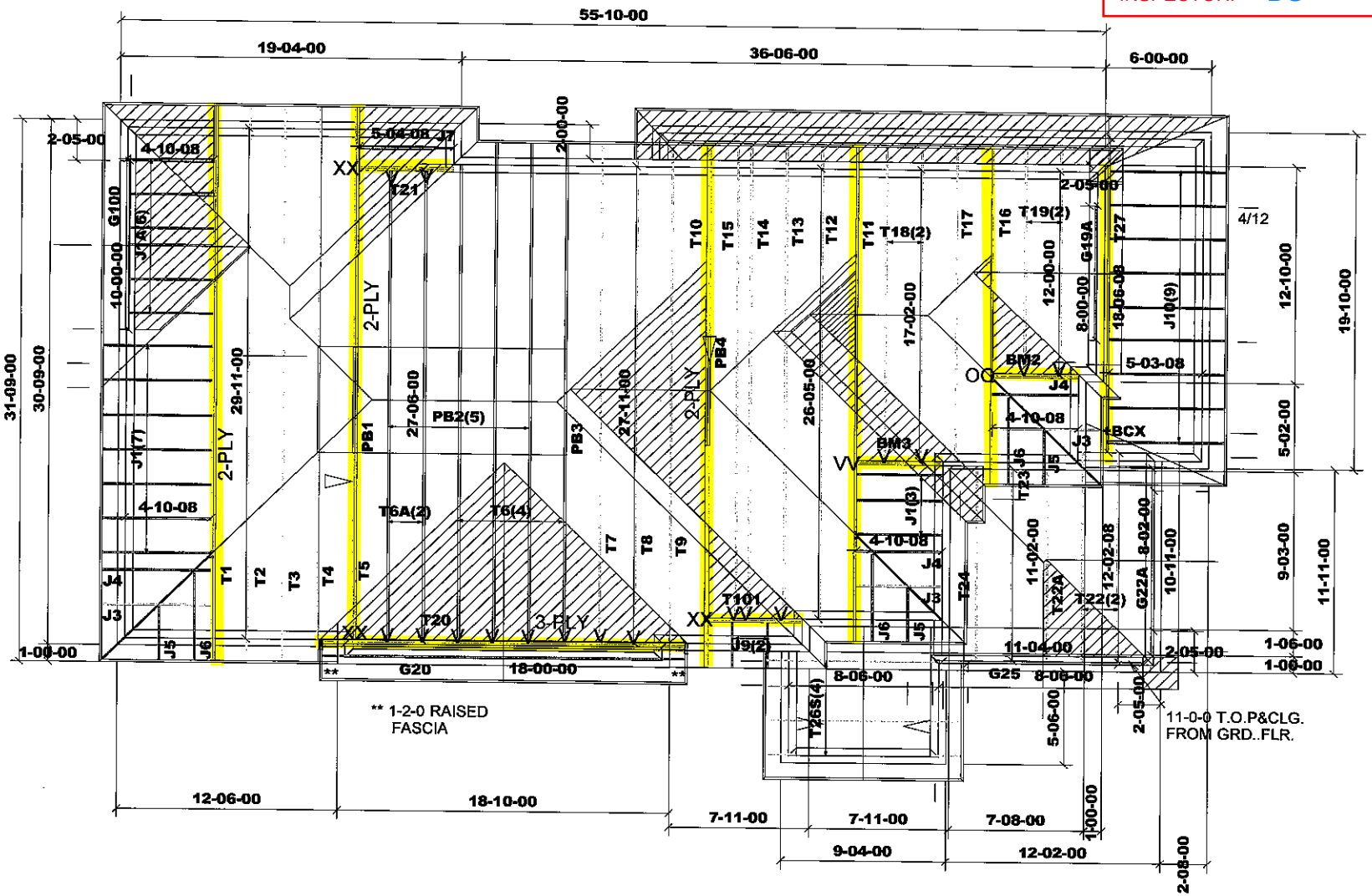
TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-11-02
INSPECTOR: BG

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



** 1-2-0 RAISED FASCIA

11-0-0 T.O.P&CLG. FROM GRD..FLR.

12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,449

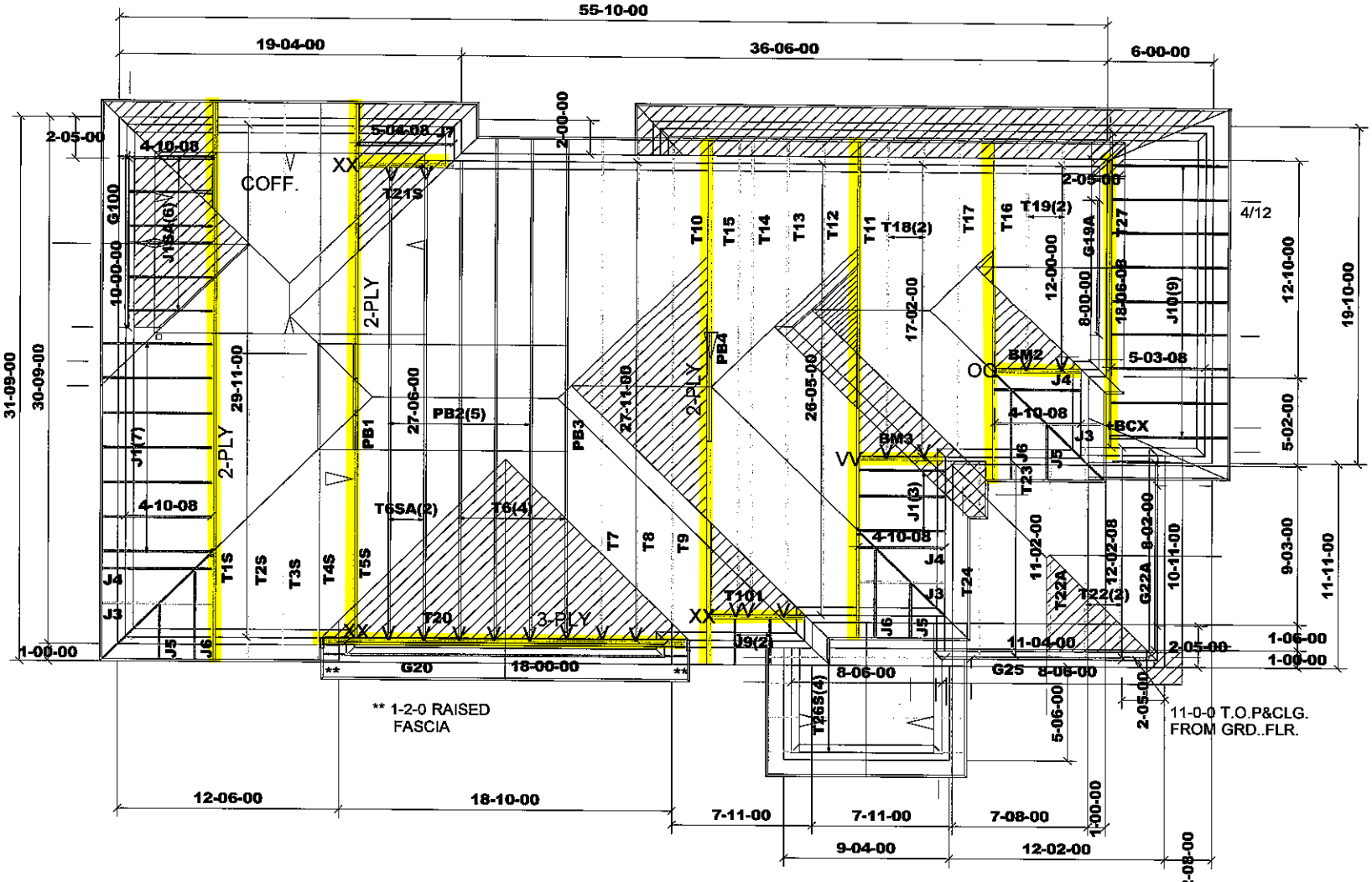
TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-7C / A
	Layout ID: 287569	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/1/2018 Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



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DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
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DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS24(O)
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,449

TAMARACK LUMBER INC. ALPA LUMBER GROUP

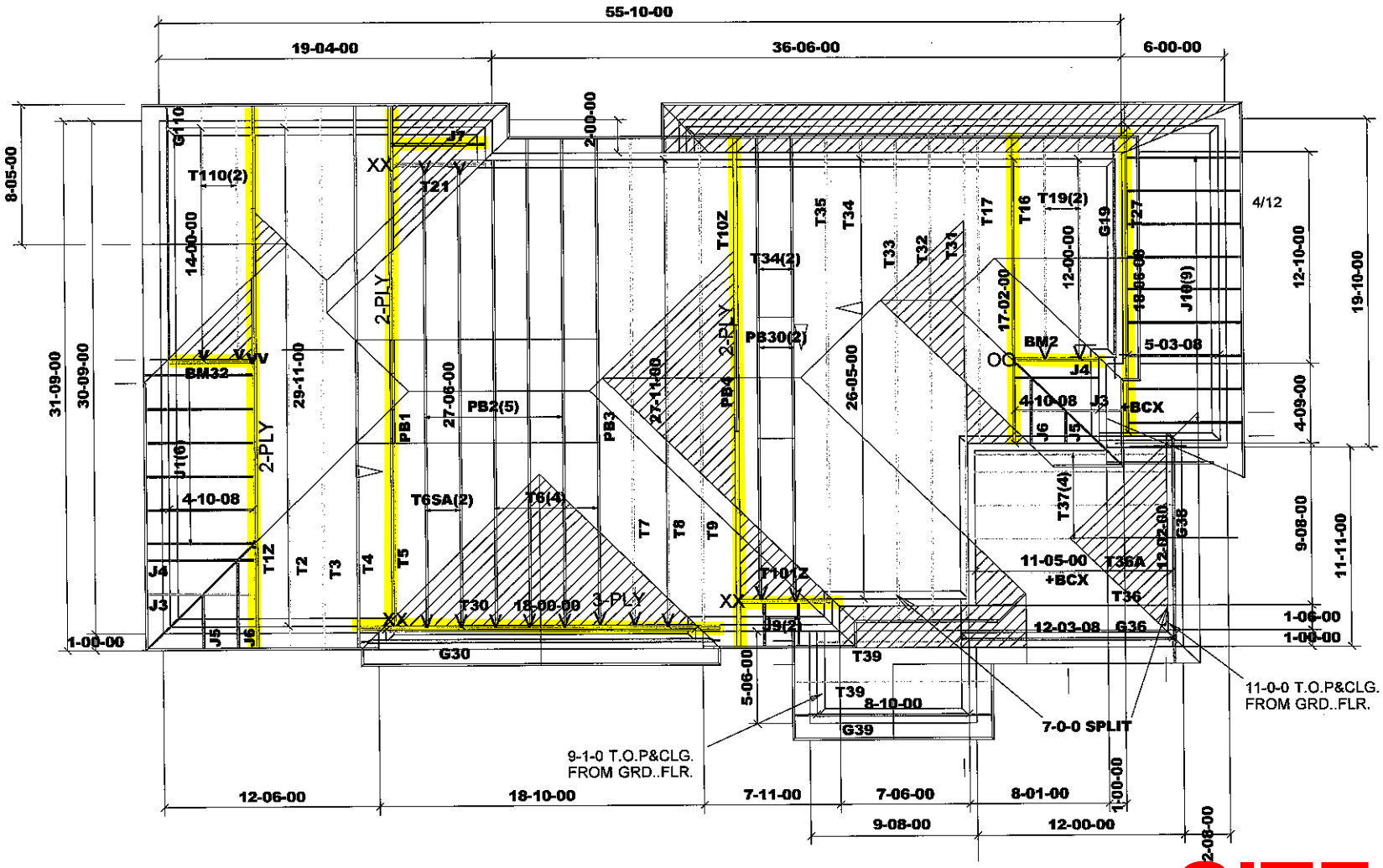
Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-7C / A-OPT.COFF
Layout ID: 287566	Project: GREEN VALLEY EAST	
Plan Log: 96660	Date: 3/1/2018 Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS26-2(VV)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)

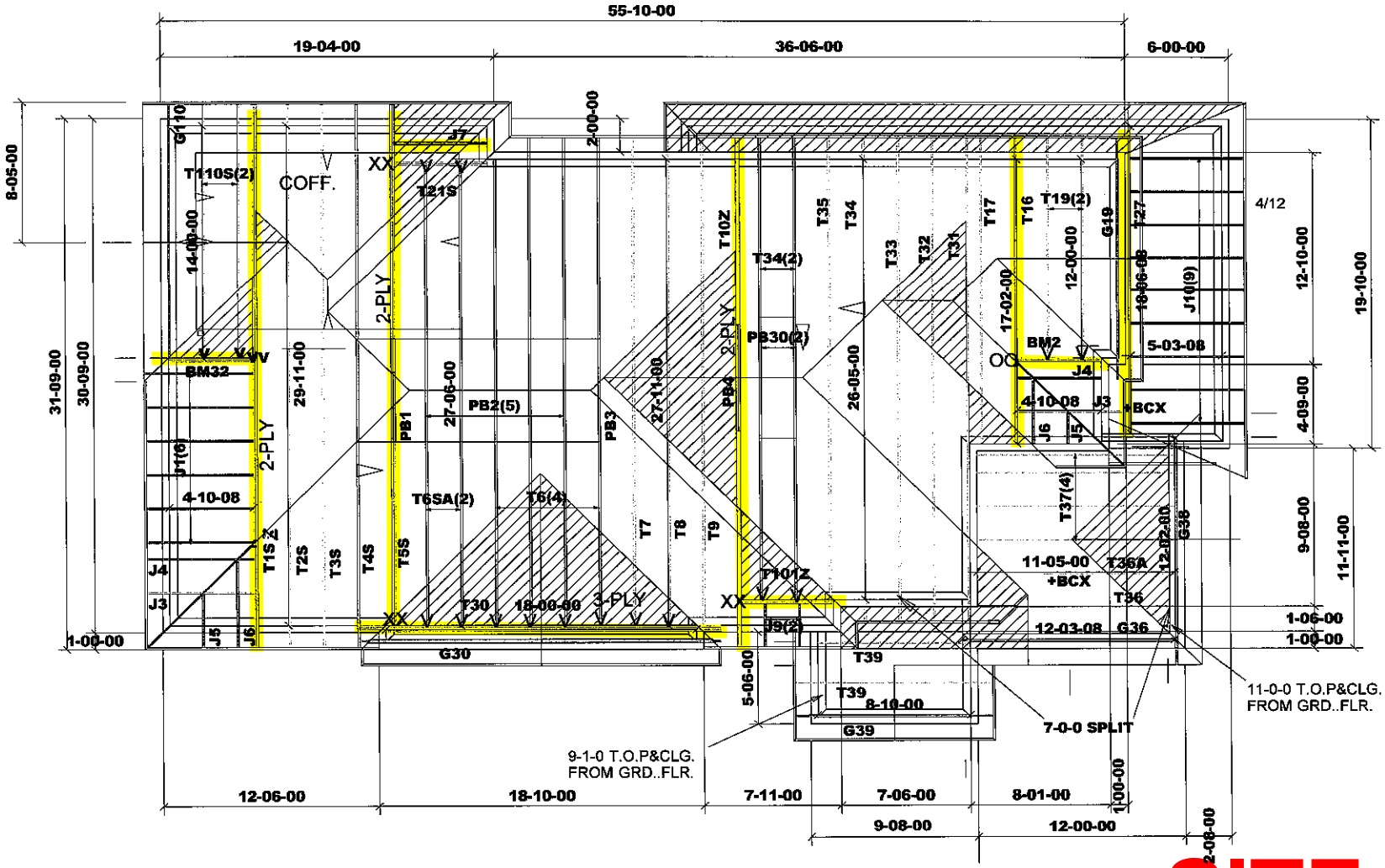
SITE COPY

M11,449

TAMARACK LUMBER INC. ALFA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-7C / B
	Layout ID: 287570	Project: GREEN VALLEY EAST	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: 96660	Date: 3/1/2018 Designer: JG	Mitek ver 7.5.0

10/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LUS26-2(VV)
LJS26DS(V)
LUS24-2(OO)
HGUS26-2(XX)

SITE COPY

M11,449

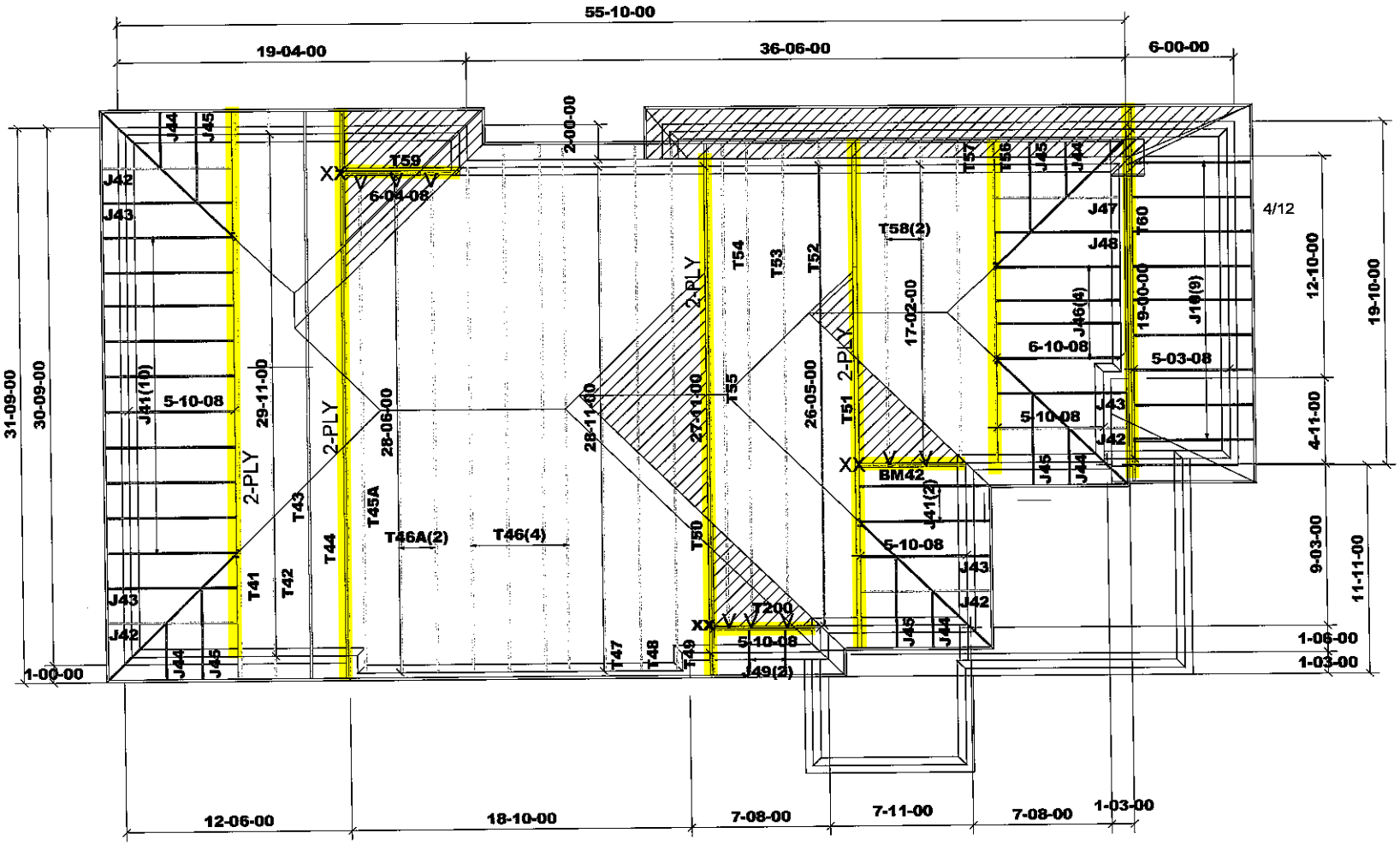
TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-7C / B-OPT.COFF
	Layout ID: 287567	Project: GREEN VALLEY EAST	
	Plan Log: 96660	Date: 3/1/2018 Designer: JG	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



12" FINISH O.H
R.T.M.C
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DESIGN CONFORMS WITH THE RELEVANT SECTION OF THE LATEST EDITION OF O.B.C. PART.9

DESIGN LOADS:
GROUND SNOW LOAD
Ss= 2.1kPa

TC DEAD	3 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,449

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	287571
Plan Log:	96660

Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**

Project: **GREEN VALLEY EAST**

Date: 3/2/2018 Designer: JG

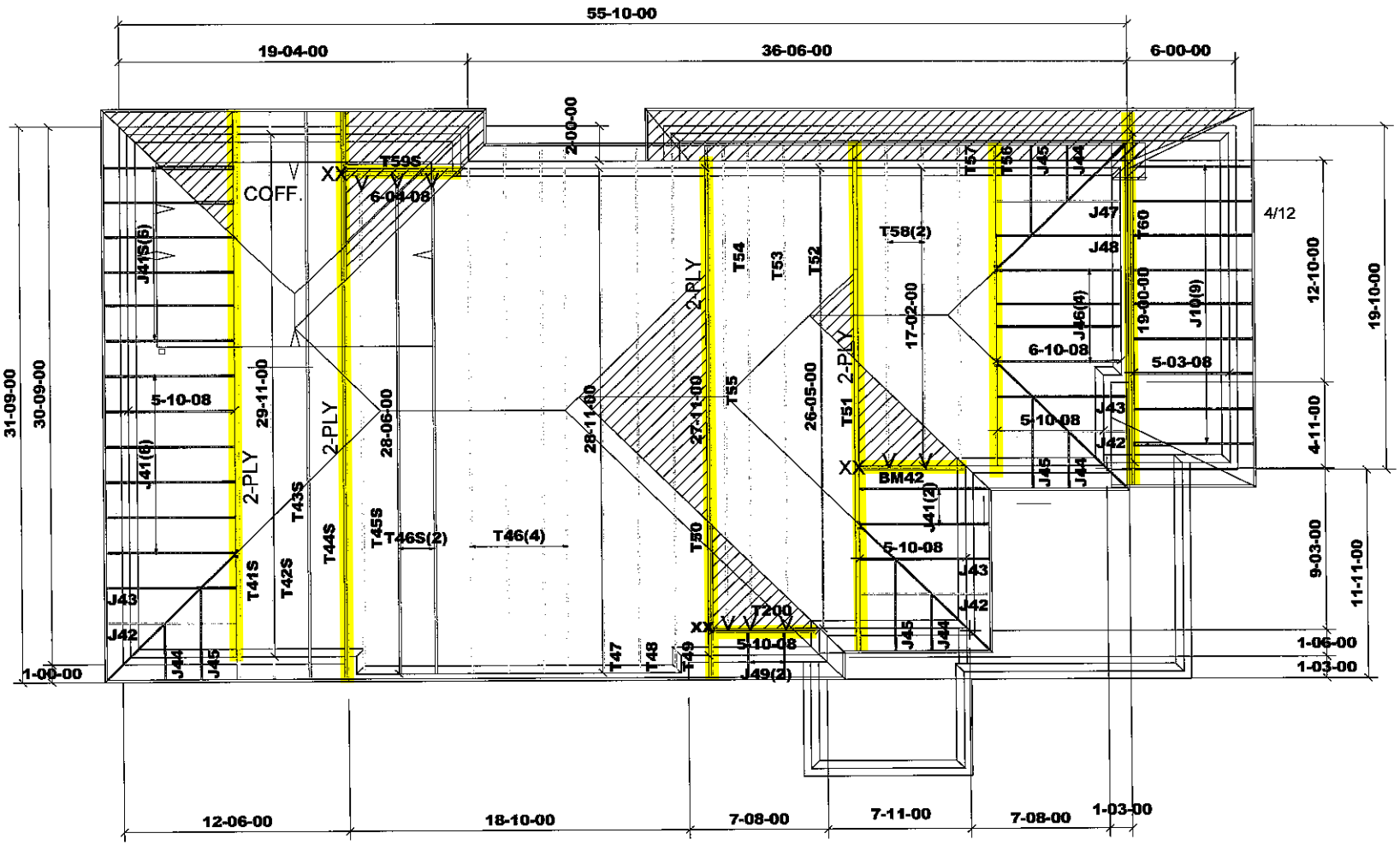
Model / Elevation: **S38-7C / C**

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

8/12 ROOF PITCH
BEAMS(BM):2-2X10
UNLESS NOTED

T-180101



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R.T.M.C
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GROUND SNOW LOAD
Ss= 2.1kPa
TC DEAD 3 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

DENOTES CONVENTIONAL FRAMING

HARDWARE
LJS26DS(V)
HGUS26-2(XX)

SITE COPY

M11,449

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S38-7C / C-OPT.COFF
	Layout ID: 287568	Project: GREEN VALLEY EAST	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.
	Plan Log: 96660	Date: 3/1/2018 Designer: JG	Mitek ver 7.5.0

DATE	03/01/18
SALES REP	Mario

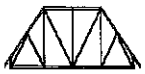
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755	LAYOUT ID: 287569	LOCATION:	BRADFORD
BUILDER:	BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL:	S38-7C	ELEVATION:	A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 HIP GIRDER	10.00 / 0.00	29-11-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	299.52 / 183.00		
	1	T2 HIP	10.00 / 0.00	29-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	139.50 / 87.17		
	1	T3 HIP	10.00 / 0.00	29-11-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	150.88 / 95.17		
	1	T4 ROOF	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	162.14 / 102.50		
	1 / 2 Ply	T5 PIGGYBACK	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 6	00-00-00 / 01-03-08	01-07-11 / 01-07-11	334.36 / 207.00		
	4	T6 PIGGYBACK	10.00 / 0.00	27-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	579.92 / 365.32		
	2	T6A PIGGYBACK	10.00 / 0.00	27-06-00	10-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 01-11-14	280.42 / 175.00		
	1	T7 HIP	10.00 / 0.00	27-11-00	11-01-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	147.69 / 91.66		
	1	T8 HIP	10.00 / 0.00	27-11-00	09-05-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.88 / 89.50		
	1	T9 HIP	10.00 / 0.00	27-11-00	07-09-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	126.81 / 80.67		
	1 / 2 Ply	T10 PIGGYBACK	10.00 / 0.00	27-11-00	10-00-00	2 X 4	2 X 6	01-03-08 / 01-05-00	01-07-11 / 01-07-11	320.16 / 201.00		
	1	T11 HIP GIRDER	10.00 / 0.00	26-05-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	137.61 / 86.83		
	1	T12 HIP	10.00 / 0.00	26-05-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	120.01 / 75.50		
	1	T13 HIP	10.00 / 0.00	26-05-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	134.80 / 85.83		
	1	T14 HIP	10.00 / 0.00	26-05-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.83 / 88.66		
	1	T15 HIP	10.00 / 0.00	26-05-00	11-03-15	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.81 / 91.00		
	1	T16 HIP GIRDER	10.00 / 0.00	17-02-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.65 / 58.83		
	1	T17 HIP	10.00 / 0.00	17-02-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	84.65 / 54.00		

SITE COPY

DATE	03/01/18
SALES REP	Mario

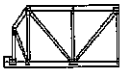
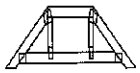
TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287569 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T18 COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	168.12 / 108.66		
	2	T19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	116.22 / 74.00		
	1	G19A COMMON	10.00 / 0.00	08-00-00	06-07-11	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	45.53 / 29.33		
	1 / 3 Ply	T20 COMMON	10.00 / 0.00	18-00-00	10-03-11	2 X 4	2 X 6	01-03-08 / 01-03-08	02-09-11 / 02-09-11	325.50 / 203.01		
	1	G20 COMMON	10.00 / 0.00	18-00-00	10-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	02-09-11 / 02-09-11	101.89 / 65.33		
	1 / 2 Ply	T21 HALF HIP	10.00 / 0.00	05-04-08	03-05-06	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 03-05-06	56.22 / 39.34		
	2	T22 COMMON	10.00 / 0.00	12-02-08	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 01-05-10	109.58 / 72.34		
	1	T22A COMMON	10.00 / 0.00	11-02-00	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	54.97 / 36.17		
	1	G22A COMMON	10.00 / 0.00	08-02-00	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	46.20 / 30.50		
	1	T23 HALF HIP	10.00 / 0.00	11-02-00	05-03-07	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-08 / 05-03-07	53.01 / 33.17		
	1	T24 HALF HIP	10.00 / 0.00	11-02-00	03-07-07	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-08 / 03-07-07	49.51 / 33.50		
	1	G25 COMMON	10.00 / 0.00	08-06-00	06-10-03	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	47.56 / 30.50		
	4	T26S SCISSOR	10.00 / 6.00	08-06-00	03-11-05	2 X 4	2 X 4	01-03-08 / 01-03-08	00-04-13 / 00-04-13	127.40 / 84.00		
	1	T27 HIP GIRDER	9.00 / 0.00	18-06-08	02-01-02	2 X 4	2 X 6	01-03-08 / 00-00-00	00-11-00 / 00-05-08	90.84 / 57.33		
	1	G100 COMMON	10.00 / 0.00	10-00-00	07-05-11	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	53.74 / 33.50		
	1 / 2 Ply	T101 HALF HIP	10.00 / 0.00	05-03-08	02-08-03	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-08-03	53.16 / 37.00		
	1 / 2 Ply	PB1 PIGGYBACK	10.00 / 0.00	04-10-14	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	31.14 / 20.66		
	5	PB2 PIGGYBACK	10.00 / 0.00	04-10-14	02-05-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	80.95 / 55.00		

SITE COPY

DATE	03/01/18
SALES REP	Mario

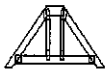
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287569 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB3 PIGGYBACK	10.00 / 0.00	04-10-14	01-11-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	16.67 / 12.00		
	1 / 2 Ply	PB4 PIGGYBACK	10.00 / 0.00	05-01-02	02-06-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	33.50 / 23.34		
	10	J1 JACK-OPEN	10.00 / 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-08-07	182.00 / 121.70		
	6	J1A JACK-OPEN	10.00 / 0.00	04-08-08	05-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 05-08-07	96.30 / 64.98		
	3	J3 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 03-01-01	01-07-11 / 00-03-08	39.48 / 27.00		
	3	J4 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 01-01-01	01-07-11 / 00-03-08	49.74 / 32.49		
	3	J5 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	25.11 / 18.00		
	3	J6 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	38.28 / 24.99		
	1	J7 JACK-OPEN	10.00 / 0.00	05-04-08	06-01-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 00-07-07	17.59 / 12.17		
	2	J9 JACK-OPEN	10.00 / 0.00	01-03-00	02-08-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-08-03	13.72 / 10.66		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		

TOTAL # TRUSS= **101.00** TOTAL BFT OF ALL TRUSSES= **3693.28** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5788.11** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
17	Hangers	LJS26DS	
1	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **22.00**

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TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

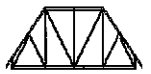
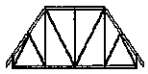
Delivery Shiplist

DATE	03/01/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287566 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-7C **ELEVATION:** A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1S HIP GIRDER	10.00 / 0.00	29-11-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	339.06 / 214.00		
	1	T2S HIP	10.00 / 0.00	29-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	146.74 / 93.33		
	1	T3S HIP	10.00 / 0.00	29-11-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	163.81 / 105.83		
	1	T4S ROOF	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	167.96 / 108.16		
	1 / 2 Ply	T5S PIGGYBACK	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 6	00-00-00 / 01-03-08	01-07-11 / 01-07-11	361.26 / 229.34		
	4	T6 PIGGYBACK	10.00 / 0.00	27-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	579.92 / 365.32		
	2	T6SA PIGGYBACK	10.00 / 0.00	27-06-00	10-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 00-11-14	288.68 / 182.00		
	1	T7 HIP	10.00 / 0.00	27-11-00	11-01-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	147.69 / 91.66		
	1	T8 HIP	10.00 / 0.00	27-11-00	09-05-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.88 / 89.50		
	1	T9 HIP	10.00 / 0.00	27-11-00	07-09-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	126.81 / 80.67		
	1 / 2 Ply	T10 PIGGYBACK	10.00 / 0.00	27-11-00	10-00-00	2 X 4	2 X 6	01-03-08 / 01-05-00	01-07-11 / 01-07-11	320.16 / 201.00		
	1	T11 HIP GIRDER	10.00 / 0.00	26-05-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	137.61 / 86.83		
	1	T12 HIP	10.00 / 0.00	26-05-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	120.01 / 75.50		
	1	T13 HIP	10.00 / 0.00	26-05-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	134.80 / 85.83		
	1	T14 HIP	10.00 / 0.00	26-05-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.83 / 88.66		
	1	T15 HIP	10.00 / 0.00	26-05-00	11-03-15	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.81 / 91.00		
	1	T16 HIP GIRDER	10.00 / 0.00	17-02-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.65 / 58.83		
	1	T17 HIP	10.00 / 0.00	17-02-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	84.65 / 54.00		

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DATE	03/01/18
SALES REP	Mario

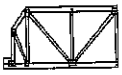
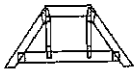
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287566 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T18 COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	168.12 / 108.66		
	2	T19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	116.22 / 74.00		
	1	G19A COMMON	10.00 / 0.00	08-00-00	06-07-11	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	45.53 / 29.33		
	1 / 3 Ply	T20 COMMON	10.00 / 0.00	18-00-00	10-03-11	2 X 4	2 X 6	01-03-08 / 01-03-08	02-09-11 / 02-09-11	325.50 / 203.01		
	1	G20 COMMON	10.00 / 0.00	18-00-00	10-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	02-09-11 / 02-09-11	101.89 / 65.33		
	1 / 2 Ply	T21S HALF HIP	10.00 / 0.00	05-04-08	03-05-06	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-05-06	57.80 / 42.66		
	2	T22 COMMON	10.00 / 0.00	12-02-08	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 01-05-10	109.58 / 72.34		
	1	T22A COMMON	10.00 / 0.00	11-02-00	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	54.97 / 36.17		
	1	G22A COMMON	10.00 / 0.00	08-02-00	06-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	46.20 / 30.50		
	1	T23 HALF HIP	10.00 / 0.00	11-02-00	05-03-07	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-08 / 05-03-07	53.01 / 33.17		
	1	T24 HALF HIP	10.00 / 0.00	11-02-00	03-07-07	2 X 4	2 X 4	00-00-00 / 00-00-00	00-03-08 / 03-07-07	49.51 / 33.50		
	1	G25 COMMON	10.00 / 0.00	08-06-00	06-10-03	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	47.56 / 30.50		
	4	T26S SCISSOR	10.00 / 6.00	08-06-00	03-11-05	2 X 4	2 X 4	01-03-08 / 01-03-08	00-04-13 / 00-04-13	127.40 / 84.00		
	1	T27 HIP GIRDER	9.00 / 0.00	18-06-08	02-01-02	2 X 4	2 X 6	01-03-08 / 00-00-00	00-11-00 / 00-05-08	90.84 / 57.33		
	1	G100 COMMON	10.00 / 0.00	10-00-00	07-05-11	2 X 4	2 X 4	00-00-00 / 00-00-00	03-03-11 / 03-03-11	53.74 / 33.50		
	1 / 2 Ply	T101 HALF HIP	10.00 / 0.00	05-03-08	02-08-03	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-08-03	53.16 / 37.00		
	1 / 2 Ply	PB1 PIGGYBACK	10.00 / 0.00	04-10-14	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	31.14 / 20.66		
	5	PB2 PIGGYBACK	10.00 / 0.00	04-10-14	02-05-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	80.95 / 55.00		

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DATE	03/01/18
SALES REP	Mario

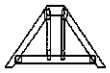
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 **LAYOUT ID:** 287566 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-7C **ELEVATION:** A-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB3 PIGGYBACK	10.00 / 0.00	04-10-14	01-11-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	16.67 / 12.00		
	1 / 2 Ply	PB4 PIGGYBACK	10.00 / 0.00	05-01-02	02-06-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	33.50 / 23.34		
	10	J1 JACK-OPEN	10.00 / 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-08-07	182.00 / 121.70		
	6	J1SA JACK-OPEN	10.00 / 0.00	04-08-08	05-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 04-08-07	115.02 / 82.98		
	3	J3 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 03-01-01	01-07-11 / 00-03-08	39.48 / 27.00		
	3	J4 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 01-01-01	01-07-11 / 00-03-08	49.74 / 32.49		
	3	J5 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	25.11 / 18.00		
	3	J6 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	38.28 / 24.99		
	1	J7 JACK-OPEN	10.00 / 0.00	05-04-08	06-01-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 00-07-07	17.59 / 12.17		
	2	J9 JACK-OPEN	10.00 / 0.00	01-03-00	02-08-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-08-03	13.72 / 10.66		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		

TOTAL # TRUSS= **101.00** TOTAL BFT OF ALL TRUSSES= **3797.42** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5909.10** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
17	Hangers	LJS26DS	
1	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **22.00**

SITE COPY

TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

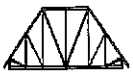
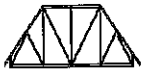
Delivery Shiplist

DATE	03/01/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287570 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1Z HIP GIRDER	10.00 / 0.00	29-11-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	299.52 / 183.00		
	1	T2 HIP	10.00 / 0.00	29-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	139.50 / 87.17		
	1	T3 HIP	10.00 / 0.00	29-11-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	150.88 / 95.17		
	1	T4 ROOF	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	162.14 / 102.50		
	1 / 2 Ply	T5 PIGGYBACK	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 6	00-00-00 / 01-03-08	01-07-11 / 01-07-11	337.72 / 209.66		
	4	T6 PIGGYBACK	10.00 / 0.00	27-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	579.92 / 365.32		
	2	T6A PIGGYBACK	10.00 / 0.00	27-06-00	10-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 01-11-14	280.40 / 175.00		
	1	T7 HIP	10.00 / 0.00	27-11-00	11-01-14	2 X 4	2 X 4	01-03-07 / 01-03-08	01-07-11 / 01-07-11	147.68 / 91.66		
	1	T8 HIP	10.00 / 0.00	27-11-00	09-05-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.88 / 89.50		
	1	T9 HIP	10.00 / 0.00	27-11-00	07-09-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	126.81 / 80.67		
	1 / 2 Ply	T10Z PIGGYBACK	10.00 / 0.00	27-11-00	10-00-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	319.78 / 201.00		
	1	T16 HIP GIRDER	10.00 / 0.00	17-02-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.65 / 58.83		
	1	T17 HIP	10.00 / 0.00	17-02-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	84.65 / 54.00		
	2	T19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	116.22 / 74.00		
	1	G19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	58.38 / 38.00		
	1 / 2 Ply	T21 HALF HIP	10.00 / 0.00	05-04-08	03-05-06	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 03-05-06	57.96 / 40.66		
	1	T27 HIP GIRDER	9.00 / 0.00	18-06-08	02-01-02	2 X 4	2 X 6	01-03-08 / 00-00-00	00-11-00 / 00-05-08	90.84 / 57.33		
	1 / 3 Ply	T30 COMMON	10.00 / 0.00	18-00-00	09-01-11	2 X 4	2 X 6	01-05-00 / 01-05-00	01-07-11 / 01-07-11	298.71 / 189.51		

SITE COPY

DATE	03/01/18
SALES REP	Mario

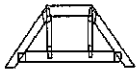
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 **LAYOUT ID:** 287570 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-7C **ELEVATION:** B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G30 COMMON	10.00 / 0.00	18-00-00	09-01-11	2 X 4	2 X 4	01-05-00 / 01-05-00	01-07-11 / 01-07-11	91.88 / 58.50		
	1	T31 HIP	10.00 / 0.00	26-05-00	04-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	106.80 / 69.33		
	1	T32 HIP	10.00 / 0.00	26-05-00	06-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	122.02 / 78.67		
	1	T33 HIP	10.00 / 0.00	26-05-00	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	123.54 / 78.67		
	3	T34 PIGGYBACK	10.00 / 0.00	26-05-00	09-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.19 / 252.00		
	1	T35 HIP	10.00 / 0.00	26-05-00	11-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.68 / 91.00		
	1	T36 COMMON	10.00 / 0.00	12-03-08	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 05-08-07	64.57 / 42.50		
	1	G36 COMMON	10.00 / 0.00	12-03-08	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 05-08-07	66.54 / 43.50		
	1	T36A COMMON	10.00 / 0.00	11-05-00	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	65.31 / 43.17		
	4	T37 MONOPITCH	10.00 / 0.00	11-05-00	11-03-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	246.80 / 154.68		
	1	G38 COMMON	10.00 / 0.00	12-02-00	06-08-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	59.20 / 39.17		
	2	T39 COMMON	10.00 / 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	81.92 / 53.66		
	1	G39 COMMON	10.00 / 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	42.96 / 28.83		
	1 / 2 Ply	T101Z HALF HIP	10.00 / 0.00	05-03-08	02-08-03	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-08-03	51.28 / 35.66		
	2	T110 COMMON	10.00 / 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	132.22 / 87.00		
	1	G110 COMMON	10.00 / 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	68.43 / 44.50		
	1 / 2 Ply	PB1 PIGGYBACK	10.00 / 0.00	04-10-14	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	31.14 / 20.66		
	5	PB2 PIGGYBACK	10.00 / 0.00	04-10-14	02-05-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	80.95 / 55.00		

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DATE	03/01/18
SALES REP	Mario

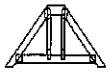
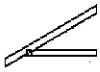
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287570 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB3 PIGGYBACK	10.00 / 0.00	04-10-14	01-11-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	16.67 / 12.00		
	1 / 2 Ply	PB4 PIGGYBACK	10.00 / 0.00	05-01-02	02-06-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	33.50 / 23.34		
	2	PB30 PIGGYBACK	10.00 / 0.00	05-11-07	02-10-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	38.60 / 24.34		
	6	J1 JACK-OPEN	10.00 / 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-05-00 / 00-00-00	01-07-11 / 05-08-07	110.28 / 73.02		
	2	J3 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 03-01-01	01-07-11 / 00-03-08	26.32 / 18.00		
	2	J4 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 01-01-01	01-07-11 / 00-03-08	33.16 / 21.66		
	2	J5 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	16.74 / 12.00		
	2	J6 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	25.52 / 16.66		
	1	J7 JACK-OPEN	10.00 / 0.00	05-04-08	06-01-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-06-04	19.51 / 12.83		
	2	J9 JACK-OPEN	10.00 / 0.00	01-03-00	02-08-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-08-03	13.72 / 10.66		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		

TOTAL # TRUSS= **91.00** TOTAL BFT OF ALL TRUSSES= **3777.96** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5920.63** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
16	Hangers	LJS26DS	
1	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **21.00**

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DATE	03/01/18
SALES REP	Mario

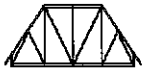
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287567 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1SZ HIP GIRDER	10.00 / 0.00	29-11-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	339.06 / 214.00		
	1	T2S HIP	10.00 / 0.00	29-11-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	146.74 / 93.33		
	1	T3S HIP	10.00 / 0.00	29-11-00	09-00-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	163.81 / 105.83		
	1	T4S ROOF	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	167.96 / 108.16		
	1 / 2 Ply	T5S PIGGYBACK	10.00 / 0.00	29-11-00	10-08-07	2 X 4	2 X 6	00-00-00 / 01-03-08	01-07-11 / 01-07-11	361.26 / 227.34		
	4	T6 PIGGYBACK	10.00 / 0.00	27-11-00	10-08-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	579.92 / 365.32		
	2	T6SA PIGGYBACK	10.00 / 0.00	27-06-00	10-08-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 00-11-14	288.68 / 182.00		
	1	T7 HIP	10.00 / 0.00	27-11-00	11-01-14	2 X 4	2 X 4	01-03-07 / 01-03-08	01-07-11 / 01-07-11	147.68 / 91.66		
	1	T8 HIP	10.00 / 0.00	27-11-00	09-05-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.88 / 89.50		
	1	T9 HIP	10.00 / 0.00	27-11-00	07-09-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	126.81 / 80.67		
	1 / 2 Ply	T10Z PIGGYBACK	10.00 / 0.00	27-11-00	10-00-00	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	319.78 / 201.00		
	1	T16 HIP GIRDER	10.00 / 0.00	17-02-00	05-08-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.65 / 58.83		
	1	T17 HIP	10.00 / 0.00	17-02-00	07-04-07	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	84.65 / 54.00		
	2	T19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	116.22 / 74.00		
	1	G19 COMMON	10.00 / 0.00	12-00-00	06-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	58.38 / 38.00		
	1 / 2 Ply	T21S HALF HIP	10.00 / 0.00	05-04-08	03-05-06	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-05-06	57.80 / 42.66		
	1	T27 HIP GIRDER	9.00 / 0.00	18-06-08	02-01-02	2 X 4	2 X 6	01-03-08 / 00-00-00	00-11-00 / 00-05-08	90.84 / 57.33		
	1 / 3 Ply	T30 COMMON	10.00 / 0.00	18-00-00	09-01-11	2 X 4	2 X 6	01-05-00 / 01-05-00	01-07-11 / 01-07-11	298.71 / 189.51		

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DATE	03/01/18
SALES REP	Mario

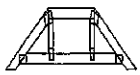
TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 **LAYOUT ID:** 287567 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-7C **ELEVATION:** B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G30 COMMON	10.00 / 0.00	18-00-00	09-01-11	2 X 4	2 X 4	01-05-00 / 01-05-00	01-07-11 / 01-07-11	91.88 / 58.50		
	1	T31 HIP	10.00 / 0.00	26-05-00	04-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	106.80 / 69.33		
	1	T32 HIP	10.00 / 0.00	26-05-00	06-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	122.02 / 78.67		
	1	T33 HIP	10.00 / 0.00	26-05-00	07-11-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	123.54 / 78.67		
	3	T34 PIGGYBACK	10.00 / 0.00	26-05-00	09-07-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.19 / 252.00		
	1	T35 HIP	10.00 / 0.00	26-05-00	11-03-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	144.68 / 91.00		
	1	T36 COMMON	10.00 / 0.00	12-03-08	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 05-08-07	64.57 / 42.50		
	1	G36 COMMON	10.00 / 0.00	12-03-08	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 05-08-07	66.54 / 43.50		
	1	T36A COMMON	10.00 / 0.00	11-05-00	08-10-06	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	65.31 / 43.17		
	4	T37 MONOPITCH	10.00 / 0.00	11-05-00	11-03-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-09-06 / 00-03-08	246.80 / 154.68		
	1	G38 COMMON	10.00 / 0.00	12-02-00	06-08-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	59.20 / 39.17		
	2	T39 COMMON	10.00 / 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	81.92 / 53.66		
	1	G39 COMMON	10.00 / 0.00	08-10-00	05-03-14	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	42.96 / 28.83		
	1 / 2 Ply	T101Z HALF HIP	10.00 / 0.00	05-03-08	02-08-03	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 02-08-03	51.28 / 35.66		
	2	T110S ROOF	10.00 / 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	136.12 / 92.66		
	1	G110 COMMON	10.00 / 0.00	14-00-00	07-05-11	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	68.43 / 44.50		
	1 / 2 Ply	PB1 PIGGYBACK	10.00 / 0.00	04-10-14	01-06-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	31.14 / 20.66		
	5	PB2 PIGGYBACK	10.00 / 0.00	04-10-14	02-05-05	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	80.95 / 55.00		

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DATE	03/01/18
SALES REP	Mario

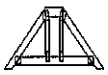
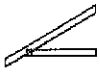
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287567 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: B-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	PB3 PIGGYBACK	10.00 / 0.00	04-10-14	01-11-15	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	16.67 / 12.00		
	1 / 2 Ply	PB4 PIGGYBACK	10.00 / 0.00	05-01-02	02-06-04	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	33.50 / 23.34		
	2	PB30 PIGGYBACK	10.00 / 0.00	05-11-07	02-10-09	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	38.60 / 24.34		
	6	J1 JACK-OPEN	10.00 / 0.00	04-10-08	05-08-07	2 X 4	2 X 4	01-05-00 / 00-00-00	01-07-11 / 05-08-07	110.28 / 73.02		
	2	J3 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 03-01-01	01-07-11 / 00-03-08	26.32 / 18.00		
	2	J4 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 01-01-01	01-07-11 / 00-03-08	33.16 / 21.66		
	2	J5 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	16.74 / 12.00		
	2	J6 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	25.52 / 16.66		
	1	J7 JACK-OPEN	10.00 / 0.00	05-04-08	06-01-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 00-06-04	19.51 / 12.83		
	2	J9 JACK-OPEN	10.00 / 0.00	01-03-00	02-08-03	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 02-08-03	13.72 / 10.66		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		

TOTAL # TRUSS= **91.00** TOTAL BFT OF ALL TRUSSES= **3863.78** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6021.72** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
14	Hangers	LJS26DS	
1	Hangers	LUS24-2	
1	Hangers	LUS26-2	

TOTAL # ITEMS= **18.00**

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DATE	03/02/18
SALES REP	Mario

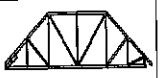
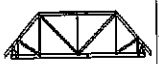
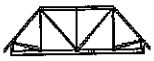
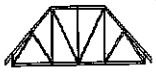
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287571 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T41 HIP GIRDER	8.00 / 0.00	29-11-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	291.12 / 178.34		
	1	T42 HIP	8.00 / 0.00	29-11-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	124.28 / 79.83		
	1	T43 ROOF	8.00 / 0.00	29-11-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	150.20 / 94.83		
	1 / 2 Ply	T44 ROOF	8.00 / 0.00	29-11-00	09-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	335.68 / 208.66		
	1	T45A HIP	8.00 / 0.00	28-06-00	09-11-13	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 01-08-02	129.21 / 81.67		
	4	T46 COMMON	8.00 / 0.00	28-06-00	10-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 01-08-02	505.64 / 318.00		
	2	T46A COMMON	8.00 / 0.00	28-06-00	10-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 01-08-02	253.46 / 159.00		
	1	T47 HIP	8.00 / 0.00	28-11-00	09-03-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	127.96 / 79.34		
	1	T48 HIP	8.00 / 0.00	28-11-00	07-11-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	131.76 / 83.17		
	1	T49 HIP	8.00 / 0.00	28-11-00	06-07-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	122.79 / 78.50		
	1 / 2 Ply	T50 HIP GIRDER	8.00 / 0.00	27-11-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	259.50 / 159.66		
	1 / 2 Ply	T51 HIP GIRDER	8.00 / 0.00	26-05-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	242.20 / 147.00		
	1	T52 HIP	8.00 / 0.00	26-05-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	114.31 / 73.17		
	1	T53 HIP	8.00 / 0.00	26-05-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	120.92 / 76.67		
	1	T54 HIP	8.00 / 0.00	26-05-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	122.91 / 77.00		
	1	T55 COMMON	8.00 / 0.00	26-05-00	10-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	112.71 / 70.33		
	1	T56 HIP GIRDER	8.00 / 0.00	17-02-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	85.10 / 53.00		
	1	T57 HIP	8.00 / 0.00	17-02-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	76.15 / 49.67		

SITE COPY

DATE	03/02/18
SALES REP	Mario

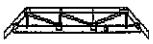
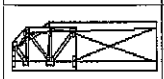
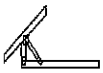
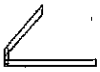
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 LAYOUT ID: 287571 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T58 COMMON	8.00 / 0.00	17-02-00	07-01-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	152.08 / 97.34		
	1 / 2 Ply	T59 HALF HIP	8.00 / 0.00	06-04-08	02-10-02	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-13 / 02-10-02	60.30 / 40.66		
	1	T60 HIP GIRDER	8.00 / 0.00	19-00-00	02-01-02	2 X 6	2 X 6	01-03-08 / 01-03-08	00-09-10 / 00-09-10	92.97 / 58.83		
	1 / 2 Ply	T200 HALF HIP	8.00 / 0.00	05-10-08	02-05-14	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-13 / 02-02-13	55.52 / 37.34		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		
	12	J41 JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	213.96 / 128.04		
	4	J42 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 04-01-01	01-04-13 / 00-03-08	53.88 / 33.32		
	4	J43 JACK-OPEN	8.00 / 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 / 02-01-01	01-04-13 / 00-03-08	66.84 / 43.32		
	5	J44 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	01-04-13 / 00-03-08	38.35 / 23.35		
	5	J45 JACK-OPEN	8.00 / 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	01-04-13 / 00-05-03	58.55 / 38.35		
	4	J46 JACK-OPEN	8.00 / 0.00	06-10-08	05-03-13	2 X 4	2 X 6	00-00-00 / 00-00-00	00-08-13 / 05-03-13	89.68 / 54.68		
	1	J47 JACK-OPEN	8.00 / 0.00	06-10-08	02-07-02	2 X 4	2 X 4	00-00-00 / -04-01-01	00-08-13 / 00-03-08	12.20 / 8.00		
	1	J48 JACK-OPEN	8.00 / 0.00	06-10-08	03-11-02	2 X 4	2 X 4	00-00-00 / -02-01-01	00-08-13 / 00-03-08	14.95 / 9.33		
	2	J49 JACK-OPEN	8.00 / 0.00	01-03-00	02-02-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-02-13	12.44 / 9.34		

TOTAL # TRUSS= **80.00** TOTAL BFT OF ALL TRUSSES= **2733.71** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4354.16** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

SITE COPY

DATE	03/01/18
SALES REP	Mario

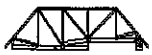
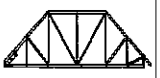
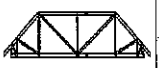
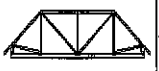
TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 **LAYOUT ID:** 287568 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S38-7C **ELEVATION:** C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T41S HIP GIRDER	8.00 / 0.00	29-05-08	05-03-13	2 X 4	2 X 6	01-03-08 / 01-09-00	01-04-13 / 01-04-13	304.52 / 190.34		
	1	T42S HIP	8.00 / 0.00	29-11-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	139.38 / 90.17		
	1	T43S ROOF	8.00 / 0.00	29-11-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	158.56 / 101.00		
	1 / 2 Ply	T44S ROOF	8.00 / 0.00	29-11-00	09-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	343.48 / 217.00		
	1	T45S HIP	8.00 / 0.00	28-06-00	09-11-13	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 00-08-02	134.89 / 84.67		
	4	T46 COMMON	8.00 / 0.00	28-06-00	10-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 01-08-02	505.64 / 318.00		
	2	T46S ROOF	8.00 / 0.00	28-06-00	10-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-08-13 / 00-08-02	272.90 / 169.34		
	1	T47 HIP	8.00 / 0.00	28-11-00	09-03-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	127.96 / 79.34		
	1	T48 HIP	8.00 / 0.00	28-11-00	07-11-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	131.76 / 83.17		
	1	T49 HIP	8.00 / 0.00	28-11-00	06-07-13	2 X 4	2 X 4	00-00-00 / 01-03-08	00-08-13 / 01-04-13	122.79 / 78.50		
	1 / 2 Ply	T50 HIP GIRDER	8.00 / 0.00	27-11-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	259.50 / 159.66		
	1 / 2 Ply	T51 HIP GIRDER	8.00 / 0.00	26-05-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	242.20 / 147.00		
	1	T52 HIP	8.00 / 0.00	26-05-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	114.31 / 73.17		
	1	T53 HIP	8.00 / 0.00	26-05-00	07-11-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	120.92 / 76.67		
	1	T54 HIP	8.00 / 0.00	26-05-00	09-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	122.91 / 77.00		
	1	T55 COMMON	8.00 / 0.00	26-05-00	10-02-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	112.71 / 70.33		
	1	T56 HIP GIRDER	8.00 / 0.00	17-02-00	05-03-13	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	85.10 / 53.00		
	1	T57 HIP	8.00 / 0.00	17-02-00	06-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	76.15 / 49.67		

SITE COPY

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

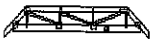
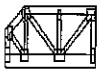
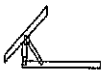
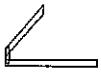
Delivery Shiplist

DATE	03/01/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287568 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S38-7C ELEVATION: C-OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	T58 COMMON	8.00 / 0.00	17-02-00	07-01-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	152.08 / 97.34		
	1 / 2 Ply	T59S HALF HIP	8.00 / 0.00	06-04-08	02-10-02	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-13 / 01-10-02	61.34 / 42.66		
	1	T60 HIP GIRDER	8.00 / 0.00	19-00-00	02-01-02	2 X 6	2 X 6	01-03-08 / 01-03-08	00-09-10 / 00-09-10	92.97 / 58.83		
	1 / 2 Ply	T200 HALF HIP	8.00 / 0.00	05-10-08	02-02-13	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-13 / 02-02-13	55.52 / 37.34		
	9	J10 JACK-OPEN	4.00 / 0.00	05-03-08	02-01-02	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-02	126.54 / 83.97		
	8	J41 JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-03-13	142.64 / 85.36		
	6	J41S JACK-OPEN	8.00 / 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 04-03-13	142.14 / 96.00		
	3	J42 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 04-01-01	01-04-13 / 00-03-08	40.41 / 24.99		
	3	J43 JACK-OPEN	8.00 / 0.00	03-09-07	03-11-02	2 X 4	2 X 4	01-03-08 / 02-01-01	01-04-13 / 00-03-08	50.13 / 32.49		
	4	J44 JACK-OPEN	8.00 / 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 / 00-01-01	01-04-13 / 00-03-08	30.68 / 18.68		
	4	J45 JACK-OPEN	8.00 / 0.00	01-10-08	03-11-02	2 X 4	2 X 4	01-03-08 / 01-10-15	01-04-13 / 00-05-03	46.84 / 30.68		
	4	J46 JACK-OPEN	8.00 / 0.00	06-10-08	05-03-13	2 X 4	2 X 6	00-00-00 / 00-00-00	00-08-13 / 05-03-13	89.68 / 54.68		
	1	J47 JACK-OPEN	8.00 / 0.00	06-10-08	02-07-02	2 X 4	2 X 4	00-00-00 / -04-01-01	00-08-13 / 00-03-08	12.20 / 8.00		
	1	J48 JACK-OPEN	8.00 / 0.00	06-10-08	03-11-02	2 X 4	2 X 4	00-00-00 / -02-01-01	00-08-13 / 00-03-08	14.95 / 9.33		
	2	J49 JACK-OPEN	8.00 / 0.00	01-03-00	02-02-13	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 02-02-13	12.44 / 9.34		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **2807.72** BFT. TOTAL WEIGHT OF ALL TRUSSES= **4446.24** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **11.00**

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Fri Mar 2 11:42:28 2018 Page 1
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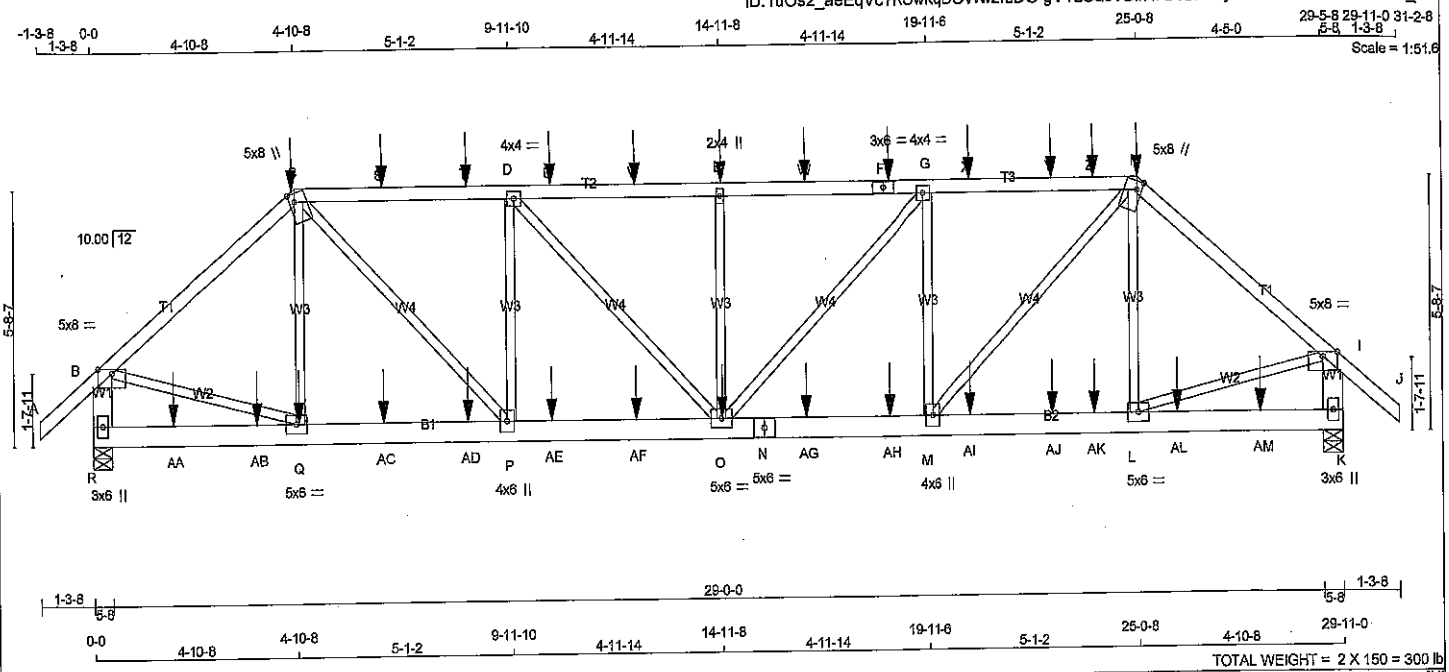

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
Q- C	1	6	SIDE(21.5)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	8.0	2.25	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.25	1.50
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3652	0	3652	0	0	5-8	5-8
K	3601	0	3601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2890	1806 / 0	549 / 0	0 / 0	0 / 0	535 / 0	0 / 0
K	2880	1749 / 0	581 / 0	0 / 0	0 / 0	549 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-3819 / 0	-104.9	-104.9	0.35 (1)	4.52
C- S	-4609 / 0	-104.9	-104.9	0.50 (1)	3.96
S- T	-4609 / 0	-104.9	-104.9	0.50 (1)	3.96
T- D	-4609 / 0	-104.9	-104.9	0.50 (1)	3.96
D- U	-5088 / 0	-104.9	-104.9	0.52 (1)	3.76
U- V	-5088 / 0	-104.9	-104.9	0.52 (1)	3.76
V- E	-5088 / 0	-104.9	-104.9	0.52 (1)	3.76
E- W	-5088 / 0	-104.9	-104.9	0.55 (1)	3.72
W- F	-5088 / 0	-104.9	-104.9	0.55 (1)	3.72
F- G	-5088 / 0	-104.9	-104.9	0.55 (1)	3.72
G- X	-4598 / 0	-104.9	-104.9	0.52 (1)	3.92
X- Y	-4598 / 0	-104.9	-104.9	0.52 (1)	3.92
Y- Z	-4598 / 0	-104.9	-104.9	0.52 (1)	3.92
Z- H	-4598 / 0	-104.9	-104.9	0.52 (1)	3.92
H- I	-3760 / 0	-104.9	-104.9	0.35 (1)	4.55
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
R- B	-3572 / 0	0.0	0.0	0.13 (1)	7.42
K- I	-3523 / 0	0.0	0.0	0.13 (1)	7.46
R-AA	0 / 0	-28.0	-28.0	0.07 (3)	10.00
AA-AB	0 / 0	-28.0	-28.0	0.07 (3)	10.00
AB- Q	0 / 0	-28.0	-28.0	0.07 (3)	10.00
Q-AC	0 / 2919	-28.0	-28.0	0.23 (1)	10.00
AC-AD	0 / 2919	-28.0	-28.0	0.23 (1)	10.00
AD- P	0 / 2919	-28.0	-28.0	0.23 (1)	10.00
P-AE	0 / 4610	-28.0	-28.0	0.35 (1)	10.00
AE-AF	0 / 4610	-28.0	-28.0	0.35 (1)	10.00
AF- O	0 / 4610	-28.0	-28.0	0.35 (1)	10.00
O- N	0 / 4598	-28.0	-28.0	0.35 (1)	10.00
N-AG	0 / 4598	-28.0	-28.0	0.35 (1)	10.00
AG-AH	0 / 4598	-28.0	-28.0	0.35 (1)	10.00
AH- M	0 / 4598	-28.0	-28.0	0.35 (1)	10.00
M-AI	0 / 2874	-28.0	-28.0	0.23 (1)	10.00
AI-AJ	0 / 2874	-28.0	-28.0	0.23 (1)	10.00
AJ-AK	0 / 2874	-28.0	-28.0	0.23 (1)	10.00
AK- L	0 / 2874	-28.0	-28.0	0.23 (1)	10.00
L-AL	0 / 0	-28.0	-28.0	0.08 (3)	10.00
AL-AM	0 / 0	-28.0	-28.0	0.08 (3)	10.00
AM- K	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-429 / 205	0.10 (1)
C- P	0 / 2442	0.30 (1)
P- D	-1521 / 0	0.36 (1)
D- O	0 / 700	0.09 (1)
O- E	-822 / 0	0.20 (1)
O- G	0 / 717	0.09 (1)
M- G	-1554 / 0	0.37 (1)
M- H	0 / 2489	0.31 (1)
L- H	-426 / 168	0.10 (1)
B- Q	0 / 3014	0.37 (1)
L- I	0 / 2988	0.37 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.55/1.00 (E-G:1) , BC=0.35/1.00 (O-P:1) , WB=0.37/1.00 (B-Q:1) , SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.38 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 2018 S. KATSOULAKOS PROVINCE OF ONTARIO

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DWG NO. TAM 11663-18 Pb 1/2

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 376.4 lbs FACTORED DOWN AT 25-0-8, 550.1 lbs FACTORED DOWN AT 4-10-8, 150.8 lbs FACTORED DOWN AT 6-11-4, 150.8 lbs FACTORED DOWN AT 8-11-4, 150.8 lbs FACTORED DOWN AT 10-11-4, 150.8 lbs FACTORED DOWN AT 12-11-4, 150.8 lbs FACTORED DOWN AT 14-11-12, 150.8 lbs FACTORED DOWN AT 16-11-12, 142.1 lbs FACTORED DOWN AT 18-11-12, 142.1 lbs FACTORED DOWN AT 20-11-12, AND 142.1 lbs FACTORED DOWN AT 22-11-12, AND 142.1 lbs FACTORED DOWN AT 23-11-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-11-4, 70.4 lbs FACTORED DOWN AT 3-11-4, 70.4 lbs FACTORED DOWN AT 4-11-4, 70.4 lbs FACTORED DOWN AT 6-11-4, 70.4 lbs FACTORED DOWN AT 8-11-4, 70.4 lbs FACTORED DOWN AT 10-11-4, 70.4 lbs FACTORED DOWN AT 12-11-4, 70.4 lbs FACTORED DOWN AT 14-11-12, 70.4 lbs FACTORED DOWN AT 16-11-12, 66.4 lbs FACTORED DOWN AT 18-11-12, 66.4 lbs FACTORED DOWN AT 20-11-12, 66.4 lbs FACTORED DOWN AT 22-11-12, 66.4 lbs FACTORED DOWN AT 23-11-12, AND 66.4 lbs FACTORED DOWN AT 25-11-12, AND 66.4 lbs FACTORED DOWN AT 27-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-550	-550	---	BACK	VERT	TOTAL
E	14-11-12	-151	-151	---	BACK	VERT	TOTAL
F	18-11-12	-142	-142	---	BACK	VERT	TOTAL
H	25-0-8	-376	-376	---	FRONT	VERT	TOTAL
O	14-11-12	-40	-70	---	BACK	VERT	TOTAL
Q	4-11-4	-40	-70	---	BACK	VERT	TOTAL
S	6-11-4	-151	-151	---	BACK	VERT	TOTAL
T	8-11-4	-151	-151	---	BACK	VERT	TOTAL
U	10-11-4	-151	-151	---	BACK	VERT	TOTAL
V	12-11-4	-151	-151	---	BACK	VERT	TOTAL
W	16-11-12	-151	-151	---	BACK	VERT	TOTAL
X	20-11-12	-142	-142	---	BACK	VERT	TOTAL
Y	22-11-12	-142	-142	---	BACK	VERT	TOTAL
Z	23-11-12	-142	-142	---	BACK	VERT	TOTAL
AA	1-11-4	-37	-70	---	BACK	VERT	TOTAL
AB	3-11-4	-40	-70	---	BACK	VERT	TOTAL
AC	6-11-4	-40	-70	---	BACK	VERT	TOTAL
AD	8-11-4	-40	-70	---	BACK	VERT	TOTAL
AE	10-11-4	-40	-70	---	BACK	VERT	TOTAL
AF	12-11-4	-40	-70	---	BACK	VERT	TOTAL
AG	16-11-12	-40	-70	---	BACK	VERT	TOTAL
AH	18-11-12	-38	-66	---	BACK	VERT	TOTAL
AI	20-11-12	-38	-66	---	BACK	VERT	TOTAL
AJ	22-11-12	-38	-66	---	BACK	VERT	TOTAL
AK	23-11-12	-38	-66	---	BACK	VERT	TOTAL
AL	25-11-12	-38	-66	---	BACK	VERT	TOTAL
AM	27-11-12	-38	-66	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
30016
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11683-188
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287570	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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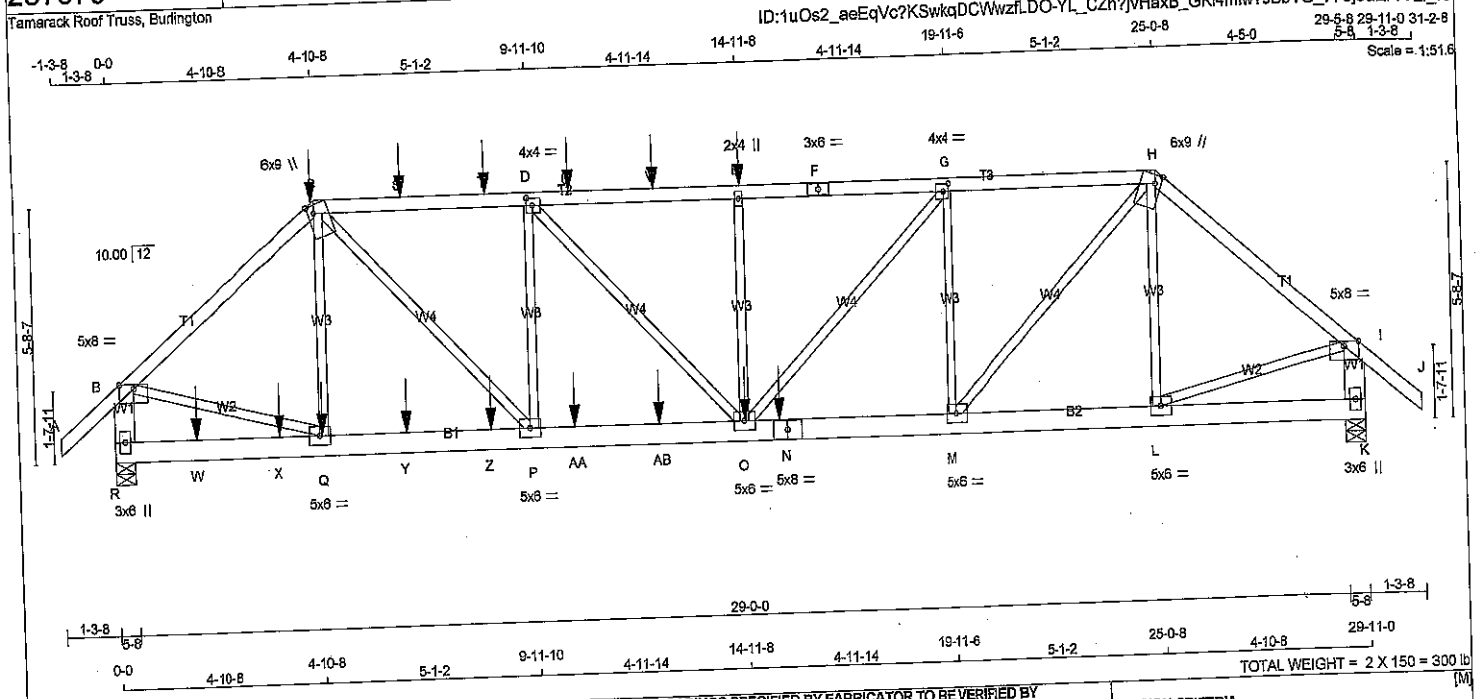

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	TOP
H- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	6	SIDE(21.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.75
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L, M, P, Q						
L	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	5.0	8.0		
O	BMWWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	4056	0	4056	0	0	5-8	5-8
K	3450	0	3450	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	3185	2031 / 0	579 / 0	0 / 0	0 / 0	576 / 0	0 / 0
K	2702	1737 / 0	462 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-4312 / 0	-104.9	-104.9	0.38 (1)	4.28
C- S	-5343 / 0	-104.9	-104.9	0.55 (1)	3.65
S- T	-5343 / 0	-104.9	-104.9	0.55 (1)	3.65
T- D	-5343 / 0	-104.9	-104.9	0.55 (1)	3.65
D- U	-6294 / 0	-104.9	-104.9	0.61 (1)	3.33
U- V	-6294 / 0	-104.9	-104.9	0.61 (1)	3.33
V- E	-6294 / 0	-104.9	-104.9	0.61 (1)	3.33
E- F	-6294 / 0	-104.9	-104.9	0.40 (1)	3.60
F- G	-6294 / 0	-104.9	-104.9	0.40 (1)	3.60
G- H	-5045 / 0	-104.9	-104.9	0.33 (1)	4.03
H- I	-3590 / 0	-104.9	-104.9	0.34 (1)	4.64
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
R- B	-3981 / 0	0.0	0.0	0.15 (1)	7.12
K- I	-3382 / 0	0.0	0.0	0.12 (1)	7.58
R- W	0 / 0	-28.0	-28.0	0.08 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.08 (2)	10.00
X- Q	0 / 0	-28.0	-28.0	0.08 (2)	10.00
Q- Y	0 / 3296	-28.0	-28.0	0.27 (1)	10.00
Y- Z	0 / 3296	-28.0	-28.0	0.27 (1)	10.00
Z- P	0 / 3296	-28.0	-28.0	0.27 (1)	10.00
P-AA	0 / 5344	-28.0	-28.0	0.51 (1)	10.00
AA-AB	0 / 5344	-28.0	-28.0	0.51 (1)	10.00
AB- O	0 / 5344	-28.0	-28.0	0.51 (1)	10.00
O- N	0 / 5045	-28.0	-28.0	0.68 (1)	10.00
N- M	0 / 5045	-28.0	-28.0	0.68 (1)	10.00
M- L	0 / 2741	-28.0	-28.0	0.28 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.04 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-484 / 193	0.12 (1)
C- P	0 / 2957	0.37 (1)
P- D	-2041 / 0	0.49 (1)
D- O	0 / 1391	0.17 (1)
O- E	-723 / 0	0.17 (1)
O- G	0 / 1828	0.23 (1)
M- G	-1903 / 0	0.45 (1)
M- H	0 / 3327	0.41 (1)
L- H	-589 / 0	0.14 (1)
B- Q	0 / 3403	0.42 (1)
L- I	0 / 2834	0.35 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-550	-550	---	BACK	VERT	TOTAL
E	14-11-12	-151	-151	---	BACK	VERT	TOTAL
N	15-9-8	-1615	-1615	---	BACK	VERT	TOTAL
O	14-11-12	-40	-70	---	BACK	VERT	TOTAL
Q	4-11-4	-40	-70	---	BACK	VERT	TOTAL
S	6-11-4	-151	-151	---	BACK	VERT	TOTAL
T	8-11-4	-151	-151	---	BACK	VERT	TOTAL
U	10-11-4	-151	-151	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.61/1.00 (D-E:1) , BC=0.68/1.00 (M-O:1) , WB=0.49/1.00 (D-P:1) , SSI=0.46/1.00 (M-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (O) (INPUT = 0.90)
SSI METAL= 0.78 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11711 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287570	T1Z	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 550.1 lbs FACTORED DOWN AT 4-10-8, 150.8 lbs FACTORED DOWN AT 6-11-4, 150.8 lbs FACTORED DOWN AT 8-11-4, 150.8 lbs FACTORED DOWN AT 10-11-4, AND 150.8 lbs FACTORED DOWN AT 12-11-4, AND 150.8 lbs FACTORED DOWN AT 14-11-12 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-11-4, 70.4 lbs FACTORED DOWN AT 3-11-4, 70.4 lbs FACTORED DOWN AT 4-11-4, 70.4 lbs FACTORED DOWN AT 6-11-4, 70.4 lbs FACTORED DOWN AT 8-11-4, 70.4 lbs FACTORED DOWN AT 10-11-4, 70.4 lbs FACTORED DOWN AT 12-11-4, AND 70.4 lbs FACTORED DOWN AT 14-11-12, AND 1614.9 lbs FACTORED DOWN AT 15-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
V	12-11-4	-151	-151	—	BACK	VERT	TOTAL
W	1-11-4	-37	-70	—	BACK	VERT	TOTAL
X	3-11-4	-40	-70	—	BACK	VERT	TOTAL
Y	6-11-4	-40	-70	—	BACK	VERT	TOTAL
Z	8-11-4	-40	-70	—	BACK	VERT	TOTAL
AA	10-11-4	-40	-70	—	BACK	VERT	TOTAL
AB	12-11-4	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3-02-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11711 -18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T1S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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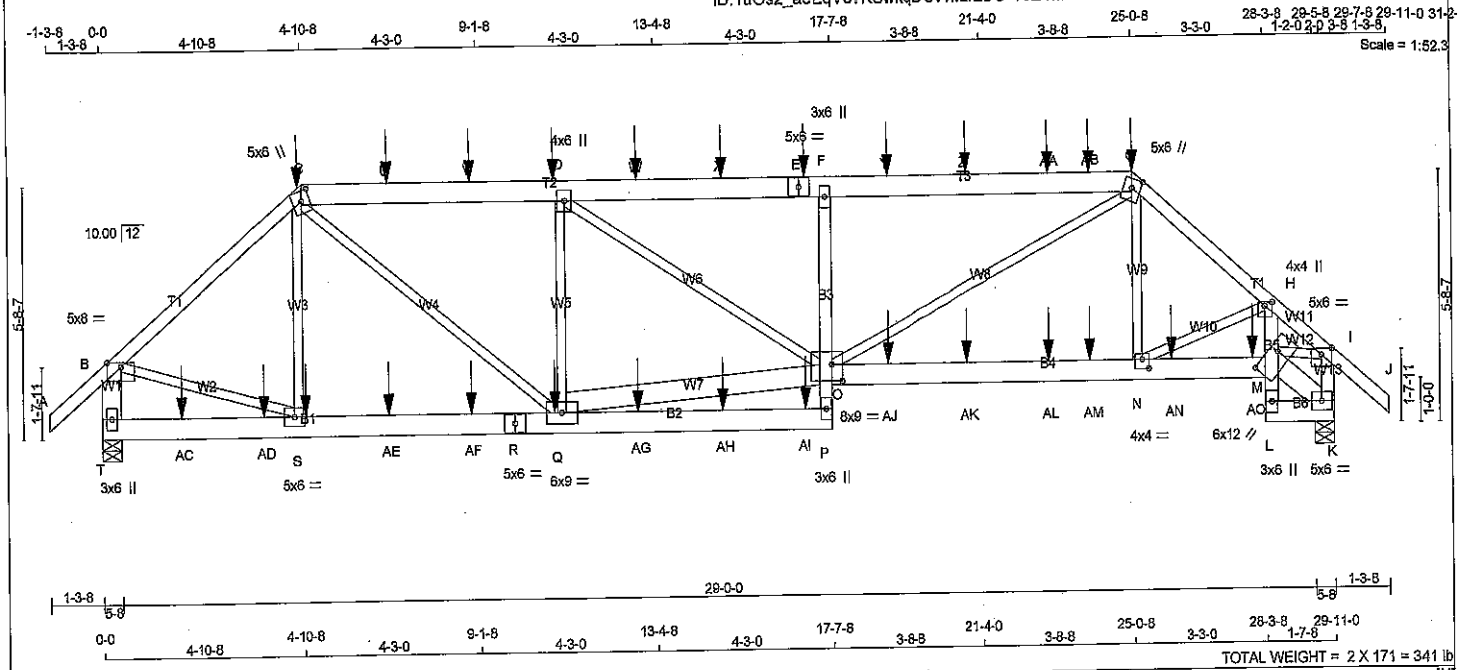

TOTAL WEIGHT = 2 X 171 = 341 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - O	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
G- J	1	12	SIDE(61.0)
K- I	1	12	TOP
C- E	2	12	SIDE(183.1)
E- G	2	12	SIDE(61.0)
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(183.1)
O- M	2	12	SIDE(0.0)
L- K	2	12	TOP
P- F	1	12	TOP
H- L	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
S- C	1	6	SIDE(21.5)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.75	2.50
D	TMVW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.50	2.50
H	TMVW+p	MT20	4.0	4.0	1.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3648	0	3648	0	0	5-8	5-8
K	3584	0	3584	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2888	1803 / 0	549 / 0	0 / 0	0 / 0	535 / 0	0 / 0
K	2855	1752 / 0	565 / 0	0 / 0	0 / 0	539 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00
B- C	-3799 / 0	-104.9	-104.9	0.35 (1)	4.53
C- U	-4911 / 0	-104.9	-104.9	0.27 (1)	4.98
U- V	-4911 / 0	-104.9	-104.9	0.27 (1)	4.98
V- D	-4911 / 0	-104.9	-104.9	0.27 (1)	4.98
D- W	-6304 / 0	-104.9	-104.9	0.41 (1)	4.33
W- X	-6304 / 0	-104.9	-104.9	0.41 (1)	4.33
X- E	-6304 / 0	-104.9	-104.9	0.41 (1)	4.33
E- F	-6304 / 0	-104.9	-104.9	0.41 (1)	4.33
F- Y	-6336 / 0	-104.9	-104.9	0.48 (1)	4.24
Y- Z	-6336 / 0	-104.9	-104.9	0.48 (1)	4.24
Z-AA	-6336 / 0	-104.9	-104.9	0.48 (1)	4.24
AA-AB	-6336 / 0	-104.9	-104.9	0.48 (1)	4.24
AB- G	-6336 / 0	-104.9	-104.9	0.48 (1)	4.24
G- H	-4632 / 0	-104.9	-104.9	0.20 (1)	4.32
H- I	-4845 / 0	-104.9	-104.9	0.15 (1)	4.26
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00
T- B	-3575 / 0	0.0	0.0	0.13 (1)	7.42
K- I	-3457 / 0	0.0	0.0	0.20 (1)	6.29
T-AC	0 / 0	-28.0	-28.0	0.08 (3)	10.00
AC-AD	0 / 0	-28.0	-28.0	0.08 (3)	10.00
AD- S	0 / 0	-28.0	-28.0	0.08 (3)	10.00
S-AE	0 / 2933	-28.0	-28.0	0.24 (1)	10.00
AE-AF	0 / 2933	-28.0	-28.0	0.24 (1)	10.00
AF- R	0 / 2933	-28.0	-28.0	0.24 (1)	10.00
R- Q	0 / 2933	-28.0	-28.0	0.24 (1)	10.00
Q-AG	0 / 135	-28.0	-28.0	0.13 (3)	10.00
AG-AH	0 / 135	-28.0	-28.0	0.13 (3)	10.00
AH-AI	0 / 135	-28.0	-28.0	0.13 (3)	10.00
AI- P	0 / 135	-28.0	-28.0	0.13 (3)	10.00
P- O	0 / 249	0.0	0.0	0.11 (1)	10.00
O- F	-1463 / 0	0.0	0.0	0.19 (1)	7.81
O-AJ	0 / 3561	-28.0	-28.0	0.31 (1)	10.00
AJ-AK	0 / 3561	-28.0	-28.0	0.31 (1)	10.00
AK-AL	0 / 3561	-28.0	-28.0	0.31 (1)	10.00
AL-AM	0 / 3561	-28.0	-28.0	0.31 (1)	10.00
AM- N	0 / 3561	-28.0	-28.0	0.31 (1)	10.00
N-AN	0 / 3821	-28.0	-28.0	0.33 (1)	10.00
AN-AO	0 / 3821	-28.0	-28.0	0.33 (1)	10.00
AO- M	0 / 3821	-28.0	-28.0	0.33 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-450 / 205	0.10 (1)
C- Q	0 / 2605	0.32 (1)
Q- D	-2104 / 0	0.47 (1)
Q- O	0 / 4867	0.32 (1)
D- O	0 / 1632	0.20 (1)
O- G	0 / 3186	0.39 (1)
N- G	0 / 472	0.06 (3)
N- H	-276 / 0	0.03 (1)
B- S	0 / 3019	0.37 (1)
M- K	-170 / 0	0.01 (1)
M- I	0 / 3696	0.46 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.48/1.00 (F-G:1) , BC=0.33/1.00 (M-N:1) , WB=0.47/1.00 (D-Q:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER 30216 S. KATSOULAKOS PROVINCE OF ONTARIO

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DWG NO. TAM 11644 -18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T1S	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMVW-p	MT20	5.0	6.0	1.75	3.00
K	BMVW1-t	MT20	5.0	6.0		
L	BMV+p	MT20	3.0	6.0		
M	BVMWW-w	MT20	6.0	12.0	2.25	7.50
N	BMWW-t	MT20	4.0	4.0	2.50	2.00
O	BVMWWW-l	MT20	8.0	9.0	4.50	3.00
P	BMV+p	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 550.1 lbs FACTORED DOWN AT 4-10-8, 150.8 lbs FACTORED DOWN AT 6-11-4, 150.8 lbs FACTORED DOWN AT 8-11-4, 150.8 lbs FACTORED DOWN AT 10-11-4, 150.8 lbs FACTORED DOWN AT 12-11-4, 150.8 lbs FACTORED DOWN AT 14-11-12, 150.8 lbs FACTORED DOWN AT 16-11-12, 160.5 lbs FACTORED DOWN AT 18-11-12, 160.5 lbs FACTORED DOWN AT 20-11-12, 160.5 lbs FACTORED DOWN AT 22-11-12, AND 160.5 lbs FACTORED DOWN AT 23-11-12, AND 376.4 lbs FACTORED DOWN AT 25-0-8 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-11-4, 70.4 lbs FACTORED DOWN AT 3-11-4, 70.4 lbs FACTORED DOWN AT 4-11-4, 70.4 lbs FACTORED DOWN AT 6-11-4, 70.4 lbs FACTORED DOWN AT 8-11-4, 70.4 lbs FACTORED DOWN AT 10-11-4, 70.4 lbs FACTORED DOWN AT 12-11-4, 70.4 lbs FACTORED DOWN AT 14-11-12, 82.7 lbs FACTORED DOWN AT 16-11-12, 34.2 lbs FACTORED DOWN AT 18-11-12, 34.2 lbs FACTORED DOWN AT 20-11-12, 34.2 lbs FACTORED DOWN AT 22-11-12, 34.2 lbs FACTORED DOWN AT 23-11-12, AND 34.2 lbs FACTORED DOWN AT 25-11-12, AND 47.5 lbs FACTORED DOWN AT 27-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- M	0 / 36	0.0	0.0	0.12 (1)	10.00
M- H	0 / 56	0.0	0.0	0.12 (1)	10.00
L- K	0 / 133	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-550	-550	—	BACK	VERT	TOTAL
D	10-11-4	-151	-151	—	BACK	VERT	TOTAL
E	16-11-12	-151	-151	—	BACK	VERT	TOTAL
G	25-0-8	-376	-376	—	FRONT	VERT	TOTAL
Q	10-11-4	-40	-70	—	BACK	VERT	TOTAL
S	4-11-4	-40	-70	—	BACK	VERT	TOTAL
U	6-11-4	-151	-151	—	BACK	VERT	TOTAL
V	8-11-4	-151	-151	—	BACK	VERT	TOTAL
W	12-11-4	-151	-151	—	BACK	VERT	TOTAL
X	14-11-12	-151	-151	—	BACK	VERT	TOTAL
Y	18-11-12	-160	-160	—	BACK	VERT	TOTAL
Z	20-11-12	-160	-160	—	BACK	VERT	TOTAL
AA	22-11-12	-160	-160	—	BACK	VERT	TOTAL
AB	23-11-12	-160	-160	—	BACK	VERT	TOTAL
AC	1-11-4	-37	-70	—	BACK	VERT	TOTAL
AD	3-11-4	-40	-70	—	BACK	VERT	TOTAL
AE	6-11-4	-40	-70	—	BACK	VERT	TOTAL
AF	8-11-4	-40	-70	—	BACK	VERT	TOTAL
AG	12-11-4	-40	-70	—	BACK	VERT	TOTAL
AH	14-11-12	-40	-70	—	BACK	VERT	TOTAL
AI	16-11-12	-47	-83	—	BACK	VERT	TOTAL
AJ	18-11-12	-20	-34	—	BACK	VERT	TOTAL
AK	20-11-12	-20	-34	—	BACK	VERT	TOTAL
AL	22-11-12	-20	-34	—	BACK	VERT	TOTAL
AM	23-11-12	-20	-34	—	BACK	VERT	TOTAL
AN	25-11-12	-20	-34	—	BACK	VERT	TOTAL
AO	27-11-12	-27	-47	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.43 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM11644
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T1SZ	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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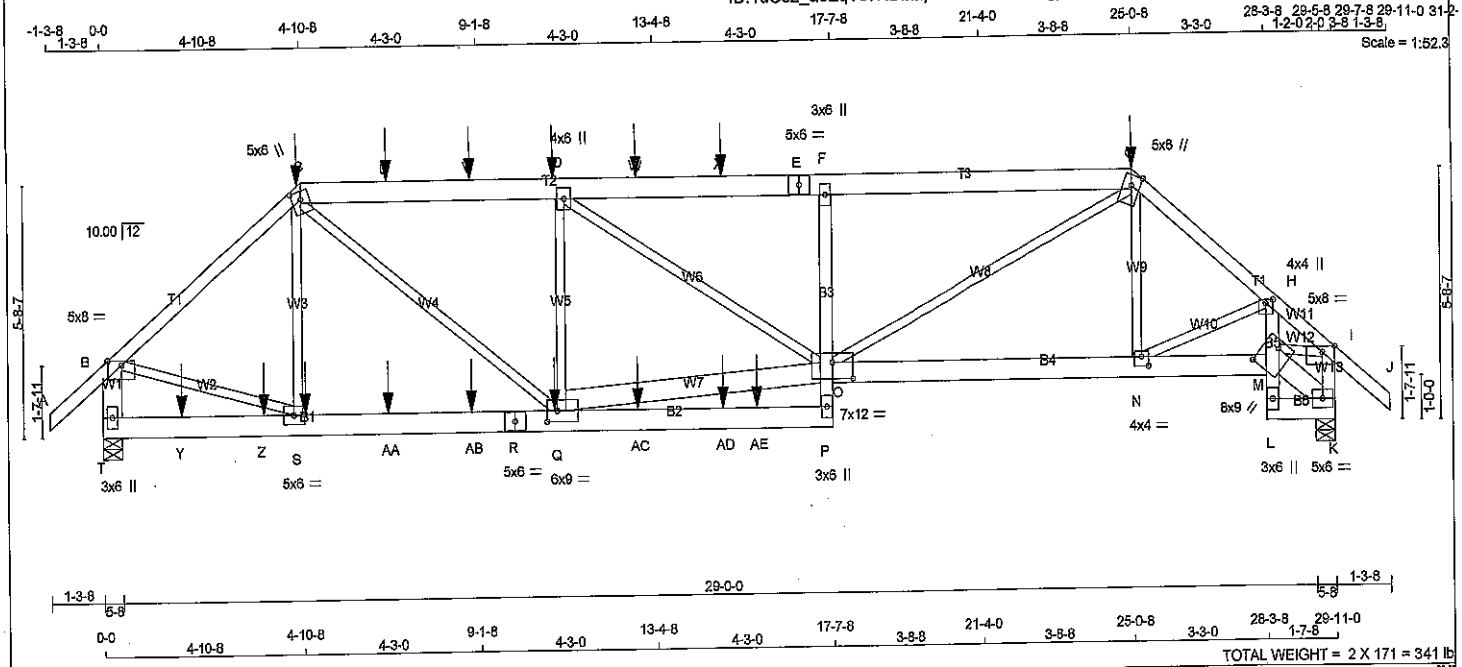

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
G- J	1	12	SIDE(61.0)
K- I	1	12	TOP
C- E	2	12	SIDE(183.1)
E- G	2	12	SIDE(61.0)
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(183.1)
O- M	2	12	TOP
L- K	2	12	TOP
P- F	1	12	TOP
H- L	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
S- C	1	6	SIDE(21.5)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	2.50
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	6.0		
G	TTWW+m	MT20	5.0	8.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.00	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4123	0	4123	0	0	5-8	5-8
K	3759	0	3759	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	3239	2064 / 0	590 / 0	0 / 0	0 / 0	586 / 0	0 / 0
K	2948	1889 / 0	530 / 0	0 / 0	0 / 0	528 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-4323 / 0	-104.9	-104.9	0.38 (1)	4.28	
C- U	-6044 / 0	-104.9	-104.9	0.30 (1)	4.54	
U- V	-6044 / 0	-104.9	-104.9	0.30 (1)	4.54	
V- D	-6044 / 0	-104.9	-104.9	0.30 (1)	4.54	
D- W	-7536 / 0	-104.9	-104.9	0.35 (1)	4.11	
W- X	-7536 / 0	-104.9	-104.9	0.35 (1)	4.11	
X- E	-7536 / 0	-104.9	-104.9	0.35 (1)	4.11	
E- F	-7536 / 0	-104.9	-104.9	0.35 (1)	4.11	
F- G	-7574 / 0	-104.9	-104.9	0.34 (1)	4.13	
G- H	-4908 / 0	-104.9	-104.9	0.21 (1)	4.20	
H- I	-5100 / 0	-104.9	-104.9	0.16 (1)	4.16	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
T- B	-4013 / 0	0.0	0.0	0.15 (1)	7.10	
K- I	-3626 / 0	0.0	0.0	0.21 (1)	6.16	
T- Y	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
Y- Z	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
Z- S	0 / 0	-28.0	-28.0	0.07 (2)	10.00	
S-AA	0 / 3342	-28.0	-28.0	0.51 (1)	10.00	
AA-AB	0 / 3342	-28.0	-28.0	0.51 (1)	10.00	
AB- R	0 / 3342	-28.0	-28.0	0.51 (1)	10.00	
R- Q	0 / 3342	-28.0	-28.0	0.51 (1)	10.00	
Q-AC	0 / 161	-28.0	-28.0	0.74 (1)	10.00	
AC-AD	0 / 161	-28.0	-28.0	0.74 (1)	10.00	
AD-AE	0 / 161	-28.0	-28.0	0.74 (1)	10.00	
AE- P	0 / 161	-28.0	-28.0	0.74 (1)	10.00	
P- O	0 / 1198	0.0	0.0	0.23 (1)	10.00	
O- F	-915 / 0	0.0	0.0	0.15 (1)	7.81	
O- N	0 / 3776	-28.0	-28.0	0.31 (1)	10.00	
N- M	0 / 4023	-28.0	-28.0	0.33 (1)	10.00	
L- M	0 / 36	0.0	0.0	0.13 (1)	10.00	
M- H	0 / 38	0.0	0.0	0.12 (1)	10.00	
L- K	0 / 140	-28.0	-28.0	0.01 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-723 / 124	0.17 (1)
C- Q	0 / 3557	0.44 (1)
Q- D	-2244 / 0	0.50 (1)
Q- O	0 / 5990	0.39 (1)
D- O	0 / 1749	0.22 (1)
O- G	0 / 4360	0.54 (1)
N- G	0 / 318	0.04 (3)
N- H	-263 / 0	0.03 (1)
B- S	0 / 3435	0.43 (1)
M- K	-179 / 0	0.01 (1)
M- I	0 / 3891	0.48 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-550	-550	---	BACK	VERT	TOTAL
D	10-11-4	-151	-151	---	BACK	VERT	TOTAL
G	25-0-8	-376	-376	---	FRONT	VERT	TOTAL
Q	10-11-4	-70	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 919 (0.39")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.74/1.00 (P-Q:1) , WB=0.54/1.00 (G-O:1) , SSI=0.40/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

LICENSED PROFESSIONAL ENGINEER 30018 S. KATSOULAKOS PROVINCE OF ONTARIO

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DWG NO. TAM 11690-B

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T1SZ	1	2	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMVW-p	MT20	5.0	8.0	Edge	3.50
K	BMVW1-t	MT20	5.0	6.0		
L	BMV+p	MT20	3.0	6.0		
M	BVMWW-w	MT20	8.0	9.0	4.00	6.75
N	BMWW-t	MT20	4.0	4.0	2.50	2.00
O	BVMWWW-l	MT20	7.0	12.0	4.50	6.00
P	BMV+p	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0	3.00	3.00
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 550.1 lbs FACTORED DOWN AT 4-10-8, 150.8 lbs FACTORED DOWN AT 6-11-4, 150.8 lbs FACTORED DOWN AT 8-11-4, 150.8 lbs FACTORED DOWN AT 10-11-4, 150.8 lbs FACTORED DOWN AT 12-11-4, AND 150.8 lbs FACTORED DOWN AT 14-11-12, AND 376.4 lbs FACTORED DOWN AT 25-0-8 ON TOP CHORD, AND 70.4 lbs FACTORED DOWN AT 1-11-4, 70.4 lbs FACTORED DOWN AT 3-11-4, 70.4 lbs FACTORED DOWN AT 4-11-4, 70.4 lbs FACTORED DOWN AT 6-11-4, 70.4 lbs FACTORED DOWN AT 8-11-4, 70.4 lbs FACTORED DOWN AT 10-11-4, 70.4 lbs FACTORED DOWN AT 12-11-4, AND 70.4 lbs FACTORED DOWN AT 14-11-12, AND 1614.7 lbs FACTORED DOWN AT 15-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	4-11-4	-40	-70	---	BACK	VERT	TOTAL
U	6-11-4	-151	-151	---	BACK	VERT	TOTAL
V	8-11-4	-151	-151	---	BACK	VERT	TOTAL
W	12-11-4	-151	-151	---	BACK	VERT	TOTAL
X	14-11-12	-151	-151	---	BACK	VERT	TOTAL
Y	1-11-4	-37	-70	---	BACK	VERT	TOTAL
Z	3-11-4	-40	-70	---	BACK	VERT	TOTAL
AA	6-11-4	-40	-70	---	BACK	VERT	TOTAL
AB	8-11-4	-40	-70	---	BACK	VERT	TOTAL
AC	12-11-4	-40	-70	---	BACK	VERT	TOTAL
AD	14-11-12	-40	-70	---	BACK	VERT	TOTAL
AE	15-9-8	-1615	-1615	---	BACK	VERT	TOTAL

JSI GRIP= 0.89 (S) (INPUT = 0.80)
JSI METAL= 0.47 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 116900 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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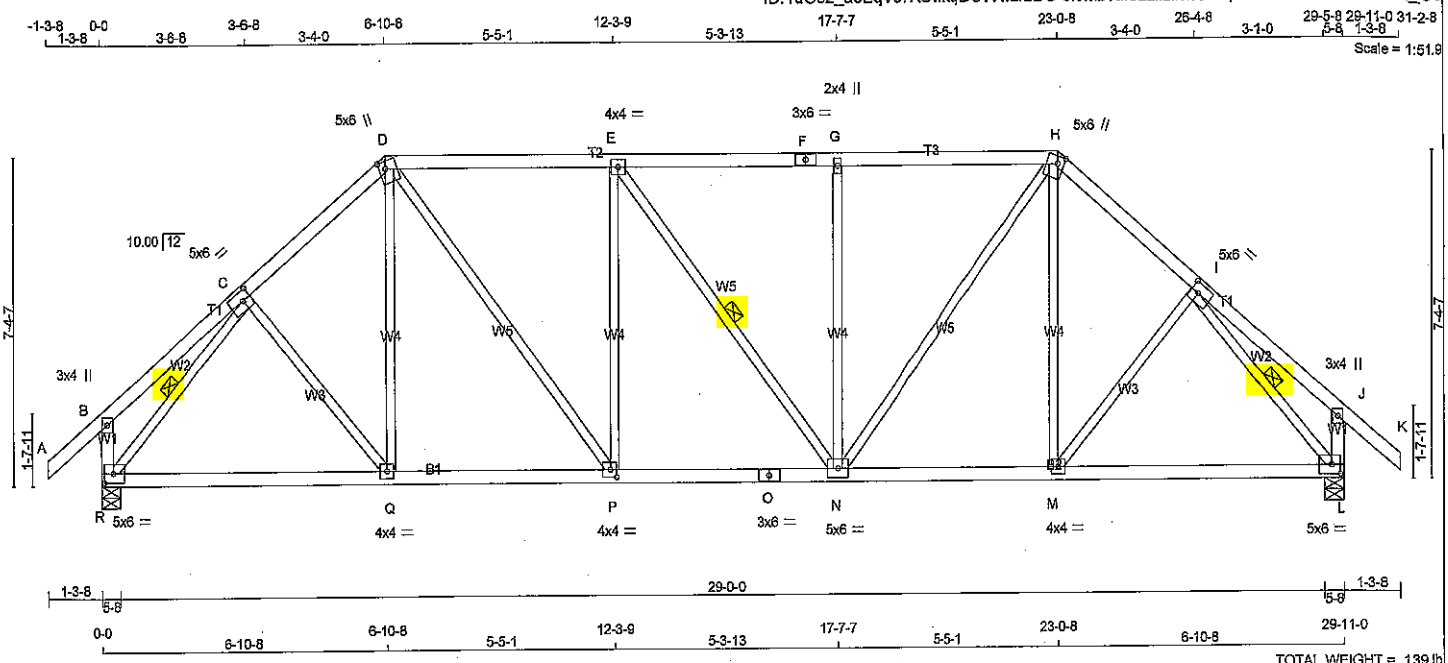

TOTAL WEIGHT = 139 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.00	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2134	0	2134	0	0	5-8	5-8
L	2134	0	2134	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
L	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, C-R, I-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	10.00
B- C	0 / 24	-104.9	-104.9	0.18 (1)	10.00	10.00
C- D	-1994 / 0	-104.9	-104.9	0.24 (1)	4.56	4.56
D- E	-2083 / 0	-104.9	-104.9	0.52 (1)	4.13	4.13
E- F	-2082 / 0	-104.9	-104.9	0.52 (1)	4.13	4.13
F- G	-2082 / 0	-104.9	-104.9	0.52 (1)	4.13	4.13
G- H	-2081 / 0	-104.9	-104.9	0.51 (1)	4.14	4.14
H- I	-1994 / 0	-104.9	-104.9	0.24 (1)	4.56	4.56
I- J	0 / 24	-104.9	-104.9	0.18 (1)	10.00	10.00
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00	10.00
R- B	-266 / 0	0.0	0.0	0.03 (1)	7.81	7.81
L- J	-266 / 0	0.0	0.0	0.03 (1)	7.81	
R- Q	0 / 1453	-28.0	-28.0	0.45 (2)	10.00	
Q- P	0 / 1511	-28.0	-28.0	0.46 (2)	10.00	
P- O	0 / 2083	-28.0	-28.0	0.41 (1)	10.00	
O- N	0 / 2083	-28.0	-28.0	0.41 (1)	10.00	
N- M	0 / 1511	-28.0	-28.0	0.46 (2)	10.00	
M- L	0 / 1453	-28.0	-28.0	0.45 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	0 / 145	0.03 (2)
Q- D	0 / 247	0.06 (3)
D- P	0 / 940	0.21 (1)
P- E	-608 / 0	0.59 (1)
E- N	-3 / 0	0.00 (1)
N- G	-607 / 0	0.59 (1)
N- H	0 / 937	0.21 (1)
M- H	0 / 248	0.06 (3)
M- I	0 / 145	0.03 (2)
R- C	-2288 / 0	0.50 (1)
I- L	-2288 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.46/1.00 (M-N:2) , WB=0.59/1.00 (E-P:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.90 (L) (INPUT = 0.90)
LSI METAL= 0.61 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11684-28
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T2S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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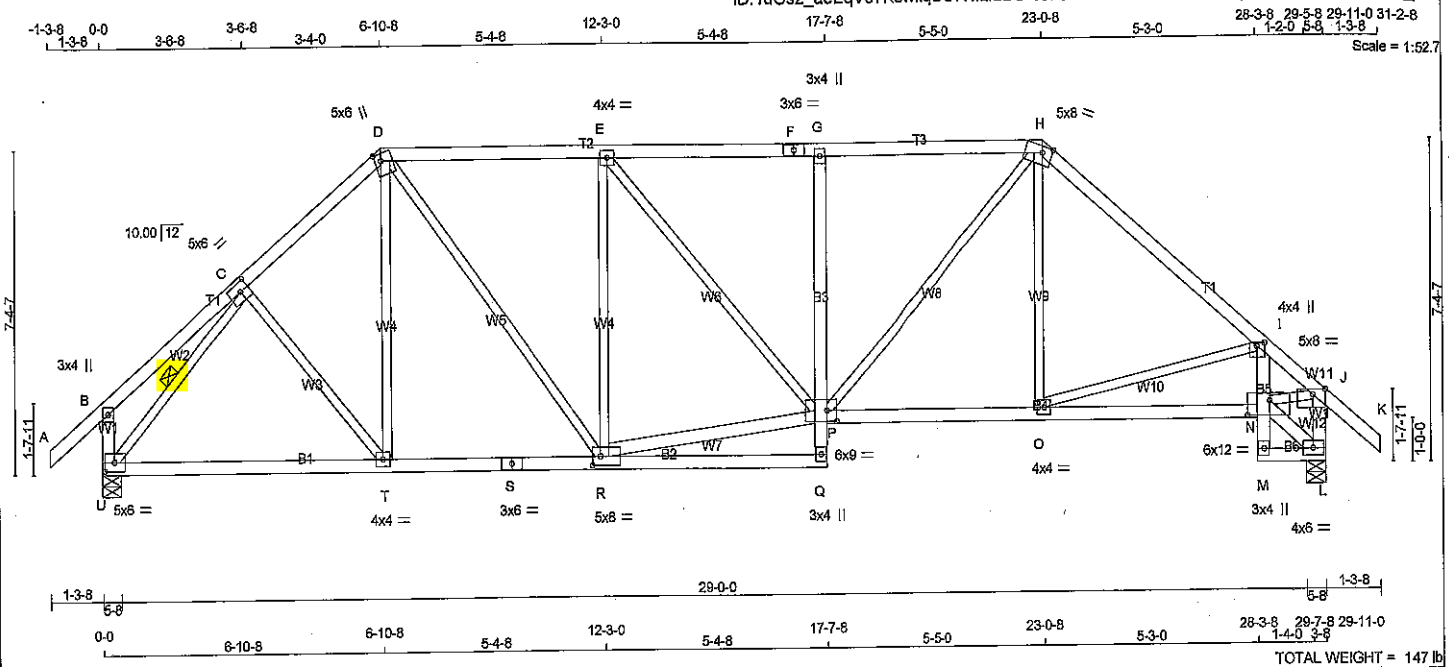

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - G	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.00
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	12.0	4.00	6.25
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-l	MT20	6.0	9.0	3.00	2.75
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.25
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2130	0	2130	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1681	1059 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
L	1665	1064 / 0	314 / 0	0 / 0	0 / 0	306 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 24	-104.9	-104.9	0.18 (1)	10.00
C- D	-1989 / 0	-104.9	-104.9	0.24 (1)	4.57
D- E	-2069 / 0	-104.9	-104.9	0.50 (1)	4.16
E- F	-2408 / 0	-104.9	-104.9	0.54 (1)	3.85
F- G	-2408 / 0	-104.9	-104.9	0.54 (1)	3.85
G- H	-2416 / 0	-104.9	-104.9	0.56 (1)	3.85
H- I	-2299 / 0	-104.9	-104.9	0.58 (1)	4.00
I- J	-2672 / 0	-104.9	-104.9	0.42 (1)	3.76
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
U- B	-286 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-2062 / 0	0.0	0.0	0.22 (1)	5.89
U- T	0 / 1450	-28.0	-28.0	0.44 (2)	10.00
T- S	0 / 1507	-28.0	-28.0	0.45 (2)	10.00
S- R	0 / 1507	-28.0	-28.0	0.45 (2)	10.00
R- Q	0 / 49	-28.0	-28.0	0.17 (2)	10.00
Q- P	0 / 108	0.0	0.0	0.08 (1)	10.00
P- G	-612 / 0	0.0	0.0	0.39 (1)	7.81
P- O	0 / 1744	-28.0	-28.0	0.44 (2)	10.00
O- N	0 / 2162	-28.0	-28.0	0.48 (1)	10.00
M- N	0 / 36	0.0	0.0	0.11 (1)	10.00
N- I	-94 / 68	0.0	0.0	0.11 (1)	7.81
M- L	0 / 69	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 145	0.03 (2)
T- D	0 / 239	0.05 (3)
D- R	0 / 935	0.21 (1)
R- E	-982 / 0	0.95 (1)
R- P	0 / 2065	0.33 (1)
E- P	0 / 506	0.11 (1)
P- H	0 / 993	0.22 (1)
O- H	0 / 351	0.08 (2)
O- I	-436 / 0	0.24 (1)
U- C	-2283 / 0	0.50 (1)
N- L	-88 / 0	0.01 (1)
N- J	0 / 2111	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.58/1.00 (H-I:1) , BC=0.48/1.00 (N-O:1) , WB=0.95/1.00 (E-R:1) , SSI=0.27/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3016
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11645-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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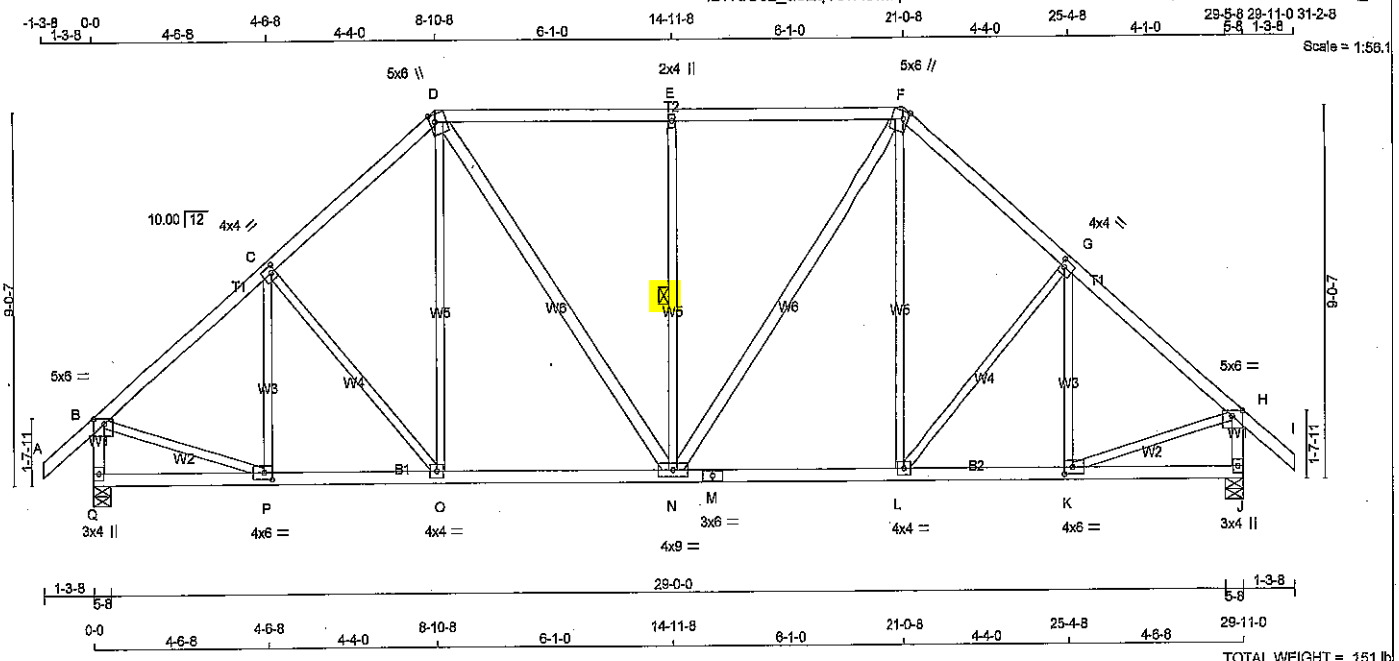

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-p	MT20	5.0	6.0	1.50	3.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.50
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2134	0	2134	0	0	5-8	5-8
J	2134	0	2134	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
J	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1997 / 0	-104.9	-104.9	0.32 (1)	4.49
C- D	-1899 / 0	-104.9	-104.9	0.31 (1)	4.59
D- E	-1764 / 0	-104.9	-104.9	0.55 (1)	4.36
E- F	-1764 / 0	-104.9	-104.9	0.55 (1)	4.36
F- G	-1899 / 0	-104.9	-104.9	0.31 (1)	4.59
G- H	-1997 / 0	-104.9	-104.9	0.32 (1)	4.49
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-2079 / 0	0.0	0.0	0.22 (1)	5.87
J- H	-2079 / 0	0.0	0.0	0.22 (1)	5.87
Q- P	0 / 0	-28.0	-28.0	0.12 (3)	10.00
P- O	0 / 1561	-28.0	-28.0	0.34 (1)	10.00
O- N	0 / 1431	-28.0	-28.0	0.37 (2)	10.00
N- M	0 / 1431	-28.0	-28.0	0.37 (2)	10.00
M- L	0 / 1431	-28.0	-28.0	0.37 (2)	10.00
L- K	0 / 1561	-28.0	-28.0	0.34 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-320 / 46	0.14 (1)
C- O	-202 / 0	0.17 (1)
O- D	0 / 342	0.08 (2)
D- N	0 / 583	0.09 (1)
N- E	-781 / 0	0.41 (1)
N- F	0 / 583	0.09 (1)
L- F	0 / 342	0.08 (2)
L- G	-202 / 0	0.17 (1)
K- G	-320 / 46	0.14 (1)
B- P	0 / 1624	0.37 (1)
K- H	0 / 1624	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.55/1.00 (D-E:1) , BC=0.37/1.00 (L-N:2) , WB=0.41/1.00 (E-N:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.39 (K) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 21685-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T3S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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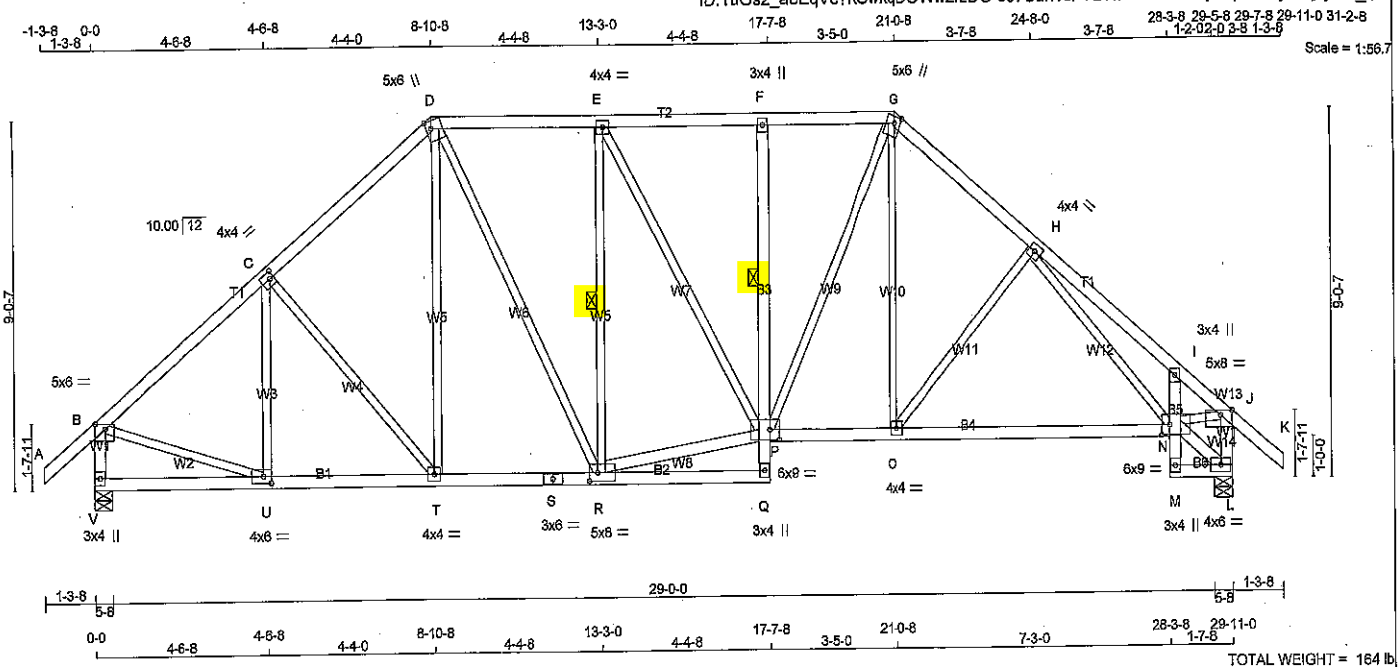

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	2.00
H	TMWW-t	MT20	4.0	4.0		
I	TMV+p	MT20	3.0	4.0		
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWWW-t	MT20	6.0	9.0	3.00	2.50
O	BMWW-t	MT20	4.0	4.0		
P	BVMWWW-t	MT20	6.0	9.0	3.00	3.00
Q	BMV+p	MT20	3.0	4.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	2.50
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMWW-t	MT20	4.0	6.0	2.00	2.50
V	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2130	0	2130	0	0	5-8	5-8
L	2137	0	2137	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1681	1059 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
L	1685	1064 / 0	314 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, E-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1995 / 0	-104.9	-104.9	0.32 (1)	4.49
C- D	-1891 / 0	-104.9	-104.9	0.31 (1)	4.60
D- E	-1699 / 0	-104.9	-104.9	0.25 (1)	4.86
E- F	-1871 / 0	-104.9	-104.9	0.25 (1)	4.68
F- G	-1875 / 0	-104.9	-104.9	0.18 (1)	4.75
G- H	-2132 / 0	-104.9	-104.9	0.24 (1)	4.46
H- I	-2264 / 0	-104.9	-104.9	0.24 (1)	4.35
I- J	-2287 / 0	-104.9	-104.9	0.16 (1)	4.40
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
V- B	-2077 / 0	0.0	0.0	0.22 (1)	5.87
L- J	-2073 / 0	0.0	0.0	0.22 (1)	5.87
V- U	0 / 0	-28.0	-28.0	0.14 (3)	10.00
U- T	0 / 1559	-28.0	-28.0	0.31 (1)	10.00
T- S	0 / 1425	-28.0	-28.0	0.30 (1)	10.00
S- R	0 / 1425	-28.0	-28.0	0.30 (1)	10.00
R- Q	0 / 30	-28.0	-28.0	0.13 (2)	10.00
Q- P	0 / 83	0.0	0.0	0.05 (1)	10.00
P- F	-417 / 0	0.0	0.0	0.06 (1)	6.25
P- O	0 / 1617	-28.0	-28.0	0.52 (2)	10.00
O- N	0 / 1836	-28.0	-28.0	0.54 (2)	10.00
M- N	0 / 36	0.0	0.0	0.09 (1)	10.00
N- I	-193 / 0	0.0	0.0	0.07 (1)	7.81
M- L	0 / 64	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-311 / 60	0.14 (1)
C- T	-209 / 0	0.18 (1)
T- D	0 / 297	0.07 (2)
D- R	0 / 619	0.14 (1)
R- E	-610 / 0	0.43 (1)
R- P	0 / 1720	0.28 (1)
E- P	0 / 348	0.08 (1)
P- G	0 / 615	0.14 (1)
O- G	0 / 486	0.11 (2)
O- H	-349 / 0	0.22 (1)
H- N	-124 / 31	0.07 (1)
B- U	0 / 1622	0.36 (1)
N- L	-77 / 0	0.01 (1)
N- J	0 / 1764	0.40 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.54/1.00 (N-O:2) , WB=0.43/1.00 (E-R:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)
SI METAL= 0.41 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11646-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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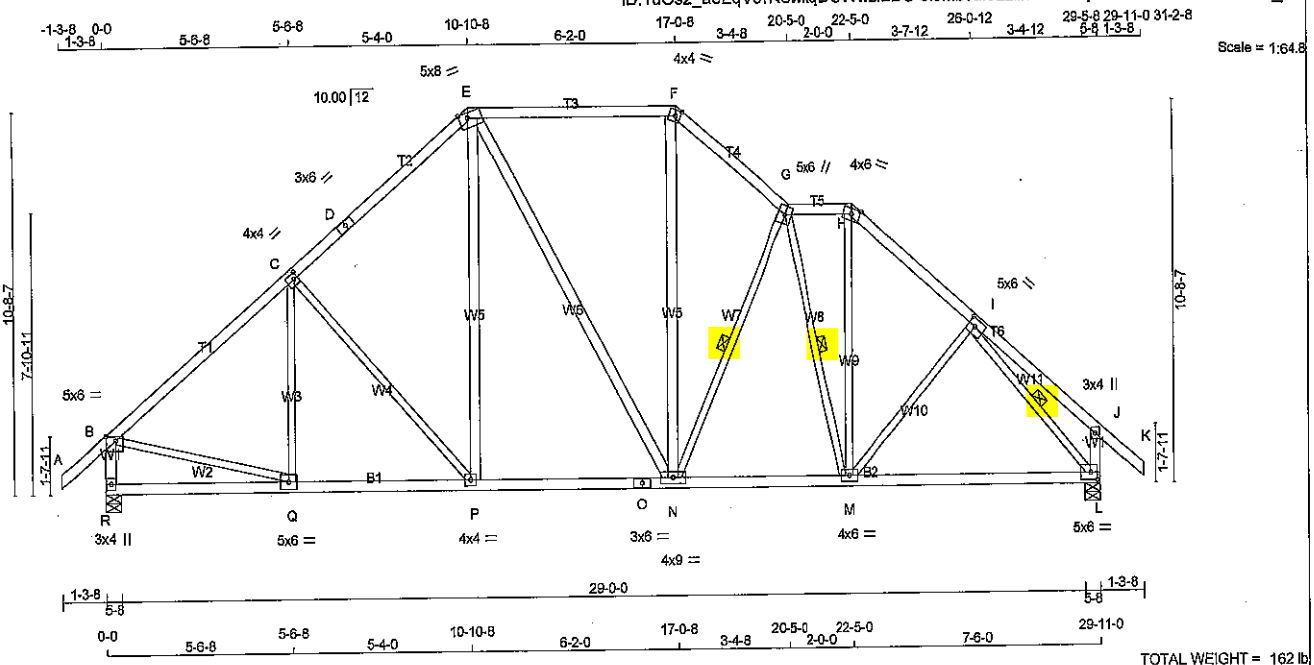

TOTAL WEIGHT = 162 lb

[MI]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TTWW+m	MT20	5.0	6.0		
H	TTW-m	MT20	4.0	6.0	Edge	
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.50
M	BMWWW-t	MT20	4.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2134	0	2134	0	0	5-8	5-8
L	2134	0	2134	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
L	1683	1062 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-N, I-L, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-2028 / 0	-104.9	-104.9	0.50 (1)	4.25
C- D	-1776 / 0	-104.9	-104.9	0.47 (1)	4.52
D- E	-1776 / 0	-104.9	-104.9	0.47 (1)	4.52
E- F	-1408 / 0	-104.9	-104.9	0.54 (1)	4.77
F- G	-1827 / 0	-104.9	-104.9	0.19 (1)	4.80
G- H	-1505 / 0	-104.9	-104.9	0.08 (1)	5.29
H- I	-1973 / 0	-104.9	-104.9	0.22 (1)	4.62
I- J	0 / 27	-104.9	-104.9	0.22 (1)	10.00
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
R- B	-2069 / 0	0.0	0.0	0.22 (1)	5.88
L- J	-299 / 0	0.0	0.0	0.03 (1)	7.81
R- Q	0 / 0	-28.0	-28.0	0.20 (3)	10.00
Q- P	0 / 1592	-28.0	-28.0	0.36 (2)	10.00
P- O	0 / 1333	-28.0	-28.0	0.33 (2)	10.00
O- N	0 / 1333	-28.0	-28.0	0.33 (2)	10.00
N- M	0 / 1703	-28.0	-28.0	0.53 (2)	10.00
M- L	0 / 1475	-28.0	-28.0	0.49 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-208 / 140	0.13 (1)
C- P	-390 / 0	0.53 (1)
P- E	0 / 464	0.07 (2)
E- N	0 / 149	0.02 (1)
N- F	0 / 677	0.11 (1)
N- G	-733 / 0	0.35 (1)
M- H	0 / 971	0.22 (1)
M- I	0 / 129	0.03 (3)
B- Q	0 / 1635	0.37 (1)
I- L	-2286 / 0	0.55 (1)
G- M	-799 / 0	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.53/1.00 (M-N:2) , WB=0.55/1.00 (I-L:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (H) (INPUT = 0.90)
SI METAL= 0.56 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11686-08
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T4S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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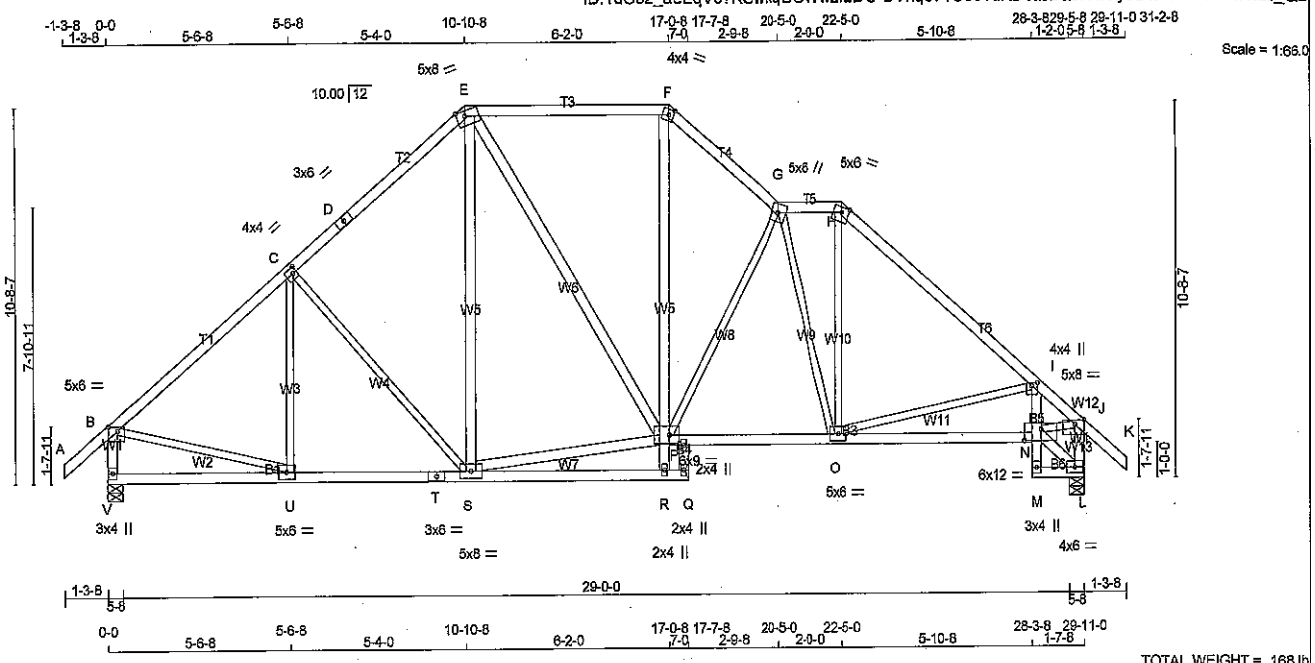

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - E	2x4	DRY	No.2	SPF
R - F	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TTW-m	MT20	4.0	4.0	2.00	1.75
G	TTWW+m	MT20	5.0	6.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	12.0	4.00	6.25
O	BMWWW-t	MT20	5.0	6.0		
P	BWMWWW*-l	MT20	6.0	9.0	3.00	3.50
Q	NP+w	MT20	2.0	4.0		
R	BMW+w	MT20	2.0	4.0		
S	BMWWW-t	MT20	5.0	8.0		
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	4.0		
W	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2140	0	2140	0	0	5-8	5-8
L	2149	0	2149	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1692	1059 / 0	321 / 0	0 / 0	0 / 0	312 / 0	0 / 0
L	1700	1064 / 0	323 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)		10.00
B- C	-2034 / 0	-104.9	-104.9	0.50 (1)		4.24
C- D	-1790 / 0	-104.9	-104.9	0.47 (1)		4.50
D- E	-1790 / 0	-104.9	-104.9	0.47 (1)		4.50
E- F	-1561 / 0	-104.9	-104.9	0.55 (1)		4.57
F- G	-2016 / 0	-104.9	-104.9	0.20 (1)		4.60
G- H	-1746 / 0	-104.9	-104.9	0.08 (1)		4.99
H- I	-2268 / 0	-104.9	-104.9	0.61 (1)		3.95
I- J	-2728 / 0	-104.9	-104.9	0.42 (1)		3.77
J- K	0 / 47	-104.9	-104.9	0.14 (1)		10.00
V- B	-2074 / 0	0.0	0.0	0.22 (1)		5.87
L- J	-2073 / 0	0.0	0.0	0.22 (1)		5.87
V- U	0 / 0	-28.0	-28.0	0.18 (3)		10.00
U- T	0 / 1596	-28.0	-28.0	0.40 (2)		10.00
T- S	0 / 1596	-28.0	-28.0	0.40 (2)		10.00
S- R	0 / 18	-28.0	-28.0	0.23 (2)		10.00
R- Q	0 / 0	-28.0	-28.0	0.02 (2)		10.00
P- O	0 / 1964	-28.0	-28.0	0.49 (2)		10.00
O- N	0 / 2232	-28.0	-28.0	0.51 (1)		10.00
M- N	0 / 36	0.0	0.0	0.12 (1)		10.00
N- I	-96 / 81	0.0	0.0	0.09 (1)		7.81
M- L	0 / 70	-28.0	-28.0	0.03 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-216 / 126	0.13 (1)
C- S	-384 / 0	0.52 (1)
S- E	0 / 333	0.05 (2)
P- G	-876 / 0	0.96 (1)
G- O	-830 / 0	0.73 (1)
O- H	0 / 1067	0.24 (1)
O- I	-535 / 0	0.38 (1)
B- U	0 / 1640	0.37 (1)
R- P	0 / 154	0.04 (2)
P- F	0 / 790	0.15 (1)
S- P	0 / 1340	0.22 (1)
E- P	0 / 412	0.07 (1)
N- L	-90 / 0	0.01 (1)
N- J	0 / 2160	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.61/1.00 (H-I:1) , BC=0.51/1.00 (N-O:1) , WB=0.96/1.00 (G-P:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

NSI GRIP= 0.89 (J) (INPUT = 0.90)
NSI METAL= 0.48 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-2016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11647-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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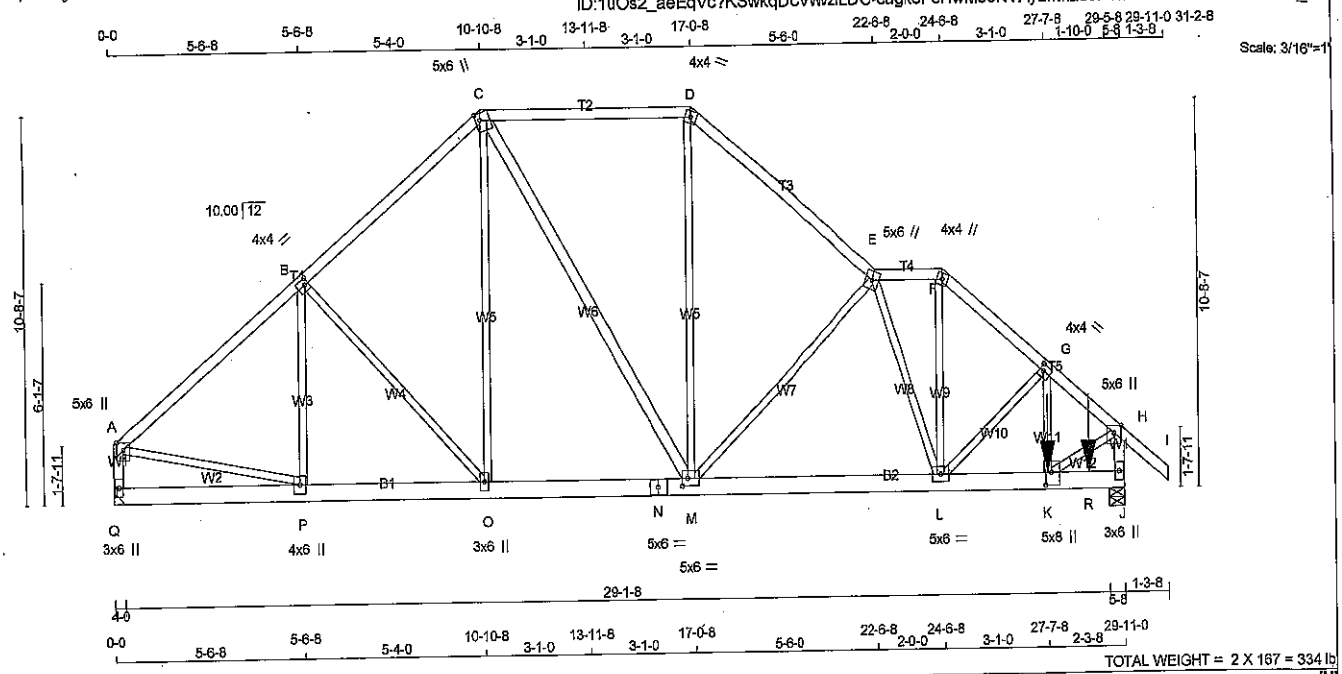

TOTAL WEIGHT = 2 X 167 = 334 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- I	1	12	TOP
Q- A	1	12	TOP
J- H	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- N	2	12	TOP
N- J	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
G- K	1	4	SIDE(350.5)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		
F	TTW+m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW+t	MT20	5.0	8.0	4.25	2.00
L	BMWWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2147	0	2147	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
J	4068	0	4068	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1710	1050 / 0	338 / 0	0 / 0	0 / 0	322 / 0	0 / 0
J	3223	2009 / 0	617 / 0	0 / 0	0 / 0	597 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2244 / 0	-104.9	-104.9	0.35 (1)	5.55	
B- C	-2008 / 0	-104.9	-104.9	0.33 (1)	5.80	
C- D	-1684 / 0	-104.9	-104.9	0.41 (1)	6.02	
D- E	-2186 / 0	-104.9	-104.9	0.36 (1)	5.59	
E- F	-2248 / 0	-104.9	-104.9	0.06 (1)	5.91	
F- G	-2908 / 0	-104.9	-104.9	0.11 (1)	5.30	
G- H	-3483 / 0	-104.9	-104.9	0.08 (1)	4.97	
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
Q- A	-2075 / 0	0.0	0.0	0.12 (1)	7.68	
J- H	-3914 / 0	0.0	0.0	0.22 (1)	5.97	
Q- P	0 / 0	-28.0	-28.0	0.05 (2)	10.00	
P- O	0 / 1757	-28.0	-28.0	0.14 (1)	10.00	
O- N	0 / 1509	-28.0	-28.0	0.14 (2)	10.00	
N- M	0 / 1509	-28.0	-28.0	0.14 (2)	10.00	
M- L	0 / 2543	-28.0	-28.0	0.20 (1)	10.00	
L- K	0 / 2687	-28.0	-28.0	0.26 (1)	10.00	
K- R	0 / 0	-28.0	-28.0	0.08 (1)	10.00	
R- J	0 / 0	-28.0	-28.0	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-234 / 134	0.07 (1)
B- O	-370 / 0	0.24 (1)
O- C	0 / 434	0.05 (2)
M- D	0 / 794	0.10 (1)
M- E	-1247 / 0	0.82 (1)
E- L	-925 / 0	0.30 (1)
L- F	0 / 1588	0.20 (1)
A- P	0 / 1799	0.22 (1)
K- G	0 / 514	0.06 (1)
L- G	-660 / 0	0.11 (1)
K- H	0 / 3045	0.38 (1)
C- M	0 / 345	0.03 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	27-7-8	-2045	-2045	---	FRONT	VERT	TOTAL
R	28-10-4	-47	-83	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.26/1.00 (K-L:1) , WB=0.82/1.00 (E-M:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.86 (H) (INPUT = 0.90)
ISI METAL= 0.35 (K) (INPUT = 1.00)

PROFESSIONAL ENGINEER LICENSED S. KATSOULAKOS PROVINCE OF ONTARIO

P644

DWG NO. TAM 11687-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BMWW+t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2045.3 lbs FACTORED DOWN AT 27-7-8, AND 82.7 lbs FACTORED DOWN AT 28-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11687-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T5S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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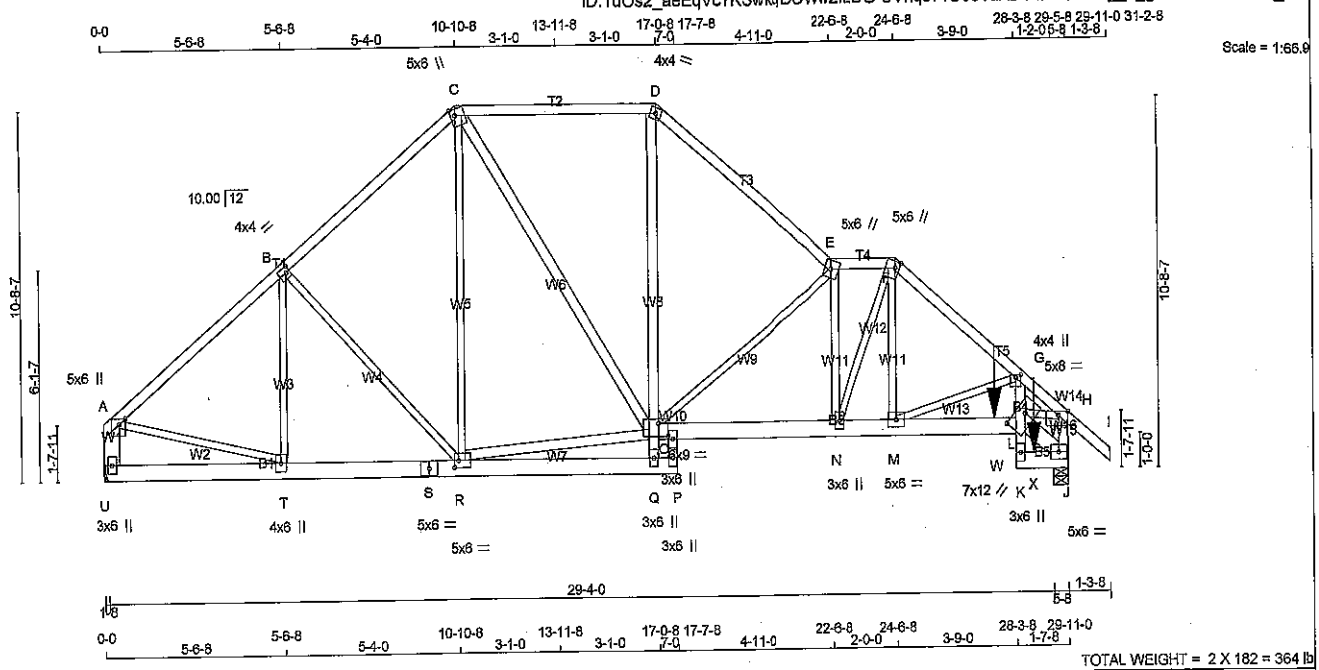

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - D	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- I	1	12	TOP
J- H	1	12	TOP
U- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- S	2	12	TOP
S- P	2	12	TOP
O- L	2	12	SIDE(0.0)
K- J	2	12	SIDE(0.0)
G- K	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2152	0	2152	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
J	4081	0	4081	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1719	1047 / 0	345 / 0	0 / 0	0 / 0	327 / 0	0 / 0
J	3249	1998 / 0	640 / 0	0 / 0	0 / 0	611 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2248 / 0	-104.9	-104.9	0.35 (1)	5.55
B- C	-2022 / 0	-104.9	-104.9	0.34 (1)	5.79
C- D	-1860 / 0	-104.9	-104.9	0.42 (1)	5.60
D- E	-2407 / 0	-104.9	-104.9	0.37 (1)	5.38
E- F	-3067 / 0	-104.9	-104.9	0.07 (1)	5.23
F- G	-3608 / 0	-104.9	-104.9	0.21 (1)	4.77
G- H	-5546 / 0	-104.9	-104.9	0.17 (1)	4.00
H- I	0 / 47	-104.9	-104.9	0.08 (1)	10.00
U- A	-2078 / 0	0.0	0.0	0.08 (1)	7.81
J- H	-3916 / 0	0.0	0.0	0.22 (1)	5.97
U- T	0 / 0	-28.0	-28.0	0.05 (2)	10.00
T- S	0 / 1760	-28.0	-28.0	0.15 (1)	10.00
S- R	0 / 1760	-28.0	-28.0	0.15 (1)	10.00
R- Q	0 / 24	-28.0	-28.0	0.06 (2)	10.00
Q- P	0 / 0	-28.0	-28.0	0.00 (2)	10.00
O- N	0 / 3087	-28.0	-28.0	0.25 (1)	10.00
N- M	0 / 2788	-28.0	-28.0	0.37 (1)	10.00
M- W	0 / 4413	-28.0	-28.0	0.75 (1)	10.00
W- L	0 / 4413	-28.0	-28.0	0.75 (1)	10.00
K- L	0 / 106	0.0	0.0	0.15 (1)	10.00
L- G	0 / 1567	0.0	0.0	0.28 (1)	10.00
K- X	0 / 161	-28.0	-28.0	0.02 (2)	10.00
X- J	0 / 161	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-245 / 112	0.07 (1)
B- R	-361 / 0	0.24 (1)
R- C	0 / 309	0.04 (2)
O- E	-1615 / 0	0.85 (1)
N- E	-896 / 0	0.17 (1)
N- F	0 / 751	0.09 (1)
M- F	0 / 1316	0.16 (1)
M- G	-1732 / 0	0.27 (1)
A- T	0 / 1803	0.22 (1)
Q- O	0 / 155	0.03 (1)
O- D	0 / 829	0.10 (1)
R- O	0 / 1513	0.13 (1)
C- O	0 / 633	0.06 (1)
L- J	-206 / 0	0.01 (1)
L- H	0 / 4239	0.52 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	27-7-8	-2035	-2035	—	FRONT	VERT	TOTAL
X	28-10-4	-55	-97	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.75/1.00 (L-M:1) , WB=0.85/1.00 (E-O:1) , SSI=0.51/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF...

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

TPI GRIP= 0.87 (F) (INPUT = 0.90)
TPI METAL= 0.48 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11648-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T5S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW+p	MT20	4.0	4.0	1.00	2.00
H	TMWW-p	MT20	5.0	8.0	1.50	3.50
J	BMVW1-t	MT20	5.0	6.0		
K	BMV+p	MT20	3.0	6.0		
L	BVMWW-w	MT20	7.0	12.0	3.25	7.00
M	BMWW-t	MT20	5.0	6.0		
N	BMWW+t	MT20	3.0	6.0		
O	BWMWWW*-l	MT20	6.0	9.0	4.50	3.50
P	NP+w	MT20	3.0	6.0		
Q	BMW+w	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	6.0	2.50	1.50
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	4.0	6.0		
U	BMV1+p	MT20	3.0	6.0		
V	NP+w	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2035.3 lbs FACTORED DOWN AT 27-7-8, AND 97.0 lbs FACTORED DOWN AT 28-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
30216
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11648-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T6	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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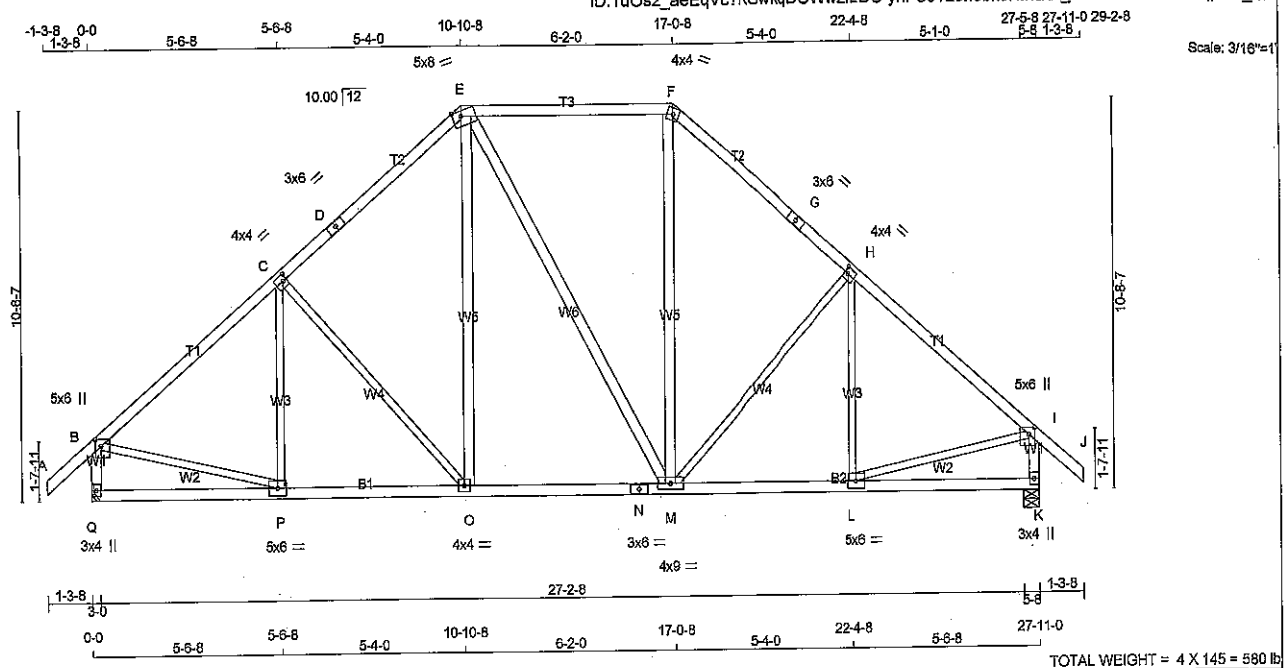

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW+p	MT20	5.0	6.0	2.25	2.00
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2001	0	2001	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
K	2001	0	2001	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0
K	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1867 / 0	-104.9	-104.9	0.49 (1)	4.41	
C- D	-1594 / 0	-104.9	-104.9	0.45 (1)	4.72	
D- E	-1594 / 0	-104.9	-104.9	0.45 (1)	4.72	
E- F	-1194 / 0	-104.9	-104.9	0.53 (1)	5.10	
F- G	-1594 / 0	-104.9	-104.9	0.45 (1)	4.72	
G- H	-1594 / 0	-104.9	-104.9	0.45 (1)	4.72	
H- I	-1867 / 0	-104.9	-104.9	0.49 (1)	4.41	
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-1937 / 0	0.0	0.0	0.21 (1)	6.04	
K- I	-1936 / 0	0.0	0.0	0.21 (1)	6.04	
Q- P	0 / 0	-28.0	-28.0	0.20 (3)	10.00	
P- O	0 / 1468	-28.0	-28.0	0.35 (2)	10.00	
O- N	0 / 1194	-28.0	-28.0	0.31 (2)	10.00	
N- M	0 / 1194	-28.0	-28.0	0.31 (2)	10.00	
M- L	0 / 1468	-28.0	-28.0	0.35 (2)	10.00	
L- K	0 / 0	-28.0	-28.0	0.20 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-177 / 154	0.11 (1)
C- O	-414 / 0	0.56 (1)
O- E	0 / 467	0.08 (2)
E- M	0 / 0	0.00 (1)
M- F	0 / 468	0.08 (2)
M- H	-413 / 0	0.56 (1)
L- H	-178 / 153	0.11 (1)
B- P	0 / 1508	0.34 (1)
L- I	0 / 1508	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.53/1.00 (E-F:1) , BC=0.35/1.00 (O-P:2) , WB=0.56/1.00 (C-O:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI: GRIP= 0.89 (B) (INPUT = 0.90)
CSI: METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-2-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11649 -08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T6A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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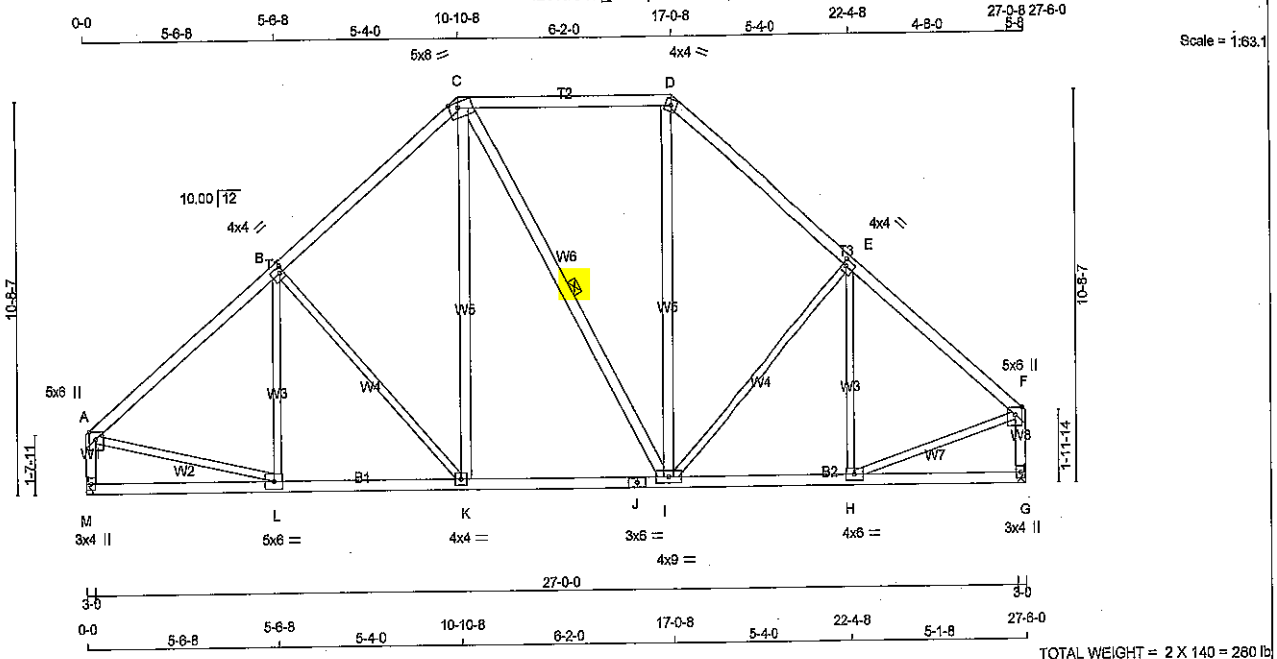

TOTAL WEIGHT = 2 X 140 = 280 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF
C - I	2x4	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.50	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW+p	MT20	5.0	6.0	2.50	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1828	0	1826	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
G	1828	0	1828	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1457	893 / 0	289 / 0	0 / 0	0 / 0	275 / 0	0 / 0
G	1457	893 / 0	289 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1833 / 0	-104.9	-104.9	0.48 (1)	4.45
B- C	-1556 / 0	-104.9	-104.9	0.45 (1)	4.78
C- D	-1149 / 0	-104.9	-104.9	0.53 (1)	5.18
D- E	-1533 / 0	-104.9	-104.9	0.42 (1)	4.85
E- F	-1723 / 0	-104.9	-104.9	0.43 (1)	4.63
M- A	-1764 / 0	0.0	0.0	0.19 (1)	6.28
G- F	-1769 / 0	0.0	0.0	0.20 (1)	6.27
M- L	0 / 0	-28.0	-28.0	0.20 (3)	10.00
L- K	0 / 1442	-28.0	-28.0	0.35 (2)	10.00
K- J	0 / 1165	-28.0	-28.0	0.31 (2)	10.00
J- I	0 / 1165	-28.0	-28.0	0.31 (2)	10.00
I- H	0 / 1357	-28.0	-28.0	0.33 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-171 / 157	0.11 (1)
B- K	-418 / 0	0.57 (1)
K- C	0 / 468	0.08 (2)
C- I	-31 / 0	0.02 (1)
I- D	0 / 441	0.07 (2)
I- E	-314 / 0	0.43 (1)
H- E	-278 / 101	0.17 (1)
A- L	0 / 1481	0.33 (1)
H- F	0 / 1425	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.53/1.00 (C-D:1) , BC=0.35/1.00 (K-L:2) , WB=0.57/1.00 (B-K:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.36 (L) (INPUT = 1.00)

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S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11663-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T6SA	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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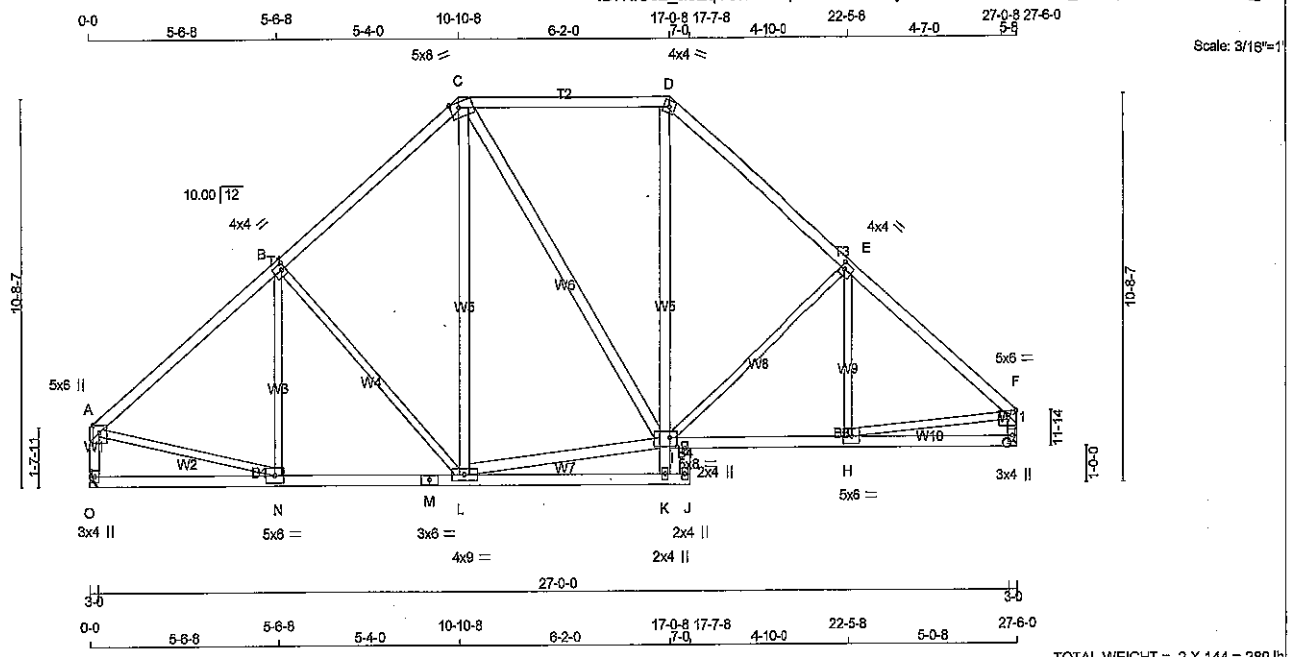

TOTAL WEIGHT = 2 X 144 = 289 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
C - 1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	2.50	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMWW-p	MT20	5.0	6.0	1.50	3.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	5.0	6.0		
I	BWMWWW*-I	MT20	5.0	8.0	3.00	2.50
J	NP+w	MT20	2.0	4.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	4.0	8.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		
P	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1836	0	1836	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
G	1840	0	1840	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1467	893 / 0	295 / 0	0 / 0	0 / 0	279 / 0	0 / 0
G	1473	893 / 0	298 / 0	0 / 0	0 / 0	281 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1841 / 0	-104.9	-104.9	0.48 (1)	4.44
B- C	-1573 / 0	-104.9	-104.9	0.45 (1)	4.76
C- D	-1276 / 0	-104.9	-104.9	0.54 (1)	4.97
D- E	-1697 / 0	-104.9	-104.9	0.44 (1)	4.64
E- F	-2105 / 0	-104.9	-104.9	0.45 (1)	4.24
O- A	-1770 / 0	0.0	0.0	0.19 (1)	6.27
G- F	-1786 / 0	0.0	0.0	0.18 (1)	6.25
O- N	0 / 0	-28.0	-28.0	0.18 (3)	10.00
N- M	0 / 1448	-28.0	-28.0	0.38 (2)	10.00
M- L	0 / 1448	-28.0	-28.0	0.38 (2)	10.00
L- K	0 / 16	-28.0	-28.0	0.23 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.02 (2)	10.00
I- H	0 / 1651	-28.0	-28.0	0.40 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-180 / 142	0.11 (1)
B- L	-410 / 0	0.56 (1)
L- C	0 / 365	0.06 (2)
I- E	-506 / 0	0.54 (1)
H- E	-35 / 229	0.05 (3)
A- N	0 / 1487	0.33 (1)
K- I	0 / 154	0.04 (2)
I- D	0 / 519	0.11 (1)
L- I	0 / 1173	0.19 (1)
C- I	0 / 190	0.03 (1)
H- F	0 / 1665	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.54/1.00 (C-D:1) , BC=0.40/1.00 (H-I:2) , WB=0.56/1.00 (B-L:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.42 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11650 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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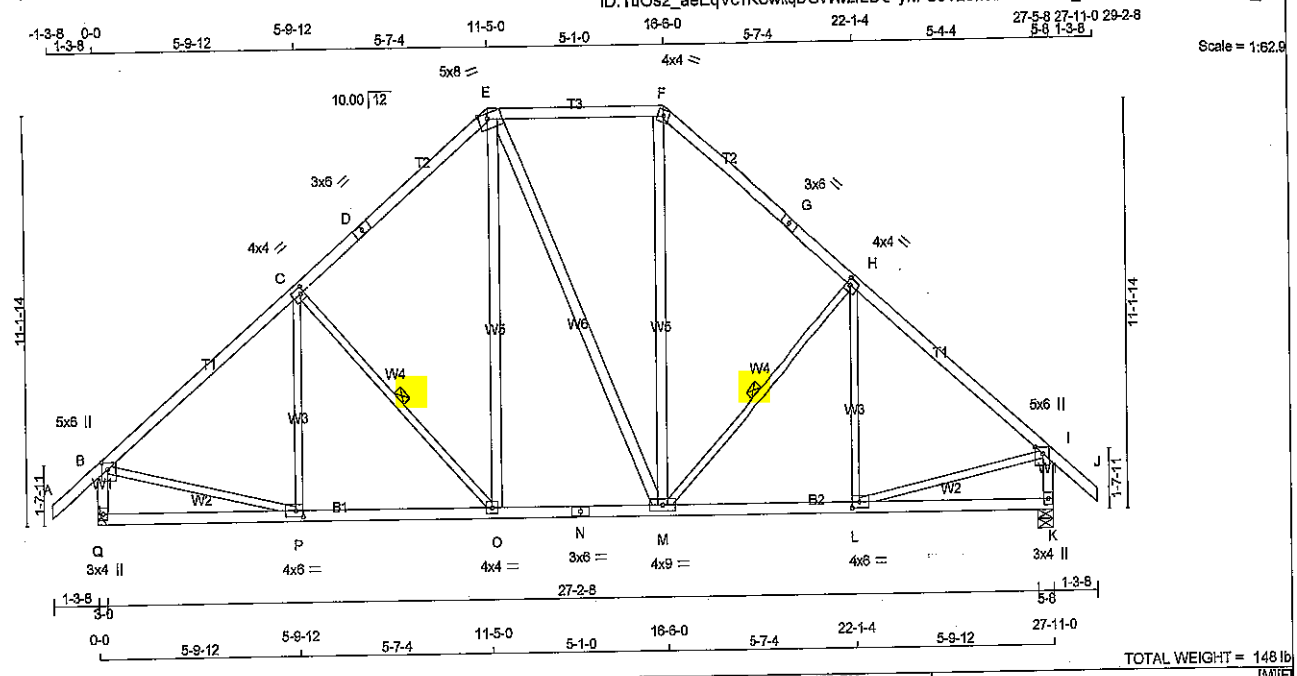

TOTAL WEIGHT = 148 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.25	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.50
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.50
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2001	0	2001	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	
K	2001	0	2001	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0
K	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1869 / 0	-104.9	-104.9	0.54 (1)	4.33
C- D	-1553 / 0	-104.9	-104.9	0.50 (1)	4.71
D- E	-1553 / 0	-104.9	-104.9	0.50 (1)	4.71
E- F	-1162 / 0	-104.9	-104.9	0.37 (1)	5.46
F- G	-1554 / 0	-104.9	-104.9	0.50 (1)	4.71
G- H	-1554 / 0	-104.9	-104.9	0.50 (1)	4.71
H- I	-1869 / 0	-104.9	-104.9	0.54 (1)	4.33
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1935 / 0	0.0	0.0	0.21 (1)	6.05
K- I	-1934 / 0	0.0	0.0	0.21 (1)	6.05
Q- P	0 / 0	-28.0	-28.0	0.23 (3)	10.00
P- O	0 / 1471	-28.0	-28.0	0.39 (2)	10.00
O- N	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
N- M	0 / 1161	-28.0	-28.0	0.27 (1)	10.00
M- L	0 / 1471	-28.0	-28.0	0.39 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-147 / 186	0.10 (1)
C- O	-464 / 0	0.22 (1)
O- E	0 / 480	0.08 (1)
E- M	0 / 1	0.00 (1)
M- F	0 / 481	0.08 (1)
M- H	-463 / 0	0.22 (1)
L- H	-149 / 184	0.10 (1)
B- P	0 / 1508	0.34 (1)
L- I	0 / 1508	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.39/1.00 (O-P:2) , WB=0.34/1.00 (B-P:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3048
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 11651-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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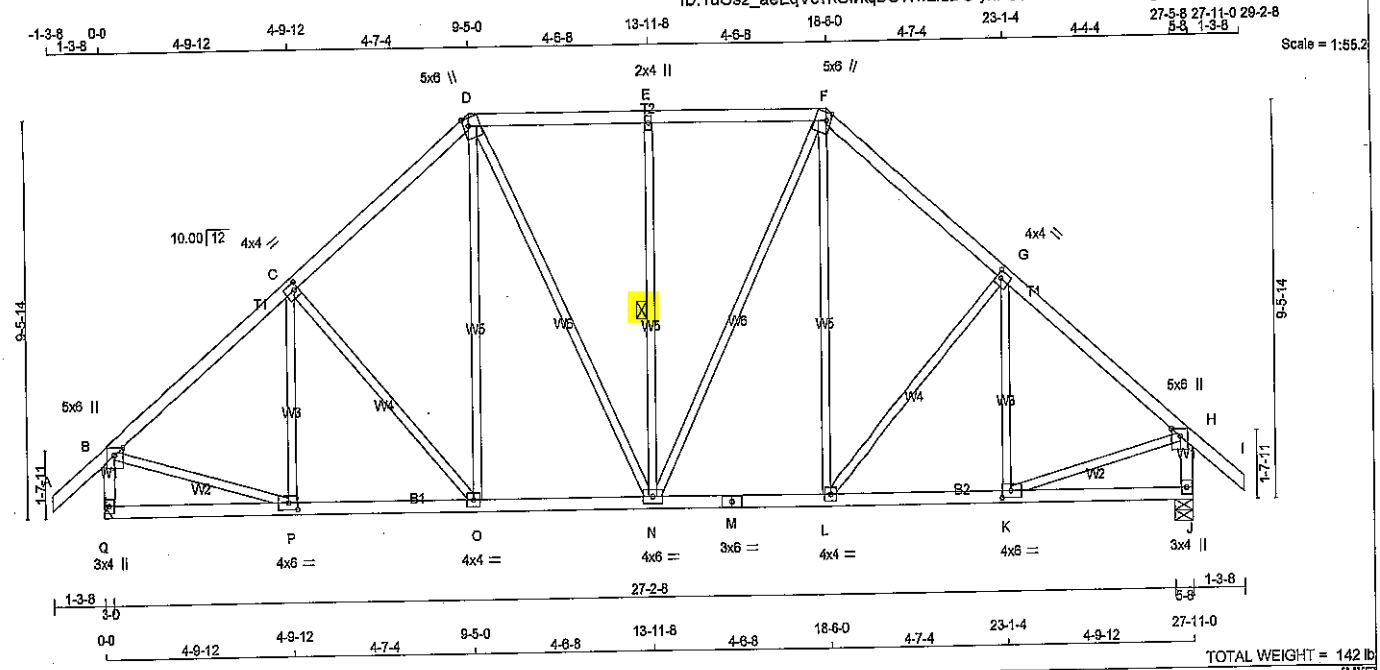

TOTAL WEIGHT = 142 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.75
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2001	0	2001	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	
J	2001	0	2001	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0
J	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.58 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1854 / 0	-104.9	-104.9	0.36 (1)	4.58
C- D	-1688 / 0	-104.9	-104.9	0.34 (1)	4.77
D- E	-1440 / 0	-104.9	-104.9	0.29 (1)	5.13
E- F	-1440 / 0	-104.9	-104.9	0.29 (1)	5.13
F- G	-1688 / 0	-104.9	-104.9	0.34 (1)	4.77
G- H	-1854 / 0	-104.9	-104.9	0.36 (1)	4.58
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1944 / 0	0.0	0.0	0.21 (1)	6.03
J- H	-1944 / 0	0.0	0.0	0.21 (1)	6.03
Q- P	0 / 0	-28.0	-28.0	0.15 (3)	10.00
P- O	0 / 1453	-28.0	-28.0	0.31 (1)	10.00
O- N	0 / 1268	-28.0	-28.0	0.27 (1)	10.00
N- M	0 / 1268	-28.0	-28.0	0.27 (1)	10.00
M- L	0 / 1268	-28.0	-28.0	0.27 (1)	10.00
L- K	0 / 1453	-28.0	-28.0	0.31 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-248 / 97	0.12 (1)
C- O	-284 / 0	0.27 (1)
O- D	0 / 352	0.08 (2)
D- N	0 / 389	0.09 (1)
N- E	-574 / 0	0.34 (1)
N- F	0 / 389	0.09 (1)
L- F	0 / 352	0.08 (2)
L- G	-284 / 0	0.27 (1)
K- G	-248 / 97	0.12 (1)
B- P	0 / 1505	0.34 (1)
K- H	0 / 1505	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.36/1.00 (G-H:1) , BC=0.31/1.00 (K-L:1) , WB=0.34/1.00 (E-N:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.37 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30216
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PROVINCE OF ONTARIO

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DWG NO. TAM 51652-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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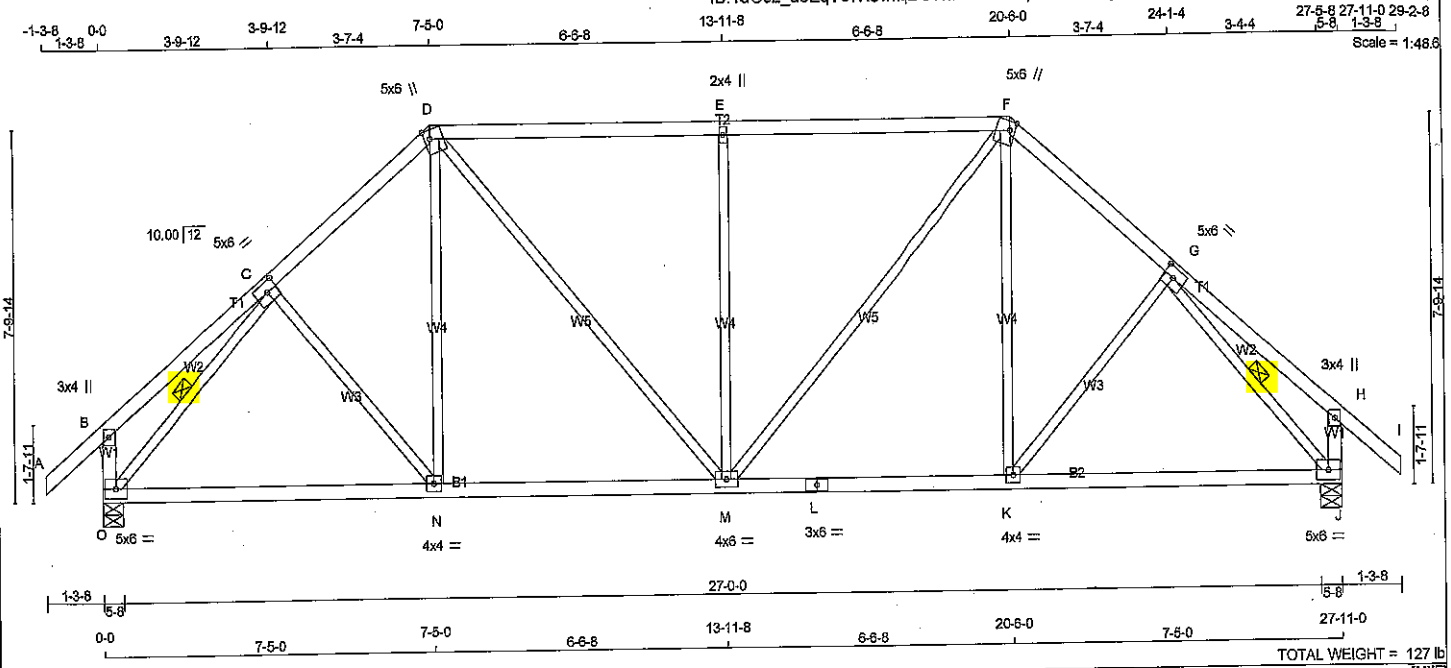

TOTAL WEIGHT = 127 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2001	0	2001	0	0	5-8	5-8
J	2001	0	2001	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0
J	1577	997 / 0	293 / 0	0 / 0	0 / 0	287 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 27	-104.9	-104.9	0.22 (1)	10.00	
C- D	-1808 / 0	-104.9	-104.9	0.21 (1)	4.80	
D- E	-1800 / 0	-104.9	-104.9	0.64 (1)	4.16	
E- F	-1800 / 0	-104.9	-104.9	0.64 (1)	4.16	
F- G	-1808 / 0	-104.9	-104.9	0.21 (1)	4.80	
G- H	0 / 27	-104.9	-104.9	0.22 (1)	10.00	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
O- B	-296 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-296 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 1364	-28.0	-28.0	0.51 (2)	10.00	
N- M	0 / 1368	-28.0	-28.0	0.52 (2)	10.00	
M- L	0 / 1368	-28.0	-28.0	0.52 (2)	10.00	
L- K	0 / 1368	-28.0	-28.0	0.52 (2)	10.00	
K- J	0 / 1364	-28.0	-28.0	0.51 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 119	0.03 (3)
N- D	0 / 330	0.07 (2)
D- M	0 / 658	0.15 (1)
M- E	-841 / 0	0.96 (1)
M- F	0 / 658	0.15 (1)
K- F	0 / 330	0.07 (2)
K- G	0 / 119	0.03 (3)
O- C	-2118 / 0	0.50 (1)
G- J	-2118 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.64/1.00 (E-F:1) , BC=0.52/1.00 (K-M:2) , WB=0.96/1.00 (E-M:1) , SSI=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.89 (C) (INPUT = 0.90)
CSI METAL= 0.52 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11653-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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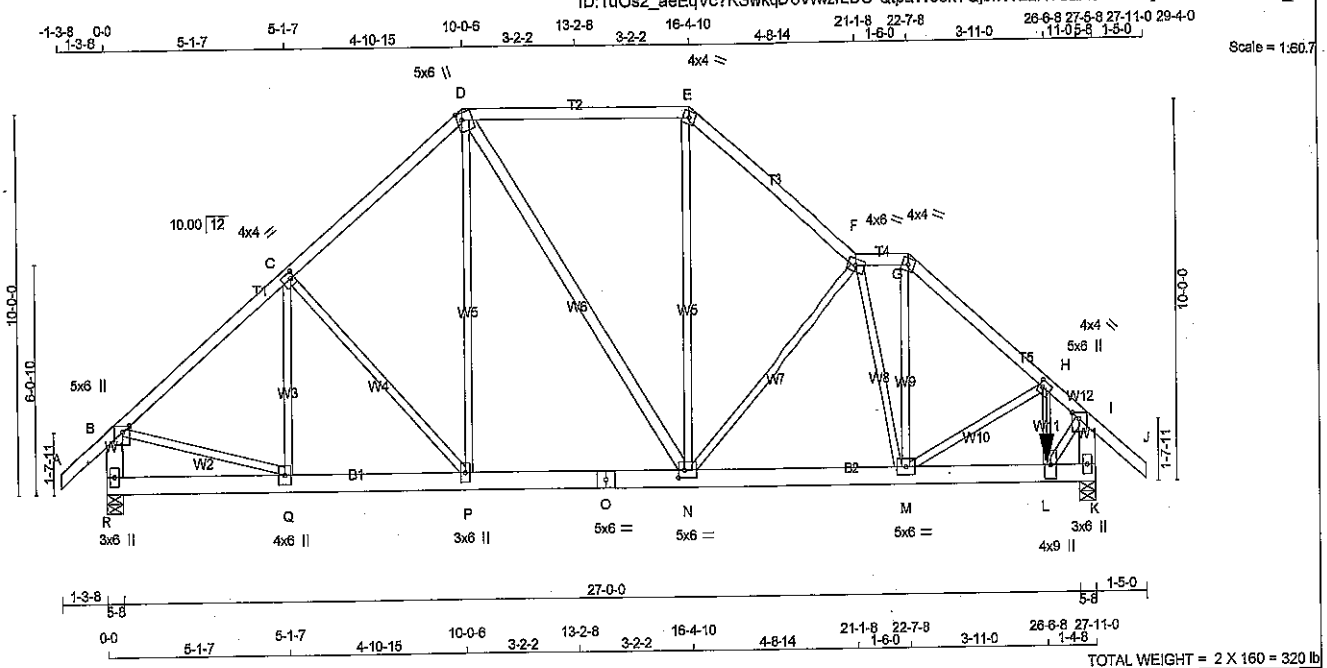

TOTAL WEIGHT = 2 X 160 = 320 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	TOP
G- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	12	TOP
O- K	2	7	SIDE(366.2)
WEBS : (0.122"X3") SPIRAL NAILS			
H- L	1	2	SIDE(435.5)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWW-m	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	4.0	9.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2148	0	2148	0	0	5-8	5-8
K	4940	0	4940	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1692	1071 / 0	314 / 0	0 / 0	0 / 0	308 / 0	0 / 0
K	3866	2491 / 0	686 / 0	0 / 0	0 / 0	688 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)		10.00
B- C	-2058 / 0	-104.9	-104.9	0.28 (1)		5.82
C- D	-1887 / 0	-104.9	-104.9	0.27 (1)		6.04
D- E	-1564 / 0	-104.9	-104.9	0.43 (1)		6.15
E- F	-2026 / 0	-104.9	-104.9	0.26 (1)		5.88
F- G	-2028 / 0	-104.9	-104.9	0.03 (1)		8.17
G- H	-2628 / 0	-104.9	-104.9	0.15 (1)		5.46
H- I	-3405 / 0	-104.9	-104.9	0.13 (1)		4.95
I- J	0 / 51	-104.9	-104.9	0.09 (1)		10.00
R- B	-2079 / 0	0.0	0.0	0.08 (1)		7.81
K- I	-4738 / 0	0.0	0.0	0.17 (1)		6.65
R- Q	0 / 0	-28.0	-28.0	0.04 (2)		10.00
Q- P	0 / 1611	-28.0	-28.0	0.13 (1)		10.00
P- O	0 / 1404	-28.0	-28.0	0.12 (2)		10.00
O- N	0 / 1404	-28.0	-28.0	0.12 (2)		10.00
N- M	0 / 2234	-28.0	-28.0	0.18 (1)		10.00
M- L	0 / 2607	-28.0	-28.0	0.26 (1)		10.00
L- K	0 / 0	-28.0	-28.0	0.09 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-250 / 103	0.06 (1)
C- P	-314 / 0	0.17 (1)
P- D	0 / 418	0.05 (2)
D- N	0 / 295	0.03 (1)
N- E	0 / 715	0.09 (1)
N- F	-1041 / 0	0.56 (1)
F- M	-859 / 0	0.25 (1)
M- G	0 / 1419	0.18 (1)
B- Q	0 / 1657	0.21 (1)
L- H	0 / 566	0.07 (1)
M- H	-702 / 0	0.13 (1)
L- I	0 / 3487	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
L	26-6-8	-3073	-3073	—	BACK	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN, C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.26/1.00 (L-M:1) , WB=0.56/1.00 (F-N:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN ONTARIO S. KATSOULAKOS 2018

Polk

DWG NO. TAM 116654-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T10	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW+t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3073.3 lbs FACTORED DOWN AT 26-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
30210
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11654 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T10Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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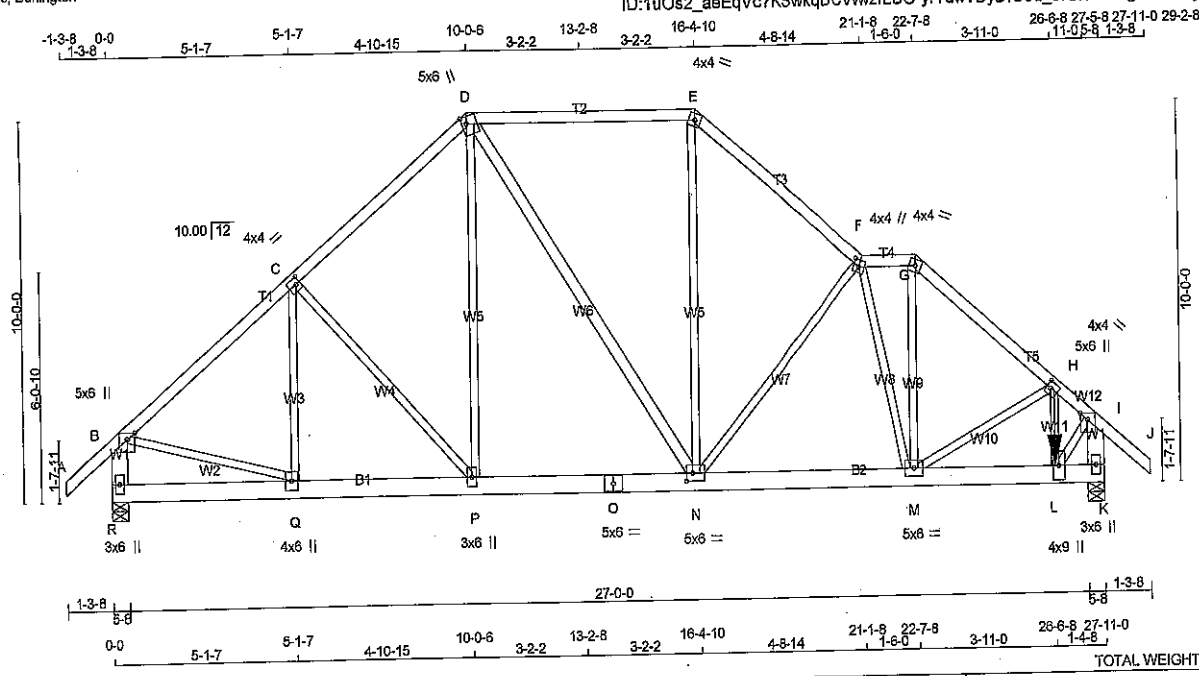

TOTAL WEIGHT = 2 X 160 = 320 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	TOP
G- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	12	TOP
O- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H- L	1	2	SIDE(439.7)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.00	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TTW-m	MT20	4.0	4.0		
F	TTWW+m	MT20	4.0	4.0	2.25	2.00
G	TTW-m	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWV+p	MT20	5.0	6.0	2.00	2.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	4.0	9.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	5.0	6.0		
P	BMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2115	0	2115	0	0	5-8	5-8
K	4274	0	4274	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1666	1054 / 0	309 / 0	0 / 0	0 / 0	303 / 0	0 / 0
K	3347	2152 / 0	597 / 0	0 / 0	0 / 0	598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
B- C	-2018 / 0	-104.9	-104.9	0.28 (1)	5.87	
C- D	-1821 / 0	-104.9	-104.9	0.27 (1)	6.10	
D- E	-1509 / 0	-104.9	-104.9	0.43 (1)	6.23	
E- F	-1959 / 0	-104.9	-104.9	0.25 (1)	5.96	
F- G	-1894 / 0	-104.9	-104.9	0.03 (1)	6.25	
G- H	-2458 / 0	-104.9	-104.9	0.15 (1)	5.61	
H- I	-2944 / 0	-104.9	-104.9	0.10 (1)	5.27	
I- J	0 / 47	-104.9	-104.9	0.08 (1)	10.00	
R- B	-2046 / 0	0.0	0.0	0.08 (1)	7.81	
K- I	-4111 / 0	0.0	0.0	0.15 (1)	7.03	
R- Q	0 / 0	-28.0	-28.0	0.04 (2)	10.00	
Q- P	0 / 1581	-28.0	-28.0	0.13 (1)	10.00	
P- O	0 / 1369	-28.0	-28.0	0.12 (2)	10.00	
O- N	0 / 1369	-28.0	-28.0	0.12 (2)	10.00	
N- M	0 / 2108	-28.0	-28.0	0.16 (1)	10.00	
M- L	0 / 2262	-28.0	-28.0	0.22 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.07 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-242 / 106	0.06 (1)
C- P	-321 / 0	0.17 (1)
P- D	0 / 420	0.05 (2)
D- N	0 / 256	0.02 (1)
N- E	0 / 679	0.08 (1)
N- F	-941 / 0	0.51 (1)
F- M	-867 / 0	0.26 (1)
M- G	0 / 1281	0.16 (1)
B- Q	0 / 1625	0.20 (1)
L- H	0 / 242	0.03 (1)
M- H	-455 / 0	0.08 (1)
L- I	0 / 3026	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	26-6-8	-2367	-2367	---	BACK	VERT	TRAC

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.22/1.00 (L-M:1) , WB=0.51/1.00 (F-N:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (F) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11691 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T10Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW+t	MT20	4.0	8.0		
R	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2387.3 lbs FACTORED DOWN AT 26-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11691-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T11	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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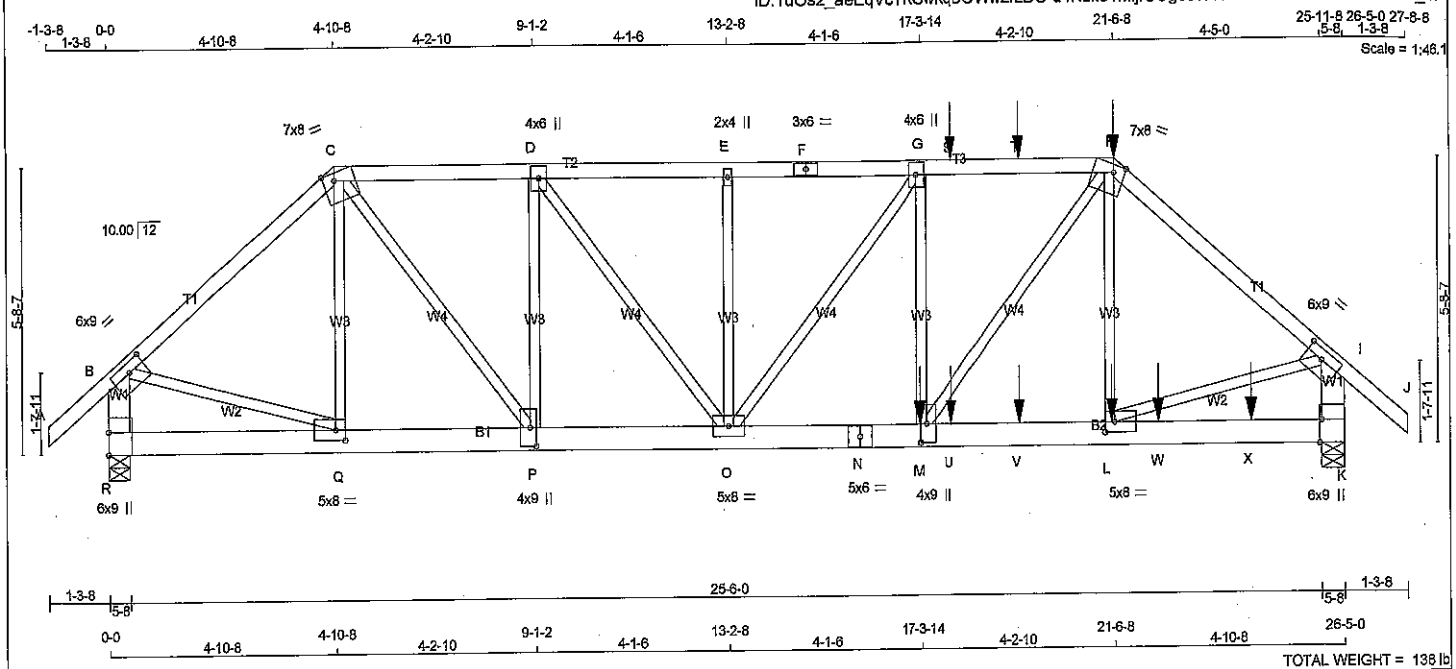

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.25	4.25
C	TTWW-m	MT20	7.0	8.0	Edge	2.75
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW-m	MT20	7.0	8.0	Edge	2.75
I	TMVW-t	MT20	6.0	9.0	2.25	4.25
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	5.0	8.0	2.50	2.50
M	BMWW+t	MT20	4.0	9.0	4.50	1.50
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	9.0	4.50	1.50
Q	BMWW-t	MT20	5.0	8.0	2.50	2.50
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 150.8 lbs FACTORED DOWN AT 17-11-12, AND 150.8 lbs FACTORED DOWN AT 19-5-12, AND 449.6 lbs FACTORED DOWN AT 21-6-8 ON TOP CHORD, AND 1468.3 lbs FACTORED DOWN AT 17-3-8, 70.4 lbs FACTORED DOWN AT 17-11-12, 70.4 lbs FACTORED DOWN AT 19-5-12, 70.4 lbs FACTORED DOWN AT 21-5-12, AND 70.4 lbs FACTORED DOWN AT 22-5-12, AND 70.4 lbs FACTORED DOWN AT 24-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2617	0	2617	0	0	5-8	5-8
K	3603	0	3603	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2052	1315 / 0	369 / 0	0 / 0	0 / 0	368 / 0	0 / 0
K	2831	1803 / 0	516 / 0	0 / 0	0 / 0	513 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)		10.00
B- C	-2599 / 0	-104.9	-104.9	0.73 (1)		3.52
C- D	-3223 / 0	-104.9	-104.9	0.47 (1)		3.42
D- E	-3961 / 0	-104.9	-104.9	0.55 (1)		3.01
E- F	-3961 / 0	-104.9	-104.9	0.70 (1)		2.78
F- G	-3961 / 0	-104.9	-104.9	0.70 (1)		2.78
G- S	-4328 / 0	-104.9	-104.9	0.82 (1)		2.45
S- T	-4328 / 0	-104.9	-104.9	0.82 (1)		2.45
T- H	-4328 / 0	-104.9	-104.9	0.82 (1)		2.45
H- I	-3761 / 0	-104.9	-104.9	0.96 (1)		2.76
I- J	0 / 47	-104.9	-104.9	0.16 (1)		10.00
R- B	-2559 / 0	0.0	0.0	0.19 (1)		6.44
K- I	-3524 / 0	0.0	0.0	0.26 (1)		5.63
R- Q	0 / 0	-28.0	-28.0	0.08 (3)		10.00
Q- P	0 / 1988	-28.0	-28.0	0.30 (1)		10.00
P- O	0 / 3223	-28.0	-28.0	0.47 (1)		10.00
O- N	0 / 4329	-28.0	-28.0	0.65 (1)		10.00
N- M	0 / 4329	-28.0	-28.0	0.65 (1)		10.00
M- U	0 / 2875	-28.0	-28.0	0.46 (1)		10.00
U- V	0 / 2875	-28.0	-28.0	0.46 (1)		10.00
V- L	0 / 2875	-28.0	-28.0	0.46 (1)		10.00
L- W	0 / 0	-28.0	-28.0	0.14 (2)		10.00
W- X	0 / 0	-28.0	-28.0	0.14 (2)		10.00
X- K	0 / 0	-28.0	-28.0	0.14 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-337 / 58	0.16 (1)
C- P	0 / 1988	0.49 (1)
P- D	-1444 / 0	0.69 (1)
D- O	0 / 1206	0.30 (1)
O- E	-362 / 0	0.17 (1)
O- G	-602 / 0	0.49 (1)
M- G	-249 / 100	0.12 (1)
M- H	0 / 2335	0.58 (1)
L- H	-398 / 217	0.19 (1)
B- Q	0 / 2052	0.51 (1)
L- I	0 / 2969	0.73 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
H	21-6-8	-450	-450	---	BACK	VERT	TOTAL
L	21-5-12	-40	-70	---	BACK	VERT	TOTAL
M	17-3-8	-1468	-1468	---	BACK	VERT	TOTAL
S	17-11-12	-151	-151	---	BACK	VERT	TOTAL
T	19-5-12	-151	-151	---	BACK	VERT	TOTAL
U	17-11-12	-40	-70	---	BACK	VERT	TOTAL
V	19-5-12	-40	-70	---	BACK	VERT	TOTAL
W	22-5-12	-40	-70	---	BACK	VERT	TOTAL
X	24-5-12	-37	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.96/1.00 (H-I:1) , BC=0.65/1.00 (M-O:1) , WB=0.73/1.00 (I-L:1) , SSI=0.42/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.74 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11655.8
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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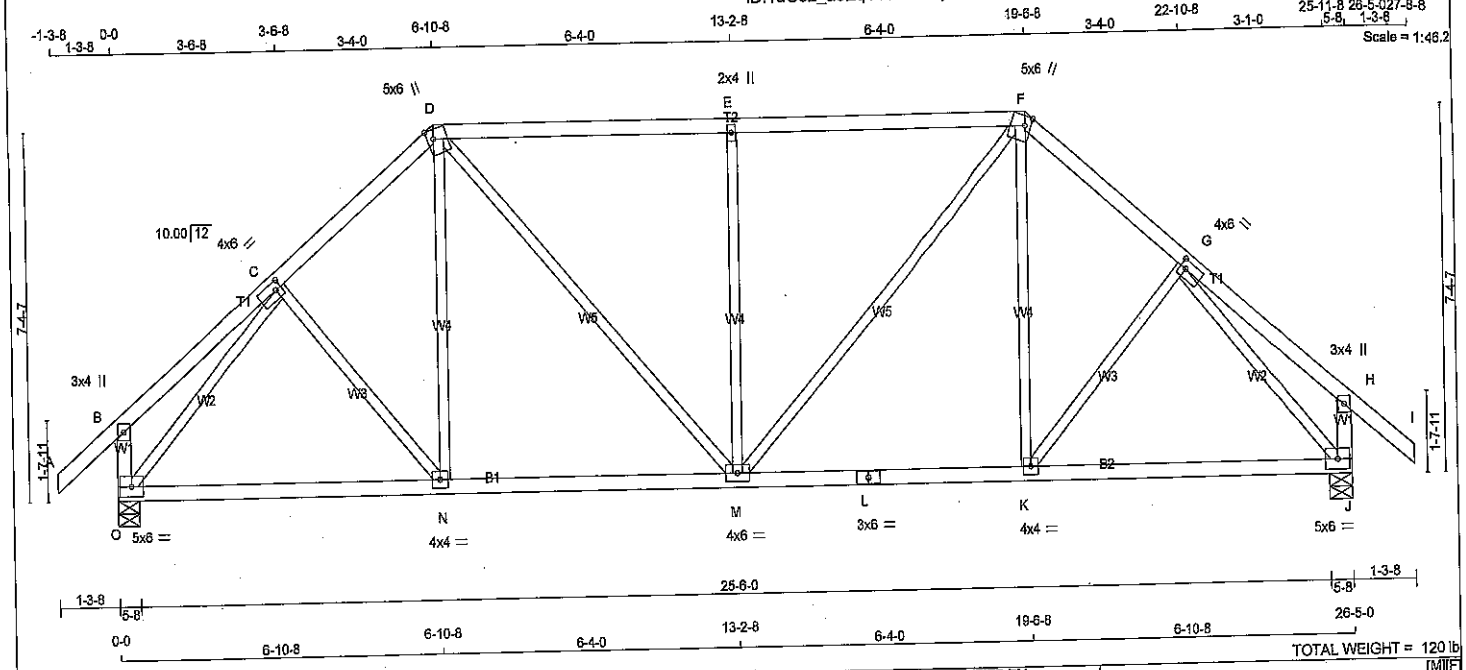

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMWW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1901	0	1901	0	0	5-8	5-8
J	1901	0	1901	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 25	-104.9	-104.9	0.18 (1)	10.00
C- D	-1700 / 0	-104.9	-104.9	0.18 (1)	4.95
D- E	-1718 / 0	-104.9	-104.9	0.59 (1)	4.33
E- F	-1718 / 0	-104.9	-104.9	0.59 (1)	4.33
F- G	-1700 / 0	-104.9	-104.9	0.18 (1)	4.95
G- H	0 / 25	-104.9	-104.9	0.18 (1)	10.00
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
O- B	-285 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-285 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1264	-28.0	-28.0	0.45 (2)	10.00
N- M	0 / 1286	-28.0	-28.0	0.46 (2)	10.00
M- L	0 / 1286	-28.0	-28.0	0.46 (2)	10.00
L- K	0 / 1286	-28.0	-28.0	0.46 (2)	10.00
K- J	0 / 1264	-28.0	-28.0	0.45 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 121	0.03 (3)
N- D	0 / 295	0.07 (2)
D- M	0 / 648	0.15 (1)
M- E	-814 / 0	0.79 (1)
M- F	0 / 648	0.15 (1)
K- F	0 / 295	0.07 (2)
K- G	0 / 121	0.03 (3)
O- C	-1990 / 0	0.93 (1)
G- J	-1990 / 0	0.93 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.59/1.00 (D-E:1) , BC=0.46/1.00 (M-N:2) , WB=0.93/1.00 (C-O:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI:GRIP= 0.88 (C) (INPUT = 0.90)
CSI:METAL= 0.49 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11656-08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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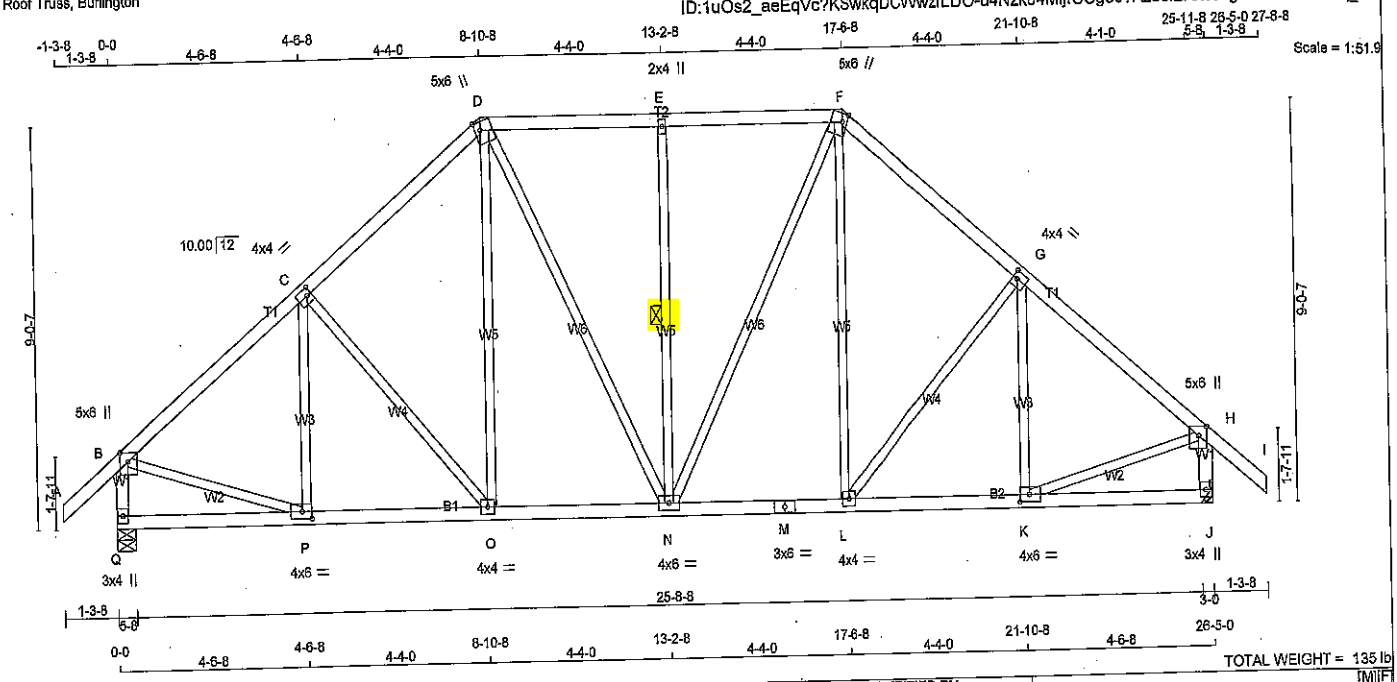

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMW+p	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1901	0	1901	0	0	IN-SX	IN-SX
J	1901	0	1901	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1729 / 0	-104.9	-104.9	0.31 (1)	4.77
C- D	-1587 / 0	-104.9	-104.9	0.30 (1)	4.94
D- E	-1356 / 0	-104.9	-104.9	0.26 (1)	5.29
E- F	-1356 / 0	-104.9	-104.9	0.26 (1)	5.29
F- G	-1587 / 0	-104.9	-104.9	0.30 (1)	4.94
G- H	-1729 / 0	-104.9	-104.9	0.31 (1)	4.77
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1848 / 0	0.0	0.0	0.20 (1)	6.16
J- H	-1848 / 0	0.0	0.0	0.20 (1)	6.16
Q- P	0 / 0	-28.0	-28.0	0.13 (3)	10.00
P- O	0 / 1355	-28.0	-28.0	0.28 (1)	10.00
O- N	0 / 1192	-28.0	-28.0	0.25 (1)	10.00
N- M	0 / 1192	-28.0	-28.0	0.25 (1)	10.00
M- L	0 / 1192	-28.0	-28.0	0.25 (1)	10.00
L- K	0 / 1355	-28.0	-28.0	0.28 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-252 / 84	0.11 (1)
C- O	-254 / 0	0.21 (1)
O- D	0 / 326	0.07 (2)
D- N	0 / 370	0.08 (1)
N- E	-547 / 0	0.29 (1)
N- F	0 / 370	0.08 (1)
L- F	0 / 326	0.07 (2)
L- G	-254 / 0	0.21 (1)
K- G	-252 / 84	0.11 (1)
B- P	0 / 1409	0.32 (1)
K- H	0 / 1409	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.26/1.00 (O-P:1) , WB=0.32/1.00 (B-P:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.87 (B) (INPUT = 0.90)
USI METAL= 0.34 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11657-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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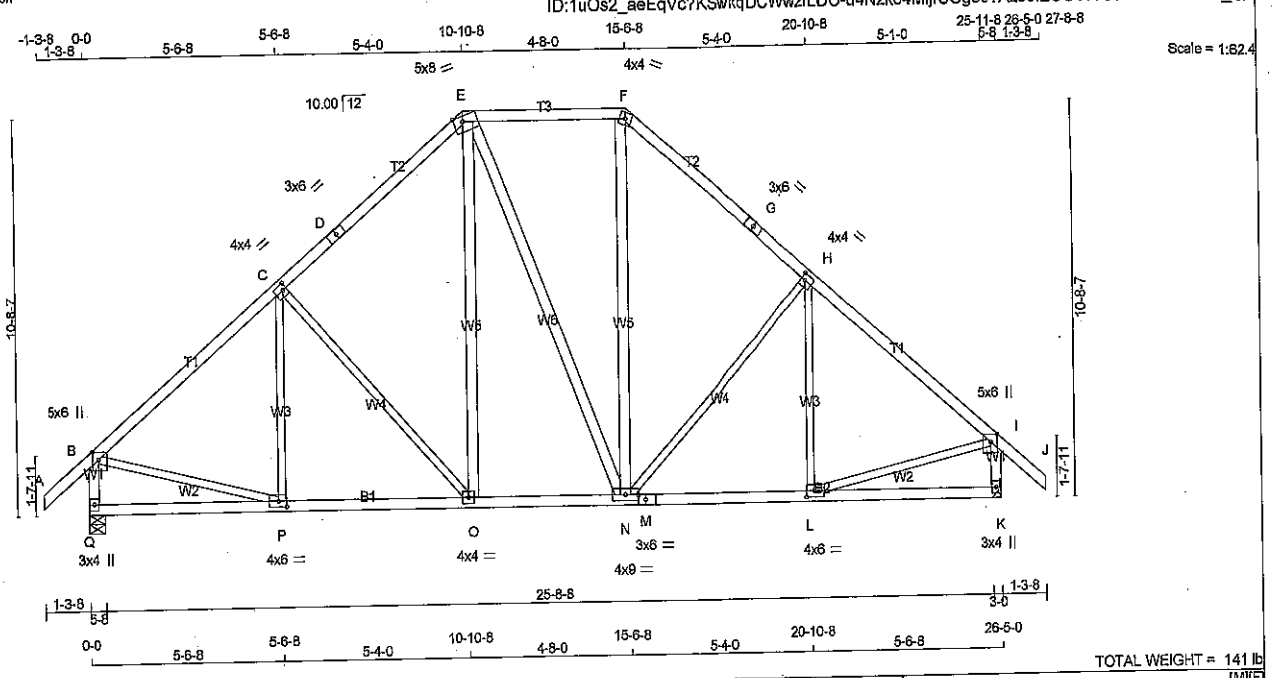

TOTAL WEIGHT = 141 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge 3.00	
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMWW+p	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BS-t	MT20	3.0	6.0	1.50	2.50
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.75
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1901	0	1901	0	0	5-8	5-8
K	1901	0	1901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1747 / 0	-104.9	-104.9	0.48 (1)	4.53
C- D	-1455 / 0	-104.9	-104.9	0.45 (1)	4.91
D- E	-1455 / 0	-104.9	-104.9	0.45 (1)	4.91
E- F	-1087 / 0	-104.9	-104.9	0.31 (1)	5.68
F- G	-1455 / 0	-104.9	-104.9	0.45 (1)	4.91
G- H	-1455 / 0	-104.9	-104.9	0.45 (1)	4.91
H- I	-1747 / 0	-104.9	-104.9	0.48 (1)	4.53
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1838 / 0	0.0	0.0	0.20 (1)	6.18
K- I	-1838 / 0	0.0	0.0	0.20 (1)	6.18
Q- P	0 / 0	-28.0	-28.0	0.21 (3)	10.00
P- O	0 / 1376	-28.0	-28.0	0.36 (2)	10.00
O- N	0 / 1087	-28.0	-28.0	0.25 (1)	10.00
N- M	0 / 1376	-28.0	-28.0	0.36 (2)	10.00
M- L	0 / 1376	-28.0	-28.0	0.36 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-149 / 174	0.09 (1)
C- O	-435 / 0	0.59 (1)
O- E	0 / 450	0.07 (1)
E- N	0 / 2	0.00 (1)
N- F	0 / 452	0.07 (1)
N- H	-434 / 0	0.59 (1)
L- H	-151 / 173	0.09 (1)
B- P	0 / 1413	0.32 (1)
L- I	0 / 1413	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.36/1.00 (O-P:2) , WB=0.59/1.00 (C-O:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.38 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11658 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T15	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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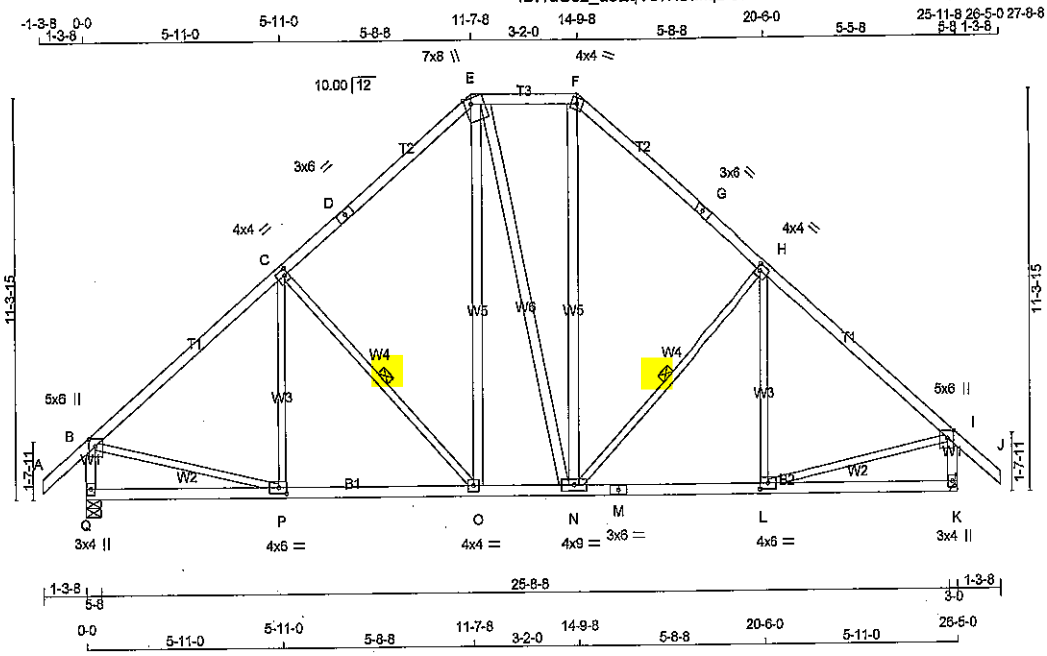

TOTAL WEIGHT = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW+p	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.75
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1901	0	1901	0	0	5-8	5-8
K	1901	0	1901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1747 / 0	-104.9	-104.9	0.55 (1)	4.43
C- D	-1399 / 0	-104.9	-104.9	0.51 (1)	4.88
D- E	-1399 / 0	-104.9	-104.9	0.51 (1)	4.88
E- F	-1043 / 0	-104.9	-104.9	0.15 (1)	5.99
F- G	-1400 / 0	-104.9	-104.9	0.51 (1)	4.88
G- H	-1400 / 0	-104.9	-104.9	0.51 (1)	4.88
H- I	-1746 / 0	-104.9	-104.9	0.55 (1)	4.43
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1835 / 0	0.0	0.0	0.20 (1)	6.18
K- I	-1834 / 0	0.0	0.0	0.20 (1)	6.18
Q- P	0 / 0	-28.0	-28.0	0.25 (3)	10.00
P- O	0 / 1378	-28.0	-28.0	0.40 (2)	10.00
O- N	0 / 1042	-28.0	-28.0	0.24 (1)	10.00
N- M	0 / 1378	-28.0	-28.0	0.39 (2)	10.00
M- L	0 / 1378	-28.0	-28.0	0.39 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-113 / 210	0.08 (1)
C- O	-501 / 0	0.25 (1)
O- E	0 / 474	0.08 (1)
E- N	0 / 4	0.00 (1)
N- F	0 / 479	0.08 (1)
N- H	-499 / 0	0.25 (1)
L- H	-116 / 208	0.08 (1)
B- P	0 / 1411	0.32 (1)
L- I	0 / 1411	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55/1.00 (B-C:1) , BC=0.40/1.00 (O-P:2) , WB=0.32/1.00 (B-P:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (P) (INPUT = 0.90)
SI METAL= 0.39 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
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PROVINCE OF ONTARIO

DWG NO. TAM 11659-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T16	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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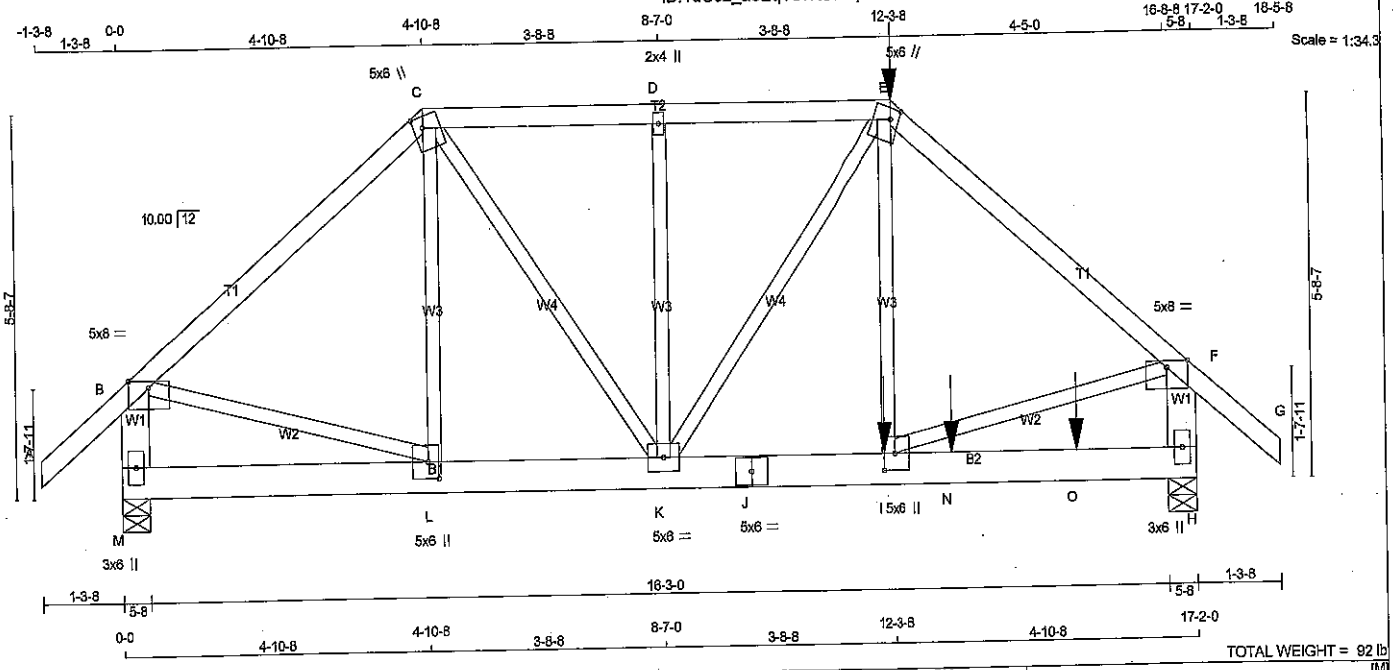

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.75
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	6.0	3.00	2.00
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	5.0	6.0	3.00	2.00
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 395.8 lbs FACTORED DOWN AT 12-3-8 ON TOP CHORD, AND 1070.2 lbs FACTORED DOWN AT 12-1-8, AND 70.4 lbs FACTORED DOWN AT 13-2-12, AND 70.4 lbs FACTORED DOWN AT 15-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1722	0	1722	0	0	5-8	5-8
H	2393	0	2393	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1337	880 / 0	225 / 0	0 / 0	0 / 0	231 / 0	0 / 0
H	1859	1221 / 0	315 / 0	0 / 0	0 / 0	323 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	
B- C	-1521 / 0	-104.9	-104.9	0.58 (1)	4.57	
C- D	-1652 / 0	-104.9	-104.9	0.29 (1)	4.80	
D- E	-1652 / 0	-104.9	-104.9	0.29 (1)	4.80	
E- F	-2279 / 0	-104.9	-104.9	0.68 (1)	3.77	
F- G	0 / 47	-104.9	-104.9	0.16 (1)	10.00	
M- B	-1664 / 0	0.0	0.0	0.12 (1)	7.62	
H- F	-2294 / 0	0.0	0.0	0.17 (1)	6.73	
M- L	0 / 0	-28.0	-28.0	0.08 (2)	10.00	
L- K	0 / 1163	-28.0	-28.0	0.19 (1)	10.00	
K- J	0 / 1763	-28.0	-28.0	0.26 (1)	10.00	
J- I	0 / 1763	-28.0	-28.0	0.26 (1)	10.00	
I- N	0 / 0	-28.0	-28.0	0.16 (2)	10.00	
N- O	0 / 0	-28.0	-28.0	0.16 (2)	10.00	
O- H	0 / 0	-28.0	-28.0	0.16 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-139 / 141	0.07 (1)
C- K	0 / 854	0.21 (1)
K- D	-457 / 0	0.22 (1)
K- E	-194 / 0	0.14 (1)
I- E	0 / 811	0.20 (1)
B- L	0 / 1200	0.30 (1)
I- F	0 / 1799	0.45 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	12-3-8	-396	-396	---	BACK	VERT	TOTAL
I	12-1-8	-1070	-1070	---	BACK	VERT	TOTAL
N	13-2-12	-40	-70	---	BACK	VERT	TOTAL
O	15-2-12	-37	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.26/1.00 (I-K:1) , WB=0.45/1.00 (F-I:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11660.08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T17	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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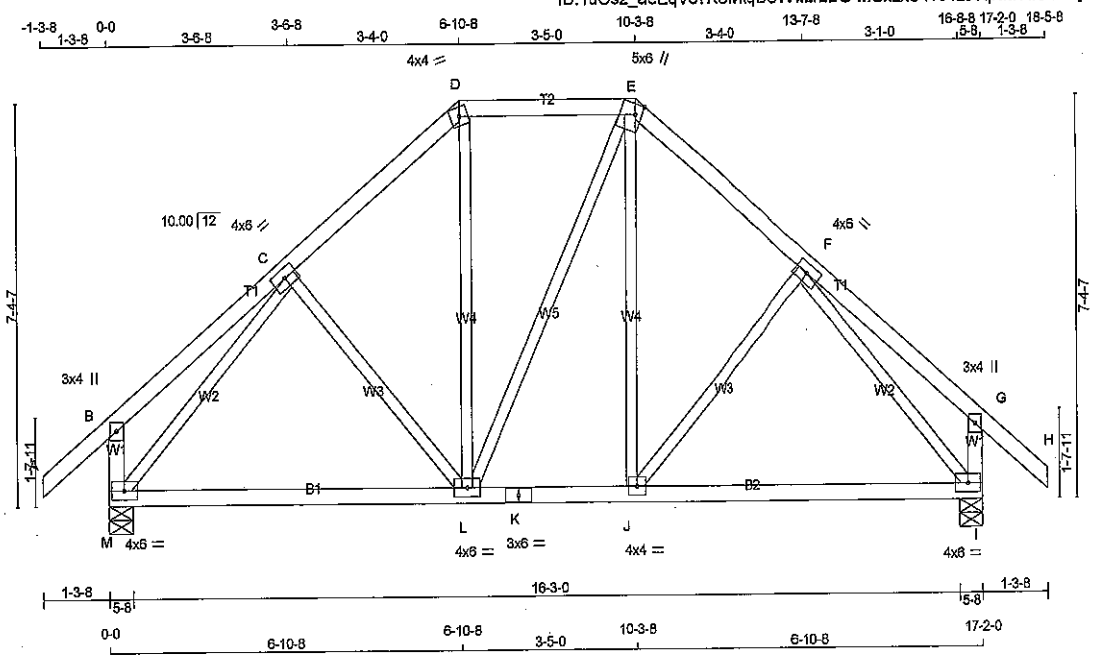

TOTAL WEIGHT = 85 lb

[M]IF

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1286	0	1286	0	0	5-8	5-8
I	1286	0	1286	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
M	1008	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
I	1008	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.19 (1)	10.00
C- D	-912 / 0	-104.9	-104.9	0.16 (1)	6.25
D- E	-682 / 0	-104.9	-104.9	0.16 (1)	6.25
E- F	-911 / 0	-104.9	-104.9	0.16 (1)	6.25
F- G	0 / 26	-104.9	-104.9	0.19 (1)	10.00
G- H	0 / 47	-104.9	-104.9	0.14 (1)	10.00
M- B	-283 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-283 / 0	0.0	0.0	0.03 (1)	7.81
M- L	0 / 757	-28.0	-28.0	0.36 (2)	10.00
L- K	0 / 682	-28.0	-28.0	0.34 (2)	10.00
K- J	0 / 682	-28.0	-28.0	0.34 (2)	10.00
J- I	0 / 757	-28.0	-28.0	0.36 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- L	-120 / 56	0.06 (1)
L- D	0 / 272	0.06 (2)
L- E	0 / 1	0.00 (2)
J- E	0 / 270	0.06 (2)
J- F	-120 / 56	0.06 (1)
M- C	-1192 / 0	0.56 (1)
F- I	-1191 / 0	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.19/1.00 (F-G:1) , BC=0.36/1.00 (I-J:2) , WB=0.56/1.00 (C-M:1) , SSI=0.15/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.79 (C) (INPUT = 0.90)
LS METAL= 0.43 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11661-B
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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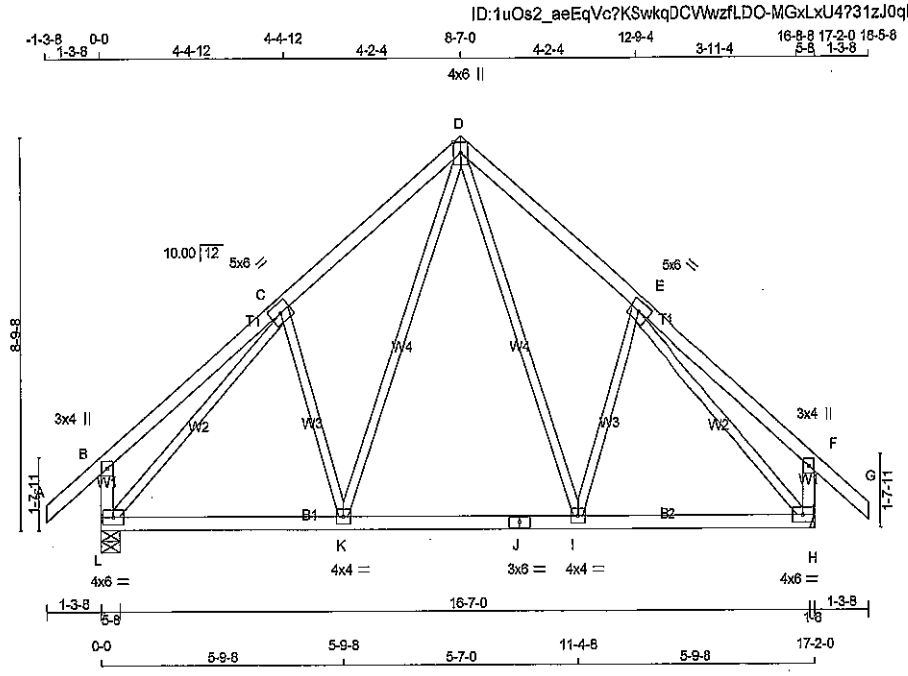

TOTAL WEIGHT = 2 X 84 = 168 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1286	0	1286	0	0	5-8	5-8
H	1286	0	1286	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1008	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1008	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 34	-104.9	-104.9	0.31 (1)	10.00
C- D	-963 / 0	-104.9	-104.9	0.25 (1)	6.03
D- E	-963 / 0	-104.9	-104.9	0.25 (1)	6.03
E- F	0 / 34	-104.9	-104.9	0.31 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- B	-315 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-315 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 793	-28.0	-28.0	0.29 (2)	10.00
K- J	0 / 576	-28.0	-28.0	0.27 (2)	10.00
J- I	0 / 576	-28.0	-28.0	0.27 (2)	10.00
I- H	0 / 793	-28.0	-28.0	0.29 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 442	0.10 (1)
I- E	-259 / 49	0.12 (1)
K- D	0 / 442	0.10 (1)
C- K	-259 / 49	0.12 (1)
L- C	-1203 / 0	0.88 (1)
E- H	-1203 / 0	0.88 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.29/1.00 (K-L:2) , WB=0.88/1.00 (C-L:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (C) (INPUT = 0.90)
JSI METAL= 0.32 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11662-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T19	2	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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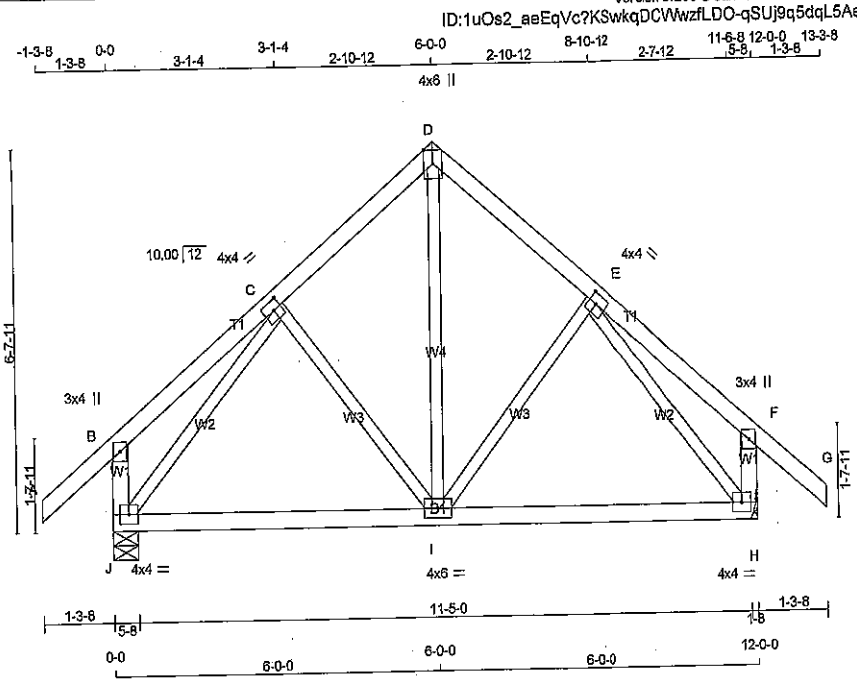

TOTAL WEIGHT = 2 X 58 = 116 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	943	0	943	0	0	5-8	5-8
H	943	0	943	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	734	480 / 0	126 / 0	0 / 0	0 / 0	128 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 23	-104.9	-104.9	0.15 (1)	10.00
C- D	-540 / 0	-104.9	-104.9	0.11 (1)	6.25
D- E	-540 / 0	-104.9	-104.9	0.11 (1)	6.25
E- F	0 / 23	-104.9	-104.9	0.15 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- B	-266 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-266 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 481	-28.0	-28.0	0.34 (2)	10.00
I- H	0 / 481	-28.0	-28.0	0.34 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 415	0.09 (1)
I- E	-133 / 39	0.05 (1)
C- I	-133 / 39	0.05 (1)
J- C	-779 / 0	0.29 (1)
E- H	-779 / 0	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.15/1.00 (B-C:1) , BC=0.34/1.00 (I-J:2) , WB=0.29/1.00 (E-H:1) , SSI=0.14/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX.	MIN.	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (C) (INPUT = 0.90)
JSI METAL= 0.29 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3018
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 21663-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G19	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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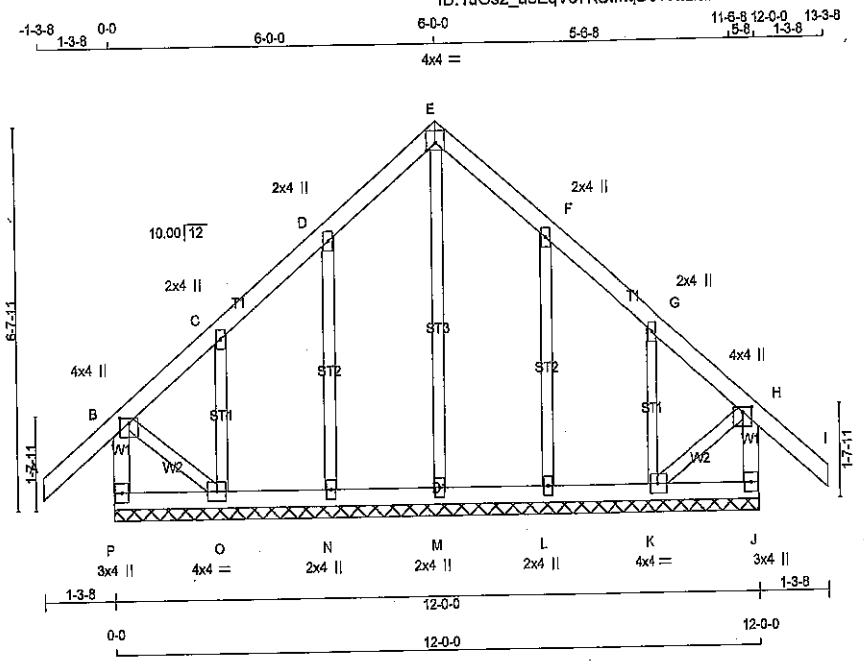

TOTAL WEIGHT = 58 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-323 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-57 / 0	-104.9	-104.9	0.14 (1)	6.25	
C- D	-12 / 0	-104.9	-104.9	0.07 (1)	6.25	
D- E	-31 / 0	-104.9	-104.9	0.07 (1)	6.25	
E- F	-31 / 0	-104.9	-104.9	0.07 (1)	6.25	
F- G	-12 / 0	-104.9	-104.9	0.07 (1)	6.25	
G- H	-57 / 0	-104.9	-104.9	0.14 (1)	6.25	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- H	-323 / 0	0.0	0.0	0.03 (1)	7.81	
P- O	0 / 0	-28.0	-28.0	0.03 (2)	10.00	
O- N	0 / 17	-28.0	-28.0	0.03 (2)	10.00	
N- M	0 / 12	-28.0	-28.0	0.02 (2)	10.00	
M- L	0 / 12	-28.0	-28.0	0.02 (2)	10.00	
L- K	0 / 17	-28.0	-28.0	0.03 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.03 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-153 / 0	0.11 (1)
N- D	-251 / 0	0.09 (1)
O- C	-139 / 0	0.03 (1)
L- F	-251 / 0	0.09 (1)
K- G	-139 / 0	0.03 (1)
B- O	0 / 26	0.01 (1)
K- H	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (N-O:2) , WB=0.11/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11692-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	G19A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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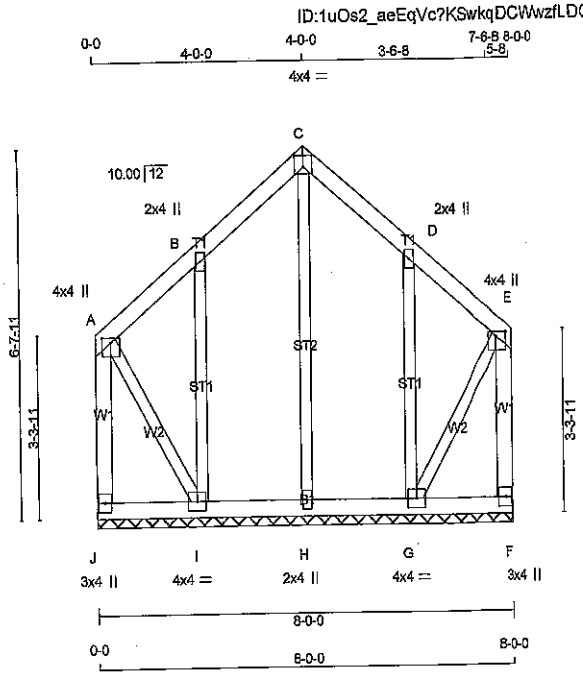

TOTAL WEIGHT = 48 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-110 / 0	0.0	0.0	0.02 (1)	7.81
A- B	-4 / 0	-104.9	-104.9	0.07 (1)	10.00
B- C	-29 / 0	-104.9	-104.9	0.07 (1)	6.25
C- D	-29 / 0	-104.9	-104.9	0.07 (1)	6.25
D- E	-4 / 0	-104.9	-104.9	0.07 (1)	10.00
F- E	-110 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 10	-28.0	-28.0	0.03 (2)	10.00
H- G	0 / 10	-28.0	-28.0	0.03 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-155 / 0	0.11 (1)
I- B	-255 / 0	0.09 (1)
G- D	-255 / 0	0.09 (1)
A- I	0 / 28	0.01 (1)
G- E	0 / 28	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (B-C:1) , BC=0.03/1.00 (H-I:2) , WB=0.11/1.00 (C-H:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (C) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-2-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11664-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T20	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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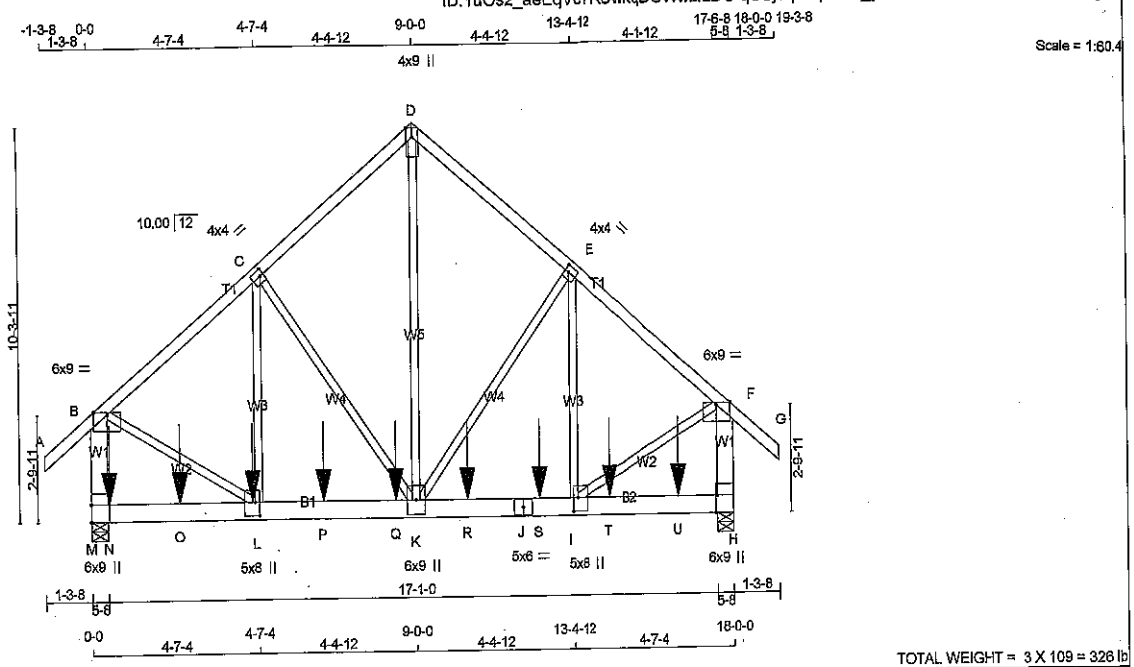

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
M- B	2	11	SIDE(130.8)
H- F	2	11	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	7	SIDE(748.4)
J- H	2	7	SIDE(514.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	9.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW-p	MT20	6.0	9.0	Edge	
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW+t	MT20	5.0	8.0	4.25	1.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW+t	MT20	6.0	9.0		
L	BMWW+t	MT20	5.0	8.0	4.25	1.50
M	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	10637	0	10637	0	0	5-8	5-8
H	9629	0	9629	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	8375	5309 / 0	1545 / 0	0 / 0	0 / 0	1520 / 0	0 / 0
H	7539	4853 / 0	1343 / 0	0 / 0	0 / 0	1343 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.05 (1)	10.00
B- C	-7495 / 0	-104.9	-104.9	0.21 (1)	4.24
C- D	-6143 / 0	-104.9	-104.9	0.16 (1)	4.63
D- E	-6142 / 0	-104.9	-104.9	0.16 (1)	4.63
E- F	-7594 / 0	-104.9	-104.9	0.22 (1)	4.21
F- G	0 / 47	-104.9	-104.9	0.05 (1)	10.00
M- B	-8158 / 0	0.0	0.0	0.26 (1)	6.33
H- F	-8261 / 0	0.0	0.0	0.26 (1)	6.30
M- N	0 / 0	-28.0	-28.0	0.41 (1)	10.00
N- O	0 / 0	-28.0	-28.0	0.41 (1)	10.00
O- L	0 / 0	-28.0	-28.0	0.41 (1)	10.00
L- P	0 / 5777	-28.0	-28.0	0.57 (1)	10.00
P- Q	0 / 5777	-28.0	-28.0	0.57 (1)	10.00
Q- K	0 / 5777	-28.0	-28.0	0.57 (1)	10.00
K- R	0 / 5853	-28.0	-28.0	0.69 (1)	10.00
R- J	0 / 5853	-28.0	-28.0	0.69 (1)	10.00
J- S	0 / 5853	-28.0	-28.0	0.69 (1)	10.00
S- I	0 / 5853	-28.0	-28.0	0.69 (1)	10.00
I- T	0 / 0	-28.0	-28.0	0.47 (1)	10.00
T- U	0 / 0	-28.0	-28.0	0.47 (1)	10.00
U- H	0 / 0	-28.0	-28.0	0.47 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 7452	0.56 (1)
K- E	-2001 / 0	0.76 (1)
I- E	0 / 2045	0.15 (1)
C- K	-1869 / 0	0.71 (1)
L- C	0 / 1873	0.14 (1)
B- L	0 / 6508	0.49 (1)
I- F	0 / 6594	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
L	4-5-4	-1808	-1808	---	BACK	VERT	TOTAL
N	6-0	-2131	-2131	---	BACK	VERT	TOTAL
O	2-5-4	-1808	-1808	---	BACK	VERT	TOTAL
P	6-5-4	-1973	-1973	---	BACK	VERT	TOTAL
Q	8-5-4	-1973	-1973	---	BACK	VERT	TOTAL
R	10-5-4	-1973	-1973	---	BACK	VERT	TOTAL
S	12-5-4	-1973	-1973	---	BACK	VERT	TOTAL
T	14-5-4	-1973	-1973	---	BACK	VERT	TOTAL
U	16-5-4	-1973	-1973	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.26/1.00 (F-H:1) , BC=0.69/1.00 (I-K:1) , WB=0.76/1.00 (E-K:1) , SSI=0.54/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
STAT SOULAKOS
PROVINCE OF ONTARIO

P6/4

DWG NO. TAM 116665-18

STRUCTURAL COMPONENT CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T20	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2130.6 lbs FACTORED DOWN AT 6-0, 1807.7 lbs FACTORED DOWN AT 2-5-4, 1807.7 lbs FACTORED DOWN AT 4-5-4, 1972.8 lbs FACTORED DOWN AT 6-5-4, 1972.8 lbs FACTORED DOWN AT 8-5-4, 1972.8 lbs FACTORED DOWN AT 10-5-4, 1972.8 lbs FACTORED DOWN AT 12-5-4, AND 1972.8 lbs FACTORED DOWN AT 14-5-4, AND 1972.8 lbs FACTORED DOWN AT 16-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 71665 -B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	G20	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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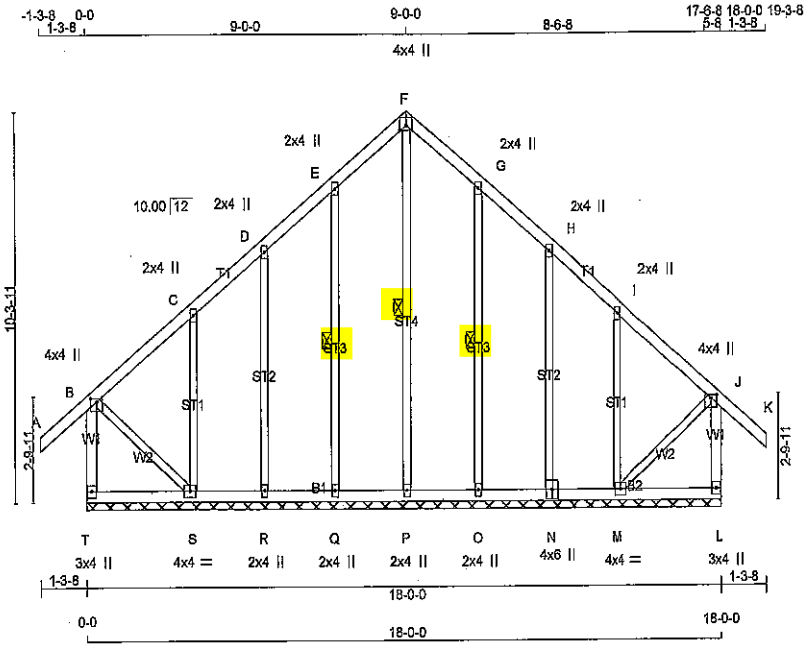

TOTAL WEIGHT = 102 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BSW1+l	MT20	4.0	6.0		
O, P, Q, R						
O	BMW1+w	MT20	2.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-P, E-Q, G-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- B	-307 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-11 / 0	-104.9	-104.9	0.11 (1)	6.25
C- D	-39 / 0	-104.9	-104.9	0.11 (1)	6.25
D- E	-12 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-28 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-28 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-11 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-36 / 0	-104.9	-104.9	0.11 (1)	6.25
I- J	-9 / 0	-104.9	-104.9	0.11 (1)	10.00
J- K	0 / 47	-104.9	-104.9	0.14 (1)	10.00
L- J	-304 / 0	0.0	0.0	0.05 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.06 (2)	10.00
S- R	0 / 16	-28.0	-28.0	0.06 (2)	10.00
R- Q	0 / 14	-28.0	-28.0	0.03 (2)	10.00
Q- P	0 / 11	-28.0	-28.0	0.03 (2)	10.00
P- O	0 / 11	-28.0	-28.0	0.03 (2)	10.00
O- N	0 / 14	-28.0	-28.0	0.03 (2)	10.00
N- M	0 / 14	-28.0	-28.0	0.06 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- F	-158 / 0	0.11 (1)
Q- E	-244 / 0	0.12 (1)
R- D	-170 / 0	0.14 (1)
S- C	-307 / 0	0.13 (1)
O- G	-244 / 0	0.12 (1)
N- H	-172 / 0	0.14 (1)
M- I	-307 / 0	0.13 (1)
B- S	0 / 29	0.01 (1)
M- J	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.06/1.00 (R-S:2) , WB=0.14/1.00 (H-N:1) , SSI=0.11/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (F) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11666-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287569	T21	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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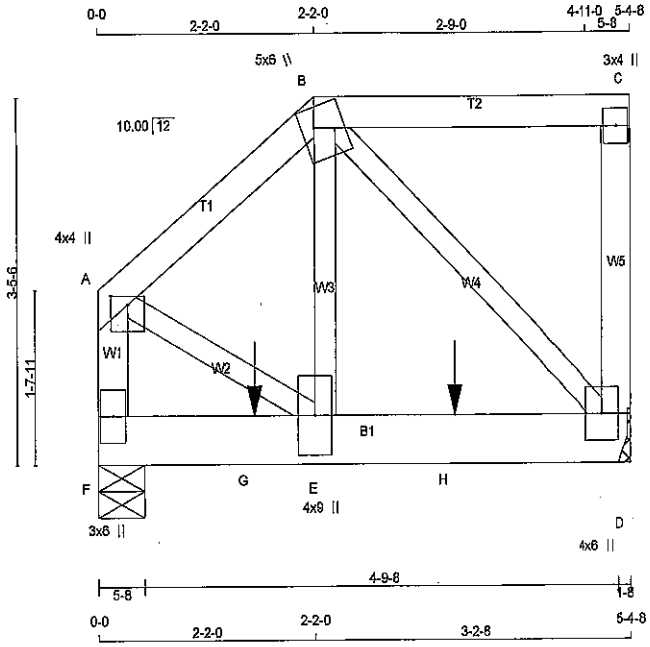

TOTAL WEIGHT = 2 X 28 = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1799.8 lbs FACTORED DOWN AT 1-6-12, AND 1799.8 lbs FACTORED DOWN AT 3-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	2074	0	2074	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	2240	0	2240	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1641	1026 / 0	312 / 0	0 / 0	0 / 0	303 / 0	0 / 0
F	1772	1109 / 0	337 / 0	0 / 0	0 / 0	327 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1743 / 0	-104.9	-104.9	0.06 (1)	6.25	
B- C	0 / 0	-104.9	-104.9	0.10 (1)	10.00	
D- C	-168 / 0	0.0	0.0	0.02 (1)	7.81	
F- A	-1955 / 0	0.0	0.0	0.11 (1)	7.81	
F- G	0 / 0	-28.0	-28.0	0.26 (1)	10.00	
G- E	0 / 0	-28.0	-28.0	0.26 (1)	10.00	
E- H	0 / 1407	-28.0	-28.0	0.45 (1)	10.00	
H- D	0 / 1407	-28.0	-28.0	0.45 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2181	0.27 (1)
B- D	-1941 / 0	0.29 (1)
A- E	0 / 1513	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-6-12	-1800	-1800	—	BACK	VERT	TOTAL
H	3-6-12	-1800	-1800	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.11/1.00 (A-F:1) , BC=0.45/1.00 (D-E:1) , WB=0.29/1.00 (B-D:1) , SSI=0.52/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.86 (A) (INPUT = 0.90)
USI METAL= 0.27 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11689-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T21S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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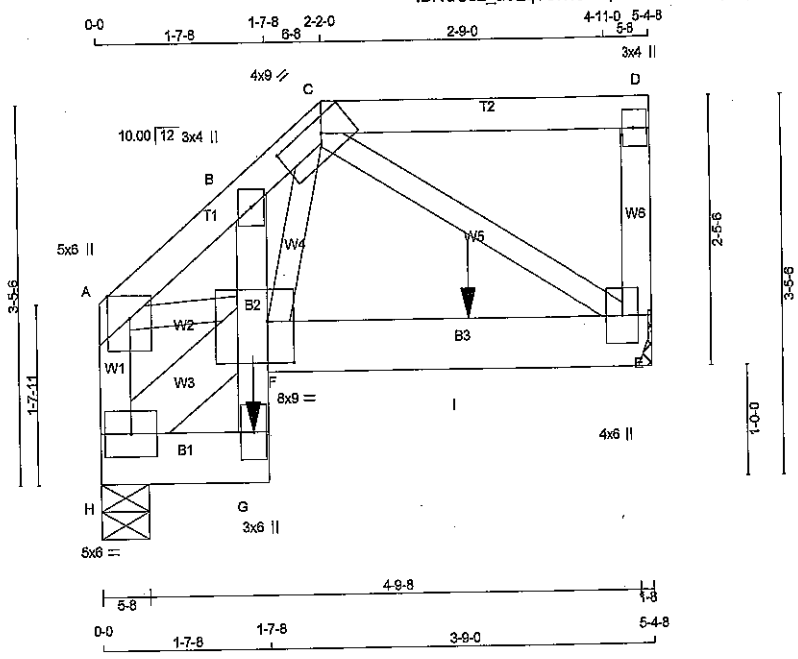

TOTAL WEIGHT = 2 X 30 = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
H- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- G	2	12	SIDE(122.0)
F- E	2	12	SIDE(0.0)
G- B	1	2	SIDE(354.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMV+p	MT20	3.0	4.0		
C	TTWW-h	MT20	4.0	9.0	1.50	5.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	4.0	6.0		
F	BVMWWW-I	MT20	8.0	9.0	4.50	3.00
G	BMV+p	MT20	3.0	6.0		
H	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	2061	0	2061	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	2291	0	2291	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1638	1012 / 0	319 / 0	0 / 0	0 / 0	308 / 0	0 / 0
H	1823	1123 / 0	358 / 0	0 / 0	0 / 0	342 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2758 / 0	-104.9	-104.9	0.14 (1)	5.35	
B- C	-2675 / 0	-104.9	-104.9	0.14 (1)	5.42	
C- D	0 / 0	-104.9	-104.9	0.10 (1)	10.00	
E- D	-168 / 0	0.0	0.0	0.01 (1)	7.81	
H- A	-2215 / 0	0.0	0.0	0.13 (1)	7.49	
H- G	0 / 82	-28.0	-28.0	0.01 (2)	10.00	
G- F	0 / 1846	0.0	0.0	0.22 (1)	10.00	
F- B	0 / 106	0.0	0.0	0.07 (1)	10.00	
F- I	0 / 1360	-28.0	-28.0	0.75 (1)	10.00	
I- E	0 / 1360	-28.0	-28.0	0.75 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- E	-1614 / 0	0.21 (1)
F- C	0 / 2413	0.30 (1)
H- F	-99 / 0	0.00 (1)
A- F	0 / 2085	0.26 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-5-12	-1825	-1825	---	BACK	VERT	TOTAL
I	3-6-12	-1812	-1812	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 915 (0.07")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.75/1.00 (E-F:1) , WB=0.30/1.00 (C-F:1) , SSI=0.34/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1556

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.42 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11667 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T21S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1825.2 lbs FACTORED DOWN AT 1-5-12, AND 1812.3 lbs FACTORED DOWN AT 3-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11667-8
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T22	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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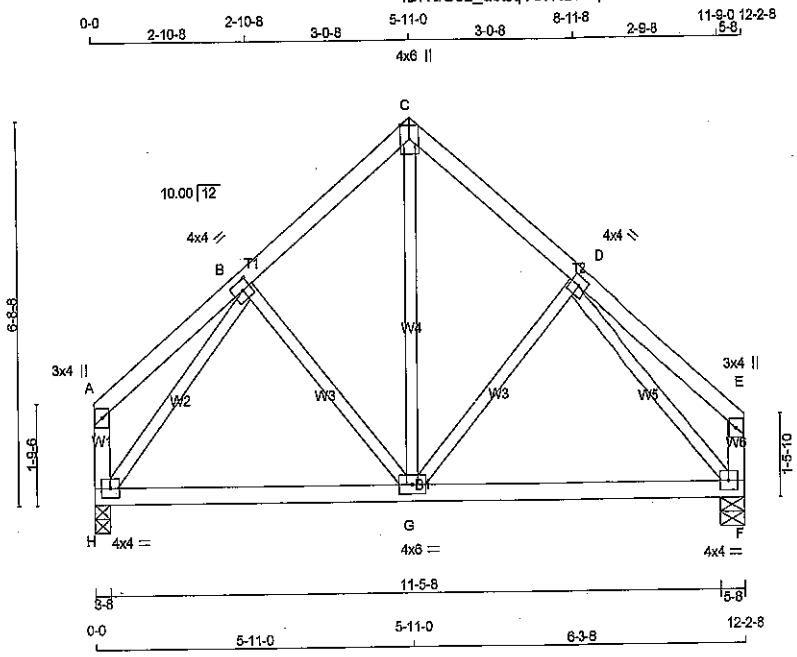

TOTAL WEIGHT = 2 X 55 = 110 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	811	0	811	0	0	3-8	3-8
F	811	0	811	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	647	396 / 0	128 / 0	0 / 0	0 / 0	122 / 0	0 / 0
F	647	396 / 0	128 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-104.9	-104.9	0.14 (1)	10.00	
B- C	-551 / 0	-104.9	-104.9	0.11 (1)	6.25	
C- D	-554 / 0	-104.9	-104.9	0.13 (1)	6.25	
D- E	0 / 24	-104.9	-104.9	0.16 (1)	10.00	
H- A	-106 / 0	0.0	0.0	0.01 (1)	7.81	
F- E	-126 / 0	0.0	0.0	0.01 (1)	7.81	
H- G	0 / 473	-28.0	-28.0	0.35 (2)	10.00	
G- F	0 / 515	-28.0	-28.0	0.35 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- G	-101 / 50	0.04 (1)
G- C	0 / 415	0.09 (1)
G- D	-167 / 30	0.07 (1)
H- B	-798 / 0	0.27 (1)
D- F	-804 / 0	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.16/1.00 (D-E:1) , BC=0.35/1.00 (F-G:2) , WB=0.30/1.00 (D-F:1) , SSI=0.15/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 116608-38
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T22A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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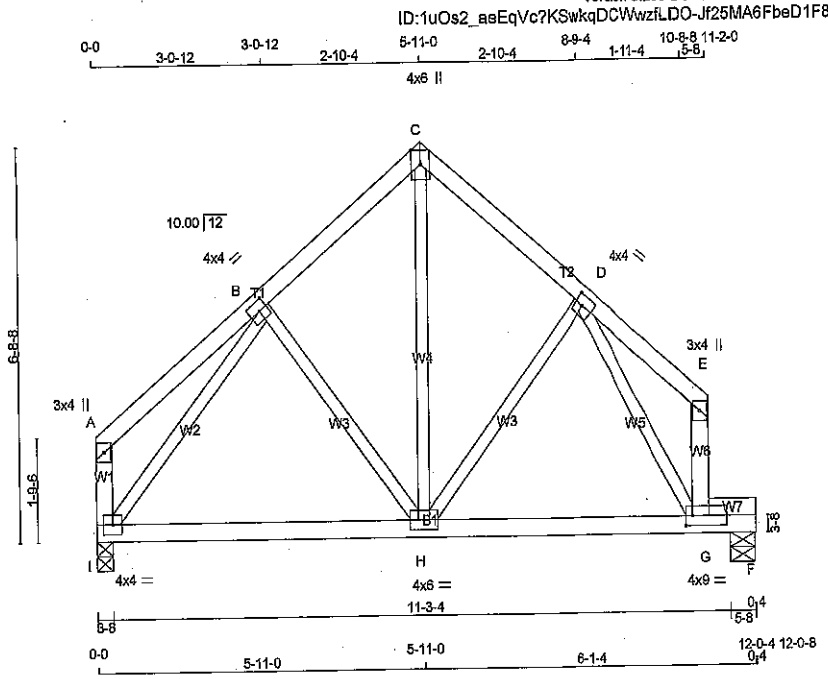

TOTAL WEIGHT = 55 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0	2.00	1.50
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	712	0	712	0	0	5-8	5-8
I	796	0	796	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	578	337 / 0	126 / 0	0 / 0	0 / 0	115 / 0	0 / 0
I	635	388 / 0	126 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-104.9	-104.9	0.14 (1)	10.00
B- C	-556 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-553 / 0	-104.9	-104.9	0.11 (1)	6.25
D- E	0 / 22	-104.9	-104.9	0.11 (1)	10.00
I- A	-119 / 0	0.0	0.0	0.01 (1)	7.81
G- E	-87 / 0	0.0	0.0	0.01 (1)	7.81
I- H	0 / 479	-28.0	-28.0	0.43 (2)	10.00
H- G	0 / 405	-28.0	-28.0	0.88 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.34 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	-110 / 47	0.04 (1)
H- C	0 / 436	0.10 (1)
H- D	0 / 84	0.02 (3)
I- B	-795 / 0	0.30 (1)
D- G	-799 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 670 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 416 (0.35")

CSI: TC=0.14/1.00 (B-C:1) , BC=0.88/1.00 (G-H:1) , WB=0.30/1.00 (B-I:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.40 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-9-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11669-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	G22A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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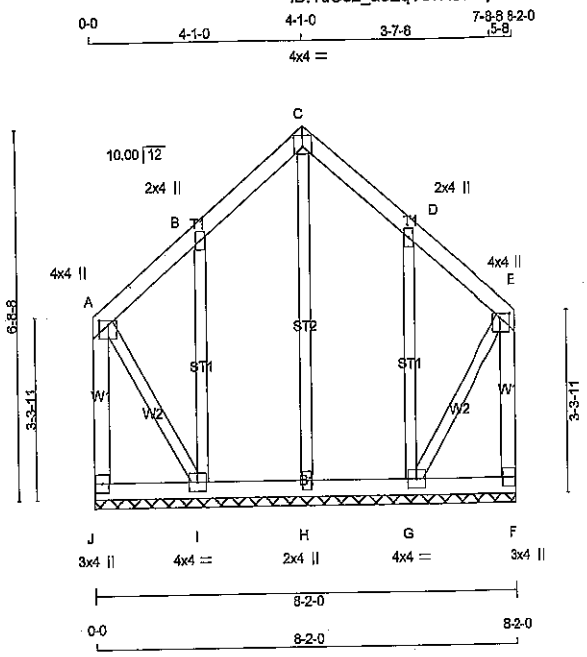

TOTAL WEIGHT = 46 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.00	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-114 / 0	0.0	0.0	0.02 (1)	7.81
A- B	-4 / 0	-104.9	-104.9	0.07 (1)	10.00
B- C	-29 / 0	-104.9	-104.9	0.07 (1)	6.25
C- D	-29 / 0	-104.9	-104.9	0.07 (1)	6.25
D- E	-4 / 0	-104.9	-104.9	0.07 (1)	10.00
F- E	-114 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 10	-28.0	-28.0	0.03 (2)	10.00
H- G	0 / 10	-28.0	-28.0	0.03 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-153 / 0	0.11 (1)
I- B	-260 / 0	0.10 (1)
G- D	-260 / 0	0.10 (1)
A- I	0 / 27	0.01 (1)
G- E	0 / 27	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (B-C:1) , BC=0.03/1.00 (G-H:2) , WB=0.11/1.00 (C-H:1) , SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (C) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11670

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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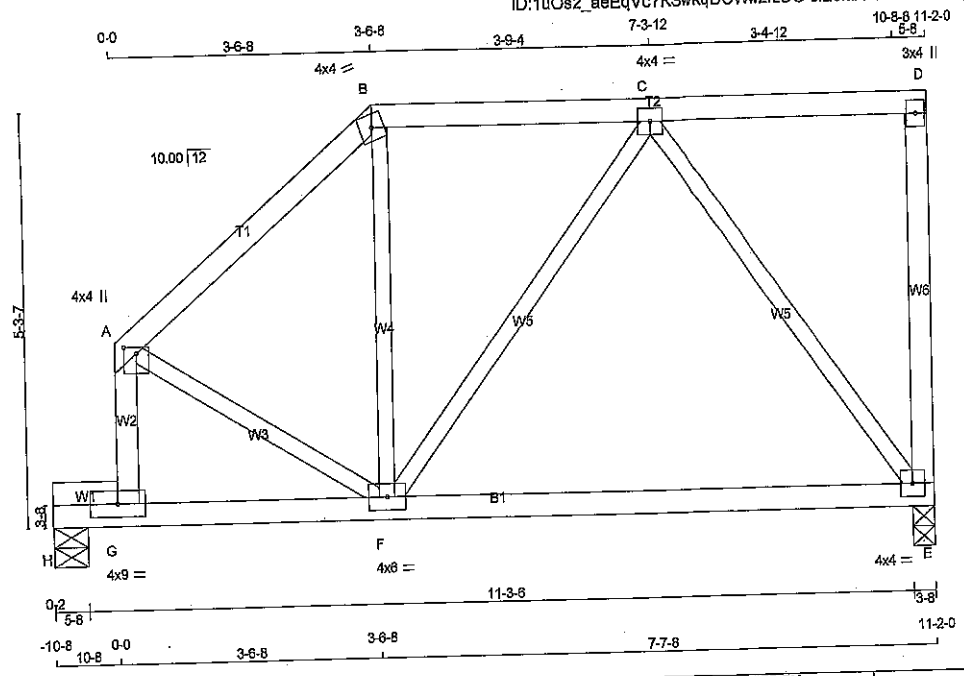

TOTAL WEIGHT = 53 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-m	MT20	4.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	4.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	796	0	796	0	0	3-8	3-8
H	712	0	712	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	635	389 / 0	126 / 0	0 / 0	0 / 0	120 / 0	0 / 0
H	578	337 / 0	126 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-642 / 0	-104.9	-104.9	0.24 (1)	6.25
B- C	-493 / 0	-104.9	-104.9	0.26 (1)	6.25
C- D	0 / 0	-104.9	-104.9	0.25 (1)	10.00
E- D	-154 / 0	0.0	0.0	0.07 (1)	7.81
G- A	-869 / 0	0.0	0.0	0.11 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.34 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.82 (1)	10.00
F- E	0 / 439	-28.0	-28.0	0.48 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	0 / 183	0.04 (3)
A- F	0 / 561	0.13 (1)
F- C	0 / 187	0.04 (3)
C- E	-718 / 0	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 761 (0.19")

CSI: TC=0.26/1.00 (B-C:1) , BC=0.82/1.00 (F-G:1) , WB=0.49/1.00 (C-E:1) , SSI=0.28/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (G) (INPUT = 0.90)
SI METAL= 0.45 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-2-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11671 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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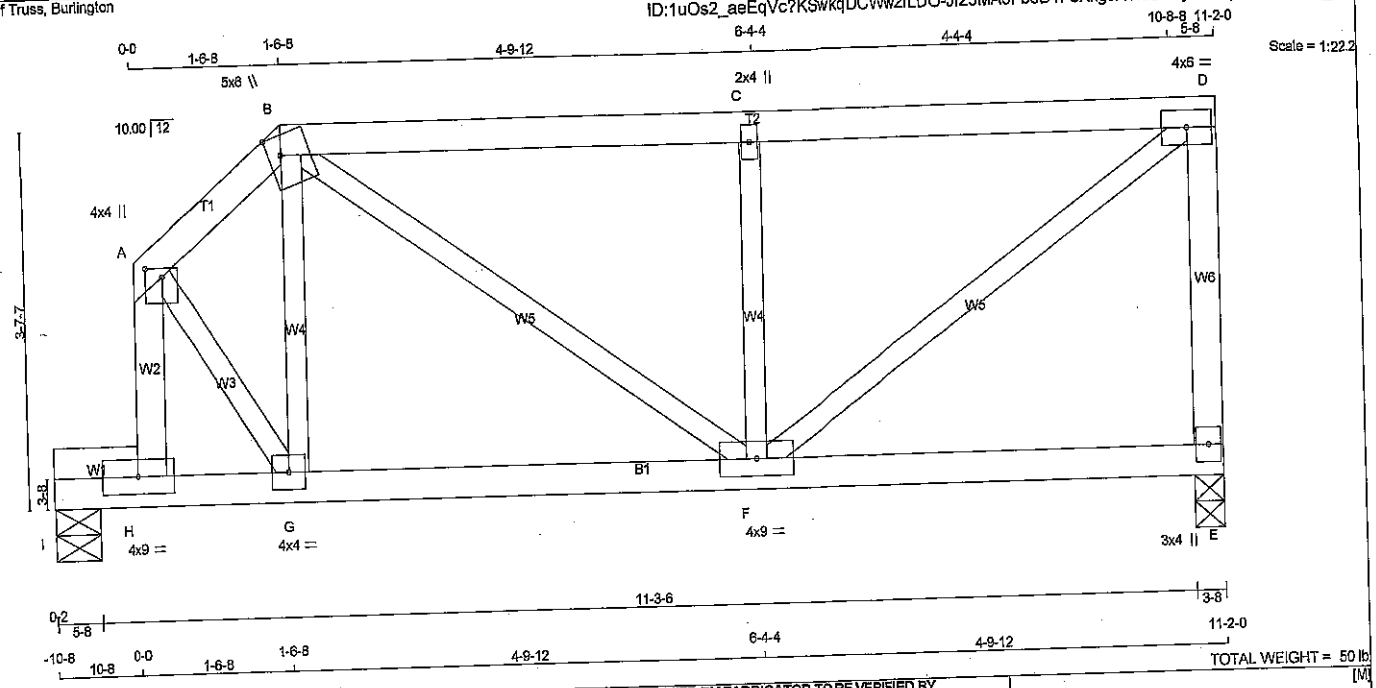

TOTAL WEIGHT = 50 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMVW-t	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMVW-t	MT20	4.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	796	0	796	0	0	3-8	3-8
I	712	0	712	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	635	389 / 0	126 / 0	0 / 0	0 / 0	120 / 0	0 / 0
I	578	337 / 0	126 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-650 / 0	-104.9	-104.9	0.05 (1)	6.25
B- C	-788 / 0	-104.9	-104.9	0.41 (1)	6.16
C- D	-788 / 0	-104.9	-104.9	0.41 (1)	6.16
E- D	-736 / 0	0.0	0.0	0.15 (1)	7.81
H- A	-1103 / 0	0.0	0.0	0.14 (1)	7.54
I- H	0 / 0	-28.0	-28.0	0.34 (1)	10.00
H- G	0 / 0	-28.0	-28.0	0.82 (1)	10.00
G- F	0 / 494	-28.0	-28.0	0.24 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-59 / 97	0.02 (3)
B- F	0 / 358	0.08 (1)
F- C	-621 / 0	0.13 (1)
F- D	0 / 956	0.22 (1)
A- G	0 / 760	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.82/1.00 (G-H:1) , WB=0.22/1.00 (D-F:1) , SSI=0.36/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.45 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/02/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11672-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	G25	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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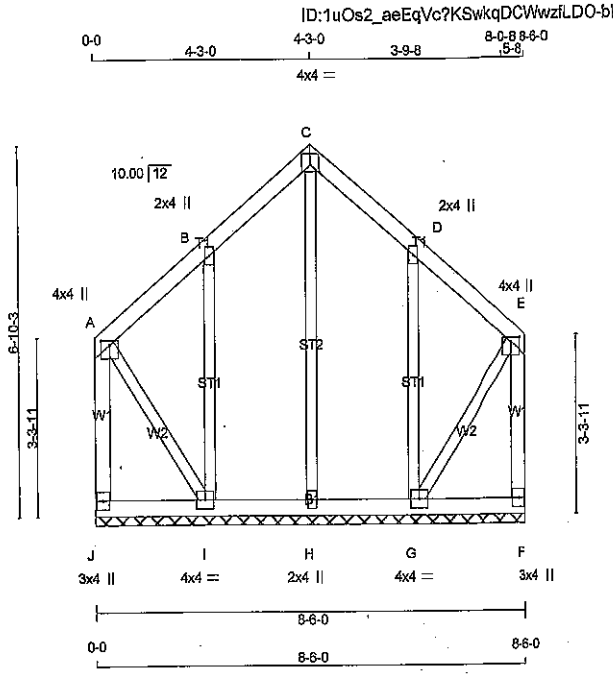

TOTAL WEIGHT = 48 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-121 / 0	0.0	0.0	0.02 (1)	7.81
A- B	-4 / 0	-104.9	-104.9	0.08 (1)	10.00
B- C	-30 / 0	-104.9	-104.9	0.08 (1)	6.25
C- D	-30 / 0	-104.9	-104.9	0.08 (1)	6.25
D- E	-4 / 0	-104.9	-104.9	0.08 (1)	10.00
F- E	-121 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 0	-28.0	-28.0	0.03 (2)	10.00
I- H	0 / 10	-28.0	-28.0	0.03 (2)	10.00
H- G	0 / 10	-28.0	-28.0	0.03 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-149 / 0	0.11 (1)
I- B	-271 / 0	0.11 (1)
G- D	-271 / 0	0.11 (1)
A- I	0 / 26	0.01 (1)
G- E	0 / 26	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1) , BC=0.03/1.00 (G-H:2) , WB=0.11/1.00 (C-H:1) , SSI=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (C) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
39218
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11673 -48
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T26S	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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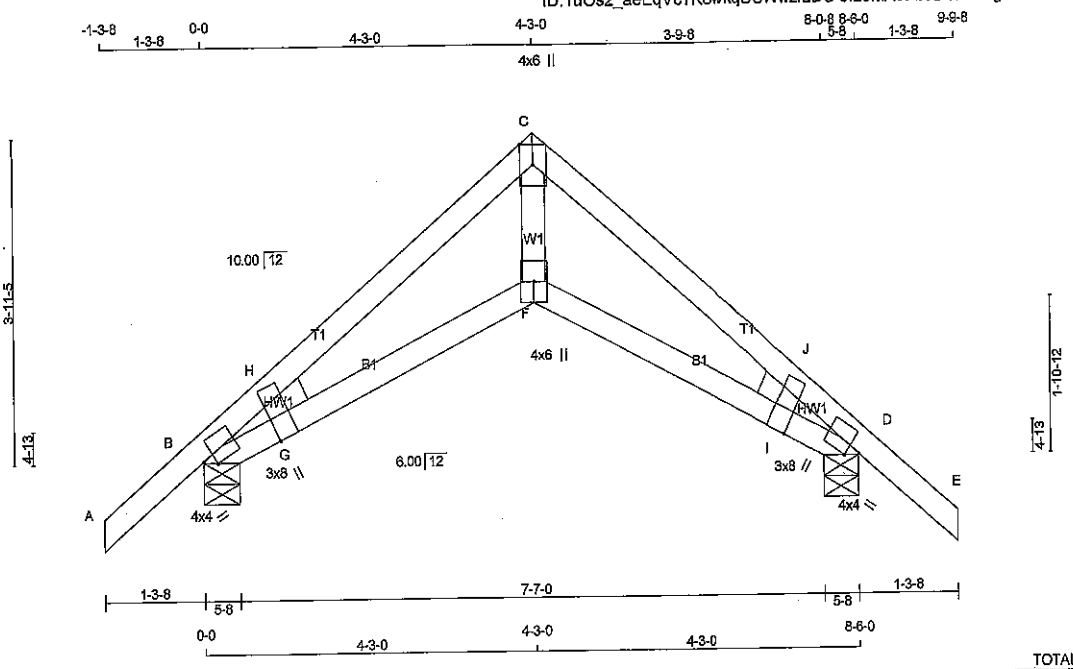

TOTAL WEIGHT = 4 X 32 = 129 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-m	MT20	4.0	4.0	Edge	1.75
B	WP+I	MT20	3.0	8.0	Edge	12.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMB1-m	MT20	4.0	4.0	Edge	1.75
D	WP+I	MT20	3.0	8.0	Edge	12.00
F	BBW+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	710	0	710	0	0	5-8	5-8	2x4 L
D	710	0	710	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	549	366 / 0	89 / 0	0 / 0	0 / 0	93 / 0	0 / 0
D	549	366 / 0	89 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-104.9	-104.9	0.14 (1)	10.00
B- H	-1052 / 0	-104.9	-104.9	0.04 (1)	6.10
H- C	-928 / 0	-104.9	-104.9	0.18 (1)	6.21
C- J	-928 / 0	-104.9	-104.9	0.18 (1)	6.21
J- D	-1052 / 0	-104.9	-104.9	0.04 (1)	6.10
D- E	0 / 39	-104.9	-104.9	0.14 (1)	10.00
B- G	0 / 756	-28.0	-28.0	0.22 (1)	10.00
G- F	0 / 802	-28.0	-28.0	0.24 (1)	10.00
F- I	0 / 802	-28.0	-28.0	0.24 (1)	10.00
I- D	0 / 756	-28.0	-28.0	0.22 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 847	0.14 (1)
G- H	-44 / 117	0.00 (1)
I- J	-44 / 117	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (C-H:1) , BC=0.24/1.00 (F-G:1) , WB=0.14/1.00 (C-F:1) , SSI=0.11/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (F) (INPUT = 0.90)
JSI METAL= 0.40 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11674-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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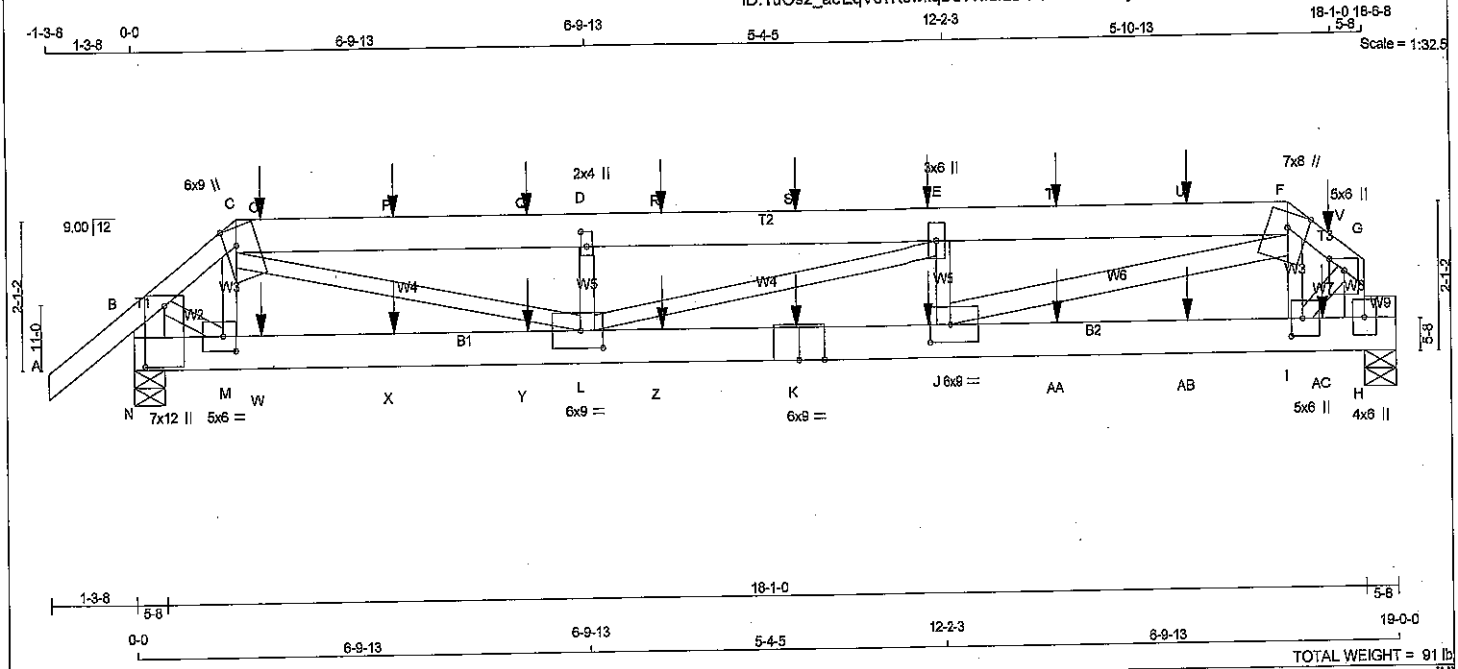

TOTAL WEIGHT = 91 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
K - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWW+t	MT20	3.0	6.0		
F	TTWW+m	MT20	7.0	8.0	Edge	
G	TMVW+p	MT20	5.0	6.0	2.00	2.50
H	BMVW1+p	MT20	4.0	6.0		
I	BMWW+t	MT20	5.0	6.0	3.00	2.00
J	BMWW-t	MT20	6.0	9.0	3.00	3.75
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	6.0	9.0	3.00	4.00
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N						
N	TMBMVW1*+p	MT20	7.0	12.0	10.25	3.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 172.5 lbs FACTORED DOWN AT 1-11-4, 139.6 lbs FACTORED DOWN AT 3-11-4, 139.6 lbs FACTORED DOWN AT 5-11-4, 139.6 lbs FACTORED DOWN AT 7-11-4, 139.6 lbs FACTORED DOWN AT 9-11-4, 139.6 lbs FACTORED DOWN AT 11-11-4, 139.6 lbs FACTORED DOWN AT 13-11-4, AND 139.6 lbs FACTORED DOWN AT 15-11-4, AND 186.5 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 79.2 lbs FACTORED DOWN AT 1-11-4, 79.2 lbs FACTORED DOWN AT 3-11-4, 79.2 lbs FACTORED DOWN AT 5-11-4, 79.2 lbs FACTORED DOWN AT 7-11-4, 79.2 lbs FACTORED DOWN AT 9-11-4, 79.2 lbs FACTORED DOWN AT 11-11-4, 79.2 lbs FACTORED DOWN AT 13-11-4, AND 79.2 lbs FACTORED DOWN AT 15-11-4, AND 87.6 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2324	0	2324	0	0	5-8	5-8
H	2340	0	2340	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1831	1159 / 0	339 / 0	0 / 0	0 / 0	333 / 0	0 / 0
H	1862	1146 / 0	356 / 0	0 / 0	0 / 0	350 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-104.9	-104.9	0.16 (1)	10.00
B- C	-2638 / 0	-104.9	-104.9	0.24 (1)	4.00
C- O	-5898 / 0	-104.9	-104.9	0.65 (1)	2.96
O- P	-5898 / 0	-104.9	-104.9	0.65 (1)	2.96
P- Q	-5898 / 0	-104.9	-104.9	0.65 (1)	2.96
Q- D	-5898 / 0	-104.9	-104.9	0.65 (1)	2.96
D- R	-5898 / 0	-104.9	-104.9	0.53 (1)	3.14
R- S	-5898 / 0	-104.9	-104.9	0.53 (1)	3.14
S- E	-5898 / 0	-104.9	-104.9	0.53 (1)	3.14
E- T	-5841 / 0	-104.9	-104.9	0.59 (1)	3.06
T- U	-5841 / 0	-104.9	-104.9	0.59 (1)	3.06
U- F	-5841 / 0	-104.9	-104.9	0.59 (1)	3.06
F- V	-1944 / 0	-104.9	-104.9	0.15 (1)	4.67
V- G	-1944 / 0	-104.9	-104.9	0.15 (1)	4.67
H- G	-2628 / 0	0.0	0.0	0.29 (1)	5.23
N- B	-2568 / 0	0.0	0.0	0.18 (1)	6.45
N- M	0 / 0	-28.0	-28.0	0.19 (1)	10.00
M- W	0 / 2104	-28.0	-28.0	0.36 (1)	10.00
W- X	0 / 2104	-28.0	-28.0	0.36 (1)	10.00
X- Y	0 / 2104	-28.0	-28.0	0.36 (1)	10.00
Y- L	0 / 2104	-28.0	-28.0	0.36 (1)	10.00
L- Z	0 / 5841	-28.0	-28.0	0.67 (1)	10.00
Z- K	0 / 5841	-28.0	-28.0	0.67 (1)	10.00
K- J	0 / 5841	-28.0	-28.0	0.67 (1)	10.00
J-AA	0 / 1619	-28.0	-28.0	0.30 (1)	10.00
AA-AB	0 / 1619	-28.0	-28.0	0.30 (1)	10.00
AB- I	0 / 1619	-28.0	-28.0	0.30 (1)	10.00
I-AC	0 / 0	-28.0	-28.0	0.17 (1)	10.00
AC- H	0 / 0	-28.0	-28.0	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- C	-146 / 141	0.03 (3)
I- F	-695 / 0	0.11 (1)
I- G	0 / 2066	0.51 (1)
B- M	0 / 2218	0.55 (1)
J- F	0 / 4427	0.78 (1)
C- L	0 / 3978	0.98 (1)
J- E	-1036 / 0	0.16 (1)
L- D	-976 / 0	0.15 (1)
L- E	0 / 59	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	11-11-4	-140	-140	---	BACK	VERT	TOTAL
J	11-11-4	-79	-79	---	BACK	VERT	TOTAL
K	9-11-4	-79	-79	---	BACK	VERT	TOTAL
O	1-11-4	-173	-173	---	BACK	VERT	TOTAL
P	3-11-4	-140	-140	---	BACK	VERT	TOTAL
Q	5-11-4	-140	-140	---	BACK	VERT	TOTAL
R	7-11-4	-140	-140	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.62")
CALCULATED VERT. DEFL.(LL) = L/ 770 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.62")
CALCULATED VERT. DEFL.(TL) = L/ 496 (0.45")

CSI: TC=0.65/1.00 (C-D:1) , BC=0.67/1.00 (J-L:1) , WB=0.98/1.00 (C-L:1) , SSI=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.96 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
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PROVINCE OF ONTARIO

DWG NO. TAM 11675 B
STRUCTURAL COMPONENT ONLY

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T27	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	9-11-4	-140	-140	—	BACK	VERT	TOTAL
T	13-11-4	-140	-140	—	BACK	VERT	TOTAL
U	15-11-4	-140	-140	—	BACK	VERT	TOTAL
V	17-11-4	-167	-187	—	BACK	VERT	TOTAL
W	1-11-4	-79	-79	—	BACK	VERT	TOTAL
X	3-11-4	-79	-79	—	BACK	VERT	TOTAL
Y	5-11-4	-79	-79	—	BACK	VERT	TOTAL
Z	7-11-4	-79	-79	—	BACK	VERT	TOTAL
AA	13-11-4	-79	-79	—	BACK	VERT	TOTAL
AB	15-11-4	-79	-79	—	BACK	VERT	TOTAL
AC	17-11-4	-85	-86	—	BACK	VERT	TOTAL

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30216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11675-B
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T30	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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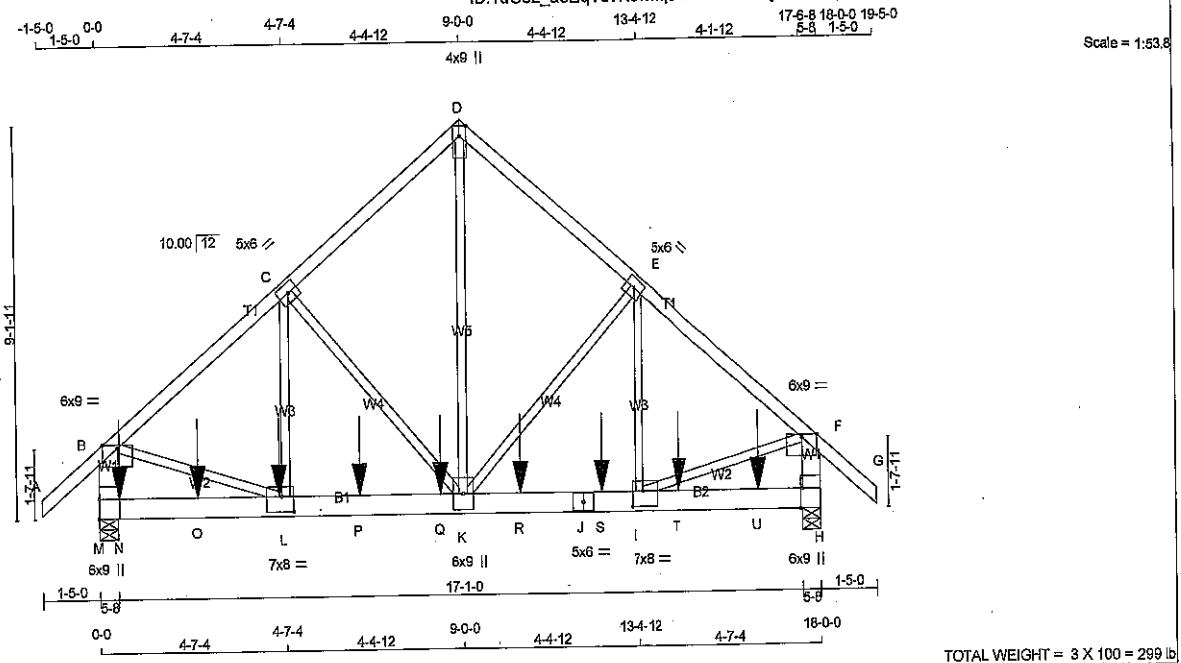

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	5	SIDE(931.9)
J- H	2	5	SIDE(514.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	6.0	9.0	Edge	
C	TMWW-t	MT20	5.0	6.0		
D	TTW+p	MT20	4.0	9.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMWW-p	MT20	6.0	9.0	Edge	
H	BMV1+t	MT20	6.0	9.0	Edge	0.50
I	BMWW-t	MT20	7.0	8.0	4.25	3.50
J	BS-t	MT20	5.0	6.0		
K	BMWWW+t	MT20	6.0	9.0		
L	BMWW-t	MT20	7.0	8.0	4.25	3.50
M	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	10650	0	10650	0	0	5-8	5-8
H	9642	0	9642	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	6384	5318 / 0	1545 / 0	0 / 0	0 / 0	1521 / 0	0 / 0
H	7547	4860 / 0	1343 / 0	0 / 0	0 / 0	1344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 51	-104.9	-104.9	0.06 (1)	10.00	
B- C	-9200 / 0	-104.9	-104.9	0.28 (1)	3.85	
C- D	-6964 / 0	-104.9	-104.9	0.18 (1)	4.39	
D- E	-6964 / 0	-104.9	-104.9	0.18 (1)	4.39	
E- F	-9321 / 0	-104.9	-104.9	0.28 (1)	3.81	
F- G	0 / 51	-104.9	-104.9	0.06 (1)	10.00	
M- B	-8149 / 0	0.0	0.0	0.18 (1)	6.33	
H- F	-8251 / 0	0.0	0.0	0.19 (1)	6.30	
M- N	0 / 0	-28.0	-28.0	0.42 (1)	10.00	
N- O	0 / 0	-28.0	-28.0	0.42 (1)	10.00	
O- L	0 / 0	-28.0	-28.0	0.42 (1)	10.00	
L- P	0 / 7085	-28.0	-28.0	0.62 (1)	10.00	
P- Q	0 / 7085	-28.0	-28.0	0.62 (1)	10.00	
Q- K	0 / 7085	-28.0	-28.0	0.62 (1)	10.00	
K- R	0 / 7178	-28.0	-28.0	0.72 (1)	10.00	
R- J	0 / 7178	-28.0	-28.0	0.72 (1)	10.00	
J- S	0 / 7178	-28.0	-28.0	0.72 (1)	10.00	
S- I	0 / 7178	-28.0	-28.0	0.72 (1)	10.00	
I- T	0 / 0	-28.0	-28.0	0.48 (1)	10.00	
T- U	0 / 0	-28.0	-28.0	0.48 (1)	10.00	
U- H	0 / 0	-28.0	-28.0	0.48 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 8499	0.64 (1)
K- E	-2815 / 0	0.76 (1)
I- E	0 / 3119	0.23 (1)
C- K	-2673 / 0	0.72 (1)
L- C	0 / 2934	0.22 (1)
B- L	0 / 7330	0.55 (1)
I- F	0 / 7427	0.56 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
L	4-5-4	-1808	-1808	---	BACK	VERT	TOTAL
N	6-0	-2131	-2131	---	BACK	VERT	TOTAL
O	2-5-4	-1808	-1808	---	BACK	VERT	TOTAL
P	6-5-4	-1973	-1973	---	BACK	VERT	TOTAL
Q	8-5-4	-1973	-1973	---	BACK	VERT	TOTAL
R	10-5-4	-1973	-1973	---	BACK	VERT	TOTAL
S	12-5-4	-1973	-1973	---	BACK	VERT	TOTAL
T	14-5-4	-1972	-1972	---	BACK	VERT	TOTAL
U	16-5-4	-1973	-1973	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.28/1.00 (E-F:1) , BC=0.72/1.00 (I-K:1) , WB=0.76/1.00 (E-K:1) , SSI=0.54/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.85 (C) (INPUT = 0.90)
(SI METAL= 0.52 (F) (INPUT = 1.00)

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DWG NO. TAM 61693-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T30	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2130.8 lbs FACTORED DOWN AT 6-0, 1807.7 lbs FACTORED DOWN AT 2-5-4, 1807.7 lbs FACTORED DOWN AT 4-5-4, 1972.8 lbs FACTORED DOWN AT 6-5-4, 1972.8 lbs FACTORED DOWN AT 8-5-4, 1972.8 lbs FACTORED DOWN AT 10-5-4, 1972.8 lbs FACTORED DOWN AT 12-5-4, AND 1972.3 lbs FACTORED DOWN AT 14-5-4, AND 1972.8 lbs FACTORED DOWN AT 16-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

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S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11693-88
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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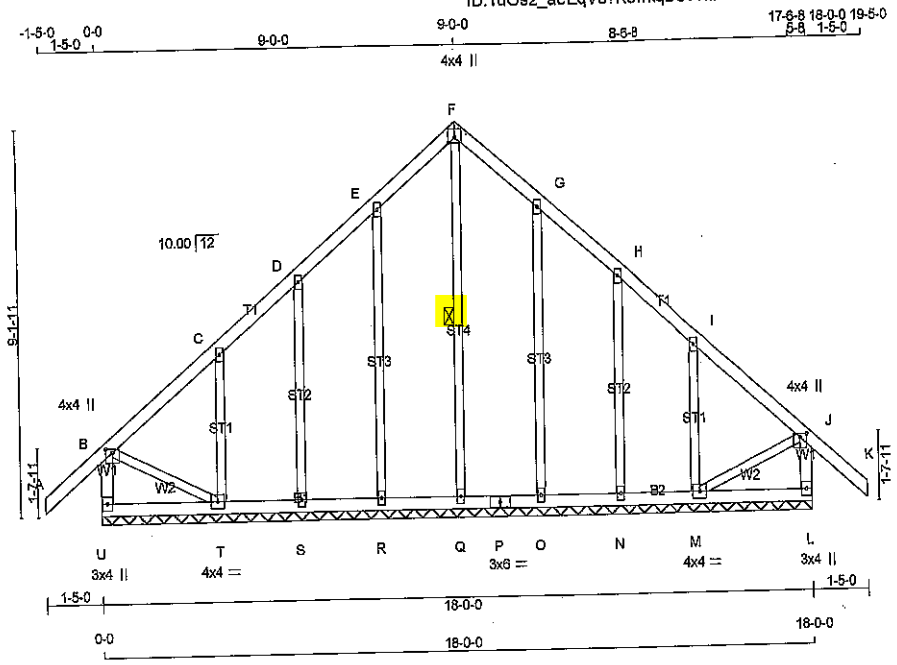

TOTAL WEIGHT = 92 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
U - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
U- B	-328 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 51	-104.9	-104.9	0.17 (1)	10.00
B- C	-28 / 0	-104.9	-104.9	0.11 (1)	6.25
C- D	-54 / 0	-104.9	-104.9	0.11 (1)	6.25
D- E	-25 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-42 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-42 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-25 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	-54 / 0	-104.9	-104.9	0.11 (1)	6.25
I- J	-28 / 0	-104.9	-104.9	0.11 (1)	6.25
J- K	0 / 51	-104.9	-104.9	0.17 (1)	10.00
L- J	-328 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 0	-28.0	-28.0	0.06 (2)	10.00
T- S	0 / 28	-28.0	-28.0	0.06 (2)	10.00
S- R	0 / 25	-28.0	-28.0	0.03 (2)	10.00
R- Q	0 / 22	-28.0	-28.0	0.03 (2)	10.00
Q- P	0 / 22	-28.0	-28.0	0.03 (2)	10.00
P- O	0 / 22	-28.0	-28.0	0.03 (2)	10.00
O- N	0 / 25	-28.0	-28.0	0.03 (2)	10.00
N- M	0 / 28	-28.0	-28.0	0.06 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- F	-140 / 0	0.07 (1)
R- E	-245 / 0	0.25 (1)
S- D	-169 / 0	0.09 (1)
T- C	-307 / 0	0.08 (1)
O- G	-245 / 0	0.25 (1)
N- H	-169 / 0	0.09 (1)
M- I	-307 / 0	0.08 (1)
B- T	0 / 39	0.01 (1)
M- J	0 / 39	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (J-K:1) , BC=0.06/1.00 (S-T:2) , WB=0.25/1.00 (E-R:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (F) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11694 -18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T31	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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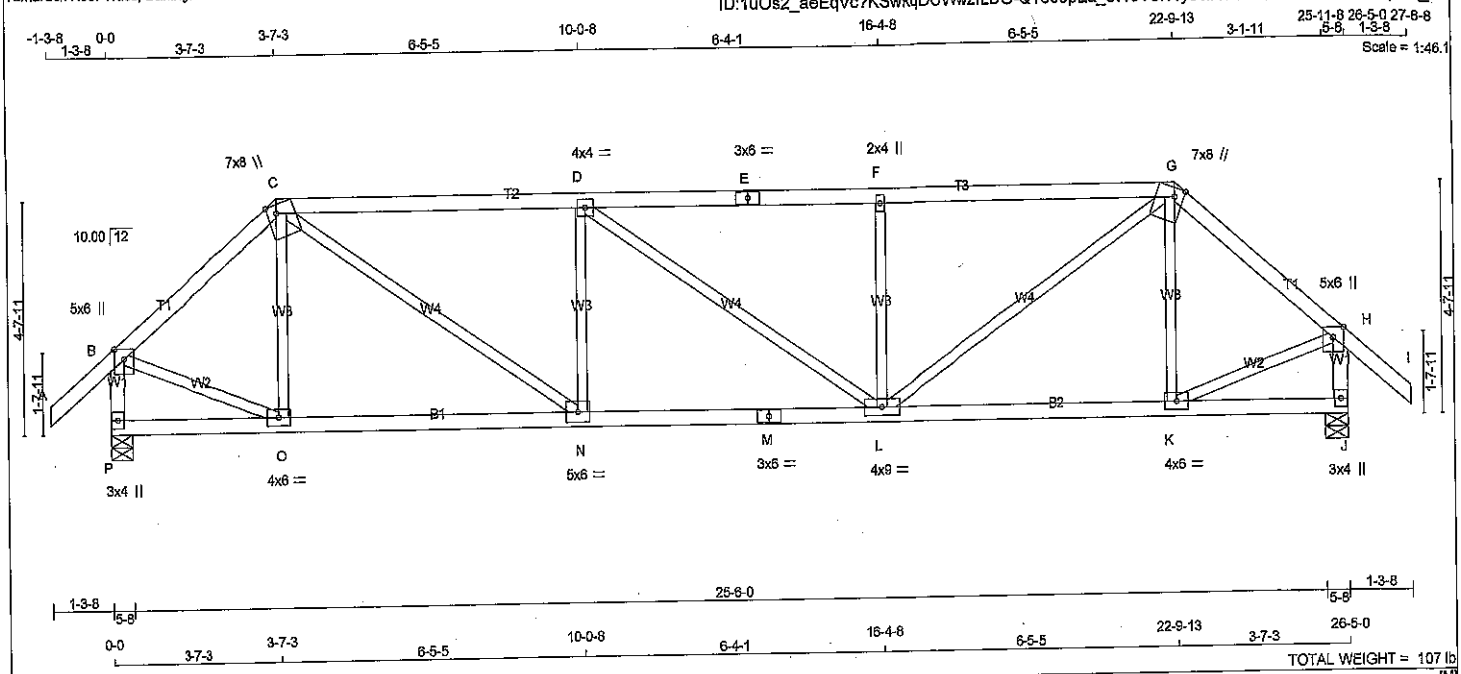

TOTAL WEIGHT = 107 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	7.0	6.0	Edge	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	7.0	6.0	Edge	2.25
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0		
L	BMWWW-t	MT20	4.0	9.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1901	0	1901	0	0	5-8	5-8
J	1901	0	1901	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B-C	-1683 / 0	-104.9	-104.9	0.29 (1)	4.82	
C-D	-2626 / 0	-104.9	-104.9	0.85 (1)	3.35	
D-E	-2625 / 0	-104.9	-104.9	0.84 (1)	3.35	
E-F	-2625 / 0	-104.9	-104.9	0.84 (1)	3.35	
F-G	-2625 / 0	-104.9	-104.9	0.84 (1)	3.36	
G-H	-1683 / 0	-104.9	-104.9	0.29 (1)	4.82	
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
P-B	-1870 / 0	0.0	0.0	0.20 (1)	6.13	
J-H	-1871 / 0	0.0	0.0	0.20 (1)	6.13	
P-O	0 / 0	-28.0	-28.0	0.18 (2)	10.00	
O-N	0 / 1282	-28.0	-28.0	0.36 (2)	10.00	
N-M	0 / 2626	-28.0	-28.0	0.53 (1)	10.00	
M-L	0 / 2626	-28.0	-28.0	0.53 (1)	10.00	
L-K	0 / 1282	-28.0	-28.0	0.36 (2)	10.00	
K-J	0 / 0	-28.0	-28.0	0.18 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-293 / 76	0.09 (1)
C-N	0 / 1620	0.36 (1)
N-D	-725 / 0	0.23 (1)
D-L	-2 / 0	0.00 (1)
L-F	-724 / 0	0.23 (1)
L-G	0 / 1618	0.36 (1)
K-G	-292 / 77	0.09 (1)
B-O	0 / 1367	0.31 (1)
K-H	0 / 1367	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.85/1.00 (C-D:1) , BC=0.53/1.00 (L-N:1) , WB=0.36/1.00 (C-N:1) , SSI=0.31/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.88 (L) (INPUT = 0.90)
CSI METAL= 0.77 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
32216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11695-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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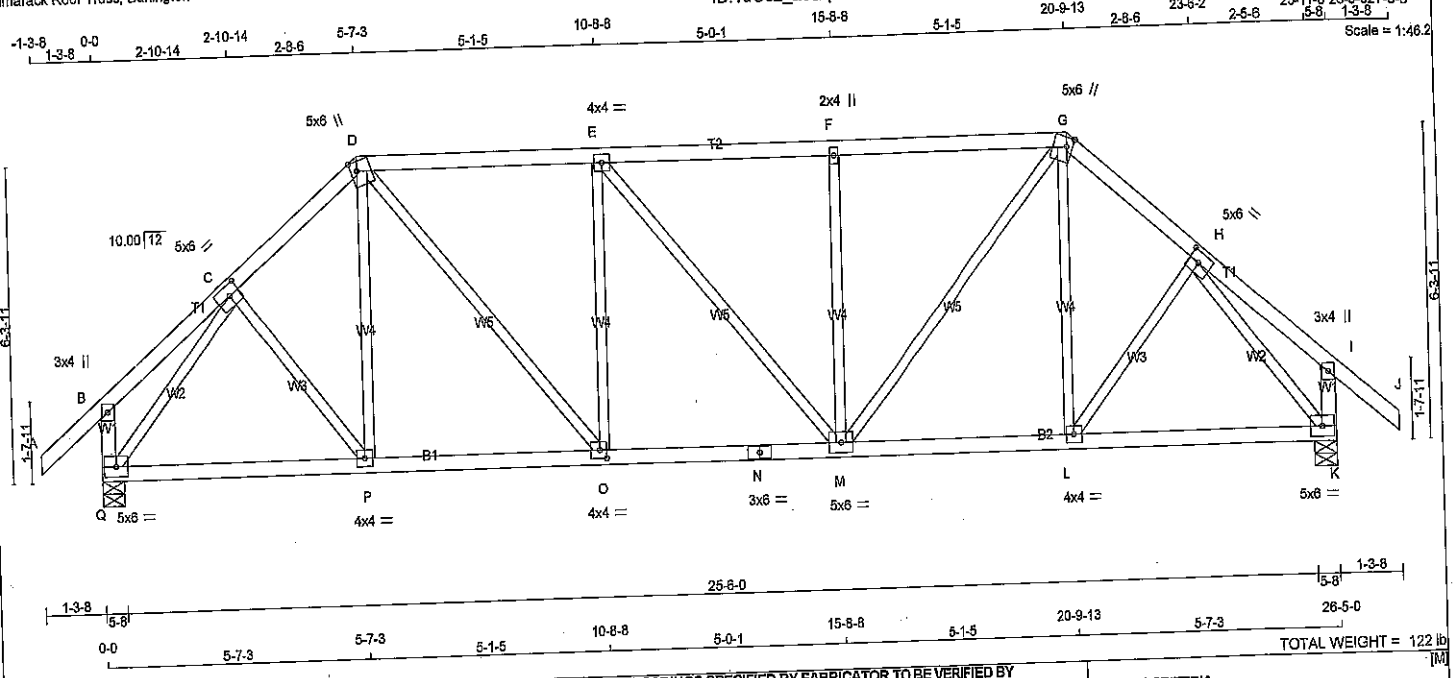

TOTAL WEIGHT = 122 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.25	1.50
H	TMWW-t	MT20	5.0	6.0	2.50	2.75
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.75
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1901	0	1901	0	0	5-8	5-8
K	1901	0	1901	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 18	-104.9	-104.9	0.11 (1)	10.00	
C- D	-1726 / 0	-104.9	-104.9	0.14 (1)	4.94	
D- E	-1909 / 0	-104.9	-104.9	0.44 (1)	4.38	
E- F	-1908 / 0	-104.9	-104.9	0.44 (1)	4.38	
F- G	-1907 / 0	-104.9	-104.9	0.43 (1)	4.39	
G- H	-1726 / 0	-104.9	-104.9	0.14 (1)	4.94	
H- I	0 / 18	-104.9	-104.9	0.11 (1)	10.00	
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
Q- B	-263 / 0	0.0	0.0	0.03 (1)	7.81	
K- I	-263 / 0	0.0	0.0	0.03 (1)	7.81	
Q- P	0 / 1206	-28.0	-28.0	0.33 (2)	10.00	
P- O	0 / 1308	-28.0	-28.0	0.35 (2)	10.00	
O- N	0 / 1909	-28.0	-28.0	0.37 (1)	10.00	
N- M	0 / 1909	-28.0	-28.0	0.37 (1)	10.00	
M- L	0 / 1308	-28.0	-28.0	0.35 (2)	10.00	
L- K	0 / 1206	-28.0	-28.0	0.33 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	0 / 178	0.04 (2)
P- D	0 / 182	0.04 (3)
D- O	0 / 927	0.21 (1)
O- E	-572 / 0	0.36 (1)
E- M	-3 / 0	0.00 (1)
M- F	-571 / 0	0.36 (1)
M- G	0 / 924	0.21 (1)
L- G	0 / 182	0.04 (3)
L- H	0 / 178	0.04 (2)
Q- C	-1982 / 0	0.66 (1)
H- K	-1983 / 0	0.66 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.37/1.00 (M-O:1) , WB=0.66/1.00 (H-K:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

TSI GRIP= 0.90 (H) (INPUT = 0.90)
TSI METAL= 0.56 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11696-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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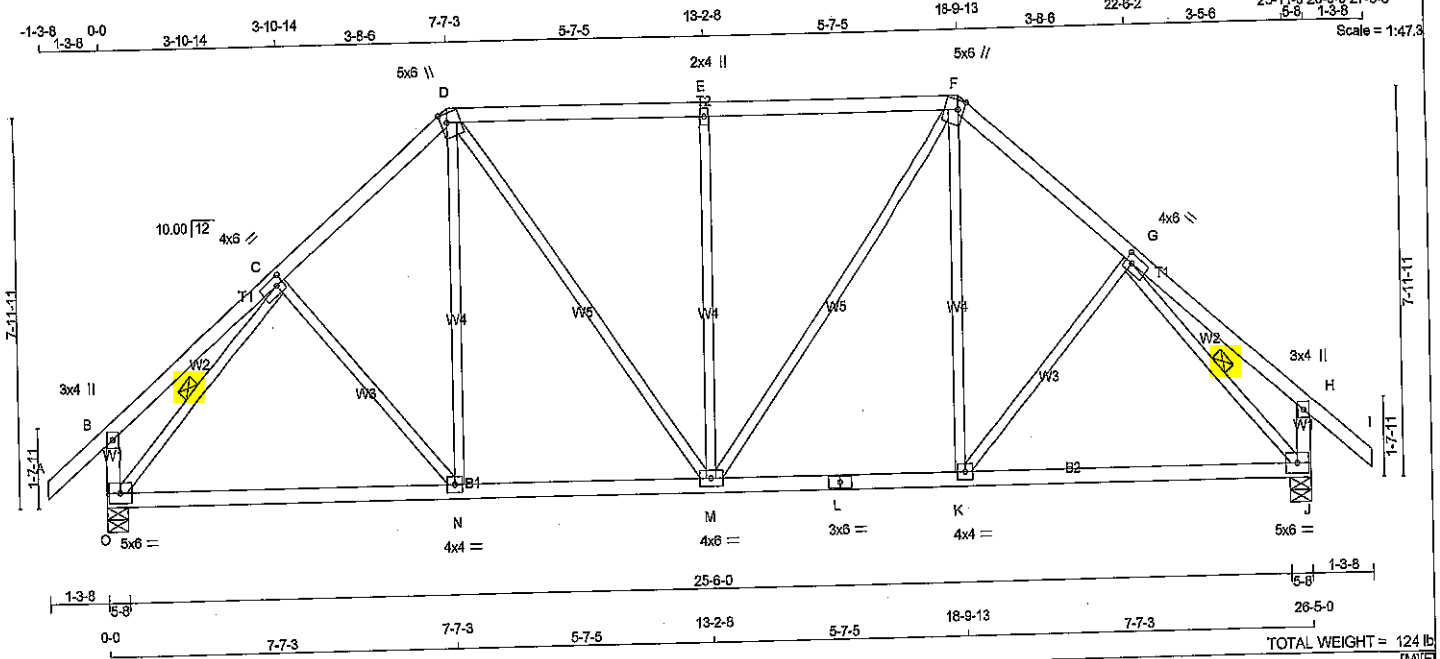

TOTAL WEIGHT = 124 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	1.75
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1901	0	1901	0	0	5-8	5-8
J	1901	0	1901	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	0 / 29	-104.9	-104.9	0.23 (1)	10.00	
C- D	-1671 / 0	-104.9	-104.9	0.22 (1)	4.94	
D- E	-1564 / 0	-104.9	-104.9	0.45 (1)	4.74	
E- F	-1564 / 0	-104.9	-104.9	0.45 (1)	4.74	
F- G	-1671 / 0	-104.9	-104.9	0.22 (1)	4.94	
G- H	0 / 29	-104.9	-104.9	0.23 (1)	10.00	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
O- B	-299 / 0	0.0	0.0	0.03 (1)	7.81	
J- H	-299 / 0	0.0	0.0	0.03 (1)	7.81	
O- N	0 / 1284	-28.0	-28.0	0.50 (2)	10.00	
N- M	0 / 1262	-28.0	-28.0	0.51 (2)	10.00	
M- L	0 / 1262	-28.0	-28.0	0.51 (2)	10.00	
L- K	0 / 1262	-28.0	-28.0	0.51 (2)	10.00	
K- J	0 / 1284	-28.0	-28.0	0.50 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-39 / 105	0.02 (3)
N- D	0 / 346	0.08 (2)
D- M	0 / 511	0.11 (1)
M- E	-718 / 0	0.87 (1)
M- F	0 / 511	0.11 (1)
K- F	0 / 346	0.08 (2)
K- G	-39 / 105	0.02 (3)
O- C	-1985 / 0	0.49 (1)
G- J	-1985 / 0	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.45/1.00 (E-F:1) , BC=0.51/1.00 (M-N:2) , WB=0.87/1.00 (E-M:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11697-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T34	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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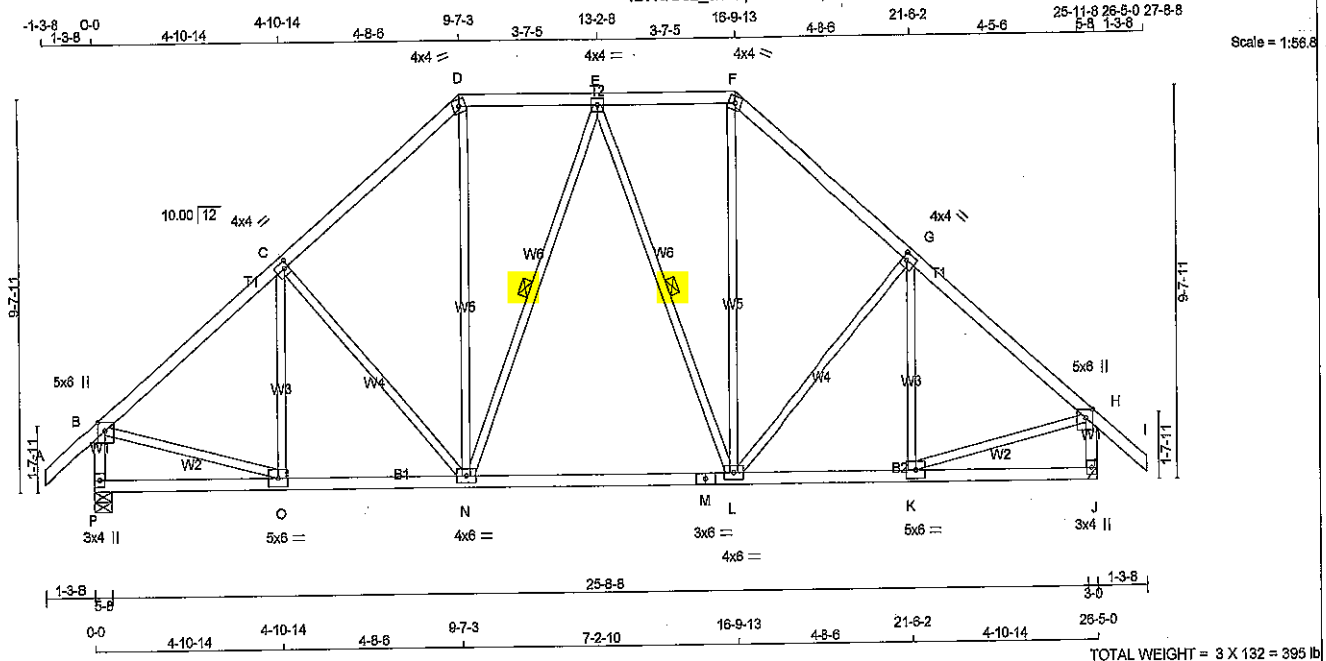

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	2.50	2.25
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1901	0	1901	0	0	5-8	5-8
J	1901	0	1901	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1736 / 0	-104.9	-104.9	0.36 (1)	4.70	
C- D	-1549 / 0	-104.9	-104.9	0.35 (1)	4.93	
D- E	-1164 / 0	-104.9	-104.9	0.17 (1)	5.73	
E- F	-1164 / 0	-104.9	-104.9	0.17 (1)	5.73	
F- G	-1549 / 0	-104.9	-104.9	0.35 (1)	4.93	
G- H	-1736 / 0	-104.9	-104.9	0.36 (1)	4.70	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
P- B	-1842 / 0	0.0	0.0	0.20 (1)	6.17	
J- H	-1842 / 0	0.0	0.0	0.20 (1)	6.17	
P- O	0 / 0	-28.0	-28.0	0.14 (2)	10.00	
O- N	0 / 1362	-28.0	-28.0	0.37 (2)	10.00	
N- M	0 / 1248	-28.0	-28.0	0.36 (2)	10.00	
M- L	0 / 1248	-28.0	-28.0	0.36 (2)	10.00	
L- K	0 / 1362	-28.0	-28.0	0.37 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.14 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-227 / 90	0.11 (1)
C- N	-309 / 0	0.31 (1)
N- D	0 / 629	0.14 (1)
N- E	-239 / 0	0.17 (1)
E- L	-239 / 0	0.17 (1)
L- F	0 / 629	0.14 (1)
L- G	-309 / 0	0.31 (1)
K- G	-227 / 90	0.11 (1)
B- O	0 / 1410	0.32 (1)
K- H	0 / 1410	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.86")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.86")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.36/1.00 (B-C:1) , BC=0.37/1.00 (N-O:2) , WB=0.32/1.00 (B-O:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.37 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30246
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11698-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T35	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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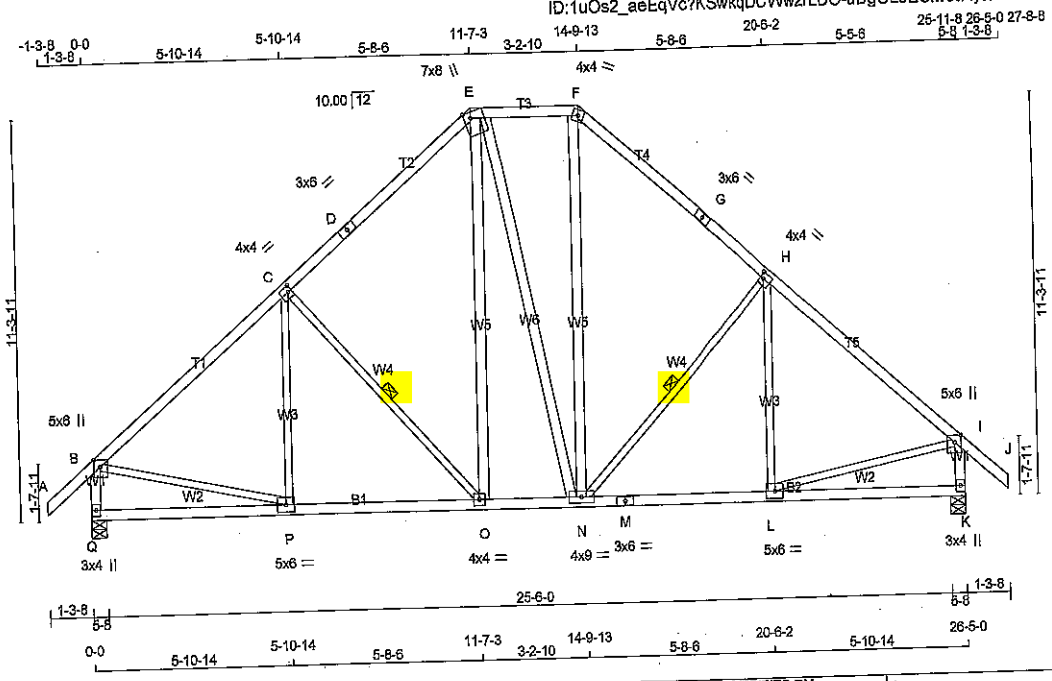

TOTAL WEIGHT = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	7.0	8.0	Edge	2.25
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW+p	MT20	5.0	6.0	2.50	2.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1901	0	1901	0	0	5-8	5-8
K	1901	0	1901	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1498	948 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-1747 / 0	-104.9	-104.9	0.55 (1)	4.43
C- D	-1401 / 0	-104.9	-104.9	0.51 (1)	4.88
D- E	-1401 / 0	-104.9	-104.9	0.51 (1)	4.88
E- F	-1044 / 0	-104.9	-104.9	0.15 (1)	5.99
F- G	-1402 / 0	-104.9	-104.9	0.51 (1)	4.88
G- H	-1402 / 0	-104.9	-104.9	0.51 (1)	4.88
H- I	-1747 / 0	-104.9	-104.9	0.55 (1)	4.43
I- J	0 / 47	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1835 / 0	0.0	0.0	0.20 (1)	6.18
K- I	-1834 / 0	0.0	0.0	0.20 (1)	6.18
Q- P	0 / 0	-28.0	-28.0	0.25 (3)	10.00
P- O	0 / 1378	-28.0	-28.0	0.40 (2)	10.00
O- N	0 / 1043	-28.0	-28.0	0.24 (1)	10.00
N- M	0 / 1378	-28.0	-28.0	0.39 (2)	10.00
M- L	0 / 1378	-28.0	-28.0	0.39 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-114 / 209	0.08 (1)
O- O	-499 / 0	0.25 (1)
O- E	0 / 473	0.08 (1)
E- N	0 / 4	0.00 (1)
N- F	0 / 478	0.08 (1)
N- H	-496 / 0	0.25 (1)
L- H	-117 / 207	0.08 (1)
B- P	0 / 1412	0.32 (1)
L- I	0 / 1411	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.55/1.00 (H-I:1) , BC=0.40/1.00 (O-P:2) , WB=0.32/1.00 (B-P:1) , SSI=0.23/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.87 (B) (INPUT = 0.90)
SSI METAL= 0.39 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3.2.18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11699 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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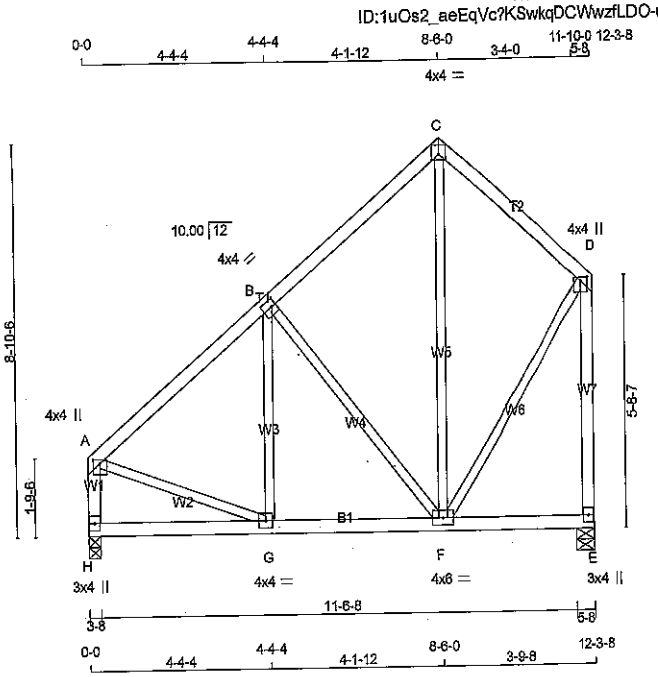

TOTAL WEIGHT = 65 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	817	0	817	0	0	3-8	3-8
E	817	0	817	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	651	399 / 0	129 / 0	0 / 0	0 / 0	123 / 0	0 / 0
E	651	399 / 0	129 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-627 / 0	-104.9	-104.9	0.25 (1)	6.25	
B- C	-369 / 0	-104.9	-104.9	0.24 (1)	6.25	
C- D	-333 / 0	-104.9	-104.9	0.19 (1)	6.25	
H- A	-767 / 0	0.0	0.0	0.08 (1)	7.81	
E- D	-774 / 0	0.0	0.0	0.50 (1)	7.81	
H- G	0 / 0	-28.0	-28.0	0.12 (3)	10.00	
G- F	0 / 507	-28.0	-28.0	0.17 (2)	10.00	
F- E	0 / 0	-28.0	-28.0	0.10 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-37 / 162	0.04 (3)
B- F	-398 / 0	0.31 (1)
F- C	0 / 166	0.04 (3)
A- G	0 / 535	0.12 (1)
F- D	0 / 443	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.17/1.00 (F-G:2) , WB=0.31/1.00 (B-F:1) , SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (A) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2016
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11702 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G36	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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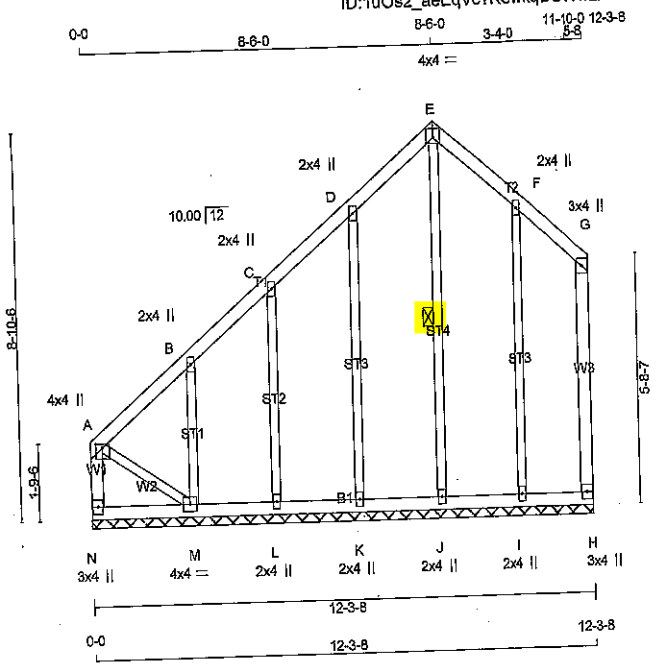

TOTAL WEIGHT = 67 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, F						
B	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-117 / 0	0.0	0.0	0.01 (1)	7.81	
A- B	0 / 2	-104.9	-104.9	0.08 (1)	10.00	
B- C	-14 / 0	-104.9	-104.9	0.08 (1)	6.25	
C- D	0 / 5	-104.9	-104.9	0.06 (1)	10.00	
D- E	-10 / 0	-104.9	-104.9	0.06 (1)	10.00	
E- F	-9 / 0	-104.9	-104.9	0.06 (1)	10.00	
F- G	0 / 16	-104.9	-104.9	0.06 (1)	10.00	
H- G	-64 / 0	0.0	0.0	0.04 (1)	7.81	
N- M	0 / 0	-28.0	-28.0	0.04 (2)	10.00	
M- L	0 / 3	-28.0	-28.0	0.04 (2)	10.00	
L- K	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
K- J	-3 / 0	-28.0	-28.0	0.02 (2)	10.00	
J- I	-3 / 0	-28.0	-28.0	0.02 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.02 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- E	-183 / 0	0.09 (1)
K- D	-242 / 0	0.22 (1)
L- C	-184 / 0	0.08 (1)
M- B	-263 / 0	0.06 (1)
I- F	-242 / 0	0.22 (1)
A- M	0 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1) , BC=0.04/1.00 (L-M:2) , WB=0.22/1.00 (D-K:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

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PROVINCE OF ONTARIO

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DWG NO. TAM 11701 -08
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T36A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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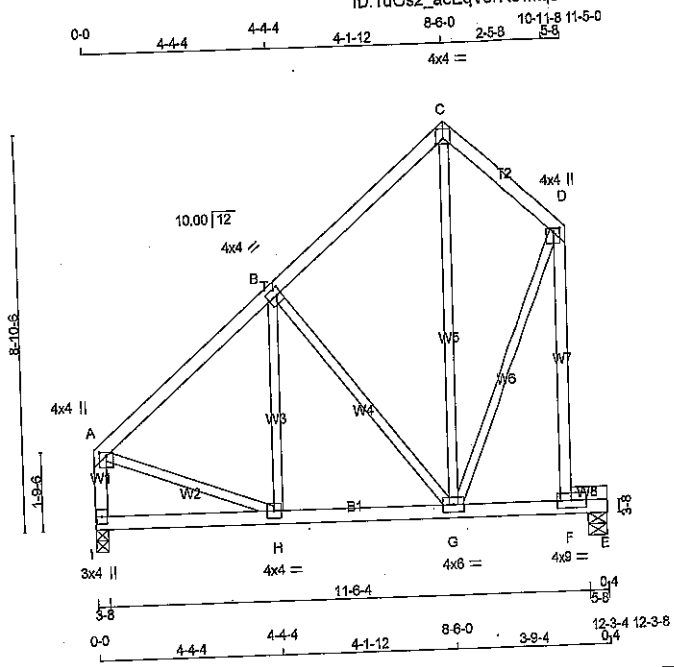

TOTAL WEIGHT = 65 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMVW-t	MT20	4.0	9.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	813	0	813	0	0	3-8	3-8
E	729	0	729	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	648	397 / 0	129 / 0	0 / 0	0 / 0	123 / 0	0 / 0
E	592	345 / 0	129 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-614 / 0	-104.9	-104.9	0.33 (1)	6.25
B- C	-381 / 0	-104.9	-104.9	0.32 (1)	6.25
C- D	-345 / 0	-104.9	-104.9	0.15 (1)	6.25
I- A	-756 / 0	0.0	0.0	0.08 (1)	7.81
F- D	-929 / 0	0.0	0.0	0.84 (1)	7.81
I- H	0 / 0	-28.0	-28.0	0.11 (2)	10.00
H- G	0 / 496	-28.0	-28.0	0.29 (1)	10.00
G- F	0 / 0	-28.0	-28.0	0.84 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- B	-76 / 149	0.03 (3)
B- G	-368 / 0	0.29 (1)
G- C	0 / 184	0.04 (2)
A- H	0 / 525	0.12 (1)
G- D	0 / 815	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 943 (0.16")

CSI: TC=0.84/1.00 (D-F:1) , BC=0.84/1.00 (F-G:1) , WB=0.29/1.00 (B-G:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (F) (INPUT = 0.90)
JSI METAL= 0.46 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11702-38
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T37	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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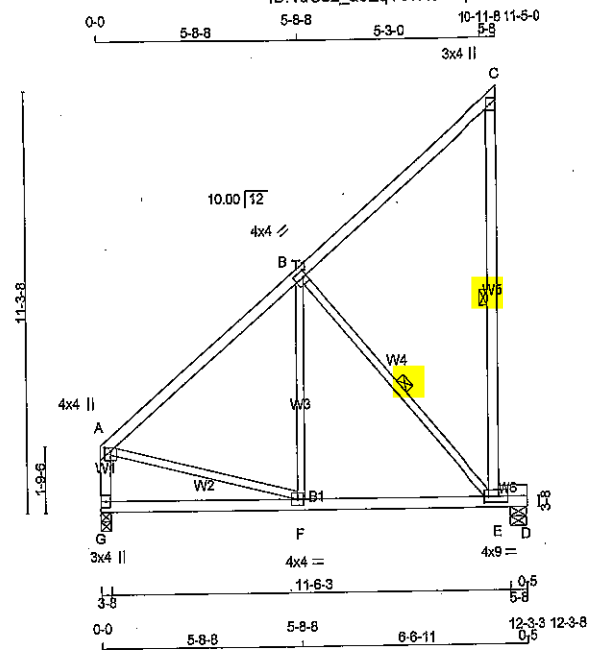

TOTAL WEIGHT = 4 X 62 = 247 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0	2.00	1.75
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	812	0	812	0	0	3-8	3-8
D	729	0	729	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	648	398 / 0	129 / 0	0 / 0	0 / 0	122 / 0	0 / 0
D	592	345 / 0	129 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-E, B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-614 / 0	-104.9	-104.9	0.62 (1)	6.25
B- C	-47 / 0	-104.9	-104.9	0.58 (1)	6.25
E- C	-225 / 0	0.0	0.0	0.15 (1)	6.25
G- A	-777 / 0	0.0	0.0	0.08 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.40 (2)	10.00
F- E	0 / 508	-28.0	-28.0	0.91 (1)	10.00
E- D	0 / 0	-28.0	-28.0	0.35 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	0 / 371	0.08 (2)
B- E	-750 / 0	0.35 (1)
A- F	0 / 524	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 588 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 362 (0.41")

CSI: TC=0.62/1.00 (A-B:1) , BC=0.91/1.00 (E-F:1) , WB=0.35/1.00 (B-E:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.40 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
32216
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11703-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G38	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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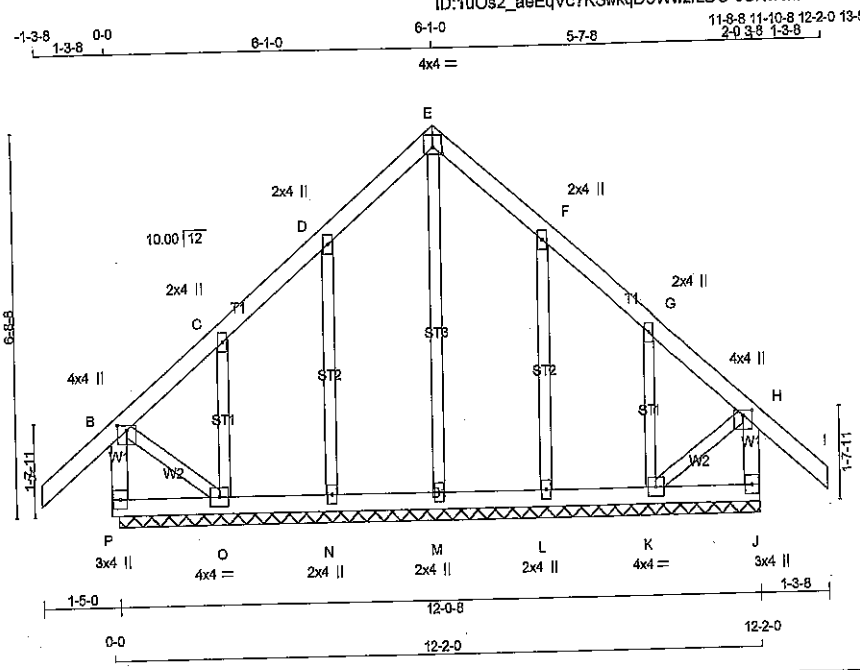

TOTAL WEIGHT = 59 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-274 / 0	0.0	0.0	0.03 (1)	7.81
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-27 / 0	-104.9	-104.9	0.06 (1)	6.25
C- D	-30 / 0	-104.9	-104.9	0.06 (1)	6.25
D- E	-38 / 0	-104.9	-104.9	0.06 (1)	6.25
E- F	-38 / 0	-104.9	-104.9	0.06 (1)	6.25
F- G	-30 / 0	-104.9	-104.9	0.06 (1)	6.25
G- H	-27 / 0	-104.9	-104.9	0.06 (1)	6.25
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- H	-274 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.03 (2)	10.00
O- N	0 / 24	-28.0	-28.0	0.03 (2)	10.00
N- M	0 / 19	-28.0	-28.0	0.02 (2)	10.00
M- L	0 / 19	-28.0	-28.0	0.02 (2)	10.00
L- K	0 / 24	-28.0	-28.0	0.03 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-146 / 0	0.11 (1)
N- D	-233 / 0	0.09 (1)
O- C	-223 / 0	0.04 (1)
L- F	-233 / 0	0.09 (1)
K- G	-223 / 0	0.04 (1)
B- O	0 / 37	0.01 (1)
K- H	0 / 37	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (N-O:2) , WB=0.11/1.00 (E-M:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (E) (INPUT = 0.90)
JSI METAL= 0.06 (F) (INPUT = 1.00)

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DWG NO. TAM 11704-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T39	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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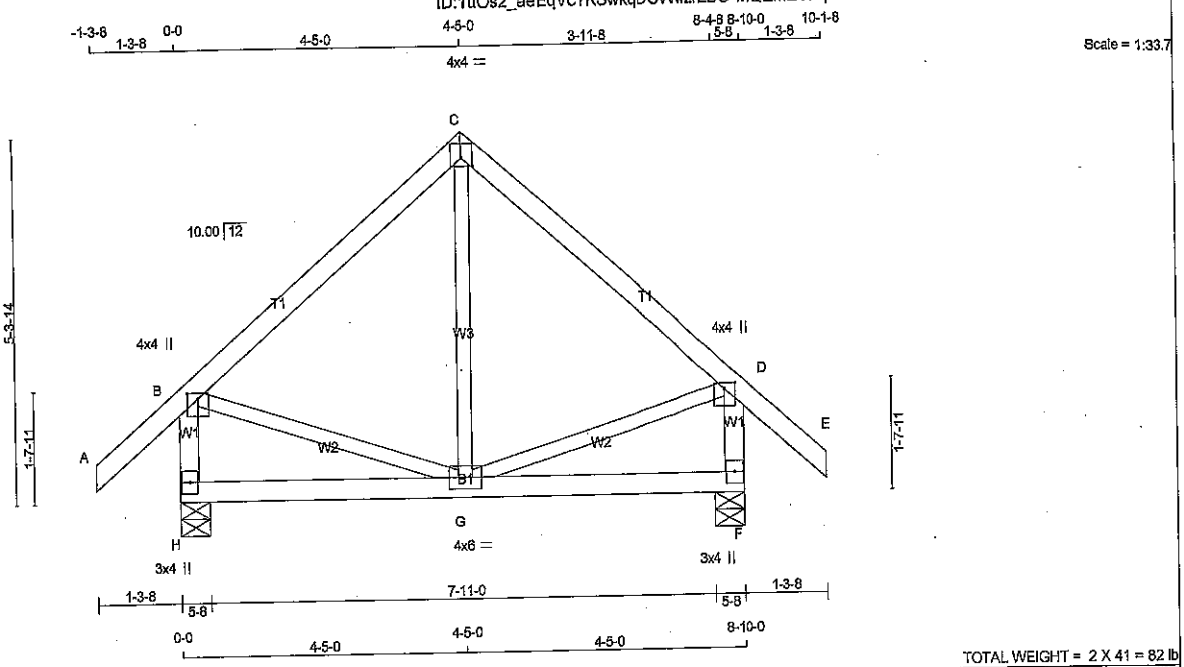

TOTAL WEIGHT = 2 X 41 = 82 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	732	0	732	0	0	5-8	5-8
F	732	0	732	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	566	377 / 0	93 / 0	0 / 0	0 / 0	97 / 0	0 / 0
F	566	377 / 0	93 / 0	0 / 0	0 / 0	97 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	-355 / 0	-104.9	-104.9	0.26 (1)	6.25
C- D	-355 / 0	-104.9	-104.9	0.26 (1)	6.25
D- E	0 / 47	-104.9	-104.9	0.14 (1)	10.00
H- B	-685 / 0	0.0	0.0	0.07 (1)	7.81
F- D	-685 / 0	0.0	0.0	0.07 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.16 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-9 / 190	0.04 (3)
B- G	0 / 284	0.06 (1)
G- D	0 / 284	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.26/1.00 (C-D:1) , BC=0.16/1.00 (F-G:3) , WB=0.06/1.00 (B-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2-2-18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11705

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G39	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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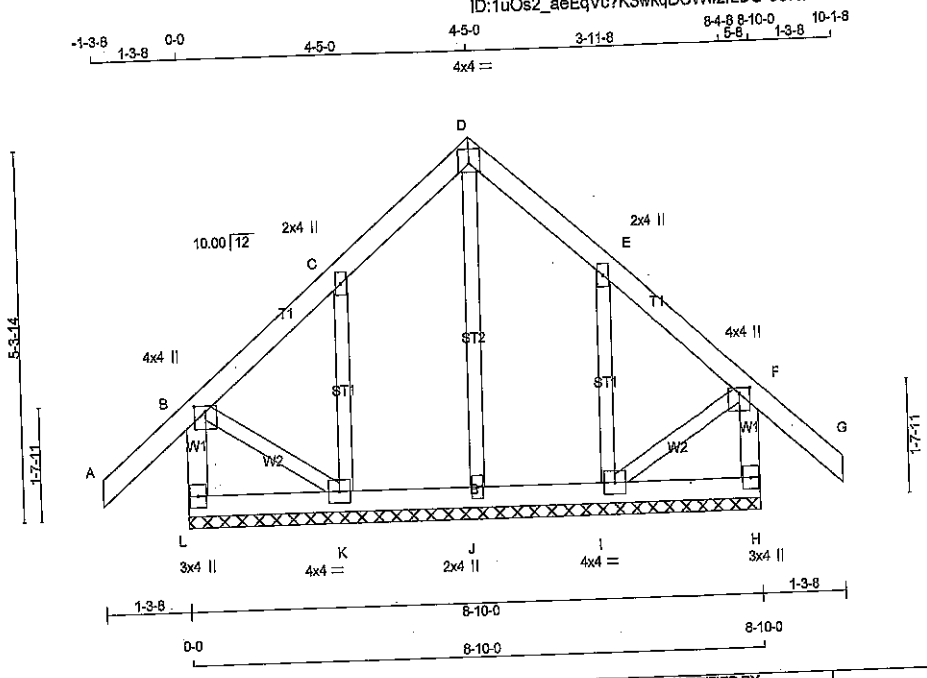

TOTAL WEIGHT = 43 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-270 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-11 / 0	-104.9	-104.9	0.08 (1)	6.25	
C- D	-37 / 0	-104.9	-104.9	0.08 (1)	6.25	
D- E	-37 / 0	-104.9	-104.9	0.08 (1)	6.25	
E- F	-11 / 0	-104.9	-104.9	0.08 (1)	6.25	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
H- F	-270 / 0	0.0	0.0	0.03 (1)	7.81	
L- K	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
K- J	0 / 14	-28.0	-28.0	0.04 (2)	10.00	
J- I	0 / 14	-28.0	-28.0	0.04 (2)	10.00	
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-137 / 0	0.06 (1)
K- C	-282 / 0	0.06 (1)
I- E	-282 / 0	0.06 (1)
B- K	0 / 25	0.01 (1)
I- F	0 / 25	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.04/1.00 (I-J:2) , WB=0.06/1.00 (E-I:1) , SSI=0.09/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (D) (INPUT = 0.90)
JSI METAL= 0.08 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 117062-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T41	1	2	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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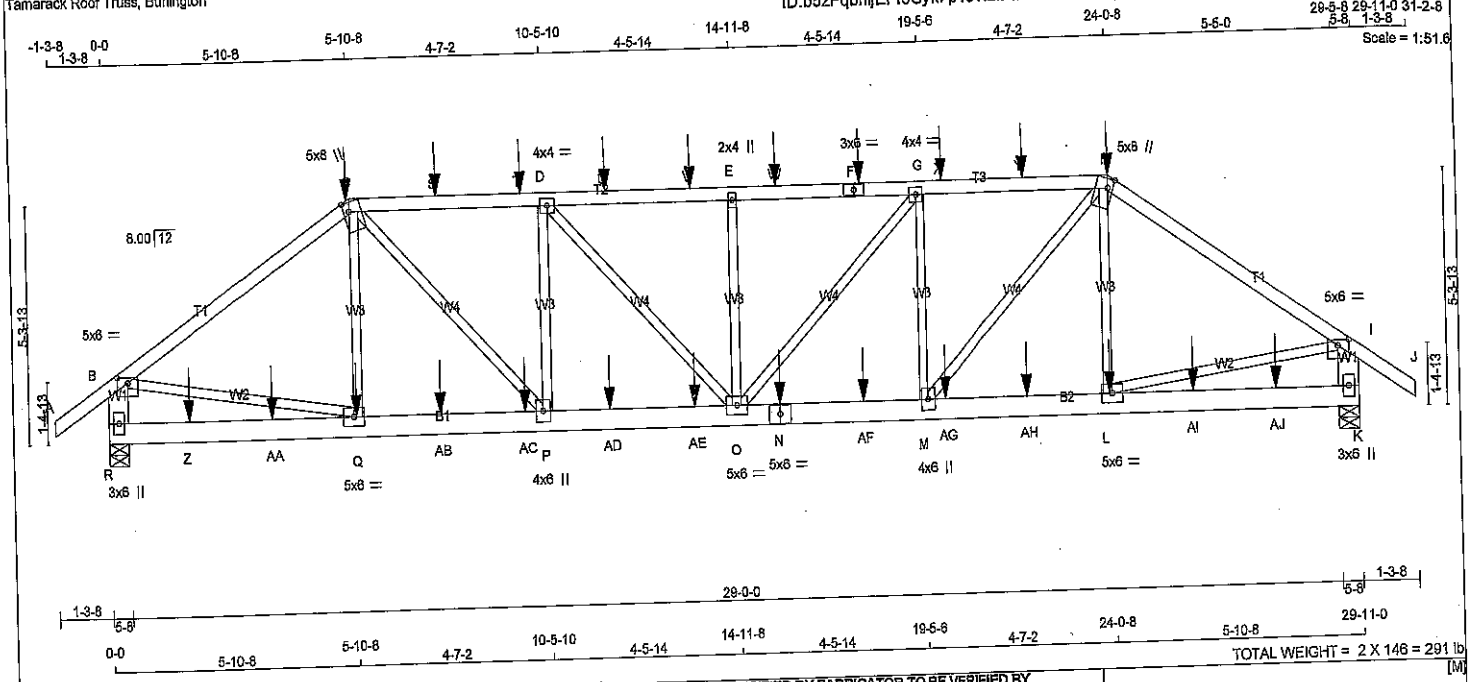

TOTAL WEIGHT = 2 X 146 = 291 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
F- H	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	12	SIDE(183.1)
N- K	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
Q- C	1	6	SIDE(31.9)
L- H	1	6	SIDE(31.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	8.0	2.50	1.50
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	8.0	2.50	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3538	0	3538	0	0	5-8	5-8
K	3538	0	3538	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2814	1735 / 0	550 / 0	0 / 0	0 / 0	529 / 0	0 / 0
K	2813	1734 / 0	550 / 0	0 / 0	0 / 0	529 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-4271 / 0	-104.9	-104.9	0.54 (1)	4.14	
C- S	-4939 / 0	-104.9	-104.9	0.39 (1)	3.97	
S- T	-4939 / 0	-104.9	-104.9	0.39 (1)	3.97	
T- D	-4939 / 0	-104.9	-104.9	0.39 (1)	3.97	
D- U	-5344 / 0	-104.9	-104.9	0.40 (1)	3.82	
U- V	-5344 / 0	-104.9	-104.9	0.40 (1)	3.82	
V- E	-5344 / 0	-104.9	-104.9	0.40 (1)	3.82	
E- W	-5344 / 0	-104.9	-104.9	0.41 (1)	3.82	
W- F	-5344 / 0	-104.9	-104.9	0.41 (1)	3.82	
F- G	-5344 / 0	-104.9	-104.9	0.41 (1)	3.82	
G- X	-4938 / 0	-104.9	-104.9	0.39 (1)	3.97	
X- Y	-4938 / 0	-104.9	-104.9	0.39 (1)	3.97	
Y- H	-4938 / 0	-104.9	-104.9	0.39 (1)	3.97	
H- I	-4270 / 0	-104.9	-104.9	0.54 (1)	4.14	
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
R- B	-3419 / 0	0.0	0.0	0.12 (1)	7.55	
K- I	-3418 / 0	0.0	0.0	0.12 (1)	7.55	
R- Z	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
Z-AA	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
AA- Q	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
Q-AB	0 / 3543	-28.0	-28.0	0.27 (1)	10.00	
AB-AC	0 / 3543	-28.0	-28.0	0.27 (1)	10.00	
AC- P	0 / 3543	-28.0	-28.0	0.27 (1)	10.00	
P-AD	0 / 4939	-28.0	-28.0	0.37 (1)	10.00	
AD-AE	0 / 4939	-28.0	-28.0	0.37 (1)	10.00	
AE- O	0 / 4939	-28.0	-28.0	0.37 (1)	10.00	
O- N	0 / 4938	-28.0	-28.0	0.37 (1)	10.00	
N-AF	0 / 4938	-28.0	-28.0	0.37 (1)	10.00	
AF- M	0 / 4938	-28.0	-28.0	0.37 (1)	10.00	
M-AG	0 / 3542	-28.0	-28.0	0.27 (1)	10.00	
AG-AH	0 / 3542	-28.0	-28.0	0.27 (1)	10.00	
AH- L	0 / 3542	-28.0	-28.0	0.27 (1)	10.00	
L-AI	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
AI-AJ	0 / 0	-28.0	-28.0	0.11 (2)	10.00	
AJ- K	0 / 0	-28.0	-28.0	0.11 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-286 / 303	0.06 (1)
C- P	0 / 2048	0.25 (1)
P- D	-1290 / 0	0.26 (1)
D- O	0 / 601	0.07 (1)
O- E	-699 / 0	0.14 (1)
O- G	0 / 603	0.07 (1)
M- G	-1289 / 0	0.26 (1)
M- H	0 / 2047	0.25 (1)
L- H	-286 / 303	0.06 (1)
B- Q	0 / 3598	0.45 (1)
L- I	0 / 3597	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.37/1.00 (M-O:1) , WB=0.45/1.00 (B-Q:1) , SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

PL GRIP= 0.86 (C) (INPUT = 0.90)
PL METAL= 0.44 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2 MAR 18
S KATSOULAKOS
PROVINCE OF ONTARIO

P66

DWG NO. TAM 11736-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44756	DRWG NO.
287571	T41	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 589.7 lbs FACTORED DOWN AT 5-10-8, 126.3 lbs FACTORED DOWN AT 7-11-4, 126.3 lbs FACTORED DOWN AT 9-11-4, 126.3 lbs FACTORED DOWN AT 11-11-4, 126.3 lbs FACTORED DOWN AT 13-11-8, 126.3 lbs FACTORED DOWN AT 15-11-12, 126.3 lbs FACTORED DOWN AT 17-11-12, 126.3 lbs FACTORED DOWN AT 19-11-12, AND 126.3 lbs FACTORED DOWN AT 21-11-12, AND 589.7 lbs FACTORED DOWN AT 24-0-8 ON TOP CHORD, AND 94.9 lbs FACTORED DOWN AT 1-11-4, 94.9 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, 71.7 lbs FACTORED DOWN AT 11-11-4, 71.7 lbs FACTORED DOWN AT 13-11-8, 71.7 lbs FACTORED DOWN AT 15-11-12, 71.7 lbs FACTORED DOWN AT 17-11-12, 71.7 lbs FACTORED DOWN AT 19-11-12, 71.7 lbs FACTORED DOWN AT 21-11-12, 71.7 lbs FACTORED DOWN AT 23-11-12, AND 94.9 lbs FACTORED DOWN AT 25-11-12, AND 94.9 lbs FACTORED DOWN AT 27-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-590	-590	—	BACK	VERT	TOTAL
F	17-11-12	-126	-126	—	BACK	VERT	TOTAL
H	24-0-8	-590	-590	—	BACK	VERT	TOTAL
L	23-11-12	-41	-72	—	BACK	VERT	TOTAL
N	15-11-12	-41	-72	—	BACK	VERT	TOTAL
Q	5-11-4	-41	-72	—	BACK	VERT	TOTAL
S	7-11-4	-126	-126	—	BACK	VERT	TOTAL
T	9-11-4	-126	-126	—	BACK	VERT	TOTAL
U	11-11-4	-126	-126	—	BACK	VERT	TOTAL
V	13-11-8	-126	-126	—	BACK	VERT	TOTAL
W	15-11-12	-126	-126	—	BACK	VERT	TOTAL
X	19-11-12	-126	-126	—	BACK	VERT	TOTAL
Y	21-11-12	-126	-126	—	BACK	VERT	TOTAL
Z	1-11-4	-51	-95	—	BACK	VERT	TOTAL
AA	3-11-4	-54	-95	—	BACK	VERT	TOTAL
AB	7-11-4	-41	-72	—	BACK	VERT	TOTAL
AC	9-11-4	-41	-72	—	BACK	VERT	TOTAL
AD	11-11-4	-41	-72	—	BACK	VERT	TOTAL
AE	13-11-8	-41	-72	—	BACK	VERT	TOTAL
AF	17-11-12	-41	-72	—	BACK	VERT	TOTAL
AG	19-11-12	-41	-72	—	BACK	VERT	TOTAL
AH	21-11-12	-41	-72	—	BACK	VERT	TOTAL
AI	25-11-12	-54	-95	—	BACK	VERT	TOTAL
AJ	27-11-12	-51	-95	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
2018
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PROVINCE OF ONTARIO

DWG NO. TAM 11736-B
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T41S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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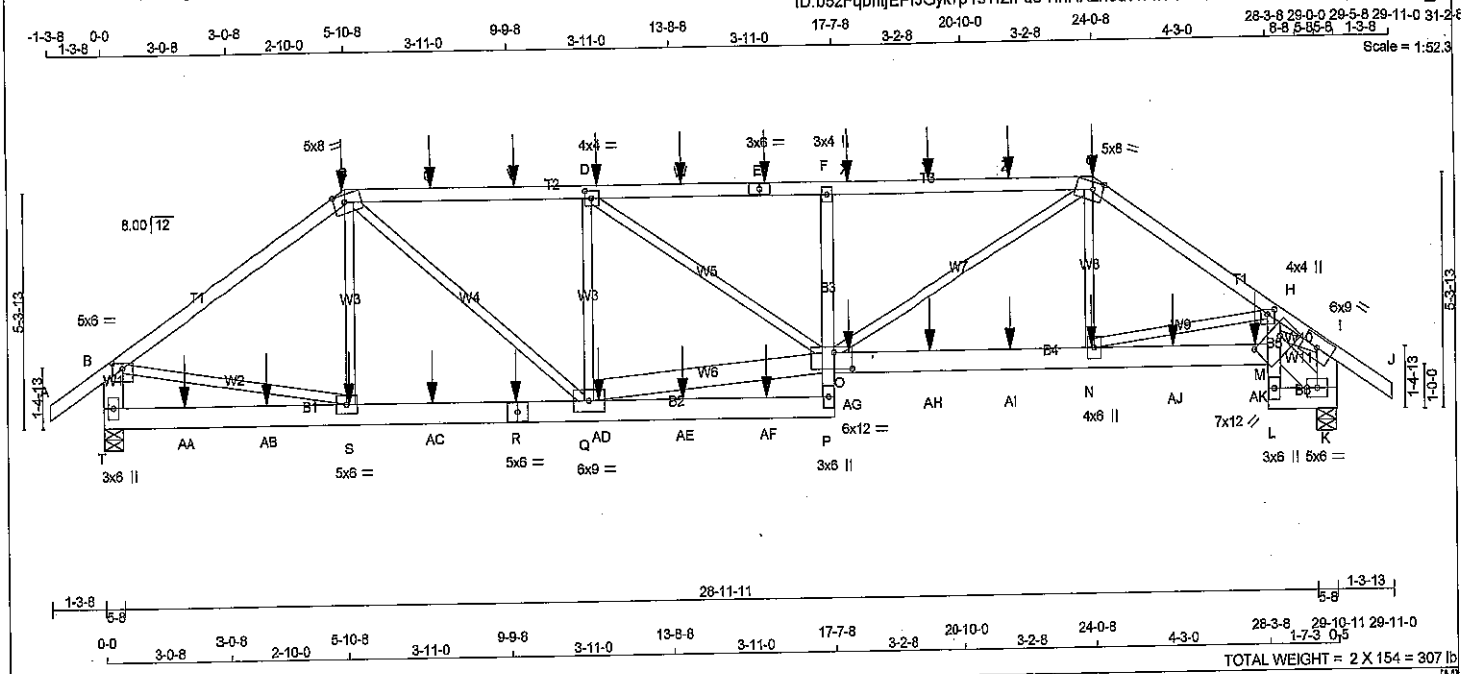

TOTAL WEIGHT = 2 X 154 = 307 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
Q - O	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	SIDE(61.0)
G- J	1	12	SIDE(61.0)
T- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(183.1)
O- M	2	12	SIDE(183.1)
L- K	2	12	TOP
P- F	1	12	TOP
H- L	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
N- G	1	6	SIDE(8.9)
S- C	1	6	SIDE(31.9)
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	5.0	8.0	2.00	3.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	2.00	3.00
H	TMVW+p	MT20	4.0	4.0	1.25	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3618	0	3618	0	0	5-8	5-8
K	3930	0	3930	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2885	1765 / 0	573 / 0	0 / 0	0 / 0	548 / 0	0 / 0
K	3143	1908 / 0	633 / 0	0 / 0	0 / 0	601 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-4388 / 0	-104.9	-104.9	0.55 (1)	4.08
C- U	-5365 / 0	-104.9	-104.9	0.60 (1)	3.63
U- V	-5365 / 0	-104.9	-104.9	0.60 (1)	3.63
V- D	-5365 / 0	-104.9	-104.9	0.60 (1)	3.63
D- W	-6935 / 0	-104.9	-104.9	0.83 (1)	2.95
W- E	-6935 / 0	-104.9	-104.9	0.83 (1)	2.95
E- F	-6935 / 0	-104.9	-104.9	0.83 (1)	2.95
F- X	-6976 / 0	-104.9	-104.9	0.93 (1)	2.94
X- Y	-6976 / 0	-104.9	-104.9	0.93 (1)	2.94
Y- Z	-6976 / 0	-104.9	-104.9	0.93 (1)	2.94
Z- G	-6976 / 0	-104.9	-104.9	0.93 (1)	2.94
G- H	-5686 / 0	-104.9	-104.9	0.37 (1)	3.80
H- I	-6701 / 0	-104.9	-104.9	0.24 (1)	3.59
I- J	0 / 47	-104.9	-104.9	0.10 (1)	10.00
T- B	-3500 / 0	0.0	0.0	0.13 (1)	7.48
T-AA	0 / 0	-28.0	-28.0	0.11 (2)	10.00
AA-AB	0 / 0	-28.0	-28.0	0.11 (2)	10.00
AB- S	0 / 0	-28.0	-28.0	0.11 (2)	10.00
S-AC	0 / 3641	-28.0	-28.0	0.28 (1)	10.00
AC- R	0 / 3641	-28.0	-28.0	0.28 (1)	10.00
R- Q	0 / 3641	-28.0	-28.0	0.28 (1)	10.00
Q-AD	0 / 163	-28.0	-28.0	0.10 (2)	10.00
AD-AE	0 / 163	-28.0	-28.0	0.10 (2)	10.00
AE-AF	0 / 163	-28.0	-28.0	0.10 (2)	10.00
AF- P	0 / 163	-28.0	-28.0	0.10 (2)	10.00
P- O	0 / 190	0.0	0.0	0.13 (1)	10.00
O- F	-1113 / 0	0.0	0.0	0.16 (1)	7.81
O-AG	0 / 4740	-28.0	-28.0	0.43 (1)	10.00
AG-AH	0 / 4740	-28.0	-28.0	0.43 (1)	10.00
AH-AI	0 / 4740	-28.0	-28.0	0.43 (1)	10.00
AI- N	0 / 4740	-28.0	-28.0	0.43 (1)	10.00
N-AJ	0 / 5976	-28.0	-28.0	0.50 (1)	10.00
AJ-AK	0 / 5976	-28.0	-28.0	0.50 (1)	10.00
AK- M	0 / 5976	-28.0	-28.0	0.50 (1)	10.00
L- M	0 / 31	0.0	0.0	0.25 (1)	10.00
M- H	0 / 610	0.0	0.0	0.30 (1)	10.00
L- K	0 / 270	-28.0	-28.0	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-289 / 308	0.06 (1)
C- Q	0 / 2271	0.28 (1)
Q- D	-2064 / 0	0.42 (1)
Q- O	0 / 5312	0.34 (1)
D- O	0 / 1860	0.23 (1)
O- G	0 / 2607	0.32 (1)
N- G	0 / 662	0.08 (2)
N- H	-1273 / 0	0.23 (1)
B- S	0 / 3697	0.46 (1)
M- K	-380 / 0	0.01 (1)
M- I	0 / 5535	0.69 (1)
K- I	-3645 / 0	0.13 (1)

LICENSED PROFESSIONAL ENGINEER
30278
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PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.93/1.00 (F-G:1) , BC=0.50/1.00 (M-N:1) , WB=0.69/1.00 (I-M:1) , SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

SITE COPY

DWG NO. TAM17773-18

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T41S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMWW-t	MT20	6.0	9.0		
K	BMWW1-t	MT20	5.0	6.0		
L	BMV+p	MT20	3.0	6.0		
M	BVMWW-w	MT20	7.0	12.0	2.75	7.75
N	BMWW+t	MT20	4.0	6.0		
O	BVMWWW-l	MT20	6.0	12.0	4.50	5.25
P	BMV+p	MT20	3.0	6.0		
Q	BMWWW-t	MT20	6.0	9.0		
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 589.7 lbs FACTORED DOWN AT 5-10-8, 126.3 lbs FACTORED DOWN AT 7-11-4, 126.3 lbs FACTORED DOWN AT 9-11-4, 126.3 lbs FACTORED DOWN AT 11-11-4, 126.3 lbs FACTORED DOWN AT 13-11-8, 126.3 lbs FACTORED DOWN AT 15-11-12, 123.8 lbs FACTORED DOWN AT 17-11-12, 123.8 lbs FACTORED DOWN AT 19-11-12, 123.8 lbs FACTORED DOWN AT 21-11-12, AND 127.3 lbs FACTORED DOWN AT 24-0-8, AND 501.8 lbs FACTORED DOWN AT 24-0-8 ON TOP CHORD, AND 94.9 lbs FACTORED DOWN AT 1-11-4, 94.9 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, 71.7 lbs FACTORED DOWN AT 7-11-4, 71.7 lbs FACTORED DOWN AT 9-11-4, 71.7 lbs FACTORED DOWN AT 11-11-4, 71.7 lbs FACTORED DOWN AT 13-11-8, 71.7 lbs FACTORED DOWN AT 15-11-12, 123.9 lbs FACTORED DOWN AT 17-11-12, 116.9 lbs FACTORED DOWN AT 19-11-12, 116.9 lbs FACTORED DOWN AT 21-11-12, 116.9 lbs FACTORED DOWN AT 23-11-12, AND 116.9 lbs FACTORED DOWN AT 25-11-12, AND 124.5 lbs FACTORED DOWN AT 27-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-590	-590	---	BACK	VERT	TOTAL
D	11-11-4	-126	-126	---	BACK	VERT	TOTAL
E	15-11-12	-126	-126	---	BACK	VERT	TOTAL
G	24-0-8	-127	-127	---	BACK	VERT	TOTAL
G	24-0-8	-502	-502	---	FRONT	VERT	TOTAL
N	23-11-12	-117	-117	---	BACK	VERT	TOTAL
R	9-11-4	-41	-72	---	BACK	VERT	TOTAL
S	5-11-4	-41	-72	---	BACK	VERT	TOTAL
U	7-11-4	-126	-126	---	BACK	VERT	TOTAL
V	9-11-4	-126	-126	---	BACK	VERT	TOTAL
W	13-11-8	-126	-126	---	BACK	VERT	TOTAL
X	17-11-12	-124	-124	---	BACK	VERT	TOTAL
Y	19-11-12	-124	-124	---	BACK	VERT	TOTAL
Z	21-11-12	-124	-124	---	BACK	VERT	TOTAL
AA	1-11-4	-51	-95	---	BACK	VERT	TOTAL
AB	3-11-4	-54	-95	---	BACK	VERT	TOTAL
AC	7-11-4	-41	-72	---	BACK	VERT	TOTAL
AD	11-11-4	-41	-72	---	BACK	VERT	TOTAL
AE	13-11-8	-41	-72	---	BACK	VERT	TOTAL
AF	15-11-12	-41	-72	---	BACK	VERT	TOTAL
AG	17-11-12	-124	-124	---	BACK	VERT	TOTAL
AH	19-11-12	-117	-117	---	BACK	VERT	TOTAL
AI	21-11-12	-117	-117	---	BACK	VERT	TOTAL
AJ	25-11-12	-117	-117	---	BACK	VERT	TOTAL
AK	27-11-12	-125	-125	---	BACK	VERT	TOTAL

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.58 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3048
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11713-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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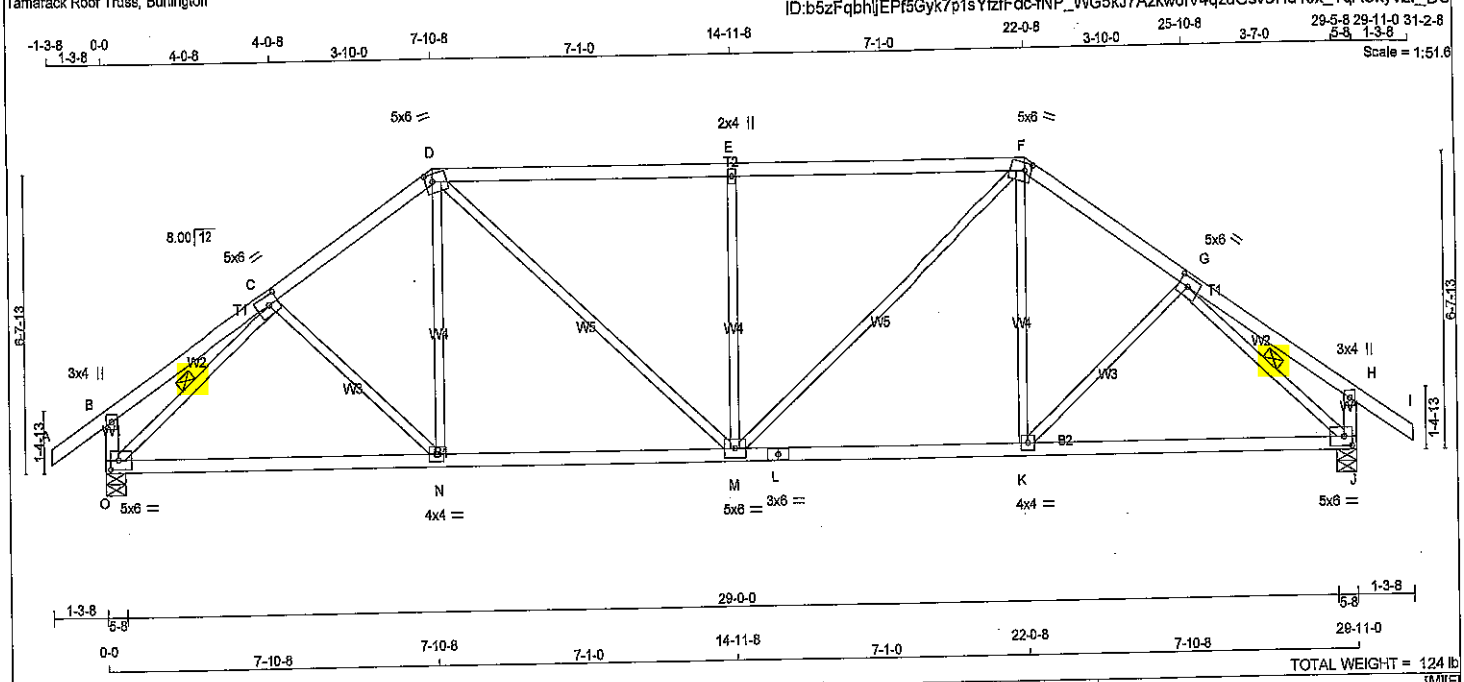

TOTAL WEIGHT = 124 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	5.0	6.0	2.50	2.75
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.50	2.25
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0	2.50	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2132	0	2132	0	0	5-8	5-8
J	2132	0	2132	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1682	1061 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
J	1682	1061 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 25	-104.9	-104.9	0.24 (1)	10.00
C- D	-2244 / 0	-104.9	-104.9	0.25 (1)	4.35
D- E	-2450 / 0	-104.9	-104.9	0.81 (1)	3.33
E- F	-2450 / 0	-104.9	-104.9	0.81 (1)	3.33
F- G	-2244 / 0	-104.9	-104.9	0.25 (1)	4.35
G- H	0 / 25	-104.9	-104.9	0.24 (1)	10.00
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-306 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1844	-28.0	-28.0	0.61 (2)	10.00
N- M	0 / 1847	-28.0	-28.0	0.62 (2)	10.00
M- L	0 / 1847	-28.0	-28.0	0.62 (2)	10.00
L- K	0 / 1847	-28.0	-28.0	0.62 (2)	10.00
K- J	0 / 1844	-28.0	-28.0	0.61 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-1 / 140	0.03 (3)
N- D	0 / 353	0.08 (2)
D- M	0 / 810	0.18 (1)
M- E	-914 / 0	0.67 (1)
M- F	0 / 810	0.18 (1)
K- F	0 / 353	0.08 (2)
K- G	-1 / 140	0.03 (3)
O- C	-2533 / 0	0.56 (1)
G- J	-2533 / 0	0.56 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.81/1.00 (D-E:1) , BC=0.62/1.00 (M-N:2) , WB=0.67/1.00 (E-M:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI-GRIP= 0.89 (J) (INPUT = 0.90)
JSI-METAL= 0.62 (C) (INPUT = 1.00)

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PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1/732-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T42S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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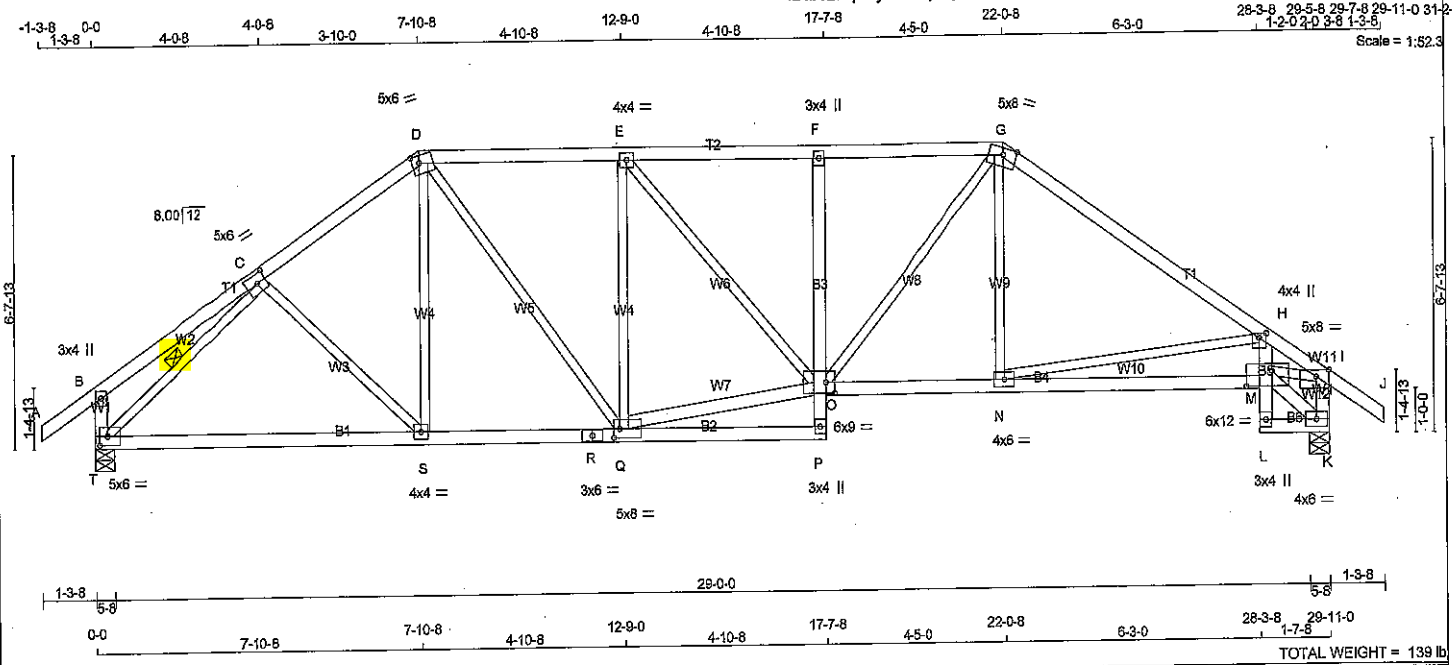

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - K	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	TMVW-p	MT20	5.0	8.0	Edge	3.50
K	BMVW1-t	MT20	4.0	6.0		
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	6.0	12.0	4.00	7.25
N	BMWW-t	MT20	4.0	6.0		
O	BVMWWW-l	MT20	6.0	9.0	3.00	2.50
P	BMV+p	MT20	3.0	4.0		
Q	BMWWW-t	MT20	5.0	6.0	2.50	1.75
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMVW1-t	MT20	5.0	6.0	2.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2129	0	2129	0	0	5-8	5-8
K	2136	0	2136	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1660	1059 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
K	1685	1063 / 0	314 / 0	0 / 0	0 / 0	308 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 25	-104.9	-104.9	0.24 (1)	10.00
C- D	-2235 / 0	-104.9	-104.9	0.25 (1)	4.36
D- E	-2318 / 0	-104.9	-104.9	0.34 (1)	4.19
E- F	-2712 / 0	-104.9	-104.9	0.35 (1)	3.91
F- G	-2722 / 0	-104.9	-104.9	0.33 (1)	3.92
G- H	-2628 / 0	-104.9	-104.9	0.75 (1)	3.49
H- I	-3736 / 0	-104.9	-104.9	0.47 (1)	3.20
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00
T- B	-306 / 0	0.0	0.0	0.03 (1)	7.81
K- I	-1994 / 0	0.0	0.0	0.21 (1)	5.97
T- S	0 / 1838	-28.0	-28.0	0.56 (2)	10.00
S- R	0 / 1839	-28.0	-28.0	0.58 (2)	10.00
R- Q	0 / 1839	-28.0	-28.0	0.58 (2)	10.00
Q- P	0 / 58	-28.0	-28.0	0.15 (2)	10.00
P- O	0 / 104	0.0	0.0	0.09 (1)	10.00
O- F	-514 / 0	0.0	0.0	0.19 (1)	7.81
O- N	0 / 2168	-28.0	-28.0	0.51 (2)	10.00
N- M	0 / 3376	-28.0	-28.0	0.69 (1)	10.00
L- M	0 / 36	0.0	0.0	0.25 (1)	10.00
M- H	0 / 272	0.0	0.0	0.29 (1)	10.00
L- K	0 / 153	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- S	-3 / 139	0.03 (3)
S- D	0 / 314	0.07 (2)
D- Q	0 / 793	0.18 (1)
Q- E	-989 / 0	0.72 (1)
O- G	0 / 855	0.19 (1)
N- G	0 / 398	0.09 (2)
N- H	-1230 / 0	0.89 (1)
T- C	-2524 / 0	0.56 (1)
M- K	-195 / 0	0.02 (1)
M- I	0 / 3181	0.72 (1)
Q- O	0 / 2320	0.37 (1)
E- O	0 / 579	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.75/1.00 (G-H:1) , BC=0.69/1.00 (M-N:1) , WB=0.99/1.00 (H-N:1) , SSI=0.25/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.62 (C) (INPUT = 1.00)

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DWG NO. TAM U714
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T43	1	1	TRUSS DESC.	

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Tamarack Roof Truss, Burlington

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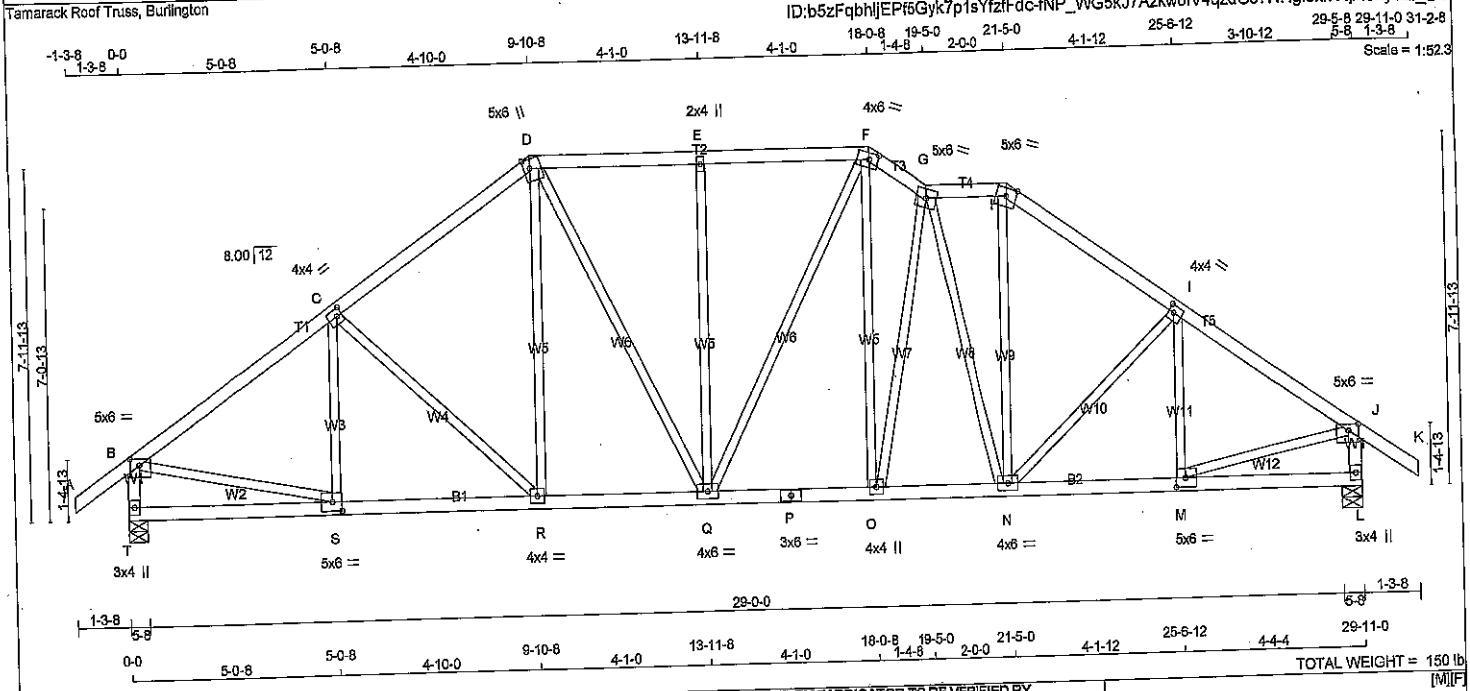

TOTAL WEIGHT = 150 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TTWW-m	MT20	5.0	6.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMWW-p	MT20	5.0	6.0	1.75	2.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW+t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2132	0	2132	0	0	5-8	5-8
L	2132	0	2132	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1682	1061 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0
L	1682	1061 / 0	314 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
B- C	-2319 / 0	-104.9	-104.9	0.41 (1)	4.12	
C- D	-2099 / 0	-104.9	-104.9	0.39 (1)	4.32	
D- E	-1955 / 0	-104.9	-104.9	0.26 (1)	4.59	
E- F	-1955 / 0	-104.9	-104.9	0.26 (1)	4.59	
F- G	-2219 / 0	-104.9	-104.9	0.08 (1)	4.54	
G- H	-1805 / 0	-104.9	-104.9	0.09 (1)	4.93	
H- I	-2185 / 0	-104.9	-104.9	0.29 (1)	4.36	
I- J	-2280 / 0	-104.9	-104.9	0.30 (1)	4.27	
J- K	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
T- B	-2073 / 0	0.0	0.0	0.21 (1)	5.87	
L- J	-2080 / 0	0.0	0.0	0.21 (1)	5.87	
T- S	0 / 0	-28.0	-28.0	0.17 (3)	10.00	
S- R	0 / 1957	-28.0	-28.0	0.39 (1)	10.00	
R- Q	0 / 1720	-28.0	-28.0	0.34 (1)	10.00	
Q- P	0 / 1858	-28.0	-28.0	0.35 (1)	10.00	
P- O	0 / 1858	-28.0	-28.0	0.35 (1)	10.00	
O- N	0 / 2002	-28.0	-28.0	0.36 (1)	10.00	
N- M	0 / 1920	-28.0	-28.0	0.36 (1)	10.00	
M- L	0 / 0	-28.0	-28.0	0.12 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-266 / 102	0.09 (1)
C- R	-325 / 0	0.26 (1)
R- D	0 / 350	0.08 (2)
D- Q	0 / 502	0.11 (1)
Q- E	-512 / 0	0.62 (1)
Q- F	0 / 206	0.05 (1)
O- F	0 / 803	0.18 (1)
O- G	-725 / 0	0.67 (1)
G- N	-727 / 0	0.69 (1)
N- H	0 / 929	0.21 (1)
N- I	-177 / 0	0.10 (1)
M- I	-346 / 38	0.10 (1)
B- S	0 / 2001	0.45 (1)
M- J	0 / 1978	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.39/1.00 (R-S:1) , WB=0.69/1.00 (G-N:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 11738-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T43S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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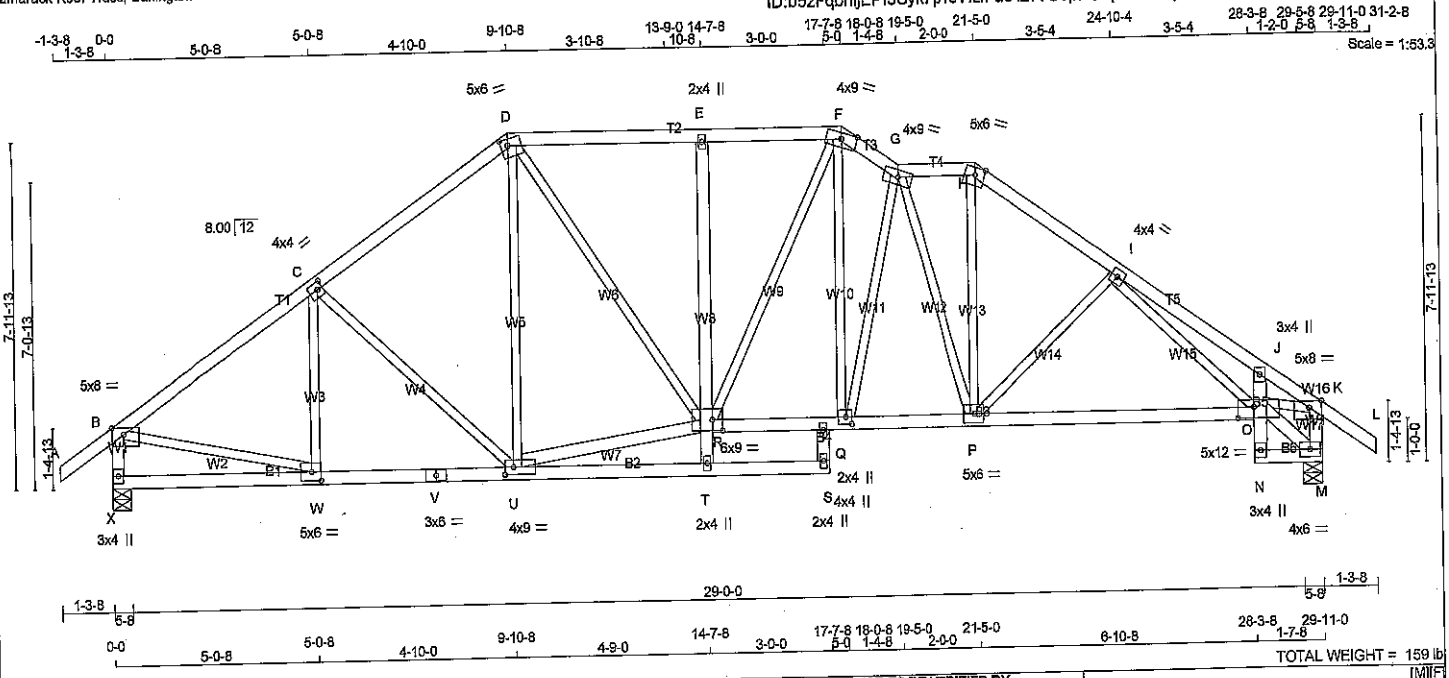

TOTAL WEIGHT = 159 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - S	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
T - E	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	4.0	9.0	Edge	
G	TTWW-m	MT20	4.0	9.0		
H	TTW-m	MT20	5.0	6.0	Edge	
I	TMWW-t	MT20	4.0	4.0		
J	TMV+p	MT20	3.0	4.0		
K	TMVW-p	MT20	5.0	8.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWWW-l	MT20	5.0	12.0	3.00	4.75
P	BMWWW-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	4.0	4.0	2.00	1.75
R	BWMWWW*-l	MT20	6.0	9.0	3.00	3.00
S	NP+w	MT20	2.0	4.0		
T	BMW+w	MT20	2.0	4.0		
U	BMWWW-t	MT20	4.0	9.0	2.00	2.50
V	BS-t	MT20	3.0	6.0		
W	BMWW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+p	MT20	3.0	4.0		
Y	NP+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2172	0	2172	0	0	5-8	5-8
M	2177	0	2177	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1734	1059 / 0	346 / 0	0 / 0	0 / 0	329 / 0	0 / 0
M	1736	1063 / 0	345 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2371 / 0	-104.9	-104.9	0.42 (1)	4.08
C- D	-2167 / 0	-104.9	-104.9	0.39 (1)	4.25
D- E	-2334 / 0	-104.9	-104.9	0.33 (1)	4.19
E- F	-2339 / 0	-104.9	-104.9	0.29 (1)	4.22
F- G	-2621 / 0	-104.9	-104.9	0.09 (1)	4.22
G- H	-2184 / 0	-104.9	-104.9	0.10 (1)	4.55
H- I	-2627 / 0	-104.9	-104.9	0.25 (1)	4.08
I- J	-3031 / 0	-104.9	-104.9	0.28 (1)	3.80
J- K	-2990 / 0	-104.9	-104.9	0.19 (1)	3.89
K- L	0 / 40	-104.9	-104.9	0.14 (1)	10.00
X- B	-2111 / 0	0.0	0.0	0.22 (1)	5.83
M- K	-2075 / 0	0.0	0.0	0.21 (1)	5.87
X- W	0 / 0	-28.0	-28.0	0.15 (3)	10.00
W- V	0 / 2001	-28.0	-28.0	0.41 (1)	10.00
V- U	0 / 2001	-28.0	-28.0	0.41 (1)	10.00
U- T	0 / 36	-28.0	-28.0	0.16 (2)	10.00
T- S	0 / 0	-28.0	-28.0	0.30 (2)	10.00
R- Q	0 / 2196	-28.0	-28.0	0.41 (1)	10.00
Q- P	0 / 2417	-28.0	-28.0	0.58 (2)	10.00
P- O	0 / 2476	-28.0	-28.0	0.57 (2)	10.00
N- O	0 / 36	0.0	0.0	0.17 (1)	10.00
O- J	-170 / 0	0.0	0.0	0.13 (1)	7.81
N- M	0 / 120	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-285 / 70	0.10 (1)
C- U	-310 / 0	0.25 (1)
U- D	-6 / 151	0.03 (3)
P- H	0 / 1202	0.27 (1)
P- I	-428 / 0	0.18 (1)
I- O	0 / 132	0.03 (2)
B- W	0 / 2046	0.46 (1)
T- R	0 / 239	0.07 (2)
R- E	-519 / 0	0.17 (1)
U- R	0 / 1774	0.29 (1)
D- R	0 / 970	0.22 (1)
Q- F	0 / 974	0.22 (1)
R- F	0 / 315	0.07 (1)
G- P	-747 / 0	0.48 (1)
Q- G	-951 / 0	0.59 (1)
O- M	-145 / 0	0.02 (1)
O- K	0 / 2446	0.55 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.58/1.00 (P-Q:2) , WB=0.59/1.00 (G-Q:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.90 (O) (INPUT = 0.90)
METAL= 0.54 (F) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 11715 -R
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T44	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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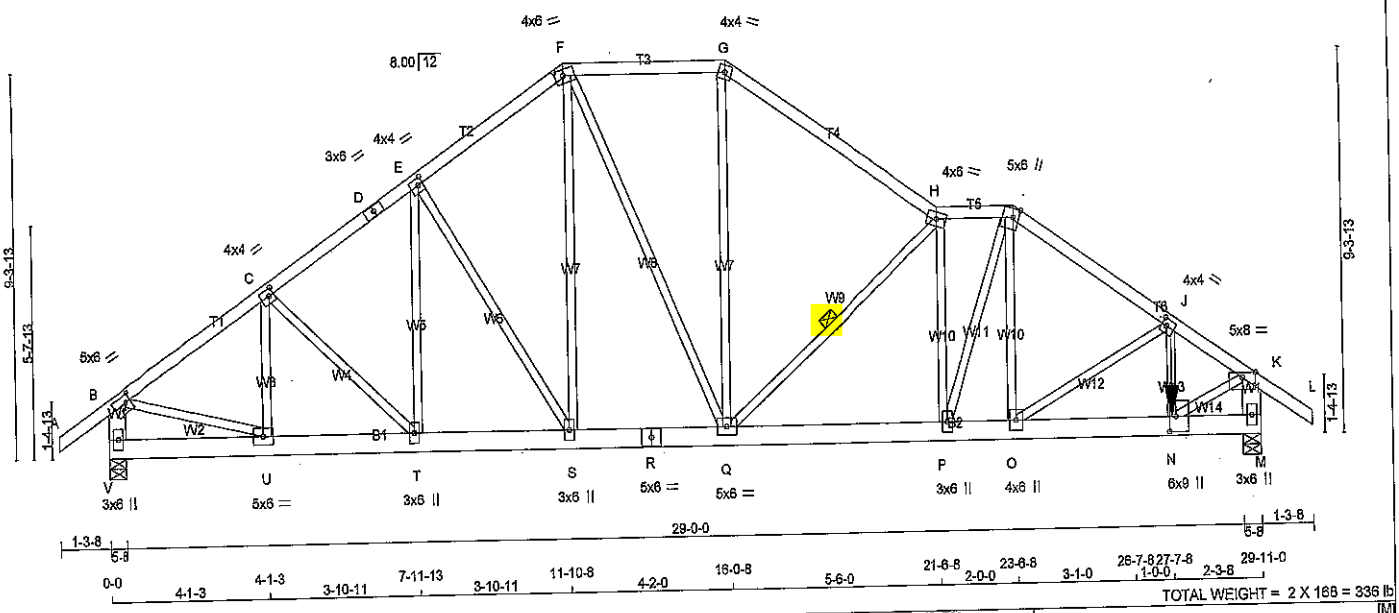

TOTAL WEIGHT = 2 X 168 = 336 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 - TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- H	1	12	TOP
H- I	1	12	TOP
I- L	1	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- R	2	12	TOP
R- M	2	6	SIDE(427.3)
WEBS : (0.122"X3") SPIRAL NAILS			
J- N	1	2	SIDE(492.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.25	1.75
C, E, J						
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
F	TTWW-m	MT20	4.0	6.0	1.75	2.50
G	TTW-m	MT20	4.0	4.0		
H	TTWW-m	MT20	4.0	6.0		
I	TTWW+m	MT20	5.0	6.0	Edge	1.75
K	TMWW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW+t	MT20	6.0	9.0	4.50	2.00
O	BMWW+t	MT20	4.0	6.0		
P, S, T						
P	BMWW+t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2403	0	2403	0	0	5-8	5-8
M	5389	0	5389	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1895	1195 / 0	354 / 0	0 / 0	0 / 0	346 / 0	0 / 0
M	4246	2685 / 0	789 / 0	0 / 0	0 / 0	774 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-2641 / 0	-104.9	-104.9	0.15 (1)	5.46
C- D	-2652 / 0	-104.9	-104.9	0.15 (1)	5.44
D- E	-2652 / 0	-104.9	-104.9	0.15 (1)	5.44
E- F	-2371 / 0	-104.9	-104.9	0.14 (1)	5.69
F- G	-2146 / 0	-104.9	-104.9	0.19 (1)	5.83
G- H	-2566 / 0	-104.9	-104.9	0.37 (1)	5.24
H- I	-3368 / 0	-104.9	-104.9	0.07 (1)	5.04
I- J	-3754 / 0	-104.9	-104.9	0.20 (1)	4.70
J- K	-5410 / 0	-104.9	-104.9	0.17 (1)	4.05
K- L	0 / 40	-104.9	-104.9	0.08 (1)	10.00
V- B	-2337 / 0	0.0	0.0	0.08 (1)	7.81
M- K	-5202 / 0	0.0	0.0	0.19 (1)	6.40
V- U	0 / 0	-28.0	-28.0	0.03 (2)	10.00
U- T	0 / 2215	-28.0	-28.0	0.17 (1)	10.00
T- S	0 / 2206	-28.0	-28.0	0.16 (1)	10.00
S- R	0 / 1958	-28.0	-28.0	0.15 (1)	10.00
R- Q	0 / 1958	-28.0	-28.0	0.15 (1)	10.00
Q- P	0 / 3381	-28.0	-28.0	0.25 (1)	10.00
P- O	0 / 3130	-28.0	-28.0	0.25 (1)	10.00
O- N	0 / 4521	-28.0	-28.0	0.42 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.13 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-443 / 0	0.06 (1)
C- T	-12 / 0	0.00 (1)
T- E	0 / 199	0.02 (2)
E- S	-475 / 0	0.26 (1)
S- F	0 / 510	0.06 (1)
F- Q	0 / 454	0.06 (1)
Q- G	0 / 916	0.11 (1)
Q- H	-1708 / 0	0.35 (1)
P- H	-648 / 0	0.15 (1)
P- I	0 / 701	0.09 (1)
O- I	0 / 1128	0.14 (1)
O- J	-1650 / 0	0.33 (1)
B- U	0 / 2279	0.28 (1)
N- J	0 / 1382	0.17 (1)
N- K	0 / 4930	0.61 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	27-7-8	-3527	-3527	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.37/1.00 (G-H:1) , BC=0.42/1.00 (N-O:1) , WB=0.61/1.00 (K-N:1) , SSI=0.10/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.87 (J) (INPUT = 0.90)
METAL= 0.49 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30218
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DWG NO. TAM 11739-18
STRUCTURAL
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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T44	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWWW-t	MT20	5.0	6.0		
R	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3528.7 lbs FACTORED DOWN AT 27-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
2018
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11739-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T44S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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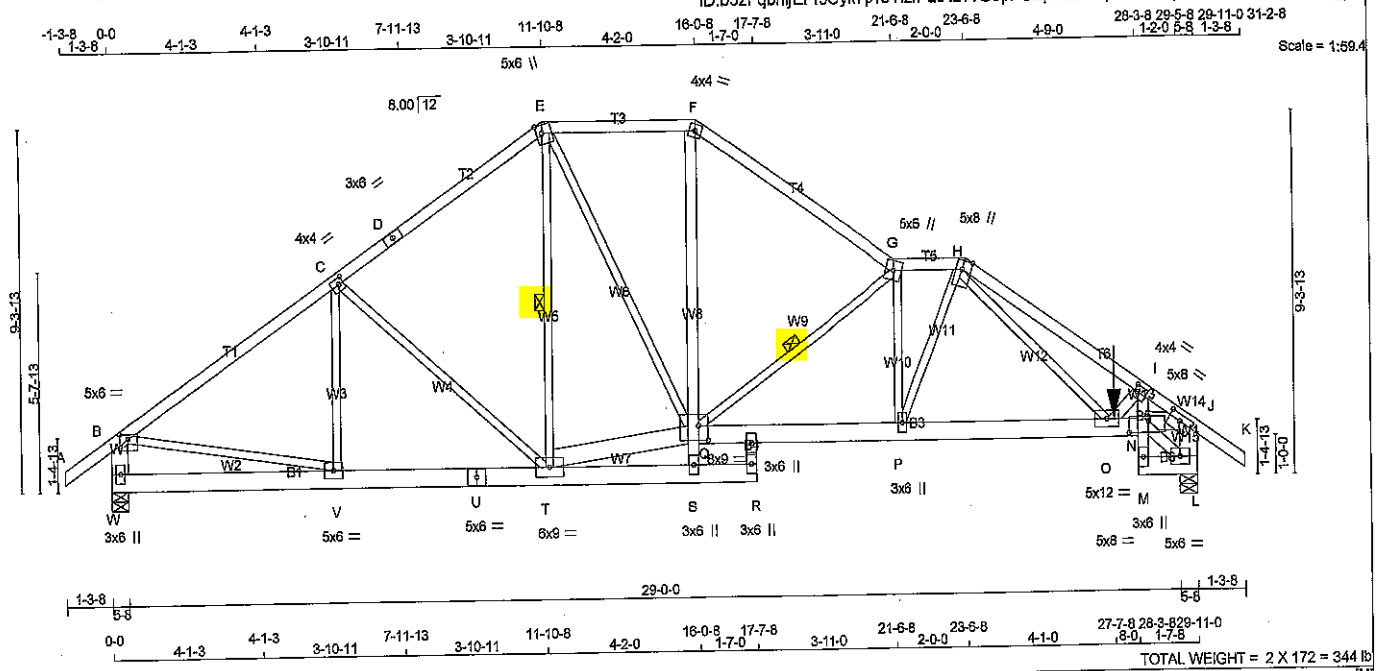

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
U - R	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - F	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
E- F	1	12	TOP
F- G	1	12	TOP
G- H	1	12	TOP
H- K	1	12	TOP
W- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- U	2	12	TOP
U- R	2	12	TOP
Q- N	2	12	SIDE(0.0)
M- L	2	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2450	0	2450	0	0	5-8	5-8
L	5411	0	5411	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1945	1206 / 0	377 / 0	0 / 0	0 / 0	362 / 0	0 / 0
L	4295	2662 / 0	832 / 0	0 / 0	0 / 0	801 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-T, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00
B- C	-2821 / 0	-104.9	-104.9	0.45 (1)	4.97
C- D	-2482 / 0	-104.9	-104.9	0.42 (1)	5.24
D- E	-2482 / 0	-104.9	-104.9	0.42 (1)	5.24
E- F	-2500 / 0	-104.9	-104.9	0.20 (1)	5.49
F- G	-2989 / 0	-104.9	-104.9	0.39 (1)	4.92
G- H	-4397 / 0	-104.9	-104.9	0.10 (1)	4.50
H- I	-8447 / 0	-104.9	-104.9	0.69 (1)	2.93
I- J	-9622 / 0	-104.9	-104.9	0.36 (1)	2.90
J- K	0 / 40	-104.9	-104.9	0.08 (1)	10.00
W- B	-2367 / 0	0.0	0.0	0.08 (1)	7.81
L- J	-5148 / 0	0.0	0.0	0.18 (1)	6.42
W- V	0 / 0	-28.0	-28.0	0.06 (2)	10.00
V- U	0 / 2381	-28.0	-28.0	0.19 (1)	10.00
U- T	0 / 2381	-28.0	-28.0	0.19 (1)	10.00
T- S	0 / 28	-28.0	-28.0	0.05 (2)	10.00
S- R	0 / 0	-28.0	-28.0	0.03 (2)	10.00
Q- P	0 / 4412	-28.0	-28.0	0.38 (1)	10.00
P- O	0 / 3966	-28.0	-28.0	0.53 (1)	10.00
O- N	0 / 8452	-28.0	-28.0	0.82 (1)	10.00
M- N	0 / 36	0.0	0.0	0.27 (1)	10.00
N- I	0 / 1638	0.0	0.0	0.41 (1)	10.00
M- L	0 / 331	-28.0	-28.0	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-227 / 154	0.05 (1)
C- T	-475 / 0	0.29 (1)
T- E	-8 / 160	0.02 (3)
S- Q	0 / 154	0.03 (1)
Q- F	0 / 1151	0.12 (1)
Q- G	-2418 / 0	0.42 (1)
P- G	-620 / 0	0.10 (1)
P- H	0 / 1085	0.13 (1)
B- V	0 / 2412	0.30 (1)
T- Q	0 / 2059	0.13 (1)
E- Q	0 / 1039	0.13 (1)
N- L	-410 / 0	0.02 (1)
N- J	0 / 7984	0.52 (1)
H- O	0 / 4546	0.56 (1)
O- I	-1843 / 0	0.10 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
O	27-7-8	-3548	-3548	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.69/1.00 (H-I:1) , BC=0.82/1.00 (N-O:1) , WB=0.56/1.00 (H-O:1) , SSI=0.24/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.91 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11716.88
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T44S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	Edge	1.75
F	TTW-m	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0		
H	TTWW+m	MT20	5.0	8.0	Edge	3.00
I	TMVW-t	MT20	4.0	4.0	2.00	1.00
J	TMVW-t	MT20	5.0	8.0	2.00	3.75
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-I	MT20	5.0	12.0	3.50	6.50
O	BMWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	3.0	6.0		
Q	BWMWWW*-I	MT20	8.0	9.0	4.50	3.00
R	NP+w	MT20	3.0	6.0		
S	BMW+w	MT20	3.0	6.0		
T	BMWWW-t	MT20	6.0	9.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMV1+p	MT20	3.0	6.0		
X	NP+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3548.4 lbs FACTORED DOWN AT 27-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
3 2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11716-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T45S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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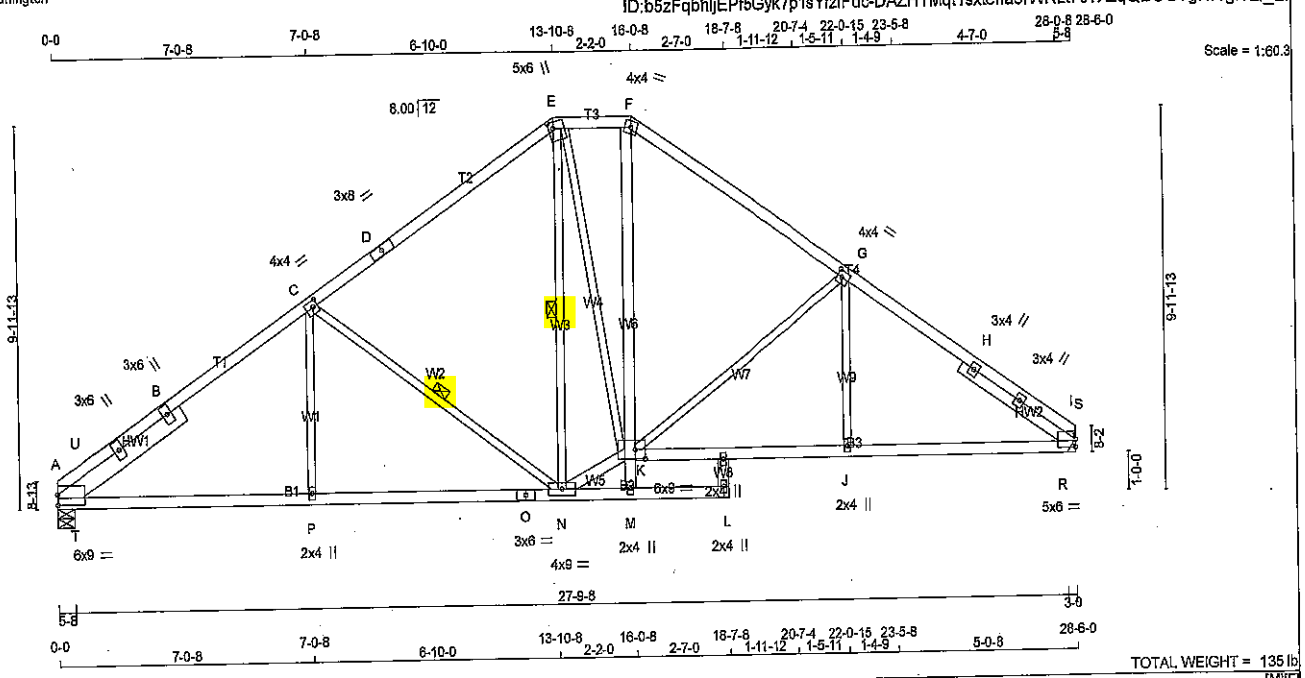

TOTAL WEIGHT = 135 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
A - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	3.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMBMR1-I	MT20	5.0	6.0	2.50	
I	RT+t	MT20	3.0	4.0		
I	RT+t	MT20	3.0	4.0		
J, M, P						
J	BMW+w	MT20	2.0	4.0		
K	BWMWWW*-I	MT20	6.0	9.0	3.00	3.25
L	NP+w	MT20	2.0	4.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
Q	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1941	0	1941	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	
A	1924	0	1924	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	1568	926 / 0	334 / 0	0 / 0	0 / 0	308 / 0	0 / 0
A	1548	926 / 0	322 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N, E-N.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- U	-2419 / 0	-104.9	-104.9	0.10 (1)	4.36
U- B	-2068 / 0	-104.9	-104.9	0.61 (1)	4.09
B- C	-2068 / 0	-104.9	-104.9	0.61 (1)	4.09
C- D	-1769 / 0	-104.9	-104.9	0.69 (1)	4.17
D- E	-1769 / 0	-104.9	-104.9	0.69 (1)	4.17
E- F	-1598 / 0	-104.9	-104.9	0.09 (1)	5.16
F- G	-1942 / 0	-104.9	-104.9	0.54 (1)	4.26
G- H	-2064 / 0	-104.9	-104.9	0.46 (1)	4.28
H- S	-2064 / 0	-104.9	-104.9	0.46 (1)	4.28
S- I	-2416 / 0	-104.9	-104.9	0.11 (1)	4.35
A- T	0 / 1399	-28.0	-28.0	0.35 (2)	10.00
T- P	0 / 2118	-28.0	-28.0	0.50 (2)	10.00
P- O	0 / 2118	-28.0	-28.0	0.50 (2)	10.00
O- N	0 / 2118	-28.0	-28.0	0.50 (2)	10.00
N- M	0 / 21	-28.0	-28.0	0.25 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.25 (2)	10.00
K- J	0 / 2191	-28.0	-28.0	0.50 (2)	10.00
J- R	0 / 2191	-28.0	-28.0	0.50 (2)	10.00
R- I	0 / 1302	-28.0	-28.0	0.32 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 345	0.08 (2)
C- N	-842 / 0	0.43 (1)
N- E	-129 / 36	0.09 (1)
M- K	0 / 200	0.05 (2)
K- F	0 / 659	0.14 (1)
N- K	0 / 1601	0.26 (1)
E- K	0 / 705	0.16 (1)
K- G	-753 / 0	0.91 (1)
J- G	0 / 340	0.08 (2)
R- S	0 / 997	0.00 (1)
R- H	-1194 / 0	0.21 (1)
T- U	0 / 856	0.00 (1)
T- B	-992 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.26")

CSI: TC=0.69/1.00 (C-E:1) , BC=0.50/1.00 (N-P:2) , WB=0.91/1.00 (G-K:1) , SSI=0.30/1.00 (I-R:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.65 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2016
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11717 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T45A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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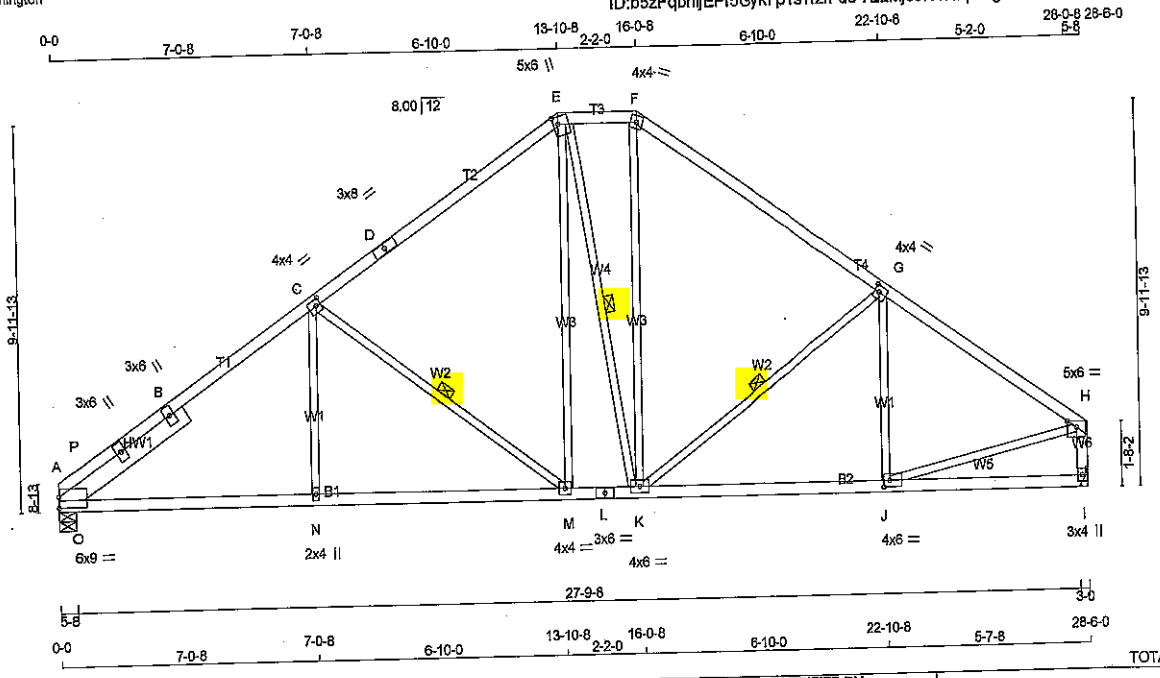

TOTAL WEIGHT = 129 lb.
[MII]F

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
A - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	3.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.00
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
A	1894	0	1894	0	0	IN-SX	IN-SX
I	1894	0	1894	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0
I	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, E-K, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- P	-2369 / 0	-104.9	-104.9	0.10 (1)	4.39	
P- B	-2029 / 0	-104.9	-104.9	0.60 (1)	4.13	
B- C	-2029 / 0	-104.9	-104.9	0.60 (1)	4.13	
C- D	-1714 / 0	-104.9	-104.9	0.68 (1)	4.24	
D- E	-1714 / 0	-104.9	-104.9	0.68 (1)	4.24	
E- F	-1384 / 0	-104.9	-104.9	0.08 (1)	5.46	
F- G	-1699 / 0	-104.9	-104.9	0.68 (1)	4.24	
G- H	-2105 / 0	-104.9	-104.9	0.83 (1)	3.95	
I- H	-1833 / 0	0.0	0.0	0.20 (1)	6.18	
A- O	0 / 1373	-28.0	-28.0	0.34 (2)	10.00	
O- N	0 / 2078	-28.0	-28.0	0.50 (2)	10.00	
N- M	0 / 2078	-28.0	-28.0	0.50 (2)	10.00	
M- L	0 / 1400	-28.0	-28.0	0.36 (2)	10.00	
L- K	0 / 1400	-28.0	-28.0	0.36 (2)	10.00	
K- J	0 / 1793	-28.0	-28.0	0.46 (2)	10.00	
J- I	0 / 0	-28.0	-28.0	0.27 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	0 / 361	0.06 (2)
C- M	-850 / 0	0.43 (1)
M- E	0 / 605	0.14 (1)
E- K	-60 / 0	0.06 (1)
K- F	0 / 524	0.12 (1)
K- G	-515 / 0	0.26 (1)
J- G	-239 / 168	0.11 (1)
J- H	0 / 1845	0.42 (1)
O- P	0 / 828	0.00 (1)
O- B	-972 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.68/1.00 (F-G:1) , BC=0.50/1.00 (N-O:2) , WB=0.43/1.00 (C-M:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.89 (H) (INPUT = 0.90)
USI METAL= 0.62 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11740-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T46	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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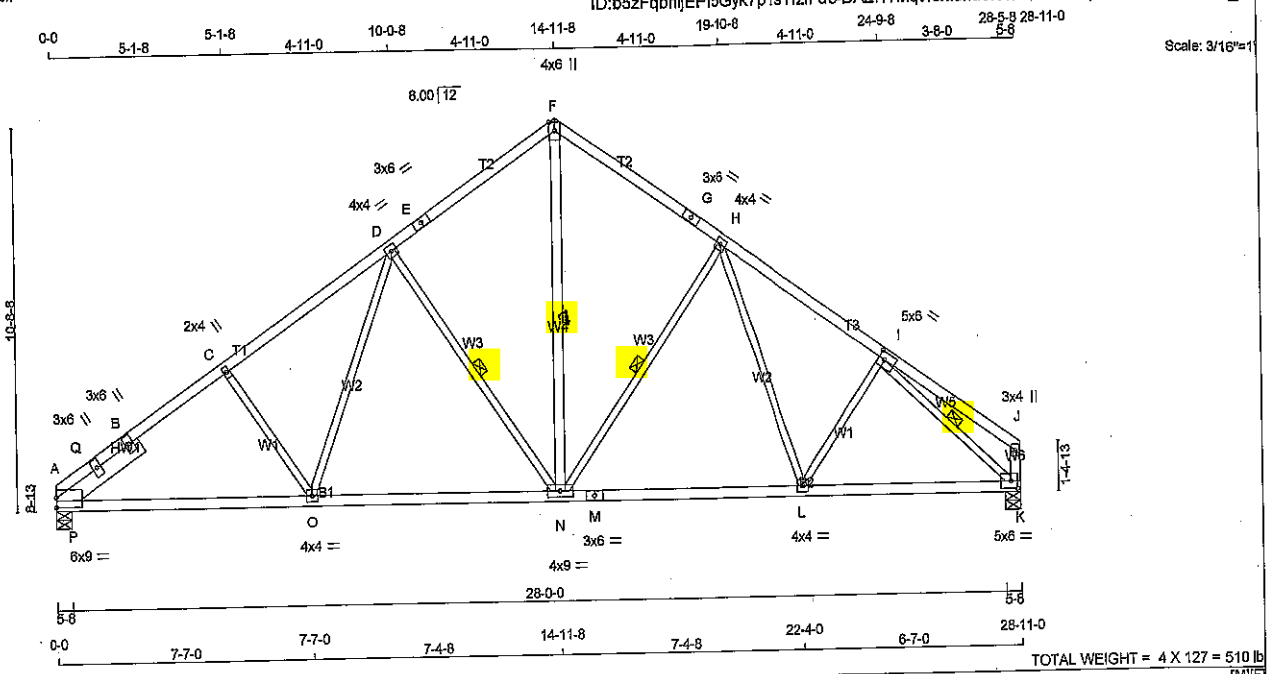

TOTAL WEIGHT = 4 X 127 = 510 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	3.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1922	0	1922	0	0	5-8	5-8
K	1922	0	1922	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1532	939 / 0	304 / 0	0 / 0	0 / 0	289 / 0	0 / 0
K	1532	939 / 0	304 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-N, H-N, I-K. F-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- Q	-2502 / 0	-104.9	-104.9	0.11 (1)	4.30
Q- B	-2067 / 0	-104.9	-104.9	0.24 (1)	4.53
B- C	-2067 / 0	-104.9	-104.9	0.24 (1)	4.53
C- D	-2335 / 0	-104.9	-104.9	0.40 (1)	4.13
D- E	-1653 / 0	-104.9	-104.9	0.35 (1)	4.80
E- F	-1653 / 0	-104.9	-104.9	0.35 (1)	4.80
F- G	-1651 / 0	-104.9	-104.9	0.34 (1)	4.82
G- H	-1651 / 0	-104.9	-104.9	0.34 (1)	4.82
H- I	-2140 / 0	-104.9	-104.9	0.36 (1)	4.32
I- J	0 / 25	-104.9	-104.9	0.25 (1)	10.00
K- J	-166 / 0	0.0	0.0	0.02 (1)	7.81
A- P	0 / 1373	-28.0	-28.0	0.40 (2)	10.00
P- O	0 / 2129	-28.0	-28.0	0.51 (2)	10.00
O- N	0 / 1805	-28.0	-28.0	0.49 (2)	10.00
N- M	0 / 1711	-28.0	-28.0	0.49 (2)	10.00
M- L	0 / 1711	-28.0	-28.0	0.49 (2)	10.00
L- K	0 / 1797	-28.0	-28.0	0.50 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-303 / 0	0.11 (1)
O- D	0 / 476	0.11 (1)
D- N	-801 / 0	0.41 (1)
N- F	0 / 1395	0.22 (1)
N- H	-638 / 0	0.32 (1)
H- L	0 / 308	0.07 (2)
L- I	-14 / 135	0.03 (3)
I- K	-2461 / 0	0.55 (1)
P- Q	0 / 1145	0.00 (1)
P- B	-1126 / 0	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.51/1.00 (O-P:2) , WB=0.55/1.00 (I-K:1) , SSI=0.27/1.00 (A-P:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.63 (A) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 11716 -18

STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T46A	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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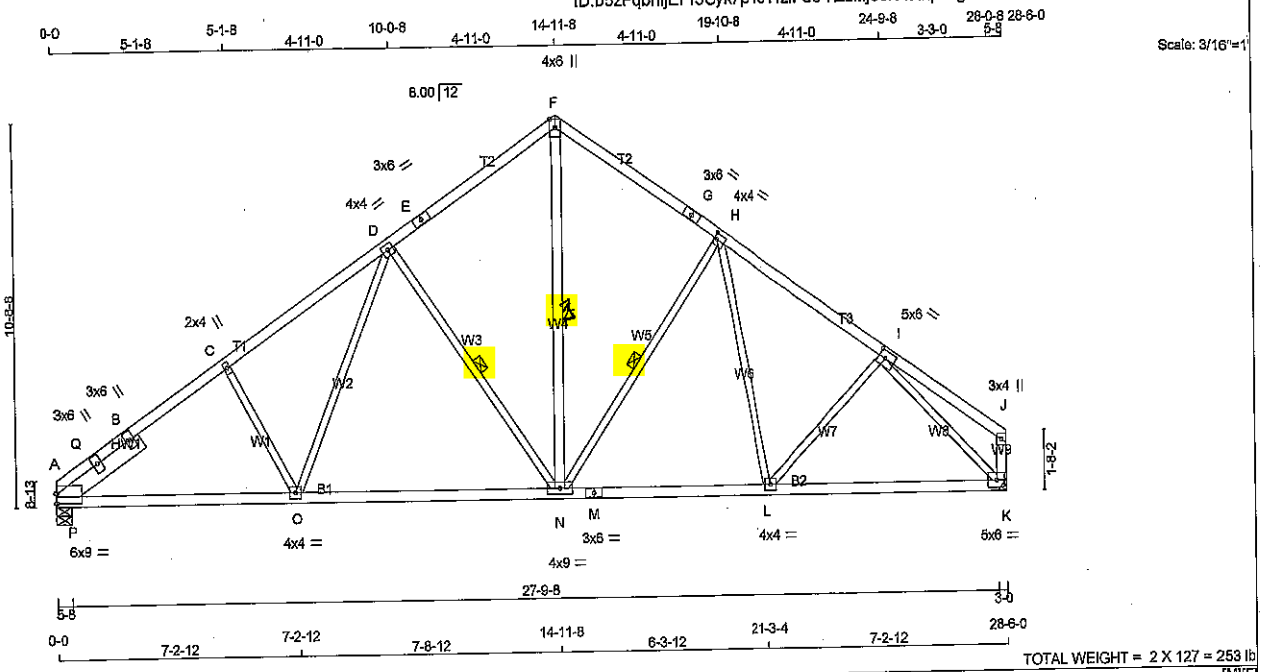

TOTAL WEIGHT = 2 X 127 = 253 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	3.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1894	0	1894	0	0	5-8	5-8
K	1894	0	1894	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-N, H-N. F-H

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- Q	-2419 / 0	-104.9	-104.9	0.10 (1)	4.36
Q- B	-2021 / 0	-104.9	-104.9	0.24 (1)	4.57
B- C	-2021 / 0	-104.9	-104.9	0.24 (1)	4.57
C- D	-2324 / 0	-104.9	-104.9	0.40 (1)	4.13
D- E	-1604 / 0	-104.9	-104.9	0.35 (1)	4.85
E- F	-1604 / 0	-104.9	-104.9	0.35 (1)	4.85
F- G	-1602 / 0	-104.9	-104.9	0.33 (1)	4.88
G- H	-1602 / 0	-104.9	-104.9	0.33 (1)	4.88
H- I	-1994 / 0	-104.9	-104.9	0.36 (1)	4.46
I- J	0 / 26	-104.9	-104.9	0.23 (1)	10.00
K- J	-142 / 0	0.0	0.0	0.02 (1)	7.81
A- P	0 / 1328	-28.0	-28.0	0.35 (2)	10.00
P- O	0 / 2097	-28.0	-28.0	0.53 (2)	10.00
O- N	0 / 1770	-28.0	-28.0	0.49 (2)	10.00
N- M	0 / 1641	-28.0	-28.0	0.48 (2)	10.00
M- L	0 / 1641	-28.0	-28.0	0.48 (2)	10.00
L- K	0 / 1596	-28.0	-28.0	0.47 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	-293 / 1	0.09 (1)
O- D	0 / 497	0.11 (1)
D- N	-799 / 0	0.41 (1)
N- F	0 / 1339	0.22 (1)
N- H	-592 / 0	0.30 (1)
H- L	0 / 250	0.06 (3)
L- I	0 / 174	0.04 (2)
I- K	-2308 / 0	1.00 (1)
P- Q	0 / 1107	0.00 (1)
P- B	-1146 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.53/1.00 (O-P:2) , WB=1.00/1.00 (I-K:1) , SSI=0.22/1.00 (A-P:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.63 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30218
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11741-A
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T46S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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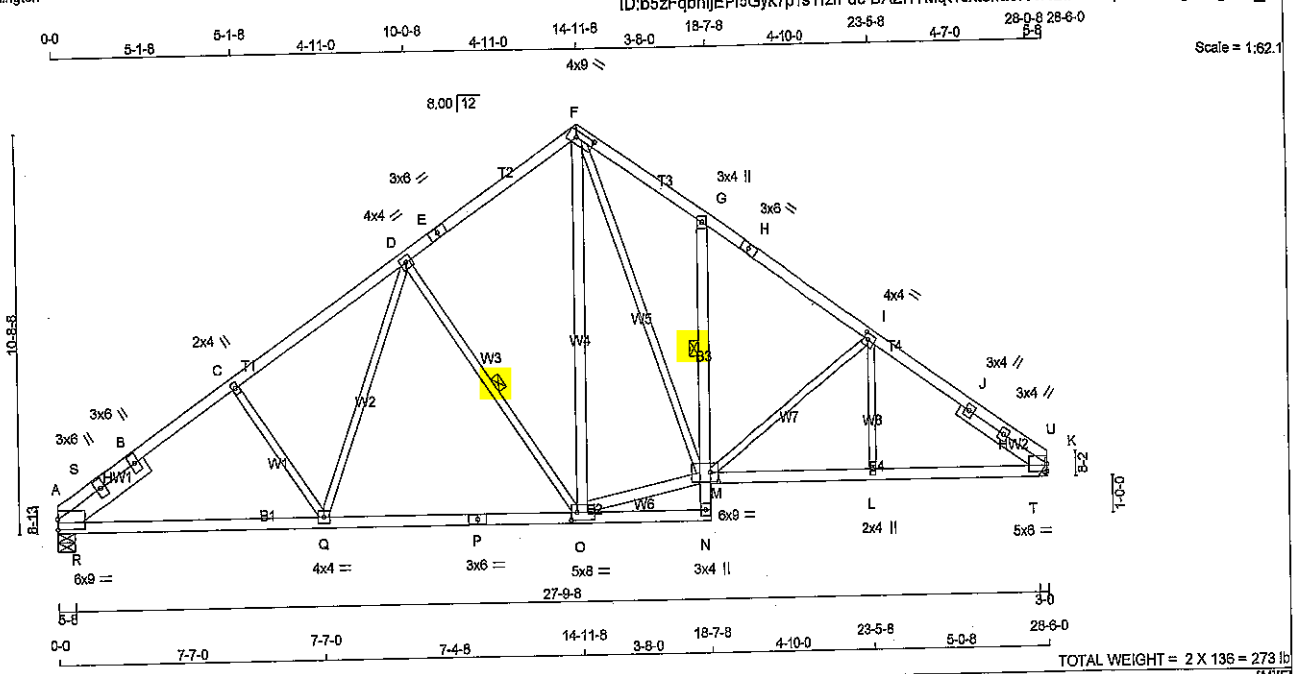

TOTAL WEIGHT = 2 X 136 = 273 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
A - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
HW2	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - F	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	3.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TTWW-I	MT20	4.0	9.0	2.00	6.00
G	TMV+p	MT20	3.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMBMR1-I	MT20	5.0	6.0	2.50	
K	RT+t	MT20	3.0	4.0		
K	RT+t	MT20	3.0	4.0		
L	BMW+w	MT20	2.0	4.0		
M	BVMWWW-I	MT20	6.0	9.0	3.00	2.75
N	BMV+p	MT20	3.0	4.0		
O	BMWWW-t	MT20	5.0	8.0	2.50	2.00
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1894	0	1894	0	0	5-8	5-8
K	1894	0	1894	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	1510	926 / 0	299 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, D-O.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- S	-2461 / 0	-104.9	-104.9	0.10 (1)		4.33
S- B	-2033 / 0	-104.9	-104.9	0.23 (1)		4.57
B- C	-2033 / 0	-104.9	-104.9	0.23 (1)		4.57
C- D	-2290 / 0	-104.9	-104.9	0.40 (1)		4.16
D- E	-1609 / 0	-104.9	-104.9	0.35 (1)		4.85
E- F	-1609 / 0	-104.9	-104.9	0.35 (1)		4.85
F- G	-2156 / 0	-104.9	-104.9	0.26 (1)		4.41
G- H	-2130 / 0	-104.9	-104.9	0.30 (1)		4.41
H- I	-2130 / 0	-104.9	-104.9	0.30 (1)		4.41
I- J	-1999 / 0	-104.9	-104.9	0.24 (1)		4.59
J- U	-1999 / 0	-104.9	-104.9	0.24 (1)		4.59
U- K	-2396 / 0	-104.9	-104.9	0.07 (1)		4.41
A- R	0 / 1352	-28.0	-28.0	0.39 (2)		10.00
R- Q	0 / 2093	-28.0	-28.0	0.52 (2)		10.00
Q- P	0 / 1766	-28.0	-28.0	0.48 (2)		10.00
P- O	0 / 1766	-28.0	-28.0	0.48 (2)		10.00
O- N	0 / 34	-28.0	-28.0	0.24 (2)		10.00
N- M	0 / 40	0.0	0.0	0.05 (1)		10.00
M- G	-509 / 0	0.0	0.0	0.06 (1)		6.25
M- L	0 / 2155	-28.0	-28.0	0.44 (1)		10.00
L- T	0 / 2155	-28.0	-28.0	0.44 (1)		10.00
T- K	0 / 1221	-28.0	-28.0	0.29 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-305 / 0	0.11 (1)
Q- D	0 / 482	0.11 (1)
D- O	-806 / 0	0.41 (1)
O- F	0 / 493	0.08 (1)
O- M	0 / 1323	0.21 (1)
F- M	0 / 1310	0.29 (1)
M- I	-469 / 0	0.32 (1)
L- I	0 / 260	0.06 (3)
R- S	0 / 1120	0.00 (1)
R- B	-1104 / 0	0.09 (1)
T- U	0 / 1187	0.00 (1)
T- J	-1308 / 0	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.95")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.95")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.52/1.00 (Q-R:2) , WB=0.41/1.00 (D-O:1) , SSI=0.27/1.00 (A-R:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.64 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11219 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T47	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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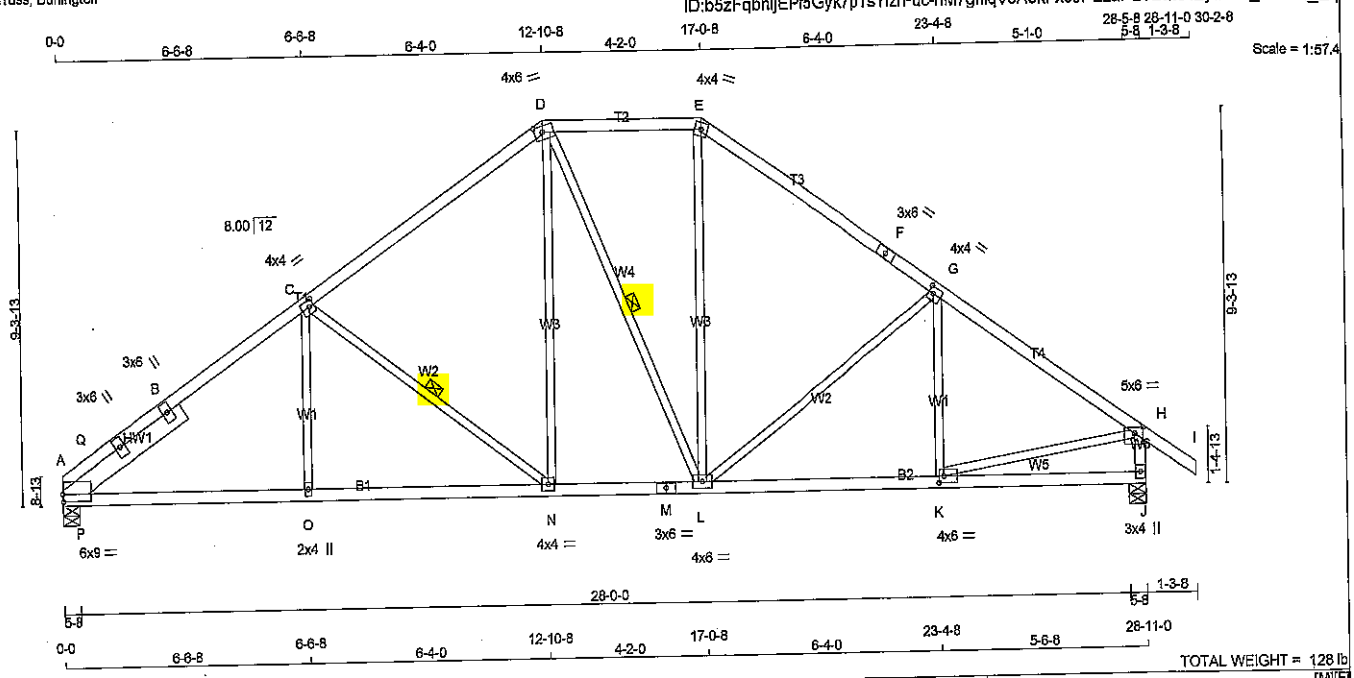

TOTAL WEIGHT = 128 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	2.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.75	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	1.50
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1922	0	1922	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1532	939 / 0	304 / 0	0 / 0	0 / 0	289 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-N, D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- Q	-2425 / 0	-104.9	-104.9	0.07 (1)	4.36
Q- B	-2063 / 0	-104.9	-104.9	0.49 (1)	4.25
B- C	-2063 / 0	-104.9	-104.9	0.49 (1)	4.25
C- D	-1866 / 0	-104.9	-104.9	0.60 (1)	4.24
D- E	-1502 / 0	-104.9	-104.9	0.26 (1)	5.08
E- F	-1839 / 0	-104.9	-104.9	0.60 (1)	4.25
F- G	-1839 / 0	-104.9	-104.9	0.60 (1)	4.25
G- H	-2258 / 0	-104.9	-104.9	0.58 (1)	3.92
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
J- H	-2004 / 0	0.0	0.0	0.21 (1)	5.96
A- P	0 / 1376	-28.0	-28.0	0.33 (2)	10.00
P- O	0 / 2132	-28.0	-28.0	0.46 (1)	10.00
O- N	0 / 2132	-28.0	-28.0	0.46 (1)	10.00
N- M	0 / 1531	-28.0	-28.0	0.34 (1)	10.00
M- L	0 / 1531	-28.0	-28.0	0.34 (1)	10.00
L- K	0 / 1917	-28.0	-28.0	0.45 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 319	0.07 (2)
C- N	-757 / 0	0.33 (1)
N- D	0 / 591	0.13 (1)
D- L	-71 / 0	0.05 (1)
L- E	0 / 521	0.12 (1)
L- G	-524 / 0	0.71 (1)
K- G	-187 / 179	0.07 (1)
K- H	0 / 1953	0.44 (1)
P- Q	0 / 935	0.00 (1)
P- B	-1057 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.60/1.00 (E-G:1) , BC=0.46/1.00 (O-P:1) , WB=0.71/1.00 (G-L:1) , SSI=0.26/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.63 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11720.18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T48	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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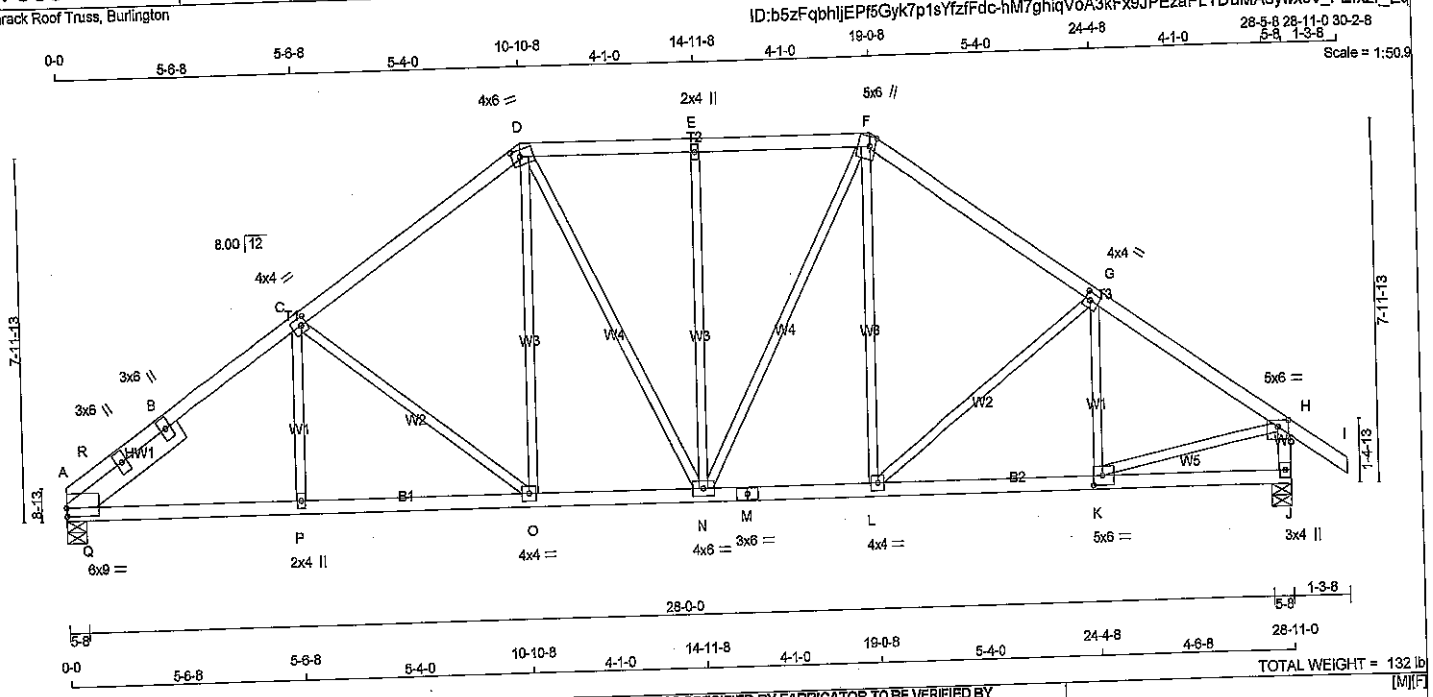

TOTAL WEIGHT = 132 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

REINFORCING MEMBERS

HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-t	MT20	6.0	9.0	2.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1922	0	1922	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1532	939 / 0	304 / 0	0 / 0	0 / 0	289 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- R	-2467 / 0	-104.9	-104.9	0.10 (1)	4.33	
R- B	-2064 / 0	-104.9	-104.9	0.34 (1)	4.41	
B- C	-2064 / 0	-104.9	-104.9	0.34 (1)	4.41	
C- D	-2069 / 0	-104.9	-104.9	0.46 (1)	4.25	
D- E	-1832 / 0	-104.9	-104.9	0.25 (1)	4.71	
E- F	-1832 / 0	-104.9	-104.9	0.25 (1)	4.71	
F- G	-1996 / 0	-104.9	-104.9	0.42 (1)	4.37	
G- H	-2220 / 0	-104.9	-104.9	0.40 (1)	4.20	
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	
J- H	-2014 / 0	0.0	0.0	0.21 (1)	5.95	
A- Q	0 / 1358	-28.0	-28.0	0.34 (1)	10.00	
Q- P	0 / 2155	-28.0	-28.0	0.47 (1)	10.00	
P- O	0 / 2155	-28.0	-28.0	0.43 (1)	10.00	
O- N	0 / 1708	-28.0	-28.0	0.36 (1)	10.00	
N- M	0 / 1636	-28.0	-28.0	0.34 (1)	10.00	
M- L	0 / 1636	-28.0	-28.0	0.34 (1)	10.00	
L- K	0 / 1879	-28.0	-28.0	0.38 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 254	0.06 (3)
C- O	-569 / 0	0.49 (1)
O- D	0 / 492	0.11 (1)
D- N	0 / 266	0.06 (1)
N- E	-514 / 0	0.62 (1)
N- F	0 / 418	0.09 (1)
L- F	0 / 358	0.06 (2)
L- G	-309 / 0	0.27 (1)
K- G	-297 / 66	0.09 (1)
K- H	0 / 1931	0.43 (1)
Q- R	0 / 1106	0.00 (1)
Q- B	-1161 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.47/1.00 (P-Q:1) , WB=0.62/1.00 (E-N:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.64 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11/2/18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T49	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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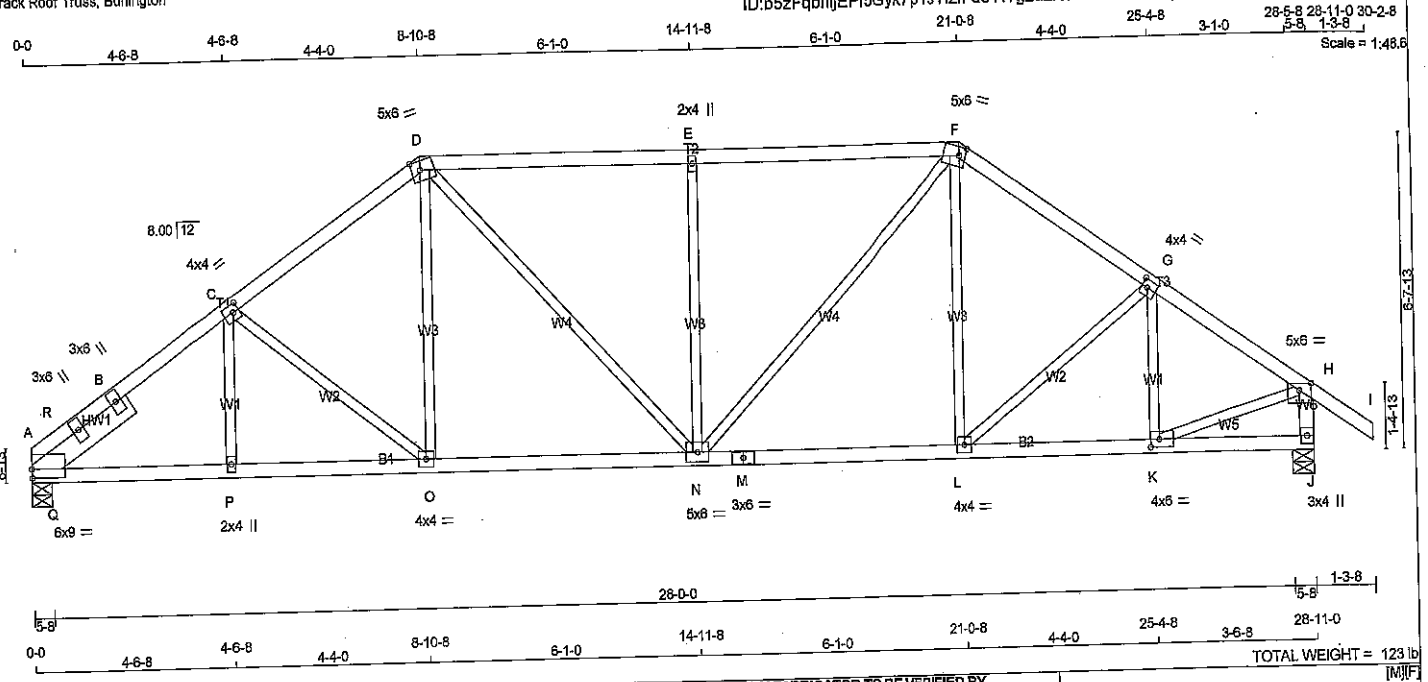

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-I	MT20	6.0	9.0	2.25	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMW-p	MT20	5.0	6.0	1.75	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	4.0	6.0	2.00	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	1922	0	1922	0	0	5-8	5-8
J	2066	0	2066	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1532	939 / 0	304 / 0	0 / 0	0 / 0	289 / 0	0 / 0
J	1629	1028 / 0	304 / 0	0 / 0	0 / 0	297 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- R	-2517 / 0	-104.9	-104.9	0.13 (1)	4.26	P- C
R- B	-2059 / 0	-104.9	-104.9	0.31 (1)	4.43	C- O
B- C	-2059 / 0	-104.9	-104.9	0.31 (1)	4.43	O- D
C- D	-2263 / 0	-104.9	-104.9	0.35 (1)	4.23	D- N
D- E	-2267 / 0	-104.9	-104.9	0.58 (1)	3.89	N- E
E- F	-2267 / 0	-104.9	-104.9	0.58 (1)	3.89	N- F
F- G	-2125 / 0	-104.9	-104.9	0.28 (1)	4.42	L- F
G- H	-2128 / 0	-104.9	-104.9	0.25 (1)	4.44	L- G
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	K- G
J- H	-2021 / 0	0.0	0.0	0.21 (1)	5.94	K- H
						Q- R
A- Q	0 / 1361	-28.0	-28.0	0.38 (1)	10.00	Q- B
Q- P	0 / 2157	-28.0	-28.0	0.51 (1)	10.00	
P- O	0 / 2157	-28.0	-28.0	0.45 (1)	10.00	
O- N	0 / 1877	-28.0	-28.0	0.41 (2)	10.00	
N- M	0 / 1748	-28.0	-28.0	0.40 (2)	10.00	
M- L	0 / 1748	-28.0	-28.0	0.40 (2)	10.00	
L- K	0 / 1795	-28.0	-28.0	0.39 (1)	10.00	
K- J	0 / 0	-28.0	-28.0	0.06 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	0 / 174	0.04 (3)
C- O	-362 / 0	0.19 (1)
O- D	0 / 410	0.09 (2)
D- N	0 / 564	0.13 (1)
N- E	-781 / 0	0.57 (1)
N- F	0 / 750	0.17 (1)
L- F	0 / 279	0.06 (2)
L- G	-63 / 0	0.03 (1)
K- G	-444 / 0	0.10 (1)
K- H	0 / 1877	0.42 (1)
Q- R	0 / 1282	0.00 (1)
Q- B	-1241 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.58/1.00 (D-E:1) , BC=0.51/1.00 (P-Q:1) , WB=0.57/1.00 (E-N:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.64 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11722-08
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T50	1	2	TRUSS DESC.	-

Tamarack Roof Truss, Burlington

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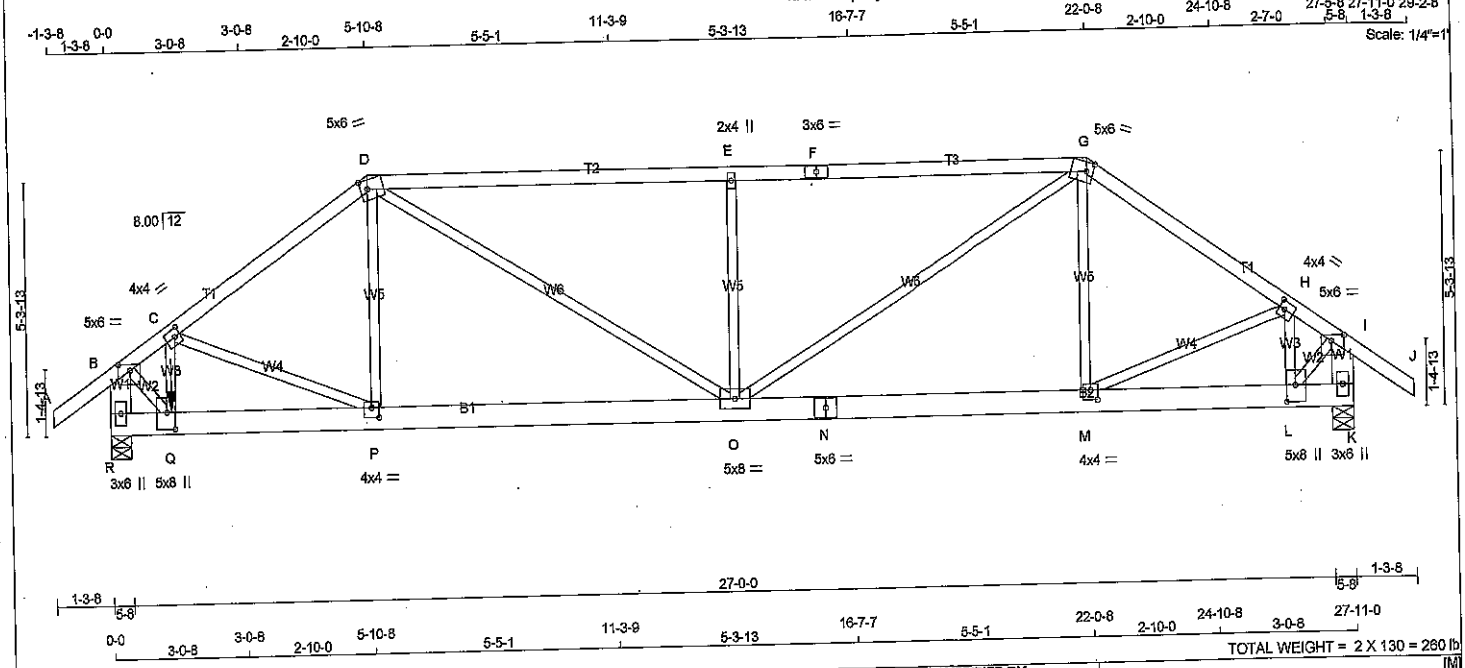

TOTAL WEIGHT = 2 X 130 = 260 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- G	1	12	TOP
G- J	1	12	TOP
R- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- N	2	4	SIDE(671.4)
N- K	2	4	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
C- Q	1	2	SIDE(284.1)
2x3	1	6	
H- L	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.25	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TTWW-m	MT20	5.0	6.0	2.25	1.75
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW+t	MT20	5.0	8.0	4.25	2.25
M	BMWW-t	MT20	4.0	4.0	2.50	2.00
N	BS-t	MT20	5.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW-t	MT20	4.0	4.0	2.50	2.00
Q	BMWW+t	MT20	5.0	8.0	4.25	2.25
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	5482	0	5482	0	0	5-8	5-8
K	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	4293	2761 / 0	766 / 0	0 / 0	0 / 0	766 / 0	0 / 0
K	1717	1087 / 0	318 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-4314 / 0	-104.9	-104.9	0.16 (1)	4.46	
C- D	-3099 / 0	-104.9	-104.9	0.21 (1)	5.07	
D- E	-3285 / 0	-104.9	-104.9	0.87 (1)	4.04	
E- F	-3285 / 0	-104.9	-104.9	0.87 (1)	4.04	
F- G	-3286 / 0	-104.9	-104.9	0.87 (1)	4.04	
G- H	-2400 / 0	-104.9	-104.9	0.17 (1)	5.63	
H- I	-1667 / 0	-104.9	-104.9	0.14 (1)	6.25	
I- J	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
R- B	-5201 / 0	0.0	0.0	0.19 (1)	6.40	
K- I	-2125 / 0	0.0	0.0	0.08 (1)	7.81	
R- Q	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
Q- P	0 / 3601	-28.0	-28.0	0.37 (1)	10.00	
P- O	0 / 2577	-28.0	-28.0	0.24 (1)	10.00	
O- N	0 / 1975	-28.0	-28.0	0.18 (1)	10.00	
N- M	0 / 1975	-28.0	-28.0	0.18 (1)	10.00	
M- L	0 / 1414	-28.0	-28.0	0.15 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	0 / 753	0.09 (1)
C- P	-1127 / 0	0.24 (1)
P- D	0 / 749	0.09 (1)
D- O	0 / 831	0.10 (1)
O- E	-1041 / 0	0.21 (1)
O- G	0 / 1535	0.19 (1)
M- G	0 / 233	0.03 (3)
M- H	0 / 607	0.08 (1)
L- H	-1021 / 0	0.08 (1)
L- I	0 / 1747	0.22 (1)
B- Q	0 / 4448	0.55 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	1-4-8	-3663	-3663	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL.	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL.	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.87/1.00 (E-G:1) , BC=0.37/1.00 (P-Q:1) , WB=0.55/1.00 (B-Q:1) , SSI=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

TPI GRIP= 0.86 (B) (INPUT = 0.90)
TPI METAL= 0.47 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11723-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T50	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3652.9 lbs FACTORED DOWN AT 1-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
3 2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11723 -18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T51	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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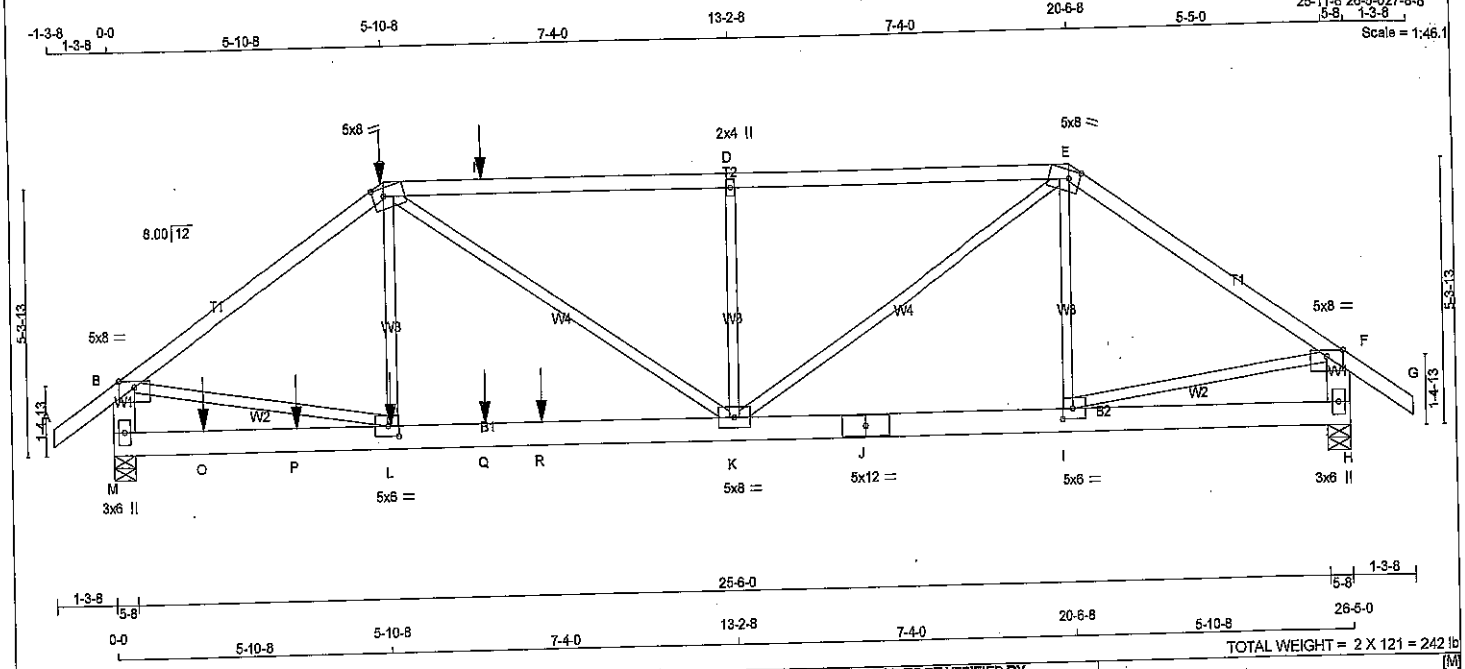

TOTAL WEIGHT = 2 X 121 = 242 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
J - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- E	1	12	SIDE(61.0)
E- G	1	12	TOP
M- B	2	12	TOP
H- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M- J	2	12	SIDE(183.1)
J- H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
L- C	1	6	SIDE(31.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	2.00	2.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	8.0	2.00	2.75
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BS-t	MT20	5.0	12.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	3754	0	3754	0	0	5-8	5-8
H	2711	0	2711	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2951	1878 / 0	538 / 0	0 / 0	0 / 0	534 / 0	0 / 0
H	2123	1366 / 0	376 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
B- C	-4847 / 0	-104.9	-104.9	0.59 (1)	3.89	
C- N	-4842 / 0	-104.9	-104.9	0.88 (1)	3.48	
N- D	-4842 / 0	-104.9	-104.9	0.88 (1)	3.48	
D- E	-4842 / 0	-104.9	-104.9	0.88 (1)	3.48	
E- F	-3105 / 0	-104.9	-104.9	0.46 (1)	4.77	
F- G	0 / 40	-104.9	-104.9	0.08 (1)	10.00	
M- B	-3819 / 0	0.0	0.0	0.14 (1)	7.23	
H- F	-2609 / 0	0.0	0.0	0.09 (1)	7.81	
M- O	0 / 0	-28.0	-28.0	0.31 (1)	10.00	
O- P	0 / 0	-28.0	-28.0	0.31 (1)	10.00	
P- L	0 / 0	-28.0	-28.0	0.31 (1)	10.00	
L- Q	0 / 4047	-28.0	-28.0	0.75 (1)	10.00	
Q- R	0 / 4047	-28.0	-28.0	0.75 (1)	10.00	
R- K	0 / 4047	-28.0	-28.0	0.75 (1)	10.00	
K- J	0 / 2572	-28.0	-28.0	0.33 (1)	10.00	
J- I	0 / 2572	-28.0	-28.0	0.33 (1)	10.00	
I- H	0 / 0	-28.0	-28.0	0.05 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	0 / 910	0.11 (2)
C- K	0 / 958	0.12 (1)
K- D	-983 / 0	0.20 (1)
K- E	0 / 2735	0.34 (1)
I- E	-403 / 119	0.08 (1)
B- L	0 / 4084	0.51 (1)
I- F	0 / 2616	0.32 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-590	-590	---	FRONT	VERT	TOTAL
L	5-11-4	-41	-72	---	FRONT	VERT	TOTAL
N	7-11-4	-126	-126	---	FRONT	VERT	TOTAL
O	1-11-4	-51	-95	---	FRONT	VERT	TOTAL
P	3-11-4	-54	-95	---	FRONT	VERT	TOTAL
Q	7-11-4	-41	-72	---	FRONT	VERT	TOTAL
R	9-1-8	-1762	-1762	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 942 (0.34")

CSI: TC=0.88/1.00 (C-D:1) , BC=0.75/1.00 (K-L:1) , WB=0.51/1.00 (B-L:1) , SSI=0.39/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.85 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11724-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T51	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 589.7 lbs FACTORED DOWN AT 5-10-8, AND 126.3 lbs FACTORED DOWN AT 7-11-4 ON TOP CHORD, AND 94.9 lbs FACTORED DOWN AT 1-11-4, 94.9 lbs FACTORED DOWN AT 3-11-4, 71.7 lbs FACTORED DOWN AT 5-11-4, AND 71.7 lbs FACTORED DOWN AT 7-11-4, AND 1762.1 lbs FACTORED DOWN AT 9-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
3-02-18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1/724-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T52	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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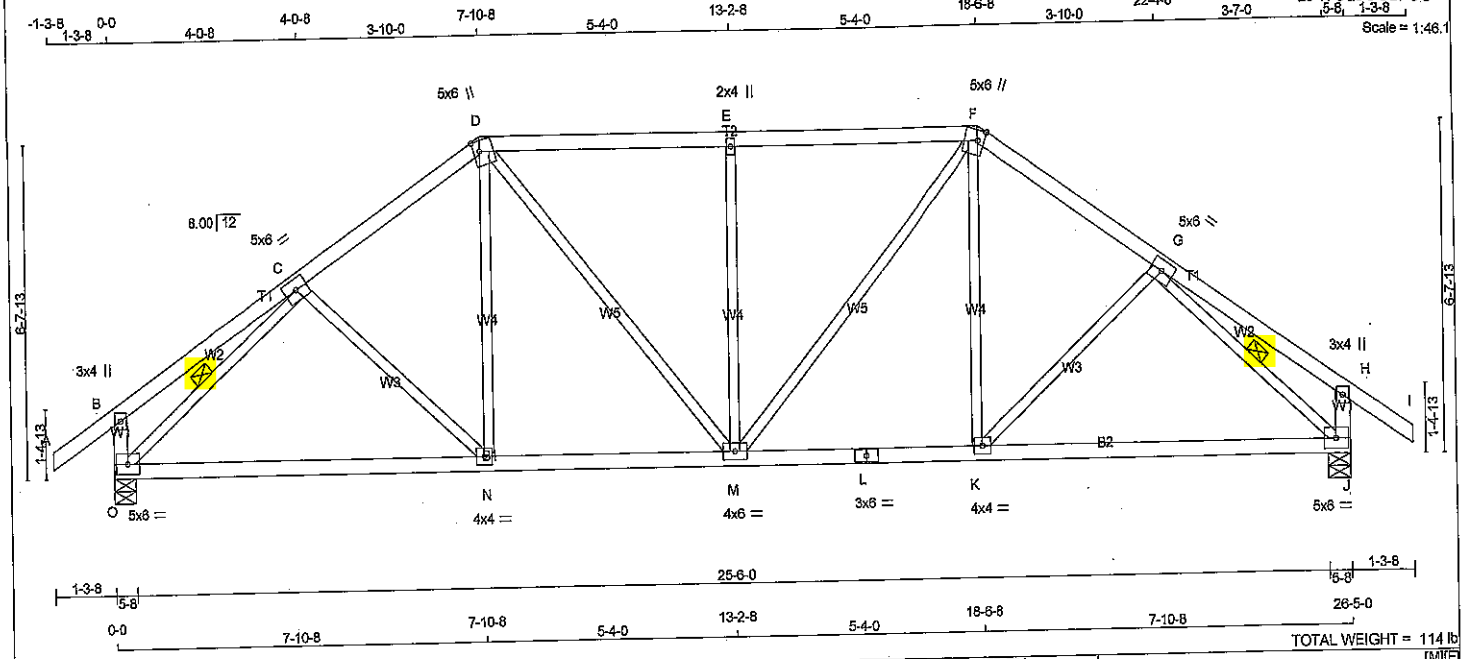

TOTAL WEIGHT = 114 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+m	MT20	5.0	6.0	2.50	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.50	1.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1900	0	1900	0	0	5-8	5-8
J	1900	0	1900	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 25	-104.9	-104.9	0.25 (1)	10.00
C- D	-1894 / 0	-104.9	-104.9	0.23 (1)	4.68
D- E	-1880 / 0	-104.9	-104.9	0.43 (1)	4.44
E- F	-1880 / 0	-104.9	-104.9	0.43 (1)	4.44
F- G	-1894 / 0	-104.9	-104.9	0.23 (1)	4.68
G- H	0 / 25	-104.9	-104.9	0.25 (1)	10.00
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- B	-305 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-305 / 0	0.0	0.0	0.03 (1)	7.81
O- N	0 / 1596	-28.0	-28.0	0.56 (2)	10.00
N- M	0 / 1557	-28.0	-28.0	0.57 (2)	10.00
M- L	0 / 1557	-28.0	-28.0	0.57 (2)	10.00
L- K	0 / 1557	-28.0	-28.0	0.57 (2)	10.00
K- J	0 / 1596	-28.0	-28.0	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	-60 / 114	0.03 (1)
N- D	0 / 362	0.08 (2)
D- M	0 / 503	0.11 (1)
M- E	-683 / 0	0.50 (1)
M- F	0 / 503	0.11 (1)
K- F	0 / 362	0.08 (2)
K- G	-60 / 114	0.03 (1)
O- C	-2193 / 0	0.49 (1)
G- J	-2193 / 0	0.49 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.43/1.00 (D-E:1) , BC=0.57/1.00 (M-N:2) , WB=0.50/1.00 (E-M:1) , SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.54 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
30216
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11725-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T53	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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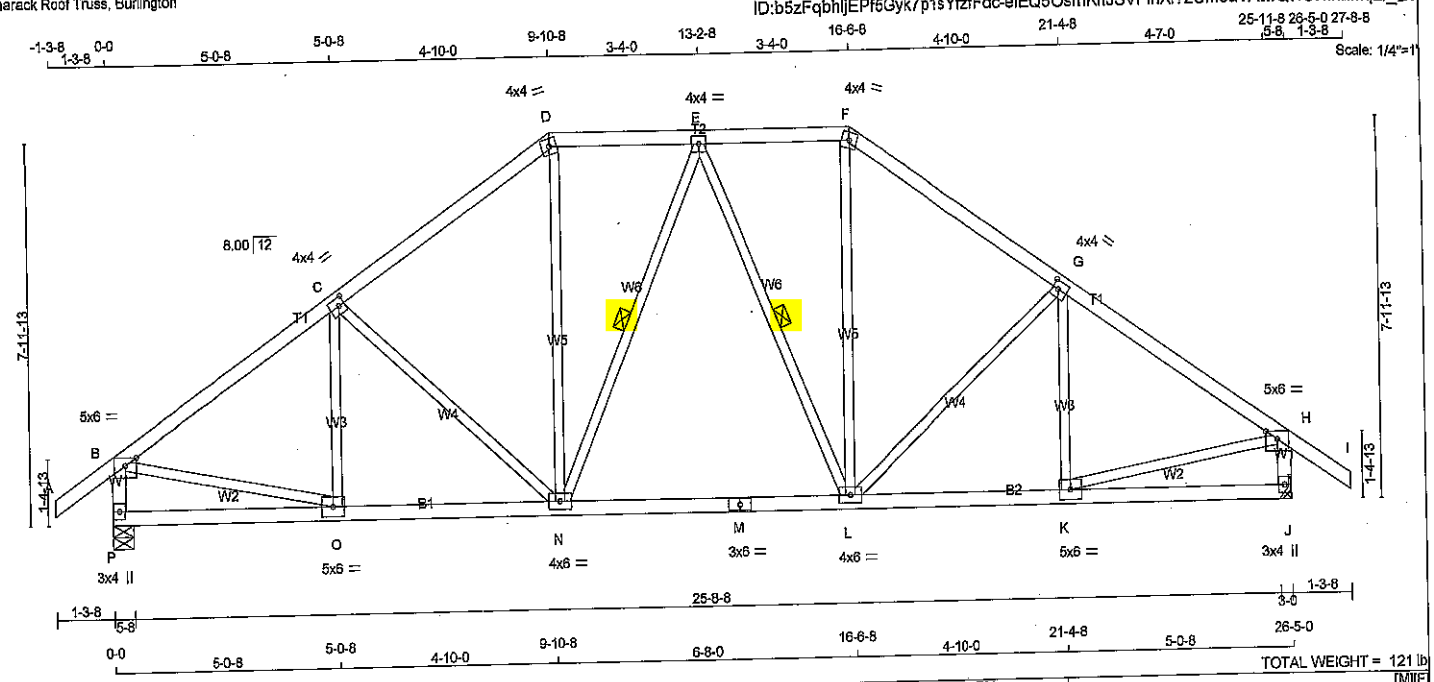

TOTAL WEIGHT = 121 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	1900	0	1900	0	0	5-8	5-8
J	1900	0	1900	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2000 / 0	-104.9	-104.9	0.39 (1)	4.41
C- D	-1744 / 0	-104.9	-104.9	0.37 (1)	4.67
D- E	-1428 / 0	-104.9	-104.9	0.15 (1)	5.32
E- F	-1428 / 0	-104.9	-104.9	0.15 (1)	5.32
F- G	-1744 / 0	-104.9	-104.9	0.37 (1)	4.67
G- H	-2000 / 0	-104.9	-104.9	0.39 (1)	4.41
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
P- B	-1839 / 0	0.0	0.0	0.19 (1)	6.18
J- H	-1839 / 0	0.0	0.0	0.19 (1)	6.18
P- O	0 / 0	-28.0	-28.0	0.15 (3)	10.00
O- N	0 / 1692	-28.0	-28.0	0.38 (1)	10.00
N- M	0 / 1515	-28.0	-28.0	0.35 (2)	10.00
M- L	0 / 1515	-28.0	-28.0	0.35 (2)	10.00
L- K	0 / 1692	-28.0	-28.0	0.38 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-220 / 106	0.07 (1)
C- N	-365 / 0	0.29 (1)
N- D	0 / 616	0.14 (1)
N- E	-223 / 0	0.11 (1)
E- L	-223 / 0	0.11 (1)
L- F	0 / 616	0.14 (1)
L- G	-365 / 0	0.29 (1)
K- G	-220 / 106	0.07 (1)
B- O	0 / 1730	0.39 (1)
K- H	0 / 1730	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.38/1.00 (N-O:1) , WB=0.39/1.00 (B-O:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.51 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11726-08
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T54	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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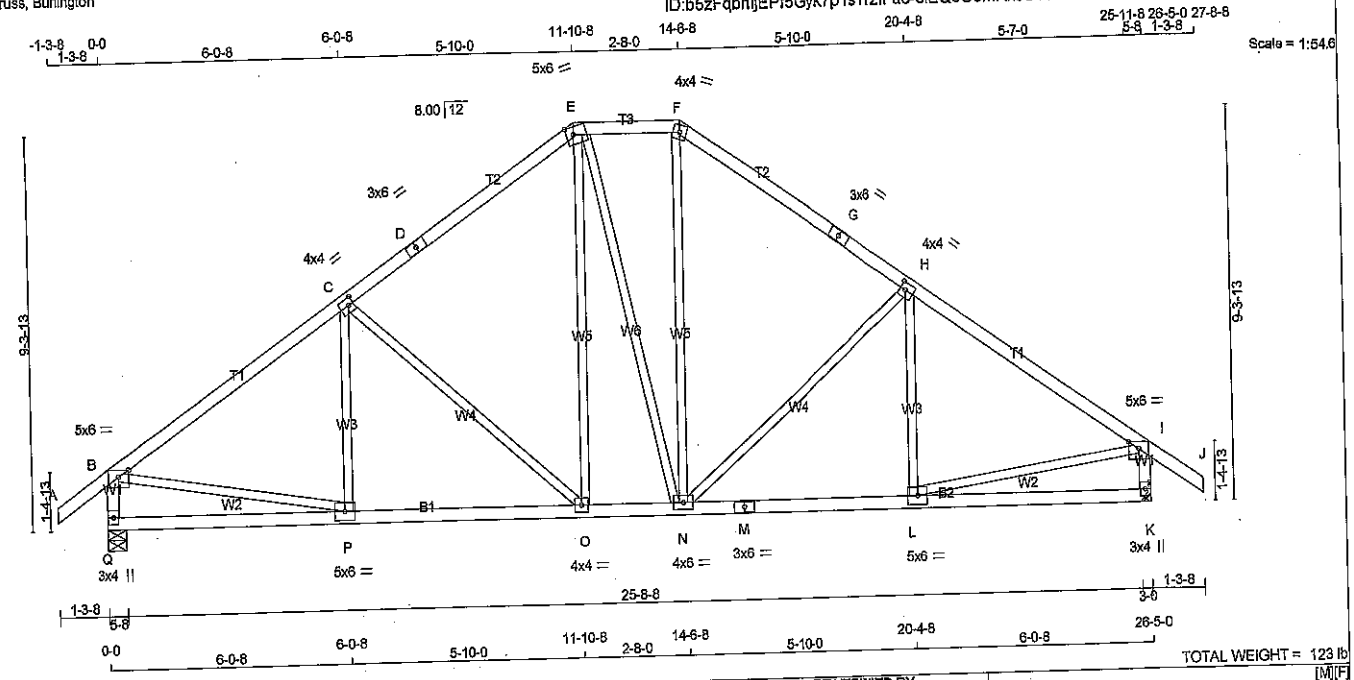

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.25	2.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1900	0	1900	0	0	IN-SX	IN-SX
K	1900	0	1900	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-0

UNFACTORED REACTIONS

JT	1ST LCASE. COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2013 / 0	-104.9	-104.9	0.58 (1)	4.14
C- D	-1574 / 0	-104.9	-104.9	0.53 (1)	4.63
D- E	-1574 / 0	-104.9	-104.9	0.53 (1)	4.63
E- F	-1280 / 0	-104.9	-104.9	0.11 (1)	5.59
F- G	-1576 / 0	-104.9	-104.9	0.53 (1)	4.62
G- H	-1576 / 0	-104.9	-104.9	0.53 (1)	4.62
H- I	-2012 / 0	-104.9	-104.9	0.58 (1)	4.14
I- J	0 / 40	-104.9	-104.9	0.14 (1)	10.00
Q- B	-1831 / 0	0.0	0.0	0.19 (1)	6.19
K- I	-1831 / 0	0.0	0.0	0.19 (1)	6.19
Q- P	0 / 0	-28.0	-28.0	0.26 (3)	10.00
P- O	0 / 1709	-28.0	-28.0	0.44 (2)	10.00
O- N	0 / 1279	-28.0	-28.0	0.28 (1)	10.00
N- M	0 / 1708	-28.0	-28.0	0.43 (2)	10.00
M- L	0 / 1708	-28.0	-28.0	0.43 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.26 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-112 / 215	0.05 (1)
C- O	-575 / 0	0.73 (1)
O- E	0 / 477	0.11 (1)
E- N	0 / 7	0.00 (1)
N- F	0 / 484	0.11 (1)
N- H	-572 / 0	0.72 (1)
L- H	-116 / 213	0.05 (1)
B- P	0 / 1736	0.39 (1)
L- I	0 / 1735	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.58/1.00 (B-C:1) , BC=0.44/1.00 (O-P:2) , WB=0.73/1.00 (C-O:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (B) (INPUT = 0.90)
SI METAL= 0.49 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3-02-18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11727 A
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T55	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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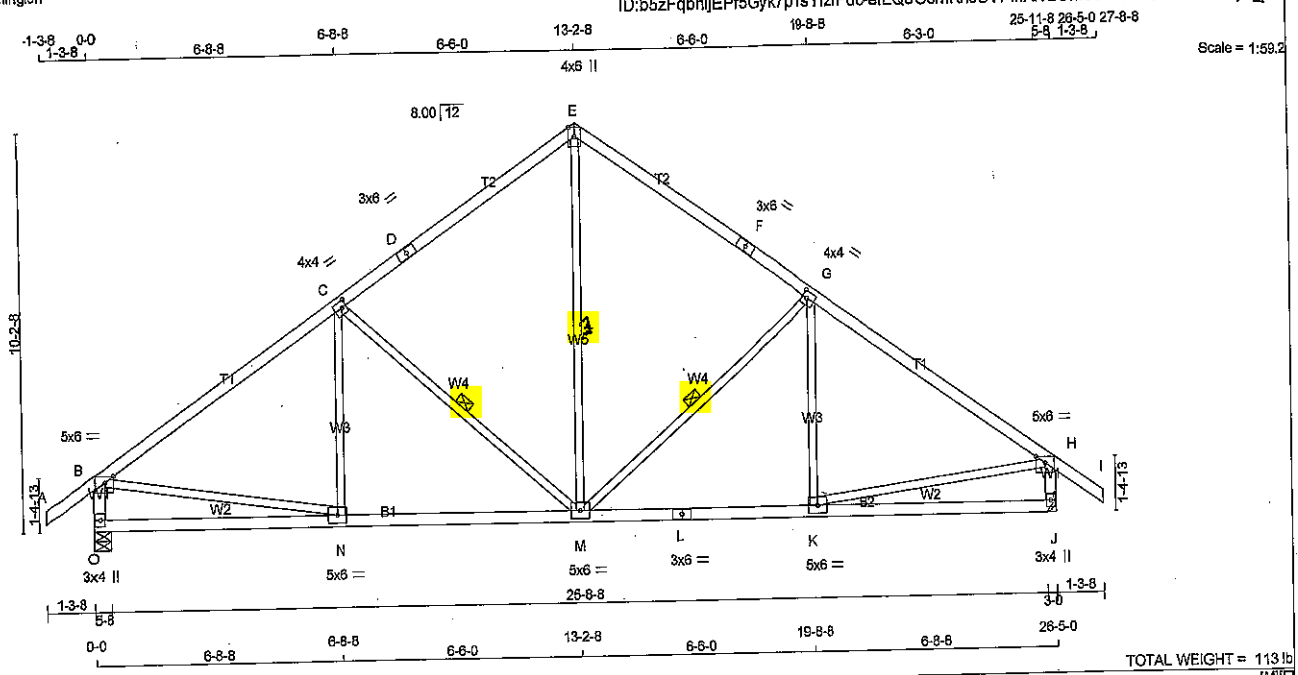

TOTAL WEIGHT = 113 lb
[MIF]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1900	0	1900	0	0	IN-SX 5-8	IN-SX 5-8
J	1900	0	1900	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0
J	1497	947 / 0	277 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-M, C-M. E-M

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-2001 / 0	-104.9	-104.9	0.73 (1)	3.89
C- D	-1468 / 0	-104.9	-104.9	0.65 (1)	4.52
D- E	-1468 / 0	-104.9	-104.9	0.65 (1)	4.52
E- F	-1468 / 0	-104.9	-104.9	0.65 (1)	4.52
F- G	-1468 / 0	-104.9	-104.9	0.65 (1)	4.52
G- H	-2001 / 0	-104.9	-104.9	0.73 (1)	3.89
H- I	0 / 40	-104.9	-104.9	0.14 (1)	10.00
O- B	-1823 / 0	0.0	0.0	0.19 (1)	6.20
J- H	-1823 / 0	0.0	0.0	0.19 (1)	6.20
O- N	0 / 0	-28.0	-28.0	0.30 (3)	10.00
N- M	0 / 1703	-28.0	-28.0	0.48 (2)	10.00
M- L	0 / 1703	-28.0	-28.0	0.48 (2)	10.00
L- K	0 / 1703	-28.0	-28.0	0.48 (2)	10.00
K- J	0 / 0	-28.0	-28.0	0.30 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	0 / 1065	0.24 (1)
M- G	-686 / 0	0.34 (1)
K- G	-68 / 253	0.06 (3)
C- M	-686 / 0	0.34 (1)
N- C	-68 / 253	0.06 (3)
B- N	0 / 1725	0.39 (1)
K- H	0 / 1725	0.39 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.88")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.88")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.48/1.00 (M-N:2) , WB=0.39/1.00 (B-N:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.53 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11728 -38
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T56	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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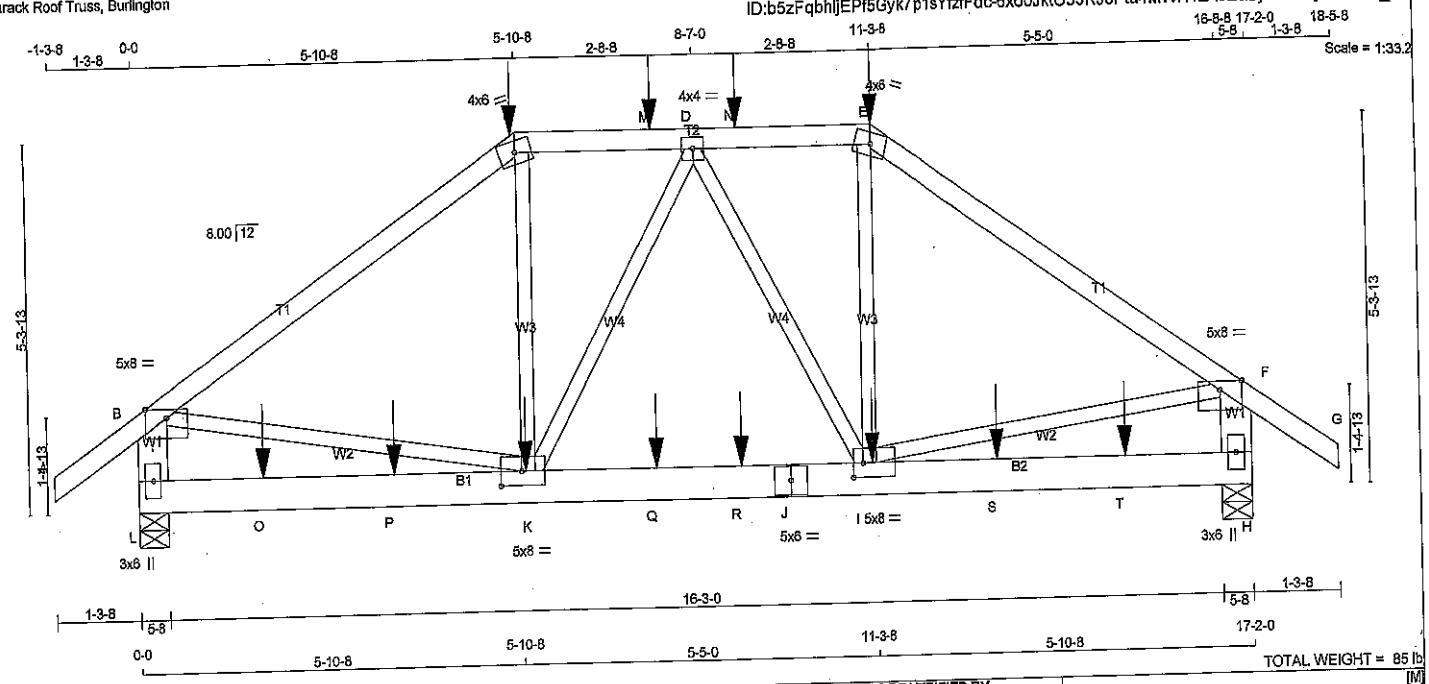

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTW-m	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	6.0		
F	TMVW-p	MT20	5.0	8.0	Edge	
H	BMV1+p	MT20	3.0	6.0		
I	BMWWW-t	MT20	5.0	8.0	2.50	1.75
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	3.75
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 689.7 lbs FACTORED DOWN AT 5-10-8, 179.4 lbs FACTORED DOWN AT 7-11-4, AND 179.4 lbs FACTORED DOWN AT 9-2-12, AND 642.8 lbs FACTORED DOWN AT 11-3-8 ON TOP CHORD, AND 94.9 lbs FACTORED DOWN AT 1-11-4, 105.5 lbs FACTORED DOWN AT 3-11-4, 144.6 lbs FACTORED DOWN AT 5-11-4, 144.6 lbs FACTORED DOWN AT 7-11-4, 144.6 lbs FACTORED DOWN AT 9-2-12, 144.6 lbs FACTORED DOWN AT 11-2-12, AND 94.9 lbs FACTORED DOWN AT 13-2-12, AND 94.9 lbs FACTORED DOWN AT 15-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2552	0	2552	0	0	5-8	5-8
H	2523	0	2523	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2016	1265 / 0	379 / 0	0 / 0	0 / 0	372 / 0	0 / 0
H	1991	1253 / 0	372 / 0	0 / 0	0 / 0	366 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.13 (1)	10.00	
B- C	-2858 / 0	-104.9	-104.9	0.81 (1)	3.72	
C- M	-2389 / 0	-104.9	-104.9	0.22 (1)	4.75	
M- D	-2389 / 0	-104.9	-104.9	0.22 (1)	4.75	
D- N	-2364 / 0	-104.9	-104.9	0.22 (1)	4.77	
N- E	-2364 / 0	-104.9	-104.9	0.22 (1)	4.77	
E- F	-2827 / 0	-104.9	-104.9	0.81 (1)	3.75	
F- G	0 / 40	-104.9	-104.9	0.13 (1)	10.00	
L- B	-2434 / 0	0.0	0.0	0.17 (1)	6.58	
H- F	-2412 / 0	0.0	0.0	0.17 (1)	6.60	
L- O	0 / 0	-28.0	-28.0	0.22 (2)	10.00	
O- P	0 / 0	-28.0	-28.0	0.22 (2)	10.00	
P- K	0 / 0	-28.0	-28.0	0.22 (2)	10.00	
K- Q	0 / 2542	-28.0	-28.0	0.47 (1)	10.00	
Q- R	0 / 2542	-28.0	-28.0	0.47 (1)	10.00	
R- J	0 / 2542	-28.0	-28.0	0.47 (1)	10.00	
J- I	0 / 2542	-28.0	-28.0	0.47 (1)	10.00	
I- S	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
S- T	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
T- H	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 548	0.14 (2)
K- D	-332 / 0	0.18 (1)
D- I	-387 / 0	0.20 (1)
I- E	0 / 568	0.15 (2)
B- K	0 / 2407	0.60 (1)
I- F	0 / 2381	0.59 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
C	5-10-8	-690	-690	---	BACK	VERT	TOTAL
E	11-3-8	-643	-643	---	BACK	VERT	TOTAL
I	11-2-12	-145	-145	---	BACK	VERT	TOTAL
K	5-11-4	-145	-145	---	BACK	VERT	TOTAL
M	7-11-4	-179	-179	---	BACK	VERT	TOTAL
N	9-2-12	-179	-179	---	BACK	VERT	TOTAL
O	1-11-4	-55	-95	---	BACK	VERT	TOTAL
P	3-11-4	-74	-105	---	BACK	VERT	TOTAL
Q	7-11-4	-145	-145	---	BACK	VERT	TOTAL
R	9-2-12	-145	-145	---	BACK	VERT	TOTAL
S	13-2-12	-54	-95	---	BACK	VERT	TOTAL
T	15-2-12	-54	-95	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.81/1.00 (B-C:1) , BC=0.47/1.00 (I-K:1) , WB=0.60/1.00 (B-K:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LSI GRIP= 0.86 (I) (INPUT = 0.90)
LSI METAL= 0.48 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
A. KAATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11729-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T57	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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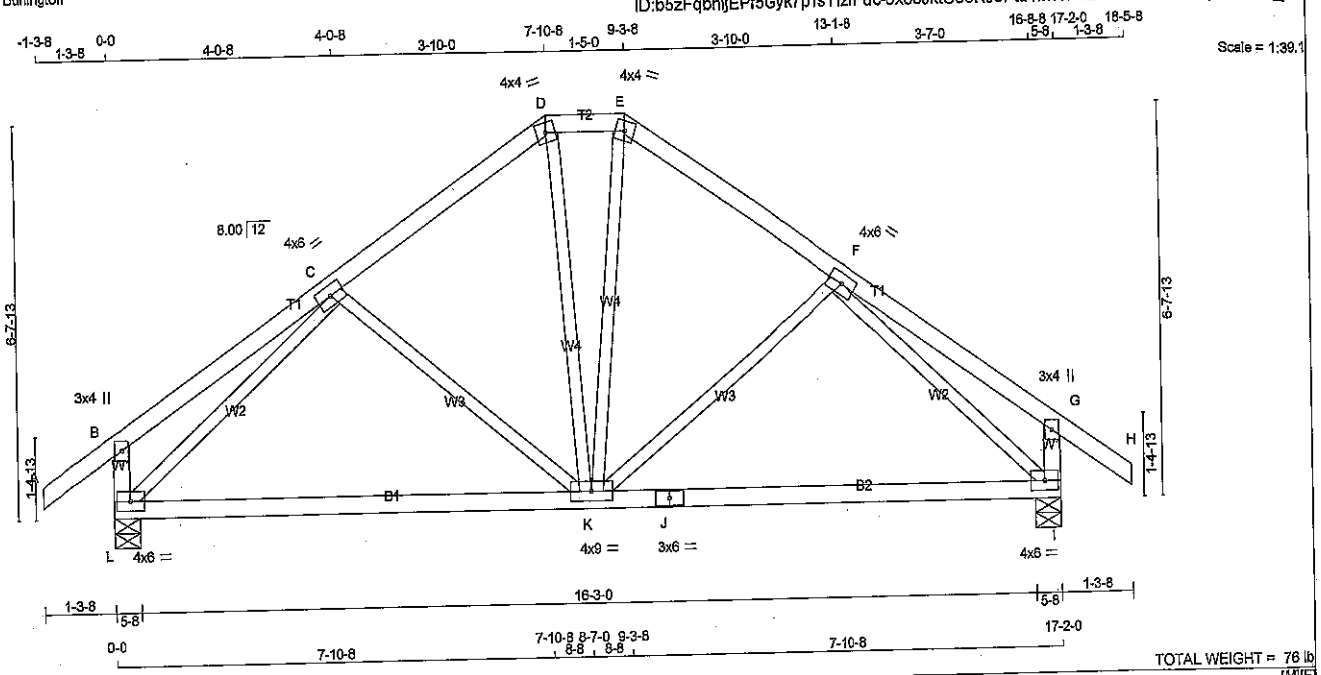

TOTAL WEIGHT = 76 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWWW*-I	MT20	4.0	9.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1285	0	1285	0	0	5-8	5-8
I	1285	0	1285	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1007	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
I	1007	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 26	-104.9	-104.9	0.26 (1)	10.00
C- D	-959 / 0	-104.9	-104.9	0.21 (1)	6.10
D- E	-807 / 0	-104.9	-104.9	0.03 (1)	6.25
E- F	-959 / 0	-104.9	-104.9	0.21 (1)	6.10
F- G	0 / 26	-104.9	-104.9	0.26 (1)	10.00
G- H	0 / 40	-104.9	-104.9	0.14 (1)	10.00
L- B	-303 / 0	0.0	0.0	0.03 (1)	7.81
I- G	-303 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 946	-28.0	-28.0	0.70 (2)	10.00
K- J	0 / 946	-28.0	-28.0	0.70 (2)	10.00
J- I	0 / 946	-28.0	-28.0	0.70 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-219 / 49	0.13 (1)
K- F	-219 / 49	0.13 (1)
L- C	-1300 / 0	0.62 (1)
F- I	-1300 / 0	0.62 (1)
K- E	0 / 287	0.06 (1)
D- K	0 / 287	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 789 (0.26")

CSI: TC=0.26/1.00 (B-C:1) , BC=0.70/1.00 (I-K:2) , WB=0.62/1.00 (C-L:1) , SSI=0.20/1.00 (K-L:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (J) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11730 -18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T58	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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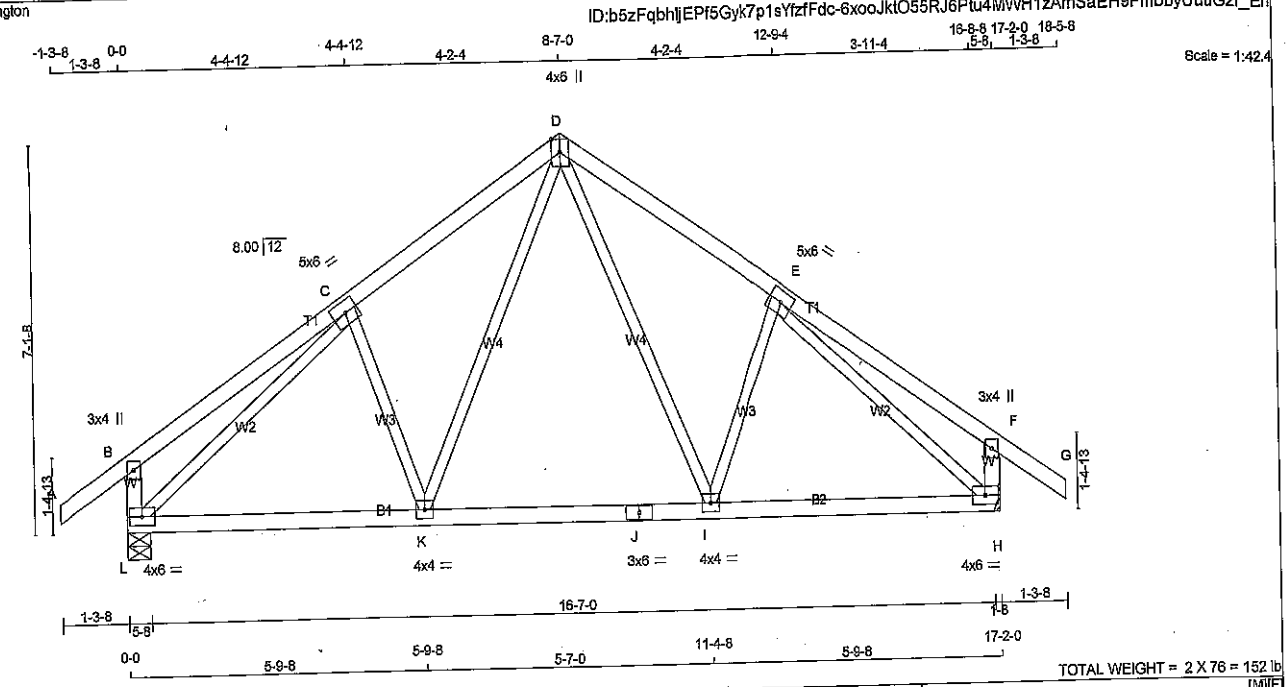

TOTAL WEIGHT = 2 X 76 = 152 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1285	0	1285	0	0	IN-SX 5-8	IN-SX 5-8
H	1285	0	1285	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1007	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0
H	1007	647 / 0	180 / 0	0 / 0	0 / 0	180 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 29	-104.9	-104.9	0.31 (1)	10.00
C- D	-1094 / 0	-104.9	-104.9	0.25 (1)	5.76
D- E	-1094 / 0	-104.9	-104.9	0.25 (1)	5.76
E- F	0 / 29	-104.9	-104.9	0.31 (1)	10.00
F- G	0 / 40	-104.9	-104.9	0.14 (1)	10.00
L- B	-316 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-316 / 0	0.0	0.0	0.03 (1)	7.81
L- K	0 / 981	-28.0	-28.0	0.31 (2)	10.00
K- J	0 / 716	-28.0	-28.0	0.28 (2)	10.00
J- I	0 / 716	-28.0	-28.0	0.28 (2)	10.00
I- H	0 / 981	-28.0	-28.0	0.31 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 450	0.10 (1)
I- E	-261 / 52	0.08 (1)
K- D	0 / 450	0.10 (1)
C- K	-261 / 52	0.08 (1)
L- C	-1333 / 0	0.75 (1)
E- H	-1333 / 0	0.75 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.31/1.00 (B-C:1) , BC=0.31/1.00 (K-L:2) , WB=0.75/1.00 (C-L:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (L) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11731-18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287571	T59	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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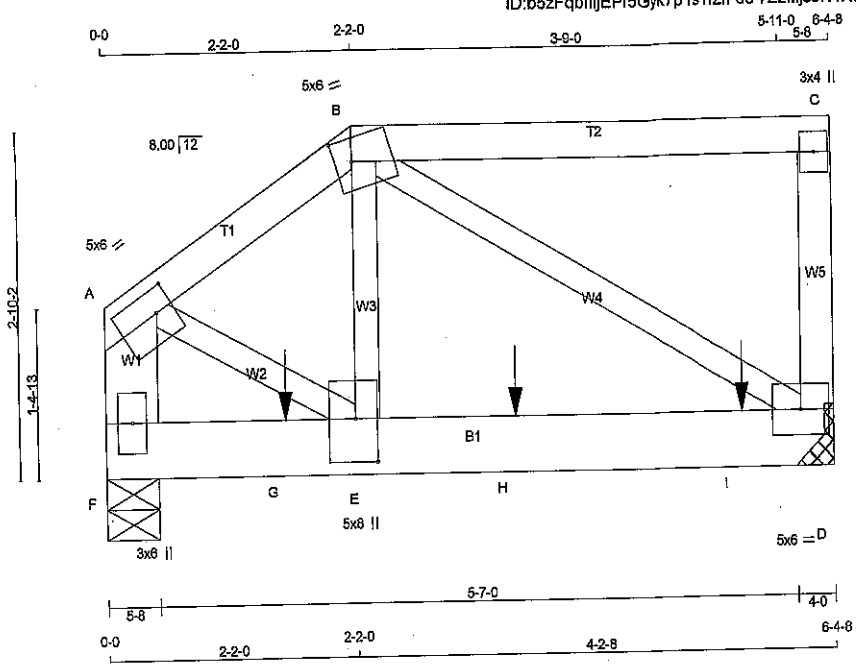

TOTAL WEIGHT = 2 X 30 = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0	2.25	1.75
B	TTWW-m	MT20	5.0	6.0	2.25	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	5.0	8.0	4.25	2.25
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1866.2 lbs FACTORED DOWN AT 1-6-12, AND 1866.2 lbs FACTORED DOWN AT 3-6-12, AND 1866.9 lbs FACTORED DOWN AT 5-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	3555	0	3555	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
F	2894	0	2894	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2813	1759 / 0	535 / 0	0 / 0	0 / 0	519 / 0	0 / 0
F	2290	1432 / 0	435 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2989 / 0	-104.9	-104.9	0.08 (1)	5.28	
B- C	0 / 0	-104.9	-104.9	0.17 (1)	10.00	
D- C	-221 / 0	0.0	0.0	0.02 (1)	7.81	
F- A	-2871 / 0	0.0	0.0	0.10 (1)	7.81	
F- G	0 / 0	-28.0	-28.0	0.49 (1)	10.00	
G- E	0 / 0	-28.0	-28.0	0.49 (1)	10.00	
E- H	0 / 2600	-28.0	-28.0	0.71 (1)	10.00	
H- I	0 / 2600	-28.0	-28.0	0.71 (1)	10.00	
I- D	0 / 2600	-28.0	-28.0	0.71 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2855	0.35 (1)
B- D	-3010 / 0	0.54 (1)
A- E	0 / 2707	0.34 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-6-12	-1866	-1866	---	BACK	VERT	TOTAL
H	3-6-12	-1866	-1866	---	BACK	VERT	TOTAL
I	5-6-12	-1869	-1869	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 980 (0.08")

CSI: TC=0.17/1.00 (B-C:1) , BC=0.71/1.00 (D-E:1) , WB=0.54/1.00 (B-D:1) , SSI=0.67/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

MSI GRIP= 0.88 (B) (INPUT = 0.90)
MSI METAL= 0.52 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11242-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T59S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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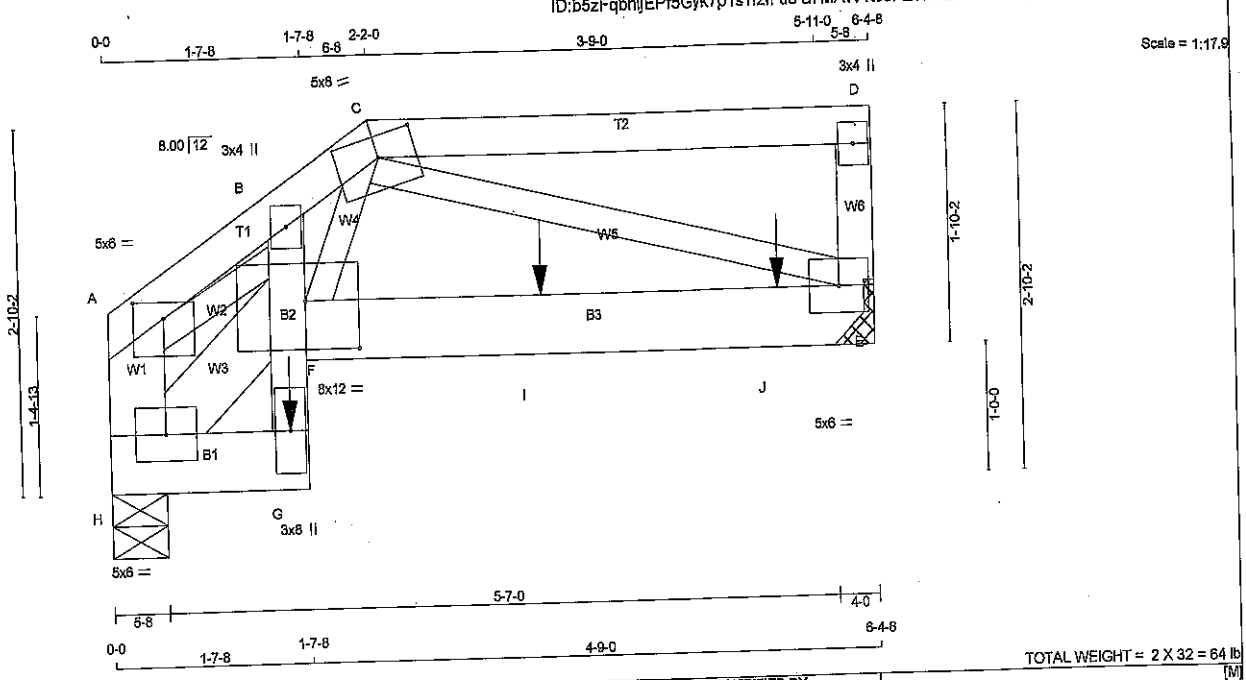

TOTAL WEIGHT = 2 X 32 = 64 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
F - E	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- E	1	12	TOP
H- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- G	2	12	SIDE(122.0)
F- E	2	12	SIDE(0.0)
G- B	2	3	SIDE(368.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TMV+p	MT20	3.0	4.0		
C	TTWW-m	MT20	5.0	8.0	2.00	3.75
D	TMV+p	MT20	3.0	4.0		
E	BMVW1-t	MT20	5.0	6.0		
F	BVMWWW-l	MT20	8.0	12.0	4.50	5.25
G	BMV+p	MT20	3.0	8.0		
H	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1879.1 lbs FACTORED DOWN AT 1-5-12, AND 1866.2 lbs FACTORED DOWN AT 3-6-12, AND 1915.3 lbs FACTORED DOWN AT 5-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	3574	0	3574	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
H	2934	0	2934	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	2846	1747 / 0	564 / 0	0 / 0	0 / 0	537 / 0	0 / 0
H	2329	1444 / 0	451 / 0	0 / 0	0 / 0	434 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-4633 / 0	-104.9	-104.9	0.28 (1)	4.17	4.17
B- C	-4612 / 0	-104.9	-104.9	0.28 (1)	4.17	4.17
C- D	0 / 0	-104.9	-104.9	0.17 (1)	10.00	10.00
E- D	-218 / 0	0.0	0.0	0.01 (1)	7.81	7.81
H- A	-2777 / 0	0.0	0.0	0.10 (1)	7.81	
H- G	0 / 201	-28.0	-28.0	0.02 (1)	10.00	
G- F	0 / 1900	0.0	0.0	0.32 (1)	10.00	
F- B	0 / 304	0.0	0.0	0.18 (1)	10.00	
F- I	0 / 2535	-28.0	-28.0	0.86 (1)	10.00	
I- J	0 / 2535	-28.0	-28.0	0.86 (1)	10.00	
J- E	0 / 2535	-28.0	-28.0	0.86 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	-243 / 0	0.01 (1)
A- F	0 / 3760	0.47 (1)
F- C	0 / 3291	0.41 (1)
C- E	-2688 / 0	0.44 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-5-12	-1879	-1879	—	BACK	VERT	TOTAL
I	3-6-12	-1866	-1866	—	BACK	VERT	TOTAL
J	5-6-12	-1915	-1915	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 637 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 408 (0.19")

CSI: TC=0.28/1.00 (A-B:1) , BC=0.86/1.00 (E-F:1) , WB=0.47/1.00 (A-F:1) , SSI=0.82/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.48 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11732-N
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T60	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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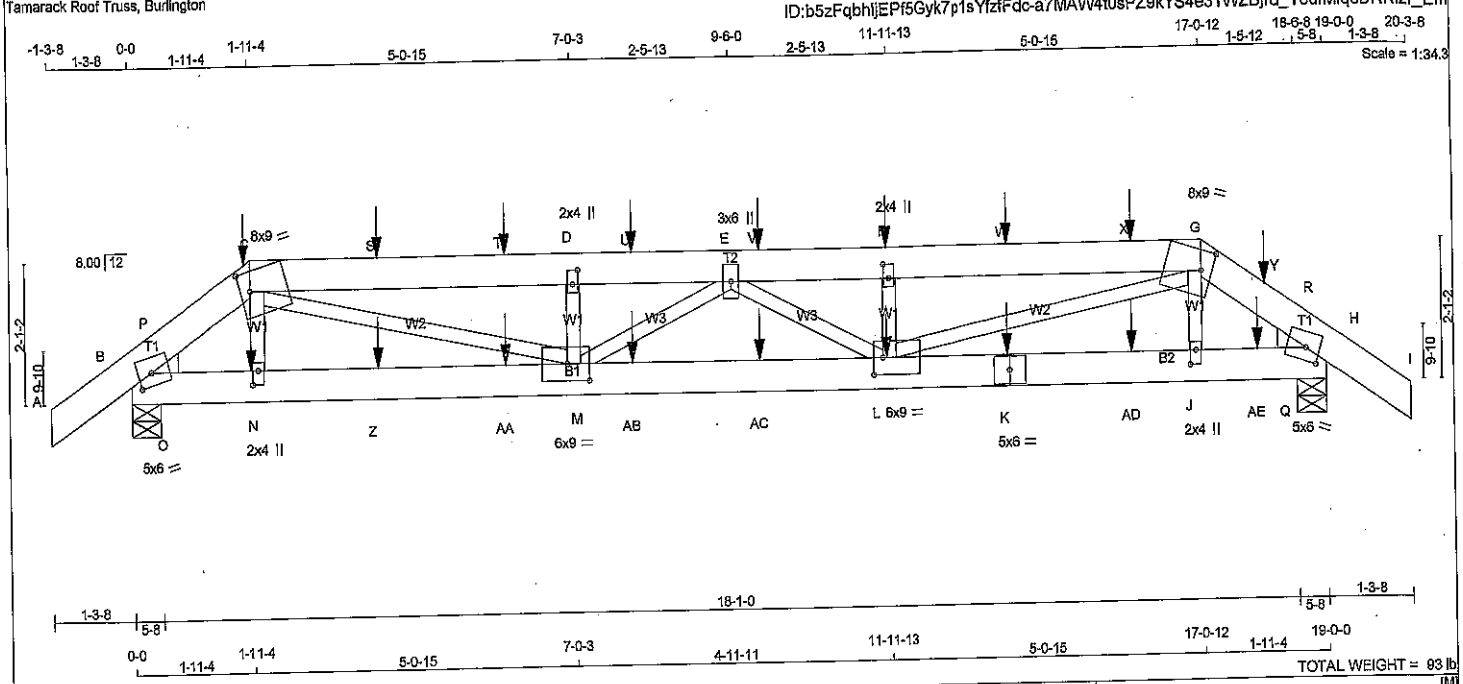

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
B - K	2x6	DRY	1650F 1.5E	SPF
K - H	2x6	DRY	1650F 1.5E	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	5.0	6.0	2.25	2.50
C	TTWW-m	MT20	8.0	9.0	3.50	2.00
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWW+t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TTWW-m	MT20	8.0	9.0	3.50	2.00
H	TMBH1-m	MT20	5.0	6.0	2.25	2.50
J	BMW+w	MT20	2.0	4.0	2.50	1.00
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	6.0	9.0	3.00	1.75
M	BMWWW-t	MT20	6.0	9.0	3.00	4.25
N	BMW+w	MT20	2.0	4.0	2.50	1.00

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 139.6 lbs FACTORED DOWN AT 1-11-4, 139.6 lbs FACTORED DOWN AT 3-11-4, 139.6 lbs FACTORED DOWN AT 5-11-4, 139.6 lbs FACTORED DOWN AT 7-11-4, 139.6 lbs FACTORED DOWN AT 9-11-4, 139.6 lbs FACTORED DOWN AT 11-11-4, 139.6 lbs FACTORED DOWN AT 13-11-4, AND 139.6 lbs FACTORED DOWN AT 15-11-4, AND 146.2 lbs FACTORED DOWN AT 17-11-4 ON TOP CHORD, AND 79.2 lbs FACTORED DOWN AT 1-11-4, 79.2 lbs FACTORED DOWN AT 3-11-4, 79.2 lbs FACTORED DOWN AT 5-11-4, 79.2 lbs FACTORED DOWN AT 7-11-4, 79.2 lbs FACTORED DOWN AT 9-11-4, 79.2 lbs FACTORED DOWN AT 11-11-4, 79.2 lbs FACTORED DOWN AT 13-11-4, AND 79.2 lbs FACTORED DOWN AT 15-11-4, AND 79.2 lbs FACTORED DOWN AT 17-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	2351	0	2351	0	0	5-8	5-8	2x4 L
H	2448	0	2448	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1855	1168 / 0	348 / 0	0 / 0	0 / 0	340 / 0	0 / 0
H	1933	1217 / 0	362 / 0	0 / 0	0 / 0	354 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 31	-104.9	-104.9	0.08 (1)	10.00	
B- P	-3695 / 0	-104.9	-104.9	0.32 (1)	4.10	
P- C	-3277 / 0	-104.9	-104.9	0.31 (1)	4.34	
C- S	-6138 / 0	-104.9	-104.9	0.66 (1)	2.87	
S- T	-6138 / 0	-104.9	-104.9	0.66 (1)	2.87	
T- D	-6138 / 0	-104.9	-104.9	0.66 (1)	2.87	
D- U	-6089 / 0	-104.9	-104.9	0.33 (1)	3.30	
U- E	-6089 / 0	-104.9	-104.9	0.33 (1)	3.30	
E- V	-6100 / 0	-104.9	-104.9	0.33 (1)	3.30	
V- F	-6100 / 0	-104.9	-104.9	0.33 (1)	3.30	
F- W	-6150 / 0	-104.9	-104.9	0.67 (1)	2.86	
W- X	-6150 / 0	-104.9	-104.9	0.67 (1)	2.86	
X- G	-6150 / 0	-104.9	-104.9	0.67 (1)	2.86	
G- Y	-3292 / 0	-104.9	-104.9	0.30 (1)	4.35	
Y- R	-3292 / 0	-104.9	-104.9	0.30 (1)	4.35	
R- H	-3724 / 0	-104.9	-104.9	0.32 (1)	4.10	
H- I	0 / 31	-104.9	-104.9	0.08 (1)	10.00	
B- O	0 / 2587	-28.0	-28.0	0.35 (1)	10.00	
O- N	0 / 2587	-28.0	-28.0	0.35 (1)	10.00	
N- Z	0 / 2608	-28.0	-28.0	0.34 (1)	10.00	
Z-AA	0 / 2608	-28.0	-28.0	0.34 (1)	10.00	
AA- M	0 / 2608	-28.0	-28.0	0.34 (1)	10.00	
M-AB	0 / 6342	-28.0	-28.0	0.72 (1)	10.00	
AB-AC	0 / 6342	-28.0	-28.0	0.72 (1)	10.00	
AC- L	0 / 6342	-28.0	-28.0	0.72 (1)	10.00	
L- K	0 / 2617	-28.0	-28.0	0.36 (1)	10.00	
K-AD	0 / 2617	-28.0	-28.0	0.36 (1)	10.00	
AD- J	0 / 2617	-28.0	-28.0	0.36 (1)	10.00	
J-AE	0 / 2595	-28.0	-28.0	0.36 (1)	10.00	
AE- Q	0 / 2595	-28.0	-28.0	0.36 (1)	10.00	
Q- H	0 / 2595	-28.0	-28.0	0.33 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	0 / 389	0.10 (2)
C- M	0 / 3698	0.91 (1)
M- D	-679 / 0	0.11 (1)
L- F	-694 / 0	0.11 (1)
L- G	0 / 3699	0.92 (1)
J- G	0 / 403	0.10 (2)
M- E	-307 / 0	0.06 (1)
E- L	-294 / 0	0.06 (1)
O- P	0 / 528	0.00 (1)
Q- R	0 / 470	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-11-4	-140	-140	—	BACK	VERT	TOTAL
F	1-11-4	-140	-140	—	BACK	VERT	TOTAL
K	13-11-4	-79	-79	—	BACK	VERT	TOTAL
L	11-11-4	-79	-79	—	BACK	VERT	TOTAL
N	1-11-4	-79	-79	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED. ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 761 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 491 (0.46")

CSI: TC=0.67/1.00 (F-G:1) , BC=0.72/1.00 (L-M:1) , WB=0.92/1.00 (G-L:1) , SSI=0.33/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

TSI GRIP= 0.89 (H) (INPUT = 0.90)
TSI METAL= 0.75 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11733-B
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T60	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	3-11-4	-140	-140	—	BACK	VERT	TOTAL
T	5-11-4	-140	-140	—	BACK	VERT	TOTAL
U	7-11-4	-140	-140	—	BACK	VERT	TOTAL
V	9-11-4	-140	-140	—	BACK	VERT	TOTAL
W	13-11-4	-140	-140	—	BACK	VERT	TOTAL
X	15-11-4	-140	-140	—	BACK	VERT	TOTAL
Y	17-11-4	-146	-146	—	BACK	VERT	TOTAL
Z	3-11-4	-79	-79	—	BACK	VERT	TOTAL
AA	5-11-4	-79	-79	—	BACK	VERT	TOTAL
AB	7-11-4	-79	-79	—	BACK	VERT	TOTAL
AC	9-11-4	-79	-79	—	BACK	VERT	TOTAL
AD	15-11-4	-79	-79	—	BACK	VERT	TOTAL
AE	17-11-4	-79	-79	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3.9.18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11733-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	G100	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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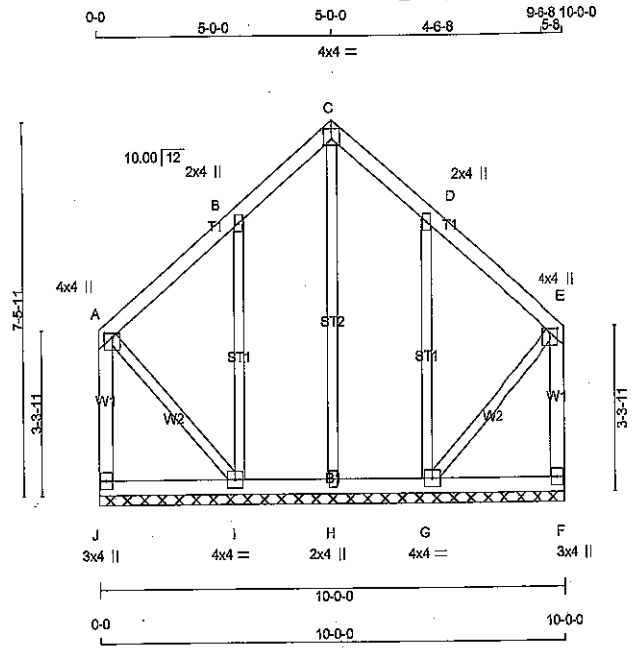

TOTAL WEIGHT = 54 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-146 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 2	-104.9	-104.9	0.12 (1)	10.00	
B- C	-35 / 0	-104.9	-104.9	0.11 (1)	6.25	
C- D	-35 / 0	-104.9	-104.9	0.11 (1)	6.25	
D- E	0 / 2	-104.9	-104.9	0.12 (1)	10.00	
F- E	-146 / 0	0.0	0.0	0.03 (1)	7.81	
J- I	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
I- H	0 / 7	-28.0	-28.0	0.06 (2)	10.00	
H- G	0 / 7	-28.0	-28.0	0.06 (2)	10.00	
G- F	0 / 0	-28.0	-28.0	0.06 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-128 / 0	0.12 (1)
I- B	-327 / 0	0.17 (1)
G- D	-327 / 0	0.17 (1)
A- I	0 / 18	0.00 (1)
G- E	0 / 18	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (D-E:1) , BC=0.06/1.00 (G-H:2) , WB=0.17/1.00 (D-G:1) , SSI=0.11/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

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DWG NO. TAM 11676-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	T101	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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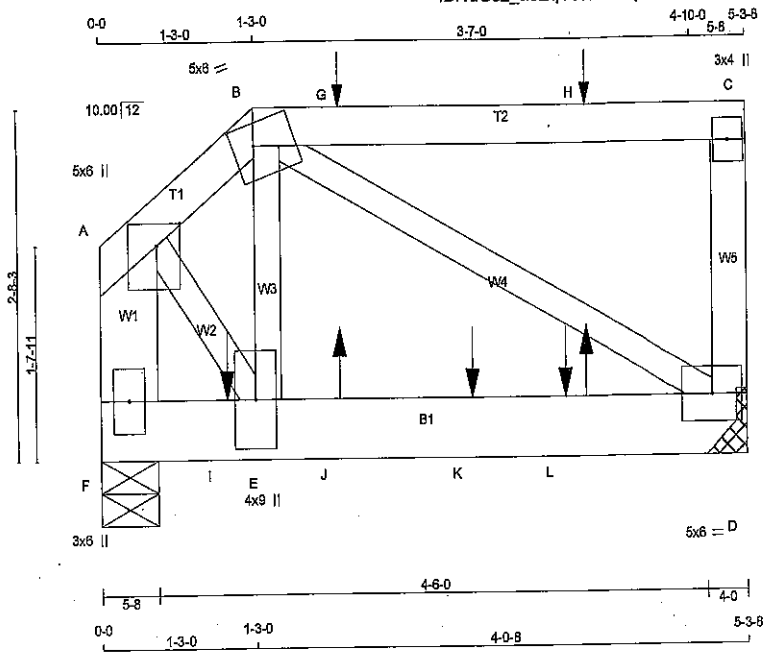

TOTAL WEIGHT = 2 X 27 = 53 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x6	DRY	No.2	SPF
F - D	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	SIDE(0.0)
C- D	1	12	TOP
F- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
E- B	1	2	SIDE(305.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TTWW-m	MT20	5.0	6.0	2.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.0 lbs FACTORED DOWN AND 138.5 lbs FACTORED UP AT 1-11-4, AND 1.0 lbs FACTORED DOWN AND 153.7 lbs FACTORED UP AT 3-11-4 ON TOP CHORD, AND 1873.1 lbs FACTORED DOWN AT 1-0-4, 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 1-11-4, 1873.1 lbs FACTORED DOWN AT 3-0-4, AND 1873.1 lbs FACTORED DOWN AT 3-9-4, AND 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 3-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	3101	0	3101	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
F	3192	0	3192	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2430	1581 / 0	435 / 0	0 / 0	0 / 0	435 / 0	0 / 0
F	2500	1607 / 0	447 / 0	0 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2894 / 0	-104.9	-104.9	0.04 (1)	5.39	
B- G	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
G- H	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
H- C	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
D- C	-212 / 0	0.0	0.0	0.01 (1)	7.81	
F- A	-3929 / 0	0.0	0.0	0.14 (1)	7.16	
F- I	0 / 0	-28.0	-28.0	0.42 (1)	10.00	
I- E	0 / 0	-28.0	-28.0	0.42 (1)	10.00	
E- J	0 / 2337	-28.0	-28.0	0.69 (1)	10.00	
J- K	0 / 2337	-28.0	-28.0	0.69 (1)	10.00	
K- L	0 / 2337	-28.0	-28.0	0.69 (1)	10.00	
L- D	0 / 2337	-28.0	-28.0	0.69 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2921	0.36 (1)
B- D	-2684 / 0	0.44 (1)
A- E	0 / 2968	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-11-4	-0	-1	138	BACK	VERT	TOTAL
H	3-11-4	-0	-1	154	BACK	VERT	TOTAL
I	1-0-4	-1873	-1873	---	FRONT	VERT	TOTAL
J	1-11-4	15	1	18	BACK	VERT	TOTAL
K	3-0-4	-1873	-1873	---	FRONT	VERT	TOTAL
L	3-9-4	-1873	-1873	---	FRONT	VERT	TOTAL
L	3-11-4	15	1	18	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= . L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 767 (0.08")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.69/1.00 (D-E:1) , WB=0.44/1.00 (B-D:1) , SSI=0.86/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.58 (E) (INPUT = 1.00)

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DWG NO. TAM 11677 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T101Z	1	2	TRUSS DESC.	

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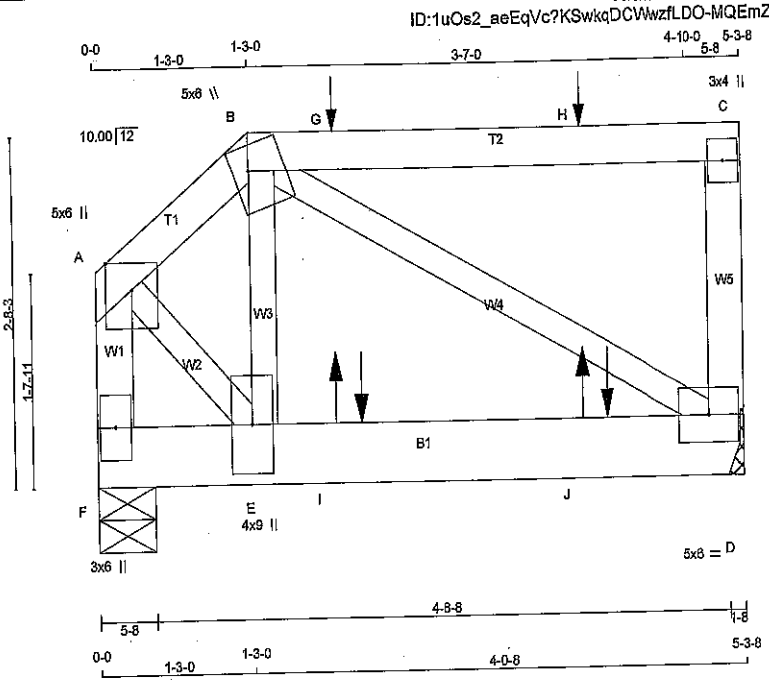

TOTAL WEIGHT = 2 X 26 = 51 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	SIDE(0.0)
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0		
E	BMWW+t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	2415	0	2415	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	2005	0	2005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1894	1213 / 0	341 / 0	0 / 0	0 / 0	340 / 0	0 / 0
F	1573	1006 / 0	284 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2097 / 0	-104.9	-104.9	0.03 (1)	6.10	
B- G	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
G- H	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
H- C	0 / 0	-104.9	-104.9	0.16 (1)	10.00	
D- C	-212 / 0	0.0	0.0	0.01 (1)	7.81	
F- A	-2866 / 0	0.0	0.0	0.16 (1)	6.78	
F- E	0 / 0	-28.0	-28.0	0.44 (1)	10.00	
E- I	0 / 1690	-28.0	-28.0	0.72 (1)	10.00	
I- J	0 / 1690	-28.0	-28.0	0.72 (1)	10.00	
J- D	0 / 1690	-28.0	-28.0	0.72 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2039	0.25 (1)
B- D	-1942 / 0	0.32 (1)
A- E	0 / 2151	0.27 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	1-11-4	-0	-1	138	BACK	VERT	TOTAL
H	3-11-4	-0	-1	154	BACK	VERT	TOTAL
I	1-11-4	15	1	18	BACK	VERT	TOTAL
I	2-1-12	-1873	-1873	---	FRONT	VERT	TOTAL
J	3-11-4	15	1	18	BACK	VERT	TOTAL
J	4-1-12	-1873	-1873	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.16/1.00 (A-F:1) , BC=0.72/1.00 (D-E:1) , WB=0.32/1.00 (B-D:1) , SSI=0.86/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

KS GRIP= 0.76 (B) (INPUT = 0.90)
KS METAL= 0.32 (E) (INPUT = 1.00)

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DWG NO. TAM 11707

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T101Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.0 lbs FACTORED DOWN AND 138.5 lbs FACTORED UP AT 1-11-4, AND 1.0 lbs FACTORED DOWN AND 153.7 lbs FACTORED UP AT 3-11-4 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 1-11-4, 1873.1 lbs FACTORED DOWN AT 2-1-12, AND 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 3-11-4, AND 1873.1 lbs FACTORED DOWN AT 4-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11707
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287570	T110	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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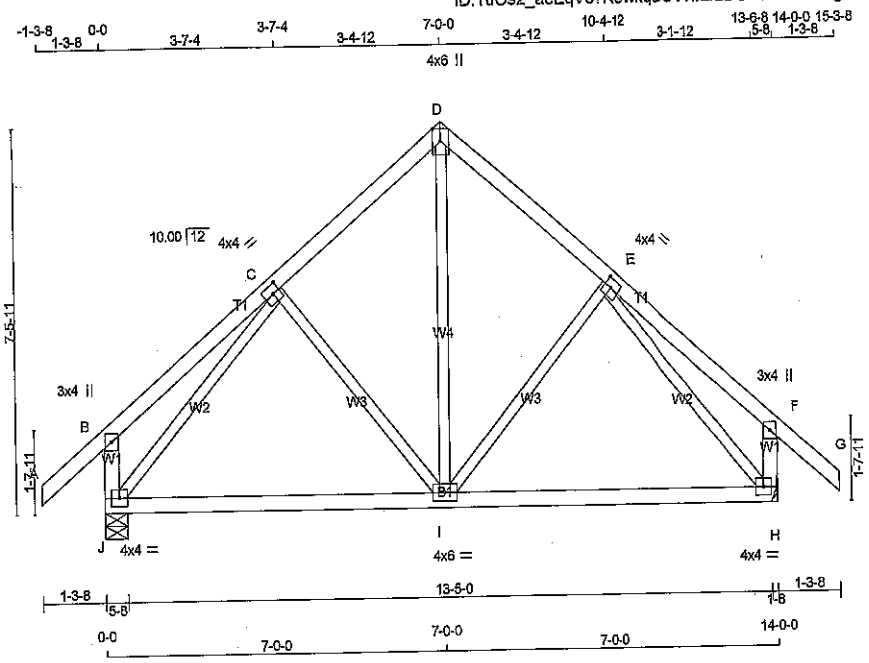

TOTAL WEIGHT = 2 X 66 = 132 lb

[M]F

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1076	0	1076	0	0	5-8	5-8
H	1076	0	1076	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00
B- C	0 / 27	-104.9	-104.9	0.20 (1)	10.00
C- D	-650 / 0	-104.9	-104.9	0.16 (1)	6.25
D- E	-650 / 0	-104.9	-104.9	0.16 (1)	6.25
E- F	0 / 27	-104.9	-104.9	0.20 (1)	10.00
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00
J- B	-285 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-285 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 592	-28.0	-28.0	0.46 (2)	10.00
I- H	0 / 592	-28.0	-28.0	0.46 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 511	0.12 (1)
I- E	-176 / 38	0.09 (1)
C- I	-176 / 38	0.09 (1)
J- C	-929 / 0	0.45 (1)
E- H	-929 / 0	0.45 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.20/1.00 (E-F:1) , BC=0.46/1.00 (I-J:2) , WB=0.45/1.00 (E-H:1) , SSI=0.17/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.34 (E) (INPUT = 1.00)

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PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 1/2/2018
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	G110	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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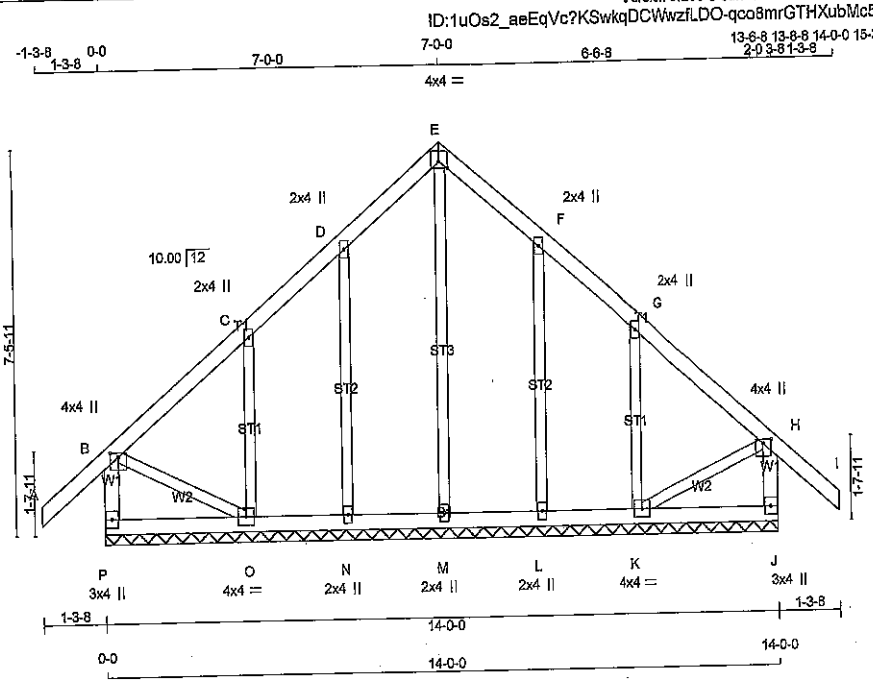

TOTAL WEIGHT = 68 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW-p	MT20	4.0	4.0	1.50	2.00
H	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-313 / 0	0.0	0.0	0.03 (1)	7.81	
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-26 / 0	-104.9	-104.9	0.11 (1)	6.25	
C- D	-48 / 0	-104.9	-104.9	0.11 (1)	6.25	
D- E	-40 / 0	-104.9	-104.9	0.06 (1)	6.25	
E- F	-40 / 0	-104.9	-104.9	0.06 (1)	6.25	
F- G	-48 / 0	-104.9	-104.9	0.11 (1)	6.25	
G- H	-26 / 0	-104.9	-104.9	0.11 (1)	6.25	
H- I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
J- H	-313 / 0	0.0	0.0	0.03 (1)	7.81	
P- O	0 / 0	-28.0	-28.0	0.06 (2)	10.00	
O- N	0 / 26	-28.0	-28.0	0.06 (2)	10.00	
N- M	0 / 22	-28.0	-28.0	0.03 (2)	10.00	
M- L	0 / 22	-28.0	-28.0	0.03 (2)	10.00	
L- K	0 / 26	-28.0	-28.0	0.06 (2)	10.00	
K- J	0 / 0	-28.0	-28.0	0.06 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-148 / 0	0.14 (1)
N- D	-208 / 0	0.11 (1)
O- C	-300 / 0	0.08 (1)
L- F	-208 / 0	0.11 (1)
K- G	-300 / 0	0.08 (1)
B- O	0 / 37	0.01 (1)
K- H	0 / 37	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (H-I:1) , BC=0.06/1.00 (N-O:2) , WB=0.14/1.00 (E-M:1) , SSI=0.11/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.08 (G) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 11709-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	T110S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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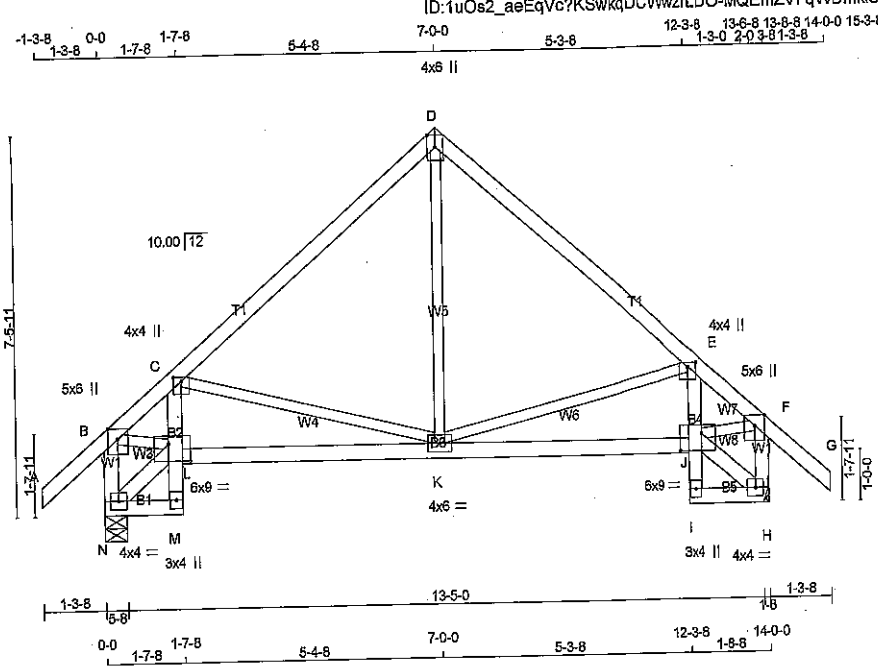

TOTAL WEIGHT = 2 X 68 = 136 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMVW1-t	MT20	4.0	4.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWW-l	MT20	6.0	9.0	4.00	5.50
K	BMWWW-t	MT20	4.0	6.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMV+p	MT20	3.0	4.0		
N	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1076	0	1076	0	0	IN-SX	IN-SX
H	1076	0	1076	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0
H	840	545 / 0	147 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
B- C	-1233 / 0	-104.9	-104.9	0.32 (1)	5.40	
C- D	-724 / 0	-104.9	-104.9	0.36 (1)	6.25	
D- E	-723 / 0	-104.9	-104.9	0.37 (1)	6.25	
E- F	-1210 / 0	-104.9	-104.9	0.30 (1)	5.46	
F- G	0 / 47	-104.9	-104.9	0.14 (1)	10.00	
N- B	-1029 / 0	0.0	0.0	0.11 (1)	7.74	
H- F	-1031 / 0	0.0	0.0	0.11 (1)	7.73	
N- M	0 / 33	-28.0	-28.0	0.02 (2)	10.00	
M- L	0 / 36	0.0	0.0	0.06 (1)	10.00	
L- C	-4 / 107	0.0	0.0	0.05 (2)	10.00	
L- K	0 / 1037	-28.0	-28.0	0.35 (2)	10.00	
K- J	0 / 1009	-28.0	-28.0	0.35 (2)	10.00	
I- J	0 / 36	0.0	0.0	0.05 (1)	10.00	
J- E	-8 / 105	0.0	0.0	0.05 (2)	10.00	
I- H	0 / 31	-28.0	-28.0	0.02 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	-525 / 0	0.30 (1)
K- D	0 / 448	0.10 (2)
K- E	-499 / 0	0.27 (1)
B- L	0 / 1012	0.23 (1)
N- L	-42 / 0	0.00 (1)
J- H	-39 / 0	0.00 (1)
J- F	0 / 988	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.35/1.00 (K-L:2) , WB=0.30/1.00 (C-K:1) , SSI=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

SITE COPY

DWG NO. TAM 11706-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T200	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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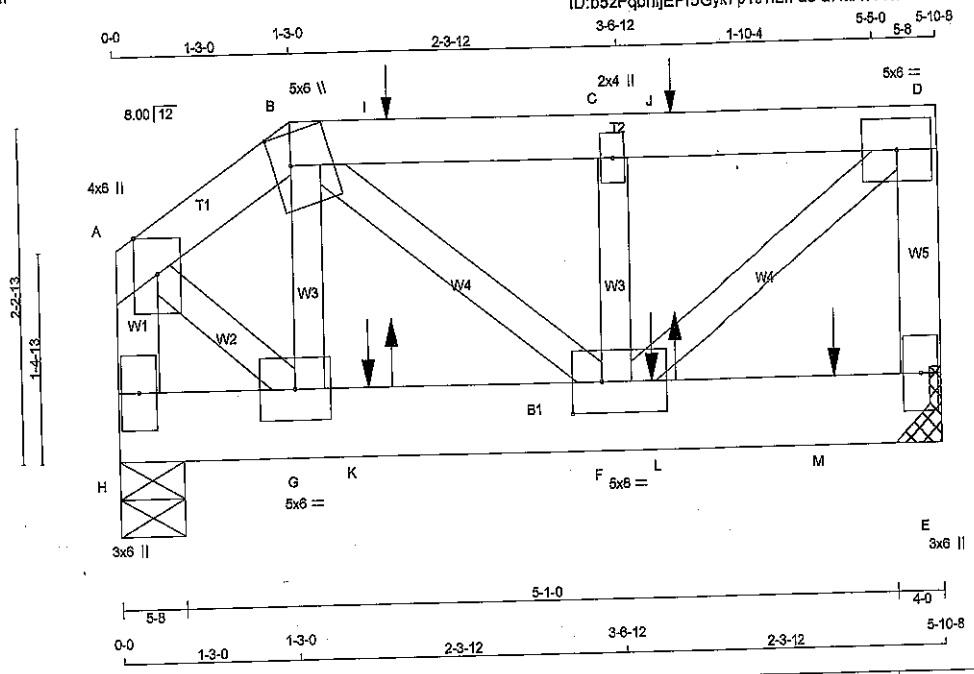

TOTAL WEIGHT = 2 X 28 = 56 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- D	1	12	SIDE(0.0)
D- E	1	12	TOP
H- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
C- F	1	2	SIDE(305.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	6.0	Edge	
B	TTWW+m	MT20	5.0	6.0	2.50	1.50
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	5.0	6.0		
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	5.0	8.0	2.50	2.50
G	BMWW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	3691	0	3691	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
H	2679	0	2679	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	2892	1857 / 0	518 / 0	0 / 0	0 / 0	517 / 0	0 / 0
H	2101	1348 / 0	378 / 0	0 / 0	0 / 0	377 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX	MAX. UNBRAC LENGTH
A- B	-2335 / 0	-104.9	-104.9	0.03 (1)	5.85	
B- I	-2901 / 0	-104.9	-104.9	0.09 (1)	5.32	
I- C	-2901 / 0	-104.9	-104.9	0.09 (1)	5.32	
C- J	-2901 / 0	-104.9	-104.9	0.09 (1)	5.32	
J- D	-2901 / 0	-104.9	-104.9	0.09 (1)	5.32	
E- D	-2428 / 0	0.0	0.0	0.15 (1)	7.23	
H- A	-2799 / 0	0.0	0.0	0.16 (1)	6.85	
H- G	0 / 0	-28.0	-28.0	0.07 (1)	10.00	
G- K	0 / 1950	-28.0	-28.0	0.30 (1)	10.00	
K- F	0 / 1950	-28.0	-28.0	0.30 (1)	10.00	
F- L	0 / 0	-28.0	-28.0	0.36 (1)	10.00	
L- M	0 / 0	-28.0	-28.0	0.36 (1)	10.00	
M- E	0 / 0	-28.0	-28.0	0.36 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	0 / 322	0.04 (1)
B- F	0 / 1225	0.15 (1)
F- C	-198 / 17	0.02 (1)
F- D	0 / 3697	0.46 (1)
A- G	0 / 2399	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	3-9-4	-1872	-1872	---	FRONT	VERT	TOTAL
I	1-11-4	-0	-1	137	BACK	VERT	TOTAL
J	3-11-4	-0	-1	152	BACK	VERT	TOTAL
K	1-9-4	-1872	-1872	---	FRONT	VERT	TOTAL
K	1-11-4	15	1	19	BACK	VERT	TOTAL
L	3-11-4	15	1	19	BACK	VERT	TOTAL
M	5-1-4	-1875	-1875	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (A-H:1) , BC=0.36/1.00 (E-F:1) , WB=0.46/1.00 (D-F:1) , SSI=0.53/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.36 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11734-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	T200	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.0 lbs FACTORED DOWN AND 136.7 lbs FACTORED UP AT 1-11-4, AND 1.0 lbs FACTORED DOWN AND 151.9 lbs FACTORED UP AT 3-11-4 ON TOP CHORD, AND 1671.8 lbs FACTORED DOWN AT 1-9-4, 1.0 lbs FACTORED DOWN AND 18.9 lbs FACTORED UP AT 1-11-4, 1871.8 lbs FACTORED DOWN AT 3-9-4, AND 1.0 lbs FACTORED DOWN AND 18.9 lbs FACTORED UP AT 3-11-4, AND 1875.0 lbs FACTORED DOWN AT 5-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 21734-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287568	J41S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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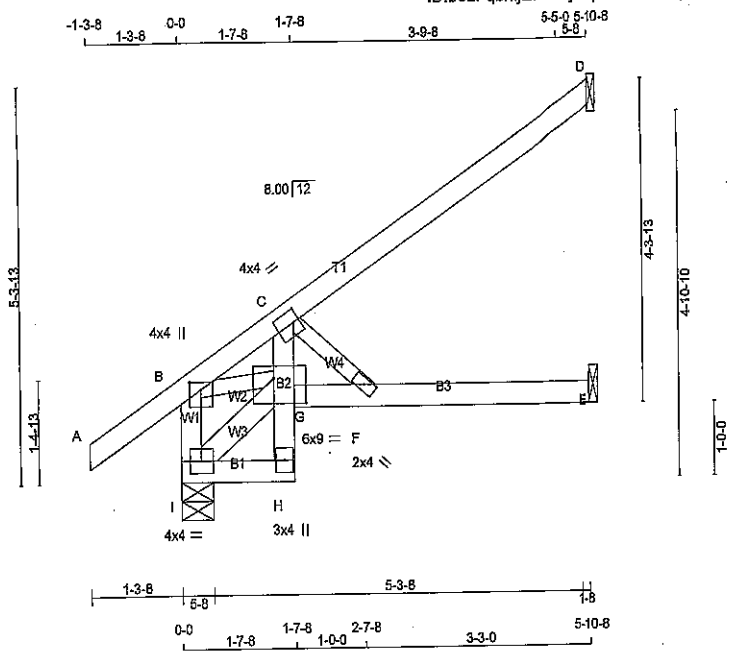

TOTAL WEIGHT = 6 X 24 = 142 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
F	BMW+w	MT20	2.0	4.0		
G	BVMWW-l	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	551	0	551	0	0	5-8	5-8
D	229	0	229	0	0	1-8	1-8
E	145	0	152	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	420	290 / 0	62 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	161	134 / 0	9 / 0	0 / 0	0 / 0	18 / 0	0 / 0
E	138	46 / 0	53 / 0	0 / 0	0 / 0	39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-512 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 40	-104.9	-104.9	0.14 (1)	10.00
B- C	-491 / 0	-104.9	-104.9	0.14 (1)	6.25
C- D	-1 / 11	-104.9	-104.9	0.34 (1)	10.00
I- H	0 / 23	-28.0	-28.0	0.02 (2)	10.00
H- G	0 / 36	0.0	0.0	0.04 (1)	10.00
G- C	0 / 387	0.0	0.0	0.10 (1)	10.00
G- F	0 / 457	-28.0	-28.0	0.51 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-29 / 0	0.00 (1)
B- G	0 / 361	0.09 (1)
C- F	-561 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 677 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 418 (0.17")

CSI: TC=0.34/1.00 (C-D:1) , BC=0.51/1.00 (F-G:1) , WB=0.09/1.00 (B-G:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.51 (B) (INPUT = 0.90)
USI METAL= 0.32 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11735
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	PB1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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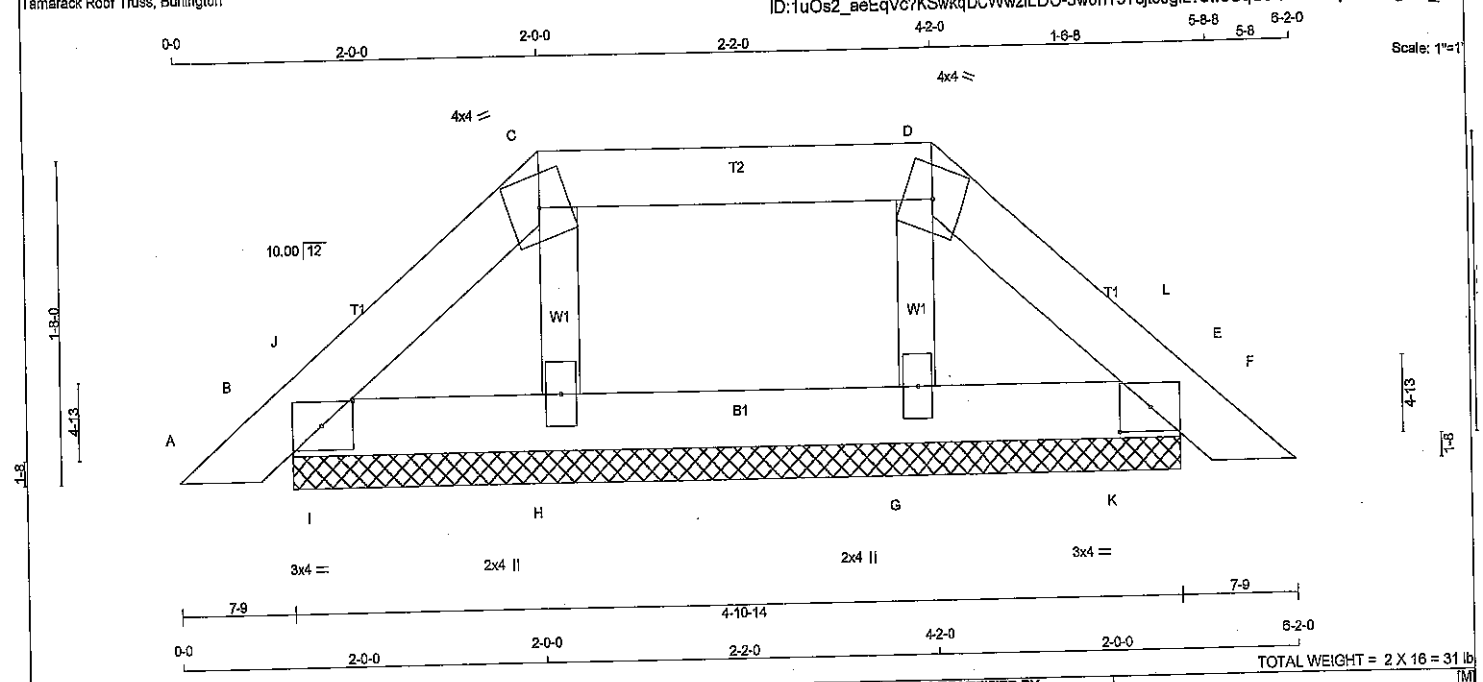

TOTAL WEIGHT = 2 X 16 = 31 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- E	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	179	0	179	0	0	4-10-14	4-10-14
E	179	0	179	0	0	4-10-14	4-10-14
H	203	0	203	0	0	4-10-14	4-10-14
G	203	0	203	0	0	4-10-14	4-10-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	130	101 / 0	12 / 0	0 / 0	0 / 0	17 / 0	0 / 0
E	130	101 / 0	12 / 0	0 / 0	0 / 0	17 / 0	0 / 0
H	168	93 / 0	40 / 0	0 / 0	0 / 0	35 / 0	0 / 0
G	168	93 / 0	40 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.01 (1)	10.00
B- J	-58 / 0	-104.9	-104.9	0.00 (3)	6.25
J- C	-55 / 0	-104.9	-104.9	0.01 (1)	6.25
C- D	-30 / 0	-104.9	-104.9	0.04 (1)	6.25
D- L	-55 / 0	-104.9	-104.9	0.01 (1)	6.25
L- E	-58 / 0	-104.9	-104.9	0.00 (3)	6.25
E- F	0 / 16	-104.9	-104.9	0.01 (1)	10.00
B- I	0 / 41	-28.0	-28.0	0.01 (1)	10.00
I- H	0 / 41	-28.0	-28.0	0.01 (1)	10.00
H- G	0 / 30	-28.0	-28.0	0.01 (2)	10.00
G- K	0 / 41	-28.0	-28.0	0.01 (1)	10.00
K- E	0 / 41	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-137 / 0	0.01 (1)
G- D	-137 / 0	0.01 (1)
I- J	-67 / 8	0.00 (1)
K- L	-67 / 8	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-D:1) , BC=0.01/1.00 (G-K:1) , WB=0.01/1.00 (D-G:1) , SSI=0.04/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (E) (INPUT = 0.90)
JSI METAL= 0.02 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11678-B
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	PB2	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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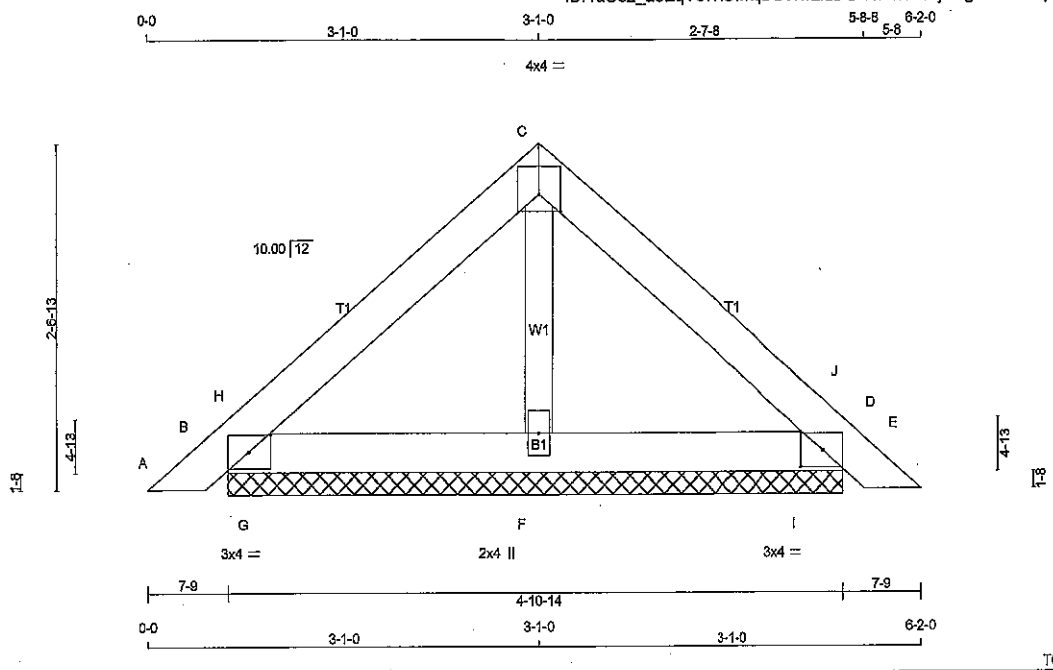

TOTAL WEIGHT = 5 X 16 = 81 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	260	0	260	0	0	4-10-14	4-10-14
D	260	0	260	0	0	4-10-14	4-10-14
F	245	0	245	0	0	4-10-14	4-10-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	193	142 / 0	23 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	193	142 / 0	23 / 0	0 / 0	0 / 0	29 / 0	0 / 0
F	209	105 / 0	56 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- H	-45 / 0	-104.9	-104.9	0.03 (1)	6.25	
H- C	-96 / 0	-104.9	-104.9	0.07 (1)	6.25	
C- J	-96 / 0	-104.9	-104.9	0.07 (1)	6.25	
J- D	-45 / 0	-104.9	-104.9	0.03 (1)	6.25	
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00	
B- G	0 / 69	-28.0	-28.0	0.08 (1)	10.00	
G- F	0 / 69	-28.0	-28.0	0.08 (1)	10.00	
F- I	0 / 69	-28.0	-28.0	0.08 (1)	10.00	
I- D	0 / 69	-28.0	-28.0	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-96 / 0	0.01 (1)
G- H	-213 / 28	0.00 (1)
I- J	-213 / 28	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-J:1) , BC=0.08/1.00 (D-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.16/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11619

STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	PB3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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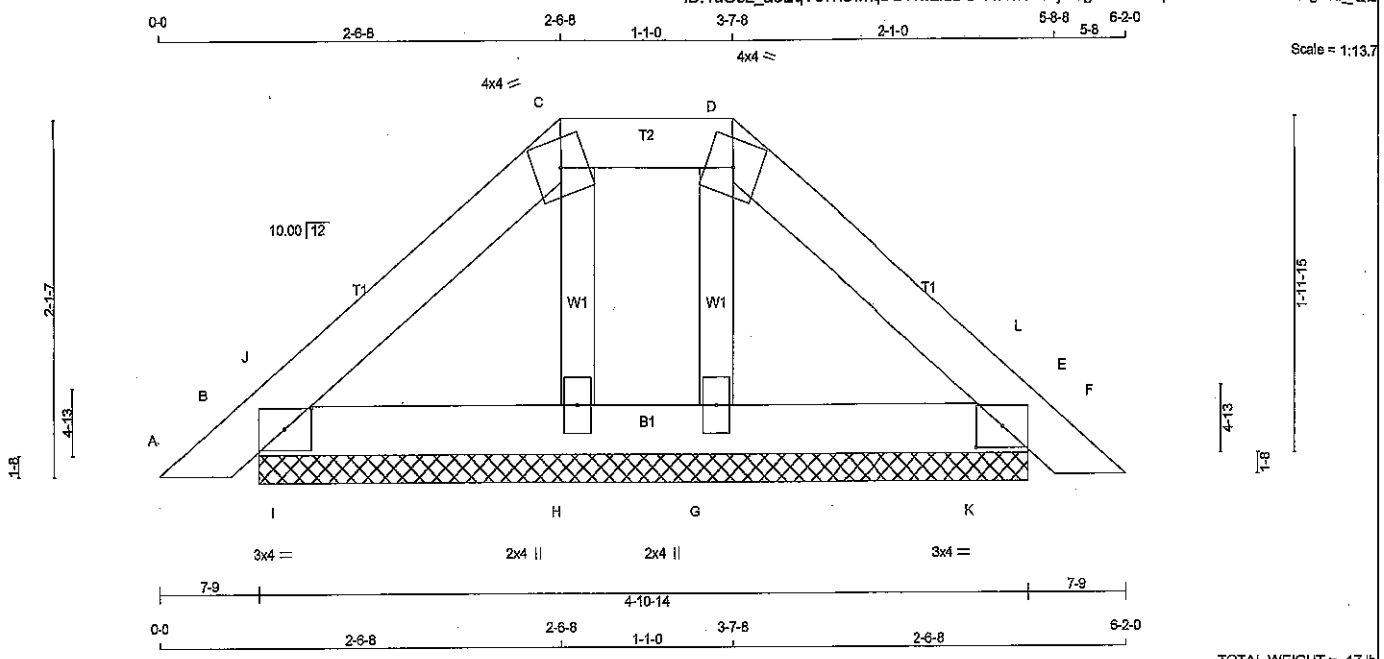

TOTAL WEIGHT = 17 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	220	0	220	0	0	4-10-14	4-10-14
E	220	0	220	0	0	4-10-14	4-10-14
H	162	0	162	0	0	4-10-14	4-10-14
G	162	0	162	0	0	4-10-14	4-10-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	165	119 / 0	21 / 0	0 / 0	0 / 0	25 / 0	0 / 0
E	165	119 / 0	21 / 0	0 / 0	0 / 0	25 / 0	0 / 0
H	133	75 / 0	31 / 0	0 / 0	0 / 0	27 / 0	0 / 0
G	133	75 / 0	31 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- J	-53 / 0	-104.9	-104.9	0.01 (3)	6.25
J- C	-67 / 0	-104.9	-104.9	0.05 (1)	6.25
C- D	-43 / 0	-104.9	-104.9	0.02 (1)	6.25
D- L	-67 / 0	-104.9	-104.9	0.05 (1)	6.25
L- E	-53 / 0	-104.9	-104.9	0.01 (3)	6.25
E- F	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- I	0 / 49	-28.0	-28.0	0.05 (1)	10.00
I- H	0 / 49	-28.0	-28.0	0.05 (1)	10.00
H- G	0 / 43	-28.0	-28.0	0.03 (1)	10.00
G- K	0 / 49	-28.0	-28.0	0.05 (1)	10.00
K- E	0 / 49	-28.0	-28.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-99 / 0	0.01 (1)
G- D	-99 / 0	0.01 (1)
I- J	-119 / 31	0.00 (1)
K- L	-119 / 31	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-L:1) , BC=0.05/1.00 (E-K:1) , WB=0.01/1.00 (D-G:1) , SSI=0.10/1.00 (B-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 11690

STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	PB4	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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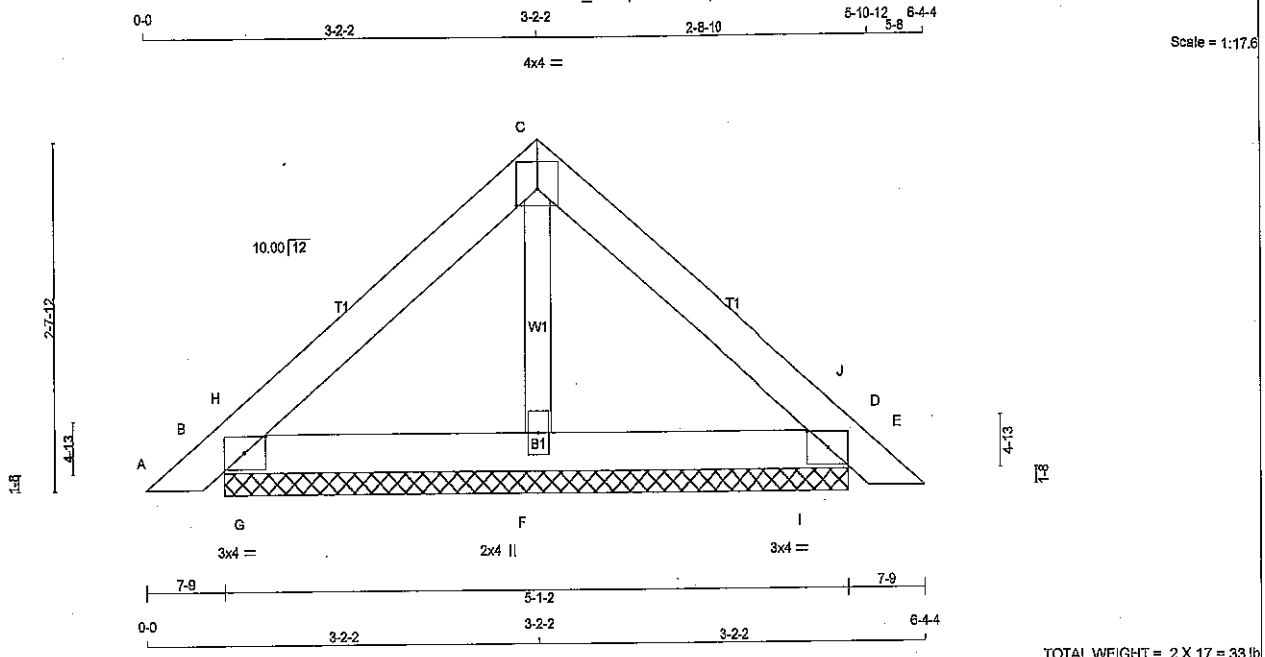

TOTAL WEIGHT = 2 X 17 = 33 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	269	0	269	0	0	5-1-2	5-1-2
D	269	0	269	0	0	5-1-2	5-1-2
F	251	0	251	0	0	5-1-2	5-1-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL.
B	201	147 / 0	24 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	201	147 / 0	24 / 0	0 / 0	0 / 0	30 / 0	0 / 0
F	214	107 / 0	59 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.01 (1)	10.00
B- H	-47 / 0	-104.9	-104.9	0.02 (1)	6.25
H- C	-103 / 0	-104.9	-104.9	0.04 (1)	6.25
C- J	-103 / 0	-104.9	-104.9	0.04 (1)	6.25
J- D	-47 / 0	-104.9	-104.9	0.02 (1)	6.25
D- E	0 / 16	-104.9	-104.9	0.01 (1)	10.00
B- G	0 / 74	-28.0	-28.0	0.04 (1)	10.00
G- F	0 / 74	-28.0	-28.0	0.04 (1)	10.00
F- I	0 / 74	-28.0	-28.0	0.04 (1)	10.00
I- D	0 / 74	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-98 / 0	0.01 (1)
G- H	-223 / 29	0.00 (1)
I- J	-223 / 29	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (C-H:1) , BC=0.04/1.00 (B-G:1) , WB=0.01/1.00 (C-F:1) , SSI=0.08/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM11681-PB
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287567	PB30	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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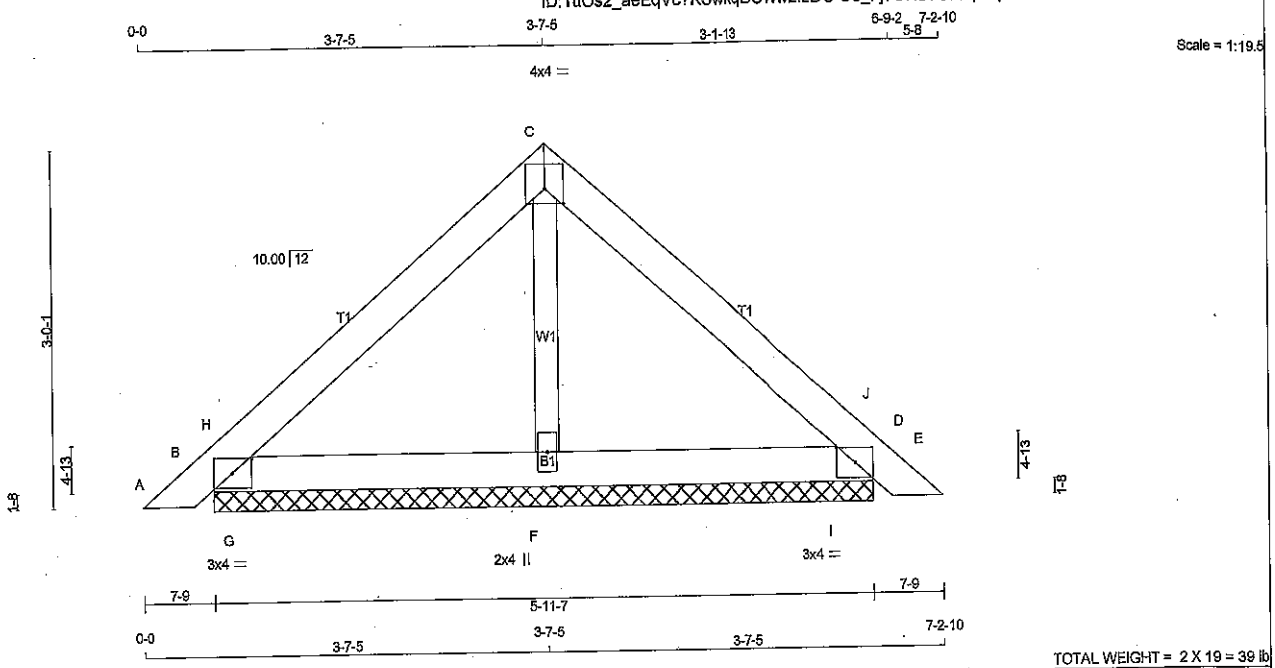

TOTAL WEIGHT = 2 X 19 = 39 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	310	0	310	0	0	5-11-7	5-11-7
D	310	0	310	0	0	5-11-7	5-11-7
F	285	0	285	0	0	5-11-7	5-11-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	232	168 / 0	29 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	232	168 / 0	29 / 0	0 / 0	0 / 0	35 / 0	0 / 0
F	244	121 / 0	67 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- H	-58 / 0	-104.9	-104.9	0.05 (1)	6.25
H- C	-128 / 0	-104.9	-104.9	0.11 (1)	6.25
C- J	-128 / 0	-104.9	-104.9	0.11 (1)	6.25
J- D	-58 / 0	-104.9	-104.9	0.05 (1)	6.25
D- E	0 / 16	-104.9	-104.9	0.02 (1)	10.00
B- G	0 / 91	-28.0	-28.0	0.11 (1)	10.00
G- F	0 / 91	-28.0	-28.0	0.11 (1)	10.00
F- I	0 / 91	-28.0	-28.0	0.11 (1)	10.00
I- D	0 / 91	-28.0	-28.0	0.11 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-106 / 2	0.02 (1)
G- H	-303 / 40	0.00 (1)
I- J	-303 / 40	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-H:1) , BC=0.11/1.00 (B-G:1) , WB=0.02/1.00 (C-F:1) , SSI=0.23/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2018
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1/7/0 -18
STRUCTURAL COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
287566	J1SA	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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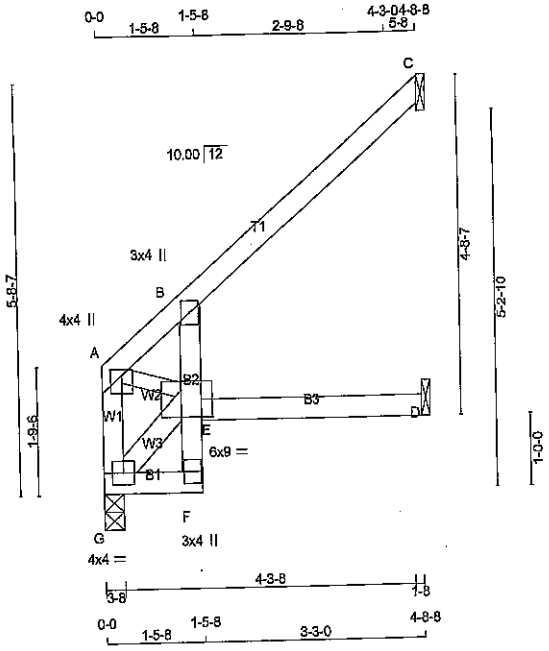

TOTAL WEIGHT = 6 X 19 = 115 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
A - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
E	BVMWW-I	MT20	6.0	9.0	4.50	5.50
F	BMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	313	0	313	0	0	3-8	3-8
C	265	0	265	0	0	1-8	1-8
D	48	0	83	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	249	153 / 0	49 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	190	153 / 0	14 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	59	0 / 0	36 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
G- A	-291 / 0	0.0	0.0	0.03 (1)	7.81
A- B	-140 / 0	-104.9	-104.9	0.39 (1)	6.25
B- C	0 / 53	-104.9	-104.9	0.47 (1)	10.00
G- F	0 / 4	-28.0	-28.0	0.01 (2)	10.00
F- E	0 / 32	0.0	0.0	0.01 (2)	10.00
E- B	0 / 116	0.0	0.0	0.02 (2)	10.00
E- D	0 / 0	-28.0	-28.0	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	-5 / 0	0.00 (1)
A- E	-11 / 0	0.00 (2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	53.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 507 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 360 (0.17")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.10/1.00 (D-E:2) , WB=0.00/1.00 (A-E:2) , SSI=0.18/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

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DWG NO. TAM 1/16 2018
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

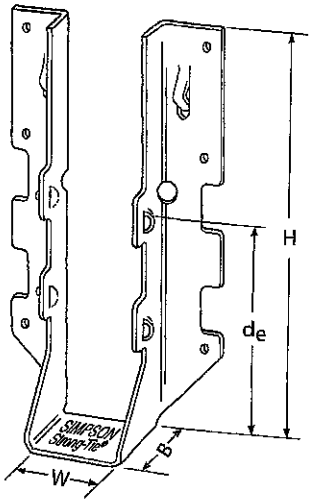
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

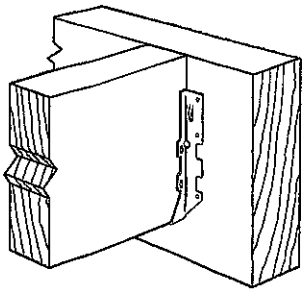
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



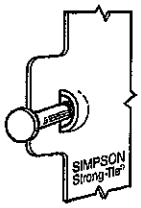
LUS28



Typical LUS Installation

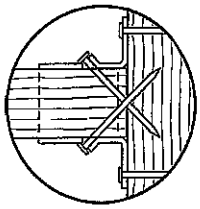
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECLUS17 3/17 exp. 6/19

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

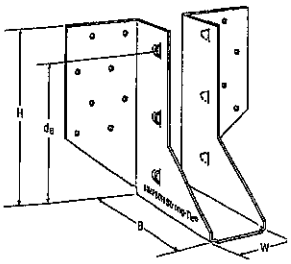
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

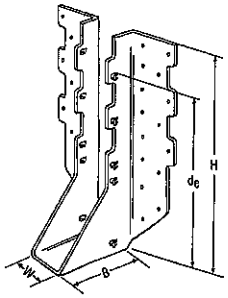
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

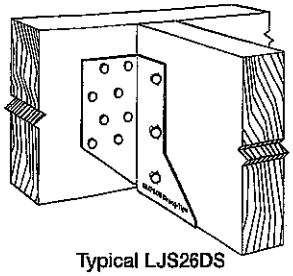
- See current catalogue for options



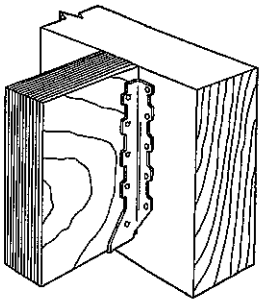
LJS26DS



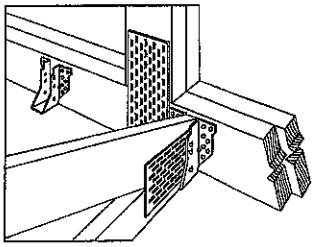
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



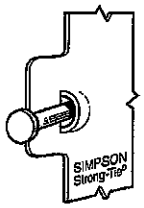
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

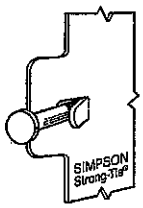
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

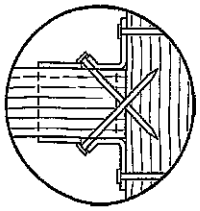


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

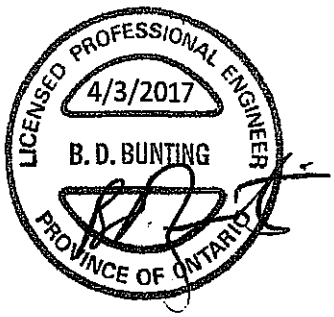
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

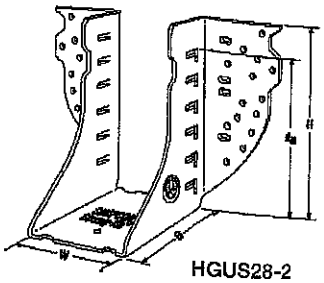
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

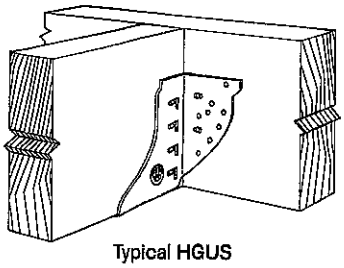
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

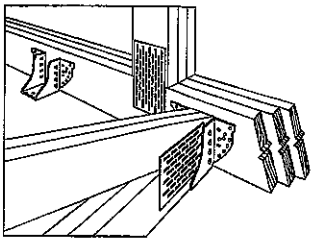
- See current catalogue for options



HGUS28-2



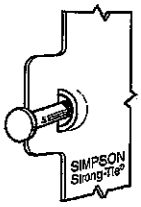
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

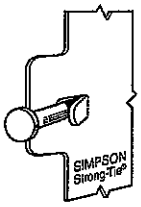
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

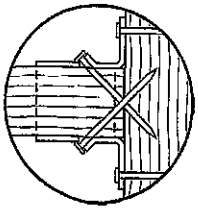


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/10/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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ONTARIO WOOD TRUSS FABRICATORS ASSOCIATION

TECH-NOTES

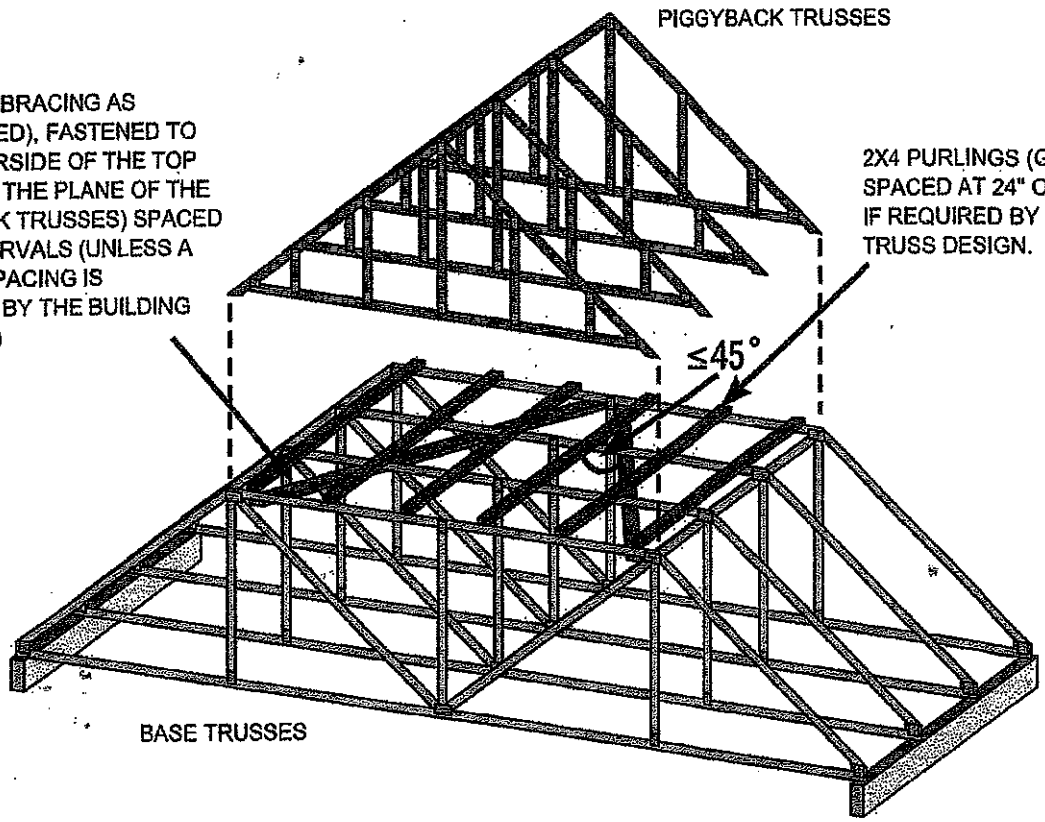
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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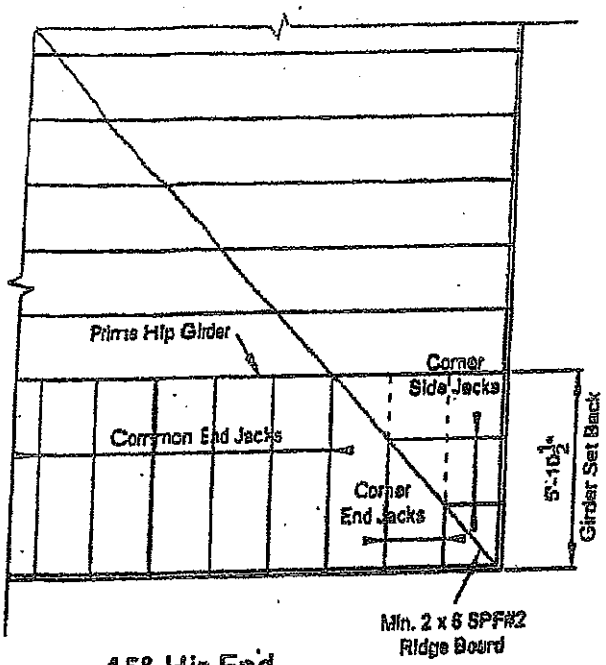
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

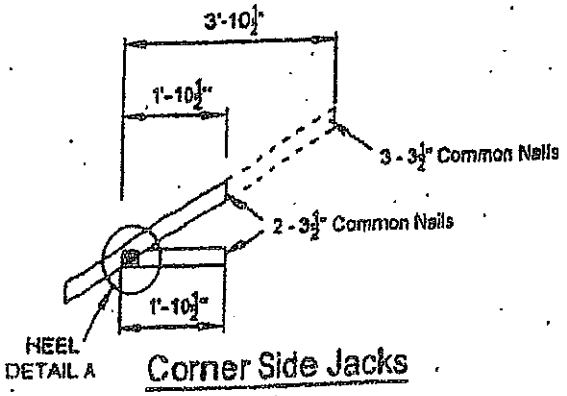
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

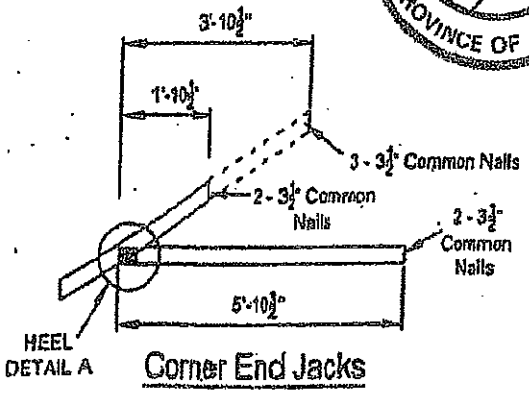
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO TAM 3495.14 R1
STRUCTURAL COMPONENT ONLY

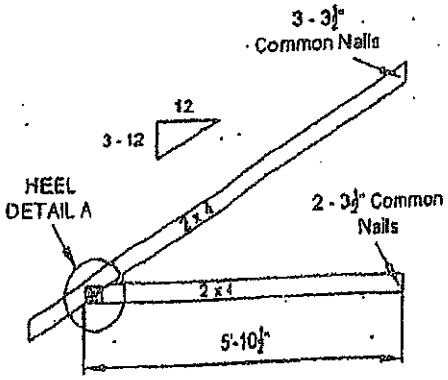
LICENSED PROFESSIONAL ENGINEER
1-2-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



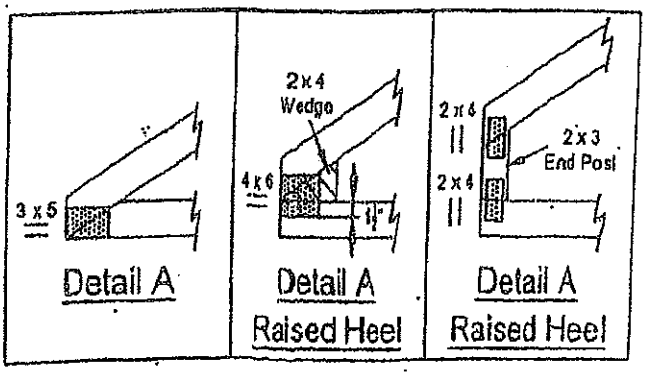
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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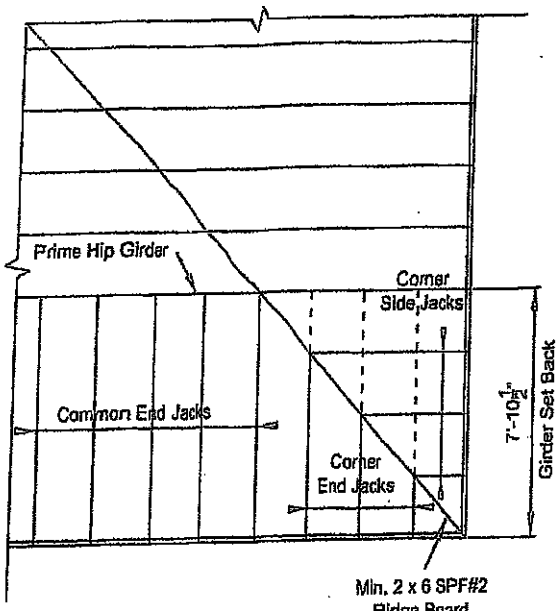
MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



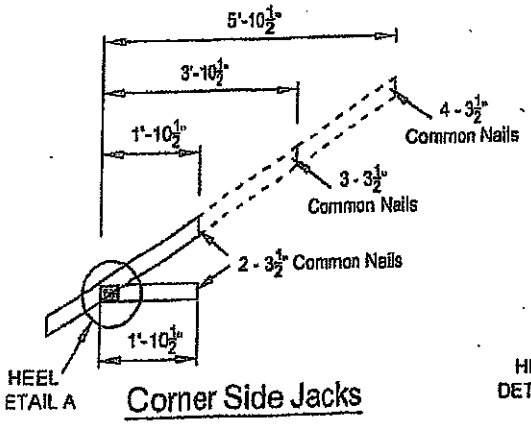
45° Hip End

DESIGN LOAD:

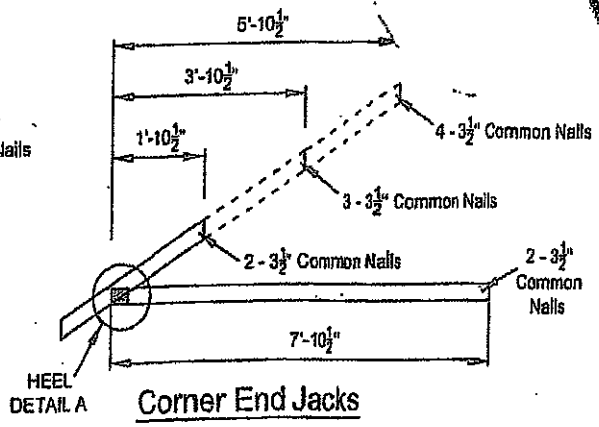
TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY

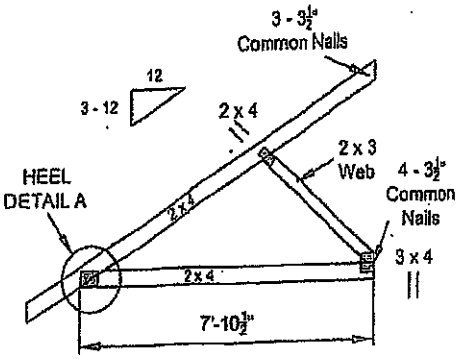
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



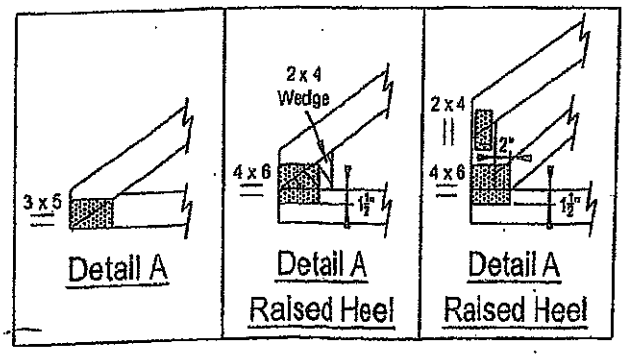
Corner Side Jacks



Corner End Jacks



Common End Jacks

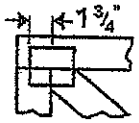


NOTE: DESIGN CONFORMS TO PART 9 O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

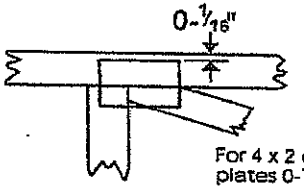
SITE COPY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

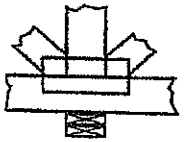
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

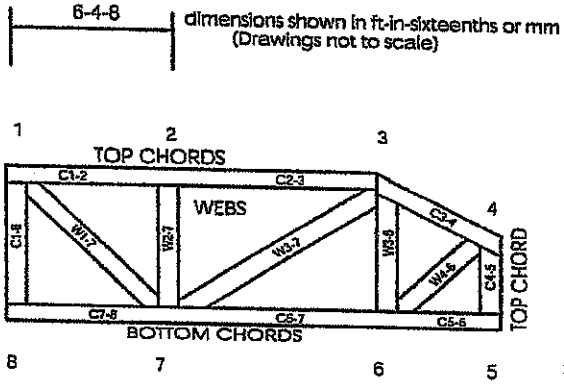
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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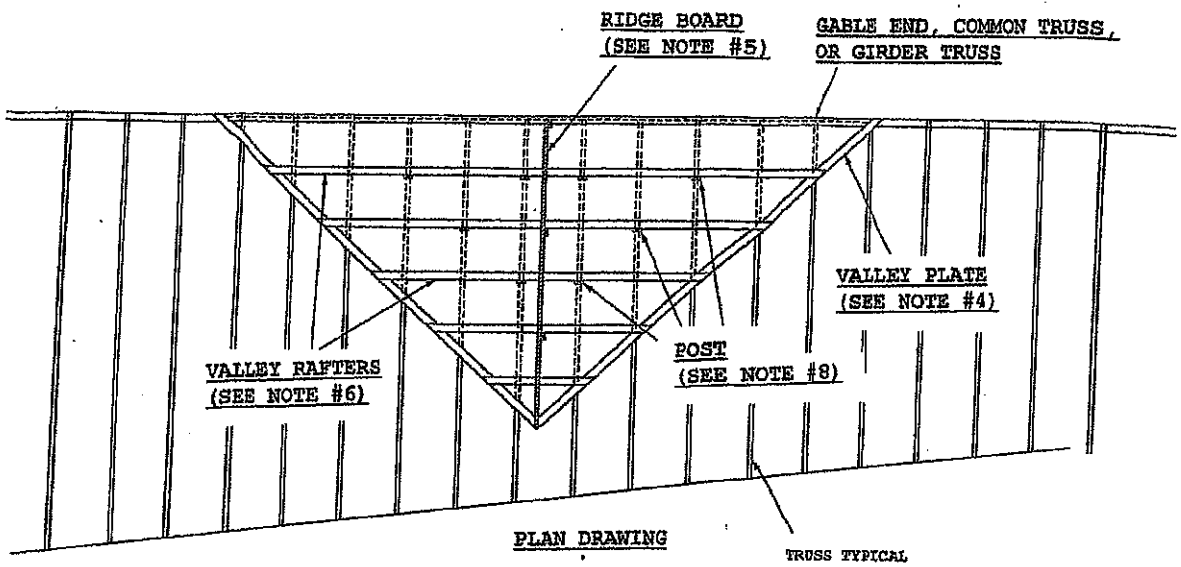
MICRO CITY
ENGINEERING SERVICES INC.

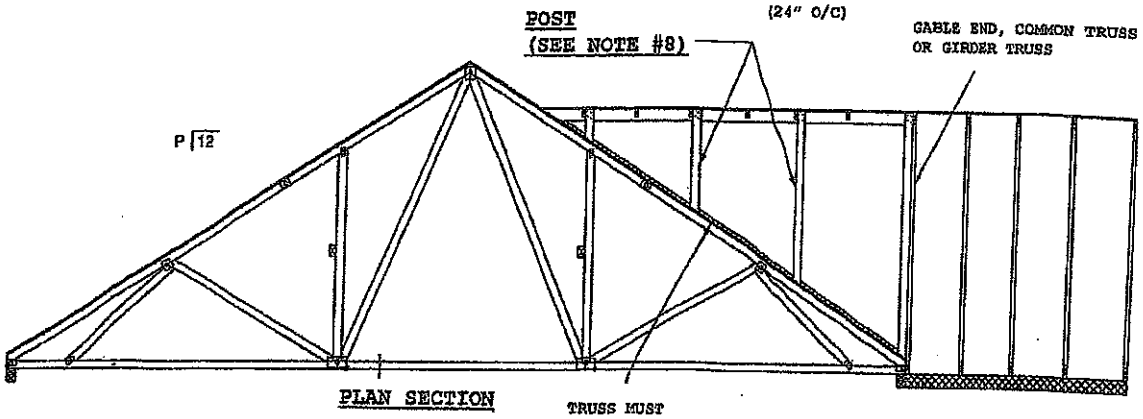
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM6305.14
STRUCTURAL
COMPONENT ONLY

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Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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