ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

DENOTES:

ALL B -2-2X10 FLUSH

* - ALL FIRST FLOOR PLATE HEIGHT 7'-10"-0 U/S @ PLATE LEVEL

V -6/12 VAULT CEILING (1'-5"-0 HIGH A.P.P)

C - 5/12 CATH. CEILING

H -12" RAISED CEILING +4" RAISED HEEL

HARDWARE:

LUS26-2 -(X)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

TH-6E
EL:B UNIT32
401423

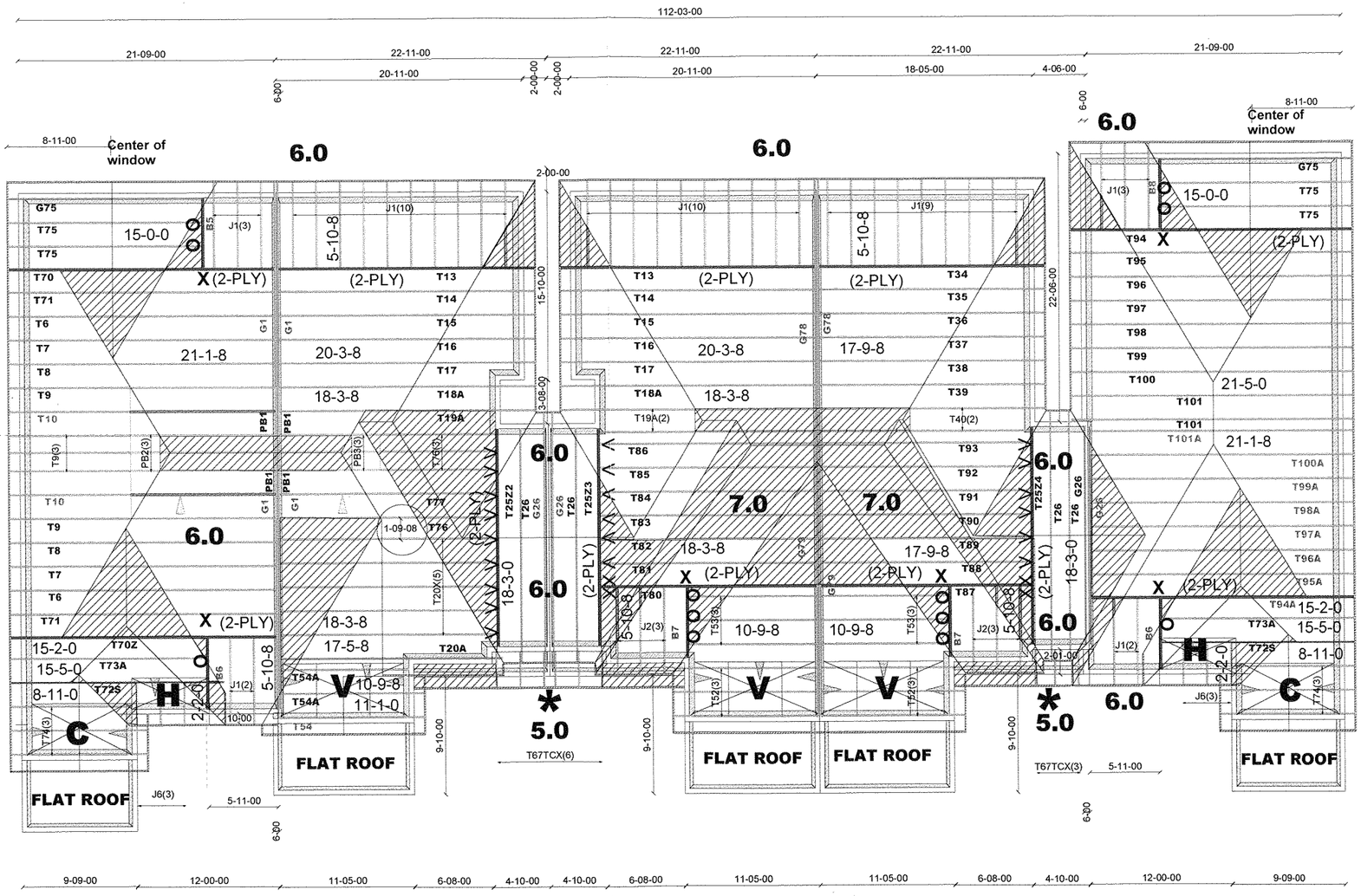
TH-2
EL:B UNIT33
401424

TH-2
EL:B2(REV) UNIT34
401425

TH-2MOD
EL:B UNIT35
401426

TH-6E
EL:B(REV) UNIT36
401427

10/12 PITCHES (TYP.)
UNLESS NOTED



T-180690

DENOTES:
CONVENTIONAL FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

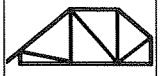
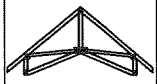
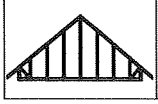
Job Track: 50264	Builder / Location: BAYVIEW WELLINGTON / ST. CATHERINES	Model / Elevation: FREEHOLD BLOCK 8 / UNITS 32 - 36	Mitek ver 8.2.3.229
Plan Log: 200524	Project: PASSAGE ON THE CANAL		
Layout ID: 401420	Date: 2/14/2019 Sales: Mario DiCano Designer: AC		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401423
Location:	ST CATHERINES	Ref #	
Model:	TH-6E	Page:	1 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 32	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

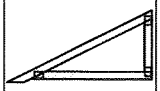
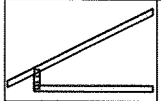
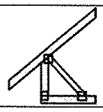
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	6 /12	21-05-00	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	230.92 149.67		
	2	T6 Half Hip	10 /12	21-01-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	191.49 122.67		
	2	T7 Half Hip	10 /12	21-01-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	2	T8 Half Hip	10 /12	21-01-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	5	T9 Half Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	573.25 364.17		
	2	T10 Half Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	229.3 145.67		
	1 2-ply	T70 Half Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	210.24 134.67		
	1 2-ply	T70Z Half Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	210.24 134.67		
	2	T71 Half Hip	10 /12	21-01-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	179.6 118.67		
	1	T72S Hip Girder	10 /12	15-05-00	4-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-11-11	74.78 48.67		
	1	T73A Hip	10 /12	15-02-00	6-05-06	2 x 4	1-03-08	1-07-11 3-02-03	69.05 44.17		
	3	T74 Scissor	10 /12 5 /12	8-11-00	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	125.83 83.00		
	2	T75 Common	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	140.22 93.33		
	1	G75 GABLE	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	74.49 49.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401423
Location:	ST CATHERINES	Ref #	
Model:	TH-6E	Page:	2 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 32	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	PB1 Piggyback	6 /12	**5-06-08**	2-09-04	2 x 4		2-09-04	29.24 20.00		
	3	PB2 Piggyback	10 /12	**12-02-00**	2-00-00	2 x 4		2-00-00	117.31 79.00		
	5	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	3	J6 Jack-Open	10 /12	**2-02-00**	3-09-06	2 x 4	1-03-08	1-11-11 3-09-06	34.05 24.50		

TOTAL # TRUSS= **42** TOTAL BFT OF ALL TRUSSES= **1916.53** BFT. TOTAL WEIGHT OF ALL TRSSES **2965.85** LBS

HARDWARE

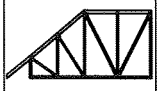
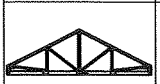
QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **5**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401424
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	1 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 33	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

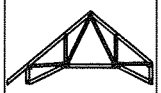
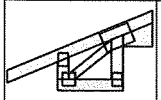
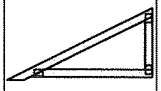
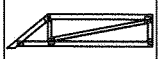
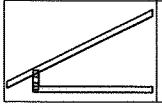
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	G1 GABLE	6 /12	21-05-00	9-01-04	2 x 4	1-03-08	1-02-00 9-01-04	230.92 149.67		
	1 2-ply	T13 Half Hip Girder	10 /12	20-03-08	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 112.33		
	1	T14 Half Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 56.50		
	1	T15 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.00		
	1	T16 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	1	T18A Half Hip	10 /12	18-03-08	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	118.33 72.67		
	1	T19A Half Hip	10 /12	18-03-08	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	133.7 83.50		
	1	T20A Common Structural Gable	10 /12	17-05-08	9-00-07	2 x 4	1-03-08	1-07-11 1-10-10	83.97 54.67		
	5	T20X Common	10 /12	18-03-08	9-00-07	2 x 4		11-06 1-10-10	419.59 270.00		
	1 2-ply	T25Z2 Common Girder	6 /12	18-03-00	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	168.25 104.00		
	1	T26 Common	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	72.57 47.33		
	1	G26 GABLE	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	1	T54 Roof Special	10 /12 6 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	54.19 36.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401424
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	2 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 33	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T54A Roof Special	10 /12 6 /12	**10-09-08**	6-04-04	2 x 4	1-07-08	1-07-11 1-10-10	103.78 69.67		
	3	T67TCX Monopitch	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	34.58 27.00		
	4	T76 Half Hip	10 /12	**18-03-08**	9-01-04	2 x 4		11-06 9-01-04	390.31 246.00		
	1	T77 Half Hip	10 /12	**18-03-08**	10-01-04	2 x 4		11-06 10-01-04	102.33 65.67		
	2	PB1 Piggyback	6 /12	**5-06-08**	2-09-04	2 x 4		2-09-04	29.24 20.00		
	3	PB3 Piggyback	10 /12	**8-06-00**	2-00-00	2 x 4		2-00-00	80.1 54.00		
	10	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		

TOTAL # TRUSS= **46** TOTAL BFT OF ALL TRUSSES= **1801.68** BFT. TOTAL WEIGHT OF ALL TRSSES **2797.58** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
9	Hardware	LJS26DS	

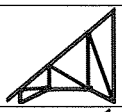
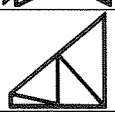
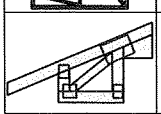
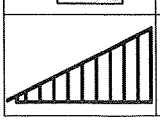
TOTAL NUMBER OF ITEMS= **9**

DELIVERY SHIPLIST

TAMARACK LUMBER INC. — ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401425
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	1 of 2
Lot #:		Date:	02/14/2019
Elevation:	B2 UNIT 34	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

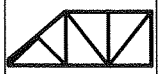
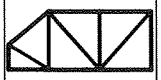
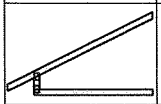
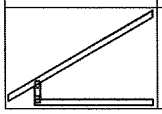
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T13 Half Hip Girder	10 /12	**20-03-08**	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	172.46 112.33		
	1	T14 Half Hip	10 /12	**20-03-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	87.39 56.50		
	1	T15 Half Hip	10 /12	**20-03-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.00		
	1	T16 Half Hip	10 /12	**20-03-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T17 Half Hip	10 /12	**20-03-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	1	T18A Half Hip	10 /12	**18-03-08**	9-01-10	2 x 6 2 x 4	3-03-08	3-03-11 9-01-04	118.33 72.67		
	2	T19A Half Hip	10 /12	**18-03-08**	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	267.41 167.00		
	1 2-ply	T25Z3 Common Girder	6 /12	**18-03-00**	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	168.25 104.00		
	1	T26 Common	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	72.57 47.33		
	1	G26 GABLE	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	3	T52 Monopitch	10 /12 6 /12	**10-09-08**	10-07-10	2 x 4	1-03-08	1-07-11 10-07-10	192.92 122.50		
	3	T53 Monopitch	10 /12	**10-09-08**	10-07-10	2 x 4		1-07-11 10-07-10	168.43 108.00		
	3	T67TCX Monopitch	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	34.58 27.00		
	1	G78 GABLE	6 /12	**19-04-08**	10-10-04	2 x 4	1-03-08	1-02-00 10-10-04	99.84 64.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401425
Location:	ST CATHERINES	Ref #	
Model:	TH-2	Page:	2 of 2
Lot #:		Date:	02/14/2019
Elevation:	B2 UNIT 34	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G79 GABLE	0 /12	24-03-08	10-07-10	2 x 4		10-07-10 10-07-10	227.57 143.33		
	1 2-ply	T80 Half Hip Girder	10 /12	18-03-08	4-08-09	2 x 4		11-06 4-08-09	153.9 99.33		
	1	T81 Half Hip	10 /12	18-03-08	5-10-09	2 x 4		11-06 5-10-09	76.73 48.33		
	1	T82 Half Hip	10 /12	18-03-08	7-00-09	2 x 4		11-06 7-00-09	84.36 53.00		
	1	T83 Half Hip	10 /12	18-03-08	6-07-04	2 x 4		3-03-11 6-07-04	81.92 53.50		
	1	T84 Half Hip	10 /12	18-03-08	7-07-04	2 x 4		3-03-11 7-07-04	86.37 55.00		
	1	T85 Half Hip	10 /12	18-03-08	8-07-04	2 x 4		3-03-11 8-07-04	95.61 61.33		
	1	T86 Half Hip	10 /12	18-03-08	9-07-04	2 x 4		3-03-11 9-07-04	107.57 67.67		
	10	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	3	J2 Jack-Open	7 /12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	51.89 32.00		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1827.15** BFT. TOTAL WEIGHT OF ALL TRSSES **2863.96** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **11**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401426
Location:	ST CATHERINES	Ref #	
Model:	TH-2MOD	Page:	1 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 35	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

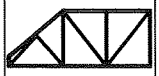
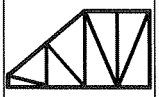
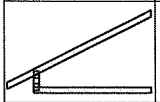
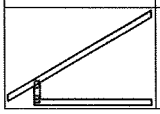
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T25Z4 Common Girder	6 /12	**18-03-00**	5-08-12	2 x 4 2 x 6		1-02-00 1-02-00	168.25 104.00		
	2	T26 Common	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	145.14 94.67		
	1	G26 GABLE	6 /12	**18-03-00**	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	1 2-ply	T34 Half Hip Girder	10 /12	**17-09-08**	4-01-04	2 x 4	1-03-08	1-07-11 4-01-04	148.81 94.33		
	1	T35 Half Hip	10 /12	**17-09-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	74.25 46.83		
	1	T36 Half Hip	10 /12	**17-09-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	78.64 50.00		
	1	T37 Half Hip	10 /12	**17-09-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	86.04 55.00		
	1	T38 Half Hip	10 /12	**17-09-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	91.24 57.67		
	1	T39 Half Hip	10 /12	**17-09-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	99.01 62.17		
	2	T40 Half Hip	10 /12	**17-09-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	207.5 130.33		
	3	T52 Monopitch	10 /12 6 /12	**10-09-08**	10-07-10	2 x 4	1-03-08	1-07-11 10-07-10	192.92 122.50		
	3	T53 Monopitch	10 /12	**10-09-08**	10-07-10	2 x 4		1-07-11 10-07-10	168.43 108.00		
	3	T67TCX Monopitch	5 /12	**1-08-00**	2-00-10	2 x 4	1-03-08	1-00-09 1-08-14	34.58 27.00		
	1	G78 GABLE	6 /12	**19-04-08**	10-10-04	2 x 4	1-03-08	1-02-00 10-10-04	99.84 64.33		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401426
Location:	ST CATHERINES	Ref #	
Model:	TH-2MOD	Page:	2 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 35	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G79 GABLE	0 /12	24-03-08	10-07-10	2 x 4		10-07-10 10-07-10	227.57 143.33		
	1 2-ply	T87 Half Hip Girder	10 /12	17-09-08	4-08-09	2 x 4 2 x 6		1-04-06 4-08-09	173.83 104.00		
	1	T88 Half Hip	10 /12	17-09-08	5-10-09	2 x 4		1-04-06 5-10-09	75.53 47.67		
	1	T89 Half Hip	10 /12	17-09-08	7-00-09	2 x 4		1-04-06 7-00-09	83.47 52.33		
	1	T90 Half Hip	10 /12	17-09-08	6-07-04	2 x 4		1-07-11 6-07-04	79.03 51.00		
	1	T91 Half Hip	10 /12	17-09-08	7-07-04	2 x 4		1-07-11 7-07-04	86.7 55.67		
	1	T92 Half Hip	10 /12	17-09-08	8-07-04	2 x 4		1-07-11 8-07-04	94.24 62.17		
	1	T93 Half Hip	10 /12	17-09-08	9-07-04	2 x 4		1-07-11 9-07-04	99.98 63.67		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	3	J2 Jack-Open	7 /12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	51.89 32.00		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1773** BFT. TOTAL WEIGHT OF ALL TRSSES **2788.85** LBS

HARDWARE

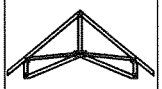
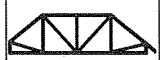
QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	
3	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **11**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401427
Location:	ST CATHERINES	Ref #	
Model:	TH-6E	Page:	1 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 36	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

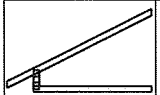
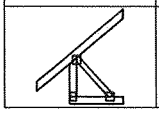
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G26 GABLE	6 /12	18-03-00	5-08-12	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	70.81 48.33		
	1	T72S Hip Girder	10 /12	15-05-00	4-09-06	2 x 4	1-03-08 1-03-08	1-07-11 1-11-11	74.78 48.67		
	1	T73A Hip	10 /12	15-02-00	6-05-06	2 x 4	1-03-08	1-07-11 3-02-03	69.05 44.17		
	3	T74 Scissor	10 /12 5 /12	8-11-00	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	125.83 83.00		
	2	T75 Common	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	140.22 93.33		
	1	G75 GABLE	10 /12	15-00-00	7-10-11	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	74.49 49.67		
	1 2-ply	T94 Hip Girder	10 /12	21-05-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	206.78 129.33		
	1 2-ply	T94A Hip Girder	10 /12	21-01-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 1-07-11	201.68 126.67		
	1	T95 Hip	10 /12	21-05-00	5-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	89.05 58.50		
	1	T95A Hip	10 /12	21-01-08	5-01-04	2 x 4	1-03-08	1-07-11 1-07-11	86.49 56.67		
	1	T96 Hip	10 /12	21-05-00	6-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	93.83 60.33		
	1	T96A Hip	10 /12	21-01-08	6-01-04	2 x 4	1-03-08	1-07-11 1-07-11	91.25 58.50		
	1	T97 Hip	10 /12	21-05-00	7-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	104.61 67.17		
	1	T97A Hip	10 /12	21-01-08	7-01-04	2 x 4	1-03-08	1-07-11 1-07-11	102.07 65.83		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200524
Project:	PASSAGE ON THE CANAL	Layout ID:	401427
Location:	ST CATHERINES	Ref #	
Model:	TH-6E	Page:	2 of 2
Lot #:		Date:	02/14/2019
Elevation:	B UNIT 36	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T98 Hip	10 /12	21-05-00	8-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	99.52 64.33		
	1	T98A Hip	10 /12	21-01-08	8-01-04	2 x 4	1-03-08	1-07-11 1-07-11	96.97 62.33		
	1	T99 Hip	10 /12	21-05-00	9-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	109.02 70.33		
	1	T99A Hip	10 /12	21-01-08	9-01-04	2 x 4	1-03-08	1-07-11 1-07-11	106.45 69.67		
	1	T100 Hip	10 /12	21-05-00	10-01-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	114.88 74.67		
	1	T100A Hip	10 /12	21-01-08	10-01-04	2 x 4	1-03-08	1-07-11 1-07-11	112.3 73.33		
	2	T101 Common	10 /12	21-05-00	10-06-12	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	217.85 136.00		
	1	T101A Common	10 /12	21-01-08	10-06-12	2 x 4	1-03-08	1-07-11 1-07-11	106.36 66.67		
	5	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	83.97 53.33		
	3	J6 Jack-Open	10 /12	2-02-00	3-09-06	2 x 4	1-03-08	1-11-11 3-09-06	34.05 24.50		

TOTAL # TRUSS= **36** TOTAL BFT OF ALL TRUSSES= **1685.33** BFT. TOTAL WEIGHT OF ALL TRSSES **2612.32** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **5**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	G1	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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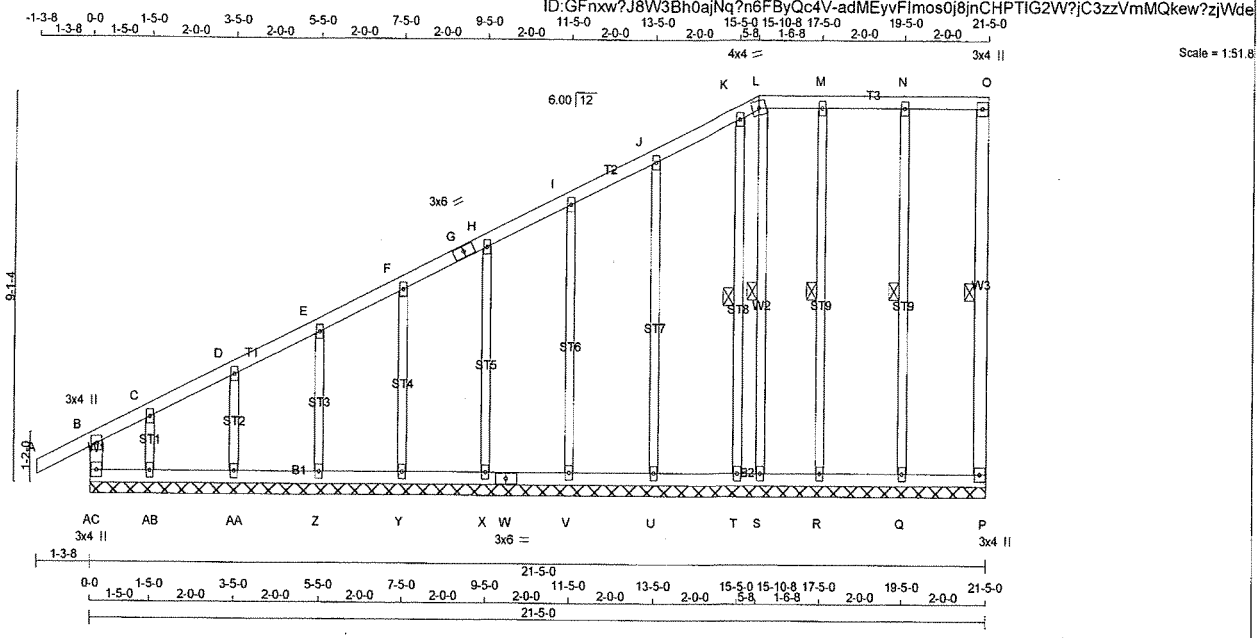


TOTAL WEIGHT = 4 X 115 = 462 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
AC- B	2x4	DRY	No.2	SPF
AC- W	2x4	DRY	No.2	SPF
W - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, F, H, I, J, K, M, N					
C	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	3.0	6.0	
L	TTW-m	MT20	4.0	4.0	
O	TMV+p	MT20	3.0	4.0	
P	BMV1+p	MT20	3.0	4.0	
Q, R, S, T, U, V, X, Y, Z, AA, AB					
Q	BMW1+w	MT20	2.0	4.0	
W	BS-t	MT20	3.0	6.0	
AC	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

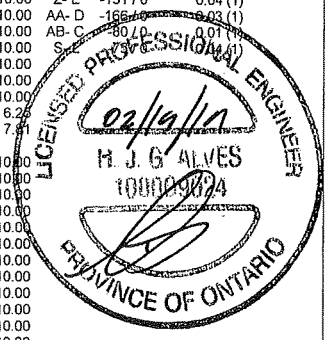
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF O-P, N-Q, M-R, K-T, L-S.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	Q- N	-169 / 0	0.09 (1)
B- C	-40 / 0	-77.7	-77.7	0.08 (1)	6.25	R- M	-135 / 0	0.07 (1)
C- D	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	T- K	-96 / 0	0.05 (1)
D- E	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	U- J	-159 / 0	0.19 (1)
E- F	-11 / 0	-77.7	-77.7	0.04 (1)	6.25	V- I	-155 / 0	0.12 (1)
F- G	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	X- H	-154 / 0	0.08 (1)
G- H	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	Y- F	-155 / 0	0.05 (1)
H- I	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	Z- E	-151 / 0	0.04 (1)
I- J	-5 / 0	-77.7	-77.7	0.04 (1)	10.00	AA- D	-166 / 0	0.03 (1)
J- K	-5 / 0	-77.7	-77.7	0.04 (1)	10.00	AB- C	-80 / 0	0.01 (1)
K- L	-4 / 0	-77.7	-77.7	0.02 (1)	10.00	S- L	-7 / 0	0.00 (1)
L- M	-1 / 0	-77.7	-77.7	0.03 (1)	10.00			
M- N	-1 / 0	-77.7	-77.7	0.04 (1)	10.00			
N- O	-1 / 0	-77.7	-77.7	0.04 (1)	10.00			
P- O	-68 / 0	0.0	0.0	0.01 (1)	6.25			
AC- B	-207 / 0	0.0	0.0	0.04 (1)	7.81			
AC-AB	0 / 20	-39.5	-39.5	0.04 (1)	10.00			
AB-AA	0 / 17	-39.5	-39.5	0.03 (2)	10.00			
AA- Z	0 / 13	-39.5	-39.5	0.03 (2)	10.00			
Z- Y	0 / 10	-39.5	-39.5	0.02 (3)	10.00			
Y- X	0 / 8	-39.5	-39.5	0.02 (3)	10.00			
X- W	0 / 6	-39.5	-39.5	0.02 (3)	10.00			
W- V	0 / 6	-39.5	-39.5	0.02 (3)	10.00			
V- U	0 / 4	-39.5	-39.5	0.03 (3)	10.00			
U- T	0 / 3	-39.5	-39.5	0.03 (3)	10.00			
T- S	0 / 2	-39.5	-39.5	0.02 (3)	10.00			
S- R	0 / 1	-39.5	-39.5	0.02 (3)	10.00			
R- Q	0 / 1	-39.5	-39.5	0.03 (3)	10.00			
Q- P	0 / 1	-39.5	-39.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.04/1.00 (AB-AC:1) , WB=0.19/1.00 (J-U:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (L) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM T1903780
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T6	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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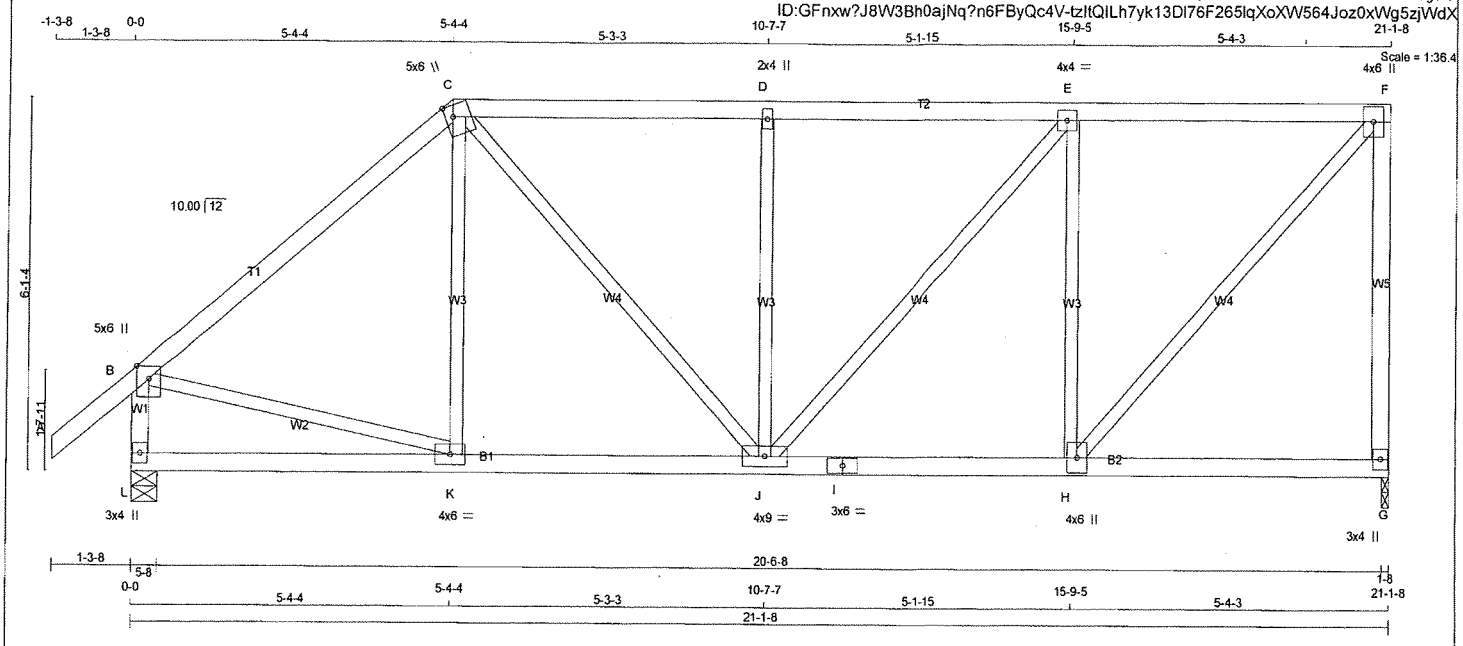


TOTAL WEIGHT = 2 X 96 = 191 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

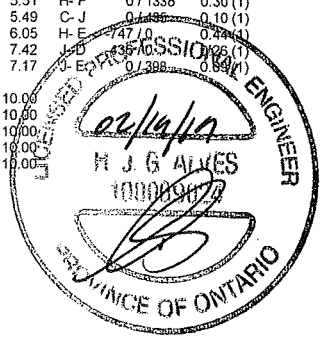
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-1144 / 0	-77.7	-77.7	0.45 (1)	5.36
C- D	-1170 / 0	-77.7	-77.7	0.31 (1)	5.51
D- E	-1170 / 0	-77.7	-77.7	0.32 (1)	5.49
E- F	-906 / 0	-77.7	-77.7	0.31 (1)	6.05
G- F	-1153 / 0	0.0	0.0	0.83 (1)	7.42
L- B	-1259 / 0	0.0	0.0	0.13 (1)	7.17
L- K	0 / 0	-39.5	-39.5	0.22 (3)	10.00
K- J	0 / 877	-39.5	-39.5	0.33 (2)	10.00
J- I	0 / 906	-39.5	-39.5	0.33 (2)	10.00
I- H	0 / 906	-39.5	-39.5	0.33 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 186	0.04 (3)
B- K	0 / 901	0.20 (1)
H- F	0 / 1338	0.30 (1)
C- J	0 / 435	0.10 (1)
H- E	-747 / 0	0.44 (1)
I- D	-435 / 0	0.26 (1)
J- E	0 / 398	0.09 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.83/1.00 (F-G:1) , BC=0.33/1.00 (J-K:2) , WB=0.44/1.00 (E-H:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM 7903781
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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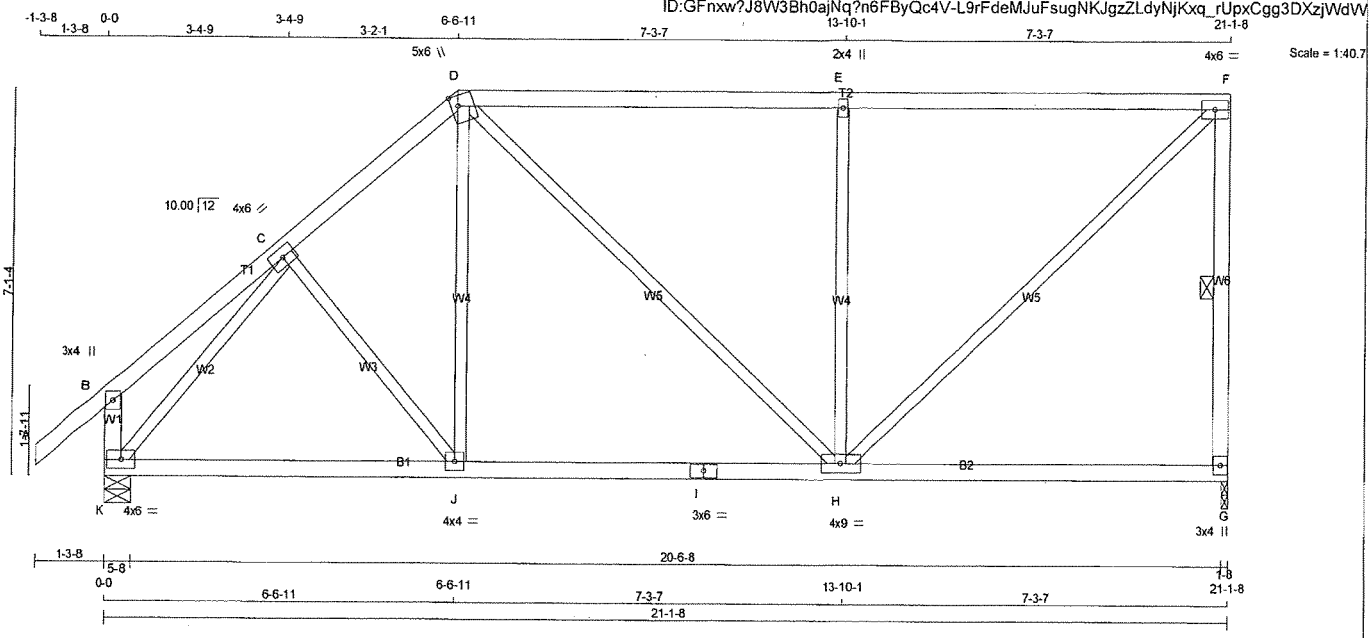

TOTAL WEIGHT = 2 X 95 = 191 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	0 / 88	0.02 (3)
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00	J- D	0 / 331	0.07 (2)
C- D	-1124 / 0	-77.7	-77.7	0.14 (1)	5.83	D- H	0 / 166	0.04 (1)
D- E	-973 / 0	-77.7	-77.7	0.78 (1)	4.91	H- E	-701 / 0	0.61 (1)
E- F	-974 / 0	-77.7	-77.7	0.78 (1)	4.91	H- F	0 / 1329	0.30 (1)
G- F	-1121 / 0	0.0	0.0	0.26 (1)	5.99	K- C	-1330 / 0	0.57 (1)
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 837	-39.5	-39.5	0.42 (2)	10.00			
J- I	0 / 852	-39.5	-39.5	0.48 (2)	10.00			
I- H	0 / 852	-39.5	-39.5	0.48 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.39 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78/1.00 (E-F:1) , BC=0.48/1.00 (H-J:2) , WB=0.61/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (F) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

DWG NO. TAM 190378Z
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T8	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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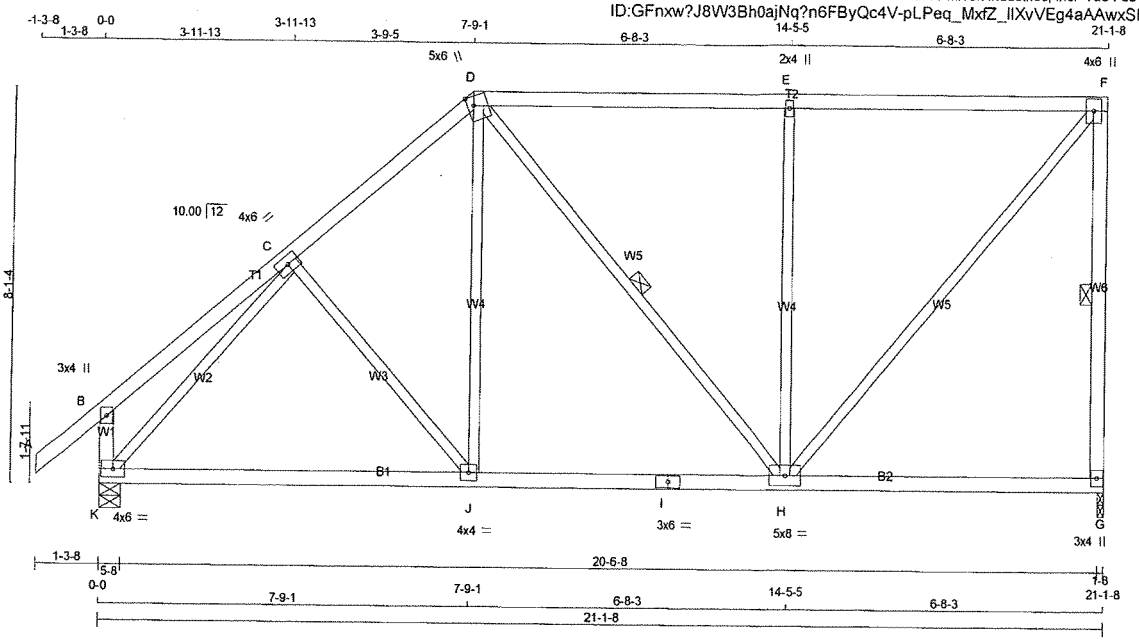


TOTAL WEIGHT = 2 X 100 = 201 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

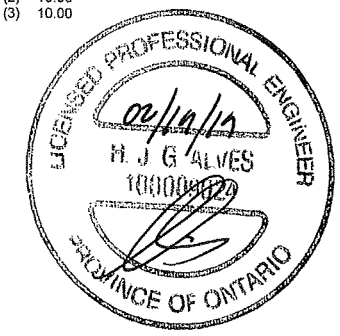
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00
C- D	-1072 / 0	-77.7	-77.7	0.20 (1)	5.85
D- E	-800 / 0	-77.7	-77.7	0.63 (1)	5.68
E- F	-800 / 0	-77.7	-77.7	0.63 (1)	5.68
G- F	-1128 / 0	0.0	0.0	0.33 (1)	5.98
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 849	-39.5	-39.5	0.50 (2)	10.00
J- I	0 / 810	-39.5	-39.5	0.50 (2)	10.00
I- H	0 / 810	-39.5	-39.5	0.50 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-66 / 57	0.04 (1)
J- D	0 / 418	0.09 (2)
D- H	-50 / 0	0.04 (3)
H- E	-643 / 0	0.81 (1)
H- F	0 / 1227	0.28 (1)
K- C	-1308 / 0	0.78 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.26")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.50/1.00 (J-K:2) , WB=0.81/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM 11905183
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T9	5	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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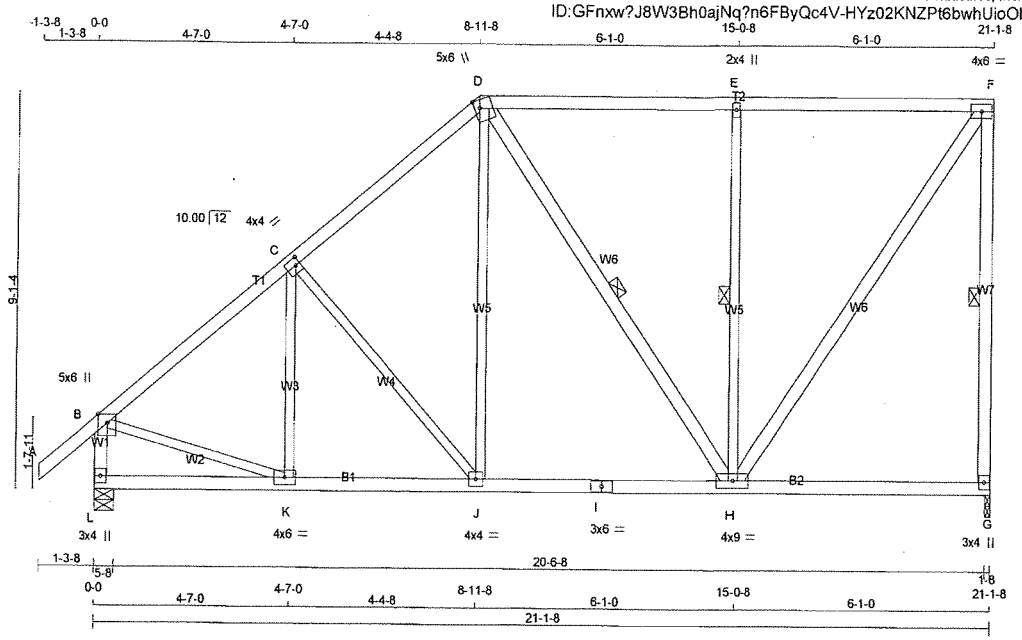


TOTAL WEIGHT = 5 X 115 = 573 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

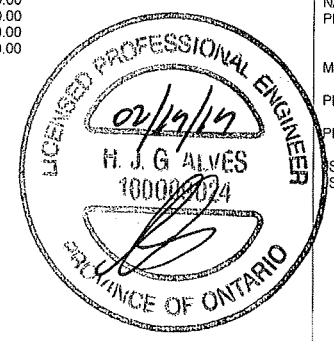
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-68 / 103	0.03 (1)
B- C	-1168 / 0	-77.7	-77.7	0.30 (1)	5.55	C- J	-281 / 0	0.24 (1)
C- D	-982 / 0	-77.7	-77.7	0.28 (1)	5.93	J- D	0 / 433	0.10 (2)
D- E	-668 / 0	-77.7	-77.7	0.51 (1)	6.25	D- H	-121 / 0	0.07 (1)
E- F	-668 / 0	-77.7	-77.7	0.51 (1)	6.25	H- E	-585 / 0	0.32 (1)
G- F	-1142 / 0	0.0	0.0	0.44 (1)	5.95	H- F	0 / 1171	0.19 (1)
L- B	-1271 / 0	0.0	0.0	0.14 (1)	7.14	B- K	0 / 953	0.21 (1)
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00			
K- J	0 / 917	-39.5	-39.5	0.27 (2)	10.00			
J- I	0 / 737	-39.5	-39.5	0.38 (2)	10.00			
I- H	0 / 737	-39.5	-39.5	0.38 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.38/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.81 (F) (INPUT = 0.90)
SI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T9037384
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 19 17:28:48 2019 Page 1

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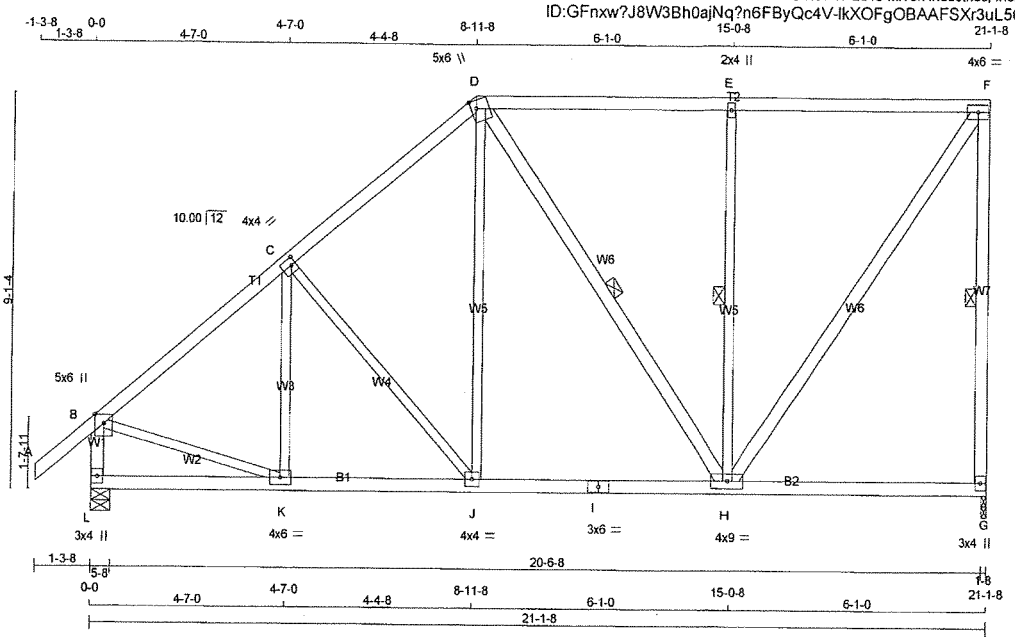


TOTAL WEIGHT = 2 X 115 = 229 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

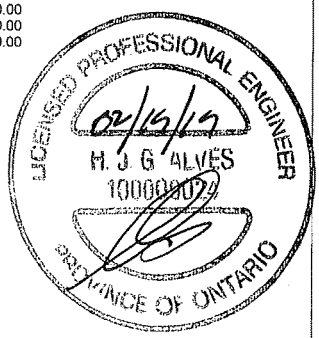
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES. (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-1168 / 0	-77.7	-77.7	0.30 (1)	5.55
C- D	-982 / 0	-77.7	-77.7	0.28 (1)	5.93
D- E	-668 / 0	-77.7	-77.7	0.51 (1)	6.25
E- F	-668 / 0	-77.7	-77.7	0.51 (1)	6.25
G- F	-1142 / 0	0.0	0.0	0.44 (1)	5.95
L- B	-1271 / 0	0.0	0.0	0.14 (1)	7.14
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00
K- J	0 / 917	-39.5	-39.5	0.27 (2)	10.00
J- I	0 / 737	-39.5	-39.5	0.38 (2)	10.00
I- H	0 / 737	-39.5	-39.5	0.38 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-68 / 103	0.03 (1)
C- J	-281 / 0	0.24 (1)
J- D	0 / 433	0.10 (2)
D- H	-121 / 0	0.07 (1)
H- E	-585 / 0	0.32 (1)
H- F	0 / 1171	0.19 (1)
B- K	0 / 953	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.38/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T1903795
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T13	2	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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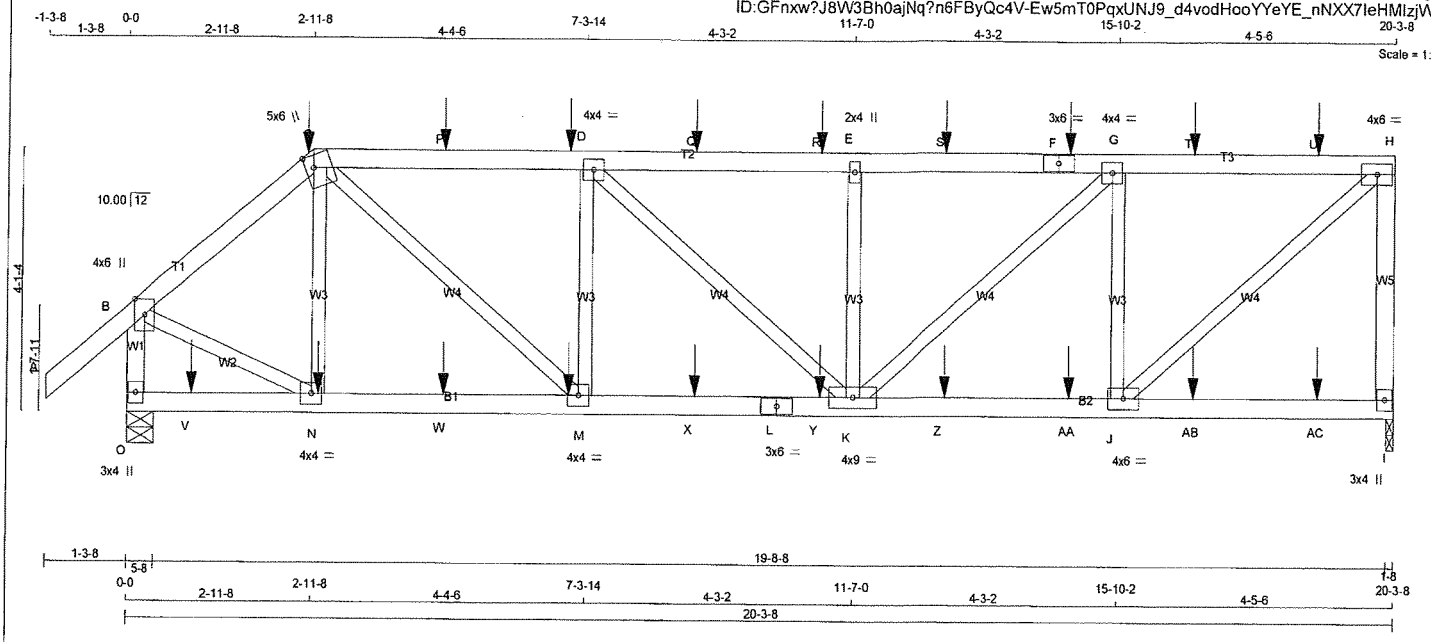


TOTAL WEIGHT = 4 X 86 = 345 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	1	12	SIDE(61.0)
L- I	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	1961	0	1961	0	0	1-8	1-8
O	2155	0	2155	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1503	688 / 0	362 / 0	0 / 0	0 / 0	453 / 0	0 / 0
O	1638	791 / 0	364 / 0	0 / 0	0 / 0	484 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00
B- C	-1919 / 0	-77.7	-77.7	0.08 (1)	6.24
C- P	-2574 / 0	-77.7	-77.7	0.19 (1)	5.45
P- D	-2574 / 0	-77.7	-77.7	0.19 (1)	5.45
D- Q	-2672 / 0	-77.7	-77.7	0.19 (1)	5.37
Q- R	-2672 / 0	-77.7	-77.7	0.19 (1)	5.37
R- E	-2672 / 0	-77.7	-77.7	0.19 (1)	5.37
E- S	-2672 / 0	-77.7	-77.7	0.22 (1)	5.33
S- F	-2672 / 0	-77.7	-77.7	0.22 (1)	5.33
F- G	-2672 / 0	-77.7	-77.7	0.22 (1)	5.33
G- T	-1908 / 0	-77.7	-77.7	0.21 (1)	6.07
T- U	-1908 / 0	-77.7	-77.7	0.21 (1)	6.07
U- H	-1908 / 0	-77.7	-77.7	0.21 (1)	6.07
I- H	-1845 / 0	0.0	0.0	0.25 (1)	7.81
O- B	-2083 / 0	0.0	0.0	0.12 (1)	7.68
O- V	0 / 0	-39.5	-39.5	0.08 (3)	10.00
V- N	0 / 0	-39.5	-39.5	0.08 (3)	10.00
N- W	0 / 1459	-39.5	-39.5	0.20 (2)	10.00
W- M	0 / 1459	-39.5	-39.5	0.20 (2)	10.00
M- X	0 / 2575	-39.5	-39.5	0.29 (2)	10.00
X- L	0 / 2575	-39.5	-39.5	0.29 (2)	10.00
L- Y	0 / 2575	-39.5	-39.5	0.29 (2)	10.00
Y- K	0 / 2575	-39.5	-39.5	0.29 (2)	10.00
K- Z	0 / 1908	-39.5	-39.5	0.28 (2)	10.00
Z-AA	0 / 1908	-39.5	-39.5	0.28 (2)	10.00
AA- J	0 / 1908	-39.5	-39.5	0.28 (2)	10.00
J-AB	0 / 0	-39.5	-39.5	0.13 (3)	10.00
AB-AC	0 / 0	-39.5	-39.5	0.13 (3)	10.00
AC- I	0 / 0	-39.5	-39.5	0.13 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- C	-350 / 4	0.05 (1)
B- N	0 / 1597	0.20 (1)
J- H	0 / 2512	0.31 (1)
C- M	0 / 1477	0.18 (1)
J- G	-1300 / 0	0.18 (1)
M- D	-676 / 0	0.09 (1)
K- G	0 / 1025	0.13 (1)
D- K	0 / 131	0.02 (2)
K- E	-497 / 6	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-100	-100	---	FRONT	VERT	TOTAL	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
D	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
F	15-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
M	7-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
N	3-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/19/19
H J G ALVES
100091704
PROVINCE OF ONTARIO

DWG NO. TAM T1405186
STRUCTURAL
COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.25/1.00 (H-I:1) , BC=0.29/1.00 (K-M:2) , WB=0.31/1.00 (H-J:1) , SSI=0.14/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

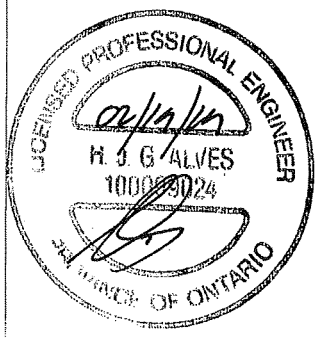
JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T13	2	2	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	5-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
Q	9-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
R	11-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
S	13-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
T	17-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
U	19-0-12	-94	-94	—	FRONT	VERT	TOTAL	—	—
V	1-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
W	5-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
X	9-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Y	11-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
Z	13-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
AA	15-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
AB	17-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—
AC	19-0-12	-56	-71	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM 1903786
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T14	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss. Burlington

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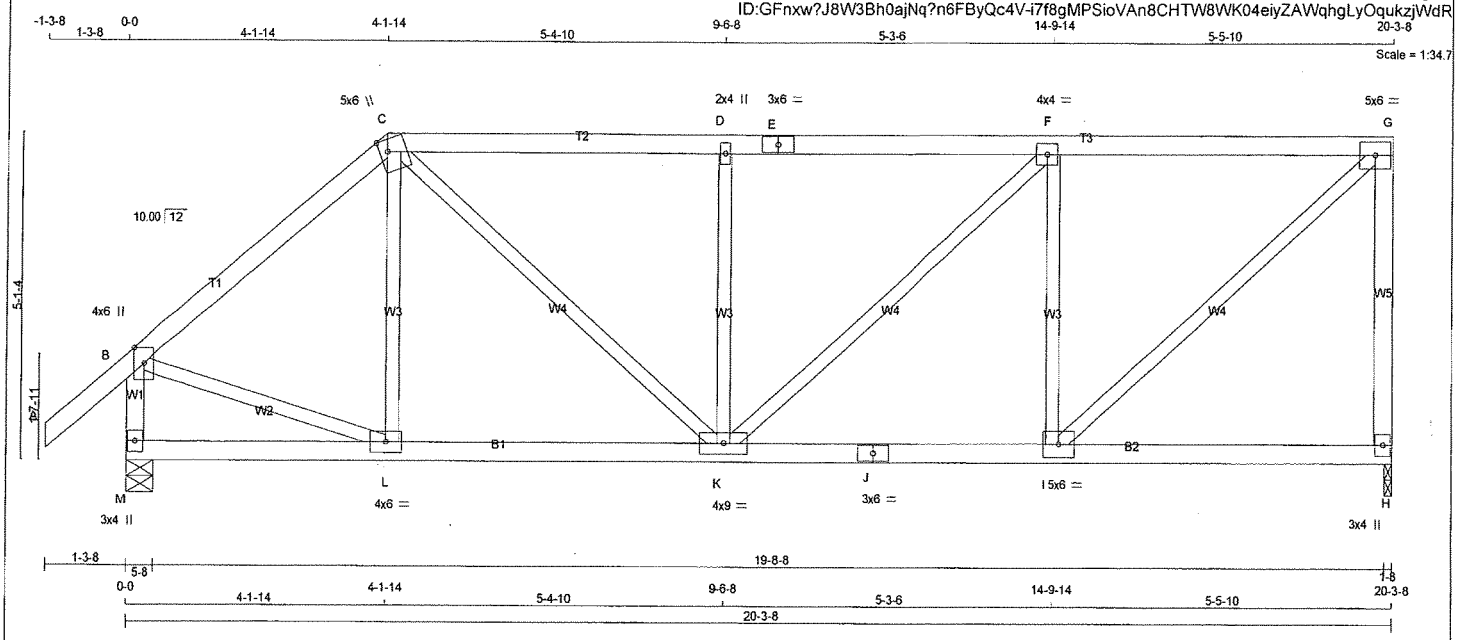


TOTAL WEIGHT = 2 X 87 = 175 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1189	0	1189	0	0	1-8	1-8
M	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
M	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

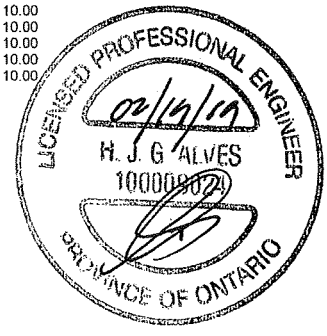
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
B- C	-1100 / 0	-77.7	-77.7	0.26 (1)	5.71	
C- D	-1307 / 0	-77.7	-77.7	0.33 (1)	5.26	
D- E	-1308 / 0	-77.7	-77.7	0.34 (1)	5.23	
E- F	-1308 / 0	-77.7	-77.7	0.34 (1)	5.23	
F- G	-1056 / 0	-77.7	-77.7	0.33 (1)	5.69	
H- G	-1102 / 0	0.0	0.0	0.49 (1)	7.54	
M- B	-1234 / 0	0.0	0.0	0.13 (1)	7.23	
M- L	0 / 0	-39.5	-39.5	0.17 (3)	10.00	
L- K	0 / 842	-39.5	-39.5	0.28 (2)	10.00	
K- J	0 / 1056	-39.5	-39.5	0.35 (2)	10.00	
J- I	0 / 1056	-39.5	-39.5	0.35 (2)	10.00	
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-45 / 121	0.03 (3)
B- L	0 / 881	0.20 (1)
I- G	0 / 1407	0.32 (1)
C- K	0 / 624	0.14 (1)
I- F	-689 / 0	0.26 (1)
K- D	-446 / 0	0.17 (1)
K- F	0 / 340	0.08 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.49/1.00 (G-H:1) , BC=0.35/1.00 (I-K:2) , WB=0.32/1.00 (G-I:1) , SSI=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM T1403787
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T15	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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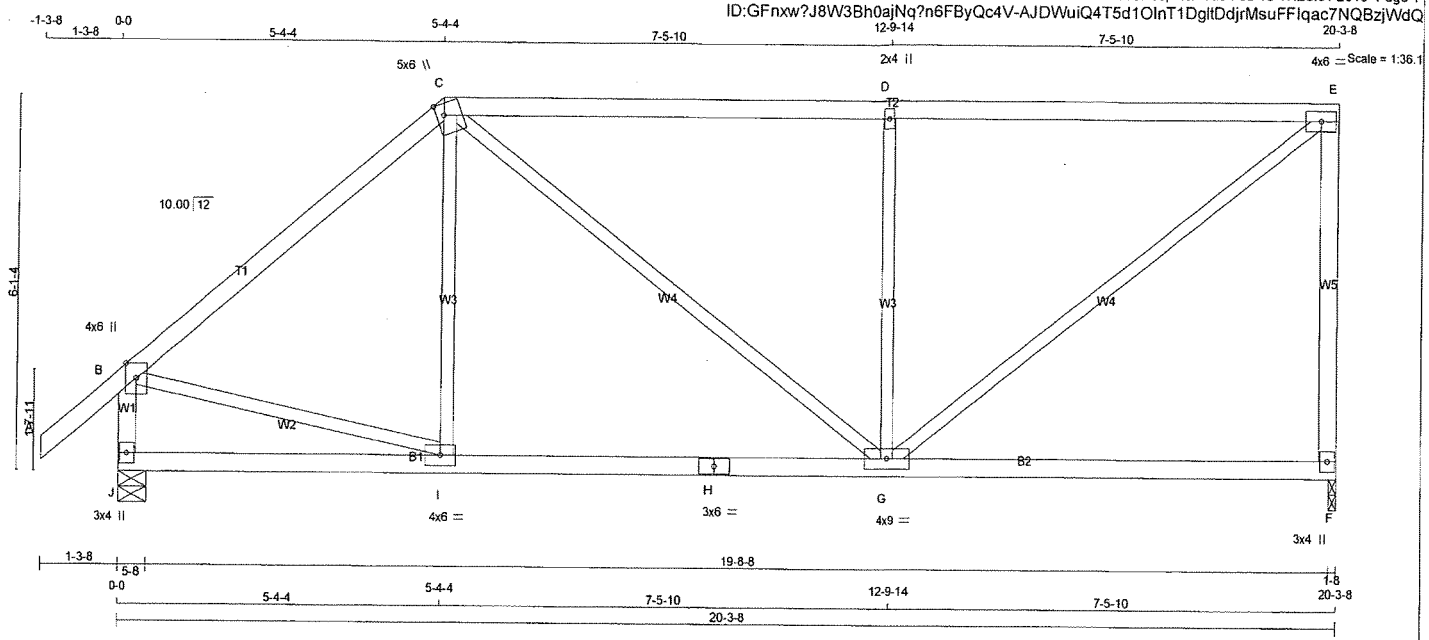


TOTAL WEIGHT = 2 X 86 = 172 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1189	0	1189	0	0	1-8	1-8
J	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

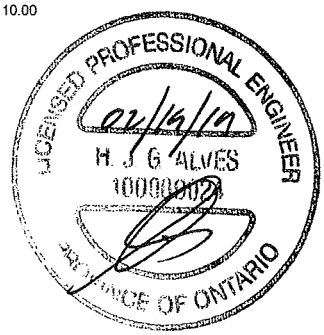
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
B- C	-1090 / 0	-77.7	-77.7	0.45 (1)	5.47	
C- D	-1097 / 0	-77.7	-77.7	0.84 (1)	4.54	
D- E	-1097 / 0	-77.7	-77.7	0.84 (1)	4.54	
F- E	-1071 / 0	0.0	0.0	0.77 (1)	7.62	
J- B	-1215 / 0	0.0	0.0	0.13 (1)	7.26	
J- I	0 / 0	-39.5	-39.5	0.27 (3)	10.00	
I- H	0 / 837	-39.5	-39.5	0.52 (2)	10.00	
H- G	0 / 837	-39.5	-39.5	0.52 (2)	10.00	
G- F	0 / 0	-39.5	-39.5	0.42 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 224	0.05 (3)
C- G	0 / 330	0.07 (1)
G- D	-719 / 0	0.42 (1)
G- E	0 / 1387	0.31 (1)
B- I	0 / 859	0.19 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.52/1.00 (G-I:2) , WB=0.42/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1403788
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T16	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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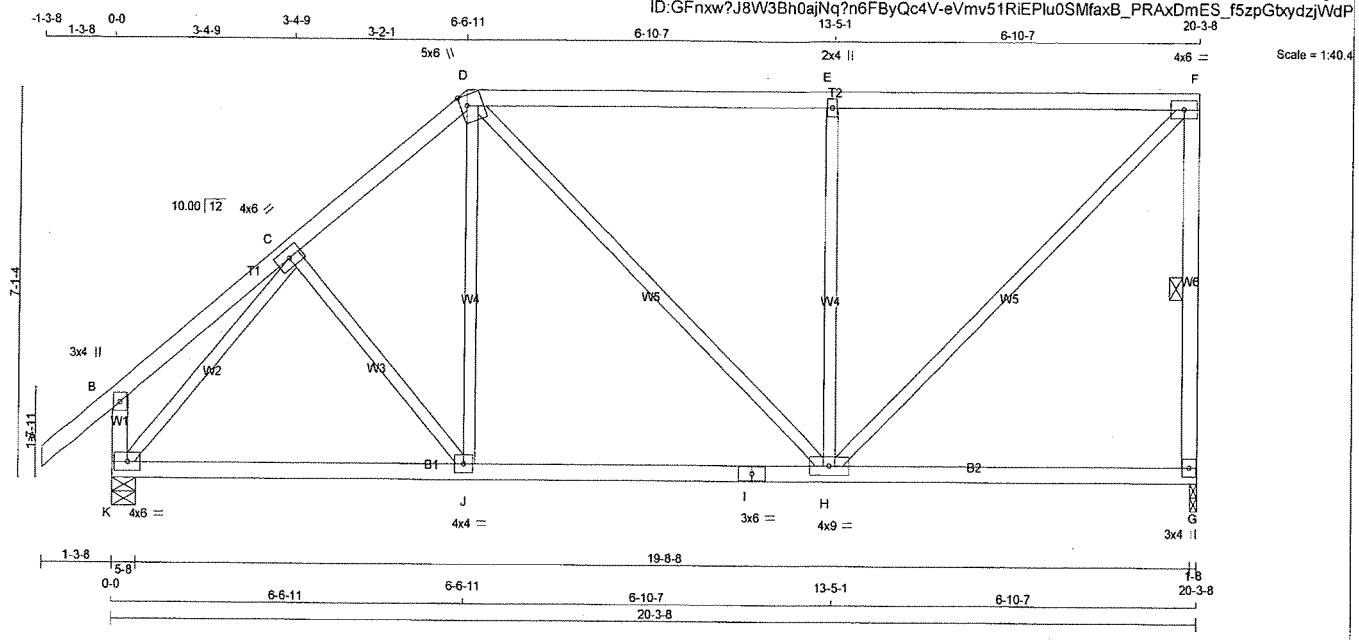


TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

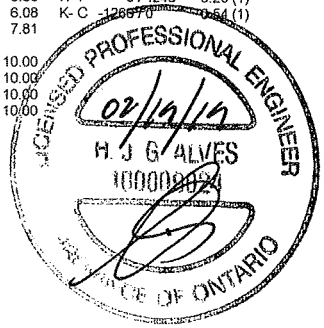
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	0 / 81	0.02 (3)
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00	J- D	0 / 332	0.07 (2)
C- D	-1062 / 0	-77.7	-77.7	0.14 (1)	5.96	D- H	0 / 117	0.03 (1)
D- E	-887 / 0	-77.7	-77.7	0.67 (1)	5.36	H- E	-661 / 0	0.58 (1)
E- F	-887 / 0	-77.7	-77.7	0.68 (1)	5.36	H- F	0 / 1246	0.28 (1)
G- F	-1078 / 0	0.0	0.0	0.25 (1)	6.08	K- C	-1266 / 0	0.54 (1)
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 797	-39.5	-39.5	0.41 (2)	10.00			
J- I	0 / 804	-39.5	-39.5	0.43 (2)	10.00			
I- H	0 / 804	-39.5	-39.5	0.43 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.34 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.43/1.00 (H-J:2) , WB=0.58/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM 1903789
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T17	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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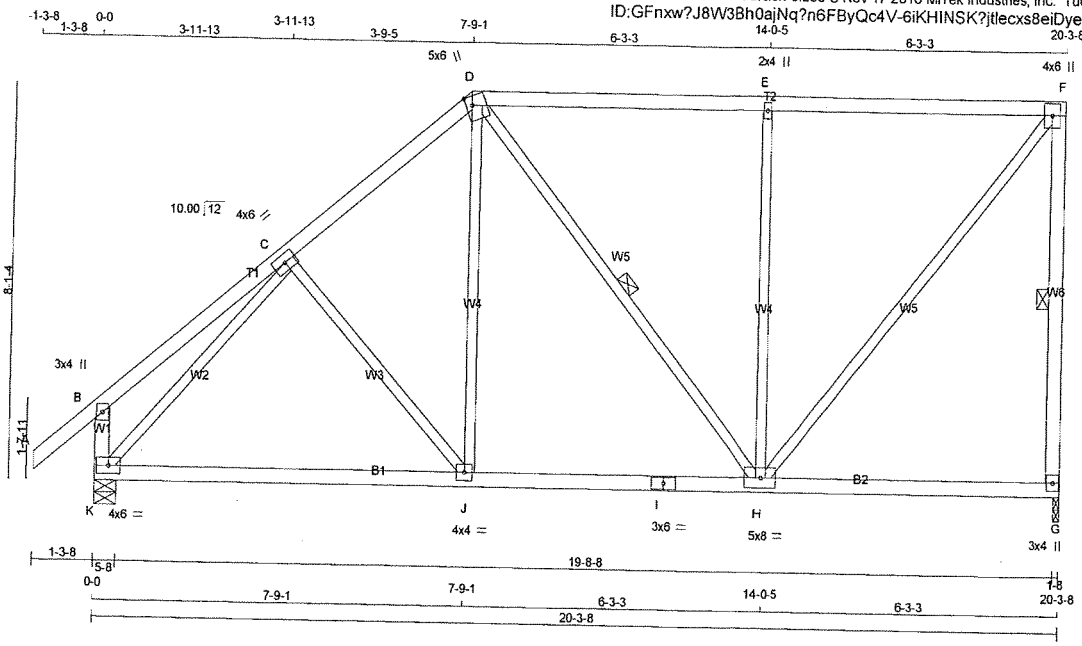


TOTAL WEIGHT = 2 X 98 = 196 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

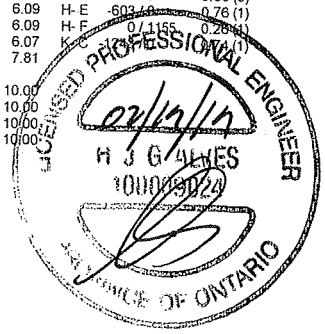
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00		C- J	-77 / 51	0.05 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00		J- D	0 / 420	0.09 (2)
C- D	-1008 / 0	-77.7	-77.7	0.20 (1)	5.98		D- H	-74 / 0	0.05 (3)
D- E	-725 / 0	-77.7	-77.7	0.54 (1)	6.09		H- E	-603 / 0	0.76 (1)
E- F	-726 / 0	-77.7	-77.7	0.54 (1)	6.09		H- F	0 / 1155	0.28 (1)
G- F	-1085 / 0	0.0	0.0	0.32 (1)	6.07		K- C	-1274 / 0	0.71 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81				
K- J	0 / 807	-39.5	-39.5	0.49 (2)	10.00				
J- I	0 / 761	-39.5	-39.5	0.49 (2)	10.00				
I- H	0 / 761	-39.5	-39.5	0.49 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.25 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 939 (0.26")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.49/1.00 (J-K:2) , WB=0.76/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

DWG NO. TAM T1903790
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T18A	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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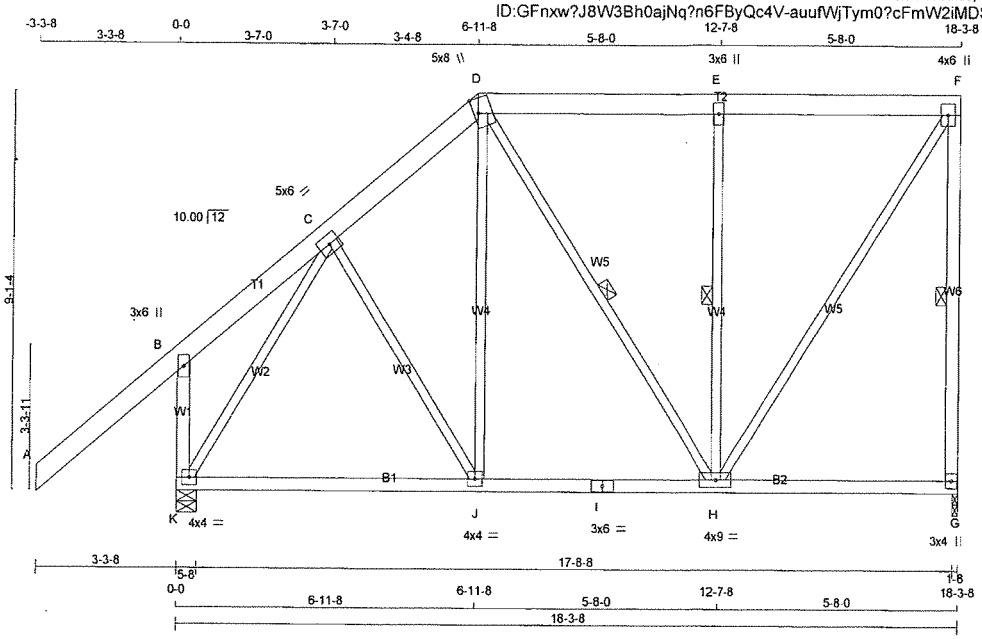


TOTAL WEIGHT = 2 X 118 = 237 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1047	0	1047	0	0	1-8	1-8
K	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	802	369 / 0	192 / 0	0 / 0	0 / 0	241 / 0	0 / 0
K	1022	540 / 0	192 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

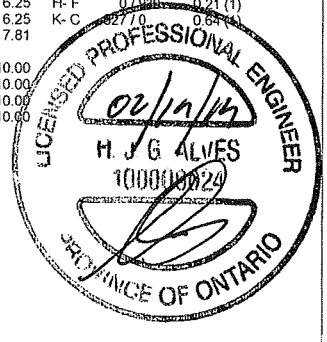
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00	C- J	0 / 180	0.04 (2)
B- C	-89 / 0	-77.7	-77.7	0.31 (1)	6.25	J- D	0 / 244	0.06 (3)
C- D	-674 / 0	-77.7	-77.7	0.09 (1)	6.25	D- H	-57 / 0	0.04 (3)
D- E	-509 / 0	-77.7	-77.7	0.18 (1)	6.25	H- E	-533 / 0	0.28 (1)
E- F	-509 / 0	-77.7	-77.7	0.18 (1)	6.25	H- F	0 / 830	0.21 (1)
G- F	-952 / 0	0.0	0.0	0.35 (1)	6.25	K- C	-927 / 0	0.64 (4)
K- B	-541 / 0	0.0	0.0	0.09 (1)	7.81			
K- J	0 / 427	-39.5	-39.5	0.37 (3)	10.00			
J- I	0 / 521	-39.5	-39.5	0.38 (2)	10.00			
I- H	0 / 521	-39.5	-39.5	0.38 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.38/1.00 (H-J:2) , WB=0.64/1.00 (C-K:1) , SSI=0.17/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.23 (F) (INPUT = 1.00)

DWG NO. TAM 7903791
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T19A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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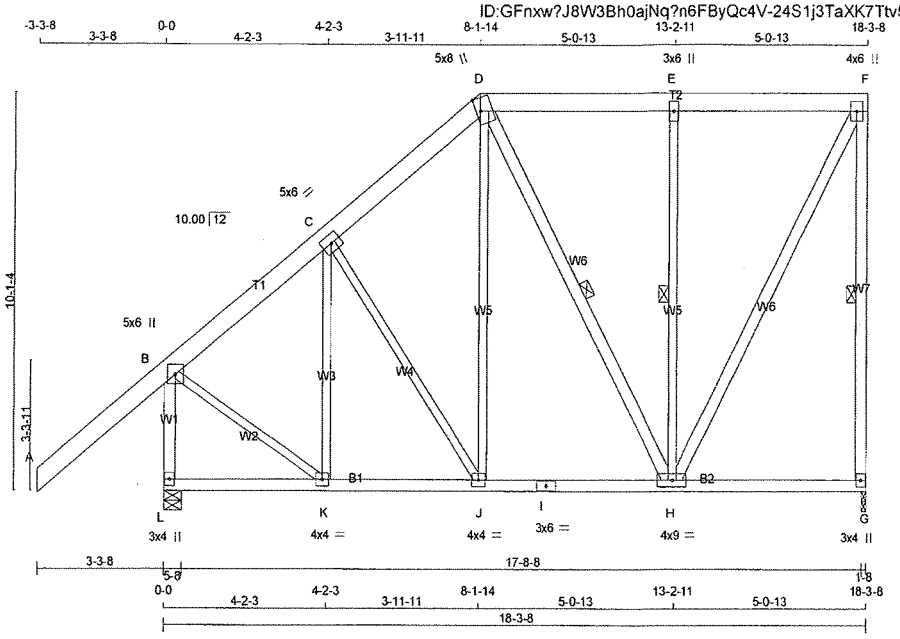


TOTAL WEIGHT = 3 X 134 = 401 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1047	0	1047	0	0	1-8	1-8
L	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	802	369 / 0	192 / 0	0 / 0	0 / 0	241 / 0	0 / 0
L	1022	540 / 0	192 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

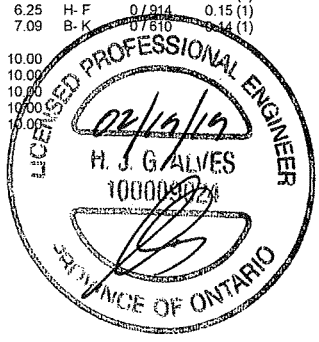
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00	K- C	-163 / 23	0.12 (1)
B- C	-721 / 0	-77.7	-77.7	0.31 (1)	6.25	C- J	-53 / 0	0.06 (2)
C- D	-629 / 0	-77.7	-77.7	0.10 (1)	6.25	J- D	0 / 253	0.06 (3)
D- E	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	D- H	-120 / 0	0.08 (1)
E- F	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	H- E	-474 / 0	0.32 (1)
G- F	-966 / 0	0.0	0.0	0.46 (1)	6.25	H- F	0 / 914	0.15 (1)
L- B	-1297 / 0	0.0	0.0	0.22 (1)	7.09	B- K	0 / 610	0.14 (1)
L- K	0 / 0	-39.5	-39.5	0.12 (3)	10.00			
K- J	0 / 503	-39.5	-39.5	0.19 (2)	10.00			
J- I	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
I- H	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.46/1.00 (F-G:1) , BC=0.25/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM 9903792
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T20A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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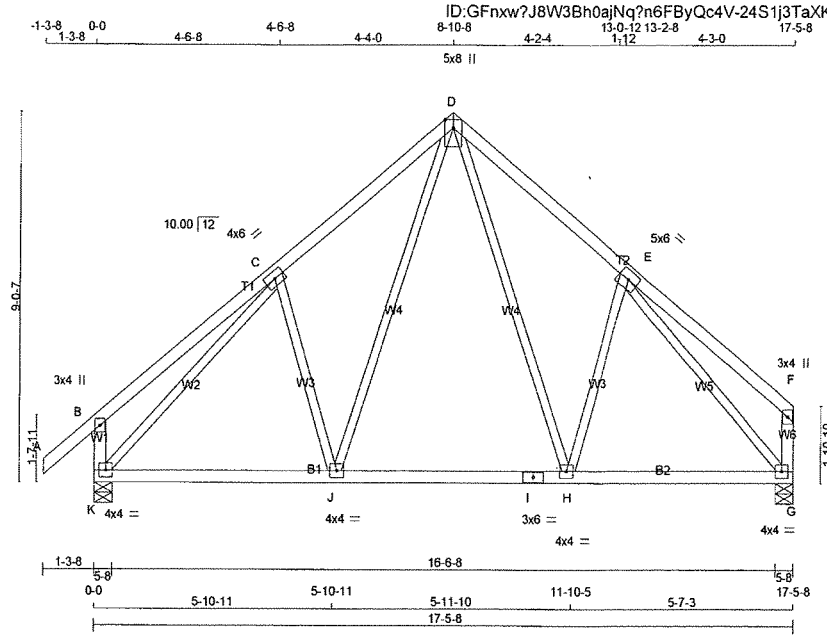


TOTAL WEIGHT = 84 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	5.0	8.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1131	0	1131	0	0	5-8	5-8
G	1023	0	1023	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	857	423 / 0	183 / 0	0 / 0	0 / 0	251 / 0	0 / 0
G	782	365 / 0	183 / 0	0 / 0	0 / 0	234 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

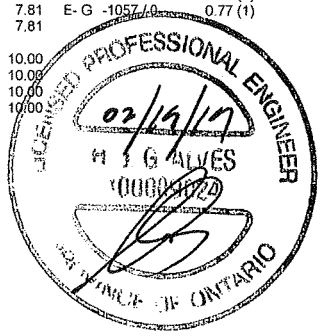
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM (PLF)	TO	LC1 MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-179 / 26	0.08 (1)
B- C	0 / 26	-77.7	-77.7	0.25 (1)	10.00	J- D	0 / 452	0.10 (1)
C- D	-875 / 0	-77.7	-77.7	0.20 (1)	6.25	K- C	-1066 / 0	0.84 (1)
D- E	-849 / 0	-77.7	-77.7	0.19 (1)	6.25	H- E	-131 / 48	0.06 (1)
E- F	0 / 26	-77.7	-77.7	0.23 (1)	10.00	D- H	0 / 393	0.09 (1)
K- B	-238 / 0	0.0	0.0	0.03 (1)	7.81	E- G	-1057 / 0	0.77 (1)
G- F	-119 / 0	0.0	0.0	0.01 (1)	7.81			
K- J	0 / 706	-39.5	-39.5	0.33 (2)	10.00			
J- I	0 / 508	-39.5	-39.5	0.31 (2)	10.00			
I- H	0 / 508	-39.5	-39.5	0.31 (2)	10.00			
H- G	0 / 673	-39.5	-39.5	0.31 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.33/1.00 (J-K:2) , WB=0.84/1.00 (C-K:1) , SSI=0.15/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.28 (C) (INPUT = 1.00)

DWG NO. TAM T7903793
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T20X	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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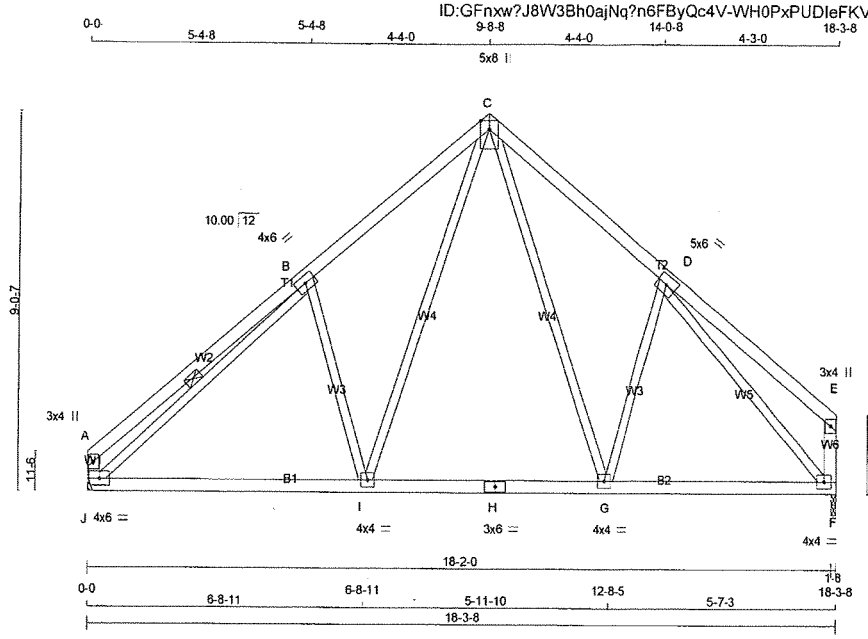


TOTAL WEIGHT = 5 X 84 = 420 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+p	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

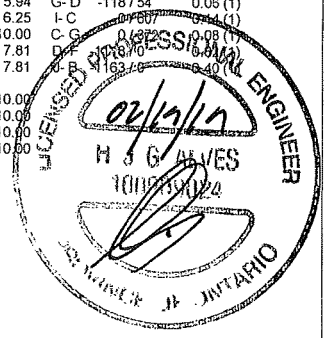
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 27	-77.7	-77.7	0.30 (1)	10.00	B- I	-307 / 0	0.15 (1)
B- C	-1010 / 0	-77.7	-77.7	0.24 (1)	5.94	G- D	-118 / 54	0.06 (1)
C- D	-905 / 0	-77.7	-77.7	0.19 (1)	6.25	I- C	0 / 607	0.44 (1)
D- E	0 / 26	-77.7	-77.7	0.23 (1)	10.00	C- G	0 / 372	0.08 (1)
F- E	-119 / 0	0.0	0.0	0.01 (1)	7.81	D- F	-163 / 0	0.40 (1)
J- A	-162 / 0	0.0	0.0	0.02 (1)	7.81	G- B	-163 / 0	0.40 (1)
J- I	0 / 843	-39.5	-39.5	0.41 (2)	10.00			
I- H	0 / 558	-39.5	-39.5	0.37 (2)	10.00			
H- G	0 / 558	-39.5	-39.5	0.37 (2)	10.00			
G- F	0 / 712	-39.5	-39.5	0.30 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.41/1.00 (I-J:2) , WB=0.82/1.00 (D-F:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.32 (B) (INPUT = 1.00)

DWG NO. TAM 71903794
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T25Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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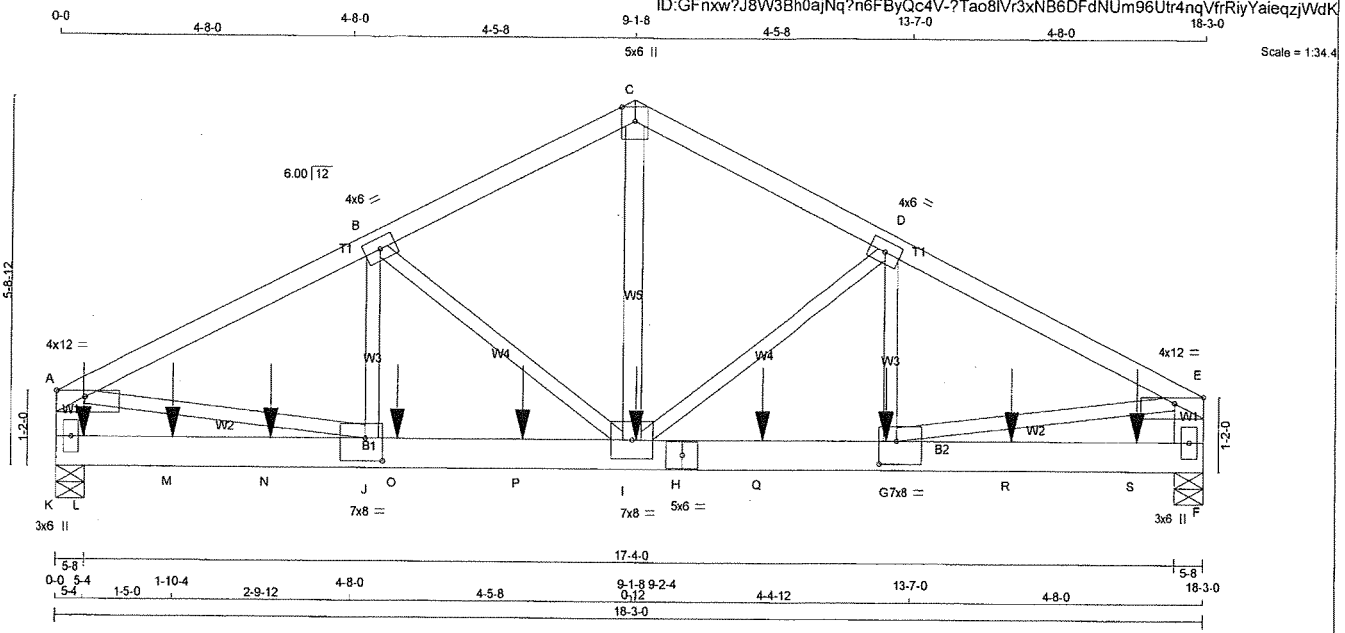


TOTAL WEIGHT = 2 X 84 = 168 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	3	2	SIDE(89.1)
F- E	3	2	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	11	SIDE(183.1)
H- F	2	11	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	6	SIDE(25.2)
2x3	1	6	
I- C	1	6	SIDE(86.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	4.0	12.0	1.25	Edge
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMVW-p	MT20	4.0	12.0	Edge	5.50
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	4.25	3.25
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	7.0	8.0		
J	BMWW-t	MT20	7.0	8.0	4.25	3.25
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	6629	0	6629	0	0	5-8	5-8
F	5845	0	5845	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	5039	2436 / 0	1117 / 0	0 / 0	0 / 0	1486 / 0	0 / 0
F	4443	2150 / 0	983 / 0	0 / 0	0 / 0	1310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

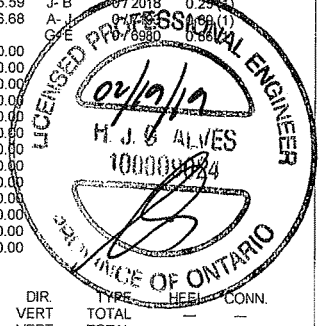
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-7938 / 0	-77.7	-77.7	0.48 (1)	3.20	I- C	0 / 4970	0.44 (1)
B- C	-5868 / 0	-77.7	-77.7	0.23 (1)	3.88	I- D	-2119 / 0	0.48 (1)
C- D	-5868 / 0	-77.7	-77.7	0.23 (1)	3.88	G- D	0 / 1743	0.22 (1)
D- E	-7702 / 0	-77.7	-77.7	0.46 (1)	3.27	B- I	-2388 / 0	0.55 (1)
K- A	-4828 / 0	0.0	0.0	0.17 (1)	6.59	J- B	0 / 2018	0.29 (1)
F- E	-4689 / 0	0.0	0.0	0.17 (1)	6.68	A- J	0 / 4752	0.69 (1)
						G- E	0 / 6980	0.86
K- L	0 / 0	-39.5	-39.5	0.66 (1)	10.00			
L- M	0 / 0	-39.5	-39.5	0.66 (1)	10.00			
M- N	0 / 0	-39.5	-39.5	0.66 (1)	10.00			
N- J	0 / 0	-39.5	-39.5	0.66 (1)	10.00			
J- O	0 / 7108	-39.5	-39.5	0.89 (1)	10.00			
O- P	0 / 7108	-39.5	-39.5	0.89 (1)	10.00			
P- I	0 / 7108	-39.5	-39.5	0.89 (1)	10.00			
I- H	0 / 6897	-39.5	-39.5	0.73 (1)	10.00			
H- Q	0 / 6897	-39.5	-39.5	0.73 (1)	10.00			
Q- G	0 / 6897	-39.5	-39.5	0.73 (1)	10.00			
G- R	0 / 0	-39.5	-39.5	0.47 (1)	10.00			
R- S	0 / 0	-39.5	-39.5	0.47 (1)	10.00			
S- F	0 / 0	-39.5	-39.5	0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	13-2-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
I	9-2-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
L	5-4	-1044	-1044	---	FRONT	VERT	TOTAL	---	---
M	1-10-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
N	3-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
O	5-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
P	7-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
Q	11-2-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
R	15-2-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
S	17-2-4	-1032	-1032	---	FRONT	VERT	TOTAL	---	---



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.48/1.00 (A-B:1) , BC=0.89/1.00 (I-J:1) , WB=0.89/1.00 (A-J:1) , SSI=0.60/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (A) (INPUT = 0.90)
JSI METAL= 0.67 (G) (INPUT = 1.00)

DWG NO. TAM T1903795
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T25Z3	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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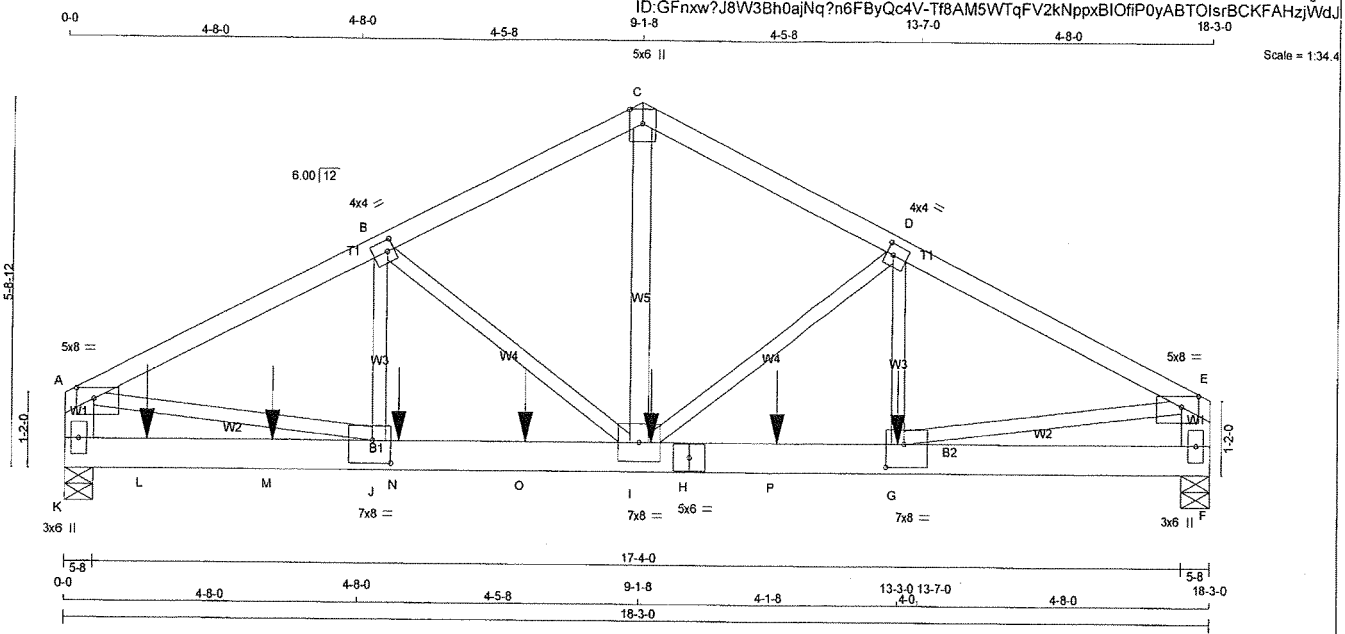


TOTAL WEIGHT = 2 X 84 = 168 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	3	SIDE(423.8)
B- J	1	3	SIDE(86.2)
2x3	1	6	
I- C	1	6	SIDE(86.3)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	2.00	3.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-p	MT20	5.0	8.0	2.00	3.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	4.25	3.50
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	7.0	8.0		
J	BMWW-t	MT20	7.0	8.0	4.25	3.50
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	5769	0	5769	0	0	5-8	5-8
F	4891	0	4891	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4382	2132 / 0	964 / 0	0 / 0	0 / 0	1287 / 0	0 / 0
F	3709	1822 / 0	807 / 0	0 / 0	0 / 0	1080 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-7782 / 0	-77.7	-77.7	0.46 (1)	3.25	I- C	0 / 5093	0.45 (1)
B- C	-6005 / 0	-77.7	-77.7	0.24 (1)	3.83	I- D	-2122 / 0	0.49 (1)
C- D	-6005 / 0	-77.7	-77.7	0.24 (1)	3.83	G- D	0 / 4747	0.29 (1)
D- E	-7841 / 0	-77.7	-77.7	0.47 (1)	3.23	B- I	-2054 / 0	0.47 (1)
K- A	-4736 / 0	0.0	0.0	0.17 (1)	6.65	J- B	0 / 4875	0.29 (1)
F- E	-4771 / 0	0.0	0.0	0.17 (1)	6.63	A- J	0 / 7056	0.87 (1)
						G- F	0 / 7106	0.88 (1)
K- L	0 / 0	-39.5	-39.5	0.49 (1)	10.00			
L- M	0 / 0	-39.5	-39.5	0.49 (1)	10.00			
M- J	0 / 0	-39.5	-39.5	0.49 (1)	10.00			
J- N	0 / 6969	-39.5	-39.5	0.78 (1)	10.00			
N- O	0 / 6969	-39.5	-39.5	0.78 (1)	10.00			
O- I	0 / 6969	-39.5	-39.5	0.78 (1)	10.00			
I- H	0 / 7022	-39.5	-39.5	0.77 (1)	10.00			
H- P	0 / 7022	-39.5	-39.5	0.77 (1)	10.00			
P- G	0 / 7022	-39.5	-39.5	0.77 (1)	10.00			
G- F	0 / 0	-39.5	-39.5	0.07 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
G	13-3-0	-2326	-2326	---	BACK	VERT	TOTAL	---	---
I	9-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
L	1-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
M	3-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
N	5-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
O	7-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---
P	11-3-12	-1032	-1032	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.78/1.00 (I-J:1) , WB=0.88/1.00 (E-G:1) , SSI=0.51/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

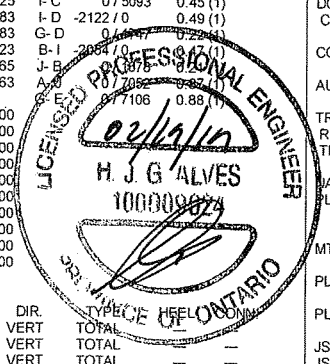
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.67 (H) (INPUT = 1.00)



DWG NO. TAM T1903796
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T25Z4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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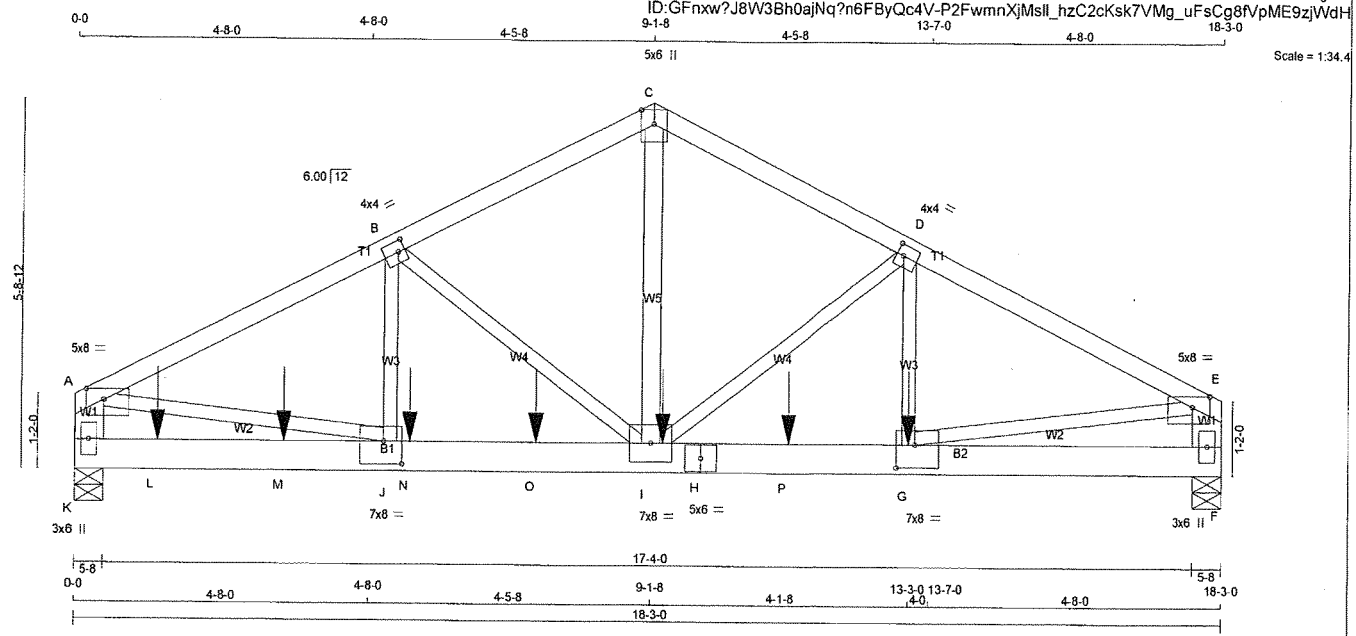

TOTAL WEIGHT = 2 X 84 = 168 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
D- G	1	3	SIDE(408.3)
B- J	1	3	SIDE(78.6)
2x3	1	6	
I- C	1	6	SIDE(78.6)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	2.00	3.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW+p	MT20	5.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TMVW-p	MT20	5.0	8.0	2.00	3.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	4.25	3.50
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	7.0	8.0		
J	BMWW-t	MT20	7.0	8.0	4.25	3.50
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	5638	0	5638	0	0	5-8	5-8
F	4787	0	4787	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	4283	2081 / 0	943 / 0	0 / 0	0 / 0	1258 / 0	0 / 0
F	3634	1776 / 0	797 / 0	0 / 0	0 / 0	1061 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-7603 / 0	-77.7	-77.7	0.45 (1)	3.30	I- C	0 / 4971	0.44 (1)
B- C	-5867 / 0	-77.7	-77.7	0.23 (1)	3.88	I- D	-2078 / 0	0.48 (1)
C- D	-5867 / 0	-77.7	-77.7	0.23 (1)	3.88	G- D	0 / 1702	0.21 (1)
D- E	-7665 / 0	-77.7	-77.7	0.45 (1)	3.28	B- I	-2007 / 0	0.46 (1)
K- A	-4631 / 0	0.0	0.0	0.16 (1)	6.71	J- B	0 / 1835	0.29 (1)
F- E	-4668 / 0	0.0	0.0	0.16 (1)	6.68	A- J	-1690 / 0	0.65 (1)
						G- E	0 / 1847	0.65 (1)
K- L	0 / 0	-39.5	-39.5	0.47 (1)	10.00			
L- M	0 / 0	-39.5	-39.5	0.47 (1)	10.00			
M- J	0 / 0	-39.5	-39.5	0.47 (1)	10.00			
J- N	0 / 6809	-39.5	-39.5	0.76 (1)	10.00			
N- O	0 / 6809	-39.5	-39.5	0.76 (1)	10.00			
O- I	0 / 6809	-39.5	-39.5	0.76 (1)	10.00			
I- H	0 / 6865	-39.5	-39.5	0.76 (1)	10.00			
H- P	0 / 6865	-39.5	-39.5	0.76 (1)	10.00			
P- G	0 / 6865	-39.5	-39.5	0.76 (1)	10.00			
G- F	0 / 0	-39.5	-39.5	0.07 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	MIN
G	13-3-0	-2267	-2267	---	FRONT	VERT	TOTAL	---	---
I	9-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
L	1-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
M	3-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
N	5-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
O	7-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---
P	11-3-12	-1003	-1003	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.76/1.00 (I-J:1) , WB=0.86/1.00 (E-G:1) , SSI=0.49/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.66 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM 77903797
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T26	4	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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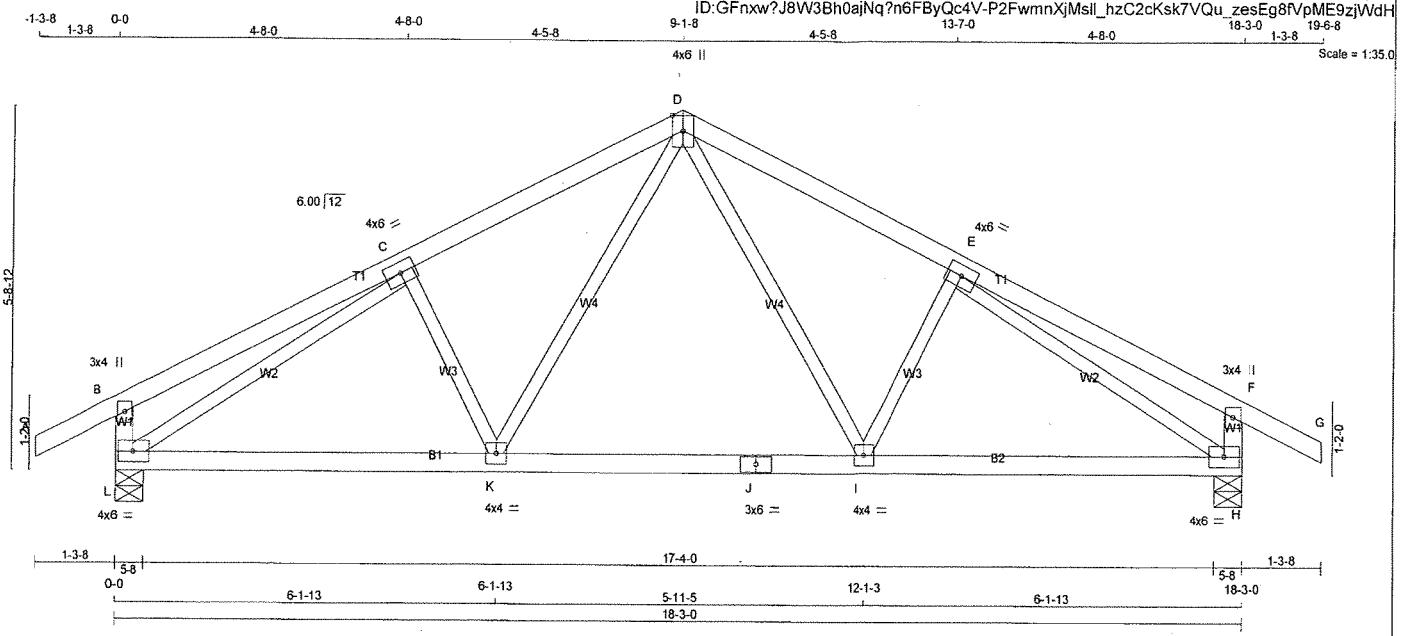


TOTAL WEIGHT = 4 X 73 = 290 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1175	0	1175	0	0	5-8	5-8
H	1175	0	1175	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0
H	891	438 / 0	192 / 0	0 / 0	0 / 0	261 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

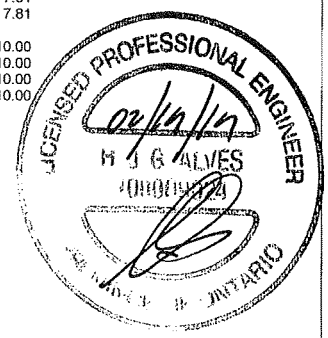
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	D- I	0 / 485	0.11 (1)
B- C	0 / 18	-77.7	-77.7	0.25 (1)	10.00	I- E	-185 / 36	0.04 (1)
C- D	-1277 / 0	-77.7	-77.7	0.21 (1)	5.48	K- D	0 / 485	0.11 (1)
D- E	-1277 / 0	-77.7	-77.7	0.21 (1)	5.48	C- K	-185 / 36	0.04 (1)
E- F	0 / 18	-77.7	-77.7	0.25 (1)	10.00	L- C	-1468 / 0	0.73 (1)
F- G	0 / 24	-77.7	-77.7	0.10 (1)	10.00	E- H	-1468 / 0	0.73 (1)
L- B	-243 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-243 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 1209	-39.5	-39.5	0.41 (2)	10.00			
K- J	0 / 893	-39.5	-39.5	0.37 (2)	10.00			
J- I	0 / 893	-39.5	-39.5	0.37 (2)	10.00			
I- H	0 / 1209	-39.5	-39.5	0.41 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.41/1.00 (K-L:2) , WB=0.73/1.00 (C-L:1) , SSI=0.15/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM T1903798
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	G26	4	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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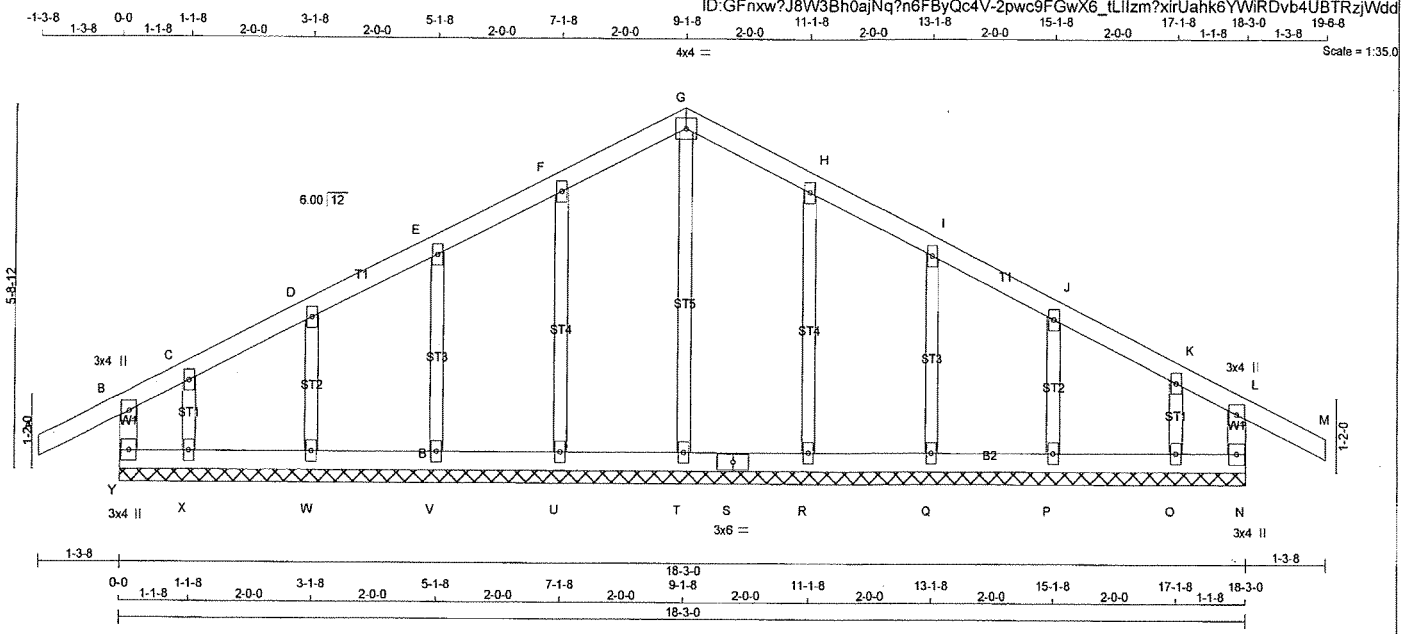

TOTAL WEIGHT = 4 X 71 = 283 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, R, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 24	-77.7	-77.7	0.10 (1)		10.00
B- C	0 / 0	-77.7	-77.7	0.07 (1)		10.00
C- D	0 / 24	-77.7	-77.7	0.04 (1)		10.00
D- E	0 / 24	-77.7	-77.7	0.04 (1)		10.00
E- F	0 / 29	-77.7	-77.7	0.04 (1)		10.00
F- G	0 / 31	-77.7	-77.7	0.04 (1)		10.00
G- H	0 / 31	-77.7	-77.7	0.04 (1)		10.00
H- I	0 / 29	-77.7	-77.7	0.04 (1)		10.00
I- J	0 / 24	-77.7	-77.7	0.04 (1)		10.00
J- K	0 / 24	-77.7	-77.7	0.04 (1)		10.00
K- L	0 / 0	-77.7	-77.7	0.07 (1)		10.00
L- M	0 / 24	-77.7	-77.7	0.10 (1)		10.00
Y- B	-174 / 0	0.0	0.0	0.03 (1)		7.81
N- L	-174 / 0	0.0	0.0	0.03 (1)		7.81
Y- X	-15 / 0	-39.5	-39.5	0.02 (3)		6.25
X- W	-18 / 0	-39.5	-39.5	0.03 (3)		6.25
W- V	-23 / 0	-39.5	-39.5	0.03 (3)		6.25
V- U	-25 / 0	-39.5	-39.5	0.02 (3)		6.25
U- T	-28 / 0	-39.5	-39.5	0.02 (3)		6.25
T- S	-28 / 0	-39.5	-39.5	0.02 (3)		6.25
S- R	-28 / 0	-39.5	-39.5	0.02 (3)		6.25
R- Q	-25 / 0	-39.5	-39.5	0.02 (3)		6.25
Q- P	-23 / 0	-39.5	-39.5	0.03 (3)		6.25
P- O	-18 / 0	-39.5	-39.5	0.03 (3)		6.25
O- N	-15 / 0	-39.5	-39.5	0.02 (3)		6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-186 / 0	0.09 (1)
U- F	-160 / 0	0.05 (1)
V- E	-150 / 0	0.03 (1)
W- D	-164 / 0	0.03 (1)
X- C	-74 / 0	0.01 (1)
R- H	-160 / 0	0.05 (1)
Q- I	-150 / 0	0.03 (1)
P- J	-164 / 0	0.03 (1)
O- K	-74 / 0	0.01 (1)

LICENSED PROFESSIONAL ENGINEER
02/19/19
M. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (P-Q:3) , WB=0.09/1.00 (G-T:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (H) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

DWG NO. TAM T1903799
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T34	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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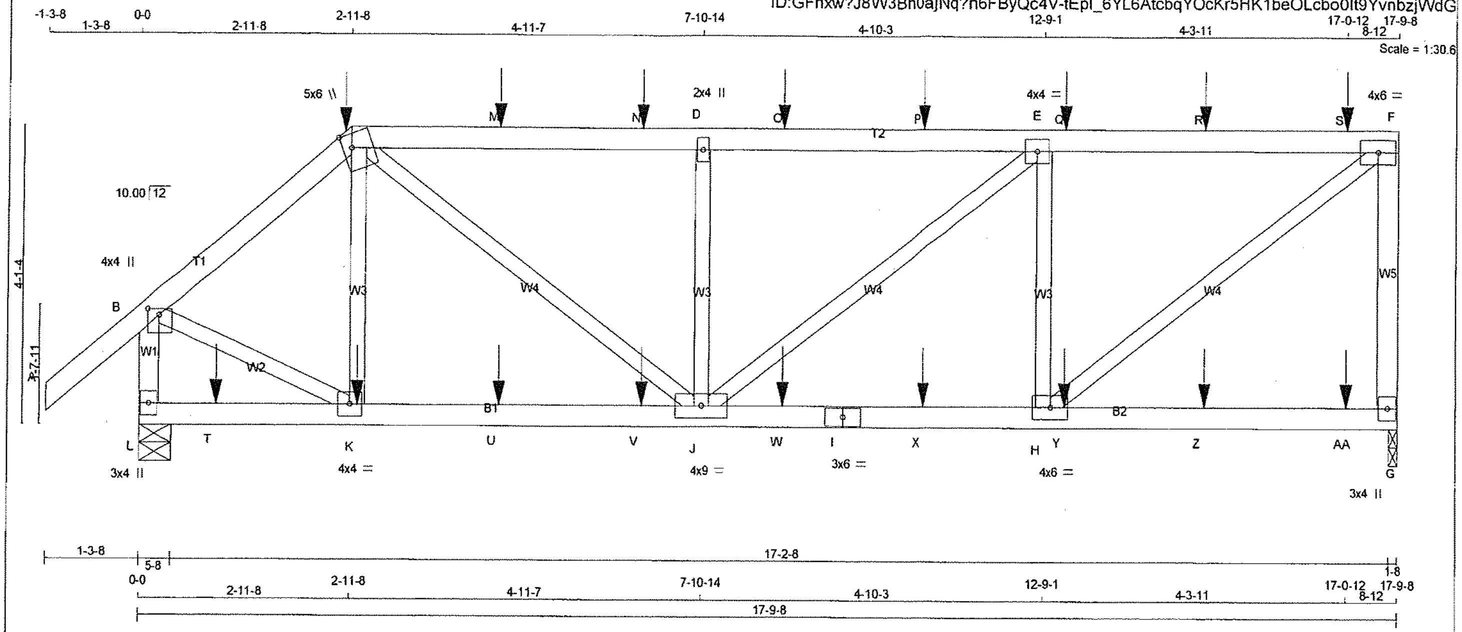

TOTAL WEIGHT = 2 X 74 = 149 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- G	1	12	TOP
L- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- I	1	12	SIDE(61.0)
I- G	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1776	0	1776	0	0	1-8	1-8
L	1912	0	1912	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
G	1361	625 / 0	327 / 0	0 / 0	0 / 0	410 / 0	0 / 0
L	1453	705 / 0	319 / 0	0 / 0	0 / 0	428 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	K- C	-239 / 87	0.03 (1)
B- C	-1677 / 0	-77.7	-77.7	0.07 (1)	6.25	B- K	0 / 1396	0.17 (1)
C- M	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	H- F	0 / 2238	0.28 (1)
M- N	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	C- J	0 / 1102	0.14 (1)
N- D	-2152 / 0	-77.7	-77.7	0.24 (1)	5.77	H- E	-971 / 0	0.13 (1)
D- O	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75	J- D	-845 / 0	0.09 (1)
O- P	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75	J- E	0 / 468	0.06 (1)
P- E	-2153 / 0	-77.7	-77.7	0.25 (1)	5.75			
E- Q	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
Q- R	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
R- S	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
S- F	-1785 / 0	-77.7	-77.7	0.24 (1)	6.18			
G- F	-1626 / 0	0.0	0.0	0.22 (1)	7.81			
L- B	-1849 / 0	0.0	0.0	0.11 (1)	7.81			
L- T	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
T- K	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
K- U	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
U- V	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
V- J	0 / 1277	-39.5	-39.5	0.21 (2)	10.00			
J- W	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
W- I	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
I- X	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
X- H	0 / 1785	-39.5	-39.5	0.30 (2)	10.00			
H- Y	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
Y- Z	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
Z-AA	0 / 0	-39.5	-39.5	0.17 (3)	10.00			
AA- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER 02/19/19 H. J. G ALVES 100009024 PROVINCE OF ONTARIO

DWG NO. TAM 1903800 STRUCTURAL COMPONENT ONLY 1/2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-100	-100	---	FRONT	VERT	TOTAL	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
K	3-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
M	5-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
N	7-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
O	9-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
P	11-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	13-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.30/1.00 (H-J:2) , WB=0.28/1.00 (F-H:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T34	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	15-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
S	17-0-12	-104	-104	---	FRONT	VERT	TOTAL	---	---
T	1-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
U	5-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
V	7-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
W	9-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
X	11-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Y	13-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
Z	15-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
AA	17-0-12	-62	-78	---	FRONT	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. G. GALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM T1903800
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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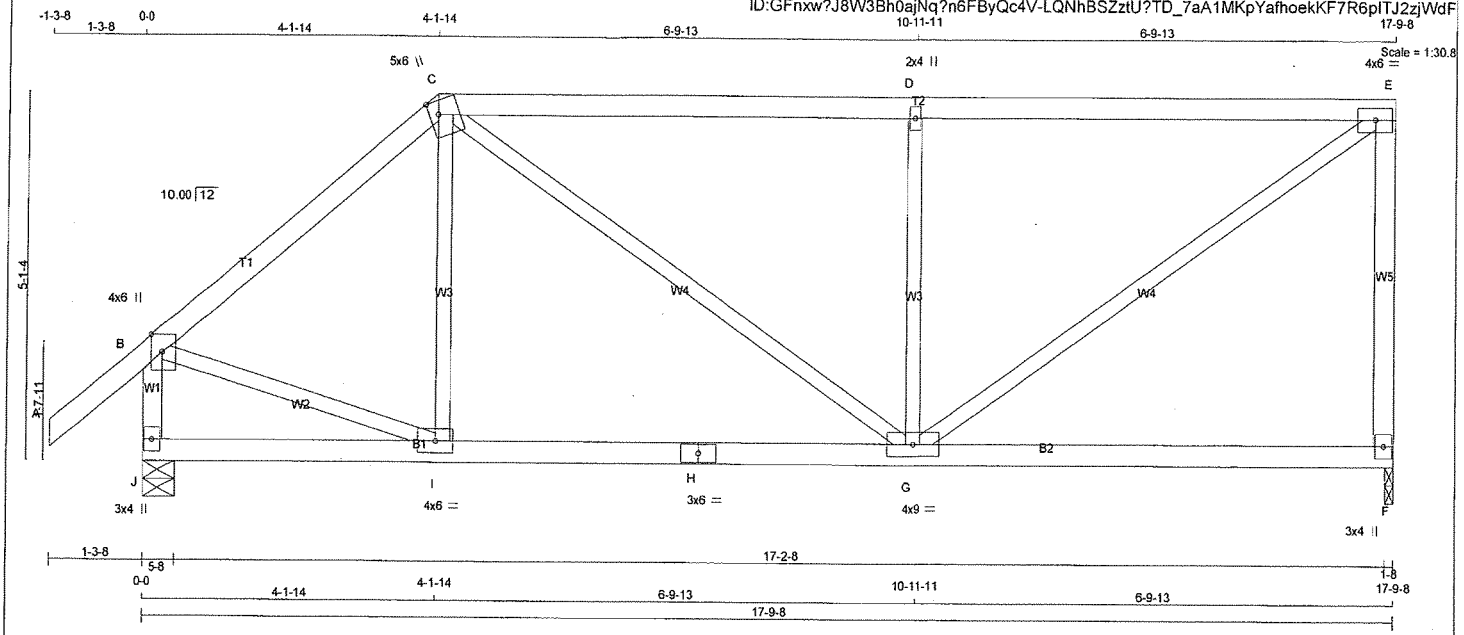


TOTAL WEIGHT = 74 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

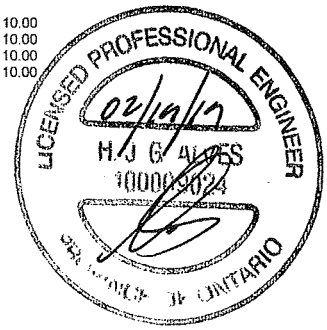
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00		I- C	0 / 162	0.04 (3)
B- C	-937 / 0	-77.7	-77.7	0.25 (1)	6.07		C- G	0 / 397	0.09 (1)
C- D	-1042 / 0	-77.7	-77.7	0.68 (1)	5.05		G- D	-655 / 0	0.25 (1)
D- E	-1043 / 0	-77.7	-77.7	0.68 (1)	5.05		G- E	0 / 1274	0.29 (1)
F- E	-934 / 0	0.0	0.0	0.41 (1)	7.81		B- I	0 / 751	0.17 (1)
J- B	-1091 / 0	0.0	0.0	0.12 (1)	7.57				
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00				
I- H	0 / 718	-39.5	-39.5	0.43 (2)	10.00				
H- G	0 / 718	-39.5	-39.5	0.43 (2)	10.00				
G- F	0 / 0	-39.5	-39.5	0.36 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.43/1.00 (G-I:2) , WB=0.29/1.00 (E-G:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM 1903801
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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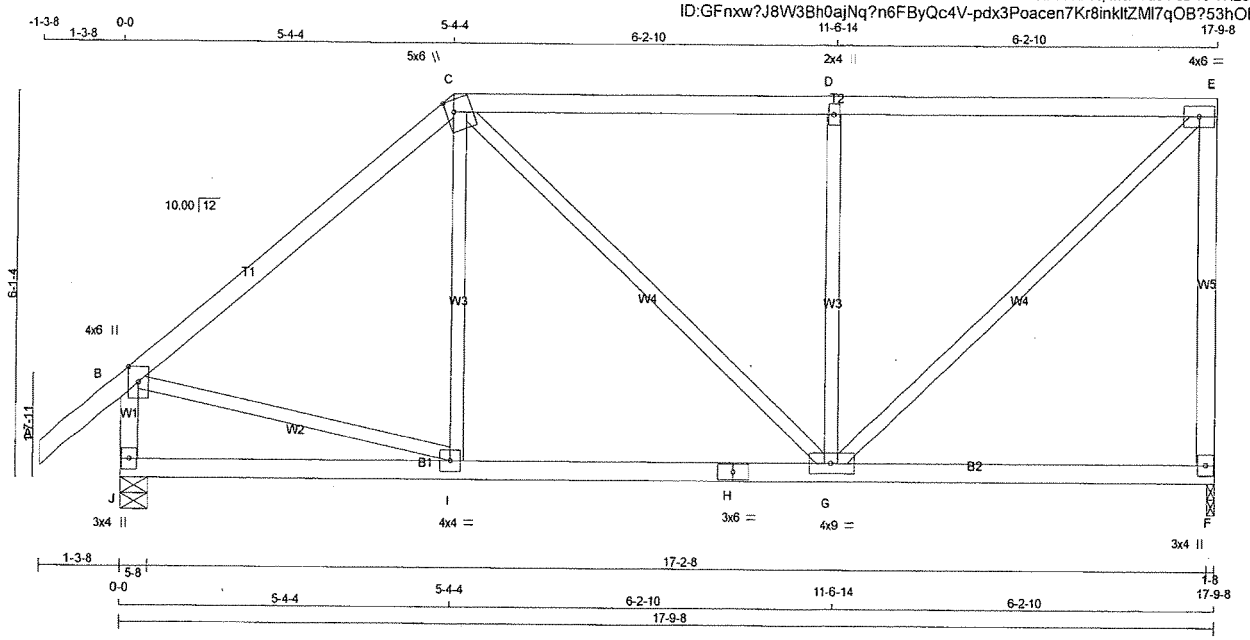


TOTAL WEIGHT = 79 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1043	0	1043	0	0	1-8	1-8
J	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

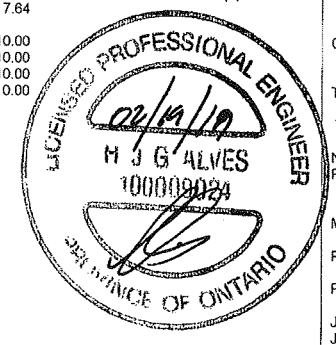
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 214	0.05 (3)
B- C	-907 / 0	-77.7	-77.7	0.43 (1)	5.87	C- G	0 / 163	0.04 (1)
C- D	-816 / 0	-77.7	-77.7	0.54 (1)	5.85	G- D	-597 / 0	0.35 (1)
D- E	-816 / 0	-77.7	-77.7	0.54 (1)	5.85	G- E	0 / 1113	0.25 (1)
F- E	-943 / 0	0.0	0.0	0.68 (1)	7.81	B- I	0 / 715	0.16 (1)
J- B	-1065 / 0	0.0	0.0	0.11 (1)	7.64			
J- I	0 / 0	-39.5	-39.5	0.22 (3)	10.00			
I- H	0 / 697	-39.5	-39.5	0.36 (2)	10.00			
H- G	0 / 697	-39.5	-39.5	0.36 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.36/1.00 (G-I:2) , WB=0.35/1.00 (D-G:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T1903802
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T37	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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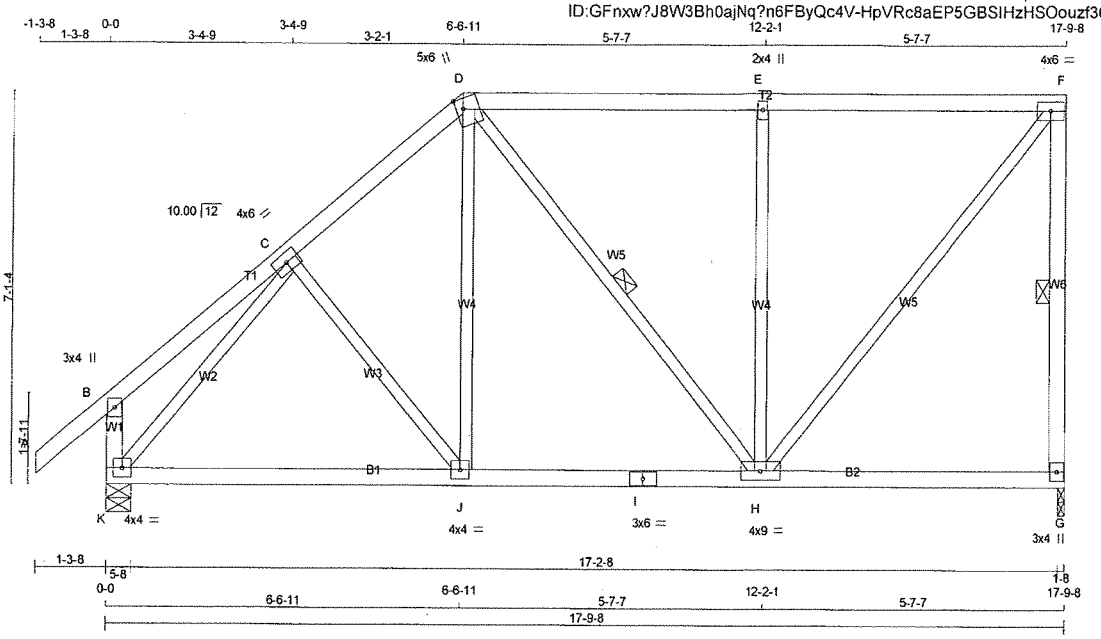


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

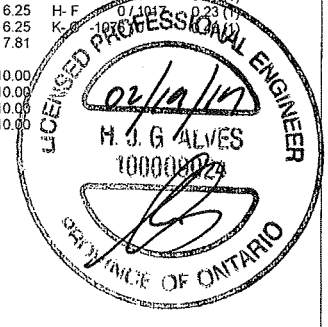
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD FROM	LC1 TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-32 / 60	0.01 (1)
B- C	0 / 18	-77.7	-77.7	0.13 (1)	10.00	J- D	0 / 337	0.08 (2)
C- D	-874 / 0	-77.7	-77.7	0.14 (1)	6.25	D- H	-44 / 0	0.02 (3)
D- E	-649 / 0	-77.7	-77.7	0.42 (1)	6.25	H- E	-538 / 0	0.47 (1)
E- F	-650 / 0	-77.7	-77.7	0.43 (1)	6.25	H- F	0 / 1013	0.23 (1)
G- F	-950 / 0	0.0	0.0	0.22 (1)	6.25	K- G	-1076 / 0	0.04 (1)
K- B	-206 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 677	-39.5	-39.5	0.37 (2)	10.00			
J- I	0 / 660	-39.5	-39.5	0.37 (2)	10.00			
I- H	0 / 660	-39.5	-39.5	0.37 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.37/1.00 (H-J:2) , WB=0.47/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (K) (INPUT = 0.90)
JSI METAL= 0.26 (C) (INPUT = 1.00)

DWG NO. TAM 17903803
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T38	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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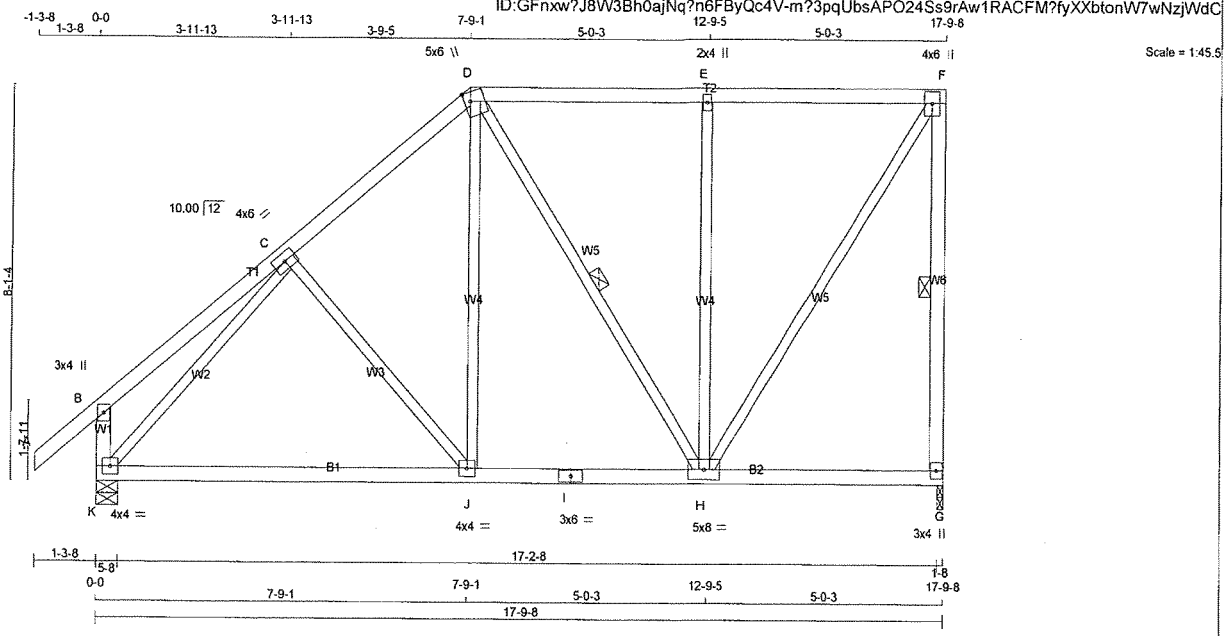


TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

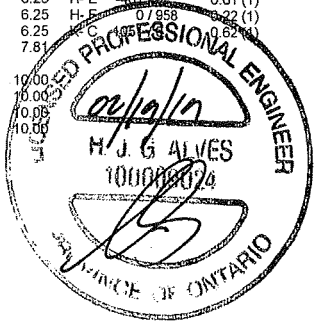
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-109 / 33	0.07 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00	J- D	0 / 432	0.10 (2)
C- D	-817 / 0	-77.7	-77.7	0.20 (1)	6.25	D- H	-182 / 0	0.11 (2)
D- E	-521 / 0	-77.7	-77.7	0.33 (1)	6.25	H- E	-481 / 0	0.61 (1)
E- F	-521 / 0	-77.7	-77.7	0.33 (1)	6.25	H- F	0 / 958	0.22 (1)
G- F	-955 / 0	0.0	0.0	0.28 (1)	6.25	I- C	1055 / 0	0.62 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 682	-39.5	-39.5	0.47 (2)	10.00			
J- I	0 / 615	-39.5	-39.5	0.46 (2)	10.00			
I- H	0 / 615	-39.5	-39.5	0.46 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.17 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 826 (0.26")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.47/1.00 (J-K:2) , WB=0.62/1.00 (C-K:1) , SSI=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.28 (I) (INPUT = 1.00)

DWG NO. TAM T1703804
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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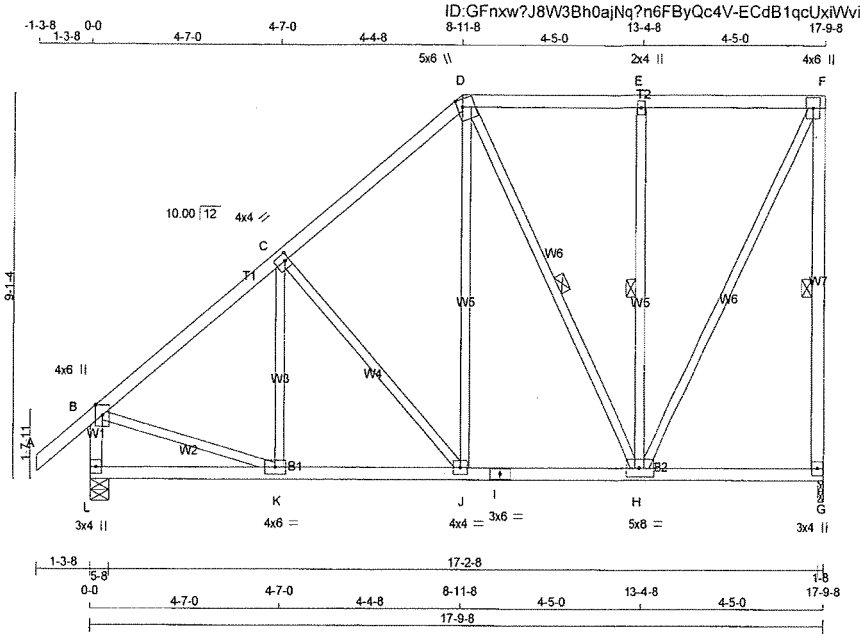


TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
L	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
L	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

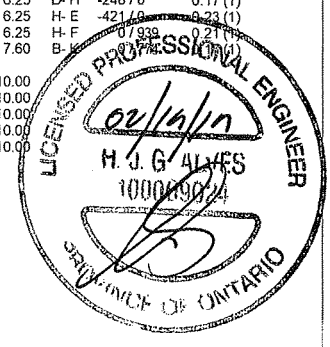
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-9 / 142	0.03 (3)
B- C	-943 / 0	-77.7	-77.7	0.29 (1)	6.02	C- J	-326 / 0	0.28 (1)
C- D	-718 / 0	-77.7	-77.7	0.27 (1)	6.25	J- D	0 / 423	0.10 (2)
D- E	-424 / 0	-77.7	-77.7	0.25 (1)	6.25	D- H	-248 / 0	0.17 (1)
E- F	-424 / 0	-77.7	-77.7	0.25 (1)	6.25	H- E	-421 / 0	0.23 (1)
G- F	-971 / 0	0.0	0.0	0.37 (1)	6.25	H- F	0 / 939	0.21 (1)
L- B	-1077 / 0	0.0	0.0	0.11 (1)	7.60	B- K	0 / 1077	0.11 (1)
L- K	0 / 0	-39.5	-39.5	0.15 (3)	10.00			
K- J	0 / 744	-39.5	-39.5	0.25 (2)	10.00			
J- I	0 / 534	-39.5	-39.5	0.21 (2)	10.00			
I- H	0 / 534	-39.5	-39.5	0.21 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.14 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.37/1.00 (F-G:1) , BC=0.25/1.00 (J-K:2) , WB=0.28/1.00 (C-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM 1903805
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T40	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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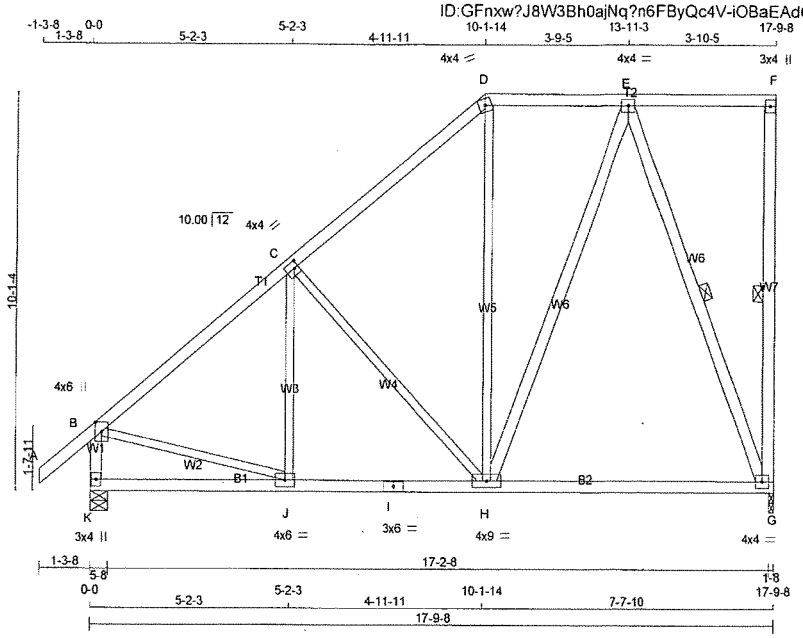


Scale = 1:56.3

TOTAL WEIGHT = 2 X 104 = 208 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1043	0	1043	0	0	1-8	1-8
K	1150	0	1150	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	872	430 / 0	187 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, E-G.

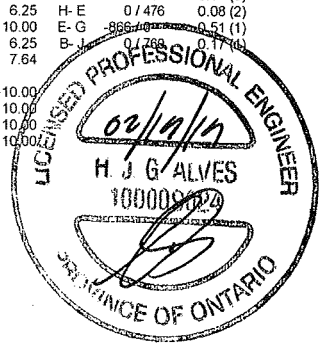
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-929 / 0	-77.7	-77.7	0.28 (1)	6.09
C- D	-658 / 0	-77.7	-77.7	0.26 (1)	6.25
D- E	-482 / 0	-77.7	-77.7	0.15 (1)	6.25
E- F	0 / 0	-77.7	-77.7	0.19 (1)	10.00
G- F	-114 / 0	0.0	0.0	0.06 (1)	6.25
K- B	-1062 / 0	0.0	0.0	0.11 (1)	7.64
K- J	0 / 0	-39.5	-39.5	0.18 (3)	10.00
J- I	0 / 737	-39.5	-39.5	0.45 (2)	10.00
I- H	0 / 737	-39.5	-39.5	0.45 (2)	10.00
H- G	0 / 317	-39.5	-39.5	0.40 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	0 / 142	0.03 (3)
C- H	-383 / 0	0.44 (1)
H- D	0 / 183	0.04 (3)
H- E	0 / 476	0.08 (2)
E- G	-866 / 0	0.51 (1)
B- J	0 / 768	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010. NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.45/1.00 (H-J:2) , WB=0.51/1.00 (E-G:1) , SSI=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

DWG NO. TAM T1903806
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T52	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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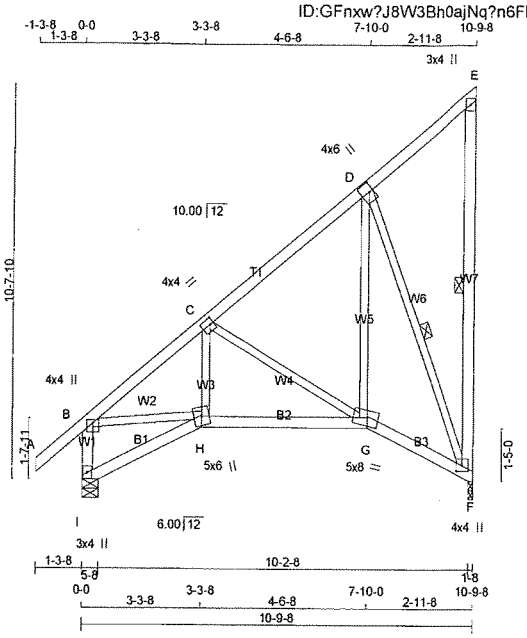


Scale = 1:59.8

TOTAL WEIGHT = 6 X 64 = 386 lb

[M][F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW+t	MT20	4.0	6.0	3.00	1.25
E	TMV+p	MT20	3.0	4.0		
F	BMVW1+p	MT20	4.0	4.0	1.75	2.00
G	BBWW-m	MT20	5.0	8.0	2.75	3.50
H	BBWW+m	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	632	0	632	0	0	1-8	1-8
I	740	0	740	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0
I	558	283 / 0	113 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

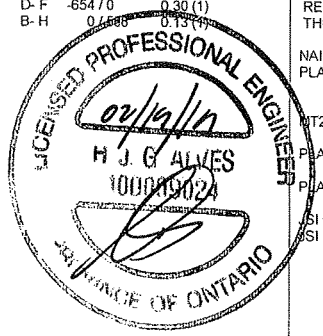
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-744 / 0	-77.7	-77.7	0.12 (1)	6.25
C- D	-302 / 0	-77.7	-77.7	0.12 (1)	6.25
D- E	-22 / 0	-77.7	-77.7	0.11 (1)	6.25
F- E	-84 / 0	0.0	0.0	0.05 (1)	6.25
I- B	-675 / 0	0.0	0.0	0.07 (1)	7.81
I- H	0 / 0	-39.5	-39.5	0.10 (3)	10.00
H- G	0 / 580	-39.5	-39.5	0.26 (2)	10.00
G- F	0 / 267	-39.5	-39.5	0.11 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	0 / 215	0.05 (3)
C- G	-409 / 0	0.18 (1)
G- D	0 / 485	0.11 (2)
D- F	-654 / 0	0.30 (1)
B- H	0 / 588	0.13 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.26/1.00 (G-H:2) , WB=0.30/1.00 (D-F:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (B) (INPUT = 0.90)
SI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM 7903807
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T53	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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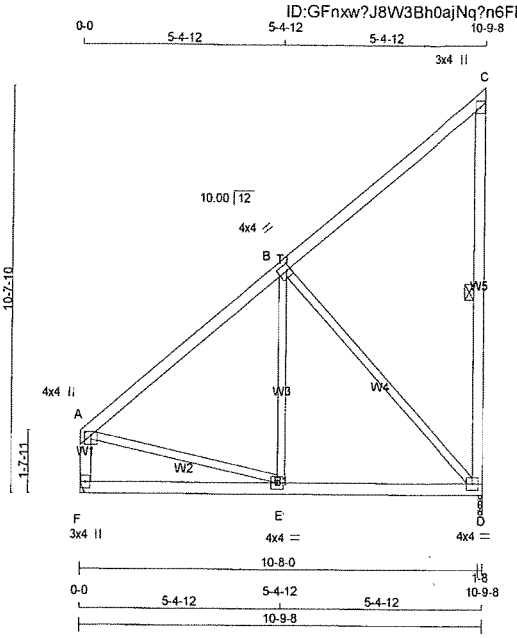


Scale = 1:58.0

TOTAL WEIGHT = 6 X 56 = 337 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	632	0	632	0	0	1-8	1-8
F	632	0	632	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0
F	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

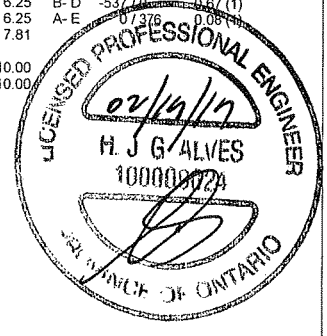
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-443 / 0	-77.7	-77.7	0.30 (1)	6.25	E- B	0 / 286	0.07 (3)
B- C	-33 / 0	-77.7	-77.7	0.29 (1)	6.25	B- D	-537 / 0	0.67 (1)
D- C	-158 / 0	0.0	0.0	0.09 (1)	6.25	A- E	0 / 376	0.08 (1)
F- A	-551 / 0	0.0	0.0	0.06 (1)	7.81			
F- E	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
E- D	0 / 366	-39.5	-39.5	0.29 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.29/1.00 (D-E:2) , WB=0.67/1.00 (B-D:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (A) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

DWG NO. TAM T1903808
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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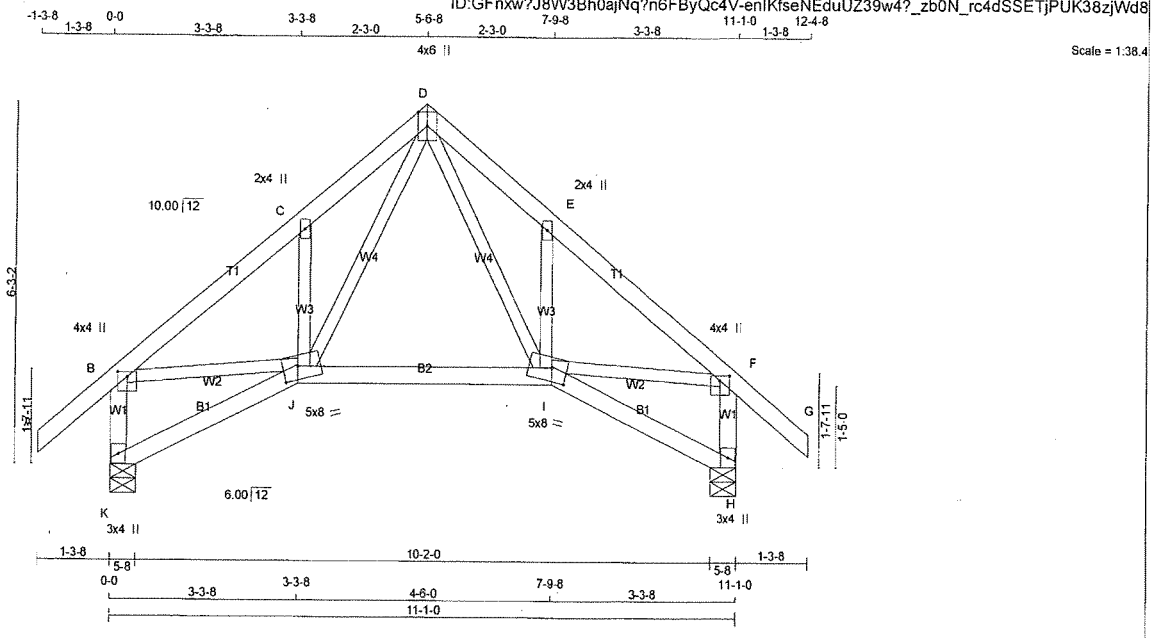


TOTAL WEIGHT = 54 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	757	0	757	0	0	5-8	5-8
H	757	0	757	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0
H	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

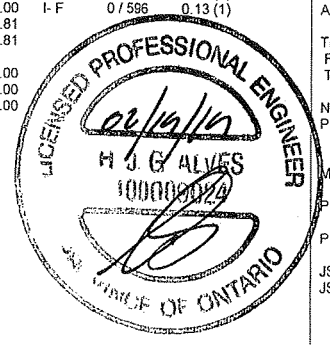
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	D- I	0 / 489	0.11 (1)
B- C	-763 / 0	-77.7	-77.7	0.11 (1)	6.25	I- E	-262 / 0	0.05 (1)
C- D	-794 / 0	-77.7	-77.7	0.07 (1)	6.25	J- D	0 / 489	0.11 (1)
D- E	-794 / 0	-77.7	-77.7	0.07 (1)	6.25	J- C	-262 / 0	0.05 (1)
E- F	-763 / 0	-77.7	-77.7	0.11 (1)	6.25	B- J	0 / 596	0.13 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- F	0 / 596	0.13 (1)
K- B	-692 / 0	0.0	0.0	0.08 (1)	7.81			
H- F	-692 / 0	0.0	0.0	0.08 (1)	7.81			
K- J	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
J- I	0 / 378	-39.5	-39.5	0.23 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.10 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.11/1.00 (A-B:1) , BC=0.23/1.00 (I-J:2) , WB=0.13/1.00 (B-J:1) , SSI=0.10/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM TM038809
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T54A	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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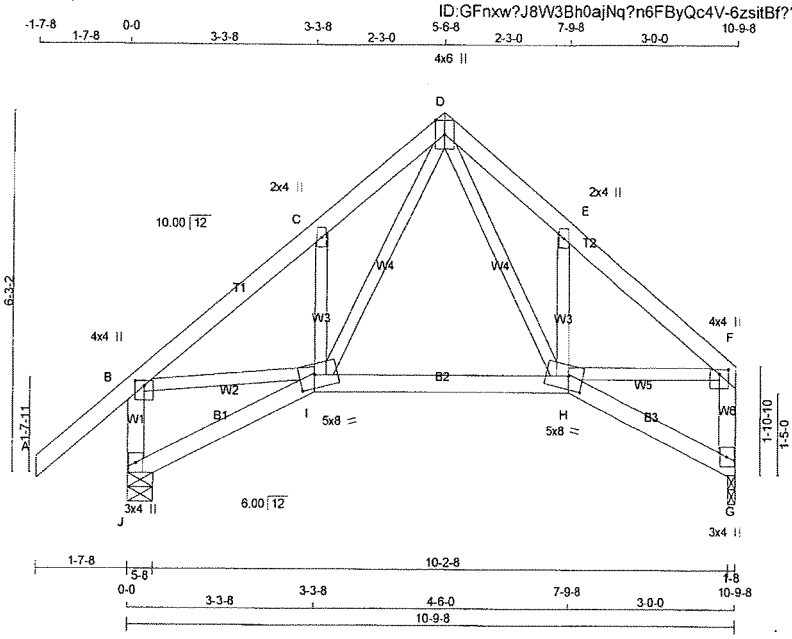


TOTAL WEIGHT = 2 X 52 = 104 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BBWWW-m	MT20	5.0	8.0	2.75	3.25
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	766	0	766	0	0	5-8	5-8
G	632	0	632	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	576	297 / 0	113 / 0	0 / 0	0 / 0	165 / 0	0 / 0
G	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

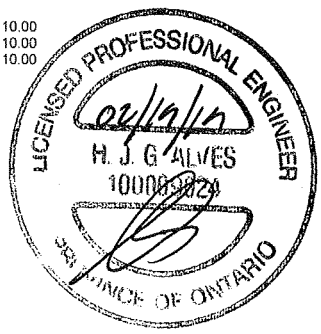
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-77.7	-77.7	0.16 (1)	10.00	I- C	-262 / 0	0.05 (1)
B- C	-736 / 0	-77.7	-77.7	0.11 (1)	6.25	I- D	0 / 488	0.11 (1)
C- D	-766 / 0	-77.7	-77.7	0.07 (1)	6.25	D- H	0 / 414	0.09 (1)
D- E	-720 / 0	-77.7	-77.7	0.06 (1)	6.25	H- E	-245 / 0	0.04 (1)
E- F	-694 / 0	-77.7	-77.7	0.09 (1)	6.25	B- I	0 / 575	0.13 (1)
J- B	-701 / 0	0.0	0.0	0.08 (1)	7.81	H- F	0 / 542	0.12 (1)
G- F	-573 / 0	0.0	0.0	0.06 (1)	7.81			
J- I	0 / 0	-39.5	-39.5	0.10 (3)	10.00			
I- H	0 / 357	-39.5	-39.5	0.22 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.08 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.22/1.00 (H-I:2) , WB=0.13/1.00 (B-I:1) , SSI=0.09/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM T1903810
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T67TCX	9	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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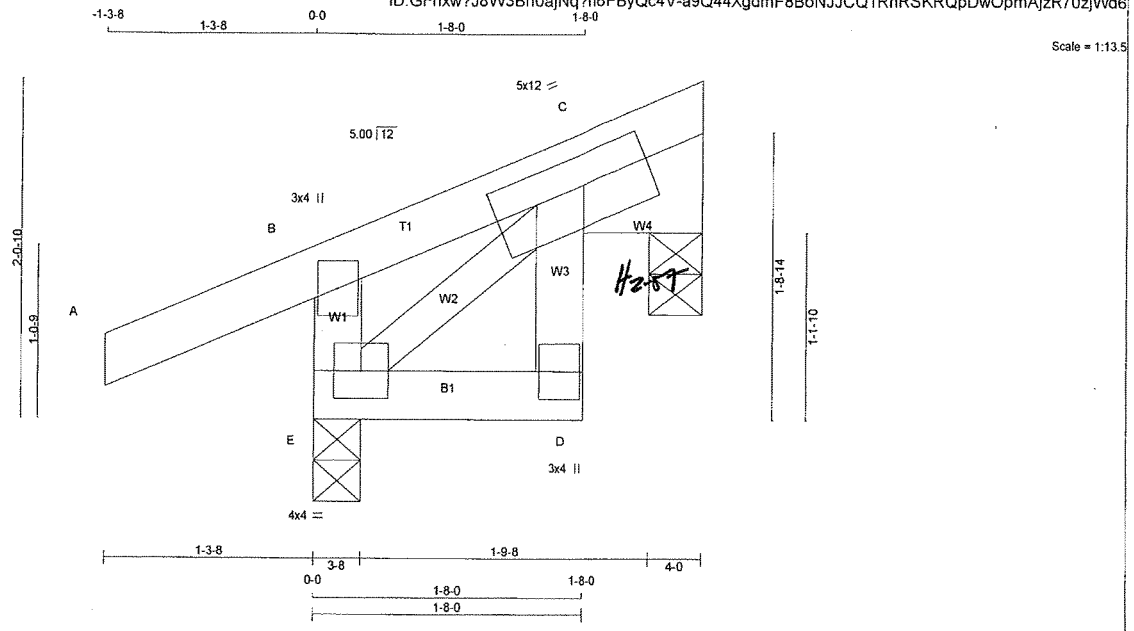


TOTAL WEIGHT = 9 X 12 = 104 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	5.0	12.0	2.00	3.25
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	55	0	58	0	-22	4-0	4-0
E	245	0	245	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT H25T

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	45	12 / -23	17 / 0	0 / 0	0 / 0	16 / 0	0 / 0
E	176	114 / 0	17 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

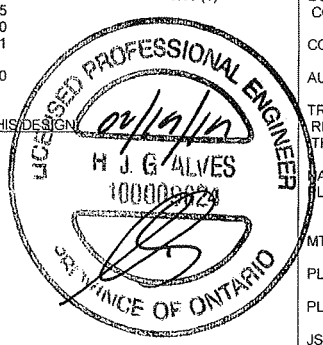
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.10 (1)		10.00	E- C	0 / 0	0.00 (1)
B- C	-16 / 0	-77.7	-77.7	0.10 (1)		6.25			
D- C	0 / 42	0.0	0.0	0.01 (3)		10.00			
E- B	-212 / 0	0.0	0.0	0.02 (1)		7.81			
E- D	0 / 0	-39.5	-39.5	0.03 (3)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN.



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.03/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM T903811
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T70	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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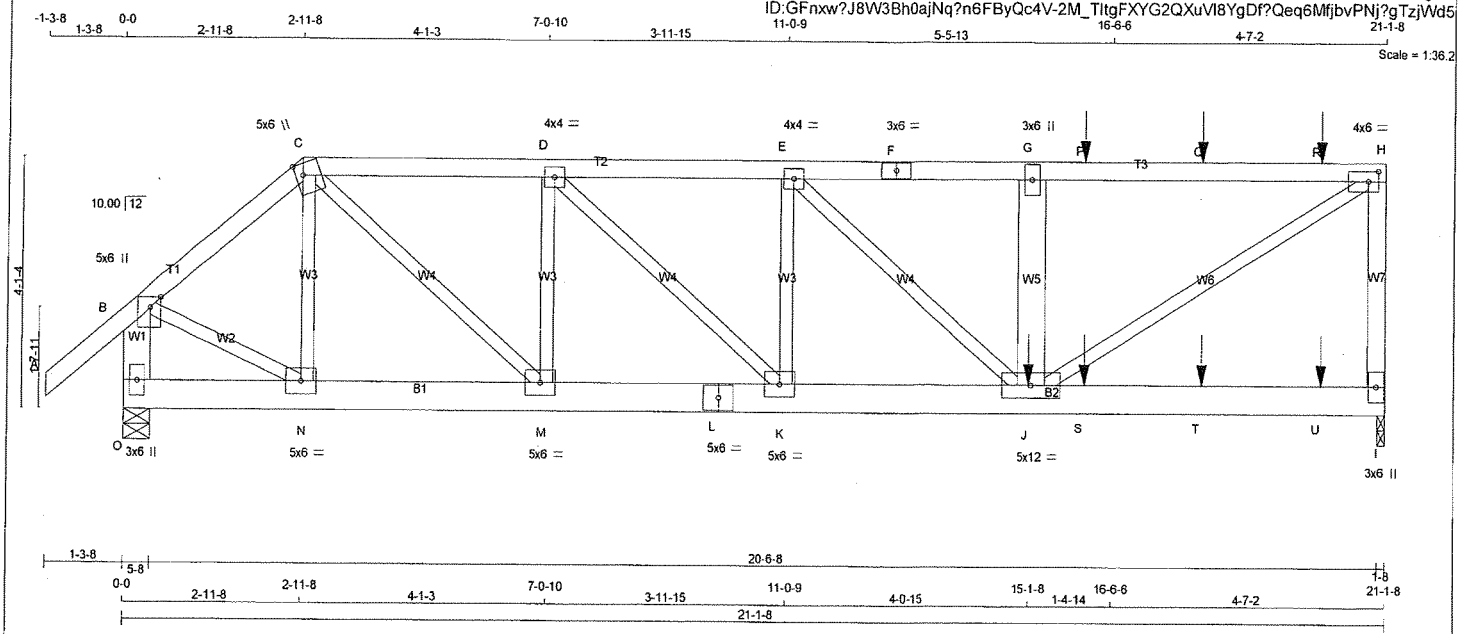

TOTAL WEIGHT = 2 X 105 = 210 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- J	2	6	SIDE(155.8)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	12.0		
K, M, N						
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	2555	0	2555	0	0	1-8	1-8
O	1777	0	1777	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1943	938 / 0	431 / 0	0 / 0	0 / 0	573 / 0	0 / 0
O	1346	666 / 0	287 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00		N- C	-311 / 0	0.04 (1)
B- C	-1597 / 0	-77.7	-77.7	0.07 (1)	6.25		J- G	-674 / 0	0.04 (1)
C- D	-2413 / 0	-77.7	-77.7	0.13 (1)	5.67		J- H	0 / 3863	0.48 (1)
D- E	-3033 / 0	-77.7	-77.7	0.14 (1)	5.18		B- N	0 / 1317	0.16 (1)
E- F	-3275 / 0	-77.7	-77.7	0.32 (1)	4.79		E- J	0 / 326	0.04 (1)
F- G	-3275 / 0	-77.7	-77.7	0.32 (1)	4.79		C- M	0 / 1617	0.20 (1)
G- P	-3275 / 0	-77.7	-77.7	0.39 (1)	4.73		K- E	-425 / 0	0.05 (1)
P- Q	-3275 / 0	-77.7	-77.7	0.39 (1)	4.73		M- D	642 / 0	0.12 (1)
Q- R	-3275 / 0	-77.7	-77.7	0.39 (1)	4.73		D- K	874 / 0	0.09 (1)
R- H	-3275 / 0	-77.7	-77.7	0.39 (1)	4.73				
I- H	-2359 / 0	0.0	0.0	0.29 (1)	7.32				
O- B	-1739 / 0	0.0	0.0	0.06 (1)	7.81				
O- N	0 / 0	-39.5	-39.5	0.03 (2)	10.00				
N- M	0 / 1214	-39.5	-39.5	0.10 (1)	10.00				
M- L	0 / 2413	-39.5	-39.5	0.19 (1)	10.00				
L- K	0 / 2413	-39.5	-39.5	0.19 (1)	10.00				
K- J	0 / 3033	-39.5	-39.5	0.22 (1)	10.00				
J- S	0 / 0	-39.5	-39.5	0.13 (2)	10.00				
S- T	0 / 0	-39.5	-39.5	0.13 (2)	10.00				
T- U	0 / 0	-39.5	-39.5	0.13 (2)	10.00				
U- I	0 / 0	-39.5	-39.5	0.13 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE		
J	15-1-8	-1299	-1299	---	BACK	VERT	TOTAL	---	---
P	16-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
Q	18-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
R	20-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
S	16-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
T	18-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---
U	20-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.39/1.00 (G-H:1) , BC=0.22/1.00 (J-K:1) , WB=0.48/1.00 (H-J:1) , SSI=0.19/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
M. J. G. ALVES
100080172
PROVINCE OF ONTARIO

DWG NO. TAM T1903812
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T70Z	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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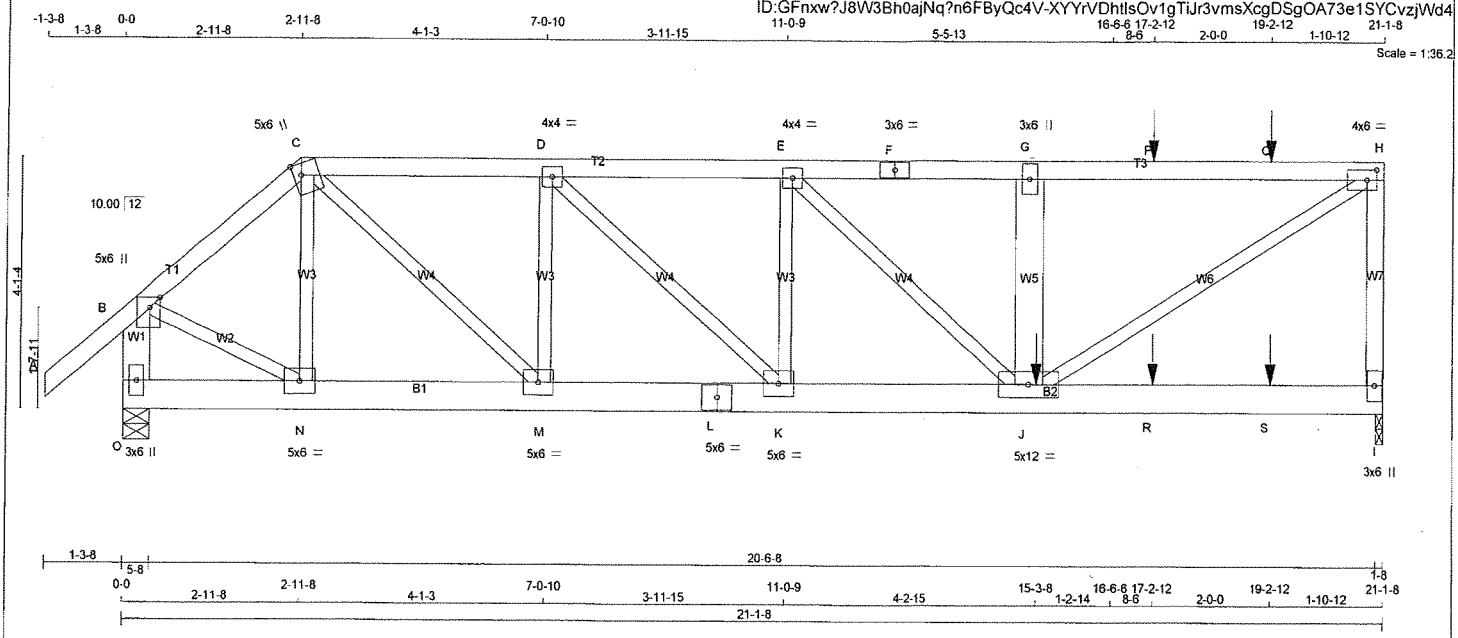

TOTAL WEIGHT = 2 X 105 = 210 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	TOP
F- H	1	12	SIDE(0.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
G- J	2	6	SIDE(151.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	12.0		
K, M, N						
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2417	0	2417	0	0	1-8	1-8
O	1748	0	1748	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1843	875 / 0	420 / 0	0 / 0	0 / 0	548 / 0	0 / 0
O	1326	649 / 0	288 / 0	0 / 0	0 / 0	390 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	N- C	-302 / 0	0.04 (1)	
B- C	-1567 / 0	-77.7	-77.7	0.07 (1)	6.25	J- G	-612 / 0	0.04 (1)	
C- D	-2359 / 0	-77.7	-77.7	0.13 (1)	5.72	J- H	0 / 3718	0.46 (1)	
D- E	-2950 / 0	-77.7	-77.7	0.14 (1)	5.24	B- N	0 / 1291	0.16 (1)	
E- F	-3151 / 0	-77.7	-77.7	0.30 (1)	4.89	E- J	0 / 272	0.03 (1)	
F- G	-3151 / 0	-77.7	-77.7	0.30 (1)	4.89	C- M	0 / 1575	0.19 (1)	
G- P	-3151 / 0	-77.7	-77.7	0.37 (1)	4.83	K- E	-396 / 0	0.05 (1)	
P- Q	-3151 / 0	-77.7	-77.7	0.37 (1)	4.83	M- D	-914 / 0	0.11 (1)	
Q- H	-3151 / 0	-77.7	-77.7	0.37 (1)	4.83	D- K	0 / 884	0.10 (1)	
I- H	-2245 / 0	0.0	0.0	0.28 (1)	7.45				
O- B	-1710 / 0	0.0	0.0	0.06 (1)	7.81				
O- N	0 / 0	-39.5	-39.5	0.03 (2)	10.00				
N- M	0 / 1191	-39.5	-39.5	0.10 (1)	10.00				
M- L	0 / 2359	-39.5	-39.5	0.18 (1)	10.00				
L- K	0 / 2359	-39.5	-39.5	0.18 (1)	10.00				
K- J	0 / 2950	-39.5	-39.5	0.22 (1)	10.00				
J- R	0 / 0	-39.5	-39.5	0.12 (2)	10.00				
R- S	0 / 0	-39.5	-39.5	0.12 (2)	10.00				
S- I	0 / 0	-39.5	-39.5	0.12 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
J	15-3-8	-1283	-1283	---	FRONT	VERT	TOTAL	---	---
P	17-2-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	19-2-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
R	17-2-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
S	19-2-12	-56	-71	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.37/1.00 (G-H:1) , BC=0.22/1.00 (J-K:1) , WB=0.46/1.00 (H-J:1) , SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (H) (INPUT = 0.90)
JSI METAL= 0.39 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
PROVINCE OF ONTARIO
02/19/19
H. J. G. ALVES
100090024

DWG NO. TAM T903813
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T71	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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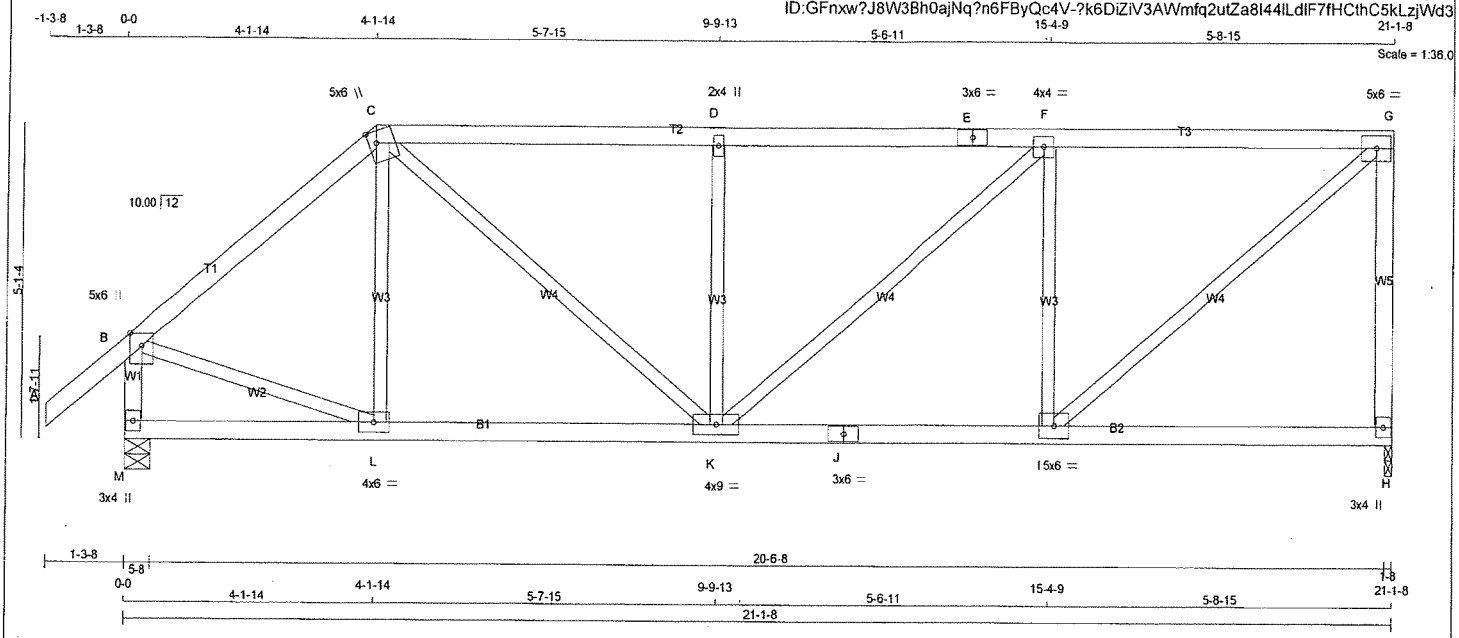


TOTAL WEIGHT = 2 X 90 = 180 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1238	0	1238	0	0	1-8	1-8
M	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
M	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

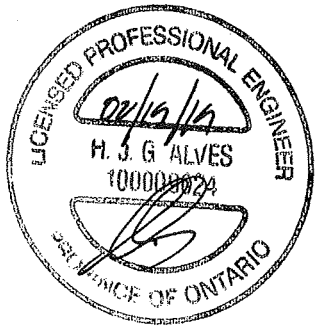
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B		0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C		-52 / 121	0.03 (3)
B- C		-1157 / 0	-77.7	-77.7	0.26 (1)	5.60	B- L		0 / 927	0.21 (1)
C- D		-1417 / 0	-77.7	-77.7	0.38 (1)	5.04	I- G		0 / 1503	0.34 (1)
D- E		-1417 / 0	-77.7	-77.7	0.39 (1)	5.01	C- K		0 / 697	0.16 (1)
E- F		-1417 / 0	-77.7	-77.7	0.39 (1)	5.01	I- F		-712 / 0	0.27 (1)
F- G		-1153 / 0	-77.7	-77.7	0.38 (1)	5.44	K- D		-470 / 0	0.18 (1)
H- G		-1146 / 0	0.0	0.0	0.51 (1)	7.43	K- F		0 / 350	0.08 (1)
M- B		-1284 / 0	0.0	0.0	0.14 (1)	7.11				
M- L		0 / 0	-39.5	-39.5	0.18 (3)	10.00				
L- K		0 / 885	-39.5	-39.5	0.30 (2)	10.00				
K- J		0 / 1153	-39.5	-39.5	0.39 (2)	10.00				
J- I		0 / 1153	-39.5	-39.5	0.39 (2)	10.00				
I- H		0 / 0	-39.5	-39.5	0.24 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.39/1.00 (I-K:2) , WB=0.34/1.00 (G-I:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

DWG NO. TAM T9038814
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T72S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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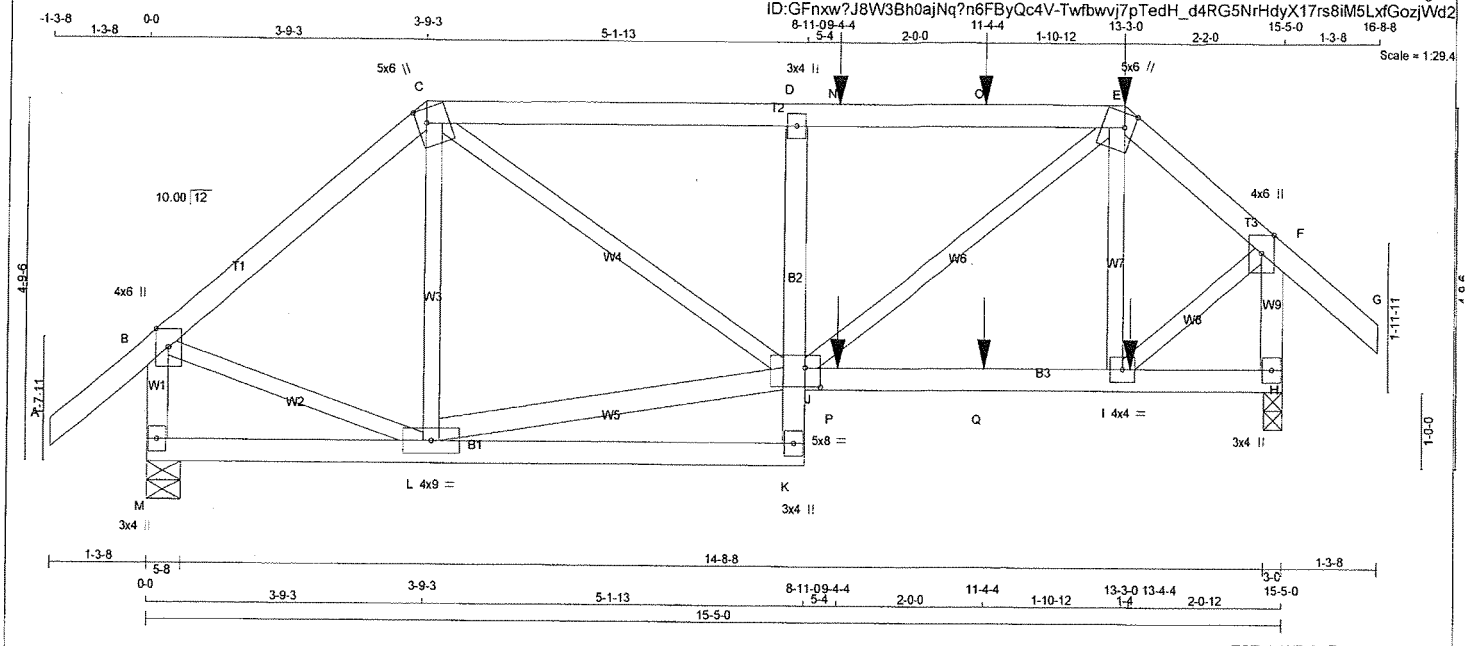

TOTAL WEIGHT = 2 X 75 = 150 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BVMWWW-I	MT20	5.0	8.0	3.00	2.50
K	BMV+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1054	0	1054	0	0	5-8	5-8
H	1230	0	1230	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	796	400 / 0	165 / 0	0 / 0	0 / 0	231 / 0	0 / 0
H	920	491 / 0	168 / 0	0 / 0	0 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.12 (1)	10.00	L- C	-122 / 84	0.04 (1)
B- C	-825 / 0	-77.7	-77.7	0.23 (1)	6.25	J- E	0 / 632	0.16 (1)
C- D	-1137 / 0	-77.7	-77.7	0.35 (1)	5.47	I- E	-287 / 0	0.07 (1)
D- N	-1146 / 0	-77.7	-77.7	0.34 (1)	5.46	B- L	0 / 670	0.17 (1)
N- O	-1146 / 0	-77.7	-77.7	0.34 (1)	5.46	I- F	0 / 808	0.20 (1)
O- E	-1146 / 0	-77.7	-77.7	0.34 (1)	5.46	L- J	0 / 614	0.11 (1)
E- F	-857 / 0	-77.7	-77.7	0.08 (1)	6.25	C- J	0 / 611	0.11 (1)
F- G	0 / 34	-77.7	-77.7	0.12 (1)	10.00			
M- B	-1007 / 0	0.0	0.0	0.12 (1)	7.77			
H- F	-1217 / 0	0.0	0.0	0.15 (1)	7.23			
M- L	0 / 0	-39.5	-39.5	0.20 (3)	10.00			
L- K	0 / 32	-39.5	-39.5	0.21 (3)	10.00			
K- J	0 / 102	0.0	0.0	0.08 (1)	10.00			
J- D	-454 / 0	0.0	0.0	0.06 (1)	7.81			
J- P	0 / 646	-39.5	-39.5	0.24 (2)	10.00			
P- Q	0 / 646	-39.5	-39.5	0.24 (2)	10.00			
Q- I	0 / 646	-39.5	-39.5	0.24 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.15 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	13-3-0	-38	-42	---	FRONT	VERT	DEAD	---	---
E	13-3-0	-13	-13	---	FRONT	VERT	TOTAL	---	---
E	13-3-0	-180	-180	---	FRONT	VERT	SNOW	---	---
I	13-4-4	-3	-4	---	FRONT	VERT	TOTAL	---	---
N	9-4-4	-6	-6	---	FRONT	VERT	TOTAL	---	---
O	11-4-4	-6	-6	---	FRONT	VERT	TOTAL	---	---
P	9-4-4	-12	-15	---	FRONT	VERT	TOTAL	---	---
Q	11-4-4	-3	-4	---	FRONT	VERT	TOTAL	---	---

DWG NO. TAM TM03815
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35/1.00 (C-D:1) , BC=0.24/1.00 (I-J:2) , WB=0.20/1.00 (F-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (I) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100099023
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T73A	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss. Burlington

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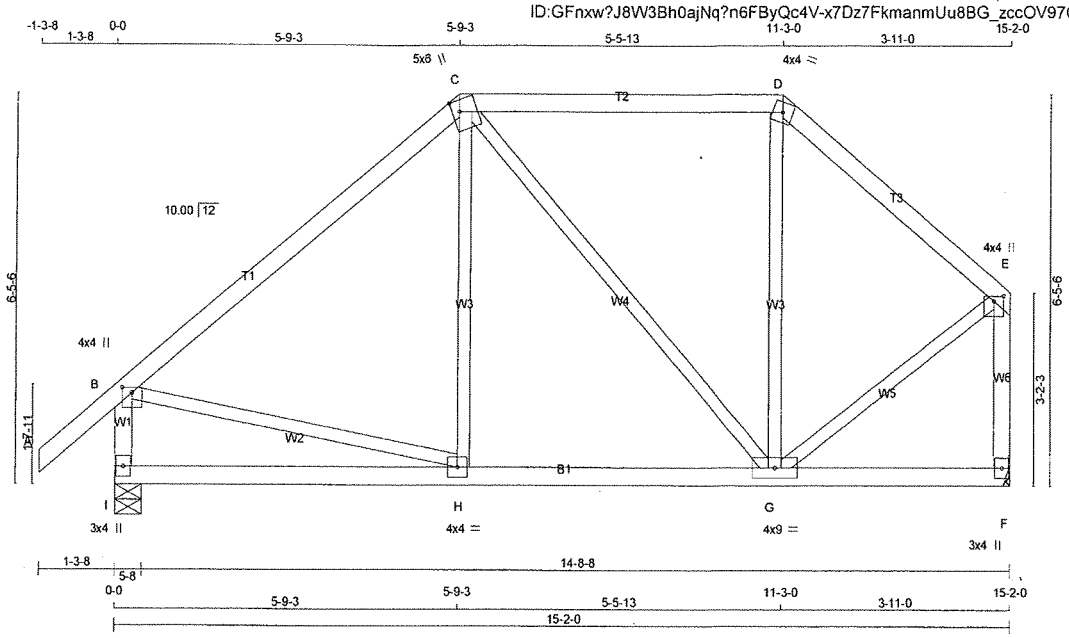


TOTAL WEIGHT = 2 X 69 = 138 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	996	0	996	0	0	5-8	5-8
F	889	0	889	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	754	375 / 0	159 / 0	0 / 0	0 / 0	220 / 0	0 / 0
F	679	317 / 0	159 / 0	0 / 0	0 / 0	203 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

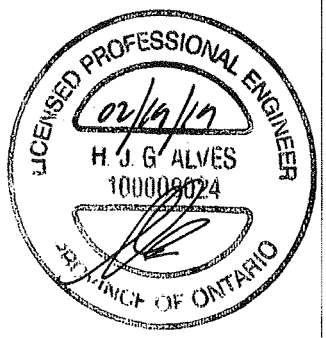
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	H- C	0 / 252	0.06 (3)
B- C	-703 / 0	-77.7	-77.7	0.34 (1)	6.25	C- G	-159 / 0	0.22 (1)
C- D	-436 / 0	-77.7	-77.7	0.30 (1)	6.25	G- D	0 / 139	0.03 (3)
D- E	-570 / 0	-77.7	-77.7	0.16 (1)	6.25	B- H	0 / 552	0.12 (1)
I- B	-903 / 0	0.0	0.0	0.10 (1)	7.81	G- E	0 / 535	0.12 (1)
F- E	-827 / 0	0.0	0.0	0.15 (1)	7.81			
I- H	0 / 0	-39.5	-39.5	0.24 (3)	10.00			
H- G	0 / 541	-39.5	-39.5	0.31 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.13 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.31/1.00 (G-H:2) , WB=0.22/1.00 (C-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

DWG NO. TAM T70734/16
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T74	6	1	UNITS 32 - 36 TRUSS DESC.	

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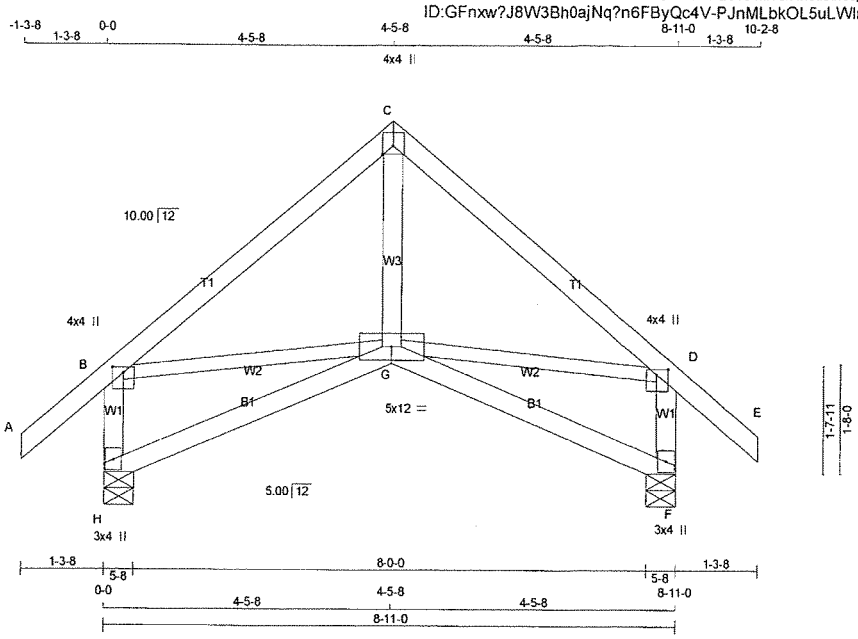


TOTAL WEIGHT = 6 X 42 = 252 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - G	2x3	DRY	No.2	SPF
G - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BBWWW-p	MT20	5.0	12.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	630	0	630	0	0	5-8	5-8
F	630	0	630	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0
F	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

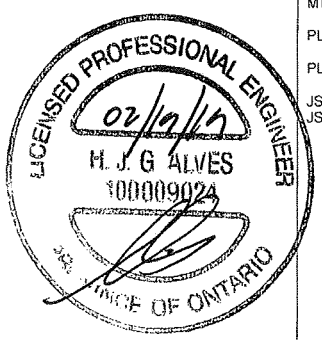
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 276	0.04 (3)
B- C	-454 / 0	-77.7	-77.7	0.20 (1)	6.25	B- G	0 / 350	0.08 (1)
C- D	-454 / 0	-77.7	-77.7	0.20 (1)	6.25	G- D	0 / 350	0.08 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-542 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-542 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.18/1.00 (G-H:3) , WB=0.08/1.00 (B-G:1) , SSI=0.10/1.00 (B-C:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

DWG NO. TAM P903817
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T75	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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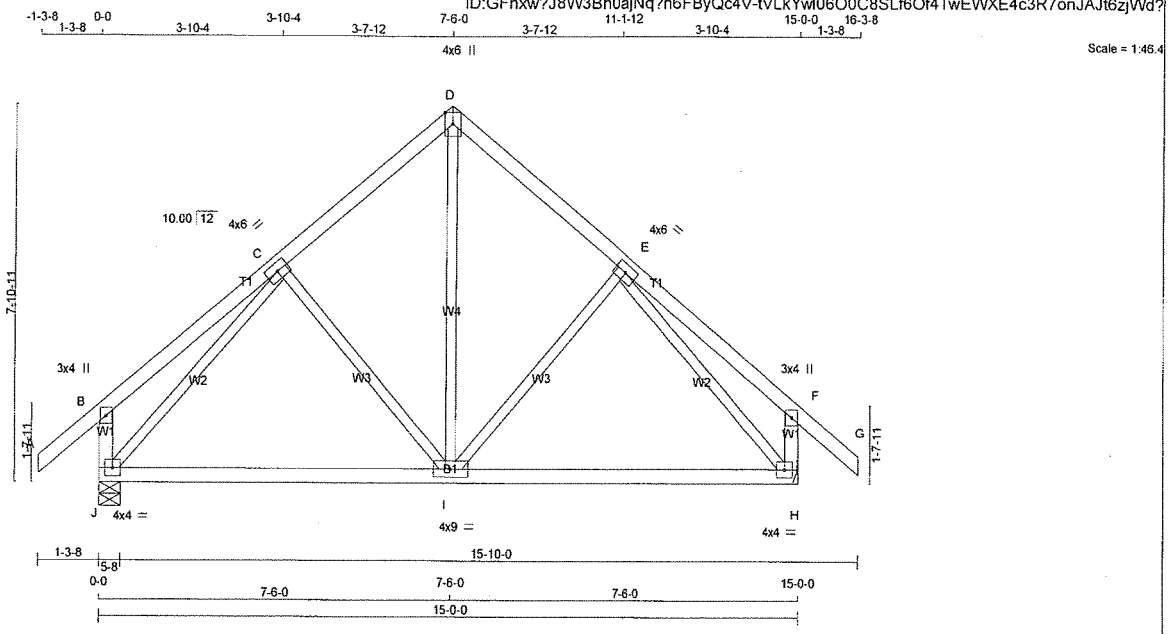

TOTAL WEIGHT = 4 X 70 = 280 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWVW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
J	987	0	987	0	0	5-8	5-8
H	987	0	987	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	747	371 / 0	158 / 0	0 / 0	0 / 0	218 / 0	0 / 0
H	747	371 / 0	158 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- D	0 / 571	0.13 (2)
B- C	0 / 21	-77.7	-77.7	0.17 (1)	10.00	I- E	-126 / 22	0.07 (1)
C- D	-635 / 0	-77.7	-77.7	0.14 (1)	6.25	C- I	-126 / 22	0.07 (1)
D- E	-635 / 0	-77.7	-77.7	0.14 (1)	6.25	J- C	-858 / 0	0.48 (1)
E- F	0 / 21	-77.7	-77.7	0.17 (1)	10.00	E- H	-858 / 0	0.48 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
J- B	-219 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-219 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 554	-39.5	-39.5	0.55 (3)	10.00			
I- H	0 / 554	-39.5	-39.5	0.55 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.55/1.00 (H-I:3) , WB=0.48/1.00 (C-J:1) , SSI=0.20/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (J) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

DWG NO. TAM T1903818
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	G75	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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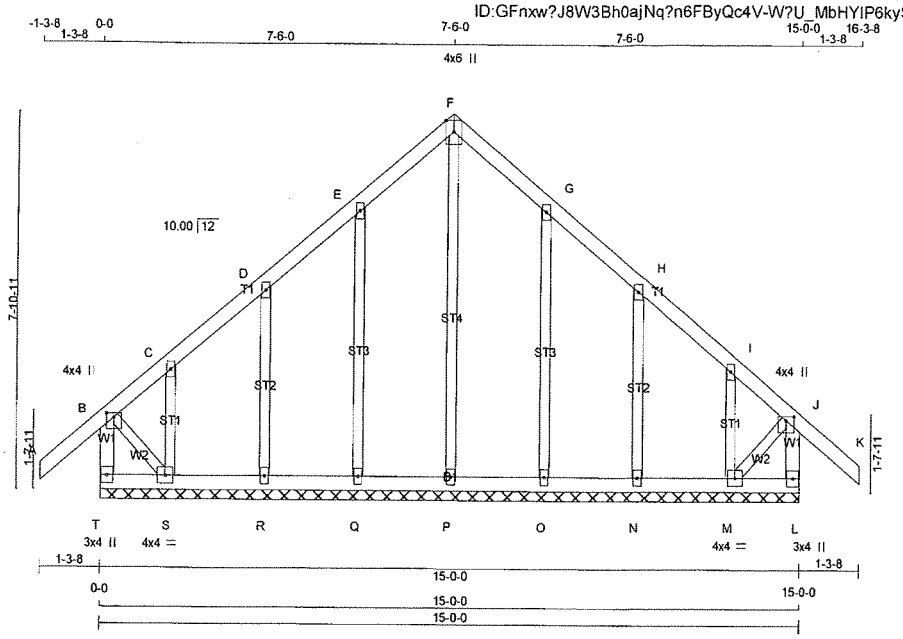

Scale = 1:46.4

TOTAL WEIGHT = 2 X 74 = 149 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
T- B	-241 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-54 / 0	-77.7	-77.7	0.10 (1)	6.25
C- D	-15 / 0	-77.7	-77.7	0.04 (1)	6.25
D- E	-15 / 0	-77.7	-77.7	0.04 (1)	6.25
E- F	-23 / 0	-77.7	-77.7	0.04 (1)	6.25
F- G	-23 / 0	-77.7	-77.7	0.04 (1)	6.25
G- H	-15 / 0	-77.7	-77.7	0.04 (1)	6.25
H- I	-15 / 0	-77.7	-77.7	0.04 (1)	6.25
I- J	-54 / 0	-77.7	-77.7	0.10 (1)	6.25
J- K	0 / 34	-77.7	-77.7	0.11 (1)	10.00
L- J	-241 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-39.5	-39.5	0.02 (3)	10.00
S- R	0 / 16	-39.5	-39.5	0.03 (3)	10.00
R- Q	0 / 13	-39.5	-39.5	0.03 (3)	10.00
Q- P	0 / 10	-39.5	-39.5	0.02 (3)	10.00
P- O	0 / 10	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 13	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 16	-39.5	-39.5	0.03 (3)	10.00
M- L	0 / 0	-39.5	-39.5	0.02 (3)	10.00

WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		
P- F	-115 / 0	0.13 (1)
Q- E	-174 / 0	0.11 (1)
R- D	-160 / 0	0.05 (1)
S- C	-74 / 0	0.01 (1)
O- G	-174 / 0	0.11 (1)
N- H	-160 / 0	0.05 (1)
M- I	-74 / 0	0.01 (1)
B- S	0 / 25	0.01 (1)
M- J	0 / 25	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (M-N:3) , WB=0.13/1.00 (F-P:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

DWG NO. TAM 119038/19
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T76	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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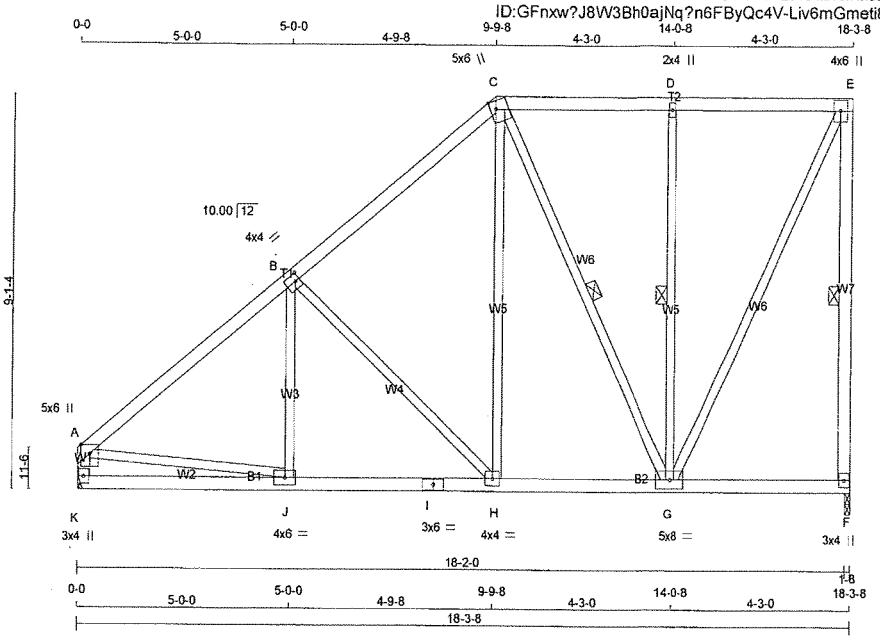


TOTAL WEIGHT = 4 X 98 = 390 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
K	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

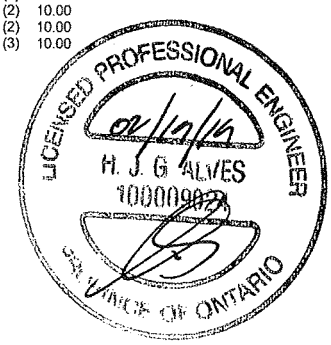
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1119 / 0	-77.7	-77.7	0.26 (1)	5.70
B- C	-755 / 0	-77.7	-77.7	0.24 (1)	6.25
C- D	-425 / 0	-77.7	-77.7	0.17 (1)	6.25
D- E	-426 / 0	-77.7	-77.7	0.17 (1)	6.25
F- E	-1002 / 0	0.0	0.0	0.39 (1)	6.25
K- A	-992 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-39.5	-39.5	0.18 (3)	10.00
J- I	0 / 882	-39.5	-39.5	0.30 (2)	10.00
I- H	0 / 882	-39.5	-39.5	0.30 (2)	10.00
H- G	0 / 562	-39.5	-39.5	0.20 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.13 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 226	0.05 (3)
B- H	-455 / 0	0.40 (1)
H- C	0 / 496	0.11 (2)
C- G	-316 / 0	0.21 (1)
G- D	-405 / 0	0.22 (1)
G- E	0 / 972	0.22 (1)
A- J	0 / 888	0.20 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.30/1.00 (H-J:2) , WB=0.40/1.00 (B-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (G) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DWG NO. TAM T1903820
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T77	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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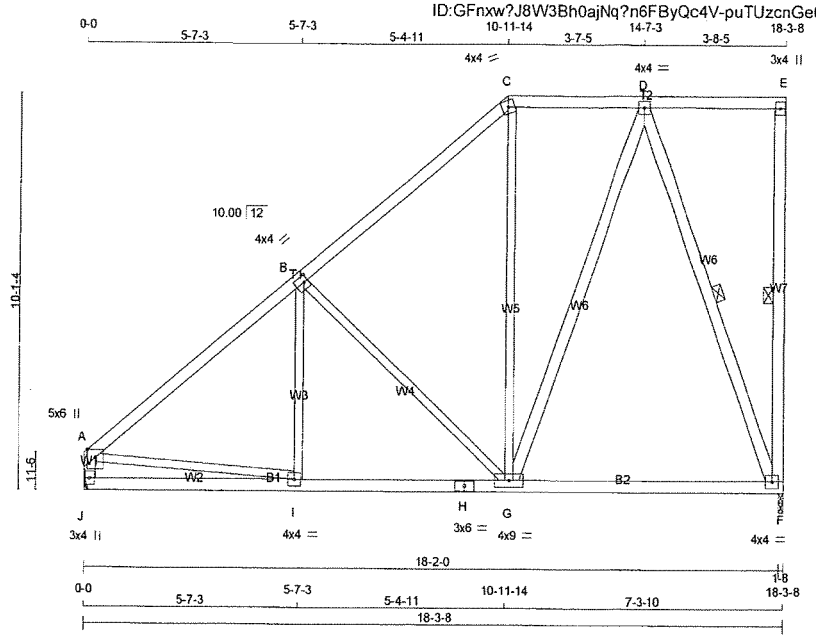


TOTAL WEIGHT = 102 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

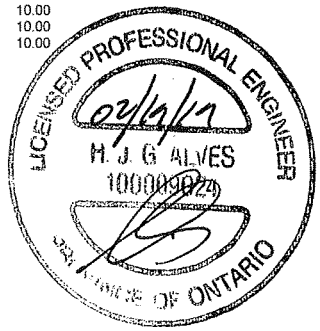
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1089 / 0	-77.7	-77.7	0.33 (1)	5.66	I- B	0 / 229	0.05 (3)
B- C	-679 / 0	-77.7	-77.7	0.31 (1)	6.25	B- G	-509 / 0	0.59 (1)
C- D	-496 / 0	-77.7	-77.7	0.13 (1)	6.25	G- C	0 / 187	0.04 (3)
D- E	0 / 0	-77.7	-77.7	0.17 (1)	10.00	G- D	0 / 528	0.08 (1)
F- E	-110 / 0	0.0	0.0	0.05 (1)	6.25	D- F	-904 / 0	0.52 (1)
J- A	-978 / 0	0.0	0.0	0.10 (1)	7.81	A- I	0 / 866	0.19 (1)
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00			
I- H	0 / 861	-39.5	-39.5	0.45 (2)	10.00			
H- G	0 / 861	-39.5	-39.5	0.45 (2)	10.00			
G- F	0 / 318	-39.5	-39.5	0.36 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.45/1.00 (G-I:2) , WB=0.59/1.00 (B-G:1) , SSI=0.17/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.44 (H) (INPUT = 1.00)

DWG NO. TAM T1903821
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	G78	2	1	TRUSS DESC.	

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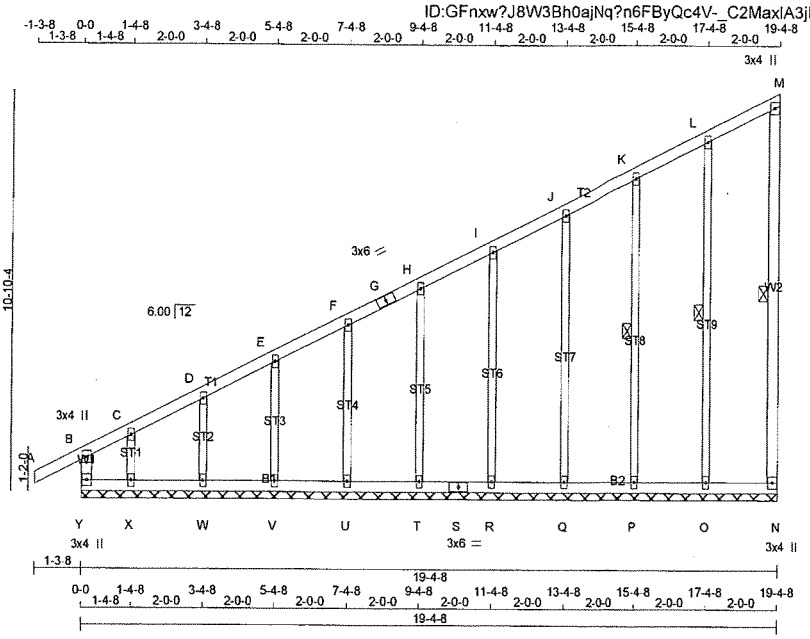


TOTAL WEIGHT = 2 X 100 = 200 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K, L						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, R, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

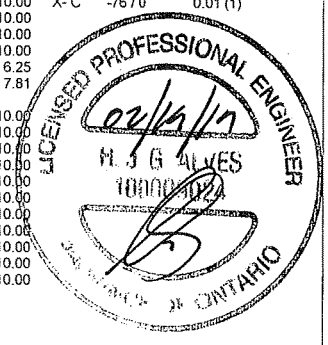
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF M-N, L-O, K-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	O- L	-168 / 0	0.11 (1)
B- C	-41 / 0	-77.7	-77.7	0.09 (1)	6.25	P- K	-154 / 0	0.08 (1)
C- D	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	Q- J	-155 / 0	0.18 (1)
D- E	-16 / 0	-77.7	-77.7	0.04 (1)	6.25	R- I	-155 / 0	0.12 (1)
E- F	-12 / 0	-77.7	-77.7	0.04 (1)	6.25	T- H	-154 / 0	0.08 (1)
F- G	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	U- F	-155 / 0	0.05 (1)
G- H	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	V- E	-151 / 0	0.04 (1)
H- I	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	W- D	-166 / 0	0.03 (1)
I- J	-5 / 0	-77.7	-77.7	0.04 (1)	10.00	X- C	-76 / 0	0.01 (1)
J- K	-4 / 0	-77.7	-77.7	0.04 (1)	10.00			
K- L	-2 / 0	-77.7	-77.7	0.04 (1)	10.00			
L- M	-6 / 0	-77.7	-77.7	0.04 (1)	10.00			
N- M	-70 / 0	0.0	0.0	0.01 (1)	6.25			
Y- B	-207 / 0	0.0	0.0	0.04 (1)	7.81			
Y- X	0 / 21	-39.5	-39.5	0.04 (1)	10.00			
X- W	0 / 18	-39.5	-39.5	0.03 (2)	10.00			
W- V	0 / 13	-39.5	-39.5	0.03 (2)	10.00			
V- U	0 / 10	-39.5	-39.5	0.02 (3)	10.00			
U- T	0 / 8	-39.5	-39.5	0.02 (3)	10.00			
T- S	0 / 6	-39.5	-39.5	0.02 (3)	10.00			
S- R	0 / 6	-39.5	-39.5	0.02 (3)	10.00			
R- Q	0 / 5	-39.5	-39.5	0.02 (3)	10.00			
Q- P	0 / 4	-39.5	-39.5	0.02 (3)	10.00			
P- O	0 / 2	-39.5	-39.5	0.03 (3)	10.00			
O- N	0 / 1	-39.5	-39.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.04/1.00 (X-Y:1) , WB=0.18/1.00 (J-Q:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (L) (INPUT = 0.90)
JSI METAL= 0.07 (L) (INPUT = 1.00)

DWG NO. TAM T1903822
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	G79	2	1	UNITS 32 - 36 TRUSS DESC.	

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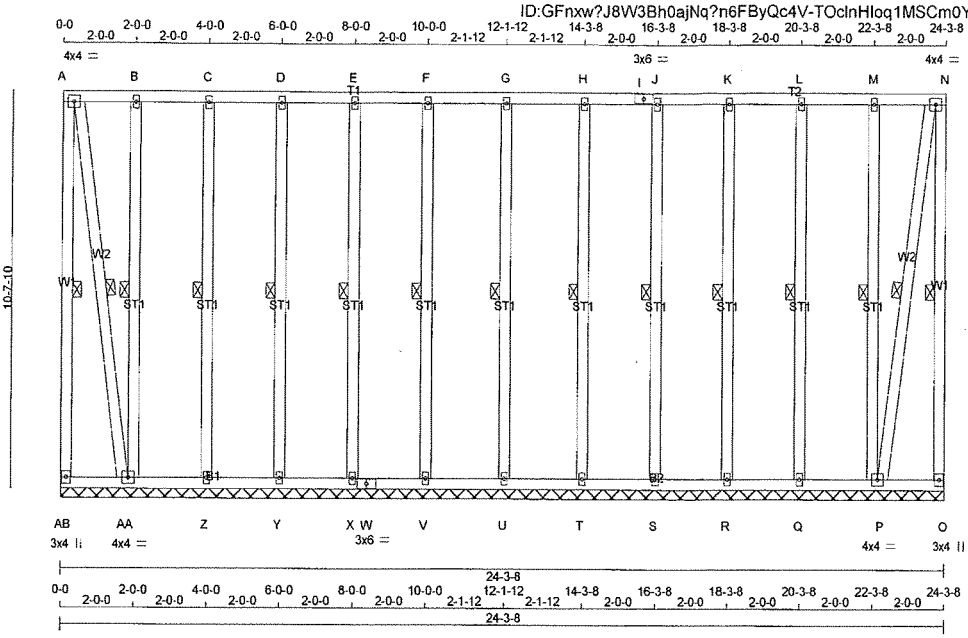


TOTAL WEIGHT = 2 X 228 = 455 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AB- A	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - N	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
AB- W	2x4	DRY	No.2	SPF
W - O	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B, C, D, E, F, G, H, J, K, L, M						
B	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	3.0	6.0		
N	TMVW-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
Q, R, S, T, U, V, X, Y, Z						
Q	BMW1+w	MT20	2.0	4.0		
W	BS-t	MT20	3.0	6.0		
AA	BMWW1-t	MT20	4.0	4.0		
AB	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

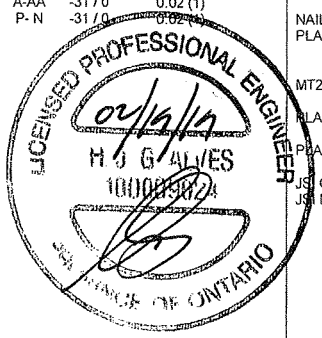
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-AB, N-O, G-U, M-P, L-Q, K-R, J-S, H-T, B-AA, C-Z, D-Y, E-X, F-V, A-AA, N-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AB- A	-33 / 0	0.0	0.0	0.02 (1)	6.25	U- G	-169 / 0	0.10 (1)
A- B	0 / 5	-77.7	-77.7	0.04 (1)	10.00	P- M	-171 / 0	0.10 (1)
B- C	0 / 0	-77.7	-77.7	0.04 (1)	10.00	Q- L	-154 / 0	0.09 (1)
C- D	0 / 0	-77.7	-77.7	0.04 (1)	10.00	R- K	-156 / 0	0.09 (1)
D- E	0 / 0	-77.7	-77.7	0.04 (1)	10.00	S- J	-154 / 0	0.09 (1)
E- F	0 / 0	-77.7	-77.7	0.04 (1)	10.00	T- H	-161 / 0	0.09 (1)
F- G	0 / 0	-77.7	-77.7	0.04 (1)	10.00	AA- B	-171 / 0	0.10 (1)
G- H	0 / 0	-77.7	-77.7	0.04 (1)	10.00	Z- C	-154 / 0	0.09 (1)
H- I	0 / 0	-77.7	-77.7	0.04 (1)	10.00	Y- D	-156 / 0	0.09 (1)
I- J	0 / 0	-77.7	-77.7	0.04 (1)	10.00	X- E	-154 / 0	0.09 (1)
J- K	0 / 0	-77.7	-77.7	0.04 (1)	10.00	V- F	-161 / 0	0.09 (1)
K- L	0 / 0	-77.7	-77.7	0.04 (1)	10.00	A-AA	-31 / 0	0.02 (1)
L- M	0 / 0	-77.7	-77.7	0.04 (1)	10.00	P- N	-31 / 0	0.02 (1)
M- N	0 / 5	-77.7	-77.7	0.04 (1)	10.00			
O- N	-33 / 0	0.0	0.0	0.02 (1)	6.25			
AB-AA	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
AA- Z	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
Z- Y	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
Y- X	0 / 0	-39.5	-39.5	0.02 (3)	10.00			
X- W	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
W- V	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
V- U	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
U- T	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
T- S	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
S- R	0 / 0	-39.5	-39.5	0.02 (3)	10.00			
R- Q	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
Q- P	0 / 0	-39.5	-39.5	0.03 (3)	10.00			
P- O	0 / 0	-39.5	-39.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (G-H:1) , BC=0.03/1.00 (Z-AA:3) , WB=0.10/1.00 (M-P:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (AA) (INPUT = 0.90)
JSI METAL= 0.04 (M) (INPUT = 1.00)

DWG NO. TAM T1903823
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T80	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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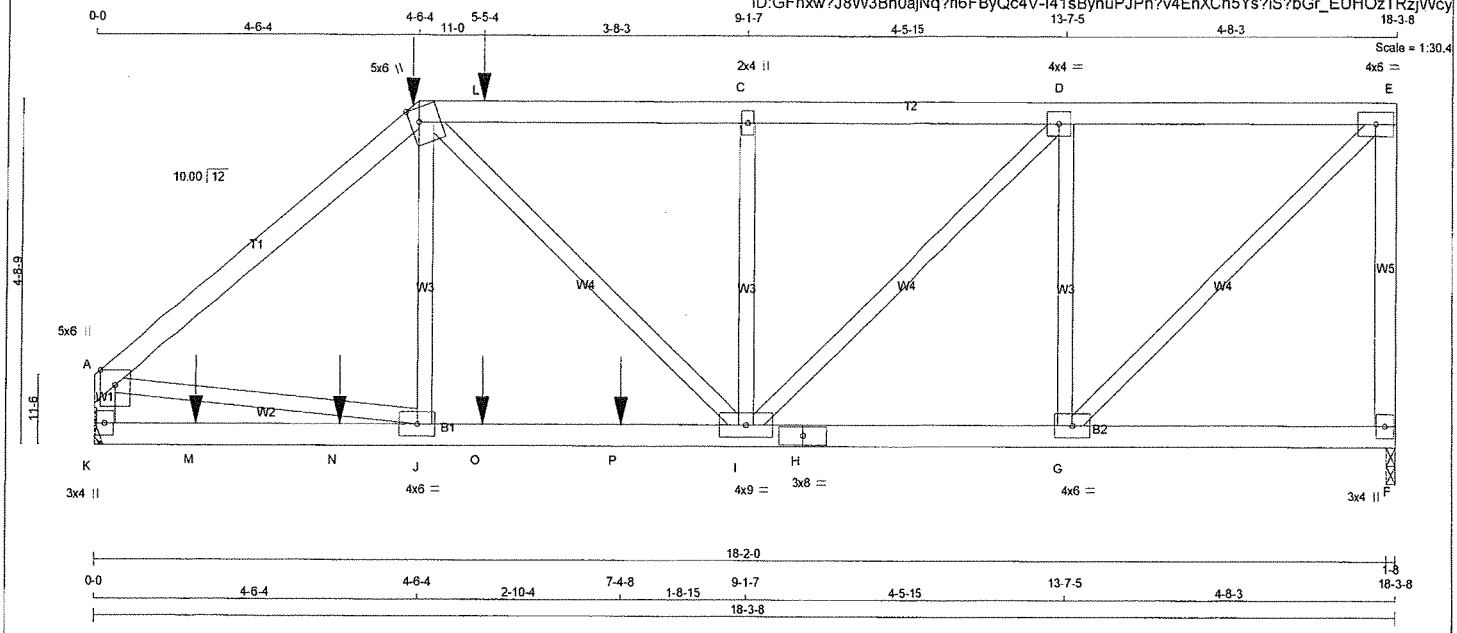

TOTAL WEIGHT = 2 X 77 = 154 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	1	12	SIDE(0.0)
H- F	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	8.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1753	0	1753	0	0	1-8	1-8
K	2366	0	2366	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1329	656 / 0	287 / 0	0 / 0	0 / 0	385 / 0	0 / 0
K	1790	897 / 0	382 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2887 / 0	-77.7	-77.7	0.20	(1)	5.22	J- B	0 / 642	0.08 (2)
B- L	-2673 / 0	-77.7	-77.7	0.18	(1)	5.39	G- E	0 / 2189	0.27 (1)
L- C	-2673 / 0	-77.7	-77.7	0.18	(1)	5.39	B- I	0 / 618	0.08 (1)
C- D	-2673 / 0	-77.7	-77.7	0.15	(1)	5.43	G- D	-1452 / 0	0.25 (1)
D- E	-1592 / 0	-77.7	-77.7	0.13	(1)	6.25	I- C	-394 / 0	0.07 (1)
F- E	-1653 / 0	0.0	0.0	0.31	(1)	7.81	I- D	0 / 1517	0.19 (1)
K- A	-2323 / 0	0.0	0.0	0.13	(1)	7.35	A- J	0 / 2282	0.28 (1)
K- M	0 / 0	-39.5	-39.5	0.35	(2)	10.00			
M- N	0 / 0	-39.5	-39.5	0.35	(2)	10.00			
N- J	0 / 0	-39.5	-39.5	0.35	(2)	10.00			
J- O	0 / 2227	-39.5	-39.5	0.92	(1)	10.00			
O- P	0 / 2227	-39.5	-39.5	0.92	(1)	10.00			
P- I	0 / 2227	-39.5	-39.5	0.92	(1)	10.00			
I- H	0 / 1592	-39.5	-39.5	0.53	(1)	10.00			
H- G	0 / 1592	-39.5	-39.5	0.53	(1)	10.00			
G- F	0 / 0	-39.5	-39.5	0.10	(2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
B	4-6-4	-63	-70	---	FRONT	VERT	DEAD	---	---
B	4-6-4	-375	-375	---	FRONT	VERT	SNOW	---	---
L	5-5-4	-97	-97	---	FRONT	VERT	TOTAL	---	---
M	1-5-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
N	3-5-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
O	5-5-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
P	7-4-8	-1269	-1269	---	FRONT	VERT	TOTAL	---	---

DWG NO. TAM T1903824
STRUCTURAL COMPONENT ONLY

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G. ALVES
100078824
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.31/1.00 (E-F:1) , BC=0.92/1.00 (I-J:1) , WB=0.28/1.00 (A-J:1) , SSI=0.38/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (A) (INPUT = 0.90)
JSI METAL= 0.71 (H) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T81	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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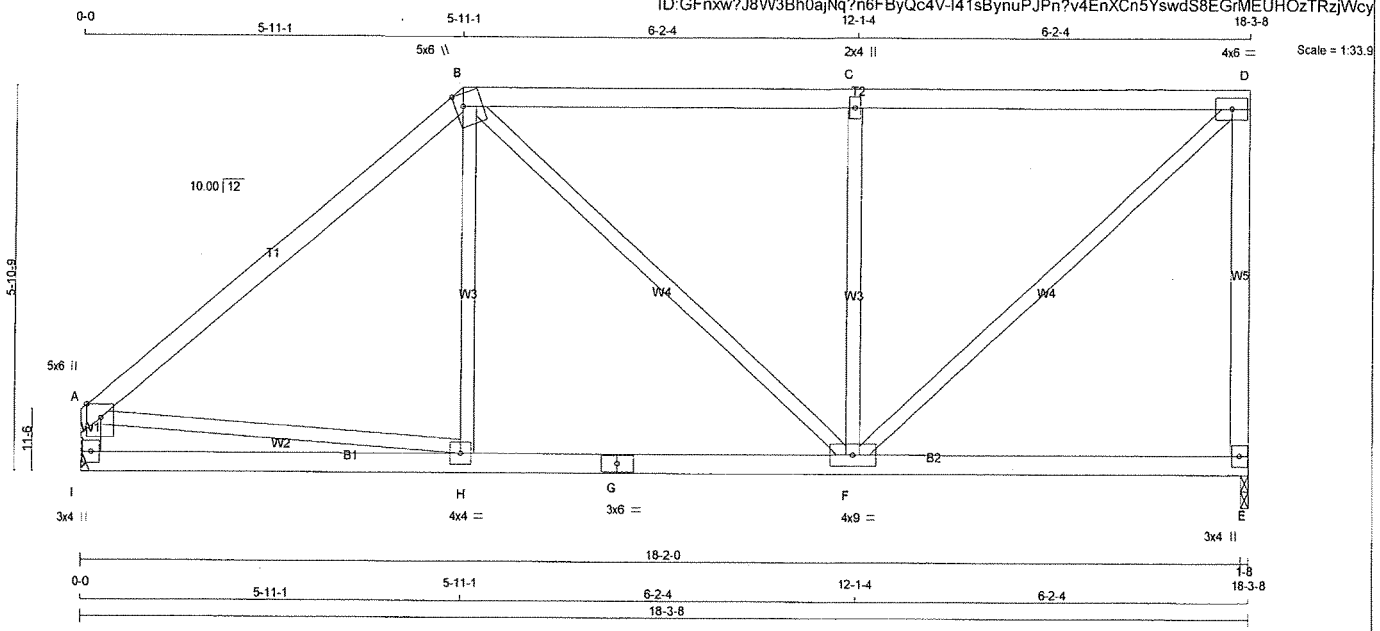

TOTAL WEIGHT = 77 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1036 / 0	-77.7	-77.7	0.55 (1)	5.40	H- B	0 / 283	0.07 (3)
B- C	-877 / 0	-77.7	-77.7	0.53 (1)	5.70	B- F	0 / 107	0.02 (1)
C- D	-877 / 0	-77.7	-77.7	0.54 (1)	5.70	F- C	-594 / 0	0.32 (1)
E- D	-972 / 0	0.0	0.0	0.63 (1)	7.81	F- D	0 / 1179	0.27 (1)
I- A	-976 / 0	0.0	0.0	0.10 (1)	7.81	A- H	0 / 799	0.18 (1)
I- H	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
H- G	0 / 798	-39.5	-39.5	0.37 (2)	10.00			
G- F	0 / 798	-39.5	-39.5	0.37 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.37/1.00 (F-H:2) , WB=0.32/1.00 (C-F:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

DWG NO. TAM TN03825
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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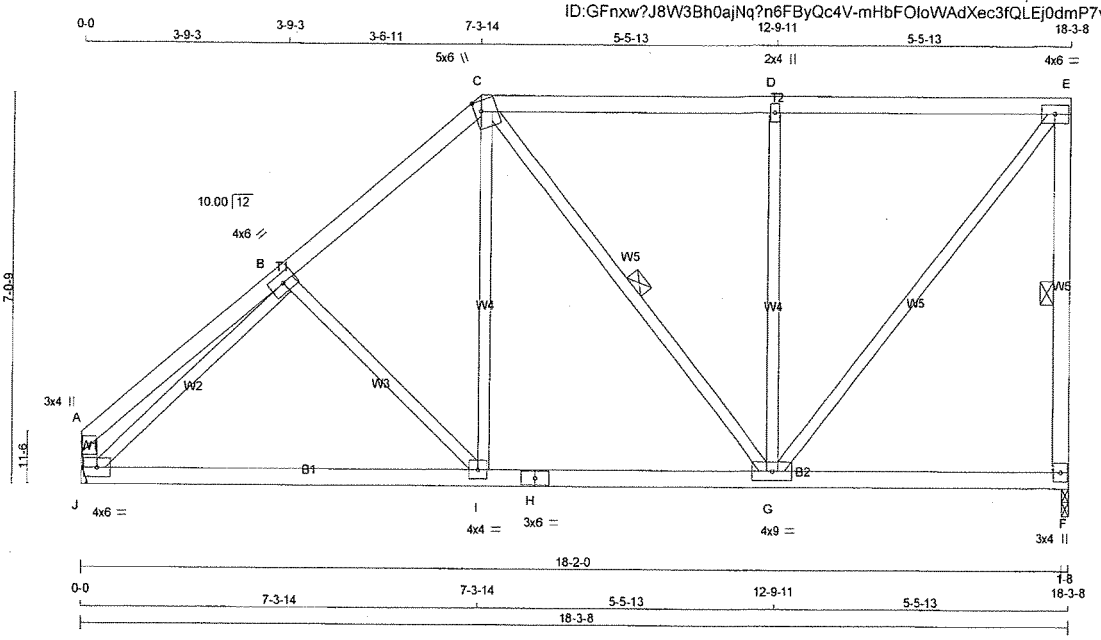


TOTAL WEIGHT = 84 lb

[M

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

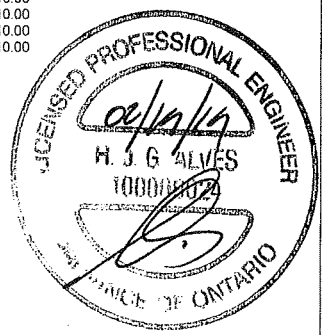
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.16 (1)	10.00	B- I	-162 / 3	0.07 (1)
B- C	-963 / 0	-77.7	-77.7	0.17 (1)	6.12	I- C	0 / 445	0.10 (2)
C- D	-667 / 0	-77.7	-77.7	0.40 (1)	6.25	C- G	-111 / 0	0.06 (2)
D- E	-667 / 0	-77.7	-77.7	0.40 (1)	6.25	G- D	-526 / 0	0.45 (1)
F- E	-979 / 0	0.0	0.0	0.22 (1)	6.25	G- E	0 / 1054	0.24 (1)
J- A	-110 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1188 / 0	0.51 (1)
J- I	0 / 840	-39.5	-39.5	0.45 (2)	10.00			
I- H	0 / 729	-39.5	-39.5	0.44 (2)	10.00			
H- G	0 / 729	-39.5	-39.5	0.44 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.45/1.00 (I-J:2) , WB=0.51/1.00 (B-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.50 (H) (INPUT = 1.00)

DWG NO. TAM 1903826
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T83	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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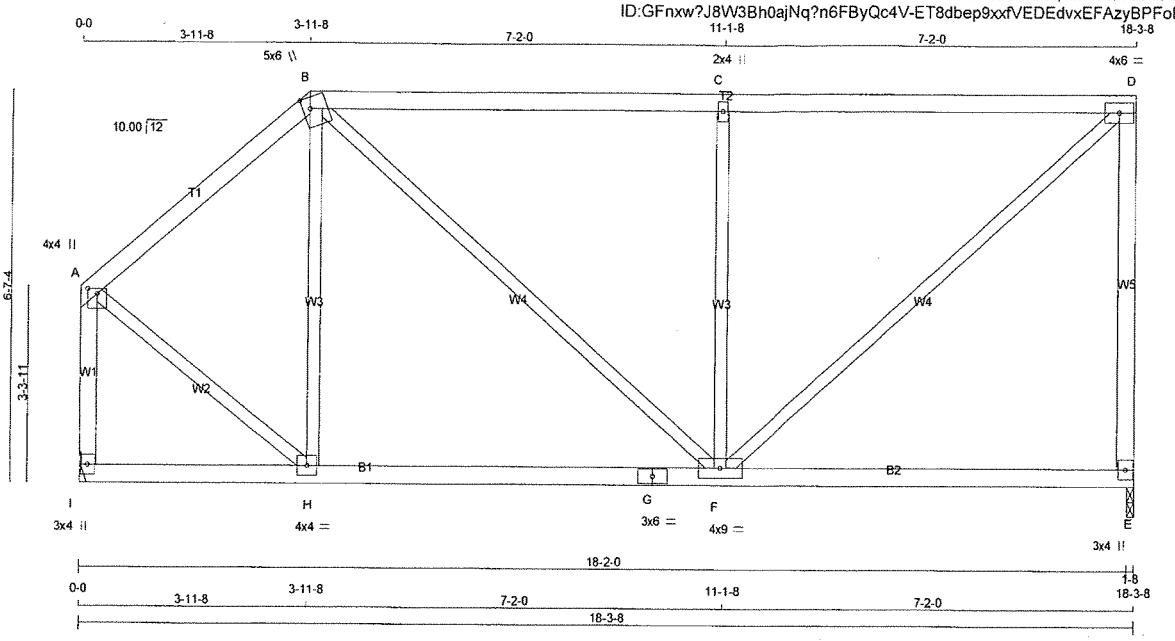


TOTAL WEIGHT = 82 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

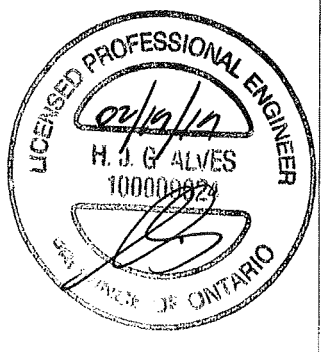
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-720 / 0	-77.7	-77.7		0.22 (1)	6.25	H- B	-170 / 61	0.12 (1)
B- C	-852 / 0	-77.7	-77.7		0.74 (1)	5.26	B- F	0 / 405	0.09 (1)
C- D	-852 / 0	-77.7	-77.7		0.74 (1)	5.26	F- C	-690 / 0	0.50 (1)
E- D	-959 / 0	0.0	0.0		0.87 (1)	7.81	F- D	0 / 1133	0.25 (1)
I- A	-1020 / 0	0.0	0.0		0.19 (1)	7.76	A- H	0 / 683	0.15 (1)
I- H	0 / 0	-39.5	-39.5		0.21 (3)	10.00			
H- G	0 / 549	-39.5	-39.5		0.45 (2)	10.00			
G- F	0 / 549	-39.5	-39.5		0.45 (2)	10.00			
F- E	0 / 0	-39.5	-39.5		0.39 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.87/1.00 (D-E:1) , BC=0.45/1.00 (F-H:2) , WB=0.50/1.00 (C-F:1) , SSI=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (A) (INPUT = 0.90)
JSI METAL= 0.36 (G) (INPUT = 1.00)

DWG NO. TAM T1903827
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T84	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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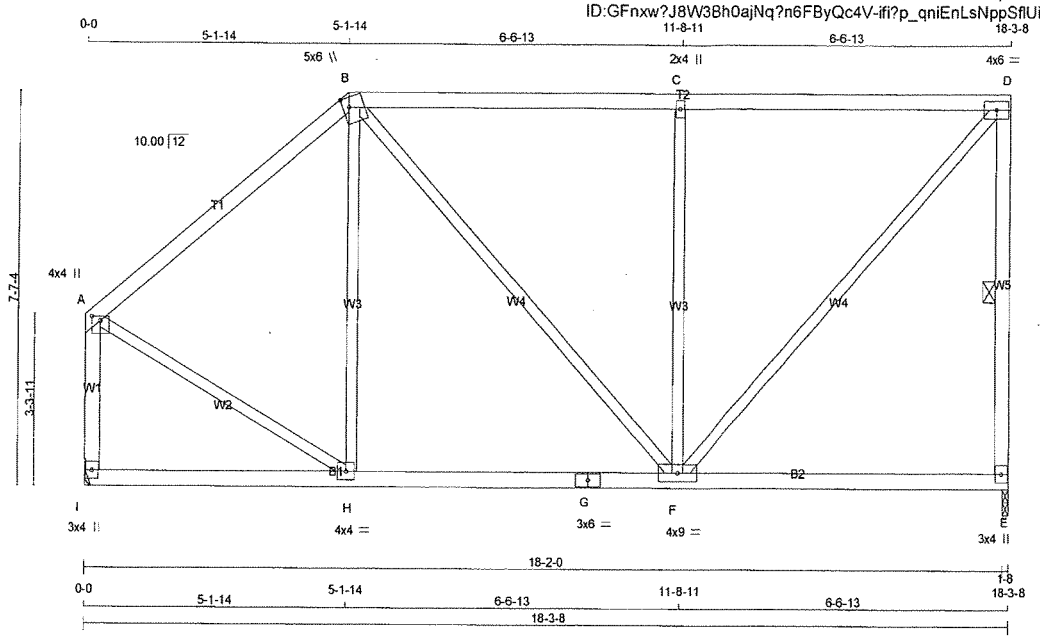

TOTAL WEIGHT = 86 lb

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

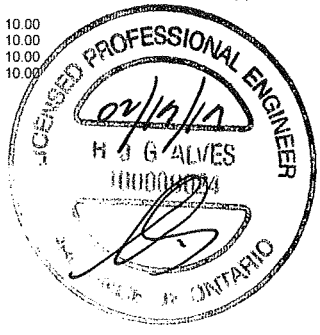
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-736 / 0	-77.7	-77.7	0.39 (1)	6.25	H- B	-72 / 128	0.08 (1)
B- C	-698 / 0	-77.7	-77.7	0.60 (1)	6.04	B- F	0 / 202	0.05 (1)
C- D	-698 / 0	-77.7	-77.7	0.60 (1)	6.04	F- C	-632 / 0	0.67 (1)
E- D	-967 / 0	0.0	0.0	0.25 (1)	6.25	F- D	0 / 1041	0.23 (1)
I- A	-991 / 0	0.0	0.0	0.19 (1)	7.81	A- H	0 / 648	0.15 (1)
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00			
H- G	0 / 563	-39.5	-39.5	0.38 (2)	10.00			
G- F	0 / 563	-39.5	-39.5	0.38 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.32 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.60/1.00 (C-D:1) , BC=0.38/1.00 (F-H:2) , WB=0.67/1.00 (C-F:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (A) (INPUT = 0.90)
JSI METAL= 0.23 (A) (INPUT = 1.00)

DWG NO. TAM T1403828
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T85	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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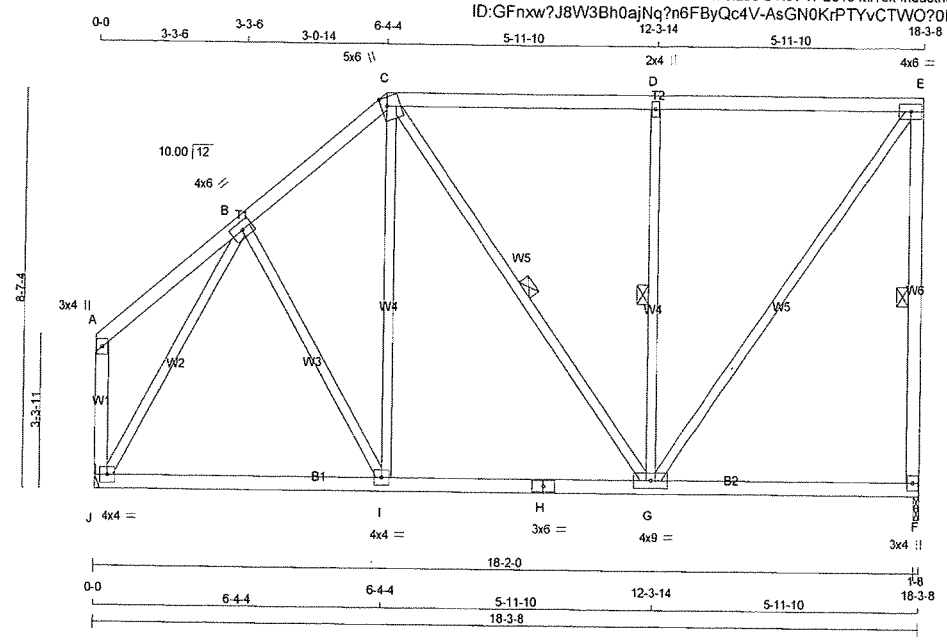


TOTAL WEIGHT = 96 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

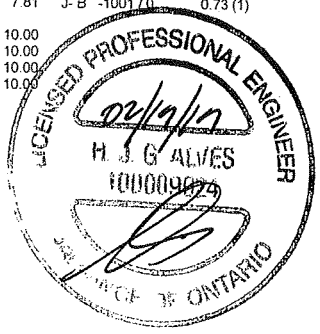
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-77.7	-77.7	0.12 (1)	10.00	B- I	0 / 144	0.03 (3)
B- C	-741 / 0	-77.7	-77.7	0.13 (1)	6.25	I- C	0 / 223	0.05 (3)
C- D	-576 / 0	-77.7	-77.7	0.48 (1)	6.25	C- G	-11 / 34	0.01 (3)
D- E	-576 / 0	-77.7	-77.7	0.48 (1)	6.25	G- D	-574 / 0	0.27 (1)
F- E	-974 / 0	0.0	0.0	0.33 (1)	6.25	G- E	0 / 983	0.22 (1)
J- A	-98 / 0	0.0	0.0	0.02 (1)	7.81	J- B	-1001 / 0	0.73 (1)
J- I	0 / 495	-39.5	-39.5	0.33 (2)	10.00			
I- H	0 / 556	-39.5	-39.5	0.35 (2)	10.00			
H- G	0 / 556	-39.5	-39.5	0.35 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.24 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.35/1.00 (G-I:2) , WB=0.73/1.00 (B-J:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM 1403824
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T86	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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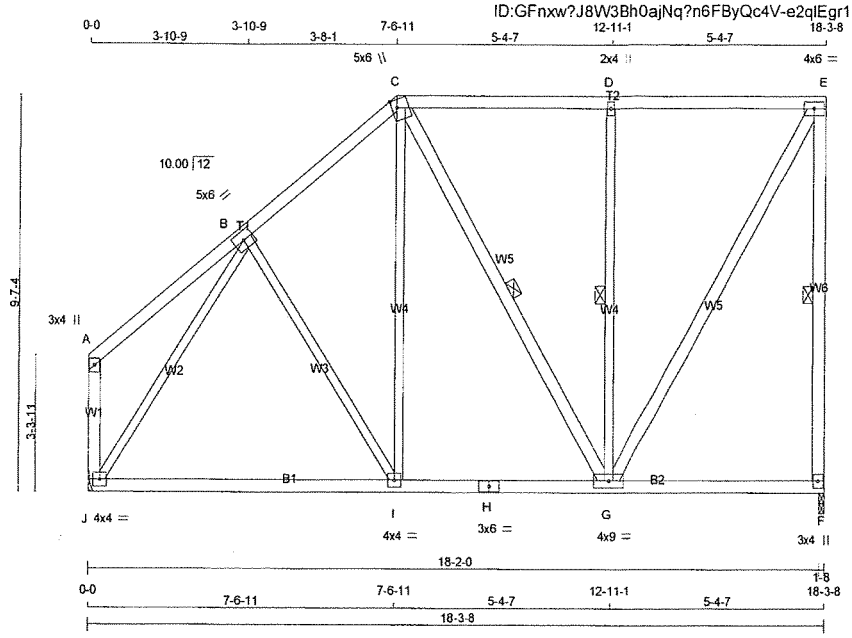


Scale = 1:53.8

TOTAL WEIGHT = 108 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

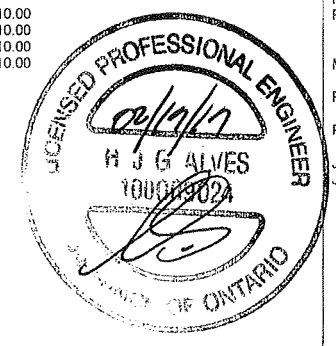
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-77.7	-77.7	0.18 (1)	10.00	B- I	0 / 103	0.02 (3)
B- C	-716 / 0	-77.7	-77.7	0.19 (1)	6.25	I- C	0 / 324	0.07 (2)
C- D	-476 / 0	-77.7	-77.7	0.38 (1)	6.25	C- G	-128 / 0	0.08 (2)
D- E	-476 / 0	-77.7	-77.7	0.38 (1)	6.25	G- D	-516 / 0	0.32 (1)
F- E	-980 / 0	0.0	0.0	0.43 (1)	6.25	G- E	0 / 948	0.15 (1)
J- A	-111 / 0	0.0	0.0	0.02 (1)	7.81	J- B	-981 / 0	0.95 (1)
J- I	0 / 514	-39.5	-39.5	0.43 (2)	10.00			
I- H	0 / 535	-39.5	-39.5	0.43 (2)	10.00			
H- G	0 / 535	-39.5	-39.5	0.43 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 948 (0.23")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.43/1.00 (G-I:2) , WB=0.95/1.00 (B-J:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (J) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM 7903830
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T87	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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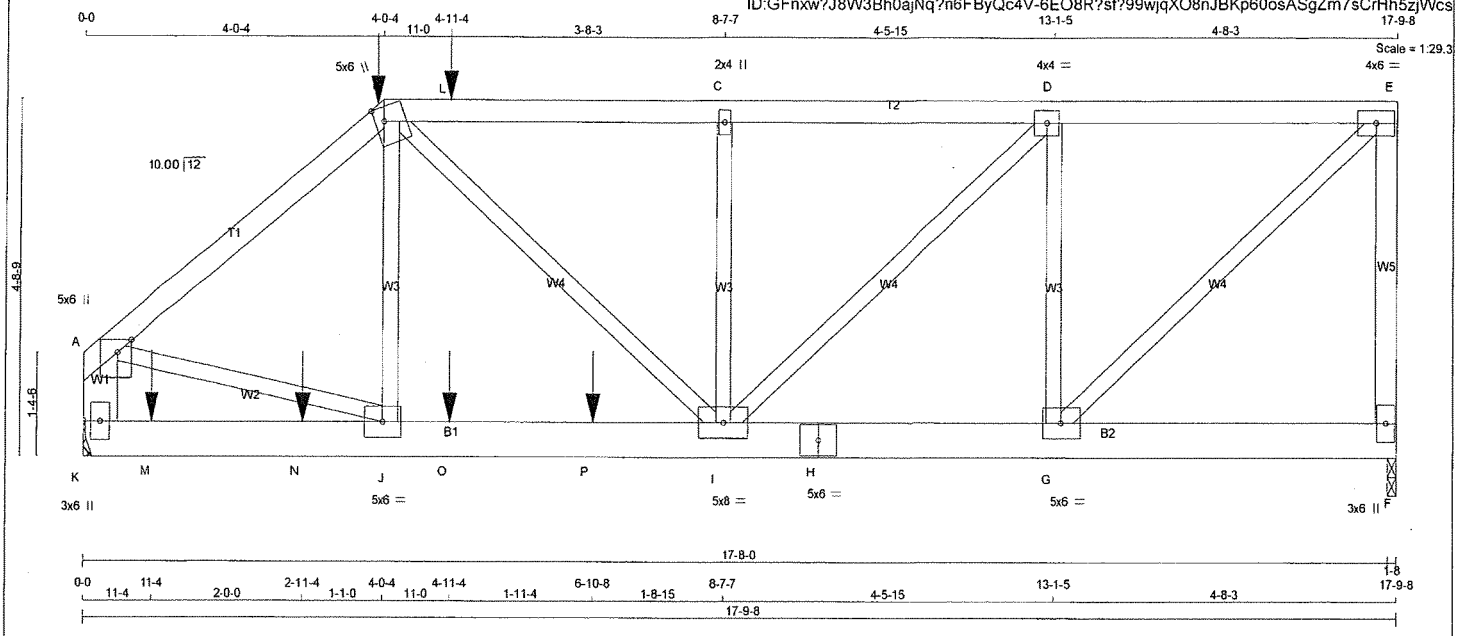

TOTAL WEIGHT = 2 X 87 = 174 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(0.0)
H- F	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1667	0	1667	0	0	1-8	1-8
K	2306	0	2306	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1265	619 / 0	277 / 0	0 / 0	0 / 0	369 / 0	0 / 0
K	1749	863 / 0	383 / 0	0 / 0	0 / 0	504 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2586 / 0	-77.7	-77.7	0.15 (1)		5.49
B- L	-2526 / 0	-77.7	-77.7	0.17 (1)		5.51
L- C	-2526 / 0	-77.7	-77.7	0.17 (1)		5.51
C- D	-2526 / 0	-77.7	-77.7	0.15 (1)		5.55
D- E	-1528 / 0	-77.7	-77.7	0.13 (1)		6.25
F- E	-1566 / 0	0.0	0.0	0.27 (1)		7.81
K- A	-2272 / 0	0.0	0.0	0.08 (1)		7.81
K- M	0 / 0	-39.5	-39.5	0.16 (1)		10.00
M- N	0 / 0	-39.5	-39.5	0.16 (1)		10.00
N- J	0 / 0	-39.5	-39.5	0.16 (1)		10.00
J- O	0 / 1992	-39.5	-39.5	0.51 (1)		10.00
O- P	0 / 1992	-39.5	-39.5	0.51 (1)		10.00
P- I	0 / 1992	-39.5	-39.5	0.51 (1)		10.00
I- H	0 / 1528	-39.5	-39.5	0.26 (1)		10.00
H- G	0 / 1528	-39.5	-39.5	0.26 (1)		10.00
G- F	0 / 0	-39.5	-39.5	0.05 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 488	0.06 (2)
A- J	0 / 2033	0.25 (1)
G- E	0 / 2084	0.26 (1)
B- I	0 / 724	0.09 (1)
G- D	-1852 / 0	0.23 (1)
I- C	-398 / 0	0.05 (1)
D- I	0 / 1387	0.17 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-0-4	-50	-50	---	FRONT	VERT	DEAD	---	---
B	4-0-4	-300	-300	---	FRONT	VERT	SNOW	---	---
L	4-11-4	-97	-97	---	BACK	VERT	TOTAL	---	---
M	11-4	-58	-74	---	BACK	VERT	TOTAL	---	---
N	2-11-4	-57	-72	---	BACK	VERT	TOTAL	---	---
O	4-11-4	-57	-72	---	BACK	VERT	TOTAL	---	---
P	6-10-8	-1269	-1269	---	BACK	VERT	TOTAL	---	---

DWG NO. TAM T190383/
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.51/1.00 (I-J:1) , WB=0.26/1.00 (E-G:1) , SSI=0.29/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (A) (INPUT = 0.90)
JSI METAL= 0.25 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T88	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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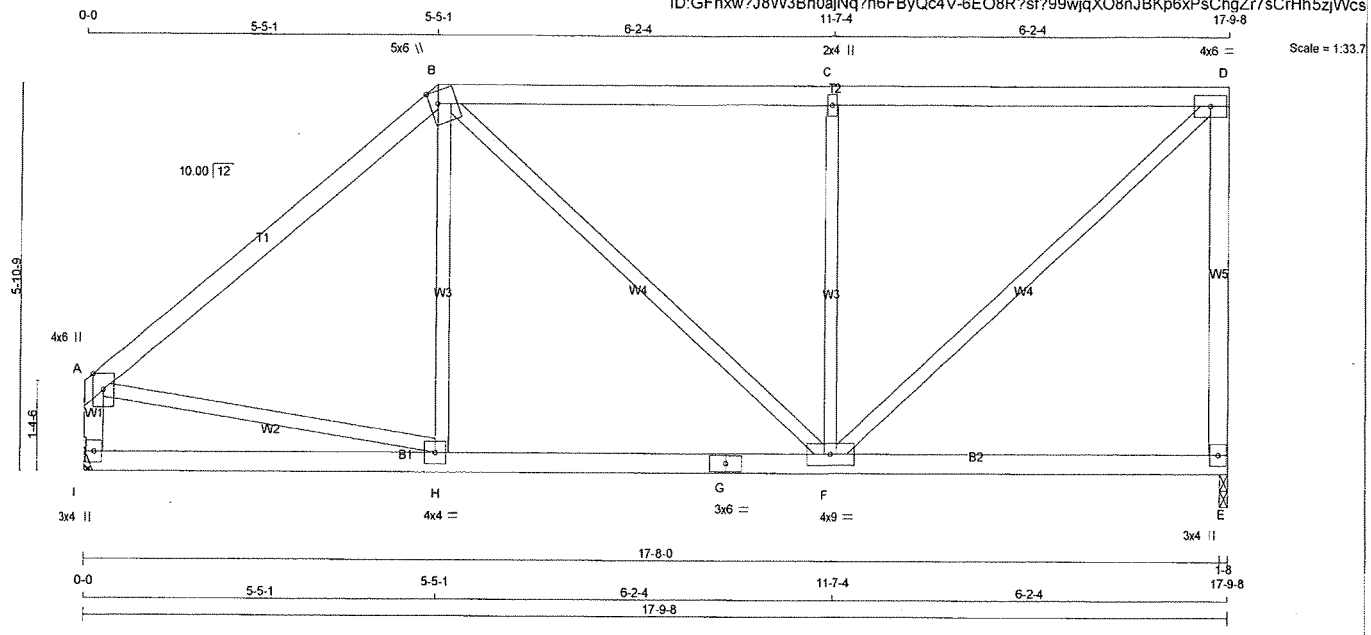


TOTAL WEIGHT = 76 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1043	0	1043	0	0	1-8	1-8
I	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

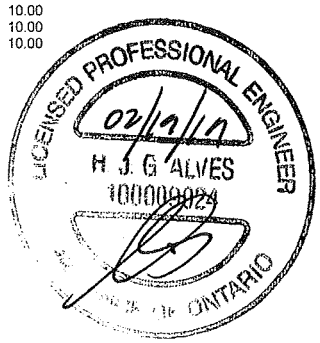
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-950 / 0	-77.7	-77.7	0.45 (1)	5.75	H- B	0 / 234	0.06 (3)
B- C	-845 / 0	-77.7	-77.7	0.53 (1)	5.79	B- F	0 / 156	0.04 (1)
C- D	-845 / 0	-77.7	-77.7	0.53 (1)	5.79	F- C	-594 / 0	0.32 (1)
E- D	-943 / 0	0.0	0.0	0.61 (1)	7.81	F- D	0 / 1136	0.26 (1)
I- A	-956 / 0	0.0	0.0	0.10 (1)	7.81	A- H	0 / 740	0.17 (1)
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00			
H- G	0 / 730	-39.5	-39.5	0.36 (2)	10.00			
G- F	0 / 730	-39.5	-39.5	0.36 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.27 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.61/1.00 (D-E:1) , BC=0.36/1.00 (F-H:2) , WB=0.32/1.00 (C-F:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.45 (A) (INPUT = 1.00)

DWG NO. TAM T1903832
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T89	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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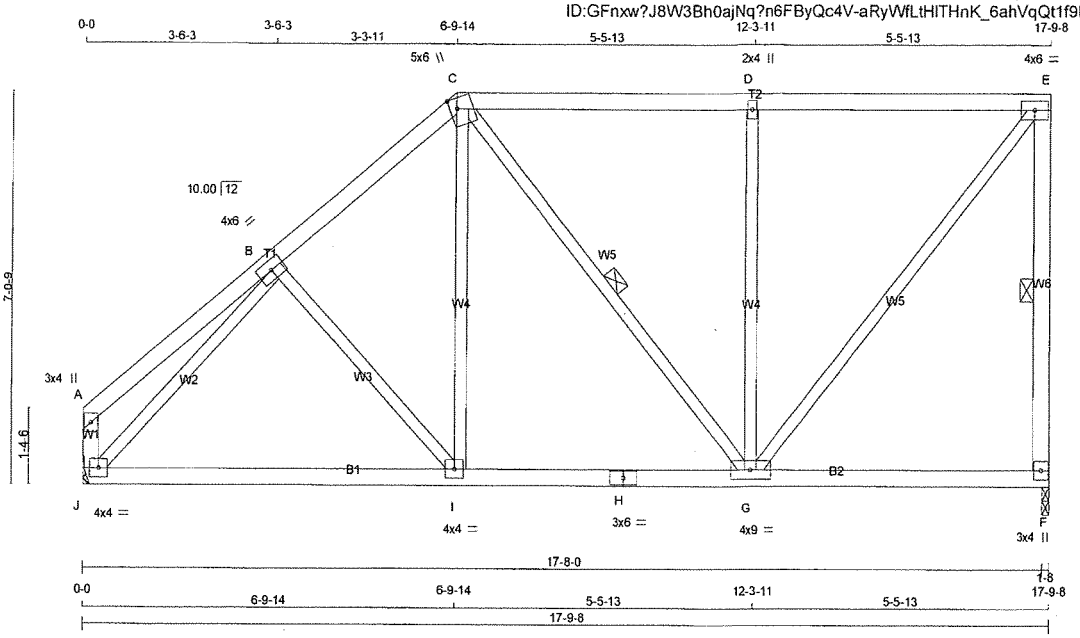


TOTAL WEIGHT = 83 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
J	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

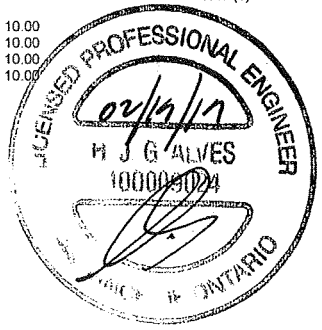
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-77.7	-77.7	0.14 (1)	10.00	B- I	-81 / 38	0.04 (1)
B- C	-897 / 0	-77.7	-77.7	0.15 (1)	6.25	I- C	0 / 378	0.08 (2)
C- D	-644 / 0	-77.7	-77.7	0.40 (1)	6.25	C- G	-68 / 0	0.03 (3)
D- E	-644 / 0	-77.7	-77.7	0.40 (1)	6.25	G- D	-526 / 0	0.45 (1)
F- E	-951 / 0	0.0	0.0	0.21 (1)	6.25	G- E	0 / 1017	0.23 (1)
J- A	-102 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1102 / 0	0.47 (1)
J- I	0 / 728	-39.5	-39.5	0.39 (2)	10.00			
I- H	0 / 678	-39.5	-39.5	0.39 (2)	10.00			
H- G	0 / 678	-39.5	-39.5	0.39 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.39/1.00 (I-J:2) , WB=0.47/1.00 (B-J:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.27 (J) (INPUT = 1.00)

DWG NO. TAM 17903833
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T90	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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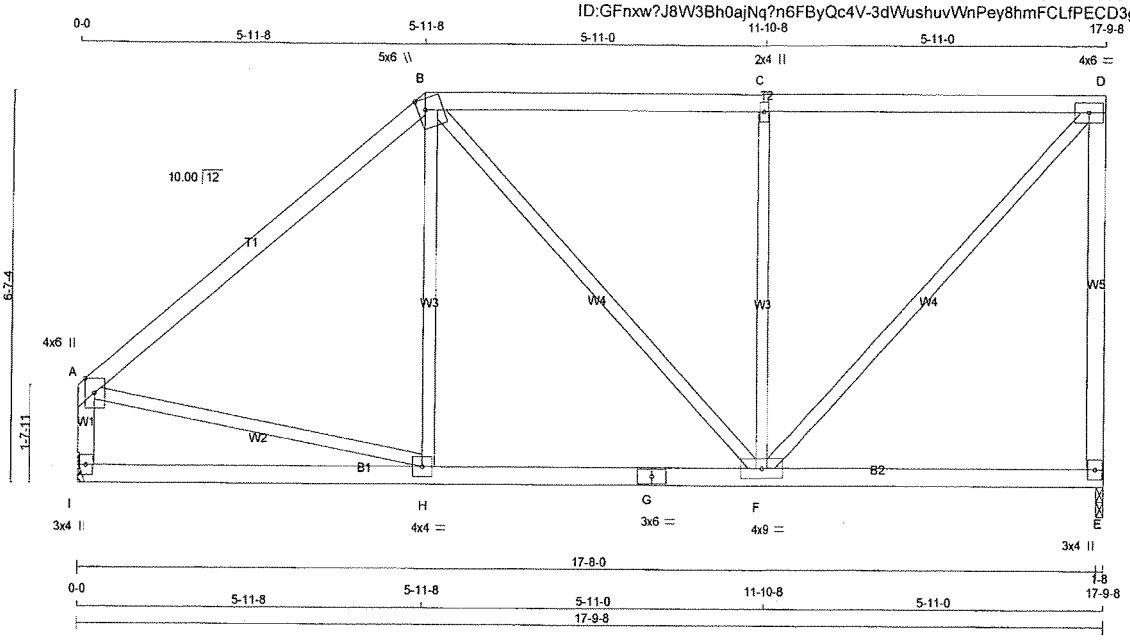

TOTAL WEIGHT = 79 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table Is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1043	0	1043	0	0	1-8	1-8
I	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
I	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-885 / 0	-77.7	-77.7	0.54 (1)	5.73	H- B	0 / 241	0.06 (3)
B- C	-727 / 0	-77.7	-77.7	0.48 (1)	6.23	B- F	0 / 68	0.02 (1)
C- D	-727 / 0	-77.7	-77.7	0.48 (1)	6.23	F- C	-568 / 0	0.41 (1)
E- D	-946 / 0	0.0	0.0	0.86 (1)	7.81	F- D	0 / 1059	0.24 (1)
I- A	-945 / 0	0.0	0.0	0.10 (1)	7.81	A- H	0 / 694	0.16 (1)
I- H	0 / 0	-39.5	-39.5	0.25 (3)	10.00			
H- G	0 / 680	-39.5	-39.5	0.34 (2)	10.00			
G- F	0 / 680	-39.5	-39.5	0.34 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.86/1.00 (D-E:1) , BC=0.34/1.00 (F-H:2) , WB=0.41/1.00 (C-F:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (H) (INPUT = 0.90)
JSI METAL= 0.43 (A) (INPUT = 1.00)

DWG NO. TAM T1903834
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T91	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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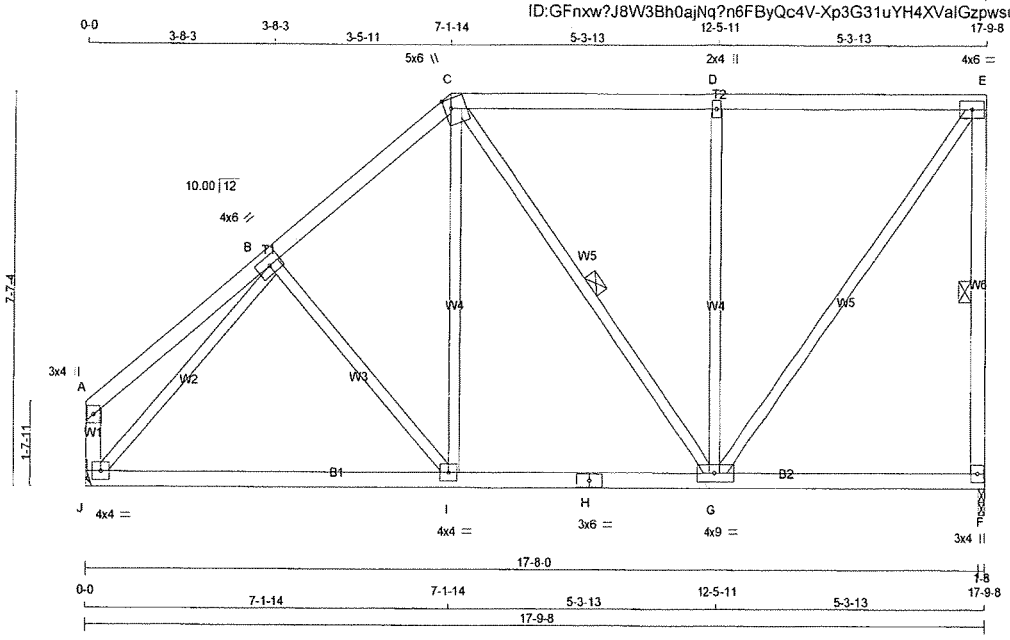


TOTAL WEIGHT = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1043	0	1043	0	0	1-8	1-8
J	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

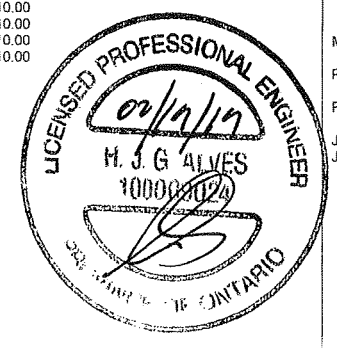
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.15 (1)	10.00	B- I	-72 / 46	0.04 (1)
B- C	-847 / 0	-77.7	-77.7	0.17 (1)	6.25	I- C	0 / 384	0.09 (2)
C- D	-581 / 0	-77.7	-77.7	0.38 (1)	6.25	C- G	-109 / 0	0.06 (2)
D- E	-582 / 0	-77.7	-77.7	0.38 (1)	6.25	G- D	-510 / 0	0.54 (1)
F- E	-952 / 0	0.0	0.0	0.25 (1)	6.25	G- E	0 / 984	0.22 (1)
J- A	-107 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1064 / 0	0.54 (1)
J- I	0 / 681	-39.5	-39.5	0.41 (2)	10.00			
I- H	0 / 638	-39.5	-39.5	0.41 (2)	10.00			
H- G	0 / 638	-39.5	-39.5	0.41 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.41/1.00 (I-J:2) , WB=0.54/1.00 (D-G:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.26 (B) (INPUT = 1.00)

DWG NO. TAM 1903835
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T92	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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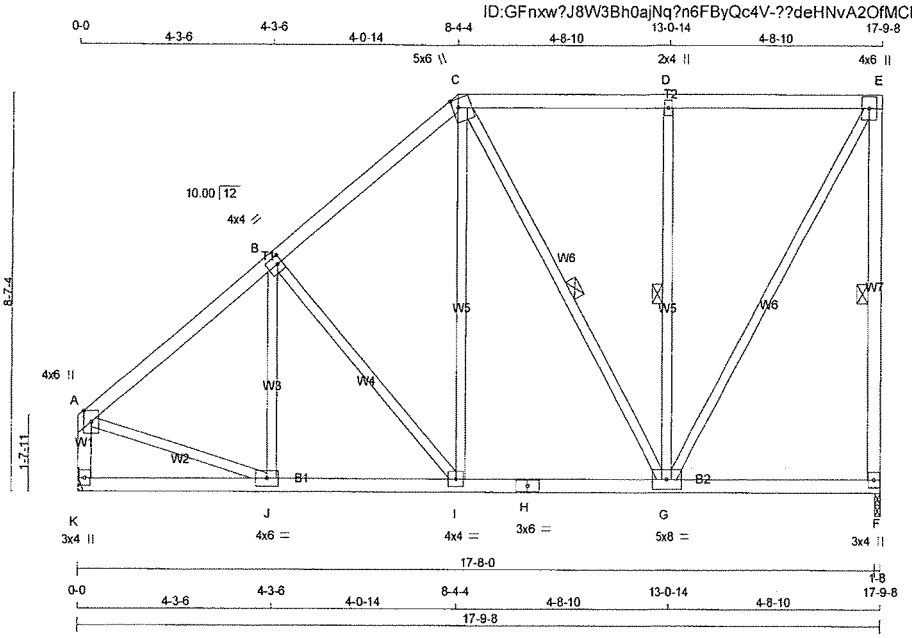


TOTAL WEIGHT = 94 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1043	0	1043	0	0	1-8	1-8
K	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

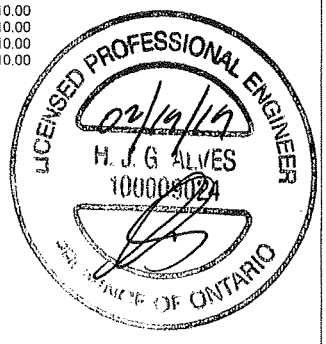
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-942 / 0	-77.7	-77.7	0.24 (1)	6.07
B- C	-756 / 0	-77.7	-77.7	0.24 (1)	6.25
C- D	-472 / 0	-77.7	-77.7	0.29 (1)	6.25
D- E	-472 / 0	-77.7	-77.7	0.29 (1)	6.25
F- E	-967 / 0	0.0	0.0	0.33 (1)	6.25
K- A	-973 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-39.5	-39.5	0.13 (3)	10.00
J- I	0 / 742	-39.5	-39.5	0.23 (2)	10.00
I- H	0 / 564	-39.5	-39.5	0.24 (2)	10.00
H- G	0 / 564	-39.5	-39.5	0.24 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-40 / 113	0.03 (3)
B- I	-280 / 0	0.21 (1)
I- C	0 / 393	0.09 (2)
C- G	-188 / 0	0.12 (1)
G- D	-451 / 0	0.21 (1)
G- E	0 / 951	0.21 (1)
A- J	0 / 776	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.33/1.00 (E-F:1) , BC=0.24/1.00 (G-I:2) , WB=0.21/1.00 (E-G:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (A) (INPUT = 0.90)
JSI METAL= 0.44 (A) (INPUT = 1.00)

DWG NO. TAM T1903836
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T93	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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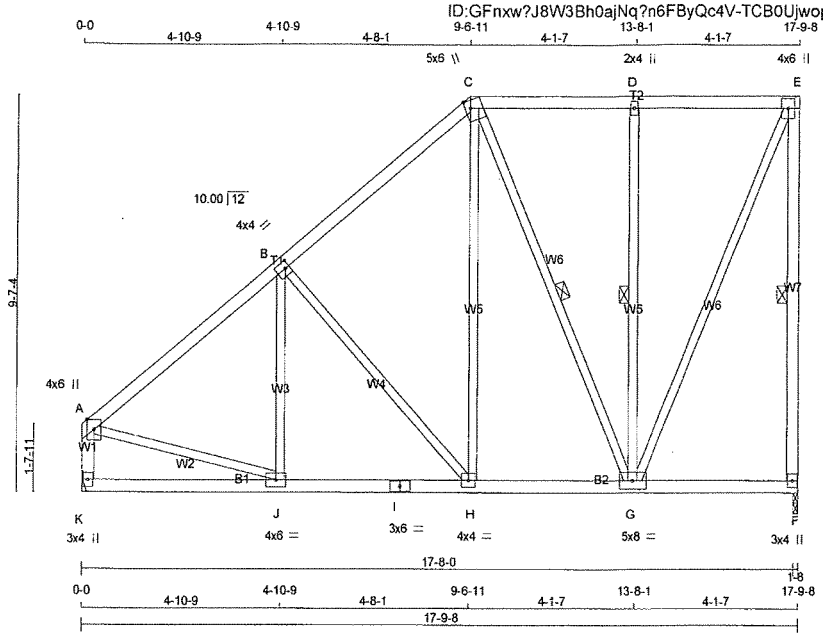


TOTAL WEIGHT = 100 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1043	0	1043	0	0	1-8	1-8
K	1043	0	1043	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0
K	797	372 / 0	187 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

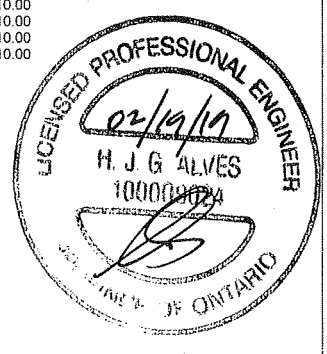
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-941 / 0	-77.7	-77.7	0.24 (1)	6.10
B- C	-679 / 0	-77.7	-77.7	0.23 (1)	6.25
C- D	-379 / 0	-77.7	-77.7	0.16 (1)	6.25
D- E	-380 / 0	-77.7	-77.7	0.16 (1)	6.25
F- E	-975 / 0	0.0	0.0	0.43 (1)	6.25
K- A	-965 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-39.5	-39.5	0.18 (3)	10.00
J- I	0 / 744	-39.5	-39.5	0.27 (2)	10.00
I- H	0 / 744	-39.5	-39.5	0.27 (2)	10.00
H- G	0 / 503	-39.5	-39.5	0.18 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 169	0.04 (3)
B- H	-371 / 0	0.37 (1)
H- C	0 / 453	0.10 (2)
C- G	-306 / 0	0.23 (1)
G- D	-391 / 0	0.24 (1)
G- E	0 / 931	0.21 (1)
A- J	0 / 770	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.27/1.00 (H-J:2) , WB=0.37/1.00 (B-H:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.44 (A) (INPUT = 1.00)

DWG NO. TAM 11403837
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T94	1	2	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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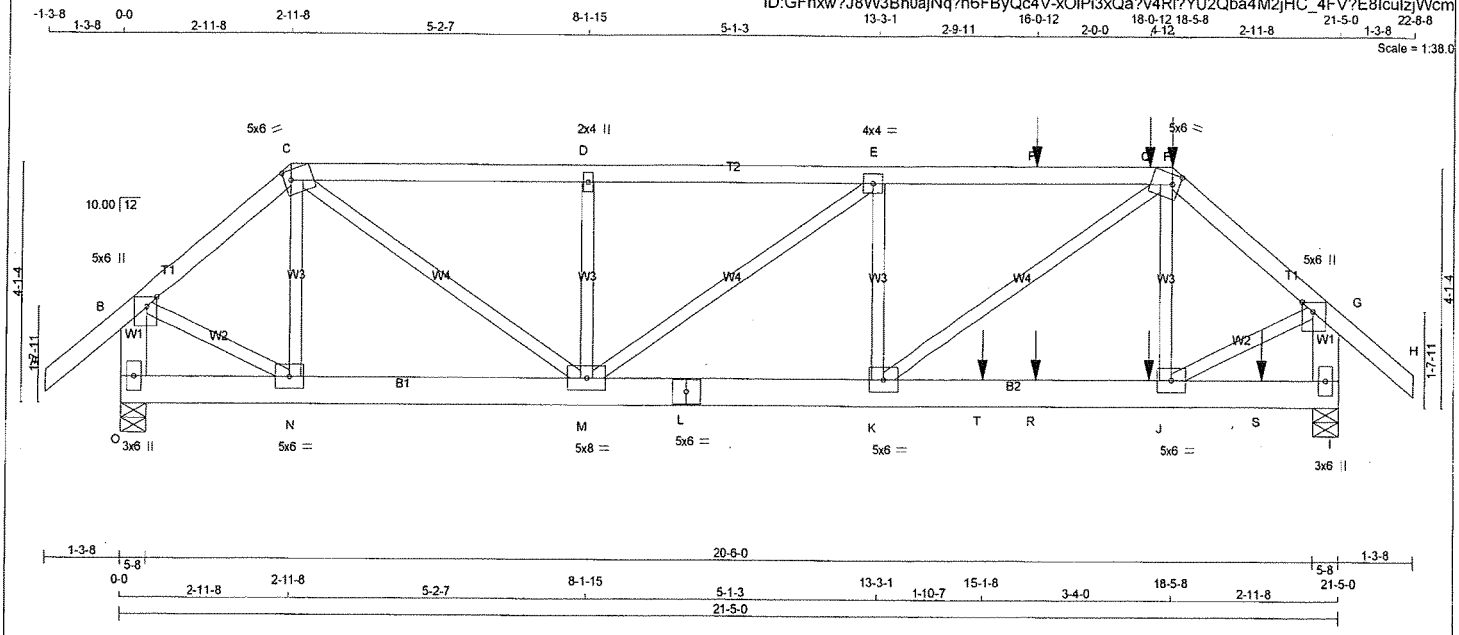


TOTAL WEIGHT = 2 X 103 = 207 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
O- B	2	12	TOP
I- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.50
G	TMVW+p	MT20	5.0	6.0	2.00	2.25
I	BMV1+p	MT20	3.0	6.0		
J, K, N						
J	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1842	0	1842	0	0	5-8	5-8
I	2780	0	2780	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1393	695 / 0	293 / 0	0 / 0	0 / 0	405 / 0	0 / 0
I	2098	1061 / 0	431 / 0	0 / 0	0 / 0	605 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

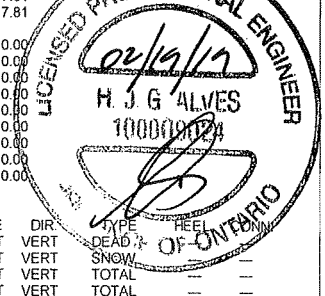
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00		N- C	-259 / 0	0.03 (1)
B- C	-1690 / 0	-77.7	-77.7	0.08 (1)	6.25		C- M	0 / 1813	0.22 (1)
C- D	-2763 / 0	-77.7	-77.7	0.18 (1)	5.33		M- D	-411 / 0	0.05 (1)
D- E	-2763 / 0	-77.7	-77.7	0.22 (1)	5.27		M- E	-1047 / 0	0.37 (1)
E- P	-3608 / 0	-77.7	-77.7	0.30 (1)	4.65		K- E	0 / 207	0.03 (3)
P- Q	-3608 / 0	-77.7	-77.7	0.30 (1)	4.65		K- F	0 / 1794	0.22 (1)
Q- F	-3608 / 0	-77.7	-77.7	0.30 (1)	4.65		J- F	0 / 261	0.03 (3)
F- G	-2801 / 0	-77.7	-77.7	0.09 (1)	5.40		B- N	0 / 1393	0.49 (1)
G- H	0 / 34	-77.7	-77.7	0.06 (1)	10.00		J- G	0 / 2801	0.79 (1)
O- B	-1827 / 0	0.0	0.0	0.07 (1)	7.81				
I- G	-2881 / 0	0.0	0.0	0.11 (1)	7.81				
O- N	0 / 0	-39.5	-39.5	0.05 (2)	10.00				
N- M	0 / 1286	-39.5	-39.5	0.14 (1)	10.00				
M- L	0 / 3608	-39.5	-39.5	0.43 (1)	10.00				
L- K	0 / 3608	-39.5	-39.5	0.43 (1)	10.00				
K- T	0 / 2147	-39.5	-39.5	0.56 (1)	10.00				
T- R	0 / 2147	-39.5	-39.5	0.56 (1)	10.00				
R- J	0 / 2147	-39.5	-39.5	0.56 (1)	10.00				
J- S	0 / 0	-39.5	-39.5	0.22 (1)	10.00				
S- I	0 / 0	-39.5	-39.5	0.22 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	18-5-8	-38	-42	---	FRONT	VERT	DEAD	---	---
F	18-5-8	-180	-180	---	FRONT	VERT	SNOW	---	---
J	18-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
P	16-0-12	-94	-94	---	FRONT	VERT	TOTAL	---	---
Q	18-0-12	-117	-117	---	FRONT	VERT	TOTAL	---	---
R	16-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
S	20-0-12	-56	-71	---	FRONT	VERT	TOTAL	---	---
T	15-1-8	-1299	-1299	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM T2038 38
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.30/1.00 (E-F:1) , BC=0.56/1.00 (J-K:1) , WB=0.37/1.00 (E-M:1) , SSI=0.31/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T94A	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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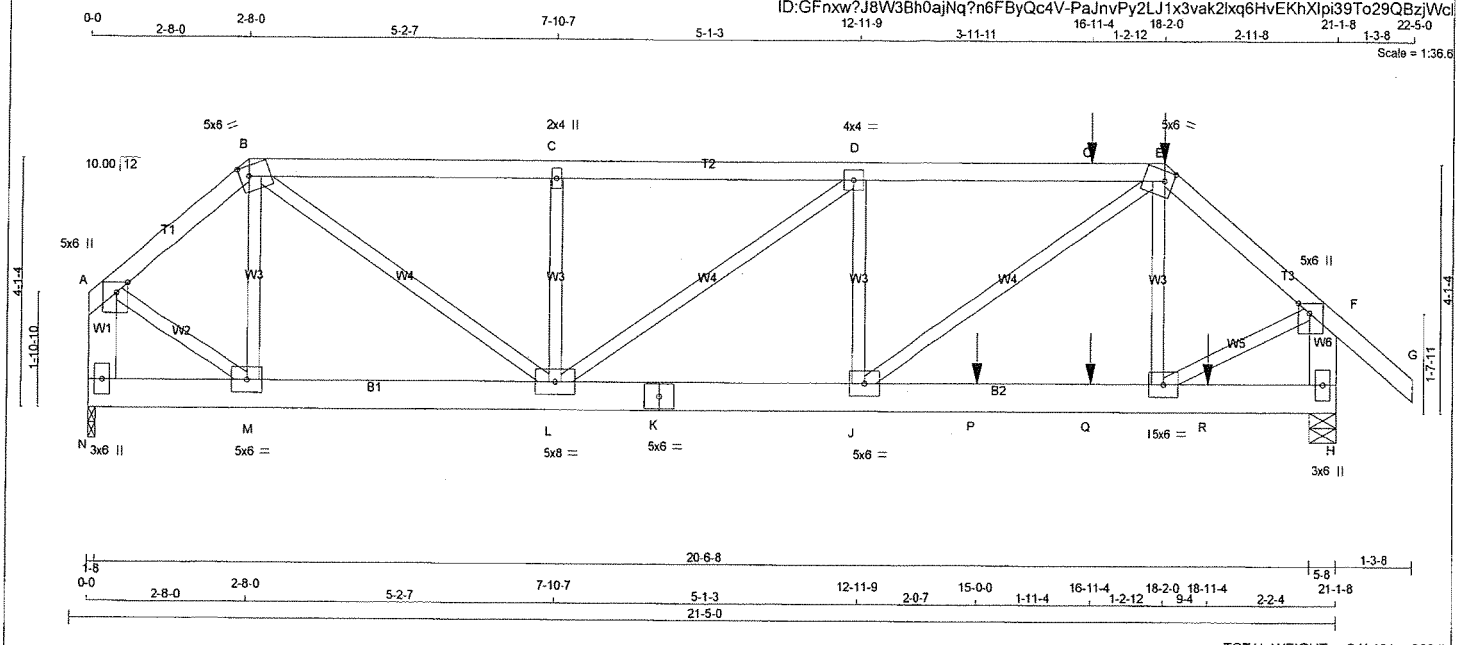

TOTAL WEIGHT = 2 X 101 = 202 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
N- A	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	TOP
K- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW-m	MT20	5.0	6.0	2.00	1.75
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	1.75
F	TMVW+p	MT20	5.0	6.0	2.00	2.25
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1676	0	1676	0	0	1-8	1-8
H	2614	0	2614	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1277	608 / 0	290 / 0	0 / 0	0 / 0	379 / 0	0 / 0
H	1978	985 / 0	418 / 0	0 / 0	0 / 0	575 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX. FACTORED MAX CSI (LC)
FR-TO						FR-TO		
A- B	-1497 / 0	-77.7	-77.7	0.06 (1)	6.25	M- B	-347 / 0	0.04 (1)
B- C	-2581 / 0	-77.7	-77.7	0.18 (1)	5.47	B- L	0 / 1774	0.22 (1)
C- D	-2581 / 0	-77.7	-77.7	0.20 (1)	5.44	L- C	-420 / 0	0.05 (1)
D- O	-3380 / 0	-77.7	-77.7	0.25 (1)	4.85	L- D	-991 / 0	0.35 (1)
O- E	-3380 / 0	-77.7	-77.7	0.25 (1)	4.85	J- D	0 / 217	0.03 (3)
E- F	-2651 / 0	-77.7	-77.7	0.09 (1)	5.51	J- E	0 / 1553	0.20 (1)
F- G	0 / 34	-77.7	-77.7	0.06 (1)	10.00	I- E	0 / 284	0.04 (3)
N- A	-1674 / 0	0.0	0.0	0.06 (1)	7.81	A- M	0 / 1265	0.17 (1)
H- F	-2739 / 0	0.0	0.0	0.10 (1)	7.81	F- I	0 / 2185	0.27 (1)
N- M	0 / 0	-39.5	-39.5	0.06 (2)	10.00			
M- L	0 / 1136	-39.5	-39.5	0.13 (1)	10.00			
L- K	0 / 3380	-39.5	-39.5	0.41 (1)	10.00			
K- J	0 / 3380	-39.5	-39.5	0.41 (1)	10.00			
J- P	0 / 2034	-39.5	-39.5	0.56 (1)	10.00			
P- Q	0 / 2034	-39.5	-39.5	0.56 (1)	10.00			
Q- I	0 / 2034	-39.5	-39.5	0.56 (1)	10.00			
I- R	0 / 0	-39.5	-39.5	0.23 (1)	10.00			
R- H	0 / 0	-39.5	-39.5	0.23 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	TOPN
E	18-5-8	-38	-42	---	FRONT	VERT	DEAD	---	---
E	18-5-8	-180	-180	---	FRONT	VERT	SNOW	---	---
O	17-2-12	-94	-94	---	BACK	VERT	TOTAL	---	---
P	15-3-8	-1283	-1283	---	BACK	VERT	TOTAL	---	---
Q	17-2-12	-56	-71	---	BACK	VERT	TOTAL	---	---
R	19-2-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DWG NO. TAM T1903837
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.56/1.00 (I-J:1) , WB=0.35/1.00 (D-L:1) , SSI=0.29/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR (PSI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
H J G ALVES
100009824
PROVINCE OF ONTARIO
02/19/19

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T95	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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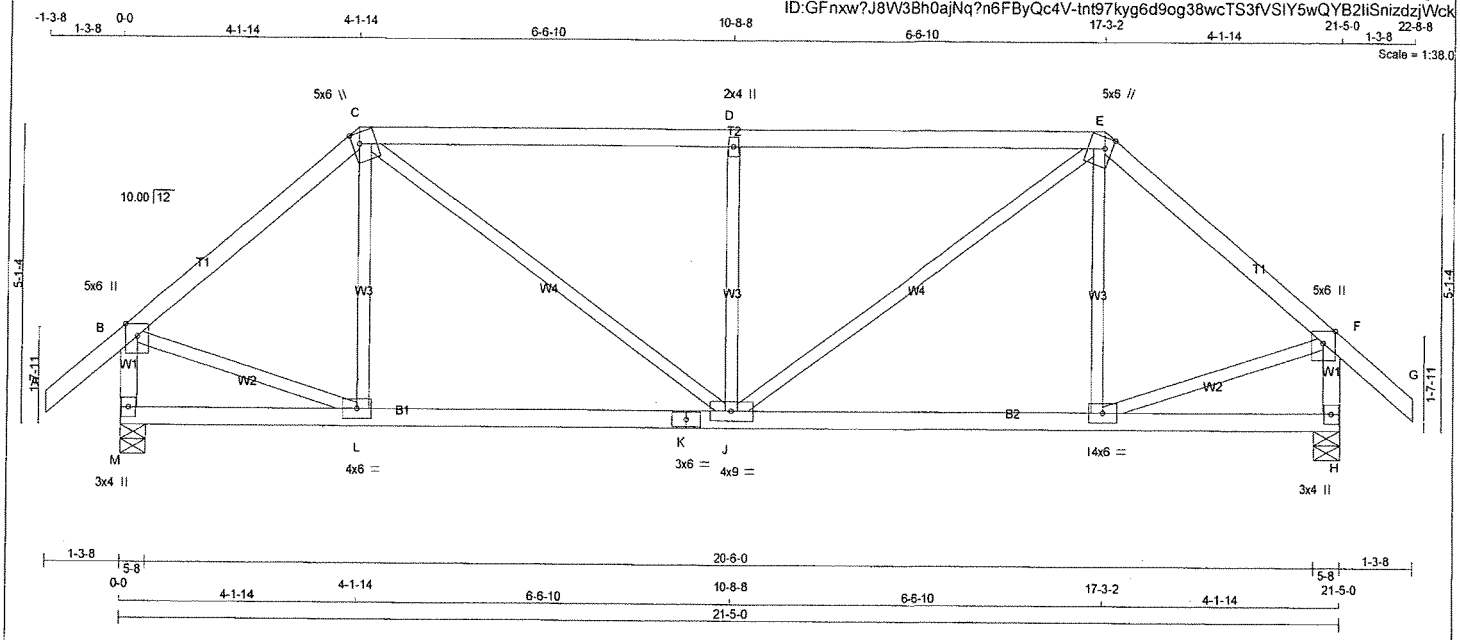


TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1363	0	1363	0	0	5-8	5-8
H	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
H	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

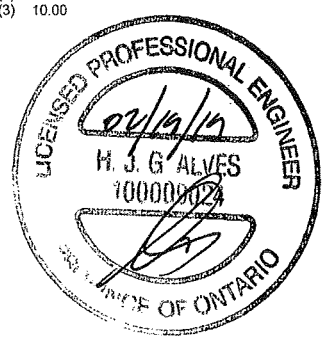
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-1181 / 0	-77.7	-77.7	0.27 (1)	5.56
C- D	-1507 / 0	-77.7	-77.7	0.67 (1)	4.44
D- E	-1507 / 0	-77.7	-77.7	0.67 (1)	4.44
E- F	-1181 / 0	-77.7	-77.7	0.27 (1)	5.56
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00
M- B	-1305 / 0	0.0	0.0	0.14 (1)	7.06
H- F	-1305 / 0	0.0	0.0	0.14 (1)	7.06
M- L	0 / 0	-39.5	-39.5	0.21 (3)	10.00
L- K	0 / 904	-39.5	-39.5	0.37 (2)	10.00
K- J	0 / 904	-39.5	-39.5	0.37 (2)	10.00
J- I	0 / 904	-39.5	-39.5	0.37 (2)	10.00
I- H	0 / 0	-39.5	-39.5	0.21 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
L- C	-42 / 137	0.03 (3)
C- J	0 / 747	0.17 (1)
J- D	-621 / 0	0.24 (1)
J- E	0 / 747	0.17 (1)
I- E	-42 / 137	0.03 (3)
B- L	0 / 947	0.21 (1)
I- F	0 / 947	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.37/1.00 (J-L:2) , WB=0.24/1.00 (D-J:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM 1903840
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T95A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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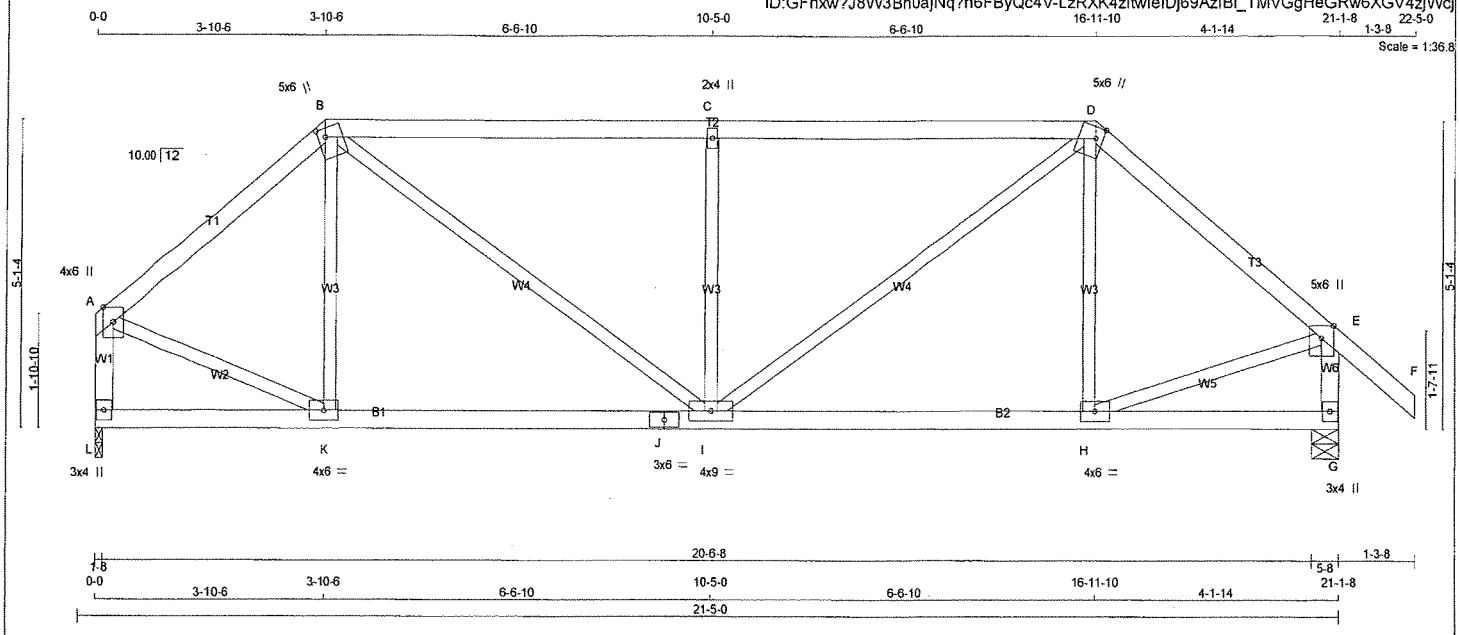


TOTAL WEIGHT = 86 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	1.75	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1238	0	1238	0	0	1-8	1-8
G	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

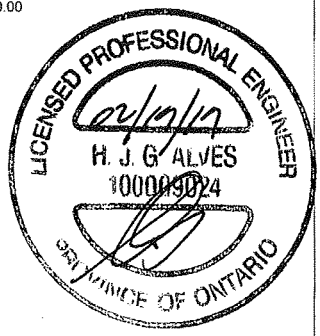
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1102 / 0	-77.7	-77.7	0.23 (1)	5.76	K- B	-99 / 100	0.04 (1)	
B- C	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	B- I	0 / 778	0.17 (1)	
C- D	-1469 / 0	-77.7	-77.7	0.66 (1)	4.49	I- C	-622 / 0	0.24 (1)	
D- E	-1162 / 0	-77.7	-77.7	0.26 (1)	5.59	I- D	0 / 719	0.16 (1)	
E- F	0 / 34	-77.7	-77.7	0.11 (1)	10.00	H- D	-38 / 139	0.03 (3)	
L- A	-1188 / 0	0.0	0.0	0.13 (1)	7.32	A- K	0 / 906	0.20 (1)	
G- E	-1288 / 0	0.0	0.0	0.14 (1)	7.11	H- E	0 / 931	0.21 (1)	
L- K	0 / 0	-39.5	-39.5	0.20 (3)	10.00				
K- J	0 / 842	-39.5	-39.5	0.36 (2)	10.00				
J- I	0 / 842	-39.5	-39.5	0.36 (2)	10.00				
I- H	0 / 889	-39.5	-39.5	0.37 (2)	10.00				
H- G	0 / 0	-39.5	-39.5	0.21 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.66/1.00 (C-D:1) , BC=0.37/1.00 (H-I:2) , WB=0.24/1.00 (C-I:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

DWG NO. TAM T1903841
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T96	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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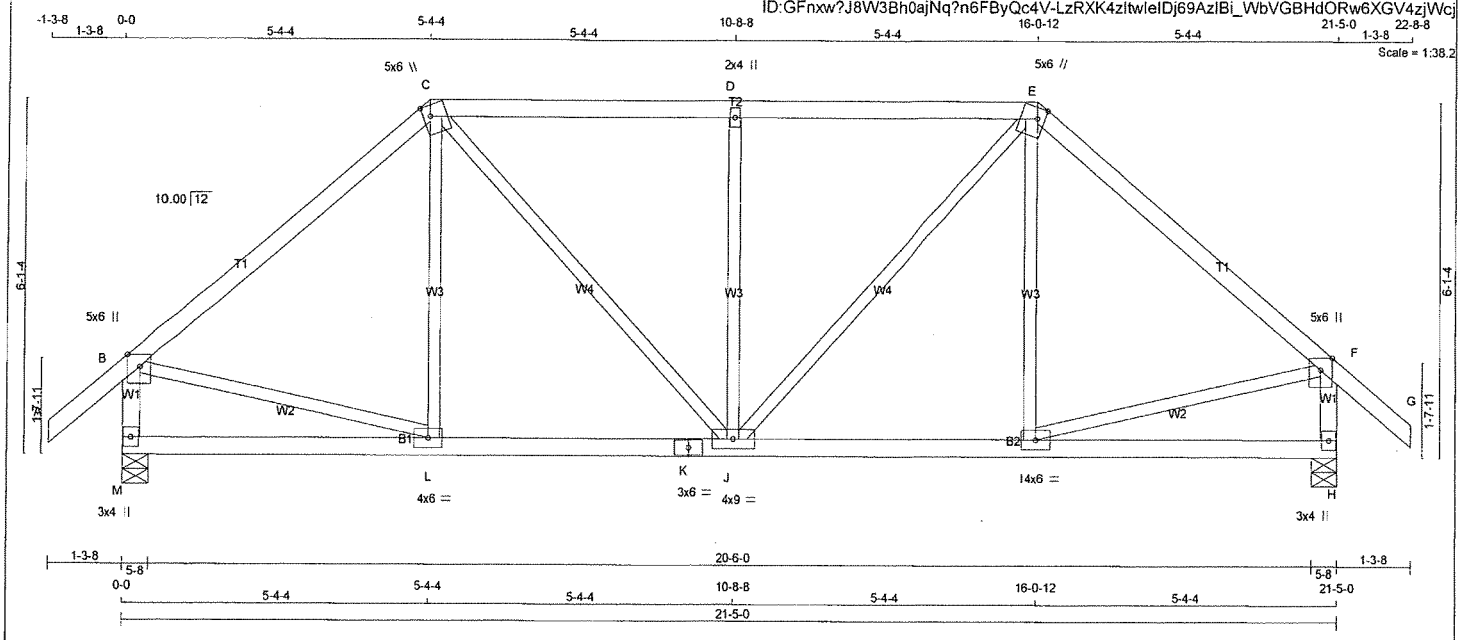


TOTAL WEIGHT = 94 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1363	0	1363	0	0	5-8	5-8
H	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
H	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

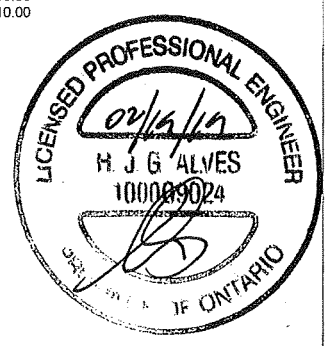
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)		10.00	L- C	0 / 186	0.04 (3)
B- C	-1164 / 0	-77.7	-77.7	0.45 (1)		5.33	C- J	0 / 471	0.11 (1)
C- D	-1213 / 0	-77.7	-77.7	0.41 (1)		5.27	J- D	-505 / 0	0.30 (1)
D- E	-1213 / 0	-77.7	-77.7	0.41 (1)		5.27	J- E	0 / 471	0.11 (1)
E- F	-1164 / 0	-77.7	-77.7	0.45 (1)		5.33	I- E	0 / 186	0.04 (3)
F- G	0 / 34	-77.7	-77.7	0.11 (1)		10.00	B- L	0 / 918	0.21 (1)
M- B	-1277 / 0	0.0	0.0	0.14 (1)		7.13	I- F	0 / 918	0.21 (1)
H- F	-1277 / 0	0.0	0.0	0.14 (1)		7.13			
M- L	0 / 0	-39.5	-39.5	0.22 (3)		10.00			
L- K	0 / 893	-39.5	-39.5	0.34 (2)		10.00			
K- J	0 / 893	-39.5	-39.5	0.34 (2)		10.00			
J- I	0 / 893	-39.5	-39.5	0.34 (2)		10.00			
I- H	0 / 0	-39.5	-39.5	0.22 (3)		10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.34/1.00 (J-L:2) , WB=0.30/1.00 (D-J:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (I) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

JOB NO. TAM 79038 42
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T96A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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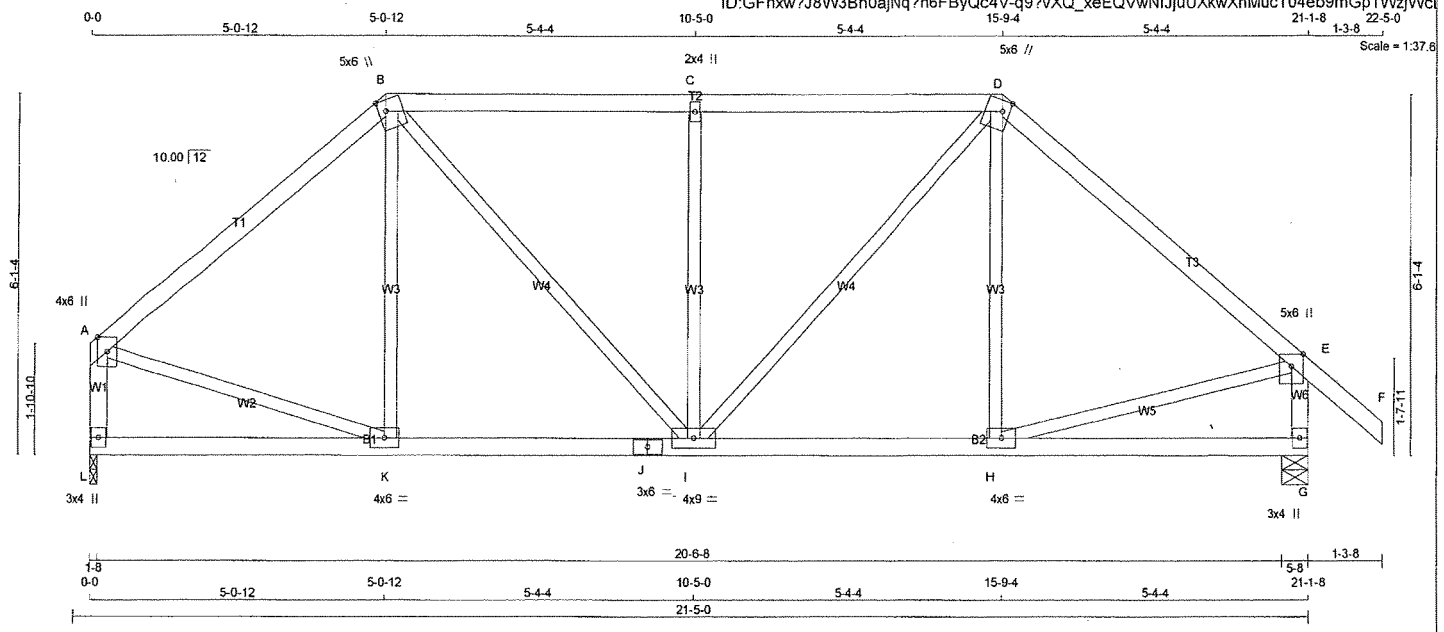


TOTAL WEIGHT = 91 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1238	0	1238	0	0	1-8	1-8
G	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

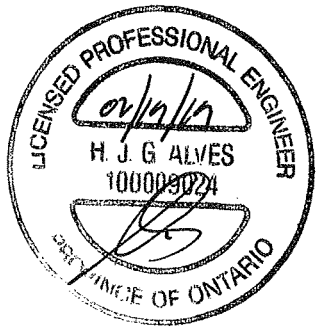
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1102 / 0	-77.7	-77.7	0.40	(1) 5.52		K- B	-17 / 153	0.04 (3)
B- C	-1182 / 0	-77.7	-77.7	0.41	(1) 5.33		B- I	0 / 497	0.11 (1)
C- D	-1182 / 0	-77.7	-77.7	0.41	(1) 5.33		I- C	-505 / 0	0.30 (1)
D- E	-1144 / 0	-77.7	-77.7	0.45	(1) 5.36		I- D	0 / 449	0.10 (1)
E- F	0 / 34	-77.7	-77.7	0.11	(1) 10.00		H- D	0 / 187	0.04 (3)
L- A	-1157 / 0	0.0	0.0	0.13	(1) 7.40		A- K	0 / 882	0.20 (1)
G- E	-1260 / 0	0.0	0.0	0.13	(1) 7.17		H- E	0 / 901	0.20 (1)
L- K	0 / 0	-39.5	-39.5	0.21	(3) 10.00				
K- J	0 / 844	-39.5	-39.5	0.32	(2) 10.00				
J- I	0 / 844	-39.5	-39.5	0.32	(2) 10.00				
I- H	0 / 877	-39.5	-39.5	0.34	(2) 10.00				
H- G	0 / 0	-39.5	-39.5	0.22	(3) 10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.34/1.00 (H-I:2) , WB=0.30/1.00 (C-I:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

DWG NO. TAM 7903843
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T97	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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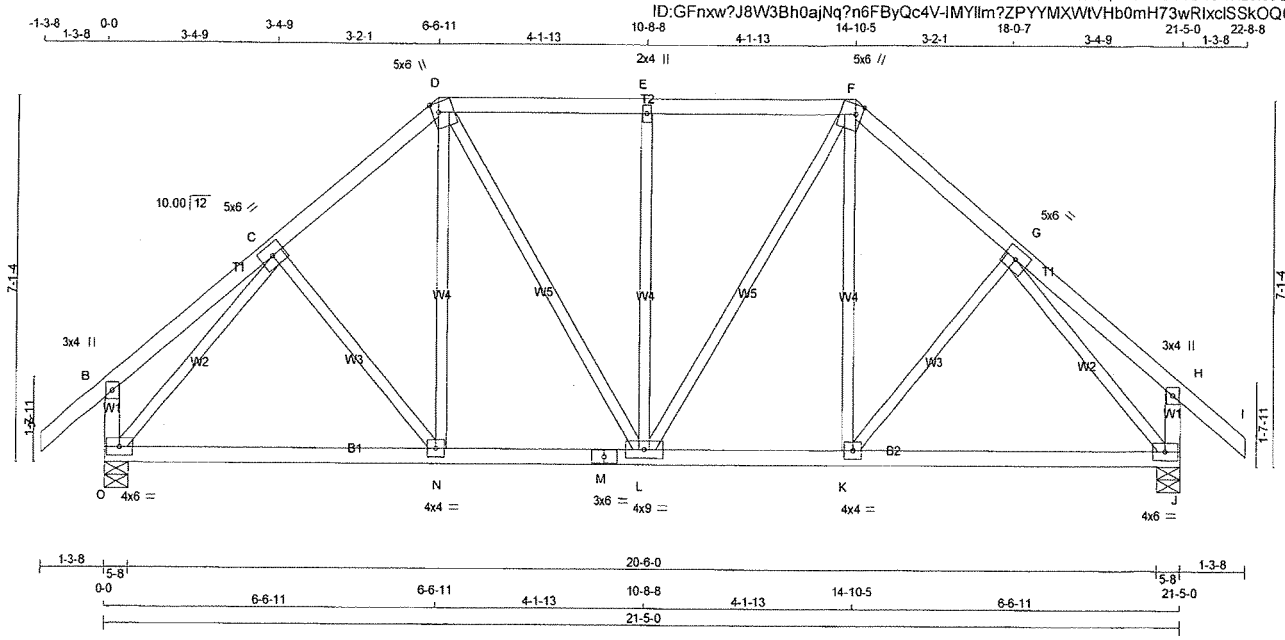


TOTAL WEIGHT = 105 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1363	0	1363	0	0	5-8	5-8
J	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
J	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

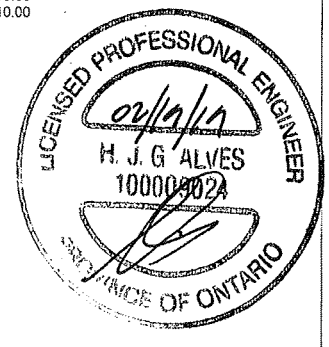
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00
C- D	-1143 / 0	-77.7	-77.7	0.11 (1)	5.84
D- E	-1008 / 0	-77.7	-77.7	0.18 (1)	6.03
E- F	-1008 / 0	-77.7	-77.7	0.18 (1)	6.03
F- G	-1143 / 0	-77.7	-77.7	0.11 (1)	5.84
G- H	0 / 17	-77.7	-77.7	0.12 (1)	10.00
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00
O- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
J- H	-207 / 0	0.0	0.0	0.02 (1)	7.81
O- N	0 / 849	-39.5	-39.5	0.40 (2)	10.00
N- M	0 / 866	-39.5	-39.5	0.41 (2)	10.00
M- L	0 / 866	-39.5	-39.5	0.41 (2)	10.00
L- K	0 / 866	-39.5	-39.5	0.41 (2)	10.00
K- J	0 / 849	-39.5	-39.5	0.40 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 89	0.02 (3)
N- D	0 / 305	0.07 (2)
D- L	0 / 273	0.06 (1)
L- E	-387 / 0	0.34 (1)
L- F	0 / 273	0.06 (1)
K- F	0 / 305	0.07 (2)
K- G	0 / 89	0.02 (3)
O- C	-1349 / 0	0.58 (1)
G- J	-1349 / 0	0.58 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.41/1.00 (L-N:2) , WB=0.58/1.00 (C-O:1) , SSI=0.17/1.00 (N-O:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (O) (INPUT = 0.90)
JSI METAL= 0.33 (G) (INPUT = 1.00)

DWG NO. TAM P903844
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T97A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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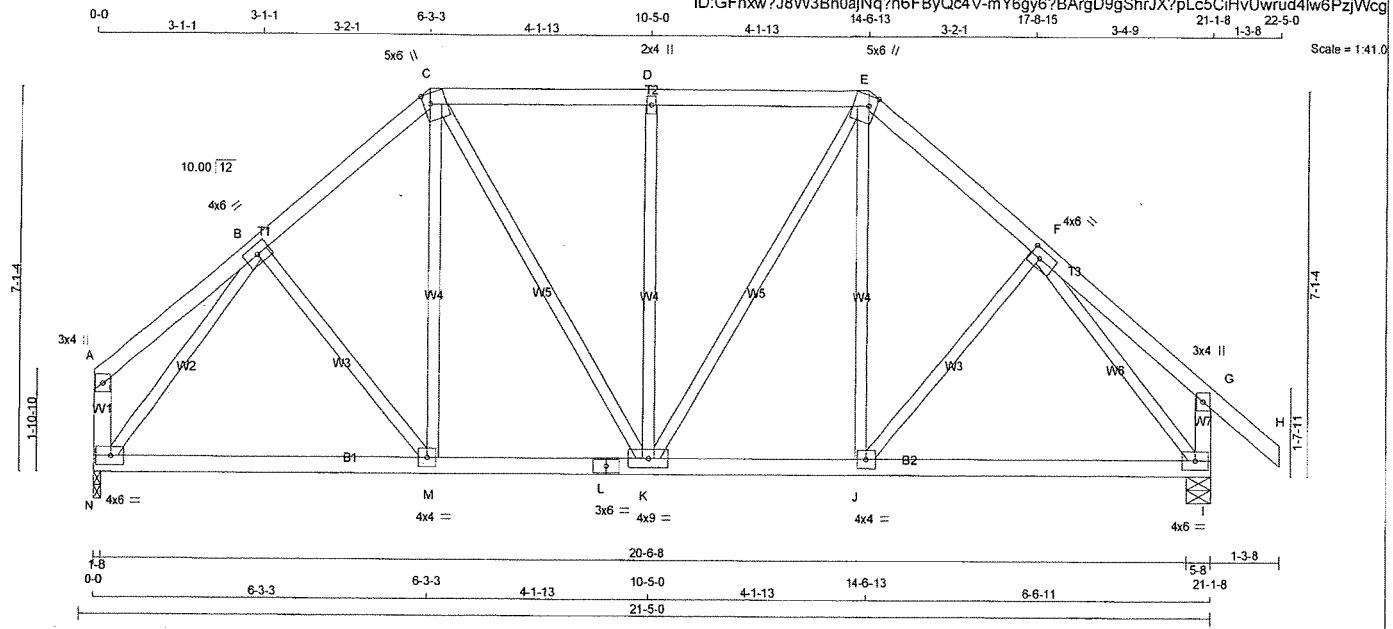

TOTAL WEIGHT = 102 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1238	0	1238	0	0	1-8	1-8
I	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
I	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-77.7	-77.7	0.11 (1)	10.00	B- M	0 / 115	0.03 (3)
B- C	-1091 / 0	-77.7	-77.7	0.10 (1)	5.95	M- C	0 / 254	0.06 (3)
C- D	-982 / 0	-77.7	-77.7	0.17 (1)	6.09	C- K	0 / 298	0.07 (1)
D- E	-982 / 0	-77.7	-77.7	0.17 (1)	6.09	K- D	-387 / 0	0.34 (1)
E- F	-1121 / 0	-77.7	-77.7	0.11 (1)	5.88	K- E	0 / 254	0.06 (1)
F- G	0 / 17	-77.7	-77.7	0.12 (1)	10.00	J- E	0 / 308	0.07 (2)
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F	0 / 86	0.02 (3)
N- A	-90 / 0	0.0	0.0	0.01 (1)	7.81	N- B	-1301 / 0	0.52 (1)
I- G	-209 / 0	0.0	0.0	0.02 (1)	7.81	F- I	-1326 / 0	0.57 (1)
N- M	0 / 774	-39.5	-39.5	0.37 (2)	10.00			
M- L	0 / 827	-39.5	-39.5	0.38 (2)	10.00			
L- K	0 / 827	-39.5	-39.5	0.38 (2)	10.00			
K- J	0 / 850	-39.5	-39.5	0.40 (2)	10.00			
J- I	0 / 837	-39.5	-39.5	0.40 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.17/1.00 (C-D:1) , BC=0.40/1.00 (J-K:2) , WB=0.57/1.00 (F-I:1) , SSI=0.16/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

...NT NO. TAM 17903845
STRUCTURAL
...CUMENTS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T98	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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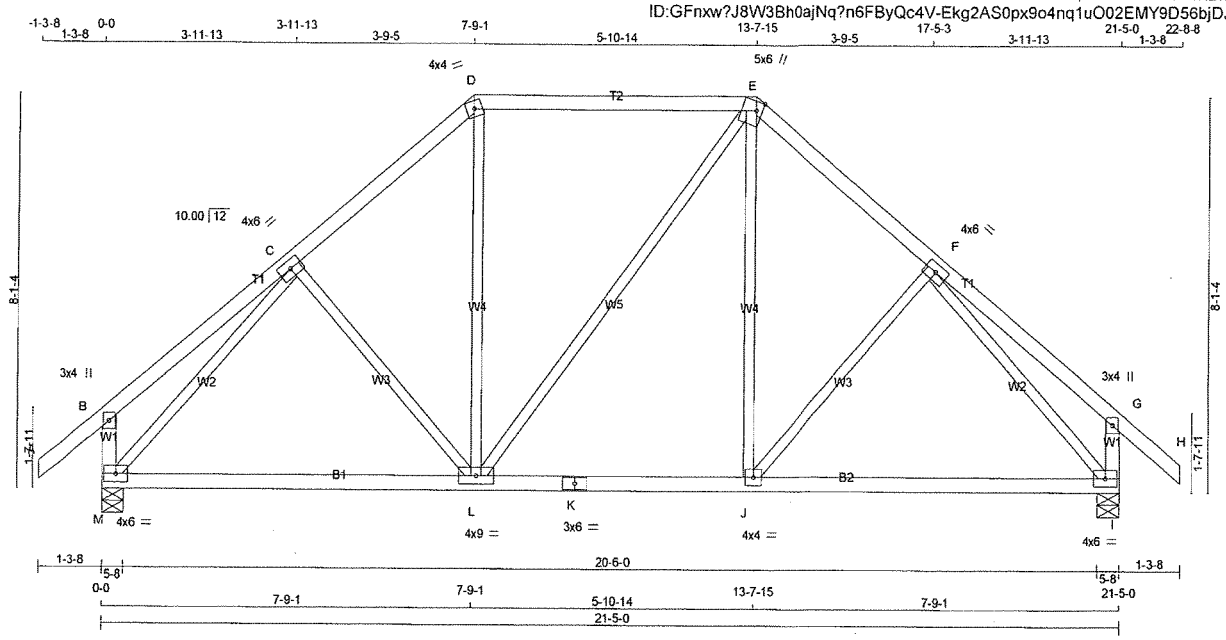


TOTAL WEIGHT = 100 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

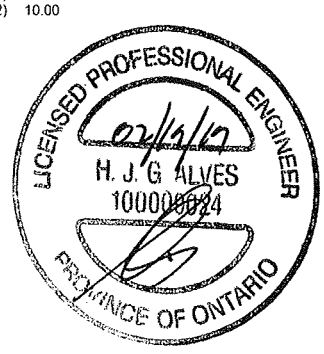
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- L	-63 / 58	0.04 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00	L- D	0 / 379	0.09 (2)
C- D	-1091 / 0	-77.7	-77.7	0.15 (1)	5.89	L- E	0 / 0	0.00 (1)
D- E	-824 / 0	-77.7	-77.7	0.36 (1)	6.21	J- E	0 / 378	0.09 (2)
E- F	-1091 / 0	-77.7	-77.7	0.15 (1)	5.89	J- F	-63 / 58	0.04 (1)
F- G	0 / 22	-77.7	-77.7	0.18 (1)	10.00	M- C	-1327 / 0	0.79 (1)
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	F- I	-1326 / 0	0.79 (1)
M- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
I- G	-223 / 0	0.0	0.0	0.02 (1)	7.81			
M- L	0 / 861	-39.5	-39.5	0.49 (2)	10.00			
L- K	0 / 823	-39.5	-39.5	0.46 (2)	10.00			
K- J	0 / 823	-39.5	-39.5	0.46 (2)	10.00			
J- I	0 / 861	-39.5	-39.5	0.49 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 930 (0.28")

CSI: TC=0.36/1.00 (D-E:1) , BC=0.49/1.00 (I-J:2) , WB=0.79/1.00 (C-M:1) , SSI=0.19/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.38 (K) (INPUT = 1.00)

DWG NO. TAM T9038846
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T98A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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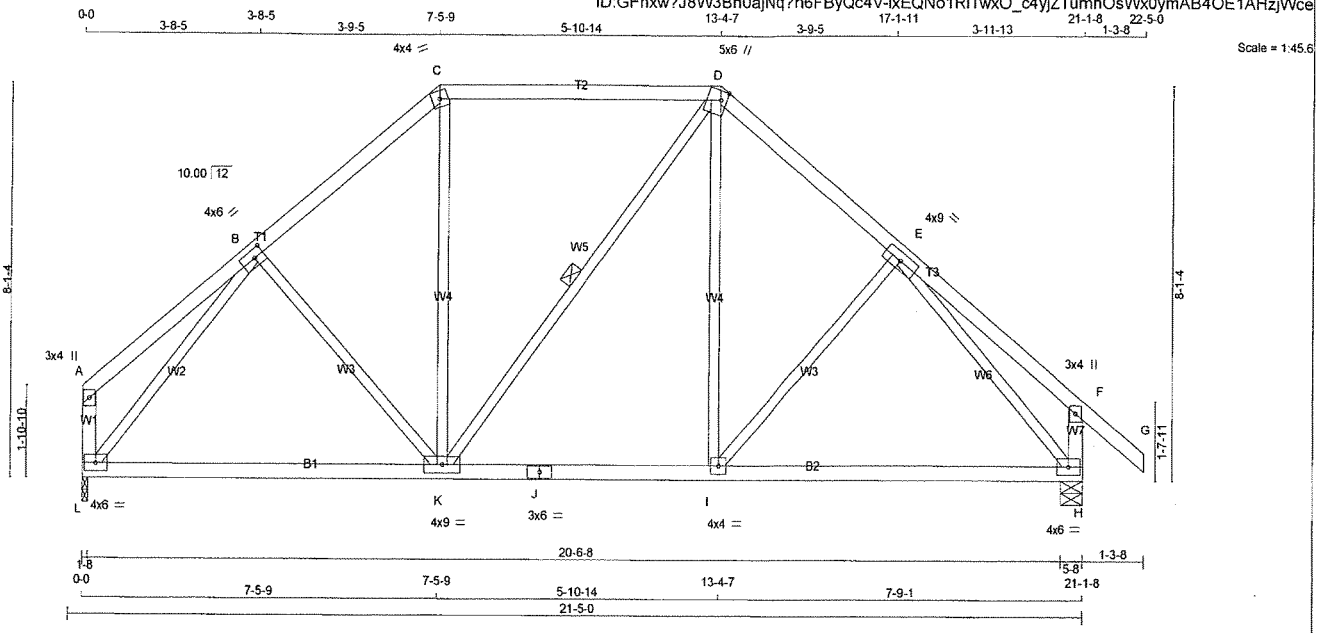


TOTAL WEIGHT = 97 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	9.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

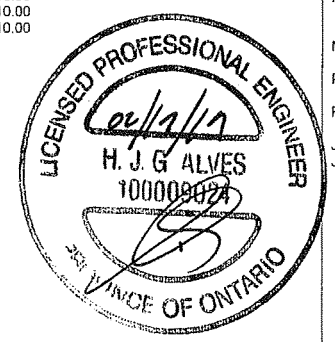
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 22	-77.7	-77.7	0.17 (1)	10.00	B- K	-13 / 80	0.02 (3)
B- C	-1049 / 0	-77.7	-77.7	0.14 (1)	5.99	K- C	0 / 354	0.08 (2)
C- D	-792 / 0	-77.7	-77.7	0.36 (1)	6.25	K- D	-25 / 0	0.02 (1)
D- E	-1069 / 0	-77.7	-77.7	0.15 (1)	5.93	I- D	0 / 387	0.09 (2)
E- F	0 / 22	-77.7	-77.7	0.18 (1)	10.00	I- E	-68 / 55	0.04 (1)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- B	-1283 / 0	0.71 (1)
L- A	-106 / 0	0.0	0.0	0.01 (1)	7.81	E- H	-1304 / 0	0.77 (1)
H- F	-225 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 796	-39.5	-39.5	0.46 (2)	10.00			
K- J	0 / 807	-39.5	-39.5	0.46 (2)	10.00			
J- I	0 / 807	-39.5	-39.5	0.46 (2)	10.00			
I- H	0 / 847	-39.5	-39.5	0.49 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 928 (0.27")

CSI: TC=0.36/1.00 (C-D:1) , BC=0.49/1.00 (H-I:2) , WB=0.77/1.00 (E-H:1) , SSI=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (L) (INPUT = 0.90)
JSI METAL= 0.35 (J) (INPUT = 1.00)

DWG NO. TAM T1903847
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T99	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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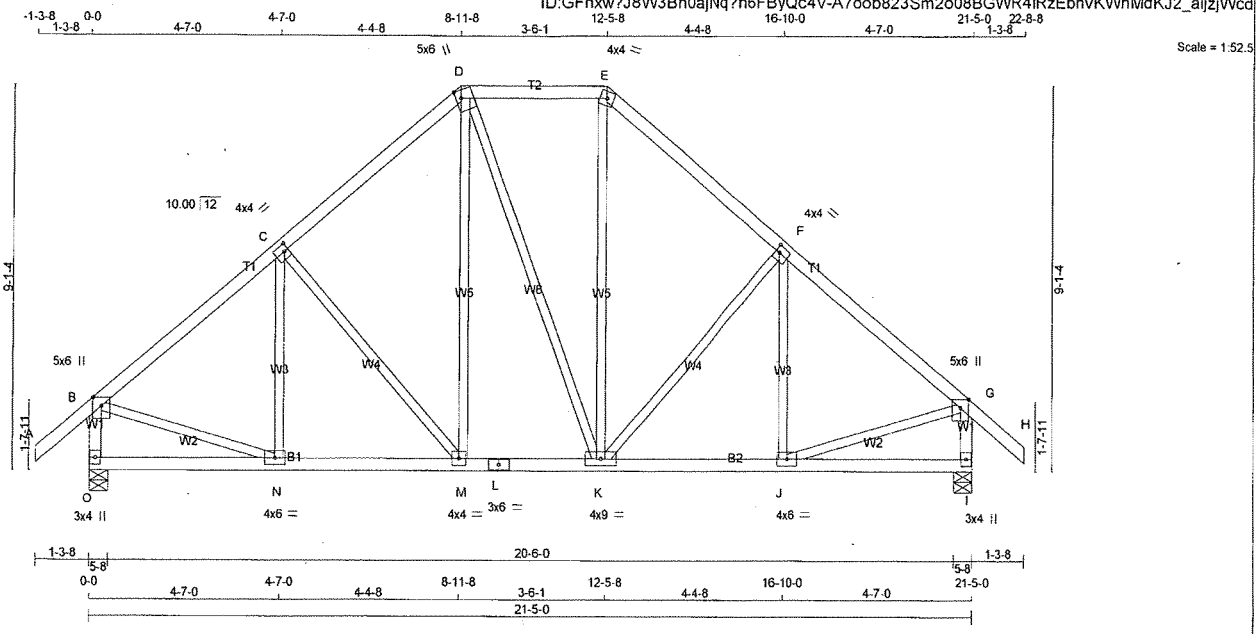


TOTAL WEIGHT = 109 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

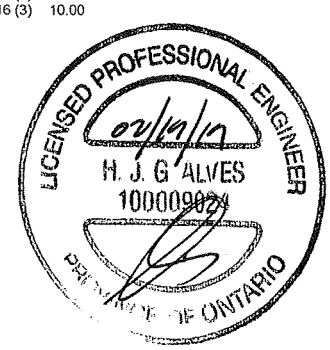
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- C	-61 / 115	0.03 (3)	
B- C	-1190 / 0	-77.7	-77.7	0.22 (1)	5.63	C- M	-287 / 0	0.25 (1)	
C- D	-999 / 0	-77.7	-77.7	0.21 (1)	6.01	M- D	0 / 374	0.08 (2)	
D- E	-750 / 0	-77.7	-77.7	0.13 (1)	6.25	D- K	0 / 3	0.00 (2)	
E- F	-1001 / 0	-77.7	-77.7	0.21 (1)	6.01	K- E	0 / 378	0.09 (2)	
F- G	-1189 / 0	-77.7	-77.7	0.22 (1)	5.63	K- F	-285 / 0	0.25 (1)	
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F	-64 / 113	0.03 (1)	
O- B	-1290 / 0	0.0	0.0	0.14 (1)	7.10	B- N	0 / 971	0.22 (1)	
I- G	-1289 / 0	0.0	0.0	0.14 (1)	7.10	J- G	0 / 971	0.22 (1)	
O- N	0 / 0	-39.5	-39.5	0.16 (3)	10.00				
N- M	0 / 934	-39.5	-39.5	0.28 (2)	10.00				
M- L	0 / 749	-39.5	-39.5	0.19 (2)	10.00				
L- K	0 / 749	-39.5	-39.5	0.19 (2)	10.00				
K- J	0 / 934	-39.5	-39.5	0.28 (2)	10.00				
J- I	0 / 0	-39.5	-39.5	0.16 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.28/1.00 (M-N:2) , WB=0.25/1.00 (C-M:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (B) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T903848
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T99A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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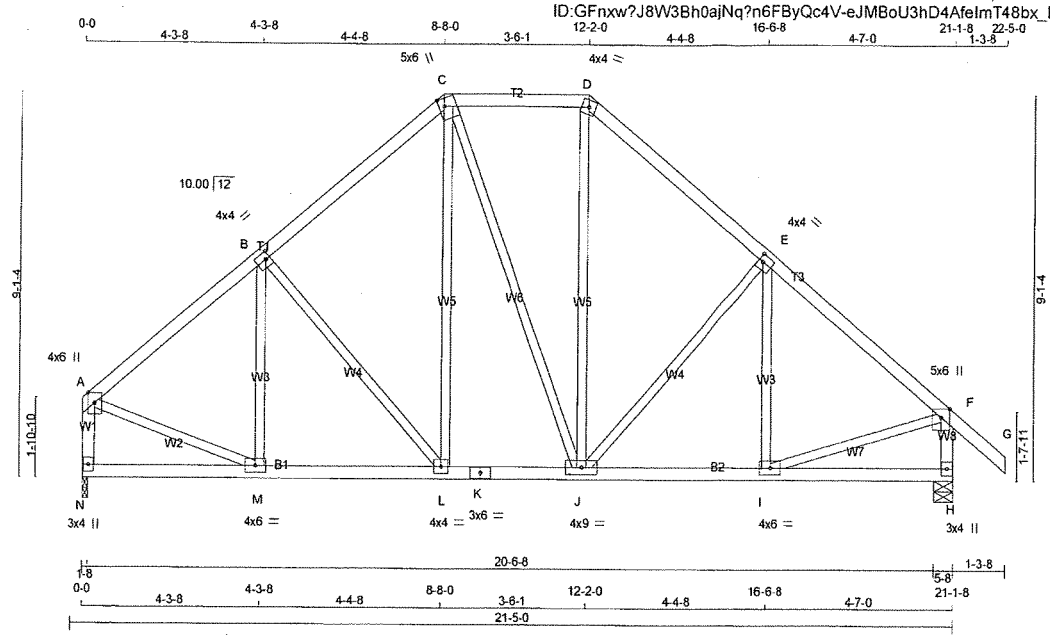

TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-1112 / 0	-77.7	-77.7	0.20 (1)	5.79	M- B	-118 / 78	0.05 (1)
B- C	-987 / 0	-77.7	-77.7	0.20 (1)	6.10	B- L	-232 / 0	0.20 (1)
C- D	-733 / 0	-77.7	-77.7	0.13 (1)	6.25	L- C	0 / 340	0.08 (2)
D- E	-978 / 0	-77.7	-77.7	0.21 (1)	6.06	C- J	0 / 20	0.00 (1)
E- F	-1170 / 0	-77.7	-77.7	0.22 (1)	5.66	J- D	0 / 365	0.08 (2)
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- E	-288 / 0	0.25 (1)
N- A	-1170 / 0	0.0	0.0	0.13 (1)	7.37	I- E	-59 / 115	0.03 (3)
H- F	-1272 / 0	0.0	0.0	0.14 (1)	7.14	A- M	0 / 929	0.21 (1)
						I- F	0 / 955	0.21 (1)
N- M	0 / 0	-39.5	-39.5	0.14 (3)	10.00			
M- L	0 / 874	-39.5	-39.5	0.26 (2)	10.00			
L- K	0 / 725	-39.5	-39.5	0.19 (2)	10.00			
K- J	0 / 725	-39.5	-39.5	0.19 (2)	10.00			
J- I	0 / 918	-39.5	-39.5	0.27 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.22/1.00 (E-F:1) , BC=0.27/1.00 (I-J:2) , WB=0.25/1.00 (E-J:1) , SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.51 (A) (INPUT = 1.00)

DWG NO. TAM TM038649
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	T100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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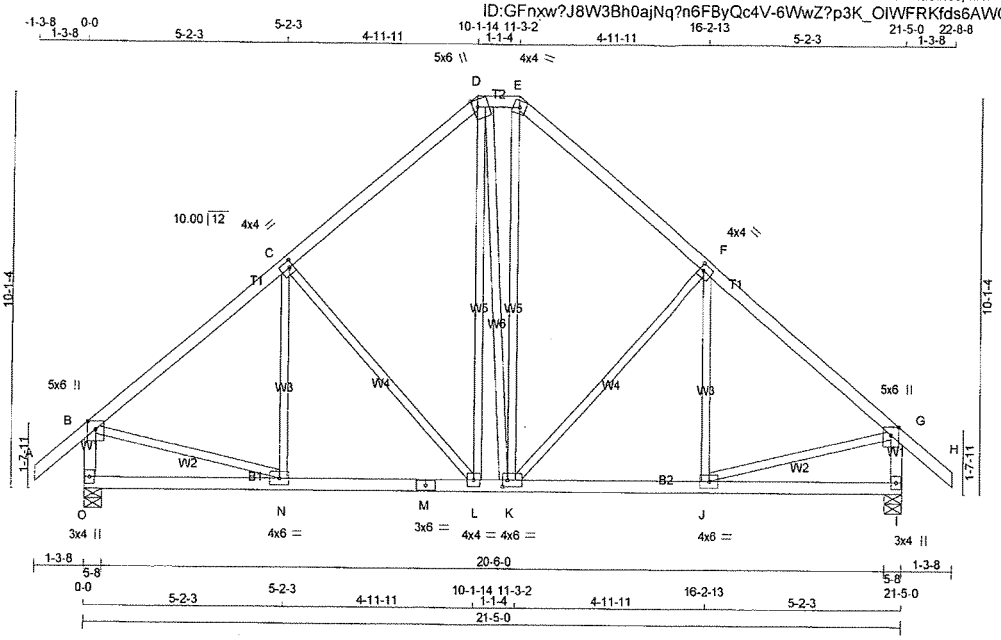


TOTAL WEIGHT = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1363	0	1363	0	0	5-8	5-8
I	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
I	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

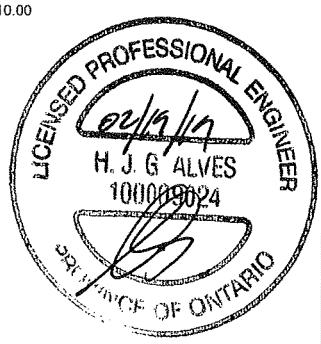
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	N- C	0 / 170	0.04 (3)	
B- C	-1193 / 0	-77.7	-77.7	0.29 (1)	5.54	C- L	-380 / 0	0.44 (1)	
C- D	-924 / 0	-77.7	-77.7	0.27 (1)	6.10	L- D	0 / 343	0.08 (2)	
D- E	-693 / 0	-77.7	-77.7	0.02 (1)	6.25	D- K	0 / 44	0.01 (2)	
E- F	-929 / 0	-77.7	-77.7	0.27 (1)	6.09	K- E	0 / 390	0.09 (2)	
F- G	-1190 / 0	-77.7	-77.7	0.29 (1)	5.54	K- F	-371 / 0	0.43 (1)	
G- H	0 / 34	-77.7	-77.7	0.11 (1)	10.00	J- F	-10 / 162	0.04 (3)	
O- B	-1281 / 0	0.0	0.0	0.14 (1)	7.12	B- N	0 / 968	0.22 (1)	
I- G	-1280 / 0	0.0	0.0	0.14 (1)	7.13	J- G	0 / 967	0.22 (1)	
O- N	0 / 0	-39.5	-39.5	0.21 (3)	10.00				
N- M	0 / 939	-39.5	-39.5	0.33 (2)	10.00				
M- L	0 / 939	-39.5	-39.5	0.33 (2)	10.00				
L- K	0 / 688	-39.5	-39.5	0.23 (2)	10.00				
K- J	0 / 937	-39.5	-39.5	0.32 (2)	10.00				
J- I	0 / 0	-39.5	-39.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.33/1.00 (L-N:2) , WB=0.44/1.00 (C-L:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (K) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM 1903850
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T100A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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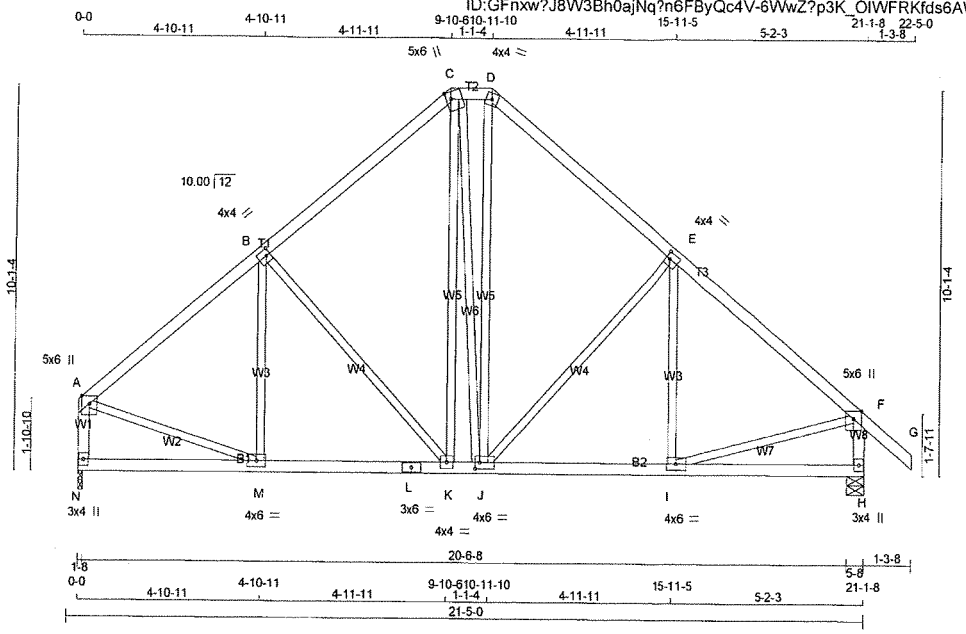


Scale = 1:58.2

TOTAL WEIGHT = 112 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	6.0	2.00	1.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1238	0	1238	0	0	1-8	1-8
H	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
H	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

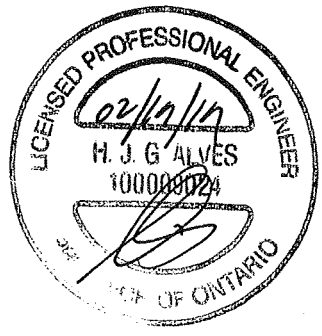
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1123 / 0	-77.7	-77.7	0.26 (1)	5.69	
B- C	-898 / 0	-77.7	-77.7	0.25 (1)	6.19	
C- D	-674 / 0	-77.7	-77.7	0.02 (1)	6.25	
D- E	-906 / 0	-77.7	-77.7	0.27 (1)	6.15	
E- F	-1170 / 0	-77.7	-77.7	0.28 (1)	5.58	
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00	
N- A	-1162 / 0	0.0	0.0	0.13 (1)	7.39	
H- F	-1263 / 0	0.0	0.0	0.13 (1)	7.16	
N- M	0 / 0	-39.5	-39.5	0.19 (3)	10.00	
M- L	0 / 886	-39.5	-39.5	0.31 (2)	10.00	
L- K	0 / 886	-39.5	-39.5	0.31 (2)	10.00	
K- J	0 / 669	-39.5	-39.5	0.22 (2)	10.00	
J- I	0 / 922	-39.5	-39.5	0.31 (2)	10.00	
I- H	0 / 0	-39.5	-39.5	0.20 (3)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-51 / 135	0.03 (3)
B- K	-328 / 0	0.38 (1)
K- C	0 / 320	0.07 (2)
C- J	0 / 53	0.01 (1)
J- D	0 / 376	0.08 (2)
J- E	-375 / 0	0.43 (1)
I- E	-5 / 165	0.04 (3)
A- M	0 / 929	0.21 (1)
I- F	0 / 951	0.21 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.31/1.00 (I-J:2) , WB=0.43/1.00 (E-J:1) , SSI=0.15/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (J) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

DWG NO. TAM 11903851
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T101	2	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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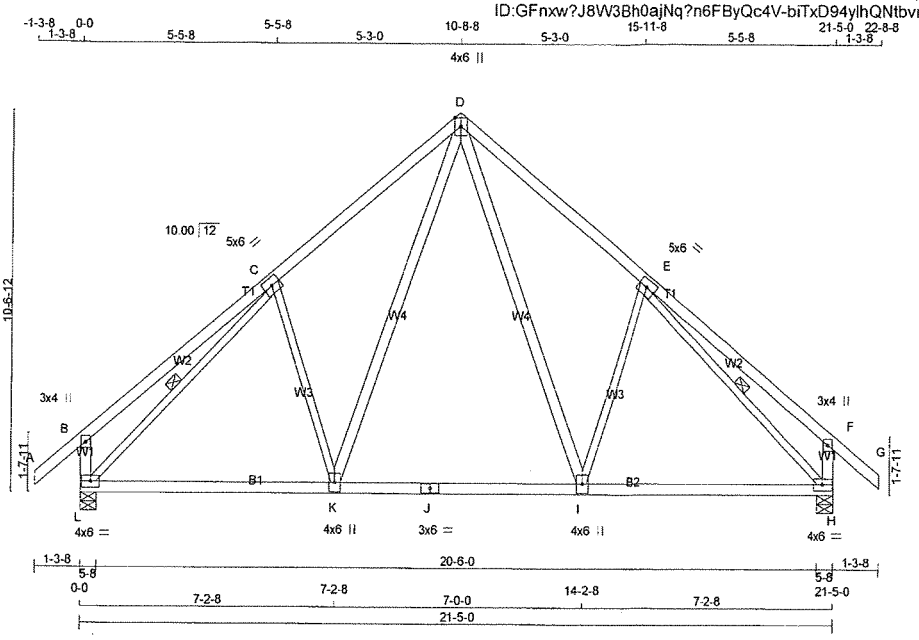

TOTAL WEIGHT = 2 X 109 = 218 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
E - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW+t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1363	0	1363	0	0	5-8	5-8
H	1363	0	1363	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0
H	1034	506 / 0	225 / 0	0 / 0	0 / 0	304 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 32	-77.7	-77.7	0.37 (1)	10.00
C- D	-1119 / 0	-77.7	-77.7	0.31 (1)	5.64
D- E	-1119 / 0	-77.7	-77.7	0.31 (1)	5.64
E- F	0 / 32	-77.7	-77.7	0.37 (1)	10.00
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00
L- B	-264 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-264 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 911	-39.5	-39.5	0.47 (2)	10.00
K- J	0 / 652	-39.5	-39.5	0.43 (2)	10.00
J- I	0 / 652	-39.5	-39.5	0.43 (2)	10.00
I- H	0 / 911	-39.5	-39.5	0.47 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 566	0.09 (1)
I- E	-240 / 17	0.16 (1)
K- D	0 / 566	0.09 (1)
C- K	-240 / 17	0.16 (1)
L- C	-1340 / 0	0.55 (1)
E- H	-1340 / 0	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.47/1.00 (K-L:2) , WB=0.55/1.00 (C-L:1) , SSI=0.18/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.35 (E) (INPUT = 1.00)

DWG NO. TAM 17703852
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	T101A	1	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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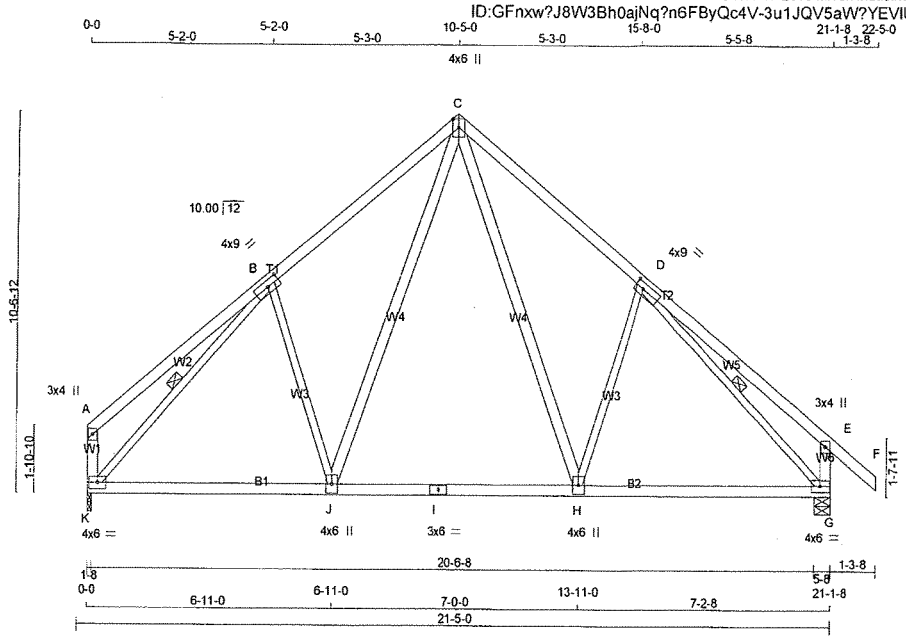


TOTAL WEIGHT = 106 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
H - D	2x3	DRY	No.2	SPF
B - J	2x3	DRY	No.2	SPF
K - B	2x3	DRY	No.2	SPF
D - G	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	9.0	2.00	4.25
C	TTWW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	9.0	2.00	3.00
E	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWW+t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1238	0	1238	0	0	1-8	1-8
G	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-K, D-G.

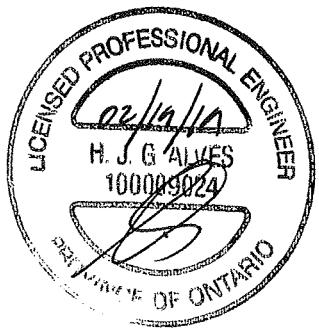
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-77.7	-77.7	0.35 (1)	10.00
B- C	-1074 / 0	-77.7	-77.7	0.29 (1)	5.75
C- D	-1101 / 0	-77.7	-77.7	0.30 (1)	5.68
D- E	0 / 31	-77.7	-77.7	0.37 (1)	10.00
E- F	0 / 34	-77.7	-77.7	0.11 (1)	10.00
K- A	-150 / 0	0.0	0.0	0.02 (1)	7.81
G- E	-269 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 861	-39.5	-39.5	0.44 (2)	10.00
J- I	0 / 634	-39.5	-39.5	0.43 (2)	10.00
I- H	0 / 634	-39.5	-39.5	0.43 (2)	10.00
H- G	0 / 895	-39.5	-39.5	0.47 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- H	0 / 579	0.09 (1)
H- D	-249 / 14	0.17 (1)
J- C	0 / 518	0.08 (1)
B- J	-200 / 36	0.13 (1)
K- B	-1303 / 0	0.50 (1)
D- G	-1313 / 0	0.53 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.37/1.00 (D-E:1) , BC=0.47/1.00 (G-H:2) , WB=0.53/1.00 (D-G:1) , SSI=0.18/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (K) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

DWG NO. TAM 1903853
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	PB1	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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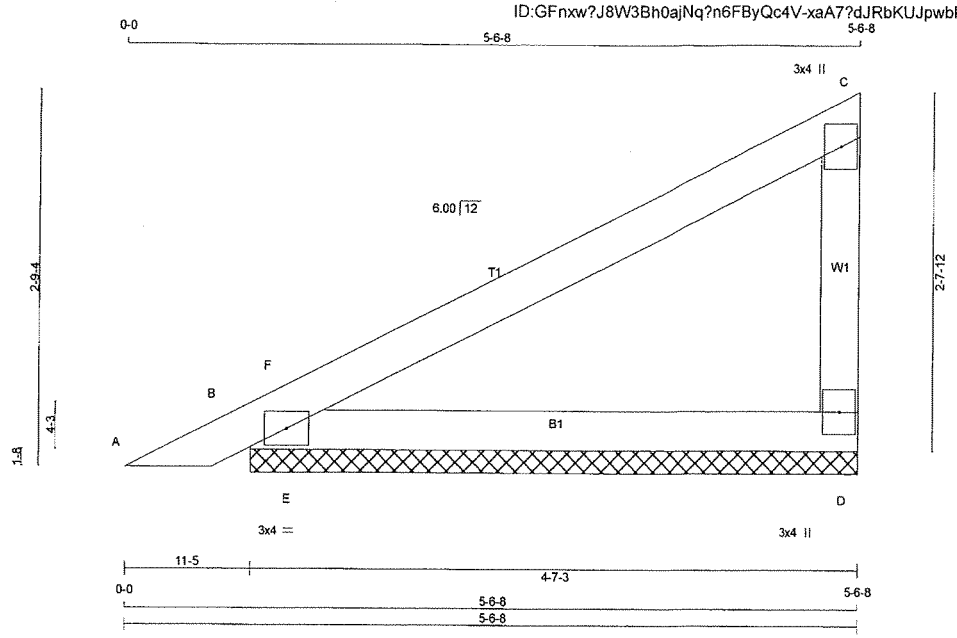


TOTAL WEIGHT = 4 X 15 = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMV+p	MT20	3.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	269	0	269	0	0	4-7-3	4-7-3
B	338	0	338	0	0	4-7-3	4-7-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	206	96 / 0	48 / 0	0 / 0	0 / 0	62 / 0	0 / 0
B	253	133 / 0	48 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

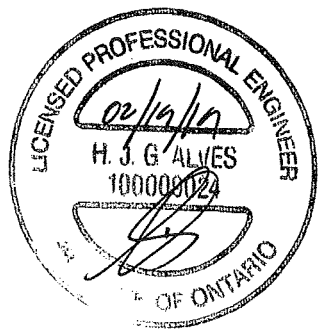
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 14	-77.7	-77.7	0.04 (1)	10.00	E- F	-172 / 120	0.00 (1)
B- F	-72 / 0	-77.7	-77.7	0.08 (3)	6.25			
F- C	0 / 6	-77.7	-77.7	0.23 (1)	10.00			
D- C	-163 / 0	0.0	0.0	0.02 (1)	7.81			
B- E	0 / 0	-39.5	-39.5	0.13 (1)	10.00			
E- D	0 / 0	-39.5	-39.5	0.19 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-F:1) , BC=0.19/1.00 (D-E:2) , WB=0.00/1.00 (E-F:1) , SSI=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 17038524
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 32 - 36	DRWG NO.
401420	PB2	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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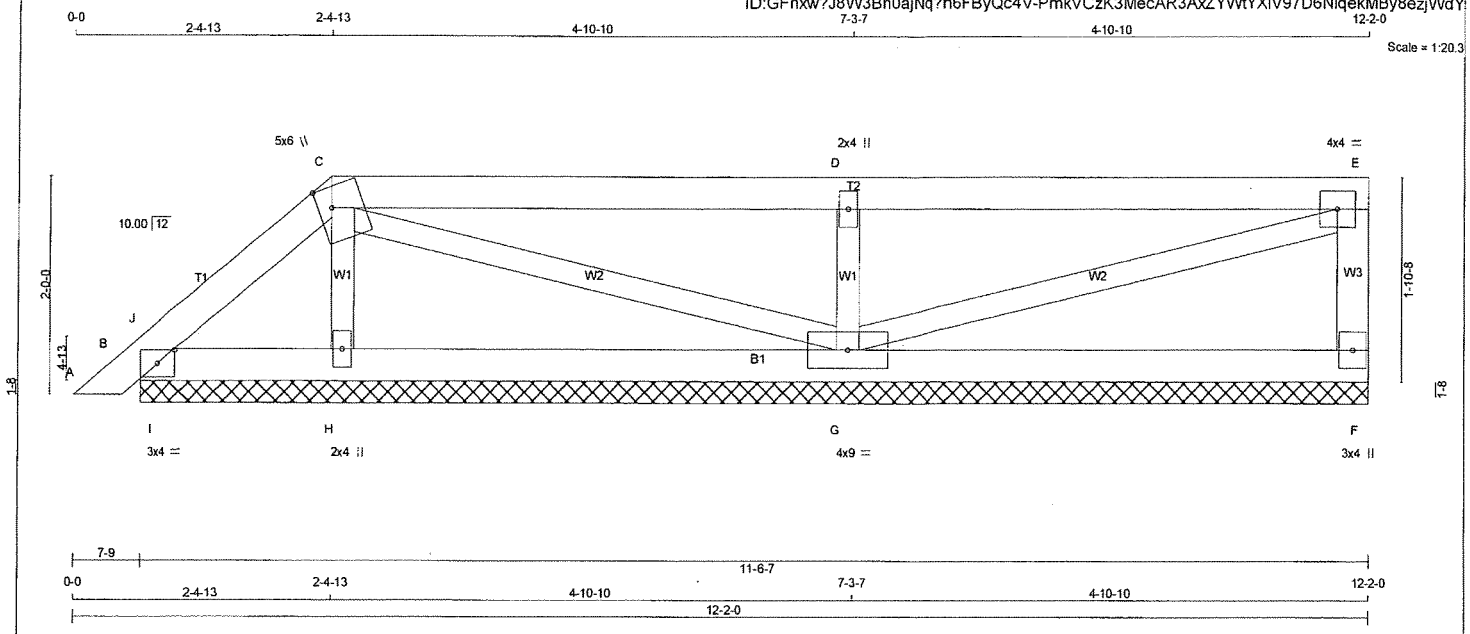

TOTAL WEIGHT = 3 X 39 = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	221	0	221	0	0	11-6-7	11-6-7
B	149	0	149	0	0	11-6-7	11-6-7
H	320	0	320	0	0	11-6-7	11-6-7
G	704	0	704	0	0	11-6-7	11-6-7

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	169	78 / 0	40 / 0	0 / 0	0 / 0	51 / 0	0 / 0
B	106	75 / 0	6 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	253	94 / 0	77 / 0	0 / 0	0 / 0	81 / 0	0 / 0
G	535	258 / 0	119 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-77.7	-77.7	0.02 (1)	10.00	H- C	-160 / 0	0.02 (1)
B- J	-19 / 9	-77.7	-77.7	0.01 (1)	6.25	C- G	-23 / 0	0.01 (1)
J- C	-46 / 0	-77.7	-77.7	0.02 (1)	6.25	G- D	-474 / 0	0.07 (1)
C- D	0 / 0	-77.7	-77.7	0.31 (1)	10.00	G- E	0 / 0	0.00 (1)
D- E	0 / 0	-77.7	-77.7	0.31 (1)	10.00	I- J	-106 / 0	0.00 (1)
F- E	-144 / 0	0.0	0.0	0.02 (1)	7.81			
B- I	0 / 32	-39.5	-39.5	0.04 (1)	10.00			
I- H	0 / 32	-39.5	-39.5	0.09 (3)	10.00			
H- G	0 / 21	-39.5	-39.5	0.19 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.31/1.00 (D-E:1) , BC=0.19/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

PROJECT NO. TAM TM03855
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401420	PB3	3	1	UNITS 32 - 36 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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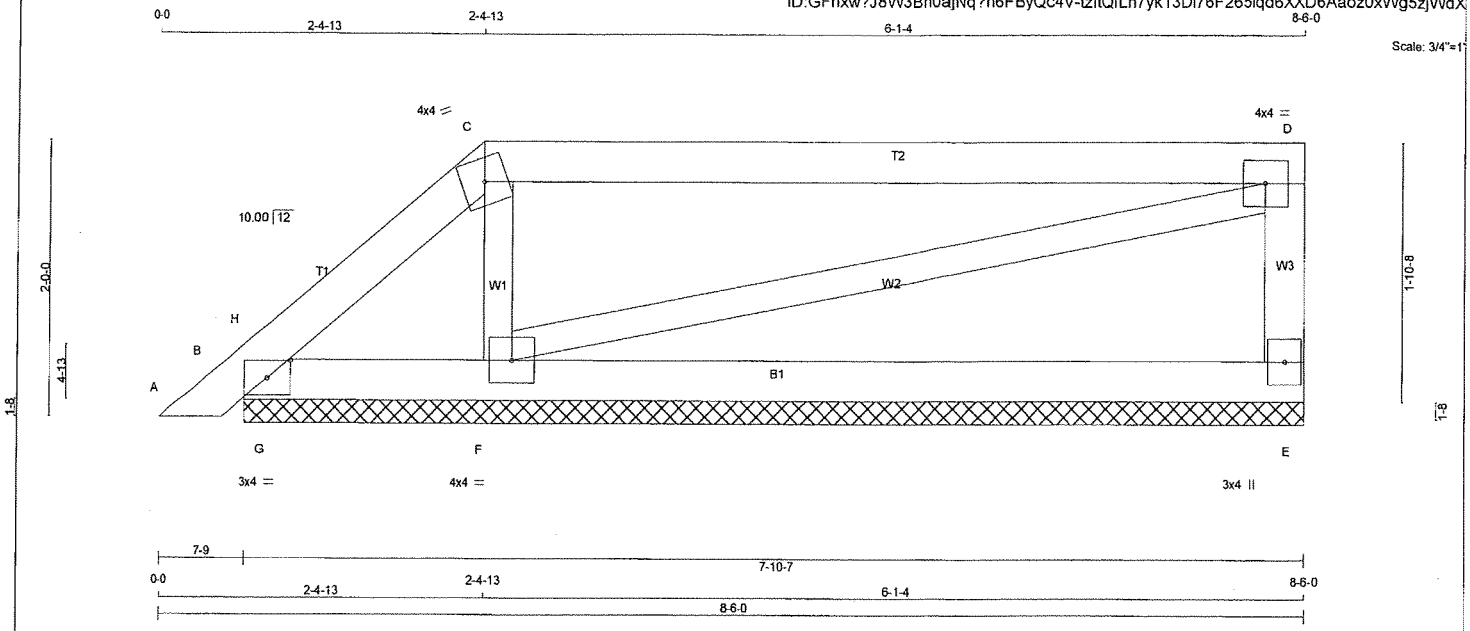


TOTAL WEIGHT = 3 X 27 = 80 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	334	0	334	0	0	7-10-7	7-10-7
B	95	0	95	0	-30	7-10-7	7-10-7
F	536	0	536	0	0	7-10-7	7-10-7

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	251	130 / 0	49 / 0	0 / 0	0 / 0	72 / 0	0 / 0
B	56	75 / 0	0 / -24	0 / 0	0 / 0	5 / 0	0 / 0
F	427	147 / 0	140 / 0	0 / 0	0 / 0	141 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-77.7	-77.7	0.02 (1)	10.00	F- C	-253 / 0	0.04 (1)
B- H	0 / 108	-77.7	-77.7	0.07 (3)	10.00	F- D	-6 / 10	0.00 (3)
H- C	-48 / 0	-77.7	-77.7	0.05 (2)	6.25	G- H	-225 / 0	0.00 (1)
C- D	-10 / 6	-77.7	-77.7	0.49 (1)	10.00			
E- D	-240 / 0	0.0	0.0	0.03 (1)	7.81			
B- G	-3 / 26	-39.5	-39.5	0.05 (1)	10.00			
G- F	-3 / 26	-39.5	-39.5	0.26 (3)	10.00			
F- E	0 / 0	-39.5	-39.5	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.49/1.00 (C-D:1) , BC=0.26/1.00 (F-G:3) , WB=0.04/1.00 (C-F:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (C) (INPUT = 0.90)
JSI METAL= 0.04 (F) (INPUT = 1.00)

DWG NO. TAM P1903856
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

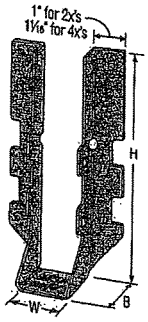
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

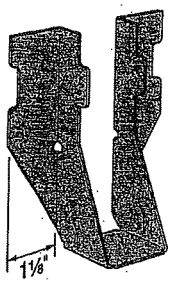
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

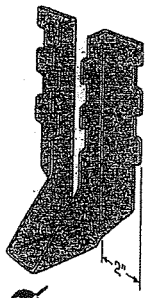
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



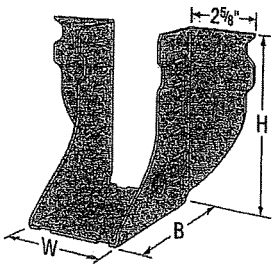
LUS28



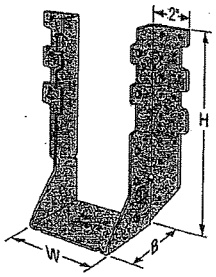
LU26L



HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

Double-Shear Nailing Top View

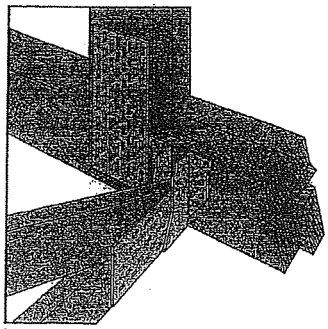
Double-Shear Nailing Side View; Do not bend tab

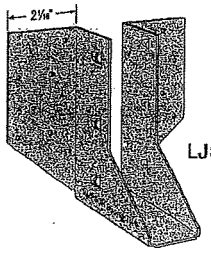
Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

Typical HUS26 Installation with Reduced Heel Height

(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON Strong-Tie®

HHUS/HGUS

See Hanger Options information on pp. 125–127.

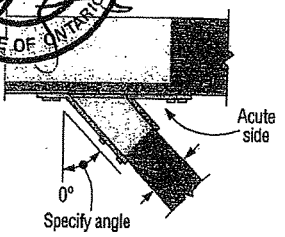
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LU24L	22	1⅜	3	1⅝	2¹⁵⁄₁₆	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
SS LUS26	18	1⅜	4¼	1¾	3¾	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 10d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅝	5⅜	5	4⅞	(20) 16d	(8) 16d	2685 / 11.95	6625 / 29.47	2685 / 11.96	5700 / 25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
SS LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.2	1290 / 5.74	1790 / 7.96
HUS28	16	1⅝	7¹⁄₁₆	3	6¹⁄₁₆	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.7	7675 / 34.13	3310 / 14.74	6900 / 30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
SS LUS210	18	1⅜	7¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

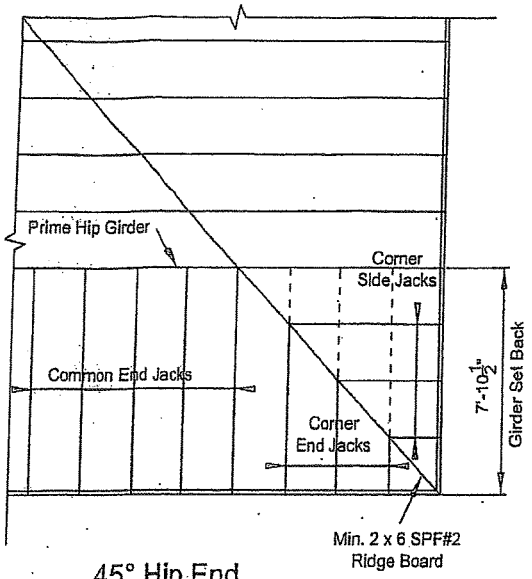
Face-Mount Hangers

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_s[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835 / 9.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⅝	5⅜	3	3⁹⁄₁₆	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS ■ LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS ■ LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⅝	9⅜	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⅝	9⅜	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹⁵⁄₁₆	9	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅝	8⅞	3	7¹⁵⁄₁₆	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⅝	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⅝	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⅝	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⅝	8¾	2	5⅞	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.



45° Hip End

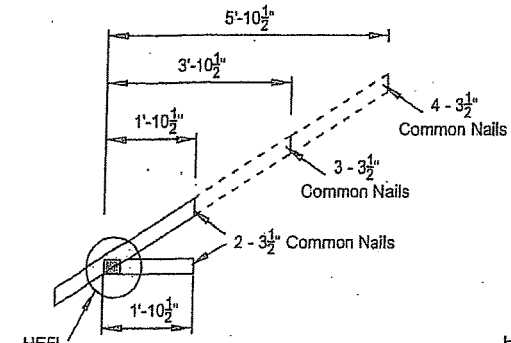
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

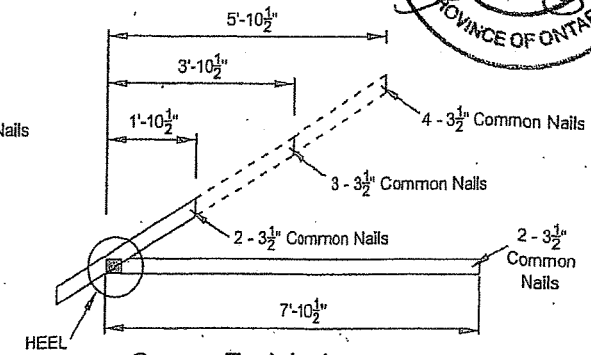
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

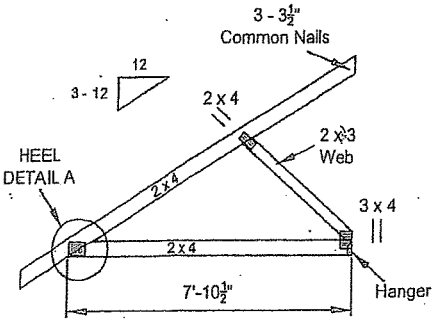
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



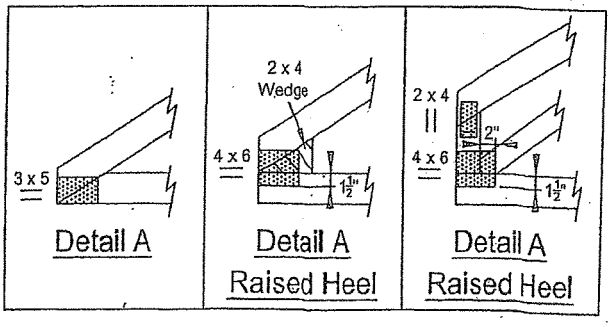
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

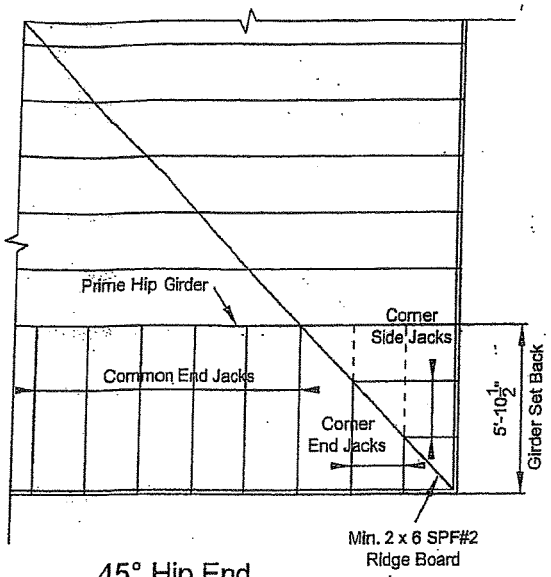
T-1800217

LUMBER SPECIFICATION

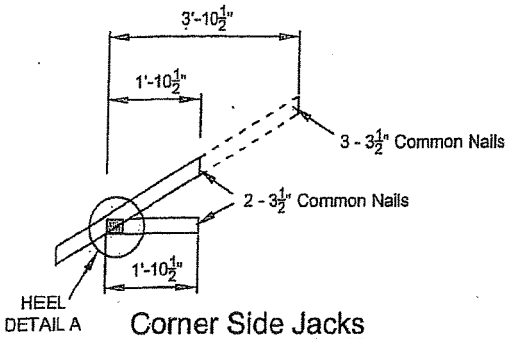
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

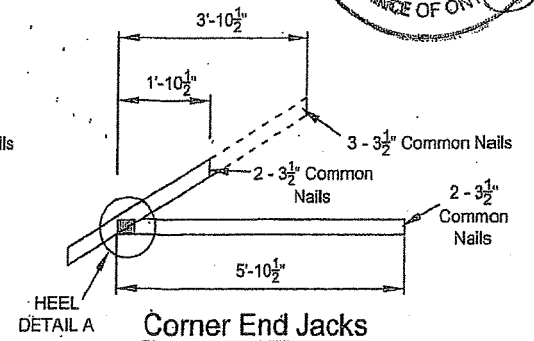
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



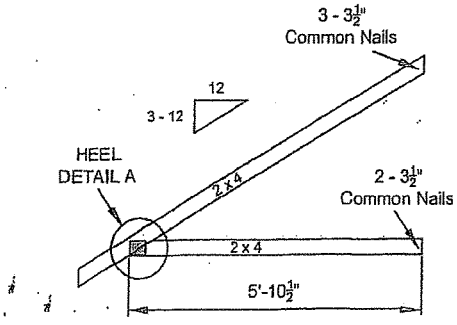
45° Hip End



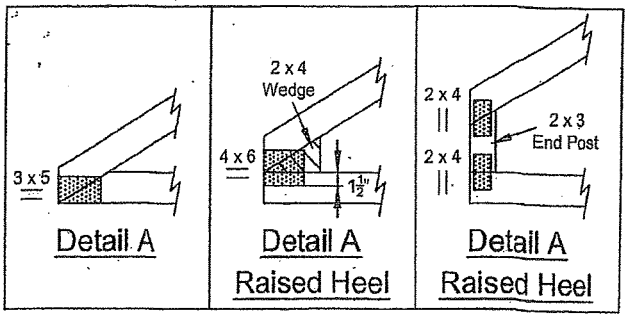
Corner Side Jacks



Corner End Jacks



Common End Jacks

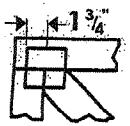


NOTE; DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

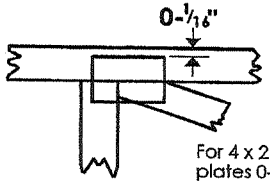
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

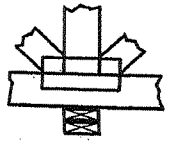
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

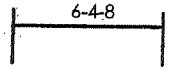
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

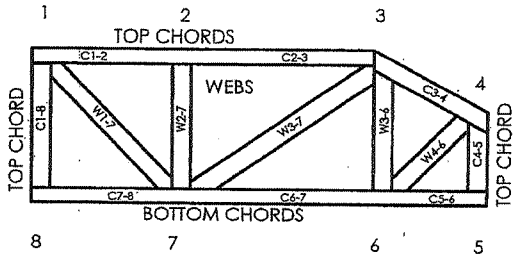
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

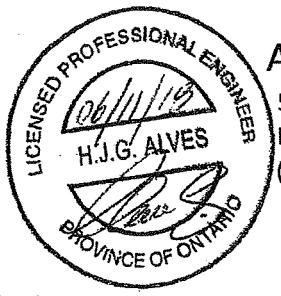
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
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RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018