ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

DENOTES:

ALL B -2-2X10 FLUSH

* - ALL FIRST FLOOR PLATE HEIGHT 7'-10"-0 U/S @ PLATE LEVEL

V -6/12 VAULT CEILING (1'-5"-0 HIGH A.P.P)

@ -1'-4"-0 HIGHER FASCIA ONLY

HARDWARE:

LUS26-2 -(X)
LJS26DS -(V)
LUS24 -(O)

TH-5E
EL:B UNIT11
401369

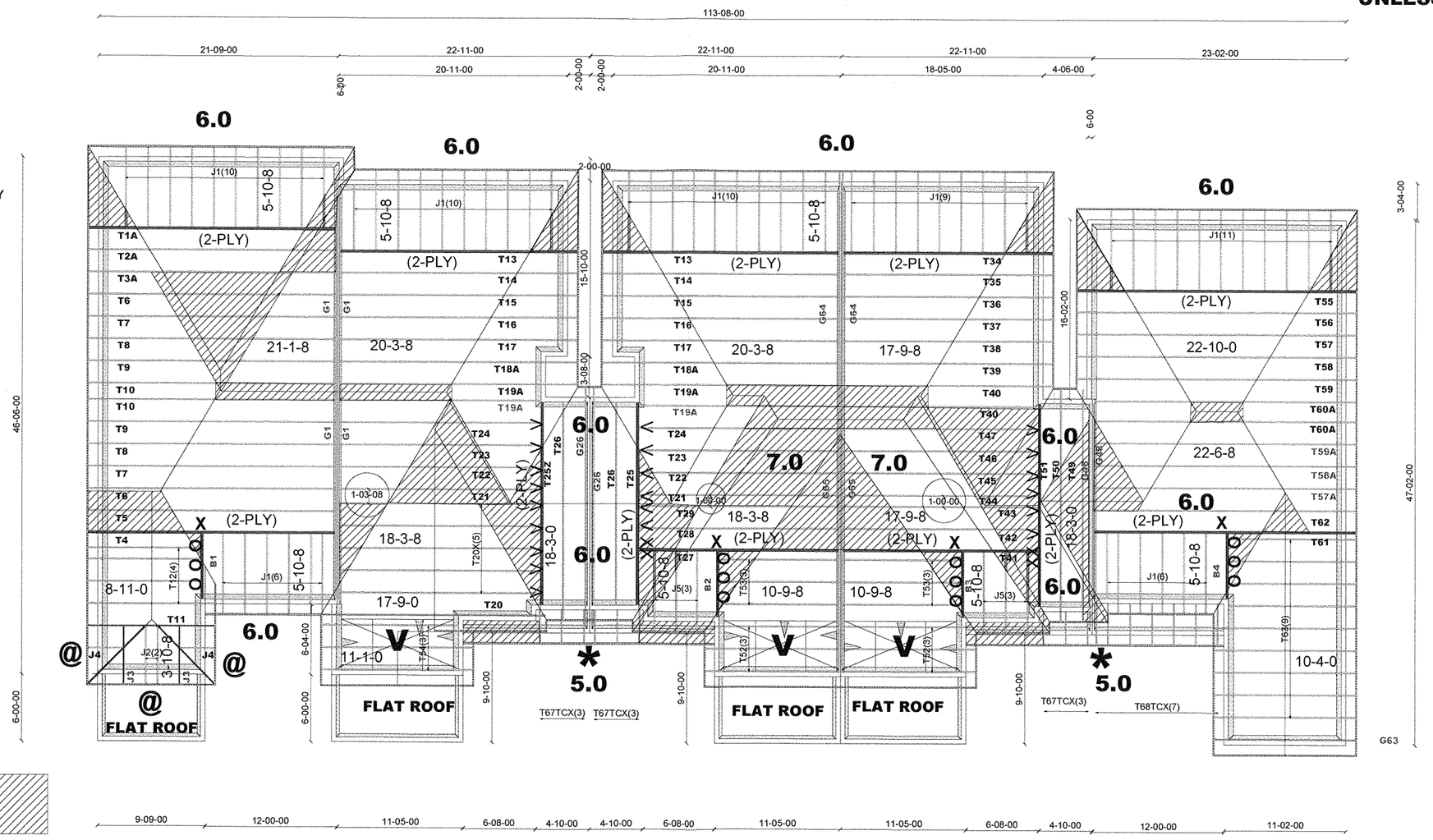
TH-2
EL:B UNIT12
401370

TH-2
EL:B2(REV) UNIT13
401371

TH-2MOD
EL:B UNIT14
401372

TH-1EMOD
EL:B(REV) UNIT15
401373

10/12 PITCHES (TYP.)
UNLESS NOTED



T-180690

DENOTES:
CONVENTIONAL
FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

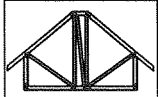
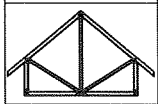
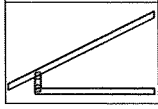
Job Track: 50264	Builder / Location: BAYVIEW WELLINGTON / ST CATHERINES	Model / Elevation: FREEHOLD BLOCK 3 / UNITS 11 - 15	Mitek ver 8.2.3.229
Plan Log: 200507	Project: PASSAGE ON THE CANAL		
Layout ID: 401368	Date: 2/12/2019 Sales: Mario DiCano Designer: xx		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401369
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 11	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

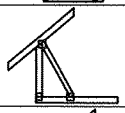
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1A Hip Girder	10 /12	**21-01-08**	4-01-04	2 x 4	1-03-08	1-07-11 1-10-10	172.94 113.67		
	2	G1 GABLE	6 /12	**18-08-00**	10-06-00	2 x 4		1-02-00 10-06-00	185.44 113.33		
	1	T2A Hip	10 /12	**21-01-08**	5-01-04	2 x 4	1-03-08	1-07-11 1-10-10	86.49 56.67		
	1	T3A Hip	10 /12	**21-01-08**	6-01-04	2 x 4	1-03-08	1-07-11 1-10-10	91.25 58.50		
	1 2-ply	T4 Half Hip Girder	10 /12	**21-01-08**	4-01-04	2 x 4 2 x 6	1-03-08	2-11-11 4-01-04	205.85 128.33		
	1	T5 Half Hip	10 /12	**21-01-08**	5-01-04	2 x 4	1-03-08	2-11-11 5-01-04	91.26 58.17		
	2	T6 Half Hip	10 /12	**21-01-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	191.49 121.33		
	2	T7 Half Hip	10 /12	**21-01-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	2	T8 Half Hip	10 /12	**21-01-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	2	T9 Half Hip	10 /12	**21-01-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	229.3 144.33		
	2	T10 Half Hip	10 /12	**21-01-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	241.17 149.67		
	1	T11 Hip Girder	10 /12	**8-11-00**	6-02-07	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	54.13 34.67		
	4	T12 Common	10 /12	**8-11-00**	6-08-04	2 x 4	1-03-08 1-03-08	2-11-11 2-11-11	185.24 116.67		
	16	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401369
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 11	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	J2 Jack-Open	10 /12	**3-10-08**	6-02-07	2 x 4	1-03-08	2-11-11 6-02-07	37.1 23.67		
	2	J3 Jack-Open Girder	10 /12	**1-09-07**	4-05-09	2 x 4	1-03-08 2-01-01	2-11-11 4-05-09	29.13 19.00		
	2	J4 Jack-Open	10 /12	**1-09-07**	4-05-09	2 x 4	1-03-08 1-01	2-11-11 4-05-09	24.58 16.33		

TOTAL # TRUSS= **46** TOTAL BFT OF ALL TRUSSES= **1575.68** BFT. TOTAL WEIGHT OF ALL TRSSES **2485.94** LBS

HARDWARE

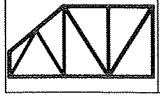
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **4**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401370
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 12	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

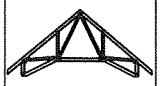
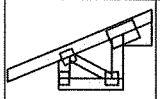
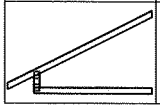
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	6 /12	18-08-00	10-06-00	2 x 4		1-02-00 10-06-00	185.44 113.33		
	1 2-ply	T13 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 111.00		
	1	T14 Half Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 55.83		
	1	T15 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	93.4 60.00		
	1	T16 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	1	T18A Half Hip	10 /12	18-03-08	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	113.9 69.67		
	2	T19A Half Hip	10 /12	18-03-08	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	267.41 168.33		
	1	T20 Common Structural Gable	10 /12	17-09-00	9-00-07	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	86.53 55.33		
	5	T20X Common	10 /12	17-05-08	9-00-07	2 x 4	1-03-08	1-07-11 1-07-11	422.04 271.67		
	1	T21 Half Hip	10 /12	18-03-08	6-01-04	2 x 4		3-03-11 6-01-04	79.81 51.17		
	1	T22 Half Hip	10 /12	18-03-08	7-01-04	2 x 4		3-03-11 7-01-04	84.11 53.33		
	1	T23 Half Hip	10 /12	18-03-08	8-01-04	2 x 4		3-03-11 8-01-04	88.7 55.50		
	1	T24 Half Hip	10 /12	18-03-08	9-01-04	2 x 4		3-03-11 9-01-04	104.75 66.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401370
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 12	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1 2-ply**	**T25Z Common Girder**	6 /12	**18-03-00**	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	184.05 112.67		
	**1**	**T26 Common**	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	72.57 47.33		
	**1**	**G26 GABLE**	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	**3**	**T54 Roof Special**	10 /12 6 /12	**11-01-00**	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	162.57 110.00		
	**3**	**T67TCX Monopitch**	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	33.95 27.00		
	**10**	**J1 Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		

TOTAL # TRUSS= **41** TOTAL BFT OF ALL TRUSSES= **1705.83** BFT. TOTAL WEIGHT OF ALL TRSSES **2669.04** LBS

HARDWARE

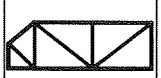
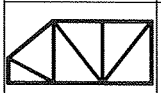
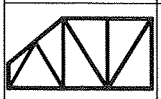
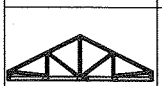
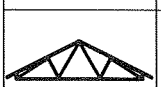
QTY	TYPE	MODEL	LENGTH
9	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **9**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401371
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 13	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

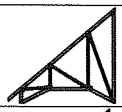
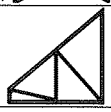
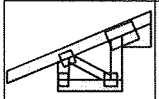
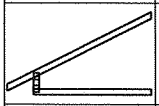
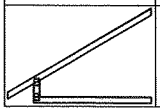
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T13 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 111.00		
	1	T14 Half Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 55.83		
	1	T15 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	93.4 60.00		
	1	T16 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	1	T18A Half Hip	10 /12	18-03-08	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	113.9 69.67		
	2	T19A Half Hip	10 /12	18-03-08	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	267.41 168.33		
	1	T21 Half Hip	10 /12	18-03-08	6-01-04	2 x 4		3-03-11 6-01-04	79.81 51.17		
	1	T22 Half Hip	10 /12	18-03-08	7-01-04	2 x 4		3-03-11 7-01-04	84.11 53.33		
	1	T23 Half Hip	10 /12	18-03-08	8-01-04	2 x 4		3-03-11 8-01-04	88.7 55.50		
	1	T24 Half Hip	10 /12	18-03-08	9-01-04	2 x 4		3-03-11 9-01-04	104.75 66.67		
	1 2-ply	T25 Common Girder	6 /12	18-03-00	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	184.05 112.67		
	1	T26 Common	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	72.57 47.33		
	1	G26 GABLE	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401371
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	2 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 13	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**1 2-ply**	**T27 Half Hip Girder**	10 /12	**18-03-08**	4-08-09	2 x 4 2 x 6		11-06 4-08-09	176.29 108.33		
	**1**	**T28 Half Hip**	10 /12	**18-03-08**	5-10-09	2 x 4		11-06 5-10-09	78.55 49.83		
	**1**	**T29 Half Hip**	10 /12	**18-03-08**	7-00-09	2 x 4		11-06 7-00-09	84.36 53.00		
	**3**	**T52 Monopitch**	10 /12 6 /12	**10-09-08**	10-07-10	2 x 4	1-03-08	1-07-11 10-07-10	192.92 122.50		
	**3**	**T53 Monopitch**	10 /12	**10-09-08**	10-07-10	2 x 4		1-07-11 10-07-10	168.43 108.00		
	**1**	**G64 GABLE**	6 /12	**20-11-11**	11-07-14	2 x 4	1-03-08	1-02-00 11-07-14	114.68 77.00		
	**1**	**G65 GABLE**	0 /12	**22-08-05**	10-07-10	2 x 4		10-07-10 10-07-10	223.92 140.67		
	**3**	**T67TCX Monopitch**	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	33.95 27.00		
	**10**	**J1 Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	**3**	**J5 Jack-Open**	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	51.89 32.00		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1846.83** BFT. TOTAL WEIGHT OF ALL TRSSES **2903.49** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

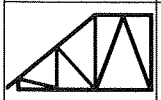
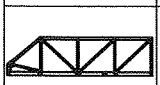
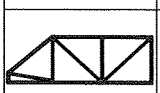
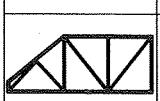
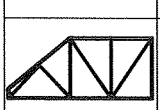
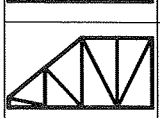
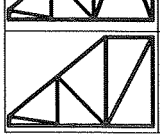
TOTAL NUMBER OF ITEMS= **11**

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200507
Project:	PASSAGE ON THE CANAL	Layout ID:	401372
Location:	ST CATHERINES	Ref #	
Model:	TH-2MOD	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 14	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T34 Half Hip Girder	10 /12	17-09-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	148.81 95.67		
	1	T35 Half Hip	10 /12	17-09-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	74.25 46.83		
	1	T36 Half Hip	10 /12	17-09-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	78.64 50.00		
	1	T37 Half Hip	10 /12	17-09-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	86.04 55.00		
	1	T38 Half Hip	10 /12	17-09-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	91.24 57.67		
	1	T39 Half Hip	10 /12	17-09-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	99.01 62.17		
	2	T40 Half Hip	10 /12	17-09-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	207.5 130.33		
	1 2-ply	T41 Half Hip Girder	10 /12	17-09-08	4-08-09	2 x 4 2 x 6		1-04-06 4-08-09	173.83 104.00		
	1	T42 Half Hip	10 /12	17-09-08	5-10-09	2 x 4		1-04-06 5-10-09	75.53 47.67		
	1	T43 Half Hip	10 /12	17-09-08	7-00-09	2 x 4		1-04-06 7-00-09	83.47 52.33		
	1	T44 Half Hip	10 /12	17-09-08	7-07-09	2 x 4		1-04-06 7-07-09	86.49 55.67		
	1	T45 Half Hip	10 /12	17-09-08	8-09-09	2 x 4		1-04-06 8-09-09	94.98 60.83		
	1	T46 Half Hip	10 /12	17-09-08	9-11-09	2 x 4		1-04-06 9-11-09	100.7 63.17		
	1	T47 Half Hip	10 /12	17-09-08	11-01-09	2 x 4		1-04-06 11-01-09	99.41 61.83		

DELIVERY SHIPLIST

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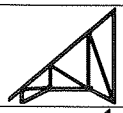
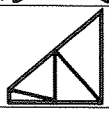
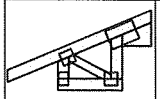
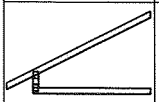
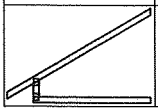
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G48 GABLE	6 /12	**17-06-00**	5-06-08	2 x 4		1-02-00 1-02-00	62.74 38.17		
	1	T49 Hip	6 /12	**18-03-00**	2-03-10	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	67.75 44.50		
	1	T50 Hip	6 /12	**18-03-00**	3-11-10	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.29 45.00		
	1 2-ply	T51 Hip Girder	6 /12	**18-03-00**	5-06-06	2 x 6		1-02-00 1-02-00	207.76 133.33		
	3	T52 Monopitch	10 /12 6 /12	**10-09-08**	10-07-10	2 x 4	1-03-08	1-07-11 10-07-10	192.92 122.50		
	3	T53 Monopitch	10 /12	**10-09-08**	10-07-10	2 x 4		1-07-11 10-07-10	168.43 108.00		
	1	G64 GABLE	6 /12	**20-11-11**	11-07-14	2 x 4	1-03-08	1-02-00 11-07-14	114.68 77.00		
	1	G65 GABLE	0 /12	**22-08-05**	10-07-10	2 x 4		10-07-10 10-07-10	223.92 140.67		
	3	T67TCX Monopitch	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	33.95 27.00		
	9	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J5 Jack-Open	7 /12	**5-10-08**	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	51.89 32.00		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1807.34** BFT. TOTAL WEIGHT OF ALL TRSSES **2845.37** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **11**

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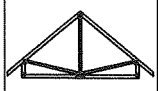
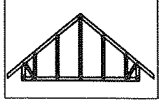
Model:	TH-1EMOD	Page:	1 of 2
Lot #:		Date:	02/12/2019
Elevation:	B UNIT 15	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G48 GABLE	6 /12	17-06-00	5-06-08	2 x 4		1-02-00 1-02-00	62.74 38.17		
	1 2-ply	T55 Hip Girder	10 /12	22-10-00	4-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	195.31 126.00		
	1	T56 Hip	10 /12	22-10-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.23 60.83		
	1	T57 Hip	10 /12	22-10-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	97.87 61.67		
	1	T57A Hip	10 /12	22-06-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	95.29 59.83		
	1	T58 Hip	10 /12	22-10-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	108.47 67.83		
	1	T58A Hip	10 /12	22-06-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	105.92 66.50		
	1	T59 Hip	10 /12	22-10-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	108.65 69.67		
	1	T59A Hip	10 /12	22-06-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	106.1 67.67		
	2	T60A Hip	10 /12	22-06-08	9-01-04	2 x 4	1-03-08	1-07-11 1-10-10	220.3 142.00		
	1 2-ply	T61 Half Hip Girder	10 /12	22-06-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	214.05 134.33		
	1	T62 Half Hip	10 /12	22-06-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	93.93 60.67		
	9	T63 Common	10 /12	10-04-00	5-11-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	419.29 270.00		
	1	G63 GABLE	10 /12	10-04-00	5-11-06	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	50.26 34.00		

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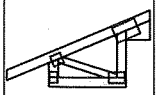
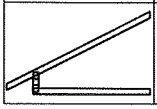
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**7**	**T68TCX Monopitch**	5 /12	**2-05-00**	2-04-06	2 x 4	1-03-08	1-00-09 2-00-10	98.16 67.67		
	**17**	**J1 Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	285.51 181.33		

TOTAL # TRUSS= **49** TOTAL BFT OF ALL TRUSSES= **1508.17** BFT. TOTAL WEIGHT OF ALL TRSSES **2355.07** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **4**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T1A	1	2	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 09:01:26 2019 Page 1
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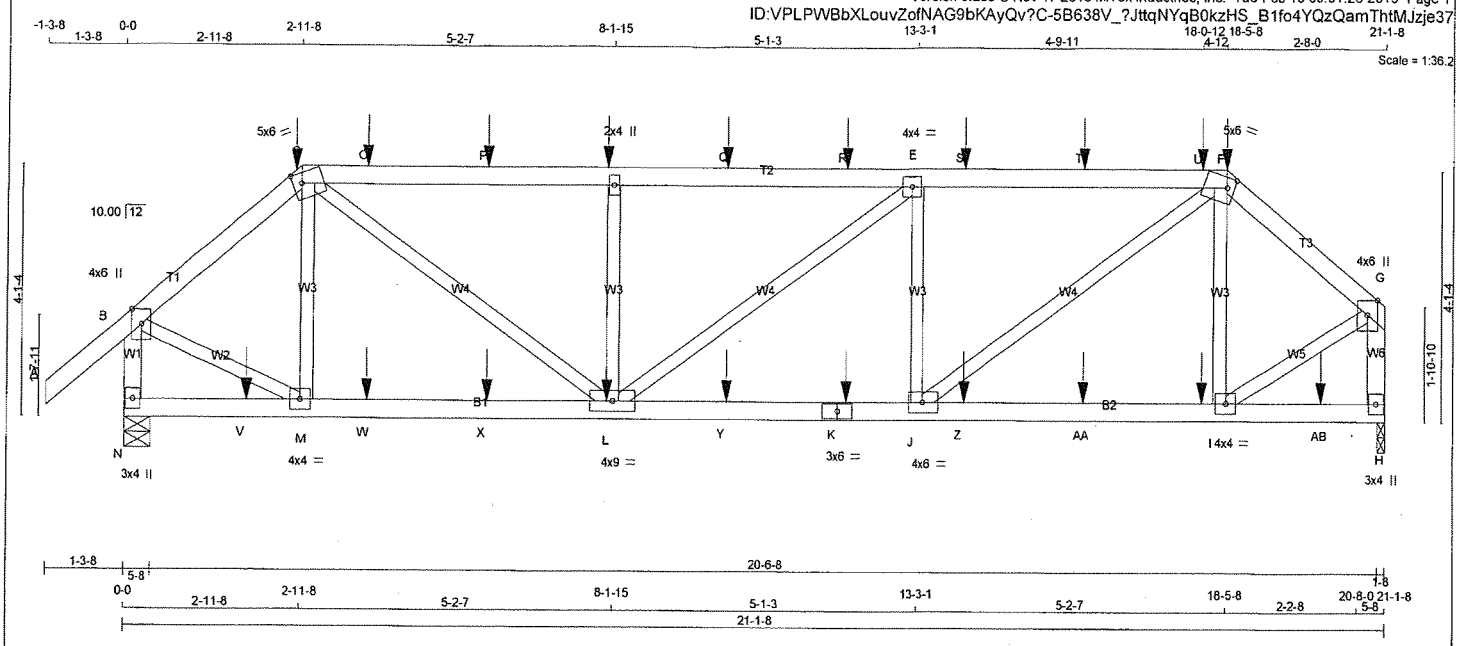


TOTAL WEIGHT = 2 X 86 = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
N- B	1	12	TOP
H- G	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	1	12	SIDE(61.0)
K- H	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2187	0	2187	0	0	5-8	5-8
H	2166	0	2166	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1660	811 / 0	364 / 0	0 / 0	0 / 0	485 / 0	0 / 0
H	1652	785 / 0	379 / 0	0 / 0	0 / 0	488 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00
B- C	-1986 / 0	-77.7	-77.7	0.08 (1)	6.16
C- O	-2905 / 0	-77.7	-77.7	0.27 (1)	5.10
O- P	-2905 / 0	-77.7	-77.7	0.27 (1)	5.10
P- D	-2905 / 0	-77.7	-77.7	0.27 (1)	5.10
D- Q	-2905 / 0	-77.7	-77.7	0.29 (1)	5.07
Q- R	-2905 / 0	-77.7	-77.7	0.29 (1)	5.07
R- E	-2905 / 0	-77.7	-77.7	0.29 (1)	5.07
E- S	-2876 / 0	-77.7	-77.7	0.29 (1)	5.09
S- T	-2876 / 0	-77.7	-77.7	0.29 (1)	5.09
T- U	-2876 / 0	-77.7	-77.7	0.29 (1)	5.09
U- F	-2876 / 0	-77.7	-77.7	0.29 (1)	5.09
F- G	-1878 / 0	-77.7	-77.7	0.06 (1)	6.25
N- B	-2149 / 0	0.0	0.0	0.13 (1)	7.58
H- G	-2116 / 0	0.0	0.0	0.13 (1)	7.62
N- V	0 / 0	-39.5	-39.5	0.12 (3)	10.00
V- M	0 / 0	-39.5	-39.5	0.12 (3)	10.00
M- W	0 / 1511	-39.5	-39.5	0.24 (2)	10.00
W- X	0 / 1511	-39.5	-39.5	0.24 (2)	10.00
X- L	0 / 1511	-39.5	-39.5	0.24 (2)	10.00
L- Y	0 / 2877	-39.5	-39.5	0.36 (2)	10.00
Y- K	0 / 2877	-39.5	-39.5	0.36 (2)	10.00
K- J	0 / 2877	-39.5	-39.5	0.36 (2)	10.00
J- Z	0 / 1423	-39.5	-39.5	0.25 (2)	10.00
Z-AA	0 / 1423	-39.5	-39.5	0.25 (2)	10.00
AA- I	0 / 1423	-39.5	-39.5	0.25 (2)	10.00
I-AB	0 / 0	-39.5	-39.5	0.11 (3)	10.00
AB- H	0 / 0	-39.5	-39.5	0.11 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-340 / 28	0.05 (1)
C- L	0 / 1725	0.21 (1)
L- D	-688 / 0	0.09 (1)
L- E	0 / 36	0.00 (2)
J- E	-714 / 0	0.10 (1)
J- F	0 / 1798	0.22 (1)
I- F	-1857 / 0	0.27 (1)
B- M	0 / 1653	0.29 (1)
I- K	0 / 1646	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	8-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
F	18-5-8	-38	-42	---	FRONT	VERT	DEAD	---	---
F	18-5-8	-180	-180	---	FRONT	VERT	SNOW	---	---
I	18-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
K	12-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
L	8-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.29/1.00 (E-F:1) , BC=0.36/1.00 (J-L:2) , WB=0.22/1.00 (F-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

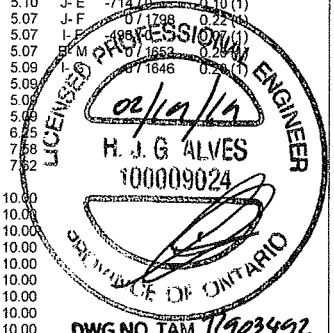
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.47 (K) (INPUT = 1.00)



DWG NO. TAM T1903492
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T1A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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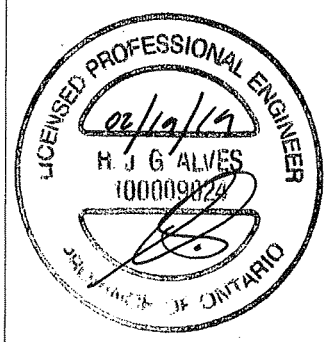
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	4-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
P	6-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	10-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	12-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	14-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
T	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
U	18-0-12	-117	-117	---	BACK	VERT	TOTAL	---	---
V	2-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
W	4-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
X	6-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Y	10-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Z	14-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AA	16-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AB	20-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 1903492
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	G1	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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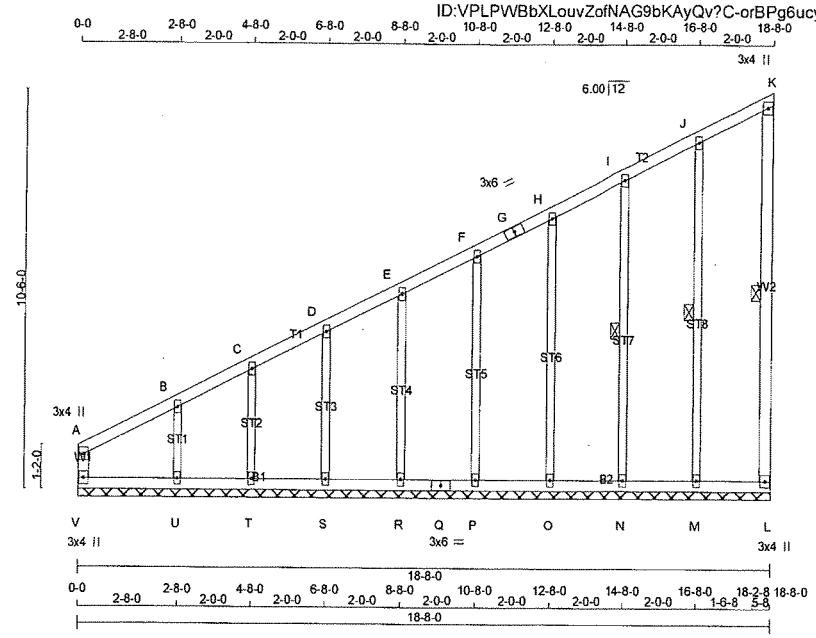


TOTAL WEIGHT = 4 X 93 = 371 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
V - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E, F, H, I, J						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, P, R, S, T, U						
M	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF K-L, J-M, I-N.

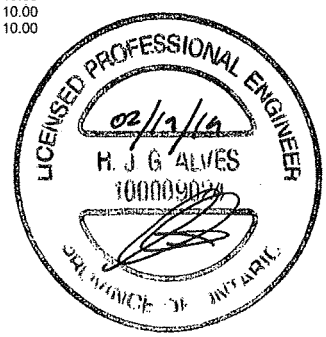
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-18 / 0	-77.7	-77.7	0.06 (1)		6.25
B- C	-21 / 0	-77.7	-77.7	0.06 (1)		6.25
C- D	-12 / 0	-77.7	-77.7	0.04 (1)		6.25
D- E	-10 / 0	-77.7	-77.7	0.04 (1)		6.25
E- F	-8 / 0	-77.7	-77.7	0.04 (1)		10.00
F- G	-6 / 0	-77.7	-77.7	0.04 (1)		10.00
G- H	-6 / 0	-77.7	-77.7	0.04 (1)		10.00
H- I	-4 / 0	-77.7	-77.7	0.04 (1)		10.00
I- J	-3 / 0	-77.7	-77.7	0.04 (1)		10.00
J- K	-6 / 0	-77.7	-77.7	0.04 (1)		10.00
L- K	-71 / 0	0.0	0.0	0.01 (1)		6.25
V- A	-101 / 0	0.0	0.0	0.02 (1)		7.81
V- U	0 / 20	-39.5	-39.5	0.05 (2)		10.00
U- T	0 / 14	-39.5	-39.5	0.05 (2)		10.00
T- S	0 / 11	-39.5	-39.5	0.03 (3)		10.00
S- R	0 / 9	-39.5	-39.5	0.03 (3)		10.00
R- Q	0 / 7	-39.5	-39.5	0.02 (3)		10.00
Q- P	0 / 7	-39.5	-39.5	0.02 (3)		10.00
P- O	0 / 5	-39.5	-39.5	0.02 (3)		10.00
O- N	0 / 4	-39.5	-39.5	0.02 (3)		10.00
N- M	0 / 3	-39.5	-39.5	0.03 (3)		10.00
M- L	0 / 2	-39.5	-39.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
M- J	-167 / 0	0.10 (1)
N- I	-154 / 0	0.07 (1)
O- H	-155 / 0	0.16 (1)
P- F	-155 / 0	0.11 (1)
R- E	-154 / 0	0.07 (1)
S- D	-157 / 0	0.05 (1)
T- C	-141 / 0	0.03 (1)
U- B	-197 / 0	0.03 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (A-B:1) , BC=0.05/1.00 (U-V:2) , WB=0.16/1.00 (H-O:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (H) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

DWG NO. TAM T1903491
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T2A	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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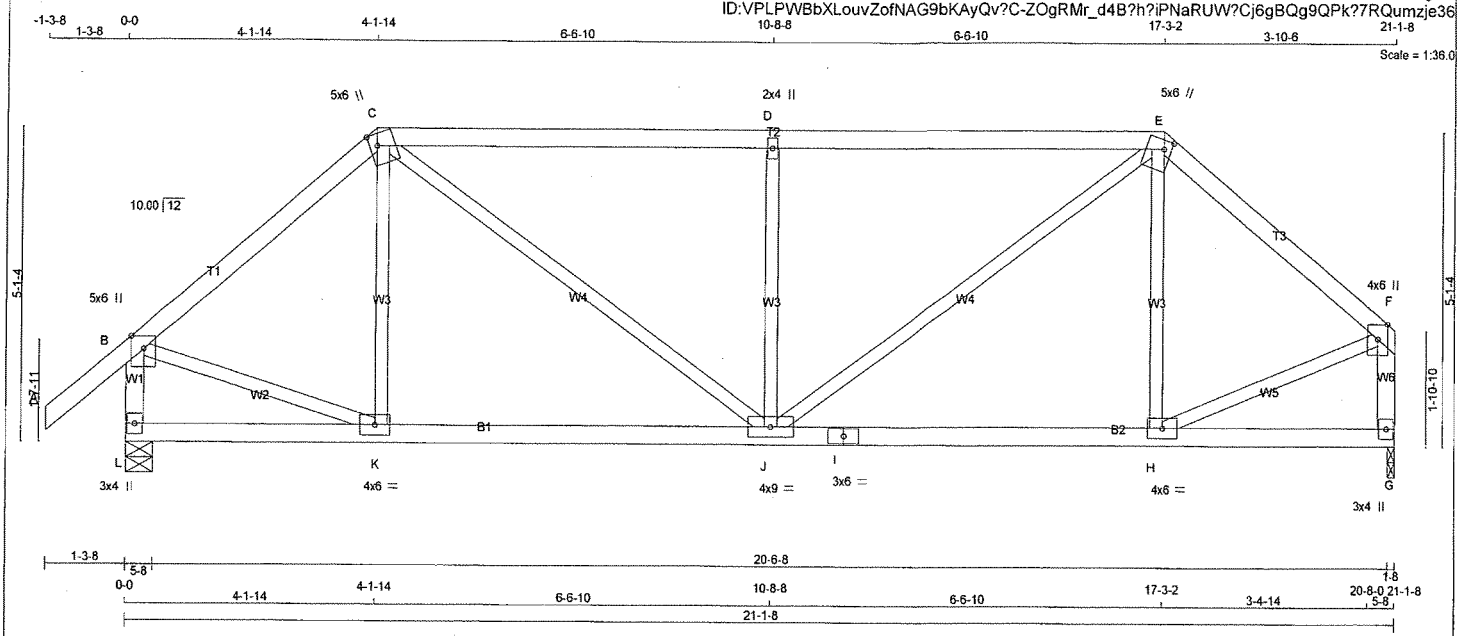


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	1.75	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1346	0	1346	0	0	5-8	5-8
G	1238	0	1238	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

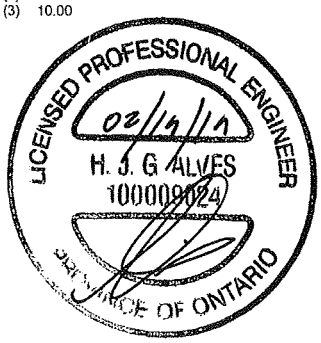
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-38 / 139	0.03 (3)
B- C	-1162 / 0	-77.7	-77.7	0.26 (1)	5.59	C- J	0 / 719	0.16 (1)
C- D	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	J- D	-622 / 0	0.24 (1)
D- E	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	J- E	0 / 778	0.17 (1)
E- F	-1102 / 0	-77.7	-77.7	0.23 (1)	5.76	H- E	-99 / 100	0.04 (1)
L- B	-1288 / 0	0.0	0.0	0.14 (1)	7.11	B- K	0 / 931	0.21 (1)
G- F	-1188 / 0	0.0	0.0	0.13 (1)	7.32	H- F	0 / 906	0.20 (1)
L- K	0 / 0	-39.5	-39.5	0.21 (3)	10.00			
K- J	0 / 889	-39.5	-39.5	0.37 (2)	10.00			
J- I	0 / 842	-39.5	-39.5	0.36 (2)	10.00			
I- H	0 / 842	-39.5	-39.5	0.36 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.66/1.00 (C-D:1) , BC=0.37/1.00 (J-K:2) , WB=0.24/1.00 (D-J:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.50 (F) (INPUT = 1.00)

DWG NO. TAM P903493
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T3A	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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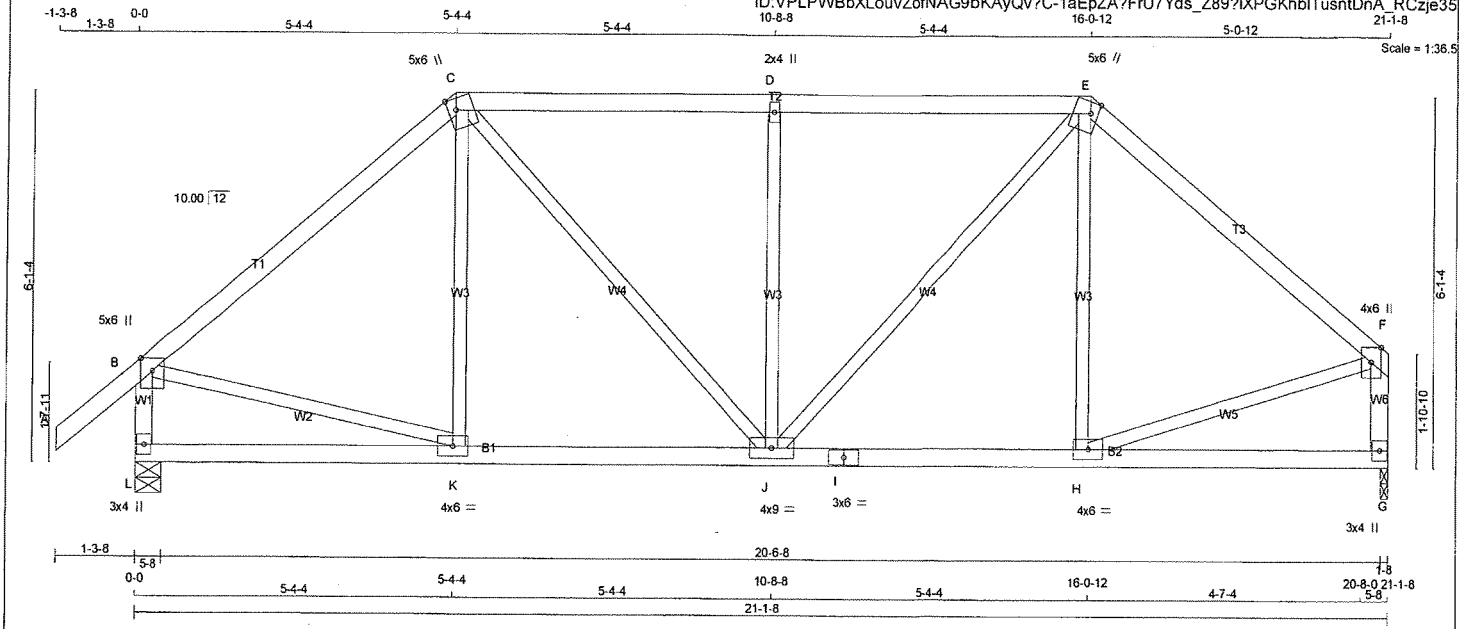


TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1346	0	1346	0	0	5-8	5-8
G	1238	0	1238	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

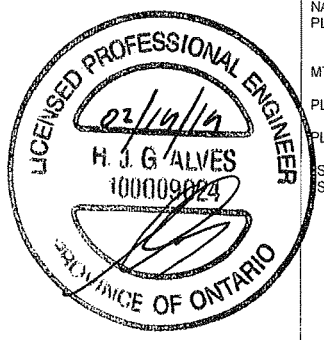
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B		0 / 34	-77.7	-77.7	0.11 (1)		10.00
B- C		-1144 / 0	-77.7	-77.7	0.45 (1)		5.36
C- D		-1182 / 0	-77.7	-77.7	0.41 (1)		5.33
D- E		-1182 / 0	-77.7	-77.7	0.41 (1)		5.33
E- F		-1102 / 0	-77.7	-77.7	0.40 (1)		5.52
L- B		-1260 / 0	0.0	0.0	0.13 (1)		7.17
G- F		-1157 / 0	0.0	0.0	0.13 (1)		7.40
L- K		0 / 0	-39.5	-39.5	0.22 (3)		10.00
K- J		0 / 877	-39.5	-39.5	0.34 (2)		10.00
J- I		0 / 844	-39.5	-39.5	0.32 (2)		10.00
I- H		0 / 844	-39.5	-39.5	0.32 (2)		10.00
H- G		0 / 0	-39.5	-39.5	0.21 (3)		10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
K- C		0 / 187	0.04 (3)
C- J		0 / 449	0.10 (1)
J- D		-505 / 0	0.30 (1)
J- E		0 / 497	0.11 (1)
H- E		-17 / 153	0.04 (3)
B- K		0 / 901	0.20 (1)
H- F		0 / 882	0.20 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.34/1.00 (J-K:2) , WB=0.30/1.00 (D-J:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.87 (F) (INPUT = 0.90)
SI METAL= 0.51 (F) (INPUT = 1.00)

DWG NO. TAM R903494
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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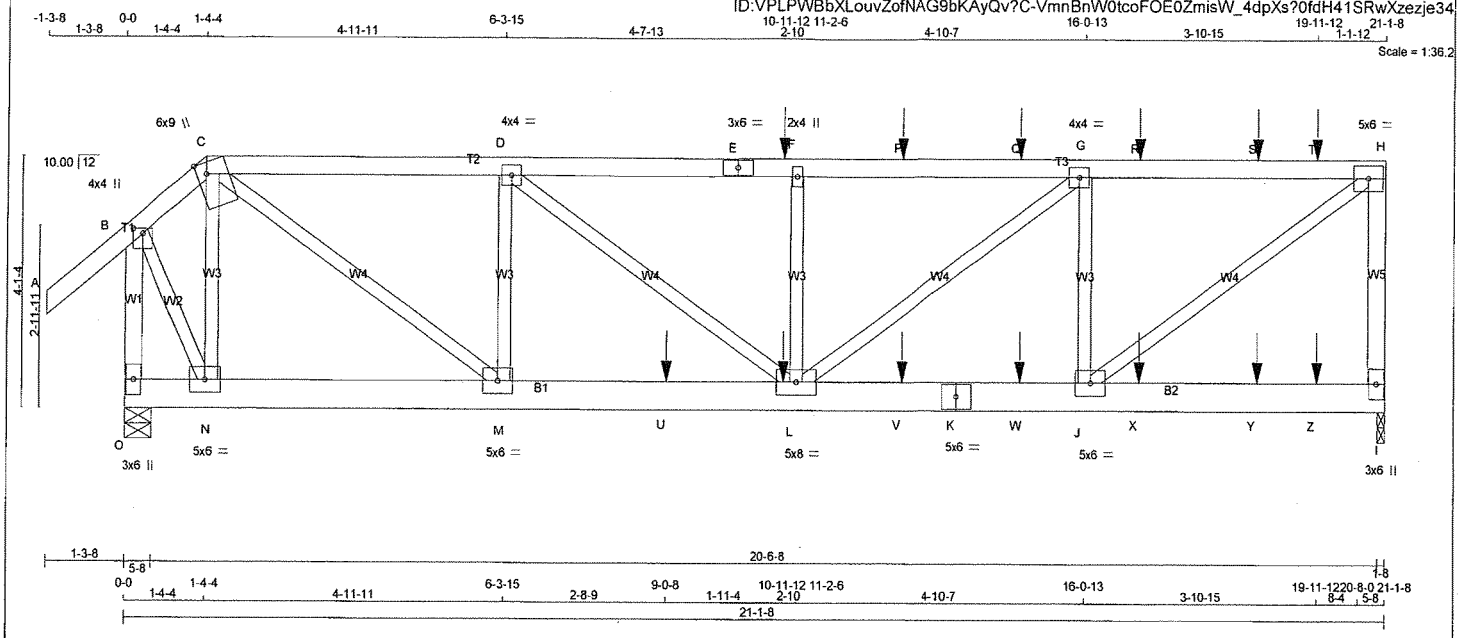

TOTAL WEIGHT = 2 X 103 = 206 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	SIDE(183.1)
K- I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	2465	0	2465	0	0	1-8	1-8
O	2316	0	2316	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1878	894 / 0	426 / 0	0 / 0	0 / 0	557 / 0	0 / 0
O	1748	882 / 0	360 / 0	0 / 0	0 / 0	507 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.06	(1)	10.00	N- C	-1378 / 0	0.17 (1)
B- C	-1070 / 0	-77.7	-77.7	0.06	(1)	6.25	B- N	0 / 1589	0.20 (1)
C- D	-3267 / 0	-77.7	-77.7	0.19	(1)	4.98	J- H	0 / 3391	0.42 (1)
D- E	-4049 / 0	-77.7	-77.7	0.22	(1)	4.55	C- M	0 / 3140	0.39 (1)
E- F	-4049 / 0	-77.7	-77.7	0.22	(1)	4.55	J- G	-1757 / 0	0.22 (1)
F- P	-4049 / 0	-77.7	-77.7	0.36	(1)	4.36	M- D	-1012 / 0	0.13 (1)
P- Q	-4049 / 0	-77.7	-77.7	0.36	(1)	4.36	L- G	0 / 1662	0.21 (1)
Q- G	-4049 / 0	-77.7	-77.7	0.36	(1)	4.36	D- L	0 / 984	0.12 (1)
G- R	-2729 / 0	-77.7	-77.7	0.32	(1)	5.16	L- F	-4957 / 0	0.06 (1)
R- S	-2729 / 0	-77.7	-77.7	0.32	(1)	5.16			
S- T	-2729 / 0	-77.7	-77.7	0.32	(1)	5.16			
T- H	-2729 / 0	-77.7	-77.7	0.32	(1)	5.16			
I- H	-2282 / 0	0.0	0.0	0.28	(1)	7.41			
O- B	-2264 / 0	0.0	0.0	0.18	(1)	7.43			
O- N	0 / 0	-39.5	-39.5	0.01	(3)	10.00			
N- M	0 / 750	-39.5	-39.5	0.22	(1)	10.00			
M- U	0 / 3267	-39.5	-39.5	0.66	(1)	10.00			
U- L	0 / 3267	-39.5	-39.5	0.66	(1)	10.00			
L- V	0 / 2729	-39.5	-39.5	0.35	(1)	10.00			
V- K	0 / 2729	-39.5	-39.5	0.35	(1)	10.00			
K- W	0 / 2729	-39.5	-39.5	0.35	(1)	10.00			
W- J	0 / 2729	-39.5	-39.5	0.35	(1)	10.00			
J- X	0 / 0	-39.5	-39.5	0.10	(2)	10.00			
X- Y	0 / 0	-39.5	-39.5	0.10	(2)	10.00			
Y- Z	0 / 0	-39.5	-39.5	0.10	(2)	10.00			
Z- I	0 / 0	-39.5	-39.5	0.10	(2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	10-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
L	10-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
P	12-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	14-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	16-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	18-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
T	19-11-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
U	9-0-8	-1298	-1298	---	FRONT	VERT	TOTAL	---	---
V	12-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
W	14-11-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.36/1.00 (F-G:1) , BC=0.66/1.00 (L-M:1) , WB=0.42/1.00 (H-J:1) , SSI=0.27/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 02/19/19 H. G. ALVES 100009083 PROVINCE OF ONTARIO

DWG NO. TAM 1903495
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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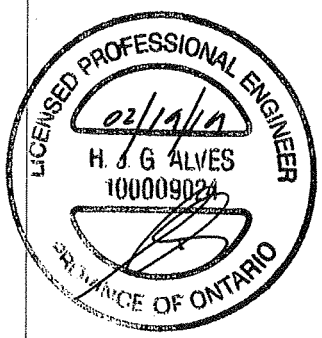
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	16-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Y	18-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Z	19-11-12	-56	-71	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM T1903495
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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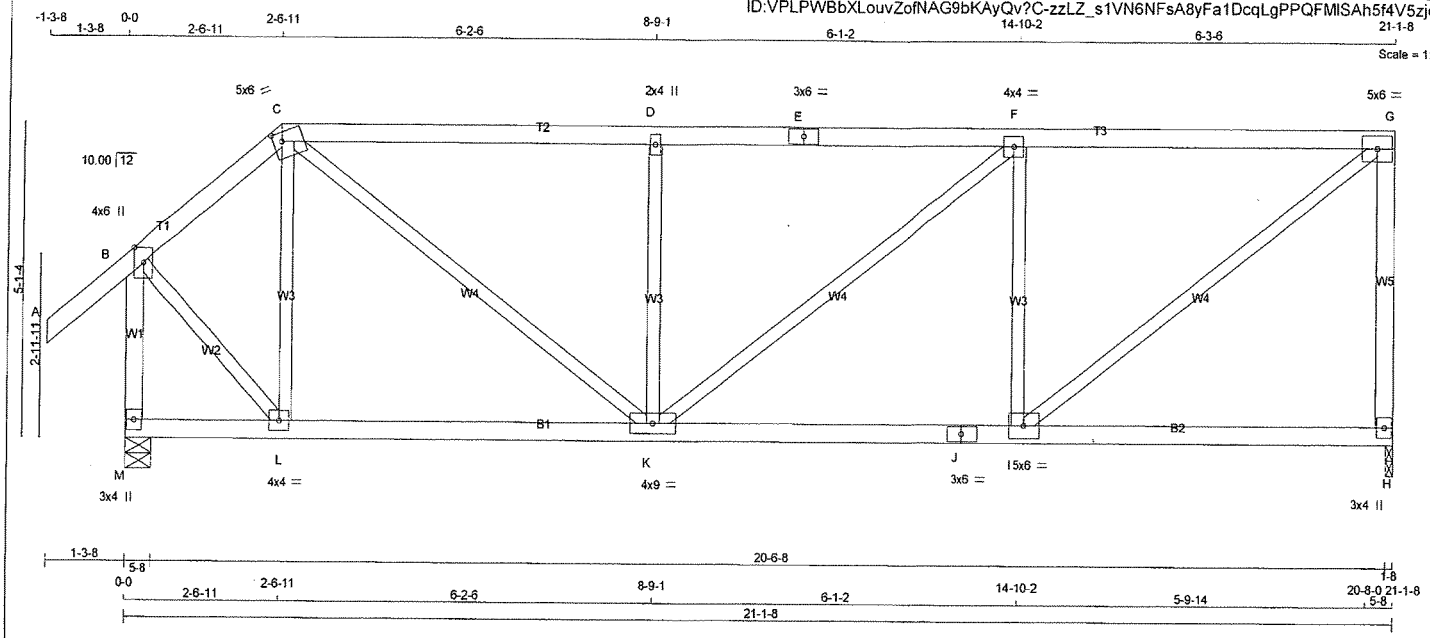


TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1238	0	1238	0	0	1-8	1-8
M	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
M	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

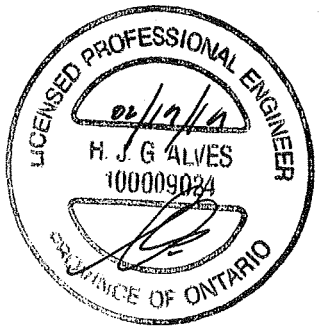
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-398 / 0	0.15 (1)
B- C	-801 / 0	-77.7	-77.7	0.09 (1)	6.25	B- L	0 / 863	0.19 (1)
C- D	-1399 / 0	-77.7	-77.7	0.46 (1)	4.94	I- G	0 / 1546	0.35 (1)
D- E	-1399 / 0	-77.7	-77.7	0.47 (1)	4.92	C- K	0 / 1004	0.23 (1)
E- F	-1399 / 0	-77.7	-77.7	0.47 (1)	4.92	I- F	-664 / 0	0.26 (1)
F- G	-1227 / 0	-77.7	-77.7	0.46 (1)	5.19	K- D	-516 / 0	0.20 (1)
H- G	-1138 / 0	0.0	0.0	0.50 (1)	7.45	K- F	0 / 219	0.05 (1)
M- B	-1331 / 0	0.0	0.0	0.21 (1)	7.02			
M- L	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
L- K	0 / 604	-39.5	-39.5	0.28 (2)	10.00			
K- J	0 / 1227	-39.5	-39.5	0.44 (2)	10.00			
J- I	0 / 1227	-39.5	-39.5	0.44 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.50/1.00 (G-H:1) , BC=0.44/1.00 (I-K:2) , WB=0.35/1.00 (G-I:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.49 (J) (INPUT = 1.00)

DWG NO. TAM 7903496
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T6	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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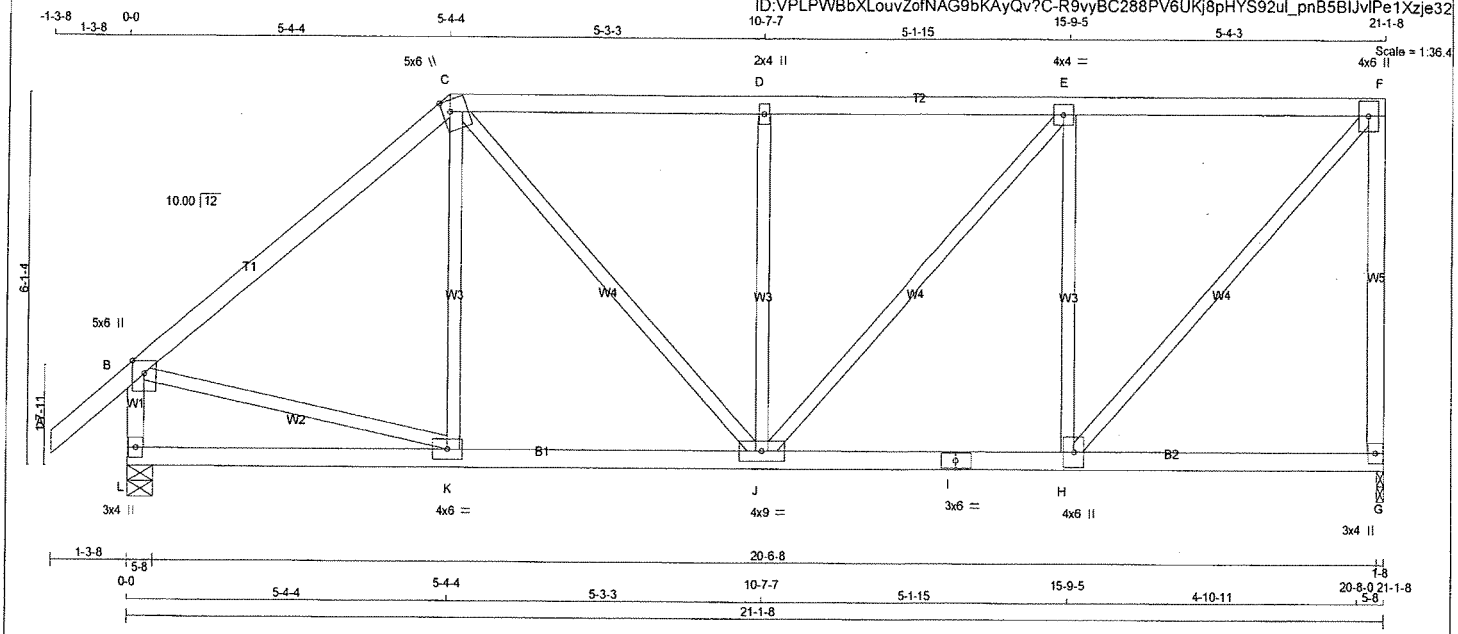


TOTAL WEIGHT = 2 X 96 = 191 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS: SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

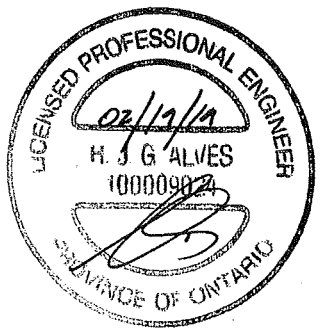
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
B- C	-1144 / 0	-77.7	-77.7	0.45 (1)	5.36	
C- D	-1170 / 0	-77.7	-77.7	0.31 (1)	5.51	
D- E	-1170 / 0	-77.7	-77.7	0.32 (1)	5.49	
E- F	-906 / 0	-77.7	-77.7	0.31 (1)	6.05	
G- F	-1153 / 0	0.0	0.0	0.83 (1)	7.42	
L- B	-1259 / 0	0.0	0.0	0.13 (1)	7.17	
L- K	0 / 0	-39.5	-39.5	0.22 (3)	10.00	
K- J	0 / 877	-39.5	-39.5	0.33 (2)	10.00	
J- I	0 / 906	-39.5	-39.5	0.33 (2)	10.00	
I- H	0 / 906	-39.5	-39.5	0.33 (2)	10.00	
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 186	0.04 (3)
B- K	0 / 901	0.20 (1)
H- F	0 / 1338	0.30 (1)
C- J	0 / 435	0.10 (1)
H- E	-747 / 0	0.44 (1)
J- D	-436 / 0	0.26 (1)
J- E	0 / 398	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.83/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.44/1.00 (E-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T1903497
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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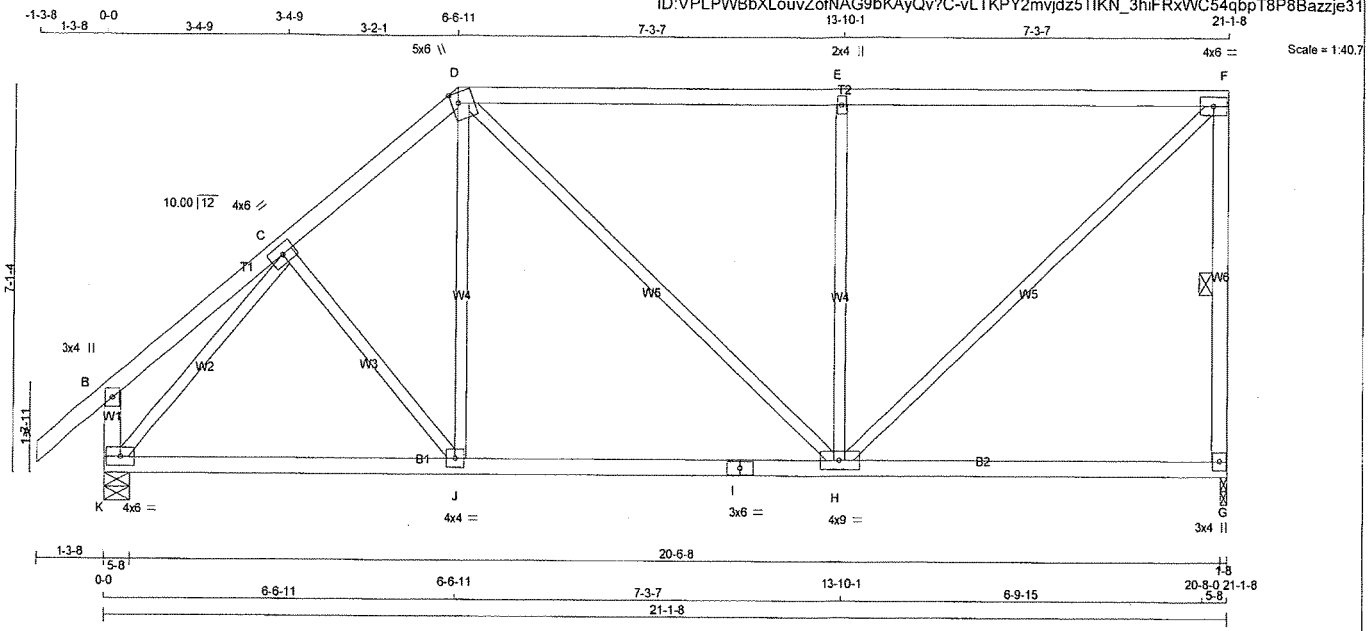


TOTAL WEIGHT = 2 X 95 = 191 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

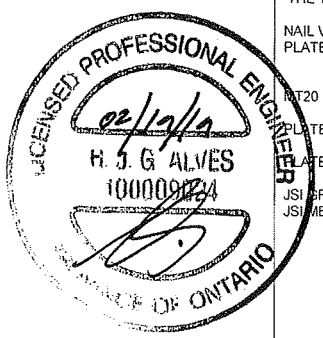
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00
C- D	-1124 / 0	-77.7	-77.7	0.14 (1)	5.83
D- E	-973 / 0	-77.7	-77.7	0.78 (1)	4.91
E- F	-974 / 0	-77.7	-77.7	0.78 (1)	4.91
G- F	-1121 / 0	0.0	0.0	0.26 (1)	5.99
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 837	-39.5	-39.5	0.42 (2)	10.00
J- I	0 / 852	-39.5	-39.5	0.48 (2)	10.00
I- H	0 / 852	-39.5	-39.5	0.48 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.39 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 88	0.02 (3)
J- D	0 / 331	0.07 (2)
D- H	0 / 166	0.04 (1)
H- E	-701 / 0	0.61 (1)
H- F	0 / 1329	0.30 (1)
K- C	-1330 / 0	0.57 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78/1.00 (E-F:1) , BC=0.48/1.00 (H-J:2) , WB=0.61/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.33 (C) (INPUT = 1.00)

DWG NO. TAM T1903498
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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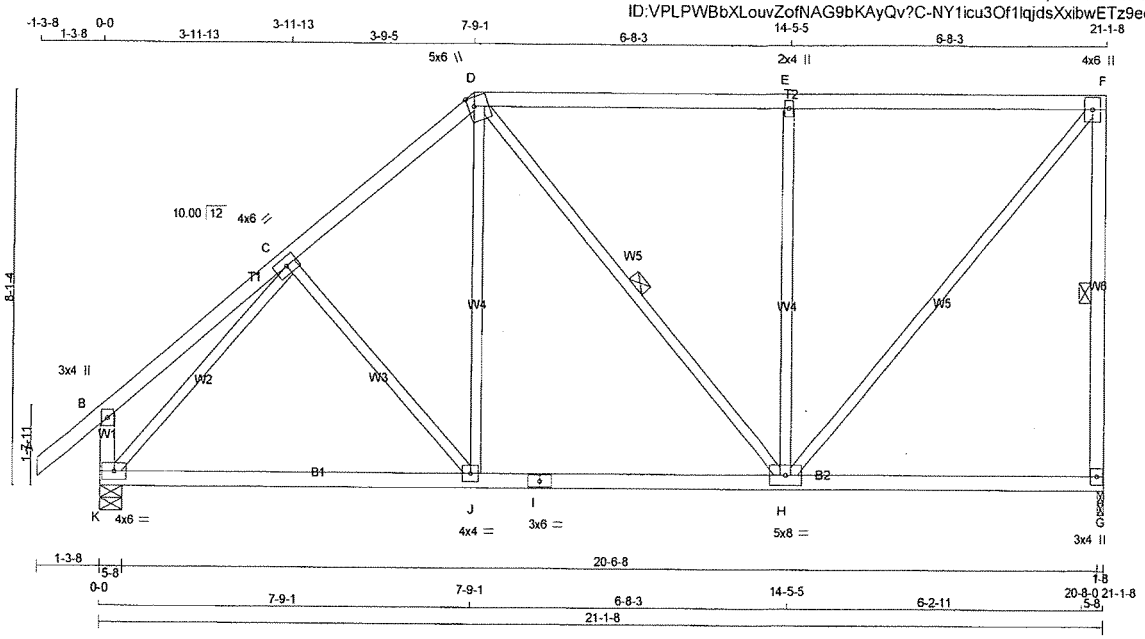


Scale = 1:45.5

TOTAL WEIGHT = 2 X 100 = 201 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

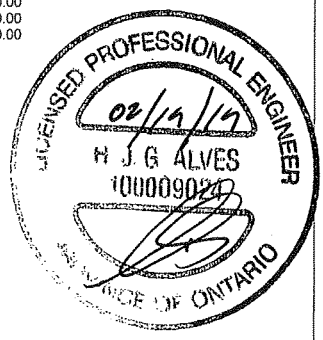
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00
C- D	-1072 / 0	-77.7	-77.7	0.20 (1)	5.85
D- E	-800 / 0	-77.7	-77.7	0.63 (1)	5.68
E- F	-800 / 0	-77.7	-77.7	0.63 (1)	5.68
G- F	-1128 / 0	0.0	0.0	0.33 (1)	5.98
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 849	-39.5	-39.5	0.50 (2)	10.00
J- I	0 / 810	-39.5	-39.5	0.50 (2)	10.00
I- H	0 / 810	-39.5	-39.5	0.50 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-66 / 57	0.04 (1)
J- D	0 / 418	0.09 (2)
D- H	-50 / 0	0.04 (3)
H- E	-643 / 0	0.81 (1)
H- F	0 / 1227	0.28 (1)
K- C	-1308 / 0	0.78 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.26")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.50/1.00 (J-K:2) , WB=0.81/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)

DWG NO. TAM T19034 99
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T9	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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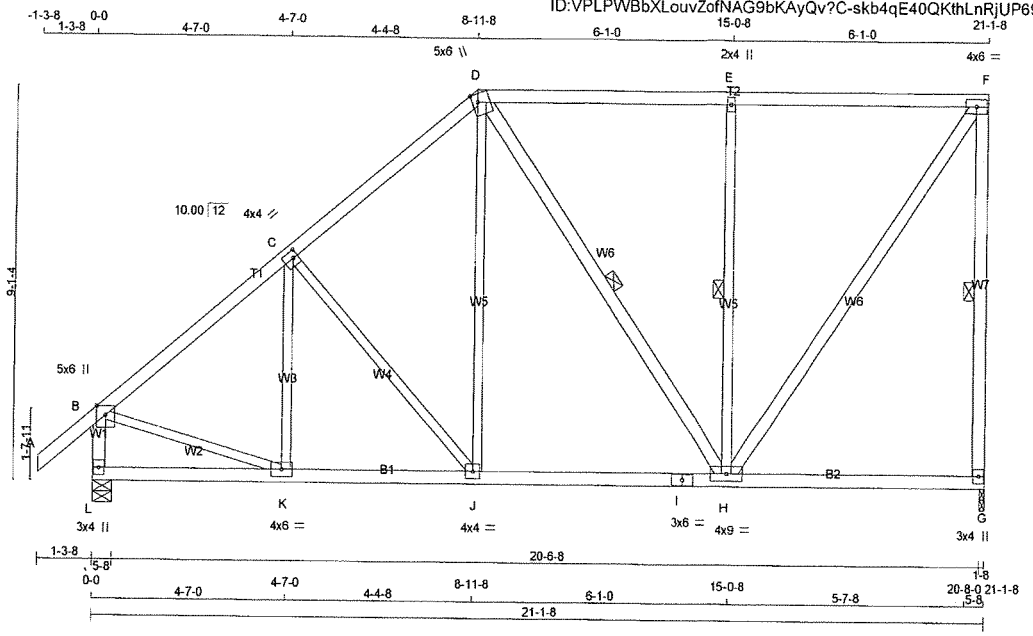


TOTAL WEIGHT = 2 X 115 = 229 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

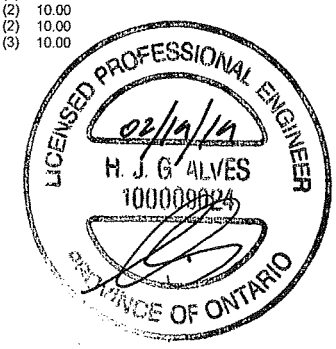
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-1168 / 0	-77.7	-77.7	0.30 (1)	5.55
C- D	-982 / 0	-77.7	-77.7	0.28 (1)	5.93
D- E	-668 / 0	-77.7	-77.7	0.51 (1)	6.25
E- F	-668 / 0	-77.7	-77.7	0.51 (1)	6.25
G- F	-1142 / 0	0.0	0.0	0.44 (1)	5.95
L- B	-1271 / 0	0.0	0.0	0.14 (1)	7.14
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00
K- J	0 / 917	-39.5	-39.5	0.27 (2)	10.00
J- I	0 / 737	-39.5	-39.5	0.38 (2)	10.00
I- H	0 / 737	-39.5	-39.5	0.38 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-68 / 103	0.03 (1)
C- J	-281 / 0	0.24 (1)
J- D	0 / 433	0.10 (2)
D- H	-121 / 0	0.07 (1)
H- E	-585 / 0	0.32 (1)
H- F	0 / 1171	0.19 (1)
B- K	0 / 953	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.38/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM 1790 3500
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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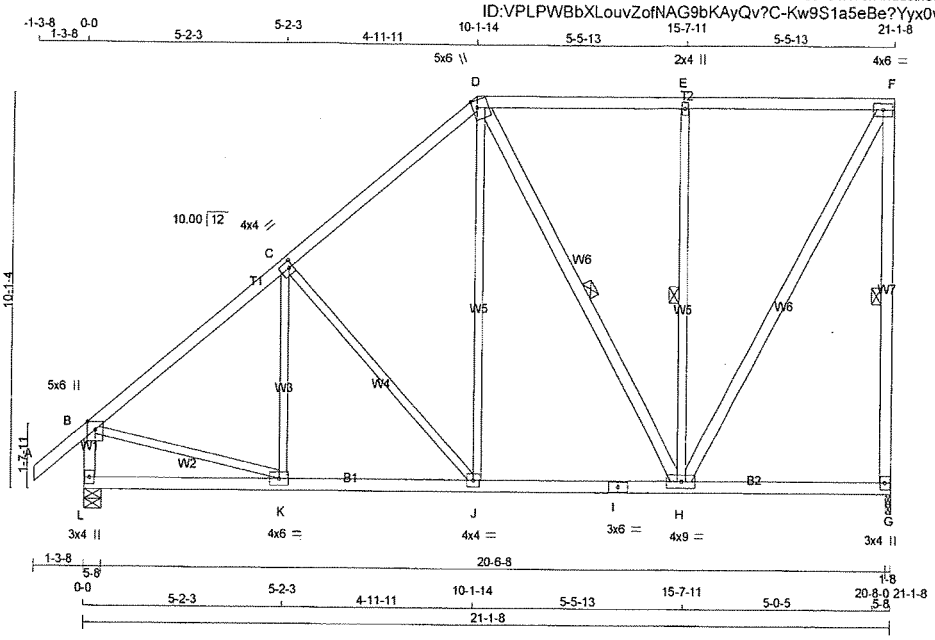


TOTAL WEIGHT = 2 X 121 = 241 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

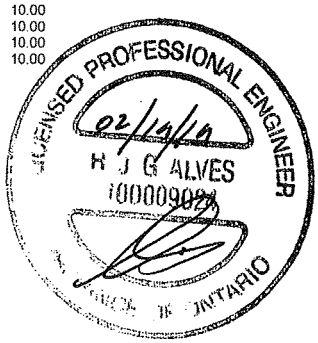
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-4 / 163	0.04 (3)
B- C	-1170 / 0	-77.7	-77.7	0.39 (1)	5.41	C- J	-375 / 0	0.43 (1)
C- D	-905 / 0	-77.7	-77.7	0.37 (1)	5.98	J- D	0 / 491	0.11 (2)
D- E	-556 / 0	-77.7	-77.7	0.40 (1)	6.25	D- H	-246 / 0	0.17 (1)
E- F	-556 / 0	-77.7	-77.7	0.40 (1)	6.25	H- E	-527 / 0	0.37 (1)
G- F	-1151 / 0	0.0	0.0	0.57 (1)	5.93	H- F	0 / 1133	0.18 (1)
L- B	-1263 / 0	0.0	0.0	0.13 (1)	7.16	B- K	0 / 951	0.21 (1)
L- K	0 / 0	-39.5	-39.5	0.19 (3)	10.00			
K- J	0 / 922	-39.5	-39.5	0.31 (2)	10.00			
J- I	0 / 675	-39.5	-39.5	0.31 (2)	10.00			
I- H	0 / 675	-39.5	-39.5	0.31 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.57/1.00 (F-G:1) , BC=0.31/1.00 (J-K:2) , WB=0.43/1.00 (C-J:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T1403501
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T11	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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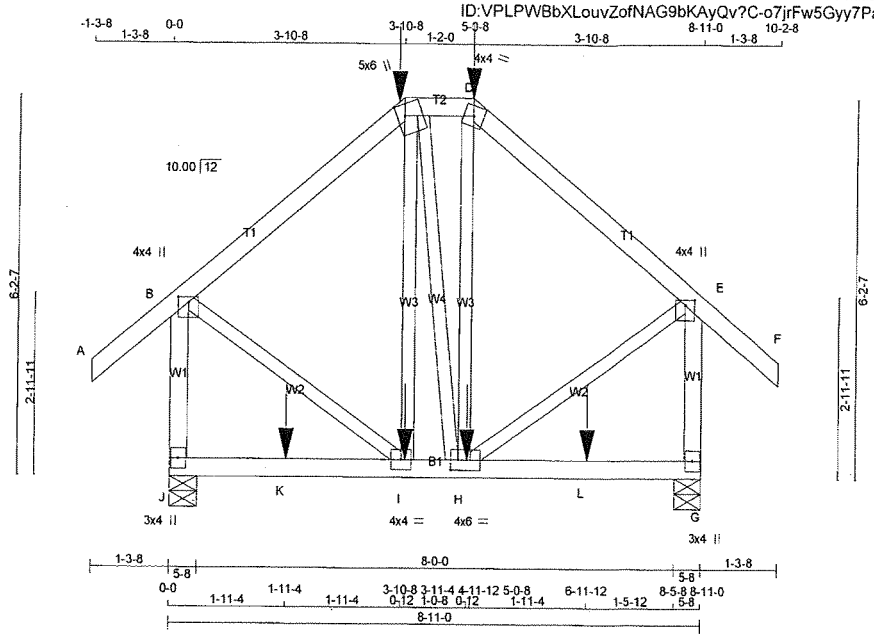

TOTAL WEIGHT = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	1.50
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	963	0	963	0	0	5-8	5-8
G	963	0	963	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	721	380 / 0	133 / 0	0 / 0	0 / 0	208 / 0	0 / 0
G	721	380 / 0	133 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.12 (1)	10.00	I- C	-97 / 74	0.06 (1)
B- C	-544 / 0	-77.7	-77.7	0.23 (1)	6.25	C- H	-8 / 6	0.00 (1)
C- D	-414 / 0	-77.7	-77.7	0.02 (1)	6.25	H- D	-106 / 80	0.07 (1)
D- E	-542 / 0	-77.7	-77.7	0.23 (1)	6.25	B- I	0 / 500	0.12 (1)
E- F	0 / 34	-77.7	-77.7	0.12 (1)	10.00	H- E	0 / 498	0.12 (1)
J- B	-884 / 0	0.0	0.0	0.15 (1)	7.81			
G- E	-882 / 0	0.0	0.0	0.15 (1)	7.81			
J- K	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
K- I	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
I- H	0 / 415	-39.5	-39.5	0.20 (2)	10.00			
H- L	0 / 0	-39.5	-39.5	0.16 (3)	10.00			
L- G	0 / 0	-39.5	-39.5	0.16 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-259	-259	---	FRONT	VERT	TOTAL	---	---
D	5-0-8	-259	-259	---	FRONT	VERT	TOTAL	---	---
H	4-11-12	-37	-47	---	FRONT	VERT	TOTAL	---	---
I	3-11-4	-37	-47	---	FRONT	VERT	TOTAL	---	---
K	1-11-4	-37	-47	---	FRONT	VERT	TOTAL	---	---
L	6-11-12	-37	-47	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.20/1.00 (H-I:2) , WB=0.12/1.00 (B-I:1) , SSI=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.19 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM P2003502
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T12	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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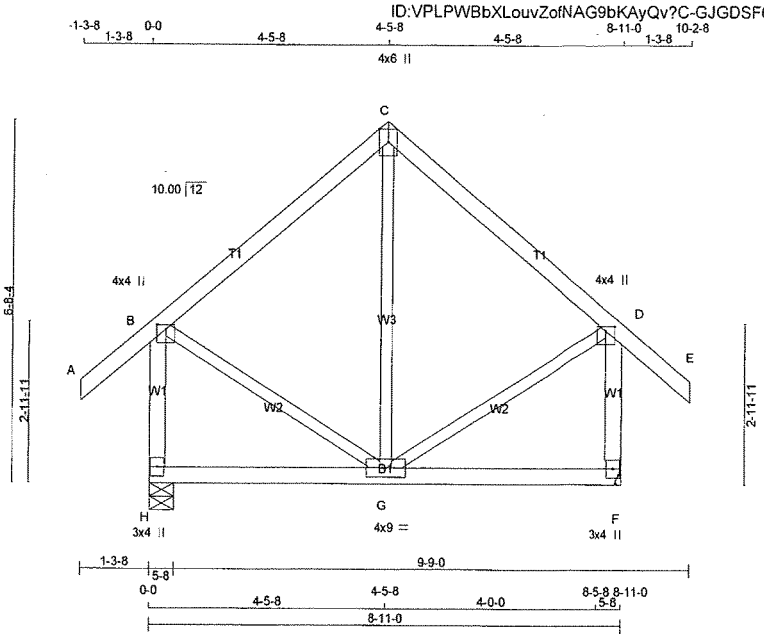


TOTAL WEIGHT = 4 X 46 = 185 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	630	0	630	0	0	5-8	5-8
F	630	0	630	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0
F	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

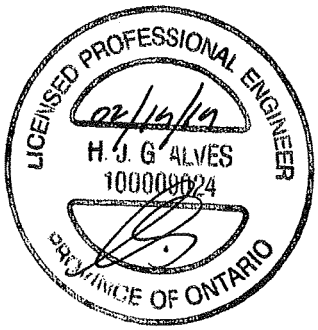
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	-16 / 134	0.03 (3)
B- C	-258 / 0	-77.7	-77.7	0.20 (1)	6.25	B- G	0 / 230	0.05 (1)
C- D	-258 / 0	-77.7	-77.7	0.20 (1)	6.25	G- D	0 / 230	0.05 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-563 / 0	0.0	0.0	0.09 (1)	7.81			
F- D	-563 / 0	0.0	0.0	0.09 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.18/1.00 (G-H:3) , WB=0.05/1.00 (D-G:1) , SSI=0.12/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

DWG NO. TAM T1903503
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T13	2	2	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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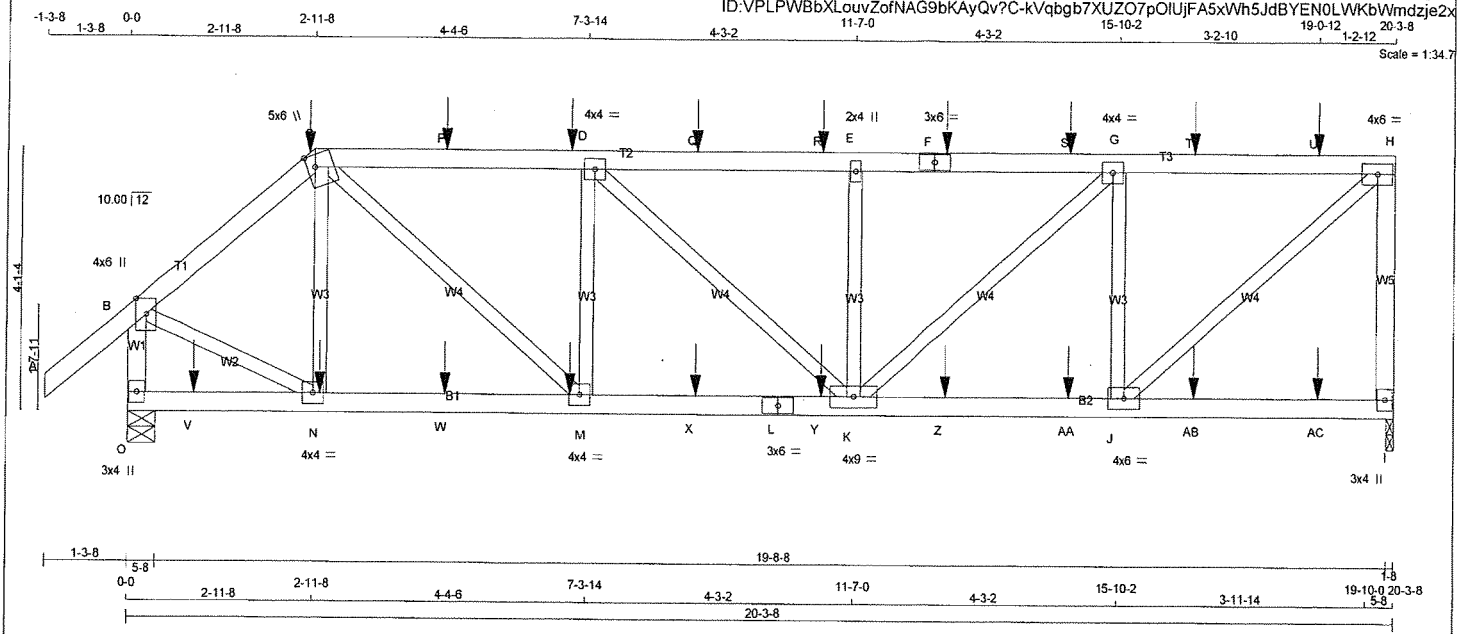

TOTAL WEIGHT = 4 X 86 = 345 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	SIDE(61.0)
C- F 1	12	SIDE(61.0)
F- H 1	12	SIDE(0.0)
H- I 1	12	TOP
O- B 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
O- L 1	12	SIDE(61.0)
L- I 1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1961	0	1961	0	0	IN-SX	IN-SX
O	2155	0	2155	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1503	688 / 0	362 / 0	0 / 0	0 / 0	453 / 0	0 / 0
O	1638	791 / 0	364 / 0	0 / 0	0 / 0	484 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 34	-77.7	-77.7	0.06 (1)	10.00	N- C		-350 / 4	0.05 (1)
B- C		-1919 / 0	-77.7	-77.7	0.08 (1)	6.24	B- N		0 / 1597	0.20 (1)
C- P		-2575 / 0	-77.7	-77.7	0.19 (1)	5.45	J- H		0 / 2510	0.31 (1)
P- D		-2575 / 0	-77.7	-77.7	0.19 (1)	5.45	C- M		0 / 1478	0.18 (1)
D- Q		-2671 / 0	-77.7	-77.7	0.19 (1)	5.37	J- G		-1299 / 0	0.17 (1)
Q- R		-2671 / 0	-77.7	-77.7	0.19 (1)	5.37	M- D		-670 / 0	0.08 (1)
R- E		-2671 / 0	-77.7	-77.7	0.19 (1)	5.37	K- G		0 / 1	0.00 (1)
E- F		-2671 / 0	-77.7	-77.7	0.22 (1)	5.33	D-		-130	0.02 (1)
F- S		-2671 / 0	-77.7	-77.7	0.22 (1)	5.33			496 / 0	0.05 (1)
S- G		-2671 / 0	-77.7	-77.7	0.22 (1)	5.33				
G- T		-1906 / 0	-77.7	-77.7	0.20 (1)	6.09				
T- U		-1906 / 0	-77.7	-77.7	0.20 (1)	6.09				
U- H		-1906 / 0	-77.7	-77.7	0.20 (1)	6.09				
I- H		-1845 / 0	0.0	0.0	0.25 (1)	7.68				
O- B		-2083 / 0	0.0	0.0	0.12 (1)	7.68				
O- V		0 / 0	-39.5	-39.5	0.08 (3)	10.00				
V- N		0 / 0	-39.5	-39.5	0.08 (3)	10.00				
N- W		0 / 1459	-39.5	-39.5	0.20 (2)	10.00				
W- M		0 / 1459	-39.5	-39.5	0.20 (2)	10.00				
M- X		0 / 2575	-39.5	-39.5	0.29 (2)	10.00				
X- L		0 / 2575	-39.5	-39.5	0.29 (2)	10.00				
L- Y		0 / 2575	-39.5	-39.5	0.29 (2)	10.00				
Y- K		0 / 2575	-39.5	-39.5	0.29 (2)	10.00				
K- Z		0 / 1906	-39.5	-39.5	0.28 (2)	10.00				
Z-AA		0 / 1906	-39.5	-39.5	0.28 (2)	10.00				
AA- J		0 / 1906	-39.5	-39.5	0.28 (2)	10.00				
J-AB		0 / 0	-39.5	-39.5	0.13 (3)	10.00				
AB-AC		0 / 0	-39.5	-39.5	0.13 (3)	10.00				
AC- I		0 / 0	-39.5	-39.5	0.13 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-100	-100	---	FRONT	VERT	TOTAL	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
F	13-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
M	7-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
N	3-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100099824
PROVINCE OF ONTARIO

DWG NO. TAM T1903504
STRUCTURAL COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.25/1.00 (H-I:1) , BC=0.29/1.00 (K-M:2) , WB=0.31/1.00 (H-J:1) , SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T13	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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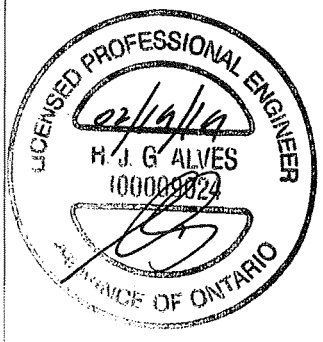
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	5-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	11-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	15-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
T	17-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
U	19-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
V	1-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
W	5-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
X	9-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Y	11-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Z	13-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AA	15-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AB	17-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AC	19-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM 7703504
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T14	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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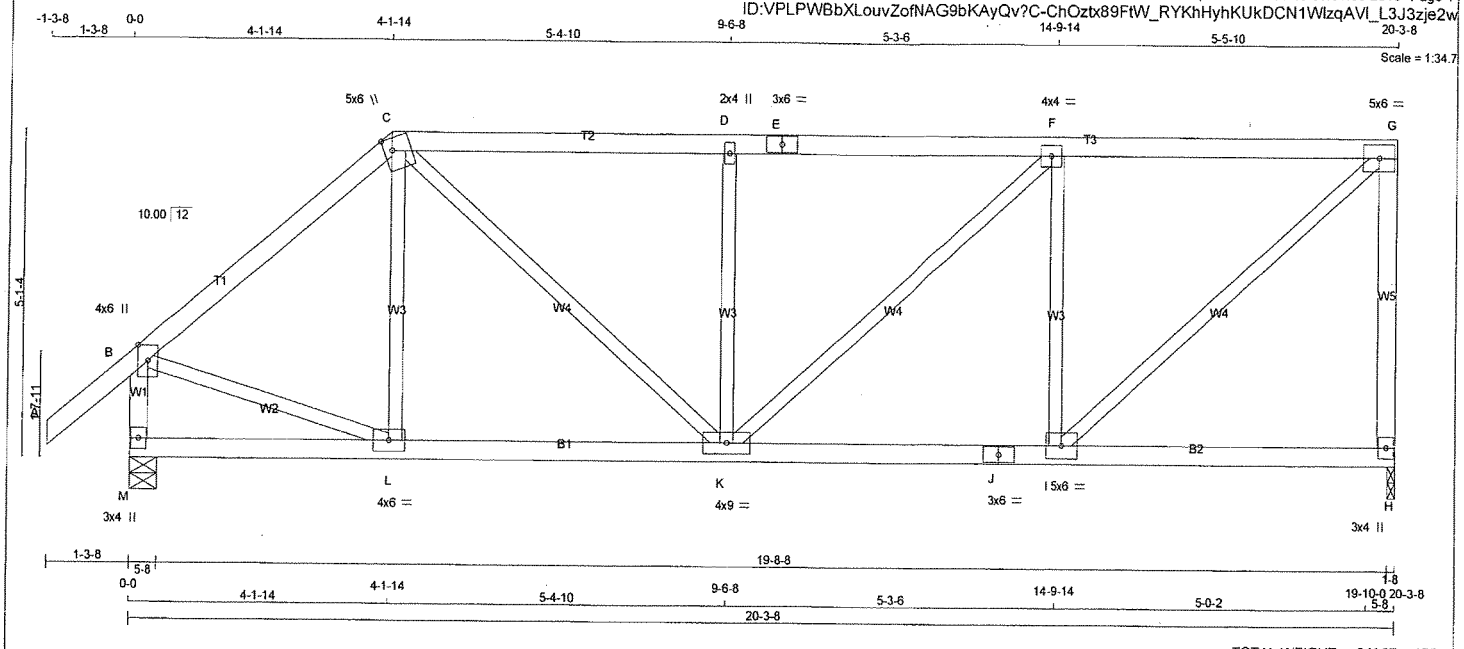


TOTAL WEIGHT = 2 X 87 = 175 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1189	0	1189	0	0	1-8	1-8
M	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
M	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

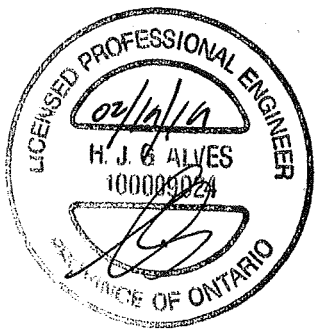
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-45 / 121	0.03 (3)
B- C	-1100 / 0	-77.7	-77.7	0.26 (1)	5.71	B- L	0 / 881	0.20 (1)
C- D	-1307 / 0	-77.7	-77.7	0.33 (1)	5.26	I- G	0 / 1407	0.32 (1)
D- E	-1308 / 0	-77.7	-77.7	0.34 (1)	5.23	C- K	0 / 624	0.14 (1)
E- F	-1308 / 0	-77.7	-77.7	0.34 (1)	5.23	I- F	-689 / 0	0.26 (1)
F- G	-1056 / 0	-77.7	-77.7	0.33 (1)	5.69	K- D	-446 / 0	0.17 (1)
H- G	-1102 / 0	0.0	0.0	0.49 (1)	7.54	K- F	0 / 340	0.08 (1)
M- B	-1234 / 0	0.0	0.0	0.13 (1)	7.23			
M- L	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
L- K	0 / 842	-39.5	-39.5	0.28 (2)	10.00			
K- J	0 / 1056	-39.5	-39.5	0.35 (2)	10.00			
J- I	0 / 1056	-39.5	-39.5	0.35 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.49/1.00 (G-H:1) , BC=0.35/1.00 (I-K:2) , WB=0.32/1.00 (G-I:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T203505
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T15	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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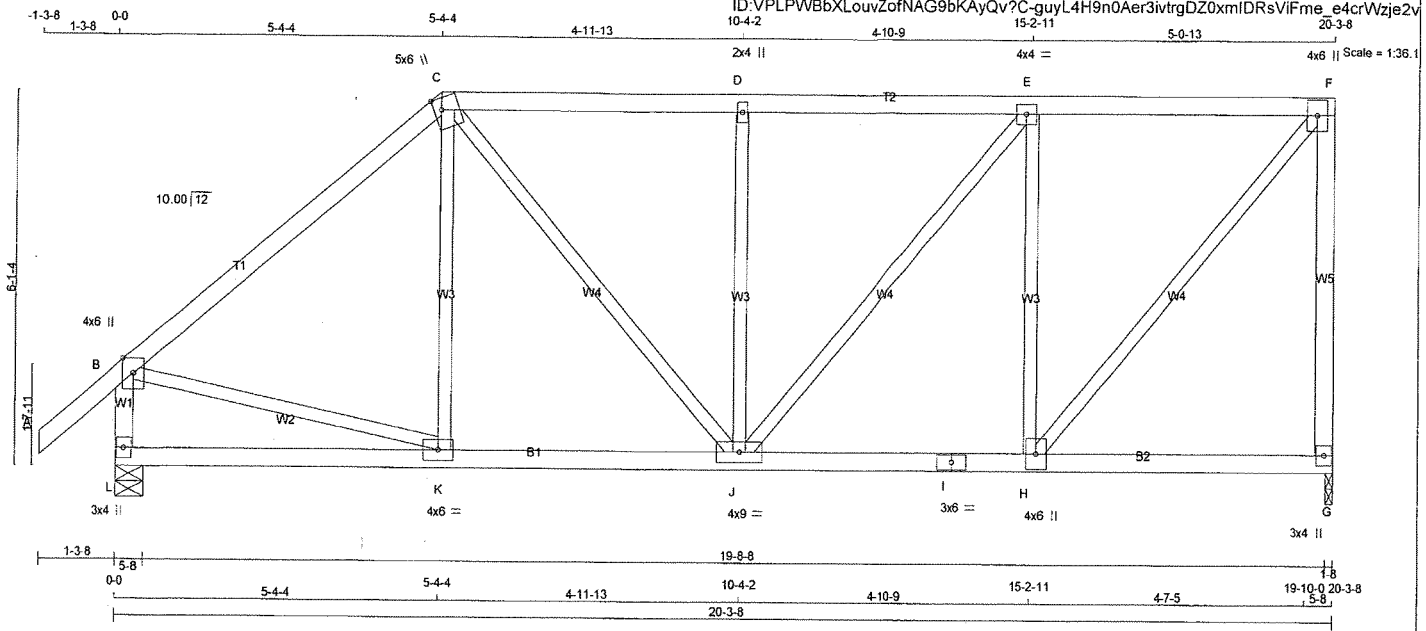


TOTAL WEIGHT = 2 X 93 = 187 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1189	0	1189	0	0	1-8	1-8
L	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
L	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

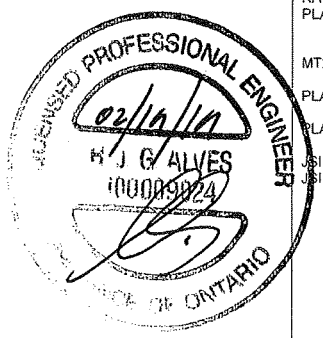
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	0 / 187	0.04 (3)	
B- C	-1084 / 0	-77.7	-77.7	0.45 (1)	5.48	B- K	0 / 854	0.19 (1)	
C- D	-1077 / 0	-77.7	-77.7	0.27 (1)	5.74	H- F	0 / 1259	0.28 (1)	
D- E	-1077 / 0	-77.7	-77.7	0.28 (1)	5.71	C- J	0 / 377	0.08 (1)	
E- F	-828 / 0	-77.7	-77.7	0.28 (1)	6.25	H- E	-723 / 0	0.42 (1)	
G- F	-1109 / 0	0.0	0.0	0.80 (1)	7.52	J- D	-413 / 0	0.24 (1)	
L- B	-1210 / 0	0.0	0.0	0.13 (1)	7.28	J- E	0 / 388	0.09 (1)	
L- K	0 / 0	-39.5	-39.5	0.21 (3)	10.00				
K- J	0 / 831	-39.5	-39.5	0.32 (2)	10.00				
J- I	0 / 828	-39.5	-39.5	0.30 (2)	10.00				
I- H	0 / 828	-39.5	-39.5	0.30 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.19 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.80/1.00 (F-G:1) , BC=0.32/1.00 (J-K:2) , WB=0.42/1.00 (E-H:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T1403506
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T16	2	1	UNITS 11 - 15 TRUSS DESC.	

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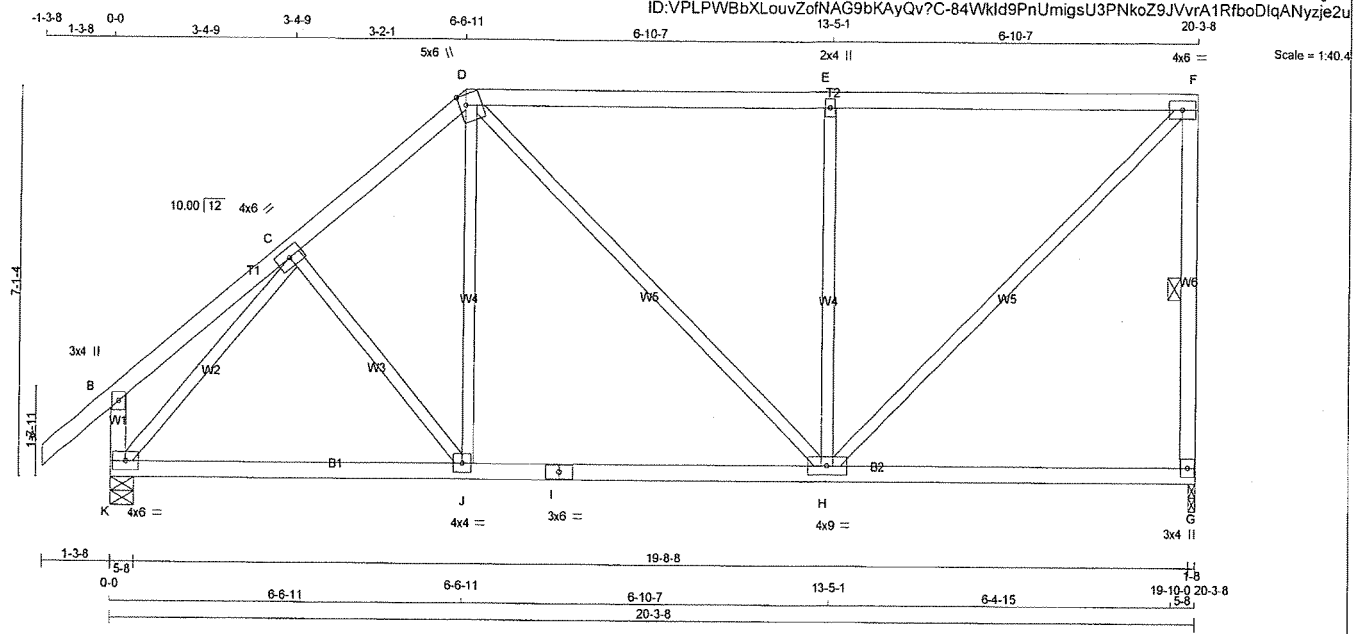


TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

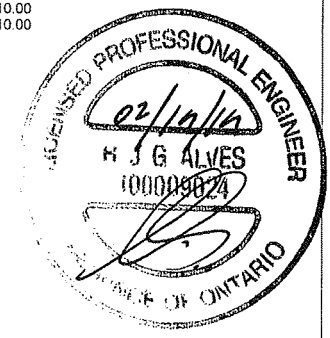
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00
C- D	-1062 / 0	-77.7	-77.7	0.14 (1)	5.96
D- E	-887 / 0	-77.7	-77.7	0.67 (1)	5.36
E- F	-887 / 0	-77.7	-77.7	0.68 (1)	5.36
G- F	-1078 / 0	0.0	0.0	0.25 (1)	6.08
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 797	-39.5	-39.5	0.41 (2)	10.00
J- I	0 / 804	-39.5	-39.5	0.43 (2)	10.00
I- H	0 / 804	-39.5	-39.5	0.43 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	0 / 81	0.02 (3)
J- D	0 / 332	0.07 (2)
D- H	0 / 117	0.03 (1)
H- E	-661 / 0	0.58 (1)
H- F	0 / 1246	0.28 (1)
K- C	-1266 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.43/1.00 (H-J:2) , WB=0.58/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM T7203507
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T17	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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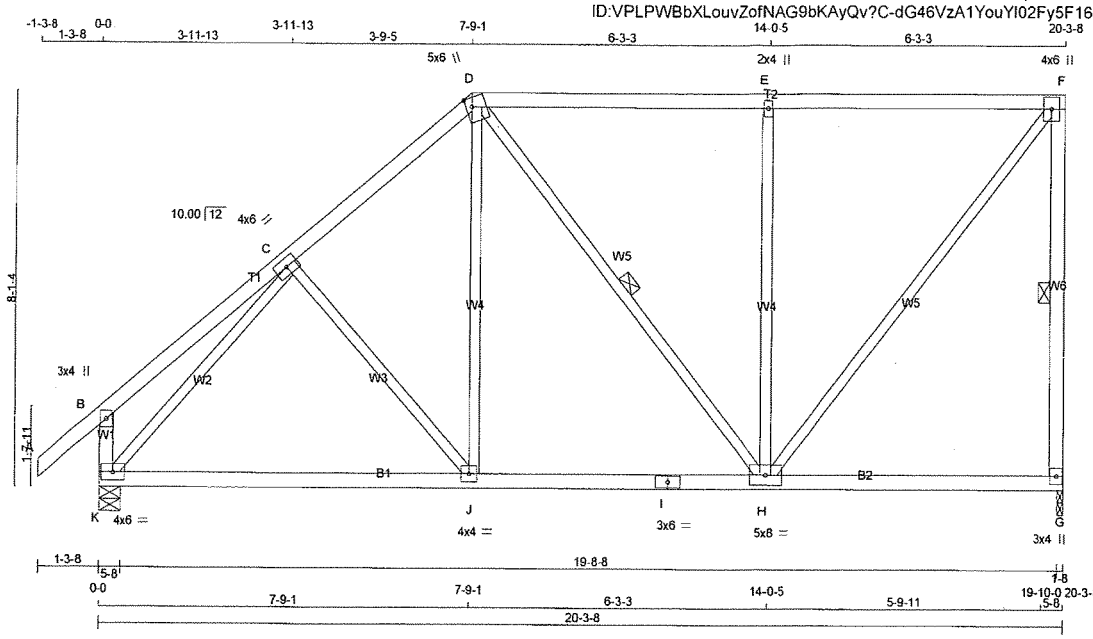


TOTAL WEIGHT = 2 X 98 = 196 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

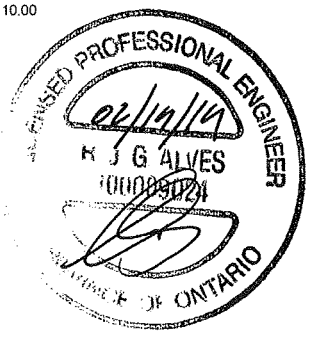
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-77 / 51	0.05 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00	J- D	0 / 420	0.09 (2)
C- D	-1008 / 0	-77.7	-77.7	0.20 (1)	5.98	D- H	-74 / 0	0.05 (3)
D- E	-725 / 0	-77.7	-77.7	0.54 (1)	6.09	H- E	-603 / 0	0.76 (1)
E- F	-726 / 0	-77.7	-77.7	0.54 (1)	6.09	H- F	0 / 1155	0.26 (1)
G- F	-1085 / 0	0.0	0.0	0.32 (1)	6.07	K- C	-1243 / 0	0.74 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 807	-39.5	-39.5	0.49 (2)	10.00			
J- I	0 / 761	-39.5	-39.5	0.49 (2)	10.00			
I- H	0 / 761	-39.5	-39.5	0.49 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.25 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 939 (0.26")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.49/1.00 (J-K:2) , WB=0.76/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

DWG NO. TAM T1903508
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T18A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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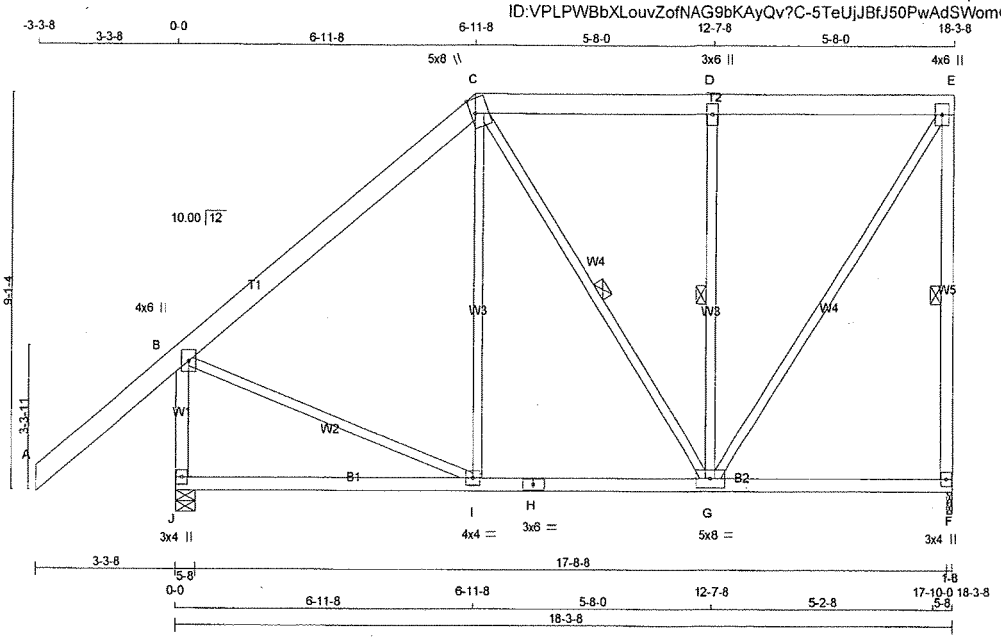


TOTAL WEIGHT = 2 X 114 = 228 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	8.0	4.00	1.25
D	TMW+w	MT20	3.0	6.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1339	0	1339	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	1004	526 / 0	192 / 0	0 / 0	0 / 0	286 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

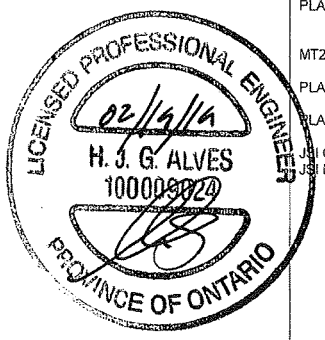
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00	I- C	0 / 232	0.06 (3)
B- C	-721 / 0	-77.7	-77.7	0.33 (1)	6.25	C- G	-64 / 0	0.05 (2)
C- D	-525 / 0	-77.7	-77.7	0.18 (1)	6.25	G- D	-533 / 0	0.28 (1)
D- E	-525 / 0	-77.7	-77.7	0.18 (1)	6.25	G- E	0 / 960	0.22 (1)
F- E	-977 / 0	0.0	0.0	0.36 (1)	6.25	B- I	0 / 596	0.13 (1)
J- B	-1225 / 0	0.0	0.0	0.21 (1)	7.25			
J- I	0 / 0	-39.5	-39.5	0.33 (3)	10.00			
I- H	0 / 553	-39.5	-39.5	0.39 (2)	10.00			
H- G	0 / 553	-39.5	-39.5	0.39 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.36/1.00 (E-F:1) , BC=0.39/1.00 (G-I:2) , WB=0.28/1.00 (D-G:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

DWG NO. TAM T1903509
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T19A	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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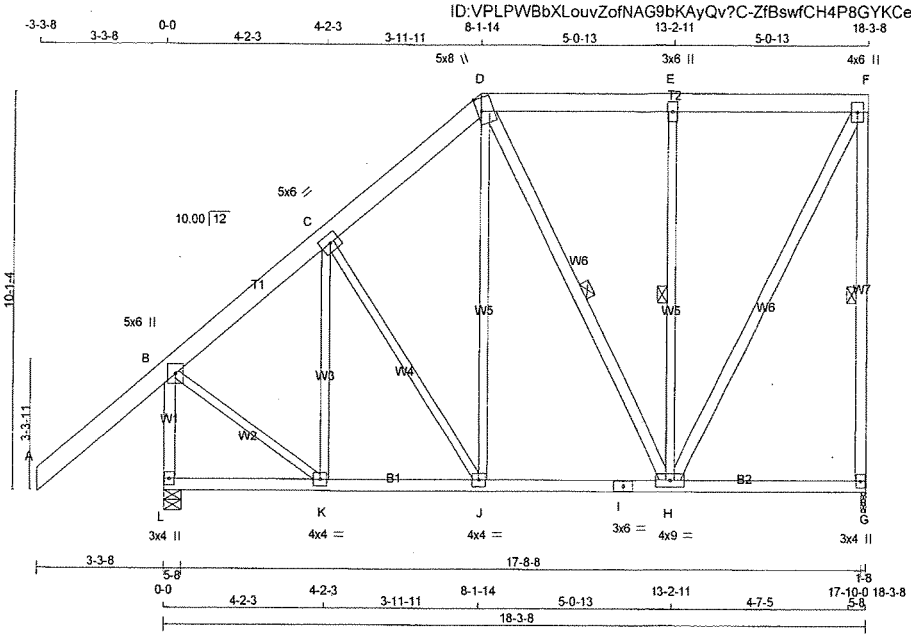


TOTAL WEIGHT = 4 X 134 = 535 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1047	0	1047	0	0	1-8	1-8
L	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	802	369 / 0	192 / 0	0 / 0	0 / 0	241 / 0	0 / 0
L	1022	540 / 0	192 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

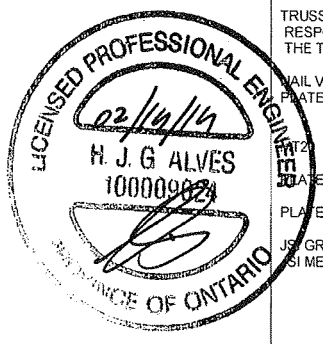
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00	K- C	-163 / 23	0.12 (1)
B- C	-721 / 0	-77.7	-77.7	0.31 (1)	6.25	C- J	-53 / 0	0.06 (2)
C- D	-629 / 0	-77.7	-77.7	0.10 (1)	6.25	J- D	0 / 253	0.06 (3)
D- E	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	D- H	-120 / 0	0.08 (1)
E- F	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	H- E	-474 / 0	0.32 (1)
G- F	-966 / 0	0.0	0.0	0.46 (1)	6.25	H- F	0 / 914	0.15 (1)
L- B	-1297 / 0	0.0	0.0	0.22 (1)	7.09	B- K	0 / 610	0.14 (1)
L- K	0 / 0	-39.5	-39.5	0.12 (3)	10.00			
K- J	0 / 503	-39.5	-39.5	0.19 (2)	10.00			
J- I	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
I- H	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.46/1.00 (F-G:1) , BC=0.25/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM T1903510
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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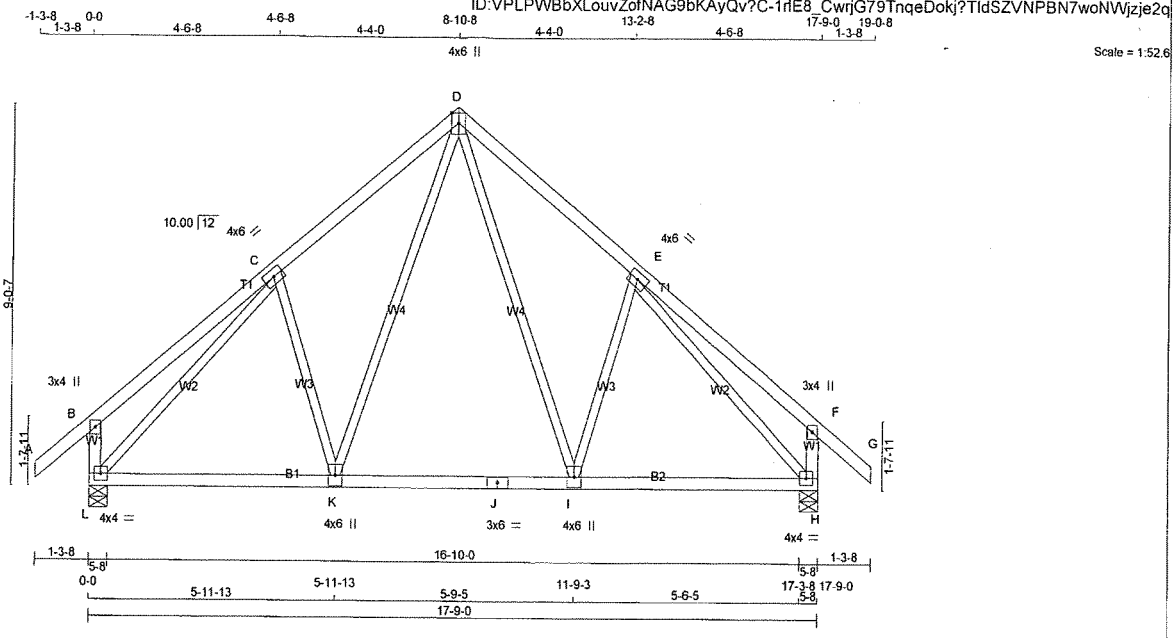


TOTAL WEIGHT = 87 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1148	0	1148	0	0	5-8	5-8
H	1148	0	1148	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	870	429 / 0	186 / 0	0 / 0	0 / 0	254 / 0	0 / 0
H	870	429 / 0	186 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	D- I	0 / 443	0.10 (1)
B- C	0 / 26	-77.7	-77.7	0.25 (1)	10.00	I- E	-175 / 27	0.08 (1)
C- D	-892 / 0	-77.7	-77.7	0.20 (1)	6.25	K- D	0 / 443	0.10 (1)
D- E	-892 / 0	-77.7	-77.7	0.20 (1)	6.25	C- K	-175 / 27	0.08 (1)
E- F	0 / 26	-77.7	-77.7	0.25 (1)	10.00	L- C	-1086 / 0	0.86 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	E- H	-1086 / 0	0.86 (1)
L- B	-238 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-238 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 720	-39.5	-39.5	0.33 (2)	10.00			
K- J	0 / 527	-39.5	-39.5	0.30 (2)	10.00			
J- I	0 / 527	-39.5	-39.5	0.30 (2)	10.00			
I- H	0 / 720	-39.5	-39.5	0.33 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.33/1.00 (H-I:2) , WB=0.86/1.00 (E-H:1) , SSI=0.15/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

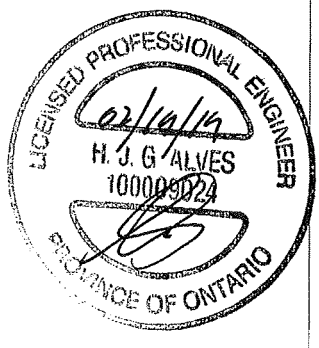
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)



DWG NO. TAM 1401351(
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T20X	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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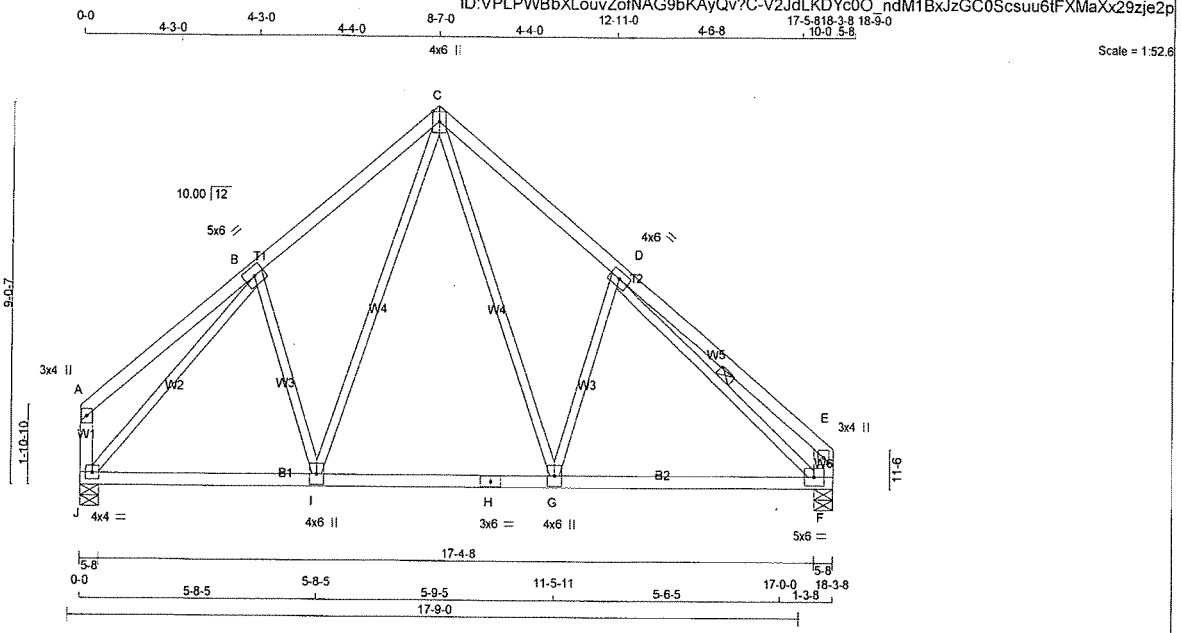


TOTAL WEIGHT = 5 X 84 = 422 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	4.0		
F	BMWW1-t	MT20	5.0	6.0		
G	BMWW+t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1058	0	1058	0	0	5-8	5-8
F	1066	0	1066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	809	377 / 0	190 / 0	0 / 0	0 / 0	242 / 0	0 / 0
F	814	381 / 0	190 / 0	0 / 0	0 / 0	243 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

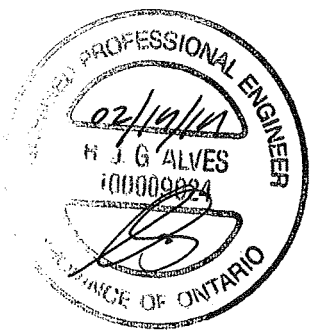
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-77.7	-77.7	0.23 (1)	10.00
B- C	-886 / 0	-77.7	-77.7	0.19 (1)	6.25
C- D	-969 / 0	-77.7	-77.7	0.24 (1)	6.04
D- E	0 / 50	-77.7	-77.7	0.29 (1)	10.00
J- A	-119 / 0	0.0	0.0	0.01 (1)	7.81
J- I	0 / 700	-39.5	-39.5	0.30 (2)	10.00
I- H	0 / 545	-39.5	-39.5	0.35 (2)	10.00
H- G	0 / 545	-39.5	-39.5	0.35 (2)	10.00
G- F	0 / 805	-39.5	-39.5	0.39 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	0 / 562	0.13 (1)
G- D	-272 / 0	0.13 (1)
I- C	0 / 375	0.08 (1)
B- I	-122 / 52	0.06 (1)
J- B	-1099 / 0	0.81 (1)
D- F	-1160 / 0	0.39 (1)
F- E	-142 / 0	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.39/1.00 (F-G:2) , WB=0.81/1.00 (B-J:1) , SSI=0.16/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

DWG NO. TAM 179035/2
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T21	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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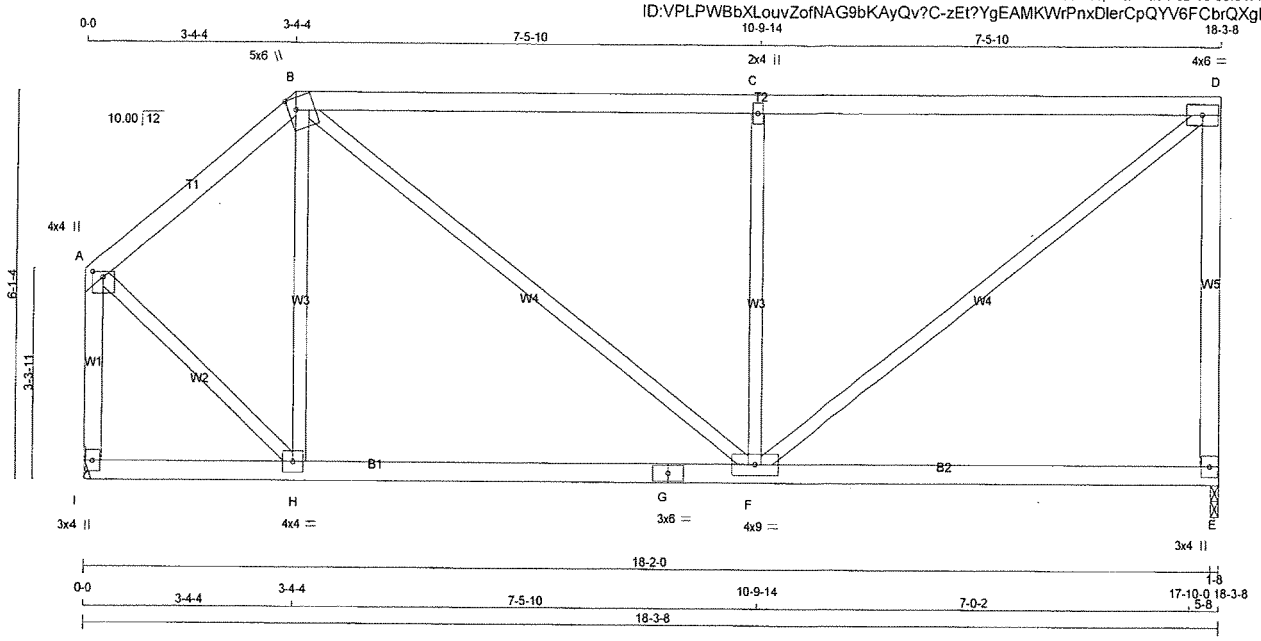


TOTAL WEIGHT = 2 X 80 = 160 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

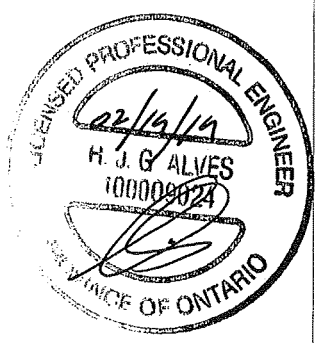
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-698 / 0	-77.7	-77.7	1	0.16 (1)	6.25	H- B	-227 / 25	0.13 (1)
B- C	-947 / 0	-77.7	-77.7	1	0.82 (1)	4.83	B- F	0 / 529	0.12 (1)
C- D	-947 / 0	-77.7	-77.7	1	0.82 (1)	4.83	F- C	-719 / 0	0.42 (1)
E- D	-954 / 0	0.0	0.0	1	0.69 (1)	7.81	F- D	0 / 1197	0.27 (1)
I- A	-1038 / 0	0.0	0.0	1	0.20 (1)	7.71	A- H	0 / 704	0.16 (1)
I- H	0 / 0	-39.5	-39.5	3	0.21 (3)	10.00			
H- G	0 / 530	-39.5	-39.5	3	0.48 (3)	10.00			
G- F	0 / 530	-39.5	-39.5	3	0.48 (3)	10.00			
F- E	0 / 0	-39.5	-39.5	3	0.43 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.82/1.00 (C-D:1) , BC=0.48/1.00 (F-H:3) , WB=0.42/1.00 (C-F:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.30 (G) (INPUT = 1.00)

DWG NO. TAM 1904035/3
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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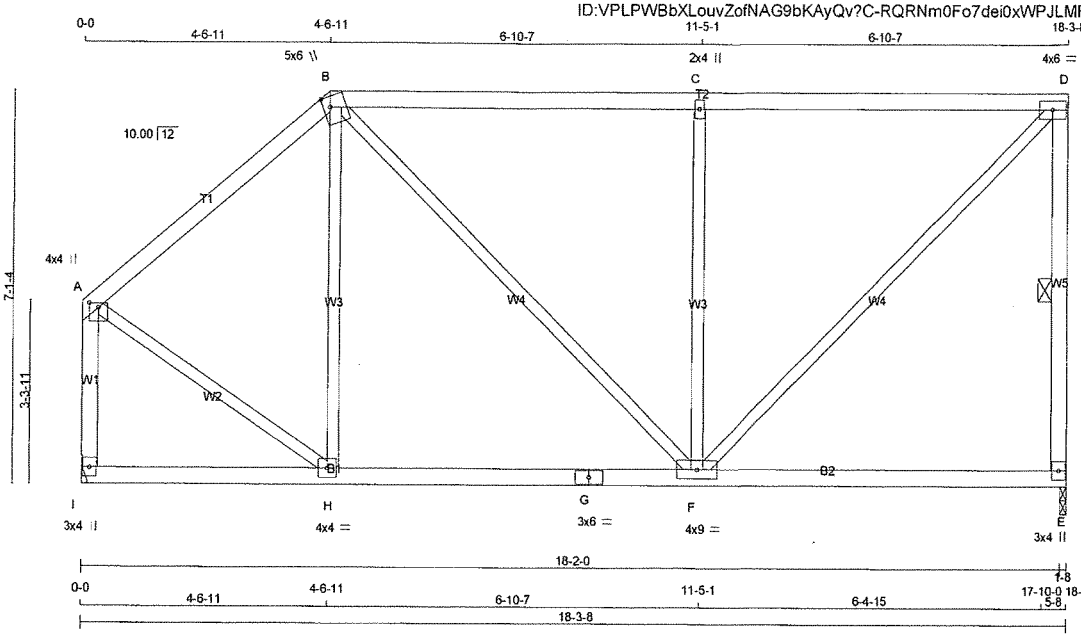


TOTAL WEIGHT = 2 X 84 = 168 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

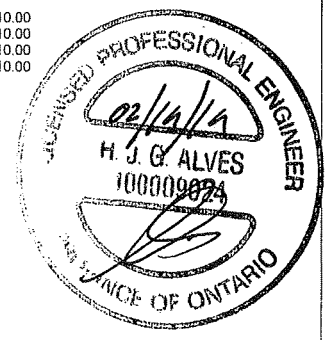
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-732 / 0	-77.7	-77.7	0.30 (1)	6.25	H- B	-119 / 95	0.10 (1)
B- C	-770 / 0	-77.7	-77.7	0.66 (1)	5.66	B- F	0 / 298	0.07 (1)
C- D	-770 / 0	-77.7	-77.7	0.67 (1)	5.66	F- C	-661 / 0	0.58 (1)
E- D	-963 / 0	0.0	0.0	0.22 (1)	6.25	F- D	0 / 1081	0.24 (1)
I- A	-1005 / 0	0.0	0.0	0.19 (1)	7.80	A- H	0 / 665	0.15 (1)
I- H	0 / 0	-39.5	-39.5	0.21 (3)	10.00			
H- G	0 / 559	-39.5	-39.5	0.42 (2)	10.00			
G- F	0 / 559	-39.5	-39.5	0.42 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.36 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.67/1.00 (C-D:1) , BC=0.42/1.00 (F-H:2) , WB=0.58/1.00 (C-F:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.24 (D) (INPUT = 1.00)

DWG NO. TAM T70314
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T23	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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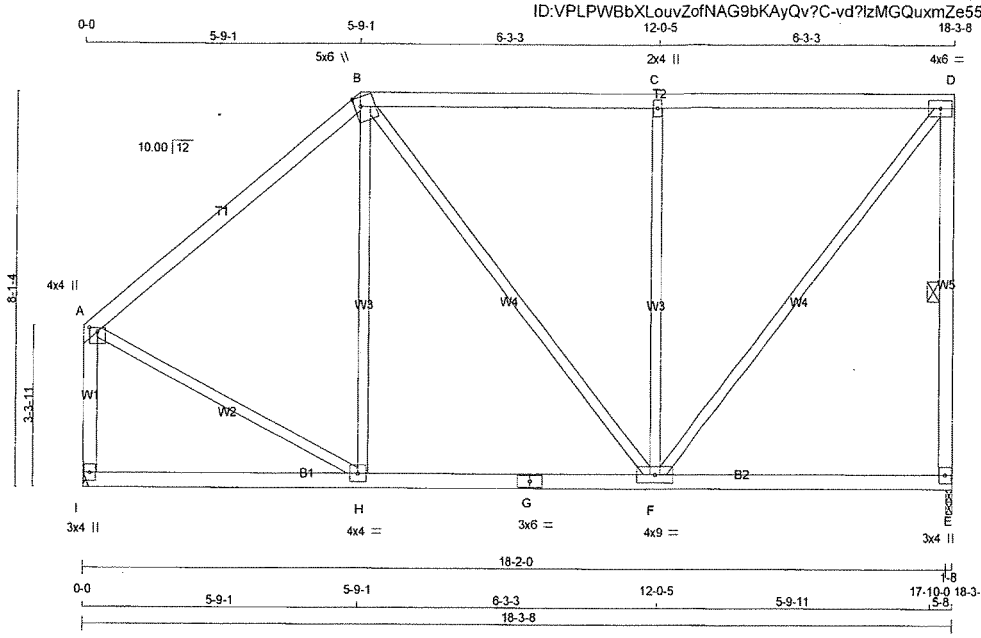


TOTAL WEIGHT = 2 X 89 = 177 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

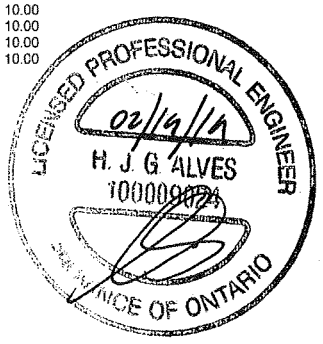
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-733 / 0	-77.7	-77.7	0.49 (1)	6.23	H- B	-27 / 161	0.04 (3)
B- C	-633 / 0	-77.7	-77.7	0.54 (1)	6.25	B- F	0 / 115	0.03 (1)
C- D	-633 / 0	-77.7	-77.7	0.54 (1)	6.25	F- C	-603 / 0	0.76 (1)
E- D	-971 / 0	0.0	0.0	0.29 (1)	6.25	F- D	0 / 1008	0.23 (1)
I- A	-979 / 0	0.0	0.0	0.18 (1)	7.81	A- H	0 / 630	0.14 (1)
I- H	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
H- G	0 / 562	-39.5	-39.5	0.34 (2)	10.00			
G- F	0 / 562	-39.5	-39.5	0.34 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.34/1.00 (F-H:2) , WB=0.76/1.00 (C-F:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (F) (INPUT = 0.90)
JSI METAL= 0.27 (G) (INPUT = 1.00)

DWG NO. TAM T1903515
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T24	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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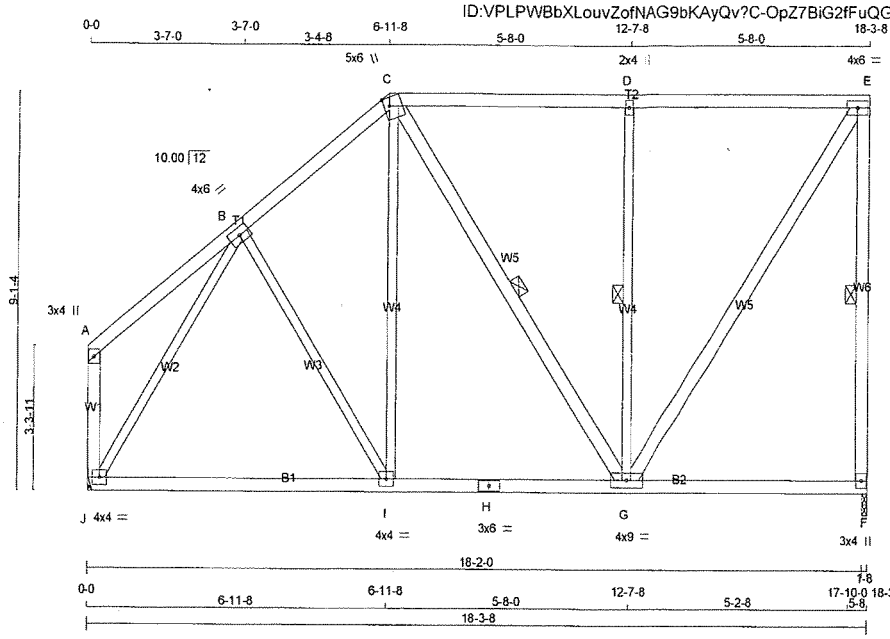


TOTAL WEIGHT = 2 X 105 = 209 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

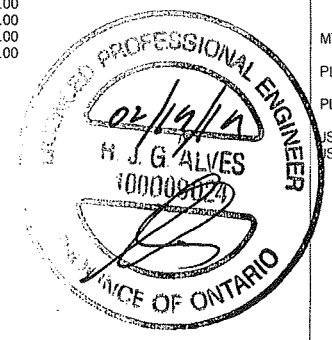
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-77.7	-77.7	0.15 (1)		10.00
B- C	-731 / 0	-77.7	-77.7	0.16 (1)		6.25
C- D	-524 / 0	-77.7	-77.7	0.43 (1)		6.25
D- E	-524 / 0	-77.7	-77.7	0.43 (1)		6.25
F- E	-978 / 0	0.0	0.0	0.38 (1)		6.25
J- A	-106 / 0	0.0	0.0	0.02 (1)		7.81
J- I	0 / 506	-39.5	-39.5	0.37 (2)		10.00
I- H	0 / 547	-39.5	-39.5	0.38 (2)		10.00
H- G	0 / 547	-39.5	-39.5	0.38 (2)		10.00
G- F	0 / 0	-39.5	-39.5	0.20 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- I	0 / 118	0.03 (3)
I- C	0 / 271	0.06 (3)
C- G	-59 / 0	0.03 (3)
G- D	-545 / 0	0.29 (1)
G- E	0 / 964	0.15 (1)
J- B	-991 / 0	0.83 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.38/1.00 (G-I:2) , WB=0.83/1.00 (B-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.73 (J) (INPUT = 0.90)
SI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM T9035 16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T25	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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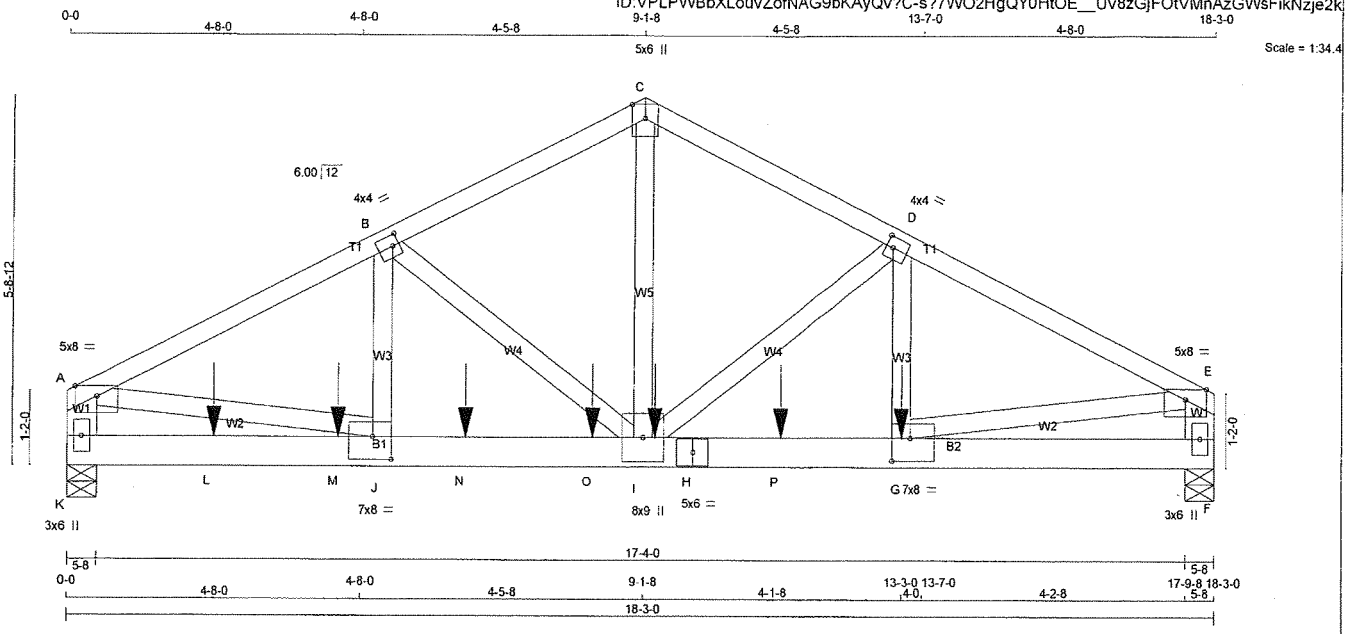

TOTAL WEIGHT = 2 X 92 = 184 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- C	1	6	SIDE(86.3)
D- G	1	3	SIDE(389.0)
2x4	1	6	
B- J	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	4.25	3.50
H	BS-t	MT20	5.0	6.0		
I	BMWWW+t	MT20	8.0	9.0		
J	BMWW-t	MT20	7.0	8.0	4.25	3.50
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	5506	0	5506	0	0	5-8	5-8
F	5020	0	5020	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4185	2025 / 0	926 / 0	0 / 0	0 / 0	1234 / 0	0 / 0
F	3816	1846 / 0	844 / 0	0 / 0	0 / 0	1125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-8062 / 0	-77.7	-77.7	0.48 (1)	3.18	I- C	0 / 5430	0.48 (1)
B- C	-6377 / 0	-77.7	-77.7	0.26 (1)	3.71	I- D	-1973 / 0	0.32 (1)
C- D	-6377 / 0	-77.7	-77.7	0.26 (1)	3.71	G- D	0 / 1592	0.14 (1)
D- E	-8081 / 0	-77.7	-77.7	0.48 (1)	3.17	B- I	-1951 / 0	0.32 (1)
K- A	-4899 / 0	0.0	0.0	0.17 (1)	6.56	J- B	0 / 1570	0.14 (1)
F- E	-4910 / 0	0.0	0.0	0.17 (1)	6.55	A- J	0 / 7306	0.65 (1)
						G- E	0 / 7324	0.65 (1)
K- L	0 / 0	-39.5	-39.5	0.48 (1)	10.00			
L- M	0 / 0	-39.5	-39.5	0.48 (1)	10.00			
M- J	0 / 0	-39.5	-39.5	0.48 (1)	10.00			
J- N	0 / 7220	-39.5	-39.5	0.81 (1)	10.00			
N- O	0 / 7220	-39.5	-39.5	0.81 (1)	10.00			
O- I	0 / 7220	-39.5	-39.5	0.81 (1)	10.00			
I- H	0 / 7238	-39.5	-39.5	0.77 (1)	10.00			
H- P	0 / 7238	-39.5	-39.5	0.77 (1)	10.00			
P- G	0 / 7238	-39.5	-39.5	0.77 (1)	10.00			
G- F	0 / 0	-39.5	-39.5	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	13-3-0	-2193	-2193	---	BACK	VERT	TOTAL	---	---
I	9-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
L	2-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
M	4-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
N	6-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
O	8-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
P	11-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.81/1.00 (I-J:1) , WB=0.65/1.00 (E-G:1) , SSI=0.55/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.70 (H) (INPUT = 1.00)

PROFESSIONAL ENGINEER
2019/02/19
M. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM 7190317
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T25Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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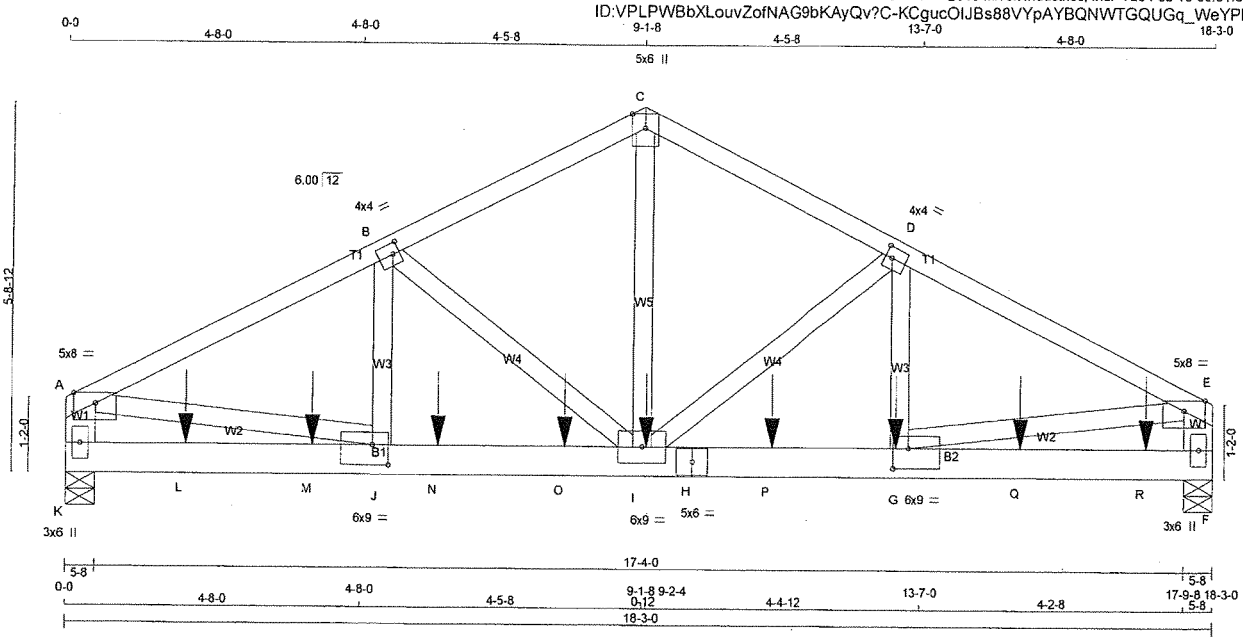


TOTAL WEIGHT = 2 X 92 = 184 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I- C	1	6	SIDE(82.8)
D- G	1	6	SIDE(82.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-p	MT20	5.0	8.0	Edge	
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	6.0	9.0	3.75	3.00
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	6.0	9.0		
J	BMWW-t	MT20	6.0	9.0	3.75	3.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	5504	0	5504	0	0	5-8	5-8
F	5859	0	5859	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4184	2024 / 0	926 / 0	0 / 0	0 / 0	1234 / 0	0 / 0
F	4453	2156 / 0	985 / 0	0 / 0	0 / 0	1313 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

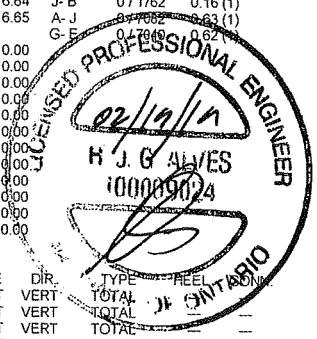
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-7814 / 0	-77.7	-77.7	0.46 (1)	3.24	I- C	0 / 5060	0.45 (1)
B- C	-5963 / 0	-77.7	-77.7	0.24 (1)	3.84	I- D	-2087 / 0	0.34 (1)
C- D	-5963 / 0	-77.7	-77.7	0.24 (1)	3.84	G- D	0 / 1707	0.15 (1)
D- E	-7767 / 0	-77.7	-77.7	0.46 (1)	3.25	B- I	-2141 / 0	0.35 (1)
K- A	-4753 / 0	0.0	0.0	0.17 (1)	6.64	J- B	0 / 1762	0.16 (1)
F- E	-4726 / 0	0.0	0.0	0.17 (1)	6.65	A- J	0 / 7052	0.63 (1)
						G- E	0 / 7040	0.62 (1)
K- L	0 / 0	-39.5	-39.5	0.50 (1)	10.00			
L- M	0 / 0	-39.5	-39.5	0.50 (1)	10.00			
M- J	0 / 0	-39.5	-39.5	0.50 (1)	10.00			
J- N	0 / 6999	-39.5	-39.5	0.85 (1)	10.00			
N- O	0 / 6999	-39.5	-39.5	0.85 (1)	10.00			
O- I	0 / 6999	-39.5	-39.5	0.85 (1)	10.00			
I- H	0 / 6957	-39.5	-39.5	0.75 (1)	10.00			
H- P	0 / 6957	-39.5	-39.5	0.75 (1)	10.00			
P- G	0 / 6957	-39.5	-39.5	0.75 (1)	10.00			
G- Q	0 / 0	-39.5	-39.5	0.46 (1)	10.00			
Q- R	0 / 0	-39.5	-39.5	0.46 (1)	10.00			
R- F	0 / 0	-39.5	-39.5	0.46 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	JOINT
G	13-2-4	-1019	-1019	---	FRONT	VERT	TOTAL	---	---
I	9-2-4	-1019	-1019	---	FRONT	VERT	TOTAL	---	---
L	1-10-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
M	3-10-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
N	5-10-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
O	7-10-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
P	11-2-4	-1019	-1019	---	FRONT	VERT	TOTAL	---	---
Q	15-2-4	-1019	-1019	---	FRONT	VERT	TOTAL	---	---
R	17-2-4	-1019	-1019	---	FRONT	VERT	TOTAL	---	---



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.46/1.00 (A-B:1) , BC=0.85/1.00 (I-J:1) , WB=0.63/1.00 (A-J:1) , SSI=0.50/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.68 (H) (INPUT = 1.00)

DWG NO. TAM P2003518
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T26	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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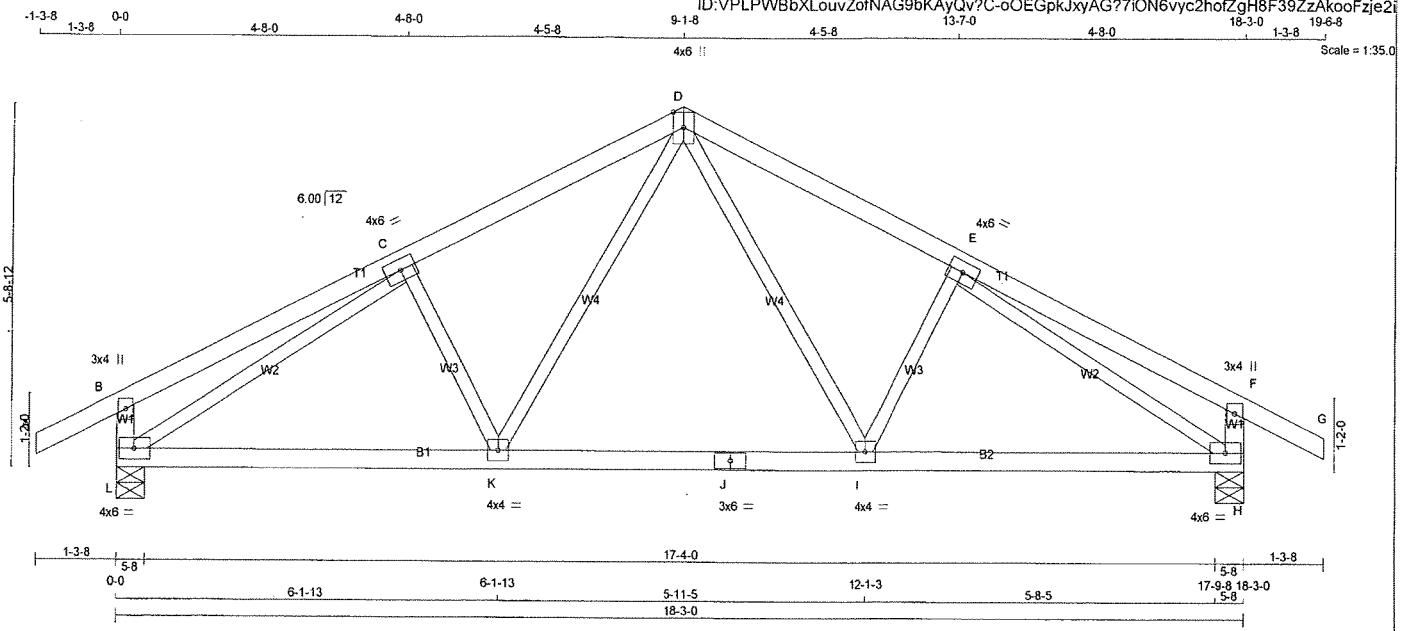


TOTAL WEIGHT = 2 X 73 = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1175	0	1175	0	0	5-8	5-8
H	1175	0	1175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0
H	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

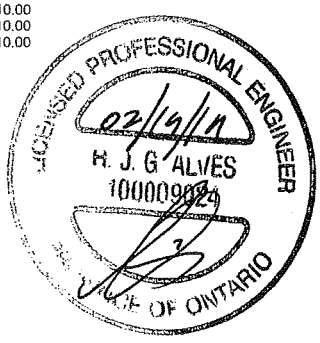
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 24	-77.7	-77.7	0.10 (1)	10.00
B- C		0 / 18	-77.7	-77.7	0.25 (1)	10.00
C- D		-1277 / 0	-77.7	-77.7	0.21 (1)	5.48
D- E		-1277 / 0	-77.7	-77.7	0.21 (1)	5.48
E- F		0 / 18	-77.7	-77.7	0.25 (1)	10.00
F- G		0 / 24	-77.7	-77.7	0.10 (1)	10.00
L- B		-243 / 0	0.0	0.0	0.02 (1)	7.81
H- F		-243 / 0	0.0	0.0	0.02 (1)	7.81
L- K		0 / 1209	-39.5	-39.5	0.41 (2)	10.00
K- J		0 / 893	-39.5	-39.5	0.37 (2)	10.00
J- I		0 / 893	-39.5	-39.5	0.37 (2)	10.00
I- H		0 / 1209	-39.5	-39.5	0.41 (2)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I		0 / 485	0.11 (1)
I- E		-185 / 36	0.04 (1)
K- D		0 / 485	0.11 (1)
C- K		-185 / 36	0.04 (1)
L- C		-1468 / 0	0.73 (1)
E- H		-1468 / 0	0.73 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.41/1.00 (K-L:2) , WB=0.73/1.00 (C-L:1) , SSI=0.15/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 7190319
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	G26	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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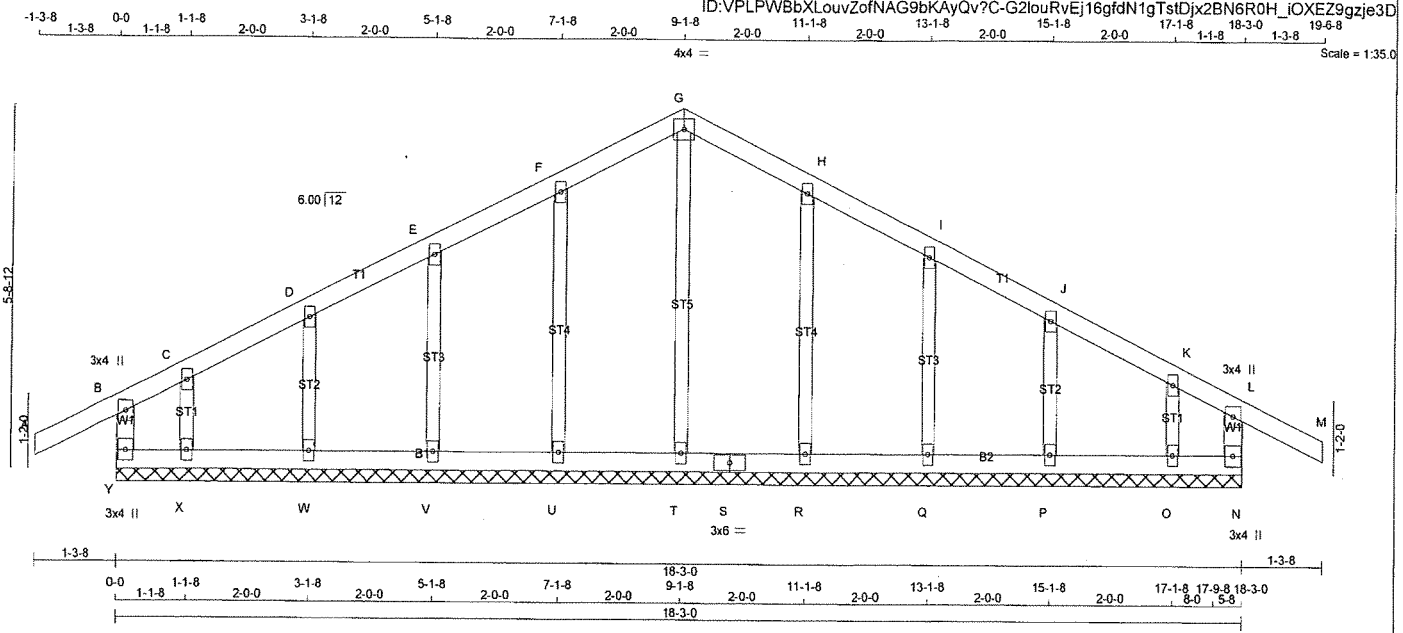


TOTAL WEIGHT = 2 X 71 = 142 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, R, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	T- G	-186 / 0	0.09 (1)
B- C	0 / 0	-77.7	-77.7	0.07 (1)	10.00	U- F	-160 / 0	0.05 (1)
C- D	0 / 24	-77.7	-77.7	0.04 (1)	10.00	V- E	-150 / 0	0.03 (1)
D- E	0 / 24	-77.7	-77.7	0.04 (1)	10.00	W- D	-164 / 0	0.03 (1)
E- F	0 / 29	-77.7	-77.7	0.04 (1)	10.00	X- C	-74 / 0	0.01 (1)
F- G	0 / 31	-77.7	-77.7	0.04 (1)	10.00	R- H	-160 / 0	0.05 (1)
G- H	0 / 31	-77.7	-77.7	0.04 (1)	10.00	Q- I	-150 / 0	0.03 (1)
H- I	0 / 29	-77.7	-77.7	0.04 (1)	10.00	P- J	-164 / 0	0.03 (1)
I- J	0 / 24	-77.7	-77.7	0.04 (1)	10.00	O- K	-74 / 0	0.01 (1)
J- K	0 / 24	-77.7	-77.7	0.04 (1)	10.00			
K- L	0 / 0	-77.7	-77.7	0.07 (1)	10.00			
L- M	0 / 24	-77.7	-77.7	0.10 (1)	10.00			
Y- B	-174 / 0	0.0	0.0	0.03 (1)	7.81			
N- L	-174 / 0	0.0	0.0	0.03 (1)	7.81			
Y- X	-15 / 0	-39.5	-39.5	0.02 (3)	6.25			
X- W	-18 / 0	-39.5	-39.5	0.03 (3)	6.25			
W- V	-23 / 0	-39.5	-39.5	0.03 (3)	6.25			
V- U	-25 / 0	-39.5	-39.5	0.02 (3)	6.25			
U- T	-28 / 0	-39.5	-39.5	0.02 (3)	6.25			
T- S	-28 / 0	-39.5	-39.5	0.02 (3)	6.25			
S- R	-28 / 0	-39.5	-39.5	0.02 (3)	6.25			
R- Q	-25 / 0	-39.5	-39.5	0.02 (3)	6.25			
Q- P	-23 / 0	-39.5	-39.5	0.03 (3)	6.25			
P- O	-18 / 0	-39.5	-39.5	0.03 (3)	6.25			
O- N	-15 / 0	-39.5	-39.5	0.02 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (P-Q:3) , WB=0.09/1.00 (G-T:1) . SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

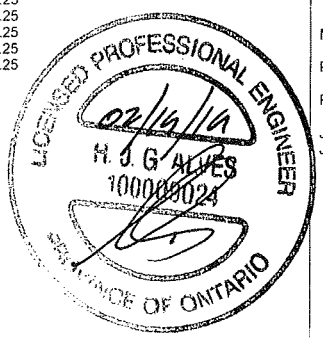
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (H) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



DWG NO. TAM 1403520
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T27	1	2	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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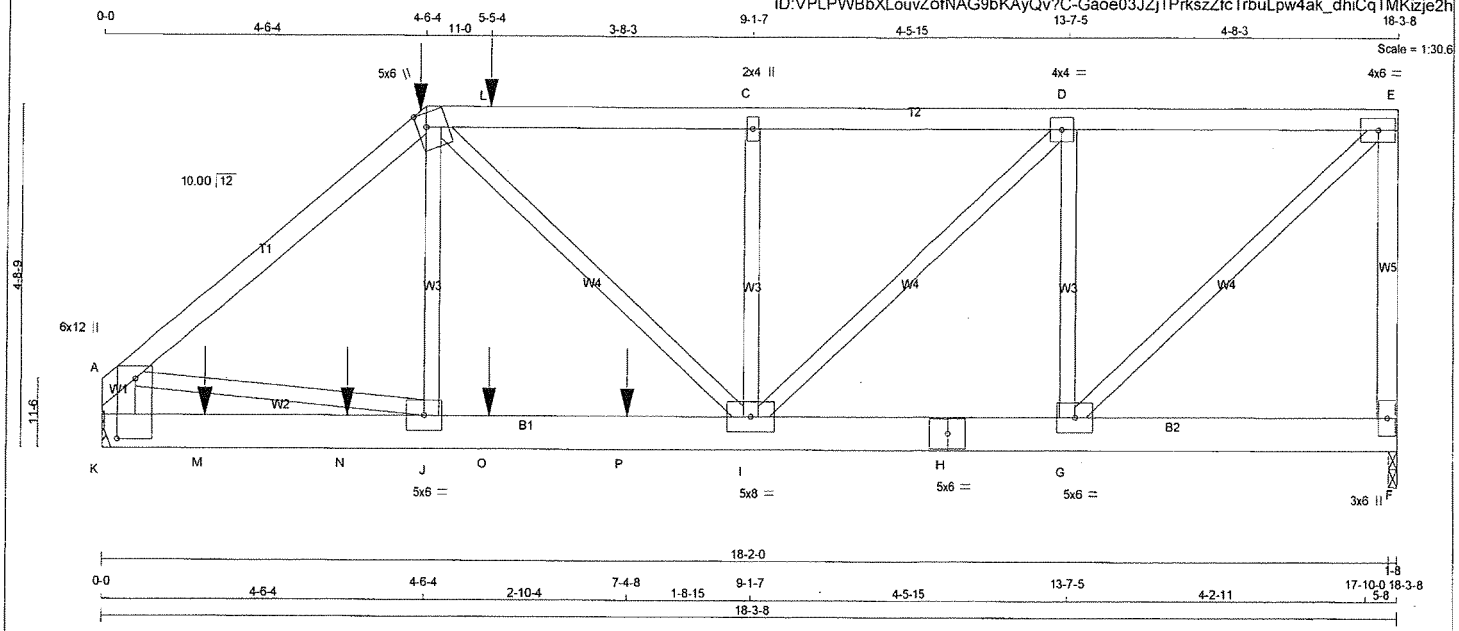


TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(0.0)
H- F	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A						
A	TMBMVW1*+p	MT20	6.0	12.0	10.00	3.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1709	0	1709	0	0	1-8	1-8
K	2232	0	2232	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1299	628 / 0	287 / 0	0 / 0	0 / 0	383 / 0	0 / 0
K	1699	815 / 0	382 / 0	0 / 0	0 / 0	503 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	-2747 / 0	-77.7	-77.7	0.20 (1)	5.32	
B- L	-2615 / 0	-77.7	-77.7	0.18 (1)	5.44	
L- C	-2615 / 0	-77.7	-77.7	0.18 (1)	5.44	
C- D	-2615 / 0	-77.7	-77.7	0.15 (1)	5.47	
D- E	-1573 / 0	-77.7	-77.7	0.13 (1)	6.25	
F- E	-1607 / 0	0.0	0.0	0.27 (1)	7.81	
K- A	-2180 / 0	0.0	0.0	0.08 (1)	7.81	
K- M	0 / 0	-39.5	-39.5	0.16 (2)	10.00	
M- N	0 / 0	-39.5	-39.5	0.16 (2)	10.00	
N- J	0 / 0	-39.5	-39.5	0.16 (2)	10.00	
J- O	0 / 2120	-39.5	-39.5	0.52 (1)	10.00	
O- P	0 / 2120	-39.5	-39.5	0.52 (1)	10.00	
P- I	0 / 2120	-39.5	-39.5	0.52 (1)	10.00	
I- H	0 / 1573	-39.5	-39.5	0.27 (1)	10.00	
H- G	0 / 1573	-39.5	-39.5	0.27 (1)	10.00	
G- F	0 / 0	-39.5	-39.5	0.05 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
J- B	0 / 681	0.08 (2)
A- J	0 / 2119	0.26 (1)
G- E	0 / 2145	0.27 (1)
B- I	0 / 679	0.08 (1)
G- D	-1395 / 0	0.22 (1)
I- C	-394 / 0	0.06 (1)
I- D	207 / 1449	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	4-6-4	-50	-56	---	FRONT	VERT	DEAD
B	4-6-4	-210	-210	---	FRONT	VERT	SNOW
L	5-5-4	-97	-97	---	FRONT	VERT	TOTAL
M	1-5-4	-57	-72	---	FRONT	VERT	TOTAL
N	3-5-4	-57	-72	---	FRONT	VERT	TOTAL
O	5-5-4	-57	-72	---	FRONT	VERT	TOTAL
P	7-4-8	-1269	-1269	---	FRONT	VERT	TOTAL

HEEL COMB

--- ---
--- ---
--- ---
--- ---
--- ---
--- ---
--- ---

DWG NO. TAM TM05521
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.52/1.00 (I-J:1) , WB=0.27/1.00 (E-G:1) , SSI=0.29/1.00 (I-J:1)

BOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

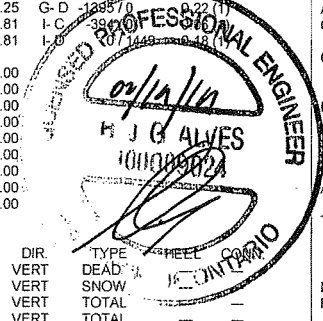
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.26 (J) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss. Burlington

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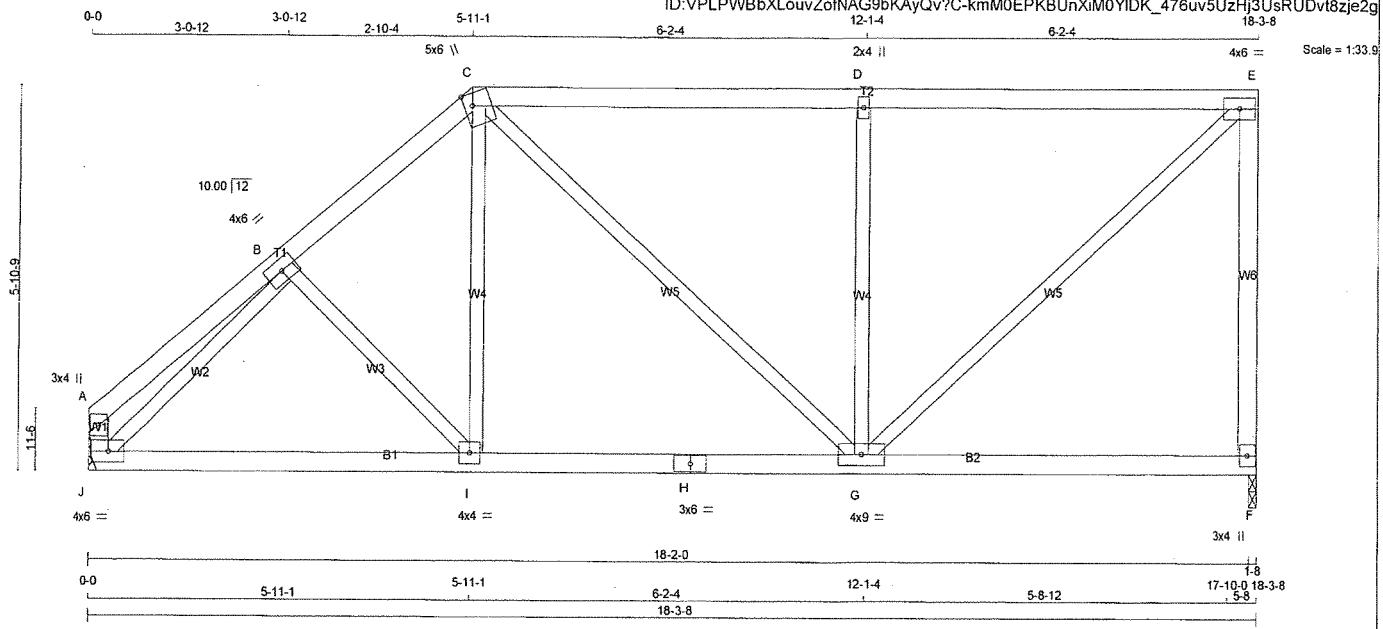


TOTAL WEIGHT = 79 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

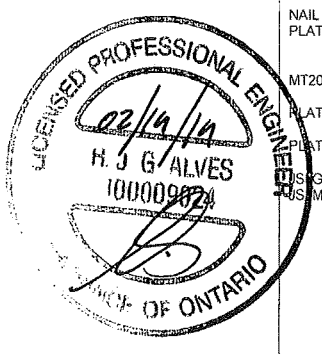
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-77.7	-77.7	0.09 (1)	10.00	B- I	-80 / 30	0.03 (1)
B- C	-1049 / 0	-77.7	-77.7	0.11 (1)	6.02	I- C	0 / 354	0.08 (2)
C- D	-877 / 0	-77.7	-77.7	0.53 (1)	5.70	C- G	0 / 108	0.02 (1)
D- E	-878 / 0	-77.7	-77.7	0.54 (1)	5.70	G- D	-594 / 0	0.32 (1)
F- E	-972 / 0	0.0	0.0	0.63 (1)	7.81	G- E	0 / 1179	0.27 (1)
J- A	-92 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1225 / 0	0.36 (1)
J- I	0 / 849	-39.5	-39.5	0.36 (2)	10.00			
I- H	0 / 798	-39.5	-39.5	0.37 (2)	10.00			
H- G	0 / 798	-39.5	-39.5	0.37 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.37/1.00 (G-I:2) , WB=0.36/1.00 (B-J:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

US GRIP= 0.78 (E) (INPUT = 0.90)
US METAL= 0.33 (H) (INPUT = 1.00)

DWG NO. TAM T1903522
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T29	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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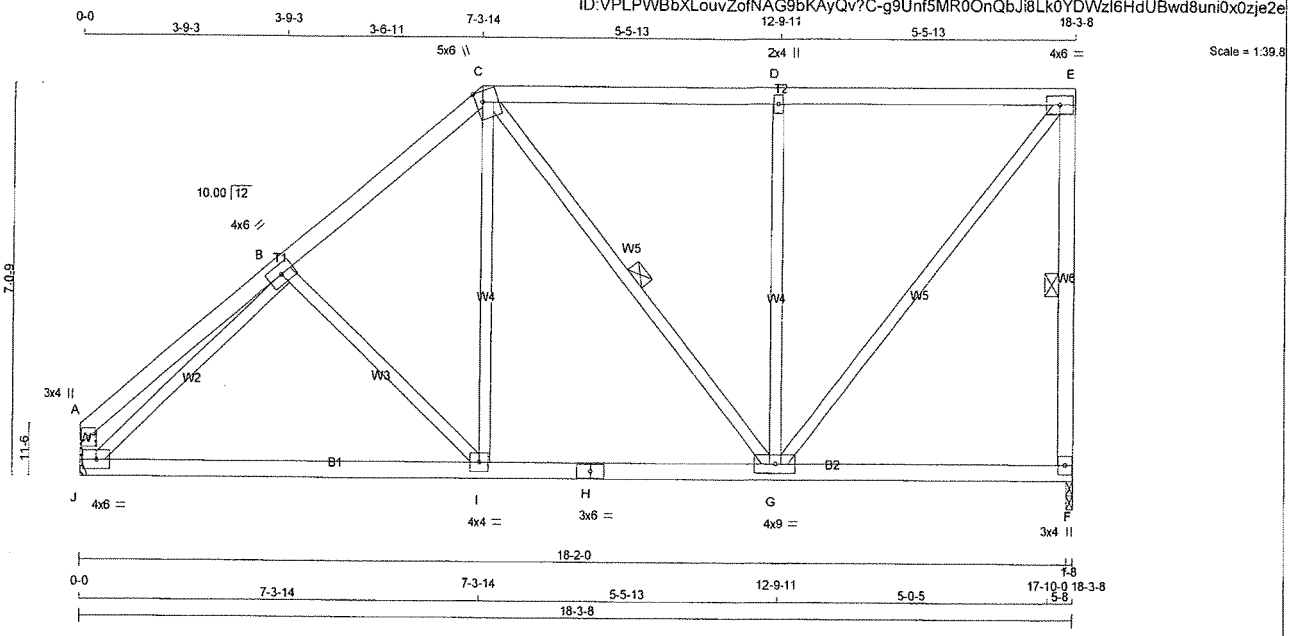


TOTAL WEIGHT = 84 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

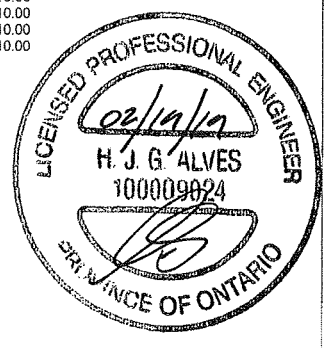
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.16	(1)	10.00	B- I	-162 / 3	0.07 (1)
B- C	-963 / 0	-77.7	-77.7	0.17	(1)	6.12	I- C	0 / 445	0.10 (2)
C- D	-667 / 0	-77.7	-77.7	0.40	(1)	6.25	C- G	-111 / 0	0.06 (2)
D- E	-667 / 0	-77.7	-77.7	0.40	(1)	6.25	G- D	-526 / 0	0.45 (1)
F- E	-979 / 0	0.0	0.0	0.22	(1)	6.25	G- E	0 / 1054	0.24 (1)
J- A	-110 / 0	0.0	0.0	0.01	(1)	7.81	J- B	-1188 / 0	0.51 (1)
J- I	0 / 840	-39.5	-39.5	0.45	(2)	10.00			
I- H	0 / 729	-39.5	-39.5	0.44	(2)	10.00			
H- G	0 / 729	-39.5	-39.5	0.44	(2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.45/1.00 (I-J:2) , WB=0.51/1.00 (B-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

DWG NO. TAM T1903523
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T34	1	2	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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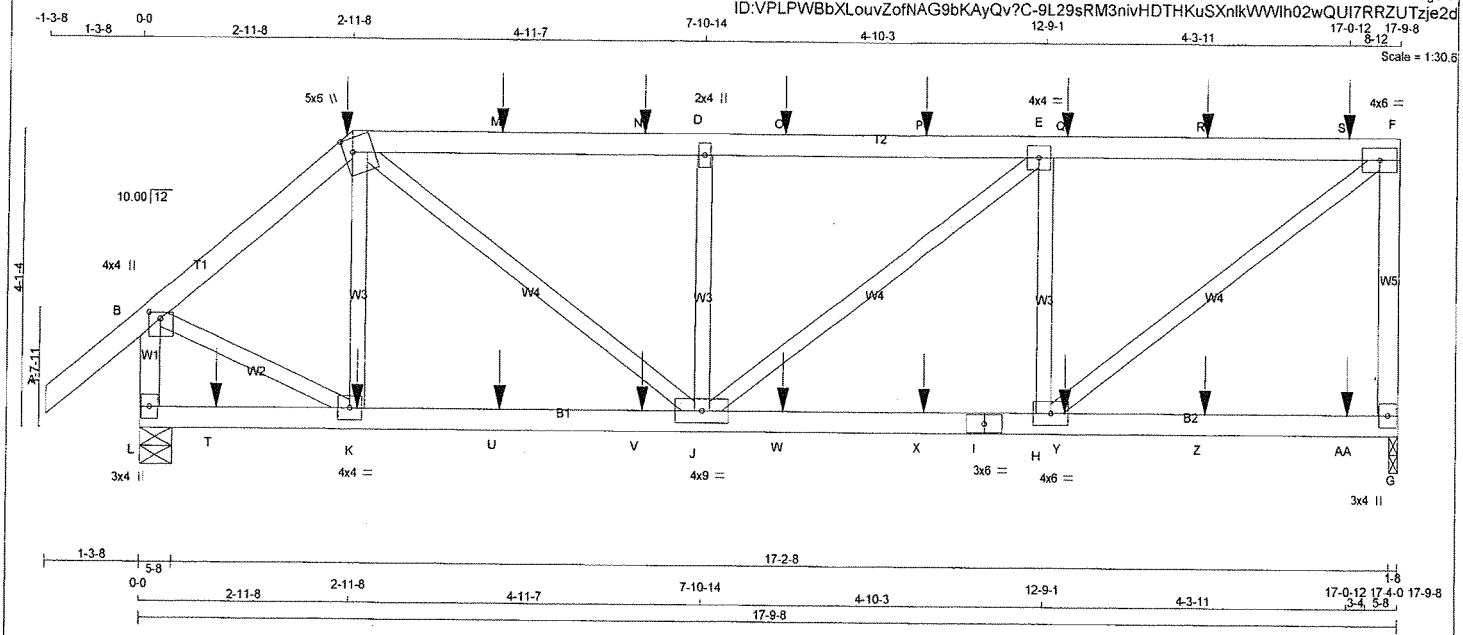

TOTAL WEIGHT = 2 X 74 = 149 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	1	12	SIDE(61.0)
I- G	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1776	0	1776	0	0	1-8	1-8
L	1912	0	1912	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1361	625 / 0	327 / 0	0 / 0	0 / 0	410 / 0	0 / 0
L	1453	705 / 0	319 / 0	0 / 0	0 / 0	428 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	K- C	-239 / 87	0.03 (1)
B- C	-1677 / 0	-77.7	-77.7	0.07 (1)	6.25	B- K	0 / 1396	0.17 (1)
C- M	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	H- F	0 / 2238	0.28 (1)
M- N	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	C- J	0 / 1102	0.14 (1)
N- D	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	H- E	-971 / 0	0.13 (1)
D- O	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75	D- J	357 / 0	0.04 (1)
O- P	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75	J- E	0 / 489	0.06 (1)
P- E	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75			
E- Q	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
Q- R	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
R- S	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
S- F	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
G- F	-1626 / 0	0.0	0.0	0.22 (1)	7.81			
L- B	-1849 / 0	0.0	0.0	0.11 (1)	7.81			
L- T	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
T- K	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
K- U	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
U- V	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
V- J	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
J- W	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
W- X	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
X- I	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
I- H	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
H- Y	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
Y- Z	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
Z-AA	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
AA- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-100	-100	---	FRONT	VERT	TOTAL	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
K	3-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
M	5-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
N	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
O	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
P	11-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	13-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---

PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100008924
PROVINCE OF ONTARIO

DWG NO. TAM 7140952A
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.30/1.00 (H-J:2) , WB=0.28/1.00 (F-H:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.31 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T34	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:VPLPWBbXLouvZofNAG9bKAyQv?C-9L29sRM3nivHDTHKuSXnlkWWlh02wQUI7RRZUTzje2d

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	15-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
S	17-0-12	-104	-104	—	FRONT	VERT	TOTAL	—	—
T	1-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
U	5-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
V	7-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
W	9-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
X	11-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Y	13-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Z	15-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
AA	17-0-12	-62	-78	—	FRONT	VERT	TOTAL	—	—

DWG NO. TAM T9035Z4
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T35	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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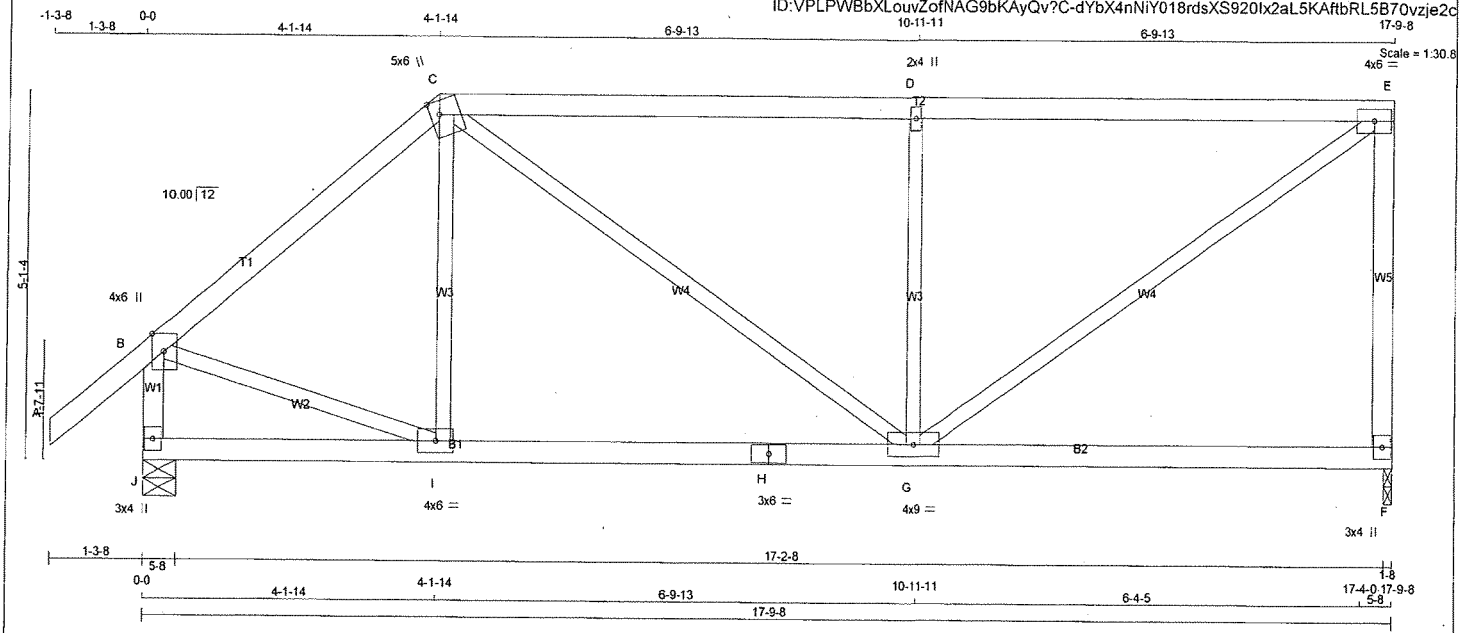

TOTAL WEIGHT = 74 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00		I- C	0 / 162	0.04 (3)
B- C	-937 / 0	-77.7	-77.7	0.25 (1)	6.07		C- G	0 / 397	0.09 (1)
C- D	-1042 / 0	-77.7	-77.7	0.68 (1)	5.05		G- D	-655 / 0	0.25 (1)
D- E	-1043 / 0	-77.7	-77.7	0.68 (1)	5.05		G- E	0 / 1274	0.29 (1)
F- E	-934 / 0	0.0	0.0	0.41 (1)	7.81		B- I	0 / 751	0.17 (1)
J- B	-1091 / 0	0.0	0.0	0.12 (1)	7.57				
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00				
I- H	0 / 718	-39.5	-39.5	0.43 (2)	10.00				
H- G	0 / 718	-39.5	-39.5	0.43 (2)	10.00				
G- F	0 / 0	-39.5	-39.5	0.36 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.29/1.00 (E-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM P403525
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T36	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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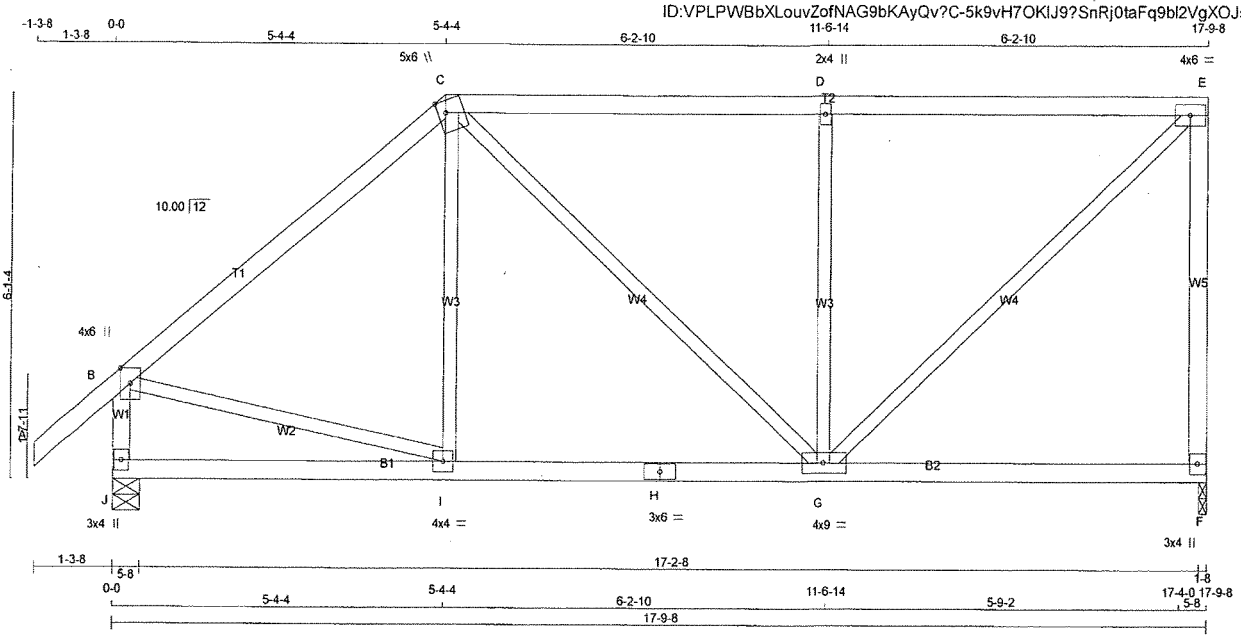


TOTAL WEIGHT = 79 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

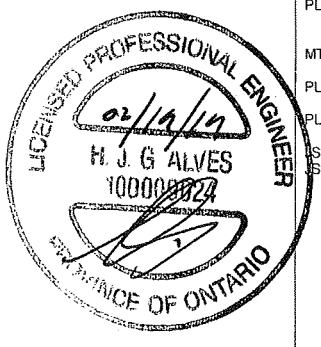
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 214	0.05 (3)	
B- C	-907 / 0	-77.7	-77.7	0.43 (1)	5.87	C- G	0 / 163	0.04 (1)	
C- D	-816 / 0	-77.7	-77.7	0.54 (1)	5.85	G- D	-597 / 0	0.35 (1)	
D- E	-816 / 0	-77.7	-77.7	0.54 (1)	5.85	G- E	0 / 1113	0.25 (1)	
F- E	-943 / 0	0.0	0.0	0.68 (1)	7.81	B- I	0 / 715	0.16 (1)	
J- B	-1065 / 0	0.0	0.0	0.11 (1)	7.64				
J- I	0 / 0	-39.5	-39.5	0.22 (3)	10.00				
I- H	0 / 697	-39.5	-39.5	0.36 (2)	10.00				
H- G	0 / 697	-39.5	-39.5	0.36 (2)	10.00				
G- F	0 / 0	-39.5	-39.5	0.28 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.36/1.00 (G-I:2) , WB=0.35/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.84 (I) (INPUT = 0.90)
SI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T1903526
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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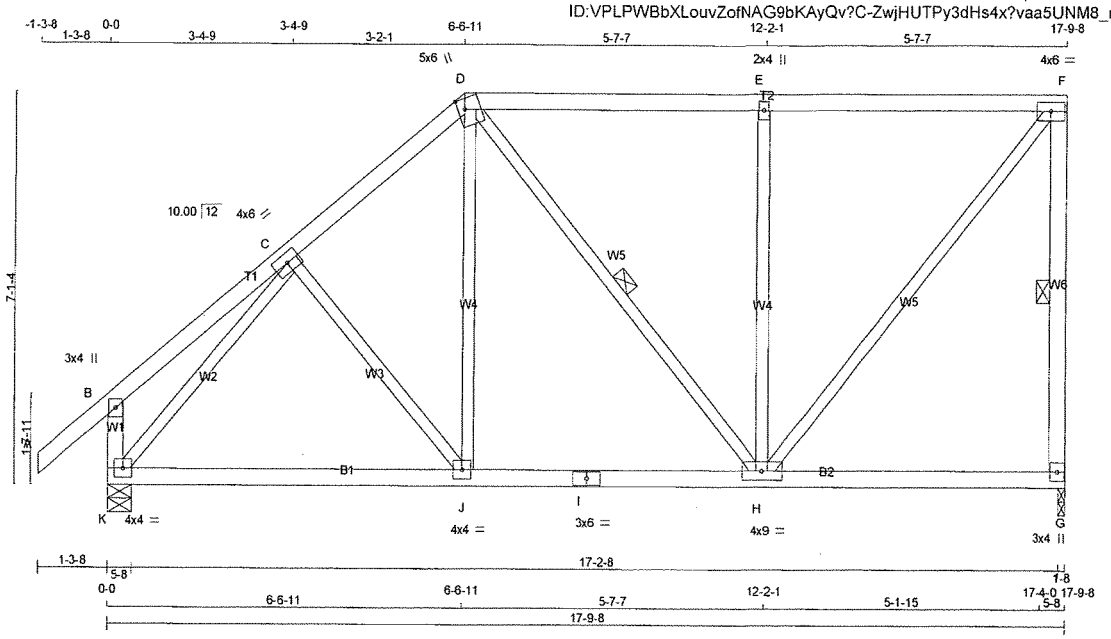


TOTAL WEIGHT = 86 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

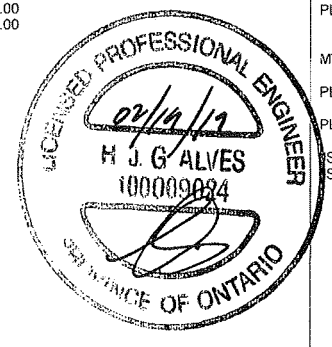
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-32 / 60	0.01 (1)
B- C	0 / 18	-77.7	-77.7	0.13 (1)	10.00	J- D	0 / 337	0.08 (2)
C- D	-874 / 0	-77.7	-77.7	0.14 (1)	6.25	D- H	-44 / 0	0.02 (3)
D- E	-649 / 0	-77.7	-77.7	0.42 (1)	6.25	H- E	-539 / 0	0.47 (1)
E- F	-650 / 0	-77.7	-77.7	0.43 (1)	6.25	H- F	0 / 1017	0.23 (1)
G- F	-950 / 0	0.0	0.0	0.22 (1)	6.25	K- C	-1076 / 0	0.46 (1)
K- B	-206 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 677	-39.5	-39.5	0.37 (2)	10.00			
J- I	0 / 660	-39.5	-39.5	0.37 (2)	10.00			
I- H	0 / 660	-39.5	-39.5	0.37 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.37/1.00 (H-J:2) , WB=0.47/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (K) (INPUT = 0.90)
SI METAL= 0.26 (C) (INPUT = 1.00)

DWG NO. TAM 179035Z7
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T38	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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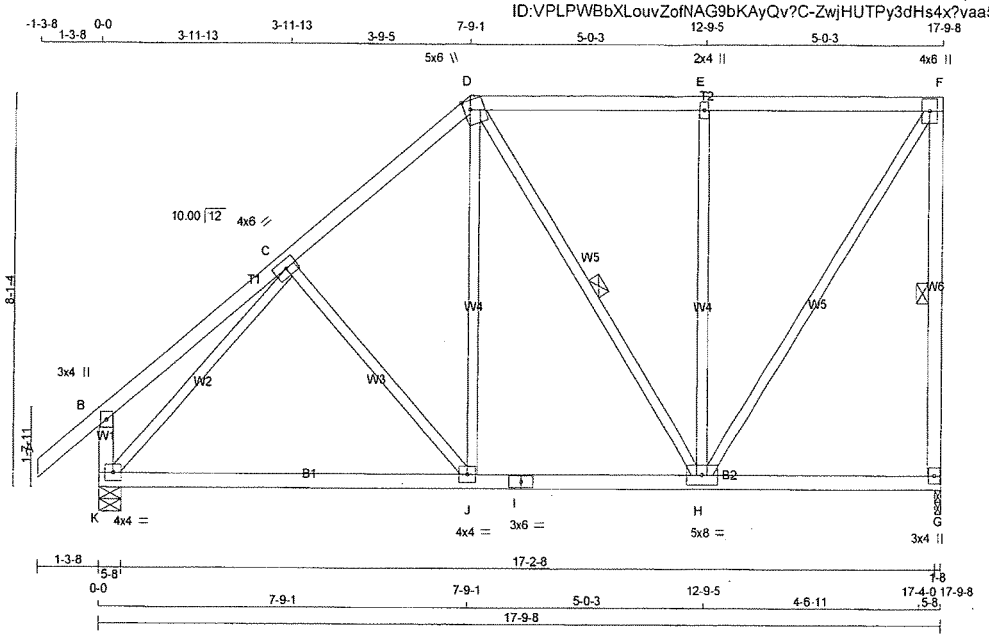


TOTAL WEIGHT = 91 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

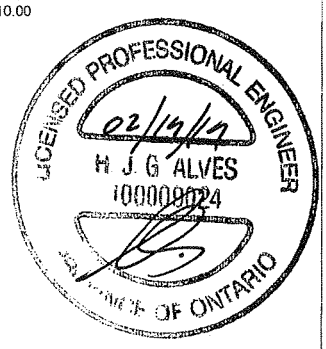
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00		C- J	-109 / 33	0.07 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00		J- D	0 / 432	0.10 (2)
C- D	-817 / 0	-77.7	-77.7	0.20 (1)	6.25		D- H	-182 / 0	0.11 (2)
D- E	-521 / 0	-77.7	-77.7	0.33 (1)	6.25		H- E	-481 / 0	0.61 (1)
E- F	-521 / 0	-77.7	-77.7	0.33 (1)	6.25		H- F	0 / 958	0.22 (1)
G- F	-955 / 0	0.0	0.0	0.28 (1)	6.25		K- C	-1051 / 0	0.62 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81				
K- J	0 / 682	-39.5	-39.5	0.47 (2)	10.00				
J- I	0 / 615	-39.5	-39.5	0.46 (2)	10.00				
I- H	0 / 615	-39.5	-39.5	0.46 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 826 (0.26")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.47/1.00 (J-K:2) , WB=0.62/1.00 (C-K:1) , SSI=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.52 (I) (INPUT = 1.00)

DWG NO. TAM 7903528
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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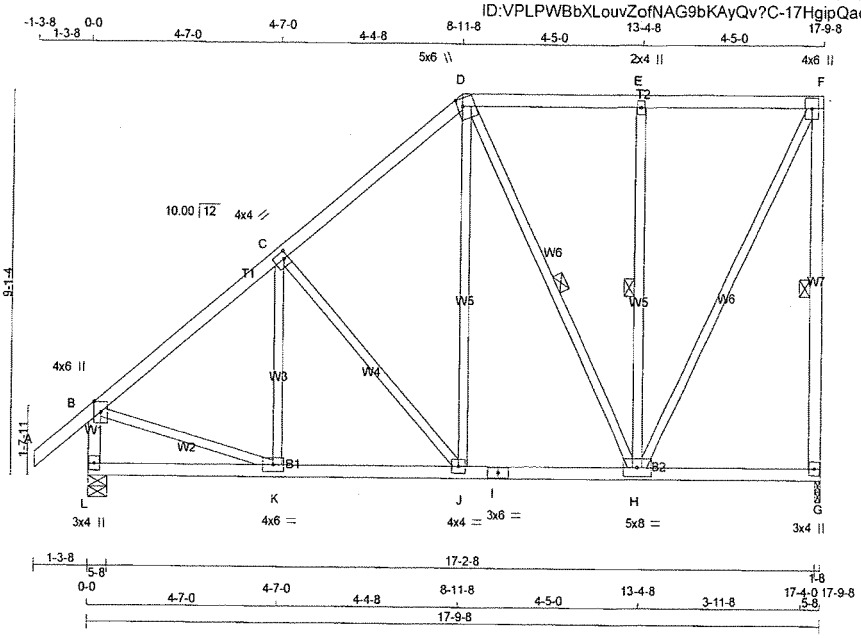


TOTAL WEIGHT = 99 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
L	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
L	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

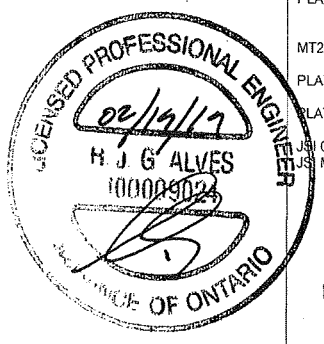
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	1 (1)	0.11	10.00
B- C	-943 / 0	-77.7	-77.7	1 (1)	0.29	6.02
C- D	-718 / 0	-77.7	-77.7	1 (1)	0.27	6.25
D- E	-424 / 0	-77.7	-77.7	1 (1)	0.25	6.25
E- F	-424 / 0	-77.7	-77.7	1 (1)	0.25	6.25
G- F	-971 / 0	0.0	0.0	1 (1)	0.37	6.25
L- B	-1077 / 0	0.0	0.0	1 (1)	0.11	7.60
L- K	0 / 0	-39.5	-39.5	1 (3)	0.15	10.00
K- J	0 / 744	-39.5	-39.5	1 (2)	0.25	10.00
J- I	0 / 534	-39.5	-39.5	1 (2)	0.21	10.00
I- H	0 / 534	-39.5	-39.5	1 (2)	0.21	10.00
H- G	0 / 0	-39.5	-39.5	1 (3)	0.14	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-9 / 142	0.03 (3)
C- J	-326 / 0	0.28 (1)
J- D	0 / 423	0.10 (2)
D- H	-248 / 0	0.17 (1)
H- E	-421 / 0	0.23 (1)
H- F	0 / 939	0.21 (1)
B- K	0 / 774	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.25/1.00 (J-K:2) , WB=0.28/1.00 (C-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903529
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T40	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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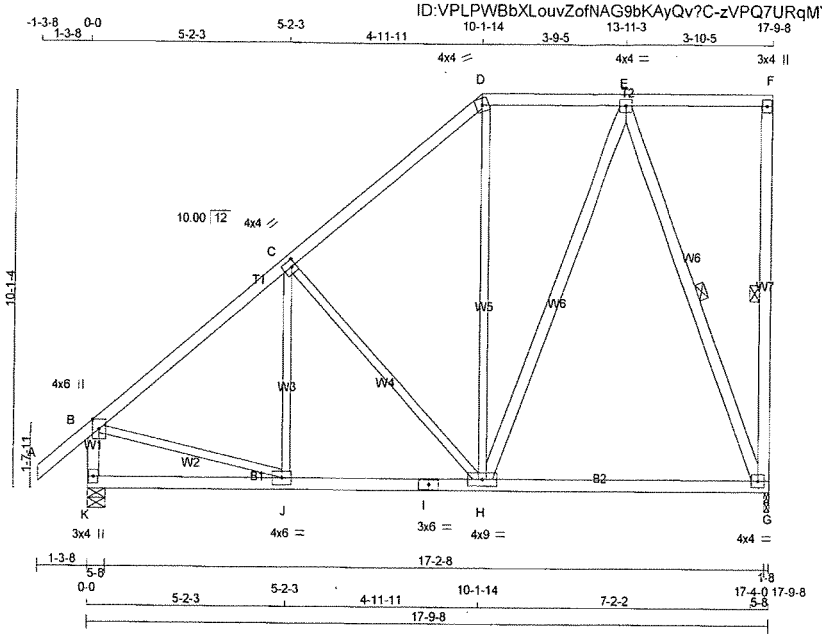


TOTAL WEIGHT = 2 X 104 = 208 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

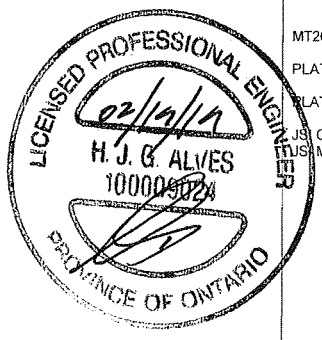
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-929 / 0	-77.7	-77.7	0.28 (1)	6.09
C- D	-658 / 0	-77.7	-77.7	0.26 (1)	6.25
D- E	-482 / 0	-77.7	-77.7	0.15 (1)	6.25
E- F	0 / 0	-77.7	-77.7	0.19 (1)	10.00
G- F	-114 / 0	0.0	0.0	0.06 (1)	6.25
K- B	-1062 / 0	0.0	0.0	0.11 (1)	7.64
K- J	0 / 0	-39.5	-39.5	0.18 (3)	10.00
J- I	0 / 737	-39.5	-39.5	0.45 (2)	10.00
I- H	0 / 737	-39.5	-39.5	0.45 (2)	10.00
H- G	0 / 317	-39.5	-39.5	0.40 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 142	0.03 (3)
C- H	-383 / 0	0.44 (1)
H- D	0 / 183	0.04 (3)
H- E	0 / 476	0.08 (2)
E- G	-866 / 0	0.51 (1)
B- J	0 / 760	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.45/1.00 (H-J:2) , WB=0.51/1.00 (E-G:1) , SSI=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.78 (B) (INPUT = 0.90)
JS METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903530
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T41	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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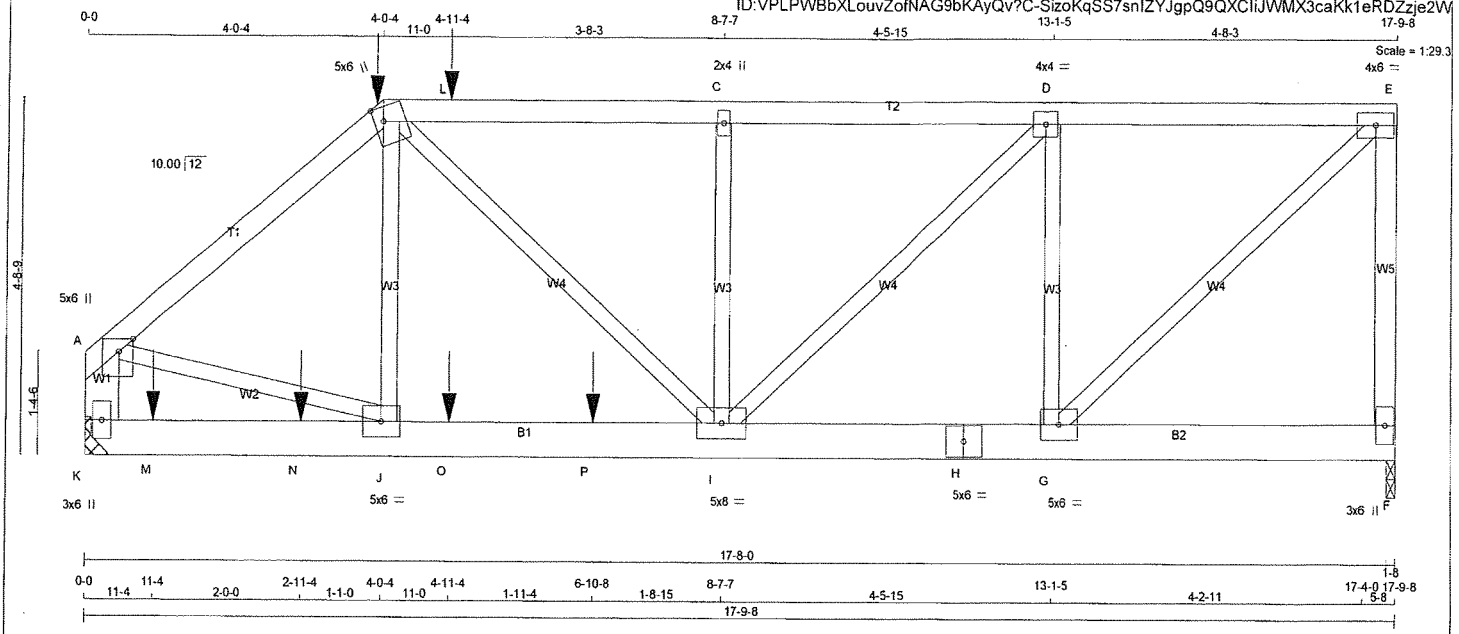


TOTAL WEIGHT = 2 X 87 = 174 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(0.0)
H- F	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1647	0	1647	0	0	1-8	1-8
K	2237	0	2237	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1252	606 / 0	277 / 0	0 / 0	0 / 0	369 / 0	0 / 0
K	1703	816 / 0	383 / 0	0 / 0	0 / 0	504 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

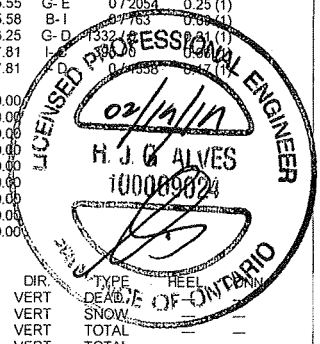
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2502 / 0	-77.7	-77.7	0.15 (1)	5.57	
B- L	-2484 / 0	-77.7	-77.7	0.17 (1)	5.55	
L- C	-2484 / 0	-77.7	-77.7	0.17 (1)	5.55	
C- D	-2484 / 0	-77.7	-77.7	0.15 (1)	5.58	
D- E	-1507 / 0	-77.7	-77.7	0.13 (1)	6.25	
F- E	-1546 / 0	0.0	0.0	0.26 (1)	7.81	
K- A	-2203 / 0	0.0	0.0	0.08 (1)	7.81	
K- M	0 / 0	-39.5	-39.5	0.17 (1)	10.00	
M- N	0 / 0	-39.5	-39.5	0.17 (1)	10.00	
N- J	0 / 0	-39.5	-39.5	0.17 (1)	10.00	
J- O	0 / 1927	-39.5	-39.5	0.50 (1)	10.00	
O- P	0 / 1927	-39.5	-39.5	0.50 (1)	10.00	
P- I	0 / 1927	-39.5	-39.5	0.50 (1)	10.00	
I- H	0 / 1507	-39.5	-39.5	0.26 (1)	10.00	
H- G	0 / 1507	-39.5	-39.5	0.26 (1)	10.00	
G- F	0 / 0	-39.5	-39.5	0.05 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 499	0.06 (2)
A- J	0 / 1966	0.24 (1)
G- E	0 / 2054	0.25 (1)
B- I	0 / 763	0.09 (1)
G- D	1332 / 0	0.21 (1)
I- C	786 / 0	0.09 (1)
D- I	0 / 1558	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	TENN
B	4-0-4	-50	-56	---	FRONT	VERT	DEAD	---	---
B	4-0-4	-210	-210	---	FRONT	VERT	SNOW	---	---
L	4-11-4	-97	-97	---	BACK	VERT	TOTAL	---	---
M	11-4	-58	-74	---	BACK	VERT	TOTAL	---	---
N	2-11-4	-57	-72	---	BACK	VERT	TOTAL	---	---
O	4-11-4	-57	-72	---	BACK	VERT	TOTAL	---	---
P	6-10-8	-1269	-1269	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM T1903531
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.26/1.00 (E-F:1) , BC=0.50/1.00 (I-J:1) , WB=0.25/1.00 (E-G:1) , SSI=0.29/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.24 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T42	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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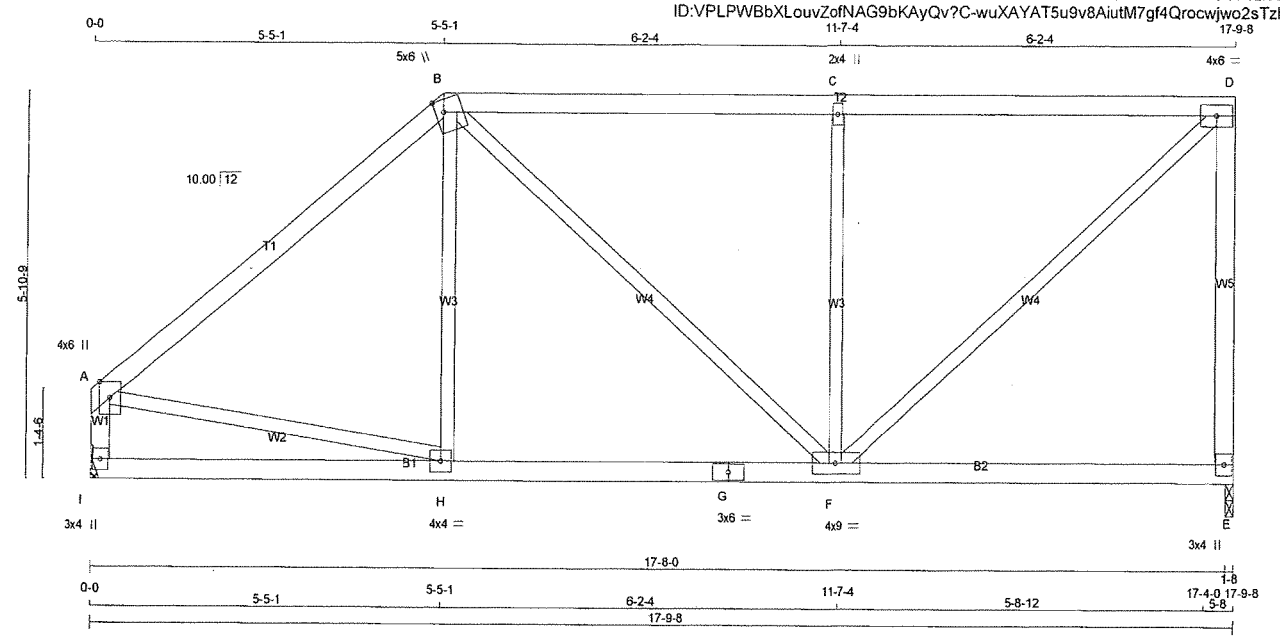


TOTAL WEIGHT = 76 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1043	0	1043	0	0	1-8	1-8
I	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

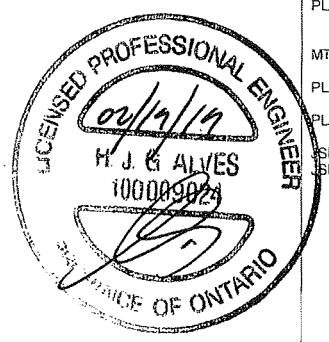
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-950 / 0	-77.7	-77.7	0.45 (1)	5.75	H- B	0 / 234	0.06 (3)
B- C	-845 / 0	-77.7	-77.7	0.53 (1)	5.79	B- F	0 / 156	0.04 (1)
C- D	-845 / 0	-77.7	-77.7	0.53 (1)	5.79	F- C	-594 / 0	0.32 (1)
E- D	-943 / 0	0.0	0.0	0.61 (1)	7.81	F- D	0 / 1136	0.26 (1)
I- A	-956 / 0	0.0	0.0	0.10 (1)	7.81	A- H	0 / 740	0.17 (1)
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00			
H- G	0 / 730	-39.5	-39.5	0.36 (2)	10.00			
G- F	0 / 730	-39.5	-39.5	0.36 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.61/1.00 (D-E:1) , BC=0.36/1.00 (F-H:2) , WB=0.32/1.00 (C-F:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.82 (H) (INPUT = 0.90)
SI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T9035S2
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T43	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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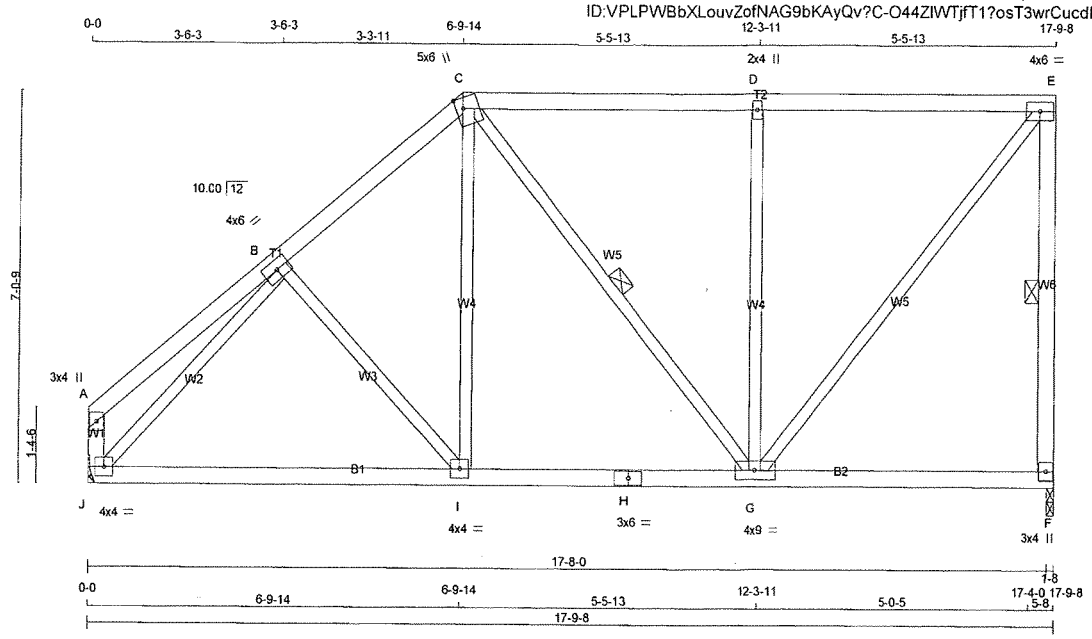


TOTAL WEIGHT = 83 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

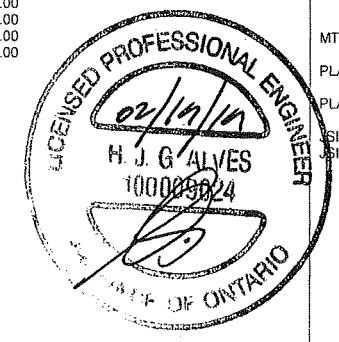
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-77.7	-77.7	0.14 (1)	10.00	B- I	-81 / 38	0.04 (1)
B- C	-897 / 0	-77.7	-77.7	0.15 (1)	6.25	I- C	0 / 378	0.08 (2)
C- D	-644 / 0	-77.7	-77.7	0.40 (1)	6.25	C- G	-68 / 0	0.03 (3)
D- E	-644 / 0	-77.7	-77.7	0.40 (1)	6.25	G- D	-526 / 0	0.45 (1)
F- E	-951 / 0	0.0	0.0	0.21 (1)	6.25	G- E	0 / 1017	0.23 (1)
J- A	-102 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1102 / 0	0.47 (1)
J- I	0 / 728	-39.5	-39.5	0.39 (2)	10.00			
I- H	0 / 678	-39.5	-39.5	0.39 (2)	10.00			
H- G	0 / 678	-39.5	-39.5	0.39 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.39/1.00 (I-J:2) , WB=0.47/1.00 (B-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.27 (J) (INPUT = 1.00)

DWG NO. TAM 7190 3533
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T44	1	1	TRUSS DESC:	

Tamarack Roof Truss, Burlington

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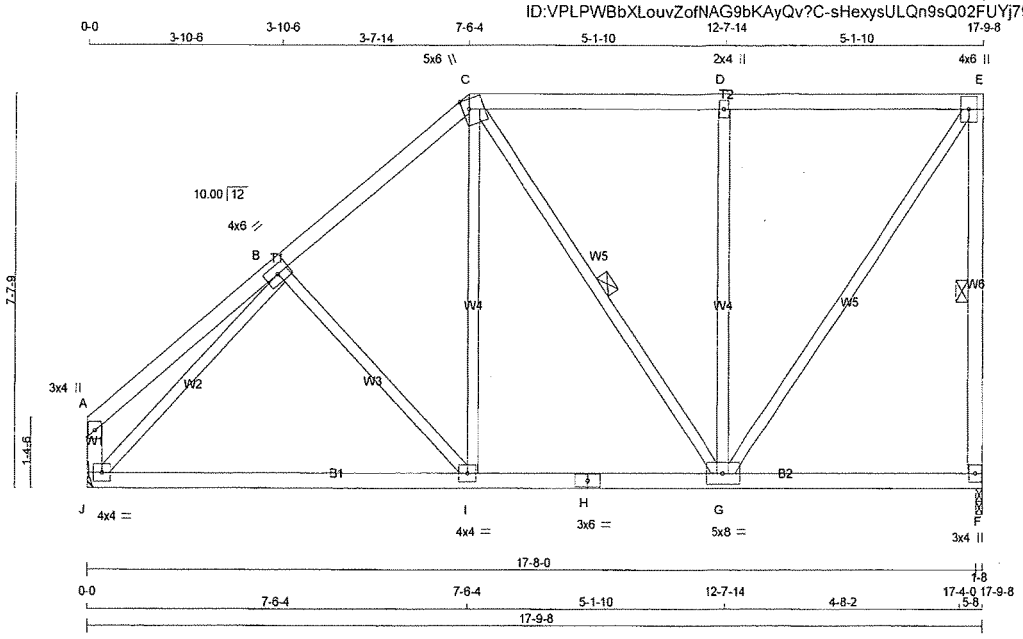


Scale = 1:43.0

TOTAL WEIGHT = 86 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

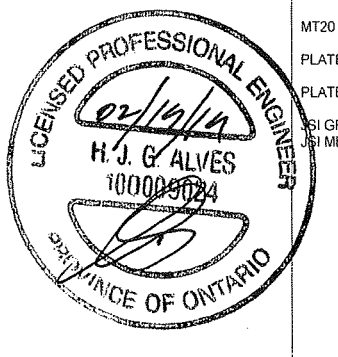
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-77.7	-77.7	0.17 (1)	10.00	B- I	-124 / 23	0.07 (1)
B- C	-859 / 0	-77.7	-77.7	0.18 (1)	6.25	I- C	0 / 430	0.10 (2)
C- D	-564 / 0	-77.7	-77.7	0.35 (1)	6.25	C- G	-155 / 0	0.08 (2)
D- E	-565 / 0	-77.7	-77.7	0.35 (1)	6.25	G- D	-492 / 0	0.52 (1)
F- E	-954 / 0	0.0	0.0	0.25 (1)	6.25	G- E	0 / 979	0.22 (1)
J- A	-112 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1086 / 0	0.55 (1)
J- I	0 / 727	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 648	-39.5	-39.5	0.44 (2)	10.00			
H- G	0 / 648	-39.5	-39.5	0.44 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 923 (0.23")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.45/1.00 (I-J:2) , WB=0.55/1.00 (B-J:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.68 (J) (INPUT = 0.90)
SSI METAL= 0.27 (J) (INPUT = 1.00)

DWG NO. TAM T1203534
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T45	1	1	TRUSS DESC.	

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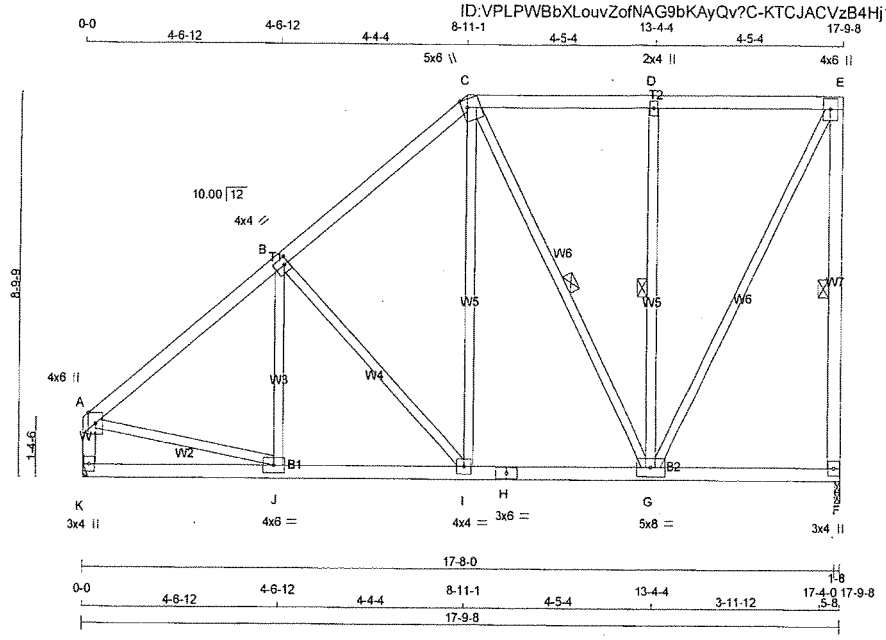


TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1043	0	1043	0	0	1-8	1-8
K	1043	0	1043	0	0	IN-SX	IN-SX
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

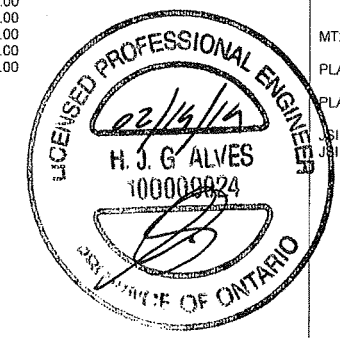
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-998 / 0	-77.7	-77.7	0.28 (1)	5.89
B- C	-743 / 0	-77.7	-77.7	0.27 (1)	6.25
C- D	-440 / 0	-77.7	-77.7	0.25 (1)	6.25
D- E	-440 / 0	-77.7	-77.7	0.25 (1)	6.25
F- E	-971 / 0	0.0	0.0	0.35 (1)	6.25
K- A	-969 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-39.5	-39.5	0.15 (3)	10.00
J- I	0 / 786	-39.5	-39.5	0.25 (2)	10.00
I- H	0 / 554	-39.5	-39.5	0.21 (2)	10.00
H- G	0 / 554	-39.5	-39.5	0.21 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 160	0.04 (3)
B- I	-351 / 0	0.28 (1)
I- C	0 / 434	0.10 (2)
C- G	-246 / 0	0.16 (1)
G- D	-423 / 0	0.21 (1)
G- E	0 / 946	0.21 (1)
A- J	0 / 806	0.18 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.25/1.00 (I-J:2) , WB=0.28/1.00 (B-I:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.85 (A) (INPUT = 0.90)
LSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM 190035 35
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T46	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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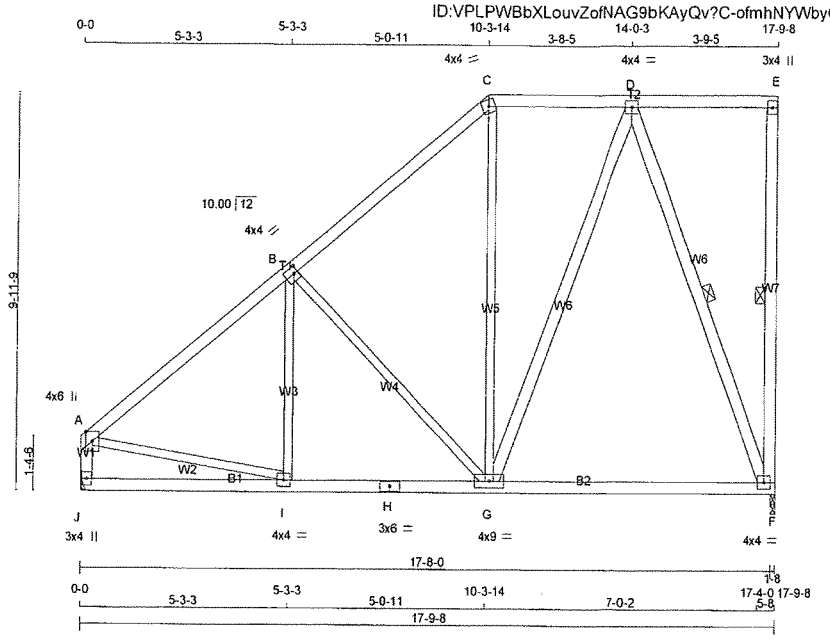


TOTAL WEIGHT = 101 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1043	0	1043	0	0	1-8	1-8
J	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

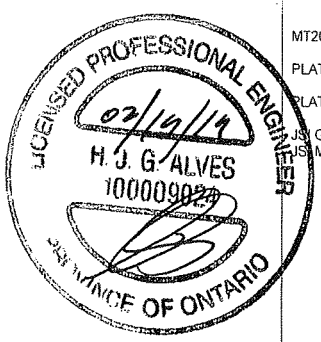
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-977 / 0	-77.7	-77.7	0.29 (1)	5.96	I- B	0 / 170	0.04 (3)
B- C	-663 / 0	-77.7	-77.7	0.27 (1)	6.25	B- G	-422 / 0	0.47 (1)
C- D	-485 / 0	-77.7	-77.7	0.14 (1)	6.25	G- C	0 / 185	0.04 (3)
D- E	0 / 0	-77.7	-77.7	0.18 (1)	10.00	G- D	0 / 487	0.08 (2)
F- E	-112 / 0	0.0	0.0	0.05 (1)	6.25	D- F	-871 / 0	0.49 (1)
J- A	-953 / 0	0.0	0.0	0.10 (1)	7.81	A- I	0 / 789	0.18 (1)
J- I	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
I- H	0 / 774	-39.5	-39.5	0.44 (2)	10.00			
H- G	0 / 774	-39.5	-39.5	0.44 (2)	10.00			
G- F	0 / 317	-39.5	-39.5	0.38 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.29/1.00 (A-B:1) , BC=0.44/1.00 (G-I:2) , WB=0.49/1.00 (D-F:1) , SSI=0.18/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.88 (I) (INPUT = 0.90)
JS METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM T190 3536
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T47	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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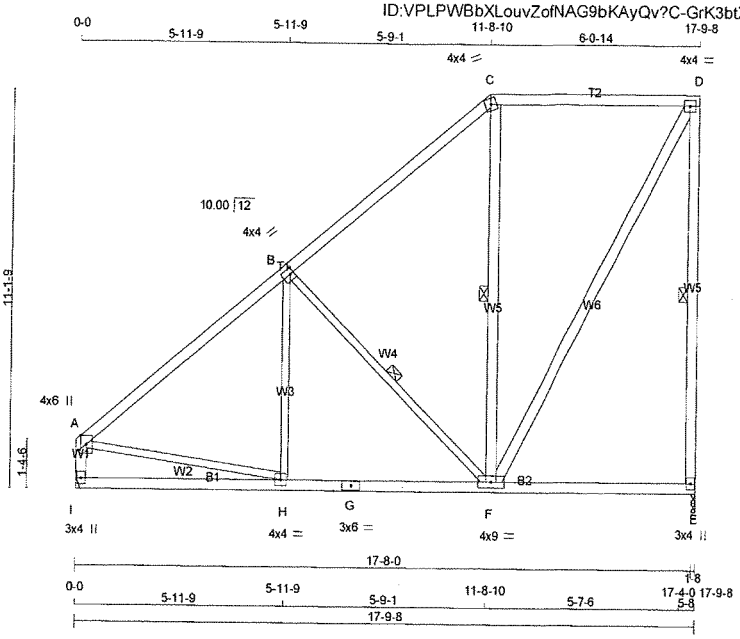


Scale = 1:62.0

TOTAL WEIGHT = 99 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1043	0	1043	0	0	1-8	1-8
I	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, B-F, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

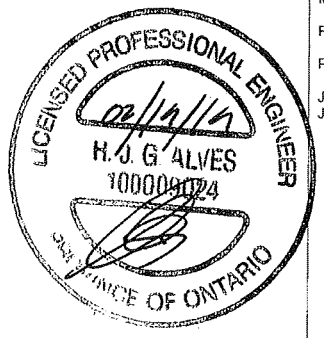
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-962 / 0	-77.7	-77.7	0.38 (1)	5.86	
B- C	-547 / 0	-77.7	-77.7	0.35 (1)	6.25	
C- D	-390 / 0	-77.7	-77.7	0.37 (1)	6.25	
E- D	-948 / 0	0.0	0.0	0.61 (1)	6.25	
I- A	-947 / 0	0.0	0.0	0.10 (1)	7.81	
I- H	0 / 0	-39.5	-39.5	0.24 (3)	10.00	
H- G	0 / 766	-39.5	-39.5	0.34 (2)	10.00	
G- F	0 / 766	-39.5	-39.5	0.34 (2)	10.00	
F- E	0 / 0	-39.5	-39.5	0.25 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	0 / 252	0.06 (3)
B- F	-534 / 0	0.26 (1)
F- C	-76 / 110	0.05 (1)
F- D	0 / 813	0.13 (1)
A- H	0 / 777	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.61/1.00 (D-E:1) , BC=0.34/1.00 (F-H:2) , WB=0.26/1.00 (B-F:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. IAM T7035537
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	G48	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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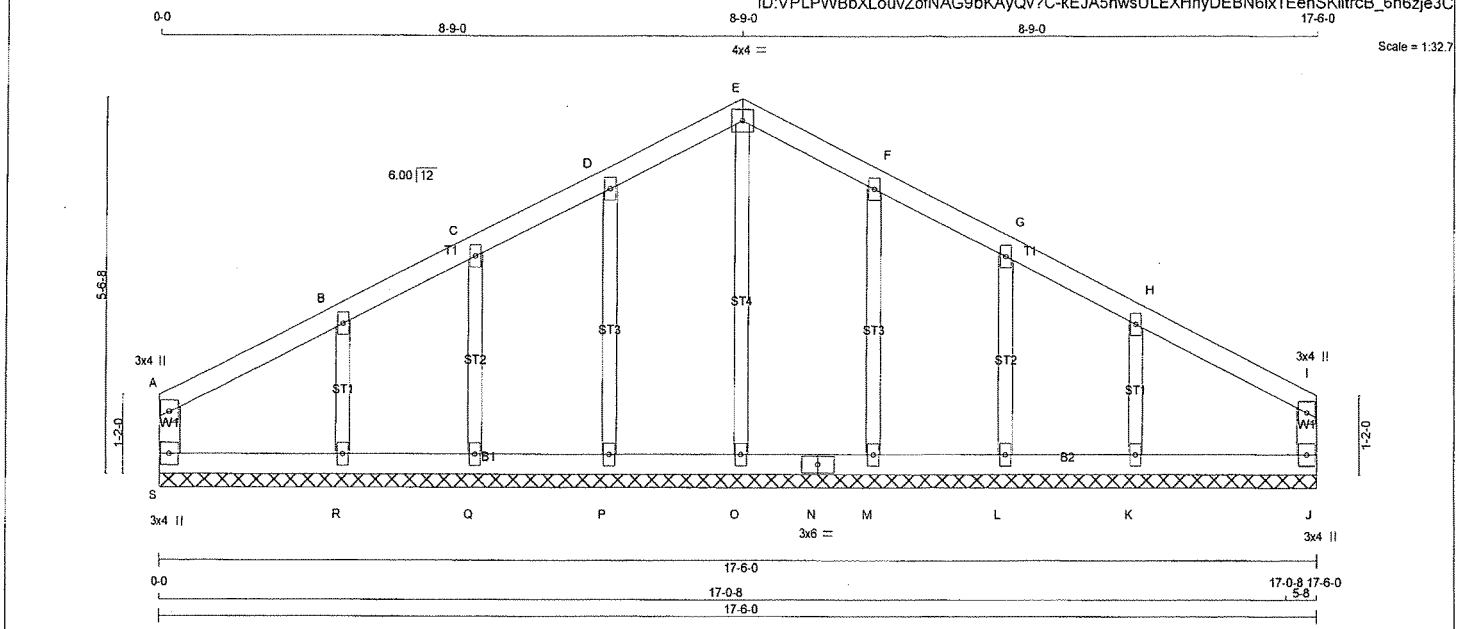

TOTAL WEIGHT = 2 X 63 = 125 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, F, G, H						
B	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, O, P, Q, R						
K	BMW1+w	MT20	2.0	4.0		
N	BS-I	MT20	3.0	6.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- A	-134 / 0	0.0	0.0	0.05 (1)		7.81	Q- E	-111 / 0	0.05 (1)
A- B	-70 / 0	-77.7	-77.7	0.06 (1)		6.25	P- D	-157 / 0	0.05 (1)
B- C	-68 / 0	-77.7	-77.7	0.06 (1)		6.25	Q- C	-143 / 0	0.03 (1)
C- D	-60 / 0	-77.7	-77.7	0.04 (1)		6.25	R- B	-191 / 0	0.03 (1)
D- E	-58 / 0	-77.7	-77.7	0.04 (1)		6.25	M- F	-157 / 0	0.05 (1)
E- F	-58 / 0	-77.7	-77.7	0.04 (1)		6.25	L- G	-143 / 0	0.03 (1)
F- G	-60 / 0	-77.7	-77.7	0.04 (1)		6.25	K- H	-191 / 0	0.03 (1)
G- H	-68 / 0	-77.7	-77.7	0.06 (1)		6.25			
H- I	-70 / 0	-77.7	-77.7	0.06 (1)		6.25			
J- I	-134 / 0	0.0	0.0	0.05 (1)		7.81			
S- R	0 / 63	-39.5	-39.5	0.05 (2)		10.00			
R- Q	0 / 57	-39.5	-39.5	0.05 (2)		10.00			
Q- P	0 / 54	-39.5	-39.5	0.03 (2)		10.00			
P- O	0 / 52	-39.5	-39.5	0.03 (2)		10.00			
O- N	0 / 52	-39.5	-39.5	0.03 (2)		10.00			
N- M	0 / 52	-39.5	-39.5	0.03 (2)		10.00			
M- L	0 / 54	-39.5	-39.5	0.03 (2)		10.00			
L- K	0 / 57	-39.5	-39.5	0.05 (2)		10.00			
K- J	0 / 63	-39.5	-39.5	0.05 (2)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (H-I:1) , BC=0.05/1.00 (J-K:2) , WB=0.05/1.00 (E-O:1) , SSI=0.07/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

DWG NO. TAM 1703538
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T49	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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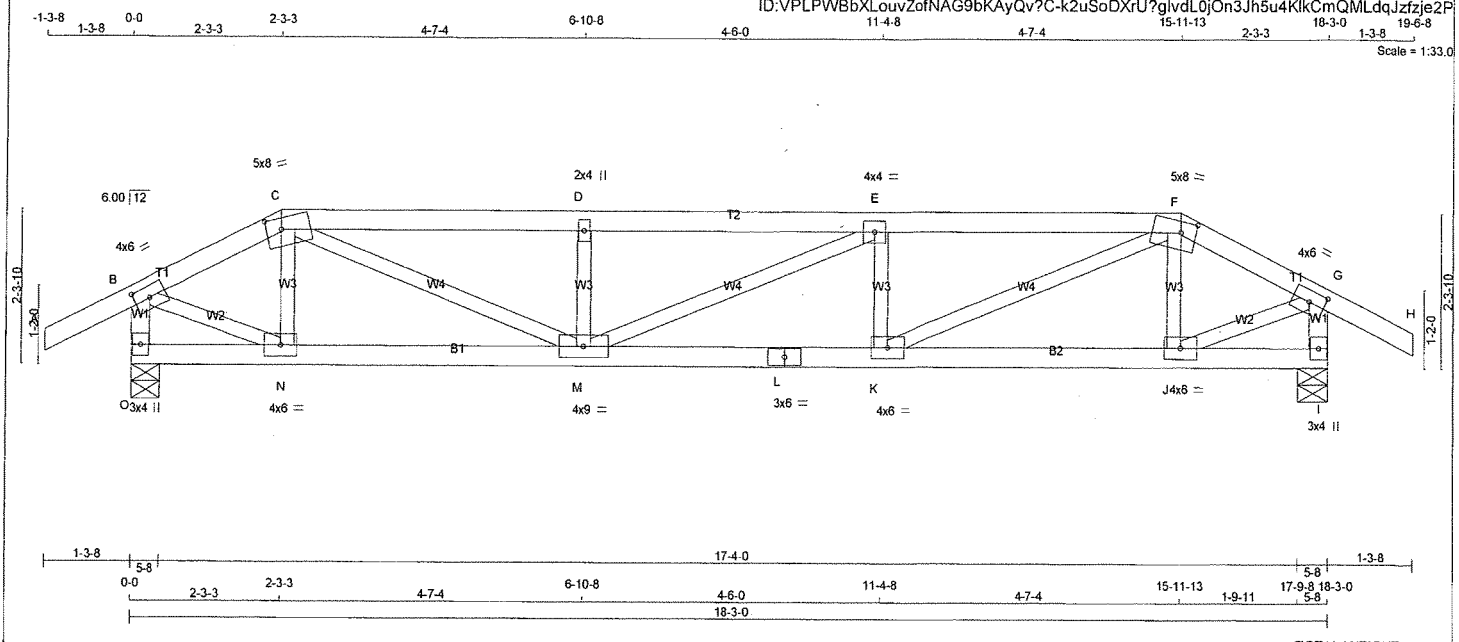


TOTAL WEIGHT = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	2.75
G	TMVW-t	MT20	4.0	6.0	2.00	3.00
I	BMV1+p	MT20	3.0	4.0		
J, K, N						
J	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1175	0	1175	0	0	5-8	5-8
I	1175	0	1175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0
I	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

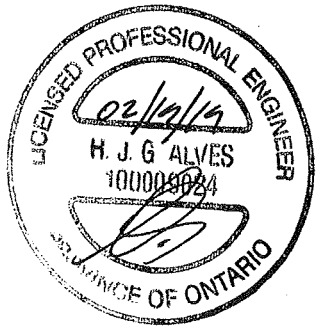
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	N- C	-235 / 0	0.04 (1)
B- C	-1242 / 0	-77.7	-77.7	0.09 (1)	5.68	J- F	-236 / 0	0.04 (1)
C- D	-2365 / 0	-77.7	-77.7	0.29 (1)	4.20	B- N	0 / 1179	0.27 (1)
D- E	-2365 / 0	-77.7	-77.7	0.27 (1)	4.22	J- G	0 / 1179	0.27 (1)
E- F	-2366 / 0	-77.7	-77.7	0.29 (1)	4.20	K- F	0 / 1384	0.31 (1)
F- G	-1241 / 0	-77.7	-77.7	0.09 (1)	5.68	C- M	0 / 1382	0.31 (1)
G- H	0 / 24	-77.7	-77.7	0.10 (1)	10.00	K- E	-378 / 0	0.06 (1)
O- B	-1153 / 0	0.0	0.0	0.12 (1)	7.42	M- D	-377 / 0	0.06 (1)
I- G	-1153 / 0	0.0	0.0	0.12 (1)	7.42	M- E	-2 / 0	0.00 (1)
O- N	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
N- M	0 / 1097	-39.5	-39.5	0.26 (2)	10.00			
M- L	0 / 2366	-39.5	-39.5	0.47 (1)	10.00			
L- K	0 / 2366	-39.5	-39.5	0.47 (1)	10.00			
K- J	0 / 1096	-39.5	-39.5	0.26 (2)	10.00			
J- I	0 / 0	-39.5	-39.5	0.10 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29/1.00 (C-D:1) , BC=0.47/1.00 (K-M:1) , WB=0.31/1.00 (F-K:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (F) (INPUT = 0.90)
JSI METAL= 0.80 (L) (INPUT = 1.00)

DWG NO. TAM 1703537
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T50	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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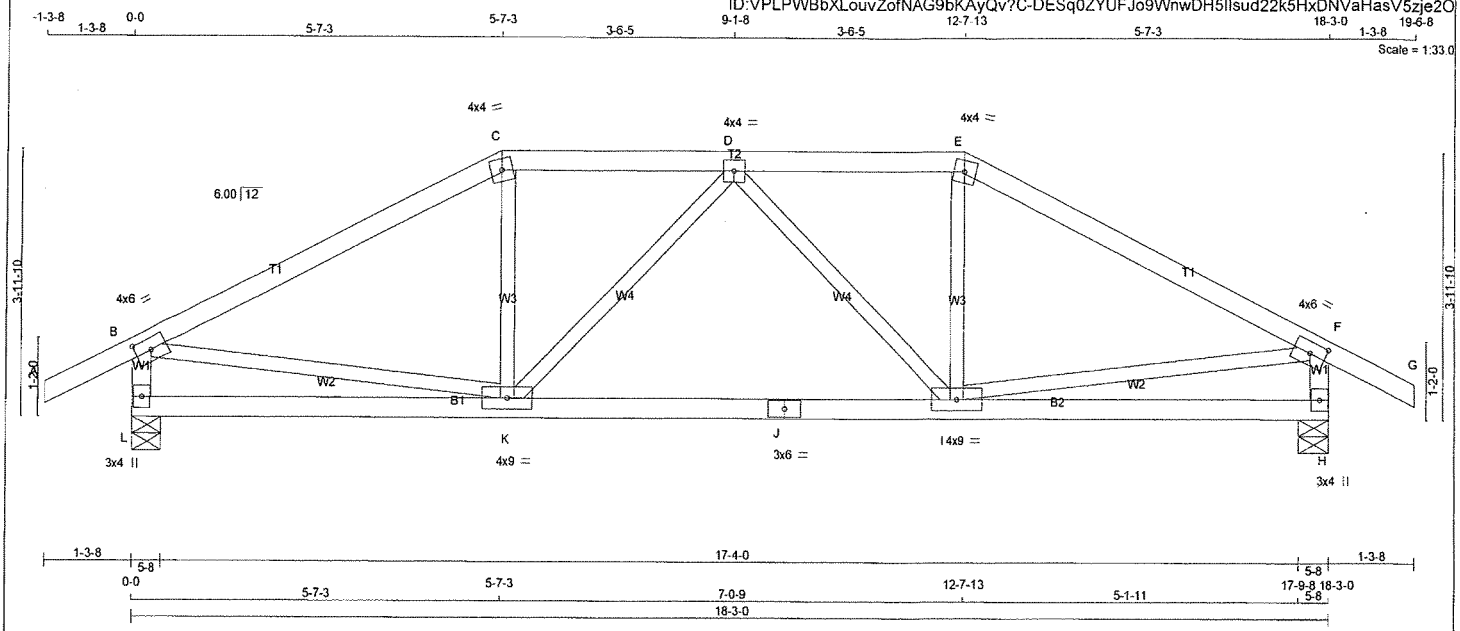

TOTAL WEIGHT = 70 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1175	0	1175	0	0	5-8	5-8
H	1175	0	1175	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0
H	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	K- C	0 / 303	0.07 (2)
B- C	-1312 / 0	-77.7	-77.7	0.34 (1)	5.26	K- D	-222 / 0	0.09 (1)
C- D	-1186 / 0	-77.7	-77.7	0.13 (1)	5.74	D- I	-222 / 0	0.09 (1)
D- E	-1186 / 0	-77.7	-77.7	0.13 (1)	5.74	I- E	0 / 303	0.07 (2)
E- F	-1312 / 0	-77.7	-77.7	0.34 (1)	5.26	B- K	0 / 1186	0.27 (1)
F- G	0 / 24	-77.7	-77.7	0.10 (1)	10.00	I- F	0 / 1186	0.27 (1)
L- B	-1086 / 0	0.0	0.0	0.11 (1)	7.58			
H- F	-1086 / 0	0.0	0.0	0.11 (1)	7.58			
L- K	0 / 0	-39.5	-39.5	0.28 (3)	10.00			
K- J	0 / 1336	-39.5	-39.5	0.45 (2)	10.00			
J- I	0 / 1336	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.45/1.00 (I-K:2) , WB=0.27/1.00 (F-I:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (I) (INPUT = 0.90)
JSI METAL= 0.55 (J) (INPUT = 1.00)

DWG NO. TAM T7035540
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T51	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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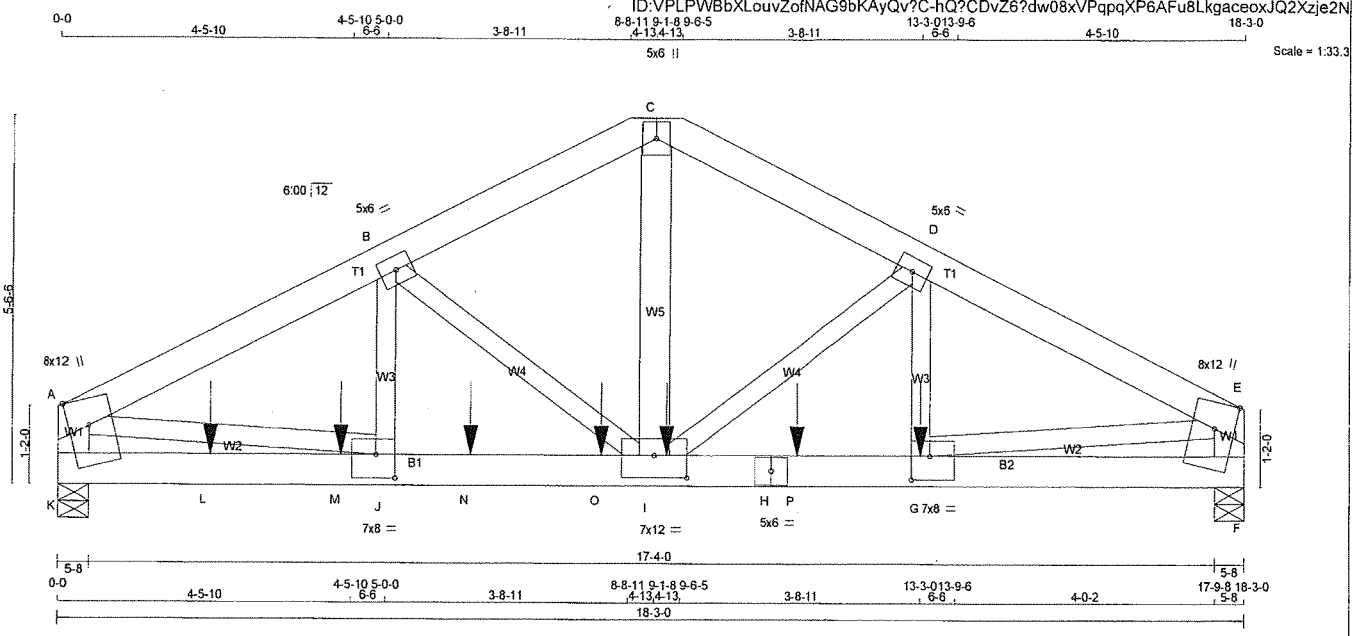

TOTAL WEIGHT = 2 X 107 = 214 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - C	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- D	1	3	SIDE(390.1)
2x4	1	3	
A- J	1	6	
B- I	1	6	
I- D	1	6	
G- E	1	6	
C- I	2	6	SIDE(78.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A, E, F, K						
A						
A	TMBMVW1*+n	MT20	8.0	12.0	Edge	3.75
B	TMWW-t	MT20	5.0	6.0		
C	TTW+p	MT20	5.0	6.0		
D	TMWW-t	MT20	5.0	6.0		
E	TMBMVW1*+n	MT20	8.0	12.0	Edge	3.75
G	BMWW-t	MT20	7.0	8.0	4.25	3.50
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	7.0	12.0	4.00	6.00
J	BMWW-t	MT20	7.0	8.0	4.25	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	5399	0	5399	0	0	5-8	5-8
F	4956	0	4956	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4103	1987 / 0	907 / 0	0 / 0	0 / 0	1209 / 0	0 / 0
F	3767	1823 / 0	833 / 0	0 / 0	0 / 0	1110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	-8091 / 0	-77.7	-77.7	0.21 (1)		4.12	J- B	0 / 1630	0.14 (1)
B- C	-6361 / 0	-77.7	-77.7	0.13 (1)		4.63	G- D	0 / 1711	0.15 (1)
C- D	-6361 / 0	-77.7	-77.7	0.13 (1)		4.63	I- C	0 / 5338	0.35 (1)
D- E	-8162 / 0	-77.7	-77.7	0.21 (1)		4.10	A- J	0 / 7298	0.65 (1)
K- A	-4811 / 0	0.0	0.0	0.17 (1)		6.61	B- I	-1937 / 0	0.29 (1)
F- E	-4851 / 0	0.0	0.0	0.17 (1)		6.58	I- D	-2016 / 0	0.30 (1)
							G- E	0 / 7361	0.65 (1)
K- L	0 / 0	-39.5	-39.5	0.46 (1)		10.00			
L- M	0 / 0	-39.5	-39.5	0.46 (1)		10.00			
M- J	0 / 0	-39.5	-39.5	0.46 (1)		10.00			
J- N	0 / 7231	-39.5	-39.5	0.82 (1)		10.00			
N- O	0 / 7231	-39.5	-39.5	0.82 (1)		10.00			
O- I	0 / 7231	-39.5	-39.5	0.82 (1)		10.00			
I- H	0 / 7294	-39.5	-39.5	0.76 (1)		10.00			
H- P	0 / 7294	-39.5	-39.5	0.76 (1)		10.00			
P- G	0 / 7294	-39.5	-39.5	0.76 (1)		10.00			
G- F	0 / 0	-39.5	-39.5	0.06 (2)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
G	13-3-0	-2197	-2197	---	FRONT	VERT	TOTAL	---	
I	9-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	
L	2-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	
M	4-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
N	6-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
O	8-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
P	11-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.82/1.00 (I-J:1) , WB=0.65/1.00 (E-G:1) , SSI=0.54/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.92 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1403541
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T52	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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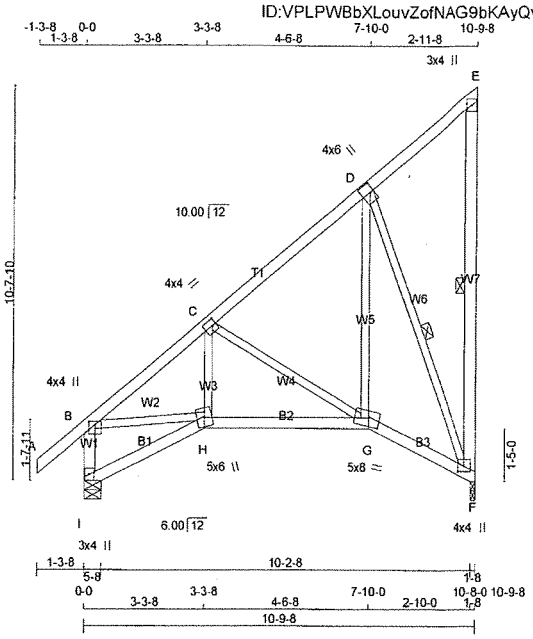

Scale = 1:59.8

TOTAL WEIGHT = 6 X 64 = 386 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW+t	MT20	4.0	6.0	3.00	1.25
E	TMV+p	MT20	3.0	4.0		
F	BMVW1+p	MT20	4.0	4.0	1.75	2.00
G	BBWW-m	MT20	5.0	8.0	2.75	3.50
H	BBWW+m	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	632	0	632	0	0	1-8	1-8
I	740	0	740	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0
I	558	283 / 0	113 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	H- C	0 / 215	0.05 (3)
B- C	-744 / 0	-77.7	-77.7	0.12 (1)	6.25	C- G	-409 / 0	0.18 (1)
C- D	-302 / 0	-77.7	-77.7	0.12 (1)	6.25	G- D	0 / 485	0.11 (2)
D- E	-22 / 0	-77.7	-77.7	0.11 (1)	6.25	D- F	-654 / 0	0.30 (1)
F- E	-84 / 0	0.0	0.0	0.05 (1)	6.25	B- H	0 / 588	0.13 (1)
I- B	-675 / 0	0.0	0.0	0.07 (1)	7.81			
I- H	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
H- G	0 / 580	-39.5	-39.5	0.26 (2)	10.00			
G- F	0 / 267	-39.5	-39.5	0.11 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.26/1.00 (G-H:2) , WB=0.30/1.00 (D-F:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM T903542
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T53	6	1	TRUSS DESC.	

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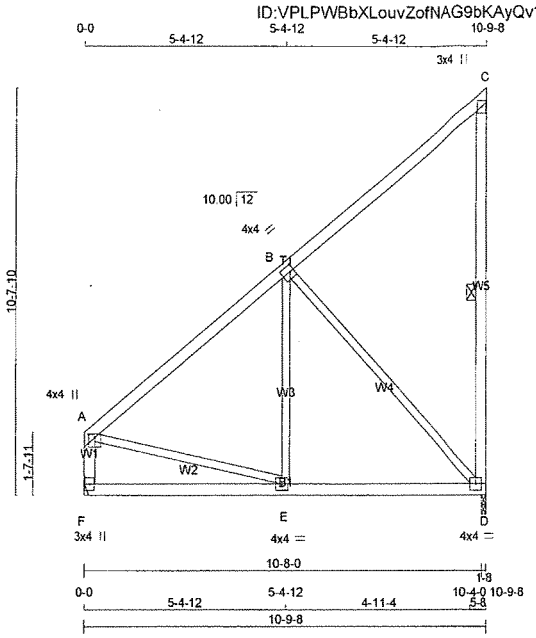


TOTAL WEIGHT = 6 X 56 = 337 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	632	0	632	0	0	1-8	1-8
F	632	0	632	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0
F	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

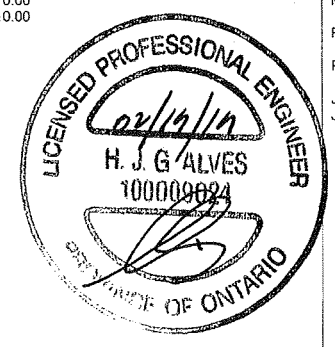
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-443 / 0	-77.7	-77.7	0.30 (1)	6.25	E- B	0 / 286	0.07 (3)
B- C	-33 / 0	-77.7	-77.7	0.29 (1)	6.25	B- D	-537 / 0	0.67 (1)
D- C	-158 / 0	0.0	0.0	0.09 (1)	6.25	A- E	0 / 376	0.08 (1)
F- A	-551 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
E- D	0 / 366	-39.5	-39.5	0.29 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.29/1.00 (D-E:2) , WB=0.67/1.00 (B-D:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (A) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

DWG NO. TAM: T7035543
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T54	3	1	TRUSS DESC.	

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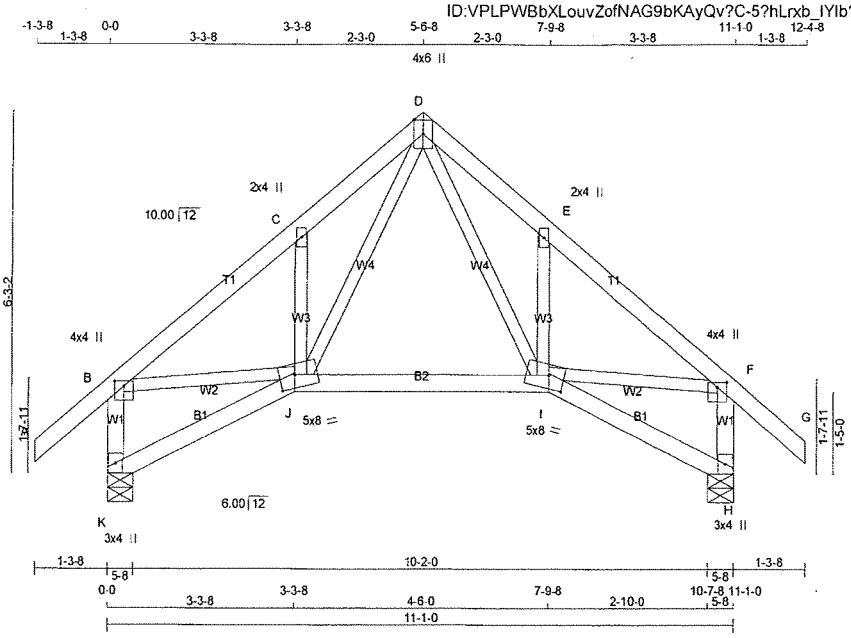

TOTAL WEIGHT = 3 X 54 = 163 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	757	0	757	0	0	5-8	5-8
H	757	0	757	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)		10.00	D- I	0 / 489	0.11 (1)
B- C	-763 / 0	-77.7	-77.7	0.11 (1)		6.25	I- E	-262 / 0	0.05 (1)
C- D	-794 / 0	-77.7	-77.7	0.07 (1)		6.25	J- D	0 / 489	0.11 (1)
D- E	-794 / 0	-77.7	-77.7	0.07 (1)		6.25	J- C	-262 / 0	0.05 (1)
E- F	-763 / 0	-77.7	-77.7	0.11 (1)		6.25	B- J	0 / 596	0.13 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)		10.00	I- F	0 / 596	0.13 (1)
K- B	-692 / 0	0.0	0.0	0.08 (1)		7.81			
H- F	-692 / 0	0.0	0.0	0.08 (1)		7.81			
K- J	0 / 0	-39.5	-39.5	0.10 (3)		10.00			
J- I	0 / 378	-39.5	-39.5	0.23 (2)		10.00			
I- H	0 / 0	-39.5	-39.5	0.10 (3)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.23/1.00 (I-J:2) , WB=0.13/1.00 (F-I:1) , SSI=0.10/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM 74903544
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T55	1	2	UNITS 11 - 15 TRUSS DESC.	

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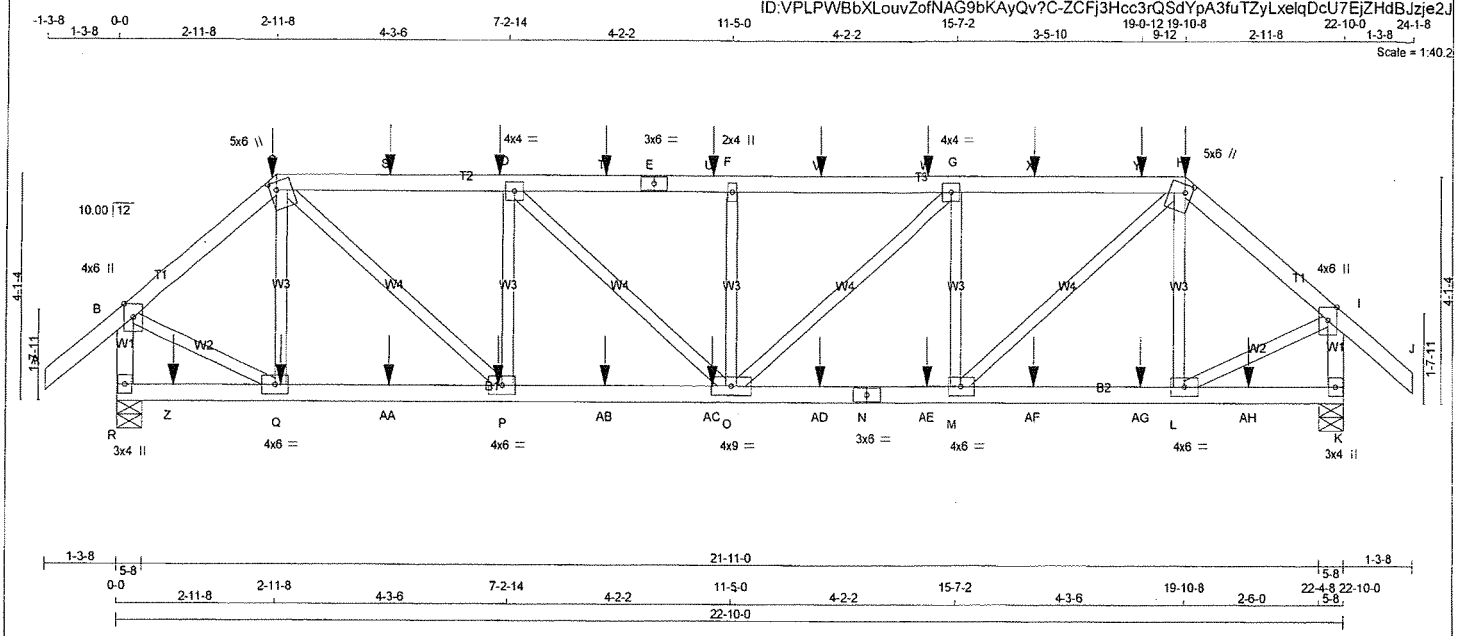


TOTAL WEIGHT = 2 X 98 = 195 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	1	12	TOP
K- I	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	1	12	SIDE(61.0)
N- K	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	1.75	1.50
I	TMVW+p	MT20	4.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L, M, P, Q						
L	BMWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2423	0	2423	0	0	5-8	5-8
K	2379	0	2379	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1842	889 / 0	409 / 0	0 / 0	0 / 0	544 / 0	0 / 0
K	1807	878 / 0	400 / 0	0 / 0	0 / 0	530 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM TO		
A- B	0 / 34	-77.7 -77.7	0.06 (1)	10.00
B- C	-2196 / 0	-77.7 -77.7	0.08 (1)	5.93
C- S	-3065 / 0	-77.7 -77.7	0.19 (1)	5.10
S- D	-3065 / 0	-77.7 -77.7	0.19 (1)	5.10
D- T	-3482 / 0	-77.7 -77.7	0.20 (1)	4.83
T- E	-3482 / 0	-77.7 -77.7	0.20 (1)	4.83
E- U	-3482 / 0	-77.7 -77.7	0.20 (1)	4.83
U- F	-3482 / 0	-77.7 -77.7	0.20 (1)	4.83
F- V	-3482 / 0	-77.7 -77.7	0.22 (1)	4.80
V- W	-3482 / 0	-77.7 -77.7	0.22 (1)	4.80
W- G	-3482 / 0	-77.7 -77.7	0.22 (1)	4.80
G- X	-3061 / 0	-77.7 -77.7	0.21 (1)	5.07
X- Y	-3061 / 0	-77.7 -77.7	0.21 (1)	5.07
Y- H	-3061 / 0	-77.7 -77.7	0.21 (1)	5.07
H- I	-2168 / 0	-77.7 -77.7	0.08 (1)	5.96
I- J	0 / 34	-77.7 -77.7	0.06 (1)	10.00
R- B	-2352 / 0	0.0 0.0	0.14 (1)	7.32
K- I	-2325 / 0	0.0 0.0	0.14 (1)	7.35
R- Z	0 / 0	-39.5 -39.5	0.08 (3)	10.00
Z- Q	0 / 0	-39.5 -39.5	0.08 (3)	10.00
Q-AA	0 / 1668	-39.5 -39.5	0.21 (2)	10.00
AA- P	0 / 1668	-39.5 -39.5	0.21 (2)	10.00
P-AB	0 / 3086	-39.5 -39.5	0.33 (1)	10.00
AB-AC	0 / 3066	-39.5 -39.5	0.33 (1)	10.00
AC- O	0 / 3066	-39.5 -39.5	0.33 (1)	10.00
O-AD	0 / 3062	-39.5 -39.5	0.33 (1)	10.00
AD- N	0 / 3062	-39.5 -39.5	0.33 (1)	10.00
N-AE	0 / 3062	-39.5 -39.5	0.33 (1)	10.00
AE- M	0 / 3062	-39.5 -39.5	0.33 (1)	10.00
M-AF	0 / 1648	-39.5 -39.5	0.22 (2)	10.00
AF-AG	0 / 1648	-39.5 -39.5	0.22 (2)	10.00
AG- L	0 / 1648	-39.5 -39.5	0.22 (2)	10.00
L-AH	0 / 0	-39.5 -39.5	0.09 (3)	10.00
AH- K	0 / 0	-39.5 -39.5	0.09 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
Q- C	-440 / 0	0.06 (1)
C- P	0 / 1866	0.23 (1)
P- D	-954 / 0	0.13 (1)
D- O	0 / 563	0.07 (1)
O- F	-483 / 0	0.06 (1)
O- G	0 / 589	0.07 (1)
M- G	-969 / 0	0.13 (1)
M- H	0 / 1889	0.23 (1)
L- H	-433 / 0	0.06 (1)
B- Q	0 / 1627	0.21 (1)
L- I	0 / 1804	0.22 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-100	-100	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.33/1.00 (M-O:1) , WB=0.23/1.00 (H-M:1) , SSI=0.13/1.00 (F-G:1)

BOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.53 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

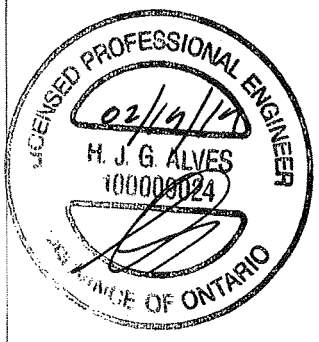
DWG NO. TAM T7903545
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T55	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
H	19-10-8	-38	-42	---	FRONT	VERT	DEAD	---	---
H	19-10-8	-180	-180	---	FRONT	VERT	SNOW	---	---
P	7-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Q	3-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
S	5-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
T	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
U	11-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
V	13-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
W	15-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
X	17-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Y	19-0-12	-101	-101	---	FRONT	VERT	TOTAL	---	---
Z	1-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AA	5-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AB	9-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AC	11-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AD	13-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AE	15-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AF	17-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AG	19-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AH	21-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM 1903545
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T56	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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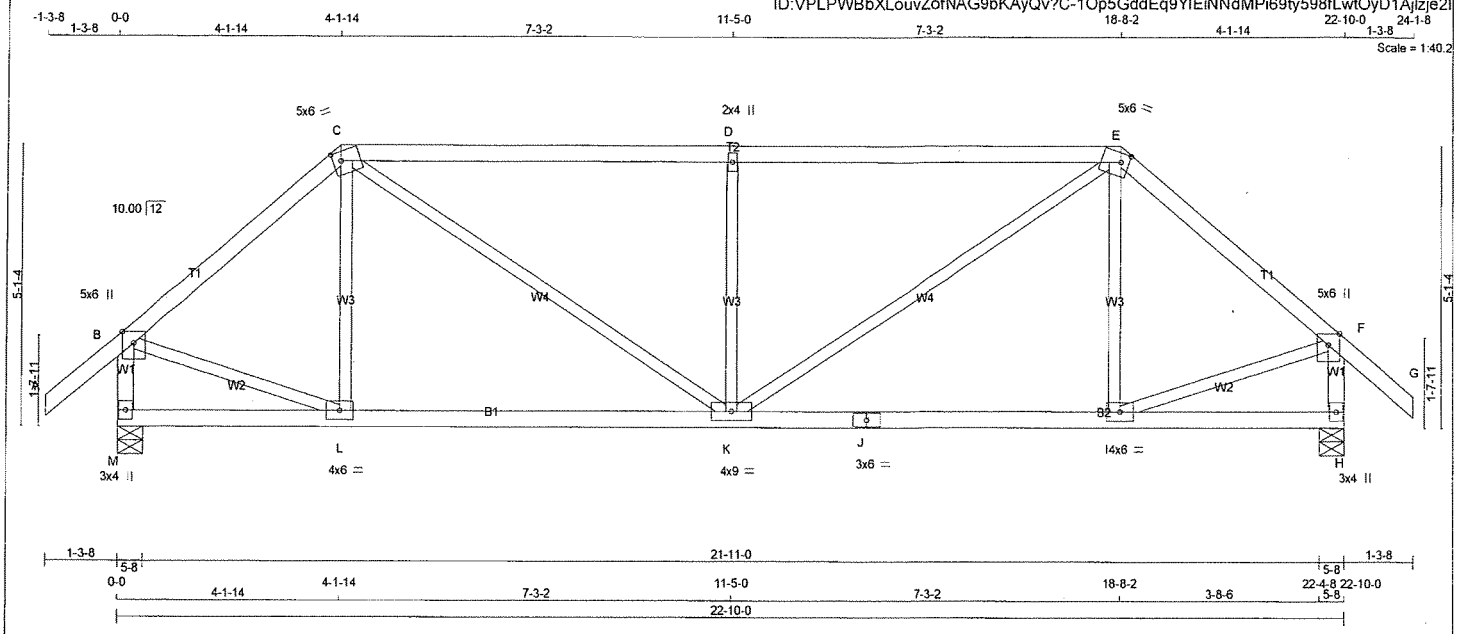


TOTAL WEIGHT = 93 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1446	0	1446	0	0	5-8	5-8
H	1446	0	1446	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0
H	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

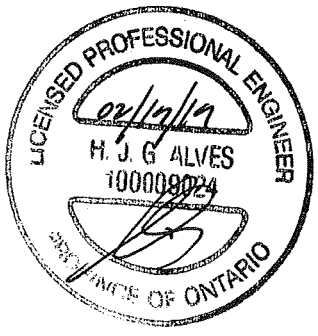
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00		L- C	-49 / 144	0.03 (3)
B- C	-1281 / 0	-77.7	-77.7	0.27 (1)	5.38		C- K	0 / 892	0.20 (1)
C- D	-1725 / 0	-77.7	-77.7	0.88 (1)	3.83		K- D	-690 / 0	0.27 (1)
D- E	-1725 / 0	-77.7	-77.7	0.88 (1)	3.83		K- E	0 / 892	0.20 (1)
E- F	-1281 / 0	-77.7	-77.7	0.27 (1)	5.38		I- E	-49 / 144	0.03 (3)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00		B- L	0 / 1027	0.23 (1)
M- B	-1393 / 0	0.0	0.0	0.15 (1)	6.90		I- F	0 / 1027	0.23 (1)
H- F	-1393 / 0	0.0	0.0	0.15 (1)	6.90				
M- L	0 / 0	-39.5	-39.5	0.24 (3)	10.00				
L- K	0 / 981	-39.5	-39.5	0.45 (2)	10.00				
K- J	0 / 981	-39.5	-39.5	0.45 (2)	10.00				
J- I	0 / 981	-39.5	-39.5	0.45 (2)	10.00				
I- H	0 / 0	-39.5	-39.5	0.24 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.88/1.00 (C-D:1) , BC=0.45/1.00 (K-L:2) , WB=0.27/1.00 (D-K:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.49 (J) (INPUT = 1.00)

DWG NO. TAM T1203546
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T57	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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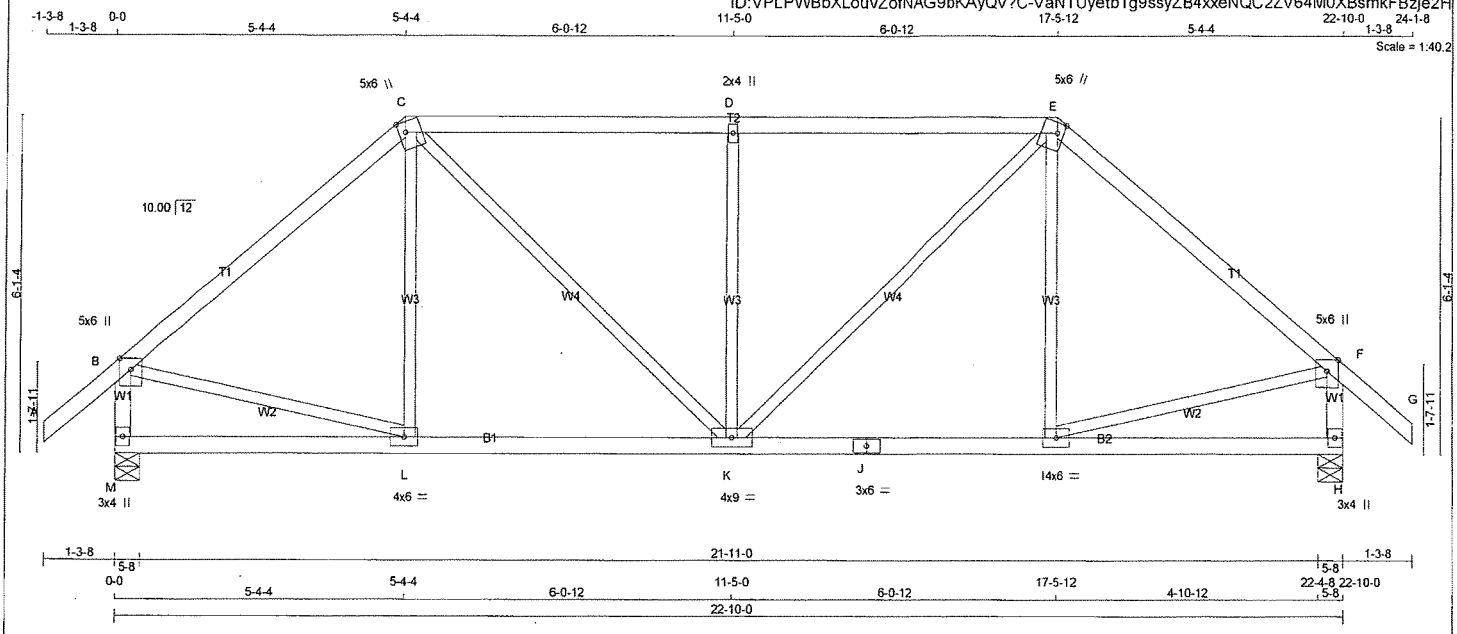


TOTAL WEIGHT = 98 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1446	0	1446	0	0	5-8	5-8
H	1446	0	1446	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0
H	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

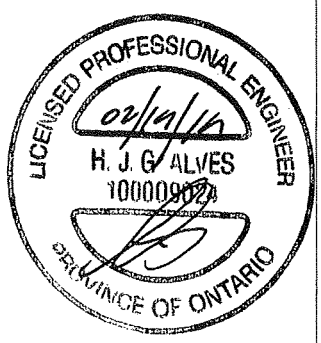
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)		10.00
B- C	-1267 / 0	-77.7	-77.7	0.46 (1)		5.15
C- D	-1390 / 0	-77.7	-77.7	0.55 (1)		4.79
D- E	-1390 / 0	-77.7	-77.7	0.55 (1)		4.79
E- F	-1267 / 0	-77.7	-77.7	0.46 (1)		5.15
F- G	0 / 34	-77.7	-77.7	0.11 (1)		10.00
M- B	-1362 / 0	0.0	0.0	0.14 (1)		6.96
H- F	-1362 / 0	0.0	0.0	0.14 (1)		6.96
M- L	0 / 0	-39.5	-39.5	0.24 (3)		10.00
L- K	0 / 972	-39.5	-39.5	0.37 (2)		10.00
K- J	0 / 972	-39.5	-39.5	0.37 (2)		10.00
J- I	0 / 972	-39.5	-39.5	0.37 (2)		10.00
I- H	0 / 0	-39.5	-39.5	0.24 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 189	0.05 (3)
C- K	0 / 578	0.13 (1)
K- D	-574 / 0	0.34 (1)
K- E	0 / 578	0.13 (1)
I- E	0 / 189	0.05 (3)
B- L	0 / 999	0.22 (1)
I- F	0 / 999	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.37/1.00 (K-L:2) , WB=0.34/1.00 (D-K:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (I) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

DWG NO. TAM 1190547
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T57A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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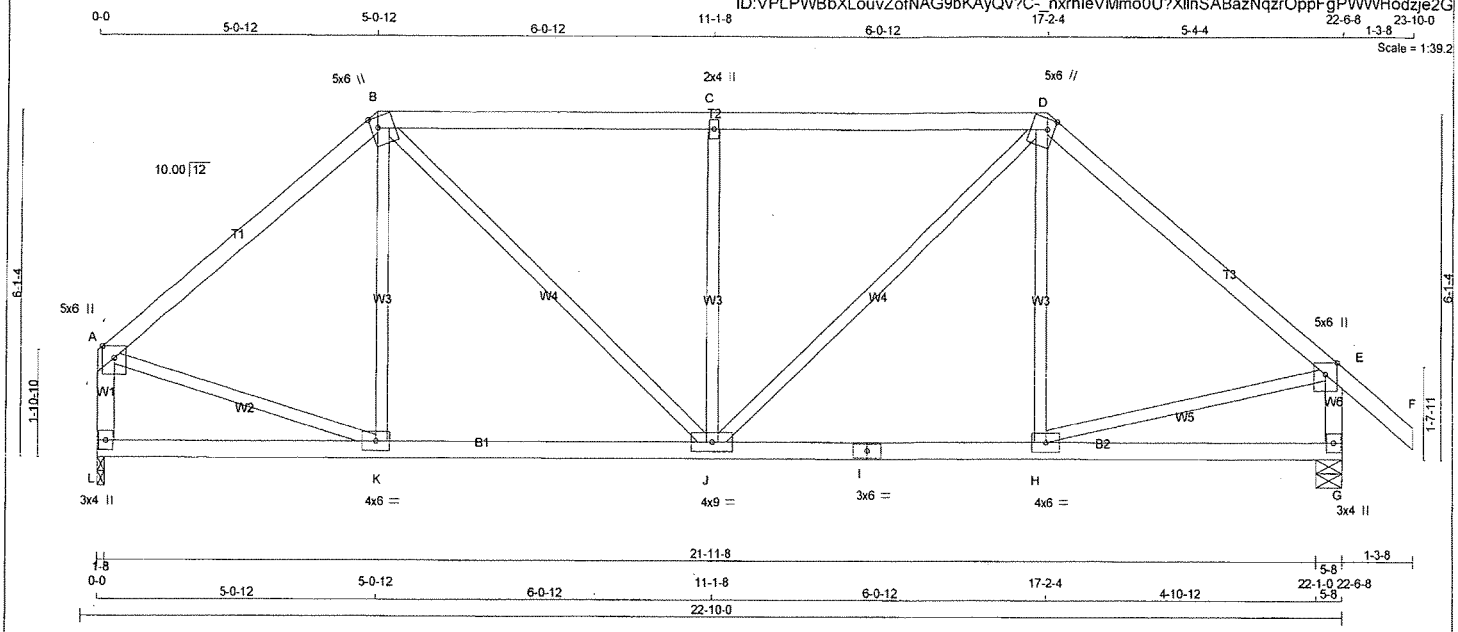

TOTAL WEIGHT = 95 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1321	0	1321	0	0	1-8	1-8
G	1429	0	1429	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1010	471 / 0	237 / 0	0 / 0	0 / 0	302 / 0	0 / 0
G	1084	529 / 0	237 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1200 / 0	-77.7	-77.7	0.40	(1)	5.34	K- B	-28 / 155	0.04 (3)
B- C	-1357 / 0	-77.7	-77.7	0.55	(1)	4.83	B- J	0 / 605	0.14 (1)
C- D	-1357 / 0	-77.7	-77.7	0.55	(1)	4.83	J- C	-574 / 0	0.34 (1)
D- E	-1247 / 0	-77.7	-77.7	0.46	(1)	5.19	J- D	0 / 554	0.12 (1)
E- F	0 / 34	-77.7	-77.7	0.11	(1)	10.00	H- D	0 / 190	0.05 (3)
L- A	-1243 / 0	0.0	0.0	0.14	(1)	7.20	A- K	0 / 960	0.22 (1)
G- E	-1344 / 0	0.0	0.0	0.14	(1)	6.99	H- E	0 / 983	0.22 (1)
L- K	0 / 0	-39.5	-39.5	0.23	(3)	10.00			
K- J	0 / 919	-39.5	-39.5	0.35	(2)	10.00			
J- I	0 / 956	-39.5	-39.5	0.37	(2)	10.00			
I- H	0 / 956	-39.5	-39.5	0.37	(2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.24	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.37/1.00 (H-J:2) , WB=0.34/1.00 (C-J:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (H) (INPUT = 0.90)
JSI METAL= 0.48 (E) (INPUT = 1.00)

DWG NO. TAM1790354B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T58	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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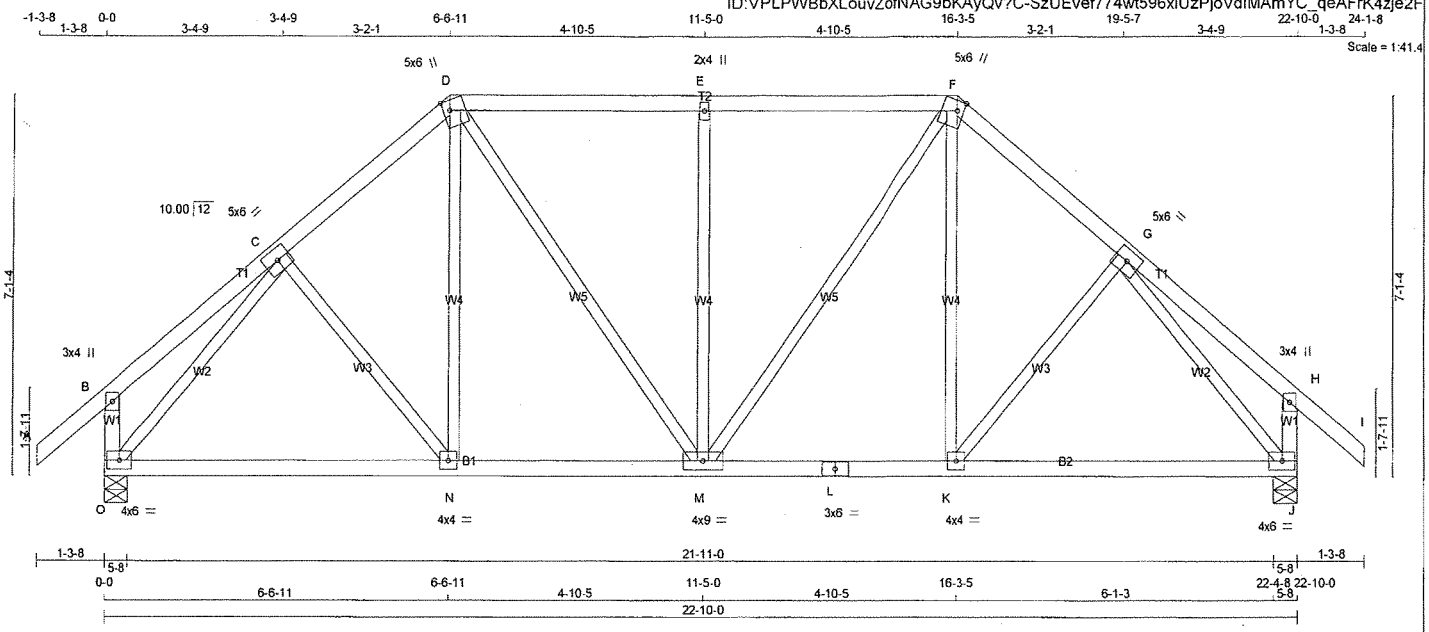


TOTAL WEIGHT = 108 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1446	0	1446	0	0	5-8	5-8
J	1446	0	1446	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0
J	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

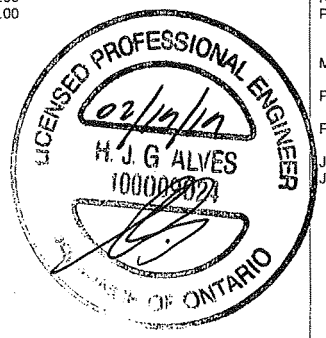
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00
C- D	-1248 / 0	-77.7	-77.7	0.11 (1)	5.65
D- E	-1157 / 0	-77.7	-77.7	0.25 (1)	5.64
E- F	-1157 / 0	-77.7	-77.7	0.25 (1)	5.64
F- G	-1248 / 0	-77.7	-77.7	0.11 (1)	5.65
G- H	0 / 17	-77.7	-77.7	0.12 (1)	10.00
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00
O- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-207 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 916	-39.5	-39.5	0.41 (2)	10.00
N- M	0 / 947	-39.5	-39.5	0.42 (2)	10.00
M- L	0 / 947	-39.5	-39.5	0.42 (2)	10.00
L- K	0 / 947	-39.5	-39.5	0.42 (2)	10.00
K- J	0 / 916	-39.5	-39.5	0.41 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 101	0.02 (3)
N- D	0 / 294	0.07 (2)
D- M	0 / 361	0.08 (1)
M- E	-457 / 0	0.40 (1)
M- F	0 / 361	0.08 (1)
K- F	0 / 294	0.07 (2)
K- G	0 / 101	0.02 (3)
O- C	-1455 / 0	0.63 (1)
G- J	-1455 / 0	0.63 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.42/1.00 (M-N:2) , WB=0.63/1.00 (C-O:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (O) (INPUT = 0.90)
USI METAL= 0.36 (G) (INPUT = 1.00)

DWG NO. TAM TM035499
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T58A	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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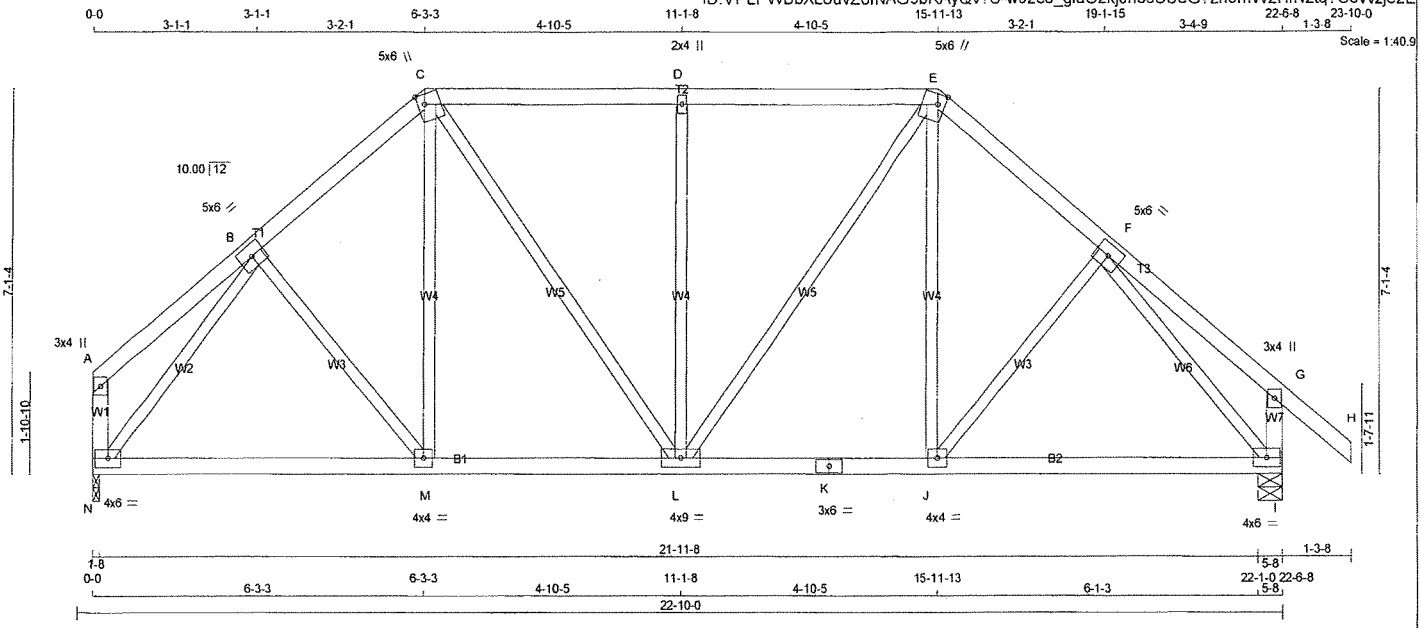


TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1321	0	1321	0	0	1-8	1-8
I	1429	0	1429	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1010	471 / 0	237 / 0	0 / 0	0 / 0	302 / 0	0 / 0
I	1084	529 / 0	237 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

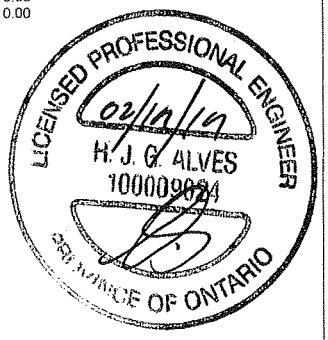
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-77.7	-77.7	1	0.11 (1)	10.00	B- M	0 / 131	0.03 (3)
B- C	-1192 / 0	-77.7	-77.7	1	0.11 (1)	5.75	M- C	0 / 241	0.06 (3)
C- D	-1128 / 0	-77.7	-77.7	1	0.24 (1)	5.69	C- L	0 / 386	0.09 (1)
D- E	-1128 / 0	-77.7	-77.7	1	0.24 (1)	5.69	L- D	-457 / 0	0.40 (1)
E- F	-1226 / 0	-77.7	-77.7	1	0.11 (1)	5.69	L- E	0 / 341	0.08 (1)
F- G	0 / 17	-77.7	-77.7	1	0.12 (1)	10.00	J- E	0 / 296	0.07 (2)
G- H	0 / 34	-77.7	-77.7	1	0.11 (1)	10.00	J- F	0 / 99	0.02 (3)
N- A	-88 / 0	0.0	0.0	1	0.01 (1)	7.81	N- B	-1404 / 0	0.56 (1)
I- G	-207 / 0	0.0	0.0	1	0.02 (1)	7.81	F- I	-1433 / 0	0.62 (1)
N- M	0 / 833	-39.5	-39.5	2	0.38 (2)	10.00			
M- L	0 / 904	-39.5	-39.5	2	0.39 (2)	10.00			
L- K	0 / 930	-39.5	-39.5	2	0.42 (2)	10.00			
K- J	0 / 930	-39.5	-39.5	2	0.42 (2)	10.00			
J- I	0 / 902	-39.5	-39.5	2	0.41 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.24/1.00 (C-D:1) , BC=0.42/1.00 (J-L:2) , WB=0.62/1.00 (F-I:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (N) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

DWG NO. TAM T1103550
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T59	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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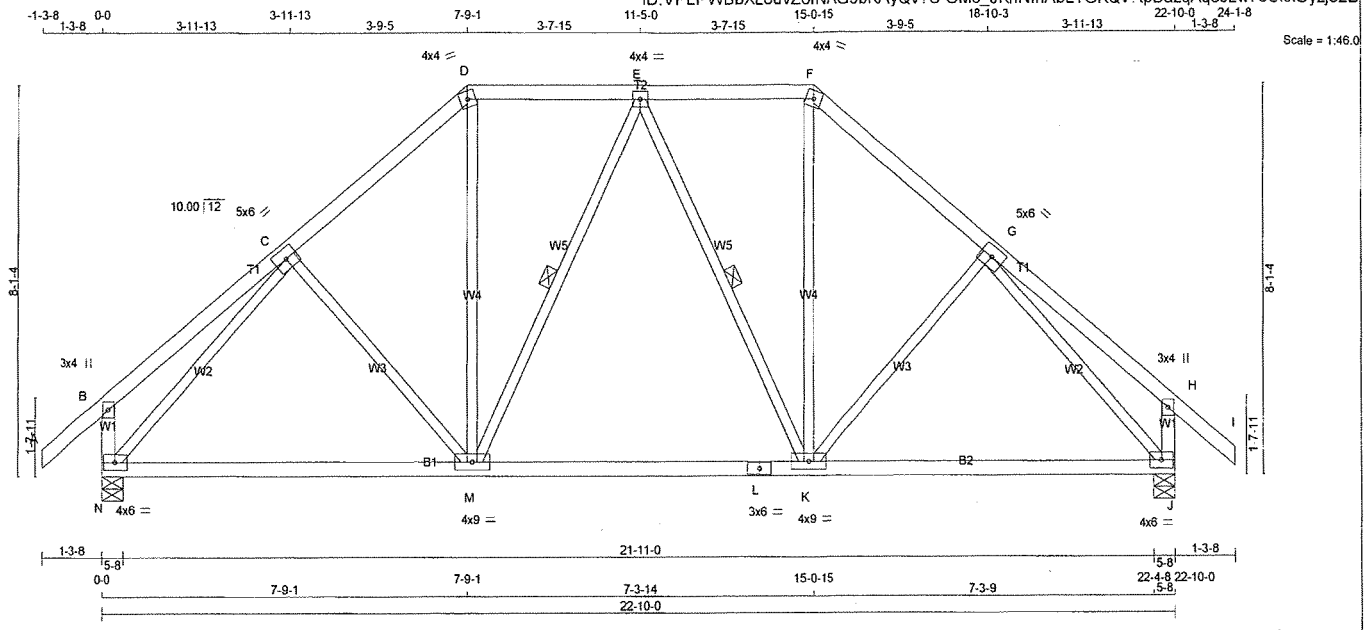


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1446	0	1446	0	0	5-8	5-8
J	1446	0	1446	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0
J	1097	535 / 0	240 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, E-K.

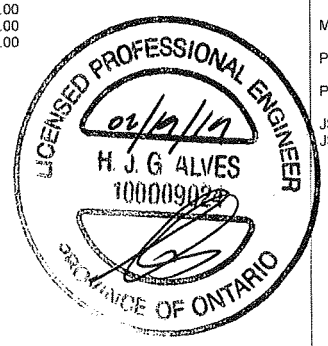
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
B- C	0 / 21	-77.7	-77.7	0.18 (1)	10.00	
C- D	-1202 / 0	-77.7	-77.7	0.16 (1)	5.68	
D- E	-912 / 0	-77.7	-77.7	0.13 (1)	6.25	
E- F	-912 / 0	-77.7	-77.7	0.13 (1)	6.25	
F- G	-1202 / 0	-77.7	-77.7	0.16 (1)	5.68	
G- H	0 / 21	-77.7	-77.7	0.18 (1)	10.00	
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
N- B	-224 / 0	0.0	0.0	0.02 (1)	7.81	
J- H	-224 / 0	0.0	0.0	0.02 (1)	7.81	
N- M	0 / 934	-39.5	-39.5	0.51 (2)	10.00	
M- L	0 / 988	-39.5	-39.5	0.52 (2)	10.00	
L- K	0 / 988	-39.5	-39.5	0.52 (2)	10.00	
K- J	0 / 934	-39.5	-39.5	0.51 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-43 / 69	0.03 (1)
M- D	0 / 526	0.12 (2)
M- E	-185 / 0	0.09 (1)
E- K	-185 / 0	0.09 (1)
K- F	0 / 526	0.12 (2)
K- G	-43 / 69	0.03 (1)
N- C	-1439 / 0	0.85 (1)
G- J	-1439 / 0	0.85 (1)



[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.52/1.00 (K-M:2) , WB=0.85/1.00 (C-N:1) , SSI=0.19/1.00 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (J) (INPUT = 0.90)
JSI METAL= 0.57 (L) (INPUT = 1.00)

DWG NO. TAM T1903551
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T59A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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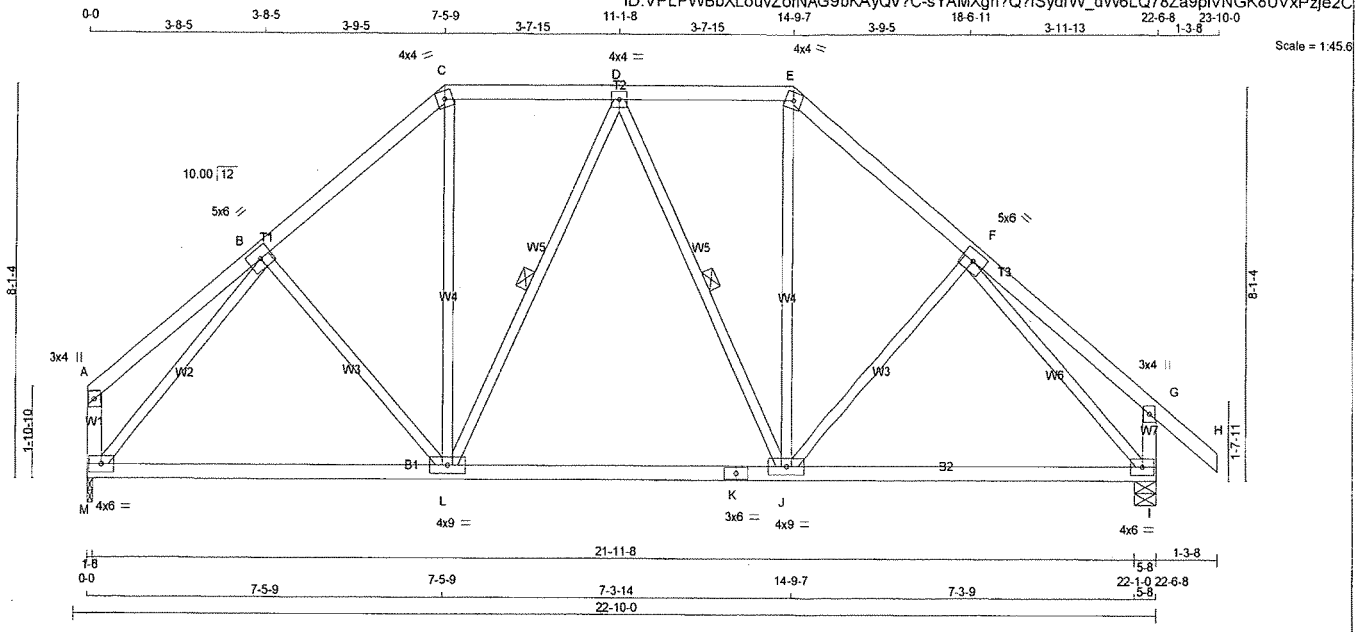


TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1321	0	1321	0	0	1-8	1-8
I	1429	0	1429	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1010	471 / 0	237 / 0	0 / 0	0 / 0	302 / 0	0 / 0
I	1084	529 / 0	237 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L, D-J.

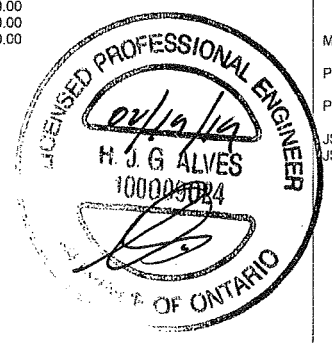
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-77.7	-77.7	0.17 (1)		10.00
B- C	-1156 / 0	-77.7	-77.7	0.15 (1)		5.77
C- D	-877 / 0	-77.7	-77.7	0.13 (1)		6.25
D- E	-895 / 0	-77.7	-77.7	0.13 (1)		6.25
E- F	-1181 / 0	-77.7	-77.7	0.16 (1)		5.72
F- G	0 / 21	-77.7	-77.7	0.18 (1)		10.00
G- H	0 / 34	-77.7	-77.7	0.11 (1)		10.00
M- A	-105 / 0	0.0	0.0	0.01 (1)		7.81
I- G	-224 / 0	0.0	0.0	0.02 (1)		7.81
M- L	0 / 862	-39.5	-39.5	0.48 (2)		10.00
L- K	0 / 963	-39.5	-39.5	0.53 (2)		10.00
K- J	0 / 963	-39.5	-39.5	0.53 (2)		10.00
J- I	0 / 920	-39.5	-39.5	0.52 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- L	0 / 94	0.02 (3)
L- C	0 / 498	0.11 (2)
L- D	-206 / 0	0.10 (1)
D- J	-164 / 0	0.08 (1)
J- E	0 / 513	0.12 (2)
J- F	-47 / 67	0.03 (1)
M- B	-1393 / 0	0.77 (1)
F- I	-1417 / 0	0.84 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.18/1.00 (F-G:1) , BC=0.53/1.00 (J-L:2) , WB=0.84/1.00 (F-I:1) , SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.57 (K) (INPUT = 1.00)

DWG NO. TAM T1203552
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T60A	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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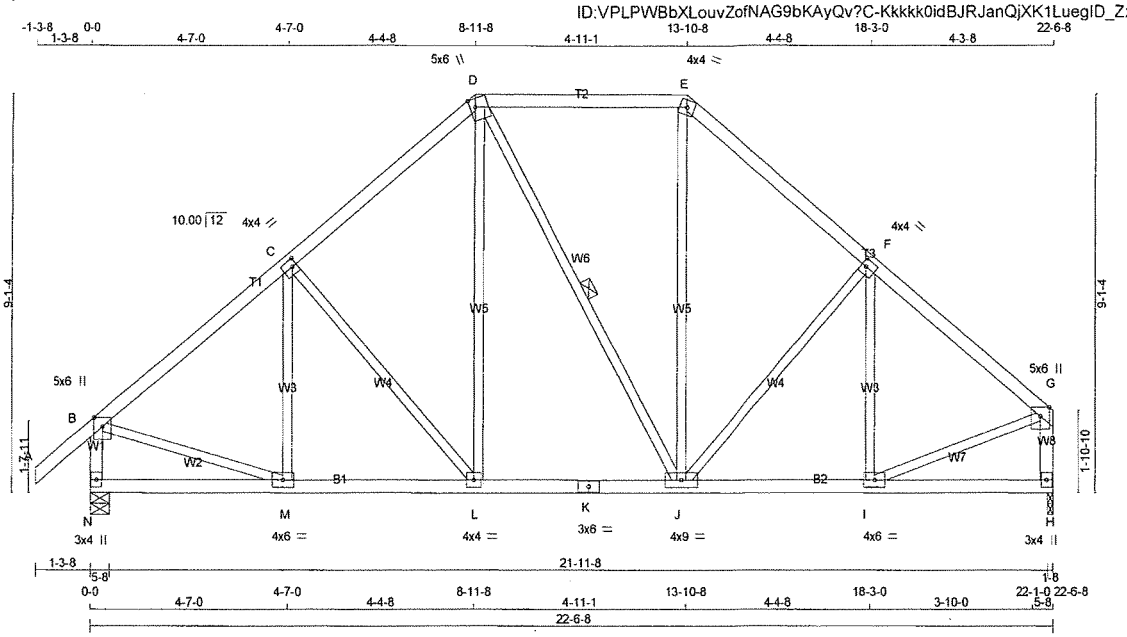


TOTAL WEIGHT = 2 X 110 = 220 lb

[M][F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1429	0	1429	0	0	5-8	5-8
H	1321	0	1321	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1084	529 / 0	237 / 0	0 / 0	0 / 0	319 / 0	0 / 0
H	1010	471 / 0	237 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

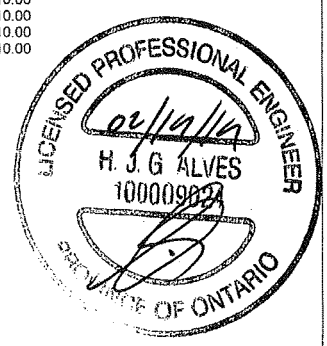
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11	(1)	10.00	M- C	-85 / 97	0.04 (1)
B- C	-1265 / 0	-77.7	-77.7	0.22	(1)	5.49	C- L	-269 / 0	0.23 (1)
C- D	-1090 / 0	-77.7	-77.7	0.21	(1)	5.82	L- D	0 / 404	0.09 (2)
D- E	-811 / 0	-77.7	-77.7	0.25	(1)	6.25	D- J	-17 / 0	0.01 (1)
E- F	-1078 / 0	-77.7	-77.7	0.20	(1)	5.86	J- E	0 / 383	0.09 (2)
F- G	-1201 / 0	-77.7	-77.7	0.20	(1)	5.62	J- F	-206 / 0	0.18 (1)
N- B	-1354 / 0	0.0	0.0	0.14	(1)	6.97	I- F	-152 / 53	0.07 (1)
H- G	-1252 / 0	0.0	0.0	0.14	(1)	7.19	B- M	0 / 1031	0.23 (1)
							I- G	0 / 1002	0.23 (1)
N- M	0 / 0	-39.5	-39.5	0.15	(3)	10.00			
M- L	0 / 992	-39.5	-39.5	0.28	(2)	10.00			
L- K	0 / 819	-39.5	-39.5	0.24	(2)	10.00			
K- J	0 / 819	-39.5	-39.5	0.24	(2)	10.00			
J- I	0 / 942	-39.5	-39.5	0.26	(2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.13	(3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.28/1.00 (L-M:2) , WB=0.23/1.00 (B-M:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (B) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

DWG NO. TAM TT903553
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T61	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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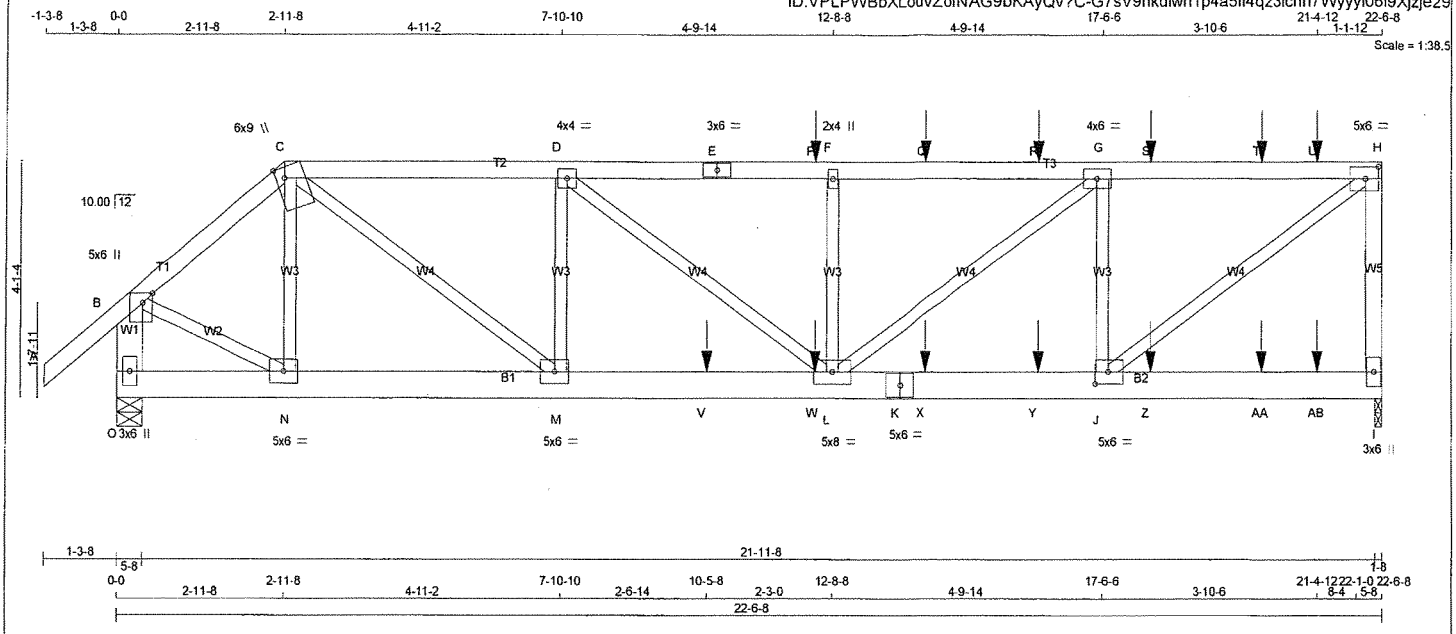


TOTAL WEIGHT = 2 X 107 = 214 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- H	1	12	SIDE(61.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- K	2	12	SIDE(183.1)
K- I	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	5.0	6.0	2.50	2.75
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2702	0	2702	0	0	1-8	1-8
O	2442	0	2442	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2059	981 / 0	467 / 0	0 / 0	0 / 0	611 / 0	0 / 0
O	1846	923 / 0	386 / 0	0 / 0	0 / 0	537 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	N- C	-664 / 0	0.08 (1)
B- C	-2261 / 0	-77.7	-77.7	0.08 (1)	5.87	B- N	0 / 1863	0.23 (1)
C- D	-4143 / 0	-77.7	-77.7	0.22 (1)	4.50	J- H	0 / 3767	0.47 (1)
D- E	-4677 / 0	-77.7	-77.7	0.24 (1)	4.27	C- M	0 / 3042	0.38 (1)
E- P	-4677 / 0	-77.7	-77.7	0.24 (1)	4.27	J- G	-2017 / 0	0.26 (1)
P- F	-4677 / 0	-77.7	-77.7	0.24 (1)	4.27	M- D	-822 / 0	0.10 (1)
F- Q	-4677 / 0	-77.7	-77.7	0.38 (1)	4.09	L- G	0 / 2093	0.26 (1)
Q- R	-4677 / 0	-77.7	-77.7	0.38 (1)	4.09	D- L	0 / 675	0.08 (1)
R- G	-4677 / 0	-77.7	-77.7	0.38 (1)	4.09	L- F	-493 / 0	0.06 (1)
G- S	-3022 / 0	-77.7	-77.7	0.32 (1)	4.96			
S- T	-3022 / 0	-77.7	-77.7	0.32 (1)	4.96			
T- U	-3022 / 0	-77.7	-77.7	0.32 (1)	4.96			
U- H	-3022 / 0	-77.7	-77.7	0.32 (1)	4.96			
I- H	-2517 / 0	0.0	0.0	0.31 (1)	7.13			
O- B	-2368 / 0	0.0	0.0	0.09 (1)	7.81			
O- N	0 / 0	-39.5	-39.5	0.03 (1)	10.00			
N- M	0 / 1712	-39.5	-39.5	0.30 (1)	10.00			
M- V	0 / 4143	-39.5	-39.5	0.78 (1)	10.00			
V- W	0 / 4143	-39.5	-39.5	0.78 (1)	10.00			
W- L	0 / 4143	-39.5	-39.5	0.78 (1)	10.00			
L- K	0 / 3022	-39.5	-39.5	0.39 (1)	10.00			
K- X	0 / 3022	-39.5	-39.5	0.39 (1)	10.00			
X- Y	0 / 3022	-39.5	-39.5	0.39 (1)	10.00			
Y- J	0 / 3022	-39.5	-39.5	0.39 (1)	10.00			
J- Z	0 / 0	-39.5	-39.5	0.10 (2)	10.00			
Z-AA	0 / 0	-39.5	-39.5	0.10 (2)	10.00			
AA-AB	0 / 0	-39.5	-39.5	0.10 (2)	10.00			
AB- I	0 / 0	-39.5	-39.5	0.10 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	12-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	14-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	16-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	18-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
T	20-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
U	21-4-12	-94	-94	---	BACK	VERT	TOTAL	---	---
V	10-5-8	-1494	-1494	---	BACK	VERT	TOTAL	---	---
W	12-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.38/1.00 (F-G:1) , BC=0.78/1.00 (L-M:1) , WB=0.47/1.00 (H-J:1) , SSI=0.31/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

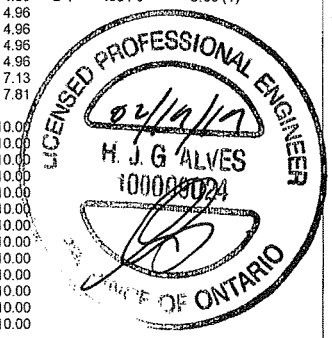
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)



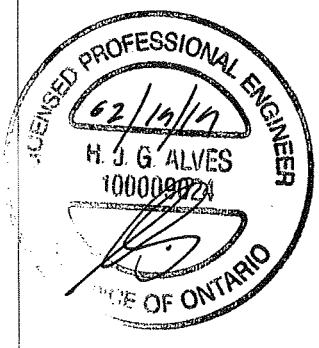
DWG NO. TAM TM035554
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T61	1	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
X	14-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Y	16-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---
Z	18-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AA	20-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---
AB	21-4-12	-56	-71	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM T7903554
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T62	1	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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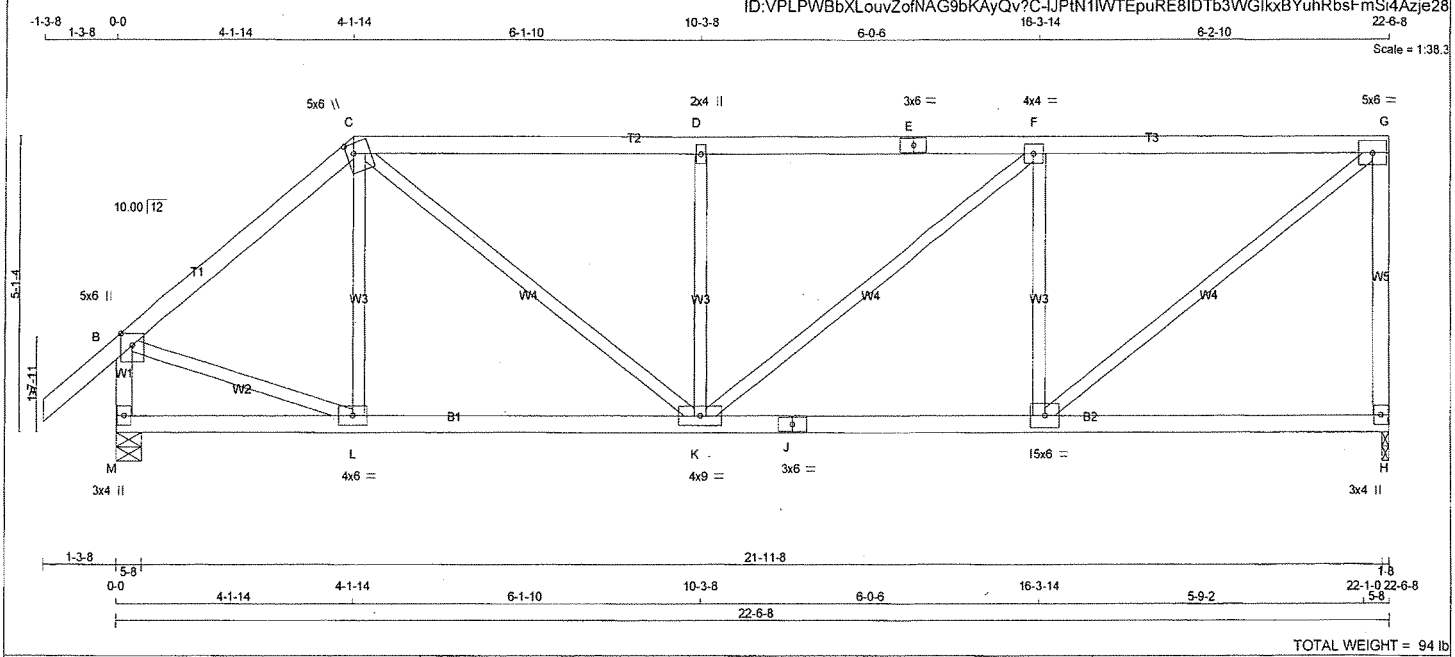


TOTAL WEIGHT = 94 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1321	0	1321	0	0	1-8	1-8
M	1429	0	1429	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1010	471 / 0	237 / 0	0 / 0	0 / 0	302 / 0	0 / 0
M	1084	529 / 0	237 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

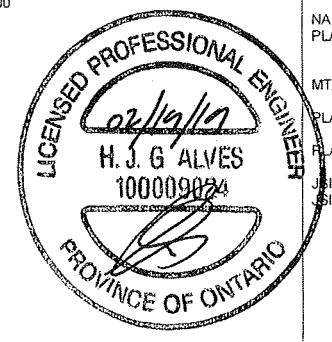
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-65 / 121	0.03 (3)
B- C	-1255 / 0	-77.7	-77.7	0.27 (1)	5.43	B- L	0 / 1005	0.23 (1)
C- D	-1613 / 0	-77.7	-77.7	0.47 (1)	4.68	I- G	0 / 1676	0.38 (1)
D- E	-1613 / 0	-77.7	-77.7	0.48 (1)	4.65	C- K	0 / 828	0.19 (1)
E- F	-1613 / 0	-77.7	-77.7	0.48 (1)	4.65	I- F	-752 / 0	0.29 (1)
F- G	-1326 / 0	-77.7	-77.7	0.46 (1)	5.04	K- D	-510 / 0	0.20 (1)
H- G	-1222 / 0	0.0	0.0	0.54 (1)	7.25	K- F	0 / 367	0.08 (1)
M- B	-1370 / 0	0.0	0.0	0.15 (1)	6.94			
M- L	0 / 0	-39.5	-39.5	0.20 (3)	10.00			
L- K	0 / 960	-39.5	-39.5	0.33 (2)	10.00			
K- J	0 / 1326	-39.5	-39.5	0.45 (2)	10.00			
J- I	0 / 1326	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.45/1.00 (I-K:2) , WB=0.38/1.00 (G-I:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T9039S5
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T63	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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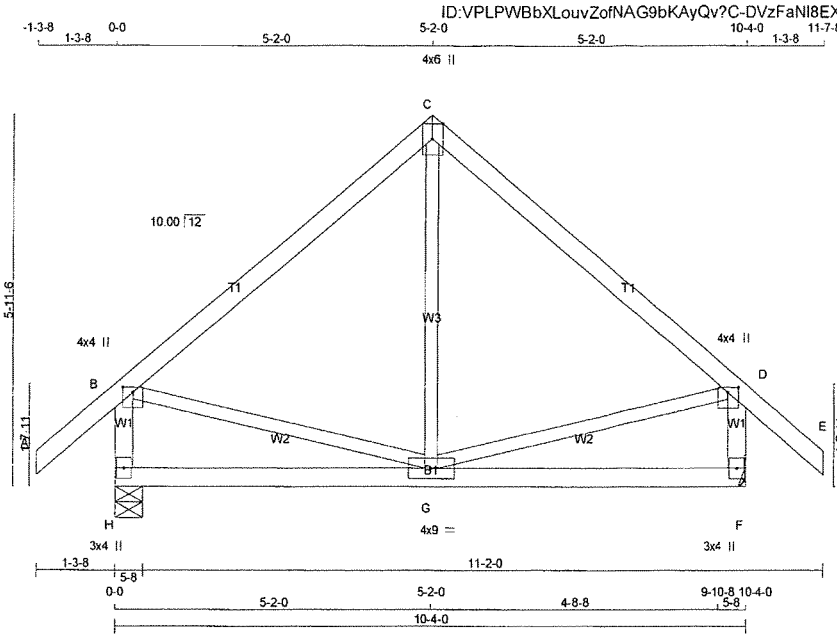


TOTAL WEIGHT = 9 X 47 = 419 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	713	0	713	0	0	5-8	5-8
F	713	0	713	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	537	274 / 0	109 / 0	0 / 0	0 / 0	155 / 0	0 / 0
F	537	274 / 0	109 / 0	0 / 0	0 / 0	155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

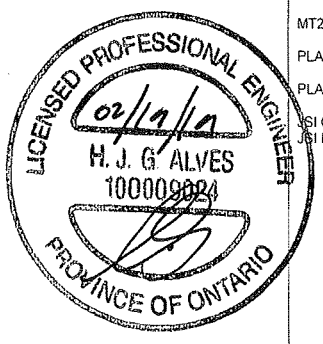
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 229	0.06 (3)
B- C	-392 / 0	-77.7	-77.7	0.27 (1)	6.25	B- G	0 / 311	0.07 (1)
C- D	-392 / 0	-77.7	-77.7	0.27 (1)	6.25	G- D	0 / 311	0.07 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-636 / 0	0.0	0.0	0.07 (1)	7.81			
F- D	-636 / 0	0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.24 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.24 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.24/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.57 (D) (INPUT = 0.90)
SI METAL= 0.16 (D) (INPUT = 1.00)

DWG NO. TAM T1903552
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	G63	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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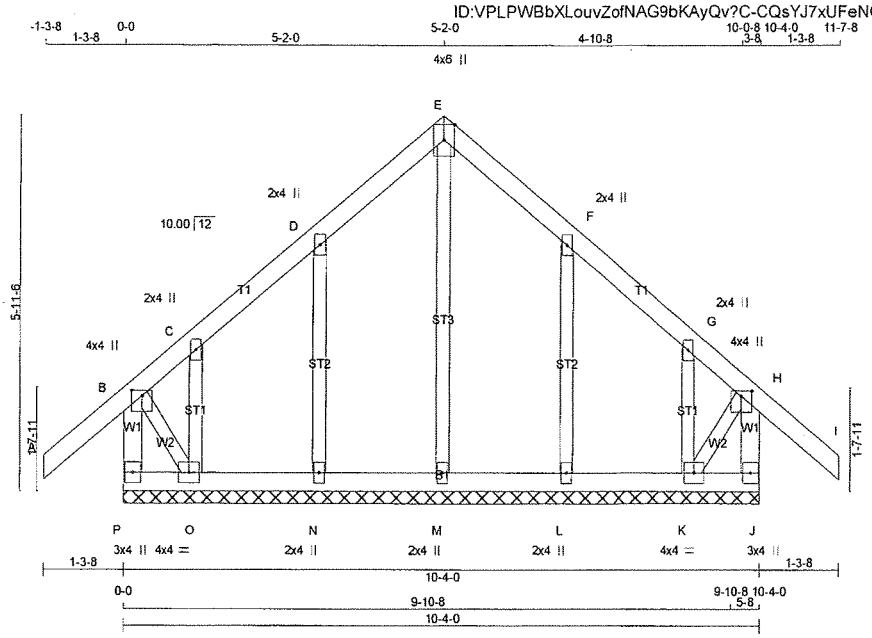


TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

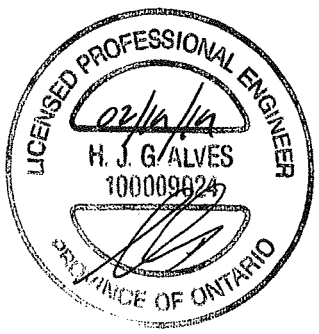
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-230 / 0	0.0	0.0	0.02 (1)	7.81	M- E	-123 / 0	0.06 (1)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- D	-188 / 0	0.05 (1)
B- C	-52 / 0	-77.7	-77.7	0.10 (1)	6.25	O- C	-41 / 0	0.01 (1)
C- D	0 / 0	-77.7	-77.7	0.05 (1)	10.00	L- F	-188 / 0	0.05 (1)
D- E	-15 / 0	-77.7	-77.7	0.05 (1)	6.25	K- G	-41 / 0	0.01 (1)
E- F	-15 / 0	-77.7	-77.7	0.05 (1)	6.25	B- O	0 / 14	0.00 (1)
F- G	0 / 0	-77.7	-77.7	0.05 (1)	10.00	K- H	0 / 14	0.00 (1)
G- H	-52 / 0	-77.7	-77.7	0.10 (1)	6.25			
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
J- H	-230 / 0	0.0	0.0	0.02 (1)	7.81			
P- O	0 / 0	-39.5	-39.5	0.02 (3)	10.00			
O- N	0 / 7	-39.5	-39.5	0.03 (3)	10.00			
N- M	0 / 3	-39.5	-39.5	0.03 (3)	10.00			
M- L	0 / 3	-39.5	-39.5	0.03 (3)	10.00			
L- K	0 / 7	-39.5	-39.5	0.03 (3)	10.00			
K- J	0 / 0	-39.5	-39.5	0.02 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (H-I:1) , BC=0.03/1.00 (K-L:3) , WB=0.06/1.00 (E-M:1) , SSI=0.06/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (H) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM T1203557
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	G64	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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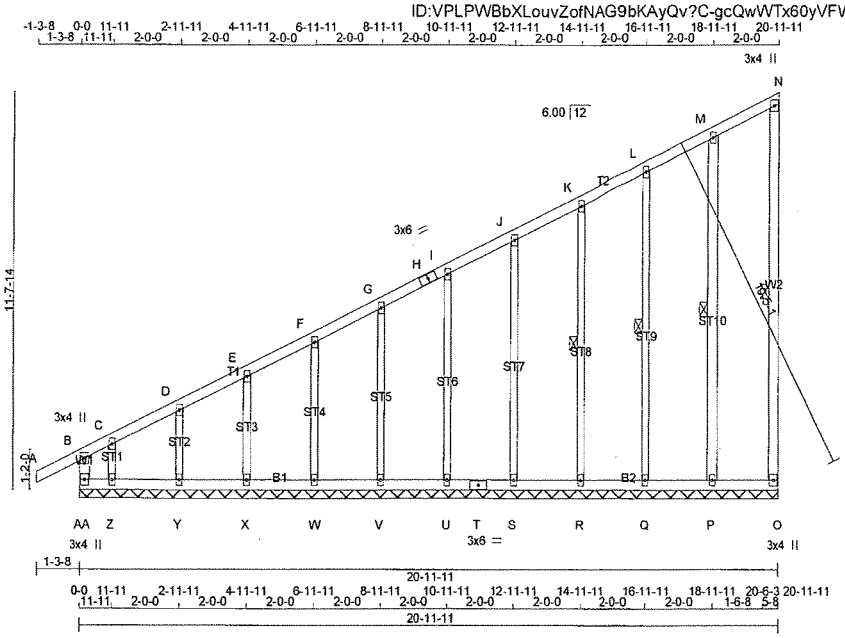


TOTAL WEIGHT = 2 X 115 = 229 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AA- B	2x4	DRY	No.2	SPF
AA- T	2x4	DRY	No.2	SPF
T - O	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, L, M						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
N	TMV+p	MT20	3.0	4.0		
O	BMV1+p	MT20	3.0	4.0		
P, Q, R, S, U, V, W, X, Y, Z						
P	BMW1+w	MT20	2.0	4.0		
T	BS-t	MT20	3.0	6.0		
AA	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF N-O, M-P, L-Q, K-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00		P- M	-173 / 0	0.10 (1)
B- C	-51 / 0	-77.7	-77.7	0.09 (1)	6.25		Q- L	-150 / 0	0.09 (1)
C- D	-17 / 0	-77.7	-77.7	0.04 (1)	6.25		R- K	-156 / 0	0.08 (1)
D- E	-17 / 0	-77.7	-77.7	0.04 (1)	6.25		S- J	-155 / 0	0.17 (1)
E- F	-13 / 0	-77.7	-77.7	0.04 (1)	6.25		U- I	-155 / 0	0.11 (1)
F- G	-10 / 0	-77.7	-77.7	0.04 (1)	6.25		V- G	-154 / 0	0.08 (1)
G- H	-8 / 0	-77.7	-77.7	0.04 (1)	10.00		W- F	-155 / 0	0.05 (1)
H- I	-8 / 0	-77.7	-77.7	0.04 (1)	10.00		X- E	-151 / 0	0.03 (1)
I- J	-6 / 0	-77.7	-77.7	0.04 (1)	10.00		Y- D	-165 / 0	0.03 (1)
J- K	-5 / 0	-77.7	-77.7	0.04 (1)	10.00		Z- C	-43 / 0	0.01 (1)
K- L	-4 / 0	-77.7	-77.7	0.04 (1)	10.00				
L- M	0 / 0	-77.7	-77.7	0.04 (1)	10.00				
M- N	-6 / 0	-77.7	-77.7	0.04 (1)	10.00				
O- N	-68 / 0	0.0	0.0	0.01 (1)	6.25				
AA- B	-211 / 0	0.0	0.0	0.03 (1)	7.81				
AA- Z	0 / 21	-39.5	-39.5	0.04 (1)	10.00				
Z- Y	0 / 19	-39.5	-39.5	0.03 (2)	10.00				
Y- X	0 / 15	-39.5	-39.5	0.03 (2)	10.00				
X- W	0 / 12	-39.5	-39.5	0.02 (3)	10.00				
W- V	0 / 9	-39.5	-39.5	0.02 (3)	10.00				
V- U	0 / 7	-39.5	-39.5	0.02 (3)	10.00				
U- T	0 / 6	-39.5	-39.5	0.02 (3)	10.00				
T- S	0 / 6	-39.5	-39.5	0.02 (3)	10.00				
S- R	0 / 4	-39.5	-39.5	0.02 (3)	10.00				
R- Q	0 / 3	-39.5	-39.5	0.02 (3)	10.00				
Q- P	0 / 2	-39.5	-39.5	0.03 (3)	10.00				
P- O	0 / 0	-39.5	-39.5	0.03 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.04/1.00 (Z-AA:1) , WB=0.17/1.00 (J-S:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

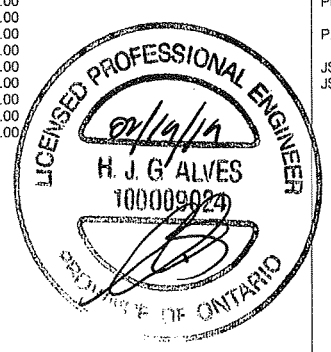
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (G) (INPUT = 0.90)
JSI METAL= 0.07 (M) (INPUT = 1.00)



DWG NO. TAM 1790355B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	G65	2	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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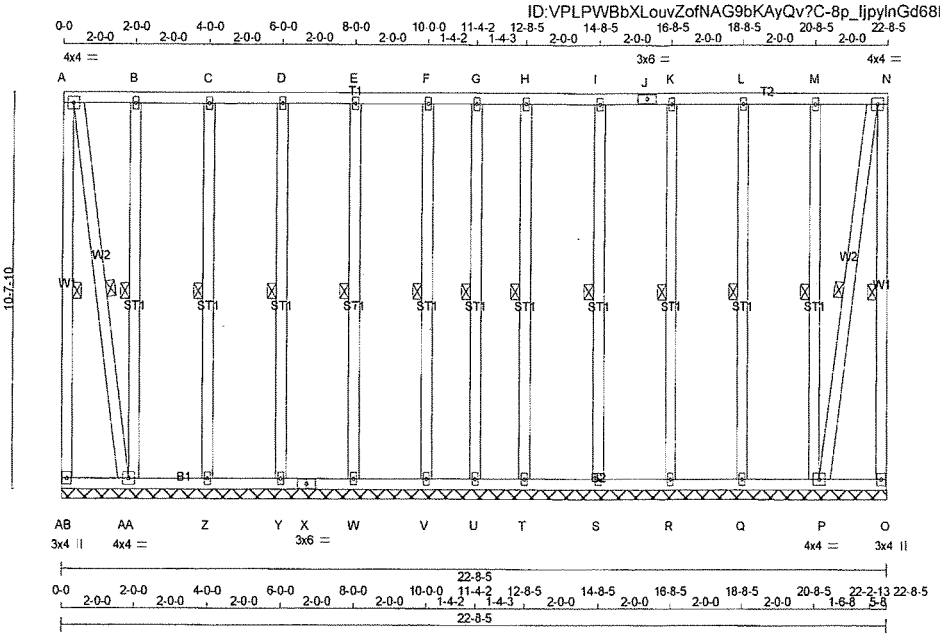

TOTAL WEIGHT = 2 X 224 = 448 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AB- A	2x4	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AB- X	2x4	DRY	No.2	SPF
X - O	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	4.0	4.0	
B, C, D, E, F, G, H, I, K, L, M					
B	TMW+w	MT20	2.0	4.0	
J	TS-t	MT20	3.0	6.0	
N	TMVW-t	MT20	4.0	4.0	
O	BMV1+p	MT20	3.0	4.0	
P	BMWW1-t	MT20	4.0	4.0	
Q, R, S, T, U, V, W, Y, Z					
Q	BMW1+w	MT20	2.0	4.0	
X	BS-t	MT20	3.0	6.0	
AA	BMWW1-t	MT20	4.0	4.0	
AB	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-AB, N-O, G-U, M-P, L-Q, K-R, I-S, H-T, B-AA, C-Z, D-Y, E-W, F-V, A-AA, N-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- A	-33 / 0	0.0	0.0	0.02 (1)	6.25		U- G	-95 / 0	0.05 (1)
A- B	0 / 5	-77.7	-77.7	0.04 (1)	10.00		P- M	-171 / 0	0.10 (1)
B- C	0 / 0	-77.7	-77.7	0.04 (1)	10.00		Q- L	-154 / 0	0.09 (1)
C- D	0 / 0	-77.7	-77.7	0.04 (1)	10.00		R- K	-155 / 0	0.09 (1)
D- E	0 / 0	-77.7	-77.7	0.04 (1)	10.00		S- I	-160 / 0	0.09 (1)
E- F	0 / 0	-77.7	-77.7	0.04 (1)	10.00		T- H	-131 / 0	0.07 (1)
F- G	0 / 0	-77.7	-77.7	0.03 (1)	10.00		AA- B	-171 / 0	0.10 (1)
G- H	0 / 0	-77.7	-77.7	0.03 (1)	10.00		Z- C	-154 / 0	0.09 (1)
H- I	0 / 0	-77.7	-77.7	0.04 (1)	10.00		Y- D	-155 / 0	0.09 (1)
I- J	0 / 0	-77.7	-77.7	0.04 (1)	10.00		W- E	-160 / 0	0.09 (1)
J- K	0 / 0	-77.7	-77.7	0.04 (1)	10.00		V- F	-131 / 0	0.07 (1)
K- L	0 / 0	-77.7	-77.7	0.04 (1)	10.00		A-AA	-31 / 0	0.02 (1)
L- M	0 / 0	-77.7	-77.7	0.04 (1)	10.00		P- N	-31 / 0	0.02 (1)
M- N	0 / 5	-77.7	-77.7	0.04 (1)	10.00				
O- N	-33 / 0	0.0	0.0	0.02 (1)	6.25				
AB-AA	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
AA- Z	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
Z- Y	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
Y- X	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
X- W	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
W- V	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
V- U	0 / 0	-39.5	-39.5	0.02 (3)	10.00				
U- T	0 / 0	-39.5	-39.5	0.02 (3)	10.00				
T- S	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
S- R	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
R- Q	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
Q- P	0 / 0	-39.5	-39.5	0.03 (3)	10.00				
P- O	0 / 0	-39.5	-39.5	0.03 (3)	10.00				

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100009929
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (A-B:1) , BC=0.03/1.00 (Z-AA:3) , WB=0.10/1.00 (B-AA:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (AA) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 1903559
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401368	T67TCX	9	1	UNITS 11 - 15 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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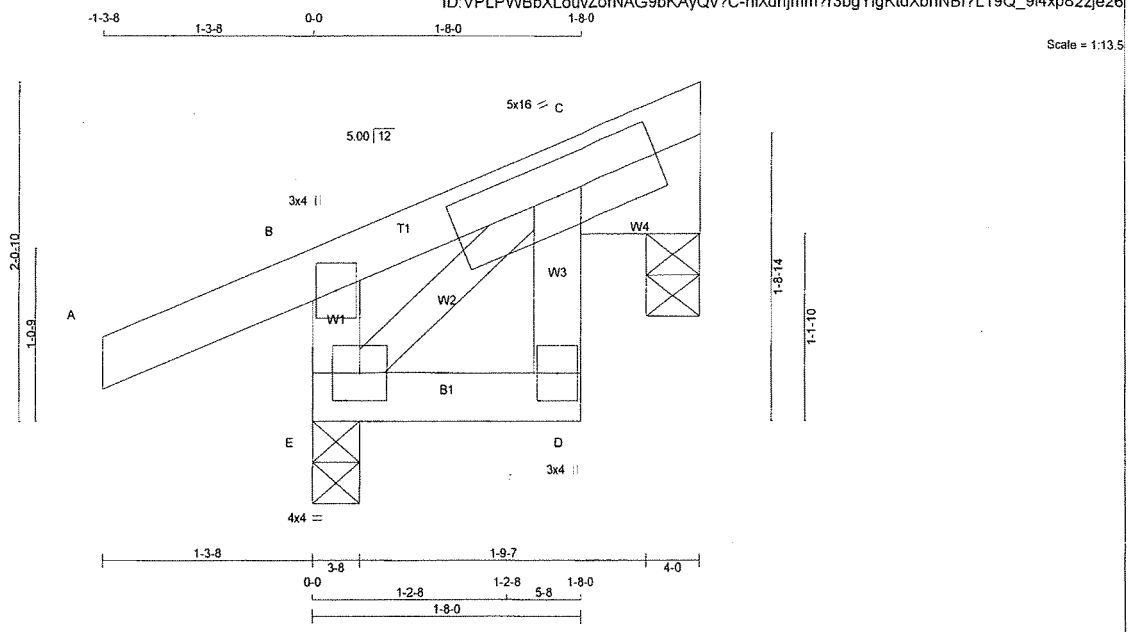


TOTAL WEIGHT = 9 X 12 = 104 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	16.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	55	0	58	0	-22	4-0	4-0
E	245	0	245	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	45	12 / -23	17 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	176	114 / 0	17 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

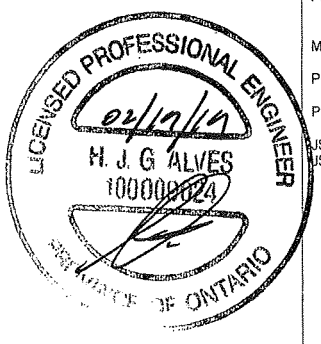
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.10 (1)	10.00		E- C	0 / 0	0.00 (1)
B- C	-16 / 0	-77.7	-77.7	0.10 (1)	6.25				
D- C	0 / 42	0.0	0.0	0.01 (3)	10.00				
E- B	-212 / 0	0.0	0.0	0.02 (1)	7.81				
E- D	0 / 0	-39.5	-39.5	0.03 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

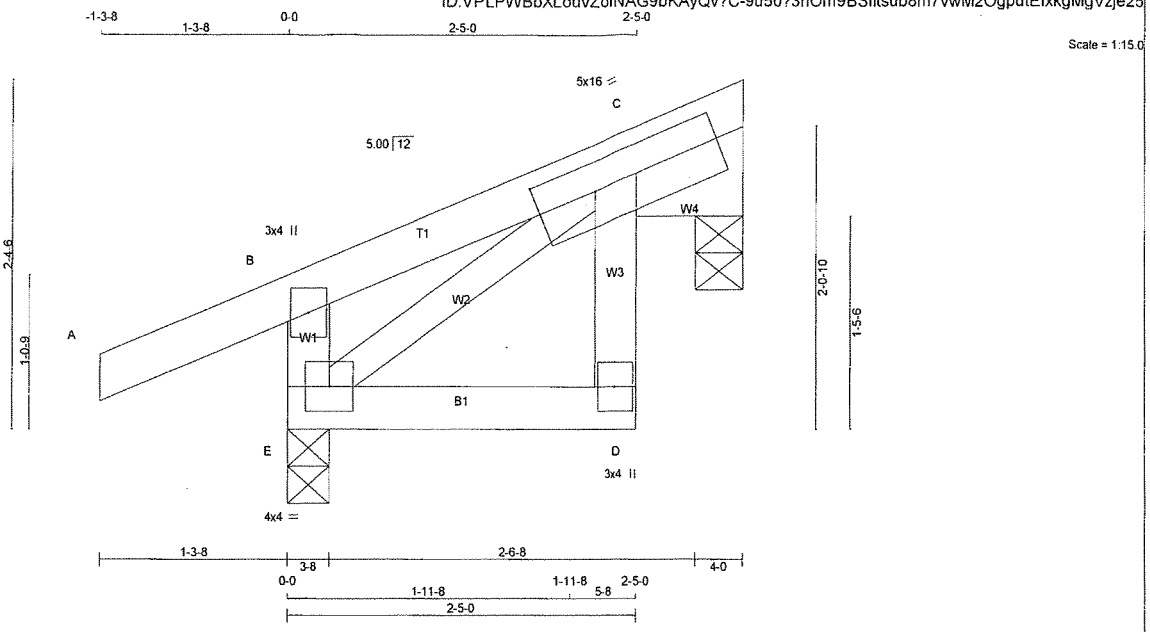
USI GRIP= 0.13 (E) (INPUT = 0.90)
USI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 7903560
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 11 - 15	DRWG NO.
401368	T68TCX	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:VPLPWBbXLouvZofNAG9bKAyQv?C-9u50?3nOm9BSlitsub8m7vwM2OgputElxkgMgVzje25



TOTAL WEIGHT = 7 X 14 = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	16.0	2.25	5.00
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	142	0	142	0	0	4-0	4-0
E	246	0	246	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	108	51 / 0	25 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	181	107 / 0	25 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

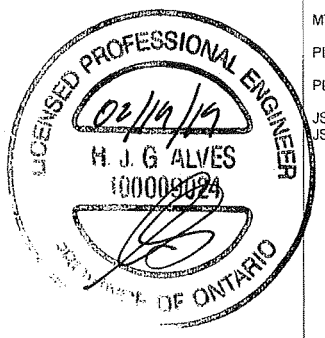
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-77.7	-77.7	0.08 (1)	10.00			
D- C	0 / 60	0.0	0.0	0.01 (3)	10.00			
E- B	-199 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-39.5	-39.5	0.06 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.06/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM T1903561
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

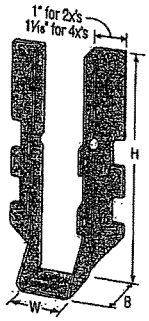
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

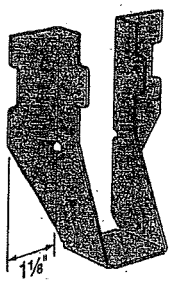
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

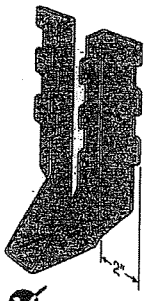
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



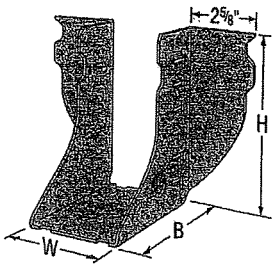
LUS28



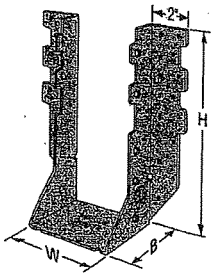
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

Double-Shear Nailing Top View

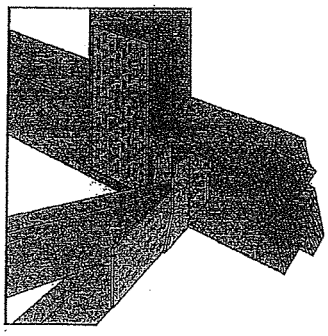
Double-Shear Nailing Side View; Do not bend tab

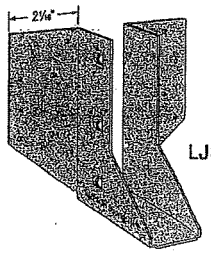
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

HHUS/HGUS

See Hanger Options information on pp. 125–127.

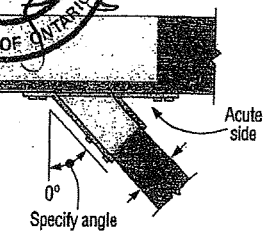
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¼	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅞	3	3⅛	(14) 16d	(6) 16d	2705 / 11.30	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(18) 16d	(6) 16d	2055 / 9.1	4265 / 19.0	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅞	5	4⅛	(20) 16d	(8) 16d	2685 / 11.95	6625 / 29.5	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.74	7675 / 34.15	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7 13/16	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

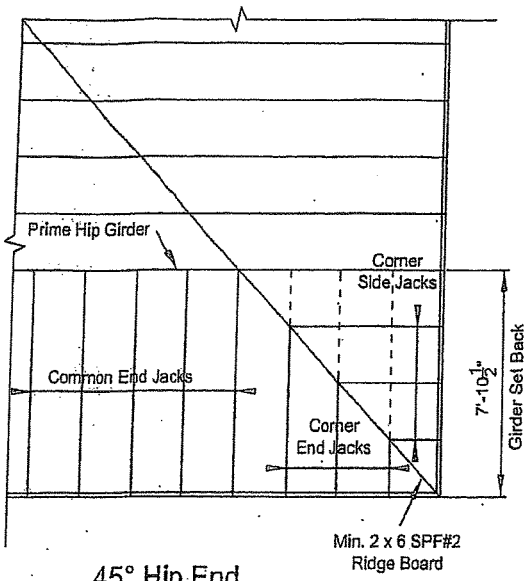
SIMPSON Strong-Tie®

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_s[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⁹⁄₁₆	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS ■ LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⁹⁄₁₆	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁹⁄₁₆	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⁹⁄₁₆	9⅛	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁹⁄₁₆	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄₁₆	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄₁₆	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⁹⁄₁₆	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⁹⁄₁₆	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⁹⁄₁₆	8¾	2	5⁹⁄₁₆	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.



45° Hip End

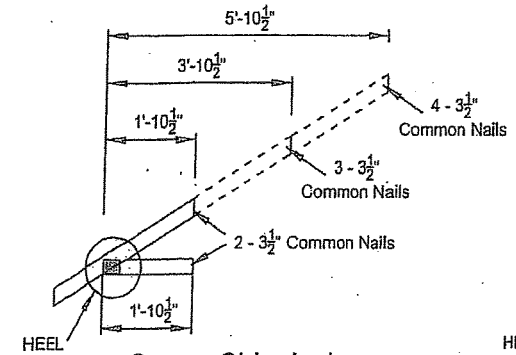
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

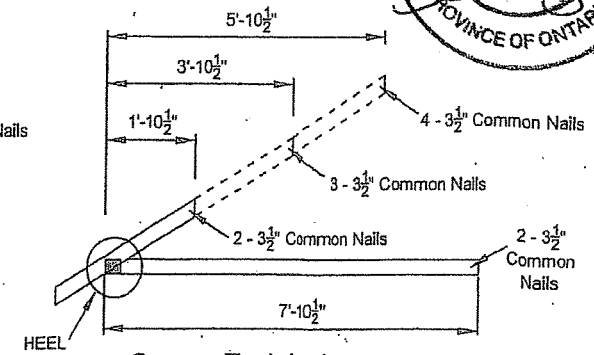
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

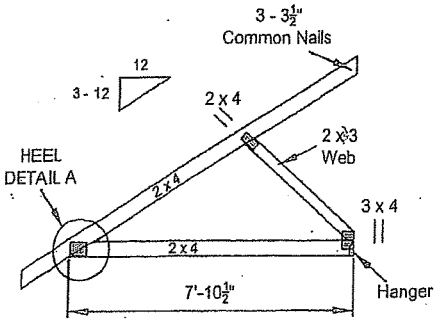
LICENSED PROFESSIONAL ENGINEER
26/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



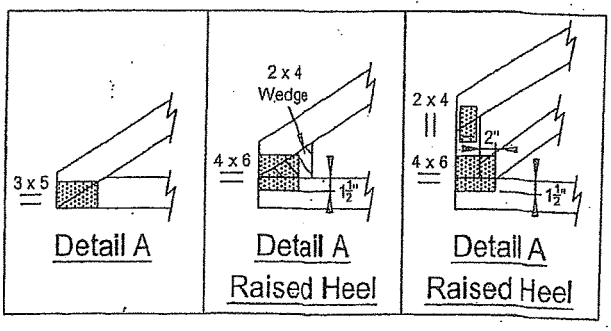
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

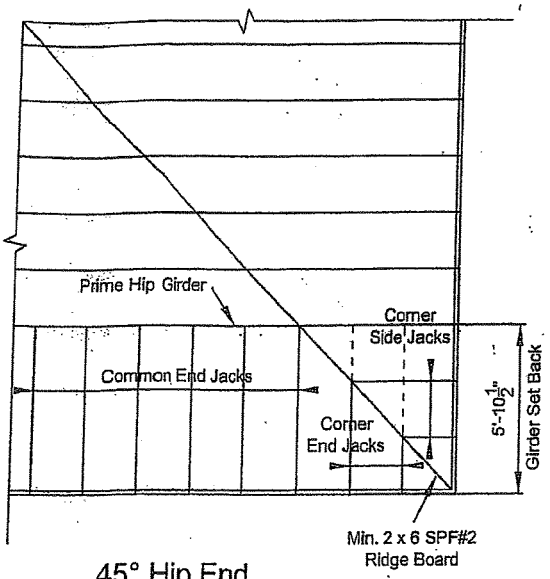
T-1800217

LUMBER SPECIFICATION

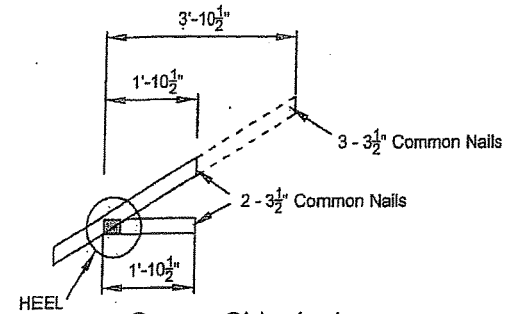
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

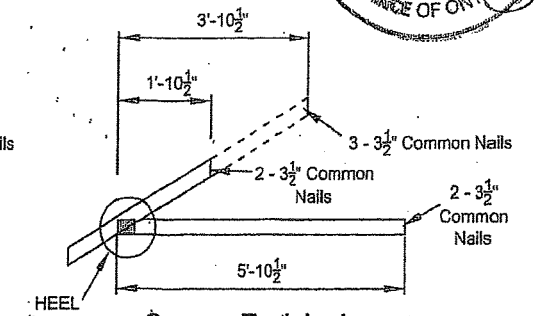
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



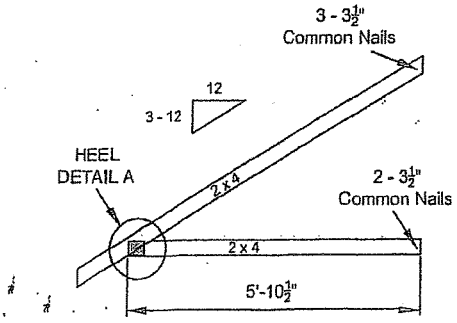
45° Hip End



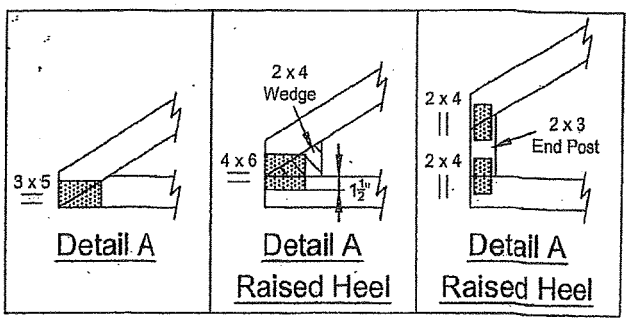
Corner Side Jacks



Corner End Jacks



Common End Jacks

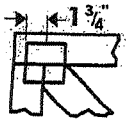


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

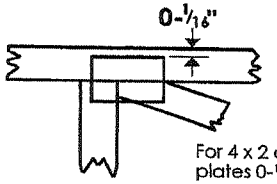
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

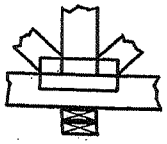
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

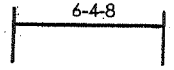


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

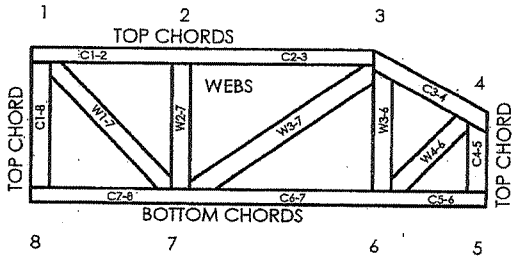
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

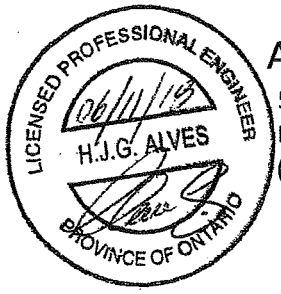
MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018