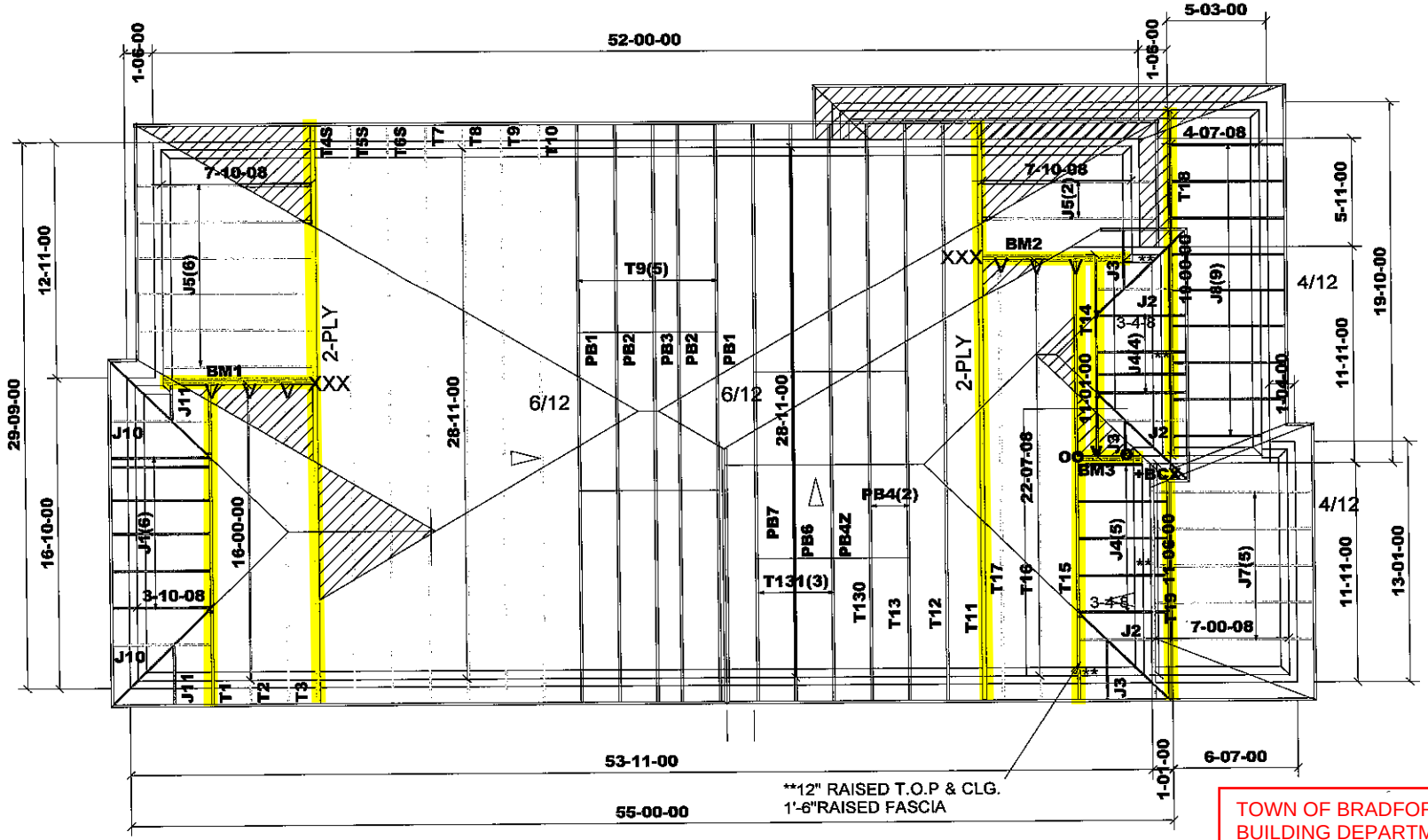
T-180101

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

BM1, 2: 3-2X10



**12" RAISED T.O.P & CLG.
1'-6"RAISED FASCIA

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-3(XXX)

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-10-26
INSPECTOR: BG

SITE COPY

M11,448

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287531
Plan Log: 96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

Date: 2/28/2018 Designer: JG

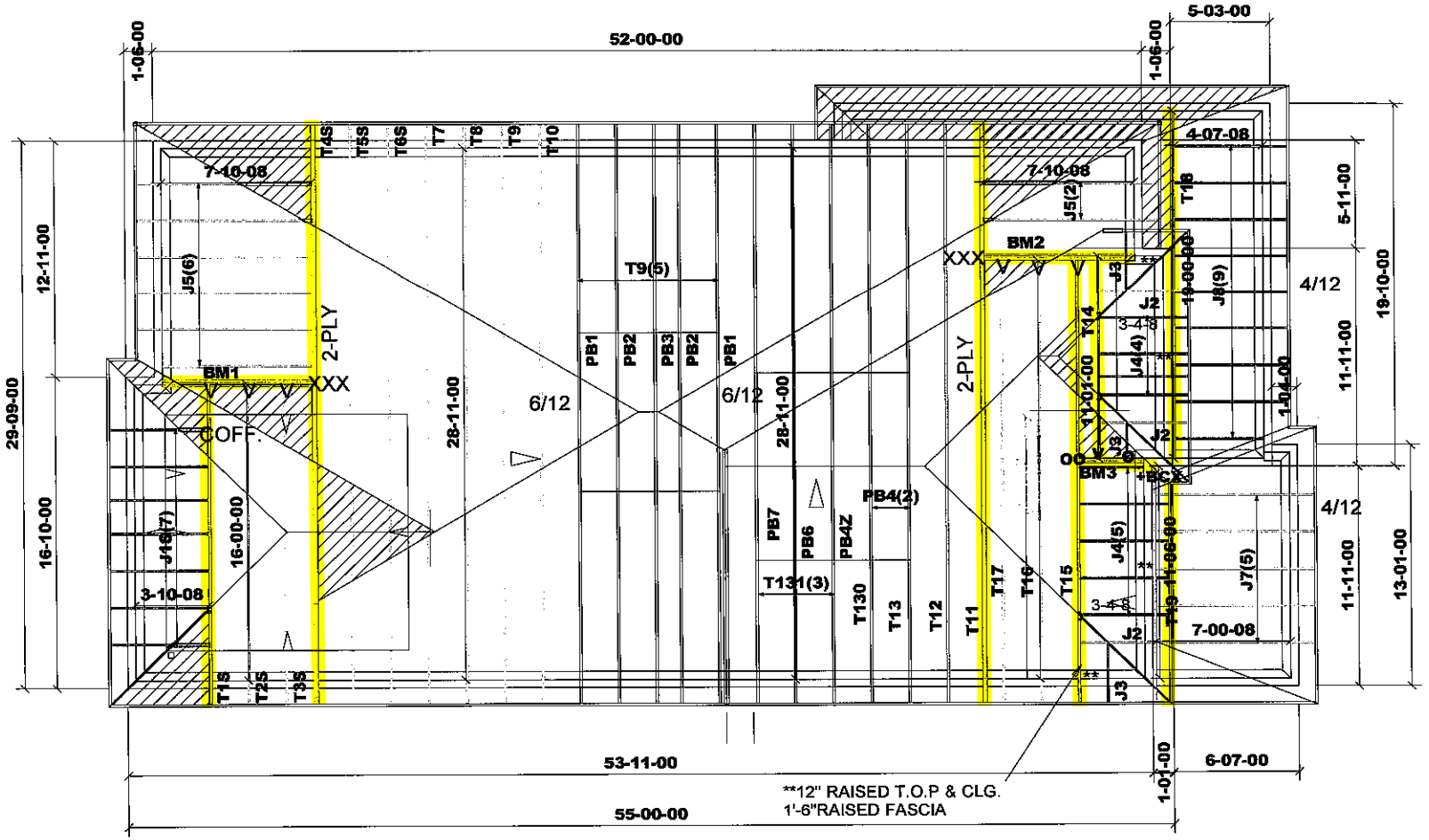
Model / Elevation:
S38-6 / A

Mitek ver 7.5.0

T-180101

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

BM1, 2: 3-2X10



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
LUS24-2(OO)
HGUS26-3(XXX)

SITE COPY

M11,448

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: **44755**
Layout ID: **287525**
Plan Log: **96660**

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2/28/2018** Designer: JG

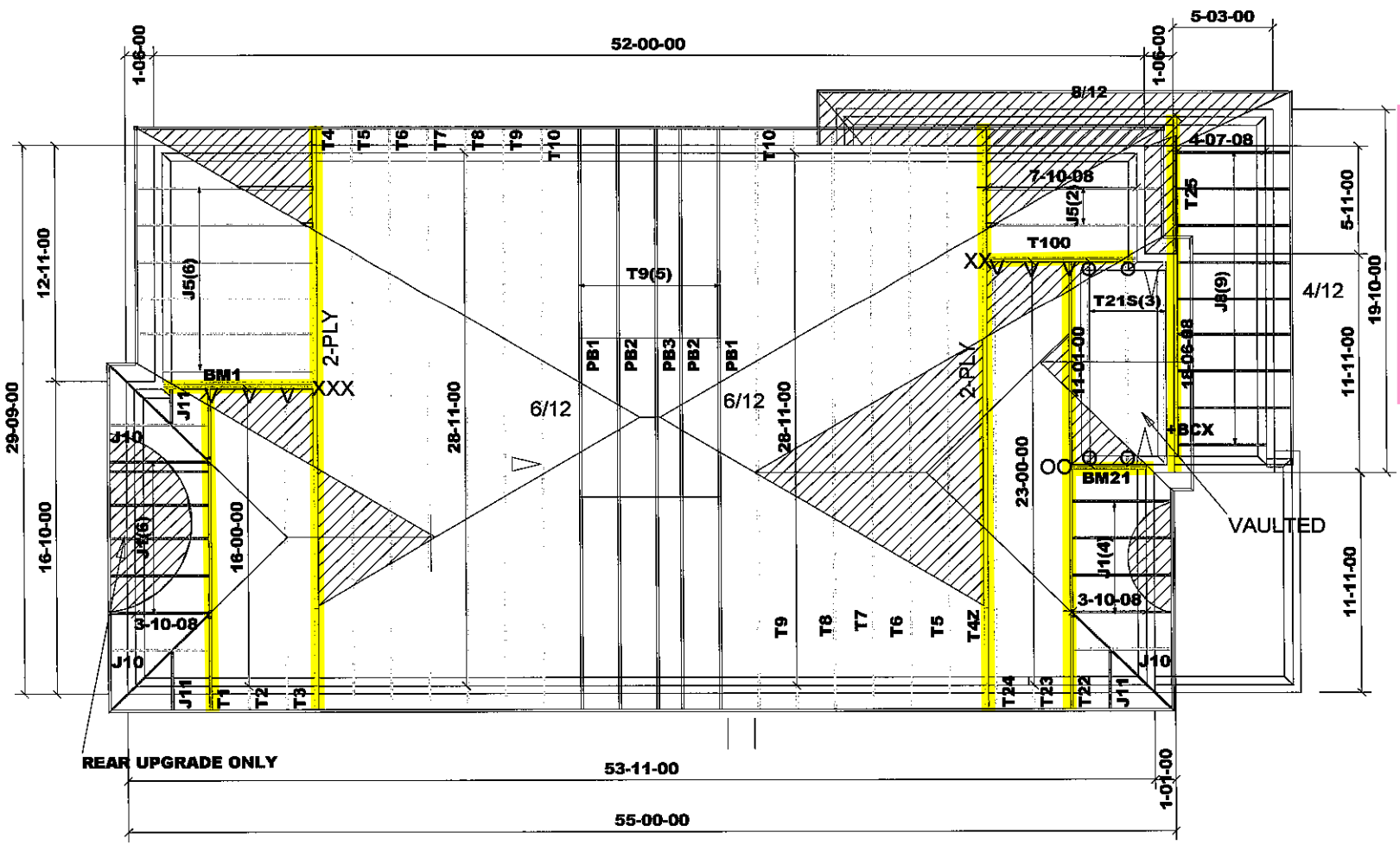
Model / Elevation:
S38-6 / A-OPT.COFF.

Mitek ver 7.5.0

T-180101

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

BM1: 2-2X10



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
HGUS26-3(XXX)

SITE COPY

M11,448

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: **44755**
Layout ID: **287532**
Plan Log: **96660**

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **2/28/2018** Designer: JG

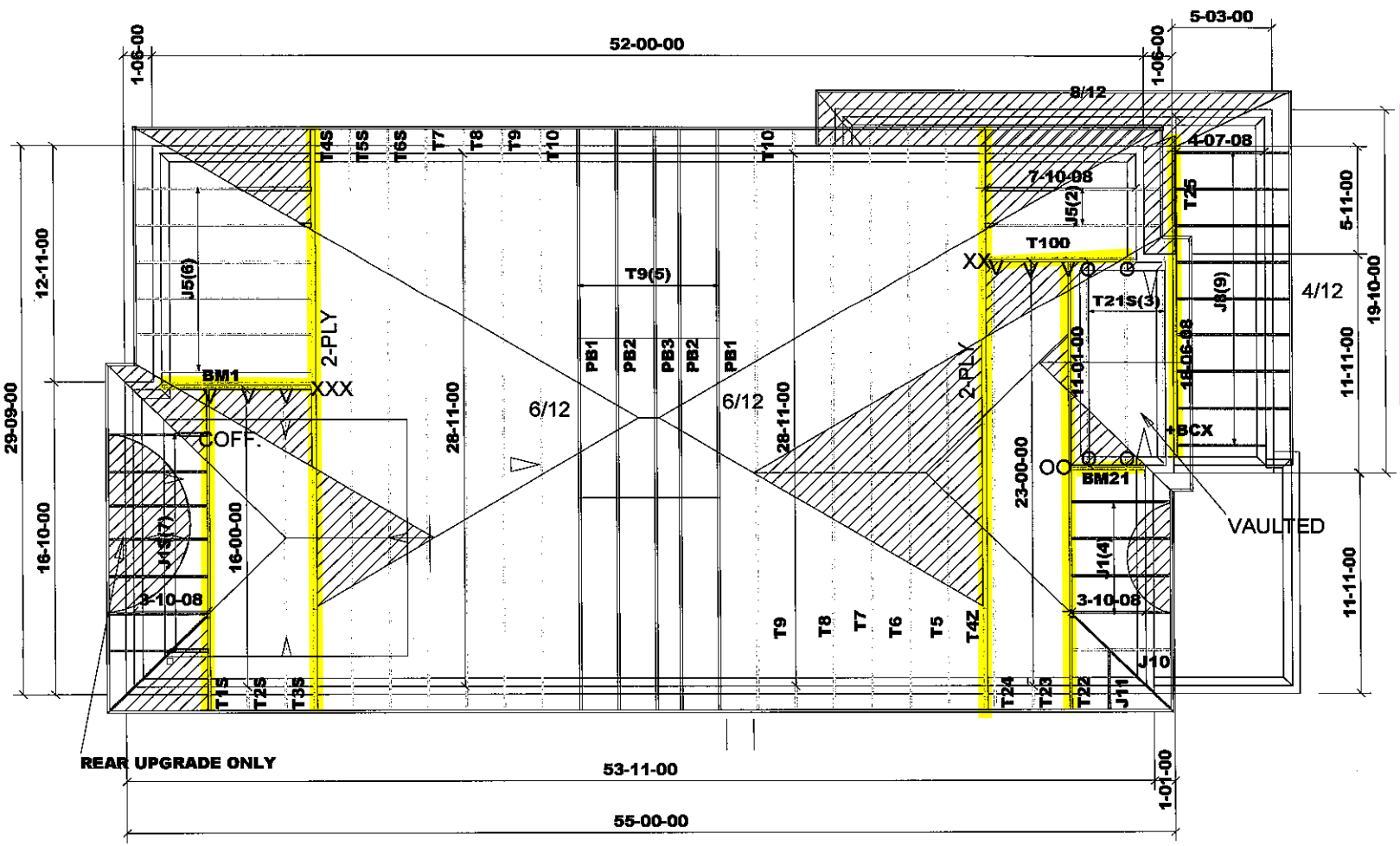
Model / Elevation:
S38-6 / B

Mitek ver 7.5.0

T-180101

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

BM1: 2-2X10



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
HGUS26-3 (XXX)

SITE COPY

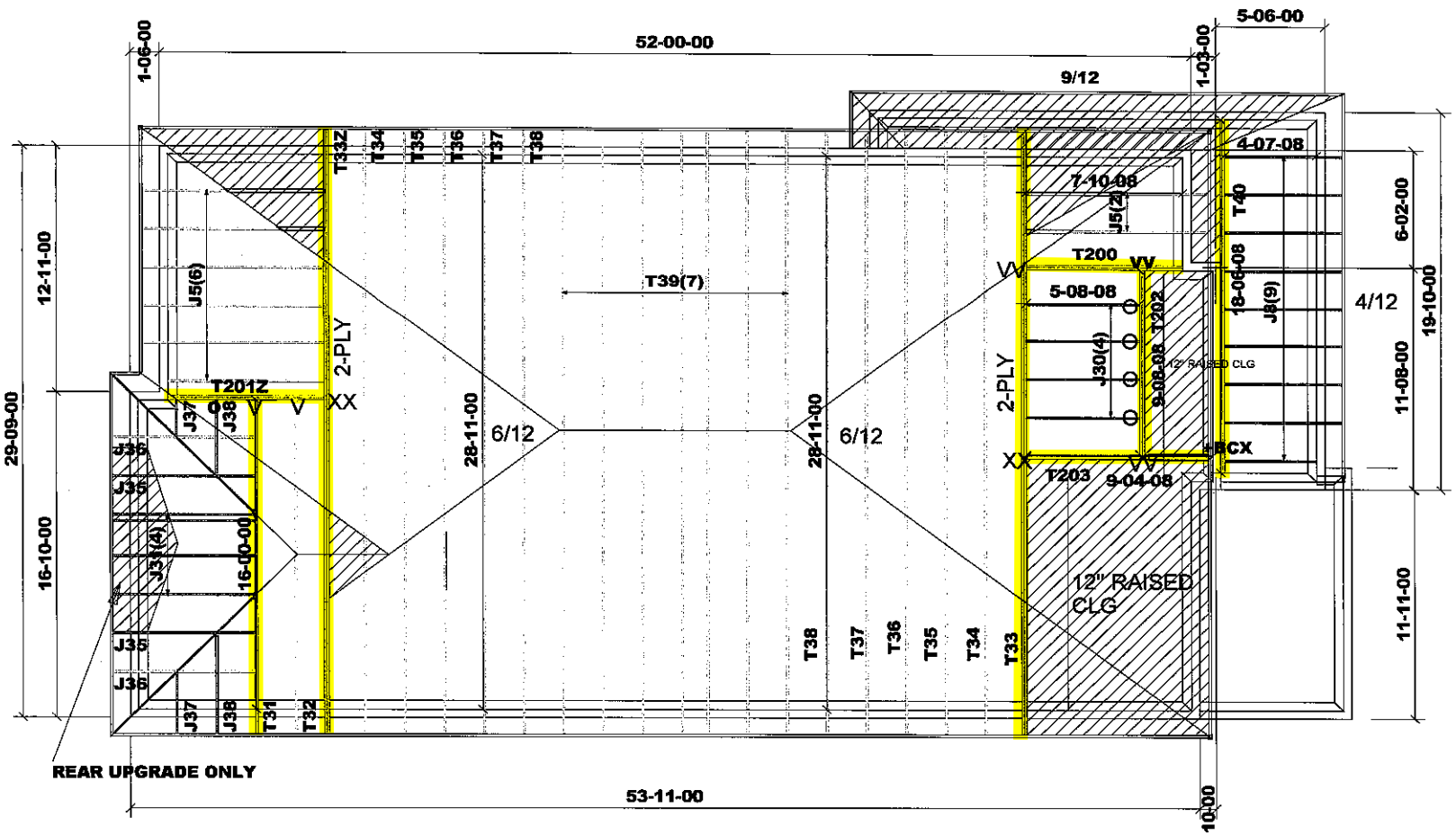
M11,448

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-6 / B-OPT.COFF
	Layout ID: 287526	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 2/28/2018 Designer: JG	

Mitek ver 7.5.0

T-180101

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
LUS26-2(VV)

SITE COPY

M11,448

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287533
Plan Log: 96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

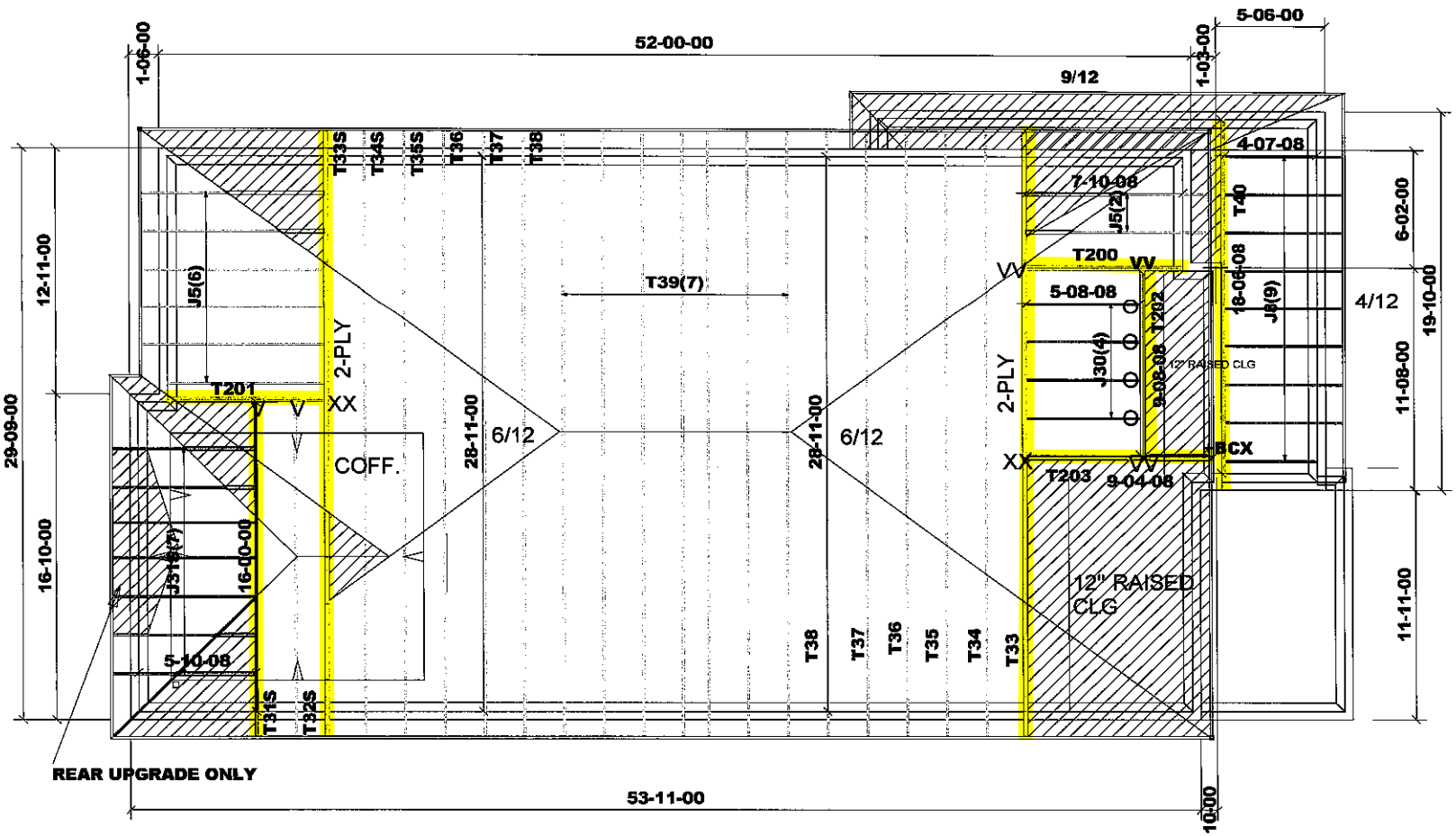
Date: 3/1/2018 Designer: JG

Model / Elevation:
S38-6 / C

Mitek ver 7.5.0

T-180101

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)
LUS26-2(VV)

SITE COPY

M11,448

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755
Layout ID: 287529
Plan Log: 96660

Builder / Location:
BAYVIEW WELLINGTON HOMES / BRADFORD

Project: GREEN VALLEY EAST

Date: 2/28/2018 Designer: JG

Model / Elevation:
S38-6 / C-OPT.COFF

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

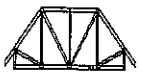
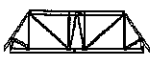
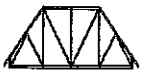
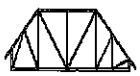
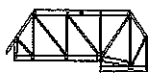
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287531 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1 HIP GIRDER	10.00 0.00	16-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	72.29 46.83		
	1	T2 HIP	10.00 0.00	16-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.58 50.67		
	1	T3 HIP	10.00 0.00	16-00-00	08-02-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 01-07-11	71.93 46.00		
	1 2 Ply	T4 HIP GIRDER	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	304.40 191.34		
	1	T5 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.81 76.67		
	1	T6 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.78 84.83		
	1	T7 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	131.25 82.67		
	1	T8 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	141.94 89.17		
	6	T9 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	933.12 588.00		
	1	T10 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	161.37 99.67		
	1 2 Ply	T11 ROOF	10.00 0.00	28-11-00	10-00-10	2 X 4	2 X 6	01-03-08 01-03-08	03-01-11 01-07-11	337.66 211.66		
	1	T12 ROOF	10.00 0.00	28-11-00	11-08-10	2 X 4	2 X 4	01-03-08 01-03-08	03-01-11 01-07-11	159.36 100.83		
	1	T13 PIGGYBACK	10.00 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	03-01-11 01-07-11	149.26 94.50		
	1	T130 PIGGYBACK	10.00 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	03-01-11 01-07-11	152.73 97.00		
	3	T131 PIGGYBACK	10.00 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 01-03-08	03-01-11 01-07-11	434.25 270.99		
	1	T14 HIP GIRDER	10.00 0.00	11-01-00	04-11-07	2 X 4	2 X 4	00-00-00 00-00-00	02-01-11 02-01-11	49.70 32.67		
	1	T15 HIP GIRDER	10.00 0.00	22-07-08	05-11-07	2 X 4	2 X 6	01-03-08 00-00-00	02-01-11 03-05-07	135.10 86.50		
	1	T16 HIP	10.00 0.00	22-07-08	07-07-07	2 X 4	2 X 4	01-03-08 00-00-00	02-01-11 03-05-07	116.22 75.17		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

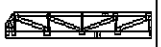
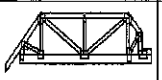
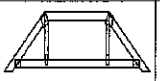
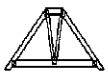
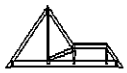
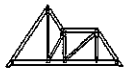
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287531 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-6 **ELEVATION:** A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T17 HIP	10.00 0.00	22-07-08	09-03-07	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-05-07	123.68 78.17		
	1	T18 HALF HIP	10.00 0.00	19-00-00	01-10-07	2 X 4	2 X 6	00-00-00 00-00-00	01-00-07 01-10-07	79.97 52.50		
	1	T19 HIP GIRDER	10.00 0.00	11-06-00	02-08-02	2 X 4	2 X 6	01-03-08 00-00-00	01-00-07 00-05-08	56.29 37.67		
	2	PB1 PIGGYBACK	10.00 0.00	07-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.88 30.66		
	2	PB2 PIGGYBACK	10.00 0.00	07-04-02	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	49.10 32.66		
	1	PB3 PIGGYBACK	10.00 0.00	07-04-02	03-05-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	23.41 14.83		
	2	PB4 PIGGYBACK	10.00 0.00	08-10-08	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	55.92 34.66		
	1	PB4Z PIGGYBACK	10.00 0.00	08-10-13	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-09	27.73 17.33		
	1	PB6 PIGGYBACK	10.00 0.00	12-11-15	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	41.65 27.00		
	1	PB7 PIGGYBACK	10.00 0.00	12-11-15	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	46.20 30.00		
	6	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	100.98 64.02		
	3	J2 JACK-OPEN	10.00 0.00	01-09-07	03-07-09	2 X 4	2 X 4	01-03-08 01-07-01	02-01-11 00-03-08	37.02 26.49		
	3	J3 JACK-OPEN	10.00 0.00	01-09-07	03-07-09	2 X 4	2 X 4	01-03-08 00-01-01	02-01-11 00-03-08	31.89 22.50		
	9	J4 JACK-OPEN	10.00 0.00	03-04-08	04-11-07	2 X 4	2 X 4	01-03-08 00-00-00	02-01-11 04-11-07	135.54 91.53		
	8	J5 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	241.76 154.64		
	5	J7 JACK-PARTIAL	4.00 0.00	07-00-08	02-08-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-08-02	103.95 66.65		
	9	J8 JACK-OPEN	4.00 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-07	112.50 72.00		
	2	J10 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	24.04 16.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287531 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	J11 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 02-09-14	19.98 14.00		

TOTAL # TRUSS= **87.00** TOTAL BFT OF ALL TRUSSES= **3209.14** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5041.24** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-3	
7	Hangers	LJS26DS	
1	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

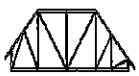
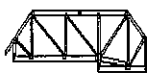
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287525 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1S HIP GIRDER	10.00 / 0.00	16-00-00	04-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	77.23 / 53.00		
	1	T2S HIP	10.00 / 0.00	16-00-00	06-06-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	78.55 / 52.83		
	1	T3S HIP	10.00 / 0.00	16-00-00	08-02-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	75.27 / 50.50		
	1 / 2 Ply	T4S HIP GIRDER	10.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	304.12 / 197.34		
	1	T5S HIP	10.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	137.91 / 89.00		
	1	T6S HIP	10.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.22 / 93.17		
	1	T7 HIP	10.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	131.25 / 82.67		
	1	T8 HIP	10.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.94 / 89.17		
	6	T9 HIP	10.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	933.12 / 588.00		
	1	T10 HIP	10.00 / 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	161.37 / 99.67		
	1 / 2 Ply	T11 ROOF	10.00 / 0.00	28-11-00	10-00-10	2 X 4	2 X 6	01-03-08 / 01-03-08	03-01-11 / 01-07-11	337.66 / 211.66		
	1	T12 ROOF	10.00 / 0.00	28-11-00	11-08-10	2 X 4	2 X 4	01-03-08 / 01-03-08	03-01-11 / 01-07-11	159.36 / 100.83		
	1	T13 PIGGYBACK	10.00 / 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 / 01-03-08	03-01-11 / 01-07-11	149.26 / 94.50		
	1	T130 PIGGYBACK	10.00 / 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 / 01-03-08	03-01-11 / 01-07-11	152.73 / 97.00		
	3	T131 PIGGYBACK	10.00 / 0.00	28-11-00	08-06-00	2 X 4	2 X 4	01-03-08 / 01-03-08	03-01-11 / 01-07-11	434.25 / 270.99		
	1	T14 HIP GIRDER	10.00 / 0.00	11-01-00	04-11-07	2 X 4	2 X 4	00-00-00 / 00-00-00	02-01-11 / 02-01-11	49.70 / 32.67		
	1	T15 HIP GIRDER	10.00 / 0.00	22-09-00	05-11-07	2 X 4	2 X 6	01-03-08 / 00-00-00	02-01-11 / 03-04-03	135.40 / 86.50		
	1	T16 HIP	10.00 / 0.00	22-09-00	07-07-07	2 X 4	2 X 4	01-03-08 / 00-00-00	02-01-11 / 03-04-03	116.48 / 74.50		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

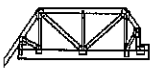
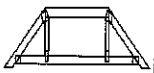
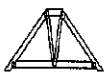
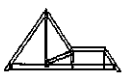
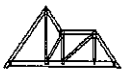
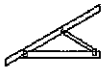
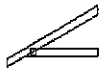
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287525 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T17 HIP	10.00 0.00	22-09-00	09-03-07	2 X 4	2 X 4	01-03-08 00-00-00	03-01-11 03-04-03	123.94 77.50		
	1	T18 HALF HIP	10.00 0.00	19-00-00	01-10-07	2 X 4	2 X 6	00-00-00 00-00-00	01-00-07 01-10-07	79.97 52.50		
	1	T19 HIP GIRDER	10.00 0.00	11-06-00	02-08-02	2 X 4	2 X 6	01-03-08 00-00-00	01-00-07 00-05-08	56.29 37.67		
	2	PB1 PIGGYBACK	10.00 0.00	07-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.88 30.66		
	2	PB2 PIGGYBACK	10.00 0.00	07-04-02	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	49.10 32.66		
	1	PB3 PIGGYBACK	10.00 0.00	07-04-02	03-05-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	23.41 14.83		
	2	PB4 PIGGYBACK	10.00 0.00	08-10-08	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	55.92 34.66		
	1	PB4Z PIGGYBACK	10.00 0.00	08-10-13	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-09	27.73 17.33		
	1	PB6 PIGGYBACK	10.00 0.00	12-11-15	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	41.65 27.00		
	1	PB7 PIGGYBACK	10.00 0.00	12-11-15	04-01-03	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	46.20 30.00		
	7	J1S JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-10-07	132.79 95.69		
	3	J2 JACK-OPEN	10.00 0.00	01-09-07	03-07-09	2 X 4	2 X 4	01-03-08 01-07-01	02-01-11 00-03-08	37.02 26.49		
	3	J3 JACK-OPEN	10.00 0.00	01-09-07	03-07-09	2 X 4	2 X 4	01-03-08 00-01-01	02-01-11 00-03-08	31.89 22.50		
	9	J4 JACK-OPEN	10.00 0.00	03-04-08	04-11-07	2 X 4	2 X 4	01-03-08 00-00-00	02-01-11 04-11-07	135.54 91.53		
	8	J5 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	241.76 154.64		
	5	J7 JACK-PARTIAL	4.00 0.00	07-00-08	02-08-02	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-08-02	103.95 66.65		
	9	J8 JACK-OPEN	4.00 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-07	112.50 72.00		

TOTAL # TRUSS= **84.00** TOTAL BFT OF ALL TRUSSES= **3248.31** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5063.36** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287525 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-6 **ELEVATION:** A-OPT.COFF

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-3	
7	Hangers	LJS26DS	
1	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **11.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

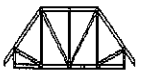
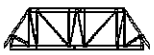
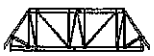
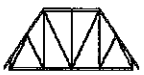
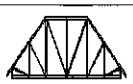
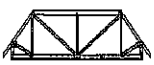
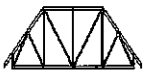
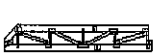
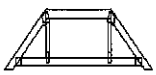
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287532 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1 HIP GIRDER	10.00 / 0.00	16-00-00	04-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	72.29 / 46.83		
	1	T2 HIP	10.00 / 0.00	16-00-00	06-06-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	77.58 / 50.67		
	1	T3 HIP	10.00 / 0.00	16-00-00	08-02-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	73.85 / 47.33		
	1 / 2 Ply	T4 HIP GIRDER	10.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	327.60 / 207.34		
	1 / 2 Ply	T4Z HIP GIRDER	10.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	327.60 / 207.34		
	2	T5 HIP	10.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	245.62 / 153.34		
	2	T6 HIP	10.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	269.56 / 169.66		
	2	T7 HIP	10.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	262.50 / 165.34		
	2	T8 HIP	10.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.88 / 178.34		
	7	T9 HIP	10.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1088.64 / 686.00		
	2	T10 HIP	10.00 / 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	322.74 / 199.34		
	3	T21S SCISSORS	10.00 / 4.00	11-01-00	06-03-02	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	151.47 / 99.99		
	1	T22 HIP GIRDER	10.00 / 0.00	23-00-00	04-10-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	117.87 / 73.33		
	1	T23 HIP	10.00 / 0.00	23-00-00	06-06-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	105.63 / 67.00		
	1	T24 HIP	10.00 / 0.00	23-00-00	08-02-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	109.65 / 70.33		
	1	T25 HALF HIP	8.00 / 0.00	18-06-08	01-10-07	2 X 6	2 X 6	00-00-00 / 00-00-00	00-09-10 / 00-05-08	88.88 / 54.50		
	1 / 2 Ply	T100 JACK-CLOSED	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 05-01-04	76.52 / 48.66		
	2	PB1 PIGGYBACK	10.00 / 0.00	07-04-02	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	43.88 / 30.66		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

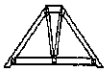
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287532 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-6 **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	2	PB2 PIGGYBACK	10.00 0.00	07-04-02	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	49.10 32.66		
	1	PB3 PIGGYBACK	10.00 0.00	07-04-02	03-05-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	23.41 14.83		
	10	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	155.70 101.70		
	8	J5 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	241.76 154.64		
	9	J8 JACK-OPEN	4.00 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-07	112.50 72.00		
	3	J10 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	36.06 24.99		
	3	J11 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	25.11 18.00		

TOTAL # TRUSS= **71.00** TOTAL BFT OF ALL TRUSSES= **2974.82** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4689.40** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HGUS26-3	
6	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **13.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

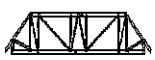
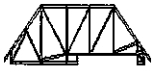
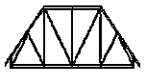
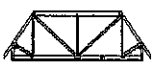
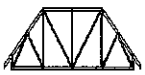
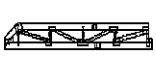
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287526 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T1S HIP GIRDER	10.00 0.00	16-00-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	77.23 53.00		
	1	T2S HIP	10.00 0.00	16-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	78.55 52.83		
	1	T3S HIP	10.00 0.00	16-00-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	75.27 50.50		
	1 2 Ply	T4S HIP GIRDER	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	304.12 197.34		
	1 2 Ply	T4Z HIP GIRDER	10.00 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	327.60 207.34		
	1	T5 HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.81 76.67		
	1	T5S HIP	10.00 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	137.91 89.00		
	1	T6 HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.78 84.83		
	1	T6S HIP	10.00 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	144.22 93.17		
	2	T7 HIP	10.00 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	262.50 165.34		
	2	T8 HIP	10.00 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	283.88 178.34		
	7	T9 HIP	10.00 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1088.64 686.00		
	2	T10 HIP	10.00 0.00	28-11-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	322.74 199.34		
	3	T21S SCISSORS	10.00 4.00	11-01-00	06-03-02	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	151.47 99.99		
	1	T22 HIP GIRDER	10.00 0.00	23-00-00	04-10-07	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	117.87 73.33		
	1	T23 HIP	10.00 0.00	23-00-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	105.63 67.00		
	1	T24 HIP	10.00 0.00	23-00-00	08-02-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	109.65 70.33		
	1	T25 HALF HIP	8.00 0.00	18-06-08	01-10-07	2 X 6	2 X 6	00-00-00 00-00-00	00-09-10 00-05-08	88.88 54.50		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

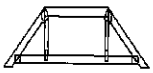
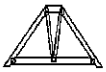
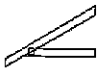
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287526 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 2 Ply	T100 JACK-CLOSED	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 05-01-04	76.52 48.66		
	2	PB1 PIGGYBACK	10.00 0.00	07-04-02	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	43.88 30.66		
	2	PB2 PIGGYBACK	10.00 0.00	07-04-02	02-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	49.10 32.66		
	1	PB3 PIGGYBACK	10.00 0.00	07-04-02	03-05-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	23.41 14.83		
	4	J1 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	62.28 40.68		
	7	J1S JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 03-10-07	132.79 95.69		
	8	J5 JACK-PARTIAL	6.00 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 05-01-04	241.76 154.64		
	9	J8 JACK-OPEN	4.00 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-07	112.50 72.00		
	1	J10 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	12.02 8.33		
	1	J11 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 00-01-01	01-07-11 00-03-08	8.37 6.00		

TOTAL # TRUSS= **68.00** TOTAL BFT OF ALL TRUSSES= **3003.00** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4696.38** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
1	Hangers	HGUS26-3	
6	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	LUS24-2	

TOTAL # ITEMS= **13.00**

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

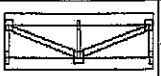
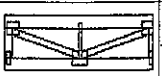
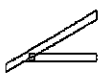
Delivery Shiplist

DATE	03/01/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287533 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T31 HIP GIRDER	8.00 / 0.00	16-00-00	05-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	70.74 / 44.50		
	1	T32 HIP	8.00 / 0.00	16-00-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	67.84 / 44.17		
	1 / 2 Ply	T33 HIP GIRDER	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	321.86 / 203.66		
	1 / 2 Ply	T33Z HIP GIRDER	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	311.74 / 196.66		
	2	T34 HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	252.00 / 158.66		
	2	T35 HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	255.52 / 161.00		
	2	T36 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	268.64 / 166.66		
	2	T37 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	263.06 / 162.68		
	2	T38 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	267.64 / 167.34		
	7	T39 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	881.30 / 536.69		
	1	T40 HALF HIP	9.00 / 0.00	18-06-08	01-10-07	2 X 6	2 X 6	00-00-00 / 00-00-00	00-11-00 / 00-05-08	89.57 / 55.50		
	1 / 2 Ply	T200 JACK-CLOSED	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 05-01-04	77.12 / 48.66		
	1 / 2 Ply	T201Z JACK-CLOSED	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 05-01-04	77.94 / 48.66		
	1 / 2 Ply	T202 FLAT GIRDER	0.00 / 0.00	09-08-08	02-01-08	2 X 6	2 X 6	00-00-00 / 00-00-00	02-01-08 / 02-01-08	92.10 / 57.34		
	1 / 2 Ply	T203 FLAT GIRDER	0.00 / 0.00	09-02-00	02-00-00	2 X 6	2 X 6	00-02-08 / 00-00-00	02-00-00 / 02-00-00	89.46 / 58.66		
	8	J5 JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-01-04	241.76 / 154.64		
	9	J8 JACK-OPEN	4.00 / 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-10-07	112.50 / 72.00		
	4	J30 JACK-OPEN	6.00 / 0.00	05-08-08	05-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	02-03-00 / 05-01-04	72.28 / 46.00		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/01/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287533 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-6 **ELEVATION:** C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	4	J31 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	95.68 58.68		
	2	J35 JACK-OPEN	8.00 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	33.42 21.66		
	2	J36 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	26.94 16.66		
	2	J37 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	15.34 9.34		
	2	J38 JACK-OPEN	8.00 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 01-10-15	01-04-13 00-05-03	23.42 15.34		

TOTAL # TRUSS= **65.00** TOTAL BFT OF ALL TRUSSES= **2505.16** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4007.87** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
5	Hangers	LUS24	
3	Hangers	LUS26-2	

TOTAL # ITEMS= **12.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

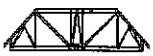
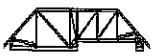
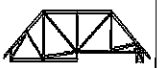
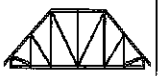
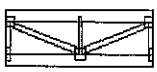
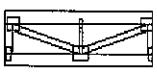
Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287529 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-6 ELEVATION: C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T31S HIP GIRDER	8.00 / 0.00	16-00-00	05-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	72.25 / 47.33		
	1	T32S HIP	8.00 / 0.00	16-00-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	70.08 / 46.00		
	1 / 2 Ply	T33 HIP GIRDER	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	317.16 / 200.34		
	1 / 2 Ply	T33S HIP GIRDER	8.00 / 0.00	28-11-00	05-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	289.80 / 187.00		
	1	T34 HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	126.00 / 79.33		
	1	T34S HIP	8.00 / 0.00	28-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	128.19 / 81.50		
	1	T35 HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	127.76 / 80.50		
	1	T35S HIP	8.00 / 0.00	28-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	135.55 / 86.00		
	2	T36 HIP	8.00 / 0.00	28-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	268.64 / 166.66		
	2	T37 HIP	8.00 / 0.00	28-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	263.06 / 162.68		
	2	T38 HIP	8.00 / 0.00	28-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	267.64 / 167.34		
	7	T39 COMMON	8.00 / 0.00	28-11-00	11-00-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	881.30 / 536.69		
	1	T40 HALF HIP	9.00 / 0.00	18-06-08	01-10-07	2 X 6	2 X 6	00-00-00 / 00-00-00	00-11-00 / 00-05-08	89.57 / 55.50		
	1 / 2 Ply	T200 JACK-CLOSED	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 05-01-04	77.12 / 48.66		
	1 / 2 Ply	T201 JACK-CLOSED	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 05-01-04	77.12 / 48.66		
	1 / 2 Ply	T202 FLAT GIRDER	0.00 / 0.00	09-08-08	02-01-08	2 X 6	2 X 6	00-00-00 / 00-00-00	02-01-08 / 02-01-08	92.10 / 57.34		
	1 / 2 Ply	T203 FLAT GIRDER	0.00 / 0.00	09-02-00	02-00-00	2 X 6	2 X 6	00-02-08 / 00-00-00	02-00-00 / 02-00-00	89.46 / 58.66		
	8	J5 JACK-PARTIAL	6.00 / 0.00	07-10-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 05-01-04	241.76 / 154.64		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	02/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287529 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-6 **ELEVATION:** C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	9	J8 JACK-OPEN	4.00 0.00	04-07-08	01-10-07	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-07	112.50 72.00		
	4	J30 JACK-OPEN	6.00 0.00	05-08-08	05-01-04	2 X 4	2 X 4	00-00-00 00-00-00	02-03-00 05-01-04	72.28 46.00		
	7	J31S JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 04-03-13	165.83 112.00		

TOTAL # TRUSS= **60.00** TOTAL BFT OF ALL TRUSSES= **2494.83** BFT. TOTAL WEIGHT OF ALL TRUSSES= **3965.17** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
4	Hangers	LUS24	
3	Hangers	LUS26-2	

TOTAL # ITEMS= **11.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:24 2018 Page 1

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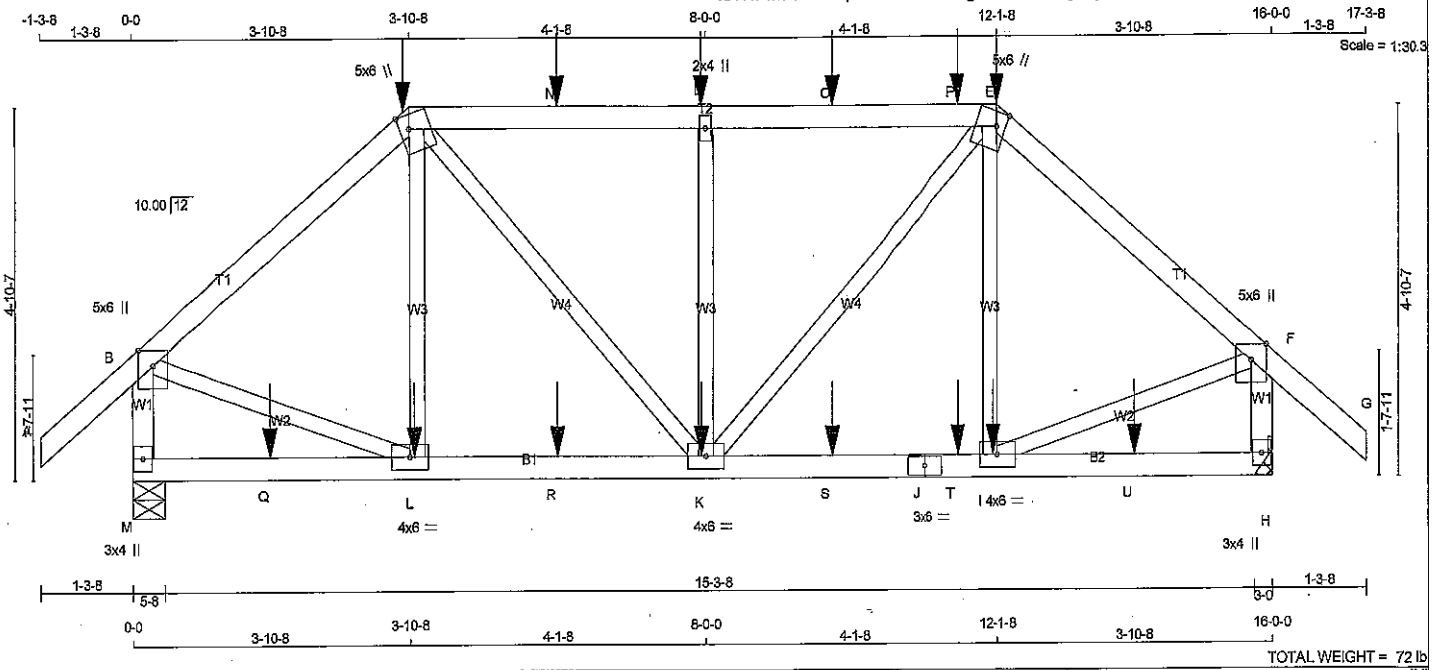

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 330.7 lbs FACTORED DOWN AT 3-10-8, 98.4 lbs FACTORED DOWN AT 5-11-4, 98.4 lbs FACTORED DOWN AT 7-11-4, 98.4 lbs FACTORED DOWN AT 9-9-0, AND 121.3 lbs FACTORED DOWN AT 11-6-12, AND 330.7 lbs FACTORED DOWN AT 12-1-8 ON TOP CHORD, AND 45.9 lbs FACTORED DOWN AT 1-11-4, 42.0 lbs FACTORED DOWN AT 3-11-4, 42.0 lbs FACTORED DOWN AT 5-11-4, 42.0 lbs FACTORED DOWN AT 7-11-4, 42.0 lbs FACTORED DOWN AT 9-9-0, 42.0 lbs FACTORED DOWN AT 11-6-12, AND 42.0 lbs FACTORED DOWN AT 12-0-12, AND 45.9 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1612	0	1812	0	0	5-8	5-8
H	1871	0	1871	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1408	922 / 0	239 / 0	0 / 0	0 / 0	247 / 0	0 / 0
H	1454	953 / 0	246 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B-C	-1578 / 0	-104.9	-104.9	0.36 (1)	4.82
C-N	-1650 / 0	-104.9	-104.9	0.50 (1)	4.46
N-D	-1650 / 0	-104.9	-104.9	0.50 (1)	4.46
D-O	-1650 / 0	-104.9	-104.9	0.50 (1)	4.46
O-P	-1650 / 0	-104.9	-104.9	0.50 (1)	4.46
P-E	-1650 / 0	-104.9	-104.9	0.50 (1)	4.46
E-F	-1646 / 0	-104.9	-104.9	0.36 (1)	4.72
F-G	0 / 47	-104.9	-104.9	0.16 (1)	10.00
M-B	-1759 / 0	0.0	0.0	0.20 (1)	6.24
H-F	-1820 / 0	0.0	0.0	0.21 (1)	6.16
M-Q	0 / 0	-28.0	-28.0	0.16 (3)	10.00
Q-L	0 / 0	-28.0	-28.0	0.16 (3)	10.00
L-R	0 / 1204	-28.0	-28.0	0.30 (1)	10.00
R-K	0 / 1204	-28.0	-28.0	0.30 (1)	10.00
K-S	0 / 1256	-28.0	-28.0	0.31 (1)	10.00
S-J	0 / 1256	-28.0	-28.0	0.31 (1)	10.00
J-T	0 / 1256	-28.0	-28.0	0.31 (1)	10.00
T-I	0 / 1256	-28.0	-28.0	0.31 (1)	10.00
I-U	0 / 0	-28.0	-28.0	0.17 (3)	10.00
U-H	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-216 / 162	0.08 (1)
C-K	0 / 665	0.16 (1)
K-D	-779 / 0	0.28 (1)
K-E	0 / 587	0.15 (1)
I-E	-212 / 193	0.08 (1)
B-L	0 / 1272	0.31 (1)
I-F	0 / 1327	0.33 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-331	-331	---	BACK	VERT	TOTAL
D	7-11-4	-98	-98	---	BACK	VERT	TOTAL
E	12-1-8	-331	-331	---	BACK	VERT	TOTAL
I	12-0-12	-24	-42	---	BACK	VERT	TOTAL
K	7-11-4	-24	-42	---	BACK	VERT	TOTAL
L	3-11-4	-24	-42	---	BACK	VERT	TOTAL
N	5-11-4	-98	-98	---	BACK	VERT	TOTAL
O	9-9-0	-98	-98	---	BACK	VERT	TOTAL
P	11-6-12	-121	-121	---	BACK	VERT	TOTAL
Q	1-11-4	-22	-46	---	BACK	VERT	TOTAL
R	5-11-4	-24	-42	---	BACK	VERT	TOTAL
S	9-9-0	-24	-42	---	BACK	VERT	TOTAL
T	11-6-12	-24	-42	---	BACK	VERT	TOTAL
U	14-0-12	-22	-46	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.31/1.00 (I-K:1) , WB=0.33/1.00 (F-I:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.32 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 22816 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 1106718
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T1S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:21 2018 Page 1

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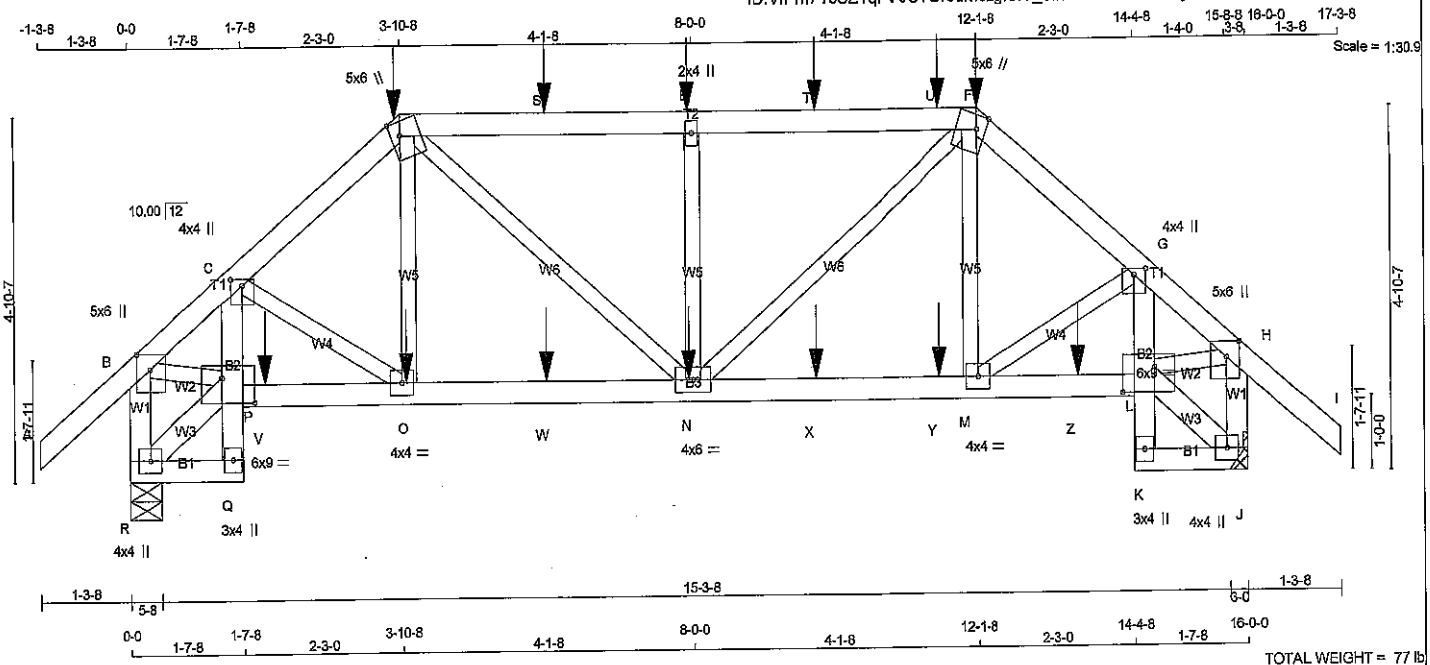

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - C	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMW+p	MT20	5.0	6.0	2.50	2.25
J	BMVW1+p	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-I	MT20	6.0	9.0	4.00	5.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BVMWW-I	MT20	6.0	9.0	4.00	5.50
Q	BMV+p	MT20	3.0	4.0		
R	BMVW1+p	MT20	4.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 125.5 lbs FACTORED DOWN AT 12-1-8, 282.3 lbs FACTORED DOWN AND 86.6 lbs FACTORED UP AT 3-10-8, 93.1 lbs FACTORED DOWN AND 103.4 lbs FACTORED UP AT 5-11-4, 93.1 lbs FACTORED DOWN AND 103.4 lbs FACTORED UP AT 7-11-4, AND 93.1 lbs FACTORED DOWN AND 103.4 lbs FACTORED UP AT 9-9-0, AND 116.1 lbs FACTORED DOWN AND 80.5 lbs FACTORED UP AT 11-6-12 ON TOP CHORD, AND 23.0 lbs FACTORED DOWN AT 1-11-4, 9.7 lbs FACTORED DOWN AT 3-11-4, 9.7 lbs FACTORED DOWN AT 5-11-4, 9.7 lbs FACTORED DOWN AT 7-11-4, 9.7 lbs FACTORED DOWN AT 9-9-0, AND 9.7 lbs FACTORED DOWN AT 11-6-12, AND 10.7 lbs FACTORED DOWN AT 13-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	1653	0	1653	0	0	5-8	5-8
J	1614	0	1614	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1296	830 / 0	232 / 0	0 / 0	0 / 0	234 / 0	0 / 0
J	1272	804 / 0	236 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-1883 / 0	-104.9	-104.9	0.20 (1)	4.67
C- D	-1748 / 0	-104.9	-104.9	0.16 (1)	4.87
D- S	-1830 / 0	-104.9	-104.9	0.50 (1)	4.27
S- E	-1830 / 0	-104.9	-104.9	0.50 (1)	4.27
E- T	-1830 / 0	-104.9	-104.9	0.50 (1)	4.27
T- U	-1830 / 0	-104.9	-104.9	0.50 (1)	4.27
U- F	-1830 / 0	-104.9	-104.9	0.50 (1)	4.27
F- G	-1699 / 0	-104.9	-104.9	0.15 (1)	4.93
G- H	-1830 / 0	-104.9	-104.9	0.19 (1)	4.72
H- I	0 / 47	-104.9	-104.9	0.16 (1)	10.00
R- B	-1594 / 0	0.0	0.0	0.19 (1)	6.50
J- H	-1556 / 0	0.0	0.0	0.18 (1)	6.56
R- Q	0 / 49	-28.0	-28.0	0.03 (7)	10.00
Q- P	0 / 36	0.0	0.0	0.09 (1)	10.00
P- C	-11 / 66	0.0	0.0	0.09 (1)	7.81
P- V	0 / 1458	-28.0	-28.0	0.31 (1)	10.00
V- O	0 / 1458	-28.0	-28.0	0.31 (1)	10.00
O- W	0 / 1339	-28.0	-28.0	0.29 (1)	10.00
W- N	0 / 1339	-28.0	-28.0	0.29 (1)	10.00
N- X	0 / 1302	-28.0	-28.0	0.28 (1)	10.00
X- Y	0 / 1302	-28.0	-28.0	0.28 (1)	10.00
Y- M	0 / 1302	-28.0	-28.0	0.28 (1)	10.00
M- Z	0 / 1416	-28.0	-28.0	0.31 (1)	10.00
Z- L	0 / 1416	-28.0	-28.0	0.31 (1)	10.00
K- L	0 / 36	0.0	0.0	0.09 (1)	10.00
L- G	-17 / 55	0.0	0.0	0.08 (1)	7.81
K- J	0 / 47	-28.0	-28.0	0.03 (7)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-157 / 0	0.03 (5)
O- D	0 / 191	0.05 (7)
D- N	0 / 649	0.16 (1)
N- E	-761 / 0	0.19 (1)
N- F	0 / 698	0.17 (1)
M- F	0 / 185	0.05 (7)
M- G	-152 / 0	0.03 (5)
R- P	-62 / 0	0.01 (1)
B- P	0 / 1419	0.35 (1)
L- J	-61 / 0	0.01 (1)
L- H	0 / 1378	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	3-10-8	-282	-282	100	BACK	VERT	TOTAL
E	7-11-4	-93	-93	103	BACK	VERT	TOTAL
F	12-1-8	-125	-125	---	FRONT	VERT	TOTAL
N	7-11-4	-6	-10	---	BACK	VERT	TOTAL
O	3-11-4	-6	-10	---	BACK	VERT	TOTAL
S	5-11-4	-93	-93	103	BACK	VERT	TOTAL
T	9-9-0	-93	-93	103	BACK	VERT	TOTAL
U	11-6-12	-116	-116	80	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.50/1.00 (E-F:1) , BC=0.31/1.00 (O-P:1) , WB=0.35/1.00 (B-P:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 2018 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 11093 -01

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T1S	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	1-11-4	-13	-23	—	BACK	VERT	TOTAL
W	5-11-4	-6	-10	—	BACK	VERT	TOTAL
X	9-9-0	-6	-10	—	BACK	VERT	TOTAL
Y	11-6-12	-6	-10	—	BACK	VERT	TOTAL
Z	13-6-12	-6	-11	—	BACK	VERT	TOTAL

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.34 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11093-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:25 2018 Page 1
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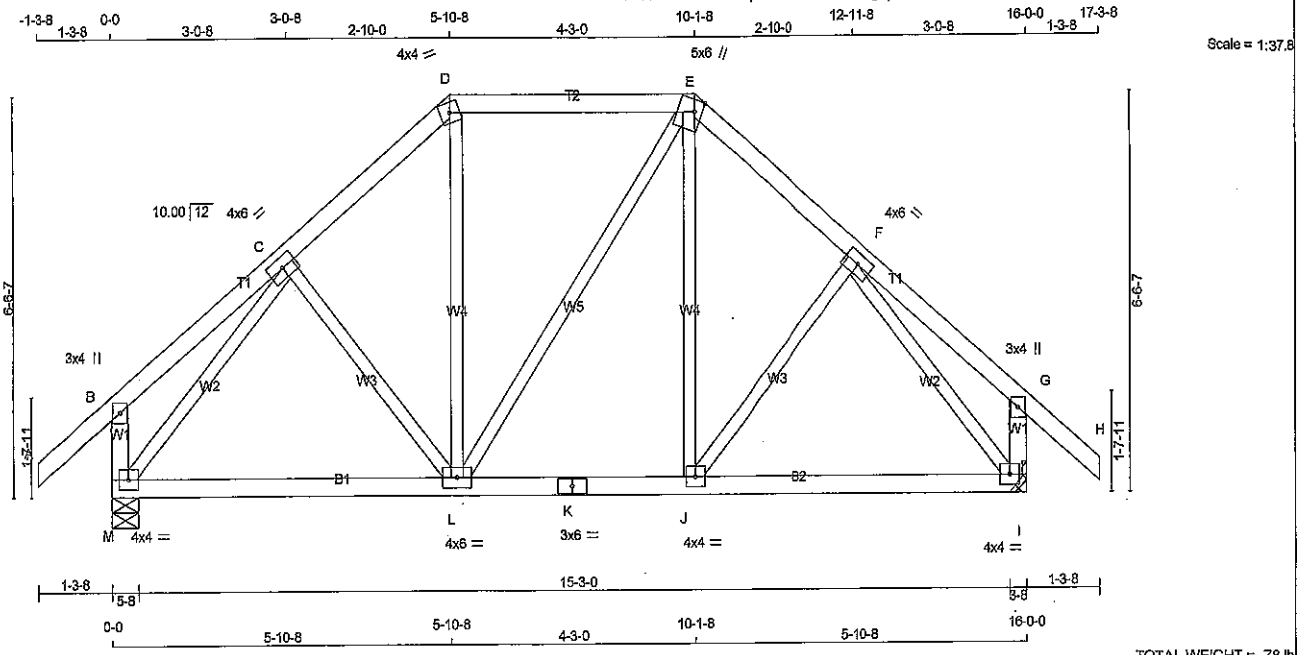

TOTAL WEIGHT = 78 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1209	0	1209	0	0	5-8	5-8
I	1209	0	1209	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
I	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 21	-104.9	-104.9	0.14 (1)	10.00	
C- D	-864 / 0	-104.9	-104.9	0.11 (1)	6.25	
D- E	-649 / 0	-104.9	-104.9	0.25 (1)	6.25	
E- F	-864 / 0	-104.9	-104.9	0.11 (1)	6.25	
F- G	0 / 21	-104.9	-104.9	0.14 (1)	10.00	
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
M- B	-264 / 0	0.0	0.0	0.03 (1)	7.81	
I- G	-264 / 0	0.0	0.0	0.03 (1)	7.81	
M- L	0 / 681	-28.0	-28.0	0.26 (2)	10.00	
L- K	0 / 648	-28.0	-28.0	0.26 (2)	10.00	
K- J	0 / 648	-28.0	-28.0	0.26 (2)	10.00	
J- I	0 / 681	-28.0	-28.0	0.26 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-57 / 66	0.02 (1)
L- D	0 / 241	0.05 (2)
L- E	0 / 0	0.00 (1)
J- E	0 / 240	0.05 (2)
J- F	-57 / 66	0.02 (1)
M- C	-1108 / 0	0.40 (1)
F- I	-1107 / 0	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.25/1.00 (D-E:1) , BC=0.28/1.00 (I-J:2) , WB=0.40/1.00 (C-M:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 110868
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Thu Mar 1 09:01:22 2018 Page 1

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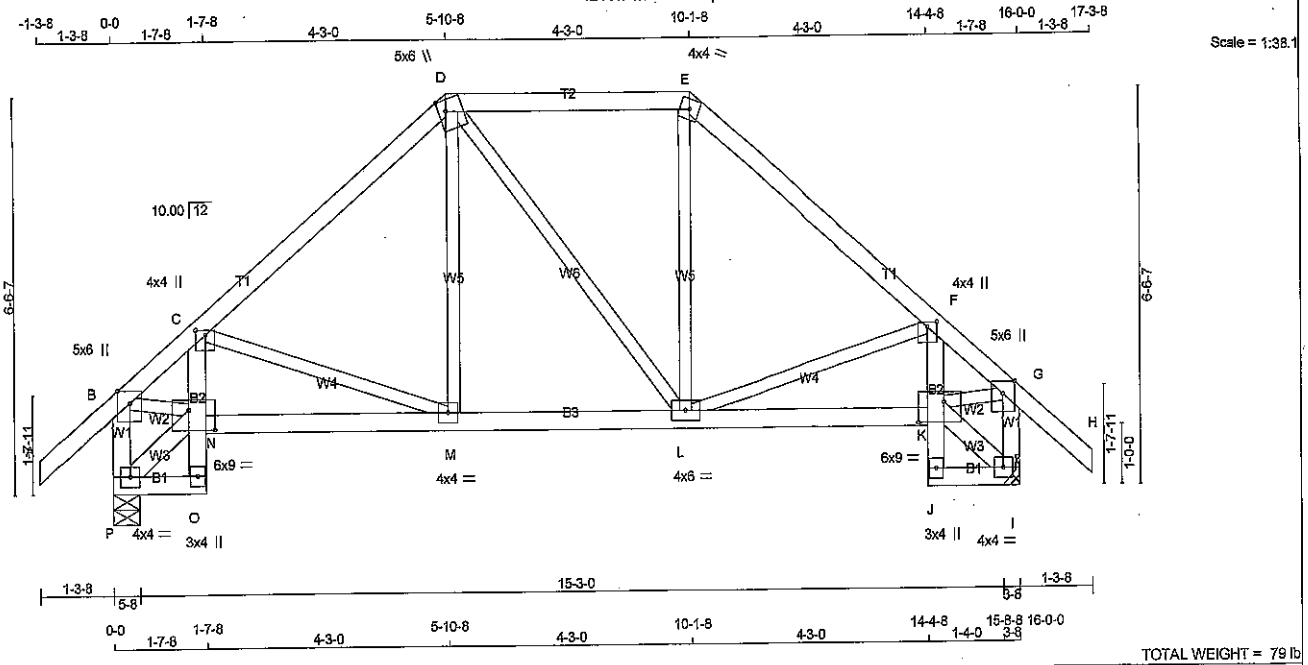

TOTAL WEIGHT = 79 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMVW1-t	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-I	MT20	6.0	9.0	4.00	5.50
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	4.00	5.50
O	BMV+p	MT20	3.0	4.0		
P	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1209	0	1209	0	0	5-8	5-8
I	1209	0	1209	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
I	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1352 / 0	-104.9	-104.9	0.19 (1)	5.39
C- D	-994 / 0	-104.9	-104.9	0.26 (1)	5.96
D- E	-746 / 0	-104.9	-104.9	0.25 (1)	6.25
E- F	-994 / 0	-104.9	-104.9	0.26 (1)	5.85
F- G	-1352 / 0	-104.9	-104.9	0.19 (1)	5.39
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P- B	-1160 / 0	0.0	0.0	0.12 (1)	7.39
I- G	-1160 / 0	0.0	0.0	0.12 (1)	7.39
P- O	0 / 36	-28.0	-28.0	0.02 (2)	10.00
O- N	0 / 36	0.0	0.0	0.06 (1)	10.00
N- C	-7 / 94	0.0	0.0	0.05 (2)	10.00
N- M	0 / 1088	-28.0	-28.0	0.24 (1)	10.00
M- L	0 / 748	-28.0	-28.0	0.21 (2)	10.00
L- K	0 / 1088	-28.0	-28.0	0.24 (1)	10.00
J- K	0 / 36	0.0	0.0	0.06 (1)	10.00
K- F	-8 / 93	0.0	0.0	0.05 (2)	10.00
J- I	0 / 36	-28.0	-28.0	0.02 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	-364 / 0	0.14 (1)
M- D	0 / 268	0.06 (2)
D- L	0 / 0	0.00 (1)
L- E	0 / 269	0.06 (2)
L- F	-363 / 0	0.14 (1)
P- N	-45 / 0	0.00 (1)
B- N	0 / 1062	0.24 (1)
K- I	-45 / 0	0.00 (1)
K- G	0 / 1062	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.26/1.00 (E-F:1) , BC=0.24/1.00 (K-L:1) , WB=0.24/1.00 (G-K:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

DWG NO. TAM 110994-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:25 2018 Page 1
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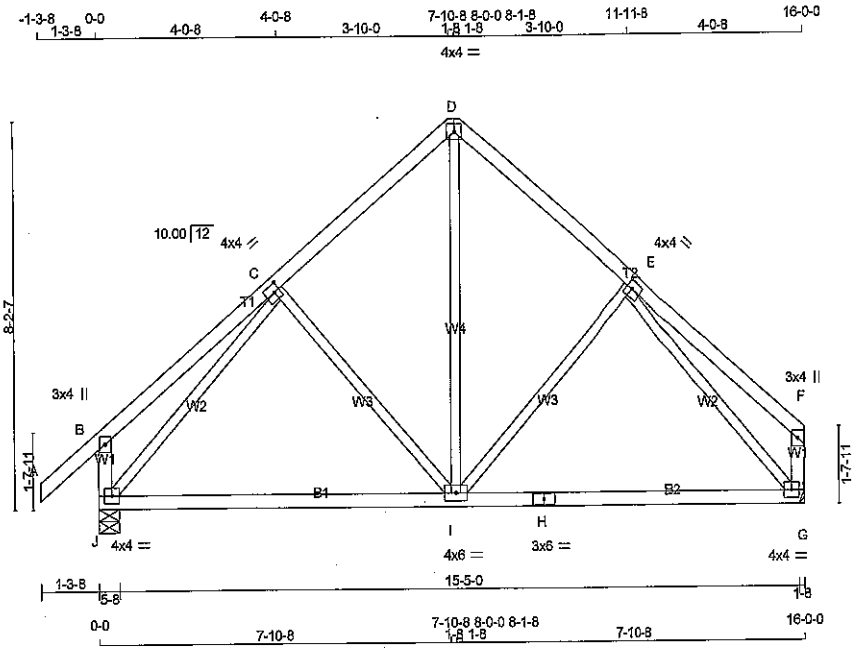


TOTAL WEIGHT = 72 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD SRG IN-SX
J	1209	0	1209	0	0	5-8	5-8
G	1063	0	1063	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
G	848	520 / 0	168 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- I	-217 / 38	0.14 (1)
B- C	0 / 32	-104.9	-104.9	0.27 (1)	10.00	I- E	-217 / 38	0.14 (1)
C- D	-760 / 0	-104.9	-104.9	0.21 (1)	6.25	J- C	-1084 / 0	0.66 (1)
D- E	-760 / 0	-104.9	-104.9	0.21 (1)	6.25	E- G	-1084 / 0	0.66 (1)
E- F	0 / 32	-104.9	-104.9	0.27 (1)	10.00	I- D	0 / 601	0.14 (1)
J- B	-300 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-155 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 705	-28.0	-28.0	0.59 (2)	10.00			
I- H	0 / 705	-28.0	-28.0	0.59 (2)	10.00			
H- G	0 / 705	-28.0	-28.0	0.59 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 962 (0.20")

CSI: TC=0.27/1.00 (B-C:1) , BC=0.59/1.00 (I-J:2) , WB=0.66/1.00 (C-J:1) , SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

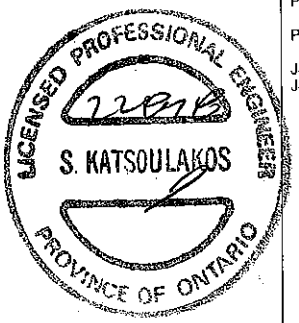
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DWG NO. TAM 11089-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:22 2018 Page 1

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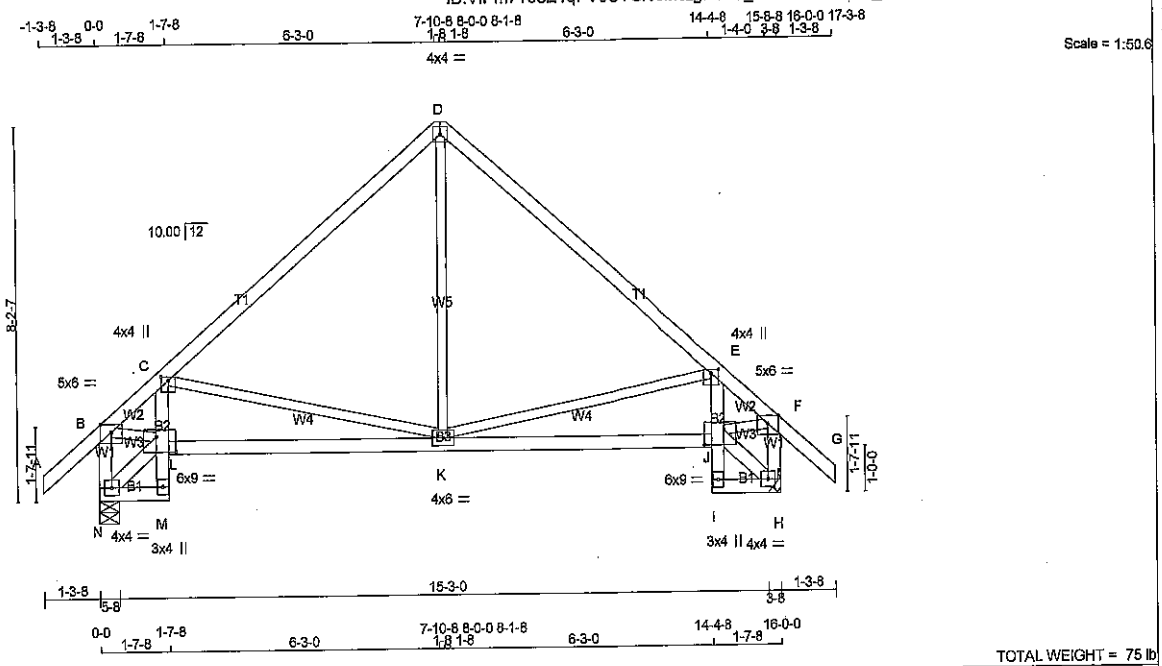

TOTAL WEIGHT = 75 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW-p	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW-p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1209	0	1209	0	0	5-8	5-8
H	1209	0	1209	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	946	610 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. (PLF)	CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- K	-681 / 0	0.57 (1)	
B- C	-1484 / 0	-104.9	-104.9	0.46 (1)	4.80	K- E	-681 / 0	0.57 (1)	
C- D	-844 / 0	-104.9	-104.9	0.55 (1)	5.80	K- D	0 / 519	0.12 (2)	
D- E	-844 / 0	-104.9	-104.9	0.55 (1)	5.80	N- L	-50 / 0	0.01 (1)	
E- F	-1484 / 0	-104.9	-104.9	0.46 (1)	4.80	B- L	0 / 1250	0.28 (1)	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	J- H	-50 / 0	0.01 (1)	
N- B	-1157 / 0	0.0	0.0	0.12 (1)	7.40	J- F	0 / 1250	0.28 (1)	
H- F	-1157 / 0	0.0	0.0	0.12 (1)	7.40				
N- M	0 / 39	-28.0	-28.0	0.02 (2)	10.00				
M- L	0 / 36	0.0	0.0	0.07 (1)	10.00				
L- C	-18 / 117	0.0	0.0	0.05 (2)	7.81				
L- K	0 / 1279	-28.0	-28.0	0.48 (2)	10.00				
K- J	0 / 1279	-28.0	-28.0	0.48 (2)	10.00				
I- J	0 / 36	0.0	0.0	0.07 (1)	10.00				
J- E	-18 / 117	0.0	0.0	0.05 (2)	7.81				
I- H	0 / 39	-28.0	-28.0	0.02 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55/1.00 (C-D:1) , BC=0.48/1.00 (K-L:2) , WB=0.57/1.00 (C-K:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.26 (F) (INPUT = 1.00)

DWG NO. TAM 110954-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:26 2018 Page 1

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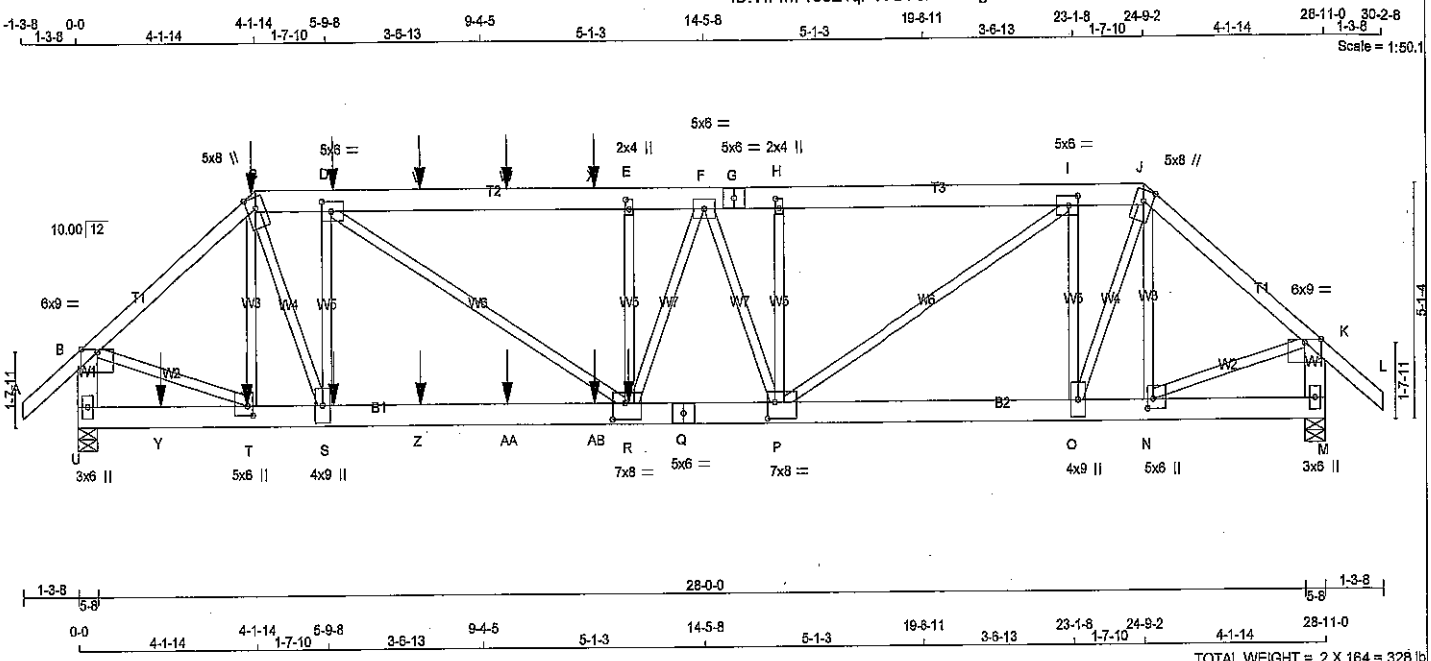

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
J- L	1	12	TOP
C- G	2	12	SIDE(183.1)
G- J	2	12	TOP
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
T- C	1	6	SIDE(30.3)
E- R	1	4	SIDE(499.6)
2x3	1	6	
H- P	1	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TMWW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	5.0	6.0	2.50	1.50
O	BMWW+t	MT20	4.0	9.0		
P	BMWWW-t	MT20	7.0	8.0	4.25	2.00
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	7.0	8.0	4.25	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	5715	0	5715	0	0	5-8	5-8
M	3856	0	3856	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	4508	2842 / 0	841 / 0	0 / 0	0 / 0	825 / 0	0 / 0
M	3028	1932 / 0	550 / 0	0 / 0	0 / 0	546 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C		-5942 / 0	-104.9	-104.9	0.36 (1)	3.74
C- D		-6053 / 0	-104.9	-104.9	0.17 (1)	4.66
D- V		-9283 / 0	-104.9	-104.9	0.38 (1)	3.74
V- W		-9283 / 0	-104.9	-104.9	0.38 (1)	3.74
W- X		-9283 / 0	-104.9	-104.9	0.38 (1)	3.74
X- E		-9283 / 0	-104.9	-104.9	0.38 (1)	3.74
E- F		-9283 / 0	-104.9	-104.9	0.20 (1)	3.89
F- G		-7889 / 0	-104.9	-104.9	0.19 (1)	4.17
G- H		-7889 / 0	-104.9	-104.9	0.19 (1)	4.17
H- I		-7889 / 0	-104.9	-104.9	0.27 (1)	4.12
I- J		-4222 / 0	-104.9	-104.9	0.11 (1)	5.46
J- K		-3982 / 0	-104.9	-104.9	0.26 (1)	4.53
K- L		0 / 47	-104.9	-104.9	0.08 (1)	10.00
U- B		-5534 / 0	0.0	0.0	0.20 (1)	6.24
M- K		-3829 / 0	0.0	0.0	0.14 (1)	7.23
U- Y		0 / 0	-28.0	-28.0	0.11 (1)	10.00
Y- T		0 / 0	-28.0	-28.0	0.11 (1)	10.00
T- S		0 / 4597	-28.0	-28.0	0.52 (1)	10.00
S- Z		0 / 6050	-28.0	-28.0	0.67 (1)	10.00
Z-AA		0 / 6050	-28.0	-28.0	0.67 (1)	10.00
AA-AB		0 / 6050	-28.0	-28.0	0.67 (1)	10.00
AB- R		0 / 6050	-28.0	-28.0	0.67 (1)	10.00
R- Q		0 / 8562	-28.0	-28.0	0.68 (1)	10.00
Q- P		0 / 8562	-28.0	-28.0	0.68 (1)	10.00
P- O		0 / 4220	-28.0	-28.0	0.34 (1)	10.00
O- N		0 / 3084	-28.0	-28.0	0.24 (1)	10.00
N- M		0 / 0	-28.0	-28.0	0.04 (1)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C		-978 / 0	0.18 (1)
R- E		-668 / 0	0.12 (1)
R- F		0 / 2090	0.26 (1)
F- P		-1951 / 0	0.38 (1)
P- H		-634 / 0	0.11 (1)
N- J		-806 / 0	0.15 (1)
B- T		0 / 4779	0.59 (1)
N- K		0 / 3203	0.40 (1)
S- D		-3010 / 0	0.52 (1)
O- I		-3090 / 0	0.54 (1)
C- S		0 / 4392	0.54 (1)
D- R		0 / 3688	0.46 (1)
O- J		0 / 3432	0.42 (1)
P- I		0 / 4412	0.55 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-1-14	-584	-584	---	FRONT	VERT	TOTAL
D	5-11-4	-54	-54	---	FRONT	VERT	TOTAL
R	12-8-12	-2617	-2617	---	FRONT	VERT	TOTAL
S	5-11-4	-337	-337	---	FRONT	VERT	TOTAL
T	3-11-4	-337	-337	---	FRONT	VERT	TOTAL
V	7-11-4	-54	-54	---	FRONT	VERT	TOTAL
W	9-11-4	-54	-54	---	FRONT	VERT	TOTAL
X	11-11-4	-54	-54	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.68/1.00 (P-R:1) , WB=0.59/1.00 (B-T:1) , SSI=0.22/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (I) (INPUT = 0.90)
JSI METAL= 0.69 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 1/090-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:VfPm7TsSZ1qPV0OYGr9uMczgf9K-JwclSizFcRHTGPdLZ0ez6WgvhHSIJdv7ObPgf1zfLnN

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMWW+t	MT20	4.0	9.0		
T	BMWW+t	MT20	5.0	6.0	2.50	1.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 583.8 lbs FACTORED DOWN AT 4-1-14, 53.9 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, AND 53.9 lbs FACTORED DOWN AT 9-11-4, AND 53.9 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, AND 336.6 lbs FACTORED DOWN AT 11-11-4, AND 2617.0 lbs FACTORED DOWN AT 12-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	1-11-4	-337	-337	---	FRONT	VERT	TOTAL
Z	7-11-4	-337	-337	---	FRONT	VERT	TOTAL
AA	9-11-4	-337	-337	---	FRONT	VERT	TOTAL
AB	11-11-4	-337	-337	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11090-T4
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T4Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:39 2018 Page 1
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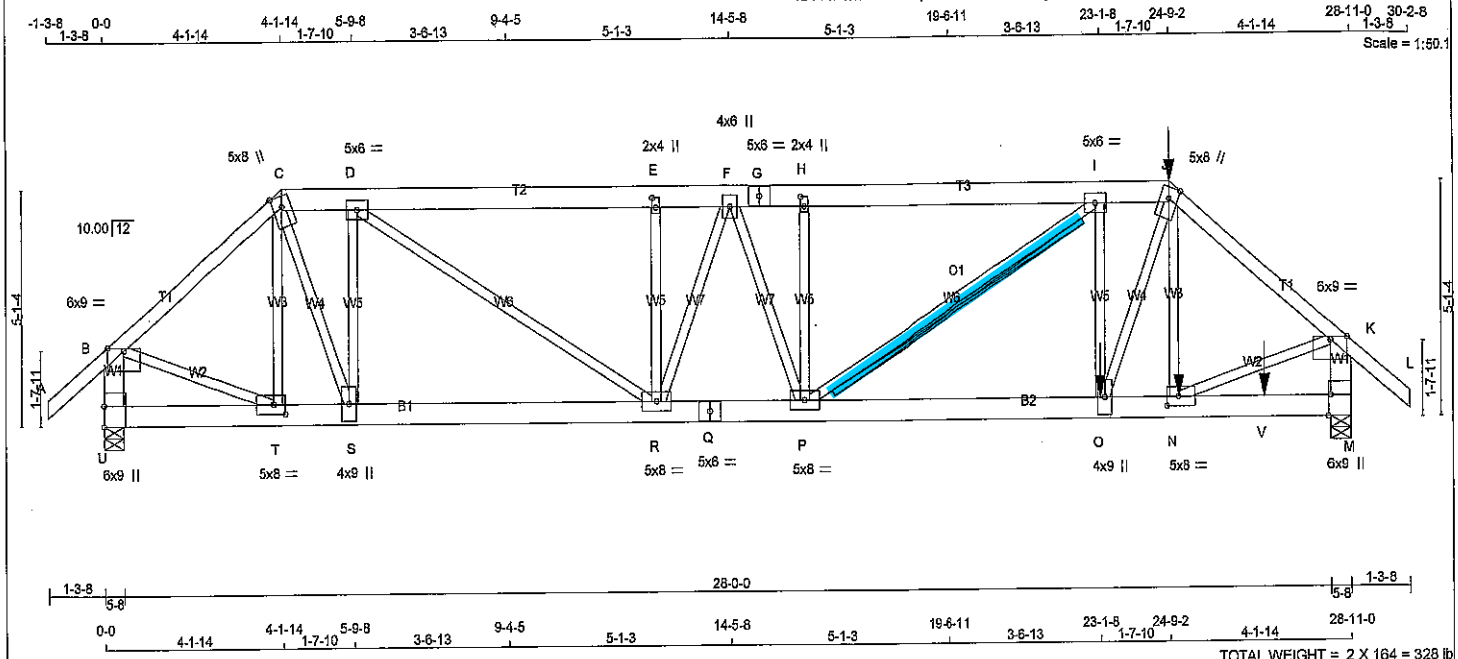

TOTAL WEIGHT = 2 X 164 = 328 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
J- L	1	12	SIDE(61.0)
C- G	2	12	TOP
G- J	2	12	SIDE(61.0)
U- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	TOP
Q- M	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
N- J	1	6	SIDE(30.3)
O- I	1	2	SIDE(1026.6)
2x3	1	6	
S- D	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	5.0	8.0	Edge	
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+t	MT20	6.0	9.0	Edge	0.50
N	BMWW-t	MT20	5.0	8.0	2.50	3.25
O	BMWW+t	MT20	4.0	9.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3150	0	3150	0	0	5-8	5-8
M	6879	0	6879	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2474	1578 / 0	450 / 0	0 / 0	0 / 0	446 / 0	0 / 0
M	5395	3455 / 0	971 / 0	0 / 0	0 / 0	969 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M
BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT I-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-3165 / 0	-104.9	-104.9	0.23 (1)	4.99
C- D	-3343 / 0	-104.9	-104.9	0.11 (1)	5.97
D- E	-5970 / 0	-104.9	-104.9	0.21 (1)	4.66
E- F	-5970 / 0	-104.9	-104.9	0.16 (1)	4.72
F- G	-6814 / 0	-104.9	-104.9	0.20 (1)	4.43
G- H	-6814 / 0	-104.9	-104.9	0.20 (1)	4.43
H- I	-6814 / 0	-104.9	-104.9	0.26 (1)	4.39
I- J	-7264 / 0	-104.9	-104.9	0.11 (1)	4.42
J- K	-7304 / 0	-104.9	-104.9	0.45 (1)	3.34
K- L	0 / 47	-104.9	-104.9	0.08 (1)	10.00
U- B	-3118 / 0	0.0	0.0	0.12 (1)	7.81
M- K	-6719 / 0	0.0	0.0	0.25 (1)	5.75
U- T	0 / 0	-28.0	-28.0	0.04 (1)	10.00
T- S	0 / 2451	-28.0	-28.0	0.20 (1)	10.00
S- R	0 / 3341	-28.0	-28.0	0.27 (1)	10.00
R- Q	0 / 6350	-28.0	-28.0	0.45 (1)	10.00
Q- P	0 / 6350	-28.0	-28.0	0.45 (1)	10.00
P- O	0 / 7263	-28.0	-28.0	0.58 (1)	10.00
O- N	0 / 5641	-28.0	-28.0	0.47 (1)	10.00
N- V	0 / 0	-28.0	-28.0	0.11 (1)	10.00
V- M	0 / 0	-28.0	-28.0	0.11 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-622 / 0	0.11 (1)
R- E	-639 / 0	0.11 (1)
R- F	-1102 / 0	0.22 (1)
F- P	0 / 1344	0.17 (1)
P- H	-718 / 0	0.13 (1)
N- J	-728 / 0	0.13 (1)
B- T	0 / 2546	0.32 (1)
N- K	0 / 5874	0.73 (1)
S- D	-2404 / 0	0.42 (1)
O- I	-240 / 84	0.04 (1)
O- R	0 / 3161	0.39 (1)
O- J	0 / 4906	0.61 (1)
P- J	-544 / 0	0.19 (1)
C- S	0 / 2688	0.33 (1)
D- R	0 / 3161	0.39 (1)
O- J	0 / 4906	0.61 (1)
P- J	-544 / 0	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	24-9-2	-584	-584		FRONT	VERT	TOTAL
N	24-11-12	-337	-337		FRONT	VERT	TOTAL
C	23-1-8	-4637	-4637		FRONT	VERT	TOTAL
V	26-11-12	-337	-337		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.45/1.00 (J-K:1) , BC=0.58/1.00 (O-P:1) , WB=0.73/1.00 (K-N:1) , SSI=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284 1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.68 (O) (INPUT = 1.00)

PROFESSIONAL ENGINEER 22678 S. KATSOULAKOS ONTARIO

DWG NO. TAM 11122-08

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T4Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMWW+t	MT20	4.0	9.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.25
U	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 583.8 lbs FACTORED DOWN AT 24-9-2 ON TOP CHORD, AND 4637.2 lbs FACTORED DOWN AT 23-1-8, AND 336.5 lbs FACTORED DOWN AT 24-11-12, AND 336.5 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
92878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11/22
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T4S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:22 2018 Page 1

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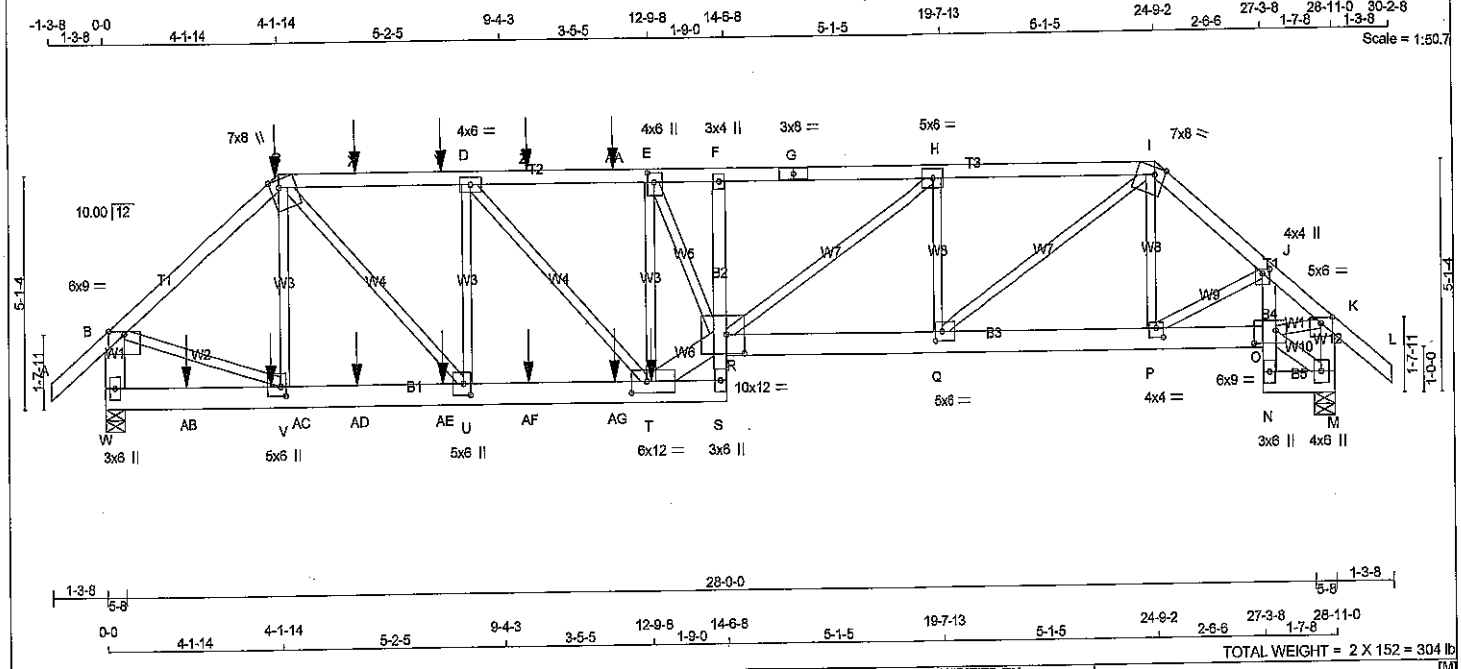

TOTAL WEIGHT = 2 X 152 = 304 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x6	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C	1 12	SIDE(61.0)
C- G	1 12	SIDE(61.0)
G- I	1 12	TOP
I- L	1 12	TOP
M- K	1 12	TOP
W- B	2 12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
W- S	2 12	SIDE(183.1)
R- O	2 12	TOP
N- M	2 12	TOP
S- F	1 12	TOP
J- N	1 12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
V- C	1 6	SIDE(30.3)
E- T	1 5	SIDE(499.6)
2x3	1 6	
2x4	1 6	
2x6	2 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.25
D	TMWW-t	MT20	4.0	6.0		
E	TMWW+t	MT20	4.0	6.0	2.50	1.75
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5707	0	5707	0	0	5-8	5-8
M	3863	0	3863	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4502	2838 / 0	840 / 0	0 / 0	0 / 0	824 / 0	0 / 0
M	3034	1937 / 0	551 / 0	0 / 0	0 / 0	547 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-5976 / 0	-104.9	-104.9	0.36 (1)	3.74	
C- X	-7497 / 0	-104.9	-104.9	0.45 (1)	3.25	
X- Y	-7497 / 0	-104.9	-104.9	0.45 (1)	3.25	
Y- D	-7497 / 0	-104.9	-104.9	0.45 (1)	3.25	
D- Z	-9090 / 0	-104.9	-104.9	0.56 (1)	2.86	
Z-AA	-9090 / 0	-104.9	-104.9	0.56 (1)	2.86	
AA- E	-9090 / 0	-104.9	-104.9	0.56 (1)	2.86	
E- F	-10633 / 0	-104.9	-104.9	0.45 (1)	2.69	
F- G	-10694 / 0	-104.9	-104.9	0.84 (1)	2.43	
G- H	-10694 / 0	-104.9	-104.9	0.84 (1)	2.43	
H- I	-7805 / 0	-104.9	-104.9	0.57 (1)	3.13	
I- J	-5057 / 0	-104.9	-104.9	0.17 (1)	4.17	
J- K	-4770 / 0	-104.9	-104.9	0.15 (1)	4.29	
K- L	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
W- B	-5516 / 0	0.0	0.0	0.20 (1)	6.25	
M- K	-3748 / 0	0.0	0.0	0.21 (1)	6.08	
W-AB	0 / 0	-28.0	-28.0	0.12 (1)	10.00	
AB-AC	0 / 0	-28.0	-28.0	0.12 (1)	10.00	
AC- V	0 / 0	-28.0	-28.0	0.12 (1)	10.00	
V-AD	0 / 4571	-28.0	-28.0	0.42 (1)	10.00	
AD-AE	0 / 4571	-28.0	-28.0	0.42 (1)	10.00	
AE- U	0 / 4571	-28.0	-28.0	0.42 (1)	10.00	
U-AF	0 / 7497	-28.0	-28.0	0.61 (1)	10.00	
AF-AG	0 / 7497	-28.0	-28.0	0.61 (1)	10.00	
AG- T	0 / 7497	-28.0	-28.0	0.61 (1)	10.00	
T- S	0 / 227	-28.0	-28.0	0.14 (1)	10.00	
S- R	-185 / 0	0.0	0.0	0.17 (1)	7.81	
R- F	-248 / 11	0.0	0.0	0.17 (1)	7.81	
R- Q	0 / 7806	-28.0	-28.0	0.56 (1)	10.00	
Q- P	0 / 3875	-28.0	-28.0	0.31 (1)	10.00	
P- O	0 / 3724	-28.0	-28.0	0.30 (1)	10.00	
N- O	0 / 36	0.0	0.0	0.11 (1)	10.00	
O- J	-447 / 0	0.0	0.0	0.11 (1)	7.81	
N- M	0 / 134	-28.0	-28.0	0.01 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-382 / 79	0.07 (1)
C- U	0 / 4301	0.53 (1)
U- D	-2396 / 0	0.45 (1)
R- H	0 / 3566	0.44 (1)
Q- H	-2718 / 0	0.34 (1)
Q- I	0 / 4623	0.60 (1)
P- I	0 / 180	0.02 (3)
P- J	0 / 167	0.02 (1)
B- V	0 / 4763	0.59 (1)
T- E	-3964 / 0	0.74 (1)
T- R	0 / 10444	0.68 (1)
D- T	0 / 2379	0.29 (1)
E- R	0 / 3905	0.48 (1)
O- K	0 / 3649	0.45 (1)
O- M	-164 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC	MAX-	MAX+	FACE	DIR	TYPE
C	4-1-14	1	-584	-584	---	FRONT VERT	TOTAL
T	12-9-8	1	-2617	-2617	---	FRONT VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 781 (0.44")

CSI: TC=0.84/1.00 (F-H:1) , BC=0.61/1.00 (T-U:1) , WB=0.74/1.00 (E-T:1) , SSI=0.20/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111096-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T4S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:VfPm7TsSZ1qPV0OYGr9uMczgf9K-wAQRSc1qrmgxd3YeqUn_mH6GzKXUthSMWH2JdczfLrA

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TTWW-m	MT20	7.0	6.0	Edge	2.75
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMVW1+p	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-I	MT20	6.0	9.0	3.25	6.00
P	BMWW-t	MT20	4.0	4.0	2.50	2.00
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BVMWWW-I	MT20	10.0	12.0	5.00	5.00
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	12.0	3.00	4.25
U	BMWW+t	MT20	5.0	6.0	3.00	2.00
V	BMWW+t	MT20	5.0	6.0	2.50	1.50
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 583.8 lbs FACTORED DOWN AT 4-1-14, 53.9 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, AND 53.9 lbs FACTORED DOWN AT 9-11-4, AND 53.9 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, AND 336.6 lbs FACTORED DOWN AT 11-11-4, AND 2617.0 lbs FACTORED DOWN AT 12-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	5-11-4	-54	-54	—	FRONT	VERT	TOTAL
Y	7-11-4	-54	-54	—	FRONT	VERT	TOTAL
Z	9-11-4	-54	-54	—	FRONT	VERT	TOTAL
AA	11-11-4	-54	-54	—	FRONT	VERT	TOTAL
AB	1-11-4	-337	-337	—	FRONT	VERT	TOTAL
AC	3-11-4	-337	-337	—	FRONT	VERT	TOTAL
AD	5-11-4	-337	-337	—	FRONT	VERT	TOTAL
AE	7-11-4	-337	-337	—	FRONT	VERT	TOTAL
AF	9-11-4	-337	-337	—	FRONT	VERT	TOTAL
AG	11-11-4	-337	-337	—	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.60 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2098
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM11096
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:26 2018 Page 1

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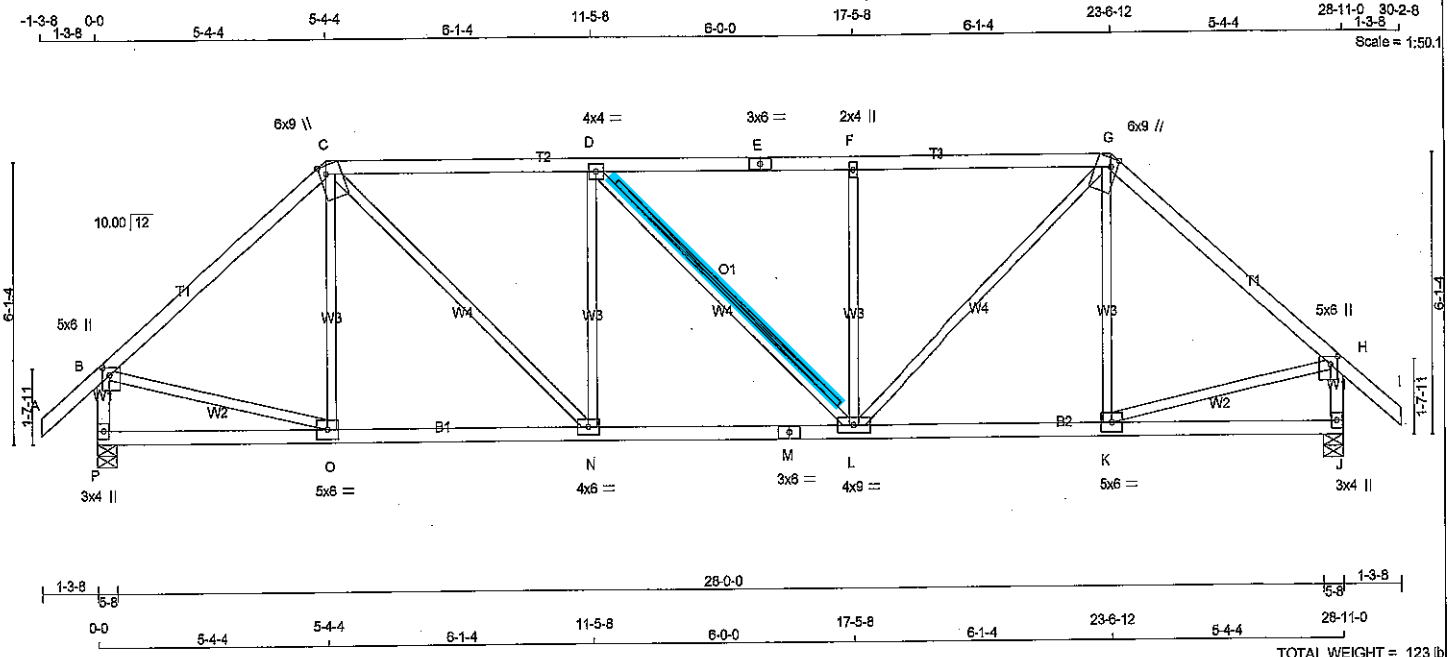

TOTAL WEIGHT = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	6.0	9.0	Edge	1.75
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1916 / 0	-104.9	-104.9	0.70 (1)	4.06
C- D	-2364 / 0	-104.9	-104.9	0.71 (1)	3.67
D- E	-2363 / 0	-104.9	-104.9	0.71 (1)	3.67
E- F	-2363 / 0	-104.9	-104.9	0.71 (1)	3.67
F- G	-2363 / 0	-104.9	-104.9	0.71 (1)	3.69
G- H	-1916 / 0	-104.9	-104.9	0.70 (1)	4.06
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
P- B	-2007 / 0	0.0	0.0	0.21 (1)	5.96
J- H	-2008 / 0	0.0	0.0	0.21 (1)	5.96
P- O	0 / 0	-28.0	-28.0	0.22 (3)	10.00
O- N	0 / 1466	-28.0	-28.0	0.38 (2)	10.00
N- M	0 / 2364	-28.0	-28.0	0.48 (1)	10.00
M- L	0 / 2364	-28.0	-28.0	0.48 (1)	10.00
L- K	0 / 1466	-28.0	-28.0	0.38 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-171 / 160	0.10 (1)
C- N	0 / 1239	0.28 (1)
N- D	-687 / 0	0.40 (1)
D- L	-2 / 0	0.00 (1)
L- F	-686 / 0	0.40 (1)
L- G	0 / 1236	0.28 (1)
K- G	-170 / 160	0.10 (1)
B- O	0 / 1510	0.34 (1)
K- H	0 / 1510	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.48/1.00 (L-N:1) , WB=0.40/1.00 (D-N:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2876
S. KATSOULAKOS
PROFESSIONAL ENGINEER PROVINCE OF ONTARIO

DWG NO. TAM 110911-MB
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T5S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:23 2018 Page 1

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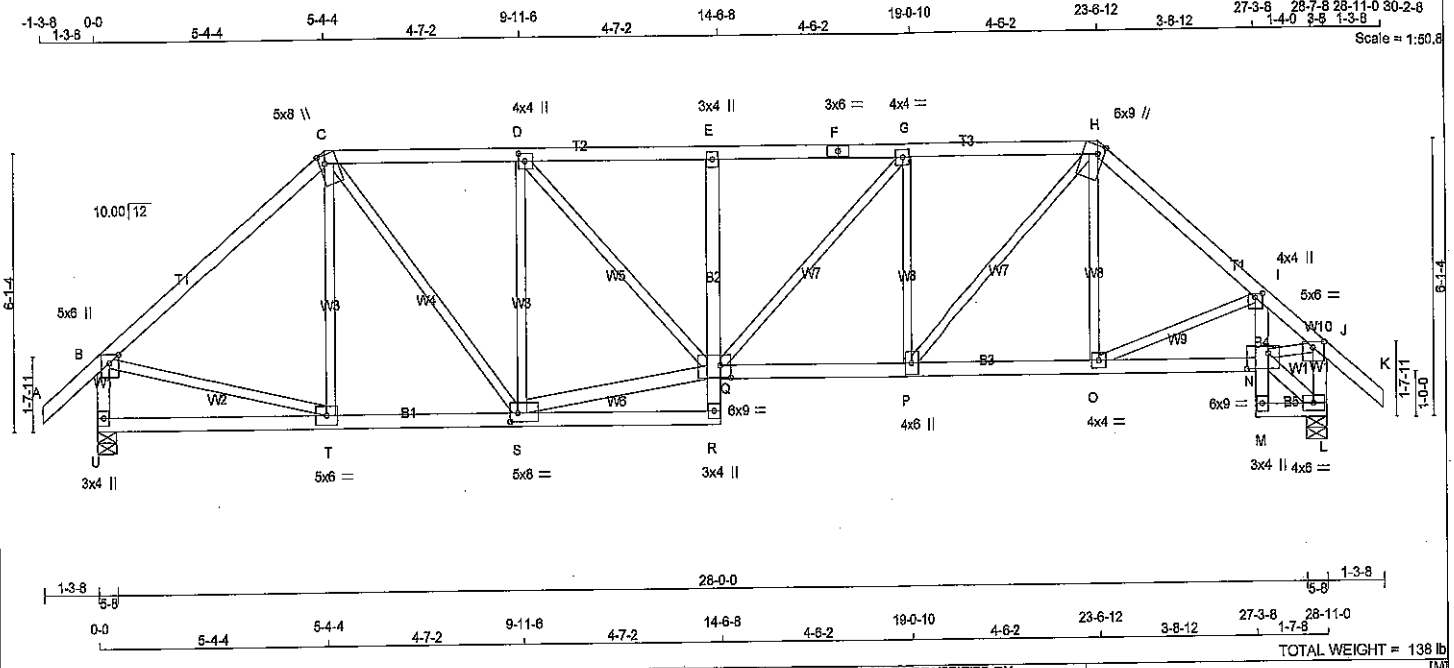


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TMWW+t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	4.00	6.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BVMWWW-I	MT20	6.0	9.0	3.25	3.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	5.0	6.0	2.25	2.25
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2064	0	2064	0	0	5-8	5-8
L	2071	0	2071	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1628	1027 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1633	1031 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

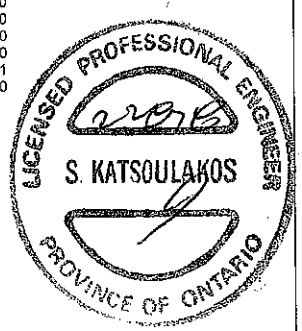
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B-C	-1908 / 0	-104.9	-104.9	0.70 (1)	4.07	
C-D	-2187 / 0	-104.9	-104.9	0.39 (1)	4.20	
D-E	-2890 / 0	-104.9	-104.9	0.45 (1)	3.67	
E-F	-2904 / 0	-104.9	-104.9	0.46 (1)	3.66	
F-G	-2904 / 0	-104.9	-104.9	0.46 (1)	3.66	
G-H	-2651 / 0	-104.9	-104.9	0.44 (1)	3.83	
H-I	-2290 / 0	-104.9	-104.9	0.31 (1)	4.25	
I-J	-2502 / 0	-104.9	-104.9	0.20 (1)	4.17	
J-K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
U-B	-2001 / 0	0.0	0.0	0.21 (1)	5.97	
L-J	-1999 / 0	0.0	0.0	0.21 (1)	5.97	
U-T	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
T-S	0 / 1459	-28.0	-28.0	0.34 (2)	10.00	
S-R	0 / 65	-28.0	-28.0	0.12 (2)	10.00	
R-Q	0 / 92	0.0	0.0	0.10 (1)	10.00	
Q-E	-431 / 0	0.0	0.0	0.14 (1)	7.81	
Q-P	0 / 2651	-28.0	-28.0	0.50 (1)	10.00	
P-O	0 / 1745	-28.0	-28.0	0.35 (1)	10.00	
O-N	0 / 1976	-28.0	-28.0	0.38 (1)	10.00	
M-N	0 / 36	0.0	0.0	0.11 (1)	10.00	
N-I	-72 / 56	0.0	0.0	0.10 (1)	7.81	
M-L	0 / 64	-28.0	-28.0	0.03 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-192 / 124	0.11 (1)
C-S	0 / 1183	0.27 (1)
S-D	-1277 / 0	0.75 (1)
S-Q	0 / 2188	0.35 (1)
D-Q	0 / 1014	0.23 (1)
Q-G	0 / 368	0.08 (1)
P-G	-804 / 0	0.31 (1)
P-H	0 / 1305	0.29 (1)
O-H	0 / 221	0.05 (2)
O-I	-249 / 0	0.06 (1)
B-T	0 / 1504	0.34 (1)
N-L	-82 / 0	0.01 (1)
N-J	0 / 1927	0.43 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.50/1.00 (P-Q:1) , WB=0.75/1.00 (D-S:1) , SSI=0.23/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.48 (J) (INPUT = 1.00)

DWG NO. TAM 11097-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287531	T6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:05:26 2018 Page 1

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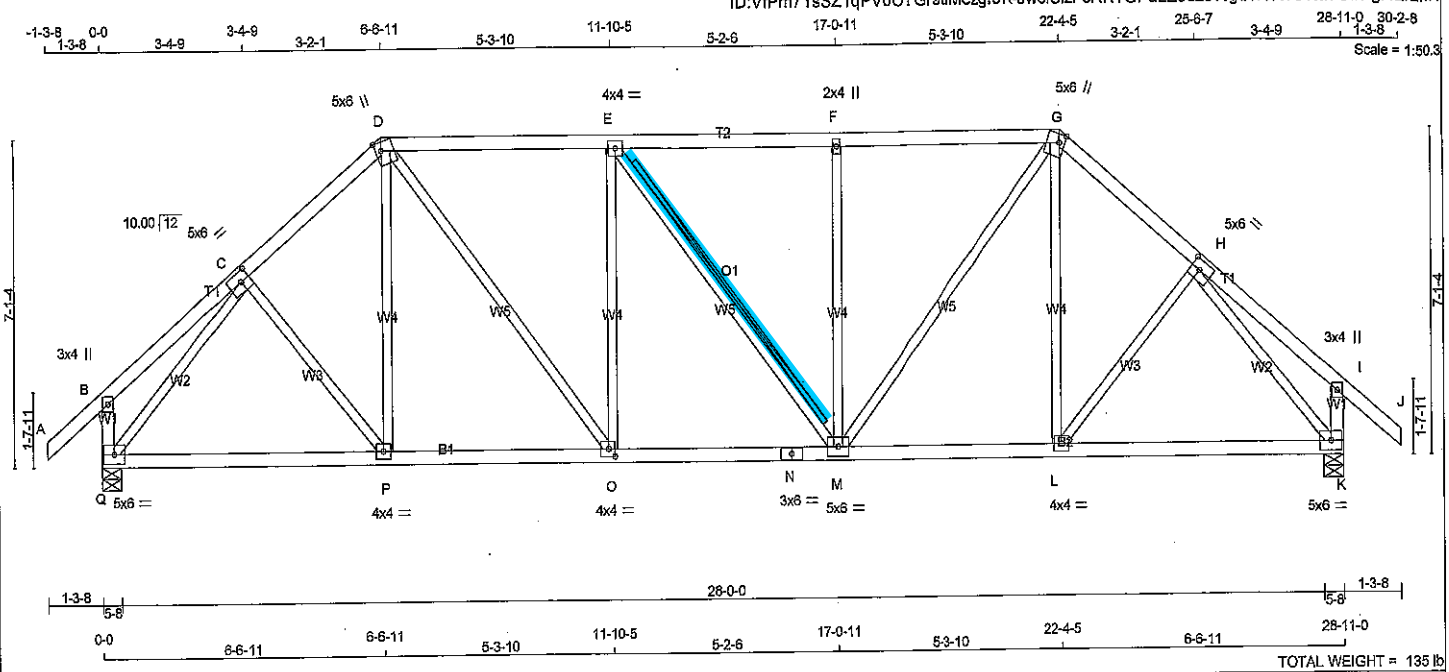

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2067	0	2067	0	0	5-8	5-8
K	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- P	0 / 152	0.03 (2)
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	P- D	0 / 231	0.05 (3)
C- D	-1917 / 0	-104.9	-104.9	0.21 (1)	4.66	D- O	0 / 925	0.21 (1)
D- E	-2022 / 0	-104.9	-104.9	0.49 (1)	4.22	O- E	-594 / 0	0.52 (1)
E- F	-2020 / 0	-104.9	-104.9	0.49 (1)	4.22	E- M	-3 / 0	0.00 (1)
F- G	-2020 / 0	-104.9	-104.9	0.48 (1)	4.23	M- F	-594 / 0	0.52 (1)
G- H	-1917 / 0	-104.9	-104.9	0.21 (1)	4.66	M- G	0 / 922	0.21 (1)
H- I	0 / 23	-104.9	-104.9	0.16 (1)	10.00	L- G	0 / 231	0.05 (3)
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	L- H	0 / 152	0.03 (2)
Q- B	-280 / 0	0.0	0.0	0.03 (1)	7.81	Q- C	-2201 / 0	0.95 (1)
K- I	-280 / 0	0.0	0.0	0.03 (1)	7.81	H- K	-2201 / 0	0.95 (1)
Q- P	0 / 1385	-28.0	-28.0	0.41 (2)	10.00			
P- O	0 / 1452	-28.0	-28.0	0.43 (2)	10.00			
O- N	0 / 2022	-28.0	-28.0	0.39 (1)	10.00			
N- M	0 / 2022	-28.0	-28.0	0.39 (1)	10.00			
M- L	0 / 1453	-28.0	-28.0	0.43 (2)	10.00			
L- K	0 / 1385	-28.0	-28.0	0.42 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.43/1.00 (L-M:2) , WB=0.95/1.00 (H-K:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.54 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11092-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T6S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:23 2018 Page 1

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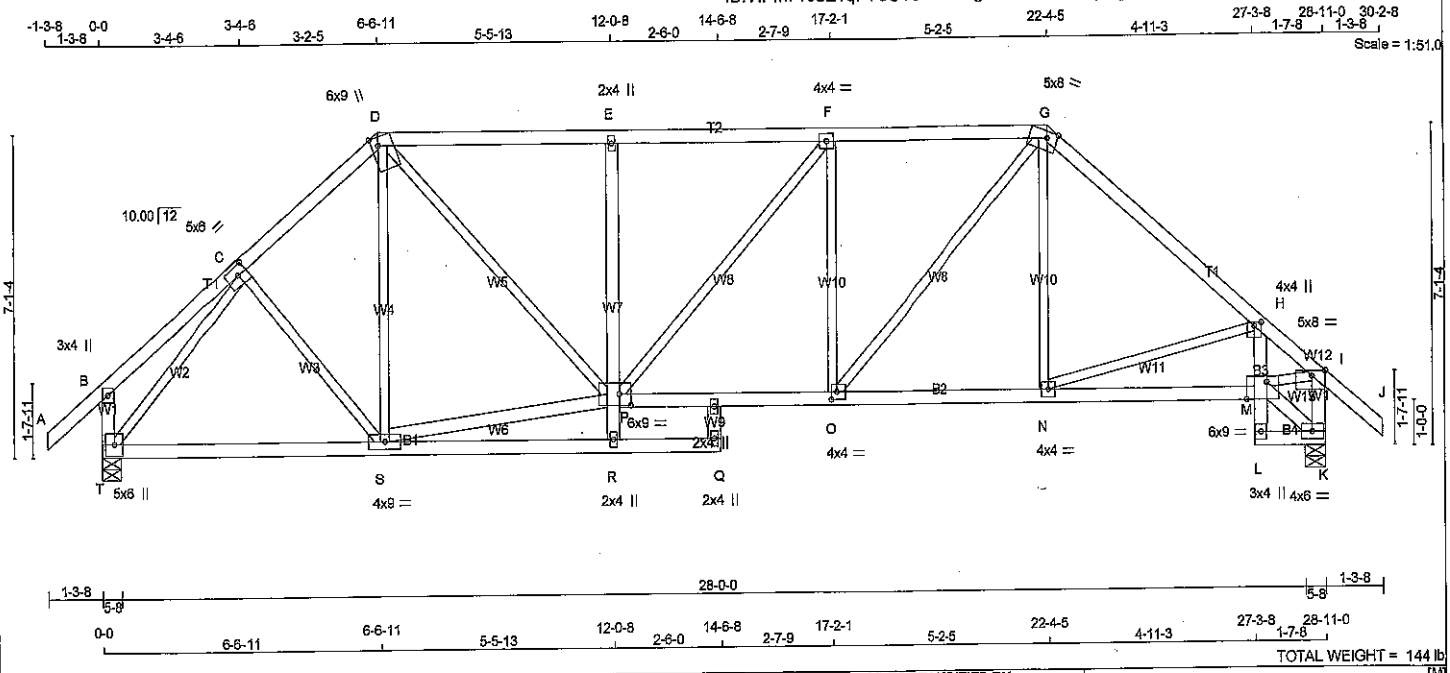

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-I	MT20	6.0	9.0	4.50	5.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.50
P	BWMWWW"-I	MT20	6.0	9.0	3.00	3.25
Q	NP+w	MT20	2.0	4.0		
R	BMW+w	MT20	2.0	4.0		
S	BMWWW-t	MT20	4.0	9.0		
T	BMVW1+p	MT20	5.0	6.0		
U	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	2100	0	2100	0	0	5-8	5-8
T	2105	0	2105	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1669	1031 / 0	325 / 0	0 / 0	0 / 0	312 / 0	0 / 0
T	1679	1027 / 0	334 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, T

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	
C- D	-1967 / 0	-104.9	-104.9	0.22 (1)	4.81	
D- E	-2443 / 0	-104.9	-104.9	0.55 (1)	3.84	
E- F	-2450 / 0	-104.9	-104.9	0.53 (1)	3.83	
F- G	-2410 / 0	-104.9	-104.9	0.49 (1)	3.92	
G- H	-2261 / 0	-104.9	-104.9	0.51 (1)	4.10	
H- I	-2603 / 0	-104.9	-104.9	0.36 (1)	3.89	
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
T- B	-279 / 0	0.0	0.0	0.03 (1)	7.81	
K- I	-2026 / 0	0.0	0.0	0.22 (1)	5.94	
T- S	0 / 1417	-28.0	-28.0	0.46 (2)	10.00	
S- R	0 / 46	-28.0	-28.0	0.31 (3)	10.00	
R- Q	0 / 0	-28.0	-28.0	0.21 (2)	10.00	
P- O	0 / 2410	-28.0	-28.0	0.47 (1)	10.00	
O- N	0 / 1716	-28.0	-28.0	0.38 (1)	10.00	
N- M	0 / 2095	-28.0	-28.0	0.44 (1)	10.00	
L- M	0 / 36	0.0	0.0	0.11 (1)	10.00	
M- H	-85 / 68	0.0	0.0	0.11 (1)	7.81	
L- K	0 / 67	-28.0	-28.0	0.03 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- D	-140 / 117	0.12 (1)
N- G	0 / 292	0.07 (2)
N- H	-396 / 0	0.20 (1)
R- P	0 / 215	0.08 (1)
P- E	-604 / 0	0.14 (1)
S- P	0 / 1466	0.24 (1)
D- P	0 / 1388	0.31 (1)
O- F	-622 / 0	0.36 (1)
O- G	0 / 1040	0.23 (1)
P- F	0 / 60	0.01 (1)
M- K	-85 / 0	0.01 (1)
M- I	0 / 2044	0.46 (1)
C- S	0 / 171	0.04 (2)
T- C	-2253 / 0	0.96 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.55/1.00 (D-E:1) , BC=0.47/1.00 (O-P:1) , WB=0.96/1.00 (C-T:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)

DWG NO. TAM 11098 -T6
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:24 2018 Page 1

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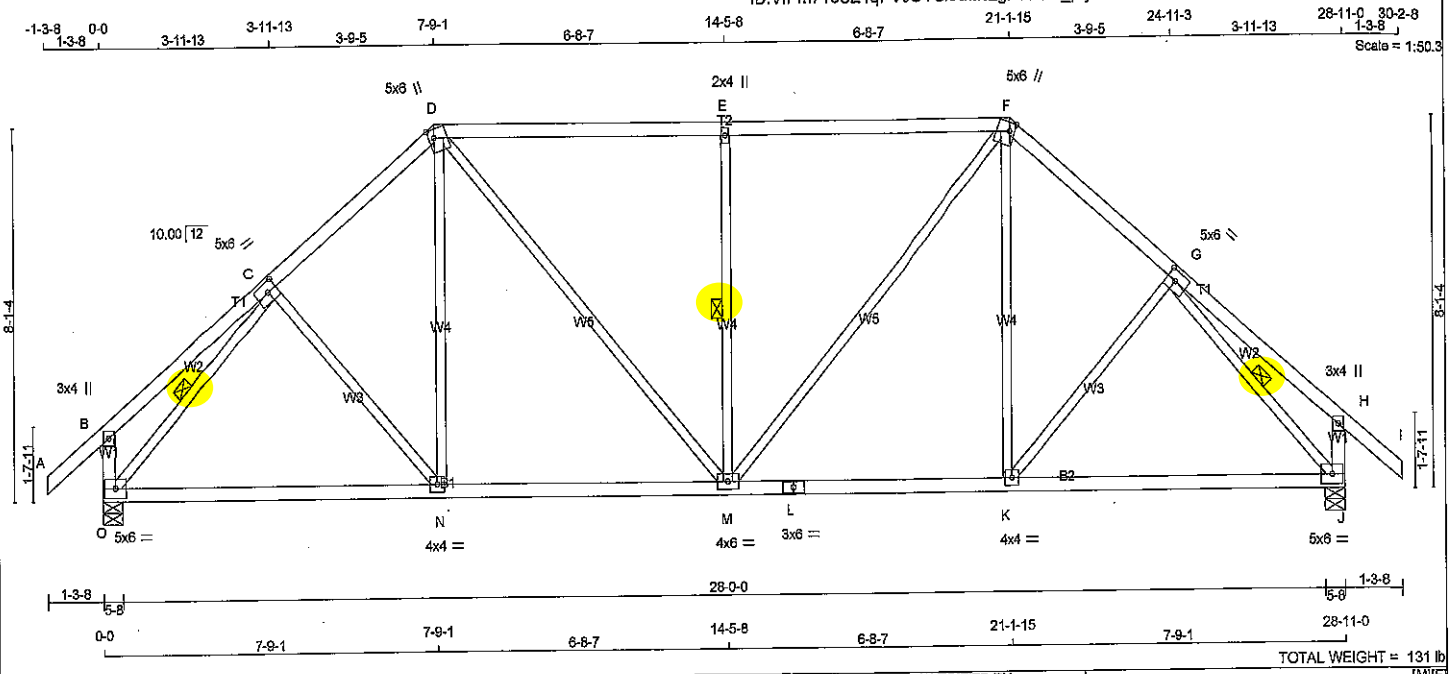

TOTAL WEIGHT = 131 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-M, C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- N	-16 / 117	0.03 (3)
B- C	0 / 29	-104.9	-104.9	0.24 (1)	10.00	N- D	0 / 351	0.08 (2)
C- D	-1882 / 0	-104.9	-104.9	0.23 (1)	4.69	D- M	0 / 670	0.15 (1)
D- E	-1859 / 0	-104.9	-104.9	0.67 (1)	4.04	M- E	-862 / 0	0.36 (1)
E- F	-1859 / 0	-104.9	-104.9	0.67 (1)	4.04	M- F	0 / 670	0.15 (1)
F- G	-1882 / 0	-104.9	-104.9	0.23 (1)	4.69	K- F	0 / 351	0.08 (2)
G- H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	K- G	-16 / 117	0.03 (3)
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O- C	-2203 / 0	0.55 (1)
O- B	-302 / 0	0.0	0.0	0.03 (1)	7.81	G- J	-2203 / 0	0.55 (1)
J- H	-302 / 0	0.0	0.0	0.03 (1)	7.81			
O- N	0 / 1430	-28.0	-28.0	0.55 (2)	10.00			
N- M	0 / 1423	-28.0	-28.0	0.56 (2)	10.00			
M- L	0 / 1423	-28.0	-28.0	0.56 (2)	10.00			
L- K	0 / 1423	-28.0	-28.0	0.56 (2)	10.00			
K- J	0 / 1430	-28.0	-28.0	0.55 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.67/1.00 (D-E:1) , BC=0.56/1.00 (M-N:2) , WB=0.55/1.00 (C-O:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.54 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 2018 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 110997-S8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:24 2018 Page 1
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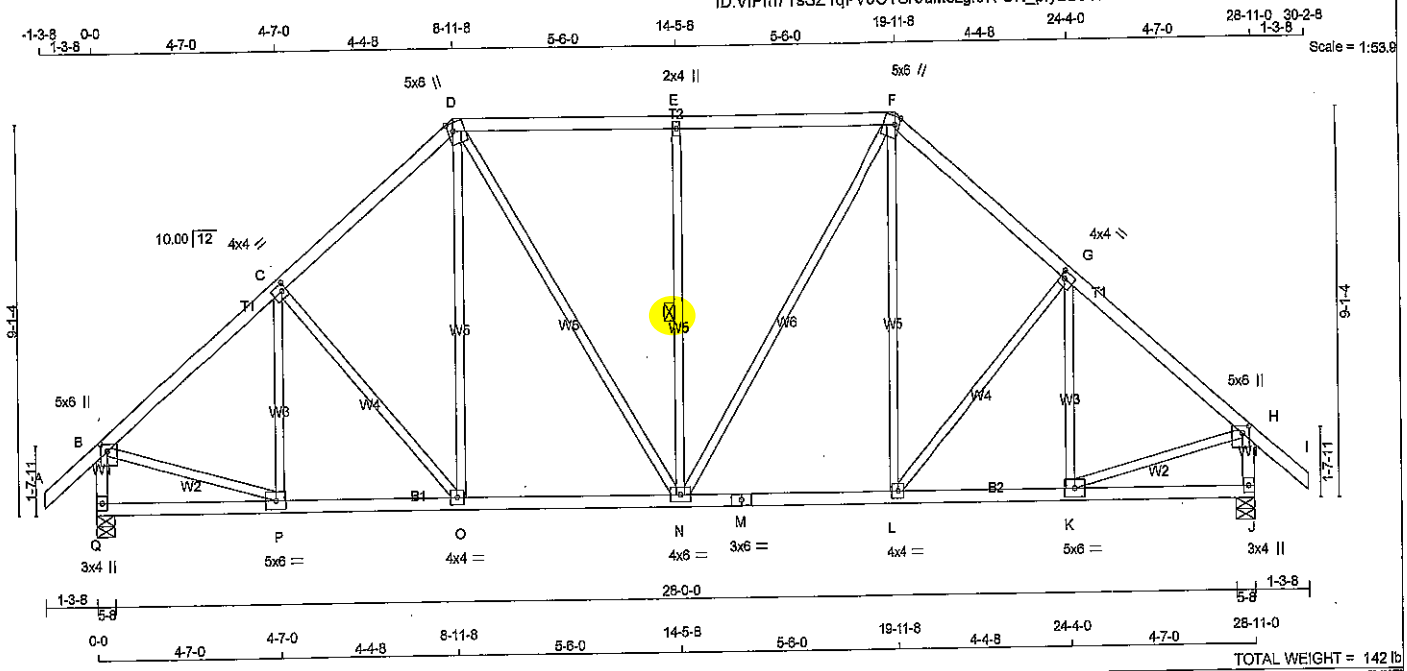

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B-C	-1922 / 0	-104.9	-104.9	0.32 (1)	4.56	
C-D	-1805 / 0	-104.9	-104.9	0.31 (1)	4.68	
D-E	-1627 / 0	-104.9	-104.9	0.44 (1)	4.68	
E-F	-1627 / 0	-104.9	-104.9	0.44 (1)	4.68	
F-G	-1805 / 0	-104.9	-104.9	0.31 (1)	4.68	
G-H	-1922 / 0	-104.9	-104.9	0.32 (1)	4.56	
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
Q-B	-2012 / 0	0.0	0.0	0.21 (1)	5.95	
J-H	-2012 / 0	0.0	0.0	0.21 (1)	5.95	
Q-P	0 / 0	-28.0	-28.0	0.13 (3)	10.00	
P-O	0 / 1503	-28.0	-28.0	0.32 (1)	10.00	
O-N	0 / 1359	-28.0	-28.0	0.32 (2)	10.00	
N-M	0 / 1359	-28.0	-28.0	0.32 (2)	10.00	
M-L	0 / 1359	-28.0	-28.0	0.32 (2)	10.00	
L-K	0 / 1503	-28.0	-28.0	0.32 (1)	10.00	
K-J	0 / 0	-28.0	-28.0	0.13 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-295 / 62	0.13 (1)
C-O	-225 / 0	0.19 (1)
O-D	0 / 340	0.08 (2)
D-N	0 / 506	0.11 (1)
N-E	-703 / 0	0.38 (1)
N-F	0 / 506	0.11 (1)
L-F	0 / 340	0.08 (2)
L-G	-225 / 0	0.19 (1)
K-G	-295 / 62	0.13 (1)
B-P	0 / 1563	0.35 (1)
K-H	0 / 1563	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.32/1.00 (N-O:2) , WB=0.38/1.00 (E-N:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.42 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11100-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T9	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, inc. Thu Mar 1 09:01:24 2018 Page 1
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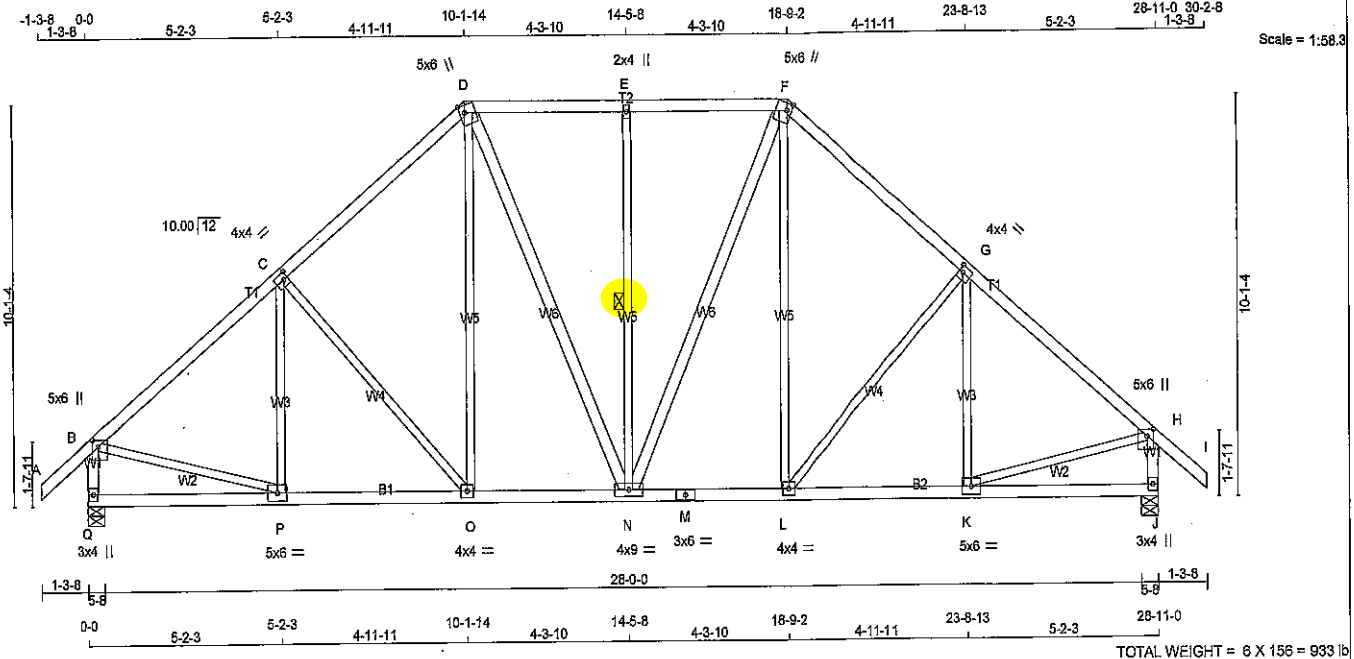

TOTAL WEIGHT = 6 X 156 = 933 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.42 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1943 / 0	-104.9	-104.9	0.42 (1)	4.42	
C- D	-1731 / 0	-104.9	-104.9	0.40 (1)	4.65	
D- E	-1443 / 0	-104.9	-104.9	0.26 (1)	5.16	
E- F	-1443 / 0	-104.9	-104.9	0.26 (1)	5.16	
F- G	-1731 / 0	-104.9	-104.9	0.40 (1)	4.65	
G- H	-1943 / 0	-104.9	-104.9	0.42 (1)	4.42	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-2007 / 0	0.0	0.0	0.21 (1)	5.96	
J- H	-2007 / 0	0.0	0.0	0.21 (1)	5.96	
Q- P	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
P- O	0 / 1523	-28.0	-28.0	0.34 (2)	10.00	
O- N	0 / 1299	-28.0	-28.0	0.27 (1)	10.00	
N- M	0 / 1299	-28.0	-28.0	0.27 (1)	10.00	
M- L	0 / 1299	-28.0	-28.0	0.27 (1)	10.00	
L- K	0 / 1523	-28.0	-28.0	0.34 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-223 / 126	0.12 (1)
C- O	-341 / 0	0.39 (1)
O- D	0 / 382	0.09 (2)
D- N	0 / 358	0.06 (1)
N- E	-542 / 0	0.38 (1)
N- F	0 / 358	0.05 (1)
L- F	0 / 382	0.09 (2)
L- G	-341 / 0	0.39 (1)
K- G	-223 / 126	0.12 (1)
B- P	0 / 1571	0.35 (1)
K- H	0 / 1571	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.34/1.00 (O-P:2) , WB=0.39/1.00 (G-L:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11101 -T9
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Thu Mar 1 09:01:25 2018 Page 1
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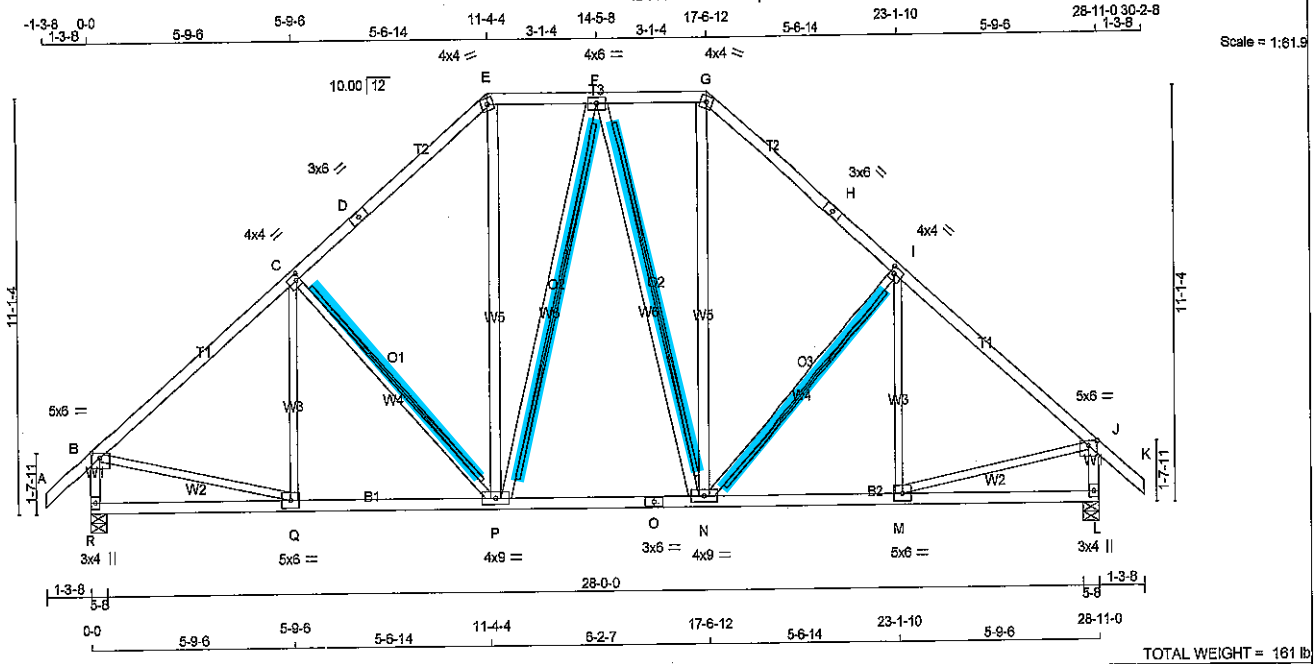

TOTAL WEIGHT = 161 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2067	0	2067	0	0	5-8	5-8
L	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-P, I-N
2x4 DRY SPF No.2 T-BRACE AT F-P, F-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	Q- C	-169 / 169	0.11 (1)
B- C	-1950 / 0	-104.9	-104.9	0.55 (1)	4.26	C- P	-444 / 0	0.52 (1)
C- D	-1651 / 0	-104.9	-104.9	0.50 (1)	4.60	P- E	0 / 873	0.11 (1)
D- E	-1651 / 0	-104.9	-104.9	0.50 (1)	4.60	P- F	-194 / 0	0.18 (1)
E- F	-1239 / 0	-104.9	-104.9	0.12 (1)	5.65	F- N	-194 / 0	0.18 (1)
F- G	-1239 / 0	-104.9	-104.9	0.12 (1)	5.65	N- G	0 / 873	0.11 (1)
G- H	-1651 / 0	-104.9	-104.9	0.50 (1)	4.60	N- I	-444 / 0	0.52 (1)
H- I	-1651 / 0	-104.9	-104.9	0.50 (1)	4.60	M- I	-169 / 169	0.11 (1)
I- J	-1950 / 0	-104.9	-104.9	0.55 (1)	4.26	B- Q	0 / 1572	0.35 (1)
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	M- J	0 / 1572	0.35 (1)
R- B	-2001 / 0	0.0	0.0	0.21 (1)	5.97			
L- J	-2001 / 0	0.0	0.0	0.21 (1)	5.97			
R- Q	0 / 0	-28.0	-28.0	0.22 (3)	10.00			
Q- P	0 / 1533	-28.0	-28.0	0.38 (2)	10.00			
P- O	0 / 1290	-28.0	-28.0	0.32 (2)	10.00			
O- N	0 / 1290	-28.0	-28.0	0.32 (2)	10.00			
N- M	0 / 1533	-28.0	-28.0	0.38 (2)	10.00			
M- L	0 / 0	-28.0	-28.0	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.55/1.00 (I-J:1) , BC=0.38/1.00 (P-Q:2) , WB=0.52/1.00 (I-N:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.41 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2207
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11102-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:25 2018 Page 1

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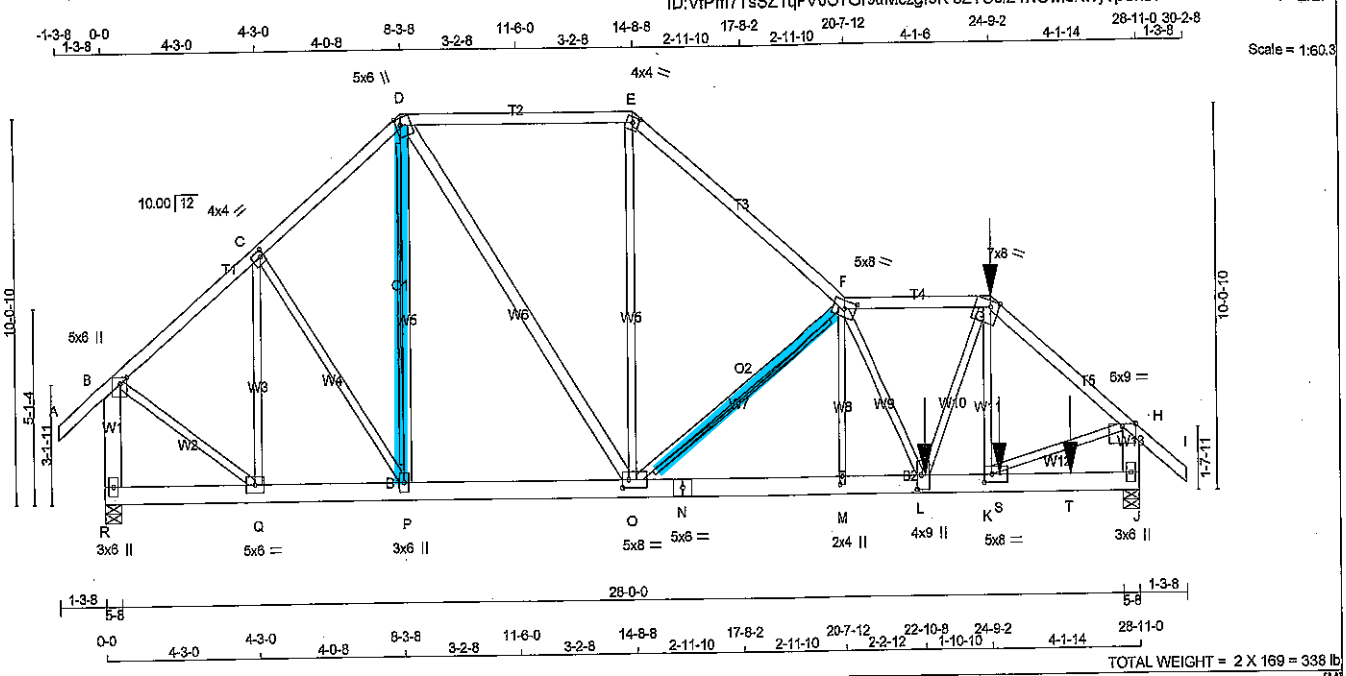

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	SIDE(61.0)
G- I	1	12	SIDE(61.0)
R- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
K- G	1	6	SIDE(30.3)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	8.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0	1.75	2.00
F	TTWWW-m	MT20	5.0	8.0	2.50	3.50
G	TTWW-m	MT20	7.0	8.0	Edge	2.75
H	TMWW-p	MT20	8.0	9.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWW+t	MT20	4.0	9.0	4.50	1.25
M	BMW+w	MT20	2.0	4.0	2.50	1.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0	2.50	2.00
P	BMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3042	0	3042	0	0	5-8	5-8
J	6228	0	6228	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2397	1515 / 0	445 / 0	0 / 0	0 / 0	437 / 0	0 / 0
J	4916	3094 / 0	920 / 0	0 / 0	0 / 0	902 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-P
2x6 DRY SPF No.2 T-BRACE AT F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	Q- C	-1047 / 0	0.37 (1)
B- C	-2325 / 0	-104.9	-104.9	0.20 (1)	5.66	C- P	0 / 384	0.05 (1)
C- D	-2675 / 0	-104.9	-104.9	0.20 (1)	5.36	P- D	-175 / 106	0.09 (1)
D- E	-2922 / 0	-104.9	-104.9	0.50 (1)	4.77	D- O	0 / 1645	0.15 (1)
E- F	-3778 / 0	-104.9	-104.9	0.55 (1)	4.38	O- E	0 / 1771	0.22 (1)
F- G	-6544 / 0	-104.9	-104.9	0.34 (1)	3.56	O- F	-4765 / 0	0.89 (1)
G- H	-6585 / 0	-104.9	-104.9	0.40 (1)	3.54	K- G	-380 / 32	0.07 (1)
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00	B- Q	0 / 2151	0.27 (1)
R- B	-2987 / 0	0.0	0.0	0.17 (1)	7.81	K- H	0 / 5249	0.65 (1)
J- H	-6042 / 0	0.0	0.0	0.22 (1)	6.01	L- G	0 / 3697	0.48 (1)
						F- L	-271 / 0	0.06 (1)
R- Q	0 / 0	-28.0	-28.0	0.03 (2)	10.00	M- F	0 / 267	0.04 (2)
Q- P	0 / 1811	-28.0	-28.0	0.14 (1)	10.00			
P- O	0 / 2021	-28.0	-28.0	0.17 (1)	10.00			
O- N	0 / 6662	-28.0	-28.0	0.49 (1)	10.00			
N- M	0 / 6662	-28.0	-28.0	0.49 (1)	10.00			
M- L	0 / 6657	-28.0	-28.0	0.60 (1)	10.00			
L- K	0 / 5037	-28.0	-28.0	0.50 (1)	10.00			
K- S	0 / 0	-28.0	-28.0	0.13 (1)	10.00			
S- T	0 / 0	-28.0	-28.0	0.13 (1)	10.00			
T- J	0 / 0	-28.0	-28.0	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	24-9-2	-584	-584	---	FRONT	VERT	TOTAL
L	22-10-8	-3879	-3879	---	FRONT	VERT	TOTAL
S	24-11-12	-337	-337	---	FRONT	VERT	TOTAL
T	26-11-12	-337	-337	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.55/1.00 (E-F;1) , BC=0.60/1.00 (L-M;1) , WB=0.89/1.00 (F-O;1) , SSI=0.20/1.00 (J-K;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.84 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11103 -17
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T11	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 583.8 lbs FACTORED DOWN AT 24-9-2 ON TOP CHORD, AND 3878.9 lbs FACTORED DOWN AT 22-10-8, AND 336.6 lbs FACTORED DOWN AT 24-11-12, AND 336.6 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
0095
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1/103-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:26 2018 Page 1

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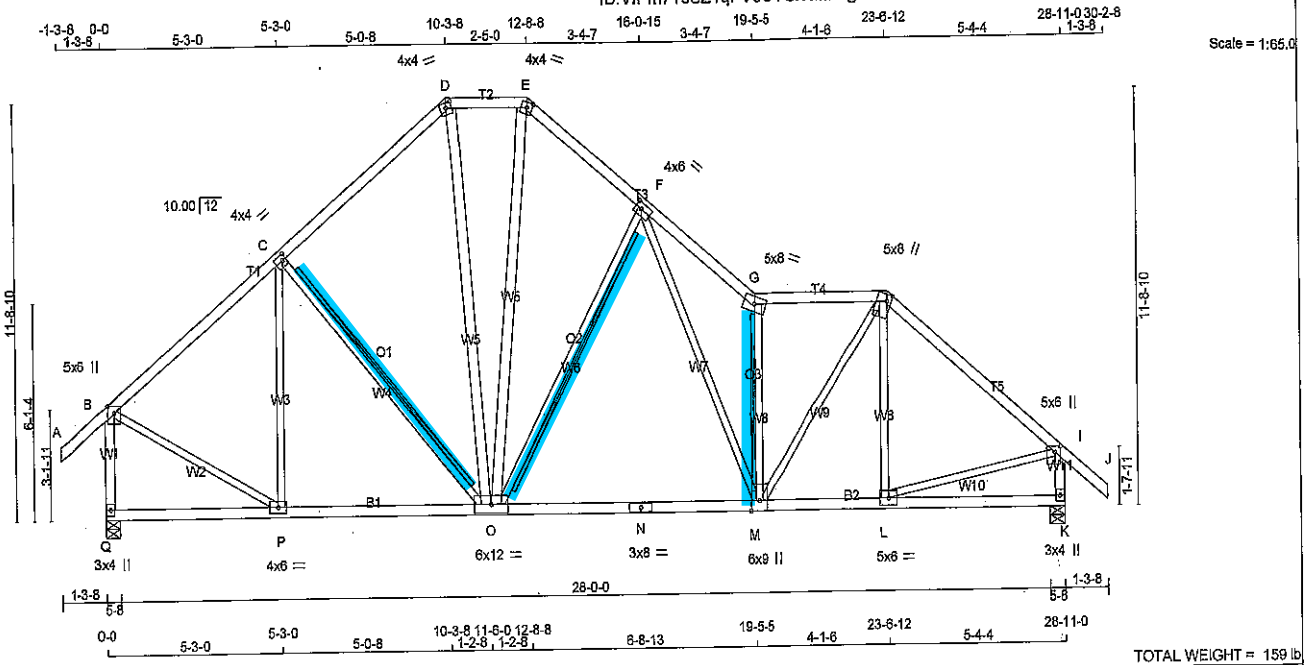


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - O	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0	1.75	2.00
E	TTW-m	MT20	4.0	4.0	1.75	2.00
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TTW-m	MT20	5.0	8.0		
H	TTWW+m	MT20	5.0	8.0	2.25	1.50
I	TMVW+p	MT20	5.0	6.0	2.00	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW+t	MT20	6.0	9.0	Edge	
N	BS-t	MT20	3.0	8.0		
O	BMWWWW*m	MT20	6.0	12.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2067	0	2067	0	0	5-8	5-8
K	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-O, F-O, G-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

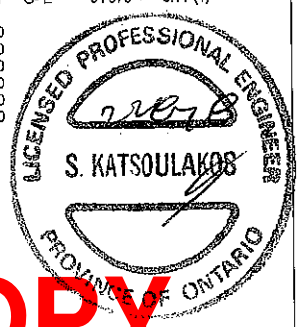
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B-C	-1530 / 0	-104.9	-104.9	0.41 (1)	4.87
C-D	-1490 / 0	-104.9	-104.9	0.40 (1)	4.93
D-E	-1173 / 0	-104.9	-104.9	0.09 (1)	5.60
E-F	-1469 / 0	-104.9	-104.9	0.19 (1)	5.22
F-G	-2712 / 0	-104.9	-104.9	0.26 (1)	4.01
G-H	-2136 / 0	-104.9	-104.9	0.28 (1)	4.39
H-I	-1910 / 0	-104.9	-104.9	0.47 (1)	4.39
I-J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q-B	-2007 / 0	0.0	0.0	0.35 (1)	5.96
K-I	-2003 / 0	0.0	0.0	0.21 (1)	5.97
Q-P	0 / 0	-28.0	-28.0	0.19 (3)	10.00
P-O	0 / 1207	-28.0	-28.0	0.46 (2)	10.00
O-N	0 / 1606	-28.0	-28.0	0.50 (2)	10.00
N-M	0 / 1606	-28.0	-28.0	0.50 (2)	10.00
M-L	0 / 1461	-28.0	-28.0	0.38 (2)	10.00
L-K	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-483 / 18	0.49 (1)
C-O	-146 / 0	0.11 (1)
D-O	0 / 608	0.10 (1)
O-F	-1069 / 0	0.81 (1)
F-M	0 / 1412	0.32 (1)
M-G	-2093 / 0	0.55 (1)
M-H	0 / 1154	0.26 (1)
L-H	-207 / 85	0.12 (1)
B-P	0 / 1368	0.31 (1)
L-I	0 / 1506	0.34 (1)
O-E	0 / 673	0.11 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.47/1.00 (H-I:1) , BC=0.50/1.00 (M-O:2) , WB=0.81/1.00 (F-O:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.67 (N) (INPUT = 1.00)

DWG NO. TAM 1/1 04 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T130	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:29 2018 Page 1
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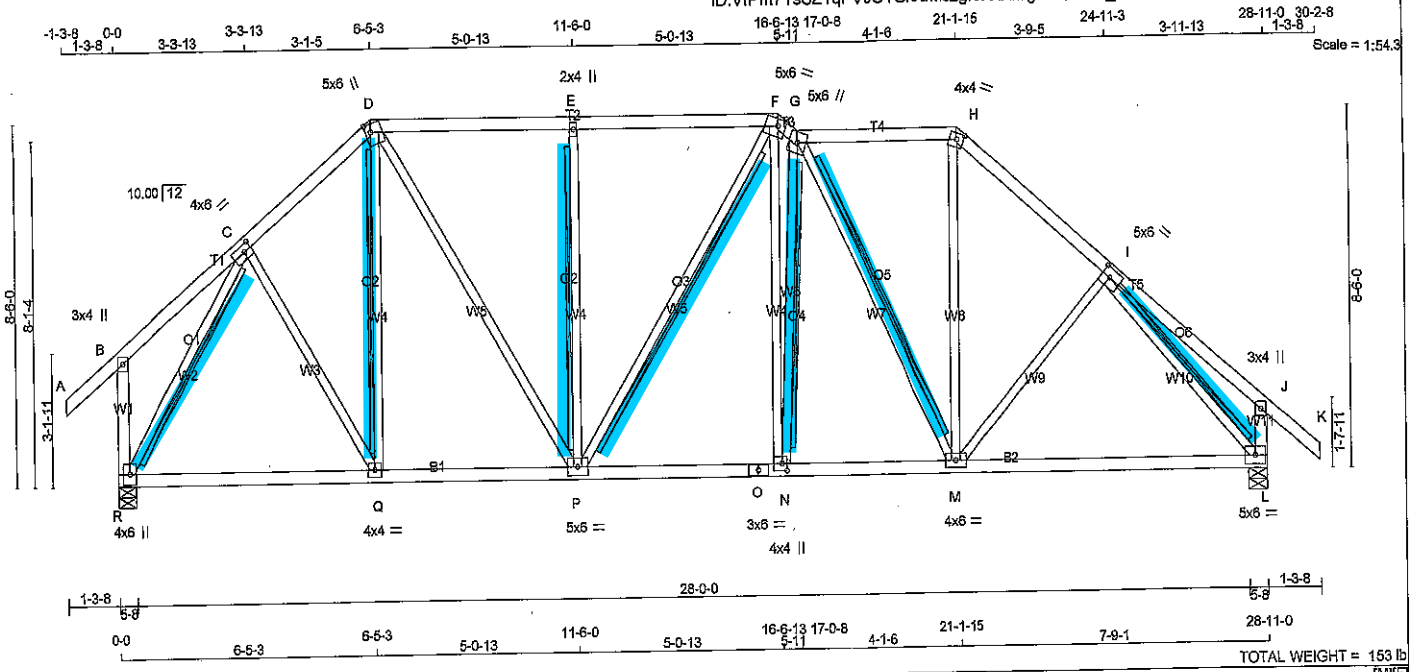

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TTWW+m	MT20	5.0	6.0		
H	TTW-m	MT20	4.0	4.0	1.75	2.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	4.0	2.00	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2067	0	2067	0	0	5-8	5-8
L	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Q, E-P, G-M
2x4 DRY SPF No.2 T-BRACE AT F-P, G-N, C-R, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C- Q	0 / 353	0.08 (1)
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	Q- D	-115 / 119	0.13 (1)
C- D	-1577 / 0	-104.9	-104.9	0.15 (1)	5.12	D- P	0 / 910	0.20 (1)
D- E	-1667 / 0	-104.9	-104.9	0.38 (1)	4.73	P- E	-644 / 0	0.73 (1)
E- F	-1667 / 0	-104.9	-104.9	0.38 (1)	4.73	P- F	-13 / 9	0.01 (3)
F- G	-2182 / 0	-104.9	-104.9	0.05 (1)	4.60	N- F	0 / 1155	0.26 (1)
G- H	-1428 / 0	-104.9	-104.9	0.25 (1)	5.19	N- G	-1057 / 0	0.50 (1)
H- I	-1878 / 0	-104.9	-104.9	0.23 (1)	4.71	M- H	0 / 812	0.18 (1)
I- J	0 / 28	-104.9	-104.9	0.24 (1)	10.00	M- I	-15 / 117	0.03 (3)
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	R- C	-1988 / 0	0.60 (1)
R- B	-279 / 0	0.0	0.0	0.05 (1)	7.81	I- L	-2199 / 0	0.59 (1)
L- J	-303 / 0	0.0	0.0	0.03 (1)	7.81	G- M	-654 / 0	0.89 (1)
R- Q	0 / 1009	-28.0	-28.0	0.36 (2)	10.00			
Q- P	0 / 1188	-28.0	-28.0	0.39 (2)	10.00			
P- O	0 / 1663	-28.0	-28.0	0.35 (1)	10.00			
O- N	0 / 1663	-28.0	-28.0	0.35 (1)	10.00			
N- M	0 / 1728	-28.0	-28.0	0.55 (2)	10.00			
M- L	0 / 1427	-28.0	-28.0	0.51 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.55/1.00 (M-N:2) , WB=0.89/1.00 (G-M:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22978
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11106-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T131	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:29 2018 Page 1

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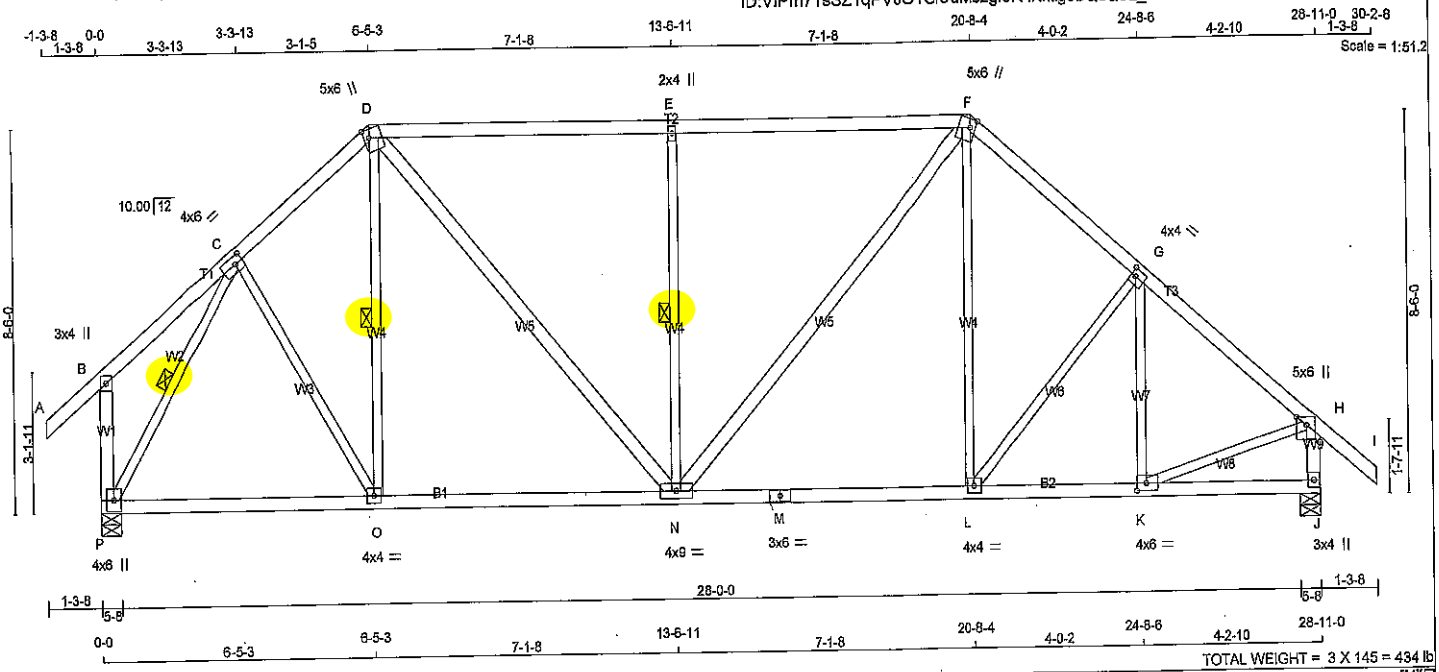

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2067	0	2067	0	0	5-8	5-8
J	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-O, E-N, C-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	10.00
B- C	0 / 23	-104.9	-104.9	0.16 (1)	10.00	10.00
C- D	-1581 / 0	-104.9	-104.9	0.15 (1)	5.11	5.11
D- E	-1779 / 0	-104.9	-104.9	0.76 (1)	3.91	3.91
E- F	-1779 / 0	-104.9	-104.9	0.76 (1)	3.91	3.91
F- G	-1848 / 0	-104.9	-104.9	0.27 (1)	4.69	4.69
G- H	-1901 / 0	-104.9	-104.9	0.27 (1)	4.63	4.63
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	10.00
P- B	-279 / 0	0.0	0.0	0.05 (1)	7.81	7.81
J- H	-2014 / 0	0.0	0.0	0.21 (1)	5.95	5.95
P- O	0 / 1011	-28.0	-28.0	0.41 (2)	10.00	
O- N	0 / 1191	-28.0	-28.0	0.44 (2)	10.00	
N- M	0 / 1394	-28.0	-28.0	0.43 (2)	10.00	
M- L	0 / 1394	-28.0	-28.0	0.43 (2)	10.00	
L- K	0 / 1485	-28.0	-28.0	0.36 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.11 (2)	10.00	

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	0 / 354	0.08 (1)
C- D	-92 / 158	0.04 (1)
D- N	0 / 896	0.14 (1)
N- E	-920 / 0	0.42 (1)
N- F	0 / 586	0.09 (1)
L- F	0 / 343	0.06 (2)
L- G	-145 / 0	0.10 (1)
K- G	-352 / 4	0.14 (1)
P- C	-1991 / 0	0.55 (1)
K- H	0 / 1554	0.35 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.76/1.00 (D-E:1) , BC=0.44/1.00 (N-O:2) , WB=0.55/1.00 (C-P:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.47 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11107-48
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:26 2018 Page 1

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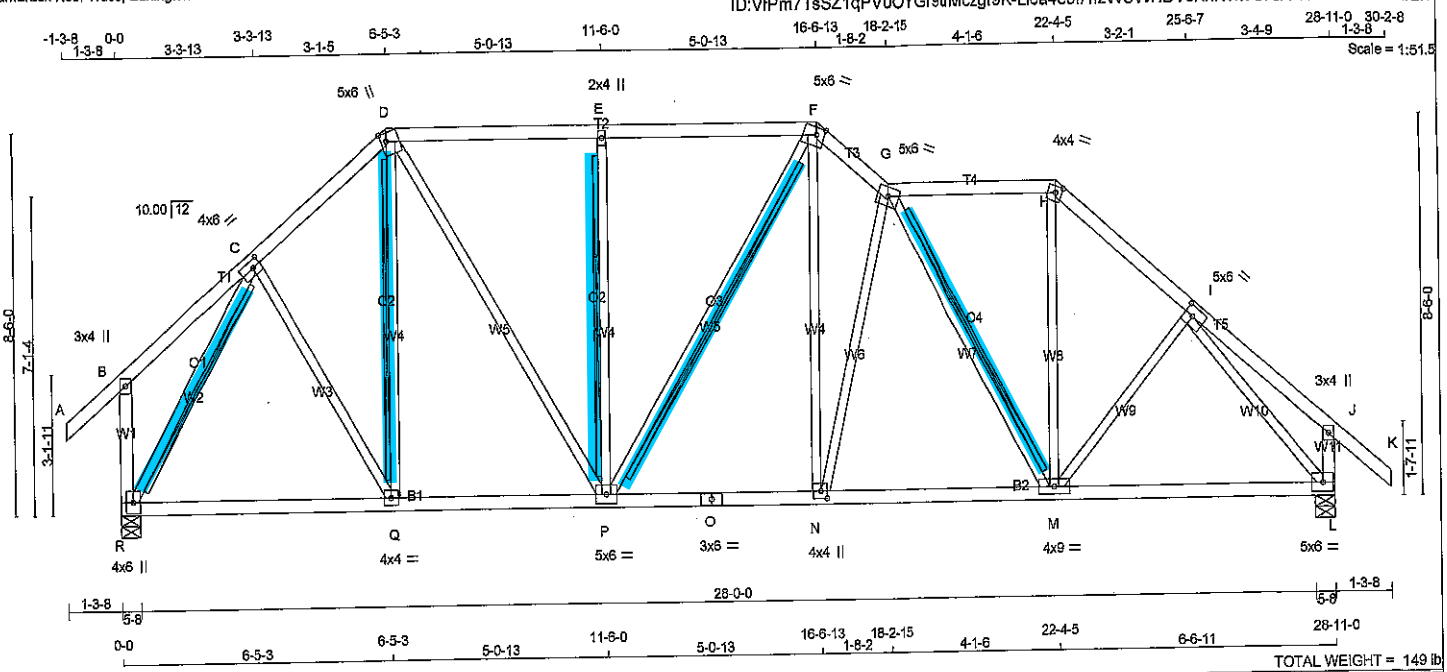

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TTWW-m	MT20	5.0	6.0		
H	TTW-m	MT20	4.0	4.0	1.75	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW+t	MT20	4.0	4.0	2.00	1.75
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2067	0	2067	0	0	5-8	5-8
L	2067	0	2067	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
L	1630	1029 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-Q, E-P, G-M
2x4 DRY SPF No.2 T-BRACE AT F-P, C-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 23	-104.9	-104.9	0.16 (1)		10.00
C- D	-1576 / 0	-104.9	-104.9	0.15 (1)		5.12
D- E	-1666 / 0	-104.9	-104.9	0.38 (1)		4.74
E- F	-1666 / 0	-104.9	-104.9	0.38 (1)		4.74
F- G	-2153 / 0	-104.9	-104.9	0.09 (1)		4.59
G- H	-1464 / 0	-104.9	-104.9	0.25 (1)		5.15
H- I	-1917 / 0	-104.9	-104.9	0.17 (1)		4.73
I- J	0 / 22	-104.9	-104.9	0.15 (1)		10.00
J- K	0 / 47	-104.9	-104.9	0.14 (1)		10.00
R- B	-279 / 0	0.0	0.0	0.05 (1)		7.81
L- J	-262 / 0	0.0	0.0	0.03 (1)		7.81
R- Q	0 / 1009	-28.0	-28.0	0.37 (2)		10.00
Q- P	0 / 1188	-28.0	-28.0	0.39 (2)		10.00
P- O	0 / 1668	-28.0	-28.0	0.33 (1)		10.00
O- N	0 / 1668	-28.0	-28.0	0.33 (1)		10.00
N- M	0 / 1901	-28.0	-28.0	0.49 (2)		10.00
M- L	0 / 1384	-28.0	-28.0	0.43 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
C- Q	0 / 353	0.08 (1)
Q- D	-112 / 125	0.13 (1)
D- P	0 / 907	0.20 (1)
P- E	-644 / 0	0.73 (1)
P- F	-35 / 0	0.03 (3)
N- F	0 / 1084	0.24 (1)
N- G	-975 / 0	0.93 (1)
M- H	0 / 863	0.19 (1)
M- I	0 / 153	0.03 (2)
R- C	-1988 / 0	0.60 (1)
I- L	-2199 / 0	0.95 (1)
G- M	-859 / 0	0.87 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.49/1.00 (M-N:2) , WB=0.95/1.00 (I-L:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.54 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2006
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11105-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:26 2018 Page 1
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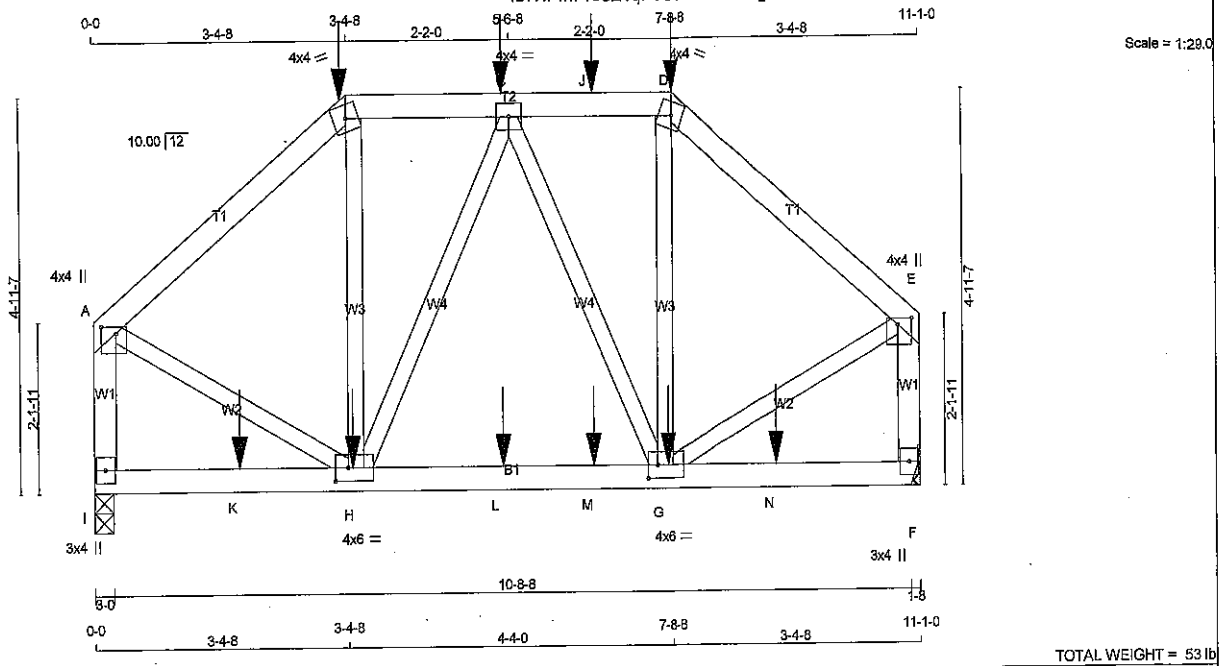

TOTAL WEIGHT = 53 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.25
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.25
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0	2.00	1.50
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 267.0 lbs FACTORED DOWN AT 3-4-8, 72.1 lbs FACTORED DOWN AT 5-5-4, AND 72.1 lbs FACTORED DOWN AT 6-7-12, AND 267.0 lbs FACTORED DOWN AT 7-8-8 ON TOP CHORD, AND 33.7 lbs FACTORED DOWN AT 1-11-4, 33.7 lbs FACTORED DOWN AT 3-5-4, 33.7 lbs FACTORED DOWN AT 5-5-4, 33.7 lbs FACTORED DOWN AT 6-7-12, AND 33.7 lbs FACTORED DOWN AT 7-7-12, AND 33.7 lbs FACTORED DOWN AT 9-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1125	0	1125	0	0	3-0	3-0
F	1142	0	1142	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	882	563 / 0	158 / 0	0 / 0	0 / 0	160 / 0	0 / 0
F	896	572 / 0	161 / 0	0 / 0	0 / 0	163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-869 / 0	-104.9	-104.9	0.24 (1)	6.23	H-B	0 / 171	0.04 (3)
B-C	-666 / 0	-104.9	-104.9	0.10 (1)	6.25	G-D	-1 / 174	0.04 (3)
C-J	-679 / 0	-104.9	-104.9	0.11 (1)	6.25	A-H	0 / 750	0.19 (1)
J-D	-679 / 0	-104.9	-104.9	0.11 (1)	6.25	G-E	0 / 765	0.19 (1)
D-E	-887 / 0	-104.9	-104.9	0.24 (1)	6.18	H-C	-224 / 0	0.10 (1)
I-A	-1081 / 0	0.0	0.0	0.14 (1)	7.56	C-G	-192 / 0	0.09 (1)
F-E	-1100 / 0	0.0	0.0	0.14 (1)	7.51			
I-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
K-H	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
H-L	0 / 757	-28.0	-28.0	0.22 (2)	10.00			
L-M	0 / 757	-28.0	-28.0	0.22 (2)	10.00			
M-G	0 / 757	-28.0	-28.0	0.22 (2)	10.00			
G-N	0 / 0	-28.0	-28.0	0.15 (3)	10.00			
N-F	0 / 0	-28.0	-28.0	0.15 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-4-8	-267	-267	---	BACK	VERT	TOTAL
C	5-5-4	-72	-72	---	BACK	VERT	TOTAL
D	7-8-8	-267	-267	---	BACK	VERT	TOTAL
G	7-7-12	-19	-34	---	BACK	VERT	TOTAL
H	3-5-4	-19	-34	---	BACK	VERT	TOTAL
J	6-7-12	-72	-72	---	BACK	VERT	TOTAL
K	1-11-4	-19	-34	---	BACK	VERT	TOTAL
L	5-5-4	-19	-34	---	BACK	VERT	TOTAL
M	6-7-12	-19	-34	---	BACK	VERT	TOTAL
N	9-1-12	-19	-34	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.22/1.00 (G-H:2) , WB=0.19/1.00 (E-G:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.25 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11108-T8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:27 2018 Page 1

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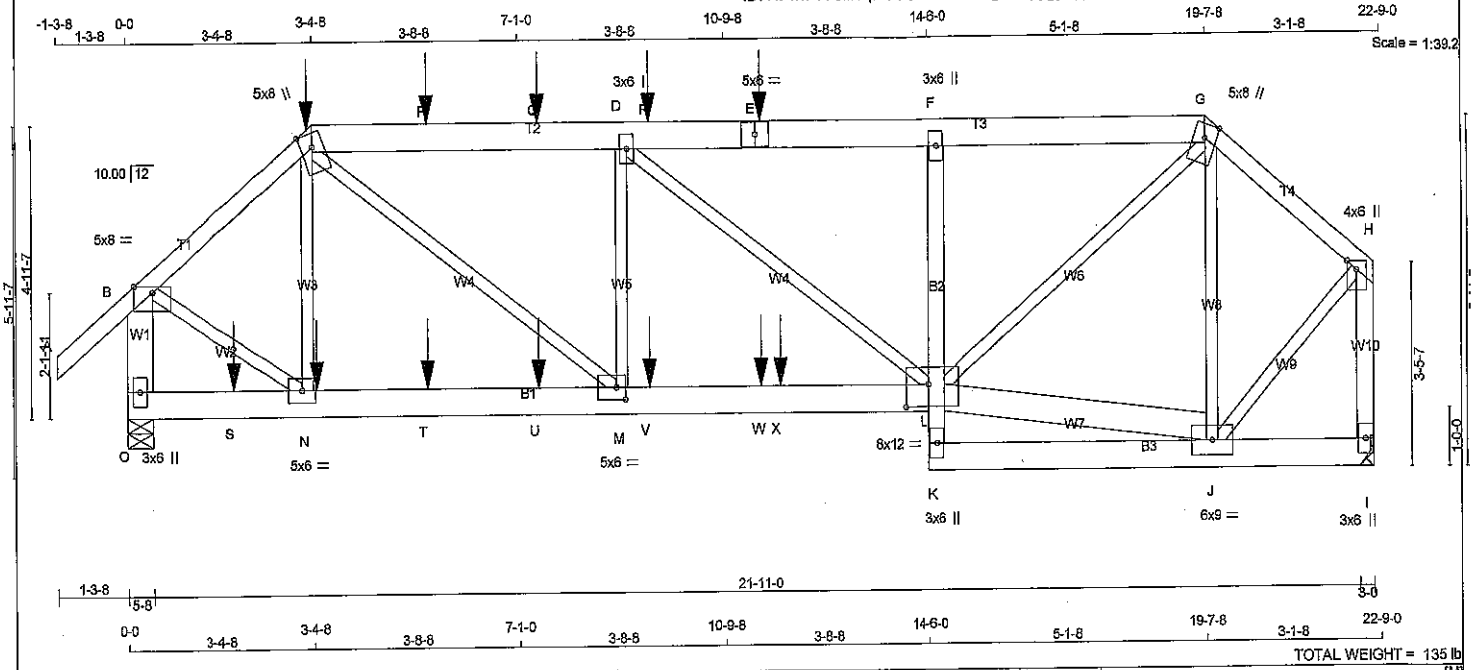

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY..	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x6	DRY	2100F 1.8E	SPF
K - F	2x4	DRY	No.2	SPF
K - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMWW+t	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	8.0	Edge	
H	TMVW+p	MT20	4.0	6.0	1.75	2.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWWW-t	MT20	6.0	9.0		
K	BMV+p	MT20	3.0	6.0		
L	BVMWWW-I	MT20	8.0	12.0	4.50	5.25
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 267.0 lbs FACTORED DOWN AT 3-4-8, 72.1 lbs FACTORED DOWN AT 5-5-4, 72.1 lbs FACTORED DOWN AT 7-5-4, AND 72.1 lbs FACTORED DOWN AT 9-5-4, AND 72.1 lbs FACTORED DOWN AT 11-5-4 ON TOP CHORD, AND 33.7 lbs FACTORED DOWN AT 1-11-4, 33.7 lbs FACTORED DOWN AT 3-5-4, 33.7 lbs FACTORED DOWN AT 5-5-4, 33.7 lbs FACTORED DOWN AT 7-5-4, 33.7 lbs FACTORED DOWN AT 9-5-4, AND 33.7 lbs FACTORED DOWN AT 11-5-4, AND 1049.4 lbs FACTORED DOWN AT 11-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2642	0	2642	0	0	5-8	5-8
I	2231	0	2231	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2058	1341 / 0	354 / 0	0 / 0	0 / 0	363 / 0	0 / 0
I	1755	1115 / 0	321 / 0	0 / 0	0 / 0	319 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	N- C	-836 / 0	0.28 (1)
B- C	-2155 / 0	-104.9	-104.9	0.31 (1)	4.30	C- M	0 / 2363	0.58 (1)
C- P	-3515 / 0	-104.9	-104.9	0.37 (1)	4.17	M- D	-535 / 86	0.18 (1)
P- Q	-3515 / 0	-104.9	-104.9	0.37 (1)	4.17	D- L	-563 / 0	0.55 (1)
Q- D	-3515 / 0	-104.9	-104.9	0.37 (1)	4.17	L- J	0 / 1089	0.14 (1)
D- R	-3072 / 0	-104.9	-104.9	0.35 (1)	4.44	L- G	0 / 2609	0.65 (1)
R- E	-3072 / 0	-104.9	-104.9	0.35 (1)	4.44	J- G	-1212 / 0	0.62 (1)
E- F	-3072 / 0	-104.9	-104.9	0.35 (1)	4.44	B- N	0 / 1876	0.46 (1)
F- G	-3059 / 0	-104.9	-104.9	0.24 (1)	4.58	J- H	0 / 1596	0.40 (1)
G- H	-1412 / 0	-104.9	-104.9	0.21 (1)	5.22			
O- B	-2570 / 0	0.0	0.0	0.21 (1)	6.43			
I- H	-2225 / 0	0.0	0.0	0.44 (1)	5.65			
O- S	0 / 0	-28.0	-28.0	0.03 (1)	10.00			
S- N	0 / 0	-28.0	-28.0	0.03 (1)	10.00			
N- T	0 / 1680	-28.0	-28.0	0.33 (1)	10.00			
T- U	0 / 1680	-28.0	-28.0	0.33 (1)	10.00			
U- M	0 / 1680	-28.0	-28.0	0.33 (1)	10.00			
M- V	0 / 3514	-28.0	-28.0	0.72 (1)	10.00			
V- W	0 / 3514	-28.0	-28.0	0.72 (1)	10.00			
W- X	0 / 3514	-28.0	-28.0	0.72 (1)	10.00			
X- L	0 / 3514	-28.0	-28.0	0.72 (1)	10.00			
K- L	0 / 102	0.0	0.0	0.09 (1)	10.00			
L- F	-633 / 0	0.0	0.0	0.17 (1)	7.81			
K- J	0 / 57	-28.0	-28.0	0.05 (2)	10.00			
J- I	0 / 0	-28.0	-28.0	0.05 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	
C	3-4-8	-267	-267	---	FRONT	VERT	TOTAL
E	11-5-4	-72	-72	---	FRONT	VERT	TOTAL
N	3-5-4	-19	-34	---	FRONT	VERT	TOTAL
P	5-5-4	-72	-72	---	FRONT	VERT	TOTAL
Q	7-5-4	-72	-72	---	FRONT	VERT	TOTAL
R	9-5-4	-72	-72	---	FRONT	VERT	TOTAL
S	1-11-4	-19	-34	---	FRONT	VERT	TOTAL
T	5-5-4	-19	-34	---	FRONT	VERT	TOTAL
U	7-5-4	-19	-34	---	FRONT	VERT	TOTAL
V	9-5-4	-19	-34	---	FRONT	VERT	TOTAL
W	11-5-4	-19	-34	---	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 936 (0.29")

CSI: TC=0.44/1.00 (H-I:1) , BC=0.72/1.00 (L-M:1) , WB=0.65/1.00 (G-L:1) , SSI=0.49/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.46 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11109-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	11-9-8	-1049	-1049	—	FRONT	VERT	TOTAL

DWG NO. TAM 11109-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:27 2018 Page 1

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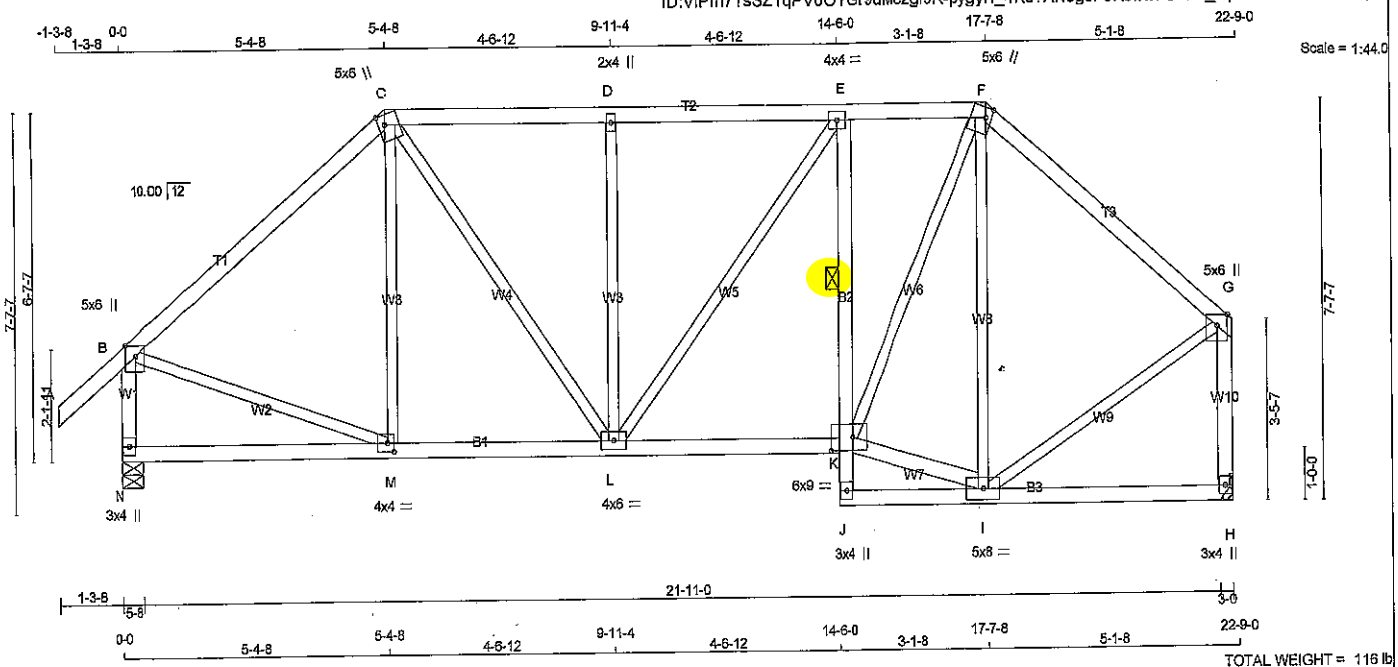

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
J - E	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMWW-t	MT20	6.0	9.0	3.00	5.50
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1649	0	1649	0	0	5-8	5-8
H	1504	0	1504	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1297	825 / 0	238 / 0	0 / 0	0 / 0	235 / 0	0 / 0
H	1199	735 / 0	238 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-1295 / 0	-104.9	-104.9	0.62 (1)		4.82
C- D	-1367 / 0	-104.9	-104.9	0.36 (1)		5.11
D- E	-1368 / 0	-104.9	-104.9	0.36 (1)		5.11
E- F	-1254 / 0	-104.9	-104.9	0.21 (1)		5.49
F- G	-1059 / 0	-104.9	-104.9	0.51 (1)		5.40
N- B	-1586 / 0	0.0	0.0	0.19 (1)		6.55
H- G	-1448 / 0	0.0	0.0	0.29 (1)		6.79
N- M	0 / 0	-28.0	-28.0	0.18 (2)		10.00
M- L	0 / 990	-28.0	-28.0	0.29 (2)		10.00
L- K	0 / 1258	-28.0	-28.0	0.28 (1)		10.00
J- K	0 / 34	0.0	0.0	0.04 (1)		10.00
K- E	-568 / 0	0.0	0.0	0.05 (1)		6.25
J- I	0 / 25	-28.0	-28.0	0.16 (2)		10.00
I- H	0 / 0	-28.0	-28.0	0.16 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-167 / 133	0.12 (1)
C- L	0 / 644	0.14 (1)
L- D	-550 / 0	0.40 (1)
L- E	0 / 183	0.04 (1)
K- I	0 / 833	0.13 (1)
K- F	0 / 1030	0.23 (1)
I- F	-832 / 0	0.67 (1)
B- M	0 / 1046	0.24 (1)
I- G	0 / 956	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.29/1.00 (L-M:2) , WB=0.67/1.00 (F-I:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.37 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11110'-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:27 2018 Page 1

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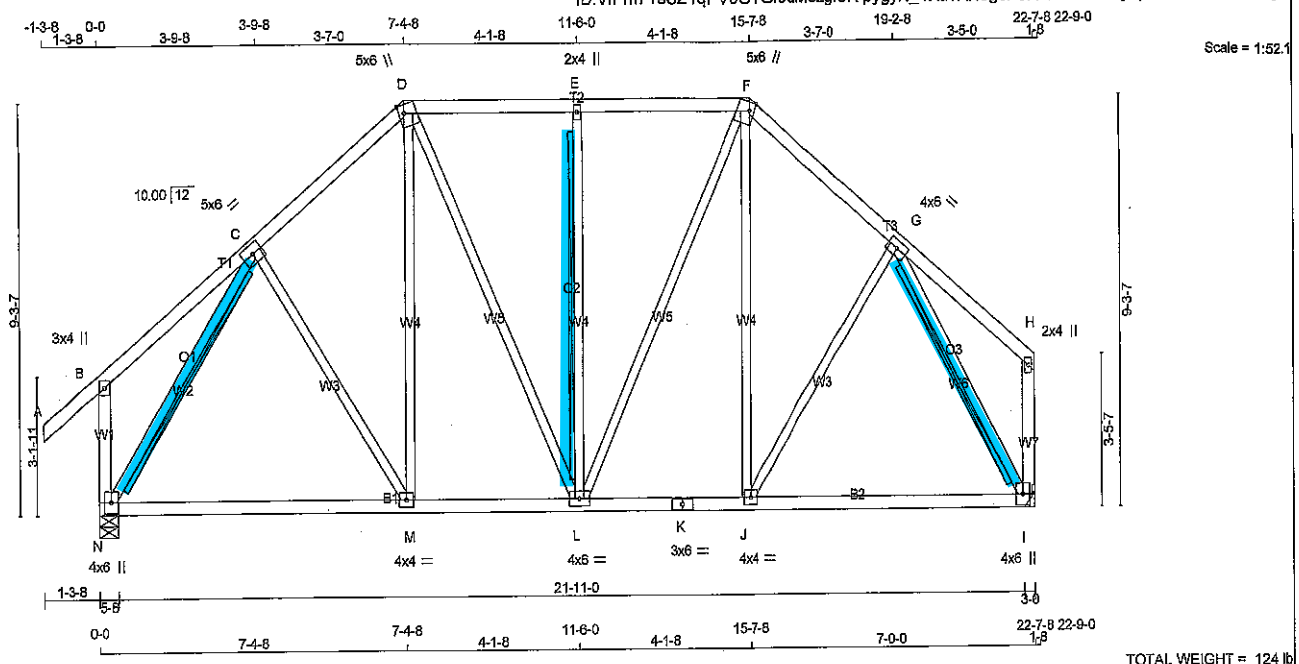

TOTAL WEIGHT = 124 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	BMWW1+t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1639	0	1639	0	0	5-8	5-8
I	1504	0	1504	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1289	820 / 0	236 / 0	0 / 0	0 / 0	233 / 0	0 / 0
I	1198	736 / 0	236 / 0	0 / 0	0 / 0	225 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-L, C-N
2x3 DRY SPF No.2 T-BRACE AT G-I

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 28	-104.9	-104.9	0.23 (1)	10.00
C- D	-1126 / 0	-104.9	-104.9	0.19 (1)	5.77
D- E	-954 / 0	-104.9	-104.9	0.23 (1)	6.09
E- F	-954 / 0	-104.9	-104.9	0.23 (1)	6.09
F- G	-1080 / 0	-104.9	-104.9	0.17 (1)	5.89
G- H	0 / 33	-104.9	-104.9	0.20 (1)	10.00
N- B	-296 / 0	0.0	0.0	0.05 (1)	7.81
N- M	0 / 792	-28.0	-28.0	0.43 (2)	10.00
M- L	0 / 842	-28.0	-28.0	0.44 (2)	10.00
L- K	0 / 808	-28.0	-28.0	0.39 (2)	10.00
K- J	0 / 808	-28.0	-28.0	0.39 (2)	10.00
J- I	0 / 700	-28.0	-28.0	0.37 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	0 / 151	0.03 (2)
M- D	0 / 259	0.06 (3)
D- L	0 / 267	0.06 (1)
L- E	-523 / 0	0.34 (1)
L- F	0 / 349	0.08 (1)
J- F	0 / 207	0.05 (3)
J- G	0 / 214	0.05 (2)
N- C	-1486 / 0	0.52 (1)
G- I	-1478 / 0	0.98 (1)
I- H	-125 / 0	0.02 (1)

LICENSED PROFESSIONAL ENGINEER
22878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.44/1.00 (L-M:2) , WB=0.98/1.00 (G-I:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.35 (C) (INPUT = 1.00)

DWG NO. TAM 11114
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T18	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:28 2018 Page 1

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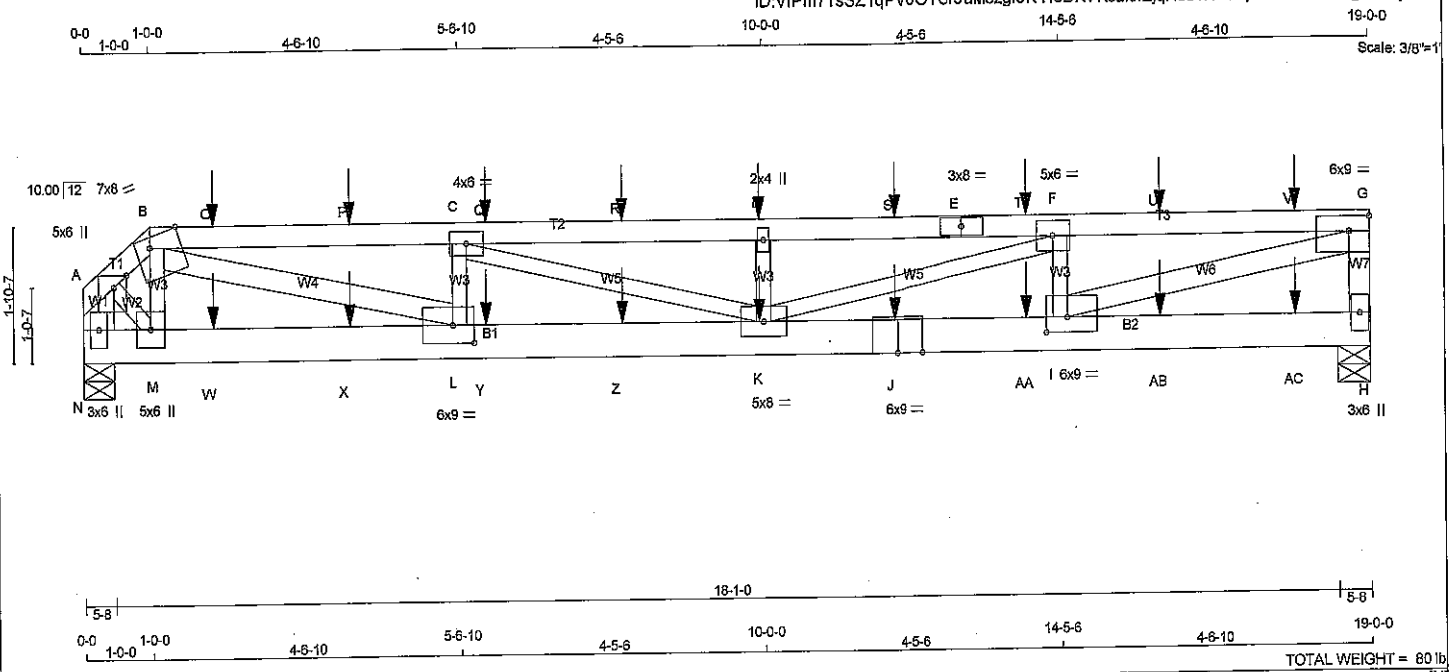

TOTAL WEIGHT = 80 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	1650F 1.5E	SPF
B - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
N - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW-m	MT20	7.0	8.0	Edge	5.50
C	TMWW-t	MT20	4.0	6.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	6.0	9.0	2.50	Edge
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	6.0	9.0	2.50	3.75
J	BS-t	MT20	6.0	9.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	6.0	9.0	3.00	3.75
M	BMWW+t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 111.2 lbs FACTORED DOWN AT 1-11-4, 107.9 lbs FACTORED DOWN AT 3-11-4, 107.9 lbs FACTORED DOWN AT 5-11-4, 107.9 lbs FACTORED DOWN AT 7-11-4, 107.9 lbs FACTORED DOWN AT 9-11-4, 107.9 lbs FACTORED DOWN AT 11-11-4, 107.9 lbs FACTORED DOWN AT 13-11-4, AND 107.9 lbs FACTORED DOWN AT 15-11-4, AND 107.9 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 66.5 lbs FACTORED DOWN AT 1-11-4, 66.5 lbs FACTORED DOWN AT 3-11-4, 66.5 lbs FACTORED DOWN AT 5-11-4, 66.5 lbs FACTORED DOWN AT 7-11-4, 66.5 lbs FACTORED DOWN AT 9-11-4, 66.5 lbs FACTORED DOWN AT 11-11-4, 66.5 lbs FACTORED DOWN AT 13-11-4, AND 66.5 lbs FACTORED DOWN AT 15-11-4, AND 66.5 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	2085	0	2085	0	0	5-8	5-8
N	2014	0	2014	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1662	1019 / 0	329 / 0	0 / 0	0 / 0	314 / 0	0 / 0
N	1605	985 / 0	318 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2082 / 0	-104.9	-104.9	0.04 (1)	5.23
B- O	-5542 / 0	-104.9	-104.9	0.64 (1)	2.92
O- P	-5542 / 0	-104.9	-104.9	0.64 (1)	2.92
P- C	-5542 / 0	-104.9	-104.9	0.64 (1)	2.92
C- Q	-6528 / 0	-104.9	-104.9	0.73 (1)	2.60
Q- R	-6528 / 0	-104.9	-104.9	0.73 (1)	2.60
R- D	-6528 / 0	-104.9	-104.9	0.73 (1)	2.60
D- S	-6528 / 0	-104.9	-104.9	0.74 (1)	2.58
S- E	-6528 / 0	-104.9	-104.9	0.74 (1)	2.58
E- T	-6528 / 0	-104.9	-104.9	0.74 (1)	2.58
T- F	-6528 / 0	-104.9	-104.9	0.74 (1)	2.58
F- U	-4976 / 0	-104.9	-104.9	0.60 (1)	3.11
U- V	-4976 / 0	-104.9	-104.9	0.60 (1)	3.11
V- G	-4976 / 0	-104.9	-104.9	0.60 (1)	3.11
H- G	-1927 / 0	0.0	0.0	0.22 (1)	6.01
N- A	-2291 / 0	0.0	0.0	0.16 (1)	6.74
N- M	0 / 0	-28.0	-28.0	0.16 (1)	10.00
M- W	0 / 1560	-28.0	-28.0	0.29 (1)	10.00
W- X	0 / 1560	-28.0	-28.0	0.29 (1)	10.00
X- L	0 / 1560	-28.0	-28.0	0.29 (1)	10.00
L- Y	0 / 5542	-28.0	-28.0	0.66 (1)	10.00
Y- Z	0 / 5542	-28.0	-28.0	0.66 (1)	10.00
Z- K	0 / 5542	-28.0	-28.0	0.66 (1)	10.00
K- J	0 / 4976	-28.0	-28.0	0.59 (1)	10.00
J-AA	0 / 4976	-28.0	-28.0	0.59 (1)	10.00
AA- I	0 / 4976	-28.0	-28.0	0.59 (1)	10.00
I-AB	0 / 0	-28.0	-28.0	0.11 (1)	10.00
AB-AC	0 / 0	-28.0	-28.0	0.11 (1)	10.00
AC- H	0 / 0	-28.0	-28.0	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-318 / 14	0.05 (1)
B- L	0 / 4186	0.74 (1)
L- C	-1121 / 0	0.15 (1)
C- K	0 / 1040	0.26 (1)
K- D	-632 / 0	0.10 (1)
K- F	0 / 1638	0.41 (1)
I- F	-1341 / 0	0.21 (1)
I- G	0 / 5237	0.93 (1)
A- M	0 / 1813	0.45 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	9-11-4	-108	-108	---	BACK	VERT	TOTAL
J	11-11-4	-67	-67	---	BACK	VERT	TOTAL
K	9-11-4	-67	-67	---	BACK	VERT	TOTAL
O	1-11-4	-111	-111	---	BACK	VERT	TOTAL
P	3-11-4	-108	-108	---	BACK	VERT	TOTAL
Q	5-11-4	-108	-108	---	BACK	VERT	TOTAL
R	7-11-4	-108	-108	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 605 (0.38")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 390 (0.58")

CSI: TC=0.74/1.00 (D-F:1) , BC=0.66/1.00 (K-L:1) , WB=0.93/1.00 (G-I:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.90 (M) (INPUT = 0.90)
MSI METAL= 0.83 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111 2-08
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T18	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	11-11-4	-108	-108	—	BACK	VERT	TOTAL
T	13-11-4	-108	-108	—	BACK	VERT	TOTAL
U	15-11-4	-108	-108	—	BACK	VERT	TOTAL
V	17-11-4	-108	-108	—	BACK	VERT	TOTAL
W	1-11-4	-67	-67	—	BACK	VERT	TOTAL
X	3-11-4	-67	-67	—	BACK	VERT	TOTAL
Y	5-11-4	-67	-67	—	BACK	VERT	TOTAL
Z	7-11-4	-67	-67	—	BACK	VERT	TOTAL
AA	13-11-4	-67	-67	—	BACK	VERT	TOTAL
AB	15-11-4	-67	-67	—	BACK	VERT	TOTAL
AC	17-11-4	-67	-67	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11112-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	T19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:28 2018 Page 1

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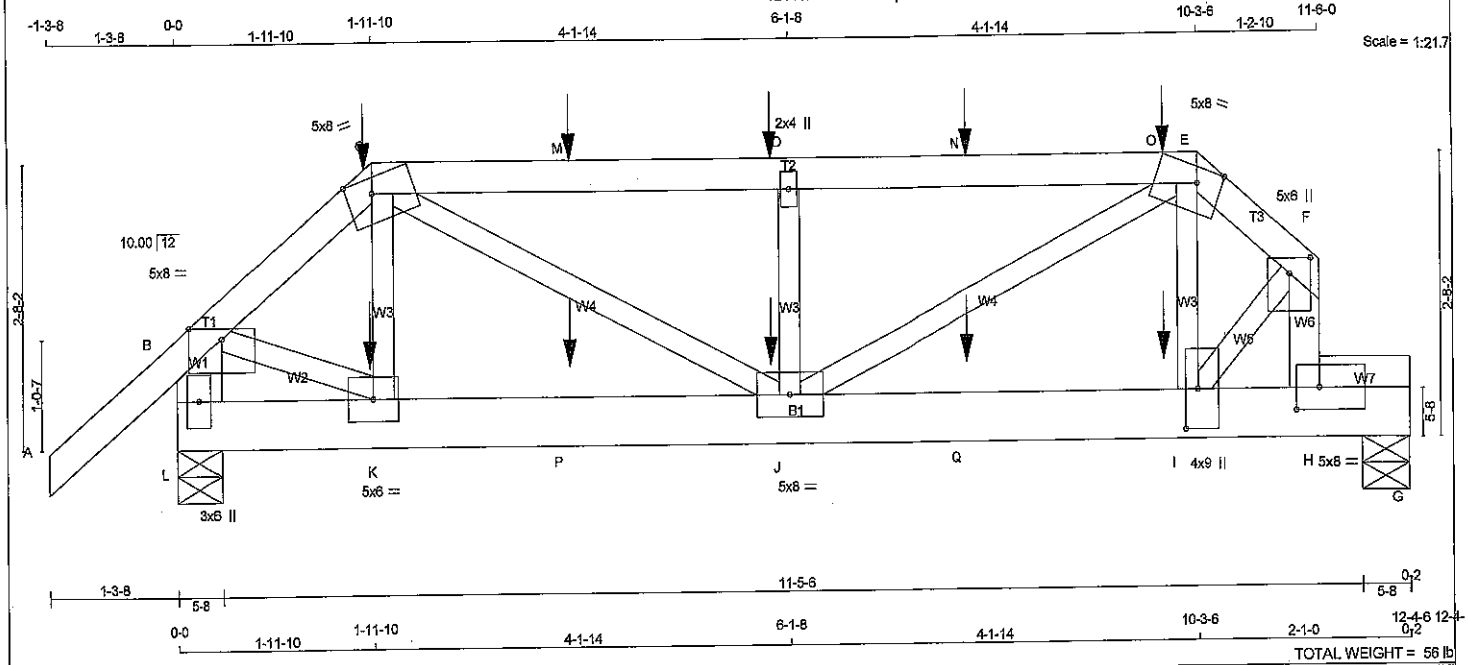

TOTAL WEIGHT = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMVW+p	MT20	5.0	6.0	1.75	2.50
H	BMVW-t	MT20	5.0	8.0	2.50	2.75
I	BMWW+t	MT20	4.0	9.0	4.50	1.50
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 25.6 lbs FACTORED DOWN AND 4.9 lbs FACTORED UP AT 1-11-10, 23.8 lbs FACTORED DOWN AND 6.8 lbs FACTORED UP AT 3-11-4, 23.8 lbs FACTORED DOWN AND 6.8 lbs FACTORED UP AT 5-11-4, AND 23.8 lbs FACTORED DOWN AND 6.8 lbs FACTORED UP AT 7-11-4, AND 56.3 lbs FACTORED DOWN AND 6.1 lbs FACTORED UP AT 9-11-4 ON TOP CHORD, AND 313.2 lbs FACTORED DOWN AT 1-11-4, 313.2 lbs FACTORED DOWN AT 3-11-4, 313.2 lbs FACTORED DOWN AT 5-11-4, AND 313.2 lbs FACTORED DOWN AT 7-11-4, AND 313.2 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1833	0	1833	0	0	5-8	5-8
G	1585	0	1585	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1443	912 / 0	265 / 0	0 / 0	0 / 0	266 / 0	0 / 0
G	1270	765 / 0	260 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-1872 / 0	-104.9	-104.9	0.12 (1)	4.77
C- M	-2581 / 0	-104.9	-104.9	0.43 (1)	3.82
M- D	-2581 / 0	-104.9	-104.9	0.43 (1)	3.82
D- N	-2582 / 0	-104.9	-104.9	0.43 (1)	3.82
N- O	-2582 / 0	-104.9	-104.9	0.43 (1)	3.82
O- E	-2582 / 0	-104.9	-104.9	0.43 (1)	3.82
E- F	-1997 / 0	-104.9	-104.9	0.08 (1)	4.70
L- B	-1883 / 0	0.0	0.0	0.13 (1)	7.27
H- F	-2788 / 0	0.0	0.0	0.32 (1)	5.10
L- K	0 / 0	-28.0	-28.0	0.12 (1)	10.00
K- P	0 / 1442	-28.0	-28.0	0.39 (1)	10.00
P- J	0 / 1442	-28.0	-28.0	0.39 (1)	10.00
J- Q	0 / 1539	-28.0	-28.0	0.41 (1)	10.00
Q- I	0 / 1539	-28.0	-28.0	0.41 (1)	10.00
I- H	0 / 0	-28.0	-28.0	1.00 (1)	10.00
H- G	0 / 0	-28.0	-28.0	0.62 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 291	0.07 (2)
C- J	0 / 1299	0.32 (1)
J- D	-560 / 0	0.10 (1)
J- E	0 / 1189	0.29 (1)
I- E	0 / 415	0.10 (1)
B- K	0 / 1493	0.37 (1)
I- F	0 / 2087	0.52 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-11-10	-26	-26	5	FRONT	VERT	TOTAL
D	5-11-4	-24	-24	7	FRONT	VERT	TOTAL
I	9-11-4	-313	-313	—	FRONT	VERT	TOTAL
J	5-11-4	-313	-313	—	FRONT	VERT	TOTAL
K	1-11-4	-313	-313	—	FRONT	VERT	TOTAL
M	3-11-4	-24	-24	7	FRONT	VERT	TOTAL
N	7-11-4	-24	-24	7	FRONT	VERT	TOTAL
O	9-11-4	-56	-56	6	FRONT	VERT	TOTAL
P	3-11-4	-313	-313	—	FRONT	VERT	TOTAL
Q	7-11-4	-313	-313	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.43/1.00 (D-E:1) , BC=1.00/1.00 (H-I:1) , WB=0.52/1.00 (F-I:1) , SSI=0.84/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.81 (H) (INPUT = 1.00)

DWG NO. TAM 11113 -S8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T21S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:39 2018 Page 1
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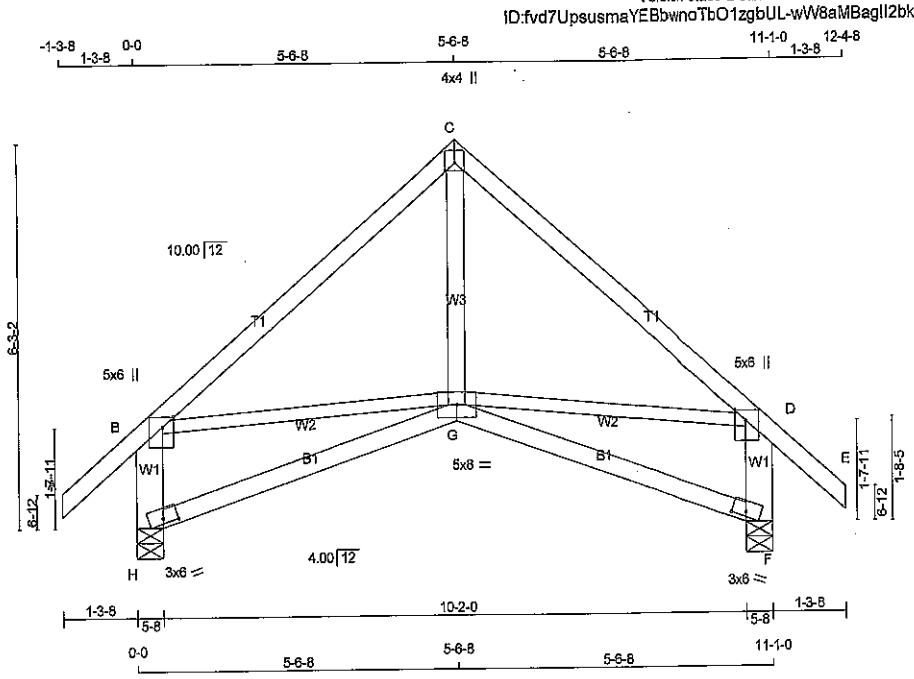

TOTAL WEIGHT = 3 X 52 = 155 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BVM1-I	MT20	3.0	6.0	1.00	3.00
G	BBWWW-p	MT20	5.0	8.0		
H	BVM1-I	MT20	3.0	6.0	1.00	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	882	0	882	0	0	5-8	5-8
F	882	0	882	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	685	450 / 0	116 / 0	0 / 0	0 / 0	119 / 0	0 / 0
F	685	450 / 0	116 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	G- C	0 / 318	0.05 (2)
B- C	-630 / 0	-104.9	-104.9	0.42 (1)	6.25	B- G	0 / 465	0.11 (1)
C- D	-630 / 0	-104.9	-104.9	0.42 (1)	6.25	G- D	0 / 465	0.11 (1)
D- E	0 / 47	-104.9	-104.9	0.14 (1)	10.00			
H- B	-804 / 0	0.0	0.0	0.06 (1)	7.81			
F- D	-804 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.26 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.26 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.26/1.00 (F-G:3) , WB=0.11/1.00 (B-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11123 -R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T22	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:40 2018 Page 1
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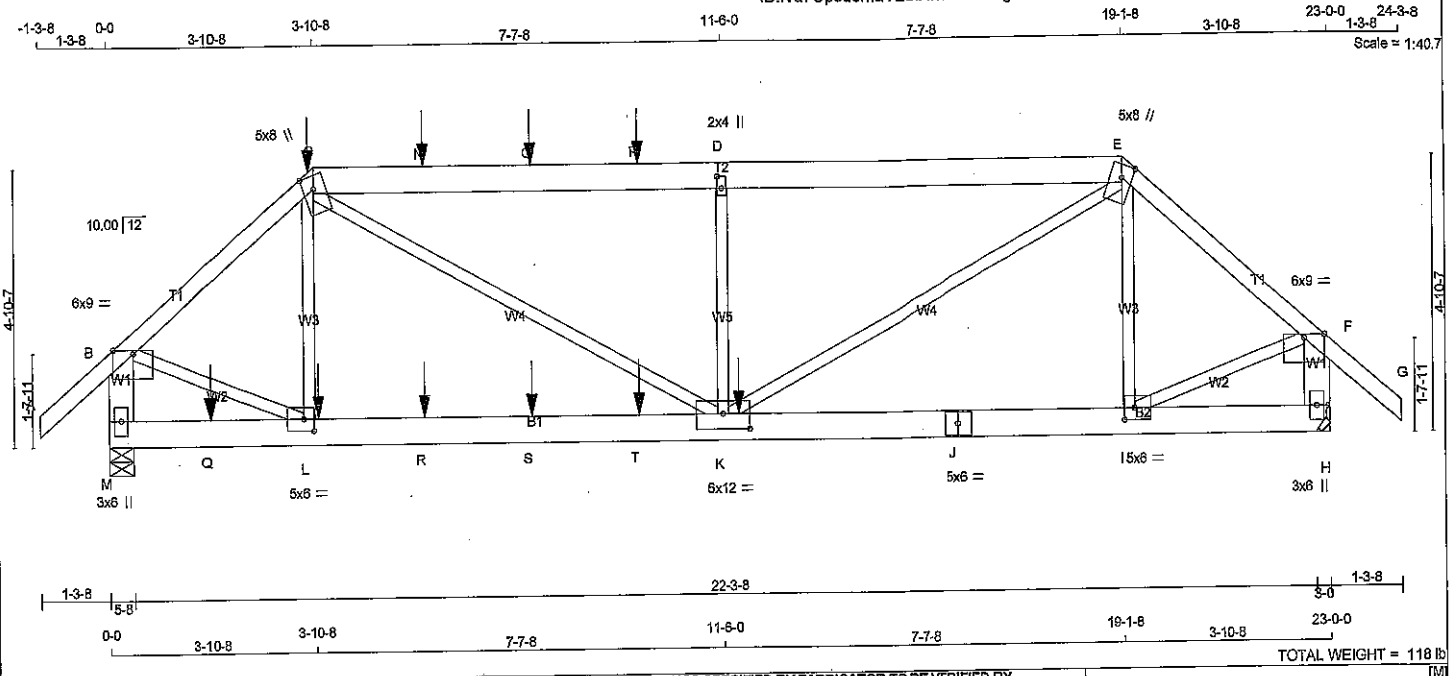

TOTAL WEIGHT = 118 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.75	2.25
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TTWW+m	MT20	5.0	8.0	2.75	2.25
F	TMVW-p	MT20	6.0	9.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.25
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	6.0	12.0	3.25	6.00
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 330.7 lbs FACTORED DOWN AT 3-10-8, 98.4 lbs FACTORED DOWN AT 5-11-4, AND 98.4 lbs FACTORED DOWN AT 7-11-4, AND 98.4 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 45.9 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 45.9 lbs FACTORED DOWN AT 5-11-4, 45.9 lbs FACTORED DOWN AT 7-11-4, AND 45.9 lbs FACTORED DOWN AT 9-11-4, AND 1103.1 lbs FACTORED DOWN AT 11-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2787	0	2787	0	0	5-8	5-8
H	2416	0	2416	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2172	1415 / 0	374 / 0	0 / 0	0 / 0	382 / 0	0 / 0
H	1883	1227 / 0	326 / 0	0 / 0	0 / 0	331 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00
B- C	-2736 / 0	-104.9	-104.9	0.46 (1)	3.72
C- N	-4114 / 0	-104.9	-104.9	0.78 (1)	3.31
N- O	-4114 / 0	-104.9	-104.9	0.78 (1)	3.31
O- P	-4114 / 0	-104.9	-104.9	0.78 (1)	3.31
P- D	-4114 / 0	-104.9	-104.9	0.78 (1)	3.31
D- E	-4114 / 0	-104.9	-104.9	0.78 (1)	3.31
E- F	-2316 / 0	-104.9	-104.9	0.42 (1)	4.05
F- G	0 / 47	-104.9	-104.9	0.16 (1)	10.00
M- B	-2777 / 0	0.0	0.0	0.20 (1)	6.23
H- F	-2404 / 0	0.0	0.0	0.16 (1)	6.81
M- Q	0 / 0	-28.0	-28.0	0.22 (2)	10.00
Q- L	0 / 0	-28.0	-28.0	0.22 (2)	10.00
L- R	0 / 2116	-28.0	-28.0	0.42 (1)	10.00
R- S	0 / 2116	-28.0	-28.0	0.42 (1)	10.00
S- T	0 / 2116	-28.0	-28.0	0.42 (1)	10.00
T- K	0 / 2116	-28.0	-28.0	0.42 (1)	10.00
K- J	0 / 1792	-28.0	-28.0	0.34 (1)	10.00
J- I	0 / 1792	-28.0	-28.0	0.34 (1)	10.00
I- H	0 / 0	-28.0	-28.0	0.14 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-373 / 216	0.12 (1)
C- K	0 / 2311	0.57 (1)
K- D	-1185 / 0	0.38 (1)
K- E	0 / 2685	0.66 (1)
I- E	-361 / 80	0.12 (1)
B- L	0 / 2215	0.55 (1)
I- F	0 / 1875	0.46 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-331	-331	---	FRONT	VERT	TOTAL
K	11-9-8	-1103	-1103	---	FRONT	VERT	TOTAL
L	3-11-4	-26	-46	---	FRONT	VERT	TOTAL
N	5-11-4	-98	-98	---	FRONT	VERT	TOTAL
O	7-11-4	-98	-98	---	FRONT	VERT	TOTAL
P	9-11-4	-98	-98	---	FRONT	VERT	TOTAL
Q	1-11-4	-22	-46	---	FRONT	VERT	TOTAL
R	5-11-4	-26	-46	---	FRONT	VERT	TOTAL
S	7-11-4	-26	-46	---	FRONT	VERT	TOTAL
T	9-11-4	-26	-46	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.78/1.00 (C-D:1) , BC=0.42/1.00 (K-L:1) , WB=0.66/1.00 (E-K:1) , SSI=0.44/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.53 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11124-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:40 2018 Page 1

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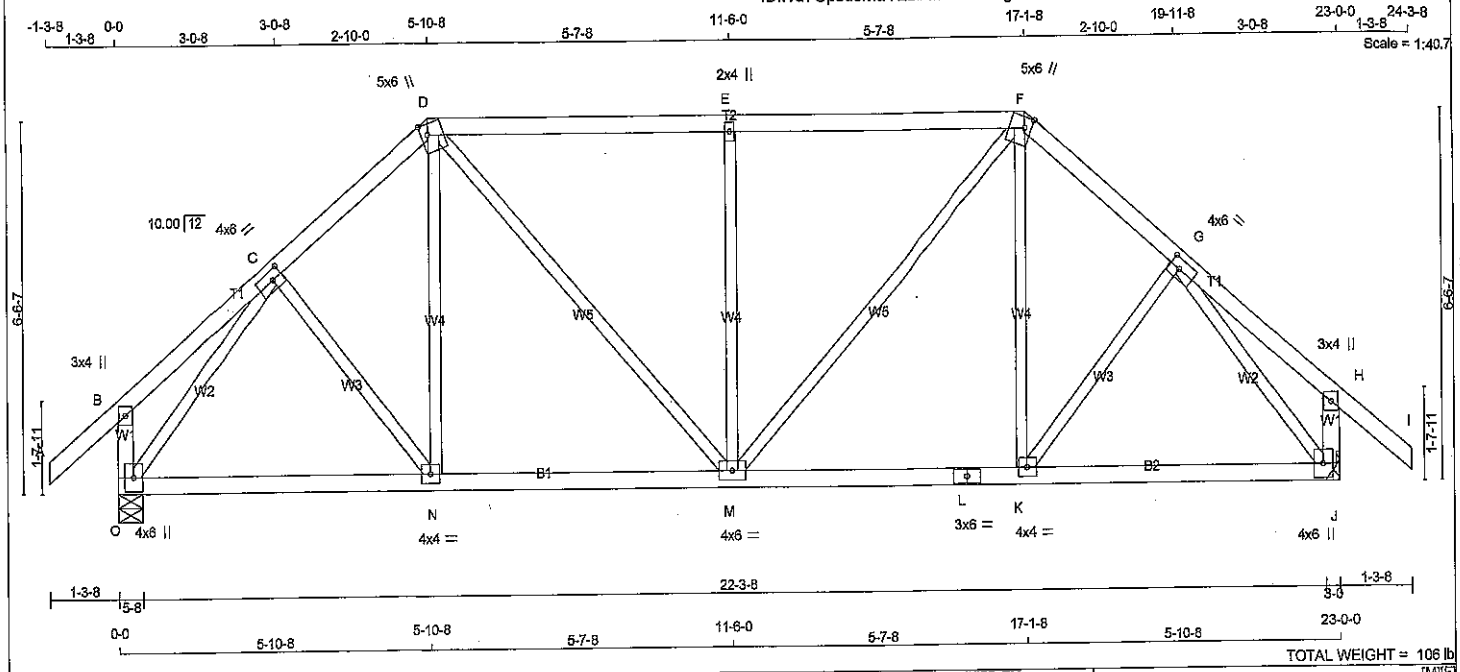

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	2.25
H	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1674	0	1674	0	0	IN-SX	IN-SX
J	1674	0	1674	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1317	837 / 0	242 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1317	837 / 0	242 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 20	-104.9	-104.9	0.13 (1)	10.00
C- D	-1443 / 0	-104.9	-104.9	0.12 (1)	5.33
D- E	-1479 / 0	-104.9	-104.9	0.45 (1)	4.84
E- F	-1479 / 0	-104.9	-104.9	0.45 (1)	4.84
F- G	-1443 / 0	-104.9	-104.9	0.12 (1)	5.33
G- H	0 / 20	-104.9	-104.9	0.13 (1)	10.00
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
O- B	-269 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-269 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1048	-28.0	-28.0	0.35 (2)	10.00
N- M	0 / 1092	-28.0	-28.0	0.36 (2)	10.00
M- L	0 / 1092	-28.0	-28.0	0.36 (2)	10.00
L- K	0 / 1092	-28.0	-28.0	0.36 (2)	10.00
K- J	0 / 1048	-28.0	-28.0	0.35 (2)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
C- N	0 / 120	0.03 (3)
N- D	0 / 233	0.05 (2)
D- M	0 / 578	0.13 (1)
M- E	-721 / 0	0.50 (1)
M- F	0 / 578	0.13 (1)
K- F	0 / 233	0.05 (2)
K- G	0 / 120	0.03 (3)
O- C	-1699 / 0	0.61 (1)
G- J	-1699 / 0	0.61 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.36/1.00 (M-N:2) , WB=0.61/1.00 (C-O:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11125

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:40 2018 Page 1

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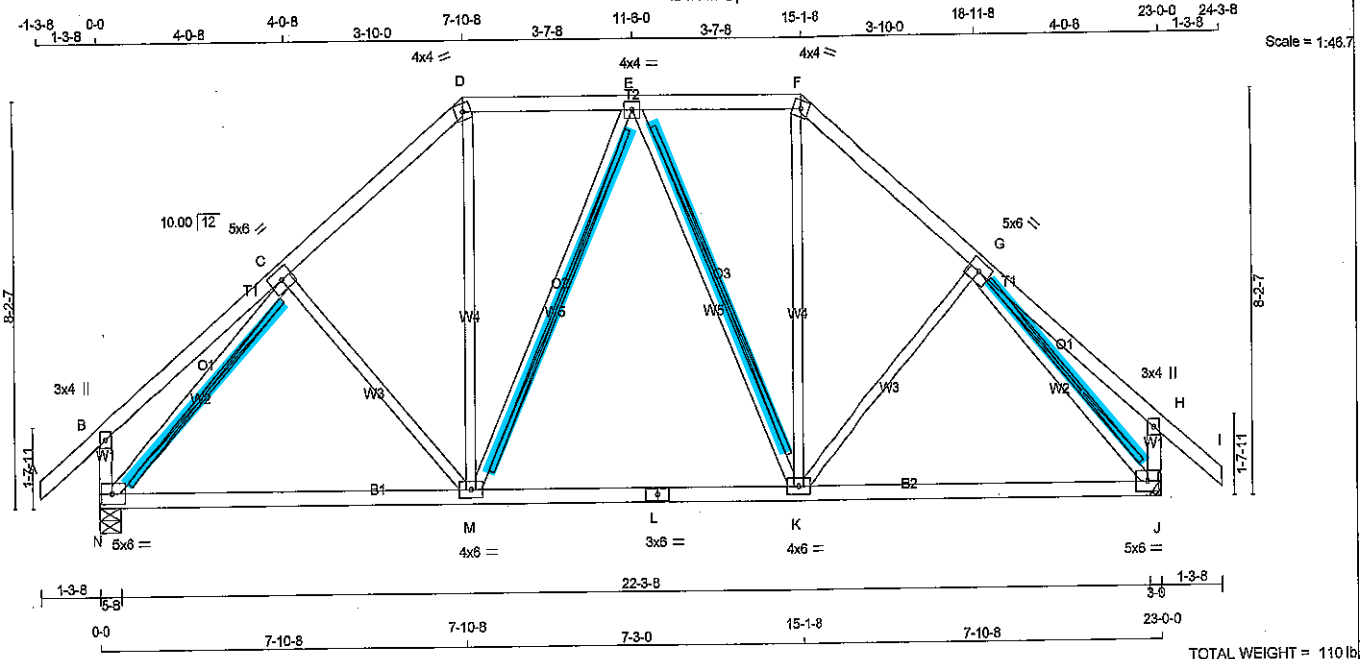

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1674	0	1674	0	0	IN-SX	IN-SX
J	1674	0	1674	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1317	837 / 0	242 / 0	0 / 0	0 / 0	238 / 0	0 / 0
J	1317	837 / 0	242 / 0	0 / 0	0 / 0	238 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-M, E-K, C-N, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B-C	0 / 30	-104.9	-104.9	0.25 (1)	10.00
C-D	-1361 / 0	-104.9	-104.9	0.22 (1)	5.34
D-E	-1027 / 0	-104.9	-104.9	0.17 (1)	5.99
E-F	-1027 / 0	-104.9	-104.9	0.17 (1)	5.99
F-G	-1361 / 0	-104.9	-104.9	0.22 (1)	5.34
G-H	0 / 30	-104.9	-104.9	0.25 (1)	10.00
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
N-B	-303 / 0	0.0	0.0	0.03 (1)	7.81
J-H	-303 / 0	0.0	0.0	0.03 (1)	7.81
N-M	0 / 1095	-28.0	-28.0	0.49 (2)	10.00
M-L	0 / 1128	-28.0	-28.0	0.50 (2)	10.00
L-K	0 / 1128	-28.0	-28.0	0.50 (2)	10.00
K-J	0 / 1095	-28.0	-28.0	0.49 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-M	-113 / 77	0.07 (1)
M-D	0 / 540	0.12 (1)
M-E	-248 / 0	0.32 (1)
E-K	-248 / 0	0.32 (1)
K-F	0 / 540	0.12 (1)
K-G	-113 / 77	0.07 (1)
N-C	-1683 / 0	0.83 (1)
G-J	-1683 / 0	0.83 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.50/1.00 (K-M:2) , WB=0.83/1.00 (C-N:1) , SSI=0.16/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111126-19
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:41 2018 Page 1

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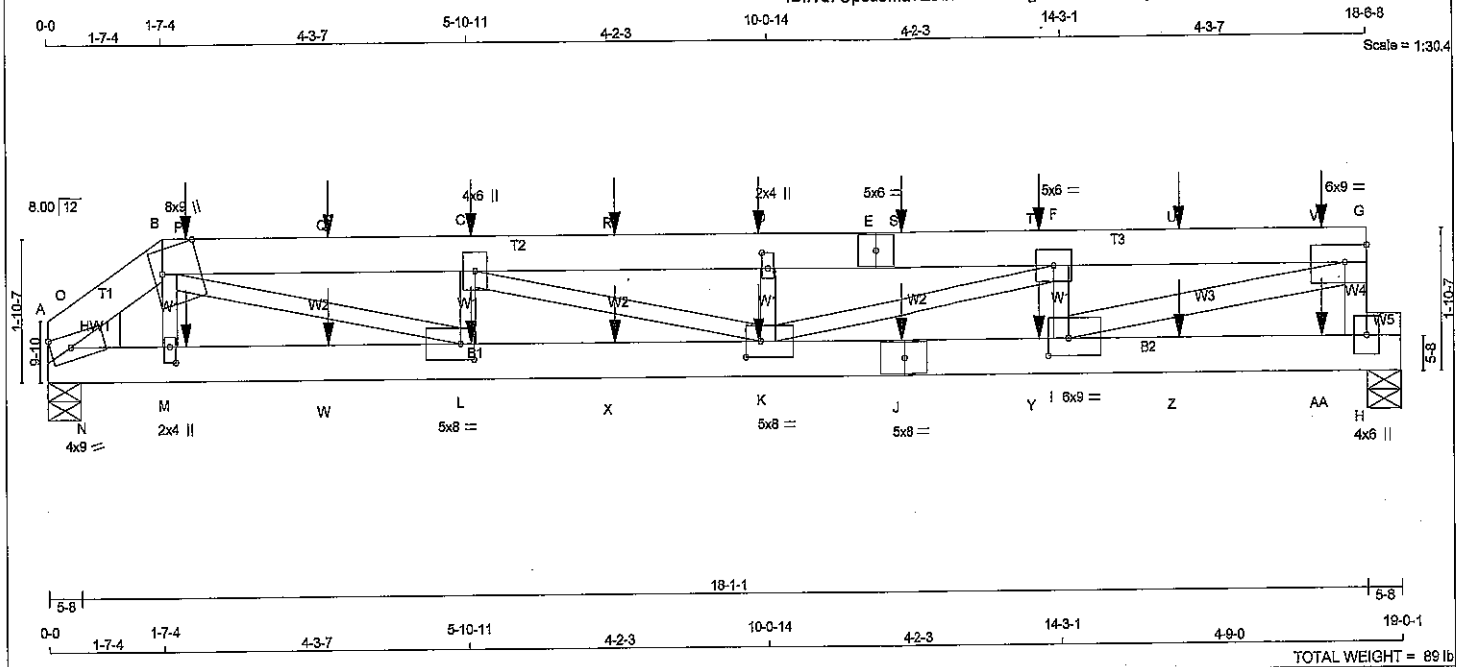

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
A - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	9.0		Edge
B	TTWW+m	MT20	8.0	9.0	Edge	6.50
C	TMWW+t	MT20	4.0	6.0		
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW-t	MT20	6.0	9.0	2.75	Edge
H	BMVW1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	6.0	9.0	2.75	3.50
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWW-t	MT20	5.0	8.0	2.50	2.25
M	BMW+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 142.9 lbs FACTORED DOWN AT 1-11-4, 107.9 lbs FACTORED DOWN AT 3-11-4, 107.9 lbs FACTORED DOWN AT 5-11-4, 107.9 lbs FACTORED DOWN AT 7-11-4, 107.9 lbs FACTORED DOWN AT 9-11-4, 107.9 lbs FACTORED DOWN AT 11-11-4, 107.9 lbs FACTORED DOWN AT 13-11-4, AND 107.9 lbs FACTORED DOWN AT 15-11-4, AND 128.7 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 66.5 lbs FACTORED DOWN AT 1-11-4, 66.5 lbs FACTORED DOWN AT 3-11-4, 66.5 lbs FACTORED DOWN AT 5-11-4, 66.5 lbs FACTORED DOWN AT 7-11-4, 66.5 lbs FACTORED DOWN AT 9-11-4, 66.5 lbs FACTORED DOWN AT 11-11-4, 66.5 lbs FACTORED DOWN AT 13-11-4, AND 66.5 lbs FACTORED DOWN AT 15-11-4, AND 72.1 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
H	2103	0	2103	0	0	5-8 (5-7)	5-8	
A	1993	0	1993	0	0	5-8	5-8	2x6 L

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1676	1028 / 0	332 / 0	0 / 0	0 / 0	316 / 0	0 / 0
A	1585	978 / 0	310 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A- O	-3267 / 0	-104.9	-104.9	0.28 (1)		4.41
O- B	-3004 / 0	-104.9	-104.9	0.25 (1)		4.58
B- P	-5688 / 0	-104.9	-104.9	0.48 (1)		3.24
P- Q	-5688 / 0	-104.9	-104.9	0.48 (1)		3.24
Q- C	-5688 / 0	-104.9	-104.9	0.48 (1)		3.24
C- R	-6491 / 0	-104.9	-104.9	0.54 (1)		2.97
R- D	-6491 / 0	-104.9	-104.9	0.54 (1)		2.97
D- E	-6491 / 0	-104.9	-104.9	0.53 (1)		2.98
E- S	-6491 / 0	-104.9	-104.9	0.53 (1)		2.98
S- T	-6491 / 0	-104.9	-104.9	0.53 (1)		2.98
T- F	-6491 / 0	-104.9	-104.9	0.53 (1)		2.98
F- U	-4803 / 0	-104.9	-104.9	0.38 (1)		3.63
U- V	-4803 / 0	-104.9	-104.9	0.38 (1)		3.63
V- G	-4803 / 0	-104.9	-104.9	0.38 (1)		3.63
H- G	-1933 / 0	0.0	0.0	0.22 (1)		6.00
A- N	0 / 2347	-28.0	-28.0	0.29 (1)		10.00
N- M	0 / 2347	-28.0	-28.0	0.31 (1)		10.00
M- W	0 / 2372	-28.0	-28.0	0.32 (1)		10.00
W- L	0 / 2372	-28.0	-28.0	0.32 (1)		10.00
L- X	0 / 5687	-28.0	-28.0	0.64 (1)		10.00
X- K	0 / 5687	-28.0	-28.0	0.64 (1)		10.00
K- J	0 / 4803	-28.0	-28.0	0.54 (1)		10.00
J- Y	0 / 4803	-28.0	-28.0	0.54 (1)		10.00
Y- I	0 / 4803	-28.0	-28.0	0.54 (1)		10.00
I- Z	0 / 0	-28.0	-28.0	0.10 (1)		10.00
Z-AA	0 / 0	-28.0	-28.0	0.10 (1)		10.00
AA- H	0 / 0	-28.0	-28.0	0.10 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	0 / 338	0.08 (2)
B- L	0 / 3485	0.86 (1)
L- C	-950 / 0	0.15 (1)
C- K	0 / 848	0.21 (1)
K- D	-596 / 0	0.09 (1)
K- F	0 / 1782	0.44 (1)
I- F	-1318 / 0	0.21 (1)
I- G	0 / 5056	0.89 (1)
N- O	0 / 323	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	5-11-4	-108	-108	---	BACK	VERT	TOTAL
D	9-11-4	-108	-108	---	BACK	VERT	TOTAL
J	11-11-4	-67	-67	---	BACK	VERT	TOTAL
K	9-11-4	-67	-67	---	BACK	VERT	TOTAL
L	5-11-4	-67	-67	---	BACK	VERT	TOTAL
M	1-11-4	-67	-67	---	BACK	VERT	TOTAL
P	1-11-4	-143	-143	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 733 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 472 (0.47")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.64/1.00 (K-L:1) , WB=0.89/1.00 (G-I:1) , SSI=0.27/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.95 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111275-S16
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	3-11-4	-108	-108	---	BACK	VERT	TOTAL
R	7-11-4	-108	-108	---	BACK	VERT	TOTAL
S	11-11-4	-108	-108	---	BACK	VERT	TOTAL
T	13-11-4	-108	-108	---	BACK	VERT	TOTAL
U	15-11-4	-108	-108	---	BACK	VERT	TOTAL
V	17-11-4	-129	-129	---	BACK	VERT	TOTAL
W	3-11-4	-67	-67	---	BACK	VERT	TOTAL
X	7-11-4	-67	-67	---	BACK	VERT	TOTAL
Y	13-11-4	-67	-67	---	BACK	VERT	TOTAL
Z	15-11-4	-67	-67	---	BACK	VERT	TOTAL
AA	17-11-4	-72	-72	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
2078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1/127

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287533	T31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:15:31 2018 Page 1

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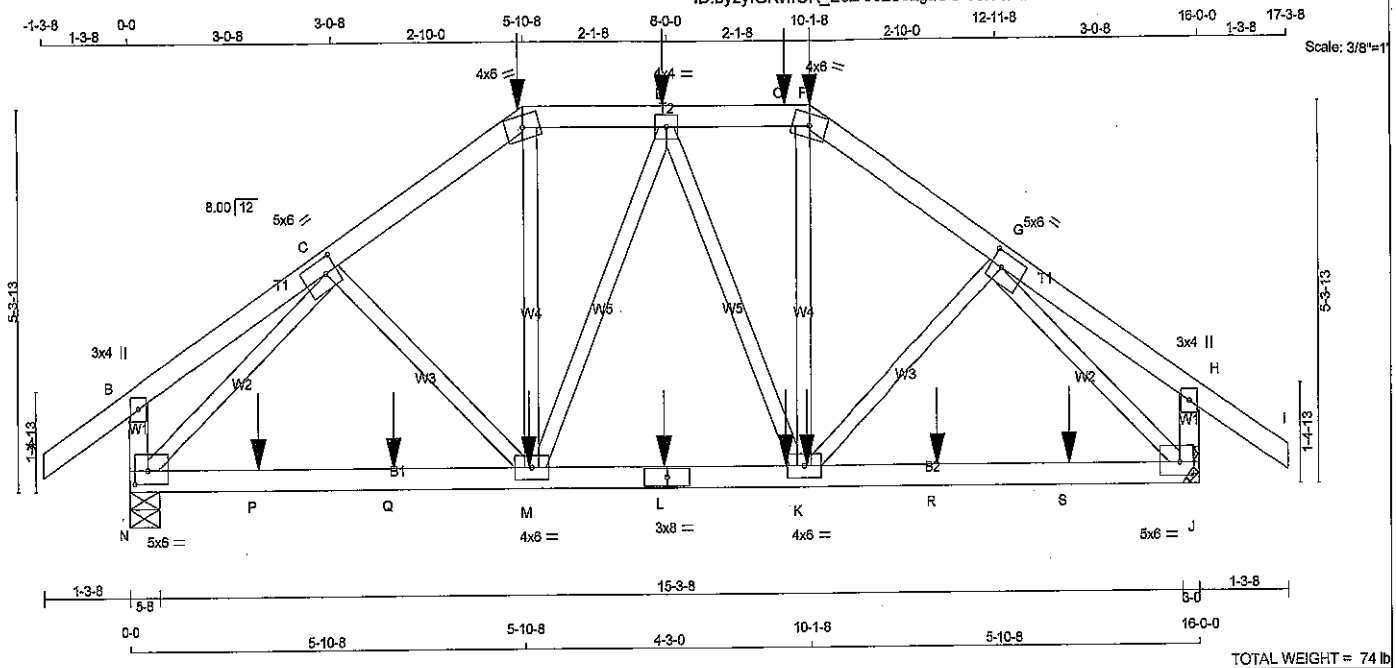

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTW-m	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.25	2.50
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0	2.25	2.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 491.5 lbs FACTORED DOWN AT 5-10-8, 28.1 lbs FACTORED DOWN AT 7-11-4, AND 60.9 lbs FACTORED DOWN AT 9-9-0, AND 491.5 lbs FACTORED DOWN AT 10-1-8 ON TOP CHORD, AND 94.9 lbs FACTORED DOWN AT 1-11-4, 94.9 lbs FACTORED DOWN AT 3-11-4, 229.4 lbs FACTORED DOWN AT 5-11-4, 229.4 lbs FACTORED DOWN AT 7-11-4, 229.4 lbs FACTORED DOWN AT 9-9-0, 229.4 lbs FACTORED DOWN AT 10-0-12, AND 94.9 lbs FACTORED DOWN AT 12-0-12, AND 94.9 lbs FACTORED DOWN AT 14-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	2273	0	2273	0	0	IN-SX	IN-SX
J	2343	0	2343	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1787	1136 / 0	326 / 0	0 / 0	0 / 0	325 / 0	0 / 0
J	1842	1171 / 0	336 / 0	0 / 0	0 / 0	335 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.15 (1)	10.00	
B- C	-1 / 14	-104.9	-104.9	0.12 (1)	10.00	
C- D	-2438 / 0	-104.9	-104.9	0.21 (1)	4.19	
D- E	-2025 / 0	-104.9	-104.9	0.12 (1)	4.63	
E- O	-2107 / 0	-104.9	-104.9	0.14 (1)	4.52	
O- F	-2107 / 0	-104.9	-104.9	0.14 (1)	4.52	
F- G	-2537 / 0	-104.9	-104.9	0.22 (1)	4.11	
G- H	-1 / 14	-104.9	-104.9	0.12 (1)	10.00	
H- I	0 / 40	-104.9	-104.9	0.15 (1)	10.00	
N- B	-272 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-273 / 0	0.0	0.0	0.03 (1)	7.81	
N- P	0 / 1828	-28.0	-28.0	0.64 (2)	10.00	
P- Q	0 / 1828	-28.0	-28.0	0.64 (2)	10.00	
Q- M	0 / 1828	-28.0	-28.0	0.64 (2)	10.00	
M- L	0 / 2119	-28.0	-28.0	0.64 (1)	10.00	
L- K	0 / 2119	-28.0	-28.0	0.64 (1)	10.00	
K- R	0 / 1895	-28.0	-28.0	0.64 (2)	10.00	
R- S	0 / 1895	-28.0	-28.0	0.64 (2)	10.00	
S- J	0 / 1895	-28.0	-28.0	0.64 (2)	10.00	

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- M	0 / 260	0.06 (1)
M- D	0 / 619	0.15 (1)
K- F	0 / 623	0.15 (1)
K- G	0 / 281	0.07 (1)
N- C	-2631 / 0	0.79 (1)
G- J	-2728 / 0	0.82 (1)
M- E	-251 / 0	0.13 (1)
E- K	-30 / 40	0.02 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-492	-492	—	BACK	VERT	TOTAL
E	7-11-4	-28	-28	—	BACK	VERT	TOTAL
F	10-1-8	-492	-492	—	BACK	VERT	TOTAL
K	9-9-0	-229	-229	—	BACK	VERT	TOTAL
K	10-0-12	-229	-229	—	BACK	VERT	TOTAL
L	7-11-4	-229	-229	—	BACK	VERT	TOTAL
M	5-11-4	-229	-229	—	BACK	VERT	TOTAL
O	9-9-0	-61	-61	—	BACK	VERT	TOTAL
P	1-11-4	-51	-95	—	BACK	VERT	TOTAL
Q	3-11-4	-54	-95	—	BACK	VERT	TOTAL
R	12-0-12	-54	-95	—	BACK	VERT	TOTAL
S	14-0-12	-51	-95	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.22/1.00 (F-G:1) , BC=0.64/1.00 (J-K:2) , WB=0.62/1.00 (G-J:1) , SSI=0.24/1.00 (M-N:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.79 (L) (INPUT = 1.00)

[M]

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 11147 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T31S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:24 2018 Page 1

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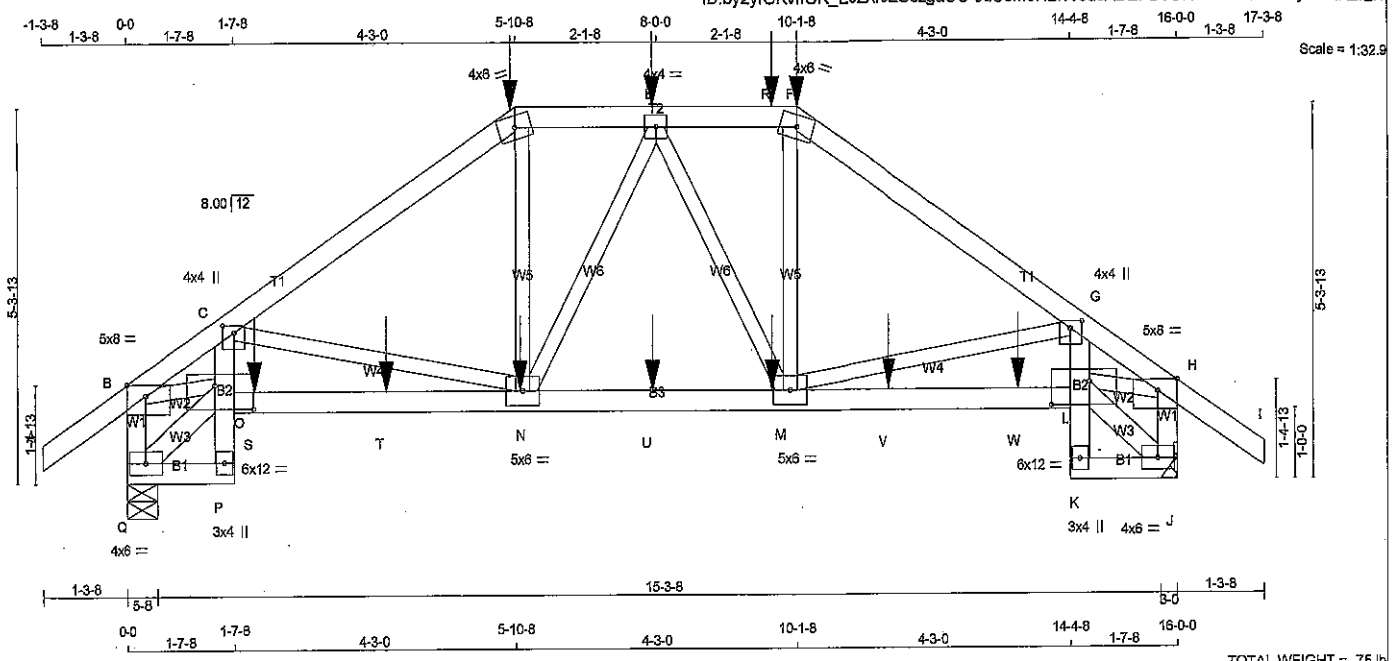

TOTAL WEIGHT = 75 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY.	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTW-m	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0		
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMVW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	12.0	4.00	7.00
M	BMWWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BVMWW-l	MT20	6.0	12.0	4.00	7.00
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 452.7 lbs FACTORED DOWN AT 5-10-8, 123.8 lbs FACTORED DOWN AT 7-11-4, AND 156.6 lbs FACTORED DOWN AT 9-9-0, AND 276.0 lbs FACTORED DOWN AT 10-1-8 ON TOP CHORD, AND 124.5 lbs FACTORED DOWN AT 1-11-4, 116.9 lbs FACTORED DOWN AT 3-11-4, 116.9 lbs FACTORED DOWN AT 5-11-4, 116.9 lbs FACTORED DOWN AT 7-11-4, 116.9 lbs FACTORED DOWN AT 9-9-0, AND 116.9 lbs FACTORED DOWN AT 11-6-12, AND 117.5 lbs FACTORED DOWN AT 13-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2142	0	2142	0	0	5-8	5-8
J	2109	0	2109	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1691	1064 / 0	316 / 0	0 / 0	0 / 0	311 / 0	0 / 0
J	1670	1041 / 0	319 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.15 (1)
B- C	-3525 / 0	-104.9	-104.9	0.30 (1)
C- D	-2623 / 0	-104.9	-104.9	0.50 (1)
D- E	-2200 / 0	-104.9	-104.9	0.12 (1)
E- R	-2192 / 0	-104.9	-104.9	0.18 (1)
R- F	-2192 / 0	-104.9	-104.9	0.18 (1)
F- G	-2612 / 0	-104.9	-104.9	0.50 (1)
G- H	-3465 / 0	-104.9	-104.9	0.30 (1)
H- I	0 / 40	-104.9	-104.9	0.15 (1)
Q- B	-2007 / 0	0.0	0.0	0.23 (1)
J- H	-1976 / 0	0.0	0.0	0.22 (1)
Q- P	0 / 144	-28.0	-28.0	0.04 (2)
P- O	0 / 36	0.0	0.0	0.26 (1)
O- C	0 / 410	0.0	0.0	0.33 (1)
O- S	0 / 3116	-28.0	-28.0	0.79 (1)
S- T	0 / 3116	-28.0	-28.0	0.79 (1)
T- N	0 / 3116	-28.0	-28.0	0.79 (1)
N- U	0 / 2288	-28.0	-28.0	0.62 (1)
U- M	0 / 2288	-28.0	-28.0	0.62 (1)
M- V	0 / 3062	-28.0	-28.0	0.77 (1)
V- W	0 / 3062	-28.0	-28.0	0.77 (1)
W- L	0 / 3062	-28.0	-28.0	0.77 (1)
K- L	0 / 36	0.0	0.0	0.25 (1)
L- G	0 / 372	0.0	0.0	0.31 (1)
K- J	0 / 142	-28.0	-28.0	0.04 (2)

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-986 / 0	0.38 (1)
N- D	0 / 678	0.17 (1)
M- F	0 / 720	0.18 (1)
M- G	-922 / 0	0.34 (1)
Q- O	-184 / 0	0.02 (1)
B- O	0 / 2929	0.73 (1)
L- J	-181 / 0	0.02 (1)
L- H	0 / 2880	0.71 (1)
N- E	-201 / 0	0.07 (1)
E- M	-220 / 26	0.08 (1)

MAX. UNBRAC LENGTH

	10.00
	3.44
	3.75
	4.47
	4.41
	4.41
	3.76
	3.47
	10.00
	5.91
	5.95
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00
	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	5-10-8	-453	-453	---	BACK	VERT	TOTAL
E	7-11-4	-124	-124	---	BACK	VERT	TOTAL
F	10-1-8	-276	-276	---	FRONT	VERT	TOTAL
M	9-9-0	-117	-117	---	BACK	VERT	TOTAL
N	5-11-4	-117	-117	---	BACK	VERT	TOTAL
R	9-9-0	-157	-157	---	BACK	VERT	TOTAL
S	1-11-4	-125	-125	---	BACK	VERT	TOTAL
T	3-11-4	-117	-117	---	BACK	VERT	TOTAL
U	7-11-4	-117	-117	---	BACK	VERT	TOTAL
V	11-6-12	-117	-117	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.50/1.00 (C-D:1) , BC=0.79/1.00 (N-O:1) , WB=0.73/1.00 (B-O:1) , SSI=0.20/1.00 (L-M:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T31S	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	13-6-12	-118	-118	—	BACK	VERT	TOTAL

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.57 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11/29 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287533	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:15:32 2018 Page 1

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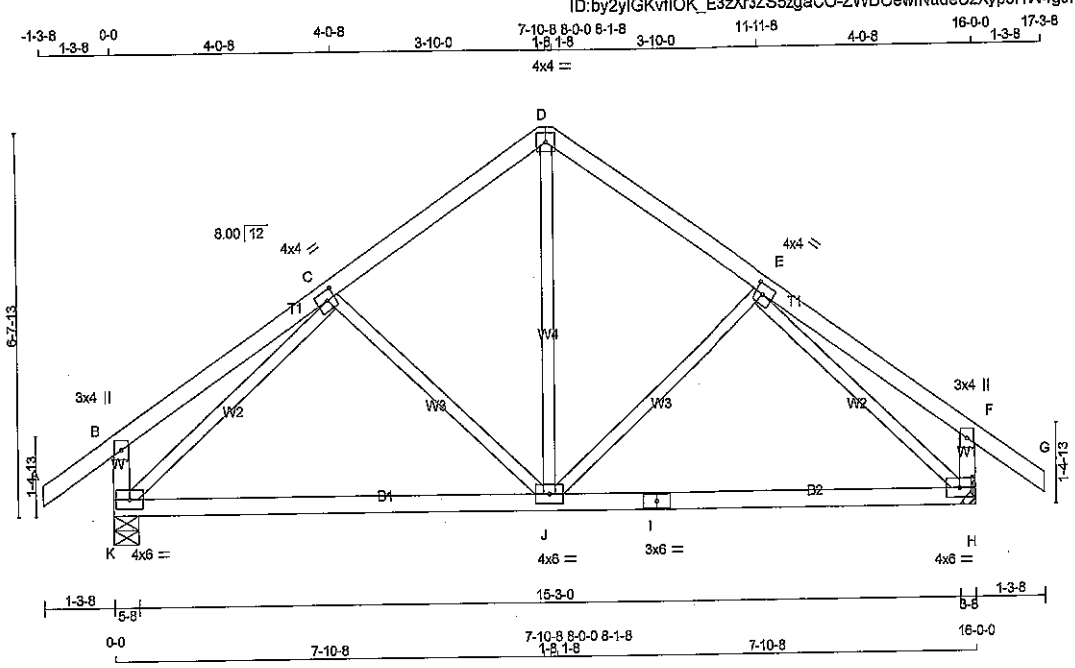


TOTAL WEIGHT = 68 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1207	0	1207	0	0	IN-SX	IN-SX
H	1207	0	1207	0	0	5-8	5-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	945	609 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	945	609 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	0 / 27	-104.9	-104.9	0.27 (1)		10.00
C- D	-863 / 0	-104.9	-104.9	0.21 (1)		6.25
D- E	-863 / 0	-104.9	-104.9	0.21 (1)		6.25
E- F	0 / 27	-104.9	-104.9	0.27 (1)		10.00
F- G	0 / 40	-104.9	-104.9	0.14 (1)		10.00
K- B	-301 / 0	0.0	0.0	0.03 (1)		7.81
H- F	-301 / 0	0.0	0.0	0.03 (1)		7.81
K- J	0 / 872	-28.0	-28.0	0.61 (2)		10.00
J- I	0 / 872	-28.0	-28.0	0.61 (2)		10.00
I- H	0 / 872	-28.0	-28.0	0.61 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-237 / 44	0.12 (1)
J- E	-237 / 44	0.12 (1)
K- C	-1198 / 0	0.57 (1)
E- H	-1198 / 0	0.57 (1)
J- D	0 / 596	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.20")

CSI: TC=0.27/1.00 (E-F:1) , BC=0.61/1.00 (J-K:2) , WB=0.57/1.00 (E-H:1) , SSI=0.19/1.00 (H-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

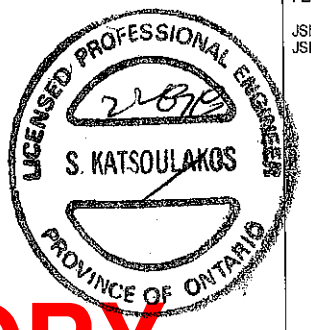
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)



DWG NO. TAM 11948-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T32S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:24 2018 Page 1

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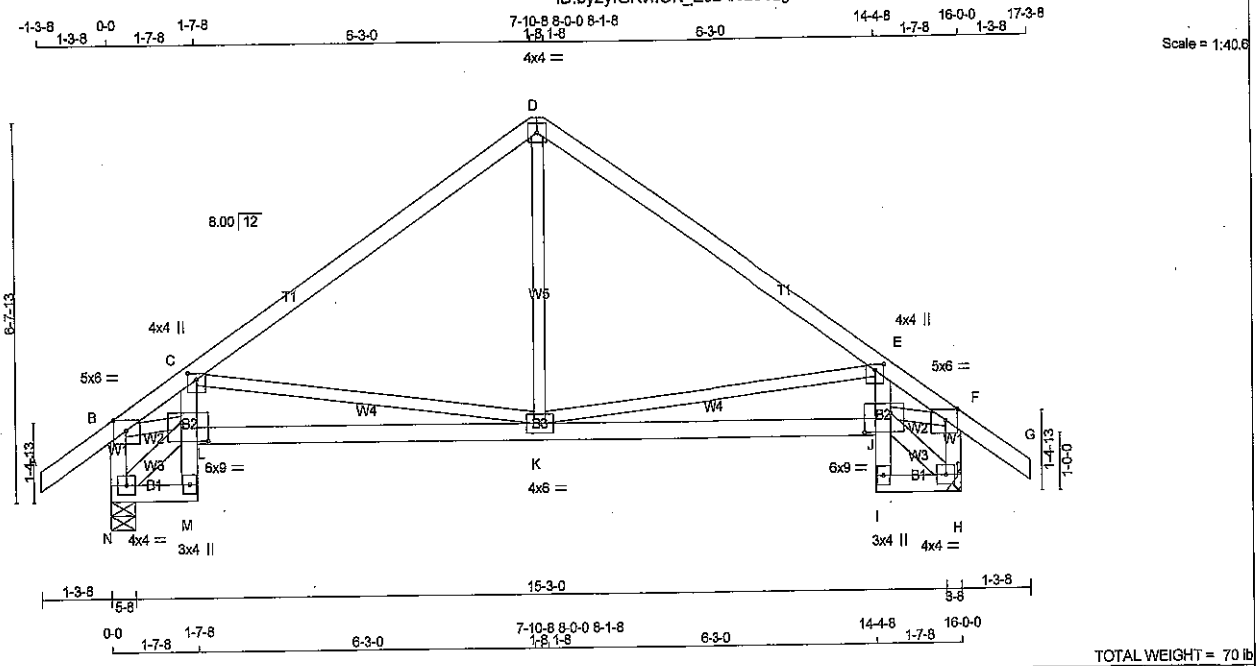

TOTAL WEIGHT = 70 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.25	2.75
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTW-p	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	TMVW-p	MT20	5.0	6.0	2.25	2.75
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	6.00
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	6.00
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1207	0	1207	0	0	5-8	5-8
H	1207	0	1207	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	945	609 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	945	609 / 0	168 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2054 / 0	-104.9	-104.9	0.43 (1)	4.24
C- D	-987 / 0	-104.9	-104.9	0.57 (1)	5.42
D- E	-987 / 0	-104.9	-104.9	0.57 (1)	5.42
E- F	-2054 / 0	-104.9	-104.9	0.43 (1)	4.24
F- G	0 / 40	-104.9	-104.9	0.14 (1)	10.00
N- B	-1119 / 0	0.0	0.0	0.12 (1)	7.49
H- F	-1119 / 0	0.0	0.0	0.12 (1)	7.49
N- M	0 / 85	-28.0	-28.0	0.03 (2)	10.00
M- L	0 / 36	0.0	0.0	0.14 (1)	10.00
L- C	0 / 214	0.0	0.0	0.17 (1)	10.00
L- K	0 / 1907	-28.0	-28.0	0.54 (2)	10.00
K- J	0 / 1907	-28.0	-28.0	0.54 (2)	10.00
I- J	0 / 36	0.0	0.0	0.14 (1)	10.00
J- E	0 / 214	0.0	0.0	0.17 (1)	10.00
I- H	0 / 85	-28.0	-28.0	0.03 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-1127 / 0	0.91 (1)
K- E	-1127 / 0	0.91 (1)
K- D	0 / 534	0.12 (2)
N- L	-109 / 0	0.01 (1)
B- L	0 / 1798	0.40 (1)
J- H	-109 / 0	0.01 (1)
J- F	0 / 1798	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.57/1.00 (C-D:1) , BC=0.54/1.00 (K-L:2) , WB=0.91/1.00 (C-K:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

DWG NO. TAM 11130

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:25 2018 Page 1

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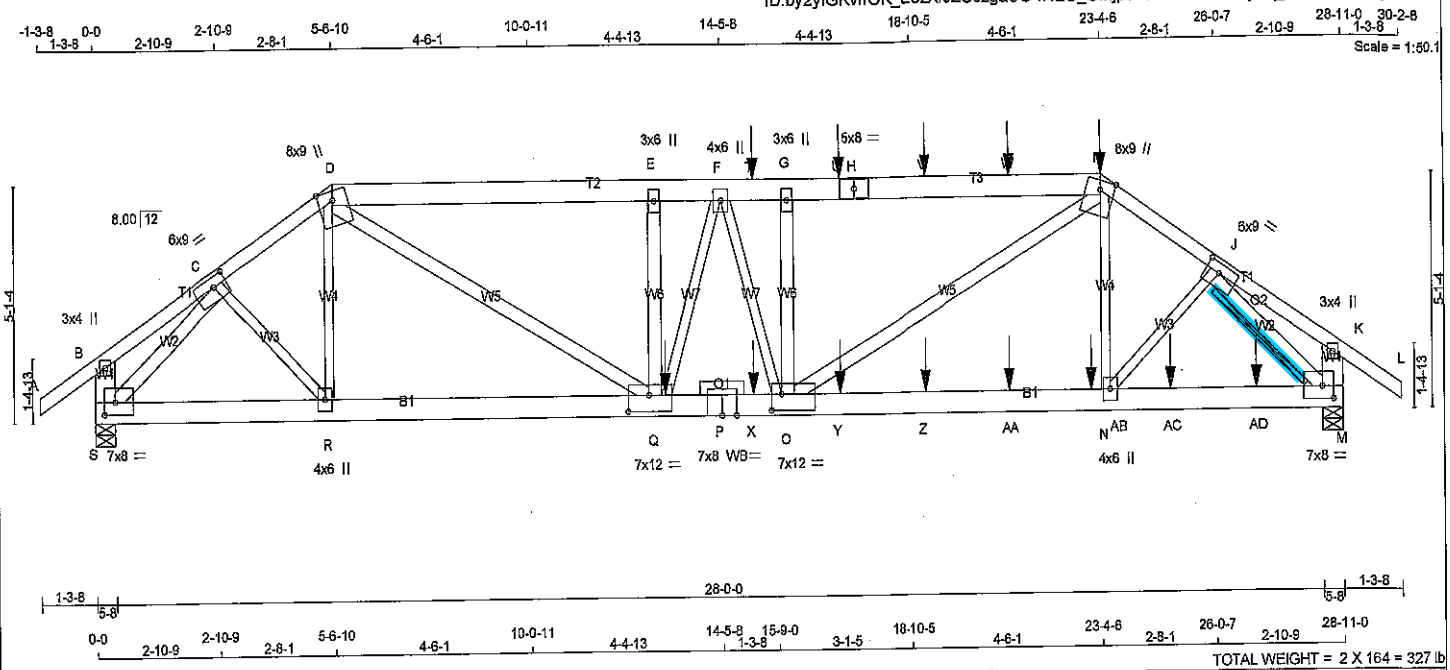

TOTAL WEIGHT = 2 X 164 = 327 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	2100F 1.8E	SPF
H - I	2x6	DRY	2100F 1.8E	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - Q	2x4	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(90.8)
I- L	1	12	SIDE(90.8)
D- H	2	12	SIDE(108.7)
H- I	2	12	SIDE(108.7)
S- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(197.8)
P- M	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
N- I	1	6	SIDE(89.8)
2x3	1	6	
E- Q	1	3	SIDE(616.5)
2x4	1	6	
G- O	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	6.0	9.0	2.50	3.75
D	TTWW+m	MT20	8.0	9.0	Edge	
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	8.0		
I	TTWW+m	MT20	8.0	9.0	Edge	
J	TMWW-t	MT20	6.0	9.0	2.50	3.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	9333	0	9333	0	0	5-8	5-8
S	8077	0	8077	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	7402	4600 / 0	1426 / 0	0 / 0	0 / 0	1376 / 0	0 / 0
S	6402	3985 / 0	1230 / 0	0 / 0	0 / 0	1188 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, S
BEARING SIZE FACTOR = 1.15 AT JNT(S) M, S (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	0 / 44	-322.3	-322.3	0.17 (1)	10.00
C- D	-9965 / 0	-322.3	-322.3	0.51 (1)	2.70
D- E	-15971 / 0	-322.3	-322.3	0.54 (1)	3.45
E- F	-15970 / 0	-322.3	-322.3	0.37 (1)	3.56
F- T	-15477 / 0	-322.3	-322.3	0.51 (1)	3.42
T- G	-15477 / 0	-322.3	-322.3	0.51 (1)	3.42
G- U	-15474 / 0	-322.3	-322.3	0.61 (1)	3.40
U- H	-15474 / 0	-322.3	-322.3	0.61 (1)	3.40
H- V	-15474 / 0	-322.3	-322.3	0.61 (1)	3.40
V- W	-15474 / 0	-322.3	-322.3	0.61 (1)	3.40
W- I	-15474 / 0	-322.3	-322.3	0.61 (1)	3.40
I- J	-11391 / 0	-322.3	-322.3	0.62 (1)	2.37
J- K	0 / 40	-322.3	-322.3	0.17 (1)	10.00
K- L	0 / 40	-104.9	-104.9	0.08 (1)	10.00
S- B	-510 / 0	0.0	0.0	0.02 (1)	10.00
M- K	-518 / 0	0.0	0.0	0.02 (1)	10.00
S- R	0 / 7323	-28.0	-28.0	0.23 (1)	10.00
R- Q	0 / 8309	-28.0	-28.0	0.29 (1)	10.00
Q- P	0 / 15491	-28.0	-28.0	0.47 (1)	10.00
P- X	0 / 15491	-28.0	-28.0	0.47 (1)	10.00
X- O	0 / 15491	-28.0	-28.0	0.47 (1)	10.00
O- Y	0 / 9478	-28.0	-28.0	0.35 (1)	10.00
Y- Z	0 / 9478	-28.0	-28.0	0.35 (1)	10.00
Z- AA	0 / 9478	-28.0	-28.0	0.35 (1)	10.00
AA- AB	0 / 9478	-28.0	-28.0	0.35 (1)	10.00
AB- N	0 / 9478	-28.0	-28.0	0.35 (1)	10.00
N- AC	0 / 8298	-28.0	-28.0	0.32 (1)	10.00
AC- AD	0 / 8298	-28.0	-28.0	0.32 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
R- D	-740 / 0	0.13 (1)
D- Q	0 / 9067	0.80 (1)
Q- E	-2426 / 0	0.30 (1)
O- G	-3416 / 0	0.42 (1)
O- I	0 / 7099	0.63 (1)
N- I	0 / 491	0.06 (2)
Q- F	0 / 1538	0.19 (1)
F- O	-131 / 0	0.02 (3)
C- R	0 / 1410	0.17 (1)
S- C	-10499 / 0	0.96 (1)
N- J	0 / 1729	0.21 (1)
J- M	-11390 / 0	0.87 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 9-0-0
END DISTANCE = 28-11-0
END SPAN CARRIED = 9-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 880 (0.39")

CSI: TC=0.62/1.00 (I-J:1) , BC=0.47/1.00 (O-Q:1) , WB=0.96/1.00 (C-S:1) , SSI=0.61/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)		SECTION (PLI)	
		MAX	MIN	MAX	MIN
MT20	565/	354	1667	822	2284

MAX MIN

1656

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T33	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	7.0	8.0	3.25	3.00
N	BMWW+t	MT20	4.0	6.0		
O	BMWWW-t	MT20	7.0	12.0	4.25	2.75
P	BS-t	MT20	7.0	8.0		
Q	BMWWW-t	MT20	7.0	12.0	4.25	5.75
R	BMWW+t	MT20	4.0	6.0		
S	BMVW1-t	MT20	7.0	8.0	3.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 194.6 lbs FACTORED DOWN AT 15-2-4, 194.6 lbs FACTORED DOWN AT 17-2-4, 194.6 lbs FACTORED DOWN AT 19-2-4, AND 194.6 lbs FACTORED DOWN AT 21-2-4, AND 526.9 lbs FACTORED DOWN AT 23-4-6 ON TOP CHORD, AND 3888.1 lbs FACTORED DOWN AT 13-2-0, 90.9 lbs FACTORED DOWN AT 15-2-4, 90.9 lbs FACTORED DOWN AT 17-2-4, 90.9 lbs FACTORED DOWN AT 19-2-4, 90.9 lbs FACTORED DOWN AT 21-2-4, 917.6 lbs FACTORED DOWN AT 23-1-8, AND 336.6 lbs FACTORED DOWN AT 24-11-12, AND 336.6 lbs FACTORED DOWN AT 26-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)		MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO						
AD-M	0 / 6298	-26.0	-28.0	0.32	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	23-4-6	-527	-527	---	FRONT	VERT	TOTAL
Q	13-2-0	-3888	-3888	---	FRONT	VERT	TOTAL
T	15-2-4	-195	-195	---	FRONT	VERT	TOTAL
U	17-2-4	-195	-195	---	FRONT	VERT	TOTAL
V	19-2-4	-195	-195	---	FRONT	VERT	TOTAL
W	21-2-4	-195	-195	---	FRONT	VERT	TOTAL
X	15-2-4	-52	-91	---	FRONT	VERT	TOTAL
Y	17-2-4	-52	-91	---	FRONT	VERT	TOTAL
Z	19-2-4	-52	-91	---	FRONT	VERT	TOTAL
AA	21-2-4	-52	-91	---	FRONT	VERT	TOTAL
AB	23-1-8	-918	-918	---	FRONT	VERT	TOTAL
AC	24-11-12	-337	-337	---	FRONT	VERT	TOTAL
AD	26-11-12	-337	-337	---	FRONT	VERT	TOTAL

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (Q) (INPUT = 0.90)
JSI METAL= 0.99 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11181-TB
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287533	T33Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Thu Mar 1 09:15:32 2018 Page 1

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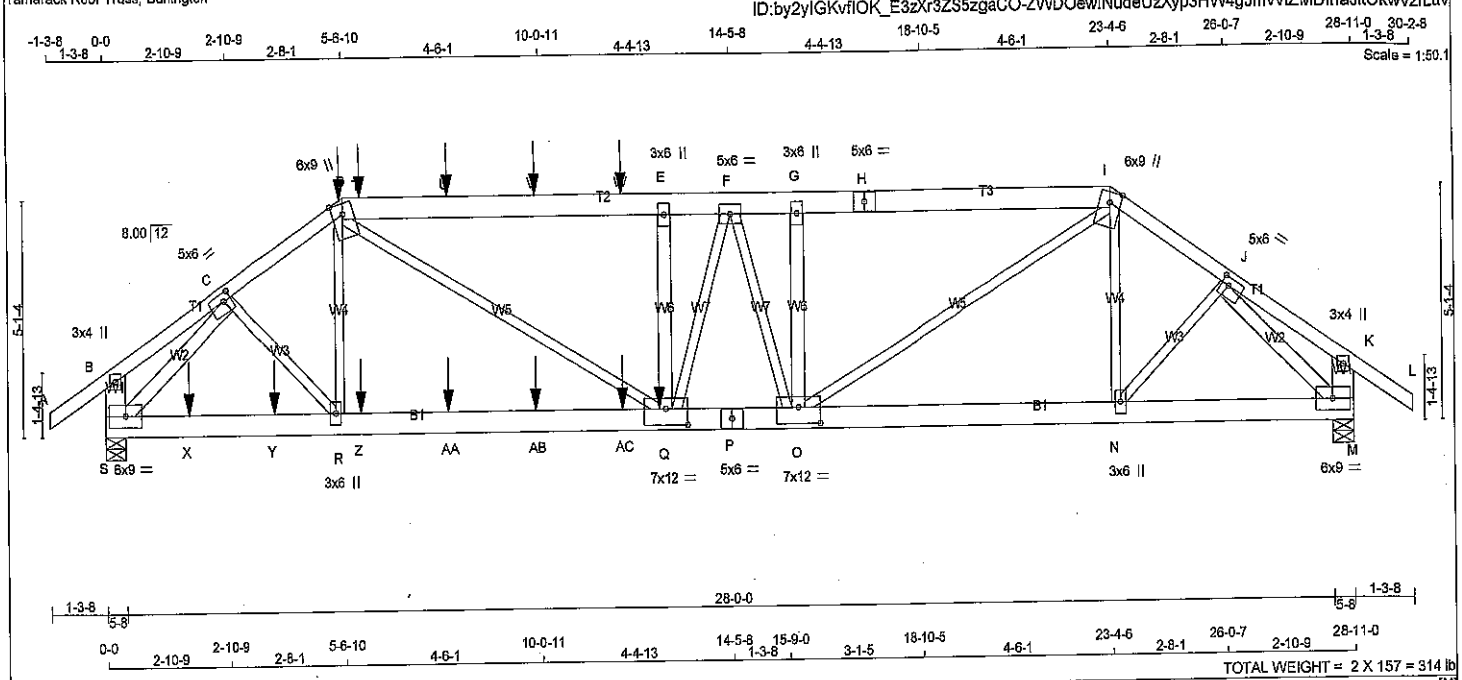

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x6	DRY	2100F 1.8E	SPF
H - I	2x6	DRY	2100F 1.8E	SPF
I - L	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
S - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - E	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	SIDE(61.0)
I- L	1	12	TOP
D- H	2	12	SIDE(65.9)
H- I	2	12	TOP
S- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	SIDE(197.8)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
R- D	1	6	SIDE(10.6)
2x3	1	6	
E- Q	1	4	SIDE(555.3)
2x4	1	4	
S- C	1	6	
J- M	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.00	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	
E	TMW+w	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	
J	TMWW-t	MT20	5.0	6.0	2.00	2.00
K	TMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	4016	0	4016	0	0	5-8	5-8
S	5797	0	5797	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	3152	2016 / 0	569 / 0	0 / 0	0 / 0	566 / 0	0 / 0
S	4570	2865 / 0	850 / 0	0 / 0	0 / 0	834 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, S

BEARING SIZE FACTOR = 1.15 AT JNT(S) M, S (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.47 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 40	-104.9	-104.9	0.08	(1)	10.00	R- D	0 / 587	0.07 (2)
B- C	-5 / 6	-104.9	-104.9	0.06	(1)	10.00	D- Q	0 / 4408	0.55 (1)
C- D	-7298 / 0	-104.9	-104.9	0.25	(1)	3.47	Q- E	-857 / 0	0.11 (1)
D- T	-9808 / 0	-104.9	-104.9	0.23	(1)	4.61	O- G	-843 / 0	0.10 (1)
T- U	-9808 / 0	-104.9	-104.9	0.23	(1)	4.61	O- I	0 / 5029	0.62 (1)
U- V	-9808 / 0	-104.9	-104.9	0.23	(1)	4.61	N- I	-357 / 74	0.06 (1)
V- W	-9808 / 0	-104.9	-104.9	0.23	(1)	4.61	Q- F	0 / 2355	0.29 (1)
W- E	-9808 / 0	-104.9	-104.9	0.23	(1)	4.61	F- O	-1997 / 0	0.38 (1)
E- F	-9808 / 0	-104.9	-104.9	0.11	(1)	4.71	C- R	0 / 1357	0.17 (1)
F- G	-8454 / 0	-104.9	-104.9	0.12	(1)	4.99	S- C	-7400 / 0	0.68 (1)
G- H	-8453 / 0	-104.9	-104.9	0.16	(1)	4.96	N- J	0 / 853	0.11 (1)
H- I	-8453 / 0	-104.9	-104.9	0.16	(1)	4.96	J- M	-5176 / 0	0.47 (1)
I- J	-5033 / 0	-104.9	-104.9	0.14	(1)	4.21			
J- K	-2 / 11	-104.9	-104.9	0.08	(1)	10.00			
K- L	0 / 40	-104.9	-104.9	0.08	(1)	10.00			
S- B	-278 / 0	0.0	0.0	0.01	(1)	7.81			
M- K	-268 / 0	0.0	0.0	0.01	(1)	7.81			
S- X	0 / 5161	-28.0	-28.0	0.32	(1)	10.00			
X- Y	0 / 5161	-28.0	-28.0	0.32	(1)	10.00			
Y- R	0 / 5161	-28.0	-28.0	0.32	(1)	10.00			
R- Z	0 / 6083	-28.0	-28.0	0.35	(1)	10.00			
Z- AA	0 / 6083	-28.0	-28.0	0.35	(1)	10.00			
AA- AB	0 / 6083	-28.0	-28.0	0.35	(1)	10.00			
AB- AC	0 / 6083	-28.0	-28.0	0.35	(1)	10.00			
AC- Q	0 / 6083	-28.0	-28.0	0.35	(1)	10.00			
Q- P	0 / 9075	-28.0	-28.0	0.31	(1)	10.00			
P- O	0 / 9075	-28.0	-28.0	0.31	(1)	10.00			
O- N	0 / 4205	-28.0	-28.0	0.15	(1)	10.00			
N- M	0 / 3610	-28.0	-28.0	0.14	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-6-11	-527	-527	---	FRONT	VERT	TOTAL
Q	12-9-8	-2887	-2887	---	FRONT	VERT	TOTAL
T	5-11-4	-86	-86	---	FRONT	VERT	TOTAL
U	7-11-4	-54	-54	---	FRONT	VERT	TOTAL
V	9-11-4	-54	-54	---	FRONT	VERT	TOTAL
W	11-11-4	-54	-54	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.35/1.00 (Q-R:1) , WB=0.68/1.00 (C-S:1) , SSI=0.30/1.00 (Q-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.93 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 11149 -R8

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287533	T33Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:by2yIGKvfIOK_E3zXr3ZS6zgaCO-ZWDOewINudeUzXyp3HW4gJmWtZMDIhaJltCkwvzfLdv

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMVW1-t	MT20	6.0	9.0		
N	BMWW+t	MT20	3.0	6.0		
O	BMWWW-t	MT20	7.0	12.0	4.25	6.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	7.0	12.0	4.25	6.00
R	BMWW+t	MT20	3.0	6.0		
S	BMVW1-t	MT20	6.0	9.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 86.3 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, 53.9 lbs FACTORED DOWN AT 9-11-4, AND 53.9 lbs FACTORED DOWN AT 11-11-4, AND 526.9 lbs FACTORED DOWN AT 5-6-11 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, AND 336.6 lbs FACTORED DOWN AT 11-11-4, AND 2886.8 lbs FACTORED DOWN AT 12-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	1-11-4	-337	-337	—	FRONT	VERT	TOTAL
Y	3-11-4	-337	-337	—	FRONT	VERT	TOTAL
Z	5-11-4	-337	-337	—	FRONT	VERT	TOTAL
AA	7-11-4	-337	-337	—	FRONT	VERT	TOTAL
AB	9-11-4	-337	-337	—	FRONT	VERT	TOTAL
AC	11-11-4	-337	-337	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
22076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11149 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T33S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:25 2018 Page 1

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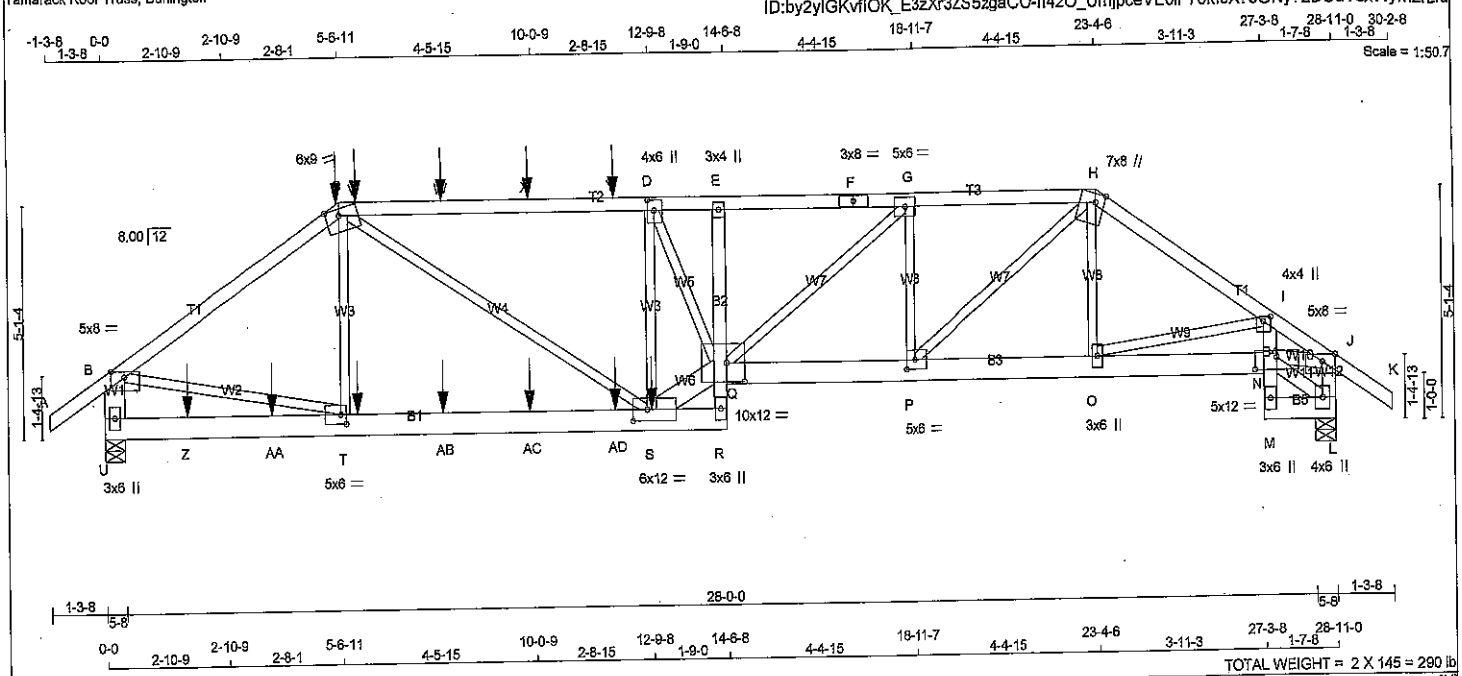

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - K	2x4	DRY	1650F 1.5E	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- C 1	12	SIDE(61.0)
C- F 1	12	SIDE(61.0)
F- H 1	12	TOP
H- K 1	12	TOP
L- J 1	12	TOP
U- B 2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
U- R 2	12	SIDE(183.1)
Q- N 2	12	TOP
M- L 2	12	TOP
R- E 1	12	TOP
I- M 1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
T- C 1	6	SIDE(15.5)
S- D 1	5	SIDE(503.4)
2x3 1	6	
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.75
C	TTWW-m	MT20	6.0	9.0	1.75	3.75
D	TMWW+t	MT20	4.0	6.0	2.75	1.75
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	5669	0	5669	0	0	5-8	5-8
L	3889	0	3889	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	4481	2810 / 0	846 / 0	0 / 0	0 / 0	824 / 0	0 / 0
L	3060	1943 / 0	562 / 0	0 / 0	0 / 0	555 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
A- B	0 / 40	-104.9	-104.9	0.06 (1)	10.00
B- C	-7049 / 0	-104.9	-104.9	0.46 (1)	3.91
C- V	-9375 / 0	-104.9	-104.9	0.93 (1)	3.14
V- W	-9375 / 0	-104.9	-104.9	0.93 (1)	3.14
W- X	-9375 / 0	-104.9	-104.9	0.93 (1)	3.14
X- Y	-9375 / 0	-104.9	-104.9	0.93 (1)	3.14
Y- D	-9375 / 0	-104.9	-104.9	0.93 (1)	3.14
D- E	-10707 / 0	-104.9	-104.9	0.57 (1)	2.89
E- F	-10767 / 0	-104.9	-104.9	0.37 (1)	3.28
F- G	-10767 / 0	-104.9	-104.9	0.37 (1)	3.28
G- H	-8316 / 0	-104.9	-104.9	0.29 (1)	3.75
H- I	-5071 / 0	-104.9	-104.9	0.21 (1)	4.35
I- J	-8525 / 0	-104.9	-104.9	0.12 (1)	4.30
J- K	0 / 40	-104.9	-104.9	0.06 (1)	10.00
U- B	-5388 / 0	0.0	0.0	0.19 (1)	6.31
L- J	-3694 / 0	0.0	0.0	0.21 (1)	6.12
U- Z	0 / 0	-28.0	-28.0	0.25 (1)	10.00
Z-AA	0 / 0	-28.0	-28.0	0.25 (1)	10.00
AA- T	0 / 0	-28.0	-28.0	0.25 (1)	10.00
T-AB	0 / 5866	-28.0	-28.0	0.64 (1)	10.00
AB-AC	0 / 5866	-28.0	-28.0	0.64 (1)	10.00
AC-AD	0 / 5866	-28.0	-28.0	0.64 (1)	10.00
AD- S	0 / 5866	-28.0	-28.0	0.64 (1)	10.00
S- R	0 / 224	-28.0	-28.0	0.26 (1)	10.00
R- Q	-393 / 0	0.0	0.0	0.17 (1)	7.81
Q- E	0 / 247	0.0	0.0	0.19 (1)	10.00
Q- P	0 / 8316	-28.0	-28.0	0.59 (1)	10.00
P- O	0 / 5050	-28.0	-28.0	0.37 (1)	10.00
O- N	0 / 5677	-28.0	-28.0	0.40 (1)	10.00
M- N	0 / 36	0.0	0.0	0.19 (1)	10.00
N- I	0 / 165	0.0	0.0	0.21 (1)	10.00
M- L	0 / 246	-28.0	-28.0	0.02 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
B- T	0 / 5948	0.74 (1)
T- C	0 / 520	0.06 (2)
C- S	0 / 4188	0.52 (1)
Q- G	0 / 3197	0.40 (1)
P- G	-2628 / 0	0.33 (1)
P- H	0 / 4230	0.52 (1)
O- H	0 / 310	0.04 (2)
O- I	-652 / 0	0.10 (1)
S- D	-4220 / 0	0.79 (1)
S- Q	0 / 10784	0.70 (1)
D- Q	0 / 3370	0.42 (1)
N- L	-302 / 0	0.02 (1)
N- J	0 / 5404	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-6-11	-527	-527	---	FRONT	VERT	TOTAL
S	12-9-8	-2631	-2631	---	FRONT	VERT	TOTAL
T	5-11-4	-337	-337	---	FRONT	VERT	TOTAL
V	5-11-4	-86	-86	---	FRONT	VERT	TOTAL
W	7-11-4	-54	-54	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 780 (0.44")

CSI: TC=0.93/1.00 (C-D:1) , BC=0.64/1.00 (S-T:1) , WB=0.79/1.00 (D-S:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T33S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TTWW+m	MT20	7.0	8.0	Edge	2.50
I	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	TMVW-p	MT20	5.0	8.0	Edge	3.50
L	BMVW1+p	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-I	MT20	5.0	12.0	3.25	6.00
O	BMWW+t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.50
Q	BVMWWW-I	MT20	10.0	12.0	5.00	5.00
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	12.0	3.00	4.00
T	BMWW-t	MT20	5.0	6.0	2.50	1.50
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 526.9 lbs FACTORED DOWN AT 5-6-11, 86.3 lbs FACTORED DOWN AT 5-11-4, 53.9 lbs FACTORED DOWN AT 7-11-4, AND 53.9 lbs FACTORED DOWN AT 9-11-4, AND 53.9 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 336.6 lbs FACTORED DOWN AT 1-11-4, 336.6 lbs FACTORED DOWN AT 3-11-4, 336.6 lbs FACTORED DOWN AT 5-11-4, 336.6 lbs FACTORED DOWN AT 7-11-4, 336.6 lbs FACTORED DOWN AT 9-11-4, AND 336.6 lbs FACTORED DOWN AT 11-11-4, AND 2631.4 lbs FACTORED DOWN AT 12-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	9-11-4	-54	-54	---	FRONT	VERT	TOTAL
Y	11-11-4	-54	-54	---	FRONT	VERT	TOTAL
Z	1-11-4	-337	-337	---	FRONT	VERT	TOTAL
AA	3-11-4	-337	-337	---	FRONT	VERT	TOTAL
AB	7-11-4	-337	-337	---	FRONT	VERT	TOTAL
AC	9-11-4	-337	-337	---	FRONT	VERT	TOTAL
AD	11-11-4	-337	-337	---	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.73 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
22878
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11132
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:26 2018 Page 1

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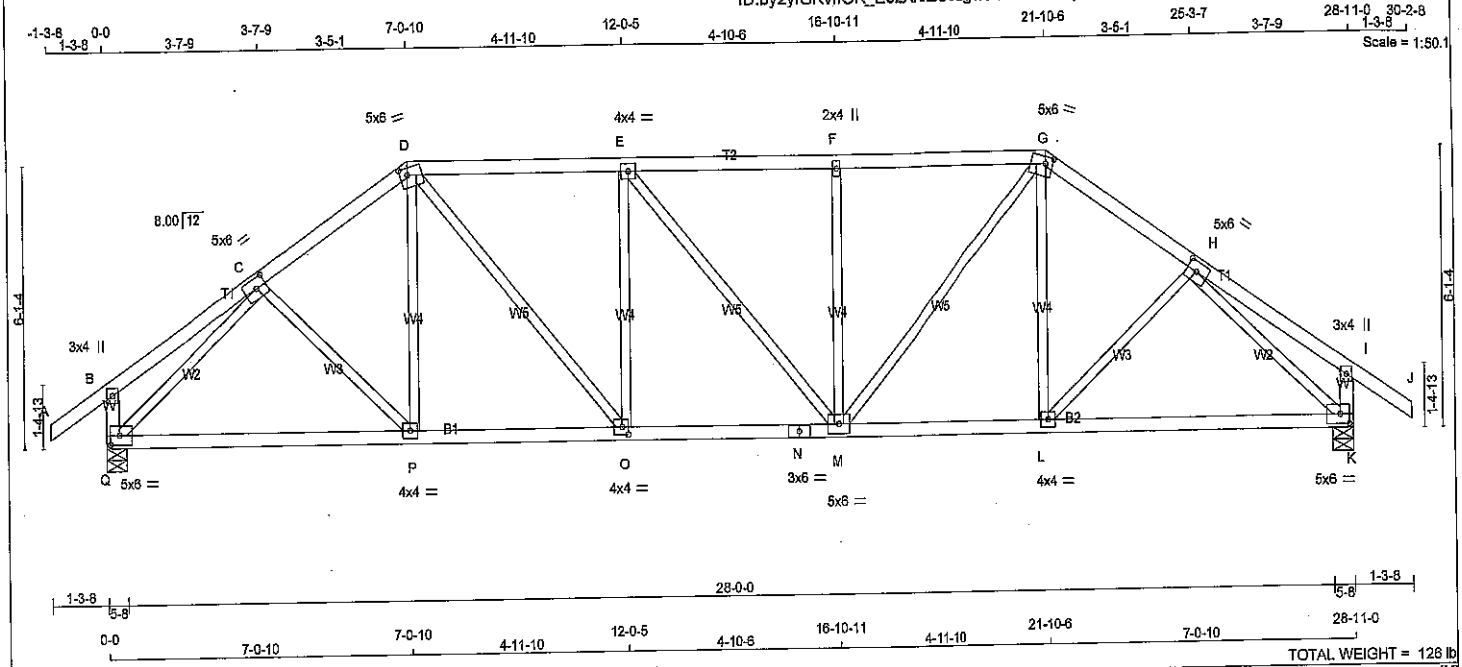

TOTAL WEIGHT = 126 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2066	0	2066	0	0	5-8	5-8
K	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1529	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 21	-104.9	-104.9	0.18 (1)	10.00	
C- D	-2174 / 0	-104.9	-104.9	0.25 (1)	4.39	
D- E	-2370 / 0	-104.9	-104.9	0.45 (1)	4.00	
E- F	-2369 / 0	-104.9	-104.9	0.45 (1)	4.00	
F- G	-2368 / 0	-104.9	-104.9	0.44 (1)	4.01	
G- H	-2174 / 0	-104.9	-104.9	0.25 (1)	4.38	
H- I	0 / 21	-104.9	-104.9	0.18 (1)	10.00	
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-291 / 0	0.0	0.0	0.03 (1)	7.81	
K- I	-291 / 0	0.0	0.0	0.03 (1)	7.81	
Q- P	0 / 1738	-28.0	-28.0	0.48 (2)	10.00	
P- O	0 / 1791	-28.0	-28.0	0.50 (2)	10.00	
O- N	0 / 2371	-28.0	-28.0	0.45 (1)	10.00	
N- M	0 / 2371	-28.0	-28.0	0.45 (1)	10.00	
M- L	0 / 1791	-28.0	-28.0	0.50 (2)	10.00	
L- K	0 / 1738	-28.0	-28.0	0.48 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 152	0.03 (3)
P- D	0 / 258	0.06 (2)
D- O	0 / 891	0.20 (1)
O- E	-557 / 0	0.33 (1)
E- M	-3 / 0	0.00 (1)
M- F	-556 / 0	0.33 (1)
M- G	0 / 888	0.20 (1)
L- G	0 / 259	0.06 (2)
L- H	0 / 152	0.03 (3)
Q- C	-2426 / 0	0.93 (1)
H- K	-2426 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.45/1.00 (D-E:1) , BC=0.50/1.00 (L-M:2) , WB=0.93/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (K) (INPUT = 0.90)
JSI METAL= 0.63 (N) (INPUT = 1.00)

DWG NO. TAM 11133-S1

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T34S	1	1	TRUSS DESC.	

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:26 2018 Page 1

Tamarack Roof Truss, Burlington

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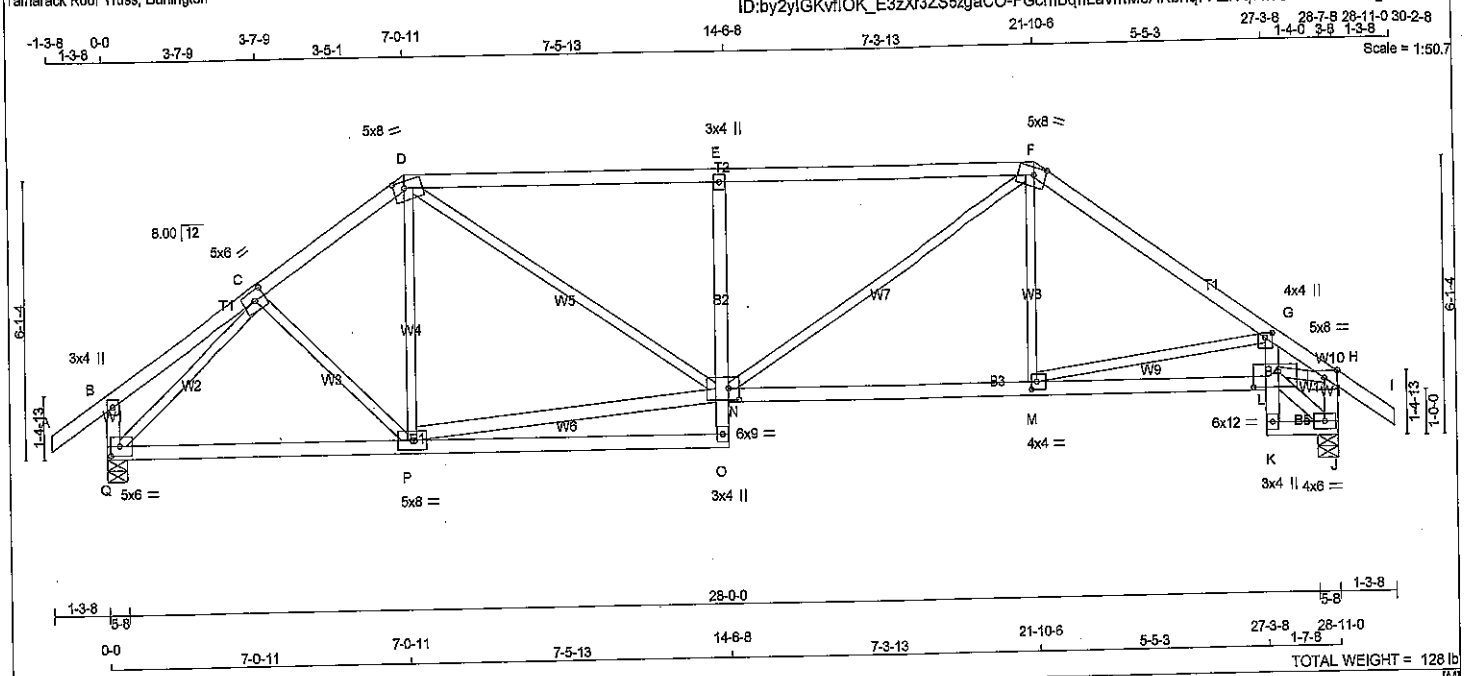

TOTAL WEIGHT = 128 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	2100F 1.8E	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.00
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	8.0	2.00	3.25
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMWW1-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-I	MT20	6.0	12.0	4.00	7.00
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BVMWWW-I	MT20	6.0	9.0	3.00	3.00
O	BMV+p	MT20	3.0	4.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMVW1-t	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2063	0	2063	0	0	5-8	5-8
J	2069	0	2069	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1627	1026 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1632	1030 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.79 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		(PLF)				FR-TO		
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	C- P	0 / 156	0.04 (3)
B- C	0 / 21	-104.9	-104.9	0.19 (1)	10.00	P- D	-33 / 226	0.05 (3)
C- D	-2181 / 0	-104.9	-104.9	0.26 (1)	4.38	P- N	0 / 1735	0.26 (1)
D- E	-3004 / 0	-104.9	-104.9	0.85 (1)	3.85	D- N	0 / 1447	0.33 (1)
E- F	-3020 / 0	-104.9	-104.9	0.86 (1)	3.83	N- F	0 / 1028	0.23 (1)
F- G	-2606 / 0	-104.9	-104.9	0.68 (1)	3.86	M- F	0 / 447	0.10 (2)
G- H	-3516 / 0	-104.9	-104.9	0.41 (1)	3.33	M- G	-1017 / 0	0.59 (1)
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	Q- C	-2433 / 0	0.94 (1)
Q- B	-290 / 0	0.0	0.0	0.03 (1)	7.81	L- J	-183 / 0	0.02 (1)
J- H	-1935 / 0	0.0	0.0	0.20 (1)	6.05	L- H	0 / 2985	0.67 (1)
Q- P	0 / 1743	-28.0	-28.0	0.60 (2)	10.00			
P- O	0 / 74	-28.0	-28.0	0.43 (2)	10.00			
O- N	0 / 137	0.0	0.0	0.11 (1)	10.00			
N- E	-951 / 0	0.0	0.0	0.34 (1)	7.79			
N- M	0 / 2155	-28.0	-28.0	0.60 (2)	10.00			
M- L	0 / 3148	-28.0	-28.0	0.71 (1)	10.00			
K- L	0 / 36	0.0	0.0	0.23 (1)	10.00			
L- G	0 / 242	0.0	0.0	0.27 (1)	10.00			
K- J	0 / 143	-28.0	-28.0	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.86/1.00 (E-F:1) , BC=0.71/1.00 (L-M:1) , WB=0.94/1.00 (C-Q:1) , SSI=0.38/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.60 (C) (INPUT = 1.00)

DWG NO. TAM 1134
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:26 2018 Page 1

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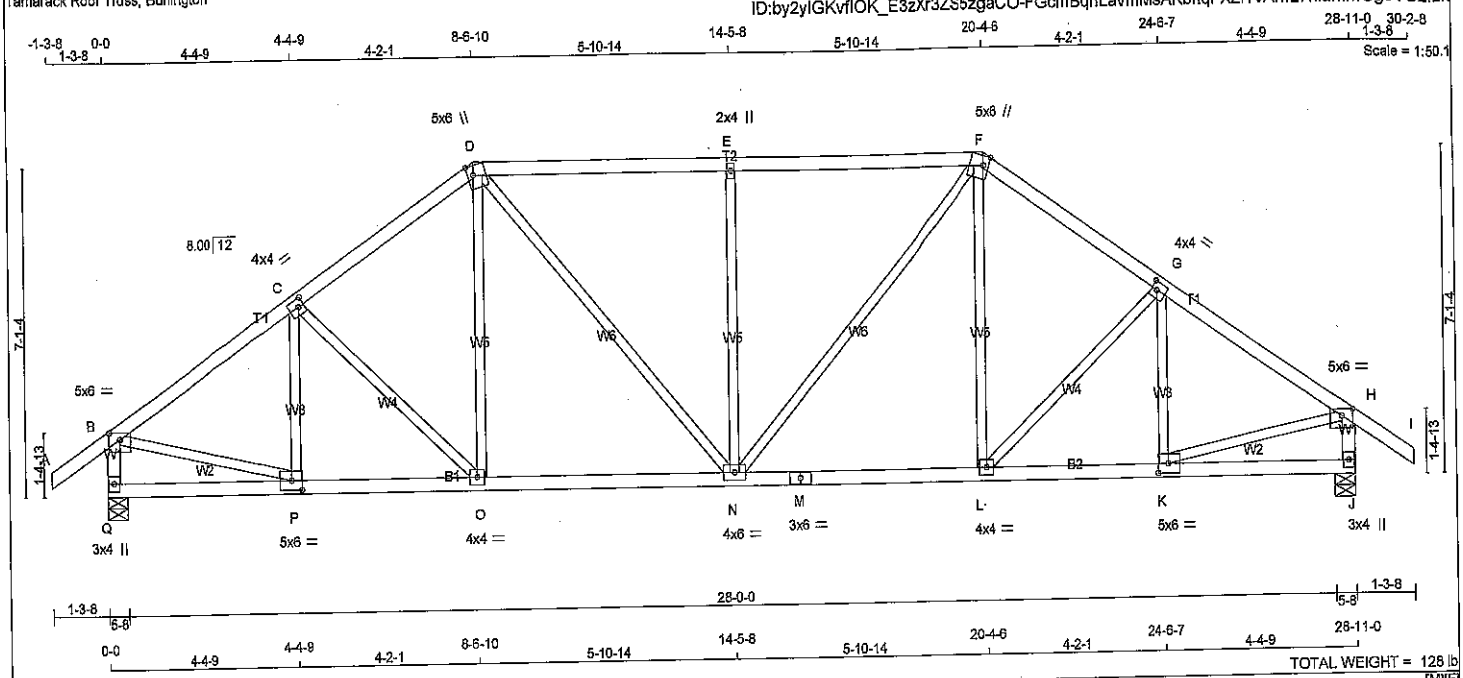

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2066	0	2066	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1529	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A-B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B-C	-2193 / 0	-104.9	-104.9	0.30 (1)		4.34
C-D	-2087 / 0	-104.9	-104.9	0.29 (1)		4.44
D-E	-2115 / 0	-104.9	-104.9	0.54 (1)		4.07
E-F	-2115 / 0	-104.9	-104.9	0.54 (1)		4.07
F-G	-2087 / 0	-104.9	-104.9	0.29 (1)		4.44
G-H	-2193 / 0	-104.9	-104.9	0.30 (1)		4.34
H-I	0 / 40	-104.9	-104.9	0.14 (1)		10.00
Q-B	-2011 / 0	0.0	0.0	0.21 (1)		5.95
J-H	-2011 / 0	0.0	0.0	0.21 (1)		5.95
Q-P	0 / 0	-28.0	-28.0	0.11 (2)		10.00
P-O	0 / 1848	-28.0	-28.0	0.38 (1)		10.00
O-N	0 / 1714	-28.0	-28.0	0.39 (2)		10.00
N-M	0 / 1714	-28.0	-28.0	0.39 (2)		10.00
M-L	0 / 1714	-28.0	-28.0	0.39 (2)		10.00
L-K	0 / 1848	-28.0	-28.0	0.38 (1)		10.00
K-J	0 / 0	-28.0	-28.0	0.11 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-336 / 32	0.09 (1)
C-O	-190 / 0	0.11 (1)
O-D	0 / 322	0.07 (2)
D-N	0 / 613	0.14 (1)
N-E	-758 / 0	0.66 (1)
N-F	0 / 613	0.14 (1)
L-F	0 / 322	0.07 (2)
L-G	-190 / 0	0.11 (1)
K-G	-336 / 32	0.09 (1)
B-P	0 / 1903	0.43 (1)
K-H	0 / 1903	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.39/1.00 (N-O:2) , WB=0.66/1.00 (E-N:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.47 (M) (INPUT = 1.00)

DWG NO. TAM 11135-R8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T35S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:27 2018 Page 1

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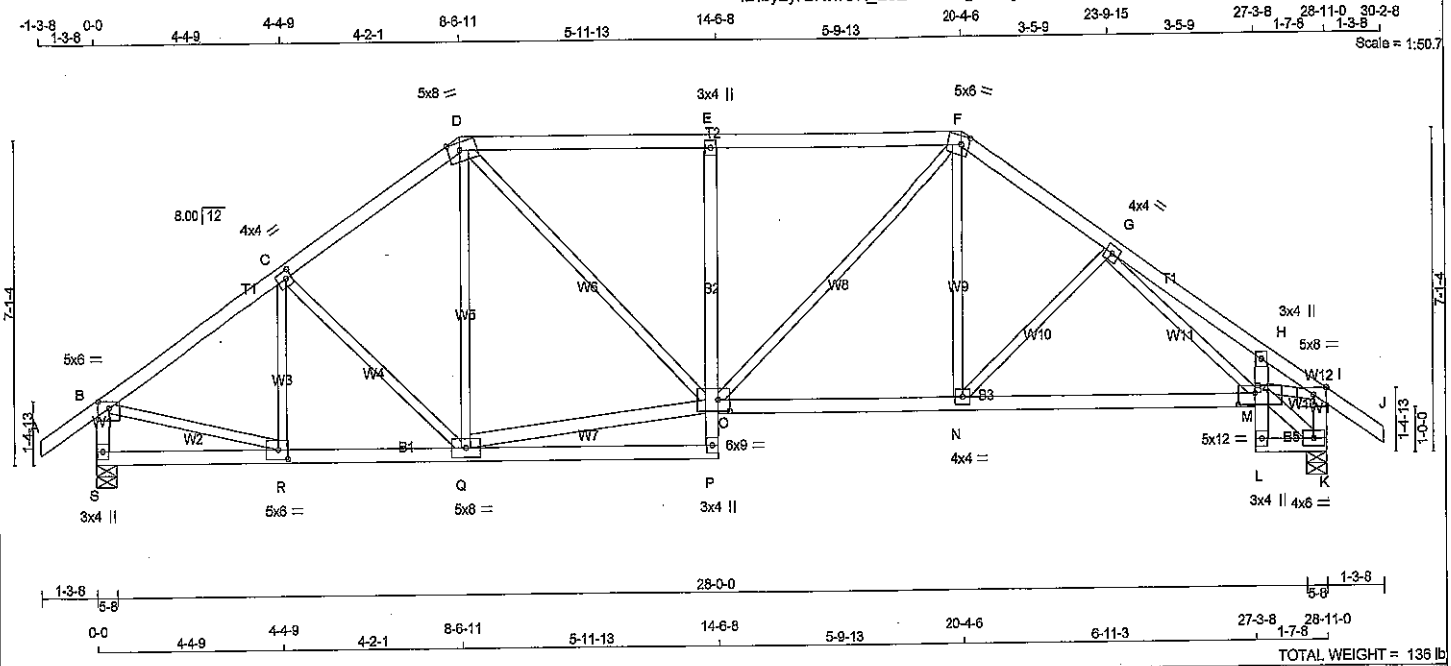

TOTAL WEIGHT = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMWW-t	MT20	4.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	TMVW-p	MT20	5.0	8.0	Edge	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWWW-I	MT20	5.0	12.0	3.00	4.75
N	BMWW-t	MT20	4.0	4.0		
O	BVMWWW-I	MT20	6.0	9.0	3.00	3.25
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2063	0	2063	0	0	5-8	5-8
K	2089	0	2089	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1627	1026 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1632	1030 / 0	304 / 0	0 / 0	0 / 0	298 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)		10.00
B- C	-2187 / 0	-104.9	-104.9	0.30 (1)		4.35
C- D	-2089 / 0	-104.9	-104.9	0.28 (1)		4.43
D- E	-2449 / 0	-104.9	-104.9	0.56 (1)		3.80
E- F	-2457 / 0	-104.9	-104.9	0.56 (1)		3.80
F- G	-2436 / 0	-104.9	-104.9	0.24 (1)		4.21
G- H	-2857 / 0	-104.9	-104.9	0.27 (1)		3.91
H- I	-2822 / 0	-104.9	-104.9	0.18 (1)		4.00
I- J	0 / 40	-104.9	-104.9	0.14 (1)		10.00
S- B	-2006 / 0	0.0	0.0	0.21 (1)		5.96
K- I	-1972 / 0	0.0	0.0	0.20 (1)		6.00
S- R	0 / 0	-28.0	-28.0	0.12 (2)		10.00
R- Q	0 / 1843	-28.0	-28.0	0.42 (1)		10.00
Q- P	0 / 47	-28.0	-28.0	0.21 (2)		10.00
P- O	0 / 117	0.0	0.0	0.08 (1)		10.00
O- E	-755 / 0	0.0	0.0	0.45 (1)		7.81
O- N	0 / 2012	-28.0	-28.0	0.58 (2)		10.00
N- M	0 / 2321	-28.0	-28.0	0.60 (2)		10.00
L- M	0 / 36	0.0	0.0	0.16 (1)		10.00
M- H	-170 / 0	0.0	0.0	0.12 (1)		7.81
L- K	0 / 113	-28.0	-28.0	0.03 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-345 / 14	0.10 (1)
C- Q	-182 / 0	0.11 (1)
Q- D	0 / 197	0.04 (3)
Q- O	0 / 1688	0.27 (1)
D- O	0 / 1040	0.23 (1)
O- F	0 / 621	0.14 (1)
N- F	0 / 532	0.12 (2)
N- G	-433 / 0	0.18 (1)
G- M	0 / 145	0.03 (2)
B- R	0 / 1998	0.43 (1)
M- K	-137 / 0	0.01 (1)
M- I	0 / 2308	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.56/1.00 (E-F:1) , BC=0.60/1.00 (M-N:2) , WB=0.52/1.00 (I-M:1) , SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	615	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.46 (R) (INPUT = 1.00)

DWG NO. TAM 11136-A
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T36	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:27 2018 Page 1

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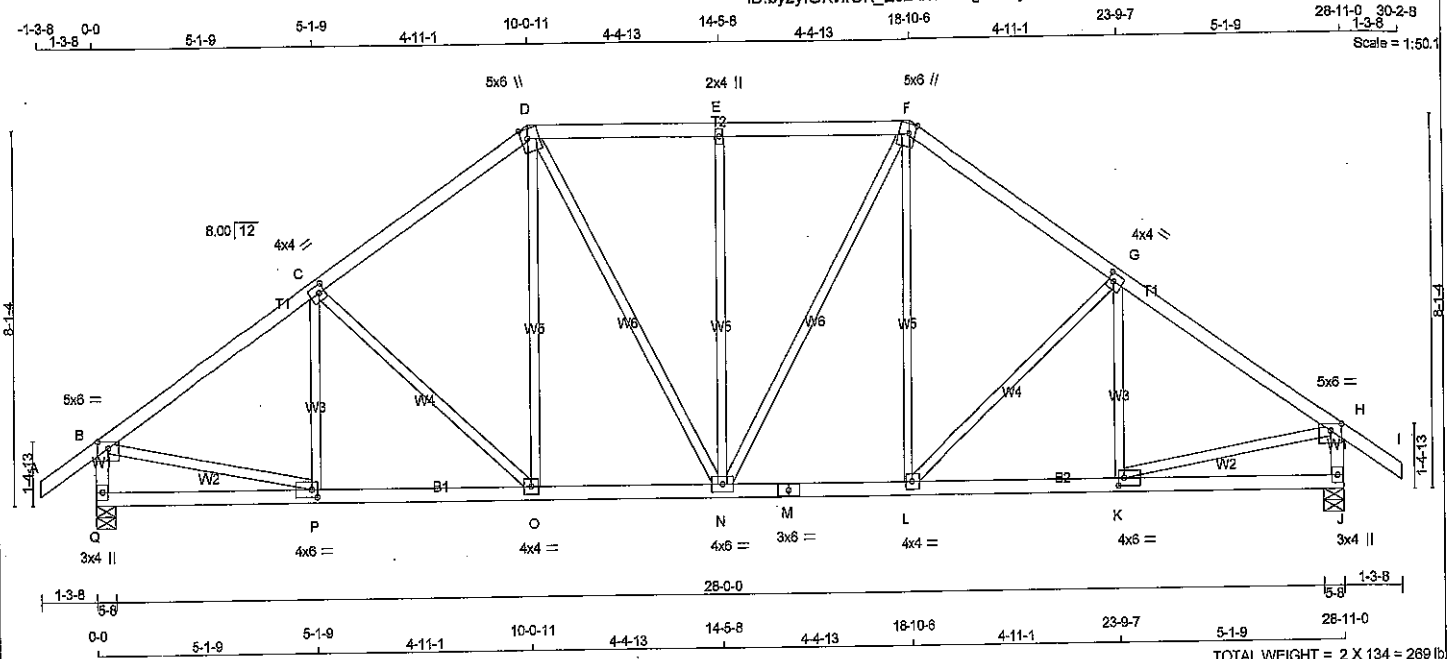

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	1.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED DOWN	MAXIMUM FACTORED HORZ	MAXIMUM FACTORED UPLIFT	INPUT BRG	REQRD BRG
Q	2066	0	2066	0	0	IN-SX 5-8	IN-SX 5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2231 / 0	-104.9	-104.9	0.42 (1)	4.17
C- D	-1983 / 0	-104.9	-104.9	0.40 (1)	4.41
D- E	-1813 / 0	-104.9	-104.9	0.29 (1)	4.69
E- F	-1813 / 0	-104.9	-104.9	0.29 (1)	4.69
F- G	-1983 / 0	-104.9	-104.9	0.39 (1)	4.41
G- H	-2231 / 0	-104.9	-104.9	0.42 (1)	4.17
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-2005 / 0	0.0	0.0	0.21 (1)	5.96
J- H	-2005 / 0	0.0	0.0	0.21 (1)	5.96
Q- P	0 / 0	-28.0	-28.0	0.17 (3)	10.00
P- O	0 / 1885	-28.0	-28.0	0.38 (1)	10.00
O- N	0 / 1624	-28.0	-28.0	0.33 (1)	10.00
N- M	0 / 1624	-28.0	-28.0	0.33 (1)	10.00
M- L	0 / 1624	-28.0	-28.0	0.33 (1)	10.00
L- K	0 / 1885	-28.0	-28.0	0.38 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-241 / 115	0.08 (1)
C- O	-357 / 0	0.30 (1)
O- D	0 / 375	0.08 (2)
D- N	0 / 385	0.09 (1)
N- E	-557 / 0	0.71 (1)
N- F	0 / 385	0.09 (1)
L- F	0 / 375	0.08 (2)
L- G	-357 / 0	0.30 (1)
K- G	-241 / 115	0.08 (1)
B- P	0 / 1926	0.43 (1)
K- H	0 / 1926	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.38/1.00 (O-P:1) , WB=0.71/1.00 (E-N:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.47 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2206
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111372-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T37	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:26 2018 Page 1

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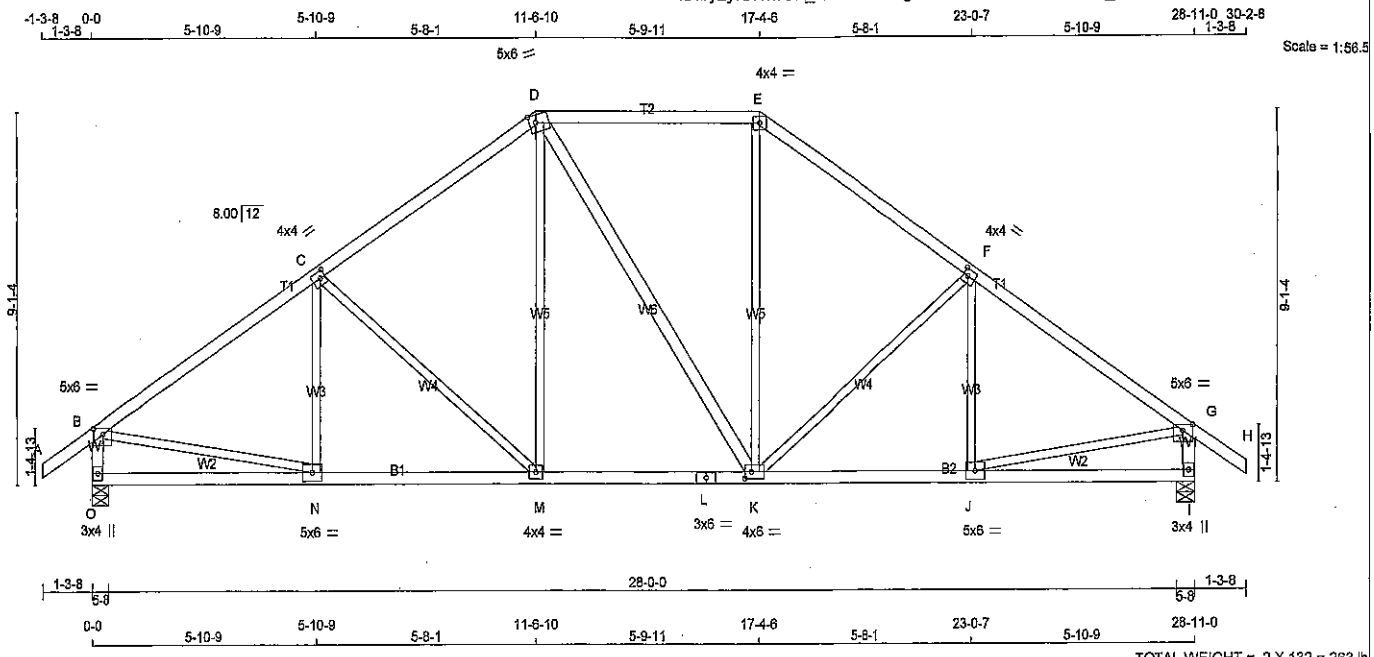

TOTAL WEIGHT = 2 X 132 = 263 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-I	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMWW-p	MT20	5.0	6.0	1.75	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2066	0	2066	0	0	5-8	5-8
I	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
I	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2245 / 0	-104.9	-104.9	0.57 (1)	3.98
C- D	-1868 / 0	-104.9	-104.9	0.52 (1)	4.35
D- E	-1525 / 0	-104.9	-104.9	0.49 (1)	4.72
E- F	-1869 / 0	-104.9	-104.9	0.52 (1)	4.35
F- G	-2245 / 0	-104.9	-104.9	0.57 (1)	3.98
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- B	-1998 / 0	0.0	0.0	0.21 (1)	5.97
I- G	-1997 / 0	0.0	0.0	0.21 (1)	5.97
O- N	0 / 0	-28.0	-28.0	0.23 (3)	10.00
N- M	0 / 1901	-28.0	-28.0	0.43 (2)	10.00
M- L	0 / 1524	-28.0	-28.0	0.34 (1)	10.00
L- K	0 / 1524	-28.0	-28.0	0.34 (1)	10.00
K- J	0 / 1901	-28.0	-28.0	0.43 (2)	10.00
J- I	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-169 / 175	0.07 (1)
C- M	-506 / 0	0.60 (1)
M- D	0 / 487	0.11 (1)
D- K	0 / 1	0.00 (1)
K- E	0 / 488	0.11 (1)
K- F	-505 / 0	0.59 (1)
J- F	-171 / 174	0.07 (1)
B- N	0 / 1933	0.43 (1)
J- G	0 / 1933	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.43/1.00 (M-N:2) , WB=0.60/1.00 (C-M:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.47 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1113B-M
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T38	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:28 2018 Page 1

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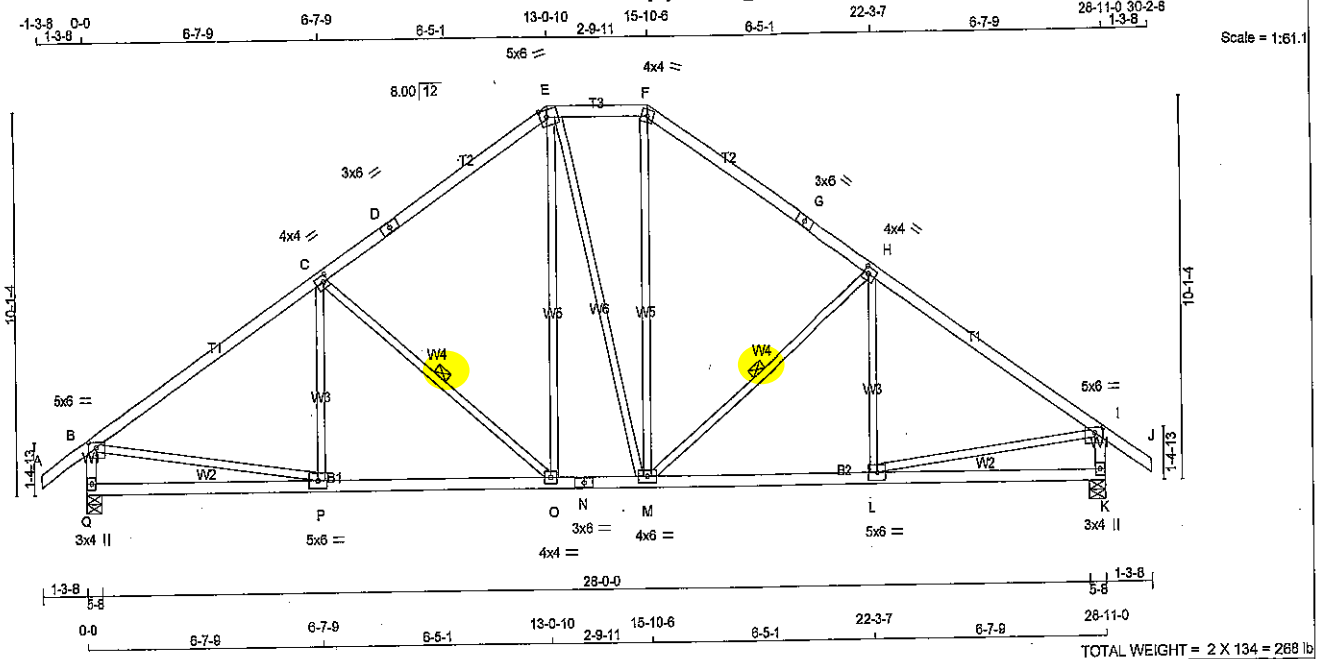

TOTAL WEIGHT = 2 X 134 = 268 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.75	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2068	0	2068	0	0	5-8	5-8
K	2056	0	2056	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
K	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2246 / 0	-104.9	-104.9	0.75 (1)	3.70
C- D	-1737 / 0	-104.9	-104.9	0.66 (1)	4.22
D- E	-1737 / 0	-104.9	-104.9	0.66 (1)	4.22
E- F	-1412 / 0	-104.9	-104.9	0.13 (1)	5.37
F- G	-1739 / 0	-104.9	-104.9	0.66 (1)	4.22
G- H	-1739 / 0	-104.9	-104.9	0.66 (1)	4.22
H- I	-2245 / 0	-104.9	-104.9	0.75 (1)	3.71
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1991 / 0	0.0	0.0	0.21 (1)	5.98
K- I	-1991 / 0	0.0	0.0	0.21 (1)	5.98
Q- P	0 / 0	-28.0	-28.0	0.32 (3)	10.00
P- O	0 / 1907	-28.0	-28.0	0.52 (2)	10.00
O- N	0 / 1410	-28.0	-28.0	0.32 (1)	10.00
N- M	0 / 1410	-28.0	-28.0	0.32 (1)	10.00
M- L	0 / 1906	-28.0	-28.0	0.51 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-97 / 247	0.06 (3)
C- O	-657 / 0	0.33 (1)
O- E	0 / 534	0.12 (1)
E- M	0 / 7	0.00 (1)
M- F	0 / 541	0.12 (1)
M- H	-653 / 0	0.33 (1)
L- H	-101 / 245	0.06 (3)
B- P	0 / 1932	0.43 (1)
L- I	0 / 1931	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.75/1.00 (B-C:1) , BC=0.52/1.00 (O-P:2) , WB=0.43/1.00 (B-P:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.46 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11139-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T39	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:28 2018 Page 1

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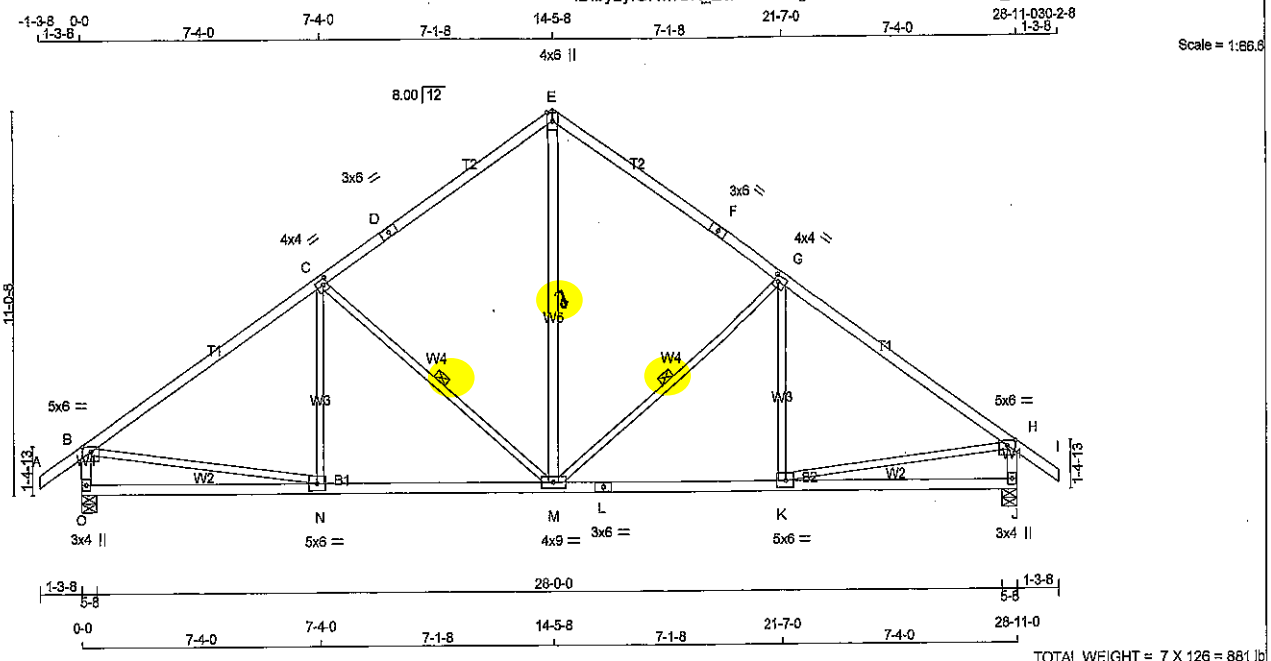

TOTAL WEIGHT = 7 X 126 = 881 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2066	0	2066	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2231 / 0	-104.9	-104.9	0.94 (1)	3.31
C- D	-1622 / 0	-104.9	-104.9	0.82 (1)	3.97
D- E	-1622 / 0	-104.9	-104.9	0.82 (1)	3.97
E- F	-1622 / 0	-104.9	-104.9	0.82 (1)	3.97
F- G	-1622 / 0	-104.9	-104.9	0.82 (1)	3.97
G- H	-2231 / 0	-104.9	-104.9	0.94 (1)	3.31
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- B	-1982 / 0	0.0	0.0	0.20 (1)	5.99
J- H	-1982 / 0	0.0	0.0	0.20 (1)	5.99
O- N	0 / 0	-28.0	-28.0	0.37 (3)	10.00
N- M	0 / 1898	-28.0	-28.0	0.56 (2)	10.00
M- L	0 / 1898	-28.0	-28.0	0.56 (2)	10.00
L- K	0 / 1898	-28.0	-28.0	0.56 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.37 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 1182	0.19 (1)
M- G	-772 / 0	0.46 (1)
K- G	-53 / 286	0.06 (3)
C- M	-772 / 0	0.46 (1)
N- C	-53 / 286	0.06 (3)
B- N	0 / 1919	0.43 (1)
K- H	0 / 1919	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.94/1.00 (G-H:1) , BC=0.56/1.00 (K-M:2) , WB=0.46/1.00 (G-M:1) , SSI=0.31/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.48 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11140-T39
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T40	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:28 2018 Page 1

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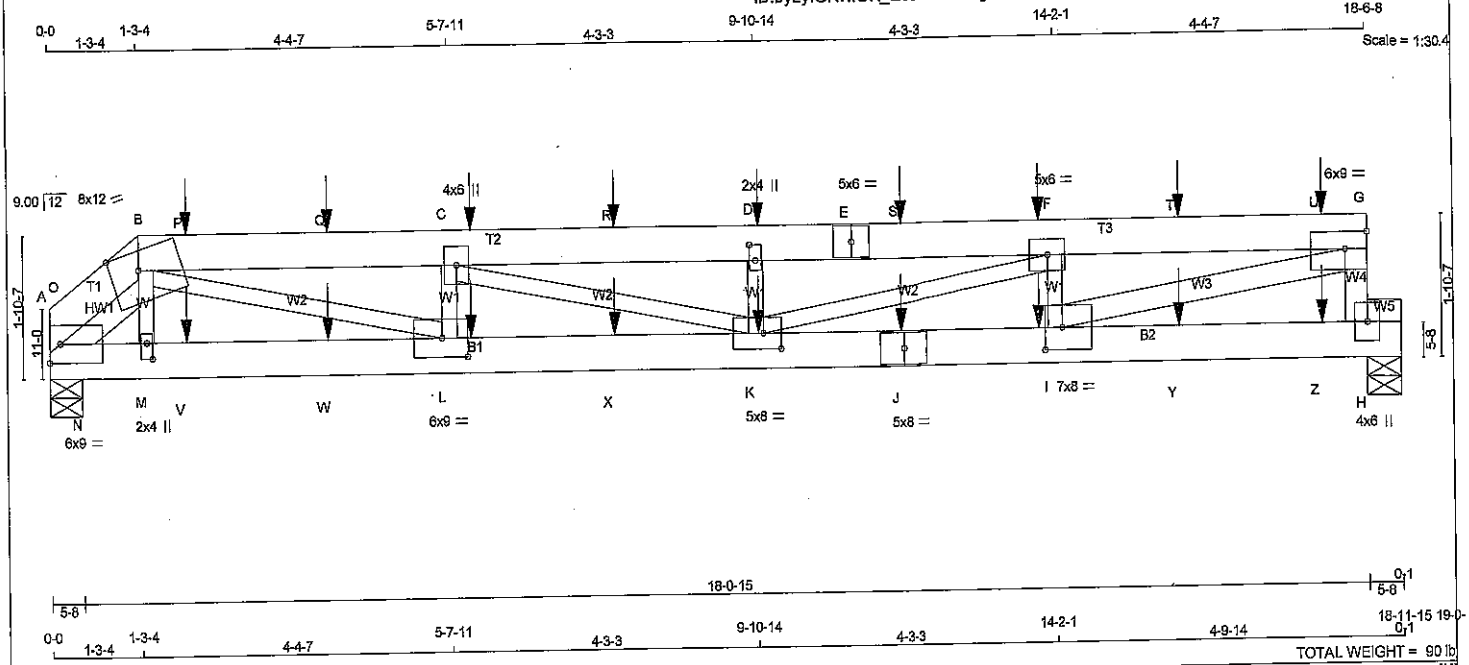


TOTAL WEIGHT = 90 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
A - J	2x6	DRY	2100F 1.8E	SPF
J - H	2x6	DRY	2100F 1.8E	SPF
REINFORCING MEMBERS				
HW1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBW1-I	MT20	6.0	9.0		Edge
B	TTWWW-m	MT20	8.0	12.0	3.00	4.75
C	TMWW+t	MT20	4.0	6.0		
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMW-t	MT20	6.0	9.0	2.75	Edge
H	BMWW1+p	MT20	4.0	6.0		
I	BMWW-t	MT20	7.0	8.0	3.50	3.00
J	BS-t	MT20	5.0	8.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	3.00
L	BMWW-t	MT20	6.0	9.0	3.00	4.25
M	BMW+w	MT20	2.0	4.0	2.50	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 130.8 lbs FACTORED DOWN AT 1-11-4, 107.9 lbs FACTORED DOWN AT 3-11-4, 107.9 lbs FACTORED DOWN AT 5-11-4, 107.9 lbs FACTORED DOWN AT 7-11-4, 107.9 lbs FACTORED DOWN AT 9-11-4, 107.9 lbs FACTORED DOWN AT 11-11-4, 107.9 lbs FACTORED DOWN AT 13-11-4, AND 107.9 lbs FACTORED DOWN AT 15-11-4, AND 128.7 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 66.5 lbs FACTORED DOWN AT 1-11-4, 66.5 lbs FACTORED DOWN AT 3-11-4, 66.5 lbs FACTORED DOWN AT 5-11-4, 66.5 lbs FACTORED DOWN AT 7-11-4, 66.5 lbs FACTORED DOWN AT 9-11-4, 66.5 lbs FACTORED DOWN AT 11-11-4, 66.5 lbs FACTORED DOWN AT 13-11-4, AND 66.5 lbs FACTORED DOWN AT 15-11-4, AND 72.1 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1983	0	1983	0	0	5-8	5-8
H	2101	0	2101	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1578	972 / 0	310 / 0	0 / 0	0 / 0	296 / 0	0 / 0
H	1674	1027 / 0	332 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- O	-3171 / 0	-104.9	-104.9	0.36 (1)	4.32
O- B	-2858 / 0	-104.9	-104.9	0.21 (1)	5.74
B- P	-5536 / 0	-104.9	-104.9	0.45 (1)	3.32
P- Q	-5536 / 0	-104.9	-104.9	0.45 (1)	3.32
Q- C	-5536 / 0	-104.9	-104.9	0.45 (1)	3.32
C- R	-6433 / 0	-104.9	-104.9	0.54 (1)	2.98
R- D	-6433 / 0	-104.9	-104.9	0.54 (1)	2.98
D- E	-6433 / 0	-104.9	-104.9	0.52 (1)	3.02
E- S	-6433 / 0	-104.9	-104.9	0.52 (1)	3.02
S- F	-6433 / 0	-104.9	-104.9	0.52 (1)	3.02
F- T	-4841 / 0	-104.9	-104.9	0.39 (1)	3.60
T- U	-4841 / 0	-104.9	-104.9	0.39 (1)	3.60
U- G	-4841 / 0	-104.9	-104.9	0.39 (1)	3.60
H- G	-1920 / 0	0.0	0.0	0.22 (1)	6.02
A- N	0 / 2086	-28.0	-28.0	0.20 (1)	10.00
N- M	0 / 2086	-28.0	-28.0	0.25 (1)	10.00
M- V	0 / 2086	-28.0	-28.0	0.25 (1)	10.00
V- W	0 / 2086	-28.0	-28.0	0.25 (1)	10.00
W- L	0 / 2086	-28.0	-28.0	0.25 (1)	10.00
L- X	0 / 5536	-28.0	-28.0	0.45 (1)	10.00
X- K	0 / 5536	-28.0	-28.0	0.45 (1)	10.00
K- J	0 / 4841	-28.0	-28.0	0.39 (1)	10.00
J- I	0 / 4841	-28.0	-28.0	0.39 (1)	10.00
I- Y	0 / 0	-28.0	-28.0	0.09 (1)	10.00
Y- Z	0 / 0	-28.0	-28.0	0.09 (1)	10.00
Z- H	0 / 0	-28.0	-28.0	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	0 / 435	0.11 (1)
B- L	0 / 3634	0.90 (1)
L- C	-1030 / 0	0.16 (1)
C- K	0 / 944	0.23 (1)
K- D	-591 / 0	0.09 (1)
K- F	0 / 1676	0.41 (1)
I- F	-1295 / 0	0.20 (1)
I- G	0 / 5088	0.90 (1)
N- O	0 / 372	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TOTAL
C	5-11-4	-108	-108	---	BACK	VERT	TOTAL
D	9-11-4	-108	-108	---	BACK	VERT	TOTAL
F	13-11-4	-108	-108	---	BACK	VERT	TOTAL
I	13-11-4	-67	-67	---	BACK	VERT	TOTAL
J	11-11-4	-67	-67	---	BACK	VERT	TOTAL
K	9-11-4	-67	-67	---	BACK	VERT	TOTAL
L	5-11-4	-67	-67	---	BACK	VERT	TOTAL
P	1-11-4	-131	-131	---	BACK	VERT	TOTAL
Q	3-11-4	-108	-108	---	BACK	VERT	TOTAL
R	7-11-4	-108	-108	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 750 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 484 (0.46")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.45/1.00 (K-L:1) , WB=0.90/1.00 (B-L:1) , SSI=0.32/1.00 (A-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

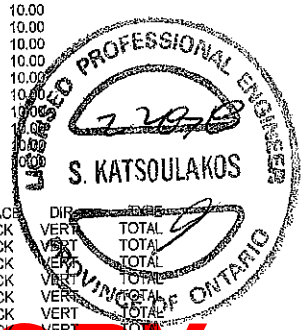
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.99 (J) (INPUT = 1.00)



DWG NO. TAM 11141 -B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T40	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	11-11-4	-108	-108	---	BACK	VERT	TOTAL
T	15-11-4	-108	-108	---	BACK	VERT	TOTAL
U	17-11-4	-129	-129	---	BACK	VERT	TOTAL
V	1-11-4	-67	-67	---	BACK	VERT	TOTAL
W	3-11-4	-67	-67	---	BACK	VERT	TOTAL
X	7-11-4	-67	-67	---	BACK	VERT	TOTAL
Y	15-11-4	-67	-67	---	BACK	VERT	TOTAL
Z	17-11-4	-72	-72	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 111 41
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287526	T100	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:07:41 2018 Page 1
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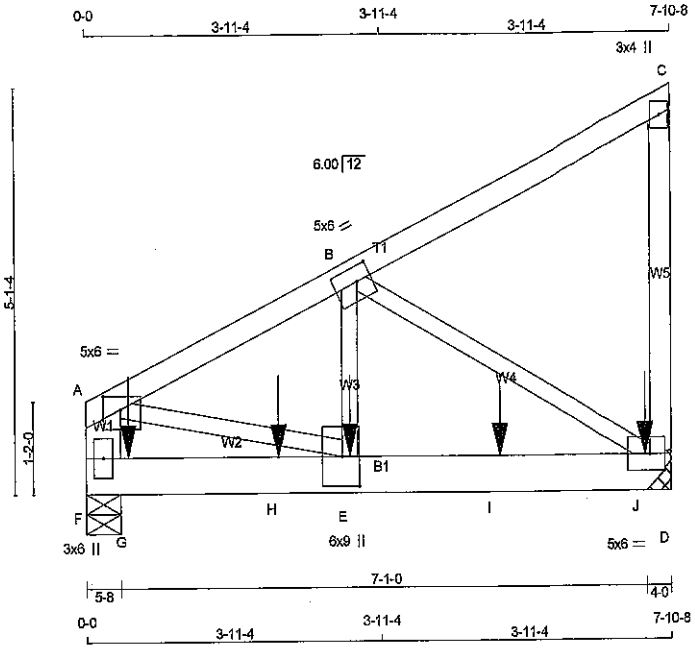

Scale = 1:29.1

TOTAL WEIGHT = 2 X 38 = 77 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	2	SIDE(41.3)
A- C	1	12	TOP
C- D	1	7	SIDE(309.7)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(440.0)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.00	2.75
B	TMWW-t	MT20	5.0	6.0	2.25	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0	2.50	2.50
E	BMWW+t	MT20	6.0	9.0	4.50	2.75
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 860.1 lbs FACTORED DOWN AT 6-12, 854.0 lbs FACTORED DOWN AT 2-6-12, 2368.4 lbs FACTORED DOWN AT 3-6-4, AND 1646.0 lbs FACTORED DOWN AT 5-6-4, AND 1655.1 lbs FACTORED DOWN AT 7-6-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	3785	0	3785	0	0	5-8	5-8
D	4665	0	4665	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2930	1943 / 0	485 / 0	0 / 0	0 / 0	503 / 0	0 / 0
D	3636	2369 / 0	629 / 0	0 / 0	0 / 0	638 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
F- A	-3029 / 0	0.0	0.0	1	0.11 (1)	7.81	A- E	0 / 4099	0.51 (1)
A- B	-4459 / 0	-104.9	-104.9	1	0.18 (1)	4.39	E- B	0 / 3850	0.48 (1)
B- C	-14 / 1	-104.9	-104.9	1	0.14 (1)	6.25	B- D	-4632 / 0	0.92 (1)
D- C	-198 / 0	0.0	0.0	1	0.04 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	1	0.32 (1)	10.00			
G- H	0 / 0	-28.0	-28.0	1	0.32 (1)	10.00			
H- E	0 / 0	-28.0	-28.0	1	0.32 (1)	10.00			
E- I	0 / 4003	-28.0	-28.0	1	0.86 (1)	10.00			
I- J	0 / 4003	-28.0	-28.0	1	0.86 (1)	10.00			
J- D	0 / 4003	-28.0	-28.0	1	0.86 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
E	3-6-4	-2388	-2388	---	BACK	VERT	TOTAL
G	6-12	-860	-860	---	BACK	VERT	TOTAL
H	2-6-12	-854	-854	---	BACK	VERT	TOTAL
I	5-6-4	-1646	-1646	---	BACK	VERT	TOTAL
J	7-6-4	-1655	-1655	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.10")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.86/1.00 (D-E:1) , WB=0.92/1.00 (B-D:1) , SSI=0.71/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11128-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T200	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:29 2018 Page 1

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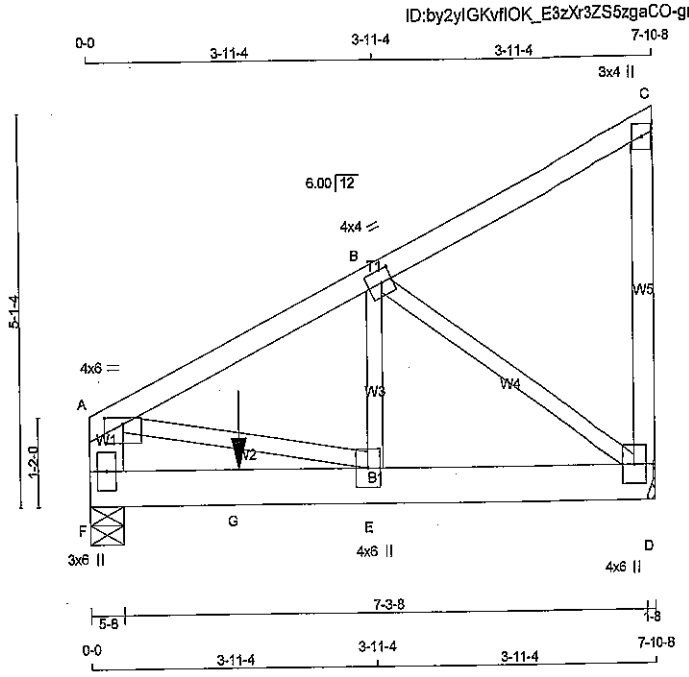

Scale = 1:30.1

TOTAL WEIGHT = 2 X 39 = 77 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x8	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMWW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1628.6 lbs FACTORED DOWN AT 2-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1730	0	1730	0	0	5-8	5-8
D	946	0	946	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1350	876 / 0	236 / 0	0 / 0	0 / 0	238 / 0	0 / 0
D	744	473 / 0	136 / 0	0 / 0	0 / 0	135 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1025 / 0	0.0	0.0	0.04 (1)	7.81	A- E	0 / 1265	0.16 (1)
A- B	-1367 / 0	-104.9	-104.9	0.15 (1)	6.25	E- B	0 / 980	0.12 (1)
B- C	-20 / 0	-104.9	-104.9	0.13 (1)	6.25	B- D	-1512 / 0	0.27 (1)
D- C	-161 / 0	0.0	0.0	0.03 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	0.51 (1)	10.00			
G- E	0 / 0	-28.0	-28.0	0.51 (1)	10.00			
E- D	0 / 1241	-28.0	-28.0	0.28 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-8	-1629	-1629	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.51/1.00 (E-F:1) , WB=0.27/1.00 (B-D:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1114-2

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T201	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:29 2018 Page 1

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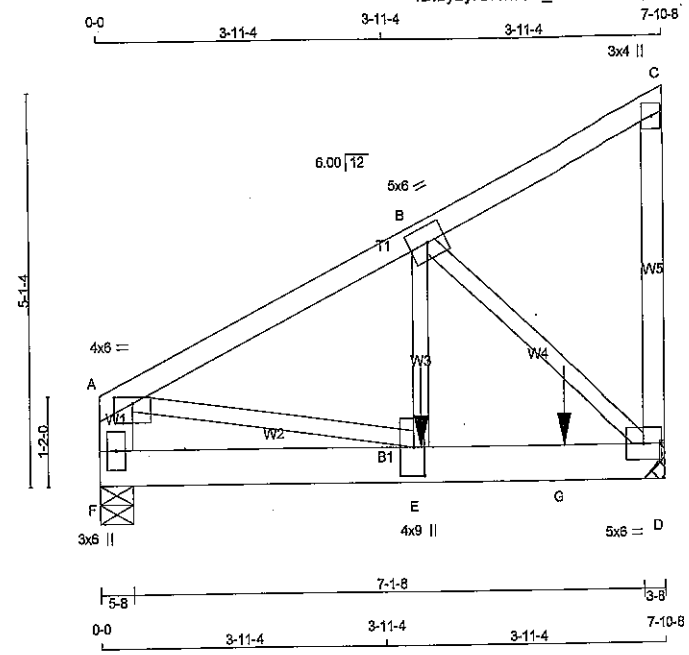

Scale = 1:30.1

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
F- A 2	12	TOP
A- C 1	12	TOP
C- D 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
F- D 2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS		
B- E 1	4	SIDE(359.4)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	5.0	6.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	9.0	4.50	1.75
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2079.5 lbs FACTORED DOWN AT 4-5-4, AND 1179.5 lbs FACTORED DOWN AT 6-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1647	0	1647	0	0	5-8	5-8
D	2659	0	2659	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1297	821 / 0	239 / 0	0 / 0	0 / 0	236 / 0	0 / 0
D	2082	1340 / 0	371 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1612 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 2134	0.26 (1)
A- B	-2333 / 0	-104.9	-104.9	0.16 (1)	5.70	E- B	0 / 2379	0.29 (1)
B- C	-23 / 0	-104.9	-104.9	0.13 (1)	6.25	B- D	-2787 / 0	0.46 (1)
D- C	-130 / 0	0.0	0.0	0.03 (1)	7.81			
F- E	0 / 0	-28.0	-28.0	0.05 (2)	10.00			
E- G	0 / 2102	-28.0	-28.0	0.50 (1)	10.00			
G- D	0 / 2102	-28.0	-28.0	0.50 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	4-5-4	-2080	-2080	—	FRONT	VERT	TOTAL
G	6-5-4	-1179	-1179	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.50/1.00 (D-E:1) , WB=0.46/1.00 (B-D:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.36 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM11143-48
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287533	T201Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:15:33 2018 Page 1
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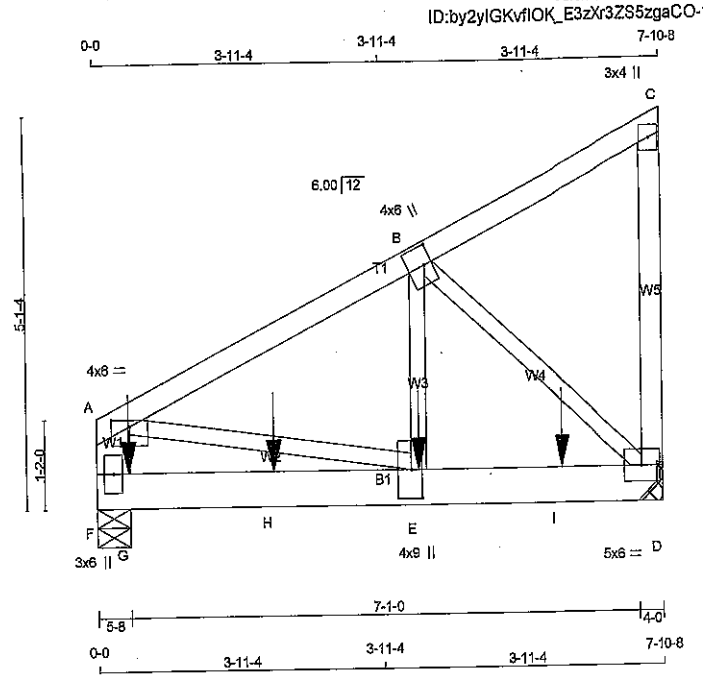

TOTAL WEIGHT = 2 X 39 = 78 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	3	SIDE(420.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMVW+t	MT20	4.0	6.0	3.00	1.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMVW+t	MT20	4.0	9.0	4.50	1.75
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 302.9 lbs FACTORED DOWN AT 5-4, 341.6 lbs FACTORED DOWN AT 2-5-4, AND 2315.3 lbs FACTORED DOWN AT 4-5-4, AND 1179.5 lbs FACTORED DOWN AT 6-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	2271	0	2271	0	0	5-8	5-8
D	2915	0	2915	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1731	1196 / 0	254 / 0	0 / 0	0 / 0	281 / 0	0 / 0
D	2263	1489 / 0	381 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-1845 / 0	0.0	0.0	0.06 (1)	7.81
A- B	-2718 / 0	-104.9	-104.9	0.18 (1)	5.36
B- C	-22 / 0	-104.9	-104.9	0.13 (1)	6.25
D- C	-132 / 0	0.0	0.0	0.03 (1)	7.81
F- G	0 / 0	-28.0	-28.0	0.13 (1)	10.00
G- H	0 / 0	-28.0	-28.0	0.13 (1)	10.00
H- E	0 / 0	-28.0	-28.0	0.13 (1)	10.00
E- I	0 / 2447	-28.0	-28.0	0.49 (1)	10.00
I- D	0 / 2447	-28.0	-28.0	0.49 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 2484	0.31 (1)
E- B	0 / 2654	0.35 (1)
B- D	-3244 / 0	0.53 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	4-5-4	-2315	-2315	---	FRONT	VERT	TOTAL
G	5-4	-303	-303	---	FRONT	VERT	TOTAL
H	2-5-4	-342	-342	---	FRONT	VERT	TOTAL
I	6-5-4	-1179	-1179	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.49/1.00 (D-E:1) , WB=0.53/1.00 (B-D:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.43 (E) (INPUT = 1.00)

DWG NO. TAM 11150 -R
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T202	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:29 2018 Page 1
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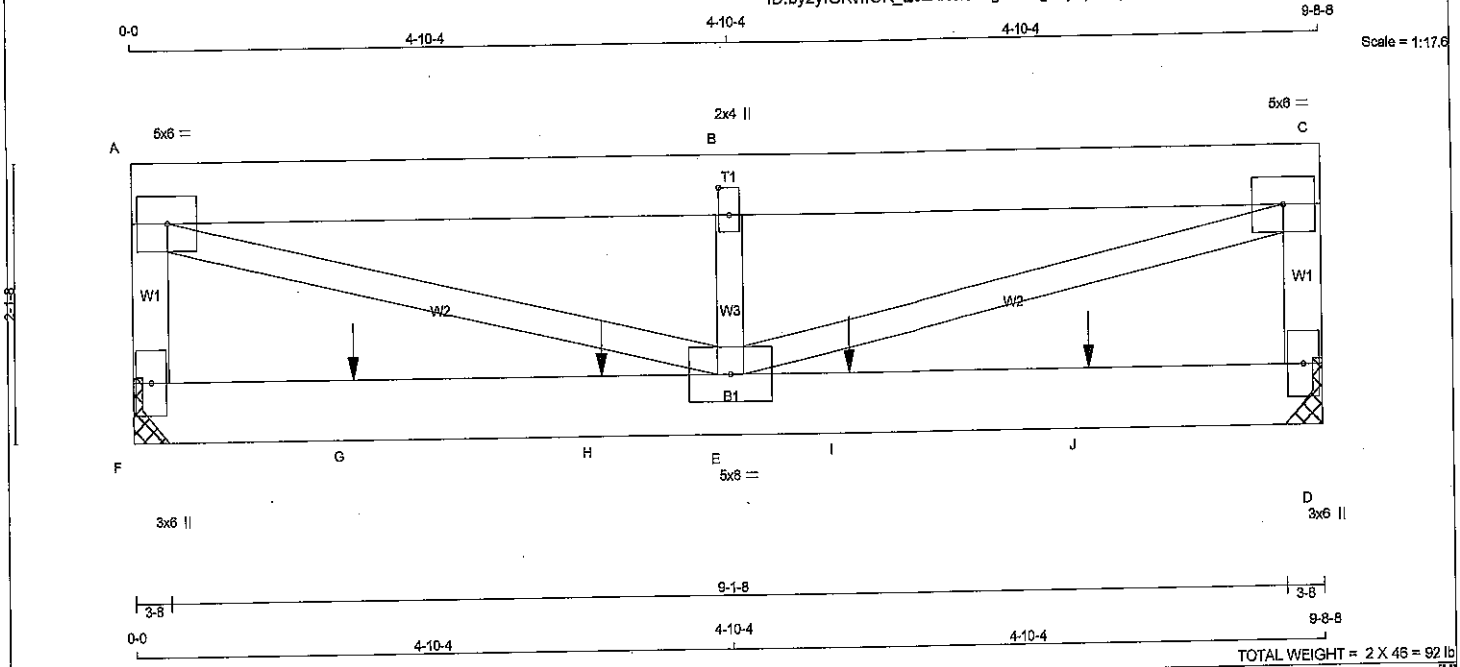

TOTAL WEIGHT = 2 X 46 = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	1	12	TOP
C- D	1	12	TOP
A- C	2	12	SIDE(31.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMVW-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 351.4 lbs FACTORED DOWN AT 1-9-12, 351.4 lbs FACTORED DOWN AT 3-9-12, AND 351.4 lbs FACTORED DOWN AT 5-9-12, AND 351.4 lbs FACTORED DOWN AT 7-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1657	0	1657	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
D	1644	0	1644	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1295	837 / 0	228 / 0	0 / 0	0 / 0	230 / 0	0 / 0
D	1286	831 / 0	227 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1351 / 0	0.0	0.0	0.08 (1)	7.81	A- E	0 / 3012	0.37 (1)
A- B	-2849 / 0	-167.2	-167.2	0.17 (1)	6.25	E- B	-879 / 0	0.07 (1)
B- C	-2849 / 0	-167.2	-167.2	0.17 (1)	6.25	E- C	0 / 3012	0.37 (1)
D- C	-1351 / 0	0.0	0.0	0.08 (1)	7.81			
F- G	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
G- H	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
H- E	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
E- I	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
I- J	0 / 0	-28.0	-28.0	0.19 (1)	10.00			
J- D	0 / 0	-28.0	-28.0	0.19 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-9-12	-351	-351	—	FRONT	VERT	TOTAL
H	3-9-12	-351	-351	—	FRONT	VERT	TOTAL
I	5-9-12	-351	-351	—	FRONT	VERT	TOTAL
J	7-9-12	-351	-351	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 4-0-0
END DISTANCE = 9-8-8
END SPAN CARRIED = 4-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.19/1.00 (E-F:1) , WB=0.37/1.00 (A-E:1) , SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (E) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 11144 -R1

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T203	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2016 MiTek Industries, Inc. Thu Mar 1 09:13:29 2018 Page 1
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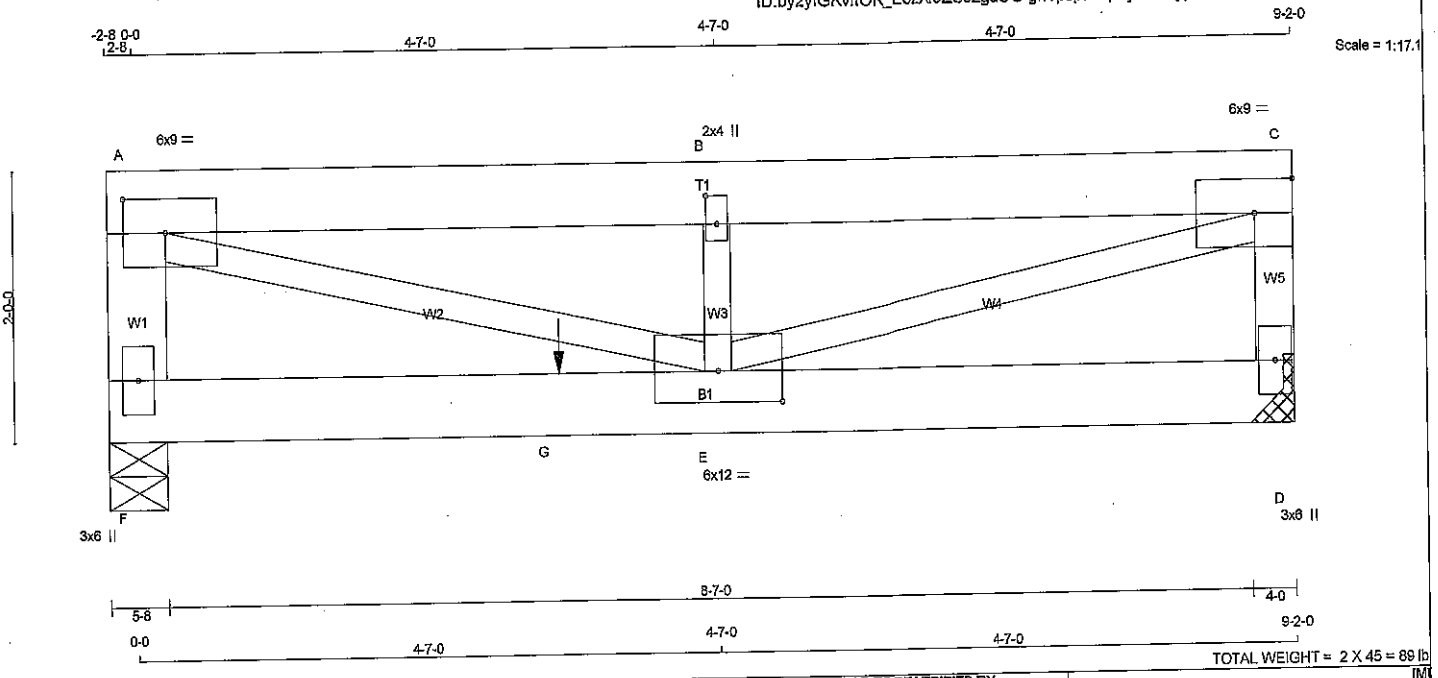

TOTAL WEIGHT = 2 X 45 = 89 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(117.0)
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(180.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	4.00
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMWW-t	MT20	6.0	9.0		Edge
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	6.0	12.0	2.75	6.00
F	BMW1+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1616.5 lbs FACTORED DOWN AT 3-4-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	3916	0	3916	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
F	4362	0	4362	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	3108	1928 / 0	601 / 0	0 / 0	0 / 0	579 / 0	0 / 0
F	3452	2157 / 0	658 / 0	0 / 0	0 / 0	638 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A- B	-7170 / 0	-338.9	-338.9	0.40 (1)	4.06	
B- C	-7170 / 0	-338.9	-338.9	0.41 (1)	4.06	
D- C	-3189 / 0	0.0	0.0	0.18 (1)	6.50	
F- G	0 / 0	-389.4	-389.4	0.64 (1)	10.00	
G- E	0 / 0	-389.4	-389.4	0.64 (1)	10.00	
E- D	0 / 0	-389.4	-389.4	0.28 (1)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
A- E	0 / 7568	0.94 (1)
E- B	-1549 / 0	0.12 (1)
E- C	0 / 7565	0.94 (1)
F- A	-3197 / 0	0.11 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
G	3-4-0	-1616	-1616	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 2-8
START SPAN CARRIED = 13-0-0
END DISTANCE = 9-2-0
END SPAN CARRIED = 13-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 2-8
START SPAN CARRIED = 9-4-8
END DISTANCE = 9-2-0
END SPAN CARRIED = 9-4-8
END WALL WIDTH = 4-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF . PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 870 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 563 (0.19")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.64/1.00 (E-F:1) , WB=0.94/1.00 (A-E:1) , SSI=0.74/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

DWG NO. TAM 11145-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	T203	1	2	TRUSS DESC.	

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (A) (INPUT = 0.90)
JSI METAL= 0.60 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11145-R1
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB1	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:19 2018 Page 1

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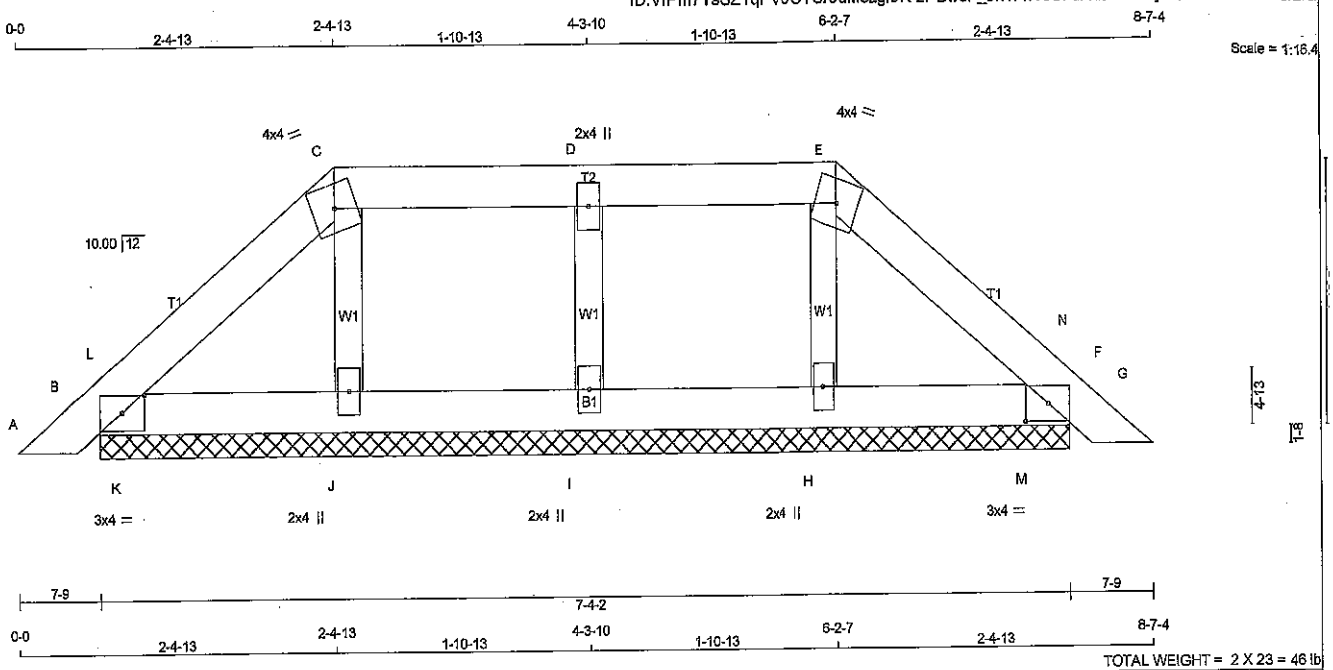

TOTAL WEIGHT = 2 X 23 = 46 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H, I, J						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	201	0	201	0	0	7-4-2	7-4-2
F	201	0	201	0	0	7-4-2	7-4-2
J	203	0	203	0	0	7-4-2	7-4-2
H	203	0	203	0	0	7-4-2	7-4-2
I	280	0	280	0	0	7-4-2	7-4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	149	110 / 0	17 / 0	0 / 0	0 / 0	22 / 0	0 / 0
F	149	110 / 0	17 / 0	0 / 0	0 / 0	22 / 0	0 / 0
J	169	91 / 0	42 / 0	0 / 0	0 / 0	36 / 0	0 / 0
H	169	91 / 0	42 / 0	0 / 0	0 / 0	36 / 0	0 / 0
I	217	144 / 0	36 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, H, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- L	-43 / 0	-104.9	-104.9	0.01 (3)	6.25	
L- C	-54 / 0	-104.9	-104.9	0.04 (1)	6.25	
C- D	-31 / 0	-104.9	-104.9	0.06 (1)	6.25	
D- E	-31 / 0	-104.9	-104.9	0.06 (1)	6.25	
E- N	-54 / 0	-104.9	-104.9	0.04 (1)	6.25	
N- F	-43 / 0	-104.9	-104.9	0.01 (3)	6.25	
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- K	0 / 39	-28.0	-28.0	0.04 (1)	10.00	
K- J	0 / 39	-28.0	-28.0	0.04 (1)	10.00	
J- I	0 / 31	-28.0	-28.0	0.03 (1)	10.00	
I- H	0 / 31	-28.0	-28.0	0.03 (1)	10.00	
H- M	0 / 39	-28.0	-28.0	0.04 (1)	10.00	
M- F	0 / 39	-28.0	-28.0	0.04 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-120 / 0	0.02 (1)
H- E	-120 / 0	0.02 (1)
I- D	-247 / 0	0.04 (1)
K- L	-107 / 22	0.00 (1)
M- N	-107 / 22	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-D:1) , BC=0.04/1.00 (F-M:1) , WB=0.04/1.00 (D-I:1) , SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

DWG NO. TAM 11114 - PB

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:19 2018 Page 1
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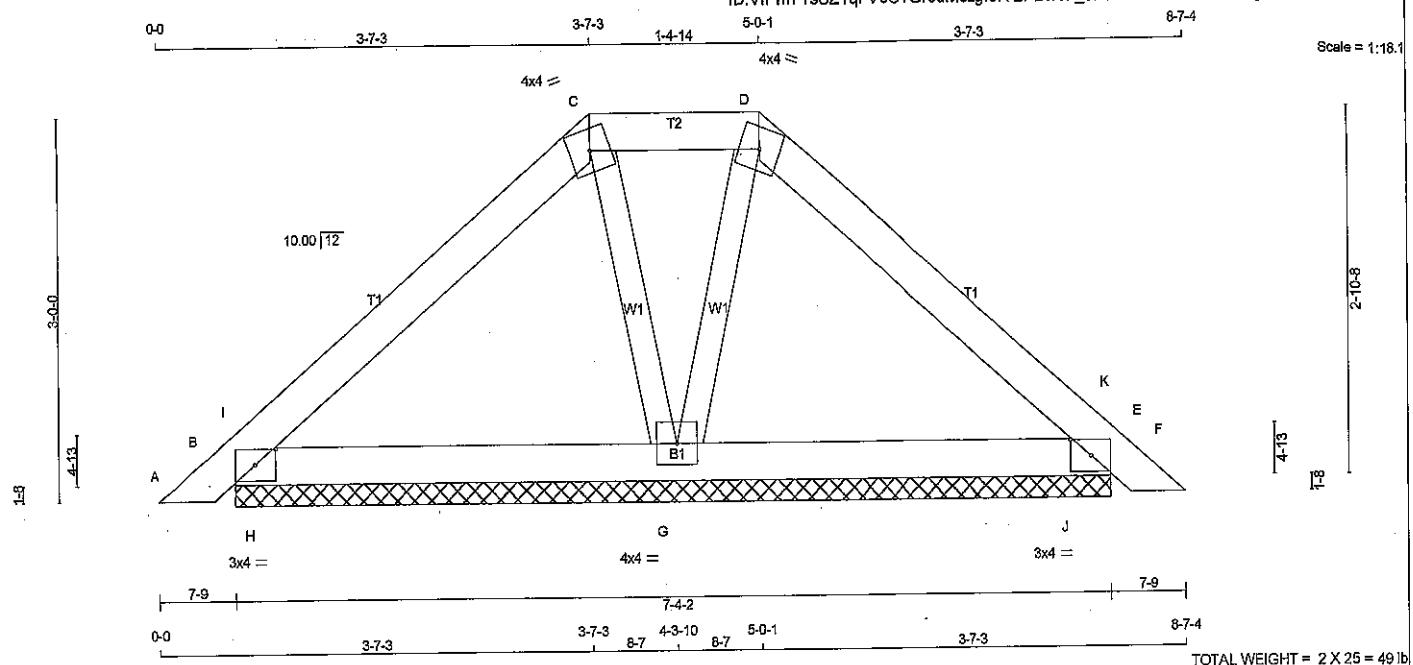

TOTAL WEIGHT = 2 X 25 = 49 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-i	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	332	0	332	0	0	7-4-2	7-4-2
E	332	0	332	0	0	7-4-2	7-4-2
G	425	0	425	0	0	7-4-2	7-4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	253	175 / 0	37 / 0	0 / 0	0 / 0	41 / 0	0 / 0
E	253	175 / 0	37 / 0	0 / 0	0 / 0	41 / 0	0 / 0
G	349	196 / 0	81 / 0	0 / 0	0 / 0	72 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	C- G	-126 / 0	0.02 (1)
B- I	-104 / 0	-104.9	-104.9	0.04 (1)	6.25	G- D	-128 / 0	0.02 (1)
I- C	-136 / 0	-104.9	-104.9	0.12 (1)	6.25	H- I	-246 / 88	0.00 (1)
C- D	-65 / 0	-104.9	-104.9	0.04 (1)	6.25	J- K	-246 / 88	0.00 (1)
D- K	-136 / 0	-104.9	-104.9	0.12 (1)	6.25			
K- E	-104 / 0	-104.9	-104.9	0.04 (1)	6.25			
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00			
B- H	0 / 99	-28.0	-28.0	0.10 (1)	10.00			
H- G	0 / 99	-28.0	-28.0	0.12 (2)	10.00			
G- J	0 / 99	-28.0	-28.0	0.12 (2)	10.00			
J- E	0 / 99	-28.0	-28.0	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-K:1) , BC=0.12/1.00 (G-J:2) , WB=0.02/1.00 (D-G:1) , SSI=0.20/1.00 (E-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11115-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:19 2018 Page 1
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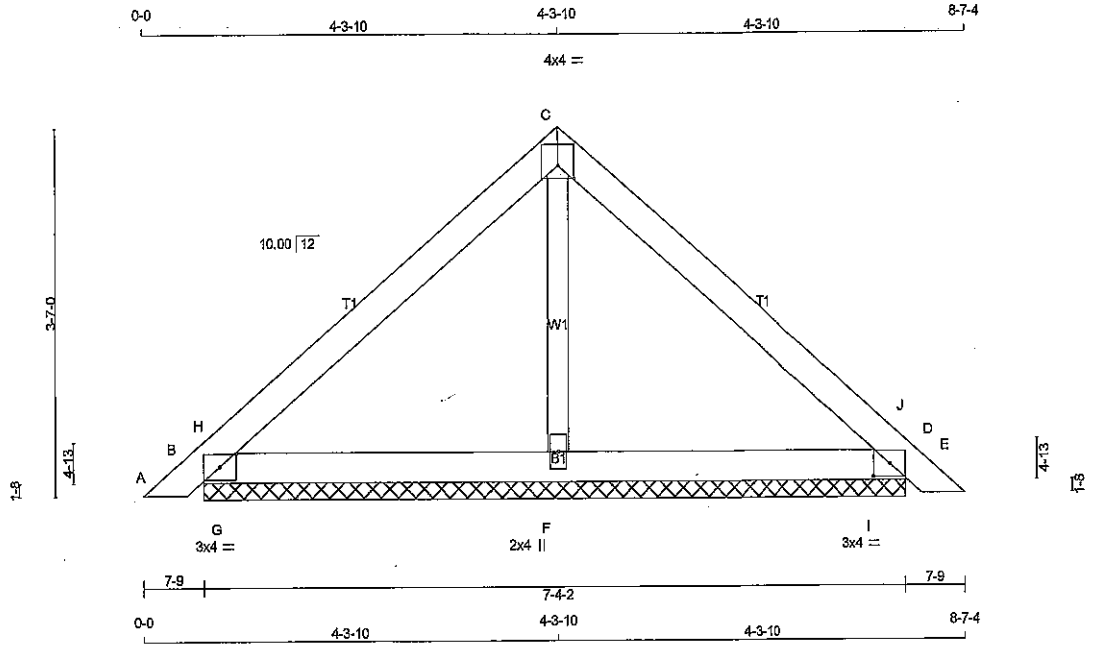

TOTAL WEIGHT = 23 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	379	0	379	0	0	7-4-2	7-4-2
D	379	0	379	0	0	7-4-2	7-4-2
F	331	0	331	0	0	7-4-2	7-4-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	284	204 / 0	37 / 0	0 / 0	0 / 0	43 / 0	0 / 0
D	284	204 / 0	37 / 0	0 / 0	0 / 0	43 / 0	0 / 0
F	285	138 / 0	81 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	F- C	-113 / 9	0.02 (1)
B- H	-84 / 14	-104.9	-104.9	0.09 (1)	6.25	G- H	-448 / 59	0.00 (1)
H- C	-175 / 0	-104.9	-104.9	0.17 (1)	6.25	I- J	-448 / 59	0.00 (1)
C- J	-175 / 0	-104.9	-104.9	0.17 (1)	6.25			
J- D	-84 / 14	-104.9	-104.9	0.09 (1)	6.25			
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00			
B- G	0 / 125	-28.0	-28.0	0.17 (1)	10.00			
G- F	0 / 125	-28.0	-28.0	0.17 (1)	10.00			
F- I	0 / 125	-28.0	-28.0	0.17 (1)	10.00			
I- D	0 / 125	-28.0	-28.0	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (C-J:1) , BC=0.17/1.00 (F-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.34/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1116
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:20 2018 Page 1
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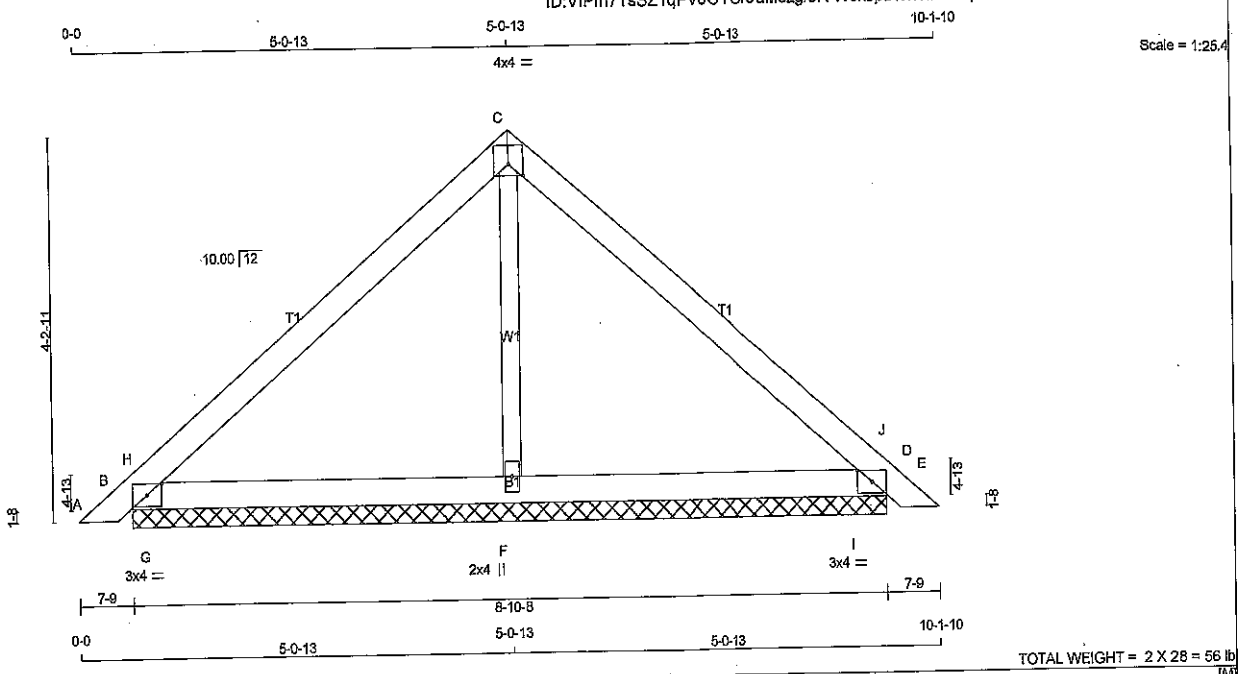

TOTAL WEIGHT = 2 X 28 = 56 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	460	0	460	0	0	8-10-8	8-10-8
D	460	0	460	0	0	8-10-8	8-10-8
F	373	0	373	0	0	8-10-8	8-10-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	346	247 / 0	46 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	346	247 / 0	46 / 0	0 / 0	0 / 0	53 / 0	0 / 0
F	325	153 / 0	95 / 0	0 / 0	0 / 0	77 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- H	-119 / 69	-104.9	-104.9	0.15 (1)	6.25
H- C	-233 / 0	-104.9	-104.9	0.25 (1)	6.25
C- J	-233 / 0	-104.9	-104.9	0.25 (1)	6.25
J- D	-119 / 69	-104.9	-104.9	0.15 (1)	6.25
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- G	0 / 167	-28.0	-28.0	0.24 (1)	10.00
G- F	0 / 167	-28.0	-28.0	0.24 (1)	10.00
F- I	0 / 167	-28.0	-28.0	0.24 (1)	10.00
I- D	0 / 167	-28.0	-28.0	0.24 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-112 / 19	0.03 (1)
G- H	-642 / 86	0.00 (1)
I- J	-642 / 86	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-H:1) , BC=0.24/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.50/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11117-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB4Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:20 2018 Page 1
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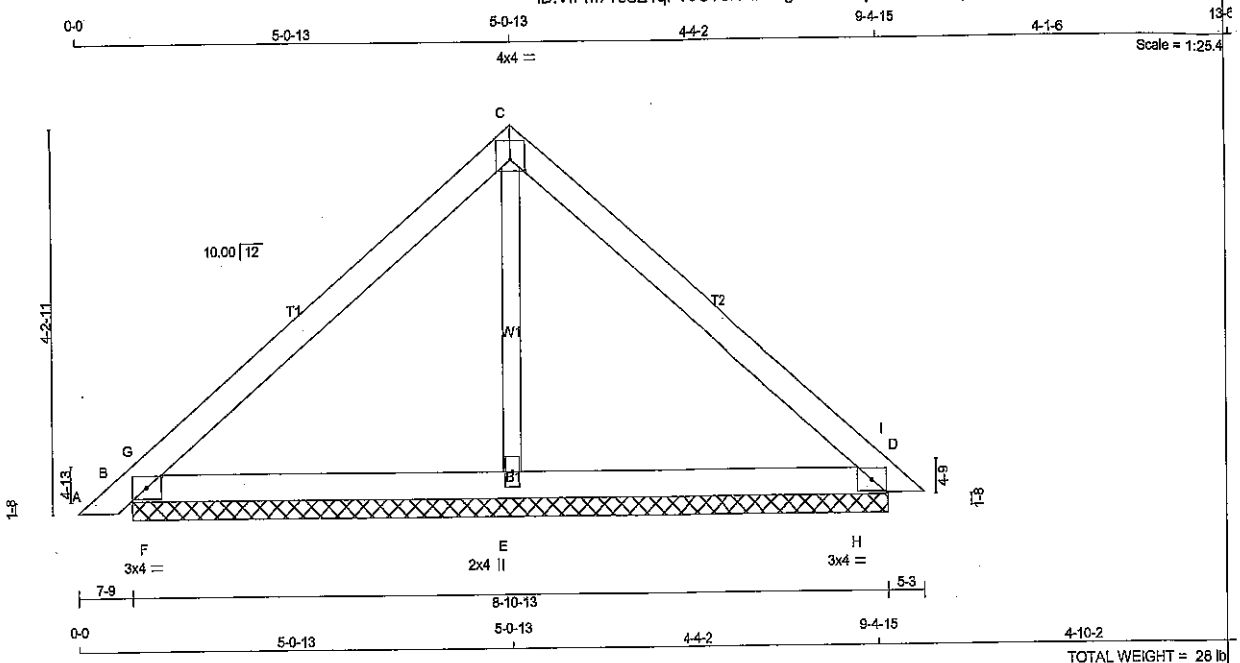

TOTAL WEIGHT = 28 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0		
E	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	448	0	448	0	0	8-10-13	8-10-13
D	393	0	393	0	0	8-10-13	8-10-13
E	398	0	398	0	0	8-10-13	8-10-13

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	337	241 / 0	45 / 0	0 / 0	0 / 0	52 / 0	0 / 0
D	300	208 / 0	45 / 0	0 / 0	0 / 0	49 / 0	0 / 0
E	344	166 / 0	98 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	E- C	-135 / 12		0.03 (1)
B- G	-112 / 89	-104.9	-104.9	0.16 (1)	6.25	F- G	-646 / 84		0.00 (1)
G- C	-216 / 0	-104.9	-104.9	0.25 (1)	6.25	H- I	-819 / 80		0.00 (1)
C- I	-216 / 0	-104.9	-104.9	0.25 (1)	6.25				
I- D	-106 / 56	-104.9	-104.9	0.15 (1)	6.25				
B- F	0 / 154	-28.0	-28.0	0.24 (1)	10.00				
F- E	0 / 154	-28.0	-28.0	0.24 (1)	10.00				
E- H	0 / 154	-28.0	-28.0	0.24 (1)	10.00				
H- D	0 / 154	-28.0	-28.0	0.24 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-I:1) , BC=0.24/1.00 (D-H:1) , WB=0.03/1.00 (C-E:1) , SSI=0.50/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11118-17
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB6	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:20 2018 Page 1

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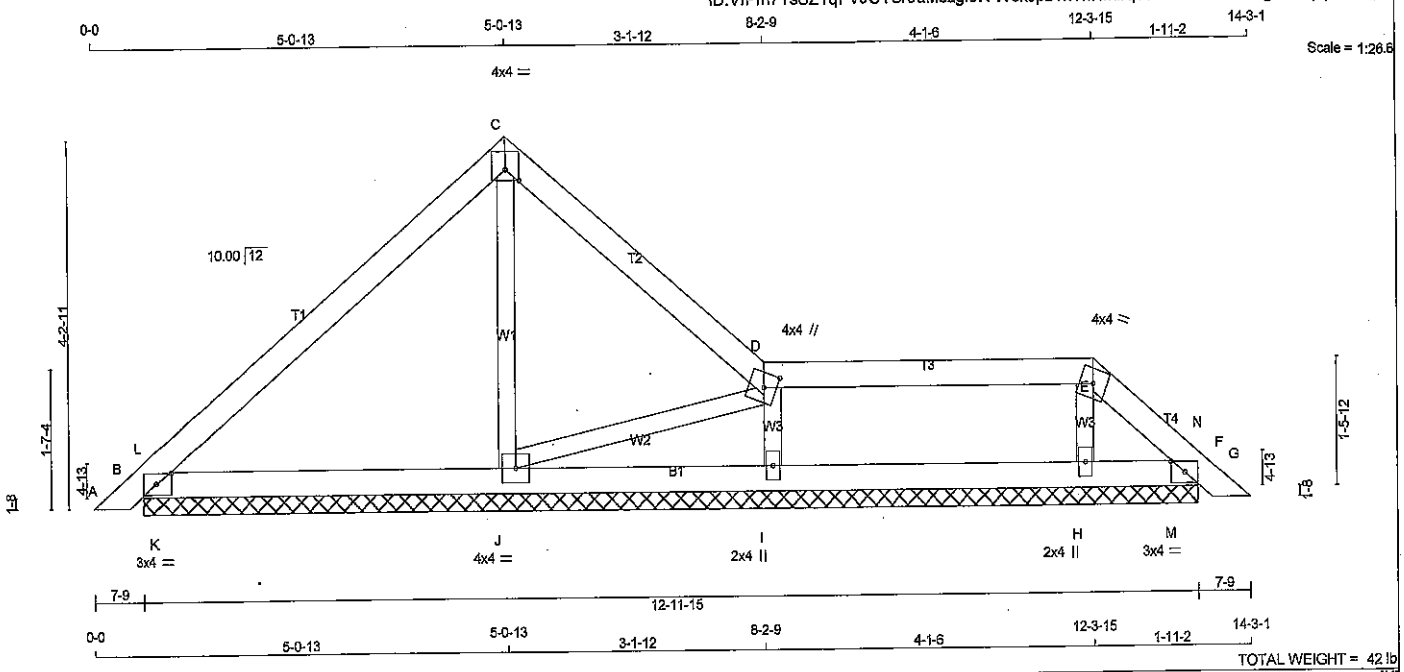

TOTAL WEIGHT = 42 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TTWW+m	MT20	4.0	4.0	2.00	1.75
E	TTW-m	MT20	4.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMW1+w	MT20	2.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	429	0	429	0	0	12-11-15	12-11-15
F	167	0	167	0	0	12-11-15	12-11-15
J	343	0	343	0	0	12-11-15	12-11-15
I	559	0	559	0	0	12-11-15	12-11-15
H	341	0	341	0	0	12-11-15	12-11-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	325	228 / 0	45 / 0	0 / 0	0 / 0	51 / 0	0 / 0
F	112	105 / 0	0 / -2	0 / 0	0 / 0	9 / 0	0 / 0
J	291	148 / 0	78 / 0	0 / 0	0 / 0	66 / 0	0 / 0
I	438	282 / 0	78 / 0	0 / 0	0 / 0	78 / 0	0 / 0
H	286	151 / 0	73 / 0	0 / 0	0 / 0	62 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- L	-122 / 104	-104.9	-104.9	0.14 (1)	6.25
L- C	-166 / 0	-104.9	-104.9	0.26 (1)	6.25
C- D	-151 / 0	-104.9	-104.9	0.18 (1)	6.25
D- E	-37 / 0	-104.9	-104.9	0.30 (1)	6.25
E- N	-79 / 0	-104.9	-104.9	0.01 (1)	6.25
N- F	-54 / 17	-104.9	-104.9	0.01 (3)	6.25
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- K	0 / 117	-28.0	-28.0	0.22 (1)	10.00
K- J	0 / 117	-28.0	-28.0	0.22 (1)	10.00
J- I	0 / 81	-28.0	-28.0	0.14 (1)	10.00
I- H	0 / 37	-28.0	-28.0	0.08 (2)	10.00
H- M	0 / 58	-28.0	-28.0	0.08 (2)	10.00
M- F	0 / 56	-28.0	-28.0	0.03 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-170 / 5	0.04 (1)
J- D	0 / 39	0.01 (1)
I- D	-493 / 0	0.07 (1)
H- E	-215 / 0	0.03 (1)
K- L	-598 / 114	0.00 (1)
M- N	-102 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.22/1.00 (J-K:1) , WB=0.07/1.00 (D-I:1) , SSI=0.47/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (D) (INPUT = 0.90)
JSI METAL= 0.09 (I) (INPUT = 1.00)

DWG NO. TAM 11119-03
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	PB7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:20 2018 Page 1
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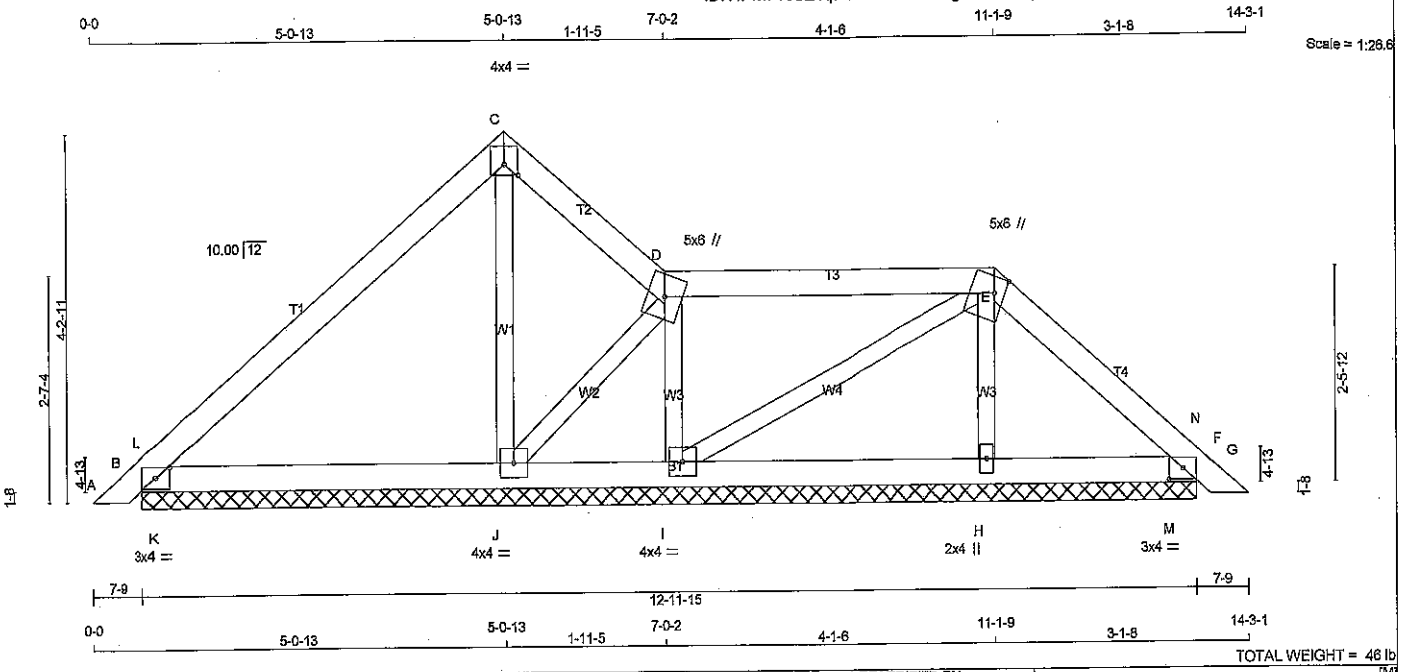


TOTAL WEIGHT = 46 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TTWW+m	MT20	5.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0	1.50	2.00
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMWW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	403	0	403	0	0	12-11-15	12-11-15
F	299	0	299	0	0	12-11-15	12-11-15
J	376	0	376	0	0	12-11-15	12-11-15
I	390	0	390	0	0	12-11-15	12-11-15
H	372	0	372	0	0	12-11-15	12-11-15

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	307	211 / 0	46 / 0	0 / 0	0 / 0	50 / 0	0 / 0
F	217	169 / 0	20 / 0	0 / 0	0 / 0	29 / 0	0 / 0
J	306	177 / 0	68 / 0	0 / 0	0 / 0	61 / 0	0 / 0
I	310	192 / 0	61 / 0	0 / 0	0 / 0	58 / 0	0 / 0
H	313	165 / 0	79 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- L	-123 / 150	-104.9	-104.9	0.15 (1)	6.25
L- C	-128 / 0	-104.9	-104.9	0.26 (1)	6.25
C- D	-113 / 0	-104.9	-104.9	0.07 (1)	6.25
D- E	-94 / 0	-104.9	-104.9	0.30 (1)	6.25
E- N	-148 / 0	-104.9	-104.9	0.08 (1)	6.25
N- F	-88 / 0	-104.9	-104.9	0.03 (1)	6.25
F- G	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- K	0 / 87	-28.0	-28.0	0.22 (1)	10.00
K- J	0 / 87	-28.0	-28.0	0.22 (1)	10.00
J- I	0 / 111	-28.0	-28.0	0.16 (1)	10.00
I- H	0 / 97	-28.0	-28.0	0.10 (2)	10.00
H- M	0 / 108	-28.0	-28.0	0.10 (2)	10.00
M- F	0 / 108	-28.0	-28.0	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-155 / 7	0.04 (1)
J- D	-36 / 4	0.01 (1)
I- D	-364 / 0	0.06 (1)
I- E	-3 / 5	0.00 (3)
H- E	-231 / 0	0.04 (1)
K- L	-609 / 116	0.00 (1)
M- N	-212 / 8	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.30/1.00 (D-E:1) , BC=0.22/1.00 (J-K:1) , WB=0.08/1.00 (D-I:1) , SSI=0.48/1.00 (B-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

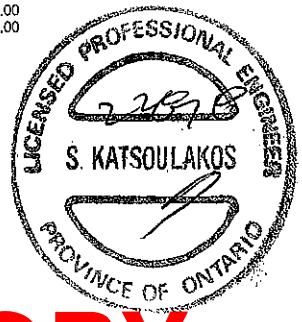
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 1112018
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287525	J1S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:01:18 2018 Page 1

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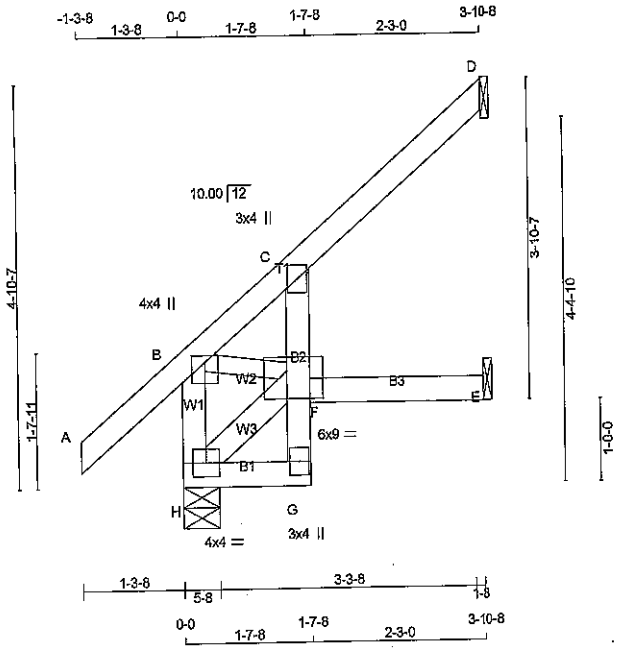

TOTAL WEIGHT = 7 X 19 = 133 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
F	BVMWW-I	MT20	6.0	9.0	4.00	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	429	0	429	0	0	5-8	5-8
D	198	0	198	0	-6	1-8	1-8
E	34	0	59	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	321	232 / 0	41 / 0	0 / 0	0 / 0	49 / 0	0 / 0
D	146	110 / -16	16 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	42	0 / 0	25 / 0	0 / 0	0 / 0	17 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
H- B	-405 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-114 / 0	-104.9	-104.9	0.23 (1)	6.25
C- D	-8 / 43	-104.9	-104.9	0.26 (1)	10.00
H- G	0 / 4	-28.0	-28.0	0.02 (3)	10.00
G- F	0 / 36	0.0	0.0	0.01 (1)	10.00
F- C	0 / 95	0.0	0.0	0.02 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	-5 / 0	0.00 (1)
B- F	-9 / 0	0.00 (2)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 662 (0.07")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.05/1.00 (E-F:3) , WB=0.00/1.00 (B-F:2) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 11121-T8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287529	J31S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Thu Mar 1 09:13:23 2018 Page 1
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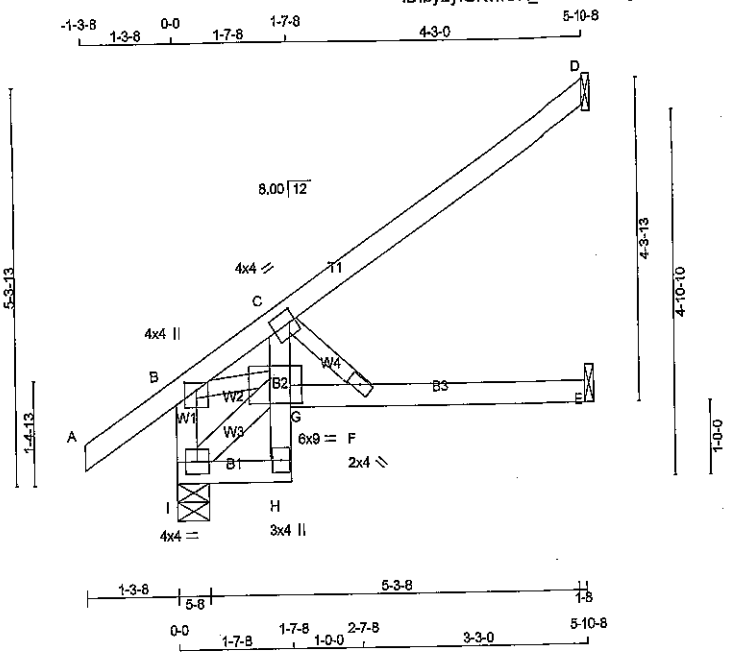


TOTAL WEIGHT = 7 X 24 = 166 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	BMW+w	MT20	2.0	4.0		
G	BVMWW-I	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	551	0	551	0	0	5-8	5-8
D	229	0	229	0	0	1-8	1-8
E	145	0	152	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	420	290 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	161	134 / 0	9 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	138	46 / 0	53 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-512 / 0	0.0	0.0	0.05 (1)	7.81	I- G	-29 / 0	0.00 (1)	
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	B- G	0 / 381	0.09 (1)	
B- C	-491 / 0	-104.9	-104.9	0.14 (1)	6.25	C- F	-561 / 0	0.08 (1)	
C- D	-1 / 11	-104.9	-104.9	0.34 (1)	10.00				
I- H	0 / 23	-28.0	-28.0	0.02 (2)	10.00				
H- G	0 / 36	0.0	0.0	0.04 (1)	10.00				
G- C	0 / 387	0.0	0.0	0.10 (1)	10.00				
G- F	0 / 457	-28.0	-28.0	0.51 (1)	10.00				
F- E	0 / 0	-28.0	-28.0	0.44 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 677 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 418 (0.17")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.51/1.00 (F-G:1) , WB=0.09/1.00 (B-G:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.32 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11146-R
STRUCTURAL COMPONENT ONLY

Boise Cascade

Triple 2 x 10 SPF #2

Dry | 1 span | No cantilevers | 0/12 slope (deg)

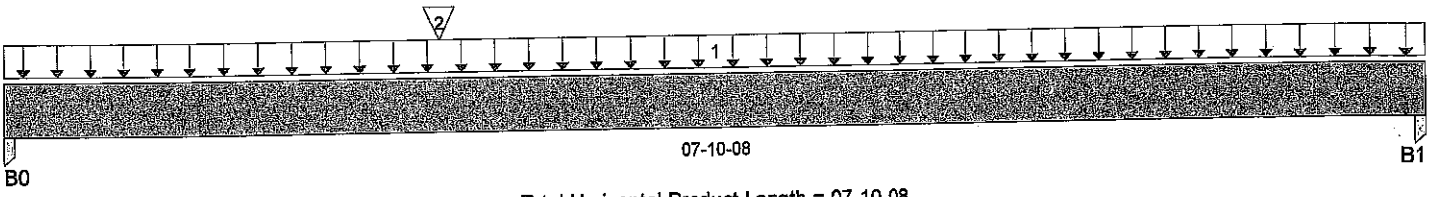
Beam\BM1

February 28, 2018 11:43:39

BC CALC® Design Report

Build 6080

Job Name:	44755/BAYVIEW WELLINGTON	File Name:	287525-BM1
Address:	GREE VALLEY EAST/S38-6	Description:	Designs\BM1
City, Province, Postal Code:	BRADFORD, ON	Specifier:	
Customer:		Designer:	JG
Code reports:	NLGA	Company:	
		Misc:	



Total Horizontal Product Length = 07-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 3-1/2"	559 / 0	561 / 0	1,740 / 0	
B1, 3-1/2"	461 / 0	463 / 0	1,408 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	07-10-08	11	10	33		09-00-00
2		Conc. Pt. (lbs)	L	02-05-00	02-05-00	240	240	810		n/a

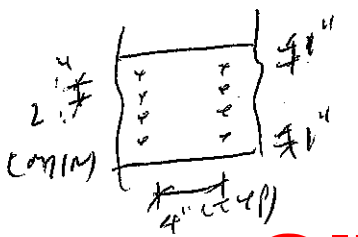
Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,236 ft-lbs	9,967 ft-lbs	62.6%	5	03-02-01
End Shear	2,933 lbs	6,575 lbs	44.6%	5	01-00-12
Total Load Defl.	L/999 (0.107")	n/a	n/a	13	03-09-08
Live Load Defl.	L/999 (0.084")	n/a	n/a	17	03-09-08
Max Defl.	0.107"	n/a	n/a	13	03-09-08
Span / Depth	9.6	n/a	n/a		00-00-00
Squash Blocks	Valid				

Bearing Supports

		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 5-1/2"	3,592 lbs	n/a	47.7%	Unspecified
B1	Post	3-1/2" x 5-1/2"	2,921 lbs	n/a	38.8%	Unspecified

Notes



PROVIDE 4 ROWS OF 3-1/2" ARDOX SPIRAL NAILS @ 4" O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. 1" LUMBER EDGE / END DISTANCE. DO NOT USE AIR NAILS. STAGGER NAILS 2" BETWEEN SIDES.

LICENSED PROFESSIONAL ENGINEER
2016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11151 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

Boise Cascade

Triple 2 x 10 SPF #2

Dry | 1 span | No cantilevers | 0/12 slope (deg)

Beam\BM1

February 28, 2018 11:43:39

BC CALC® Design Report

Build 6080

Job Name:	44755/BAYVIEW WELLINGTON	File Name:	287525-BM1
Address:	GREE VALLEY EAST/S38-6	Description:	Designs\BM1
City, Province, Postal Code:	BRADFORD, ON	Specifier:	
Customer:		Designer:	JG
Code reports:	NLGA	Company:	
		Misc:	

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
CONFORMS TO OBC 2012
Design based on Dry Service Condition.
The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
Importance Factor : Normal Part code : Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11157
STRUCTURAL
COMPONENT ONLY

Boise Cascade

Triple 2 x 10 SPF #2

Dry | 1 span | No cantilevers | 0/12 slope (deg)

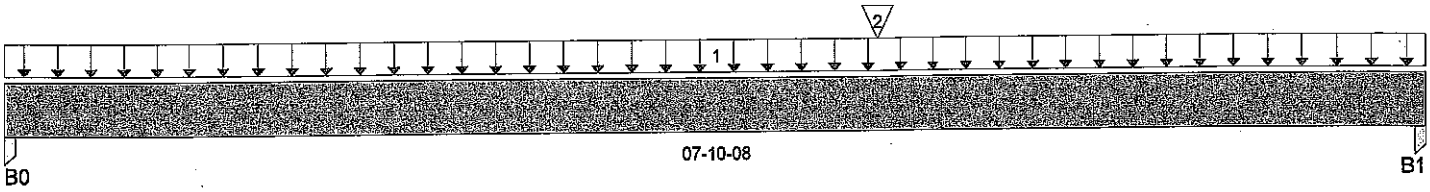
Beam\BM2

February 28, 2018 11:42:40

BC CALC® Design Report

Build 6080

Job Name:	44755/BAYVIEW WELLINGTON	File Name:	287525
Address:	GREE VALLEY EAST/S38-6	Description:	Designs\BM2
City, Province, Postal Code:	BRADFORD, ON	Specifier:	
Customer:		Designer:	JG
Code reports:	NLGA	Company:	
		Misc:	



Total Horizontal Product Length = 07-10-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 5-1/2"	723 / 0	707 / 0	2,224 / 0	
B1, 3-1/2"	771 / 0	756 / 0	2,401 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	Standard Load	Unf. Area (lb/ft^2)	L	00-00-00	07-10-08	11	10	33		13-06-00
2		Conc. Pt. (lbs)	L	04-10-00	04-10-00	325	325	1,117		n/a

Controls Summary

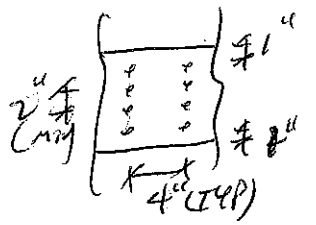
	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	9,622 ft-lbs	9,967 ft-lbs	96.5%	5	04-10-00
End Shear	3,951 lbs	6,575 lbs	60.1%	5	06-09-12
Total Load Defl.	L/576 (0.151")	0.363"	41.7%	13	04-01-14
Live Load Defl.	L/999 (0.119")	n/a	n/a	17	04-01-14
Max Defl.	0.151"	1"	15.1%	13	04-01-14
Span / Depth	9.4	n/a	n/a		00-00-00
Squash Blocks	Valid				

Bearing Supports

		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	5-1/2" x 4-1/2"	4,582 lbs	n/a	30.1%	Unspecified
B1	Post	3-1/2" x 4-1/2"	4,932 lbs	n/a	65.5%	Unspecified

[illegible] required at [illegible]s bearing.

Notes



PROVIDE 4 ROWS OF 3-1/2" ARDOX SPIRAL NAILS @ 4 " O/C FOR MULTI-PLY NAILING. MAINTAIN A MIN. 1" LUMBER EDGE / END DISTANCE. DO NOT USE AIR NAILS.
STAGGER NAILS 2" BETWEEN PLIES.

LICENSED PROFESSIONAL ENGINEER
2676
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11152
STRUCTURAL COMPONENT ONLY

Boise Cascade

Triple 2 x 10 SPF #2

Dry | 1 span | No cantilevers | 0/12 slope (deg)

Beam\BM2

February 28, 2018 11:42:40

BC CALC® Design Report

Build 6080

Job Name:	44755/BAYVIEW WELLINGTON	File Name:	287525
Address:	GREE VALLEY EAST/S38-6	Description:	Designs\BM2
City, Province, Postal Code:	BRADFORD, ON	Specifier:	
Customer:		Designer:	JG
Code reports:	NLGA	Company:	
		Misc:	

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
CONFORMS TO OBC 2012
Design based on Dry Service Condition.
The analysis of solid sawn wood members is in accordance with CSA 086 and is limited to the output shown above. All other support and design for these products, including but not limited to notching, connections, installation, and engineer/architect certification is the responsibility of the project's design professional of record.
Importance Factor : Normal Part code : Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11152-18
STRUCTURAL
COMPONENT ONLY

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

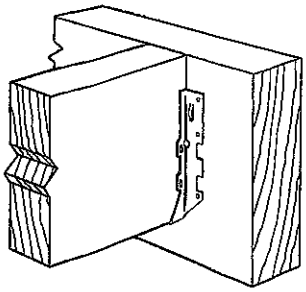
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

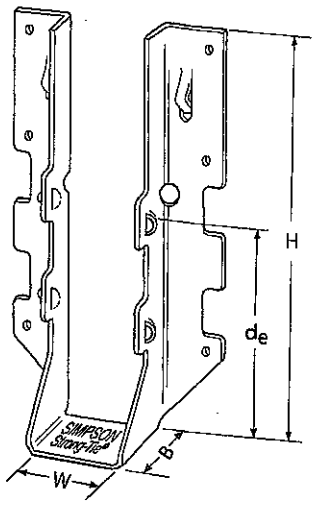
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



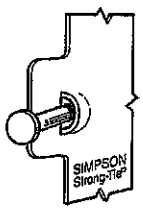
Typical LUS Installation



LUS28

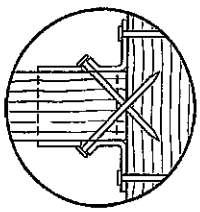
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.

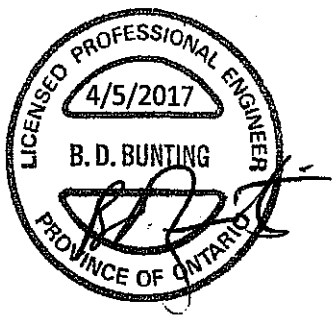


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

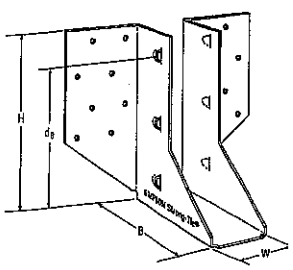
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

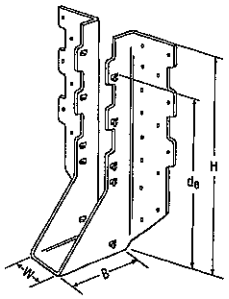
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

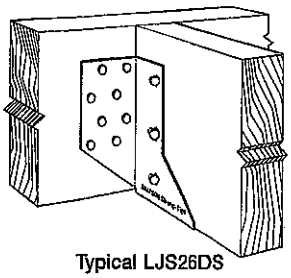
- See current catalogue for options



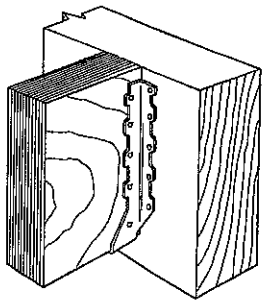
LJS26DS



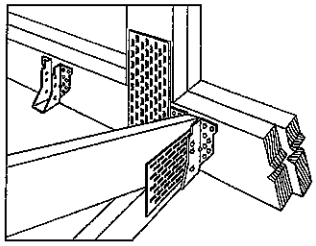
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



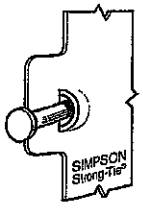
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

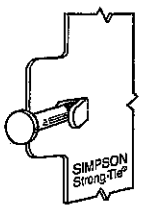
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

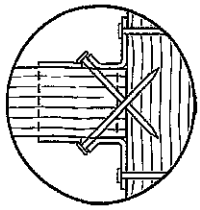


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

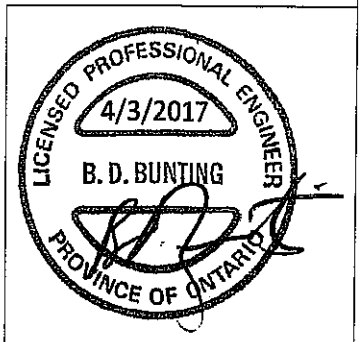
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

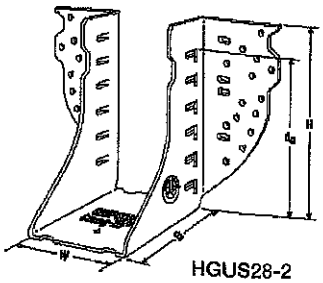
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

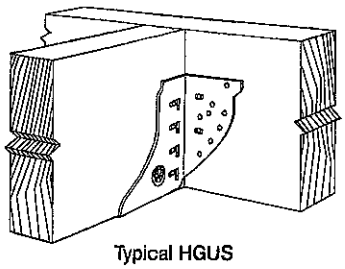
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

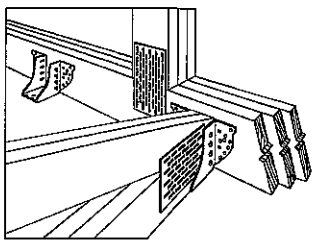
Options:

- See current catalogue for options



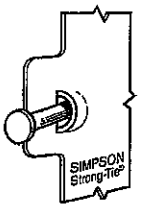
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

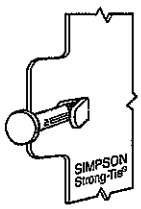
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 9/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

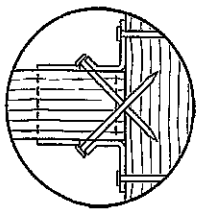


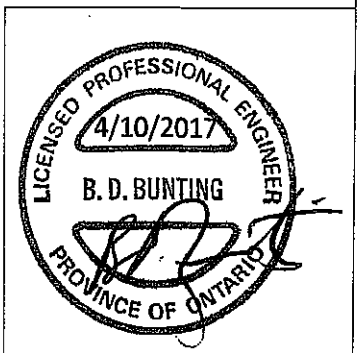
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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(800) 999-5099
strongtie.com

ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

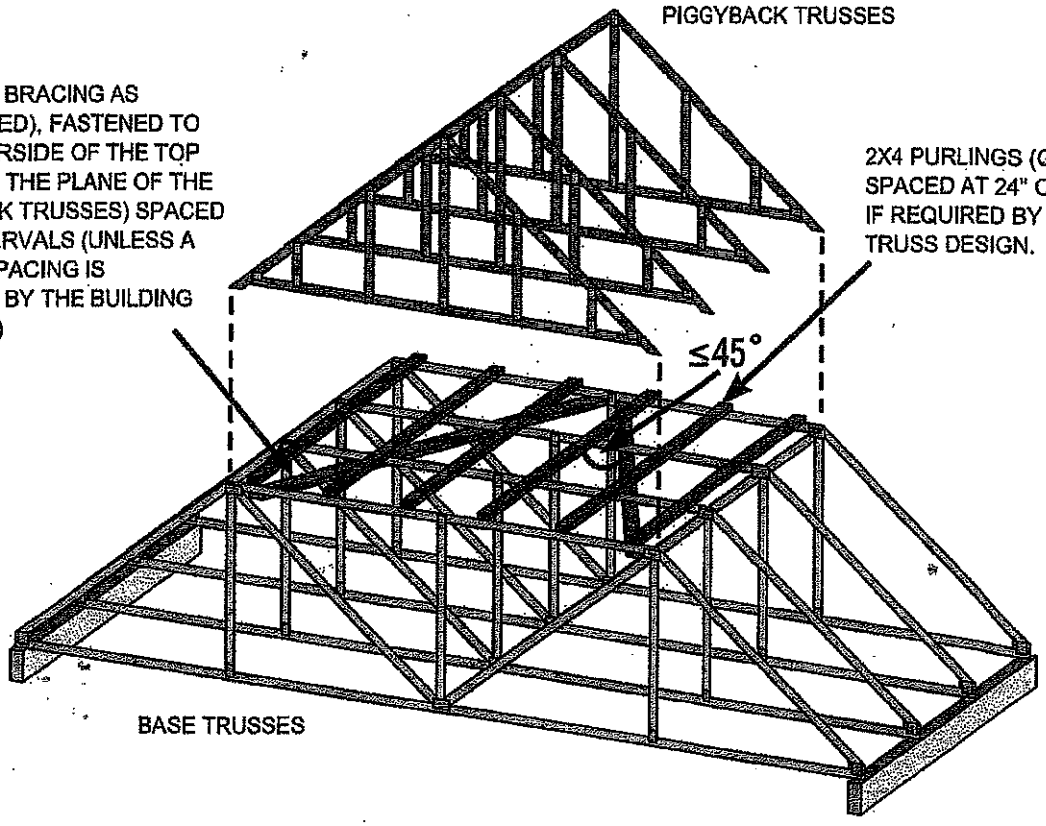
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

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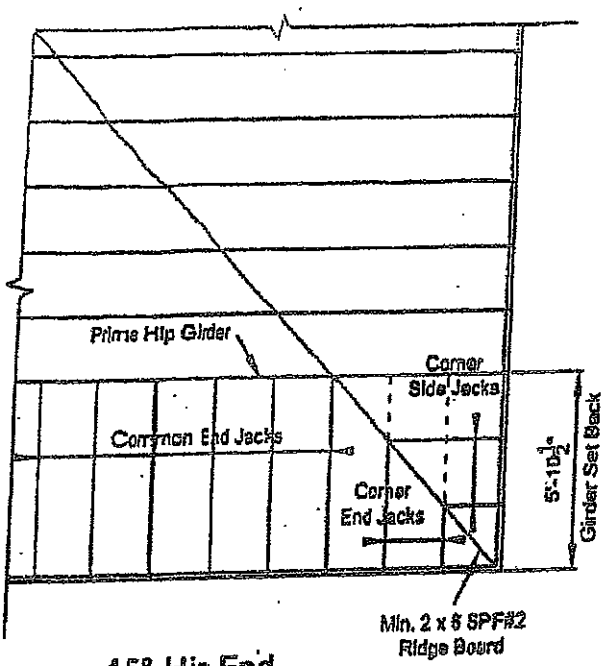

45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

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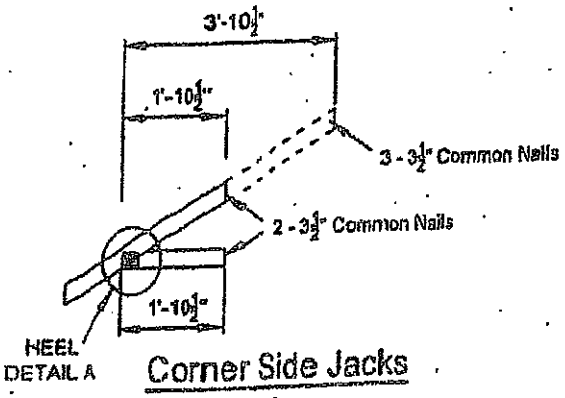
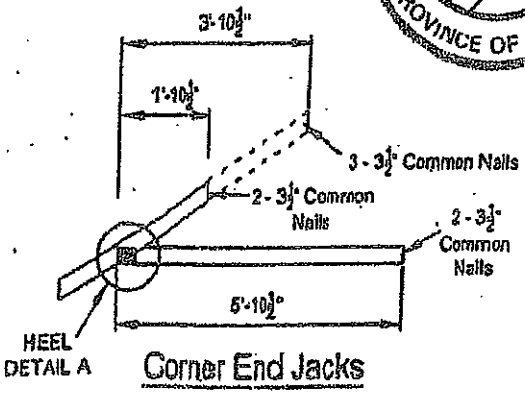
STRUCTURAL COMPONENT ONLY

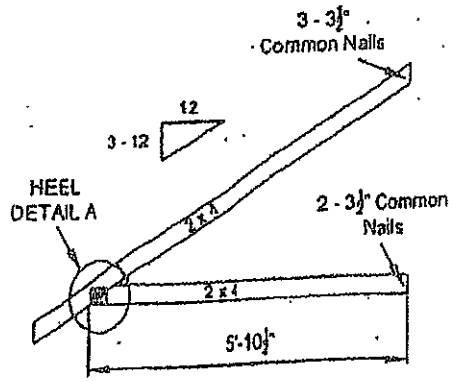
LICENSED PROFESSIONAL ENGINEER
1-2-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



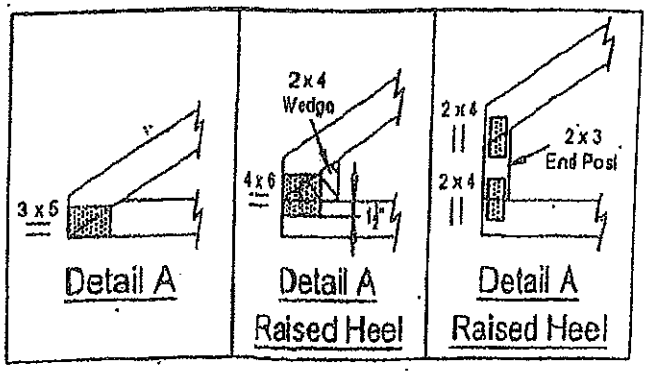
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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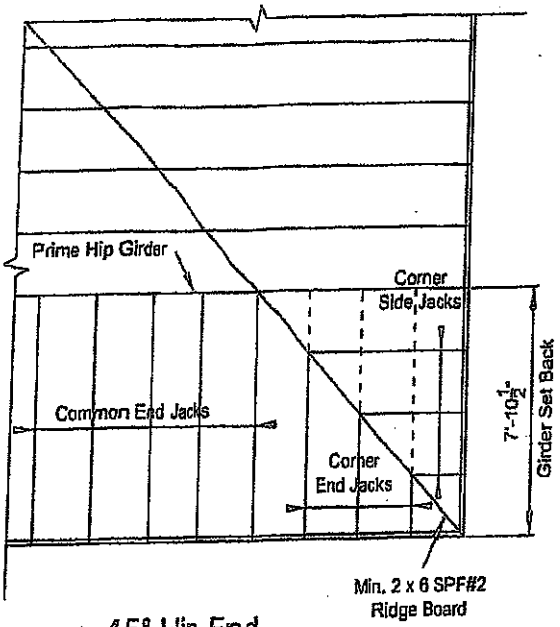
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOAD:

TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY

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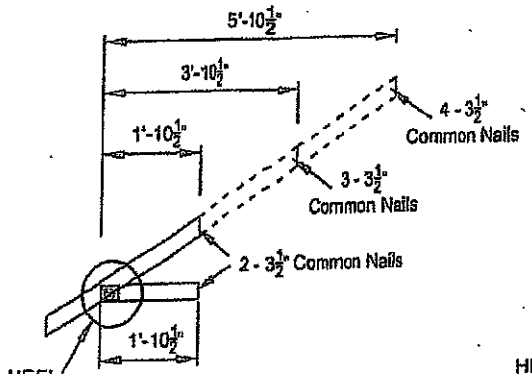
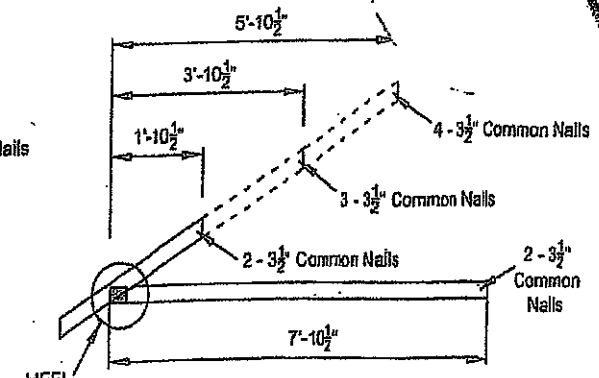
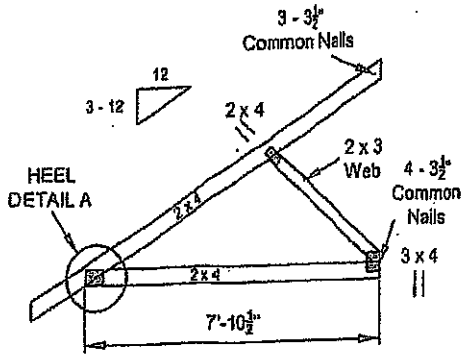


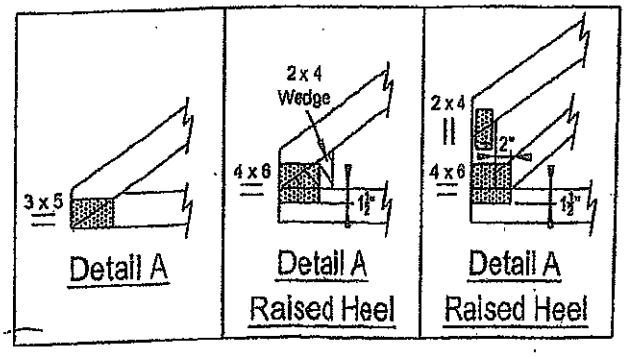
Corner Side Jacks



Corner End Jacks



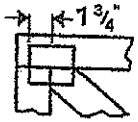
Common End Jacks



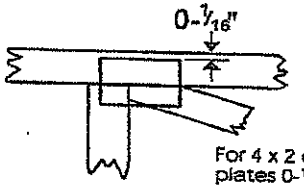
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

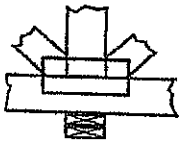
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

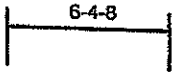
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

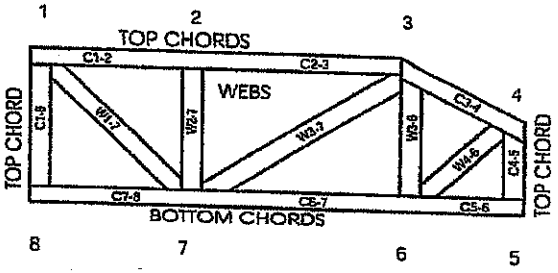
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.®

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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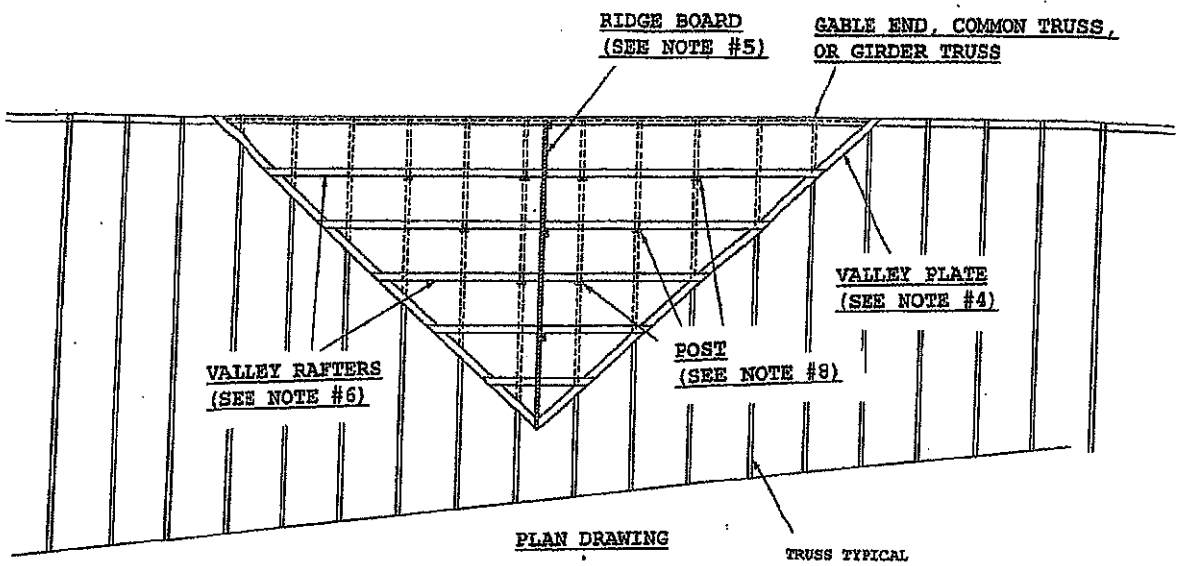
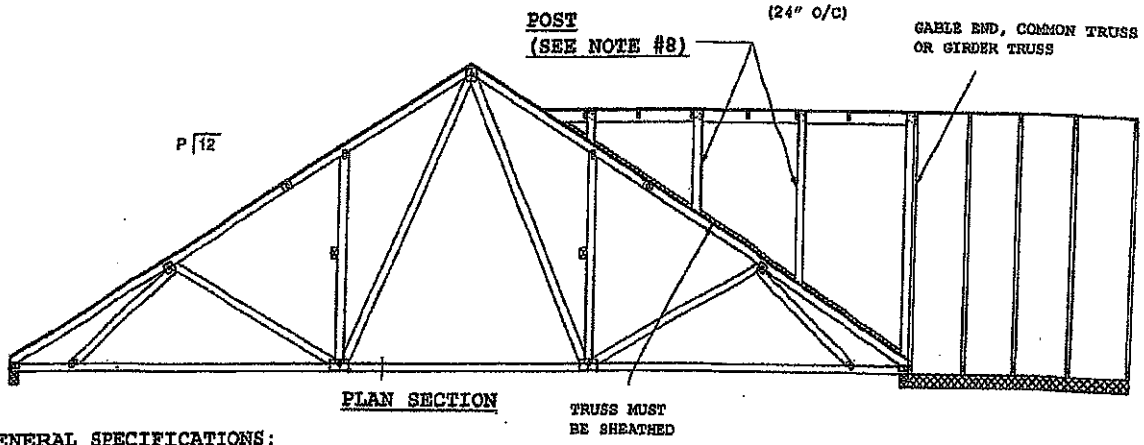
CONVENTIONAL VALLEY FRAMING DETAIL





GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 1/7/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.