

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

DENOTES:

ALL B -2-2X10 FLUSH

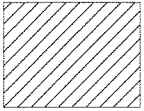
- *** - ALL FIRST FLOOR
PLATE HEIGHT 7'-10"-0
U/S @ PLATE LEVEL
- V** -6/12 VAULT CEILING
(1'-5"-0 HIGH A.P.P)
- C** - 5/12 CATH. CEILING
- H** -12" RAISED CEILING
+4" RAISED HEEL

HARDWARE:

- LUS26-2 -(X)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

T-180690

DENOTES:
CONVENTIONAL
FRAMING

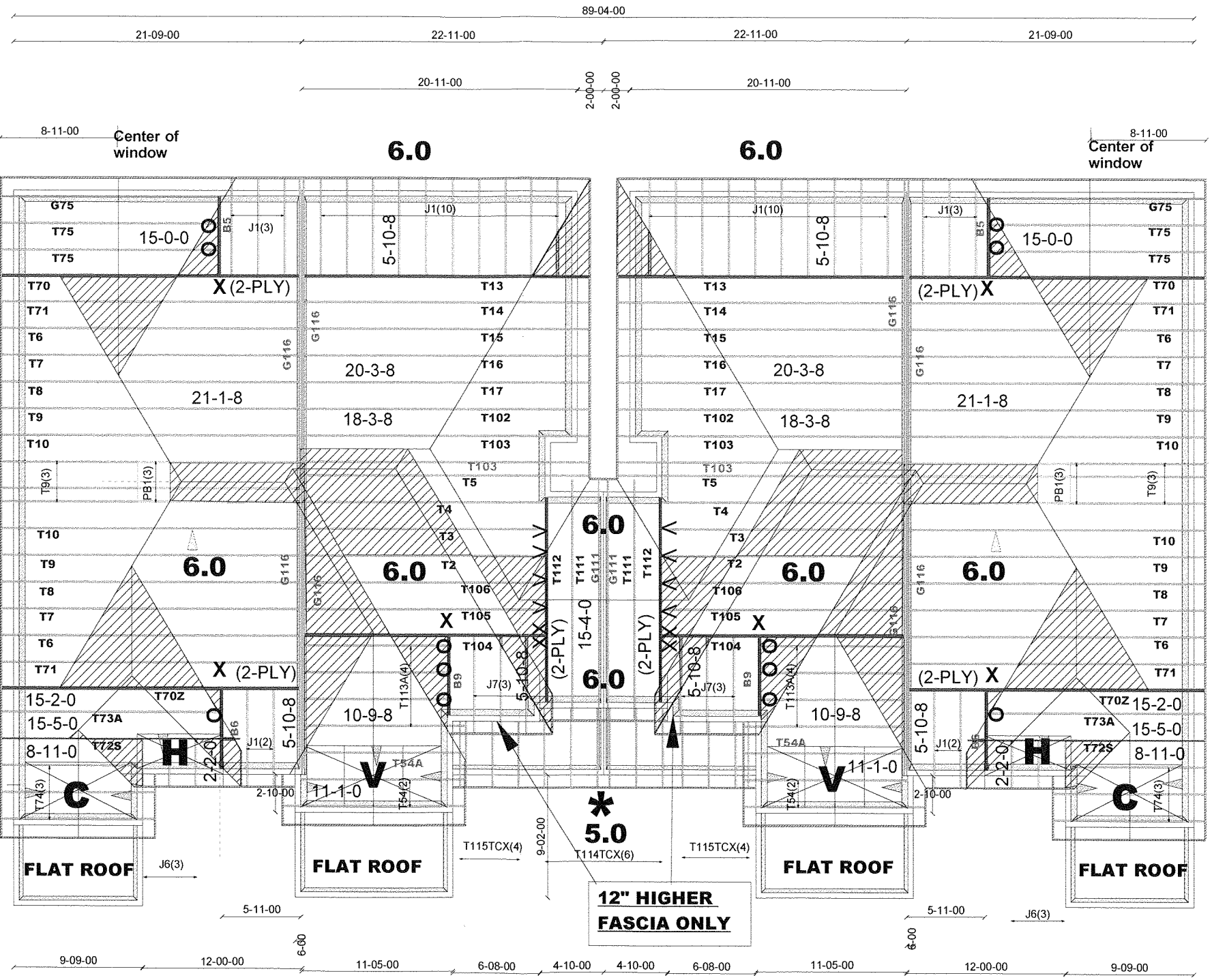


TH-6E
EL:B UNIT41
401392

TH-3
EL:B UNIT42
401393

TH-3
EL:B(REV) UNIT43
401394

TH-6E
EL:B(REV) UNIT44
401395



10/12 PITCHES (TYP.)
UNLESS NOTED

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

- SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



Job Track: 50264
Plan Log: 200511
Layout ID: 401391

Builder / Location:
BAYVIEW WELLINGTON / ST CATHERINES

Project: **PASSAGE ON THE CANAL**

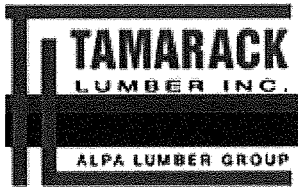
Date: 2/13/2019 Sales: Mario DiCano Designer: AC

Model / Elevation:
FREEHOLD BLOCK 6 / UNITS 41 - 44

Mitek ver 8.2.3.229

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REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

DELIVERY SHIPLIST



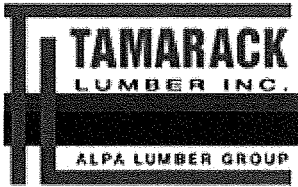
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-6E
 Lot #:
 Elevation: B UNIT 41

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401392
 Ref #
 Page: 1 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T6 Half Hip	10 /12	21-01-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	191.49 122.67		
	2	T7 Half Hip	10 /12	21-01-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	2	T8 Half Hip	10 /12	21-01-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	5	T9 Half Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	573.25 364.17		
	2	T10 Half Hip	10 /12	21-01-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	241.17 151.00		
	1 2-ply	T70 Half Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	1 2-ply	T70Z Half Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	2	T71 Half Hip	10 /12	21-01-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	179.6 118.67		
	1	T72S Hip Girder	10 /12	15-05-00	4-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-11-11	74.78 48.67		
	1	T73A Hip	10 /12	15-02-00	6-05-06	2 x 4	1-03-08	1-07-11 3-02-03	69.05 44.17		
	3	T74 Scissor	10 /12 5 /12	8-11-00	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	125.83 83.00		
	2	T75 Common	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	140.22 93.33		
	1	G75 GABLE	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	74.49 49.67		
	2	G116 GABLE	6 /12	21-05-00	10-07-10	2 x 4	1-03-08	1-02-00 10-07-10	259.11 166.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-6E
 Lot #:
 Elevation: B UNIT 41

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401392
 Ref #
 Page: 2 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	PB1 Piggyback	10 /12	12-02-00	2-00-00	2 x 4		2-00-00	117.31 79.00		
	5	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	3	J6 Jack-Open	10 /12	2-02-00	3-09-06	2 x 4	1-03-08	1-11-11 3-09-06	34.05 24.50		

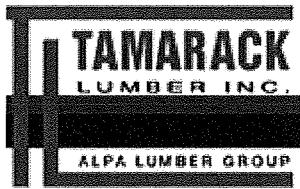
TOTAL # TRUSS= 40

TOTAL BFT OF ALL TRUSSES= 1908.19 BFT. TOTAL WEIGHT OF ALL TRSSES 2963.98 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 5



DELIVERY SHIPLIST

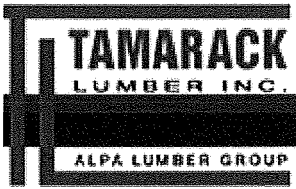
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-3
 Lot #:
 Elevation: B UNIT 42

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401393
 Ref #
 Page: 1 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T2 Half Hip	10 /12	18-03-08	6-07-04	2 x 4		3-03-11 6-07-04	81.92 53.50		
	1	T3 Half Hip	10 /12	18-03-08	7-07-04	2 x 4		3-03-11 7-07-04	86.37 55.00		
	1	T4 Half Hip	10 /12	18-03-08	8-07-04	2 x 4		3-03-11 8-07-04	95.61 61.33		
	1	T5 Half Hip	10 /12	18-03-08	9-07-10	2 x 6 2 x 4	3-03-08	3-03-11 9-07-04	127.72 79.33		
	1 2-ply	T13 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 113.67		
	1	T14 Half Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 56.50		
	1	T15 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.67		
	1	T16 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	2	T54 Roof Special	10 /12 6 /12	11-01-00	6-03-02	2 x 3	1-03-08 1-03-08	1-07-11 1-07-11	93.69 63.00		
	1	T54A Roof Special	10 /12 6 /12	10-09-08	6-03-02	2 x 3	1-03-08	1-07-11 1-07-11	45.12 30.00		
	1	T102 Half Hip	10 /12	20-03-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	112.23 72.17		
	2	T103 Half Hip	10 /12	20-03-08	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	236.5 149.67		
	1 2-ply	T104 Half Hip Girder	10 /12	18-03-08	5-01-04	2 x 4 2 x 6		1-11-06 5-01-04	181.53 116.67		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-3
 Lot #:
 Elevation: B UNIT 42

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401393
 Ref #
 Page: 2 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T105 Half Hip	10 /12	18-03-08	6-01-04	2 x 4		1-11-06 6-01-04	78.39 49.50		
	1	T106 Half Hip	10 /12	18-03-08	7-01-04	2 x 4		1-11-06 7-01-04	82.98 52.83		
	1	T111 Common	6 /12	15-04-00	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	60.25 39.17		
	1	G111 GABLE	6 /12	15-04-00	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	57.71 38.17		
	1 2-ply	T112 Common Girder	6 /12	15-04-00	5-00-00	2 x 4 2 x 6		1-02-00 1-02-00	142.58 89.00		
	4	T113A Common	10 /12	10-09-08	6-03-02	2 x 4	1-03-08	1-07-11 1-10-10	187.29 124.00		
	3	T114TCX Monopitch	5 /12	4-07-00	3-03-04	2 x 4	1-03-08	1-00-09 2-11-08	67.1 45.50		
	4	T115TCX Monopitch	5 /12	3-07-00	2-10-04	2 x 4	1-03-08	1-00-09 2-06-08	74.56 50.67		
	2	G116 GABLE	6 /12	21-05-00	10-07-10	2 x 4	1-03-08	1-02-00 10-07-10	259.11 166.00		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	3	J7 Jack-Open	6 /12	5-10-08	5-01-04	2 x 4	1-03-08	2-02-00 5-01-04	66.99 43.00		

TOTAL # TRUSS= 50

TOTAL BFT OF ALL TRUSSES= 1833.02

BFT.

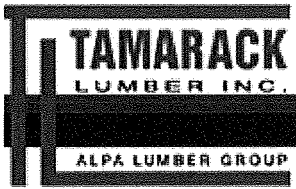
TOTAL WEIGHT OF ALL TRSSES 2842.56 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 9

DELIVERY SHIPLIST



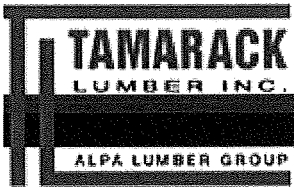
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-3
 Lot #:
 Elevation: B UNIT 43

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401394
 Ref #
 Page: 1 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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	1	T4 Half Hip	10 /12	18-03-08	8-07-04	2 x 4		3-03-11 8-07-04	95.61 61.33		
	1	T5 Half Hip	10 /12	18-03-08	9-07-10	2 x 6 2 x 4	3-03-08	3-03-11 9-07-04	127.72 79.33		
	1 2-ply	T13 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 113.67		
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	1	T15 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.67		
	1	T16 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	2	T54 Roof Special	10 /12 6 /12	11-01-00	6-03-02	2 x 3	1-03-08 1-03-08	1-07-11 1-07-11	93.69 63.00		
	1	T54A Roof Special	10 /12 6 /12	10-09-08	6-03-02	2 x 3	1-03-08	1-07-11 1-07-11	45.12 30.00		
	1	T102 Half Hip	10 /12	20-03-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	112.23 72.17		
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DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-3
 Lot #:
 Elevation: B UNIT 43

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401394
 Ref #
 Page: 2 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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TOTAL # TRUSS= 50

TOTAL BFT OF ALL TRUSSES= 1833.02

BFT.

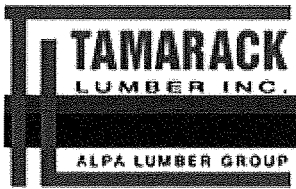
TOTAL WEIGHT OF ALL TRSSES 2842.56 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
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TOTAL NUMBER OF ITEMS= 9

DELIVERY SHIPLIST



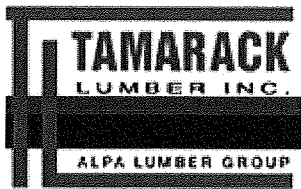
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-6E
 Lot #:
 Elevation: B UNIT 44

Job Track: 50264
 PlanLog: 200511
 Layout ID: 401395
 Ref #
 Page: 1 of 2
 Date: 02/13/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
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	5	T9 Half Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	573.25 364.17		
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DELIVERY SHIPLIST



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 Builder: BAYVIEW WELLINGTON /
 Project: PASSAGE ON THE CANAL
 Location: ST CATHERINES
 Model: TH-6E
 Lot #:
 Elevation: B UNIT 44

Job Track: 50264
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 Page: 2 of 2
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 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

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TOTAL # TRUSS= 40

TOTAL BFT OF ALL TRUSSES= 1908.19

BFT.

TOTAL WEIGHT OF ALL TRSSES 2963.98 LBS

HARDWARE

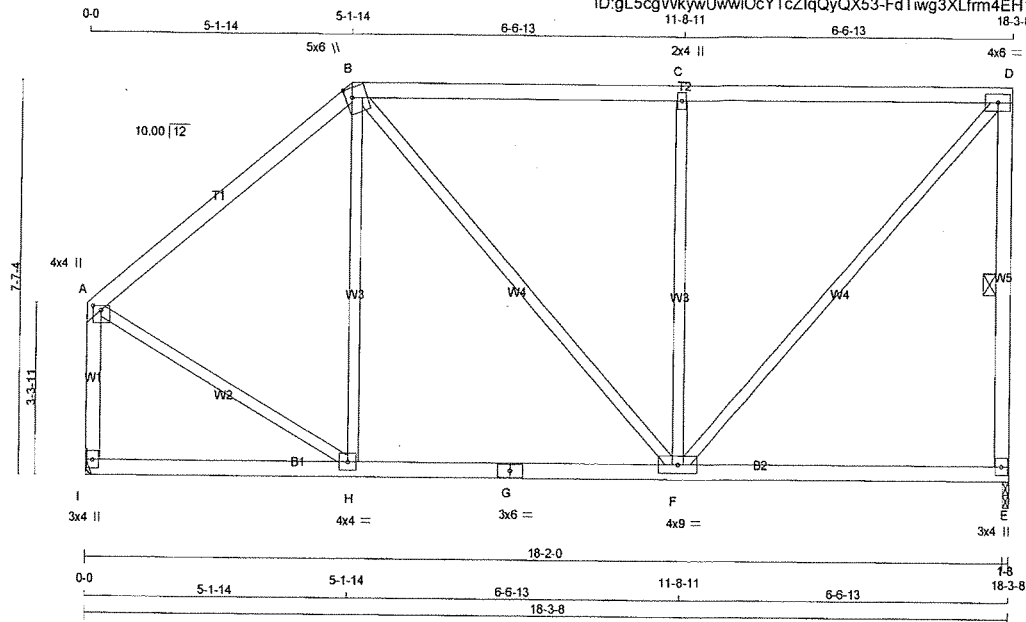
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 5

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T3	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 86 = 173 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTVW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-l	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWVW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMVWV-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	1066	0	1066	0	0	1-8	1-8
I	1066	0	1066	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

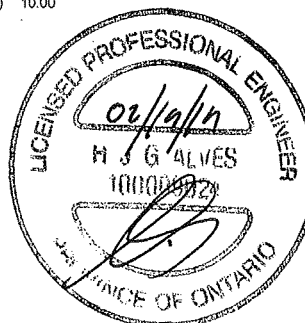
1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
A-B		-731 / 0	-78.0	-78.0	0.39 (1)	6.25	H-B	-76 / 125	0.08 (1)
B-C		-694 / 0	-78.0	-78.0	0.60 (1)	6.05	B-F	0 / 202	0.05 (1)
C-D		-694 / 0	-78.0	-78.0	0.60 (1)	6.05	F-C	-835 / 0	0.67 (1)
E-D		-963 / 0	0.0	0.0	0.25 (1)	6.25	F-D	0 / 1035	0.23 (1)
I-A		-987 / 0	0.0	0.0	0.19 (1)	7.81	A-H	0 / 644	0.14 (1)
I-H		0 / 0	-38.5	-38.5	0.22 (3)	10.00			
H-G		0 / 559	-38.5	-38.5	0.37 (2)	10.00			
G-F		0 / 559	-38.5	-38.5	0.37 (2)	10.00			
F-E		0 / 0	-38.5	-38.5	0.31 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	44.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.12")

CSI: TC=0.60/1.00 (C-D-1), BC=0.37/1.00 (F-H-2), WB=0.67/1.00 (C-F-1), SSI=0.25/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (A) (INPUT = 0.90)
JSI METAL= 0.33 (G) (INPUT = 1.00)

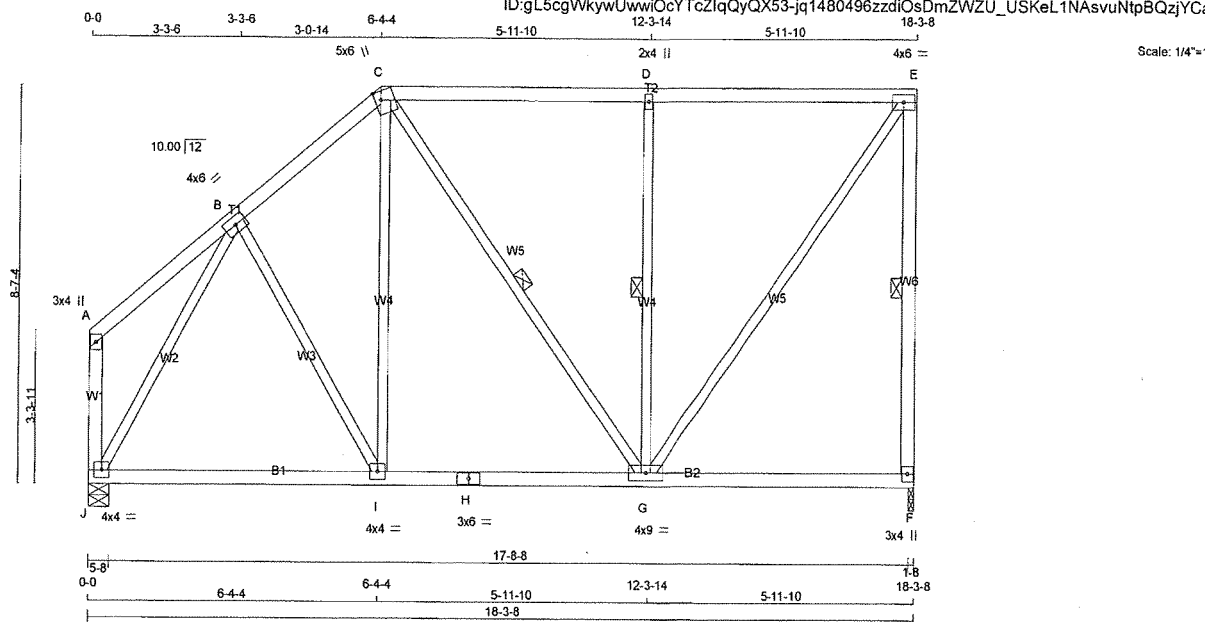
DWG NO. TAM 17203695
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T4	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 96 = 191 lb

LUMBER

N L G A RULES	CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1066	0	1066	0	0	1-8	1-8
J	1066	0	1066	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM TO		FR-TO			
A-B	0 / 17	-78.0	-78.0 0.12 (1)	10.00	B-I	0 / 141	0.03 (3)
B-C	-737 / 0	-78.0	-78.0 0.13 (1)	6.25	I-C	0 / 219	0.05 (3)
C-D	-572 / 0	-78.0	-78.0 0.48 (1)	6.25	C-G	-11 / 35	0.01 (3)
D-E	-573 / 0	-78.0	-78.0 0.48 (1)	6.25	G-D	-578 / 0	0.27 (1)
E-F	-970 / 0	0.0	0.0 0.33 (1)	6.25	G-E	0 / 977	0.22 (1)
J-A	-98 / 0	0.0	0.0 0.02 (1)	7.81	J-B	-996 / 0	0.72 (1)
J-I	0 / 492	-38.5	-38.5 0.33 (2)	10.00			
I-H	0 / 552	-38.5	-38.5 0.34 (2)	10.00			
H-G	0 / 552	-38.5	-38.5 0.34 (2)	10.00			
G-F	0 / 0	-38.5	-38.5 0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.12")

CSI: TC=0.48/1.00 (D-E-1), BC=0.34/1.00 (G-I-2), WB=0.72/1.00 (B-J-1), SSI=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

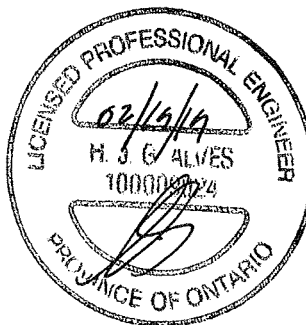
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



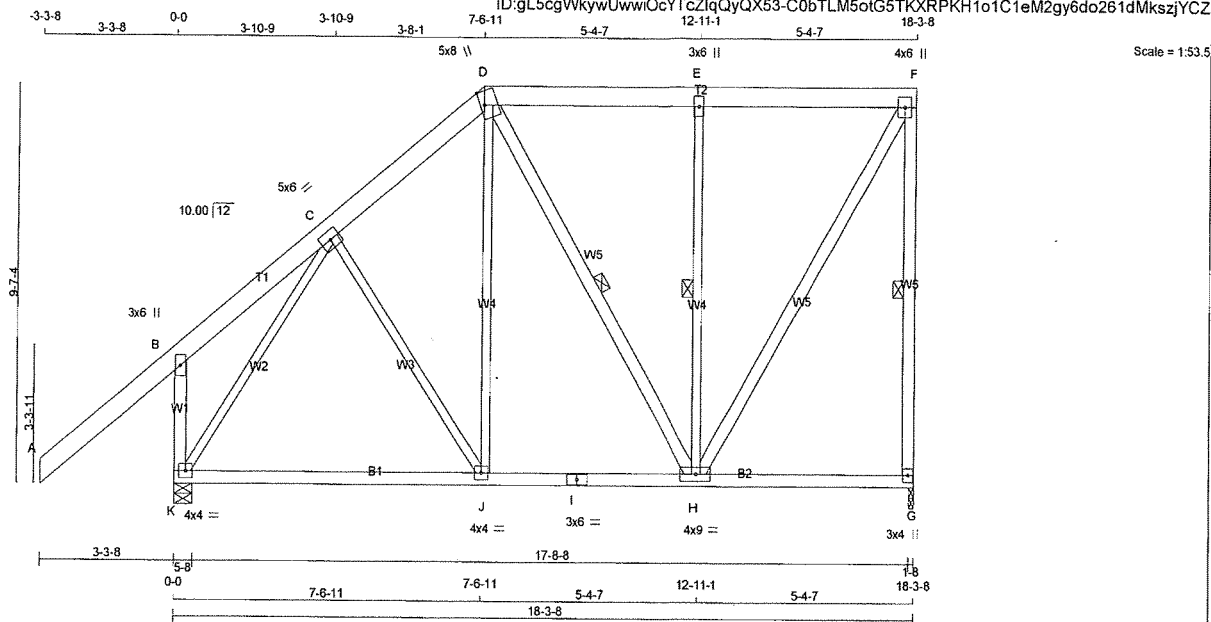
DWG NO. TAM 7120 3696
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T5	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 128 = 255 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMVWV-t	MT20	5.0	6.0		
D	TTWV+m	MT20	5.0	8.0	4.00	1.25
E	TMVW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVWV-t	MT20	4.0	4.0		
K	BMVWV-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
JT	VERT	HORZ	BRG	BRG
G	1040	0	1-8	1-8
K	1359	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
G	797
K	1017

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WEBS
MEMB.	MEMB.
MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)
FROM TO	FROM TO
A-B	C-J
B-C	D-H
C-D	E-F
D-E	G-H
E-F	K-C
F-G	
G-H	
H-I	
I-J	
J-K	
K-L	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (TL) = $L/978$ (0.22")

CSI: TC=0.40/1.00 (F-G:1), BC=0.42/1.00 (H-J:2), VB=0.74/1.00 (C-K:1), SSI=0.18/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
		788	1987
			1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.69 (H) (INPUT = 0.90)
JSI METAL = 0.21 (F) (INPUT = 1.00)

DWG NO. TAM 7203677
STRUCTURAL
COMPONENT ONLY

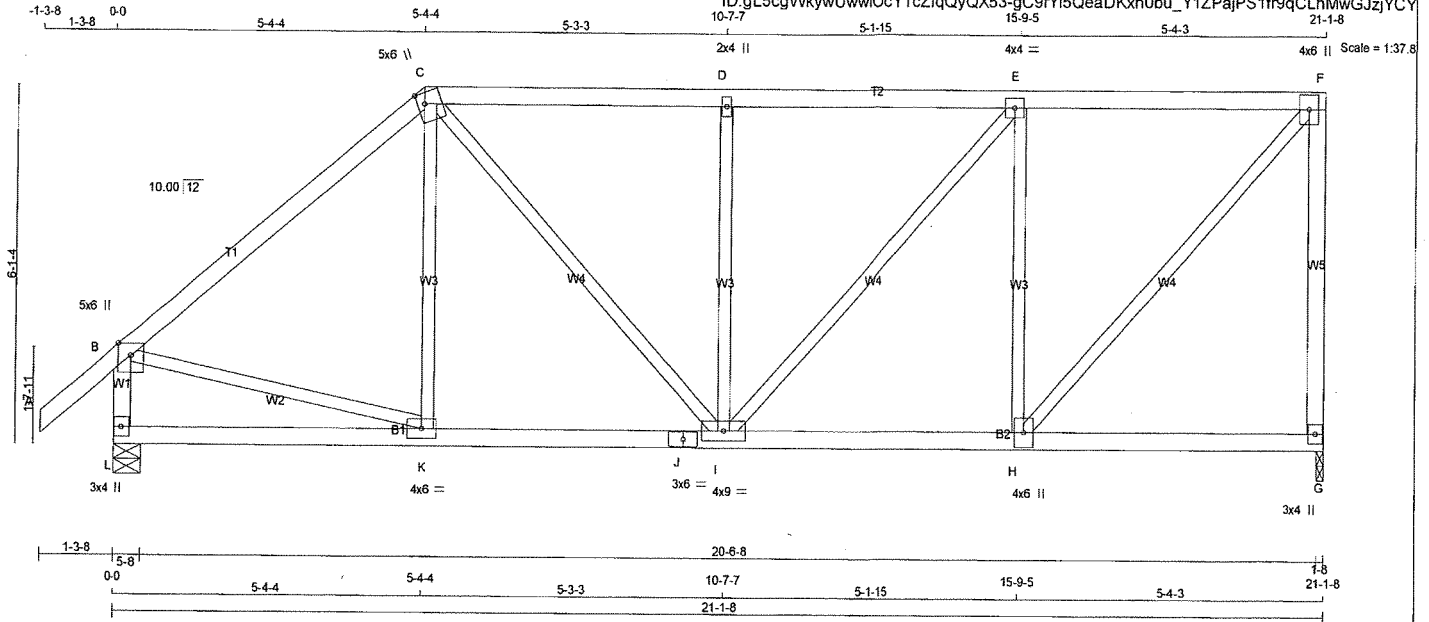


JOB NAME 401391	TRUSS NAME T6	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 19 15:40:59 2019 Page 1

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TOTAL WEIGHT = 4 X 96 = 383 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTVW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW+t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW+t	MT20	4.0	6.0		
I	BMVW+t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMVW+t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1231	0	1231	0
L	1339	0	1339	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
L	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)	MAX. VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 35	-78.0	-78.0 0.11 (1)	10.00	K-C	0 / 182	0.04 (3)
B-C	-1136 / 0	-78.0	-78.0 0.45 (1)	5.38	B-K	0 / 896	0.20 (1)
C-D	-1163 / 0	-78.0	-78.0 0.31 (1)	5.52	H-F	0 / 1330	0.30 (1)
D-E	-1163 / 0	-78.0	-78.0 0.32 (1)	5.50	C-I	0 / 433	0.10 (1)
E-F	-901 / 0	-78.0	-78.0 0.31 (1)	6.06	H-E	-747 / 0	0.44 (1)
G-F	-1148 / 0	0.0	0.0 0.83 (1)	7.42	I-D	-438 / 0	0.26 (1)
L-B	-1255 / 0	0.0	0.0 0.13 (1)	7.18	I-E	0 / 396	0.09 (1)
L-K	0 / 0	-38.5	-38.5 0.21 (3)	10.00			
K-J	0 / 872	-38.5	-38.5 0.33 (2)	10.00			
J-I	0 / 872	-38.5	-38.5 0.33 (2)	10.00			
I-H	0 / 901	-38.5	-38.5 0.32 (2)	10.00			
H-G	0 / 0	-38.5	-38.5 0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.07")

CSI: TC=0.83/1.00 (F-G:1), BC=0.33/1.00 (I-K:2),
WB=0.44/1.00 (E-H:1), SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

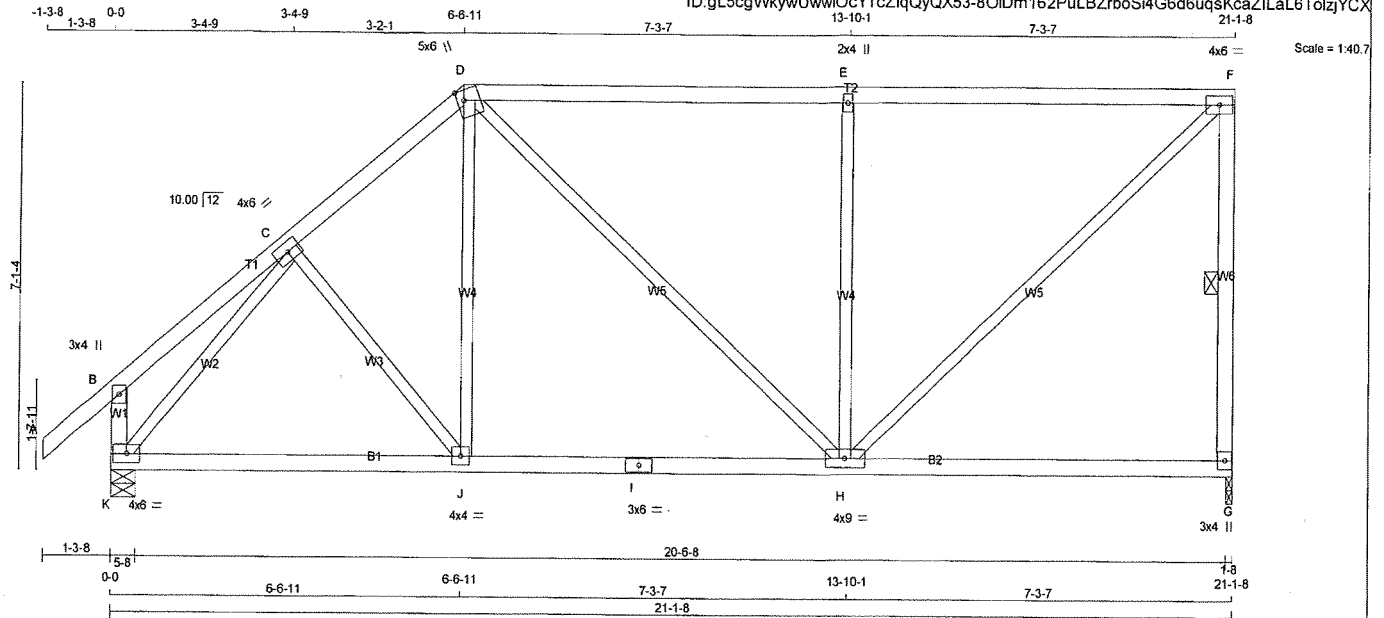


DWG NO. TAM 11903698
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T7	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 95 = 382 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0		
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-t	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BMVW-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
G	1231	0	1231	0
K	1339	0	1339	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
G	COMBINED	SNOW	LIVE	0/0	0/0	0/0
K	COMBINED	SNOW	LIVE	0/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO							
A-B	0/35	-78.0	-78.0	0.11 (1)	10.00	C-J	0/833
B-C	0/17	-78.0	-78.0	0.12 (1)	10.00	J-D	0/326
C-D	-1117/0	-78.0	-78.0	0.14 (1)	5.84	D-H	0/166
D-E	-968/0	-78.0	-78.0	0.78 (1)	4.91	H-E	-704/0
E-F	-968/0	-78.0	-78.0	0.78 (1)	4.91	H-F	0/1322
G-F	-1117/0	0.0	0.0	0.25 (1)	6.00	K-C	-1323/0
K-B	-208/0	0.0	0.0	0.02 (1)	7.81		
K-J	0/833	-38.5	-38.5	0.42 (2)	10.00		
J-I	0/847	-38.5	-38.5	0.48 (2)	10.00		
I-H	0/847	-38.5	-38.5	0.48 (2)	10.00		
H-G	0/0	-38.5	-38.5	0.37 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.11")
ALLOWABLE DEFL. (TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.19")

CSI: TC=0.78/1.00 (E-F:1), BC=0.48/1.00 (H-J:2),
WB=0.62/1.00 (E-H:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)

JSI METAL= 0.41 (I) (INPUT = 1.00)

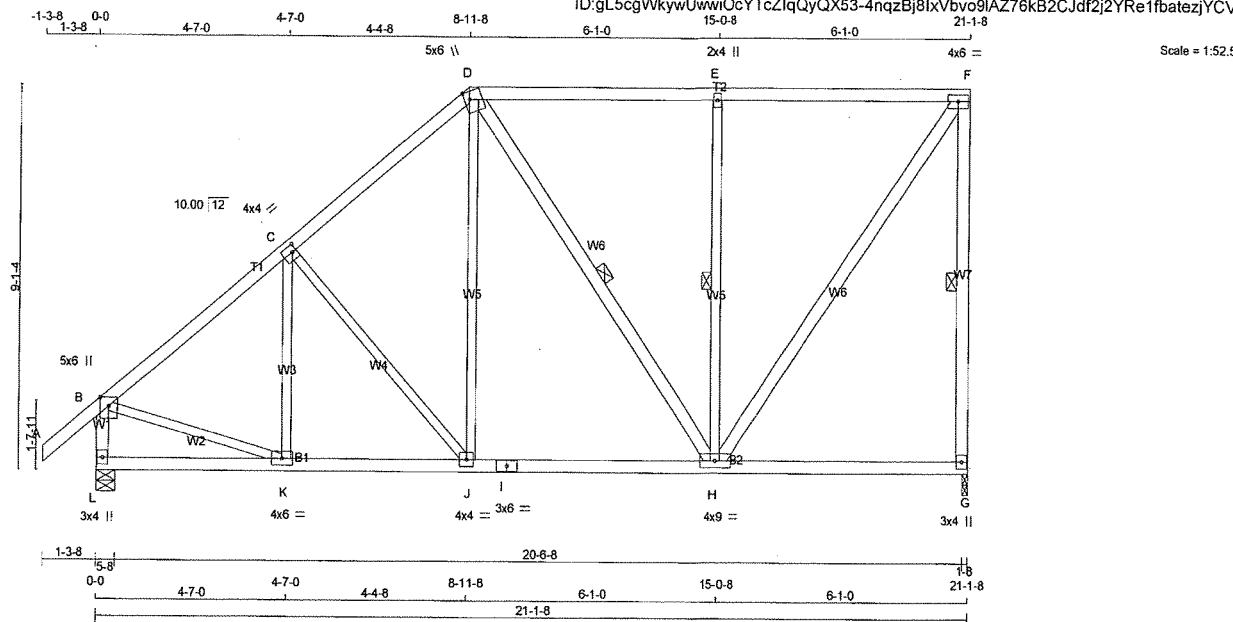


DWG NO. TAM T903699
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T9	10	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 10 X 115 = 1146 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
D - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
G	1231 0	1231 0	1-8
L	1339 0	1339 0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
G	940	444 / 0	222 / 0	0 / 0	275 / 0
L	1015	502 / 0	222 / 0	0 / 0	291 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-78.0	-78.0 0.11 (1)	10.00	K-C	-72 / 101	0.03 (1)
B-C	-1161 / 0	-78.0	-78.0 0.30 (1)	5.55	C-J	-280 / 0	0.24 (1)
C-D	-976 / 0	-78.0	-78.0 0.28 (1)	5.94	J-D	0 / 427	0.10 (2)
D-E	-684 / 0	-78.0	-78.0 0.51 (1)	6.25	D-H	-120 / 0	0.07 (1)
E-F	-684 / 0	-78.0	-78.0 0.51 (1)	6.25	H-E	-588 / 0	0.32 (1)
G-F	-1137 / 0	0.0	0.0 0.44 (1)	5.96	H-F	0 / 1164	0.19 (1)
L-B	-1266 / 0	0.0	0.0 0.13 (1)	7.16	B-K	0 / 948	0.21 (1)
L-K	0 / 0	-38.5	-38.5 0.14 (3)	10.00			
K-J	0 / 912	-38.5	-38.5 0.26 (2)	10.00			
J-I	0 / 732	-38.5	-38.5 0.37 (2)	10.00			
I-H	0 / 732	-38.5	-38.5 0.37 (2)	10.00			
H-G	0 / 0	-38.5	-38.5 0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.51/1.00 (E-F:1), BC=0.37/1.00 (H-J:2), WB=0.32/1.00 (E-H:1), SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

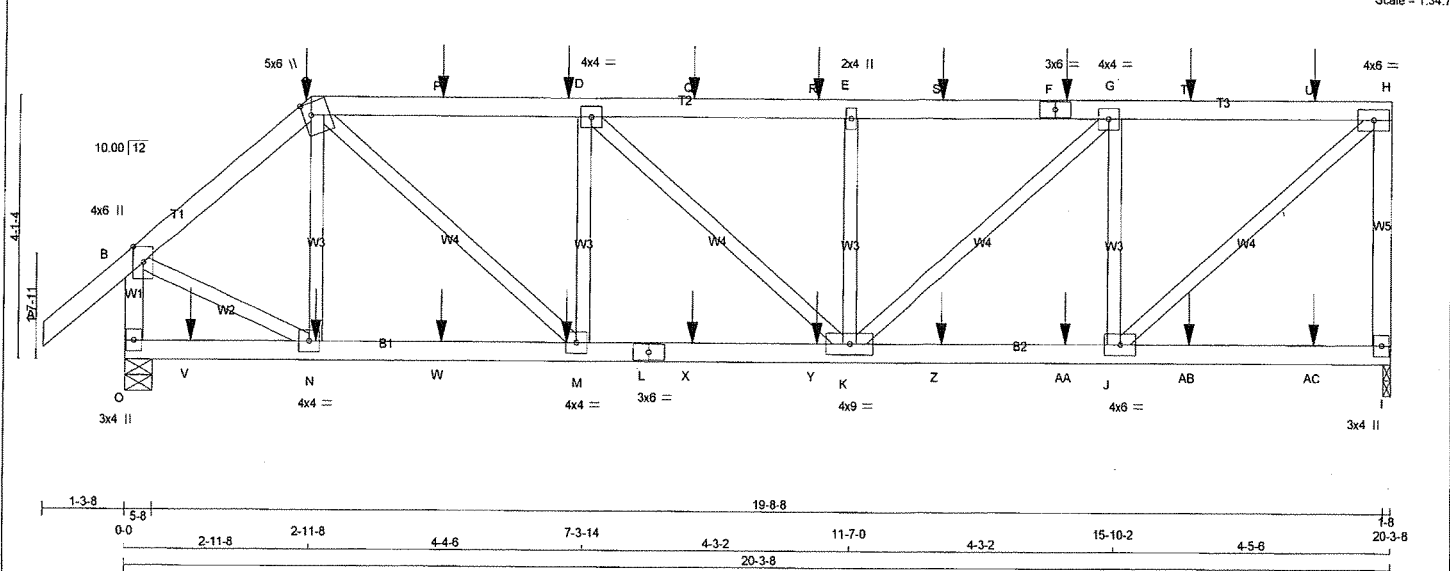


DWG NO. TAM 71903701
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T13	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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LUMBER			
N L G A RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
I - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - F	12	SIDE(61.0)
F - H	12	SIDE(0.0)
H - I	12	TOP
O - B	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - L	12	SIDE(61.0)
L - I	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMWw+p	MT20	4.0	6.0 Edge
C	TTWw+m	MT20	5.0	6.0 2.25 1.50
D	TMWw-t	MT20	4.0	4.0
E	TS-t	MT20	2.0	4.0
F	TS-t	MT20	3.0	6.0
G	TMWw-t	MT20	4.0	4.0
H	TMWw-t	MT20	4.0	6.0
I	BMV1+p	MT20	3.0	4.0
J	BMWw-t	MT20	4.0	6.0
K	BMWw-t	MT20	4.0	9.0
L	BS-t	MT20	3.0	6.0
M	BMWw-t	MT20	4.0	4.0
N	BMWw-t	MT20	4.0	4.0
O	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	BRG	IN-SX	BRG
I	1948	0	0	1948	0	0	1-8	1-8	5-8
O	2142	0	0	2142	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE
I	1493	692 / 0	362 / 0
O	1628	794 / 0	364 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	CS (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	CS (LC)	MAX. UNBRAC	MEMB.
FR-TO					FR-TO				
A-B	0 / 35	-78.0	-78.0 0.06 (1)	10.00	N-C	-353 / 3	0.05 (1)		
B-C	-1907 / 0	-78.0	-78.0 0.08 (1)	6.25	B-N	0 / 1587	0.20 (1)		
C-P	-2559 / 0	-78.0	-78.0 0.19 (1)	5.46	J-H	0 / 2492	0.09 (1)		
P-D	-2559 / 0	-78.0	-78.0 0.19 (1)	5.46	C-M	0 / 469	0.18 (1)		
D-Q	-2655 / 0	-78.0	-78.0 0.19 (1)	5.38	J-G	-1288 / 0	0.13 (1)		
Q-R	-2655 / 0	-78.0	-78.0 0.19 (1)	5.38	M-D	-678 / 0	0.03 (1)		
R-E	-2655 / 0	-78.0	-78.0 0.19 (1)	5.38	K-G	-50 / 1045	0.12 (1)		
E-S	-2655 / 0	-78.0	-78.0 0.22 (1)	5.34	D-O	-60 / 330	0.02 (2)		
S-F	-2655 / 0	-78.0	-78.0 0.22 (1)	5.34	K-E	-499 / 0	0.02 (1)		
F-G	-2655 / 0	-78.0	-78.0 0.22 (1)	5.34					
G-T	-1896 / 0	-78.0	-78.0 0.21 (1)	6.09					
T-U	-1896 / 0	-78.0	-78.0 0.21 (1)	6.09					
U-H	-1896 / 0	-78.0	-78.0 0.21 (1)	6.09					
H-I	-1836 / 0	0.0	0.0 0.25 (1)	7.81					
O-B	-2073 / 0	0.0	0.0 0.12 (1)	7.68					
O-V	0 / 0	-38.5	-38.5 0.08 (3)	10.00					
V-N	0 / 0	-38.5	-38.5 0.08 (3)	10.00					
N-W	0 / 1450	-38.5	-38.5 0.20 (2)	10.00					
W-M	0 / 1450	-38.5	-38.5 0.20 (2)	10.00					
M-L	0 / 2559	-38.5	-38.5 0.28 (2)	10.00					
L-X	0 / 2559	-38.5	-38.5 0.28 (2)	10.00					
X-Y	0 / 2559	-38.5	-38.5 0.28 (2)	10.00					
Y-K	0 / 2559	-38.5	-38.5 0.28 (2)	10.00					
K-Z	0 / 1896	-38.5	-38.5 0.27 (2)	10.00					
Z-AA	0 / 1896	-38.5	-38.5 0.27 (2)	10.00					
AA-J	0 / 1896	-38.5	-38.5 0.27 (2)	10.00					
J-AB	0 / 0	-38.5	-38.5 0.13 (3)	10.00					
AB-AC	0 / 0	-38.5	-38.5 0.13 (3)	10.00					
AC-I	0 / 0	-38.5	-38.5 0.13 (3)	10.00					

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX.	FRONT	VERT	DEAD	TYPE	HEEL	CONN.	HEEL	CONN.
C	2-11-8	-38	-42	---	---	---	---	---	---	---	---
C	2-11-8	-101	-101	---	---	---	---	---	---	---	---
C	2-11-8	-180	-180	---	---	---	---	---	---	---	---
D	7-0-12	-94	-94	---	---	---	---	---	---	---	---
F	15-0-12	-94	-94	---	---	---	---	---	---	---	---
M	7-0-12	-55	-70	---	---	---	---	---	---	---	---
N	3-0-12	-55	-70	---	---	---	---	---	---	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.68")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.68")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

SSI=0.25/1.00 (H-I), BC=0.28/1.00 (K-M-2),
SSI=0.31/1.00 (H-J-1), SSI=0.14/1.00 (G-H-1)

DOJ LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

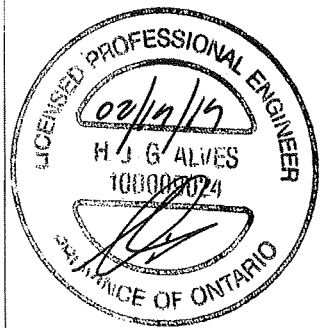
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (N) (INPUT = 0.90)

JSI METAL= 0.43 (B) (INPUT = 1.00)

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	5-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
P	9-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
Q	11-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
R	13-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
S	17-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
T	19-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
U	1-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
V	5-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
W	9-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
X	11-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	13-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	15-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	17-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	19-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC									



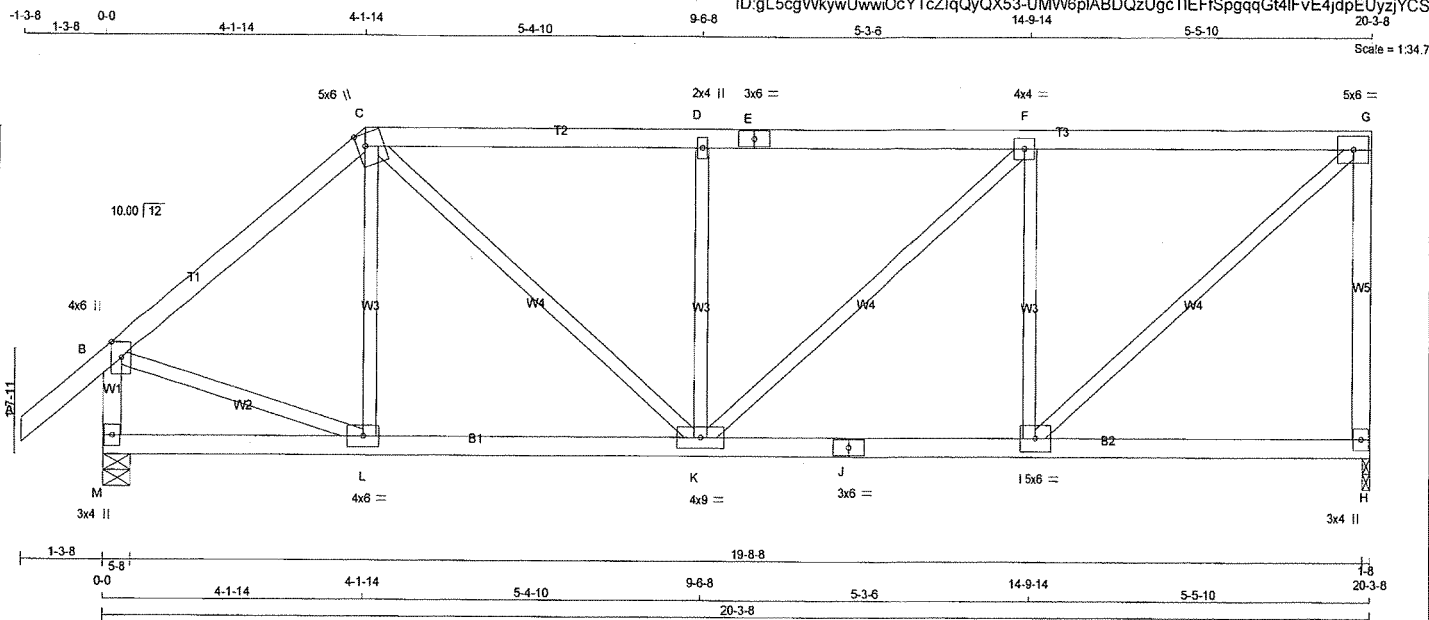
DWG NO. TAM *P903703*
 STRUCTURAL
 COMPONENT ONLY *2/2*

JOB NAME 401391	TRUSS NAME T14	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: gL5cgWkywUwwiOcYtCZlqQyQX53-UMW6pIABDQzUgcTIEffSpqqGt4IFvE4jdpEUyzjYCS



TOTAL WEIGHT = 2 X 87 = 175 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
M - B	2x4	DRY	No.2
M - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	1.50
C	TTWW+m	MT20	5.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWV-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWV-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWVW-t	MT20	4.0	9.0		
L	BMWV-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
H	1182	0	1-8	1-8
M	1290	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	903	426 / 0	213 / 0	0 / 0	0 / 0	0 / 0	264 / 0	0 / 0
M	978	484 / 0	213 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO								
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	L-C	-49 / 118	0.03 (3)
B-C	-1093 / 0	-78.0	-78.0	0.26 (1)	5.73	B-L	0 / 876	0.20 (1)
C-D	-1300 / 0	-78.0	-78.0	0.33 (1)	5.26	I-G	0 / 1399	0.31 (1)
D-E	-1300 / 0	-78.0	-78.0	0.35 (1)	5.24	C-K	0 / 621	0.14 (1)
E-F	-1300 / 0	-78.0	-78.0	0.35 (1)	5.24	I-F	-889 / 0	0.26 (1)
F-G	-1050 / 0	-78.0	-78.0	0.33 (1)	5.69	K-D	-448 / 0	0.17 (1)
H-G	-1097 / 0	0.0	0.0	0.48 (1)	7.55	K-F	0 / 338	0.08 (1)
M-B	-1229 / 0	0.0	0.0	0.13 (1)	7.23			
M-L	0 / 0	-38.5	-38.5	0.16 (3)	10.00			
L-K	0 / 836	-38.5	-38.5	0.28 (2)	10.00			
K-J	0 / 1050	-38.5	-38.5	0.35 (2)	10.00			
J-I	0 / 1050	-38.5	-38.5	0.35 (2)	10.00			
I-H	0 / 0	-38.5	-38.5	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.07")

CSI: $TC=0.48/1.00$ (G-H:1), $BC=0.35/1.00$ (I-K:2),
 $WB=0.31/1.00$ (G-I:1), $SSI=0.20/1.00$ (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

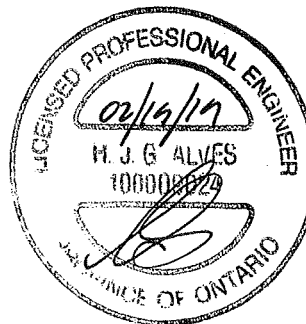
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

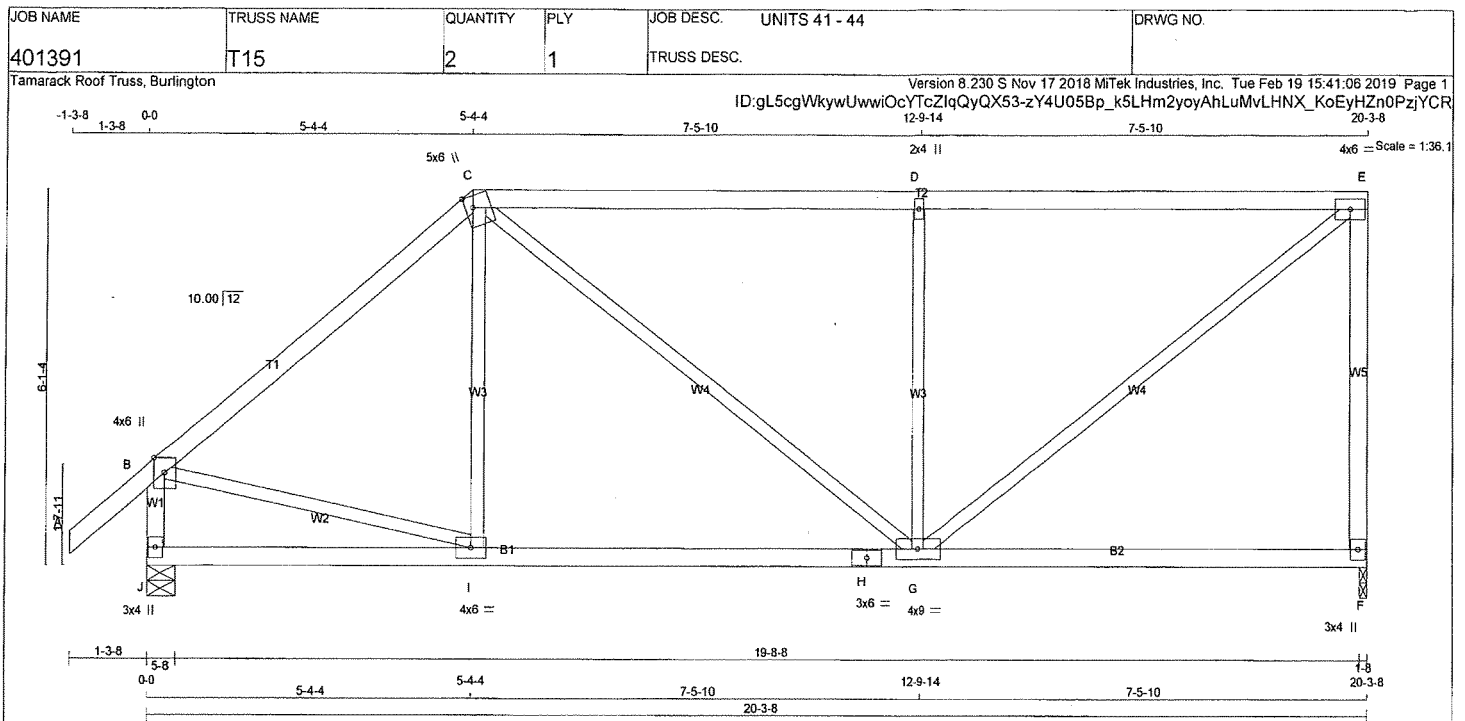
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)



DWG NO. TAM 17503704
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No. 2
C - E	2x4	DRY	No. 2
F - E	2x4	DRY	No. 2
J - B	2x4	DRY	No. 2
J - H	2x4	DRY	No. 2
H - F	2x4	DRY	No. 2

ALL WEBS 2x3 DRY No. 2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMVWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	GROSS REACTION	DOWN	GROSS REACTION	UP	BRG	IN-SX	BRG	IN-SX
F	1182	0	1182	0	0	1-8	1-8		
J	1290	0	1290	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
J	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 35	-78.0	-78.0 0.11 (1)	10.00	I-C	0 / 219	0.05 (3)
B-C	-1083 / 0	-78.0	-78.0 0.45 (1)	5.48	C-G	0 / 329	0.07 (1)
C-D	-1091 / 0	-78.0	-78.0 0.84 (1)	4.54	G-D	-721 / 0	0.42 (1)
D-E	-1091 / 0	-78.0	-78.0 0.85 (1)	4.54	G-E	0 / 1379	0.31 (1)
F-E	-1067 / 0	0.0	0.0 0.77 (1)	7.63	B-I	0 / 654	0.19 (1)
J-B	-1211 / 0	0.0	0.0 0.13 (1)	7.28			
J-I	0 / 0	-38.5	-38.5 0.26 (3)	10.00			
I-H	0 / 832	-38.5	-38.5 0.51 (2)	10.00			
H-G	0 / 832	-38.5	-38.5 0.51 (2)	10.00			
G-F	0 / 0	-38.5	-38.5 0.41 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.85/1.00 (D-E:1), BC=0.51/1.00 (G:2),
WB=0.42/1.00 (D-G:1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.57 (H) (INPUT = 1.00)



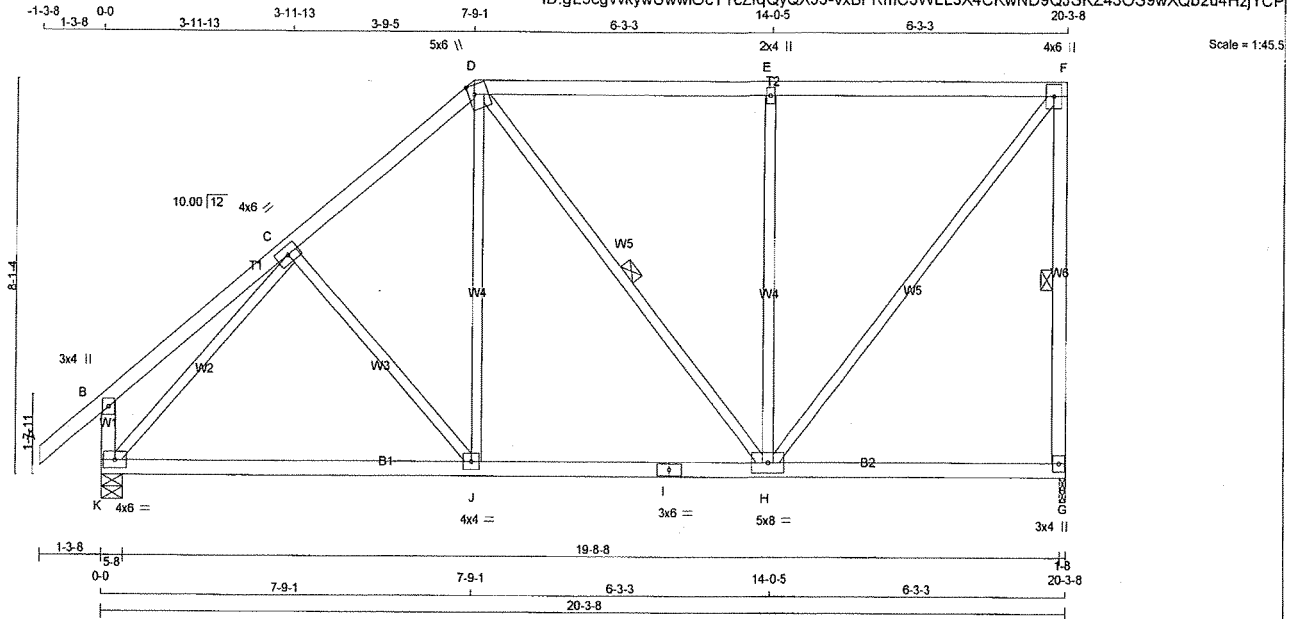
DRWG NO. TAM 17403705
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T17	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 98 = 196 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY

D - F 2x4 DRY

G - F 2x4 DRY

K - B 2x4 DRY

K - I 2x4 DRY

I - G 2x4 DRY

LUMBER

No.2

No.2

No.2

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

ALL WEBS 2x3 DRY

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES

B TMV+p MT20

C TMWW-t MT20

D TTWW+m MT20

E TMW+w MT20

F TMVW+p MT20

G BMV1+p MT20

H BMVWW-t MT20

I BS-t MT20

J BMVW-t MT20

K BMVW1-t MT20

W LEN Y X

3.0 4.0

4.0 6.0

5.0 6.0 2.25 1.50

2.0 4.0

4.0 6.0

3.0 4.0

5.0 8.0

3.0 6.0

4.0 4.0

4.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
G	1182	0	1182	0	0	1-8	1-8	5-8
K	1290	0	1290	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	284 / 0	0 / 0
K	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		
A-B	0 / 35	-78.0 -78.0 0.11 (1)	10.00	C-J -79 / 49 0.05 (1)
B-C	0 / 22	-78.0 -78.0 0.18 (1)	10.00	J-D -0 / 413 0.09 (2)
C-D	-1002 / 0	-78.0 -78.0 0.20 (1)	5.99	D-H -72 / 0 0.05 (3)
D-E	-721 / 0	-78.0 -78.0 0.54 (1)	6.10	H-E -605 / 0 0.77 (1)
E-F	-722 / 0	-78.0 -78.0 0.54 (1)	6.10	H-F -0 / 1148 0.26 (1)
G-F	-1081 / 0	0.0 0.0 0.32 (1)	6.08	K-C -1238 / 0 0.73 (1)
K-B	-224 / 0	0.0 0.0 0.02 (1)	7.81	
K-J	0 / 803	-38.5 -38.5 0.48 (2)	10.00	
J-I	0 / 756	-38.5 -38.5 0.48 (2)	10.00	
I-H	0 / 756	-38.5 -38.5 0.48 (2)	10.00	
H-G	0 / 0	-38.5 -38.5 0.24 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (TL) = $L/960$ (0.25")

CSI: TC=0.54/1.00 (E-F:1), BC=0.48/1.00 (J-K:2), WB=0.77/1.00 (E-H:1), SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

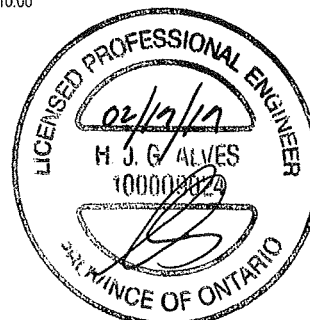
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)



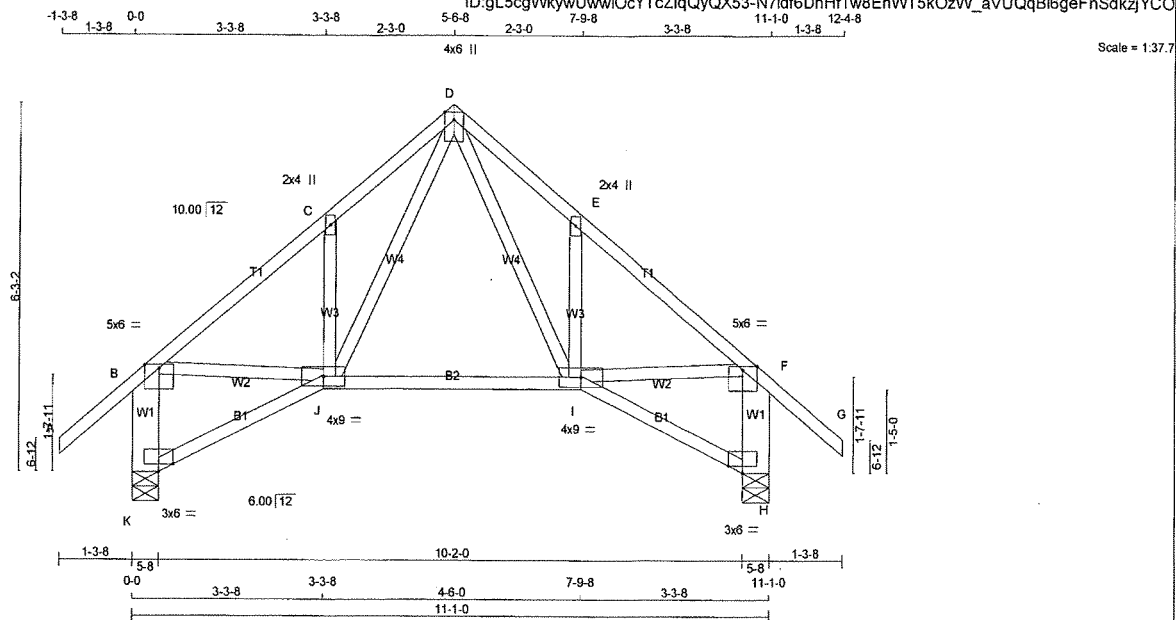
DWG NO. TAM 7703707
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T54	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:41:09 2019 Page 1

ID: gL5cgWkywUwwiOcYTcZlqQyQX53-N7Idf6DhHfTw8EnWT5kOzW_aVUQqBl6geFnSdKzYCO



TOTAL WEIGHT = 4 X 47 = 187 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x3	DRY	No.2
D - G	2x3	DRY	No.2
K - B	2x6	DRY	No.2
H - F	2x6	DRY	No.2
K - J	2x3	DRY	No.2
J - I	2x3	DRY	No.2
I - H	2x3	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BVM1-p	MT20	3.0	6.0	Edge	3.00
I	BBWWW-p	MT20	4.0	9.0		
J	BBWWW-p	MT20	4.0	9.0		
K	BVM1-p	MT20	3.0	6.0	Edge	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
K	752	0	752	0
H	752	0	752	0

UNFACTORED REACTIONS

1ST LOASE	MAX /MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	SOIL
K	567	290 / 0	116 / 0	0 / 0	0 / 0	160 / 0
H	567	290 / 0	116 / 0	0 / 0	0 / 0	160 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	UNBRAC	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT.	(PLF)	CSI	(LC)	LENGTH	FR-TO	MEMB.	FORCE
FR-TO	(LBS)	FROM	TO						(LBS)
A-B	0 / 34	-78.0	-78.0	0.20	(1)	10.00	D-I	0 / 478	0.11
B-C	-734 / 0	-78.0	-78.0	0.21	(1)	5.95	I-E	-276 / 0	0.05
C-D	-775 / 0	-78.0	-78.0	0.17	(1)	5.88	J-D	0 / 478	0.11
D-E	-775 / 0	-78.0	-78.0	0.17	(1)	5.88	J-C	-276 / 0	0.05
E-F	-734 / 0	-78.0	-78.0	0.21	(1)	5.95	B-J	0 / 575	0.13
F-G	0 / 34	-78.0	-78.0	0.20	(1)	10.00	I-F	0 / 575	0.13
K-B	-688 / 0	0.0	0.0	0.05	(1)	7.81			
H-F	-688 / 0	0.0	0.0	0.05	(1)	7.81			
K-J	0 / 0	-38.5	-38.5	0.19	(3)	10.00			
J-I	0 / 368	-38.5	-38.5	0.40	(2)	10.00			
I-H	0 / 0	-38.5	-38.5	0.19	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	44.5	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (TL) = $L/939$ (0.14")

CSI: TC=0.21/1.00 (E-F-1), BC=0.40/1.00 (I-J-2),
WB=0.13/1.00 (F-I-1), SSI=0.13/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

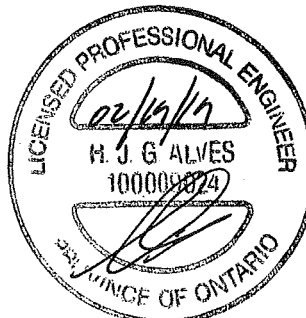
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (J) (INPUT = 0.90)
JSI METAL= 0.20 (F) (INPUT = 1.00)



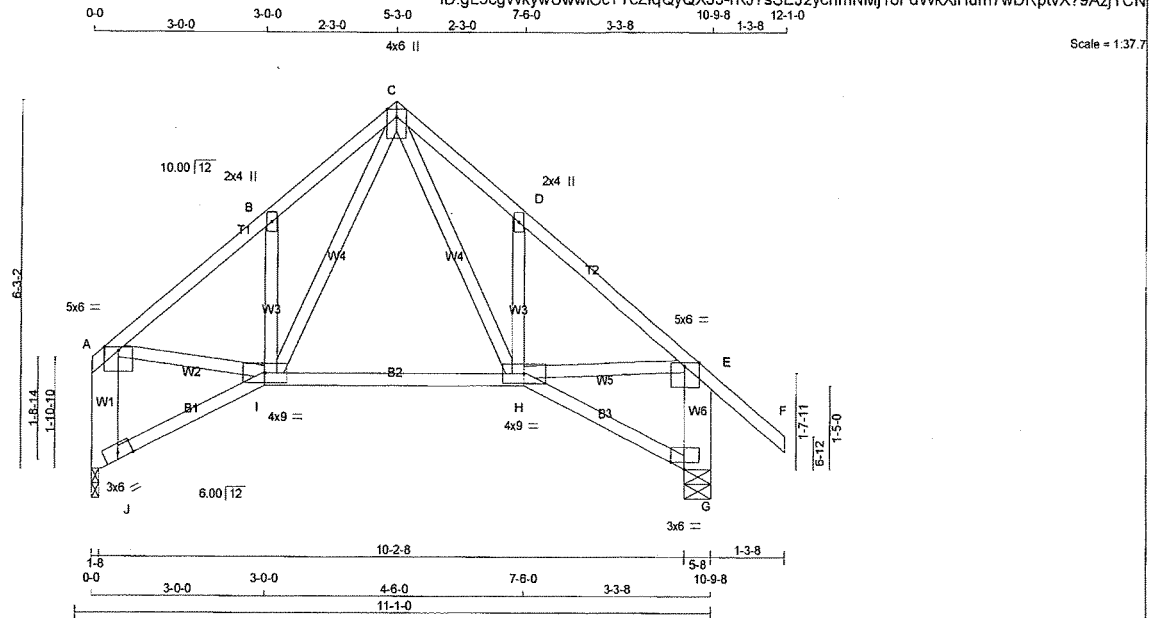
DWG NO. TAM 71903708
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T54A	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:41:10 2019 Page 1

ID: gL5cgWkywUwwiOcYTcZlqQyQX53-rKJ?SEJ2ycnmNMj1oFdWkXlHum7wDRptvX?9AzjYCN



TOTAL WEIGHT = 2 X 45 = 90 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x3 DRY No.2

C - F 2x3 DRY No.2

J - A 2x6 DRY No.2

G - E 2x6 DRY No.2

J - I 2x3 DRY No.2

I - H 2x3 DRY No.2

H - G 2x3 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-w	MT20	2.0	4.0		
C	TTVVW+p	MT20	4.0	6.0	Edge	
D	TMVW-w	MT20	2.0	4.0		
E	TMVW-p	MT20	5.0	6.0	Edge	
G	BVM1-p	MT20	3.0	6.0	Edge	3.00
H	BBVVWV-p	MT20	4.0	9.0		
I	BBVVWV-p	MT20	4.0	9.0		
J	BVM1-I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ
JT	629	0	629	0
J	629	0	629	0
G	735	0	735	0

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
JT	480	227 / 0	113 / 0	0 / 0
J	480	227 / 0	113 / 0	0 / 0
G	554	284 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	-666 / 0	-78.0	-78.0	0.17 (1)
B-C	-703 / 0	-78.0	-78.0	0.14 (1)
C-D	-748 / 0	-78.0	-78.0	0.17 (1)
D-E	-707 / 0	-78.0	-78.0	0.21 (1)
E-F	0 / 34	-78.0	-78.0	0.20 (1)
J-A	-571 / 0	0.0	0.0	0.05 (1)
J-E	-671 / 0	0.0	0.0	0.05 (1)
J-I	0 / 0	-38.5	-38.5	0.16 (3)
I-H	0 / 348	-38.5	-38.5	0.40 (2)
H-G	0 / 0	-38.5	-38.5	0.19 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/932 (0.14")

CSI: TC=0.21/1.00 (D-E:1), BC=0.40/1.00 (H-I:2), WB=0.12/1.00 (E-H:1), SSI=0.13/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (H) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 1.00)



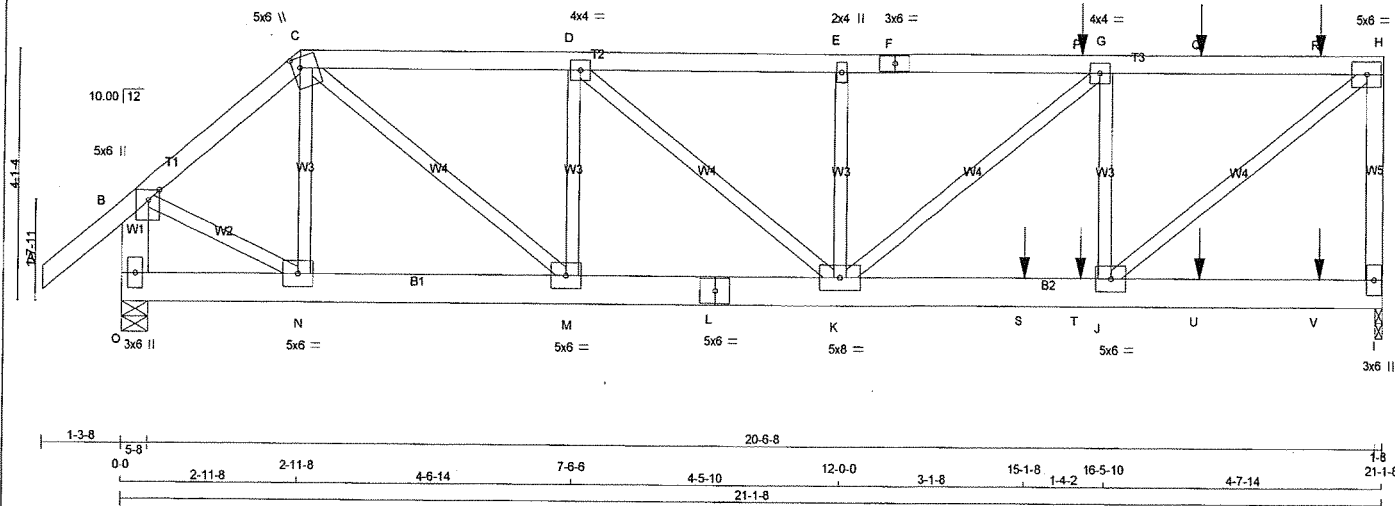
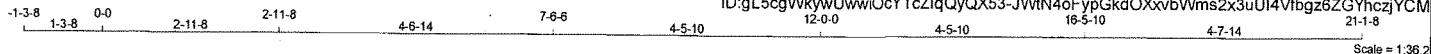
DWG NO. TAM 71903709
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T70	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: gL5cgWkywUwwiOcyTcZlqQyQX53-JWtN4oFypGkdOXxbVwms2x3uU14Vfbgz6ZGYhcZjYCM



TOTAL WEIGHT = 4 X 102 = 408 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - H	12	SIDE(0.0)
H - I	12	TOP
O - B	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
O - L	12	TOP
L - I	12	SIDE(0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWw+p	MT20	5.0	6.0	2.00 2.25
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMWw-t	MT20	4.0	4.0	
E	TMWw+w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMWw-t	MT20	4.0	4.0	
H	TMWw-t	MT20	5.0	6.0	
I	BMV1+p	MT20	3.0	6.0	
J, M, N					
K	BMWw-t	MT20	5.0	6.0	
L	BS-t	MT20	5.0	6.0	
O	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
I	2539	0	2539	0
O	1771	0	1771	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	1929	942 / 0	431 / 0	0 / 0	0 / 0	556 / 0	0 / 0
O	1340	670 / 0	287 / 0	0 / 0	0 / 0	383 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO			
A-B	0 / 35	-78.0	-78.0	0.06 (1)	N-C	-281 / 0	0.04 (1)	
B-C	-1601 / 0	-78.0	-78.0	0.07 (1)	B-N	0 / 1319	0.16 (1)	
C-D	-2501 / 0	-78.0	-78.0	0.15 (1)	J-H	0 / 3596	0.45 (1)	
D-E	-3194 / 0	-78.0	-78.0	0.17 (1)	C-M	0 / 1652	0.20 (1)	
E-F	-3194 / 0	-78.0	-78.0	0.22 (1)	J-G	-932 / 0	0.12 (1)	
F-P	-3194 / 0	-78.0	-78.0	0.22 (1)	M-D	-974 / 0	0.12 (1)	
P-G	-3194 / 0	-78.0	-78.0	0.22 (1)	K-G	0 / 505	0.06 (1)	
G-Q	-2807 / 0	-78.0	-78.0	0.22 (1)	D-K	0 / 903	0.11 (1)	
Q-R	-2807 / 0	-78.0	-78.0	0.22 (1)	K-F	-303 / 0	0.04 (1)	
R-H	-2807 / 0	-78.0	-78.0	0.22 (1)				
I-H	-2484 / 0	0.0	0.0	0.31 (1)				
O-B	-1743 / 0	0.0	0.0	0.06 (1)				
O-N	0 / 0	-38.5	-38.5	0.04 (2)				
N-M	0 / 1217	-38.5	-38.5	0.11 (1)				
M-L	0 / 2501	-38.5	-38.5	0.26 (1)				
L-K	0 / 2501	-38.5	-38.5	0.26 (1)				
K-S	0 / 2807	-38.5	-38.5	0.52 (1)				
S-T	0 / 2807	-38.5	-38.5	0.52 (1)				
T-J	0 / 2807	-38.5	-38.5	0.52 (1)				
J-U	0 / 0	-38.5	-38.5	0.17 (1)				
U-V	0 / 0	-38.5	-38.5	0.17 (1)				
V-I	0 / 0	-38.5	-38.5	0.17 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
P	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---
Q	18-0-12	-94	-94	---	BACK	VERT	TOTAL	---
R	20-0-12	-94	-94	---	BACK	VERT	TOTAL	---
S	15-1-8	-1294	-1294	---	BACK	VERT	TOTAL	---
T	16-0-12	-55	-70	---	BACK	VERT	TOTAL	---
U	18-0-12	-55	-70	---	BACK	VERT	TOTAL	---
V	20-0-12	-55	-70	---	BACK	VERT	TOTAL	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	44.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.11")

CSI: TC=0.31/1.00 (H-I:1), BC=0.52/1.00 (J-K:1), WB=0.45/1.00 (H-J:1), SS=0.36/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.38 (J) (INPUT = 1.00)

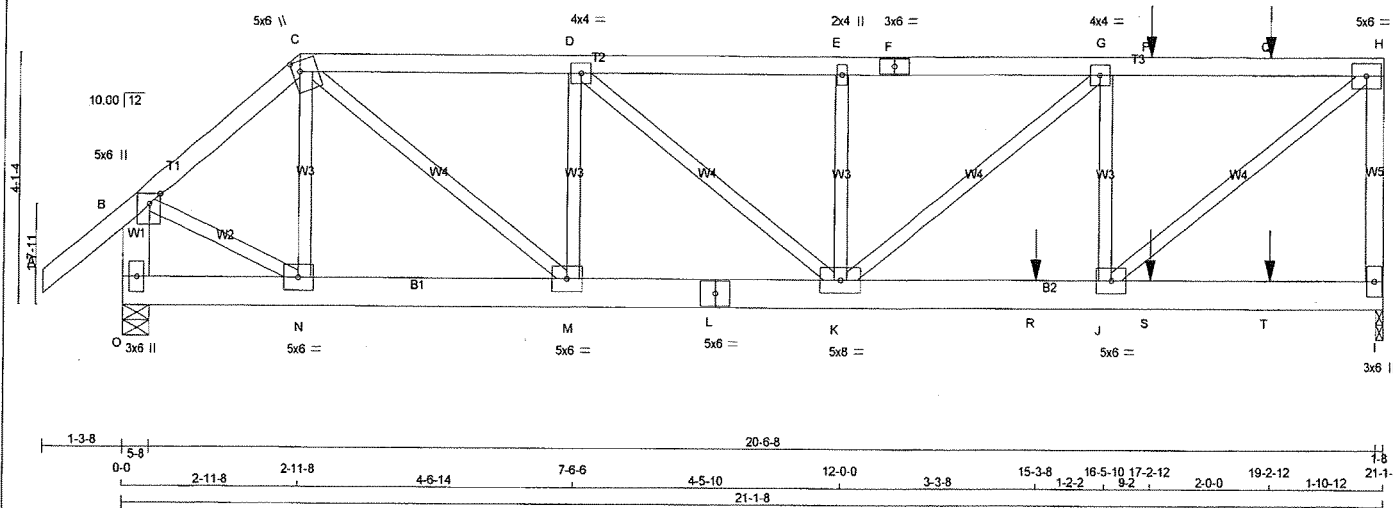
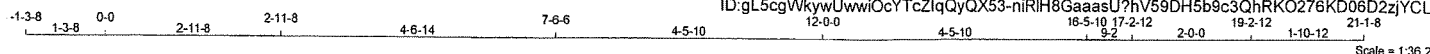
DWG NO. TAM 77203710
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T70Z	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 102 = 408 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - B	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	
L - I	2x6	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - F	12	TOP
F - H	12	SIDE(0.0)
H - I	12	TOP
O - B	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O - L	12	TOP
L - I	12	SIDE(0.0)

WEBS : (0.122"x3") SPIRAL NAILS

2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	5.0	6.0	2.00	2.25
C TTWV+m	MT20	5.0	6.0	2.00	1.50
D TMWV-t	MT20	4.0	4.0		
E TMWV+w	MT20	2.0	4.0		
F TS-t	MT20	3.0	6.0		
G TMWV-t	MT20	4.0	4.0		
H TMWV-t	MT20	5.0	6.0		
I BMV1+p	MT20	3.0	6.0		
J, M, N					
K BMWV-t	MT20	5.0	6.0		
L BMWV-t	MT20	5.0	8.0		
L BS-t	MT20	5.0	6.0		
O BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
I	2429	0	0	2429	0	0	1-8	1-8	
O	1738	0	0	1738	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1849	891 / 0	421 / 0	0 / 0	0 / 0	536 / 0	0 / 0
O	1317	653 / 0	286 / 0	0 / 0	0 / 0	378 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.06 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 35	-78.0 -78.0	0.06 (1)	10.00	N-C	-274 / 0	0.03 (1)
B-C	-1566 / 0	-78.0 -78.0	0.07 (1)	6.25	B-N	0 / 1290	0.16 (1)
C-D	-2438 / 0	-78.0 -78.0	0.15 (1)	5.62	J-H	0 / 3491	0.43 (1)
D-E	-3079 / 0	-78.0 -78.0	0.17 (1)	5.12	C-M	0 / 1606	0.20 (1)
E-F	-3079 / 0	-78.0 -78.0	0.21 (1)	5.06	J-G	-847 / 0	0.11 (1)
F-G	-3079 / 0	-78.0 -78.0	0.21 (1)	5.06	M-D	-932 / 0	0.12 (1)
G-P	-2725 / 0	-78.0 -78.0	0.23 (1)	5.29	K-G	0 / 451	0.08 (1)
P-Q	-2725 / 0	-78.0 -78.0	0.23 (1)	5.29	D-K	0 / 1387	0.04 (1)
Q-H	-2725 / 0	-78.0 -78.0	0.23 (1)	5.29			
I-H	-2388 / 0	0.0 0.0	0.30 (1)	7.28			
O-B	-1709 / 0	0.0 0.0	0.06 (1)	7.81			
O-N	0 / 0	-38.5 -38.5	0.04 (2)	10.00			
N-M	0 / 1191	-38.5 -38.5	0.11 (1)	10.00			
M-L	0 / 2438	-38.5 -38.5	0.25 (1)	10.00			
L-K	0 / 2438	-38.5 -38.5	0.25 (1)	10.00			
K-R	0 / 2725	-38.5 -38.5	0.48 (1)	10.00			
R-J	0 / 2725	-38.5 -38.5	0.48 (1)	10.00			
J-S	0 / 0	-38.5 -38.5	0.16 (1)	10.00			
S-T	0 / 0	-38.5 -38.5	0.16 (1)	10.00			
T-I	0 / 0	-38.5 -38.5	0.16 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TOTAL	TYPE	HEEL	CORN.
P	17-2-12	-94	-94	---	FRONT	VERT	TOTAL	---	---	---
Q	19-2-12	-94	-94	---	FRONT	VERT	TOTAL	---	---	---
R	15-3-8	-1300	-1300	---	FRONT	VERT	TOTAL	---	---	---
S	17-2-12	-55	-70	---	FRONT	VERT	TOTAL	---	---	---
T	19-2-12	-55	-70	---	FRONT	VERT	TOTAL	---	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.10")

CSI: TC=0.30/1.00 (H-I), BC=0.48/1.00 (J-K-1), WB=0.43/1.00 (H-J-1), SSI=0.36/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

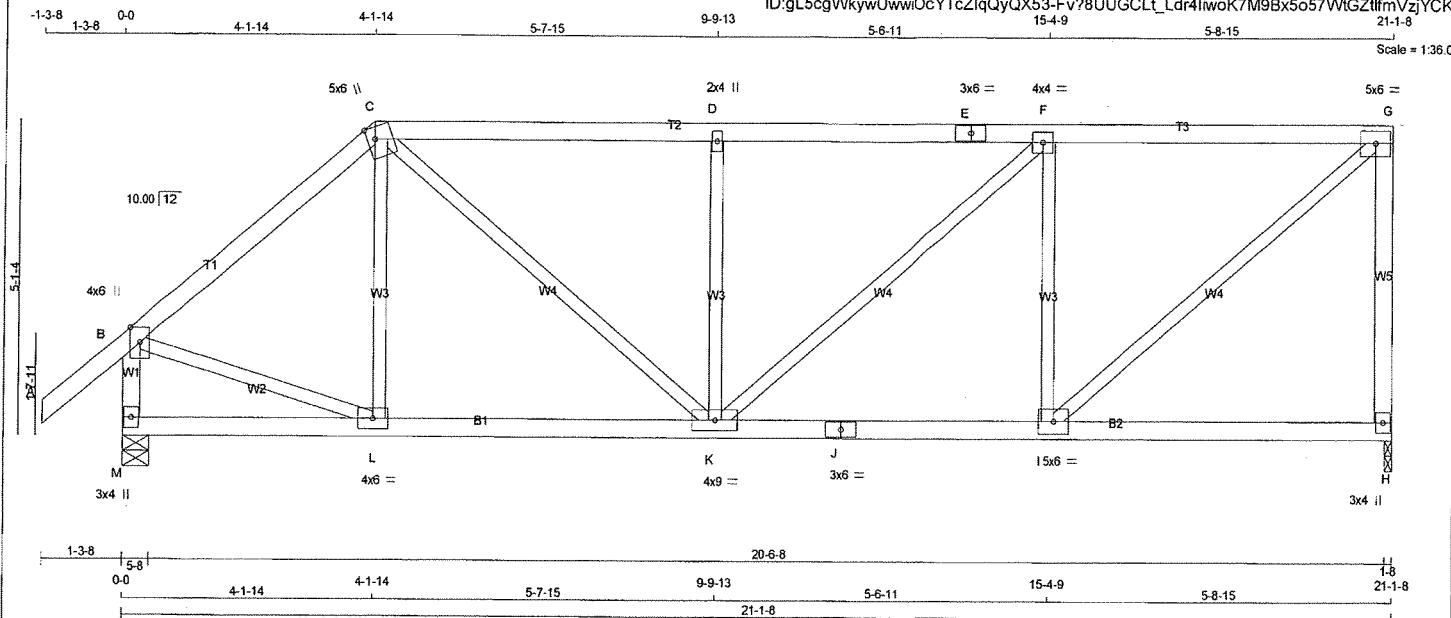
JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.36 (J) (INPUT = 1.00)

DWG NO. TAM 17003711
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T71	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 90 = 359 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	No.2	SPF
H - G	2x4 DRY	No.2	SPF
M - B	2x4 DRY	No.2	SPF
M - J	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	1231	0	1231	0	1-8	1-8
M	1339	0	1339	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	940	444 / 0	222 / 0	0 / 0	0 / 0	275 / 0	0 / 0
M	1015	502 / 0	222 / 0	0 / 0	0 / 0	291 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				
A-B	0 / 35	-78.0	-78.0	0.11 (1)
B-C	-1150 / 0	-78.0	-78.0	0.26 (1)
C-D	-1409 / 0	-78.0	-78.0	0.38 (1)
D-E	-1409 / 0	-78.0	-78.0	0.39 (1)
E-F	-1409 / 0	-78.0	-78.0	0.39 (1)
F-G	-1146 / 0	-78.0	-78.0	0.38 (1)
H-G	-1141 / 0	0.0	0.0	0.50 (1)
M-B	-1279 / 0	0.0	0.0	0.14 (1)
M-L	0 / 0	-38.5	-38.5	0.17 (3)
L-K	0 / 880	-38.5	-38.5	0.29 (2)
K-J	0 / 1146	-38.5	-38.5	0.38 (2)
J-I	0 / 1146	-38.5	-38.5	0.38 (2)
I-H	0 / 0	-38.5	-38.5	0.23 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.70")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CSI: TC=0.50/1.00 (G-H-1), BC=0.38/1.00 (H-K-2),
WB=0.34/1.00 (G-I-1), SSI=0.21/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

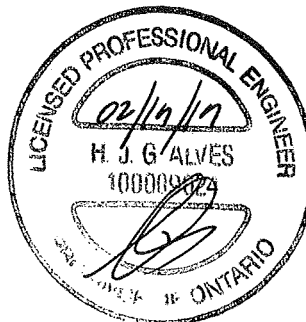
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.55 (B) (INPUT = 1.00)

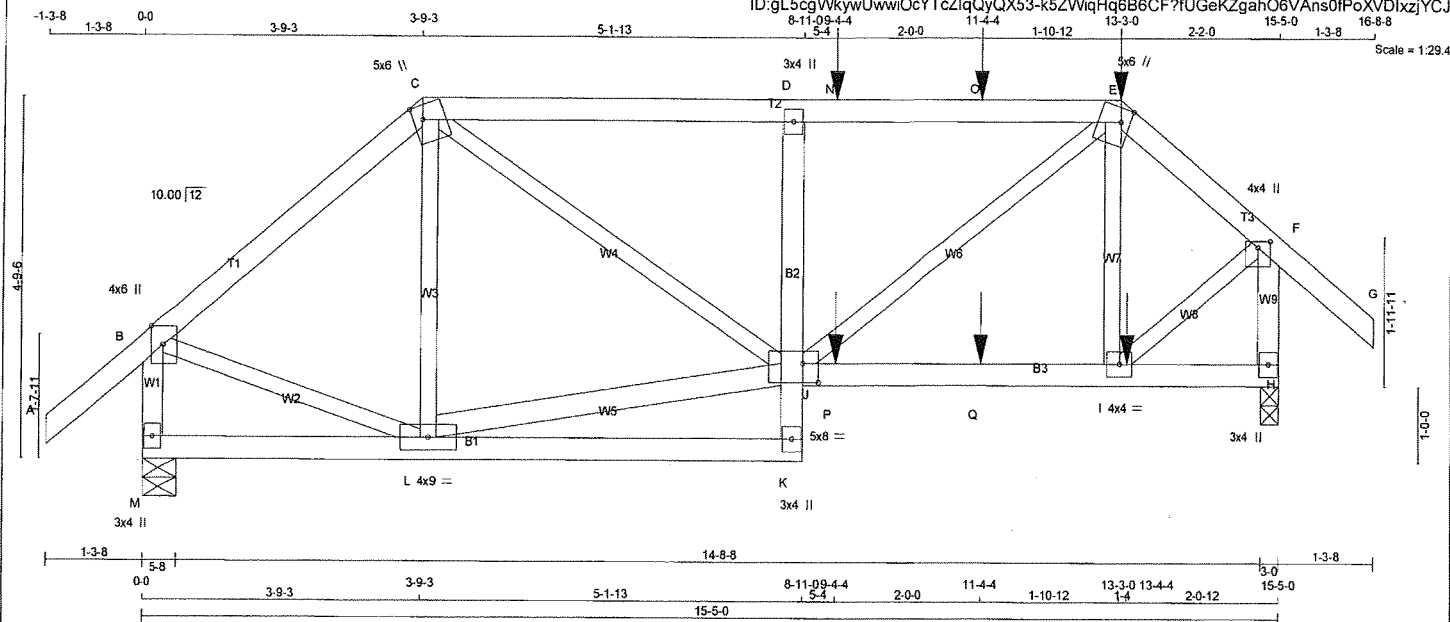


JWG NO. TAM 11003712
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T72S	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 75 = 150 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVWW-t	MT20	5.0	8.0	3.00	2.50
K	BMV+p	MT20	3.0	4.0		
L	BMVWW-t	MT20	4.0	9.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UPLIFT	IN-SX
JT	1028	0	0	5-8
M	1100	0	0	5-8
H	1100	0	0	3-0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	777	390 / 0	165 / 0	0 / 0	0 / 0	222 / 0	0 / 0
H	829	424 / 0	168 / 0	0 / 0	0 / 0	237 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	UNBRAC LENGTH	FR-TO			
A-B	0 / 35	-78.0	-78.0 0.12 (1)	10.00	L-C	-116 / 83	0.04 (1)
B-C	-798 / 0	-78.0	-78.0 0.23 (1)	6.25	J-E	0 / 662	0.16 (1)
C-D	-1089 / 0	-78.0	-78.0 0.35 (1)	5.58	I-E	-231 / 3	0.06 (1)
D-N	-1089 / 0	-78.0	-78.0 0.34 (1)	5.57	B-L	0 / 662	0.16 (1)
N-O	-1089 / 0	-78.0	-78.0 0.34 (1)	5.57	I-F	0 / 662	0.16 (1)
O-E	-1089 / 0	-78.0	-78.0 0.34 (1)	5.57	L-J	0 / 662	0.16 (1)
E-F	-748 / 0	-78.0	-78.0 0.08 (1)	6.25	C-G	0 / 662	0.16 (1)
F-G	0 / 35	-78.0	-78.0 0.12 (1)	10.00			
M-B	-982 / 0	0.0	0.0 0.11 (1)	7.81			
H-F	-1088 / 0	0.0	0.0 0.13 (1)	7.55			
M-L	0 / 0	-38.5	-38.5 0.20 (3)	10.00			
L-K	0 / 30	-38.5	-38.5 0.20 (3)	10.00			
K-J	0 / 100	0.0	0.0 0.06 (1)	7.81			
J-D	-457 / 0	0.0	0.0 0.06 (1)	7.81			
J-P	0 / 565	-38.5	-38.5 0.23 (2)	10.00			
P-Q	0 / 565	-38.5	-38.5 0.23 (2)	10.00			
Q-I	0 / 565	-38.5	-38.5 0.23 (2)	10.00			
I-H	0 / 0	-38.5	-38.5 0.14 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-3-0	-13	-14	---	FRONT	VERT	DEAD	---	---
E	13-3-0	-13	-13	---	FRONT	VERT	TOTAL	---	---
E	13-3-0	-60	-60	---	FRONT	VERT	SNOW	---	---
I	13-4-4	-3	-4	---	FRONT	VERT	TOTAL	---	---
N	9-4-4	-7	-7	---	FRONT	VERT	TOTAL	---	---
O	11-4-4	-7	-7	---	FRONT	VERT	TOTAL	---	---
P	9-4-4	-11	-14	---	FRONT	VERT	TOTAL	---	---
Q	11-4-4	-3	-4	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 44.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.35/1.00 (C-D:1), BC=0.23/1.00 (I-J:2), WB=0.17/1.00 (F-I:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

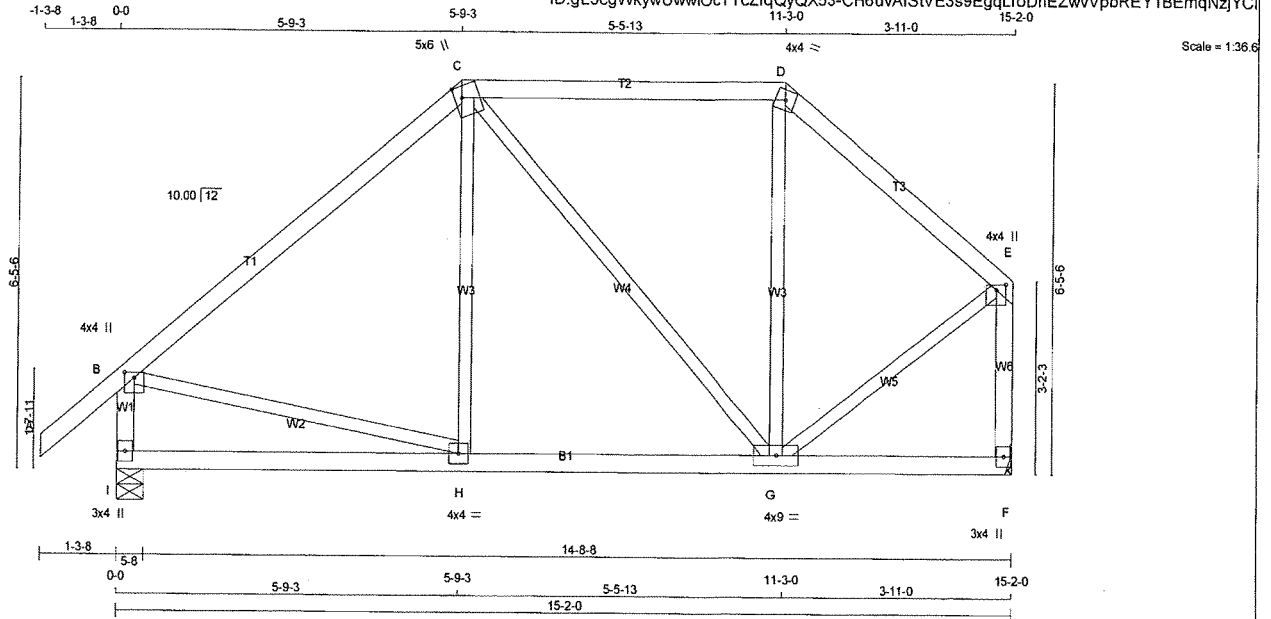
JSI GRIP= 0.82 (F) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 71209713
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T73A	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:41:15 2019 Page 1
ID: gL5cgWkywUwwiOcYTcZlqQyQX53-CH6uvAISTVE3s9EgqLroDnEZwVpbREY1BEmqNzjYCI



TOTAL WEIGHT = 2 X 69 = 138 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - D	2x4 DRY	No.2	SPF
D - E	2x4 DRY	No.2	SPF
I - B	2x4 DRY	No.2	SPF
F - E	2x4 DRY	No.2	SPF
I - F	2x4 DRY	No.2	SPF

ALL WEBS
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
I	992	0	992	0
F	884	0	884	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
I	750	377 / 0	159 / 0	0 / 0	214 / 0	0 / 0
F	675	319 / 0	159 / 0	0 / 0	197 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	WEBS	MAX. FACTORED	MAX.
FR-TO		FORCE (LBS)	VERT. (PLP)	FROM	TO	CSI (LC)	UNBRAC	FR-TO		FORCE (LBS)	CSI (LC)
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	H-C	0 / 247	0.06 (3)			
B-C	-698 / 0	-78.0	-78.0	0.34 (1)	6.25	C-G	-158 / 0	0.22 (1)			
C-D	-434 / 0	-78.0	-78.0	0.30 (1)	6.25	G-D	-3 / 136	0.03 (3)			
D-E	-566 / 0	-78.0	-78.0	0.16 (1)	6.25	B-H	0 / 549	0.12 (1)			
I-B	-900 / 0	0.0	0.0	0.10 (1)	7.81	G-E	0 / 531	0.12 (1)			
F-E	-823 / 0	0.0	0.0	0.15 (1)	7.81						
I-H	0 / 0	-38.5	-38.5	0.24 (3)	10.00						
H-G	0 / 538	-38.5	-38.5	0.30 (2)	10.00						
G-F	0 / 0	-38.5	-38.5	0.13 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 44.5 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.51")
CALCULATED VERT. DEFL. (LL) = L/999 (0.05")
ALLOWABLE DEFL. (TL) = L/360 (0.51")
CALCULATED VERT. DEFL. (TL) = L/999 (0.08")

CSI: TC=0.34/1.00 (B-C:1), BC=0.30/1.00 (G-H:2), WB=0.22/1.00 (C-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

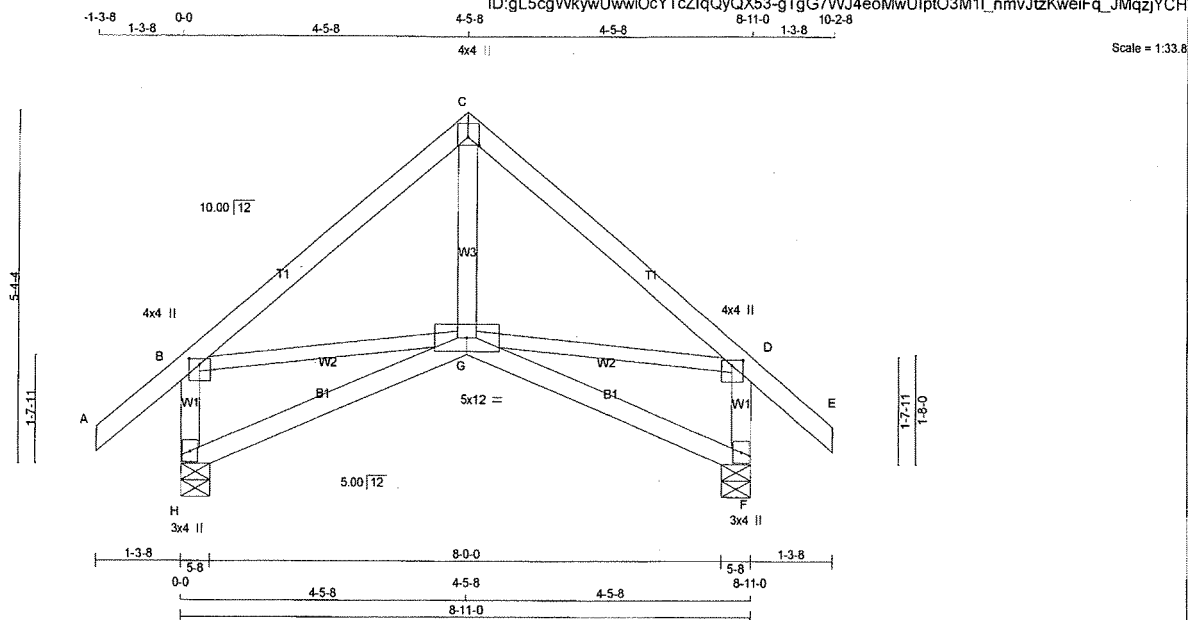
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)



DRWG NO. TAM 71203714
STRUCTURAL
COMPONENT ONLY

Version: 0.250 © NOV 17 2018 INTER INDUSTRIES, INC. Tue Feb 19 15:41:10 2019 Page 1
ID: gL5cqWkvwUwwiOcYTcZlqQvQX53-gTqG7WJ4epMwUlpO3M1l nmyJtzKweiFq JMaziYCH



TOTAL WEIGHT = 6 X 42 = 252 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTW+p	MT20	4.0	4.0	1.50 2.00
D	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BBWWWW-p	MT20	5.0	12.0	
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	627	0	627	0	0	5-8	5-8
F	627	0	627	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	472	245 / 0	94 / 0	0 / 0	0 / 0	133 / 0	0 / 0
F	472	245 / 0	94 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H. F.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (MAX)	CS1 (L/C)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX CS1 (L/C)	
FR-TO		FROM	TO			FR-TO			
A-B	0/35	-78.0	-78.0	0.11 (1)	10.00	G-C	0/271	0.04 (3)	
B-C	-451/0	-78.0	-78.0	0.20 (1)	6.25	B-G	0/348	0.08 (1)	
C-D	-451/0	-78.0	-78.0	0.20 (1)	6.25	G-D	0/348	0.08 (1)	
D-E	0/35	-78.0	-78.0	0.11 (1)	10.00				
H-B	-542/0	0.0	0.0	0.06 (1)	7.81				
F-D	-542/0	0.0	0.0	0.06 (1)	7.81				
H-G	0/0	-38.5	-38.5	0.18 (3)	10.00				
G-F	0/0	-38.5	-38.5	0.18 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.20/1.00 (B-C:1), BC=0.18/1.00 (G-H:3),
WB=0.08/1.00 (D-G:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

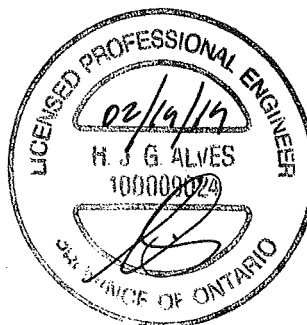
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

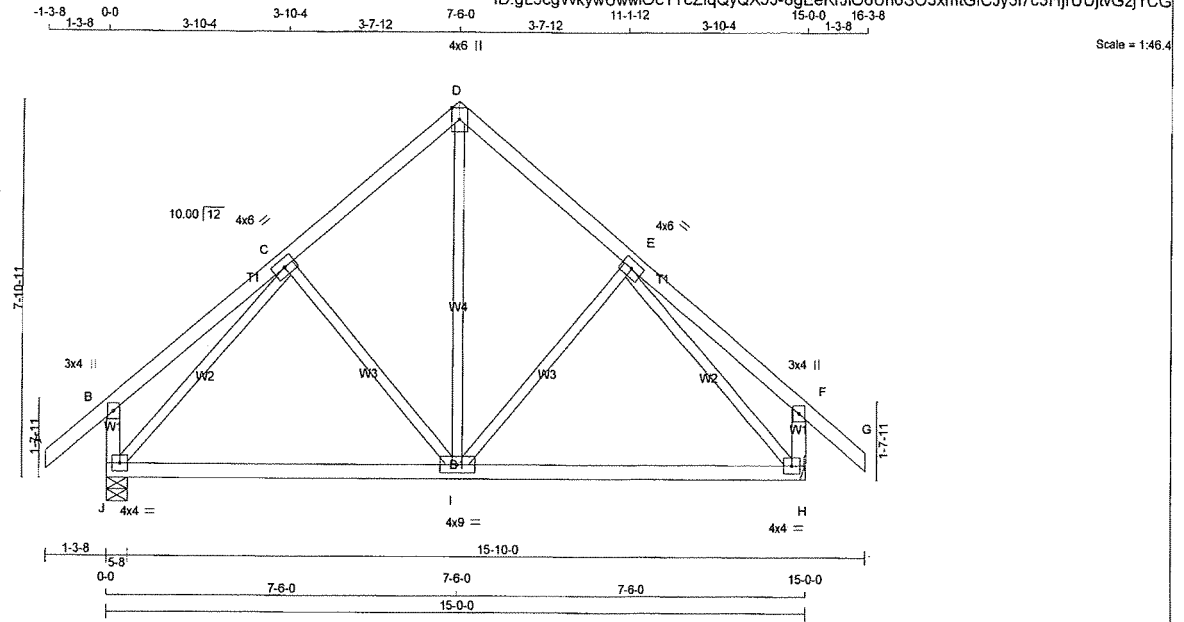


DWG NO. TAM 71903715
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T75	QUANTITY 4	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Feb 19 15:41:17 2019 Page 1
ID: gL5cgWkywUwwiOcYTcZlqQyQX53-8gEeKrJiO6Un6SO3xmtGlCJy5i7c3HjrUUjtvGzjYCG



TOTAL WEIGHT = 4 X 70 = 280 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWW1-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	982	0	982	0
H	982	0	982	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	742	373 / 0	158 / 0	0 / 0	0 / 0	212 / 0	0 / 0
H	742	373 / 0	158 / 0	0 / 0	0 / 0	212 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-78.0 -78.0	0.11 (1)	I-D	0 / 564	0.13 (2)	
B-C	0 / 21	-78.0 -78.0	0.17 (1)	I-E	-127 / 21	0.07 (1)	
C-D	-631 / 0	-78.0 -78.0	0.14 (1)	C-I	-127 / 21	0.07 (1)	
D-E	-631 / 0	-78.0 -78.0	0.14 (1)	J-C	-855 / 0	0.47 (1)	
E-F	0 / 21	-78.0 -78.0	0.17 (1)	E-H	-855 / 0	0.47 (1)	
F-G	0 / 35	-78.0 -78.0	0.11 (1)				
J-B	-220 / 0	0.0 0.0	0.02 (1)				
H-F	-220 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 551	-38.5 -38.5	0.54 (3)				
I-H	0 / 551	-38.5 -38.5	0.54 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 44.5	PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.50")
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.17/1.00 (B-C:1), BC=0.54/1.00 (I-J:3), WB=0.47/1.00 (C-J:1), SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (J) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)



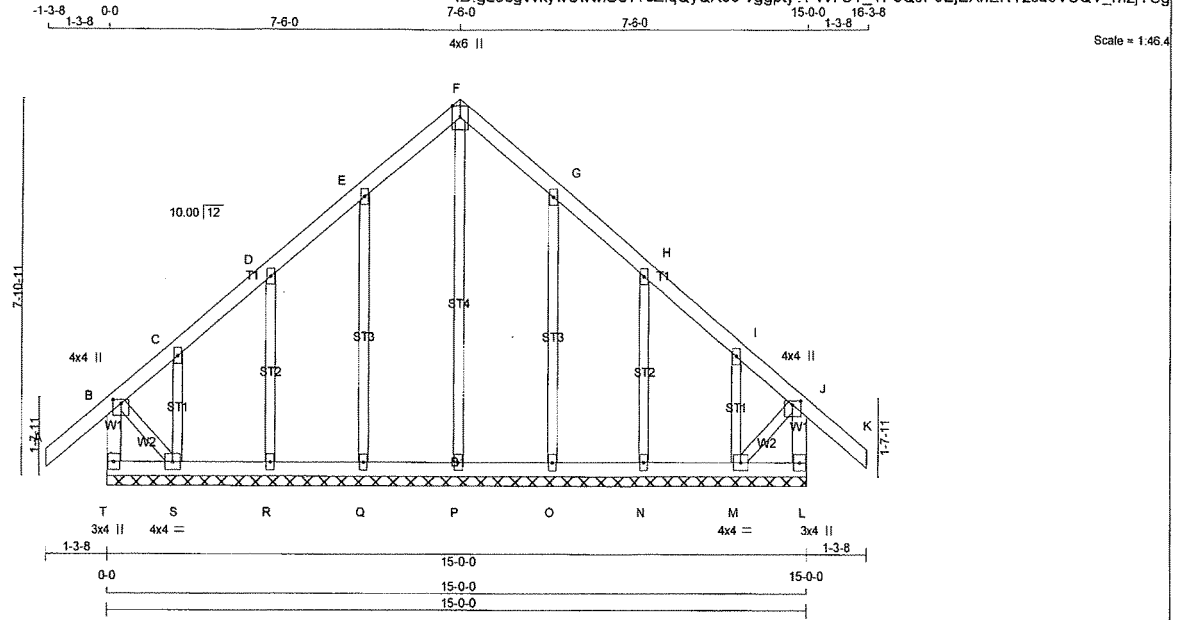
DWG NO. TAM **T7203716**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	G75	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:40:51 2019 Page 1

ID: gL5cgWkywUwwiOcyTcZlqQyQX53-vggpty?PW7CT_TP3QJP9EjEXhENYzcd0VSQV_mzjYCg



TOTAL WEIGHT = 2 X 74 = 149 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
T - B	2x4	DRY	No.2
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMVW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-l	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMV1+w	MT20	2.0	4.0		
S	BMVW1-l	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX. FACTORED	FACTORED	VERT. LOAD	LC1
	(LBS)	(PLF)	(LBS)	(LC)	(LBS)	(PLF)	(LBS)	(LC)
FR-TO		FROM TO			FR-TO			
T-B	-242 / 0	0.0	0.0	0.03 (1)	7.81	P-F	-116 / 0	0.13 (1)
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	Q-E	-175 / 0	0.11 (1)
B-C	-55 / 0	-78.0	-78.0	0.10 (1)	6.25	R-D	-160 / 0	0.05 (1)
C-D	-15 / 0	-78.0	-78.0	0.04 (1)	6.25	S-C	-74 / 0	0.01 (1)
D-E	-15 / 0	-78.0	-78.0	0.04 (1)	6.25	O-G	-175 / 0	0.11 (1)
E-F	-23 / 0	-78.0	-78.0	0.04 (1)	6.25	N-H	-160 / 0	0.05 (1)
F-G	-23 / 0	-78.0	-78.0	0.04 (1)	6.25	M-I	-74 / 0	0.01 (1)
G-H	-15 / 0	-78.0	-78.0	0.04 (1)	6.25	B-S	0 / 25	0.01 (1)
H-I	-15 / 0	-78.0	-78.0	0.04 (1)	6.25	M-J	0 / 25	0.01 (1)
I-J	-55 / 0	-78.0	-78.0	0.10 (1)	6.25			
J-K	0 / 35	-78.0	-78.0	0.11 (1)	10.00			
L-J	-242 / 0	0.0	0.0	0.03 (1)	7.81			
T-S	0 / 0	-38.5	-38.5	0.02 (3)	10.00			
S-R	0 / 16	-38.5	-38.5	0.02 (3)	10.00			
R-Q	0 / 13	-38.5	-38.5	0.02 (3)	10.00			
Q-P	0 / 10	-38.5	-38.5	0.02 (3)	10.00			
P-O	0 / 10	-38.5	-38.5	0.02 (3)	10.00			
O-N	0 / 13	-38.5	-38.5	0.02 (3)	10.00			
N-M	0 / 16	-38.5	-38.5	0.02 (3)	10.00			
M-L	0 / 0	-38.5	-38.5	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1), BC=0.02/1.00 (M-N:3), WB=0.13/1.00 (F-P:1), SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)

JSI METAL= 0.09 (E) (INPUT = 1.00)



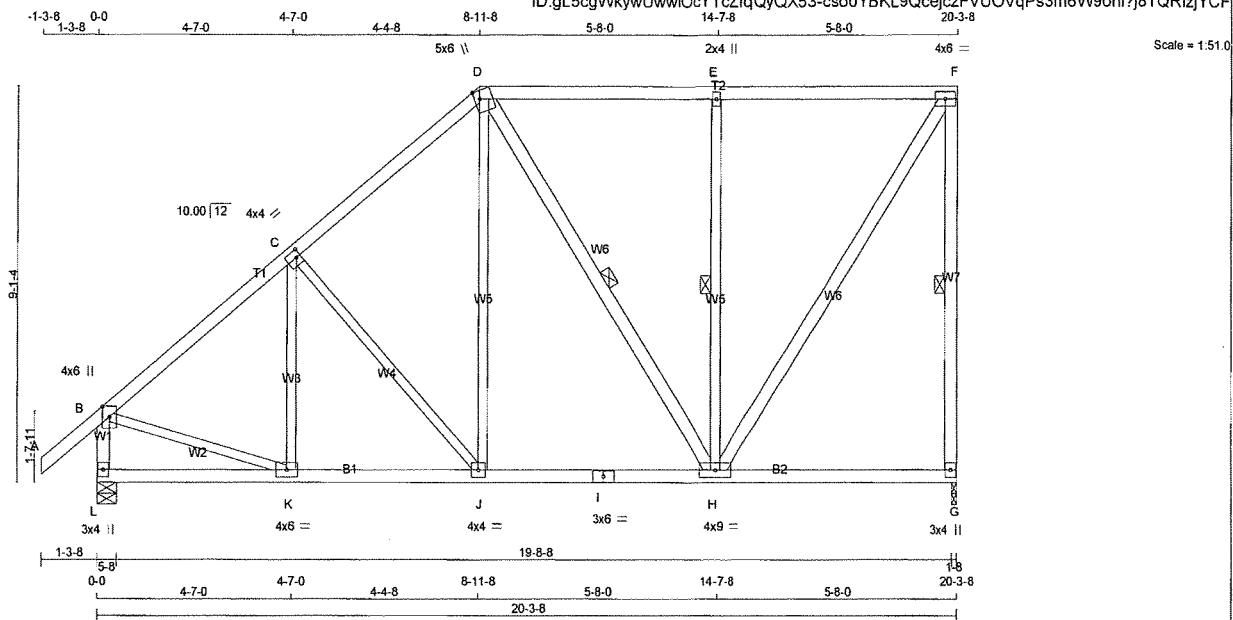
DWG NO. TAM 11903717
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T102	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 112 = 224 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMVW-t	MT20	4.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	1182	0	1182	0	1-8	1-8
L	1290	0	1290	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	284 / 0	0 / 0
L	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 35	-78.0	-78.0 0.11 (1)	10.00	K-C	-57 / 111	0.03 (3)
B-C	-1105 / 0	-78.0	-78.0 0.29 (1)	5.66	C-J	-291 / 0	0.25 (1)
C-D	-911 / 0	-78.0	-78.0 0.28 (1)	6.09	J-D	0 / 424	0.10 (2)
D-E	-599 / 0	-78.0	-78.0 0.43 (1)	6.25	D-H	-153 / 0	0.09 (1)
E-F	-599 / 0	-78.0	-78.0 0.43 (1)	6.25	H-E	-547 / 0	0.30 (1)
G-F	-1095 / 0	0.0	0.0 0.42 (1)	6.04	H-F	0 / 1102	0.18 (1)
L-B	-1217 / 0	0.0	0.0 0.13 (1)	7.26	B-K	0 / 903	0.20 (1)
L-K	0 / 0	-38.5	-38.5 0.14 (3)	10.00			
K-J	0 / 869	-38.5	-38.5 0.25 (2)	10.00			
J-I	0 / 682	-38.5	-38.5 0.33 (2)	10.00			
I-H	0 / 682	-38.5	-38.5 0.33 (2)	10.00			
H-G	0 / 0	-38.5	-38.5 0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL)= L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL)= L/999 (0.07")

CSI: TC=0.43/1.00 (E-F:1), BC=0.33/1.00 (H-J:2), WB=0.30/1.00 (E-H:1), SS=0.22/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

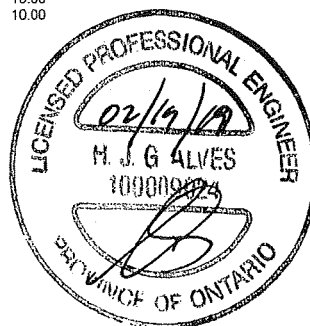
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

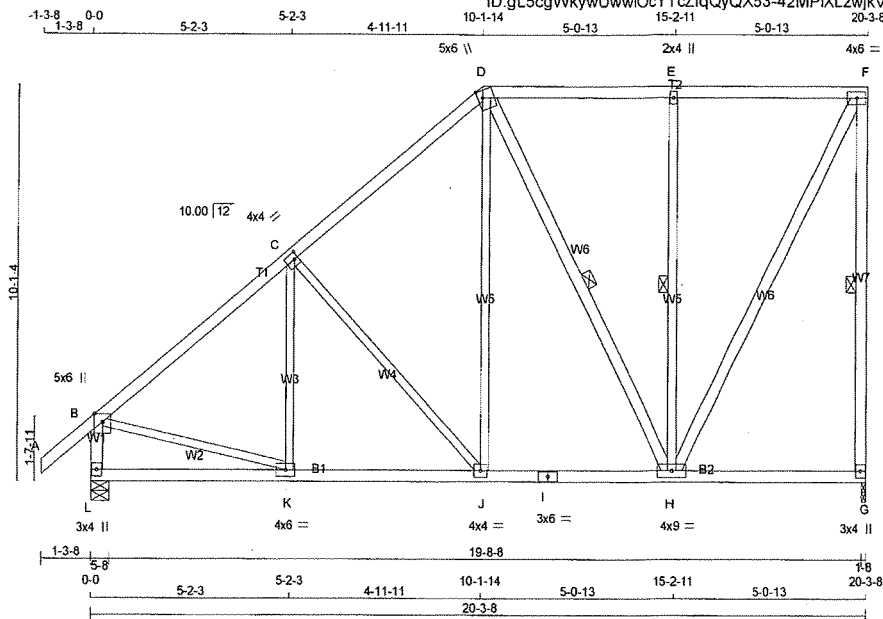


DWG NO. TAM 19903718
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T103	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 118 = 473 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.25
D	TTVW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+w	MT20	2.0	4.0		
F	TMVW-i	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMVWVW-i	MT20	4.0	9.0		
I	BS-i	MT20	3.0	6.0		
J	BMVW-i	MT20	4.0	4.0		
K	BMVW-i	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	1182	0	1182	0	1-8	1-8
L	1290	0	1290	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	903	426 / 0	213 / 0	0 / 0	0 / 0	264 / 0	0 / 0
L	978	484 / 0	213 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-78.0 -78.0	0.11 (1)	10.00	K-C	0 / 167	0.04 (3)
B-C	-1105 / 0	-78.0 -78.0	0.39 (1)	5.53	C-J	-383 / 0	0.44 (1)
C-D	-834 / 0	-78.0 -78.0	0.37 (1)	6.16	J-D	0 / 482	0.11 (2)
D-E	-495 / 0	-78.0 -78.0	0.34 (1)	6.25	D-H	-274 / 0	0.18 (1)
E-F	-495 / 0	-78.0 -78.0	0.34 (1)	6.25	H-E	-488 / 0	0.34 (1)
G-F	-1103 / 0	0.0 0.0	0.55 (1)	6.03	H-F	0 / 1072	0.17 (1)
L-B	-1210 / 0	0.0 0.0	0.13 (1)	7.28	B-K	0 / 899	0.20 (1)
L-K	0 / 0	-38.5 -38.5	0.19 (3)	10.00			
K-J	0 / 872	-38.5 -38.5	0.31 (2)	10.00			
J-I	0 / 620	-38.5 -38.5	0.27 (2)	10.00			
I-H	0 / 620	-38.5 -38.5	0.27 (2)	10.00			
H-G	0 / 0	-38.5 -38.5	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	44.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9. NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.68")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.06")

CSI: TC=0.55/1.00 (F-G), BC=0.31/1.00 (J-K-2), WB=0.44/1.00 (C-J-1), SSI=0.19/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

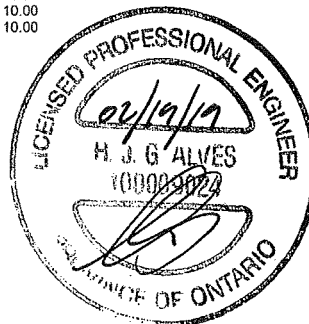
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

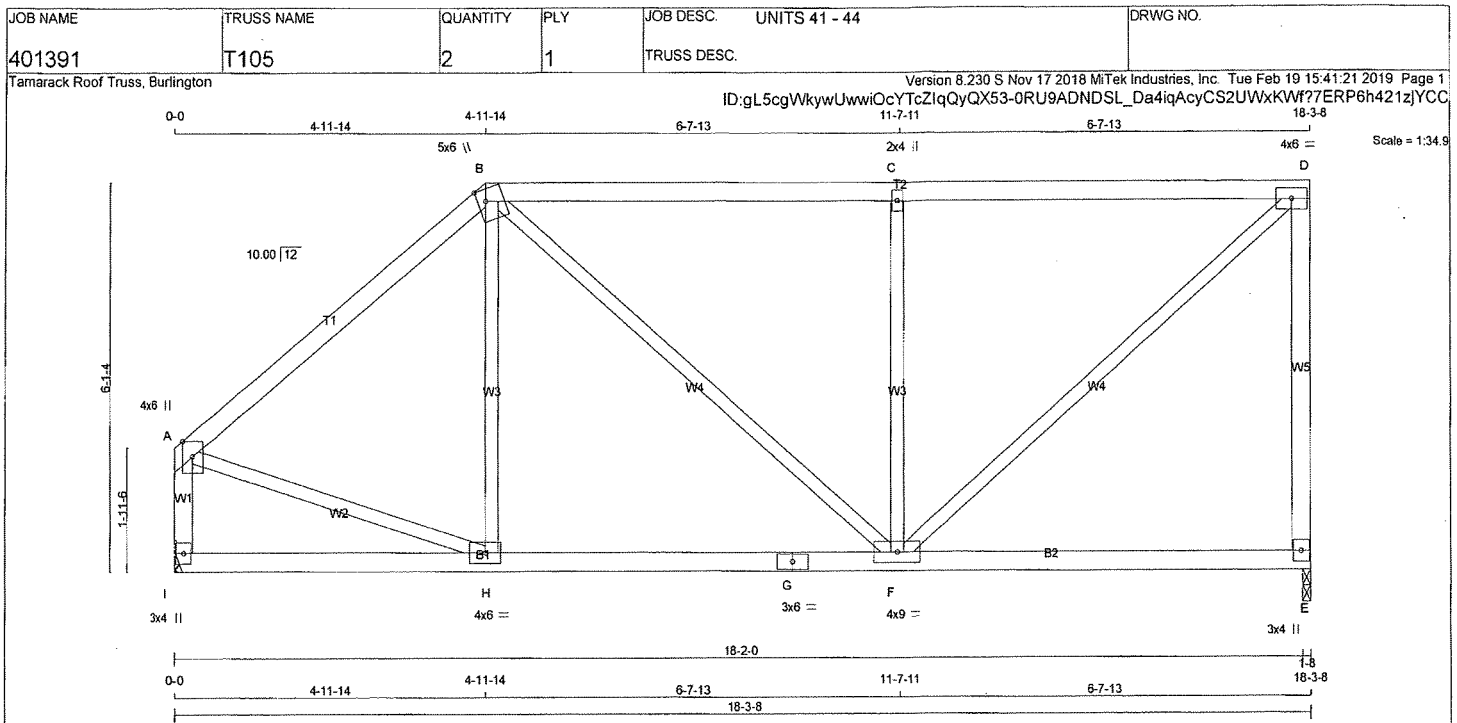
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)



DWG NO. TAM 11903719
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 78 = 157 lb (M)

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
I - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-l	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-l	MT20	4.0	9.0		
G	BS-l	MT20	3.0	6.0		
H	BMVWW-l	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
E	1066	0	1066	0	1-8	1-8
I	1066	0	1066	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO				FR-TO			
A-B	-900 / 0	-78.0	-78.0 0.37 (1)	5.98	H-B	0 / 178	0.04 (3)
B-C	-879 / 0	-78.0	-78.0 0.63 (1)	5.48	B-F	0 / 252	0.06 (1)
C-D	-879 / 0	-78.0	-78.0 0.63 (1)	5.48	F-C	-642 / 0	0.38 (1)
E-D	-962 / 0	0.0	0.0 0.69 (1)	7.81	F-D	0 / 1165	0.26 (1)
I-A	-990 / 0	0.0	0.0 0.11 (1)	7.81	A-H	0 / 723	0.16 (1)
I-H	0 / 0	-38.5	-38.5 0.21 (3)	10.00			
H-G	0 / 690	-38.5	-38.5 0.40 (2)	10.00			
G-F	0 / 690	-38.5	-38.5 0.40 (2)	10.00			
F-E	0 / 0	-38.5	-38.5 0.32 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 44.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.13")

CSI: TC=0.69/1.00 (D-E:1), BC=0.40/1.00 (F-H:2), WB=0.38/1.00 (C-F:1), SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

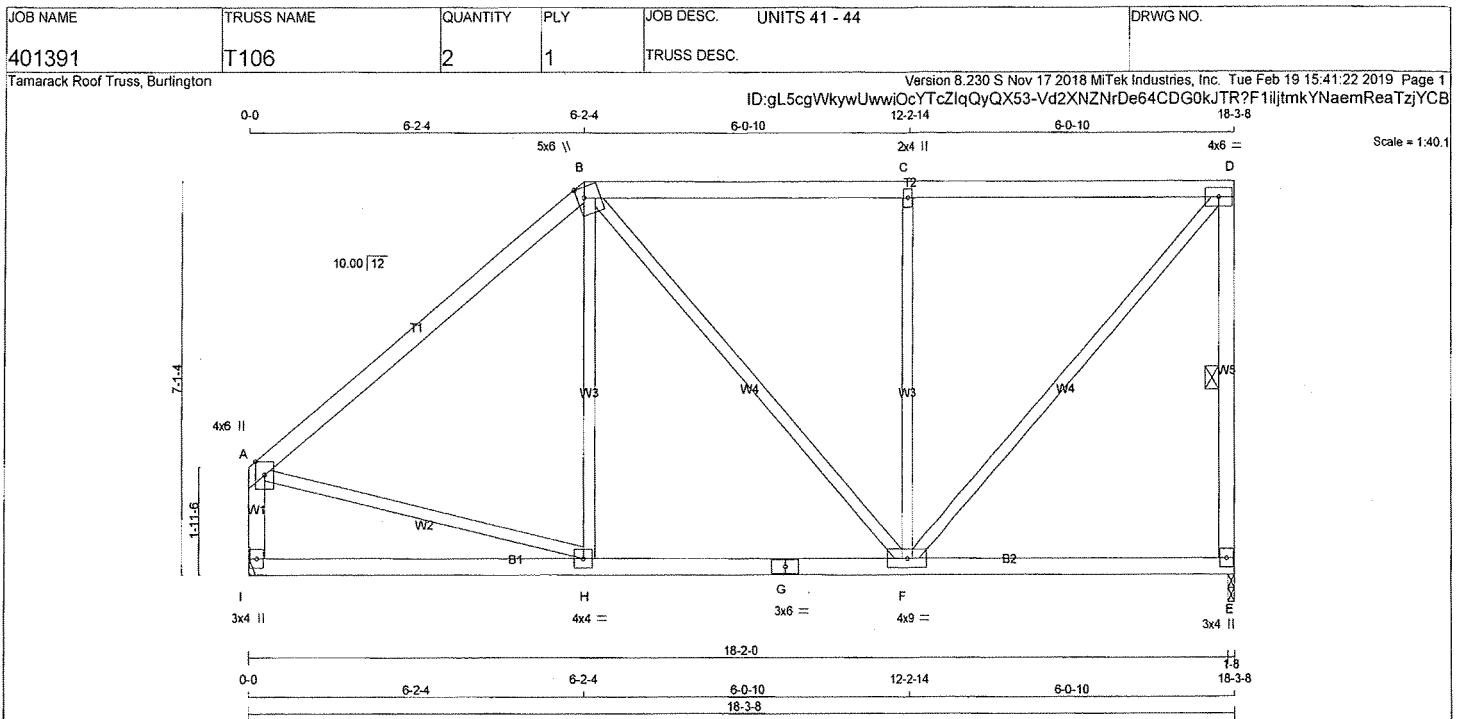
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)



DWG NO. TAM 77903721
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 83 = 166 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF
I - A	2x4 DRY	No.2	SPF
I - G	2x4 DRY	No.2	SPF
G - E	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMVW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMVWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
E	1066	0	0	1-8
I	1066	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	814	384 / 0	192 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD			MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE		
		FROM	TO	CSI (LC)			MEMB.	FORCE (LBS)	CSI (LC)
FR-TO									
A-B	-866 / 0	-78.0	-78.0	0.59 (1)	5.68	H-B	0 / 233	0.06 (3)	
B-C	-705 / 0	-78.0	-78.0	0.50 (1)	6.24	B-F	0 / 60	0.01 (1)	
C-D	-705 / 0	-78.0	-78.0	0.50 (1)	6.24	F-C	-584 / 0	0.51 (1)	
E-D	-969 / 0	0.0	0.0	0.22 (1)	6.25	F-D	0 / 1058	0.24 (1)	
I-A	-867 / 0	0.0	0.0	0.11 (1)	7.81	A-H	0 / 686	0.15 (1)	
I-H	0 / 0	-38.5	-38.5	0.26 (3)	10.00				
H-G	0 / 665	-38.5	-38.5	0.35 (2)	10.00				
G-F	0 / 665	-38.5	-38.5	0.35 (2)	10.00				
F-E	0 / 0	-38.5	-38.5	0.25 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.59/1.00 (A-B:1), BC=0.35/1.00 (F-H:2),
WB=0.51/1.00 (C-F:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

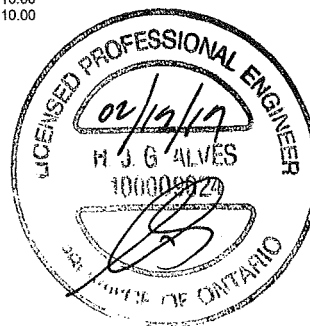
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.42 (A) (INPUT = 1.00)



DWG NO. TAM 1703722
STRUCTURAL
COMPONENT ONLY

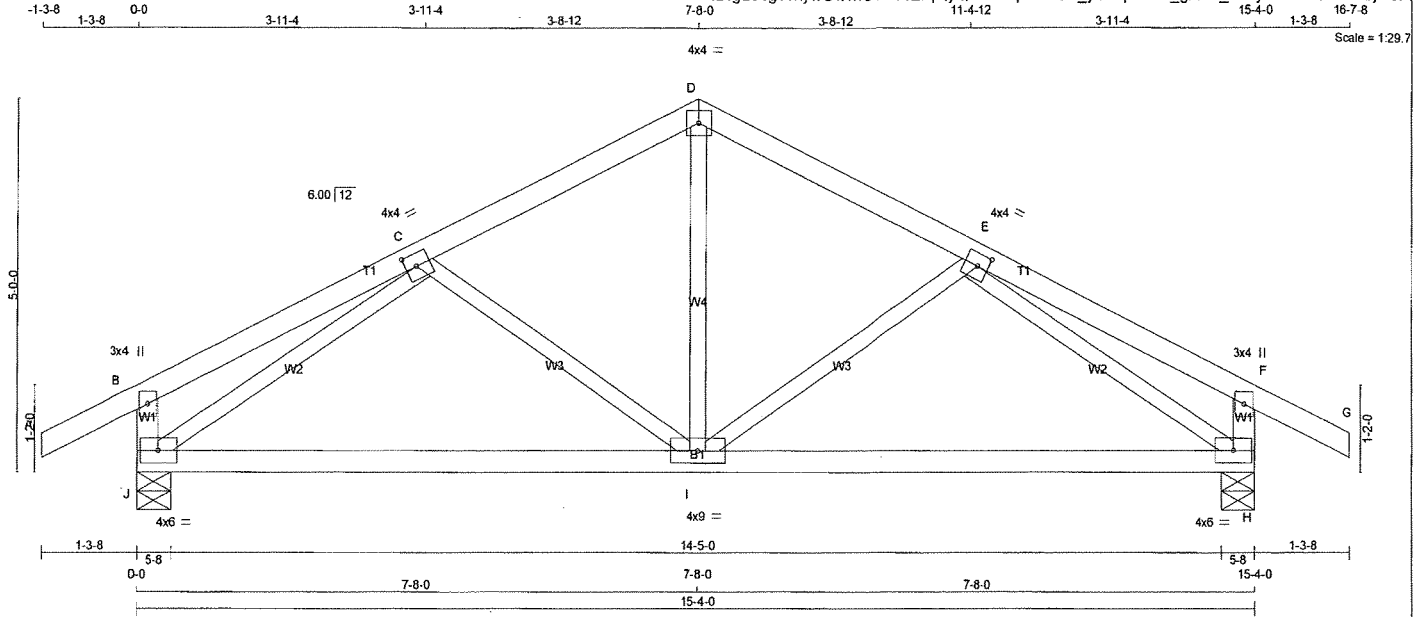
JOB NAME 401391	TRUSS NAME T111	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:41:23 2019 Page 1

ID: gL5cgWkywUwwiOcYtCZlqQyQX53-zqbvbvOT_yEwqNrDl1_gXTZ_c79yT09ksQAB6wzjYCA

Scale = 1:29.7



TOTAL WEIGHT = 2 X 60 = 121 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-p	MT20	4.0	4.0	
E	TMWW-t	MT20	4.0	4.0	2.00 1.75
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMWWW-t	MT20	4.0	9.0	
J	BMVW1-t	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
J	999	0	999	0	0	5-8	5-8	5-8	5-8
H	999	0	999	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
J	756	379 / 0	161 / 0	0 / 0	216 / 0
H	756	379 / 0	161 / 0	0 / 0	216 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		LENGTH	FR-TO			
A-B	0 / 24	-78.0	-78.0	0.10 (1)	10.00	I-D	0 / 567	0.13 (2)	
B-C	0 / 14	-78.0	-78.0	0.17 (1)	10.00	I-E	-160 / 33	0.06 (1)	
C-D	-895 / 0	-78.0	-78.0	0.14 (1)	6.25	C-I	-160 / 33	0.06 (1)	
D-E	-895 / 0	-78.0	-78.0	0.14 (1)	6.25	J-C	-1132 / 0	0.41 (1)	
E-F	0 / 14	-78.0	-78.0	0.17 (1)	10.00	E-H	-1132 / 0	0.41 (1)	
F-G	0 / 24	-78.0	-78.0	0.10 (1)	10.00				
J-B	-224 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-224 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 917	-38.5	-38.5	0.61 (2)	10.00				
I-H	0 / 917	-38.5	-38.5	0.61 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 44.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.51")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.51")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CSI: TC=0.17/1.00 (E-F:1), BC=0.61/1.00 (I-J:2), WB=0.41/1.00 (E-H:1), SS=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

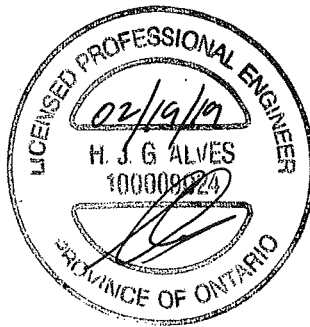
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
MT20	618	354	1667	788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT= 0.90)
JSI METAL= 0.39 (C) (INPUT= 1.00)



DRWG NO. TAM 1740 3723
STRUCTURAL
COMPONENT ONLY

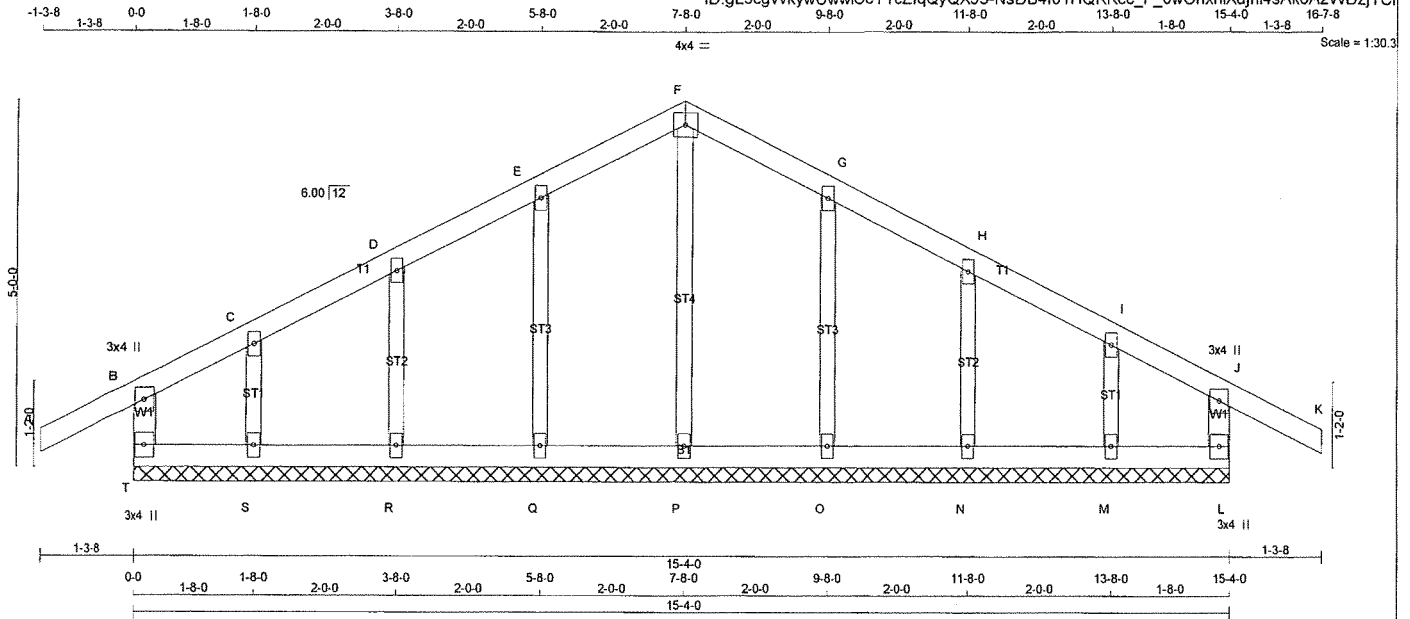
JOB NAME 401391	TRUSS NAME G111	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 9.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:40:52 2019 Page 1

ID: gL5cgWkywUwwiOcYTcZlqQyQX53-NsDB4I01HQKKcc_F_0wOnxniXdjini4sAk6A2WDzjYCF

Scale = 1:30.3



TOTAL WEIGHT = 2 X 58 = 115 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - F	2x4	DRY	No.2
F - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M, N, O, P, Q, R, S						
M	BMW1+w	MT20	2.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0/24	-78.0	-78.0 0.10 (1)	10.00	P-F	-188/0	0.07 (1)
B-C	-3/10	-78.0	-78.0 0.06 (1)	10.00	Q-E	-157/0	0.04 (1)
C-D	0/27	-78.0	-78.0 0.04 (1)	10.00	R-D	-161/0	0.03 (1)
D-E	0/28	-78.0	-78.0 0.04 (1)	10.00	S-C	-112/0	0.02 (1)
E-F	0/31	-78.0	-78.0 0.04 (1)	10.00	O-G	-157/0	0.04 (1)
F-G	0/31	-78.0	-78.0 0.04 (1)	10.00	N-H	-161/0	0.03 (1)
G-H	0/28	-78.0	-78.0 0.04 (1)	10.00	M-I	-112/0	0.02 (1)
H-I	0/27	-78.0	-78.0 0.04 (1)	10.00			
I-J	-3/10	-78.0	-78.0 0.06 (1)	10.00			
J-K	0/24	-78.0	-78.0 0.10 (1)	10.00			
T-B	-179/0	0.0	0.0 0.03 (1)	7.81			
L-J	-179/0	0.0	0.0 0.03 (1)	7.81			
T-S	-17/1	-38.5	-38.5 0.02 (3)	6.25			
S-R	-22/0	-38.5	-38.5 0.02 (3)	6.25			
R-Q	-26/0	-38.5	-38.5 0.02 (3)	6.25			
Q-P	-28/0	-38.5	-38.5 0.02 (3)	6.25			
P-O	-28/0	-38.5	-38.5 0.02 (3)	6.25			
O-N	-26/0	-38.5	-38.5 0.02 (3)	6.25			
N-M	-22/0	-38.5	-38.5 0.02 (3)	6.25			
M-L	-17/1	-38.5	-38.5 0.02 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 44.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1), BC=0.02/1.00 (Q-R:3), WB=0.07/1.00 (F-P:1), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (F) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)



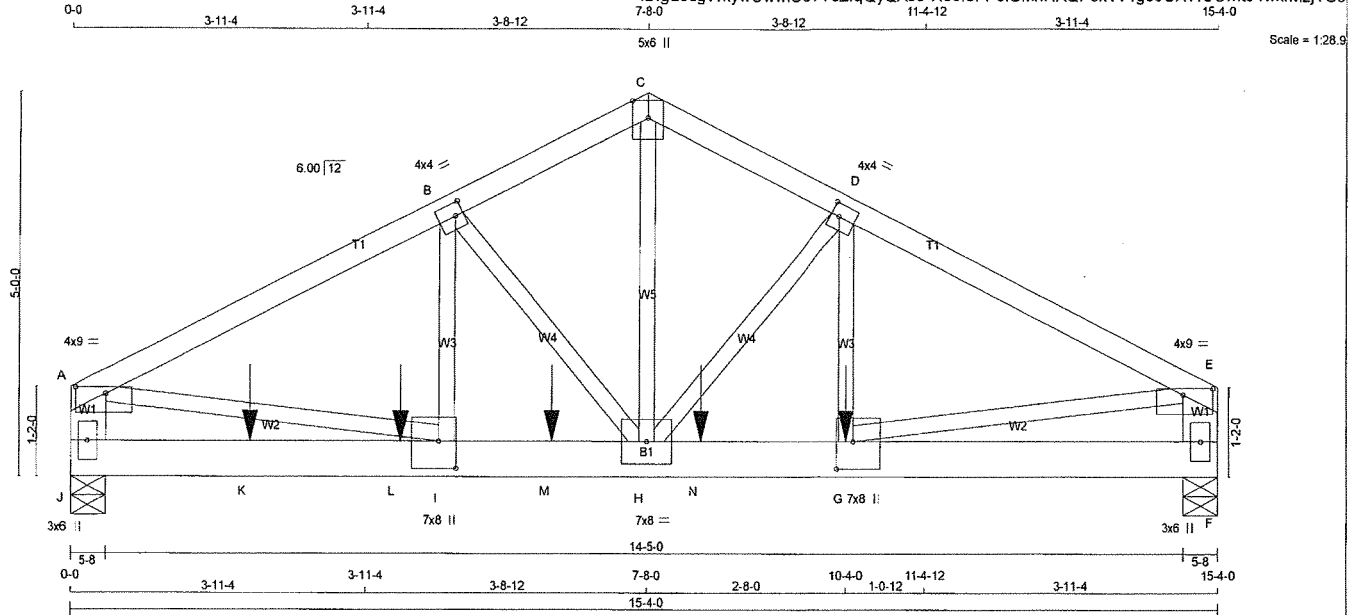
DWG NO. TAM 71903724
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T112	QUANTITY 2	PLY 2	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:41:24 2019 Page 1

ID: gL5cgWkywUwwiOcYTcZlqQyQX53-R09loFP5iGMnRXQPskVv4g66CXVfComt54wkfMzjYc9



TOTAL WEIGHT = 4 X 71 = 285 lb

LUMBER			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
J - A	2x6	DRY	No.2 SPF
F - E	2x6	DRY	No.2 SPF
J - F	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	TOP
C - E	12	TOP
J - A	2	TOP
F - E	2	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J - F	2	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
D - G	1	SIDE (475.2)
2x3	1	
B - I	1	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TM/VV-p	MT20	4.0	9.0	1.00	4.75
B	TM/VV-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	6.0	Edge	
D	TM/VV-t	MT20	4.0	4.0	2.00	1.25
E	TM/VV-p	MT20	4.0	9.0	1.00	4.75
F	BMV1+p	MT20	3.0	6.0		
G	BM/VV-t	MT20	7.0	8.0	4.25	2.75
H	BM/VV-t	MT20	7.0	8.0		
I	BM/VV-t	MT20	7.0	8.0	4.25	2.75
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS										
FACTORED				MAXIMUM FACTORED				INPUT	REQRD	
GROSS REACTION				GROSS REACTION				BRG	BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX		IN-SX	
J	4379	0		4379	0	0	5-8		5-8	
F	4040	0		4040	0	0	5-8		5-8	

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED
J	3321 1639 / 0 731 / 0 0 / 0 0 / 0 951 / 0 0 / 0
F	3059 1527 / 0 684 / 0 0 / 0 0 / 0 868 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH	
FR-TO				FR-TO			
A - B	-6136 / 0	-78.0 -78.0 0.36 (1)	3.70	H - C	0 / 4294	0.53 (1)	
B - C	-4980 / 0	-78.0 -78.0 0.14 (1)	4.22	H - D	-1939 / 0	0.29 (1)	
C - D	-4979 / 0	-78.0 -78.0 0.14 (1)	4.23	G - D	0 / 1772	0.22 (1)	
D - E	-6328 / 0	-78.0 -78.0 0.37 (1)	3.64	B - H	-1667 / 0	0.25 (1)	
J - A	-3777 / 0	0.0 0.0 0.13 (1)	7.26	I - B	0 / 1473	0.18 (1)	
F - E	-3890 / 0	0.0 0.0 0.14 (1)	7.18	A - I	0 / 5560	0.69 (1)	
				G - E	0 / 5734	0.71 (1)	
J - K	0 / 0	-38.5 -38.5 0.49 (1)	10.00				
K - L	0 / 0	-38.5 -38.5 0.49 (1)	10.00				
L - I	0 / 0	-38.5 -38.5 0.49 (1)	10.00				
I - M	0 / 5494	-38.5 -38.5 0.64 (1)	10.00				
M - H	0 / 5494	-38.5 -38.5 0.64 (1)	10.00				
H - N	0 / 5667	-38.5 -38.5 0.51 (1)	10.00				
N - G	0 / 5667	-38.5 -38.5 0.51 (1)	10.00				
G - F	0 / 0	-38.5 -38.5 0.11 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	10-4-0	-2524	-2524	---	FRONT	VERT	TOTAL	---	---
K	2-4-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	---
L	4-4-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	---
M	6-4-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	---
N	8-4-12	-1027	-1027	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.51")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.51")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.13")

CSI: TC=0.37/1.00 (D-E-1), BC=0.64/1.00 (H-I-1), WB=0.71/1.00 (E-G-1), SSI=0.55/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

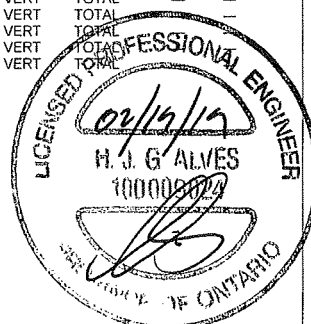
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	MAX	MIN	MAX
	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.73 (G) (INPUT = 1.00)



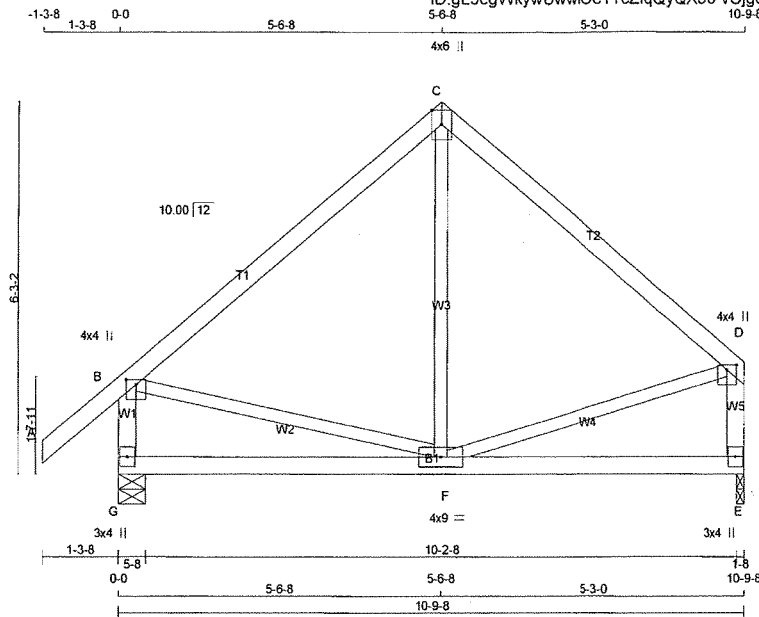
DRWG NO. TAM **T1903725**
STRUCTURAL
COMPONENT ONLY

JOB NAME 401391	TRUSS NAME T113A	QUANTITY 8	PLY 1	JOB DESC. TRUSS DESC.	UNITS 41 - 44	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:37.5

TOTAL WEIGHT = 8 X 47 = 375 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

C - D 2x4 DRY

G - B 2x4 DRY

E - D 2x4 DRY

G - E 2x4 DRY

LUMBER

No. 2

No. 2

No. 2

No. 2

No. 2

DESCR.

SPF

SPF

SPF

SPF

SPF

ALL WEBS 2x3 DRY
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
G	737	0	737	0	5-8	5-8
E	629	0	629	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
G	555	285 / 0	113 / 0	0 / 0	0 / 0	157 / 0	0 / 0
E	480	227 / 0	113 / 0	0 / 0	0 / 0	140 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	LC1		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 35	-78.0	-78.0	0.11 (1)	10.00	F-C	0 / 230	0.06 (3)	
B-C	-400 / 0	-78.0	-78.0	0.31 (1)	6.25	B-F	0 / 315	0.07 (1)	
C-D	-404 / 0	-78.0	-78.0	0.28 (1)	6.25	F-D	0 / 324	0.07 (1)	
G-B	-650 / 0	0.0	0.0	0.07 (1)	7.81				
E-D	-558 / 0	0.0	0.0	0.06 (1)	7.81				
G-F	0 / 0	-38.5	-38.5	0.26 (3)	10.00				
F-E	0 / 0	-38.5	-38.5	0.26 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.36")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.31/1.00 (B-C:1), BC=0.26/1.00 (F-G:3),
WB=0.07/1.00 (D-F:1), SSI=0.14/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)



DWG NO. TAM 71903726
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T114TCX	6	1	TRUSS DESC.		

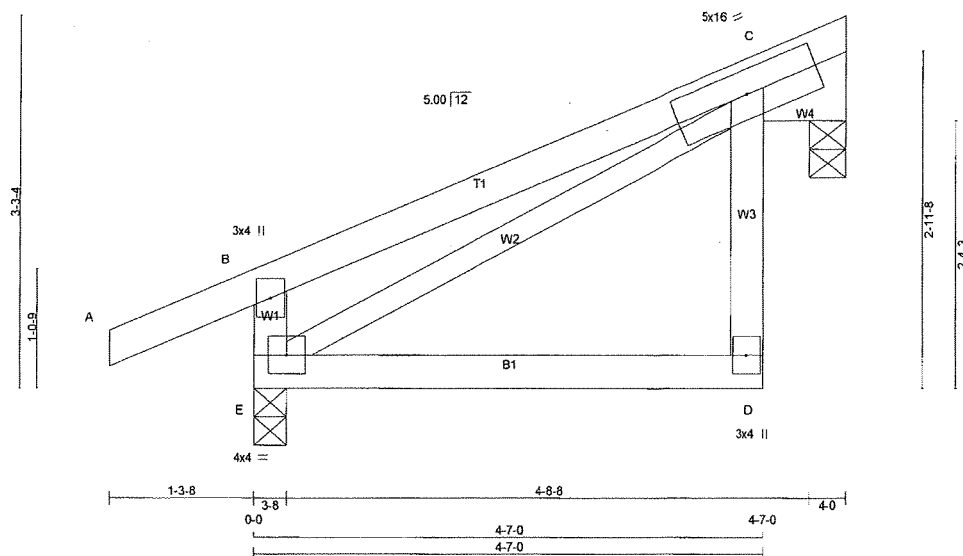
Tamarack Roof Truss, Burlington

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-1-3-8 1-3-8 0-0 4-3-8 4-3-8 4-7-0 4-3-8 4-7-0

Scale = 1:19.5



TOTAL WEIGHT = 6 X 22 = 134 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	SIZE	LUMBER		
CHORDS				
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS 2x3 DRY				SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVWW1-l	MT20	5.0	16.0	
D	BMV+p	MT20	3.0	4.0	
E	BMVW1-l	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
C	267	0	267	0	0	4-0	4-0		
E	372	0	372	0	0	3-8	3-8		

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
C	204	96 / 0	48 / 0	0 / 0	0 / 0	60 / 0	0 / 0	
E	277	153 / 0	48 / 0	0 / 0	0 / 0	76 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS	
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (LC)	MAX. UNBRACED LENGTH	FR-TO	MAX. FORCE (LBS)	MAX. CSI (LC)
A-B	0 / 20	-78.0	-78.0 0.10 (1)	10.00	E-C	0 / 0	0.00 (1)
B-C	0 / 0	-78.0	-78.0 0.28 (1)	10.00			
D-C	0 / 112	0.0	0.0 0.02 (3)	10.00			
E-B	-284 / 0	0.0	0.0 0.03 (1)	7.81			
E-D	0 / 0	-38.5	-38.5 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	44.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.05")

CSI: TC=0.28/1.00 (B-C:1), BC=0.19/1.00 (D-E:3), WB=0.00/1.00 (C-E:1), SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

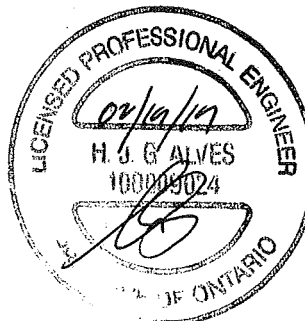
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



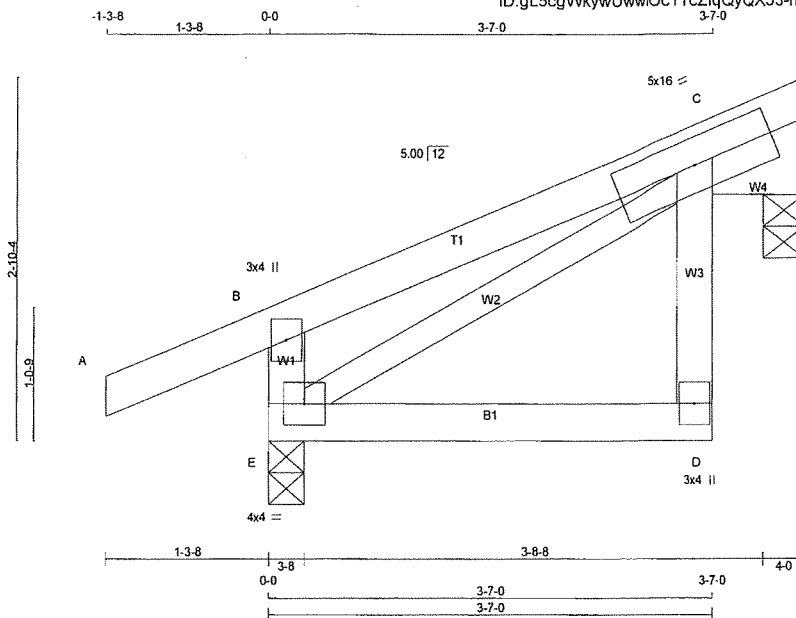
DRWG NO. TAM 17903727
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	T115TCX	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1:17.4

TOTAL WEIGHT = 8 X 19 = 149 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
E - B	2x4	DRY	No.2
E - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

LUMBER

DESCR.

SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	209	0	209	0	4-0	4-0
E	314	0	314	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	159	75 / 0	38 / 0	0 / 0	0 / 0	47 / 0	0 / 0
E	232	132 / 0	38 / 0	0 / 0	0 / 0	63 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FROM	TO				
A-B	0 / 20	-78.0	-78.0	0.10 (1)	10.00	E-C	0 / 0	0.00 (1)
B-C	0 / 0	-78.0	-78.0	0.17 (1)	10.00			
D-C	0 / 88	0.0	0.0	0.02 (3)	10.00			
E-B	-245 / 0	0.0	0.0	0.02 (1)	7.81			
E-D	0 / 0	-38.5	-38.5	0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 44.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.17/1.00 (B-C:1), BC=0.12/1.00 (D-E:3),
WB=0.00/1.00 (C-E:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



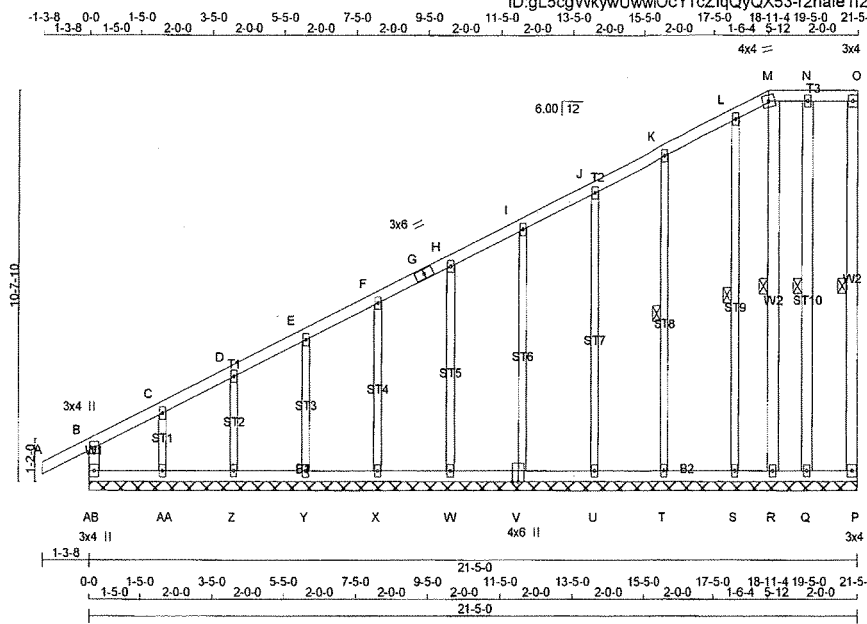
DWG NO. TAM 17203728
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	G116	8	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 8 X 130 = 1036 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
M - O	2x4	DRY	No.2	SPF
P - Q	2x4	DRY	No.2	SPF
AB - B	2x4	DRY	No.2	SPF
AB - V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - N	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT				
ST1	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P, N-Q, L-S, K-T, M-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1), BC=0.03/1.00 (U-V:3), WB=0.20/1.00 (J-U:1), SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

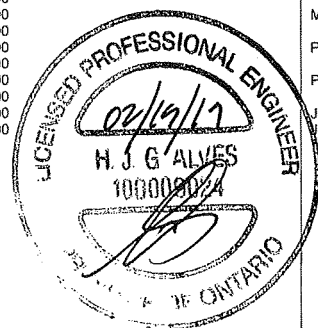
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C, D, E, F, H, I, J, K, L, N				
C TMV+w	MT20	2.0	4.0	
G TS-t	MT20	3.0	6.0	
M TTW-m	MT20	4.0	4.0	
O TMV+p	MT20	3.0	4.0	
P BMV1+p	MT20	3.0	4.0	
Q, R, S, T, U, W, X, Y, Z, AA				
Q BMV1+w	MT20	2.0	4.0	
V BSW1+H	MT20	4.0	6.0	
AB BMV1+p	MT20	3.0	4.0	

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0/24	-78.0 -78.0	0.10 (1)	10.00	Q-N	-95/0 0.05 (1)	
B-C	-22/0	-78.0 -78.0	0.04 (1)	6.25	S-L	-112/0 0.08 (1)	
C-D	-15/0	-78.0 -78.0	0.04 (1)	6.25	T-K	-162/0 0.09 (1)	
D-E	-11/0	-78.0 -78.0	0.04 (1)	6.25	U-J	-155/0 0.20 (1)	
E-F	-8/0	-78.0 -78.0	0.04 (1)	10.00	V-I	-156/0 0.14 (1)	
F-G	-6/0	-78.0 -78.0	0.04 (1)	10.00	W-H	-155/0 0.09 (1)	
G-H	-6/0	-78.0 -78.0	0.04 (1)	10.00	X-F	-155/0 0.06 (1)	
H-I	-4/0	-78.0 -78.0	0.04 (1)	10.00	Y-E	-155/0 0.04 (1)	
I-J	-5/0	-78.0 -78.0	0.04 (1)	10.00	Z-D	-155/0 0.03 (1)	
J-K	-4/0	-78.0 -78.0	0.04 (1)	10.00	AA-C	-151/0 0.02 (1)	
K-L	-5/0	-78.0 -78.0	0.04 (1)	10.00	R-M	-82/0 0.05 (1)	
L-M	-3/0	-78.0 -78.0	0.02 (1)	10.00			
M-N	0/0	-78.0 -78.0	0.02 (1)	10.00			
N-O	0/0	-78.0 -78.0	0.02 (1)	10.00			
P-O	-51/0	0.0 0.0	0.01 (1)	6.25			
AB-B	-192/0	0.0 0.0	0.01 (2)	7.81			
AB-AA	0/19	-38.5 -38.5	0.03 (3)	10.00			
AA-Z	0/14	-38.5 -38.5	0.03 (3)	10.00			
Z-Y	0/10	-38.5 -38.5	0.02 (3)	10.00			
Y-X	0/8	-38.5 -38.5	0.02 (3)	10.00			
X-W	0/6	-38.5 -38.5	0.03 (3)	10.00			
W-V	0/4	-38.5 -38.5	0.03 (3)	10.00			
V-U	0/5	-38.5 -38.5	0.03 (3)	10.00			
U-T	0/4	-38.5 -38.5	0.03 (3)	10.00			
T-S	0/2	-38.5 -38.5	0.02 (3)	10.00			
S-R	0/2	-38.5 -38.5	0.02 (3)	10.00			
R-Q	0/0	-38.5 -38.5	0.01 (3)	10.00			
Q-P	0/0	-38.5 -38.5	0.01 (3)	10.00			



Professional Engineer Seal
02/19/17

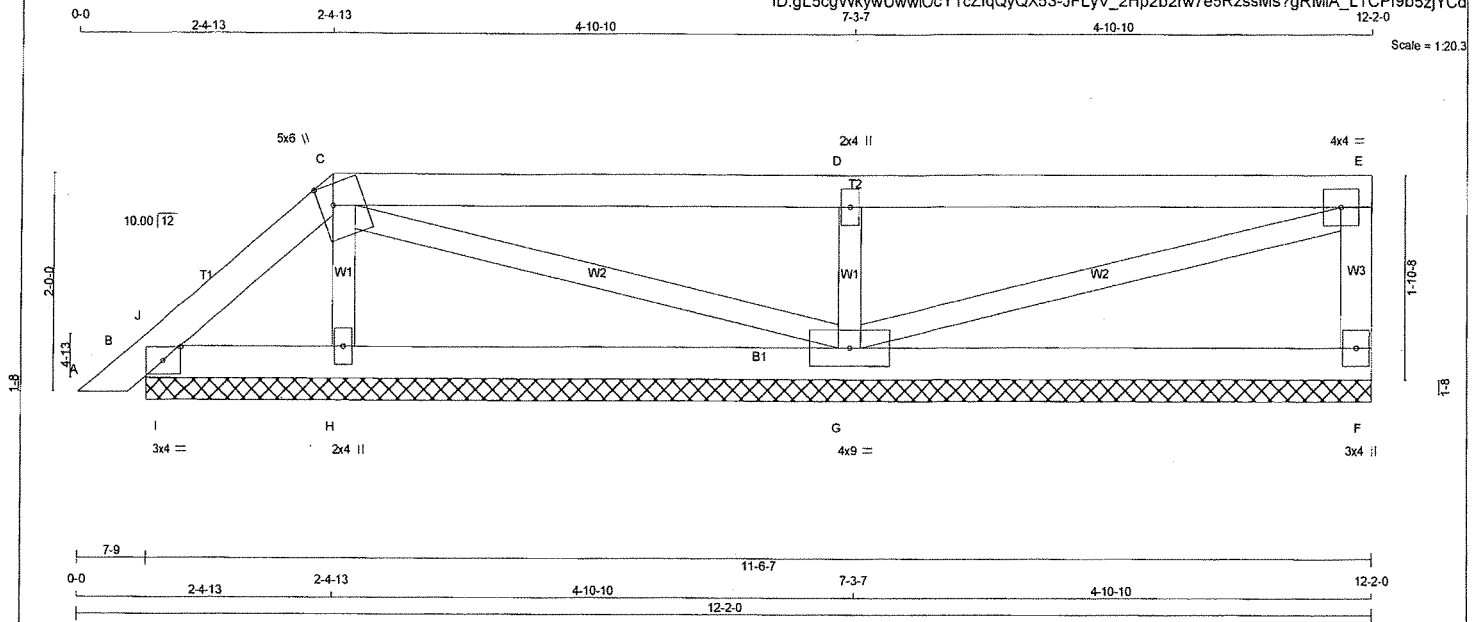


DRWG NO. TAM 71903729
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	UNITS 41 - 44	DRWG NO.
401391	PB1	6	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 15:40:54 2019 Page 1
ID: gL5cgWkywUwwiOcYTcZlqQyQX53-JFLyV_2Hp2b2rw7e5RzssMs?gRMIA_LTCPf9b5zjYCd



TOTAL WEIGHT = 6 X 39 = 235 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWV-I	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWVWV-I	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	219	0	219	0	0	11-6-7	11-6-7		
B	150	0	150	0	0	11-6-7	11-6-7		
H	317	0	317	0	0	11-6-7	11-6-7		
G	700	0	700	0	0	11-6-7	11-6-7		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	168	78 / 0	40 / 0	0 / 0	0 / 0	49 / 0	0 / 0
B	106	75 / 0	6 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	250	94 / 0	77 / 0	0 / 0	0 / 0	78 / 0	0 / 0
G	532	260 / 0	119 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0/12	-78.0	-78.0 0.02 (1)	10.00	H-C	-160/0	0.02 (1)
B-J	-19/8	-78.0	-78.0 0.01 (1)	6.25	C-G	-23/0	0.01 (1)
J-C	-46/0	-78.0	-78.0 0.02 (1)	6.25	G-D	-476/0	0.07 (1)
C-D	0/0	-78.0	-78.0 0.32 (1)	10.00	G-E	0/0	0.00 (1)
D-E	0/0	-78.0	-78.0 0.32 (1)	10.00	I-J	-106/0	0.00 (1)
F-E	-145/0	0.0	0.0 0.02 (1)	7.81			
B-I	0/32	-38.5	-38.5 0.04 (1)	10.00			
I-H	0/32	-38.5	-38.5 0.09 (2)	10.00			
H-G	0/22	-38.5	-38.5 0.18 (3)	10.00			
G-F	0/0	-38.5	-38.5 0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 44.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-D:1), BC=0.18/1.00 (G-H:3), WB=0.07/1.00 (D-G:1), SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

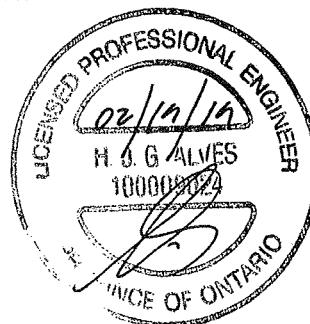
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DWG NO. TAM 17703730
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

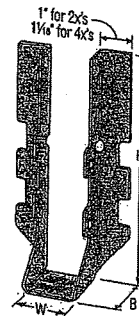
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

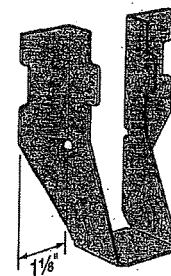
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



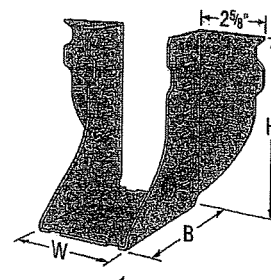
LUS28



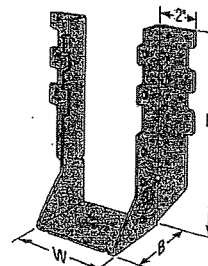
LU26L



HUS210
(HUS26, HUS28,
and HHUS similar)



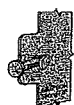
HGUS28-2



HHUS210-2



Double-Shear
Nailing
Top View

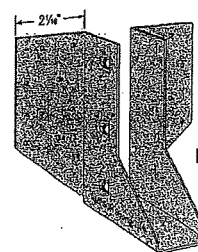
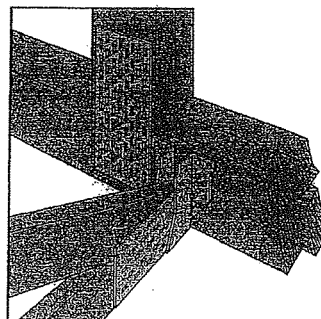


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

**Typical HUS26
Installation
with Reduced
Heel Height**
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS**HHUS/HGUS**

See Hanger Options information on pp. 125–127.

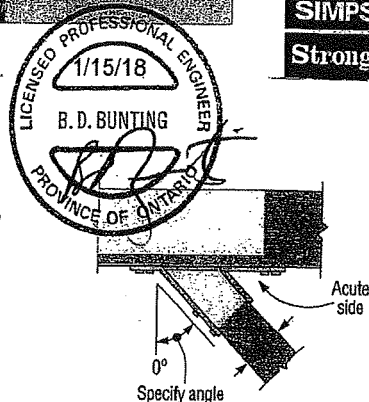
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.67 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value

**Top View HHUS Hanger Skewed Right**

(joist must be bevel cut)

All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-F-H		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K ₀ = 1.15)	(K ₀ = 1.00)	(K ₀ = 1.15)	(K ₀ = 1.00)
lb.	lb.	lb.	lb.								
kN	kN	kN	kN								
Single 2x Sizes											
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
								316	723	287	514
								136	300	119	215
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								166	464	142	322
								720	1605	645	1140
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								320	874	287	507
								1420	2170	1290	1630
SS LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								632	935	574	725
								2705	4940	2065	3875
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
								1210	2400	920	1724
								2055	4265	1460	4115
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1460	4115
								914	1837	649	1631
								2685	6625	2685	5700
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
								1196	2951	1196	2535
								1140	2185	1020	1550
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								507	972	454	689
								1420	2520	1290	1790
SS LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2520	1290	1790
								632	1121	574	796
								3605	5365	2675	4345
HUS28	16	1 1/8	7 1/4	3	6 1/8	(22) 16d	(8) 16d	3605	5365	2675	4345
								1604	2406	1190	1933
								3310	7675	3310	6900
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7675	3310	6900
								1474	3419	1474	3073
								1140	2495	1020	1770
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								507	1130	454	787
								1420	2785	1290	2210
SS LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								632	1239	574	983

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.

2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.

3. d_0 is the distance from the bearing seat to the top joist nail.

4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.

5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

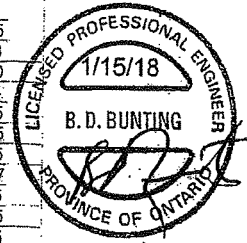
SIMPSON
Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

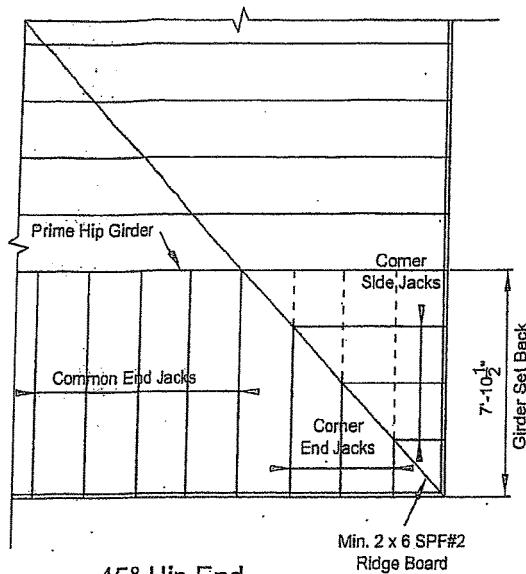
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance							
		W	H	B	d _s ¹	Header	Joist	D-Fir-L		S-P-F					
								Uplift	Normal	Uplift	Normal				
								(K _D = 1.15)	(K _D = 1.00)	(K _D = 1.15)	(K _D = 1.00)				
								lb.	lb.	lb.	lb.				
kN												kN	kN	kN	kN
Double 2x Sizes															
LUS24-2	18	3 3/8	3 3/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435				
LUS26-2	18	3 3/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920				
HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2850	7335	2065	5205				
HGUS26-2	12	3 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355				
LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575				
HHUS28-2	14	3 3/8	7 1/8	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345				
HGUS28-2	12	3 3/8	7 3/8	4	6 1/4	(36) 16d	(12) 16d	6070	12980	4310	9215				
LUS210-2	18	3 3/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195				
HHUS210-2	14	3 3/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000				
HGUS210-2	12	3 3/8	9 3/8	4	8 1/4	(46) 16d	(16) 16d	6840	14015	4855	10270				
Triple 2x Sizes															
HGUS26-3	12	4 1/8	5 1/2	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355				
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215				
HHUS210-3	14	4 1/8	9	3	7 1/4	(30) 16d	(10) 16d	4670	9670	4235	6865				
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400				
Quadruple 2x Sizes															
HGUS26-4	12	6 1/8	5 7/8	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355				
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215				
HHUS210-4	14	6 1/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	10155	4235	7210				
HGUS210-4	12	6 1/8	9 3/8	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400				
HGUS212-4	12	6 1/8	10 3/8	4	10 1/4	(56) 16d	(20) 16d	7640	14995	5425	10645				
HGUS214-4	12	6 1/8	12 3/8	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645				
4x Sizes															
LUS46	18	3 3/8	4 3/4	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920				
HHUS46	14	3 3/8	5 3/4	3	3 3/8	(14) 16d	(6) 16d	2540	7335	2065	5205				
HGUS46	12	3 3/8	5 1/4	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355				
LUS48	18	3 3/8	6 3/4	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2575				
HHUS48	14	3 3/8	7 3/8	3	6 1/4	(22) 16d	(8) 16d	3765	8940	2675	6345				
HGUS48	12	3 3/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215				
LUS410	18	3 3/8	8 3/4	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195				
HGUS410	12	3 3/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270				
HGUS412	12	3 3/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645				
HGUS414	12	3 3/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645				

See footnotes on p. 258.



Plated Truss Connectors



45° Hip End

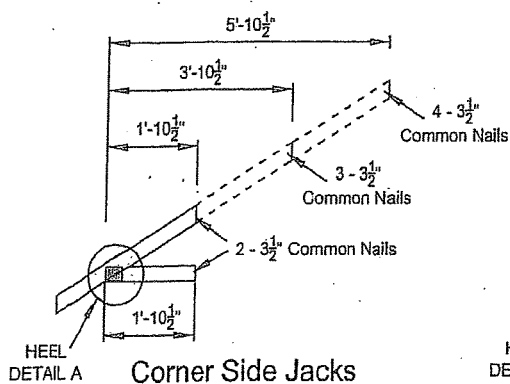
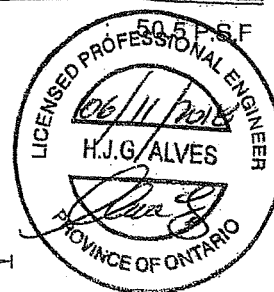
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

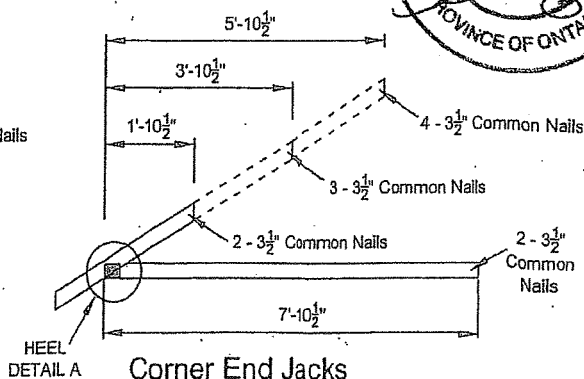
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

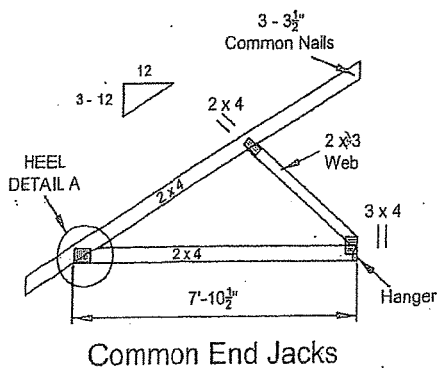
TOTAL LOAD



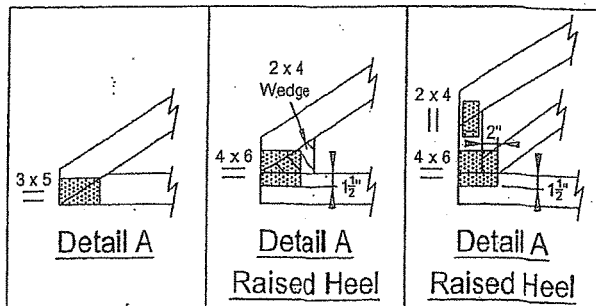
Corner Side Jacks



Corner End Jacks

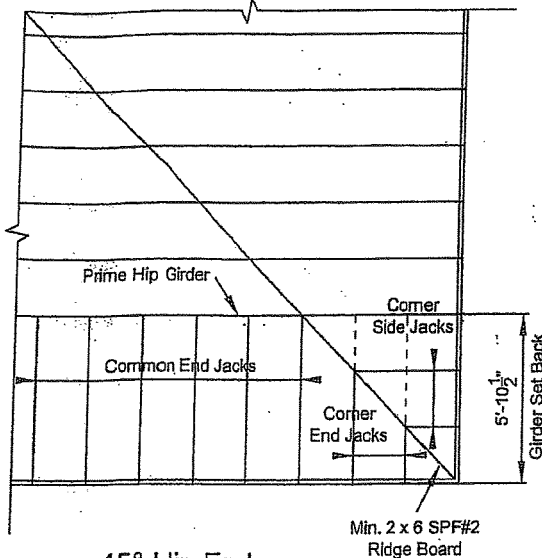


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

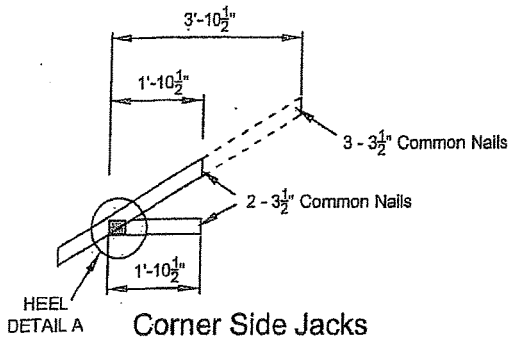
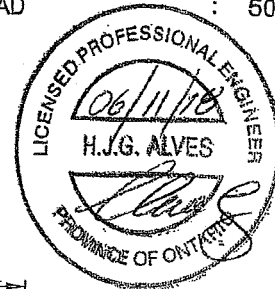
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

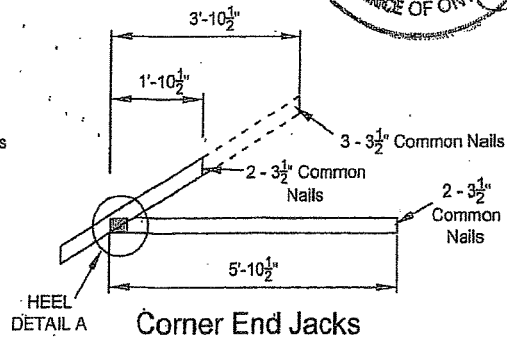
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

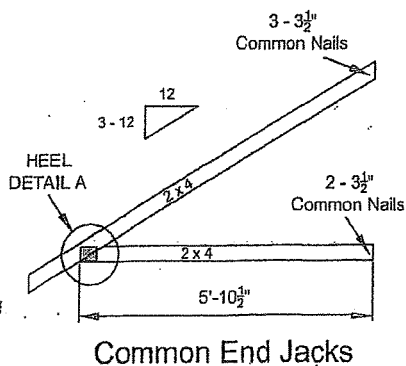
TOTAL LOAD : 50.5 P.S.F



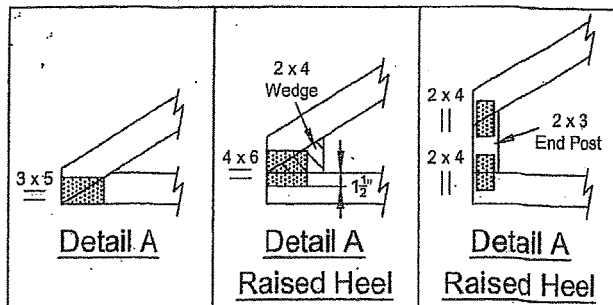
Corner Side Jacks



Corner End Jacks



Common End Jacks

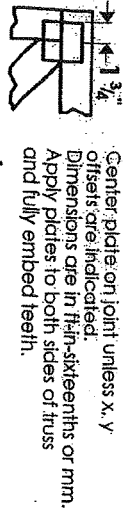


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

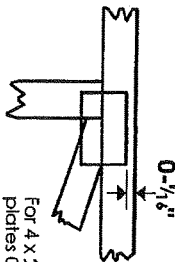
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in fractions of an inch or millimeters. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

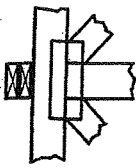
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



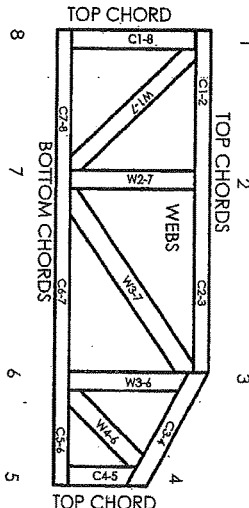
Indicates location where bearings (support) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in fractions of an inch or millimeters (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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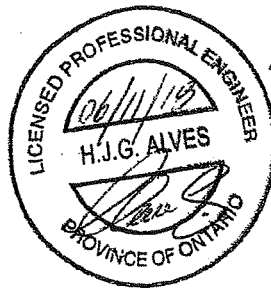
MITEK
POWER TO PERFORMANCE

Mitek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing members may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018