ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

TH-5E
EL:A UNIT28
401438

TH-3
EL:A UNIT29
401439

TH-3
EL:(REV) A UNIT30
401440

TH-5E
EL:A(REV) UNIT31
401441

-6" WALL DIFF.

10/12 PITCHES (TYP.) UNLESS NOTED

DENOTES:

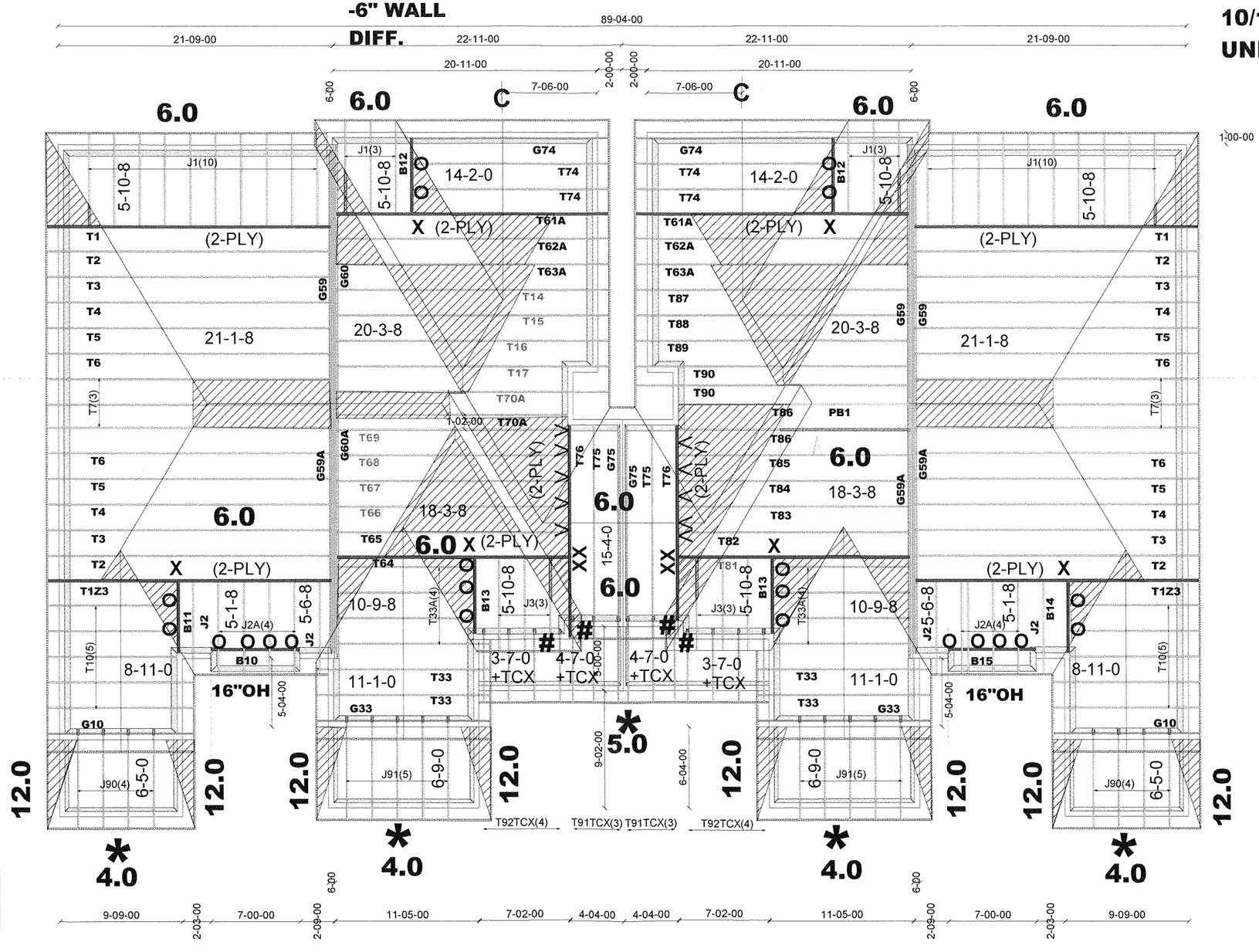
ALL B -2-2X10 FLUSH

* - ALL FIRST FLOOR PLATE HEIGHT 7'-10"-0 U/S @ PLATE LEVEL

\# -11" HIGHER FASCIA ONLY

HARDWARE:

LUS26-2 -(X)
HGUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)



T-180690

DENOTES:
CONVENTIONAL FRAMING

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c.
WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

SNOW LOAD 21.0 psf
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

TAMARACK LUMBER INC
A LPA LUMBER GROUP

Job Track: 50264	Builder / Location: BAYVIEW WELLINGTON / ST CATHERINES	Model / Elevation: FREEHOLD BLOCK 9 / UNITS 28 - 31	Mitek ver 8.2.3.229
Plan Log: 200531			
Layout ID: 401437	Project: PASSAGE ON THE CANAL		
	Date: 2/15/2019 Sales: Mario DiCano Designer: AC		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401438
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	1 of 2
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 28	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

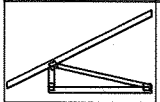
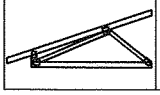
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**1 2-ply**	**T1 Half Hip Girder**	10 /12	**21-01-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	**1 2-ply**	**T1Z3 Half Hip Girder**	10 /12	**21-01-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	**2**	**T2 Half Hip**	10 /12	**21-01-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	179.6 118.67		
	**2**	**T3 Half Hip**	10 /12	**21-01-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	176.67 112.67		
	**2**	**T4 Half Hip**	10 /12	**21-01-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	**2**	**T5 Half Hip**	10 /12	**21-01-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	**2**	**T6 Half Hip**	10 /12	**21-01-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	229.3 145.67		
	**3**	**T7 Half Hip**	10 /12	**21-01-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	361.76 226.50		
	**5**	**T10 Common**	10 /12	**8-11-00**	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	206.39 134.17		
	**1**	**G10 GABLE**	10 /12	**8-11-00**	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	43.33 28.83		
	**1**	**G59 GABLE**	6 /12	**19-10-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.24 64.33		
	**1**	**G59A GABLE**	6 /12	**19-06-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.77 65.33		
	**10**	**J1 Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	**2**	**J2 Jack-Open**	6 /12	**5-06-08**	4-01-04	2 x 4	1-07-08	1-04-00 4-01-04	33.21 21.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401438
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	2 of 2
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 28	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**4**	**J2A Jack-Open**	6 /12	**5-01-08**	4-01-04	2 x 4	2-00-08	1-06-08 4-01-04	81.6 55.33		
	**4**	**J90 Monopitch**	4 /12	**6-05-08**	3-01-00	2 x 4	1-03-08	11-03 3-01-00	92.37 61.33		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1650.84** BFT. TOTAL WEIGHT OF ALL TRSSES **2574.84** LBS

HARDWARE

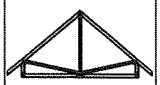
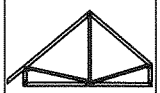
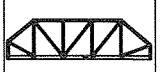
QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **7**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401439
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	1 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 29	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

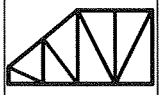
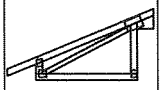
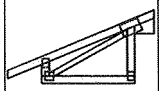
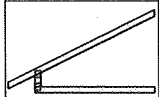
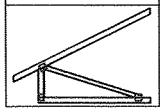
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T14 Half Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	85.89 55.00		
	1	T15 Half Hip	10 /12	20-03-08	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	93.1 59.33		
	1	T16 Half Hip	10 /12	20-03-08	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	98.11 62.67		
	1	T17 Half Hip	10 /12	20-03-08	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	112.23 72.17		
	2	T33 Common	10 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	98.81 67.33		
	4	T33A Common	10 /12	10-09-08	6-03-02	2 x 4	1-03-08	1-07-11 1-10-10	187.29 124.00		
	1	G33 GABLE	10 /12	11-01-00	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	53.85 37.67		
	1	G60 GABLE	6 /12	19-10-00	10-07-00	2 x 4		8-00 10-07-00	98.44 65.00		
	1	G60A GABLE	6 /12	19-06-00	10-07-00	2 x 4		8-00 10-07-00	98.38 65.67		
	1 2-ply	T61A Hip Girder	10 /12	20-03-08	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	193.62 123.33		
	1	T62A Hip	10 /12	20-03-08	5-01-04	2 x 4	1-03-08	1-07-11 1-10-10	84.05 55.33		
	1	T63A Hip	10 /12	20-03-08	6-01-04	2 x 4	1-03-08	1-07-11 1-10-10	88.9 57.17		
	1 2-ply	T64 Hip Girder	10 /12	18-03-08	5-00-04	2 x 4 2 x 6		2-03-05 1-10-10	186.36 121.33		
	1	T65 Hip	10 /12	18-03-08	6-00-04	2 x 4		2-03-05 1-10-10	81.4 51.67		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401439
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	2 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 29	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

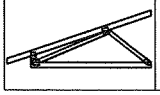
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T66 Half Hip	10 /12	18-03-08	6-06-04	2 x 4		2-03-05 6-06-04	80.52 50.00		
	1	T67 Half Hip	10 /12	18-03-08	7-06-04	2 x 4		2-03-05 7-06-04	88.53 55.33		
	1	T68 Half Hip	10 /12	18-03-08	8-06-04	2 x 4		2-03-05 8-06-04	93.7 58.50		
	1	T69 Half Hip	10 /12	18-03-08	9-06-04	2 x 4		2-03-05 9-06-04	101.68 63.33		
	2	T70A Half Hip	10 /12	18-03-08	10-01-10	2 x 6 2 x 4	3-03-08	3-03-11 10-01-04	267.41 167.00		
	2	T74 Common	10 /12	14-02-00	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.54 92.33		
	1	G74 GABLE	10 /12	14-02-00	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.95 48.17		
	1	T75 Common	6 /12	15-04-00	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	60.25 39.17		
	1	G75 GABLE	6 /12	15-04-00	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	57.71 38.17		
	1 2-ply	T76 Common Girder	6 /12	15-04-00	5-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	145.91 91.33		
	3	T91TCX Monopitch	5 /12	4-07-00	3-03-04	2 x 4	1-03-08	1-00-09 2-11-08	67.1 45.50		
	4	T92TCX Monopitch	5 /12	3-07-00	2-10-04	2 x 4	1-03-08	1-00-09 2-06-08	74.56 50.67		
	3	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	50.38 32.00		
	3	J3 Jack-Open	6 /12	5-10-08	5-00-04	2 x 4	1-03-08	2-01-00 5-00-04	66.65 43.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401439
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	3 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 29	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	**5**	**J91 Monopitch**	4 /12	**6-09-00**	3-02-03	2 x 4	1-03-08	11-03 3-02-03	120.19 79.17		

TOTAL # TRUSS= **51** TOTAL BFT OF ALL TRUSSES= **1971.34** BFT. TOTAL WEIGHT OF ALL TRSSES **3038.54** LBS

HARDWARE

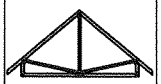
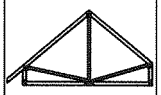
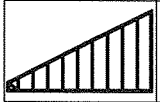
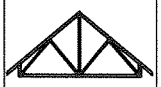
QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
5	Hardware	LUS24	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **13**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401440
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	1 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 30	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

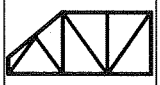
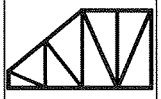
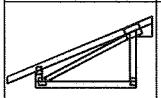
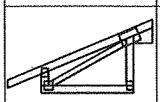
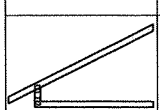
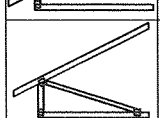
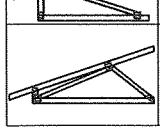
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	T33 Common	10 /12	**11-01-00**	6-03-02	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	98.81 67.33		
	4	T33A Common	10 /12	**10-09-08**	6-03-02	2 x 4	1-03-08	1-07-11 1-10-10	187.29 124.00		
	1	G59 GABLE	6 /12	**19-10-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.24 64.33		
	1	G59A GABLE	6 /12	**19-06-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.77 65.33		
	1 2-ply	T61A Hip Girder	10 /12	**20-03-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 1-10-10	193.62 123.33		
	1	T62A Hip	10 /12	**20-03-08**	5-01-04	2 x 4	1-03-08	1-07-11 1-10-10	84.05 55.33		
	1	T63A Hip	10 /12	**20-03-08**	6-01-04	2 x 4	1-03-08	1-07-11 1-10-10	88.9 57.17		
	2	T74 Common	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	133.54 92.33		
	1	G74 GABLE	10 /12	**14-02-00**	7-06-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	69.95 48.17		
	1	T75 Common	6 /12	**15-04-00**	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	60.25 39.17		
	1	G75 GABLE	6 /12	**15-04-00**	5-00-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	57.71 38.17		
	1 2-ply	T76 Common Girder	6 /12	**15-04-00**	5-00-00	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	145.91 91.33		
	1 2-ply	T81 Half Hip Girder	10 /12	**18-03-08**	5-00-04	2 x 4 2 x 6		2-03-06 5-00-04	181.16 115.33		
	1	T82 Half Hip	10 /12	**18-03-08**	6-00-04	2 x 4		2-03-06 6-00-04	78.31 49.50		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401440
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	2 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 30	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T83 Half Hip	10 /12	**18-03-08**	7-00-04	2 x 4		2-03-06 7-00-04	82.81 51.67		
	1	T84 Half Hip	10 /12	**18-03-08**	8-00-04	2 x 4		2-03-06 8-00-04	91.09 58.67		
	1	T85 Half Hip	10 /12	**18-03-08**	9-00-04	2 x 4		2-03-06 9-00-04	98.87 62.67		
	2	T86 Half Hip	10 /12	**18-03-08**	10-00-04	2 x 4		2-03-06 10-00-04	222.51 139.00		
	1	T87 Half Hip	10 /12	**20-03-08**	6-07-04	2 x 4	1-03-08	1-07-11 6-07-04	88.11 57.33		
	1	T88 Half Hip	10 /12	**20-03-08**	7-07-04	2 x 4	1-03-08	1-07-11 7-07-04	95.58 62.67		
	1	T89 Half Hip	10 /12	**20-03-08**	8-07-04	2 x 4	1-03-08	1-07-11 8-07-04	102.94 66.17		
	2	T90 Half Hip	10 /12	**18-03-08**	10-04-08	2 x 6 2 x 4	3-03-08	3-03-11 10-04-02	270.42 164.00		
	3	T91TCX Monopitch	5 /12	**4-07-00**	3-03-04	2 x 4	1-03-08	1-00-09 2-11-08	67.1 45.50		
	4	T92TCX Monopitch	5 /12	**3-07-00**	2-10-04	2 x 4	1-03-08	1-00-09 2-06-08	74.56 50.67		
	1	PB1 Piggyback	10 /12	**10-02-07**	2-00-00	2 x 4		2-00-00	33.1 22.00		
	3	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	50.38 32.00		
	3	J3 Jack-Open	6 /12	**5-10-08**	5-00-04	2 x 4	1-03-08	2-01-00 5-00-04	66.65 43.00		
	5	J91 Monopitch	4 /12	**6-09-00**	3-02-03	2 x 4	1-03-08	11-03 3-02-03	120.19 79.17		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401440
Location:	ST CATHERINES	Ref #	
Model:	TH-3	Page:	3 of 3
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 30	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS

TOTAL # TRUSS= **51** TOTAL BFT OF ALL TRUSSES= **1965.34** BFT. TOTAL WEIGHT OF ALL TRSSES **3046.81** LBS

HARDWARE

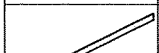
QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	
5	Hardware	LUS24	
2	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **13**

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401441
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	1 of 2
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 31	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

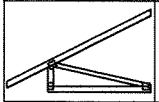
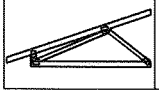
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**1** **2-ply**	**T1** **Half Hip** **Girder**	10 /12	**21-01-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	**1** **2-ply**	**T1Z3** **Half Hip** **Girder**	10 /12	**21-01-08**	4-01-04	2 x 4 2 x 6	1-03-08	1-07-11 4-01-04	203.9 129.67		
	**2**	**T2** **Half Hip**	10 /12	**21-01-08**	5-01-04	2 x 4	1-03-08	1-07-11 5-01-04	179.6 118.67		
	**2**	**T3** **Half Hip**	10 /12	**21-01-08**	6-01-04	2 x 4	1-03-08	1-07-11 6-01-04	176.67 112.67		
	**2**	**T4** **Half Hip**	10 /12	**21-01-08**	7-01-04	2 x 4	1-03-08	1-07-11 7-01-04	190.98 122.67		
	**2**	**T5** **Half Hip**	10 /12	**21-01-08**	8-01-04	2 x 4	1-03-08	1-07-11 8-01-04	200.88 128.00		
	**2**	**T6** **Half Hip**	10 /12	**21-01-08**	9-01-04	2 x 4	1-03-08	1-07-11 9-01-04	229.3 145.67		
	**3**	**T7** **Half Hip**	10 /12	**21-01-08**	10-01-04	2 x 4	1-03-08	1-07-11 10-01-04	361.76 226.50		
	**5**	**T10** **Common**	10 /12	**8-11-00**	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	206.39 134.17		
	**1**	**G10** **GABLE**	10 /12	**8-11-00**	5-04-04	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	43.33 28.83		
	**1**	**G59** **GABLE**	6 /12	**19-10-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.24 64.33		
	**1**	**G59A** **GABLE**	6 /12	**19-06-00**	11-01-00	2 x 4		1-02-00 11-01-00	101.77 65.33		
	**10**	**J1** **Jack-Open**	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	167.94 106.67		
	**2**	**J2** **Jack-Open**	6 /12	**5-06-08**	4-01-04	2 x 4	1-07-08	1-04-00 4-01-04	33.21 21.33		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50264
Builder:	BAYVIEW WELLINGTON /	PlanLog:	200531
Project:	PASSAGE ON THE CANAL	Layout ID:	401441
Location:	ST CATHERINES	Ref #	
Model:	TH-5E	Page:	2 of 2
Lot #:		Date:	02/15/2019
Elevation:	A UNIT 31	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	J2A Jack-Open	6 /12	**5-01-08**	4-01-04	2 x 4	2-00-08	1-06-08 4-01-04	81.6 55.33		
	4	J90 Monopitch	4 /12	**6-05-08**	3-01-00	2 x 4	1-03-08	11-03 3-01-00	92.37 61.33		

TOTAL # TRUSS= **45** TOTAL BFT OF ALL TRUSSES= **1650.84** BFT. TOTAL WEIGHT OF ALL TRSSES **2574.84** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
1	Hardware	LUS26-2	

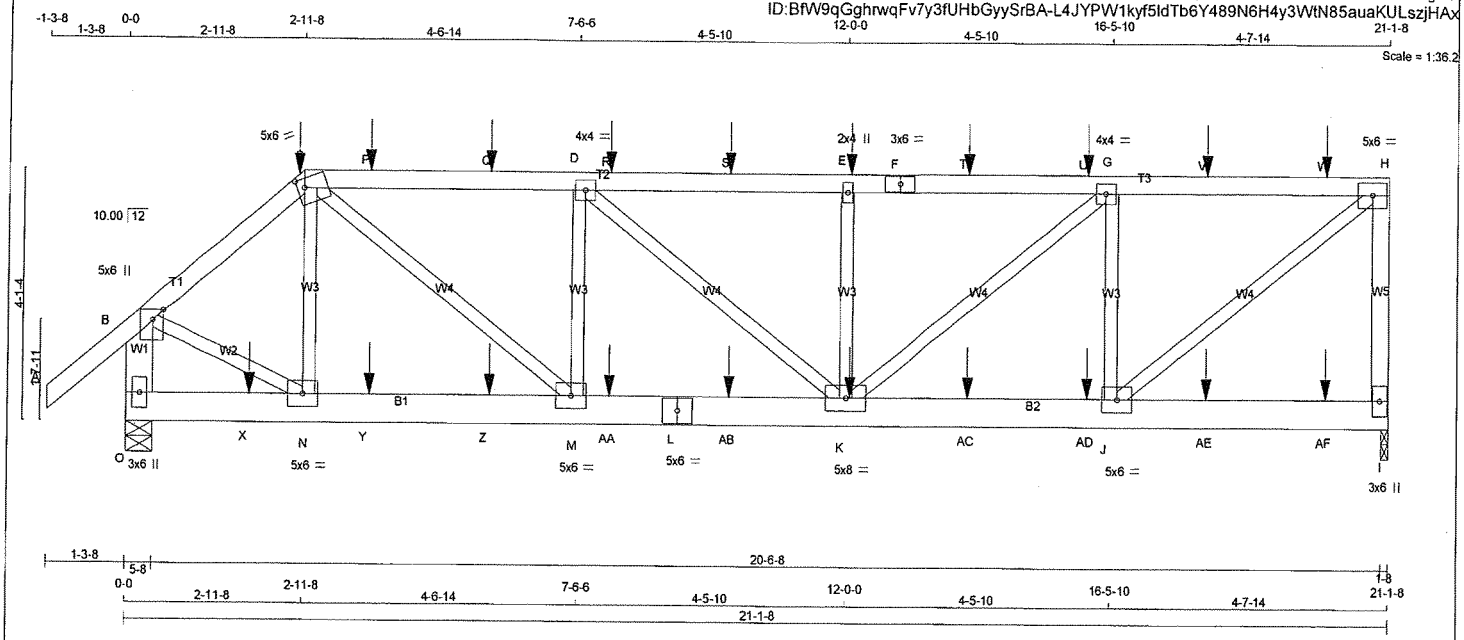
TOTAL NUMBER OF ITEMS= **7**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T1	2	2	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Feb 20 11:03:14 2019 Page 1

ID:BfW9qGghrwqFv7y3fUHbGyySrBA-L4JYPW1kyf5ldTb6Y489N6H4y3WtN85auaKULszjHAx



TOTAL WEIGHT = 4 X 102 = 408 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(0.1)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(0.0)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2044	0	2044	0	0	1-8	1-8
O	2163	0	2163	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1567	717 / 0	379 / 0	0 / 0	0 / 0	472 / 0	0 / 0
O	1644	797 / 0	365 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00
B- C	-1998 / 0	-77.7	-77.7	0.08 (1)	6.15
C- P	-2796 / 0	-77.7	-77.7	0.24 (1)	5.22
P- Q	-2796 / 0	-77.7	-77.7	0.24 (1)	5.22
Q- D	-2796 / 0	-77.7	-77.7	0.24 (1)	5.22
D- R	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
R- S	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
S- E	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
E- F	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
F- T	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
T- U	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
U- G	-2916 / 0	-77.7	-77.7	0.24 (1)	5.13
G- V	-2099 / 0	-77.7	-77.7	0.23 (1)	5.83
V- W	-2099 / 0	-77.7	-77.7	0.23 (1)	5.83
W- H	-2099 / 0	-77.7	-77.7	0.23 (1)	5.83
I- H	-1909 / 0	0.0	0.0	0.24 (1)	7.81
O- B	-2119 / 0	0.0	0.0	0.08 (1)	7.81
O- X	0 / 0	-39.5	-39.5	0.05 (3)	10.00
X- N	0 / 0	-39.5	-39.5	0.05 (3)	10.00
N- Y	0 / 1521	-39.5	-39.5	0.14 (1)	10.00
Y- Z	0 / 1521	-39.5	-39.5	0.14 (1)	10.00
Z- M	0 / 1521	-39.5	-39.5	0.14 (1)	10.00
M-AA	0 / 2796	-39.5	-39.5	0.24 (1)	10.00
AA- L	0 / 2798	-39.5	-39.5	0.24 (1)	10.00
L-AB	0 / 2796	-39.5	-39.5	0.24 (1)	10.00
AB- K	0 / 2796	-39.5	-39.5	0.24 (1)	10.00
K-AC	0 / 2099	-39.5	-39.5	0.18 (1)	10.00
AC-AD	0 / 2099	-39.5	-39.5	0.18 (1)	10.00
AD- J	0 / 2099	-39.5	-39.5	0.18 (1)	10.00
J-AE	0 / 0	-39.5	-39.5	0.06 (3)	10.00
AE-AF	0 / 0	-39.5	-39.5	0.06 (3)	10.00
AF- I	0 / 0	-39.5	-39.5	0.06 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-315 / 21	0.04 (1)
B- N	0 / 1647	0.20 (1)
J- H	0 / 2689	0.33 (1)
C- M	0 / 1642	0.20 (1)
J- G	-1333 / 0	0.17 (1)
M- D	-733 / 0	0.09 (1)
K- G	0 / 1064	0.13 (1)
D- K	0 / 157	0.02 (1)
K- E	-507 / 0	0.06 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	2-11-8	-38	-42	---	FRONT	VERT	DEAD	---	---
C	2-11-8	-180	-180	---	FRONT	VERT	SNOW	---	---
E	12-0-12	-94	-94	---	BACK	VERT	TOTAL	---	---
K	12-0-12	-56	-71	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.24/1.00 (E-G:1) , BC=0.24/1.00 (K-M:1) , WB=0.33/1.00 (H-J:1) , SSI=0.14/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.65 (H) (INPUT = 0.90)
JSI METAL= 0.29 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
02/20/19
H J G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 1903858
STRUCTURAL
COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T1	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	4-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
Q	6-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
R	8-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
S	10-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
T	14-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
U	16-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
V	18-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
W	20-0-12	-94	-94	—	BACK	VERT	TOTAL	—	—
X	2-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
Y	4-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
Z	6-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AA	8-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AB	10-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AC	14-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AD	16-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AE	18-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—
AF	20-0-12	-56	-71	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM T1903858
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T1Z3	2	2	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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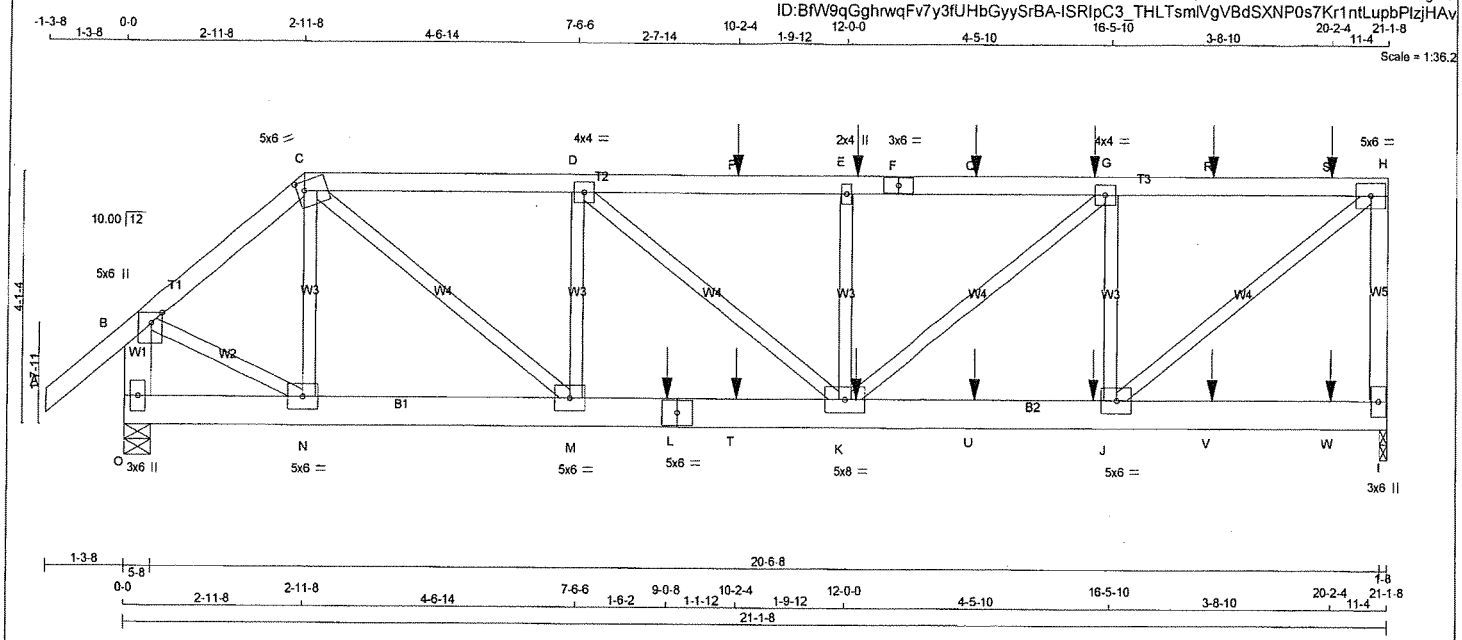


TOTAL WEIGHT = 4 X 102 = 408 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- I	1	12	TOP
O- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	SIDE(0.0)
L- I	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	1.75	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J, M, N						
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BS-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	2329	0	2329	0	0	1-8	1-8
O	2111	0	2111	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1772	851 / 0	397 / 0	0 / 0	0 / 0	524 / 0	0 / 0
O	1596	799 / 0	333 / 0	0 / 0	0 / 0	464 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	0 / 34	-77.7	-77.7	0.06 (1)	10.00	
B- C	-1927 / 0	-77.7	-77.7	0.08 (1)	6.23	
C- D	-3285 / 0	-77.7	-77.7	0.18 (1)	4.98	
D- P	-3564 / 0	-77.7	-77.7	0.19 (1)	4.81	
P- E	-3564 / 0	-77.7	-77.7	0.19 (1)	4.81	
E- F	-3564 / 0	-77.7	-77.7	0.27 (1)	4.71	
F- Q	-3564 / 0	-77.7	-77.7	0.27 (1)	4.71	
Q- G	-3564 / 0	-77.7	-77.7	0.27 (1)	4.71	
G- R	-2424 / 0	-77.7	-77.7	0.24 (1)	5.52	
R- S	-2424 / 0	-77.7	-77.7	0.24 (1)	5.52	
S- H	-2424 / 0	-77.7	-77.7	0.24 (1)	5.52	
I- H	-2179 / 0	0.0	0.0	0.27 (1)	7.54	
O- B	-2051 / 0	0.0	0.0	0.08 (1)	7.81	
O- N	0 / 0	-39.5	-39.5	0.02 (2)	10.00	
N- M	0 / 1460	-39.5	-39.5	0.21 (1)	10.00	
M- L	0 / 3285	-39.5	-39.5	0.49 (1)	10.00	
L- T	0 / 3285	-39.5	-39.5	0.49 (1)	10.00	
T- K	0 / 3285	-39.5	-39.5	0.49 (1)	10.00	
K- U	0 / 2424	-39.5	-39.5	0.24 (1)	10.00	
U- J	0 / 2424	-39.5	-39.5	0.24 (1)	10.00	
J- V	0 / 0	-39.5	-39.5	0.07 (2)	10.00	
V- W	0 / 0	-39.5	-39.5	0.07 (2)	10.00	
W- I	0 / 0	-39.5	-39.5	0.07 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
N- C	-511 / 0	0.06 (1)
B- N	0 / 1588	0.20 (1)
J- H	0 / 3106	0.38 (1)
C- M	0 / 2349	0.29 (1)
J- G	-1667 / 0	0.21 (1)
M- D	-646 / 0	0.08 (1)
K- G	0 / 1484	0.18 (1)
D- K	0 / 364	0.04 (1)
K- F	-544 / 0	0.07 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	12-2-4	-121	-121	---	FRONT	VERT	TOTAL	---	---
G	16-2-4	-121	-121	---	FRONT	VERT	TOTAL	---	---
J	16-2-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
K	12-2-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
L	9-0-8	-867	-867	---	FRONT	VERT	TOTAL	---	---
P	10-2-4	-84	-84	---	FRONT	VERT	TOTAL	---	---
Q	14-2-4	-121	-121	---	FRONT	VERT	TOTAL	---	---
R	18-2-4	-121	-121	---	FRONT	VERT	TOTAL	---	---
S	20-2-4	-86	-86	---	FRONT	VERT	TOTAL	---	---
T	10-2-4	-52	-66	---	FRONT	VERT	TOTAL	---	---
U	14-2-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
V	18-2-4	-57	-72	---	FRONT	VERT	TOTAL	---	---
W	20-2-4	-54	-68	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.27/1.00 (H-I:1) , BC=0.49/1.00 (K-M:1) , WB=0.38/1.00 (H-J:1) , SSI=0.22/1.00 (K-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

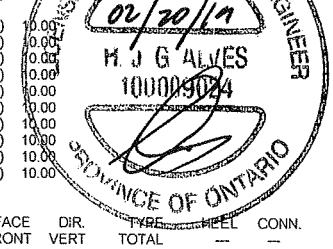
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.64 (L) (INPUT = 1.00)



DWG NO. TAM 1903854
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T2	4	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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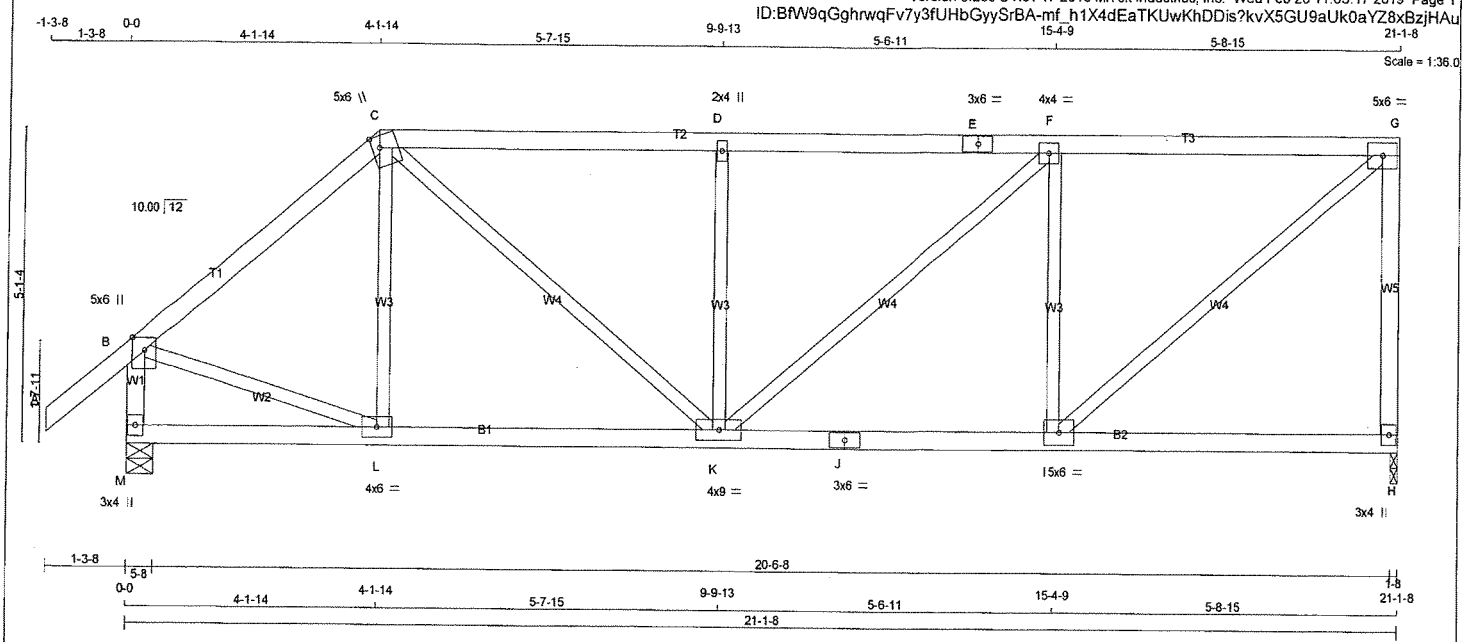

TOTAL WEIGHT = 4 X 90 = 359 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1238	0	1238	0	0	1-8	1-8
M	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
M	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	L- C	-52 / 121	0.03 (3)
B- C	-1157 / 0	-77.7	-77.7	0.26 (1)	5.60	B- L	0 / 927	0.21 (1)
C- D	-1417 / 0	-77.7	-77.7	0.38 (1)	5.04	I- G	0 / 1503	0.34 (1)
D- E	-1417 / 0	-77.7	-77.7	0.39 (1)	5.01	C- K	0 / 697	0.16 (1)
E- F	-1417 / 0	-77.7	-77.7	0.39 (1)	5.01	I- F	-712 / 0	0.27 (1)
F- G	-1153 / 0	-77.7	-77.7	0.38 (1)	5.44	K- D	-470 / 0	0.18 (1)
H- G	-1146 / 0	0.0	0.0	0.51 (1)	7.43	K- F	0 / 350	0.08 (1)
M- B	-1284 / 0	0.0	0.0	0.14 (1)	7.11			
M- L	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
L- K	0 / 885	-39.5	-39.5	0.30 (2)	10.00			
K- J	0 / 1153	-39.5	-39.5	0.39 (2)	10.00			
J- I	0 / 1153	-39.5	-39.5	0.39 (2)	10.00			
I- H	0 / 0	-39.5	-39.5	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.39/1.00 (I-K:2) , WB=0.34/1.00 (G-I:1) , SSI=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.44 (B) (INPUT = 1.00)

DWG NO. TAM P1103860
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T3	4	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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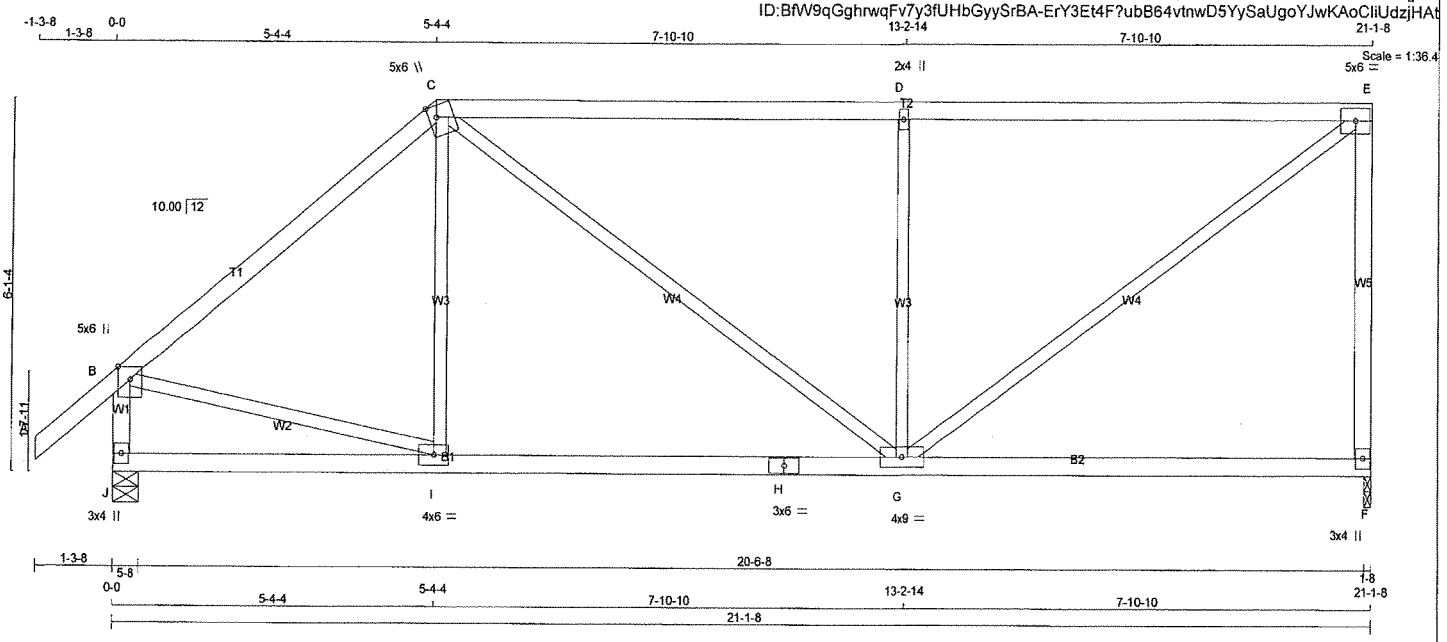

TOTAL WEIGHT = 4 X 88 = 353 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1238	0	1238	0	0	1-8	1-8
J	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
J	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 228	0.05 (3)	
B- C	-1152 / 0	-77.7	-77.7	0.45 (1)	5.35	C- G	0 / 392	0.09 (1)	
C- D	-1199 / 0	-77.7	-77.7	0.97 (1)	3.98	G- D	-759 / 0	0.44 (1)	
D- E	-1200 / 0	-77.7	-77.7	0.98 (1)	3.98	G- E	0 / 1488	0.33 (1)	
F- E	-1113 / 0	0.0	0.0	0.80 (1)	7.51	B- I	0 / 908	0.20 (1)	
J- B	-1266 / 0	0.0	0.0	0.13 (1)	7.16				
J- I	0 / 0	-39.5	-39.5	0.28 (3)	10.00				
I- H	0 / 884	-39.5	-39.5	0.57 (2)	10.00				
H- G	0 / 884	-39.5	-39.5	0.57 (2)	10.00				
G- F	0 / 0	-39.5	-39.5	0.48 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.98/1.00 (D-E:1) , BC=0.57/1.00 (G-I:2) , WB=0.44/1.00 (D-G:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM 1903861
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T4	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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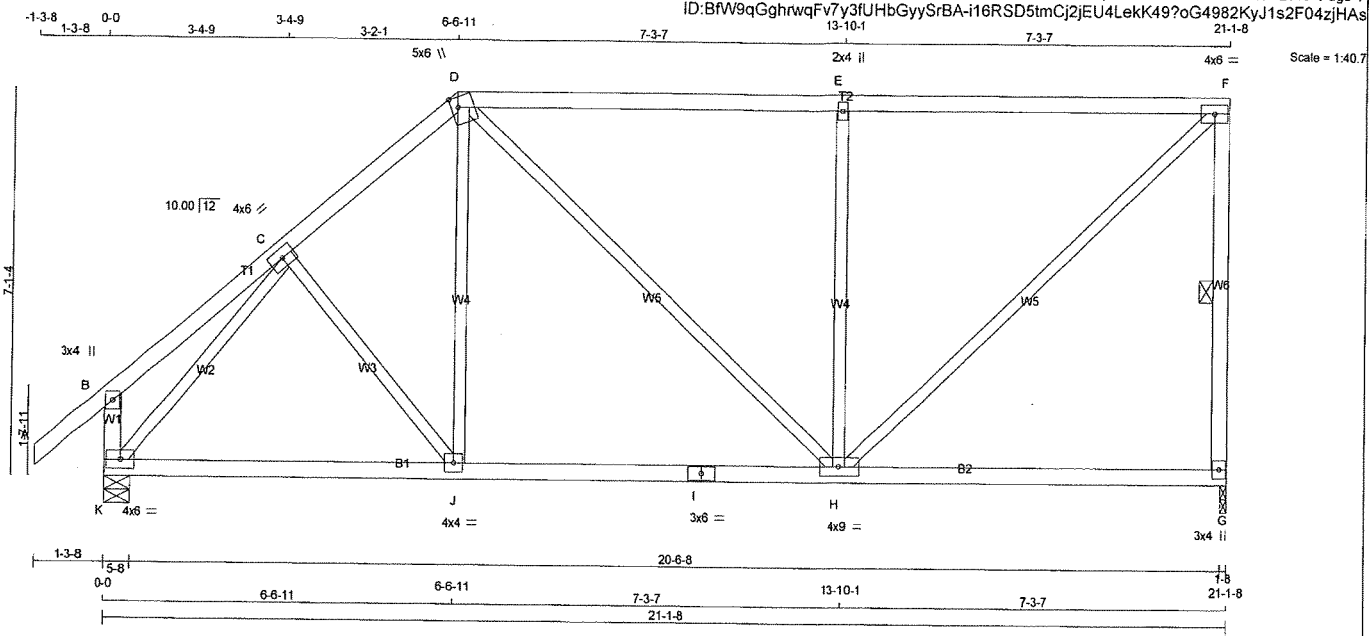

TOTAL WEIGHT = 4 X 95 = 382 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	0 / 88	0.02 (3)
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00	J- D	0 / 331	0.07 (2)
C- D	-1124 / 0	-77.7	-77.7	0.14 (1)	5.83	D- H	0 / 166	0.04 (1)
D- E	-973 / 0	-77.7	-77.7	0.78 (1)	4.91	H- E	-701 / 0	0.61 (1)
E- F	-974 / 0	-77.7	-77.7	0.78 (1)	4.91	H- F	0 / 1329	0.30 (1)
G- F	-1121 / 0	0.0	0.0	0.26 (1)	5.99	K- C	-1330 / 0	0.57 (1)
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 837	-39.5	-39.5	0.42 (2)	10.00			
J- I	0 / 852	-39.5	-39.5	0.48 (2)	10.00			
I- H	0 / 852	-39.5	-39.5	0.48 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.39 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.78/1.00 (E-F:1) , BC=0.48/1.00 (H-J:2) , WB=0.61/1.00 (E-H:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

DWG NO. TAM 77903862
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T5	4	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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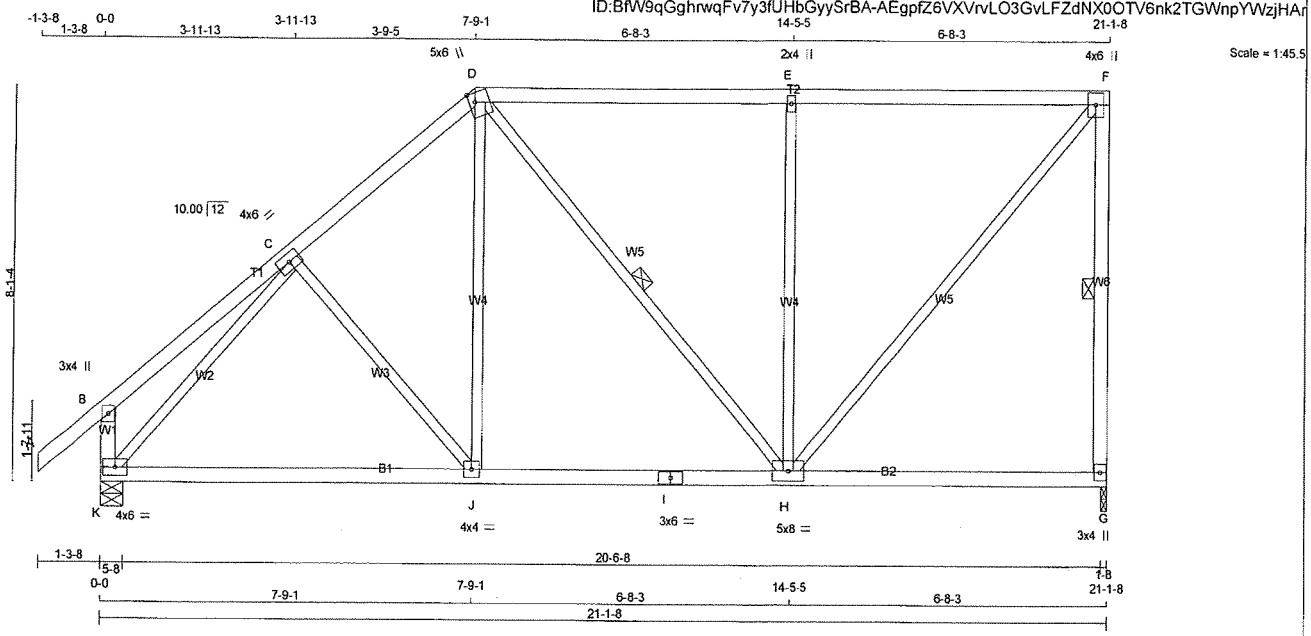


TOTAL WEIGHT = 4 X 100 = 402 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
K	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
K	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

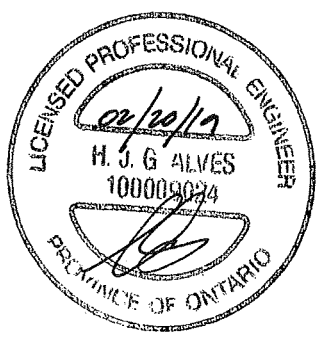
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-66 / 57	0.04 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00	J- D	0 / 418	0.09 (2)
C- D	-1072 / 0	-77.7	-77.7	0.20 (1)	5.85	D- H	-50 / 0	0.04 (3)
D- E	-800 / 0	-77.7	-77.7	0.63 (1)	5.68	H- E	-643 / 0	0.81 (1)
E- F	-800 / 0	-77.7	-77.7	0.63 (1)	5.68	H- F	0 / 1227	0.28 (1)
G- F	-1128 / 0	0.0	0.0	0.33 (1)	5.98	K- C	-1308 / 0	0.78 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 849	-39.5	-39.5	0.50 (2)	10.00			
J- I	0 / 810	-39.5	-39.5	0.50 (2)	10.00			
I- H	0 / 810	-39.5	-39.5	0.50 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 982 (0.26")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.50/1.00 (J-K:2) , WB=0.81/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM 1903863
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T6	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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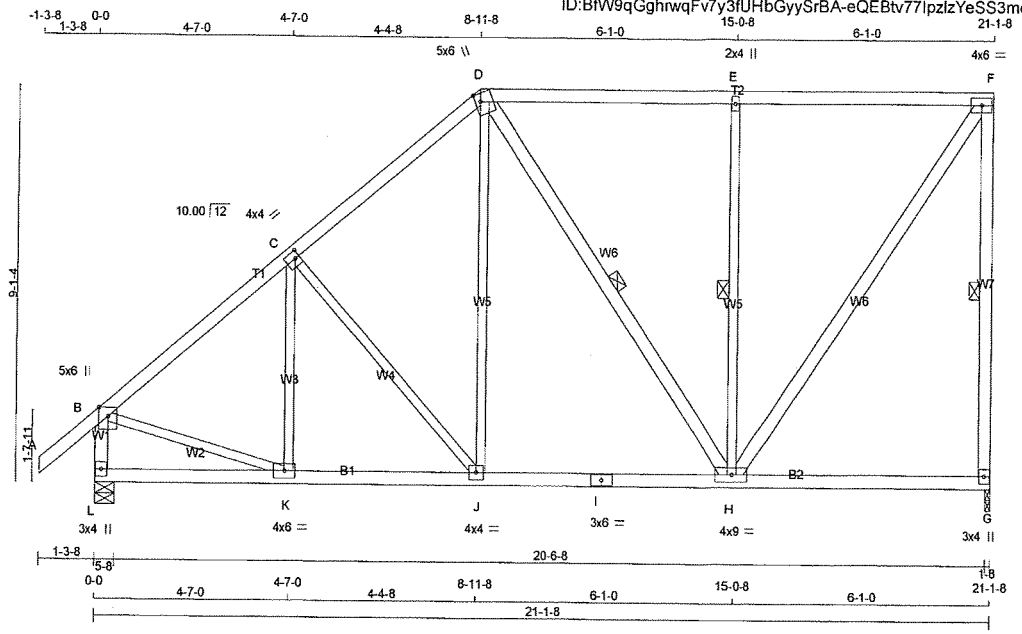


TOTAL WEIGHT = 4 X 115 = 459 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

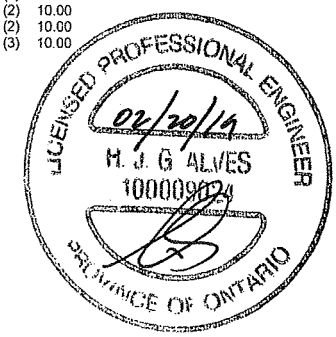
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-68 / 103	0.03 (1)
B- C	-1168 / 0	-77.7	-77.7	0.30 (1)	5.55	C- J	-281 / 0	0.24 (1)
C- D	-982 / 0	-77.7	-77.7	0.28 (1)	5.93	J- D	0 / 433	0.10 (2)
D- E	-668 / 0	-77.7	-77.7	0.51 (1)	6.25	D- H	-121 / 0	0.07 (1)
E- F	-668 / 0	-77.7	-77.7	0.51 (1)	6.25	H- E	-585 / 0	0.32 (1)
G- F	-1142 / 0	0.0	0.0	0.44 (1)	5.95	H- F	0 / 1171	0.19 (1)
L- B	-1271 / 0	0.0	0.0	0.14 (1)	7.14	B- K	0 / 953	0.21 (1)
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00			
K- J	0 / 917	-39.5	-39.5	0.27 (2)	10.00			
J- I	0 / 737	-39.5	-39.5	0.38 (2)	10.00			
I- H	0 / 737	-39.5	-39.5	0.38 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.38/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM T1903864
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T7	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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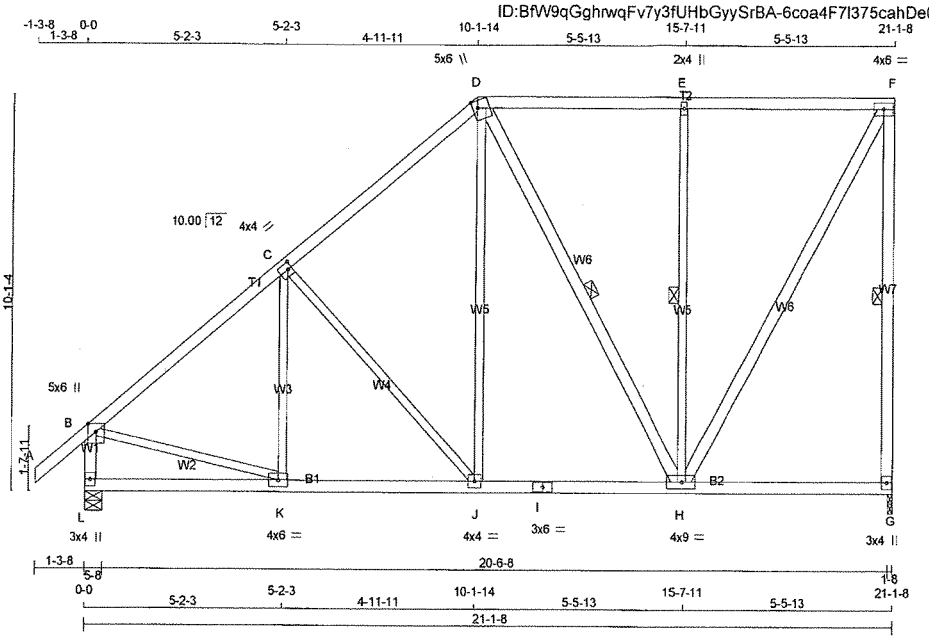


TOTAL WEIGHT = 6 X 121 = 724 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1238	0	1238	0	0	1-8	1-8
L	1346	0	1346	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	946	442 / 0	222 / 0	0 / 0	0 / 0	283 / 0	0 / 0
L	1021	499 / 0	222 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

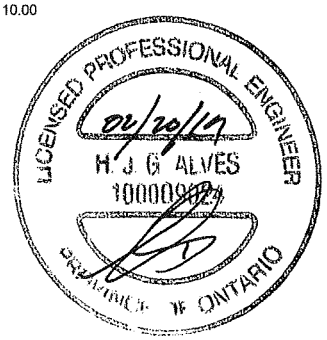
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-4 / 163	0.04 (3)
B- C	-1170 / 0	-77.7	-77.7	0.39 (1)	5.41	C- J	-375 / 0	0.43 (1)
C- D	-905 / 0	-77.7	-77.7	0.37 (1)	5.98	J- D	0 / 491	0.11 (2)
D- E	-556 / 0	-77.7	-77.7	0.40 (1)	6.25	D- H	-246 / 0	0.17 (1)
E- F	-556 / 0	-77.7	-77.7	0.40 (1)	6.25	H- E	-527 / 0	0.37 (1)
G- F	-1151 / 0	0.0	0.0	0.57 (1)	5.93	H- F	0 / 1133	0.18 (1)
L- B	-1263 / 0	0.0	0.0	0.13 (1)	7.16	B- K	0 / 951	0.21 (1)
L- K	0 / 0	-39.5	-39.5	0.19 (3)	10.00			
K- J	0 / 922	-39.5	-39.5	0.31 (2)	10.00			
J- I	0 / 675	-39.5	-39.5	0.31 (2)	10.00			
I- H	0 / 675	-39.5	-39.5	0.31 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.23 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.57/1.00 (F-G:1) , BC=0.31/1.00 (J-K:2) , WB=0.43/1.00 (C-J:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

DWG NO. TAM T7903865
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T10	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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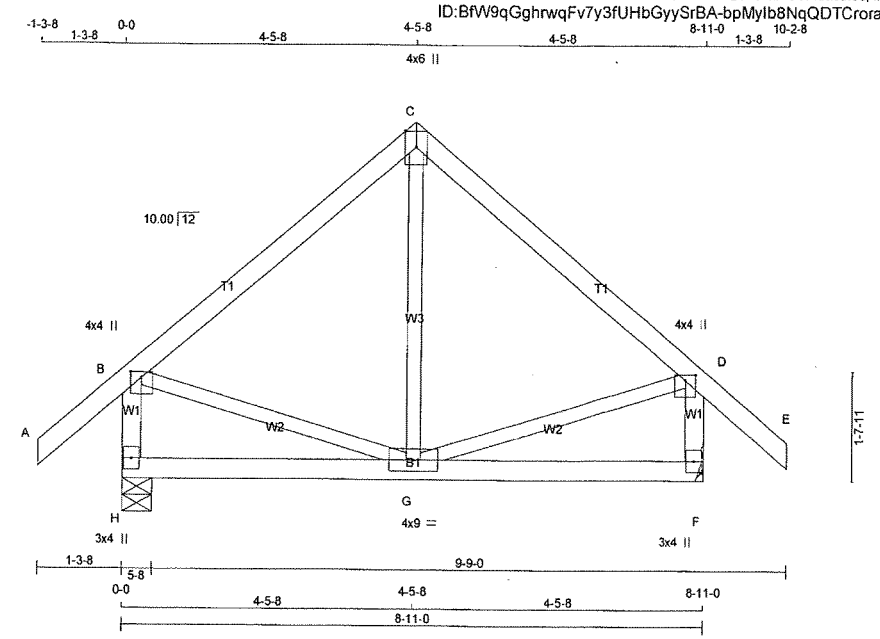

TOTAL WEIGHT = 10 X 41 = 413 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	630	0	630	0	0	5-8	5-8
F	630	0	630	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0
F	474	244 / 0	94 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 188	0.05 (3)
B- C	-326 / 0	-77.7	-77.7	0.20 (1)	6.25	B- G	0 / 261	0.06 (1)
C- D	-326 / 0	-77.7	-77.7	0.20 (1)	6.25	G- D	0 / 261	0.06 (1)
D- E	0 / 34	-77.7	-77.7	0.11 (1)	10.00			
H- B	-563 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-563 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-39.5	-39.5	0.18 (3)	10.00			
G- F	0 / 0	-39.5	-39.5	0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.18/1.00 (F-G:3) , WB=0.06/1.00 (B-G:1) , SSI=0.12/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

DWG NO. TAM T1903968
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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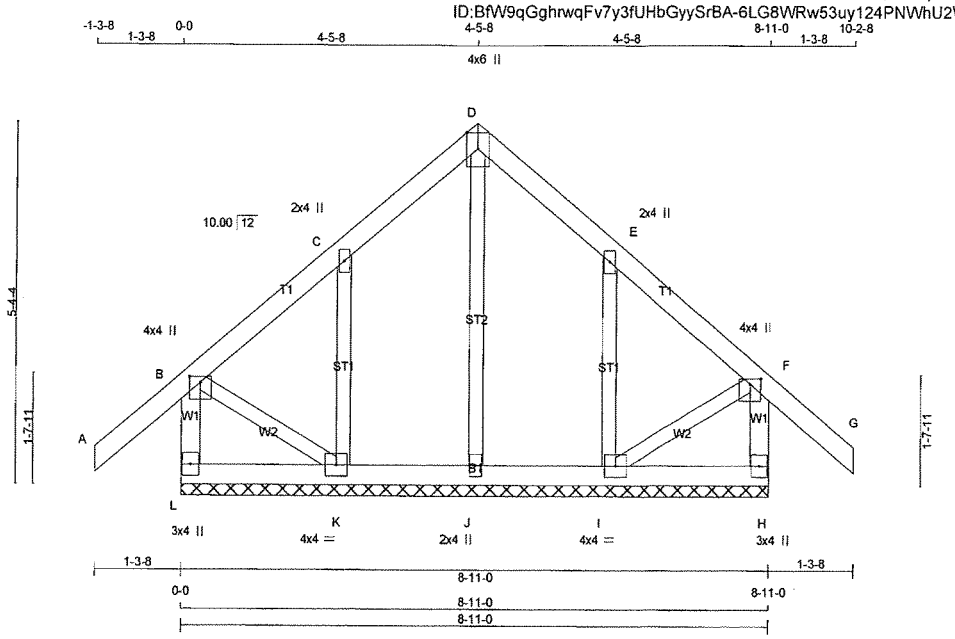


TOTAL WEIGHT = 2 X 43 = 87 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

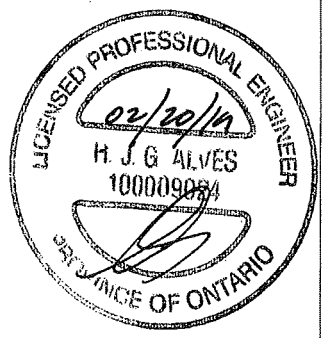
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-201 / 0	0.0	0.0		0.02 (1)	7.81	J- D	-101 / 0	0.04 (1)
A- B	0 / 34	-77.7	-77.7		0.11 (1)	10.00	K- C	-211 / 0	0.05 (1)
B- C	-8 / 0	-77.7	-77.7		0.06 (1)	10.00	I- E	-211 / 0	0.05 (1)
C- D	-28 / 0	-77.7	-77.7		0.06 (1)	6.25	B- K	0 / 18	0.00 (1)
D- E	-28 / 0	-77.7	-77.7		0.06 (1)	6.25	I- F	0 / 18	0.00 (1)
E- F	-8 / 0	-77.7	-77.7		0.06 (1)	10.00			
F- G	0 / 34	-77.7	-77.7		0.11 (1)	10.00			
H- F	-201 / 0	0.0	0.0		0.02 (1)	7.81			
L- K	0 / 0	-39.5	-39.5		0.04 (3)	10.00			
K- J	0 / 10	-39.5	-39.5		0.04 (3)	10.00			
J- I	0 / 10	-39.5	-39.5		0.04 (3)	10.00			
I- H	0 / 0	-39.5	-39.5		0.04 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (F-G:1) , BC=0.04/1.00 (J-K:3) , WB=0.05/1.00 (C-K:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.11 (C) (INPUT = 1.00)

DWG NO. TAM 1903867
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T14	1	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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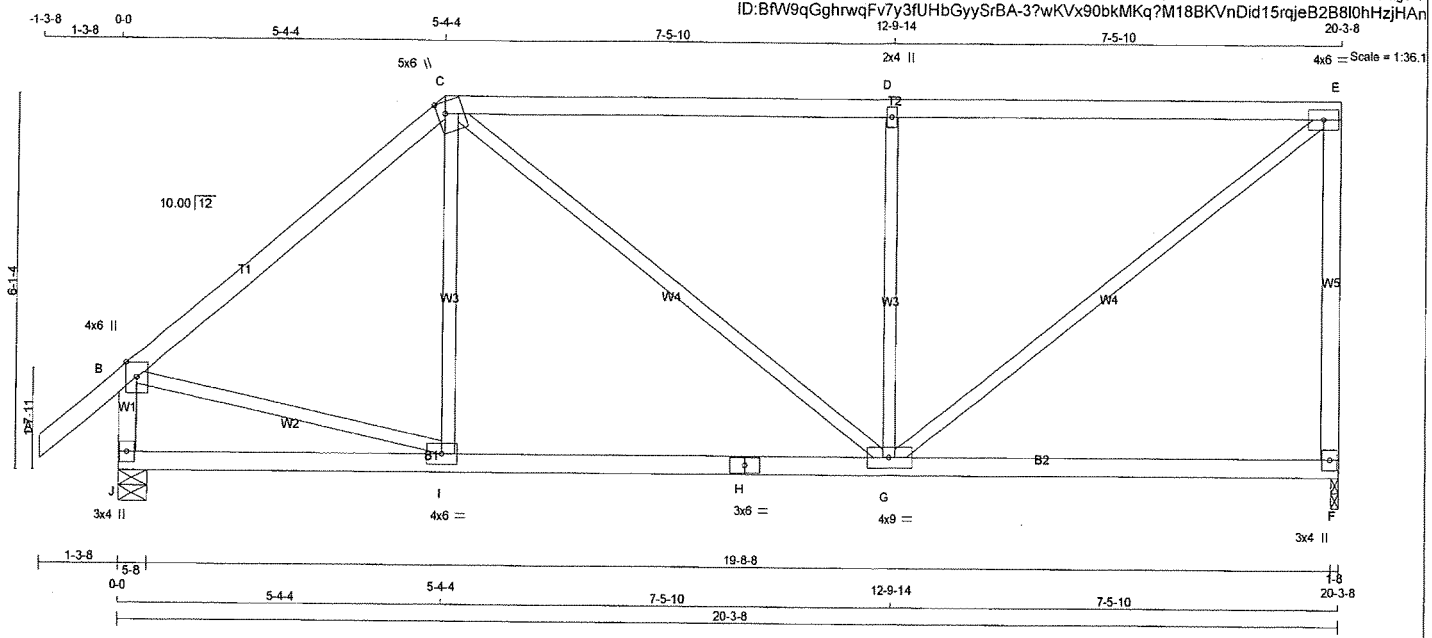

TOTAL WEIGHT = 86 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1189	0	1189	0	0	1-8	1-8
J	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 224	0.05 (3)
B- C	-1090 / 0	-77.7	-77.7	0.45 (1)	5.47	C- G	0 / 330	0.07 (1)
C- D	-1097 / 0	-77.7	-77.7	0.84 (1)	4.54	G- D	-719 / 0	0.42 (1)
D- E	-1097 / 0	-77.7	-77.7	0.84 (1)	4.54	G- E	0 / 1387	0.31 (1)
F- E	-1071 / 0	0.0	0.0	0.77 (1)	7.62	B- I	0 / 859	0.19 (1)
J- B	-1215 / 0	0.0	0.0	0.13 (1)	7.26			
J- I	0 / 0	-39.5	-39.5	0.27 (3)	10.00			
I- H	0 / 837	-39.5	-39.5	0.52 (2)	10.00			
H- G	0 / 837	-39.5	-39.5	0.52 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.42 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.52/1.00 (G-I:2) , WB=0.42/1.00 (D-G:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. TAM T1703868
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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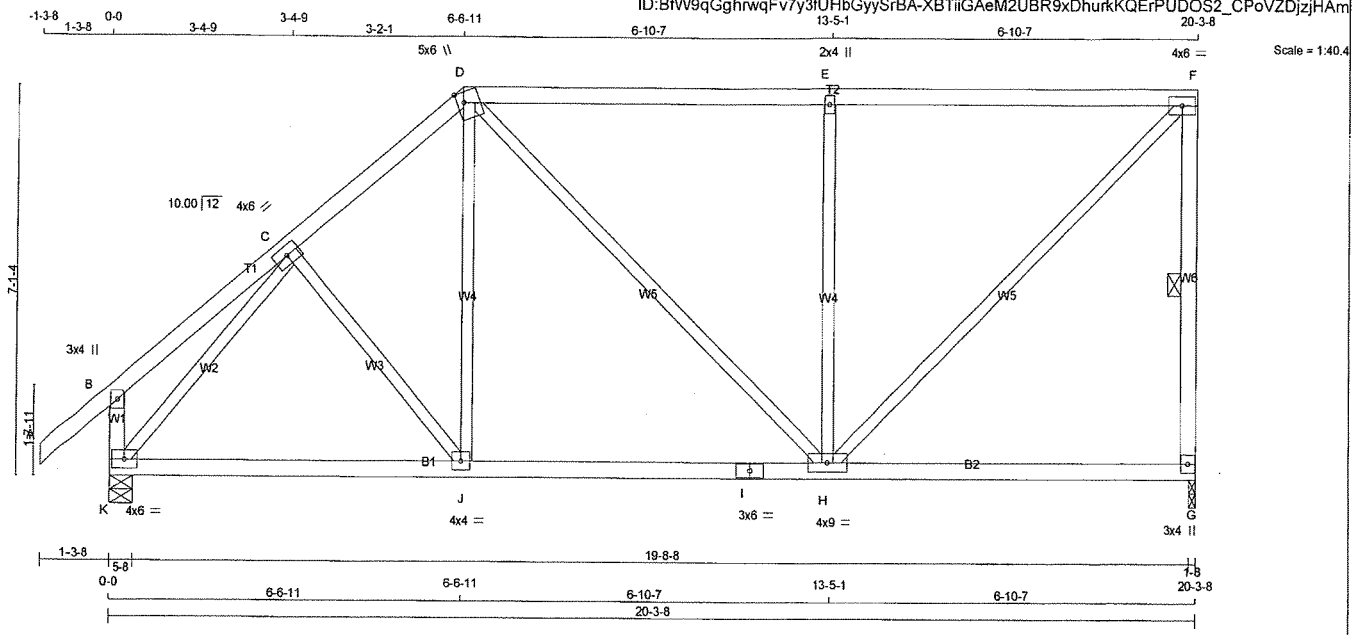


TOTAL WEIGHT = 93 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

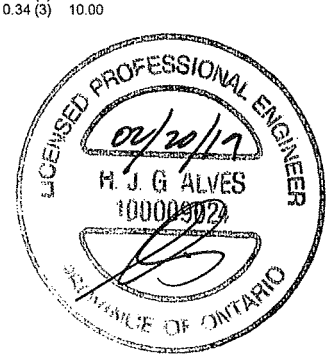
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 17	-77.7	-77.7	0.12 (1)	10.00
C- D	-1062 / 0	-77.7	-77.7	0.14 (1)	5.96
D- E	-887 / 0	-77.7	-77.7	0.67 (1)	5.36
E- F	-887 / 0	-77.7	-77.7	0.68 (1)	5.36
G- F	-1078 / 0	0.0	0.0	0.25 (1)	6.08
K- B	-207 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 797	-39.5	-39.5	0.41 (2)	10.00
J- I	0 / 804	-39.5	-39.5	0.43 (2)	10.00
I- H	0 / 804	-39.5	-39.5	0.43 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	CSI (LC)
C- J	0 / 81	0.02 (3)
J- D	0 / 332	0.07 (2)
D- H	0 / 117	0.03 (1)
H- E	-661 / 0	0.58 (1)
H- F	0 / 1246	0.28 (1)
K- C	-1266 / 0	0.54 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.43/1.00 (H-J:2) , WB=0.58/1.00 (E-H:1) , SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

DWG NO. TAM T1903869
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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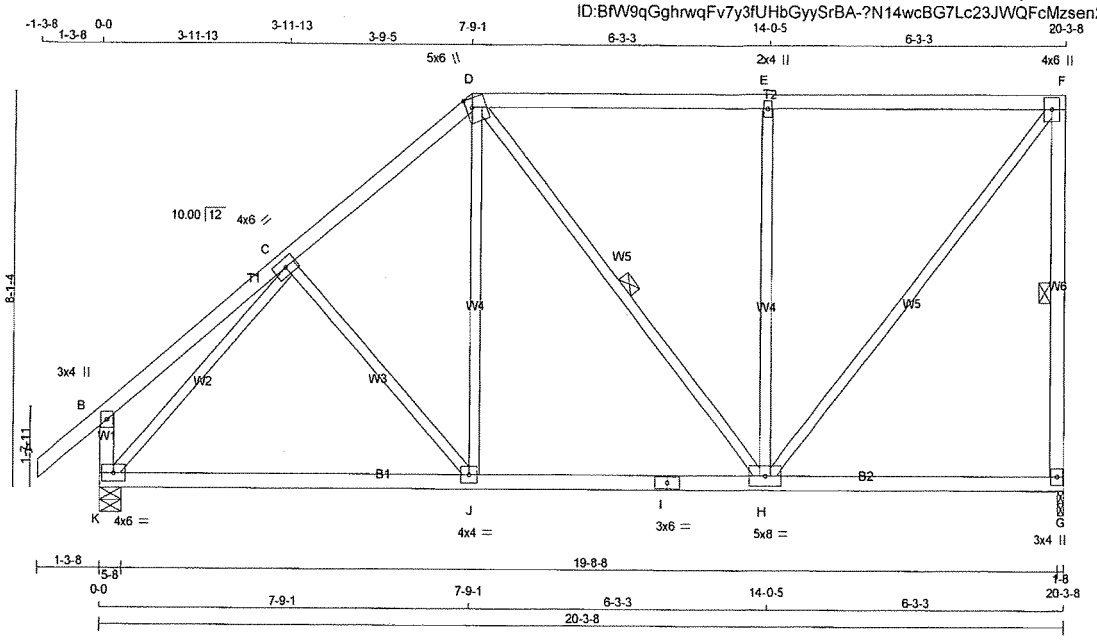

TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	C- J	-77 / 51	0.05 (1)
B- C	0 / 22	-77.7	-77.7	0.18 (1)	10.00	J- D	0 / 420	0.09 (2)
C- D	-1008 / 0	-77.7	-77.7	0.20 (1)	5.98	D- H	-74 / 0	0.05 (3)
D- E	-725 / 0	-77.7	-77.7	0.54 (1)	6.09	H- E	-603 / 0	0.76 (1)
E- F	-726 / 0	-77.7	-77.7	0.54 (1)	6.09	H- F	0 / 1155	0.26 (1)
G- F	-1085 / 0	0.0	0.0	0.32 (1)	6.07	K- C	-1243 / 0	0.74 (1)
K- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
K- J	0 / 807	-39.5	-39.5	0.49 (2)	10.00			
J- I	0 / 761	-39.5	-39.5	0.49 (2)	10.00			
I- H	0 / 761	-39.5	-39.5	0.49 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 939 (0.26")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.49/1.00 (J-K:2) , WB=0.76/1.00 (E-H:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.33 (F) (INPUT = 1.00)

DWG NO. TAM T1903870
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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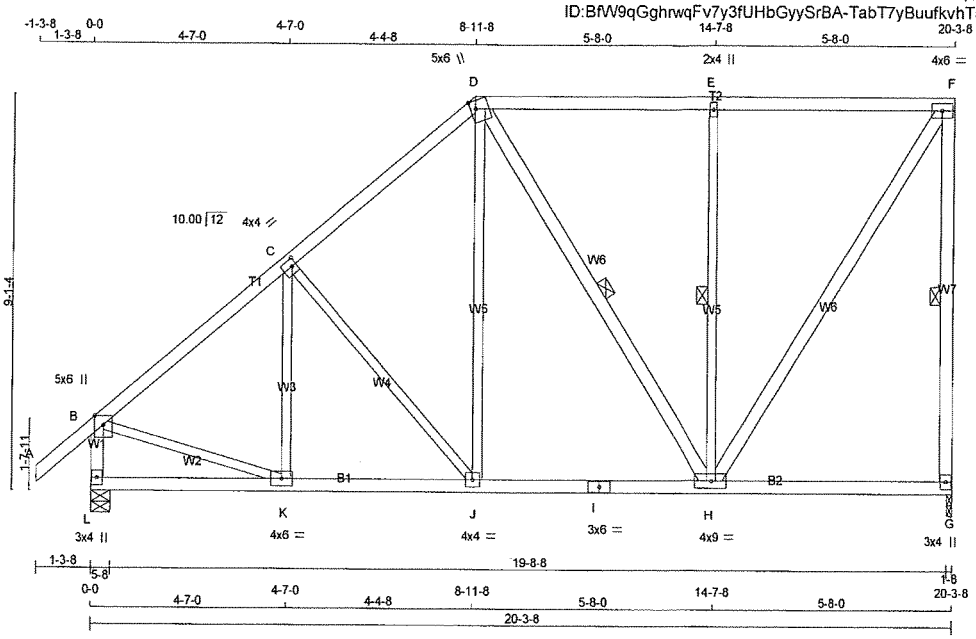


TOTAL WEIGHT = 112 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1189	0	1189	0	0	1-8	1-8
L	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
L	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

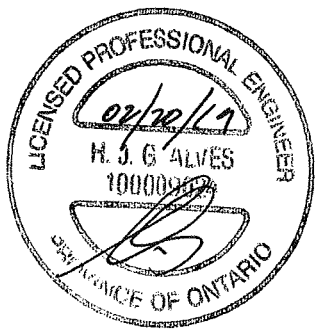
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-53 / 114	0.03 (3)
B- C	-1111 / 0	-77.7	-77.7	0.29 (1)	5.65	C- J	-293 / 0	0.25 (1)
C- D	-916 / 0	-77.7	-77.7	0.28 (1)	6.09	J- D	0 / 430	0.10 (2)
D- E	-603 / 0	-77.7	-77.7	0.43 (1)	6.25	D- H	-154 / 0	0.09 (1)
E- F	-603 / 0	-77.7	-77.7	0.43 (1)	6.25	H- E	-545 / 0	0.29 (1)
G- F	-1099 / 0	0.0	0.0	0.42 (1)	6.04	H- F	0 / 1109	0.18 (1)
L- B	-1222 / 0	0.0	0.0	0.13 (1)	7.25	B- K	0 / 908	0.20 (1)
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00			
K- J	0 / 874	-39.5	-39.5	0.26 (2)	10.00			
J- I	0 / 686	-39.5	-39.5	0.33 (2)	10.00			
I- H	0 / 686	-39.5	-39.5	0.33 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.25 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.33/1.00 (H-J:2) , WB=0.29/1.00 (E-H:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

DWG NO. TAM T1903871
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T33	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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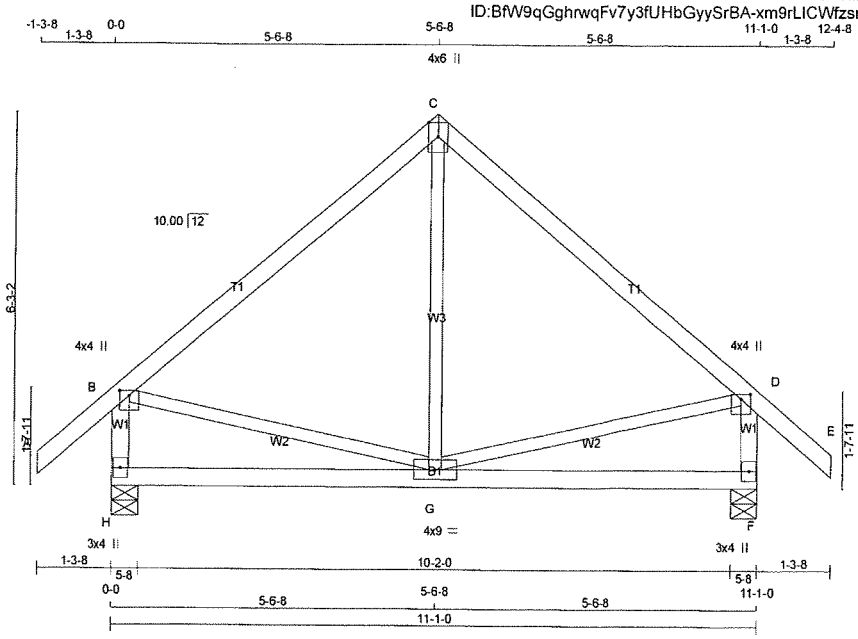

TOTAL WEIGHT = 4 X 49 = 198 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	757	0	757	0	0	5-8	5-8
F	757	0	757	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	571	290 / 0	116 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34		-77.7	-77.7	0.11 (1)	10.00	G- C	0 / 251	0.06 (3)
B- C	-428 / 0		-77.7	-77.7	0.31 (1)	6.25	B- G	0 / 337	0.08 (1)
C- D	-428 / 0		-77.7	-77.7	0.31 (1)	6.25	G- D	0 / 337	0.08 (1)
D- E	0 / 34		-77.7	-77.7	0.11 (1)	10.00			
H- B	-674 / 0		0.0	0.0	0.07 (1)	7.81			
F- D	-674 / 0		0.0	0.0	0.07 (1)	7.81			
H- G	0 / 0		-39.5	-39.5	0.28 (3)	10.00			
G- F	0 / 0		-39.5	-39.5	0.28 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.28/1.00 (G-H:3) , WB=0.08/1.00 (D-G:1) , SSI=0.15/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

DWG NO. TAM T1703872
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T33A	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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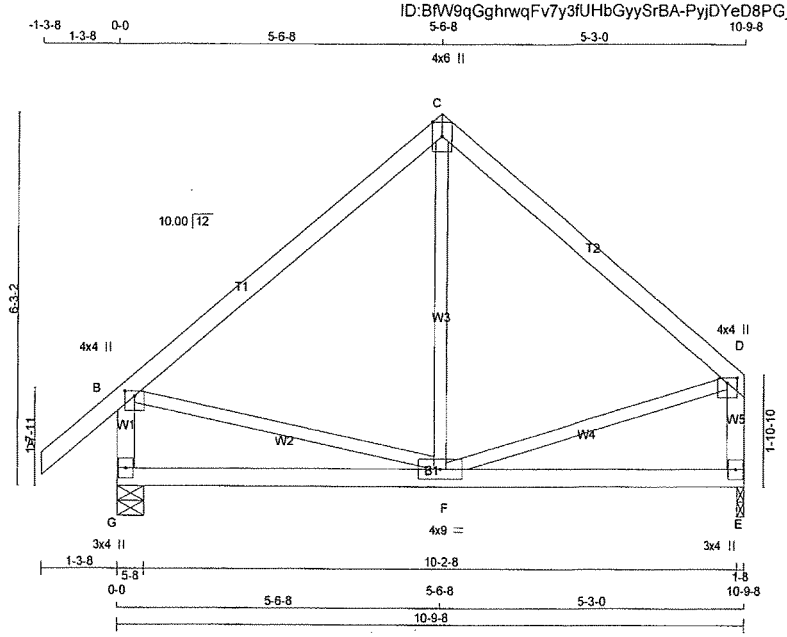


TOTAL WEIGHT = 8 X 47 = 375 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	740	0	740	0	0	5-8	5-8
E	632	0	632	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	558	283 / 0	113 / 0	0 / 0	0 / 0	161 / 0	0 / 0
E	483	226 / 0	113 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING

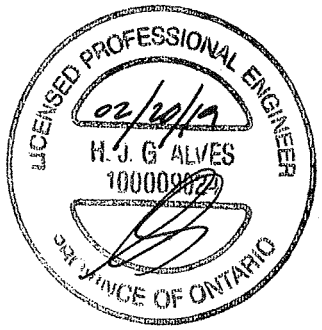
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	F- C	0 / 235	0.06 (3)
B- C	-403 / 0	-77.7	-77.7	0.31 (1)	6.25	B- F	0 / 317	0.07 (1)
C- D	-407 / 0	-77.7	-77.7	0.28 (1)	6.25	F- D	0 / 327	0.07 (1)
G- B	-651 / 0	0.0	0.0	0.07 (1)	7.81			
E- D	-560 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-39.5	-39.5	0.26 (3)	10.00			
F- E	0 / 0	-39.5	-39.5	0.26 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.26/1.00 (E-F:3) , WB=0.07/1.00 (D-F:1) , SSI=0.15/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM 17703874
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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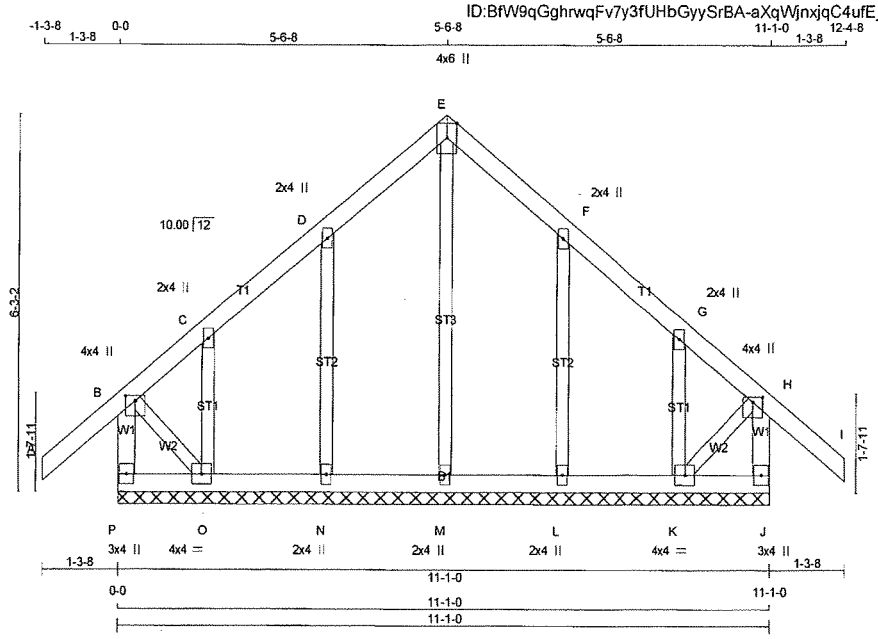


TOTAL WEIGHT = 2 X 54 = 108 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

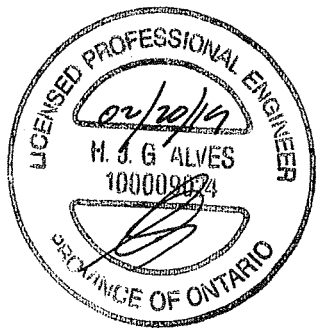
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
P- B	-233 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-47 / 0	-77.7	-77.7	0.10 (1)	6.25
C- D	-4 / 0	-77.7	-77.7	0.05 (1)	10.00
D- E	-20 / 0	-77.7	-77.7	0.05 (1)	6.25
E- F	-20 / 0	-77.7	-77.7	0.05 (1)	6.25
F- G	-4 / 0	-77.7	-77.7	0.05 (1)	10.00
G- H	-47 / 0	-77.7	-77.7	0.10 (1)	6.25
H- I	0 / 34	-77.7	-77.7	0.11 (1)	10.00
J- H	-233 / 0	0.0	0.0	0.02 (1)	7.81
P- O	0 / 0	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 11	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 6	-39.5	-39.5	0.03 (3)	10.00
M- L	0 / 6	-39.5	-39.5	0.03 (3)	10.00
L- K	0 / 11	-39.5	-39.5	0.03 (3)	10.00
K- J	0 / 0	-39.5	-39.5	0.02 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
M- E	-117 / 0	0.07 (1)
N- D	-188 / 0	0.06 (1)
O- C	-70 / 0	0.01 (1)
L- F	-188 / 0	0.06 (1)
K- G	-70 / 0	0.01 (1)
B- O	0 / 17	0.00 (1)
K- H	0 / 17	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT. CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (K-L:3) , WB=0.07/1.00 (E-M:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

DWG NO. TAM 7903873
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G59	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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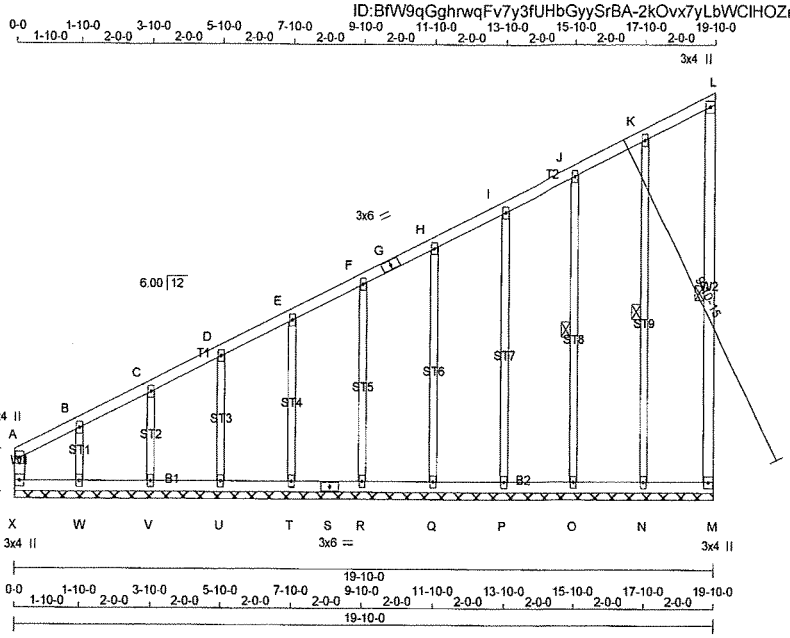


TOTAL WEIGHT = 3 X 101 = 304 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
X - S	2x4	DRY	No.2	SPF
S - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B, C, D, E, F, H, I, J, K						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, R, T, U, V, W						
N	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

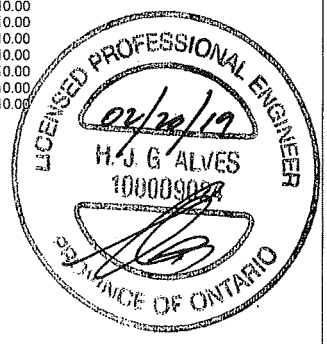
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-22 / 0	-77.7	-77.7	0.03 (1)	6.25	N- K	-167 / 0	0.12 (1)
B- C	-18 / 0	-77.7	-77.7	0.04 (1)	6.25	O- J	-154 / 0	0.08 (1)
C- D	-15 / 0	-77.7	-77.7	0.04 (1)	6.25	P- I	-155 / 0	0.20 (1)
D- E	-11 / 0	-77.7	-77.7	0.04 (1)	6.25	Q- H	-155 / 0	0.14 (1)
E- F	-9 / 0	-77.7	-77.7	0.04 (1)	10.00	R- F	-155 / 0	0.09 (1)
F- G	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	T- E	-154 / 0	0.06 (1)
G- H	-7 / 0	-77.7	-77.7	0.04 (1)	10.00	U- D	-154 / 0	0.04 (1)
H- I	-5 / 0	-77.7	-77.7	0.04 (1)	10.00	V- C	-154 / 0	0.03 (1)
I- J	-4 / 0	-77.7	-77.7	0.04 (1)	10.00	W- B	-150 / 0	0.02 (1)
J- K	-2 / 0	-77.7	-77.7	0.04 (1)	10.00			
K- L	-6 / 0	-77.7	-77.7	0.04 (1)	10.00			
M- L	-70 / 0	0.0	0.0	0.01 (1)	6.25			
X- A	-73 / 0	0.0	0.0	0.02 (1)	7.81			
X- W	0 / 22	-39.5	-39.5	0.03 (3)	10.00			
W- V	0 / 17	-39.5	-39.5	0.03 (3)	10.00			
V- U	0 / 13	-39.5	-39.5	0.02 (3)	10.00			
U- T	0 / 10	-39.5	-39.5	0.02 (3)	10.00			
T- S	0 / 8	-39.5	-39.5	0.02 (3)	10.00			
S- R	0 / 8	-39.5	-39.5	0.02 (3)	10.00			
R- Q	0 / 6	-39.5	-39.5	0.02 (3)	10.00			
Q- P	0 / 5	-39.5	-39.5	0.02 (3)	10.00			
P- O	0 / 4	-39.5	-39.5	0.02 (3)	10.00			
O- N	0 / 2	-39.5	-39.5	0.03 (3)	10.00			
N- M	0 / 1	-39.5	-39.5	0.03 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (K-L:1) , BC=0.03/1.00 (N-O:3) , WB=0.20/1.00 (I-P:1) , SSI=0.06/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (J) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)

DWG NO. TAM T7703875
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G59A	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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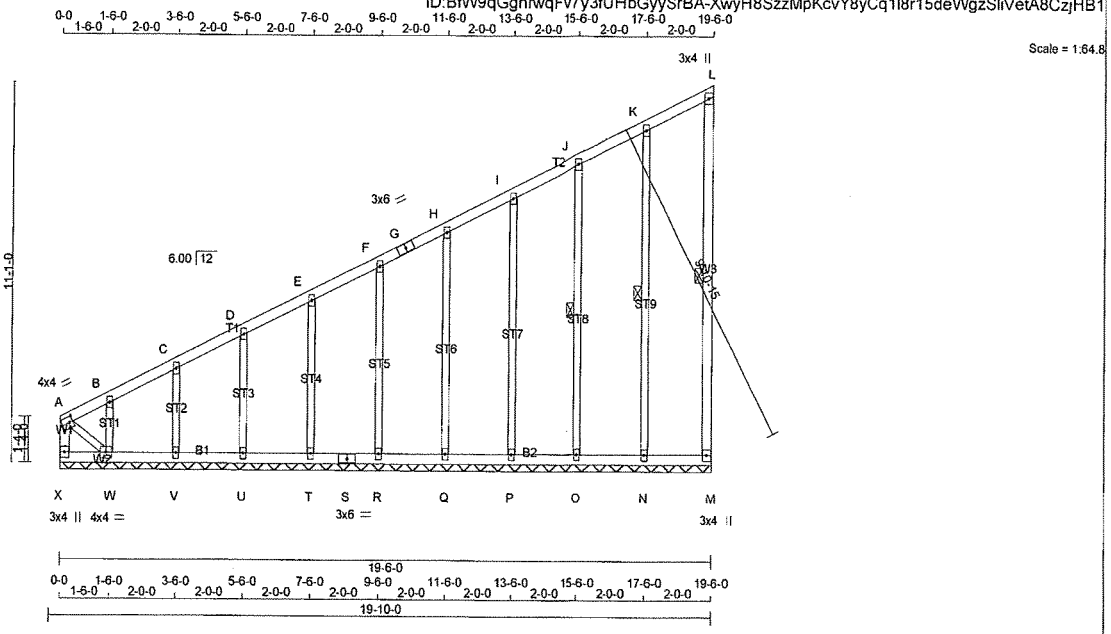


TOTAL WEIGHT = 3 X 102 = 305 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
X - S	2x4	DRY	No.2	SPF
S - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, F, H, I, J, K						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, R, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
W	BMWW1-t	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-16 / 0	-77.7	-77.7	0.03 (1)	6.25
B- C	-16 / 0	-77.7	-77.7	0.04 (1)	6.25
C- D	-13 / 0	-77.7	-77.7	0.04 (1)	6.25
D- E	-10 / 0	-77.7	-77.7	0.04 (1)	10.00
E- F	-8 / 0	-77.7	-77.7	0.04 (1)	10.00
F- G	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
G- H	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
H- I	-4 / 0	-77.7	-77.7	0.04 (1)	10.00
I- J	-3 / 0	-77.7	-77.7	0.04 (1)	10.00
J- K	0 / 0	-77.7	-77.7	0.04 (1)	10.00
K- L	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
M- L	-66 / 0	0.0	0.0	0.04 (1)	6.25
X- A	-64 / 0	0.0	0.0	0.01 (1)	7.81
X- W	0 / 0	-39.5	-39.5	0.02 (3)	10.00
W- V	0 / 15	-39.5	-39.5	0.02 (2)	10.00
V- U	0 / 12	-39.5	-39.5	0.02 (3)	10.00
U- T	0 / 9	-39.5	-39.5	0.02 (3)	10.00
T- S	0 / 7	-39.5	-39.5	0.02 (3)	10.00
S- R	0 / 7	-39.5	-39.5	0.02 (3)	10.00
R- Q	0 / 5	-39.5	-39.5	0.02 (3)	10.00
Q- P	0 / 4	-39.5	-39.5	0.02 (3)	10.00
P- O	0 / 2	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 1	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 0	-39.5	-39.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- K	-173 / 0	0.12 (1)
O- J	-153 / 0	0.08 (1)
P- I	-155 / 0	0.20 (1)
Q- H	-155 / 0	0.14 (1)
R- F	-155 / 0	0.09 (1)
T- E	-154 / 0	0.06 (1)
U- D	-154 / 0	0.04 (1)
V- C	-156 / 0	0.03 (1)
W- B	-145 / 0	0.02 (1)
A- W	0 / 25	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (L-M:1) , BC=0.03/1.00 (M-N:3) , WB=0.20/1.00 (I-P:1) , SSI=0.07/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

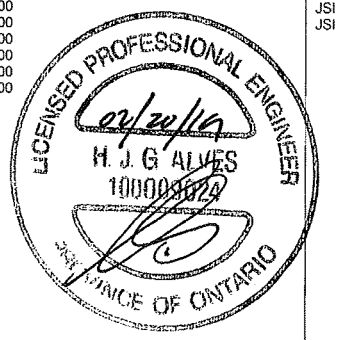
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)



DWG NO. TAM 17903876
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G60	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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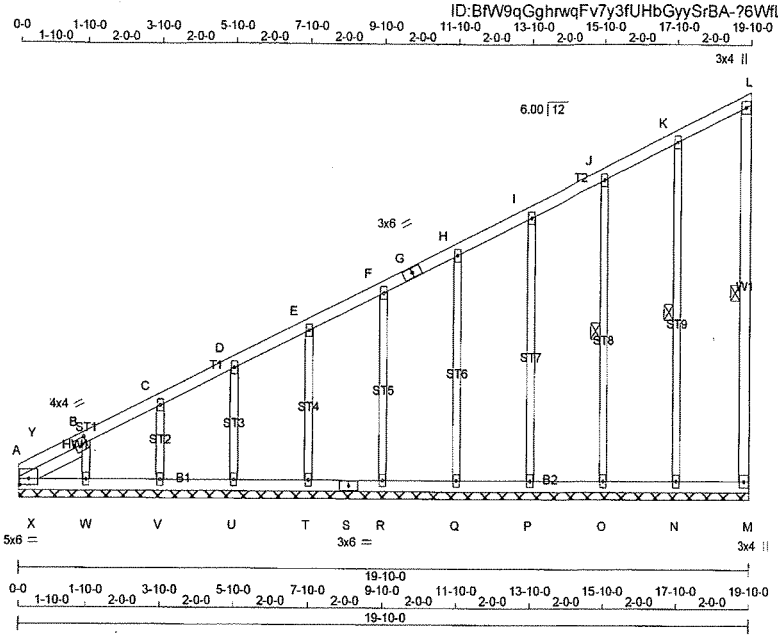


TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
S - M	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-l	MT20	5.0	6.0	2.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, R, T, U, V, W						
N	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, K-N, J-O.

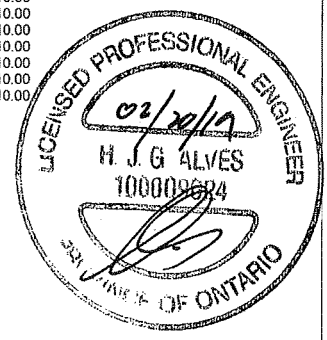
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- Y	-78 / 0	-77.7	-77.7	0.00 (1)	6.25
Y- B	-48 / 0	-77.7	-77.7	0.03 (1)	6.25
B- C	-18 / 0	-77.7	-77.7	0.03 (1)	6.25
C- D	-14 / 0	-77.7	-77.7	0.04 (1)	6.25
D- E	-11 / 0	-77.7	-77.7	0.04 (1)	6.25
E- F	-8 / 0	-77.7	-77.7	0.04 (1)	10.00
F- G	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
G- H	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
H- I	-4 / 0	-77.7	-77.7	0.04 (1)	10.00
I- J	-3 / 0	-77.7	-77.7	0.04 (1)	10.00
J- K	0 / 0	-77.7	-77.7	0.04 (1)	10.00
K- L	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
M- L	-66 / 0	0.0	0.0	0.04 (1)	6.25
A- X	0 / 49	-39.5	-39.5	0.02 (1)	10.00
X- W	0 / 17	-39.5	-39.5	0.02 (2)	10.00
W- V	0 / 17	-39.5	-39.5	0.02 (3)	10.00
V- U	0 / 13	-39.5	-39.5	0.02 (3)	10.00
U- T	0 / 10	-39.5	-39.5	0.02 (3)	10.00
T- S	0 / 7	-39.5	-39.5	0.02 (3)	10.00
S- R	0 / 7	-39.5	-39.5	0.02 (3)	10.00
R- Q	0 / 5	-39.5	-39.5	0.02 (3)	10.00
Q- P	0 / 4	-39.5	-39.5	0.02 (3)	10.00
P- O	0 / 2	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 1	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 0	-39.5	-39.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- K	-173 / 0	0.11 (1)
O- J	-153 / 0	0.07 (1)
P- I	-155 / 0	0.16 (1)
Q- H	-155 / 0	0.11 (1)
R- F	-154 / 0	0.07 (1)
T- E	-154 / 0	0.05 (1)
U- D	-154 / 0	0.03 (1)
V- C	-153 / 0	0.02 (1)
W- B	-155 / 0	0.02 (1)
X- Y	-20 / 15	0.00 (1)
X- B	0 / 42	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (K-L:1) , BC=0.03/1.00 (N-O:3) , WB=0.16/1.00 (I-P:1) , SSI=0.07/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (B) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)

DWG NO. TAM 17903877
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G60A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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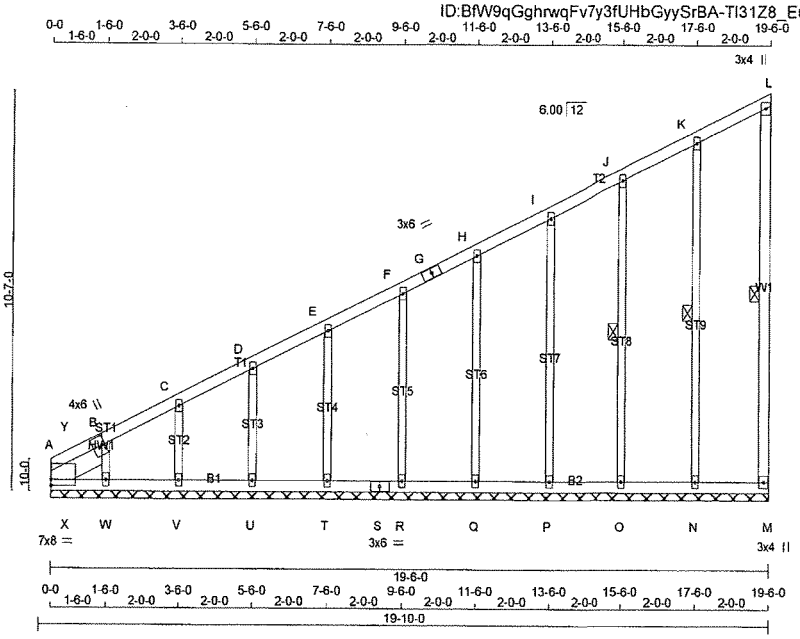


TOTAL WEIGHT = 98 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
S - M	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMW1-I	MT20	7.0	8.0	2.00	
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, R, T, U, V, W						
N	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, J-O.

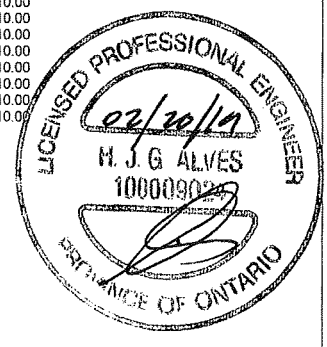
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- Y	-49 / 0	-77.7	-77.7	0.00 (1)	6.25
Y- B	-21 / 0	-77.7	-77.7	0.03 (1)	6.25
B- C	-18 / 0	-77.7	-77.7	0.04 (1)	6.25
C- D	-14 / 0	-77.7	-77.7	0.04 (1)	6.25
D- E	-11 / 0	-77.7	-77.7	0.04 (1)	6.25
E- F	-8 / 0	-77.7	-77.7	0.04 (1)	10.00
F- G	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
G- H	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
H- I	-4 / 0	-77.7	-77.7	0.04 (1)	10.00
I- J	-3 / 0	-77.7	-77.7	0.04 (1)	10.00
J- K	0 / 0	-77.7	-77.7	0.04 (1)	10.00
K- L	-6 / 0	-77.7	-77.7	0.04 (1)	10.00
M- L	-66 / 0	0.0	0.0	0.04 (1)	6.25
A- X	0 / 25	-39.5	-39.5	0.01 (1)	10.00
X- W	0 / 17	-39.5	-39.5	0.02 (2)	10.00
W- V	0 / 17	-39.5	-39.5	0.03 (3)	10.00
V- U	0 / 13	-39.5	-39.5	0.03 (3)	10.00
U- T	0 / 10	-39.5	-39.5	0.02 (3)	10.00
T- S	0 / 7	-39.5	-39.5	0.02 (3)	10.00
S- R	0 / 7	-39.5	-39.5	0.02 (3)	10.00
R- Q	0 / 5	-39.5	-39.5	0.02 (3)	10.00
Q- P	0 / 4	-39.5	-39.5	0.02 (3)	10.00
P- O	0 / 2	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 1	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 0	-39.5	-39.5	0.03 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- K	-173 / 0	0.11 (1)
O- J	-153 / 0	0.07 (1)
P- I	-155 / 0	0.16 (1)
Q- H	-155 / 0	0.11 (1)
R- F	-154 / 0	0.07 (1)
T- E	-154 / 0	0.05 (1)
U- D	-154 / 0	0.03 (1)
V- C	-155 / 0	0.02 (1)
W- B	-136 / 0	0.02 (1)
X- Y	-11 / 8	0.00 (1)
X- B	0 / 15	0.00 (2)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (K-L:1) , BC=0.03/1.00 (M-N:3) , WB=0.16/1.00 (I-P:1) , SSI=0.07/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (K) (INPUT = 0.90)
JSI METAL= 0.07 (K) (INPUT = 1.00)

DWG NO. TAM 179038 78
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T61A	2	2	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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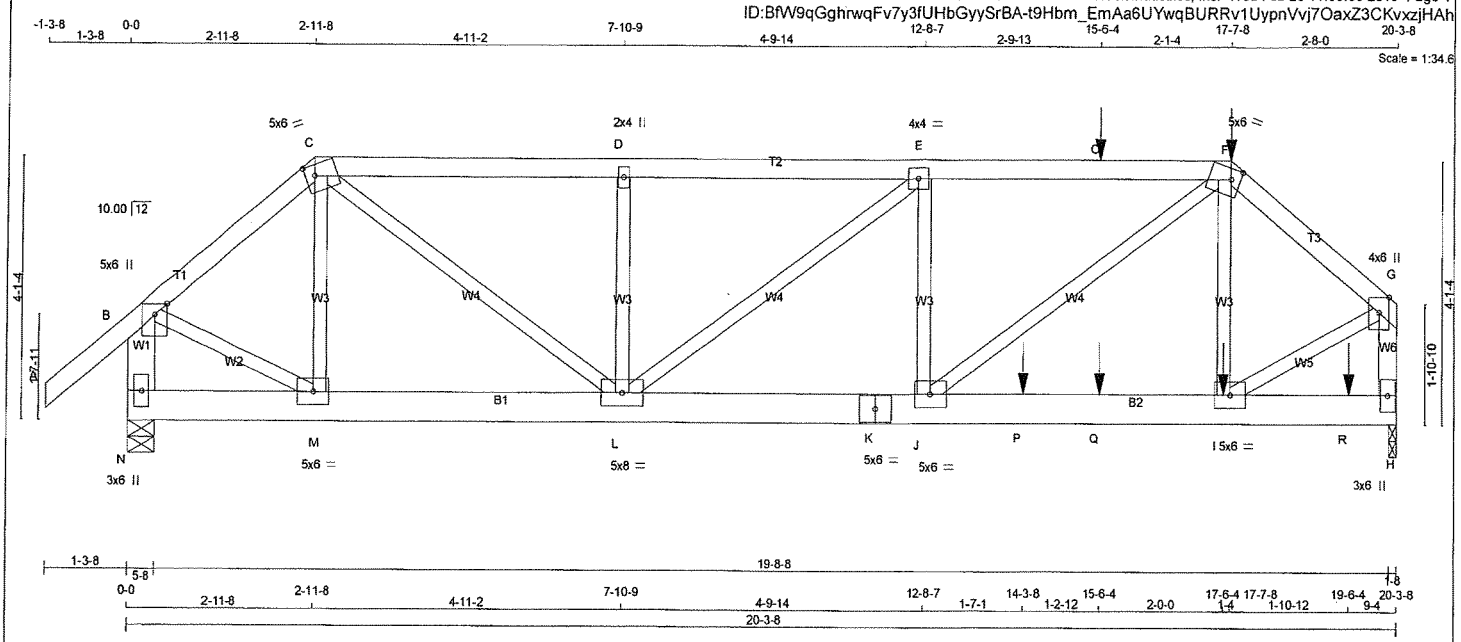


TOTAL WEIGHT = 4 X 97 = 387 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
H- G	1	12	TOP
N- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	TOP
K- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTWW-m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	1.75
G	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I, J, M						
I	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1752	0	1752	0	0	5-8	5-8
H	2565	0	2565	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1325	662 / 0	277 / 0	0 / 0	0 / 0	386 / 0	0 / 0
H	1944	958 / 0	416 / 0	0 / 0	0 / 0	569 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

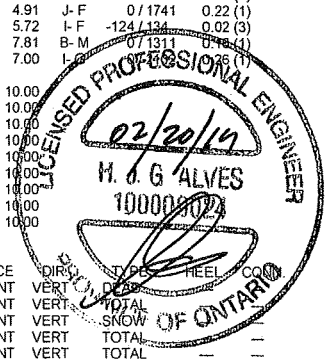
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.06	(1)	10.00	M- C	-243 / 0	0.03 (1)
B- C	-1590 / 0	-77.7	-77.7	0.07	(1)	6.25	C- L	0 / 1632	0.20 (1)
C- D	-2514 / 0	-77.7	-77.7	0.15	(1)	5.55	L- D	-392 / 0	0.05 (1)
D- E	-2514 / 0	-77.7	-77.7	0.18	(1)	5.51	L- E	-925 / 0	0.30 (1)
E- O	-3245 / 0	-77.7	-77.7	0.25	(1)	4.91	J- E	0 / 188	0.02 (3)
O- F	-3245 / 0	-77.7	-77.7	0.25	(1)	4.91	J- F	0 / 1741	0.22 (1)
F- G	-2428 / 0	-77.7	-77.7	0.07	(1)	5.72	I- F	-124 / 134	0.02 (3)
N- B	-1732 / 0	0.0	0.0	0.06	(1)	7.81	B- M	0 / 1311	0.18 (1)
H- G	-2650 / 0	0.0	0.0	0.16	(1)	7.00	I- G	-1026 / 0	0.26 (1)
N- M	0 / 0	-39.5	-39.5	0.05	(2)	10.00			
M- L	0 / 1210	-39.5	-39.5	0.13	(1)	10.00			
L- K	0 / 3245	-39.5	-39.5	0.38	(1)	10.00			
K- J	0 / 3245	-39.5	-39.5	0.38	(1)	10.00			
J- P	0 / 1854	-39.5	-39.5	0.48	(1)	10.00			
P- Q	0 / 1854	-39.5	-39.5	0.48	(1)	10.00			
Q- I	0 / 1854	-39.5	-39.5	0.48	(1)	10.00			
I- R	0 / 0	-39.5	-39.5	0.19	(1)	10.00			
R- H	0 / 0	-39.5	-39.5	0.19	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEEL	COMM.
F	17-7-8	-38	-42	---	FRONT	VERT	DEAD	---	---
F	17-7-8	-100	-100	---	FRONT	VERT	TOTAL	---	---
F	17-7-8	-180	-180	---	FRONT	VERT	SNOW	---	---
I	17-6-4	-56	-71	---	FRONT	VERT	TOTAL	---	---
O	15-6-4	-94	-94	---	FRONT	VERT	TOTAL	---	---
P	14-3-8	-1247	-1247	---	FRONT	VERT	TOTAL	---	---
Q	15-6-4	-56	-71	---	FRONT	VERT	TOTAL	---	---
R	19-6-4	-61	-77	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM T1703879
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.25/1.00 (E-F:1) , BC=0.48/1.00 (I-J:1) , WB=0.30/1.00 (E-L:1) , SSI=0.31/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.52 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T62A	2	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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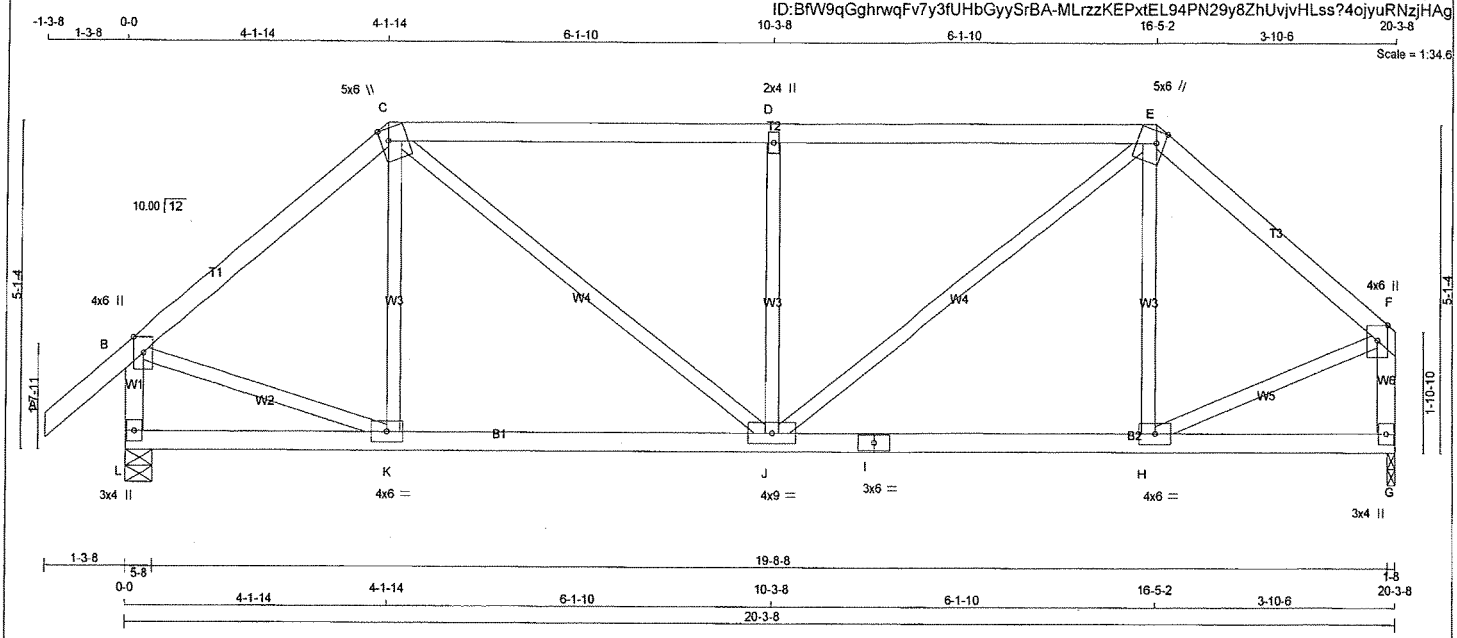


TOTAL WEIGHT = 2 X 84 = 168 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1297	0	1297	0	0	5-8	5-8
G	1189	0	1189	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

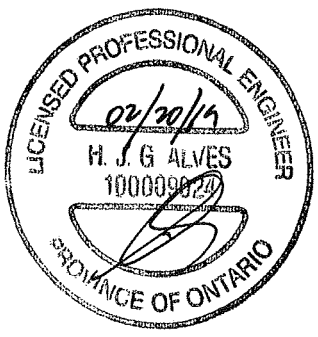
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	K- C	-33 / 136	0.03 (3)
B- C	-1103 / 0	-77.7	-77.7	0.26 (1)	5.71	C- J	0 / 641	0.14 (1)
C- D	-1350 / 0	-77.7	-77.7	0.56 (1)	4.82	J- D	-581 / 0	0.22 (1)
D- E	-1350 / 0	-77.7	-77.7	0.56 (1)	4.82	J- E	0 / 698	0.16 (1)
E- F	-1047 / 0	-77.7	-77.7	0.22 (1)	5.87	H- E	-92 / 98	0.04 (1)
L- B	-1237 / 0	0.0	0.0	0.13 (1)	7.22	B- K	0 / 884	0.20 (1)
G- F	-1136 / 0	0.0	0.0	0.13 (1)	7.45	H- F	0 / 861	0.19 (1)
L- K	0 / 0	-39.5	-39.5	0.19 (3)	10.00			
K- J	0 / 845	-39.5	-39.5	0.33 (2)	10.00			
J- I	0 / 800	-39.5	-39.5	0.32 (2)	10.00			
I- H	0 / 800	-39.5	-39.5	0.32 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.18 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.33/1.00 (J-K:2) , WB=0.22/1.00 (D-J:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM 1190380
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T63A	2	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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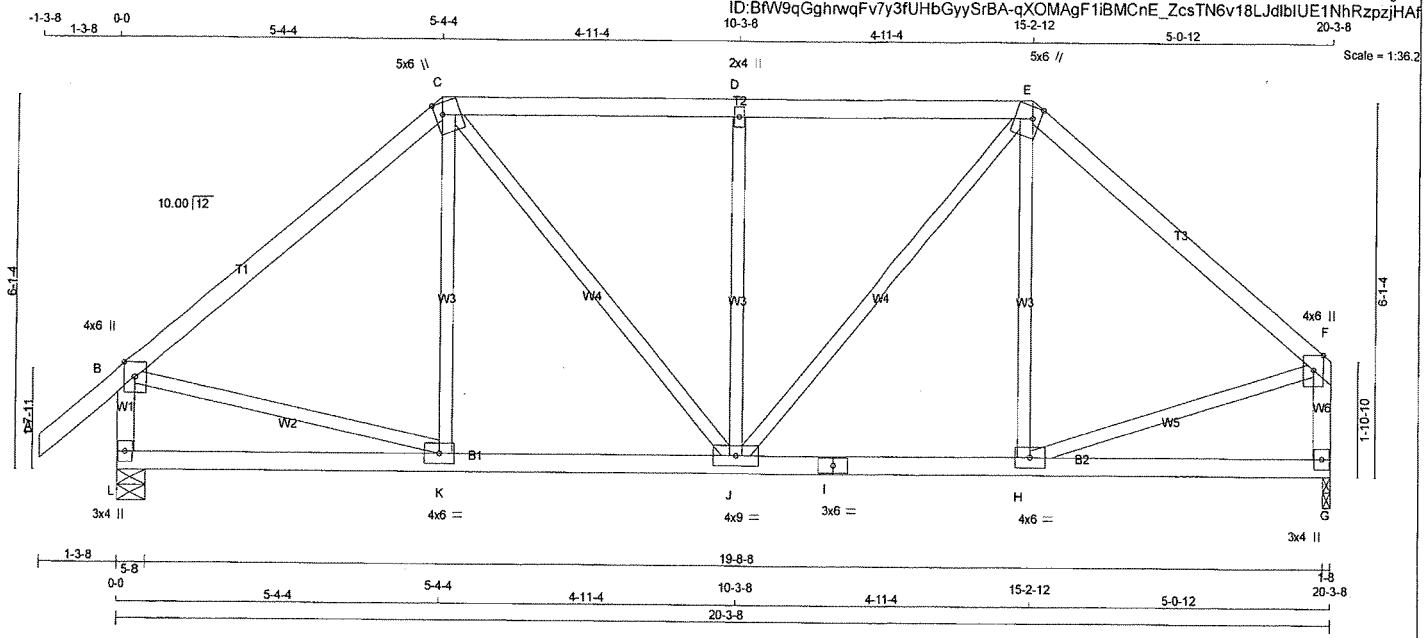


TOTAL WEIGHT = 2 X 89 = 178 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1297	0	1297	0	0	5-8	5-8
G	1189	0	1189	0	0	1-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

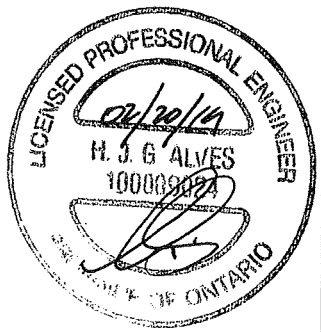
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 34		-77.7	-77.7	0.11 (1)	10.00	K- C	0 / 186		0.04 (3)
B- C	-1083 / 0		-77.7	-77.7	0.31 (1)	5.70	C- J	0 / 391		0.09 (1)
C- D	-1085 / 0		-77.7	-77.7	0.25 (1)	5.77	J- D	-465 / 0		0.27 (1)
D- E	-1085 / 0		-77.7	-77.7	0.25 (1)	5.77	J- E	0 / 438		0.10 (1)
E- F	-1045 / 0		-77.7	-77.7	0.28 (1)	5.82	H- E	-10 / 153		0.04 (3)
L- B	-1210 / 0		0.0	0.0	0.13 (1)	7.28	B- K	0 / 854		0.19 (1)
G- F	-1108 / 0		0.0	0.0	0.12 (1)	7.52	H- F	0 / 836		0.19 (1)
L- K	0 / 0		-39.5	-39.5	0.21 (3)	10.00				
K- J	0 / 831		-39.5	-39.5	0.32 (2)	10.00				
J- I	0 / 801		-39.5	-39.5	0.30 (2)	10.00				
I- H	0 / 801		-39.5	-39.5	0.30 (2)	10.00				
H- G	0 / 0		-39.5	-39.5	0.20 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.32/1.00 (J-K:2) , WB=0.27/1.00 (D-J:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM A1903881
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T64	1	2	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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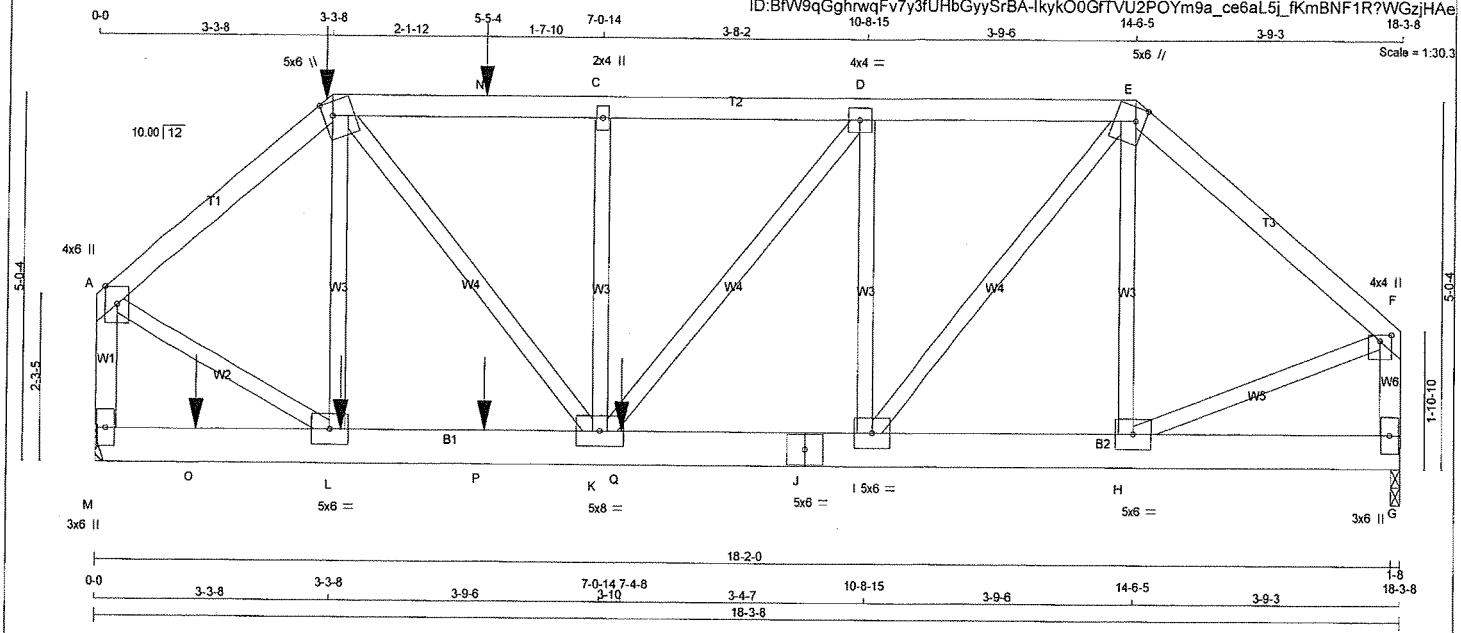

TOTAL WEIGHT = 2 X 93 = 186 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
M- A	1	12	TOP
G- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(183.1)
J- G	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
C- K	1	6	SIDE(202.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	6.0		
H, I, L						
H	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	2613	0	2613	0	0	MECHANICAL	
G	1836	0	1836	0	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M. MINIMUM BEARING LENGTH AT JOINT M = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1974	991 / 0	410 / 0	0 / 0	0 / 0	573 / 0	0 / 0
G	1390	689 / 0	295 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.29 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2257 / 0	-77.7	-77.7	0.10 (1)	5.85
B- N	-2796 / 0	-77.7	-77.7	0.18 (1)	5.29
N- C	-2796 / 0	-77.7	-77.7	0.18 (1)	5.29
C- D	-2796 / 0	-77.7	-77.7	0.13 (1)	5.35
D- E	-2303 / 0	-77.7	-77.7	0.10 (1)	5.81
E- F	-1742 / 0	-77.7	-77.7	0.12 (1)	6.25
M- A	-2525 / 0	0.0	0.0	0.16 (1)	7.13
G- F	-1771 / 0	0.0	0.0	0.10 (1)	7.81
M- O	0 / 0	-39.5	-39.5	0.04 (3)	10.00
O- L	0 / 0	-39.5	-39.5	0.04 (3)	10.00
L- P	0 / 1714	-39.5	-39.5	0.14 (1)	10.00
P- K	0 / 1714	-39.5	-39.5	0.14 (1)	10.00
K- Q	0 / 2303	-39.5	-39.5	0.28 (1)	10.00
Q- J	0 / 2303	-39.5	-39.5	0.28 (1)	10.00
J- I	0 / 2303	-39.5	-39.5	0.28 (1)	10.00
I- H	0 / 1326	-39.5	-39.5	0.12 (1)	10.00
H- G	0 / 0	-39.5	-39.5	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-641 / 0	0.12 (1)
B- K	0 / 1710	0.21 (1)
K- C	-407 / 0	0.07 (1)
K- D	0 / 795	0.10 (1)
I- D	-909 / 0	0.16 (1)
I- E	0 / 1543	0.19 (1)
H- E	-368 / 0	0.06 (1)
A- L	0 / 1756	0.18 (1)
H- F	0 / 1427	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONC
B	3-3-8	-50	-56	---	FRONT	VERT	TOTAL	---	---
B	3-3-8	-157	-157	---	BACK	VERT	TOTAL	---	---
B	3-3-8	-240	-240	---	FRONT	VERT	SNOW	---	---
K	7-4-8	-1478	-1478	---	BACK	VERT	TOTAL	---	---
L	3-5-4	-77	-97	---	BACK	VERT	TOTAL	---	---
N	5-5-4	-151	-151	---	BACK	VERT	TOTAL	---	---
O	1-5-4	-77	-97	---	BACK	VERT	TOTAL	---	---
P	5-5-4	-77	-97	---	BACK	VERT	TOTAL	---	---

LICENSED PROFESSIONAL ENGINEER
02/20/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T1903882
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.28/1.00 (I-K:1) , WB=0.24/1.00 (A-L:1) , SSI=0.47/1.00 (I-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.49 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T65	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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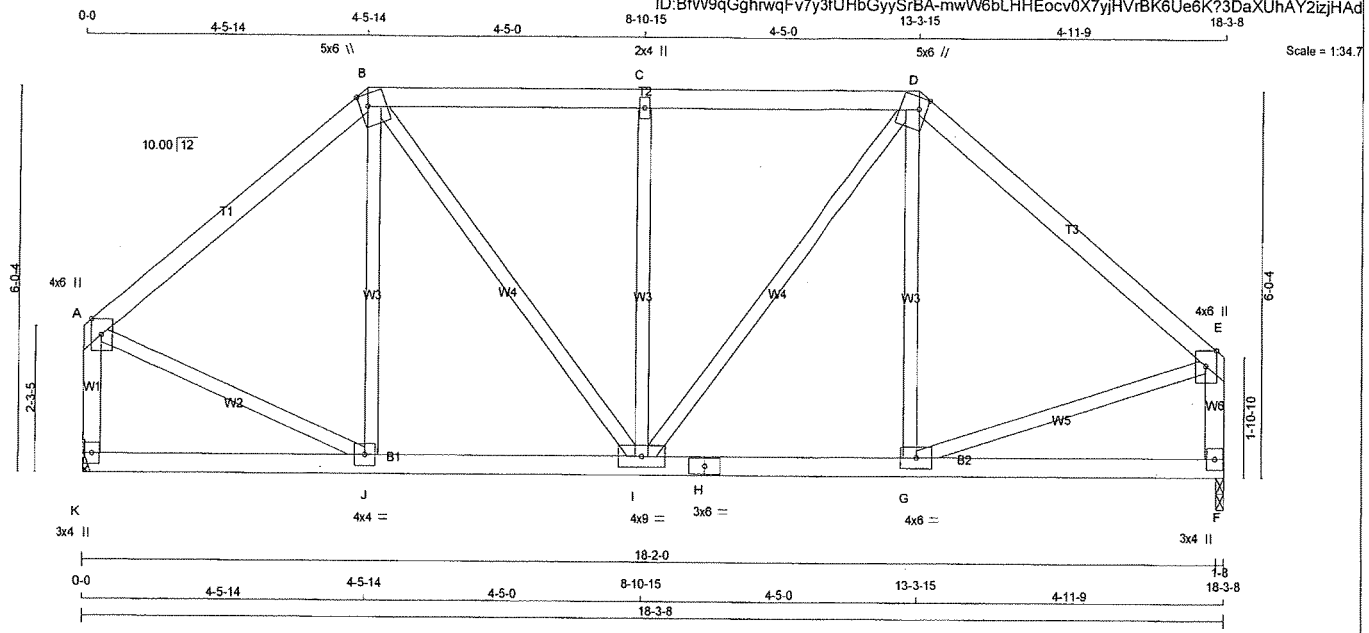


TOTAL WEIGHT = 81 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	6.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1072	0	1072	0	0	IN-SX	IN-SX
F	1072	0	1072	0	0	MECHANICAL	
						1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

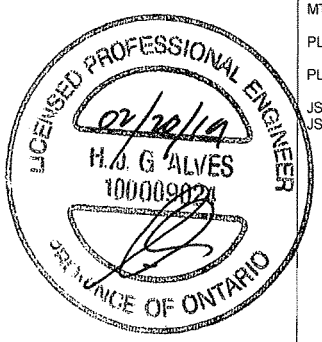
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-854 / 0	-77.7	-77.7	0.21 (1)		6.25
B- C	-892 / 0	-77.7	-77.7	0.20 (1)		6.25
C- D	-892 / 0	-77.7	-77.7	0.20 (1)		6.25
D- E	-911 / 0	-77.7	-77.7	0.26 (1)		6.15
K- A	-1000 / 0	0.0	0.0	0.12 (1)		7.81
F- E	-991 / 0	0.0	0.0	0.11 (1)		7.81
K- J	0 / 0	-39.5	-39.5	0.15 (3)		10.00
J- I	0 / 653	-39.5	-39.5	0.24 (2)		10.00
I- H	0 / 698	-39.5	-39.5	0.27 (2)		10.00
H- G	0 / 698	-39.5	-39.5	0.27 (2)		10.00
G- F	0 / 0	-39.5	-39.5	0.18 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-71 / 101	0.04 (1)
B- I	0 / 391	0.09 (1)
I- C	-415 / 0	0.23 (1)
I- D	0 / 317	0.07 (1)
G- D	0 / 153	0.04 (3)
A- J	0 / 711	0.16 (1)
G- E	0 / 730	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26/1.00 (D-E:1) , BC=0.27/1.00 (G-I:2) , WB=0.23/1.00 (C-I:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (J) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)

DWG NO. TAM T1903883
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T66	1	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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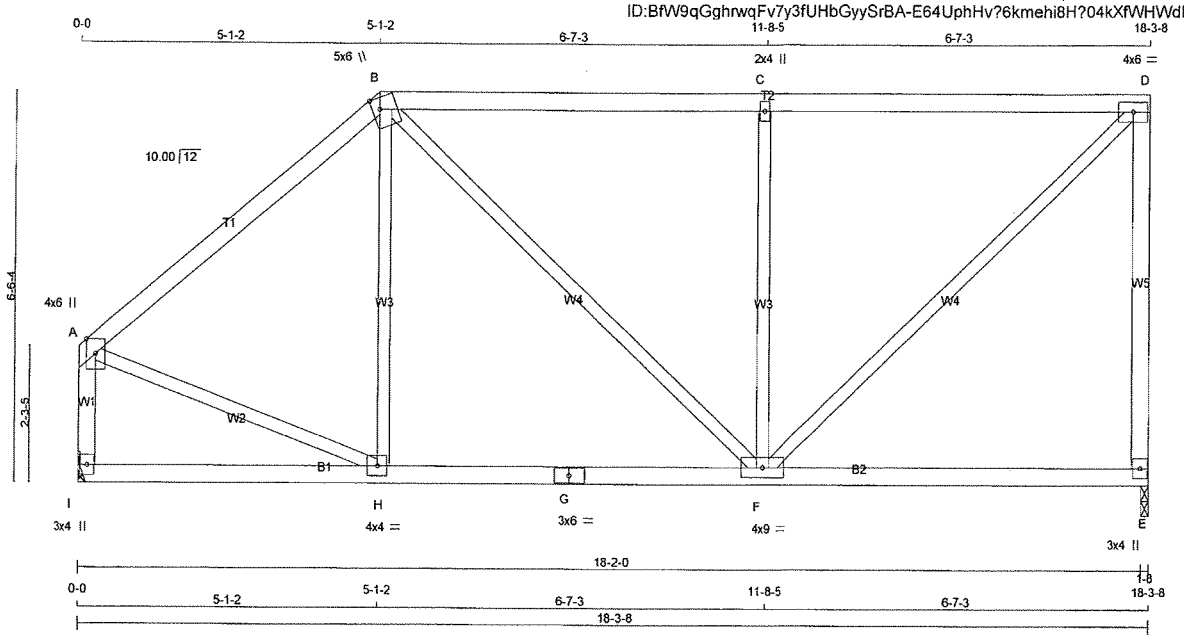


TOTAL WEIGHT = 81 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

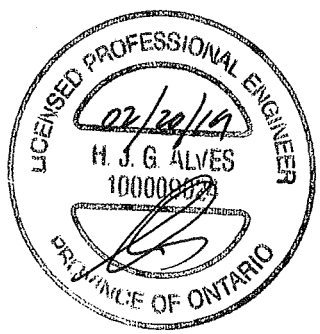
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-856 / 0	-77.7	-77.7	0.38 (1)	6.07	H- B	0 / 169	0.04 (3)
B- C	-821 / 0	-77.7	-77.7	0.61 (1)	5.66	B- F	0 / 228	0.05 (1)
C- D	-821 / 0	-77.7	-77.7	0.61 (1)	5.66	F- C	-635 / 0	0.44 (1)
E- D	-967 / 0	0.0	0.0	0.84 (1)	7.81	F- D	0 / 1126	0.25 (1)
I- A	-993 / 0	0.0	0.0	0.12 (1)	7.81	A- H	0 / 700	0.16 (1)
I- H	0 / 0	-39.5	-39.5	0.22 (3)	10.00			
H- G	0 / 656	-39.5	-39.5	0.40 (2)	10.00			
G- F	0 / 656	-39.5	-39.5	0.40 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.32 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.84/1.00 (D-E:1) , BC=0.40/1.00 (F-H:2) , WB=0.44/1.00 (C-F:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DWG NO. TAM T1903884
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T67	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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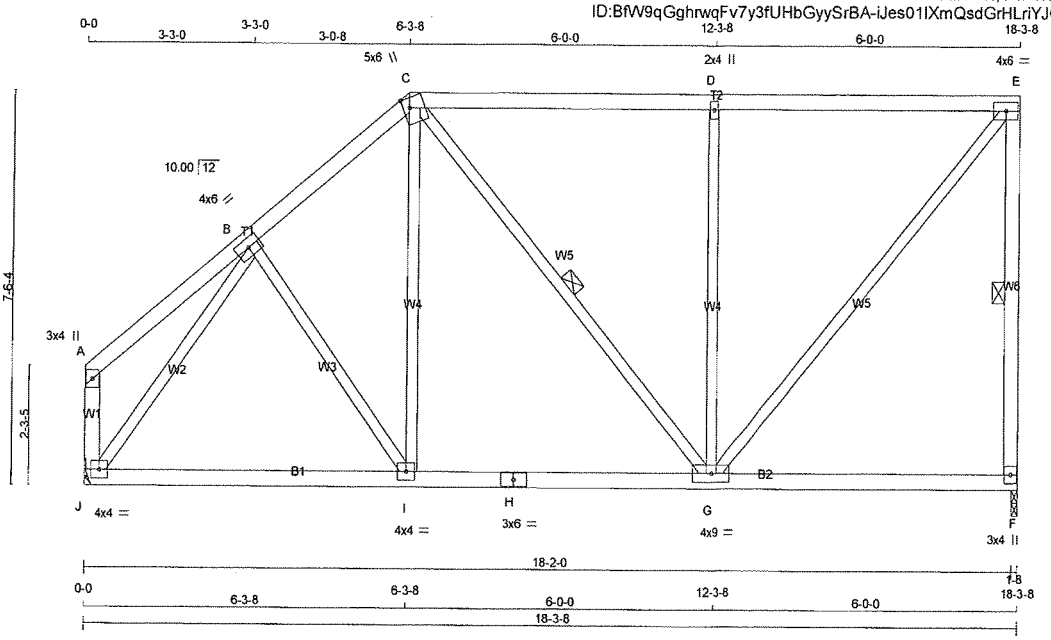


TOTAL WEIGHT = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

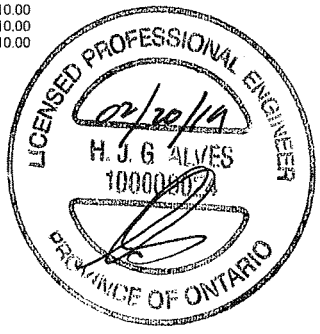
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-77.7	-77.7	0.12 (1)	10.00	B- I	0 / 103	0.02 (3)
B- C	-844 / 0	-77.7	-77.7	0.12 (1)	6.25	I- C	0 / 269	0.06 (3)
C- D	-664 / 0	-77.7	-77.7	0.49 (1)	6.25	C- G	-7 / 45	0.01 (1)
D- E	-665 / 0	-77.7	-77.7	0.49 (1)	6.25	G- D	-577 / 0	0.59 (1)
F- E	-974 / 0	0.0	0.0	0.25 (1)	6.25	G- E	0 / 1037	0.23 (1)
J- A	-94 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1061 / 0	0.53 (1)
J- I	0 / 600	-39.5	-39.5	0.34 (2)	10.00			
I- H	0 / 636	-39.5	-39.5	0.35 (2)	10.00			
H- G	0 / 636	-39.5	-39.5	0.35 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.24 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.35/1.00 (G-I:2) , WB=0.59/1.00 (D-G:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM 1903885
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T68	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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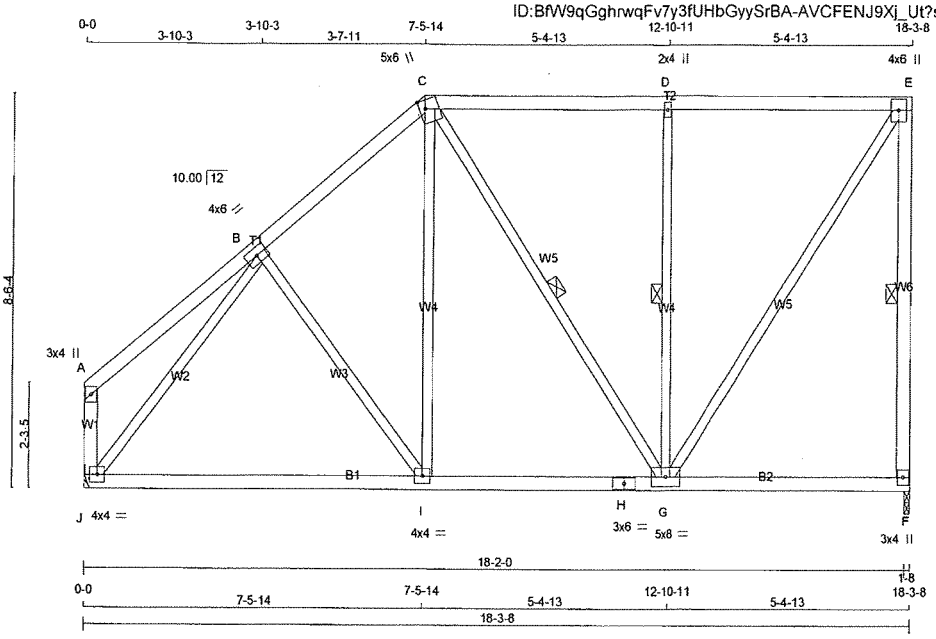


Scale: 1/4"=1'

TOTAL WEIGHT = 94 lb

[M

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

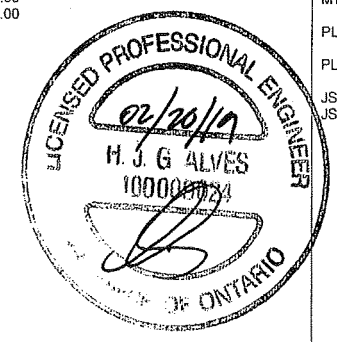
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 21	-77.7	-77.7	0.17 (1)	10.00	B- I	-29 / 70	0.02 (1)
B- C	-804 / 0	-77.7	-77.7	0.18 (1)	6.25	I- C	0 / 369	0.08 (2)
C- D	-541 / 0	-77.7	-77.7	0.39 (1)	6.25	C- G	-125 / 0	0.08 (2)
D- E	-541 / 0	-77.7	-77.7	0.39 (1)	6.25	G- D	-518 / 0	0.24 (1)
F- E	-980 / 0	0.0	0.0	0.32 (1)	6.25	G- E	0 / 981	0.22 (1)
J- A	-111 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1040 / 0	0.71 (1)
J- I	0 / 617	-39.5	-39.5	0.43 (2)	10.00			
I- H	0 / 604	-39.5	-39.5	0.44 (2)	10.00			
H- G	0 / 604	-39.5	-39.5	0.44 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.19 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 970 (0.23")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.44/1.00 (G-I:2) , WB=0.71/1.00 (B-J:1) , SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM T1903586
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T69	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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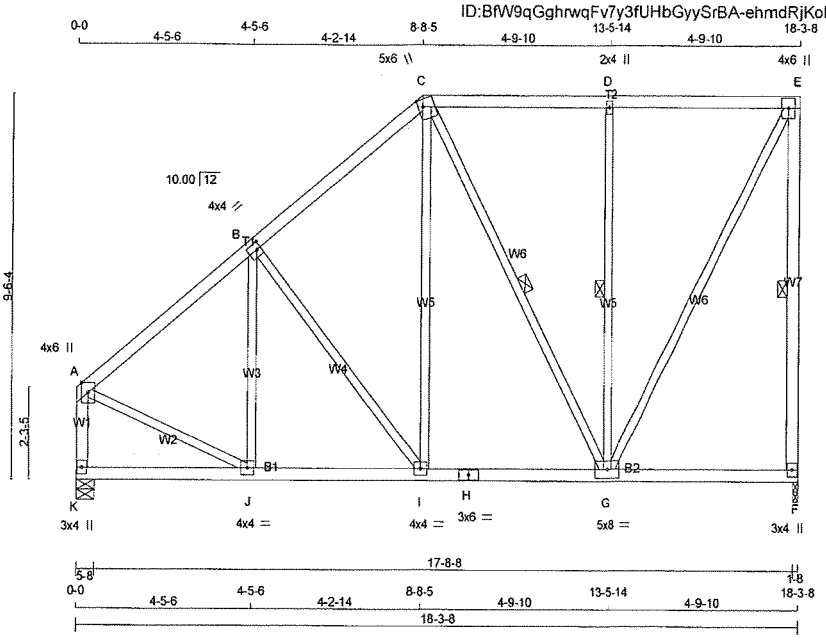


TOTAL WEIGHT = 102 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1072	0	1072	0	0	1-8	1-8
K	1072	0	1072	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

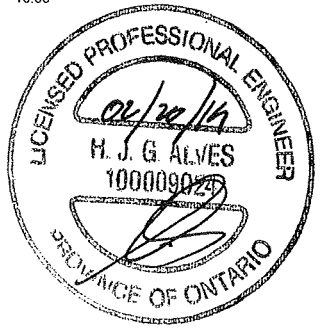
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-864 / 0	-77.7	-77.7	0.26 (1)	6.24
B- C	-723 / 0	-77.7	-77.7	0.26 (1)	6.25
C- D	-446 / 0	-77.7	-77.7	0.30 (1)	6.25
D- E	-446 / 0	-77.7	-77.7	0.30 (1)	6.25
F- E	-995 / 0	0.0	0.0	0.43 (1)	6.25
K- A	-1001 / 0	0.0	0.0	0.12 (1)	7.81
K- J	0 / 0	-39.5	-39.5	0.14 (3)	10.00
J- I	0 / 683	-39.5	-39.5	0.23 (2)	10.00
I- H	0 / 538	-39.5	-39.5	0.24 (2)	10.00
H- G	0 / 538	-39.5	-39.5	0.24 (2)	10.00
G- F	0 / 0	-39.5	-39.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-103 / 84	0.06 (1)
B- I	-245 / 0	0.24 (1)
I- C	0 / 381	0.09 (2)
C- G	-200 / 0	0.16 (1)
G- D	-459 / 0	0.28 (1)
G- E	0 / 960	0.22 (1)
A- J	0 / 745	0.17 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.43/1.00 (E-F:1) , BC=0.24/1.00 (G-I:2) , WB=0.28/1.00 (D-G:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (J) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DWG NO. TAM R7035887
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T70A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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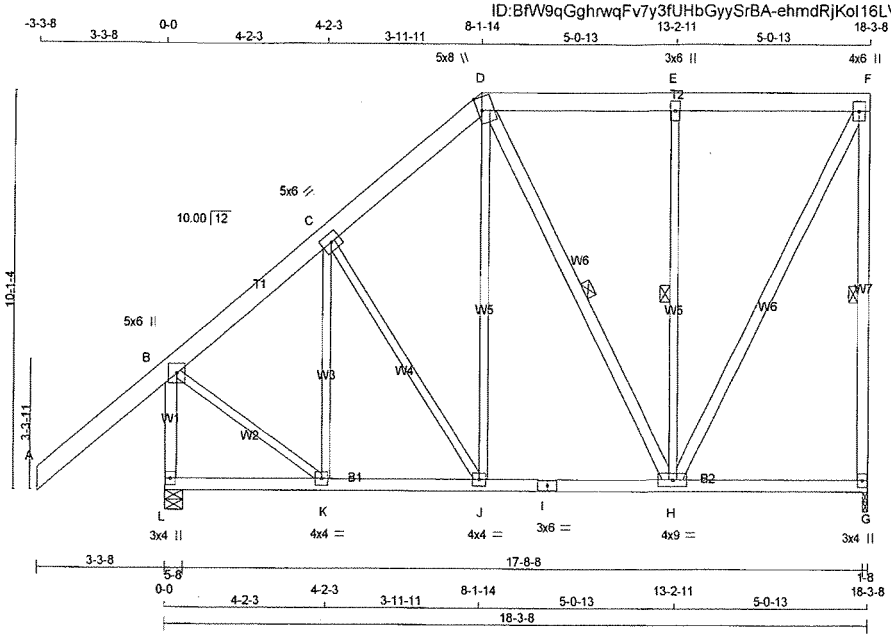


Scale = 1:56.3

TOTAL WEIGHT = 2 X 134 = 267 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1047	0	1047	0	0	1-8	1-8
L	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	802	369 / 0	192 / 0	0 / 0	0 / 0	241 / 0	0 / 0
L	1022	540 / 0	192 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

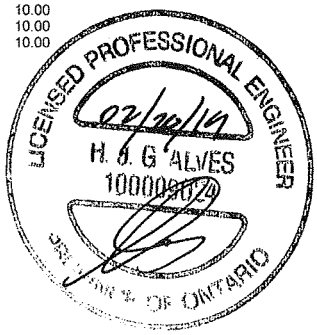
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO			
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00	K- C	-163 / 23	0.12 (1)
B- C	-721 / 0	-77.7	-77.7	0.31 (1)	6.25	C- J	-53 / 0	0.06 (2)
C- D	-629 / 0	-77.7	-77.7	0.10 (1)	6.25	J- D	0 / 253	0.06 (3)
D- E	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	D- H	-120 / 0	0.08 (1)
E- F	-425 / 0	-77.7	-77.7	0.14 (1)	6.25	H- E	-474 / 0	0.32 (1)
G- F	-966 / 0	0.0	0.0	0.46 (1)	6.25	H- F	0 / 914	0.15 (1)
L- B	-1297 / 0	0.0	0.0	0.22 (1)	7.09	B- K	0 / 610	0.14 (1)
L- K	0 / 0	-39.5	-39.5	0.12 (3)	10.00			
K- J	0 / 503	-39.5	-39.5	0.19 (2)	10.00			
J- I	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
I- H	0 / 480	-39.5	-39.5	0.25 (2)	10.00			
H- G	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.46/1.00 (F-G:1) , BC=0.25/1.00 (H-J:2) , WB=0.32/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM 179038888
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T74	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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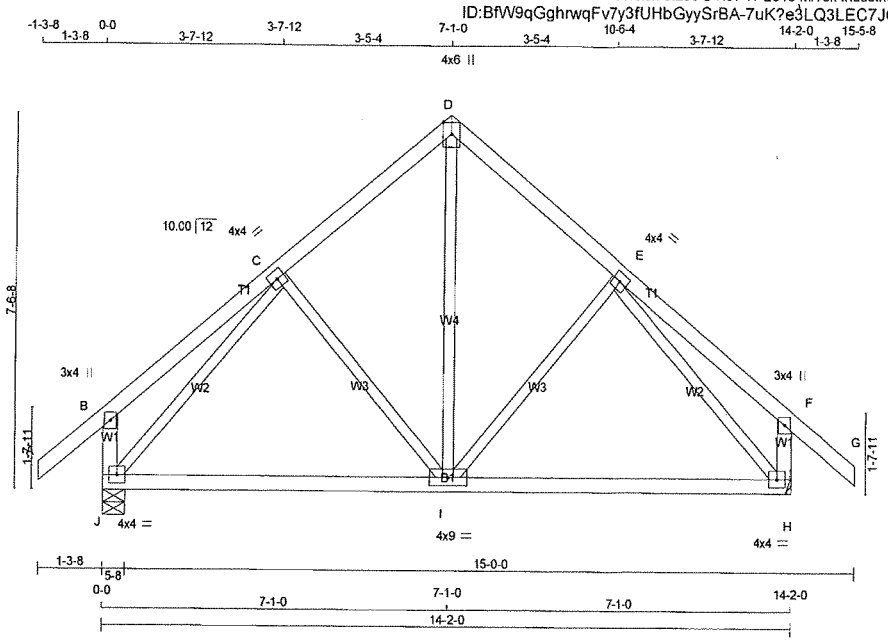

TOTAL WEIGHT = 4 X 67 = 267 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	938	0	938	0	0	5-8	5-8
H	938	0	938	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	709	354 / 0	149 / 0	0 / 0	0 / 0	206 / 0	0 / 0
H	709	354 / 0	149 / 0	0 / 0	0 / 0	206 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 20	-77.7	-77.7	0.15 (1)	10.00
C- D	-594 / 0	-77.7	-77.7	0.12 (1)	6.25
D- E	-594 / 0	-77.7	-77.7	0.12 (1)	6.25
E- F	0 / 20	-77.7	-77.7	0.15 (1)	10.00
F- G	0 / 34	-77.7	-77.7	0.11 (1)	10.00
J- B	-213 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-213 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 514	-39.5	-39.5	0.50 (3)	10.00
I- H	0 / 514	-39.5	-39.5	0.50 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 531	0.12 (2)
I- E	-112 / 24	0.06 (1)
C- I	-112 / 24	0.06 (1)
J- C	-804 / 0	0.40 (1)
E- H	-804 / 0	0.40 (1)

LICENSED PROFESSIONAL ENGINEER
02/20/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.50/1.00 (I-J:3) , WB=0.40/1.00 (C-J:1) , SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

DWG NO. TAM T1903887
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G74	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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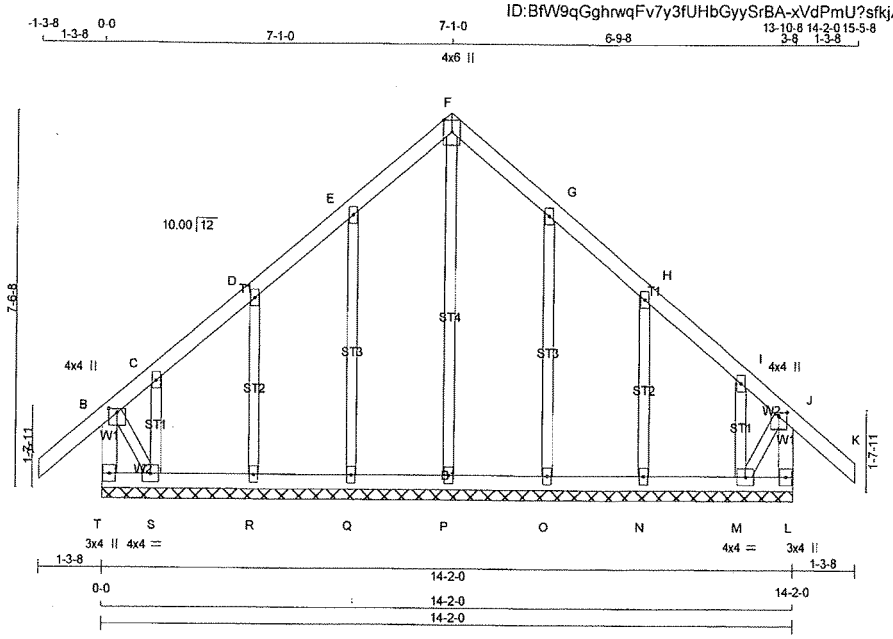


TOTAL WEIGHT = 2 X 70 = 140 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, R						
N	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

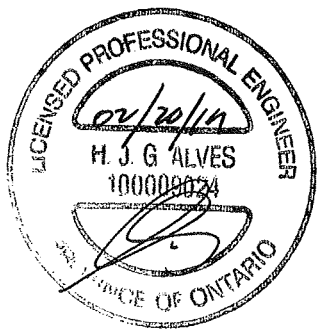
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-241 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-61 / 0	-77.7	-77.7	0.10 (1)	6.25
C- D	-11 / 0	-77.7	-77.7	0.04 (1)	6.25
D- E	-10 / 0	-77.7	-77.7	0.04 (1)	6.25
E- F	-18 / 0	-77.7	-77.7	0.04 (1)	6.25
F- G	-18 / 0	-77.7	-77.7	0.04 (1)	6.25
G- H	-10 / 0	-77.7	-77.7	0.04 (1)	6.25
H- I	-11 / 0	-77.7	-77.7	0.04 (1)	6.25
I- J	-61 / 0	-77.7	-77.7	0.10 (1)	6.25
J- K	0 / 34	-77.7	-77.7	0.11 (1)	10.00
L- J	-241 / 0	0.0	0.0	0.03 (1)	7.81
T- S	0 / 0	-39.5	-39.5	0.02 (3)	10.00
S- R	0 / 13	-39.5	-39.5	0.03 (3)	10.00
R- Q	0 / 9	-39.5	-39.5	0.03 (3)	10.00
Q- P	0 / 6	-39.5	-39.5	0.02 (3)	10.00
P- O	0 / 6	-39.5	-39.5	0.02 (3)	10.00
O- N	0 / 9	-39.5	-39.5	0.03 (3)	10.00
N- M	0 / 13	-39.5	-39.5	0.03 (3)	10.00
M- L	0 / 0	-39.5	-39.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-121 / 0	0.12 (1)
Q- E	-175 / 0	0.09 (1)
R- D	-158 / 0	0.04 (1)
S- C	-42 / 0	0.01 (1)
O- G	-175 / 0	0.09 (1)
N- H	-158 / 0	0.04 (1)
M- I	-42 / 0	0.01 (1)
B- S	0 / 23	0.01 (1)
M- J	0 / 23	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	20.9	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.03/1.00 (M-N:3) , WB=0.12/1.00 (F-P:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (B) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

DWG NO. TAM T1903890
STRUCTURAL COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T75	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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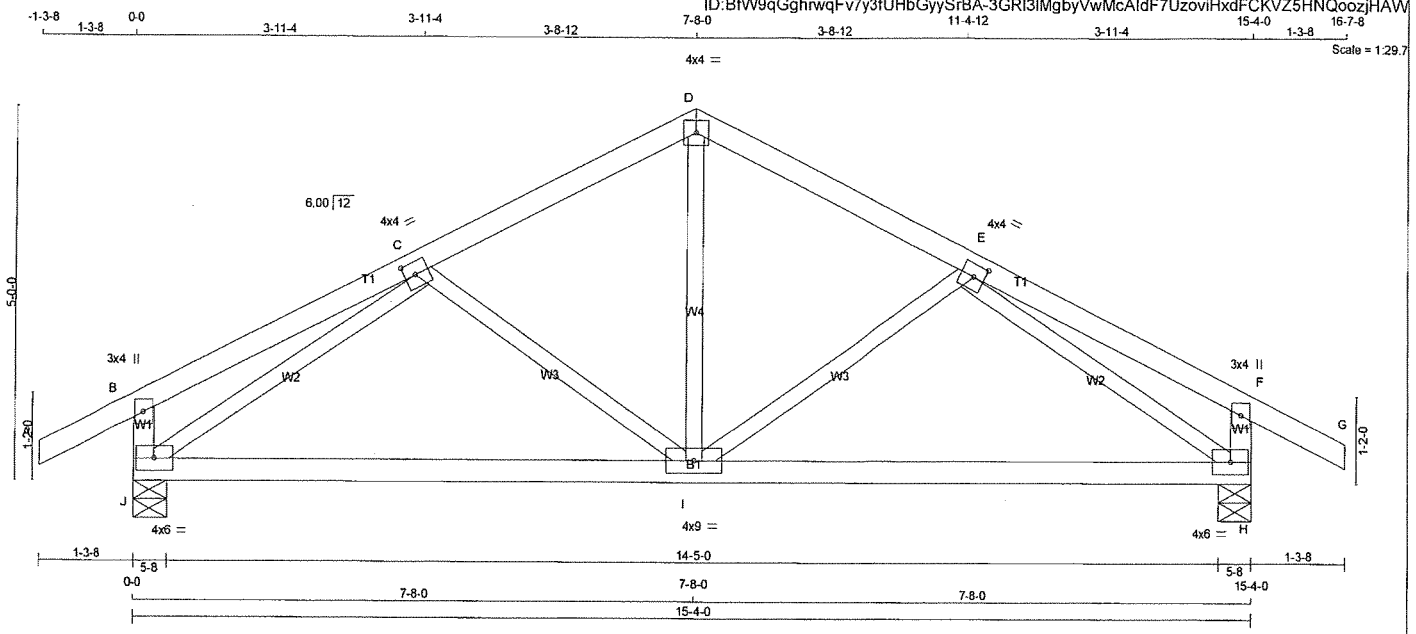

TOTAL WEIGHT = 2 X 60 = 121 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1004	0	1004	0	0	5-8	5-8
H	1004	0	1004	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	760	377 / 0	161 / 0	0 / 0	0 / 0	222 / 0	0 / 0
H	760	377 / 0	161 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	I- D	0 / 574	0.13 (2)
B- C	0 / 14	-77.7	-77.7	0.17 (1)	10.00	I- E	-158 / 35	0.06 (1)
C- D	-902 / 0	-77.7	-77.7	0.14 (1)	6.25	C- I	-158 / 35	0.06 (1)
D- E	-902 / 0	-77.7	-77.7	0.14 (1)	6.25	J- C	-1137 / 0	0.41 (1)
E- F	0 / 14	-77.7	-77.7	0.17 (1)	10.00	E- H	-1137 / 0	0.41 (1)
F- G	0 / 24	-77.7	-77.7	0.10 (1)	10.00			
J- B	-223 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-223 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 921	-39.5	-39.5	0.62 (2)	10.00			
I- H	0 / 921	-39.5	-39.5	0.62 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.17/1.00 (E-F:1) , BC=0.62/1.00 (I-J:2) , WB=0.41/1.00 (E-H:1) , SSI=0.20/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (C) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

DWG NO. TAM 7903891
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	G75	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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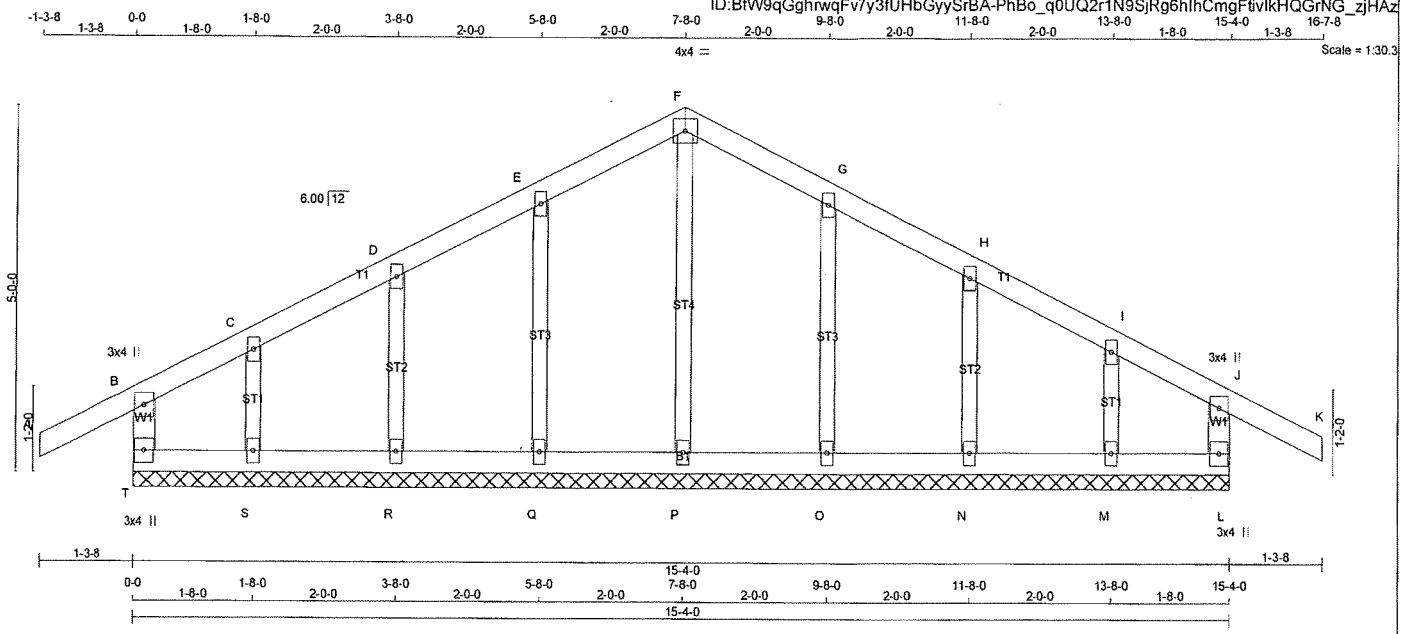


TOTAL WEIGHT = 2 X 58 = 115 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C, D, E, G, H, I					
C	TMW+w	MT20	2.0	4.0	
F	TTW-p	MT20	4.0	4.0	
J	TMV+p	MT20	3.0	4.0	
L	BMV1+p	MT20	3.0	4.0	
M, N, O, P, Q, R, S					
M	BMW1+w	MT20	2.0	4.0	
T	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

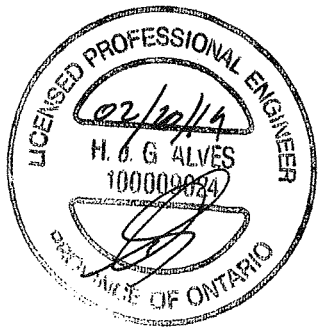
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-77.7	-77.7	0.10 (1)	10.00	P- F	-187 / 0	0.07 (1)
B- C	-3 / 10	-77.7	-77.7	0.06 (1)	10.00	Q- E	-157 / 0	0.04 (1)
C- D	0 / 27	-77.7	-77.7	0.04 (1)	10.00	R- D	-160 / 0	0.03 (1)
D- E	0 / 28	-77.7	-77.7	0.04 (1)	10.00	S- C	-112 / 0	0.02 (1)
E- F	0 / 31	-77.7	-77.7	0.04 (1)	10.00	O- G	-157 / 0	0.04 (1)
F- G	0 / 31	-77.7	-77.7	0.04 (1)	10.00	N- H	-160 / 0	0.03 (1)
G- H	0 / 28	-77.7	-77.7	0.04 (1)	10.00	M- I	-112 / 0	0.02 (1)
H- I	0 / 27	-77.7	-77.7	0.04 (1)	10.00			
I- J	-3 / 10	-77.7	-77.7	0.06 (1)	10.00			
J- K	0 / 24	-77.7	-77.7	0.10 (1)	10.00			
T- B	-179 / 0	0.0	0.0	0.03 (1)	7.81			
L- J	-179 / 0	0.0	0.0	0.03 (1)	7.81			
T- S	-17 / 1	-39.5	-39.5	0.02 (3)	6.25			
S- R	-21 / 0	-39.5	-39.5	0.02 (3)	6.25			
R- Q	-25 / 0	-39.5	-39.5	0.02 (3)	6.25			
Q- P	-28 / 0	-39.5	-39.5	0.02 (3)	6.25			
P- O	-28 / 0	-39.5	-39.5	0.02 (3)	6.25			
O- N	-25 / 0	-39.5	-39.5	0.02 (3)	6.25			
N- M	-21 / 0	-39.5	-39.5	0.02 (3)	6.25			
M- L	-17 / 1	-39.5	-39.5	0.02 (3)	6.25			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (J-K:1) , BC=0.02/1.00 (M-N:3) , WB=0.07/1.00 (F-P:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (F) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

DWG NO. TAM 1703892
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T76	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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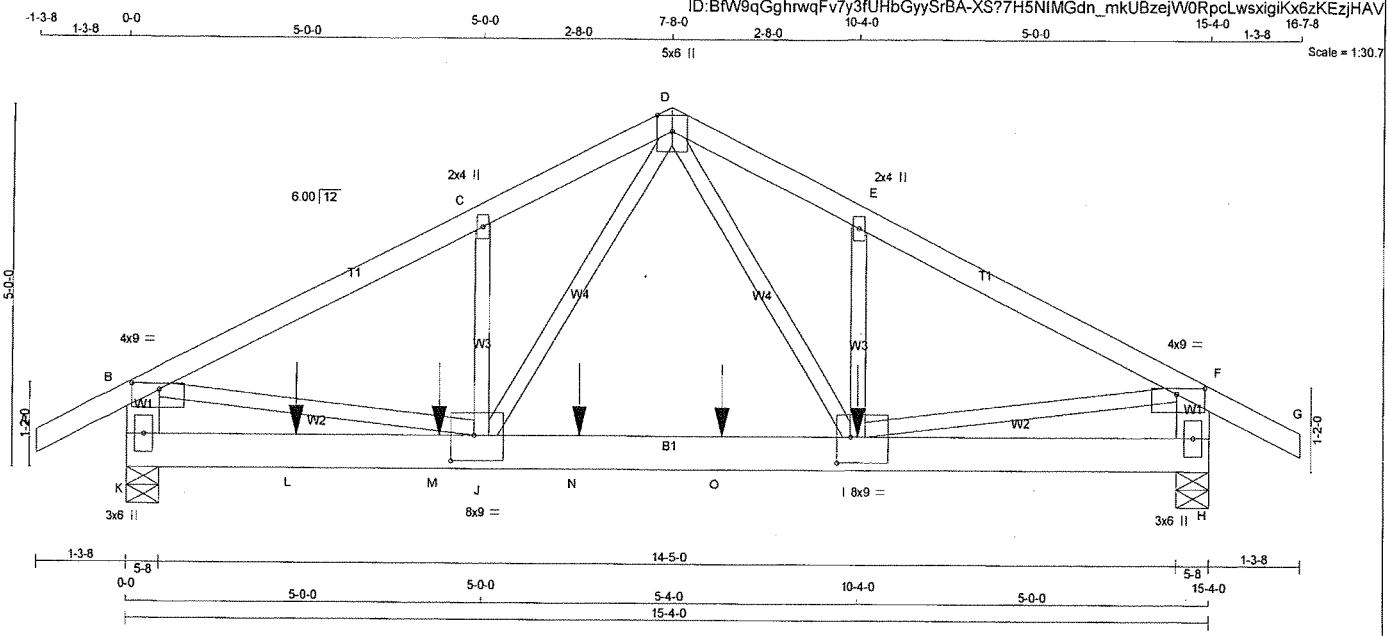


TOTAL WEIGHT = 4 X 73 = 292 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
K- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
E- I	1	3	SIDE(488.2)
2x3	1	3	
J- D	1	6	
I- D	1	6	
B- J	1	6	
F- I	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	9.0	1.00	4.75
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMVW-p	MT20	4.0	9.0	1.00	4.75
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	8.0	9.0	4.25	2.50
J	BMWWW-t	MT20	8.0	9.0	4.25	4.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	4530	0	4530	0	0	5-8	5-8
H	4181	0	4181	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	3431	1695 / 0	733 / 0	0 / 0	0 / 0	1003 / 0	0 / 0
H	3162	1579 / 0	662 / 0	0 / 0	0 / 0	920 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

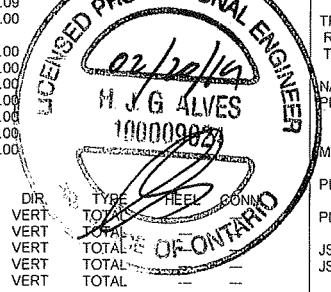
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
A- B	0 / 24		-77.7	-77.7	0.06 (1)	10.00		J- C	-328 / 0		0.04 (1)
B- C	-6398 / 0		-77.7	-77.7	0.37 (1)	3.62		J- D	0 / 3057		0.38 (1)
C- D	-6400 / 0		-77.7	-77.7	0.18 (1)	3.75		I- E	-325 / 0		0.04 (1)
D- E	-6598 / 0		-77.7	-77.7	0.19 (1)	3.70		D- I	0 / 3418		0.42 (1)
E- F	-6597 / 0		-77.7	-77.7	0.39 (1)	3.56		B- J	0 / 5795		0.72 (1)
F- G	0 / 24		-77.7	-77.7	0.06 (1)	10.00		I- F	0 / 5795		0.72 (1)
K- B	-4025 / 0		0.0	0.0	0.14 (1)	7.09					
H- F	-4142 / 0		0.0	0.0	0.15 (1)	7.00					
K- L	0 / 0		-39.5	-39.5	0.51 (1)	10.00					
L- M	0 / 0		-39.5	-39.5	0.51 (1)	10.00					
M- J	0 / 0		-39.5	-39.5	0.51 (1)	10.00					
J- N	0 / 4210		-39.5	-39.5	0.78 (1)	10.00					
N- O	0 / 4210		-39.5	-39.5	0.78 (1)	10.00					
O- I	0 / 4210		-39.5	-39.5	0.78 (1)	10.00					
I- H	0 / 0		-39.5	-39.5	0.12 (2)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
I	10-4-0	-2573	-2573	---	FRONT	VERT	TOTAL	---	---
L	2-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
M	4-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
N	6-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---
O	8-4-12	-1032	-1032	---	FRONT	VERT	TOTAL	---	---



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/ 997 (0.18")

CSI: TC=0.39/1.00 (E-F:1) , BC=0.78/1.00 (I-J:1) , WB=0.74/1.00 (F-I:1) , SSI=0.58/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.56 (B) (INPUT = 1.00)

DWG NO. TAM 1903893
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T81	1	2	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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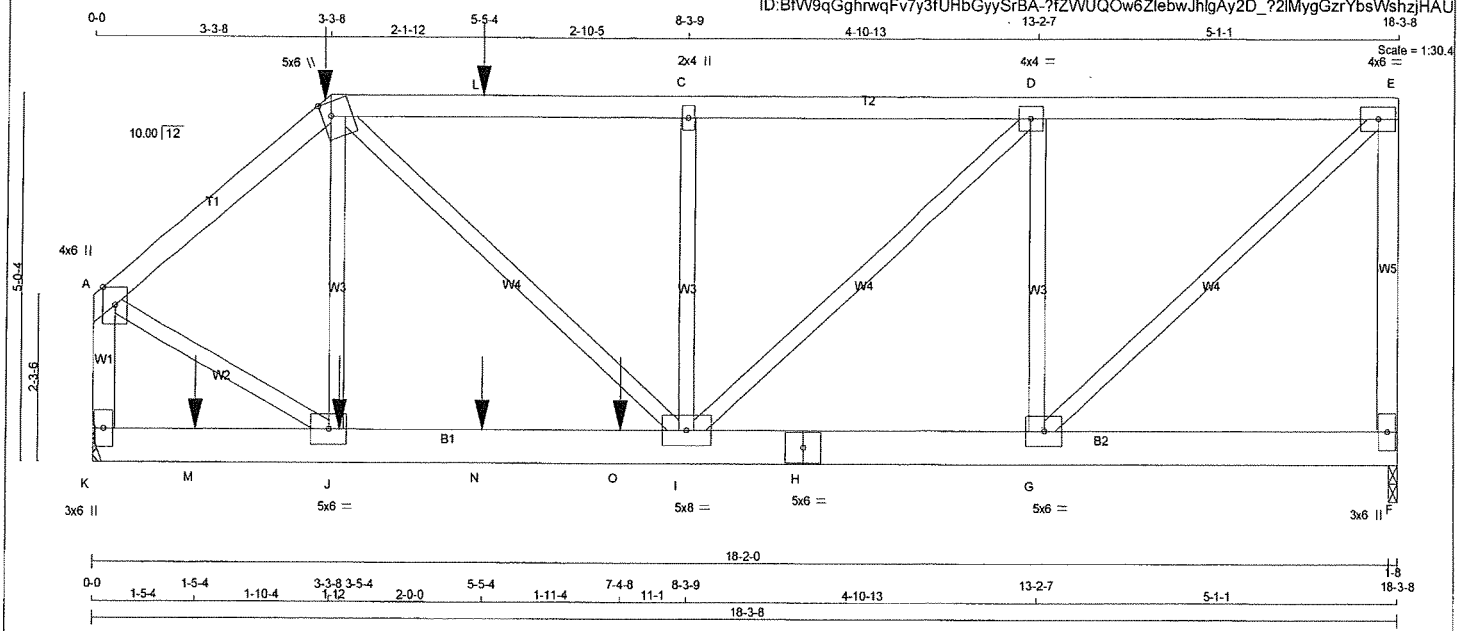


TOTAL WEIGHT = 2 X 91 = 181 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(61.0)
E- F	1	12	TOP
K- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(183.1)
H- F	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1836	0	1836	0	0	1-8	1-8
K	2613	0	2613	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1390	689 / 0	295 / 0	0 / 0	0 / 0	406 / 0	0 / 0
K	1974	991 / 0	410 / 0	0 / 0	0 / 0	573 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

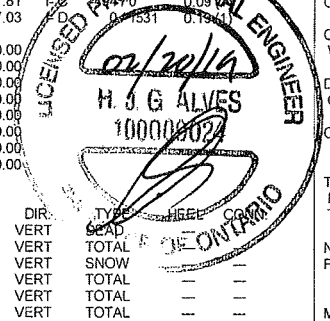
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2340 / 0	-77.7	-77.7	0.10 (1)	5.77	J- B	-281 / 118	0.05 (1)
B- L	-2829 / 0	-77.7	-77.7	0.29 (1)	5.12	A- J	0 / 2043	0.25 (1)
L- C	-2829 / 0	-77.7	-77.7	0.29 (1)	5.12	G- E	0 / 2325	0.29 (1)
C- D	-2829 / 0	-77.7	-77.7	0.22 (1)	5.22	B- I	-97 / 1421	0.18 (1)
D- E	-1718 / 0	-77.7	-77.7	0.15 (1)	6.25	G- D	-1464 / 0	0.19 (1)
F- E	-1731 / 0	0.0	0.0	0.34 (1)	7.81	I- G	-97 / 0	0.09 (1)
K- A	-2615 / 0	0.0	0.0	0.17 (1)	7.03	I- D	0 / 1331	0.19 (1)
K- M	0 / 0	-39.5	-39.5	0.14 (1)	10.00			
M- J	0 / 0	-39.5	-39.5	0.14 (1)	10.00			
J- N	0 / 1785	-39.5	-39.5	0.41 (1)	10.00			
N- O	0 / 1785	-39.5	-39.5	0.41 (1)	10.00			
O- I	0 / 1785	-39.5	-39.5	0.41 (1)	10.00			
I- H	0 / 1718	-39.5	-39.5	0.27 (1)	10.00			
H- G	0 / 1718	-39.5	-39.5	0.27 (1)	10.00			
G- F	0 / 0	-39.5	-39.5	0.06 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	COM
B	3-3-8	-50	-56	---	FRONT	VERT	SEAT	---	---
B	3-3-8	-157	-157	---	FRONT	VERT	TOTAL	---	---
B	3-3-8	-240	-240	---	FRONT	VERT	SNOW	---	---
J	3-5-4	-77	-97	---	FRONT	VERT	TOTAL	---	---
L	5-5-4	-151	-151	---	FRONT	VERT	TOTAL	---	---
M	1-5-4	-77	-97	---	FRONT	VERT	TOTAL	---	---
N	5-5-4	-77	-97	---	FRONT	VERT	TOTAL	---	---
O	7-4-8	-1478	-1478	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM 77703894
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.41/1.00 (I-J:1) , WB=0.29/1.00 (E-G:1) , SSI=0.44/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.50 (A) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T82	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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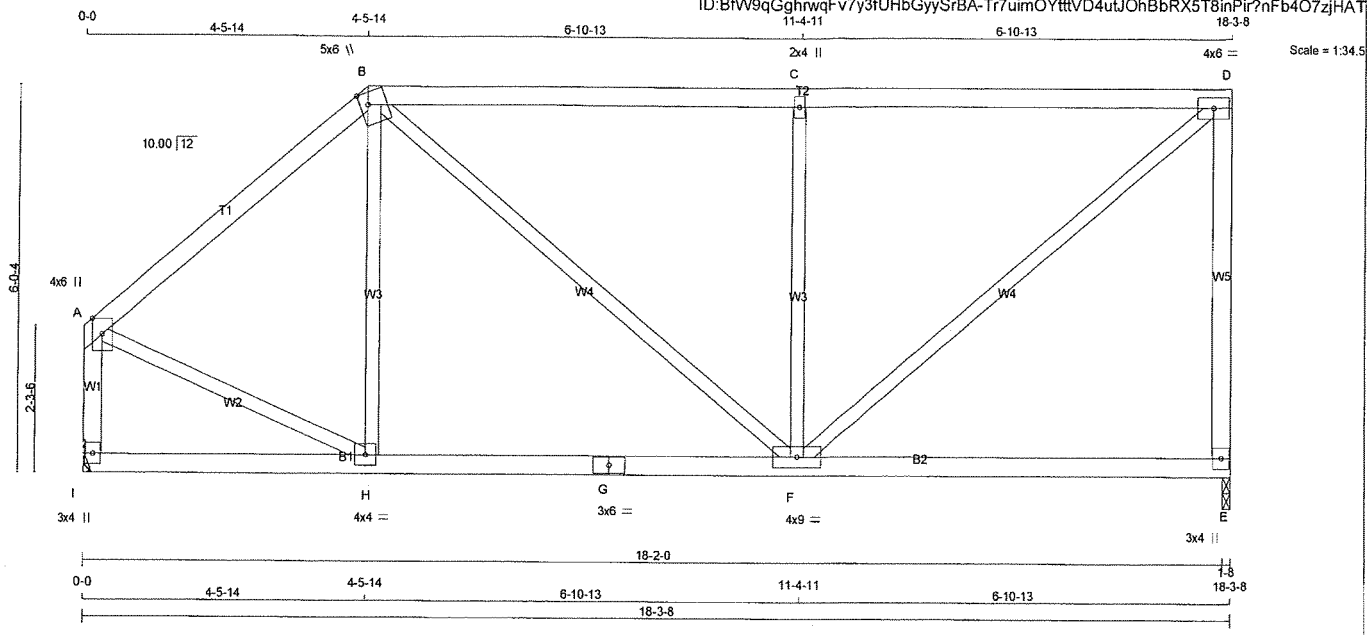

TOTAL WEIGHT = 78 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-860 / 0	-77.7	-77.7	0.29 (1)	6.20
B- C	-918 / 0	-77.7	-77.7	0.68 (1)	5.27
C- D	-918 / 0	-77.7	-77.7	0.69 (1)	5.27
E- D	-962 / 0	0.0	0.0	0.67 (1)	7.81
I- A	-1006 / 0	0.0	0.0	0.12 (1)	7.80
I- H	0 / 0	-39.5	-39.5	0.21 (3)	10.00
H- G	0 / 659	-39.5	-39.5	0.43 (2)	10.00
G- F	0 / 659	-39.5	-39.5	0.43 (2)	10.00
F- E	0 / 0	-39.5	-39.5	0.36 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-41 / 139	0.03 (3)
B- F	0 / 337	0.08 (1)
F- C	-664 / 0	0.38 (1)
F- D	0 / 1190	0.27 (1)
A- H	0 / 716	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.69/1.00 (C-D:1) , BC=0.43/1.00 (F-H:2) , WB=0.38/1.00 (C-F:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (H) (INPUT = 0.90)
JSI METAL= 0.40 (A) (INPUT = 1.00)

DWG NO. TAM 11903895
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T83	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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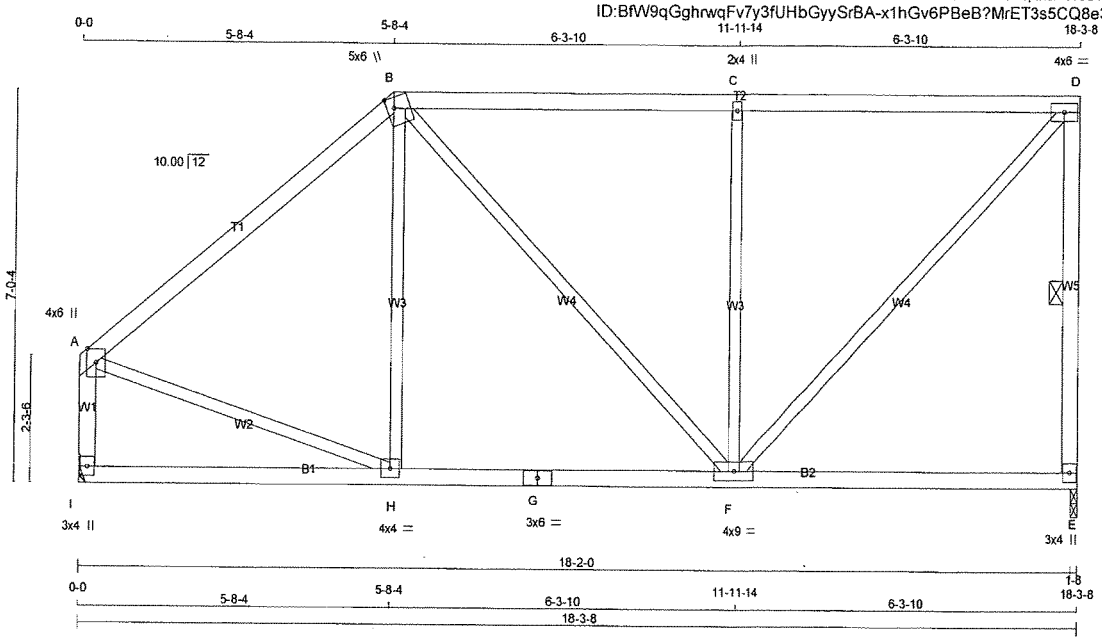


TOTAL WEIGHT = 83 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BS-t	MT20	3.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	1072	0	1072	0	0	1-8	1-8
I	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
I	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

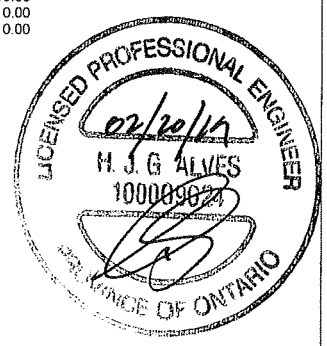
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-845 / 0	-77.7	-77.7	0.49 (1)	5.93	H- B	0 / 198	0.05 (3)
B- C	-738 / 0	-77.7	-77.7	0.55 (1)	6.04	B- F	0 / 132	0.03 (1)
C- D	-738 / 0	-77.7	-77.7	0.55 (1)	6.04	F- C	-606 / 0	0.51 (1)
E- D	-970 / 0	0.0	0.0	0.22 (1)	6.25	F- D	0 / 1076	0.24 (1)
I- A	-980 / 0	0.0	0.0	0.12 (1)	7.81	A- H	0 / 683	0.15 (1)
I- H	0 / 0	-39.5	-39.5	0.24 (3)	10.00			
H- G	0 / 648	-39.5	-39.5	0.36 (2)	10.00			
G- F	0 / 648	-39.5	-39.5	0.36 (2)	10.00			
F- E	0 / 0	-39.5	-39.5	0.28 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.36/1.00 (F-H:2) , WB=0.51/1.00 (C-F:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DWG NO. TAM T1903896
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T84	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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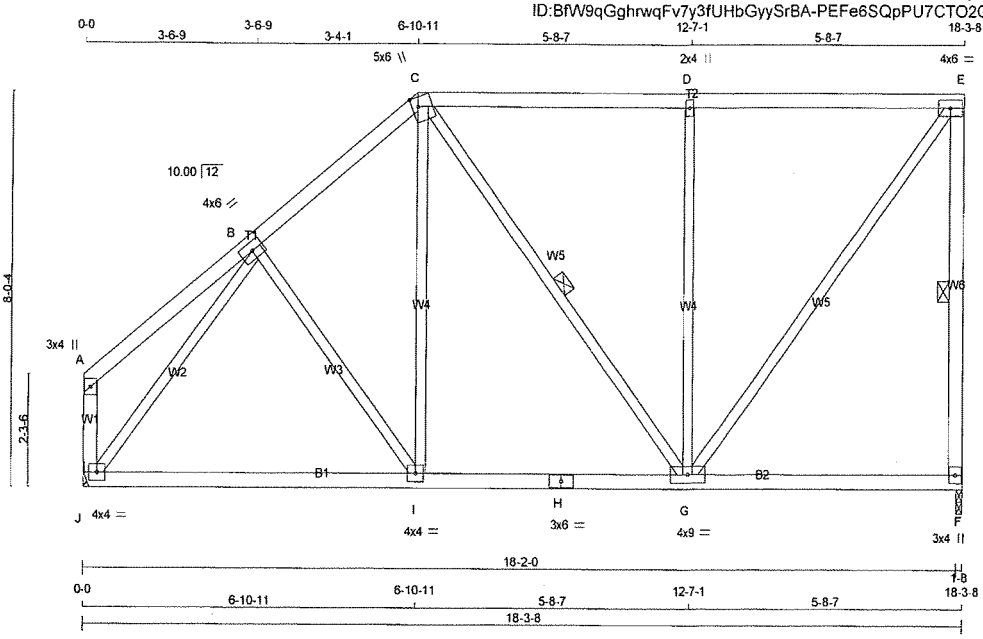


TOTAL WEIGHT = 91 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1072	0	1072	0	0	1-8	1-8
J	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
J	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

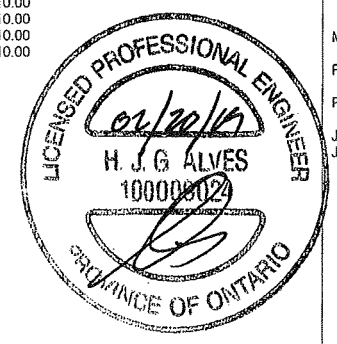
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 19	-77.7	-77.7	0.14 (1)	10.00	B- I	0 / 86	0.02 (3)
B- C	-825 / 0	-77.7	-77.7	0.15 (1)	6.25	I- C	0 / 319	0.07 (2)
C- D	-599 / 0	-77.7	-77.7	0.44 (1)	6.25	C- G	-56 / 0	0.04 (3)
D- E	-600 / 0	-77.7	-77.7	0.44 (1)	6.25	G- D	-548 / 0	0.67 (1)
F- E	-977 / 0	0.0	0.0	0.28 (1)	6.25	G- E	0 / 1006	0.23 (1)
J- A	-103 / 0	0.0	0.0	0.01 (1)	7.81	J- B	-1051 / 0	0.61 (1)
J- I	0 / 610	-39.5	-39.5	0.38 (2)	10.00			
I- H	0 / 621	-39.5	-39.5	0.39 (2)	10.00			
H- G	0 / 621	-39.5	-39.5	0.39 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.39/1.00 (G-I:2) , WB=0.67/1.00 (D-G:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM 17903897
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T85	1	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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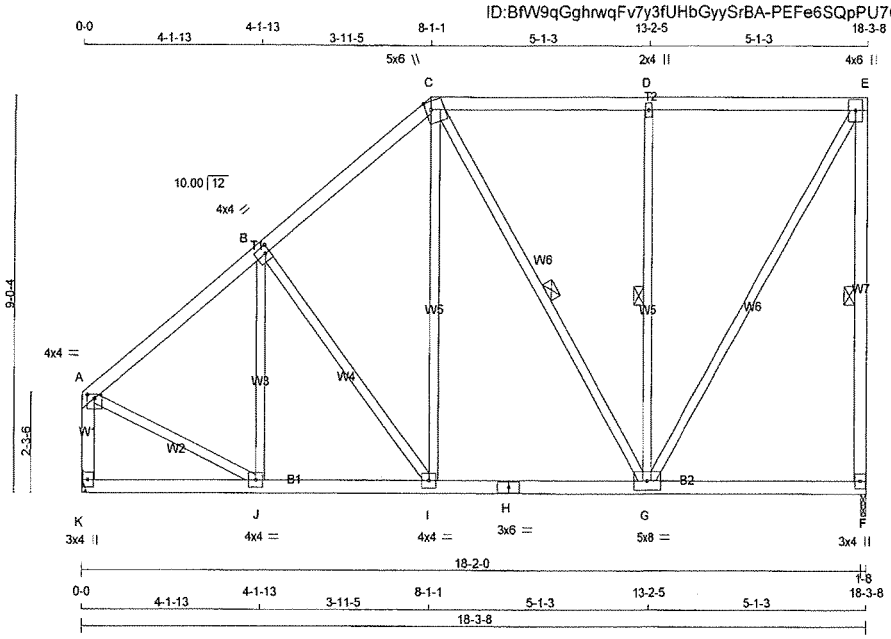


TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
K	1072	0	1072	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 1-8.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

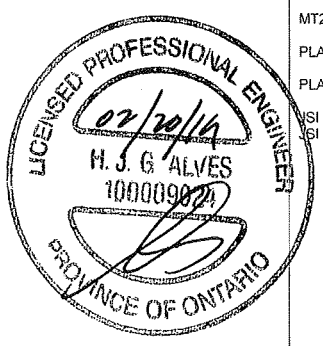
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-856 / 0	-77.7	-77.7	0.23 (1)	6.25	J- B	-139 / 53	0.07 (1)
B- C	-754 / 0	-77.7	-77.7	0.22 (1)	6.25	B- I	-195 / 0	0.16 (1)
C- D	-493 / 0	-77.7	-77.7	0.34 (1)	6.25	I- C	0 / 348	0.08 (2)
D- E	-493 / 0	-77.7	-77.7	0.34 (1)	6.25	C- G	-138 / 0	0.10 (1)
F- E	-991 / 0	0.0	0.0	0.37 (1)	6.25	G- D	-489 / 0	0.26 (1)
K- A	-1005 / 0	0.0	0.0	0.12 (1)	7.80	G- E	0 / 972	0.22 (1)
						A- J	0 / 746	0.17 (1)
K- J	0 / 0	-39.5	-39.5	0.12 (3)	10.00			
J- I	0 / 676	-39.5	-39.5	0.21 (2)	10.00			
I- H	0 / 563	-39.5	-39.5	0.27 (2)	10.00			
H- G	0 / 563	-39.5	-39.5	0.27 (2)	10.00			
G- F	0 / 0	-39.5	-39.5	0.20 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	20.9	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.37/1.00 (E-F:1) , BC=0.27/1.00 (G-I:2) , WB=0.26/1.00 (D-G:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (A) (INPUT = 0.90)
SI METAL= 0.27 (H) (INPUT = 1.00)

DWG NO. TAM 1790398
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T86	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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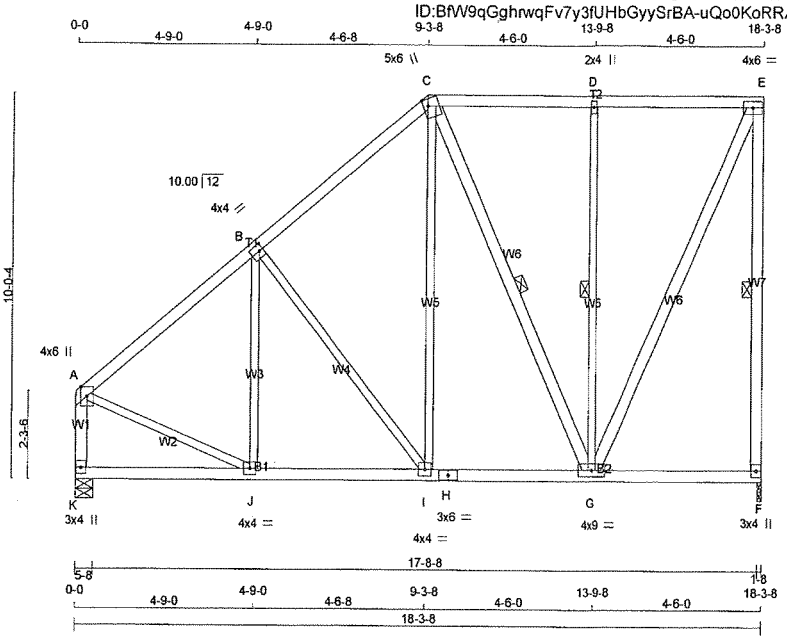

TOTAL WEIGHT = 2 X 111 = 223 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1072	0	1072	0	0	1-8	1-8
K	1072	0	1072	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0
K	819	382 / 0	192 / 0	0 / 0	0 / 0	245 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-868 / 0	-77.7	-77.7	0.22 (1)		6.25
B- C	-690 / 0	-77.7	-77.7	0.22 (1)		6.25
C- D	-403 / 0	-77.7	-77.7	0.20 (1)		6.25
D- E	-403 / 0	-77.7	-77.7	0.20 (1)		6.25
F- E	-999 / 0	0.0	0.0	0.49 (1)		6.25
K- A	-997 / 0	0.0	0.0	0.12 (1)		7.81
K- J	0 / 0	-39.5	-39.5	0.17 (3)		10.00
J- I	0 / 688	-39.5	-39.5	0.25 (2)		10.00
I- H	0 / 511	-39.5	-39.5	0.21 (2)		10.00
H- G	0 / 511	-39.5	-39.5	0.21 (2)		10.00
G- F	0 / 0	-39.5	-39.5	0.15 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
J- B	-70 / 114	0.04 (1)
B- I	-292 / 0	0.33 (1)
I- C	0 / 413	0.09 (2)
C- G	-258 / 0	0.16 (1)
G- D	-430 / 0	0.29 (1)
G- E	0 / 953	0.15 (1)
A- J	0 / 743	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.25/1.00 (I-J:2) , WB=0.33/1.00 (B-I:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.41 (A) (INPUT = 1.00)

DWG NO. TAM T1903899
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T87	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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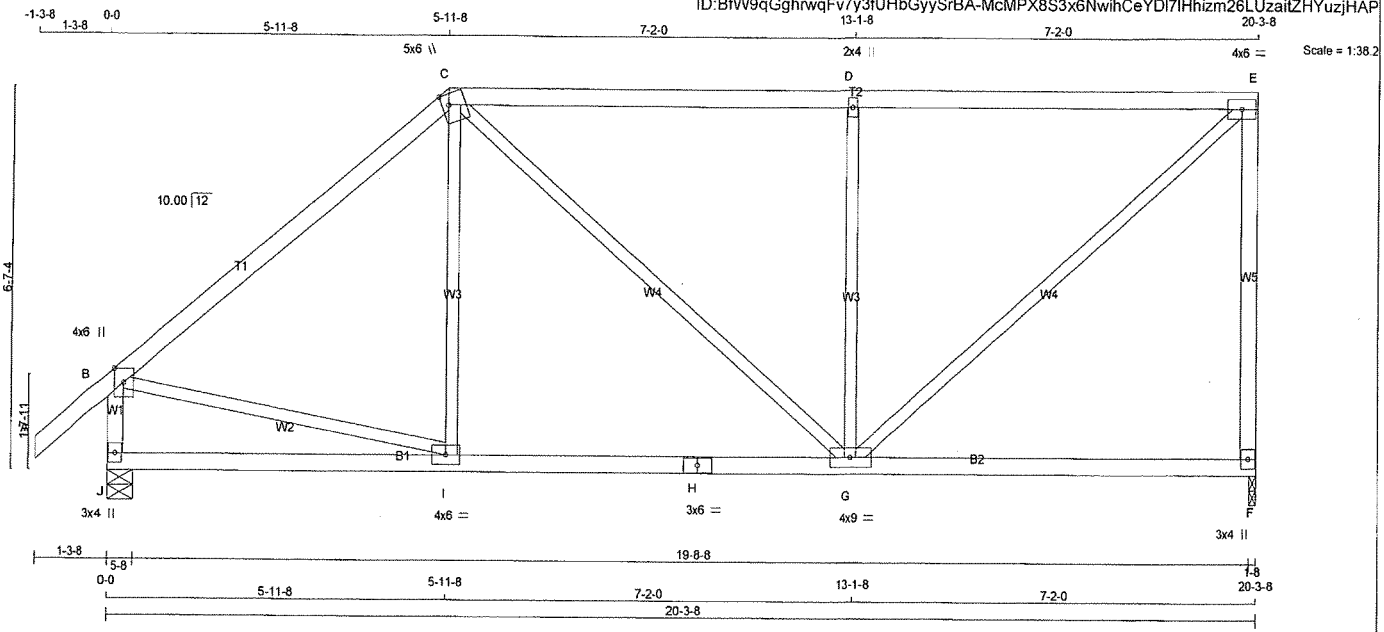

TOTAL WEIGHT = 88 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	1189	0	1189	0	0	1-8	1-8
J	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00	I- C	0 / 249	0.06 (3)	
B- C	-1070 / 0	-77.7	-77.7	0.57 (1)	5.32	C- G	0 / 216	0.05 (1)	
C- D	-984 / 0	-77.7	-77.7	0.75 (1)	4.97	G- D	-690 / 0	0.50 (1)	
D- E	-984 / 0	-77.7	-77.7	0.76 (1)	4.97	G- E	0 / 1309	0.29 (1)	
F- E	-1075 / 0	0.0	0.0	0.97 (1)	7.61	B- I	0 / 839	0.19 (1)	
J- B	-1203 / 0	0.0	0.0	0.13 (1)	7.29				
J- I	0 / 0	-39.5	-39.5	0.29 (3)	10.00				
I- H	0 / 822	-39.5	-39.5	0.47 (2)	10.00				
H- G	0 / 822	-39.5	-39.5	0.47 (2)	10.00				
G- F	0 / 0	-39.5	-39.5	0.38 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.97/1.00 (E-F:1) , BC=0.47/1.00 (G-I:2) , WB=0.50/1.00 (D-G:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.54 (B) (INPUT = 1.00)

DWG NO. T1903700
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	T88	1	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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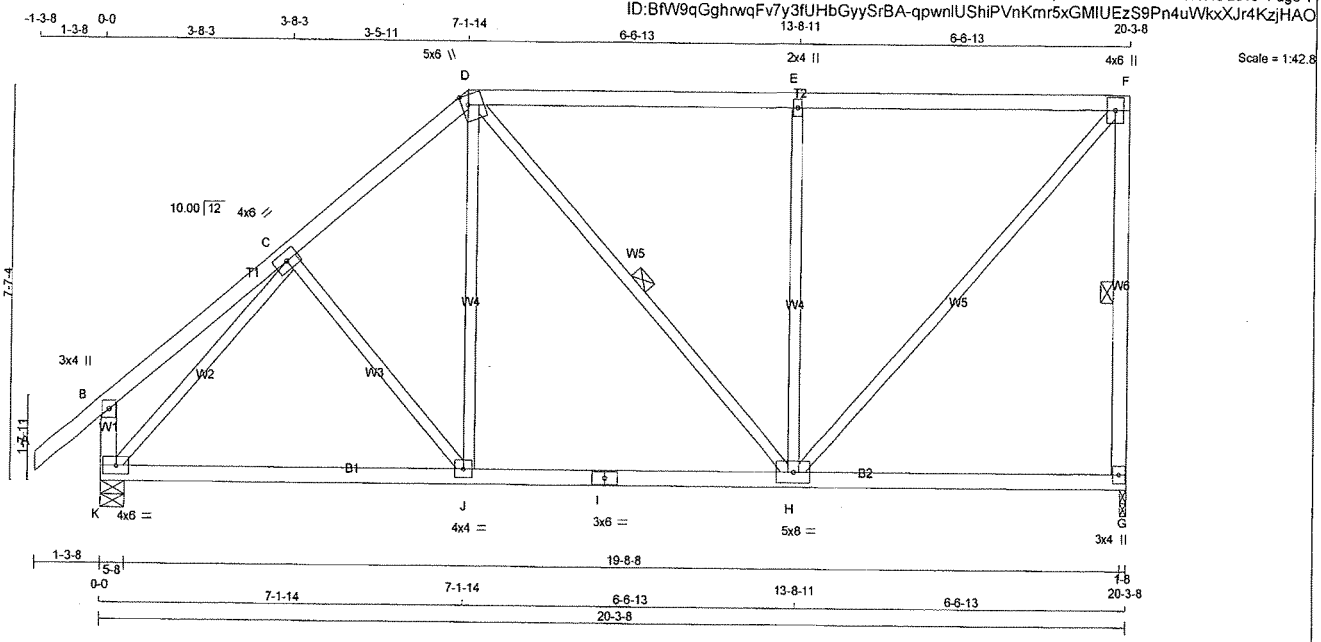


TOTAL WEIGHT = 96 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1189	0	1189	0	0	1-8	1-8
K	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

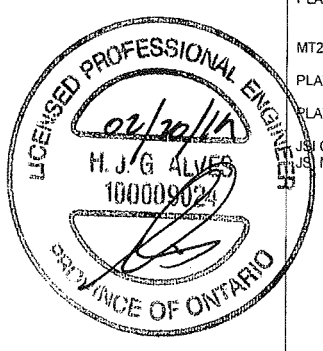
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	0 / 20	-77.7	-77.7	0.15 (1)	10.00
C- D	-1036 / 0	-77.7	-77.7	0.17 (1)	5.97
D- E	-801 / 0	-77.7	-77.7	0.61 (1)	5.73
E- F	-802 / 0	-77.7	-77.7	0.61 (1)	5.73
G- F	-1082 / 0	0.0	0.0	0.28 (1)	6.07
K- B	-215 / 0	0.0	0.0	0.02 (1)	7.81
K- J	0 / 803	-39.5	-39.5	0.44 (2)	10.00
J- I	0 / 784	-39.5	-39.5	0.45 (2)	10.00
I- H	0 / 784	-39.5	-39.5	0.45 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.29 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-37 / 65	0.02 (1)
J- D	0 / 376	0.08 (2)
D- H	-23 / 27	0.02 (3)
H- E	-632 / 0	0.67 (1)
H- F	0 / 1195	0.27 (1)
K- C	-1255 / 0	0.63 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.61/1.00 (E-F:1) , BC=0.45/1.00 (H-J:2) , WB=0.67/1.00 (E-H:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.36 (F) (INPUT = 1.00)

DWG NO. TAM T1903901
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T89	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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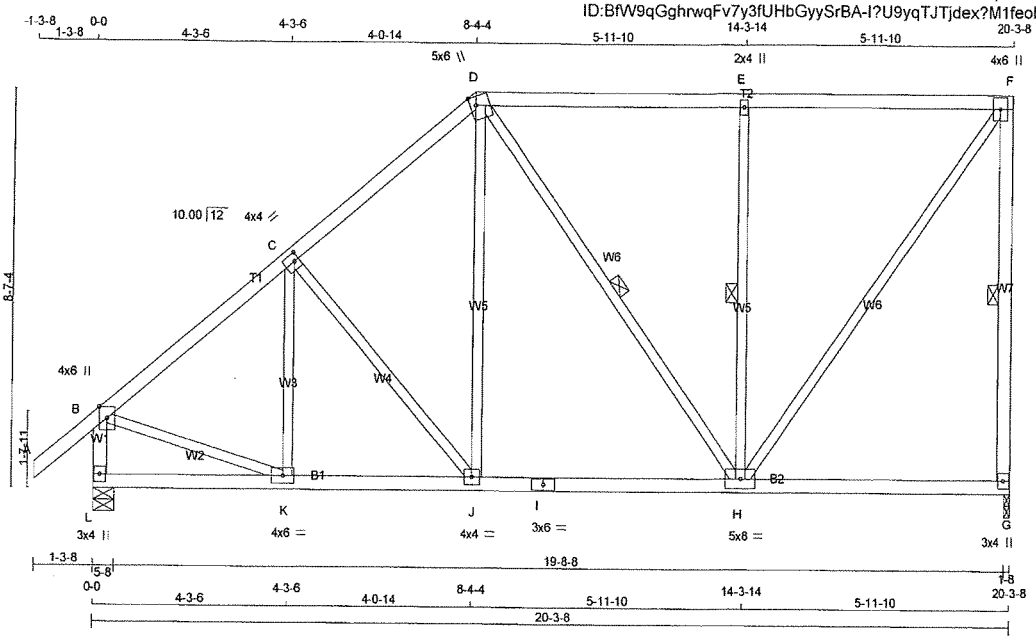


TOTAL WEIGHT = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1189	0	1189	0	0	IN-SX	IN-SX
L	1297	0	1297	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	909	424 / 0	213 / 0	0 / 0	0 / 0	272 / 0	0 / 0
L	984	482 / 0	213 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

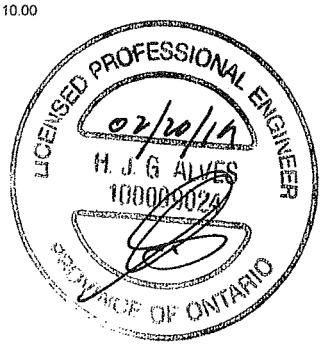
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-77.7	-77.7	0.11 (1)	10.00
B- C	-1108 / 0	-77.7	-77.7	0.25 (1)	5.71
C- D	-952 / 0	-77.7	-77.7	0.24 (1)	6.05
D- E	-663 / 0	-77.7	-77.7	0.48 (1)	6.25
E- F	-663 / 0	-77.7	-77.7	0.48 (1)	6.25
G- F	-1095 / 0	0.0	0.0	0.37 (1)	6.04
L- B	-1226 / 0	0.0	0.0	0.13 (1)	7.24
L- K	0 / 0	-39.5	-39.5	0.12 (3)	10.00
K- J	0 / 869	-39.5	-39.5	0.25 (2)	10.00
J- I	0 / 715	-39.5	-39.5	0.36 (2)	10.00
I- H	0 / 715	-39.5	-39.5	0.36 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.28 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-87 / 82	0.04 (1)
C- J	-244 / 0	0.18 (1)
J- D	0 / 401	0.09 (2)
D- H	-90 / 0	0.07 (1)
H- E	-573 / 0	0.27 (1)
H- F	0 / 1132	0.25 (1)
B- K	0 / 909	0.20 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.48/1.00 (E-F:1) , BC=0.36/1.00 (H-J:2) , WB=0.27/1.00 (E-H:1) , SSI=0.23/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

DWG NO. TAM 17903702
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T90	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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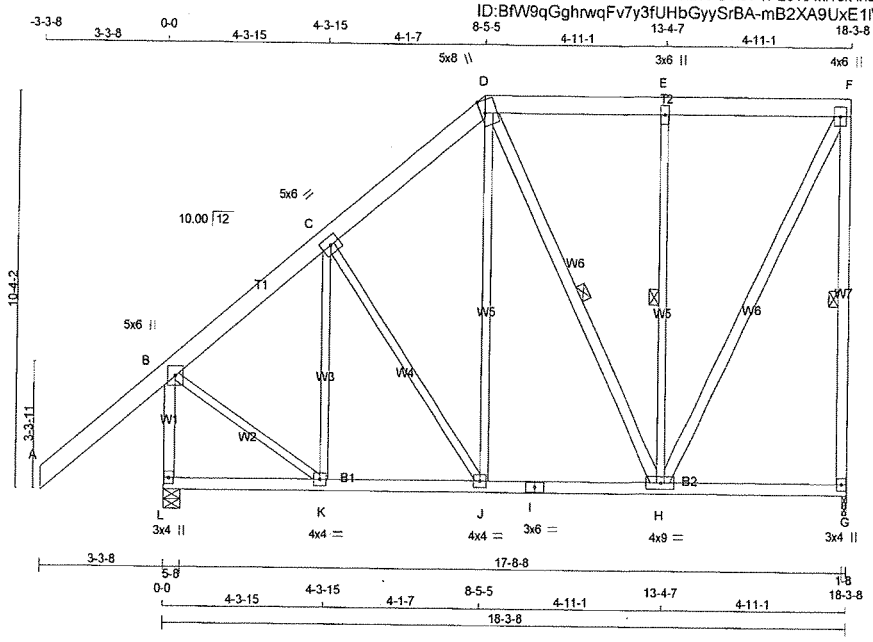

TOTAL WEIGHT = 2 X 135 = 270 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	8.0	4.00	1.25
E	TMW+w	MT20	3.0	6.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1047	0	1047	0	0	1-8	1-8
L	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	802	369 / 0	192 / 0	0 / 0	0 / 0	241 / 0	0 / 0
L	1022	540 / 0	192 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 86	-77.7	-77.7	0.32 (1)	10.00
B- C	-726 / 0	-77.7	-77.7	0.31 (1)	6.25
C- D	-619 / 0	-77.7	-77.7	0.10 (1)	6.25
D- E	-406 / 0	-77.7	-77.7	0.13 (1)	6.25
E- F	-406 / 0	-77.7	-77.7	0.13 (1)	6.25
G- F	-968 / 0	0.0	0.0	0.49 (1)	6.25
L- B	-1295 / 0	0.0	0.0	0.22 (1)	7.10
L- K	0 / 0	-39.5	-39.5	0.14 (3)	10.00
K- J	0 / 510	-39.5	-39.5	0.20 (2)	10.00
J- I	0 / 472	-39.5	-39.5	0.24 (2)	10.00
I- H	0 / 472	-39.5	-39.5	0.24 (2)	10.00
H- G	0 / 0	-39.5	-39.5	0.19 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-148 / 38	0.11 (1)
C- J	-78 / 0	0.09 (2)
J- D	0 / 273	0.06 (3)
D- H	-150 / 0	0.10 (1)
H- E	-459 / 0	0.33 (1)
H- F	0 / 910	0.15 (1)
B- K	0 / 611	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.49/1.00 (F-G:1) , BC=0.24/1.00 (H-J:2) , WB=0.33/1.00 (E-H:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)

DWG NO. TAM T1903903
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T91TCX	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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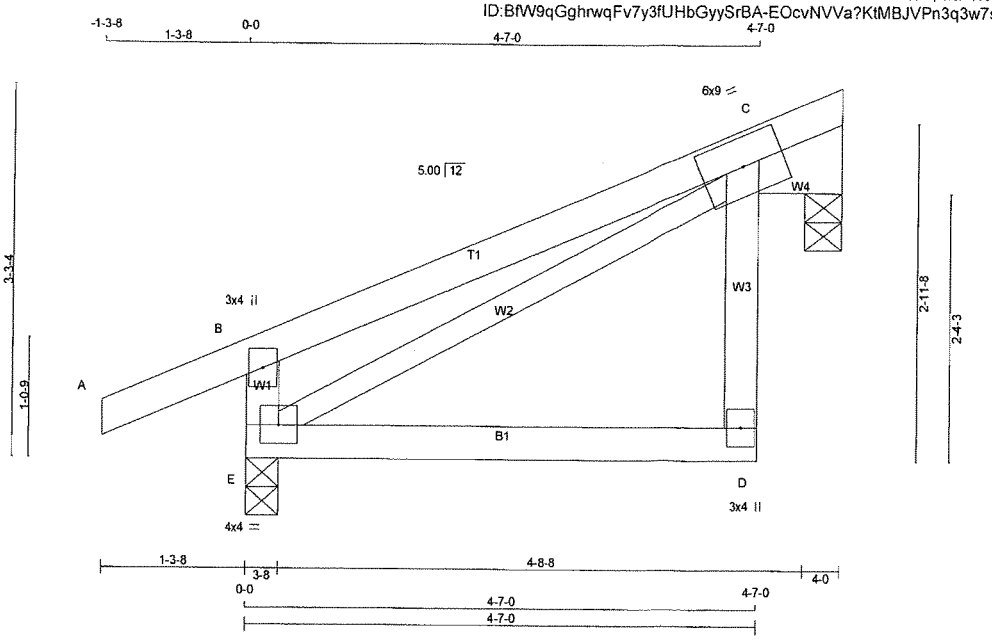

TOTAL WEIGHT = 6 X 22 = 134 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
C	269	0	269	0	0	4-0	4-0
E	373	0	373	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	205	96 / 0	48 / 0	0 / 0	0 / 0	61 / 0	0 / 0
E	278	152 / 0	48 / 0	0 / 0	0 / 0	78 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-77.7	-77.7	0.10 (1)	10.00
B- C	0 / 0	-77.7	-77.7	0.28 (1)	10.00
D- C	0 / 115	0.0	0.0	0.02 (3)	10.00
E- B	-283 / 0	0.0	0.0	0.03 (1)	7.81
E- D	0 / 0	-39.5	-39.5	0.19 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.19/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM T9039 04
STRUCTURAL
COMPONENT

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. UNITS 28 - 31	DRWG NO.
401437	T92TCX	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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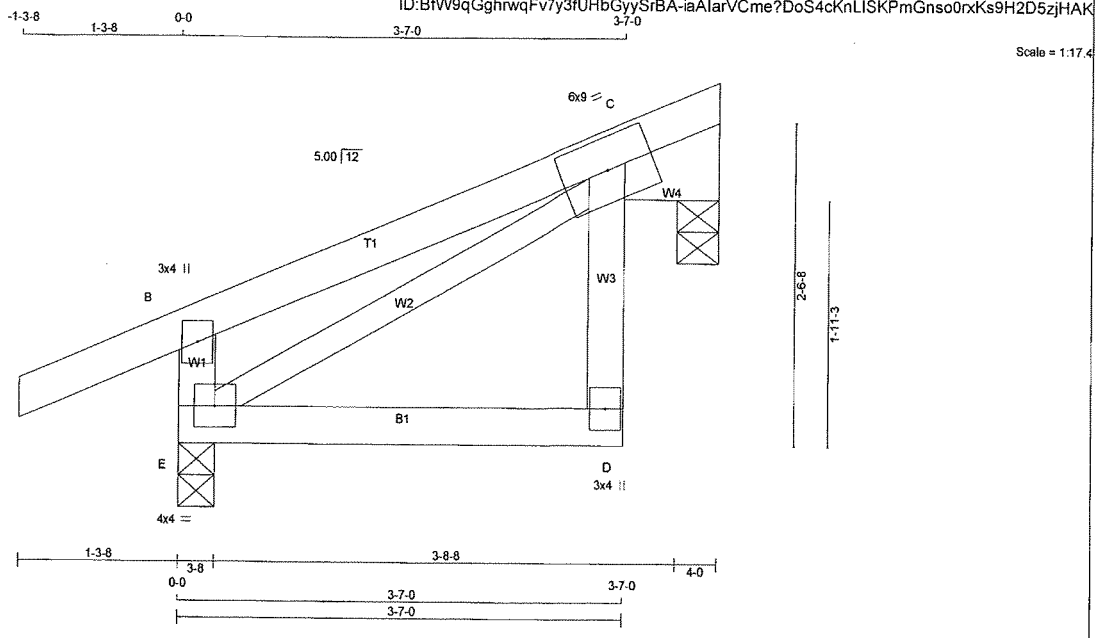


TOTAL WEIGHT = 8 X 19 = 149 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	210	0	210	0	0	4-0	4-0
E	315	0	315	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	161	75 / 0	38 / 0	0 / 0	0 / 0	48 / 0	0 / 0
E	233	131 / 0	38 / 0	0 / 0	0 / 0	64 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 20	-77.7	-77.7	0.10 (1)	10.00	E- C	0 / 0	0.00 (1)
B- C	0 / 0	-77.7	-77.7	0.17 (1)	10.00			
D- C	0 / 90	0.0	0.0	0.02 (3)	10.00			
E- B	-244 / 0	0.0	0.0	0.02 (1)	7.81			
E- D	0 / 0	-39.5	-39.5	0.12 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.12/1.00 (D-E:3) , WB=0.00/1.00 (C-E:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

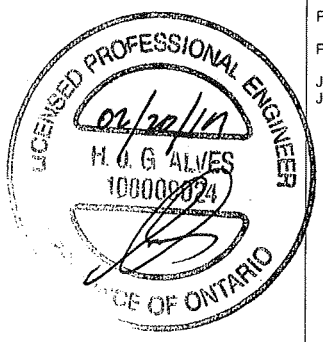
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAM T1903905
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401437	PB1	1	1	UNITS 28 - 31 TRUSS DESC.	

Tamarack Roof Truss, Burlington

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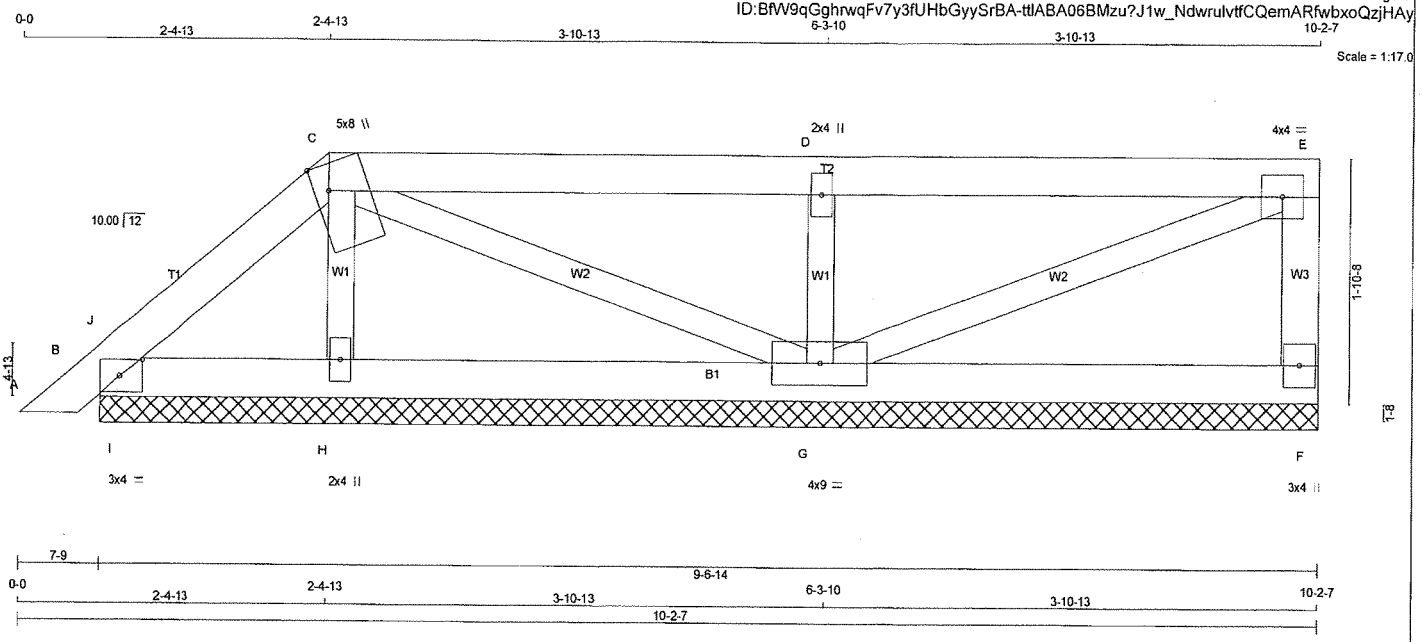


TOTAL WEIGHT = 33 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW1-t	MT20	4.0	9.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	177	0	177	0	0	9-6-14	9-6-14
B	158	0	158	0	0	9-6-14	9-6-14
H	263	0	263	0	0	9-6-14	9-6-14
G	566	0	566	0	0	9-6-14	9-6-14

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	135	62 / 0	33 / 0	0 / 0	0 / 0	41 / 0	0 / 0
B	114	74 / 0	11 / 0	0 / 0	0 / 0	29 / 0	0 / 0
H	207	79 / 0	62 / 0	0 / 0	0 / 0	66 / 0	0 / 0
G	430	208 / 0	95 / 0	0 / 0	0 / 0	127 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, B, H, G

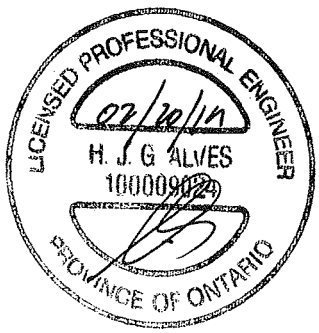
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-77.7	-77.7	0.02 (1)	10.00
B- J	-31 / 0	-77.7	-77.7	0.01 (1)	6.25
J- C	-44 / 0	-77.7	-77.7	0.03 (1)	6.25
C- D	0 / 1	-77.7	-77.7	0.20 (1)	10.00
D- E	0 / 1	-77.7	-77.7	0.20 (1)	10.00
F- E	-115 / 0	0.0	0.0	0.01 (1)	7.81
B- I	0 / 32	-39.5	-39.5	0.03 (1)	10.00
I- H	0 / 32	-39.5	-39.5	0.06 (2)	10.00
H- G	0 / 23	-39.5	-39.5	0.12 (3)	10.00
G- F	0 / 0	-39.5	-39.5	0.12 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-131 / 0	0.02 (1)
C- G	-26 / 0	0.01 (1)
G- D	-378 / 0	0.06 (1)
G- E	-1 / 0	0.00 (1)
I- J	-86 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	20.9	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	44.8	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(60 % OF 20.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.9 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.20/1.00 (C-D:1) , BC=0.12/1.00 (G-H:3) , WB=0.06/1.00 (D-G:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.08 (D) (INPUT = 1.00)

DWG NO. TAM T1903906
STRUCTURAL
COMPONENT ONLY

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

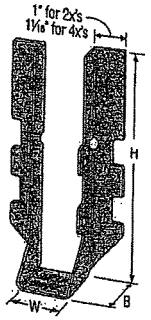
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

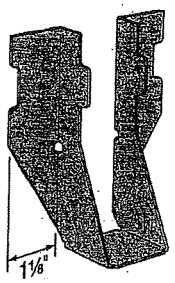
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.

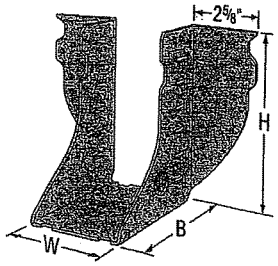


LUS28

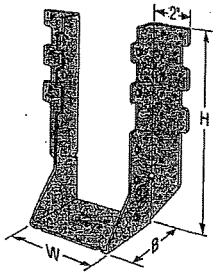


LU26L

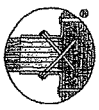
HUS210
(HUS26, HUS28, and HHUS similar)



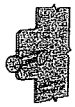
HGUS28-2



HHUS210-2



Double-Shear Nailing Top View

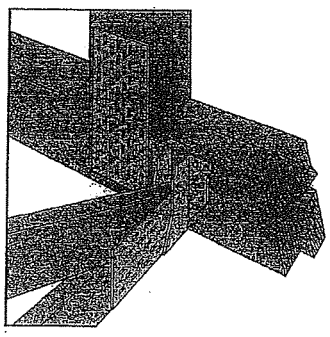


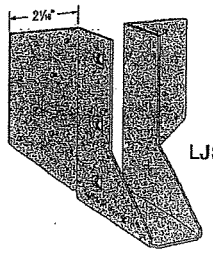
Double-Shear Nailing Side View; Do not bend tab

Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580

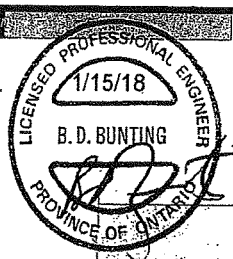
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)





LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS



SIMPSON Strong-Tie®

HHUS/HGUS

See Hanger Options information on pp. 125–127.

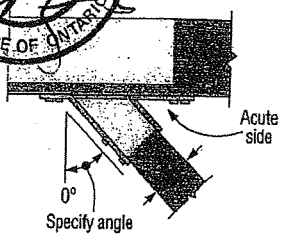
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb.	kN	Normal (K_D = 1.00) lb.	kN	S-P-F Uplift (K_D = 1.15) lb.	kN	Normal (K_D = 1.00) lb.	kN
Single 2x Sizes															
LUS24	18	1⅜	3⅛	1¾	2¼	(4) 10d	(2) 10d	710	3.16	1625	7.23	645	2.87	1155	5.14
LU24L	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360	1.60	1020	4.54	320	1.42	725	3.22
LU26L	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720	3.20	1605	7.14	645	2.87	1140	5.07
SS LUS26	18	1⅜	4¾	1¾	3¾	(4) 10d	(4) 10d	1420	6.32	2170	9.65	1290	5.74	1630	7.25
HUS26	16	1⅝	5⅜	3	3⅛	(14) 16d	(6) 16d	2705	12.03	4940	21.97	2065	9.20	3875	17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055	9.14	4265	18.97	1460	6.49	4115	18.31
HGUS26	12	1⅝	5⅜	5	4⅛	(20) 16d	(8) 16d	2685	11.95	6625	29.47	2685	11.96	5700	25.35
LU28L	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140	5.07	2185	9.72	1020	4.54	1550	6.89
SS LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420	6.32	2520	11.21	1290	5.74	1790	7.96
HUS28	16	1⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3605	16.04	5365	23.86	2675	11.90	4345	19.33
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	14.73	7675	34.15	3310	14.74	6900	30.73
LU210L	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140	5.07	2495	11.10	1020	4.54	1770	7.87
SS LUS210	18	1⅜	7 13⁄16	1¾	3⅞	(8) 10d	(4) 10d	1420	6.32	2785	12.39	1290	5.74	2210	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

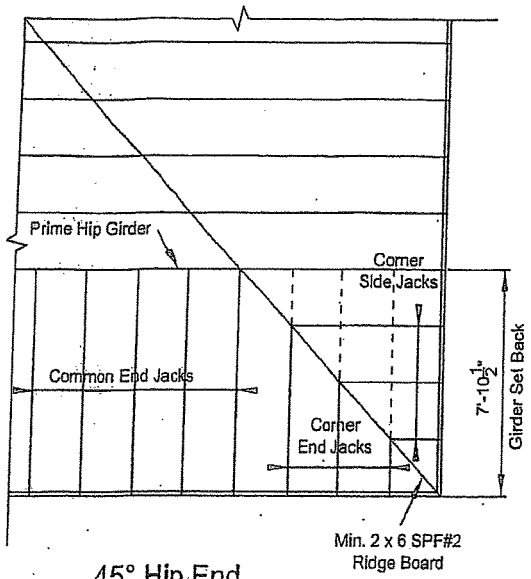
SIMPSON Strong-Tie®

■ These products are available with additional corrosion protection. For more information, see p. 24.

◤ These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_B[3]	Fasteners Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Double 2x Sizes											
■ LUS24-2	18	3⅛	31⁄16	2	1½	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.99	590 / 2.62	1435 / 6.38
SS ■ LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS26-2	14	3⁹⁄16	5⅜	3	3¹⁵⁄16	(14) 16d	(6) 16d	2850 / 12.68	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS26-2	12	3⁹⁄16	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
SS ■ LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS28-2	14	3⁹⁄16	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS28-2	12	3⁹⁄16	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
SS ■ LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
■ HHUS210-2	14	3⁹⁄16	9⅛	3	8	(30) 16d	(10) 16d	4670 / 20.77	9660 / 42.97	4235 / 18.84	7000 / 31.14
HGUS210-2	12	3⁹⁄16	9⅛	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
Triple 2x Sizes											
HGUS26-3	12	4¹⁵⁄16	5½	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-3	12	4¹⁵⁄16	7¼	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-3	14	4¹⁵⁄16	9	3	7¹⁵⁄16	(30) 16d	(10) 16d	4670 / 20.77	9670 / 43.02	4235 / 18.84	6865 / 30.54
HGUS210-3	12	4¹⁵⁄16	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⁹⁄16	5⅞	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
HGUS28-4	12	6⁹⁄16	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ HHUS210-4	14	6⅛	8⅞	3	7¹⁵⁄16	(30) 16d	(10) 16d	4670 / 20.77	10155 / 45.17	4235 / 18.84	7210 / 32.07
HGUS210-4	12	6⁹⁄16	9¼	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14645 / 65.14	4855 / 21.60	10400 / 46.26
HGUS212-4	12	6⁹⁄16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS214-4	12	6⁹⁄16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80
4x Sizes											
■ LUS46	18	3⁹⁄16	4¾	2	3⅞	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54
■ HHUS46	14	3⅝	5¼	3	3¹⁵⁄16	(14) 16d	(6) 16d	2540 / 11.30	7335 / 32.63	2065 / 9.20	5205 / 23.15
HGUS46	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385 / 19.51	8950 / 39.81	3110 / 13.83	6355 / 28.27
■ LUS48	18	3⁹⁄16	6¾	2	3⅞	(6) 16d	(4) 16d	1720 / 7.65	3325 / 14.79	1545 / 6.87	2575 / 11.45
■ HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765 / 16.75	8940 / 39.77	2675 / 11.90	6345 / 28.22
HGUS48	12	3⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070 / 27.00	12980 / 57.74	4310 / 19.17	9215 / 40.99
■ LUS410	18	3⁹⁄16	8¾	2	5⁹⁄16	(8) 16d	(6) 16d	2580 / 11.48	4500 / 20.02	2320 / 10.32	3195 / 14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840 / 30.43	14015 / 62.34	4855 / 21.60	10270 / 45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640 / 33.98	14995 / 66.70	5425 / 24.13	10645 / 47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130 / 45.06	16400 / 72.95	7195 / 32.00	11645 / 51.80

See footnotes on p. 258.



45° Hip End

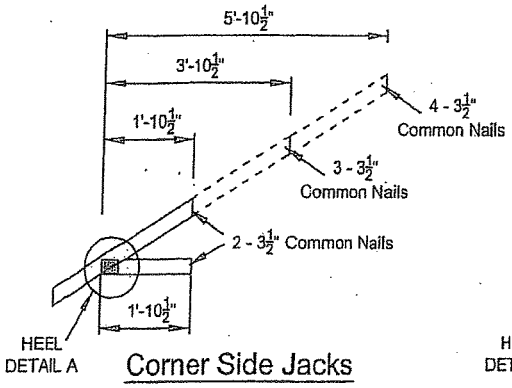
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

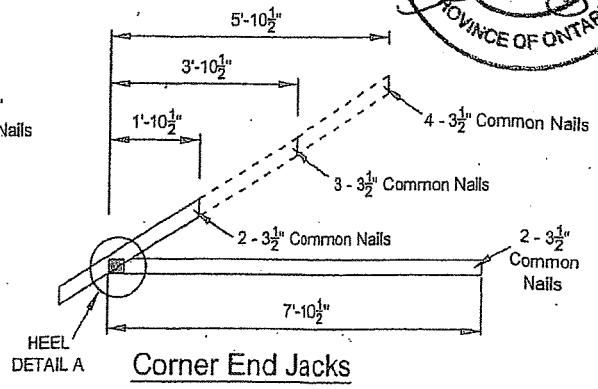
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

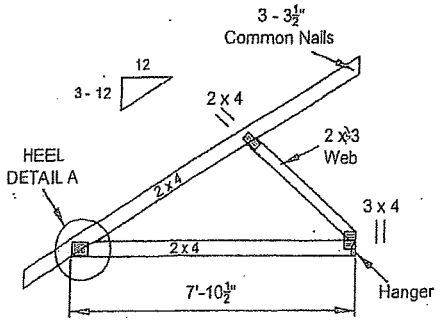
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



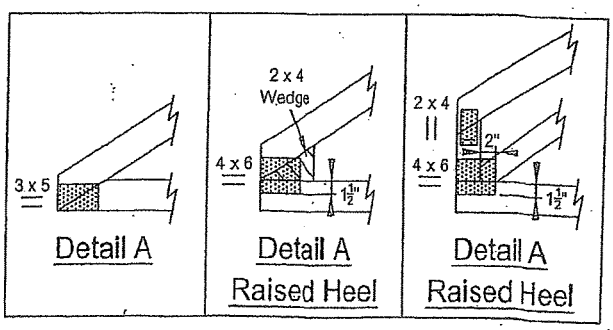
Corner Side Jacks



Corner End Jacks

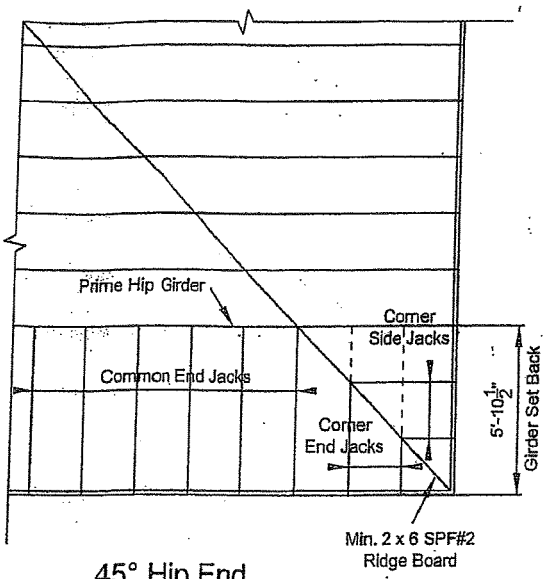


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



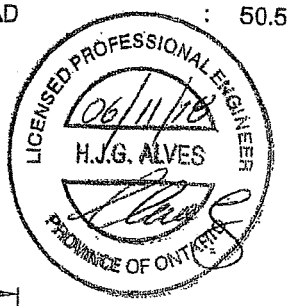
45° Hip End

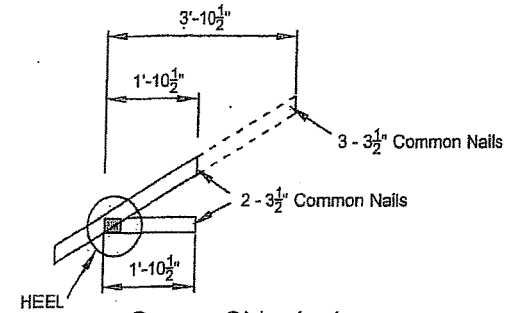
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

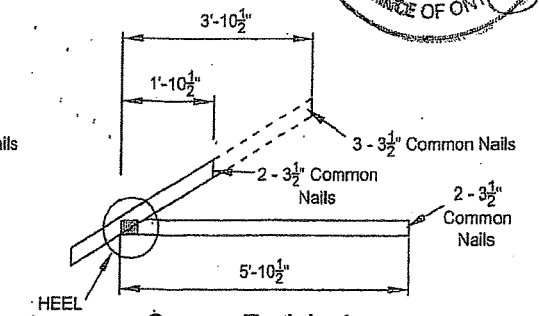
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

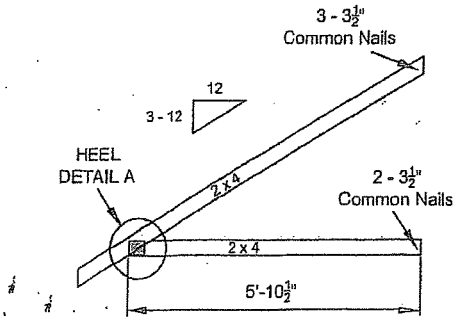




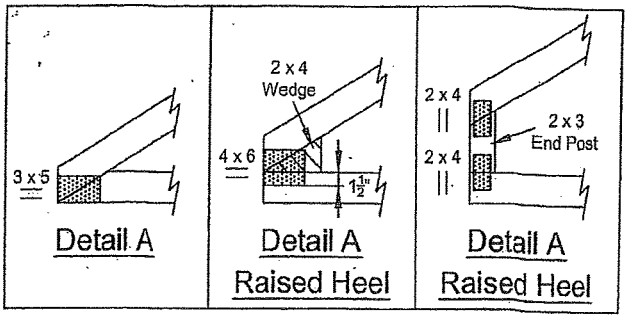
Corner Side Jacks



Corner End Jacks



Common End Jacks

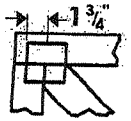


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

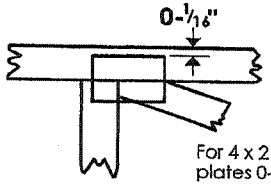
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

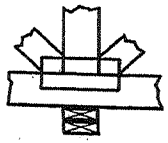
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

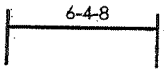
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

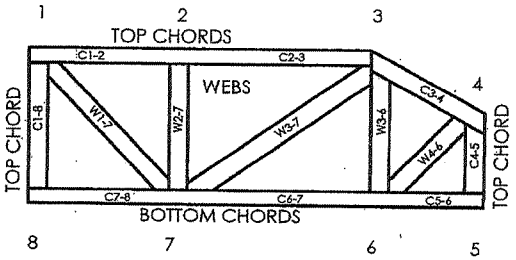
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

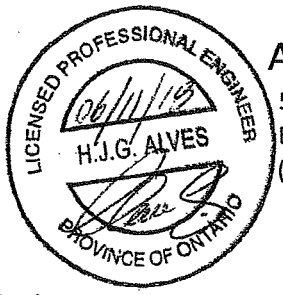
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mii-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
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(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018