

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2018-11-07

INSPECTOR: BG

ALL CONV. FRAMING TO
CONFORM WITH PART 9 OF
O.B.C. 2012 ROOF RAFTERS THAT
CROSS MEET OVER TRUSSES TO
BE 2x4 SPP#2 @ 24" O.C. WITH A
VERTICAL POST TO THE TRUSS
UNDERNEATH AT EACH CROSS
POINT. VERTICAL POST LONGER
THAN 8' TO HAVE LATERAL
BRACING SO THAT THE
DISTANCE BETWEEN END POINT
AND BETWEEN ROWS OF
BRACING DOES NOT EXCEED 6'

ASHPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL
FRAMING

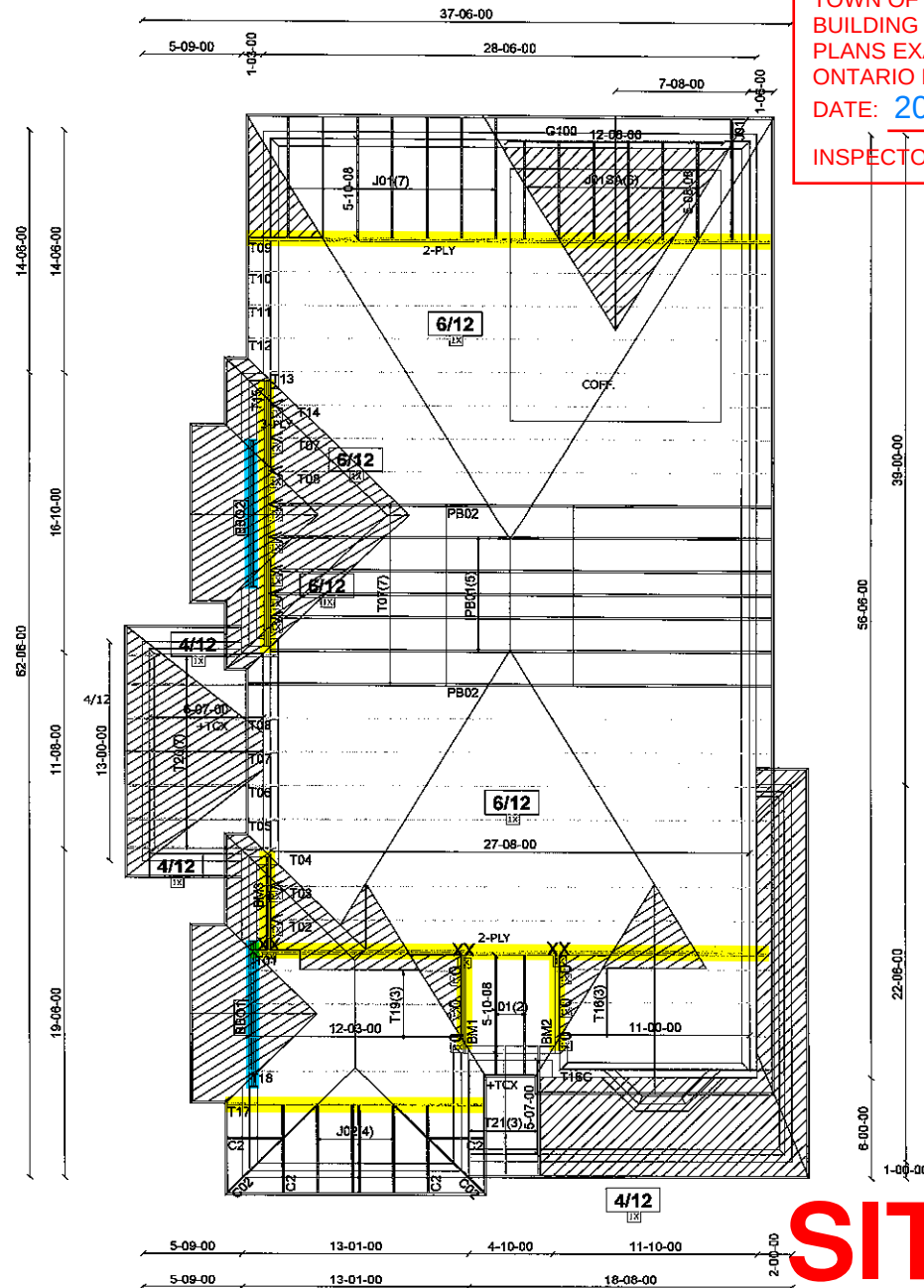
O LUS24
V LJS26DS
XX HGUS26-2

"PIGGYBACKS"
PURLINS BY OTHERS

"PORCH BEAMS (A.P.P.)
BY OTHERS
(DROPPED)

"COFFERED CL." (CONFIRM)
- RAISED 12"
- INSET 14"

BM1,2, 3:2-2X10



SITE COPY

mil, 250

	Job Track: 44755 Layout ID: 288713 Plan Log: 93767	Builder / Location: BAYVIEW WELLINGTON / BRADFORD Project: GREEN VALLEY EAST Date: 3/2/2018 Designer: Peter Dahlberg/JG	Model / Elevation: S38-8C / A OPT.COFF - 4 OR 5 BR THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Mitek ver 7.6.4
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Mitek ver 7.6.4

ALL CONV. FRAMING TO CONFORM WITH PART 8 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

ASPHALT SHINGLES
2x8 BRG/BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x8 FASCIA

BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 10/12 (U.N.O.)

CONVENTIONAL FRAMING

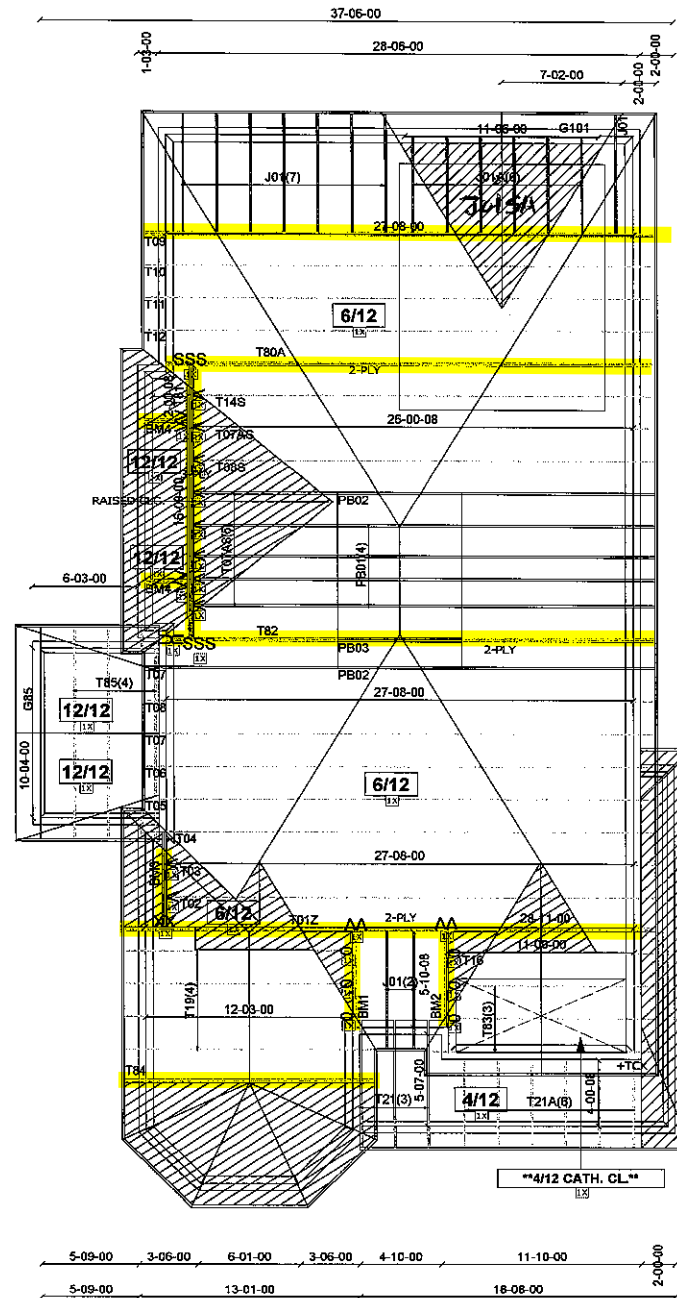
O LUS24
V LJS26DS
XX HGUS26-2
SSS HGUS26-3
TBE6

PIGGYBACKS
PURLINS BY OTHERS

**PORCH BEAMS (A.P.F.)
BY OTHERS
(DROPPED)

**COFFERED CL. (CONFIRM)
- RAISED 12"
- INSET 14"

T-180101



SITE COPY

111,250

	Job Track: 44755 Layout ID: 288737 Plan Log: 96660	Builder / Location: BAYVIEW WELLINGTON / BRADFORD Project: GREEN VALLEY EAST Date: 3/2/2018 Designer: Peter Dahlberg/JG	Model / Elevation: S38-8C / B OPT.COFF - 4 OR 5 BR THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE. Milek ver 7.6.4
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ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

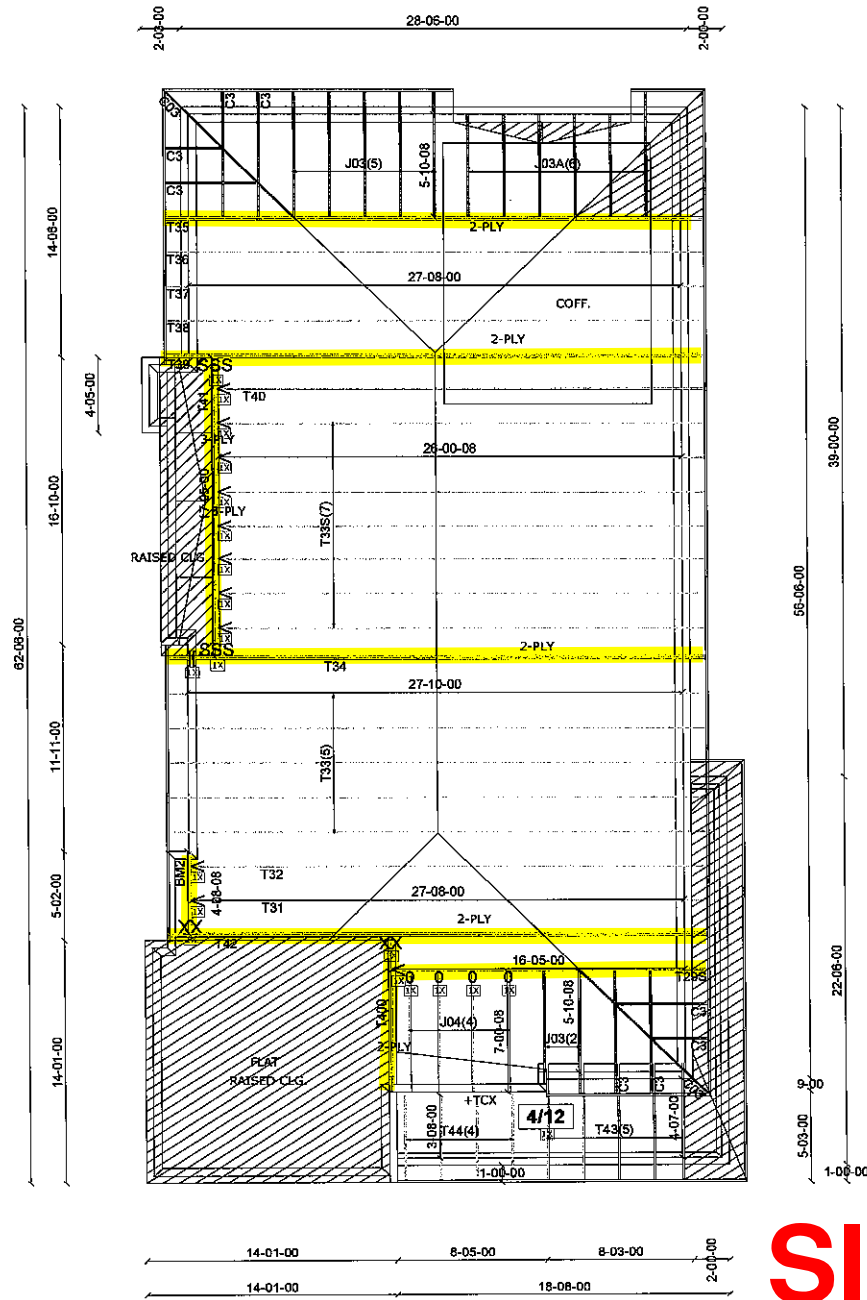
BRADFORD
GSL: 43.859 PSF
TCDL: 3 PSF
BCLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

O LUS24
V LJS28DS
XX HGUS26-2
SSS HGUS28-3
II TBE4

COFFERED CL. (CONFIRM)
- RAISED 12"
- INSET 14"



SITE COPY

ml, 260



Job Track: **44755**
Layout ID: **288739**
Plan Log: **96660**

Builder / Location:
BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **3/5/2018** Designer: **Peter Dahlberg / JG**

Model / Elevation:
S38-8C / C OPT.COFF - 4/5 BR

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.
Mitek ver 7.6.4

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPP#2 @ 24" O.C. WITH A VERTICAL POST TO THE TRUSS UNDERNEATH AT EACH CROSS POINT. VERTICAL POST LONGER THAN 8' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'

ASPHALT SHINGLES
2x6 BRG./BRICK
12" FINISHED OVERHANG
R.T.M.C.
W/ 2x6 FASCIA

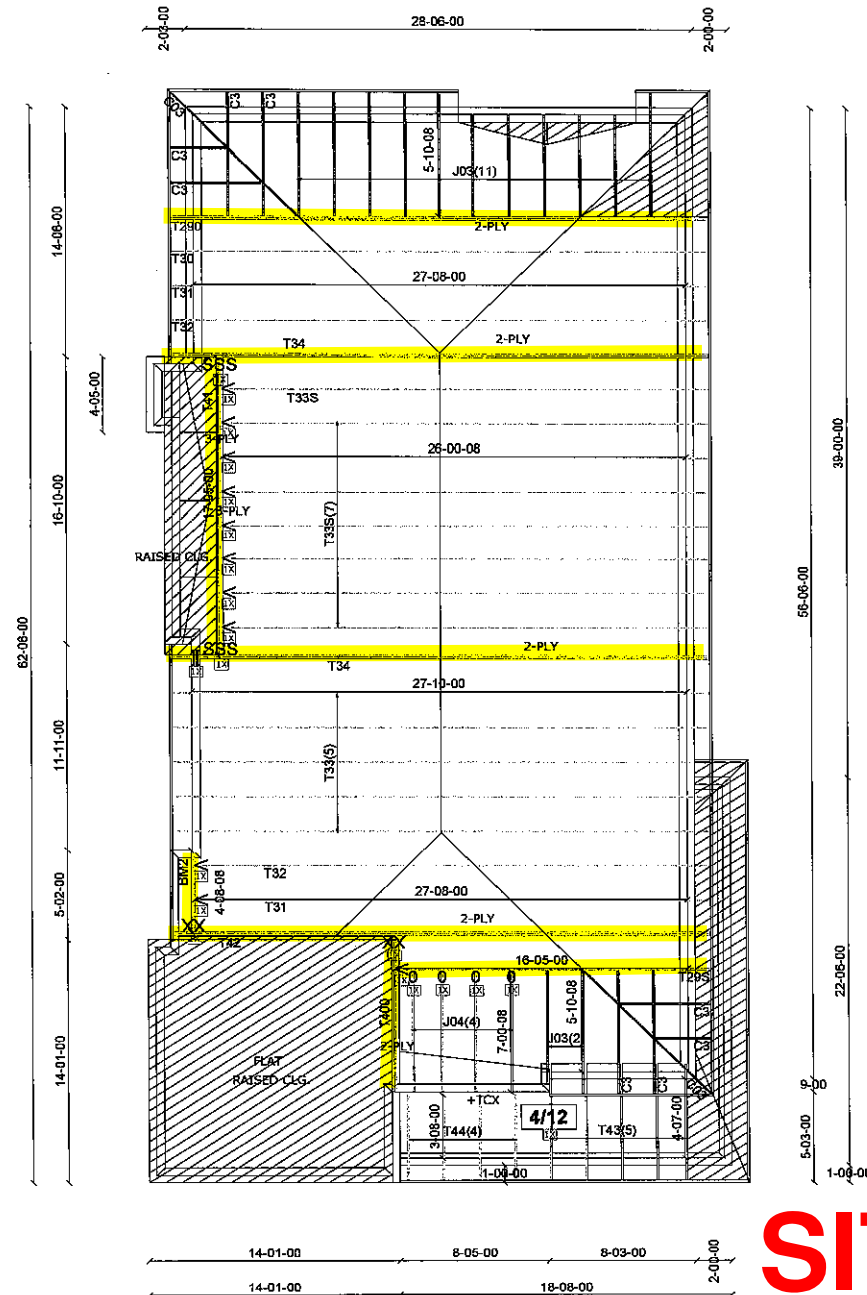
BRADFORD
GSL: 43,859 PSF
TCDL: 3 PSF
BOLL: 10 PSF
BCDL: 7 PSF

PITCH: 8/12 (U.N.O.)

CONVENTIONAL FRAMING

O LUS24
V L/S26DS
XX HGUS26-2
SSS HGUS28-3
II TBE4

COFFERED CL(CONFIRM)
- RAISED 12"
- INSET 14"



SITE COPY

MU, 250



Job Track: **44755**
Layout ID: **288746**
Plan Log: **96660**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST**

Date: **3/5/2018** Designer: **Peter Dahlberg / JG**

Model / Elevation:

S38-8C / C STD. - 4/5 BR

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 7.6.4



Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755	LAYOUT ID: 288713	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: A OPT.COFF	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T01 HIP GIRDER	10.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	327.74		
	2 Ply		0.00							206.68		
	1	T02 HIP	10.00 0.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.13 72.33		
	1	T03 HIP	10.00 0.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	119.22 74.67		
	1	T04 HIP	10.00 0.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.09 76.50		
	1	T05 HIP	10.00 0.00	27-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	127.75 81.33		
	1	T06 HIP	10.00 0.00	27-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.57 87.17		
	9	T07 HIP	10.00 0.00	27-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1300.14 825.03		
	2	T08 HIP	10.00 0.00	27-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	293.34 182.00		
	1	T09 HIP GIRDER	10.00	27-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	324.78		
	2 Ply		0.00							206.66		
	1	T10 HIP	10.00 0.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.78 82.50		
	1	T11 HIP	10.00 0.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.07 85.83		
	1	T12 HIP	10.00 0.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	136.54 87.17		
	1	T13 HIP	10.00 0.00	27-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	133.83 86.33		
	1	T14 HIP	10.00 0.00	27-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	143.99 91.67		
	1	T15 HIP GIRDER	10.00	16-00-00	02-04-07	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 01-07-11	258.60		
	3 Ply		0.00							171.99		
	3	T16 COMMON	10.00 0.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.24 96.99		
	1	T16G COMMON	10.00 0.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	50.75 33.33		
	1	T17 HIP GIRDER	10.00 0.00	12-03-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.77 40.00		

SITE COPY



Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288713 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: A OPT.COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T18 HIPP	10.00 0.00	12-03-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.52 38.17		
	3	T19 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	165.75 110.49		
	7	T20 ROOF	4.00 0.00	06-07-00	02-09-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-04	180.32 115.50		
	3	T21 ROOF	4.00 0.00	05-07-00	02-05-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-02-04	59.01 38.01		
	5	PB01 PIGGYBACK	10.00 0.00	06-01-01	02-11-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	98.30 64.15		
	2	PB02 PIGGYBACK	10.00 0.00	06-01-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	38.08 28.00		
	1	G100 COMMON	10.00 0.00	12-06-00	06-10-03	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	57.01 37.83		
	10	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	6	J01SA JACK-OPEN	6.00 0.00	05-08-08	04-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-03-00 03-01-04	113.28 84.00		
	4	J02 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	60.48 38.68		
	2	C02 JACK-OPEN	10.00 0.00	03-10-08	04-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	69.74 44.66		

TOTAL # TRUSS= 77.00

TOTAL BFT OF ALL TRUSSES=

3294.37 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5129.72 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
8	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= 16.00

SITE COPY



Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288740 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: A STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T01 HIP GIRDER	10.00 0.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	327.74		
	2 Ply									206.68		
	1	T090 HIP GIRDER	10.00 0.00	27-08-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	260.08		
	2 Ply									163.34		
	2	T02 HIPP	10.00 0.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	226.26		
										144.66		
	2	T03 HIPP	10.00 0.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	238.44		
										149.34		
	2	T04 HIPP	10.00 0.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	244.18		
										153.00		
	2	T05 HIP	10.00 0.00	27-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	255.50		
										162.66		
	2	T06 HIP	10.00 0.00	27-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	277.14		
										174.34		
	9	T07 HIP	10.00 0.00	27-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	1300.14		
										825.03		
	2	T08 HIP	10.00 0.00	27-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	293.34		
										182.00		
	1	T15 HIP GIRDER	10.00 0.00	16-00-00	02-04-07	2 X 6	2 X 6	00-00-00 00-00-00	01-07-11 01-07-11	258.60		
	3 Ply									171.99		
	3	T16 COMMON	10.00 0.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	147.24		
										96.99		
	1	T16G COMMON	10.00 0.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	50.75		
										33.33		
	1	T17 HIP GIRDER	10.00 0.00	12-03-00	04-10-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	62.77		
										40.00		
	1	T18 HIPP	10.00 0.00	12-03-00	06-06-07	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.52		
										38.17		
	3	T19 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	165.75		
										110.49		
	7	T20 ROOF	4.00 0.00	06-07-00	02-09-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-06-04	180.32		
										115.50		
	3	T21 ROOF	4.00 0.00	05-07-00	02-05-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 02-02-04	59.01		
										38.01		
	1	G100 COMMON	10.00 0.00	12-06-00	06-10-03	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	57.01		
										37.83		

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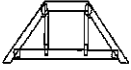
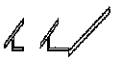
Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288740 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: A STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	PB01 PIGGYBACK	10.00 0.00	06-01-01	02-11-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	98.30 64.15		
	2	PB02 PIGGYBACK	10.00 0.00	06-01-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	38.08 28.00		
	10	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	167.90 106.70		
	6	J01A JACK-OPEN	6.00 0.00	05-08-08	04-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-03-00 04-01-04	89.04 55.98		
	4	J02 JACK-OPEN	10.00 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-10-07	60.48 38.68		
	2	C02 JACK-OPEN	10.00 0.00	03-10-08	04-03-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-03-08	69.74 44.66		

TOTAL # TRUSS= 77.00

TOTAL BFT OF ALL TRUSSES=

3181.53 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4986.33 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
8	Hangers	LJS26DS	
6	Hangers	LUS24	

TOTAL # ITEMS= 16.00

SITE COPY



Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288737 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: B-OPT.COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T01Z HIP GIRDER	10.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	327.74		
	2 Ply		0.00					01-03-08	01-07-11	206.68		
	1	T02 HIP	10.00 0.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	113.13 72.33		
	1	T03 HIP	10.00 0.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	119.22 74.67		
	1	T04 HIP	10.00 0.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	122.09 76.50		
	1	T05 HIP	10.00 0.00	27-08-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	127.75 81.33		
	1	T06 HIP	10.00 0.00	27-08-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	138.57 87.17		
	2	T07 HIP	10.00 0.00	27-08-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	288.92 183.34		
	6	T07AS PIGGYBACK	10.00 0.00	26-00-08	10-01-04	2 X 4	2 X 4	00-00-00 01-03-08	02-11-15 01-07-11	834.66 534.00		
	1	T08 HIP	10.00 0.00	27-08-00	11-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	146.67 91.00		
	1	T08S HIP	10.00 0.00	26-00-08	11-01-04	2 X 4	2 X 4	00-00-00 01-03-08	02-11-15 01-07-11	141.25 87.83		
	1	T09 HIP GIRDER	10.00	27-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	331.38		
	2 Ply		0.00					01-03-08	01-07-11	210.66		
	1	T10 HIP	10.00 0.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	126.76 82.50		
	1	T11 HIP	10.00 0.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	134.07 85.83		
	1	T12 HIP	10.00 0.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	136.54 87.17		
	1	T14S HIP	10.00 0.00	26-00-08	09-01-04	2 X 4	2 X 4	00-00-00 01-03-08	02-11-15 01-07-11	138.99 88.83		
	1	T16 COMMON	10.00 0.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	49.08 32.33		
	4	T19 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	236.36 152.68		
	3	T21 ROOF	4.00	05-07-00	02-05-12	2 X 4	2 X 4	01-03-08	00-03-15	59.01		
			0.00					00-10-08	02-02-04	38.01		

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Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755	LAYOUT ID: 288737	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: B-OPT.COFF.	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	6	T21A MONOPITCH	4.00 0.00	04-00-08	01-11-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-08-02	92.88 64.02		
	1 2 Ply	T80A HIP GIRDER	10.00 0.00	27-08-00	08-01-04	2 X 4	2 X 8	00-00-00 01-03-08	01-07-11 01-07-11	385.08 246.66		
	1 3 Ply	T81 COMMON	12.00 0.00	16-09-00	09-10-08	2 X 6	2 X 6	00-00-00 00-00-00	01-06-00 01-06-00	362.55 222.99		
	1 2 Ply	T82 PIGGYBACK	10.00 0.00	27-08-00	10-01-04	2 X 4	2 X 8	01-03-08 01-03-08	01-07-11 01-07-11	364.84 232.00		
	3	T83 SCISSORS	10.00 4.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	150.54 96.99		
	1 2 Ply	T84 COMMON	10.00 0.00	12-03-00	06-08-15	2 X 4	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11	135.62 88.66		
	4	T85 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	199.64 127.32		
	1	G85 COMMON	12.00 0.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	55.75 36.83		
	1	G101 COMMON	10.00 0.00	11-06-00	06-05-03	2 X 4	2 X 4	00-00-00 00-00-00	01-07-11 01-07-11	52.04 34.33		
	4	PB01 PIGGYBACK	10.00 0.00	06-01-01	02-11-04	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	78.64 51.32		
	2	PB02 PIGGYBACK	10.00 0.00	06-01-01	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	38.08 28.00		
	1 2 Ply	PB03 PIGGYBACK	10.00 0.00	06-01-01	02-08-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	42.18 30.00		
	10	J01 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-02-08 00-00-00	01-02-00 04-01-04	166.90 106.70		
	6	J01SA JACK-OPEN	6.00 0.00	05-08-08	04-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-03-00 03-01-04	113.28 84.00		

TOTAL # TRUSS= 79.00

TOTAL BFT OF ALL TRUSSES=

3722.68 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5810.21 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	HGUS28-3	
10	Hangers	LJS26DS	
6	Hangers	LUS24	

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Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755	LAYOUT ID: 288737	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: B-OPT.COFF.	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	LUS26-2	
1	Hangers	TBE6	

TOTAL # ITEMS= 24.00

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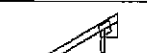
Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288742 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T01Z HIP GIRDER	10.00	28-11-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	327.74		
	2 Ply		0.00					01-03-08	01-07-11	206.68		
	2	T02 HIP	10.00	27-08-00	05-01-04	2 X 4	2 X 4	01-03-08	01-07-11	226.26		
			0.00					01-03-08	01-07-11	144.66		
	2	T03 HIP	10.00	27-08-00	06-01-04	2 X 4	2 X 4	01-03-08	01-07-11	238.44		
			0.00					01-03-08	01-07-11	149.34		
	2	T04 HIP	10.00	27-08-00	07-01-04	2 X 4	2 X 4	01-03-08	01-07-11	244.18		
			0.00					01-03-08	01-07-11	153.00		
	1	T05 HIP	10.00	27-08-00	08-01-04	2 X 4	2 X 4	01-03-08	01-07-11	127.75		
			0.00					01-03-08	01-07-11	81.33		
	1	T06 HIP	10.00	27-08-00	09-01-04	2 X 4	2 X 4	01-03-08	01-07-11	138.57		
			0.00					01-03-08	01-07-11	87.17		
	2	T07 HIP	10.00	27-08-00	10-01-04	2 X 4	2 X 4	01-03-08	01-07-11	288.92		
			0.00					01-03-08	01-07-11	183.34		
	6	T07AS PIGGYBACK	10.00	26-00-08	10-01-04	2 X 4	2 X 4	00-00-00	02-11-15	834.66		
			0.00					01-03-08	01-07-11	534.00		
	1	T08 HIP	10.00	27-08-00	11-01-04	2 X 4	2 X 4	01-03-08	01-07-11	146.67		
			0.00					01-03-08	01-07-11	91.00		
	1	T08S HIP	10.00	26-00-08	11-01-04	2 X 4	2 X 4	00-00-00	02-11-15	141.25		
			0.00					01-03-08	01-07-11	87.83		
	1	T090 HIP GIRDER	10.00	27-08-00	04-01-04	2 X 4	2 X 6	01-03-08	01-07-11	256.68		
	2 Ply		0.00					01-03-08	01-07-11	160.66		
	1	T16 COMMON	10.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08	01-07-11	49.08		
			0.00					01-03-08	01-07-11	32.33		
	4	T19 COMMON	10.00	12-03-00	06-08-15	2 X 4	2 X 4	01-03-08	01-07-11	236.36		
			0.00					01-03-08	01-07-11	152.68		
	3	T21 ROOF	4.00	05-07-00	02-05-12	2 X 4	2 X 4	01-03-08	00-03-15	59.01		
			0.00					00-10-08	02-02-04	38.01		
	6	T21A MONOPITCH	4.00	04-00-08	01-11-10	2 X 4	2 X 4	01-03-08	00-03-15	92.88		
			0.00					00-00-00	01-08-02	64.02		
	1	T6A HIP	10.00	26-00-08	09-01-04	2 X 4	2 X 4	00-00-00	02-11-15	131.65		
			0.00					01-03-08	01-07-11	83.00		
	1	T80 HIP GIRDER	10.00	27-08-00	08-01-04	2 X 4	2 X 8	01-03-08	01-07-11	336.74		
	2 Ply		0.00					01-03-08	01-07-11	214.66		
	1	T81 COMMON	12.00	16-09-00	09-10-08	2 X 6	2 X 6	00-00-00	01-06-00	362.55		
	3 Ply		0.00					00-00-00	01-06-00	222.99		

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Delivery Shiplist

DATE	03/02/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288742 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T82 PIGGYBACK	10.00	27-08-00	10-01-04	2 X 4	2 X 8	01-03-08	01-07-11	364.84		
	2 Ply		0.00					01-03-08	01-07-11	232.00		
	3	T83 SCISSORS	10.00	11-00-00	06-02-11	2 X 4	2 X 4	01-03-08	01-07-11	150.54		
			4.00					01-03-08	01-07-11	96.99		
	1	T84 COMMON	10.00	12-03-00	06-08-15	2 X 4	2 X 6	01-03-08	01-07-11	135.62		
	2 Ply		0.00					01-03-08	01-07-11	88.66		
	4	T85 COMMON	12.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08	01-10-08	199.64		
			0.00					01-03-08	01-10-08	127.32		
	1	G85 COMMON	12.00	10-04-00	07-00-08	2 X 4	2 X 4	01-03-08	01-10-08	55.75		
			0.00					01-03-08	01-10-08	36.83		
	1	G101 COMMON	10.00	11-06-00	06-05-03	2 X 4	2 X 4	00-00-00	01-07-11	52.04		
			0.00					00-00-00	01-07-11	34.33		
	4	PB01 PIGGYBACK	10.00	06-01-01	02-11-04	2 X 4	2 X 4	00-00-00	00-04-13	78.64		
			0.00					00-00-00	00-04-13	51.32		
	2	PB02 PIGGYBACK	10.00	06-01-01	01-10-08	2 X 4	2 X 4	00-00-00	00-04-13	38.08		
			0.00					00-00-00	00-04-13	28.00		
	1	PB03 PIGGYBACK	10.00	06-01-01	02-08-08	2 X 4	2 X 4	00-00-00	00-04-13	42.18		
	2 Ply		0.00					00-00-00	00-04-13	30.00		
	10	J01 JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-02-08	01-02-00	166.90		
			0.00					00-00-00	04-01-04	106.70		
	6	J01A JACK-OPEN	6.00	05-08-08	04-01-04	2 X 4	2 X 4	00-00-00	01-03-00	89.04		
			0.00					00-00-00	04-01-04	55.98		

TOTAL # TRUSS= 79.00

TOTAL BFT OF ALL TRUSSES=

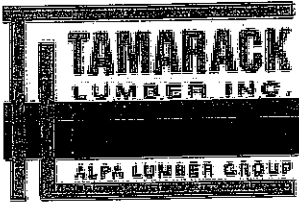
3574.83 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5612.66 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-2	
2	Hangers	HGUS28-3	
10	Hangers	LJS26DS	
6	Hangers	LUS24	
4	Hangers	LUS26-2	
1	Hangers	TBE6	

TOTAL # ITEMS= 24.00

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Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

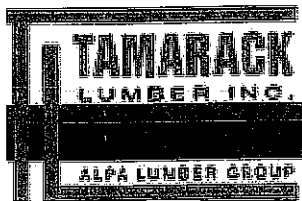
JOB TRACK: 44755	LAYOUT ID: 288739	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: C-OPT.COFF	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T29S HALF HIP	8.00 0.00	16-05-00	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	70.72 43.67		
	1	T31 HIP	8.00 0.00	27-08-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	124.20 78.00		
	1	T32 HIP	8.00 0.00	27-08-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	126.07 78.33		
	5	T33 COMMON	8.00 0.00	27-08-00	10-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	588.20 361.70		
	7	T33S COMMON	8.00 0.00	26-00-08	10-07-08	2 X 4	2 X 4	00-00-00 01-03-08	02-05-13 01-04-13	783.79 488.81		
	1 2 Ply	T34 COMMON	8.00 0.00	27-10-00	10-07-08	2 X 4	2 X 6	01-01-08 01-03-08	01-03-08 01-04-13	305.22 189.34		
	1 2 Ply	T35 HIP GIRDER	8.00 0.00	27-08-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	305.78 189.34		
	1	T36 HIP	8.00 0.00	27-08-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	126.27 81.67		
	1	T37 HIP	8.00 0.00	27-08-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	138.05 88.50		
	1	T38 HIP	8.00 0.00	27-08-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	139.85 88.17		
	1 2 Ply	T39 ROOF	8.00 0.00	27-08-00	10-07-08	2 X 4	2 X 8	01-03-08 01-03-08	01-04-13 01-04-13	349.58 226.00		
	1	T40 ROOF	8.00 0.00	26-00-08	10-07-08	2 X 4	2 X 4	00-00-00 01-03-08	02-05-13 01-04-13	121.02 76.67		
	1 3 Ply	T41 FLAT GIRDER	0.00 0.00	17-05-00	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	259.83 165.00		
	1 2 Ply	T42 HIP GIRDER	8.00 0.00	28-11-08	06-07-13	2 X 4	2 X 6	00-00-00 01-03-08	00-06-08 01-04-13	305.70 190.00		
	5	T43 ROOF	4.00 0.00	04-07-00	02-01-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 01-10-04	84.75 56.65		
	4	T44 MONOPITCH	4.00 0.00	03-08-00	01-08-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-06-10	52.32 33.32		
	1 2 Ply	T400 JACK-CLOSED	8.00 0.00	09-00-08	06-07-13	2 X 4	2 X 6	00-00-00 00-00-00	00-07-08 06-07-13	98.22 62.34		
	2	C03 CORNER	5.66 0.00	10-00-02	00-00-00	2 X 6		00-00-00 00-00-00	00-04-13 00-06-13	40.42 24.00		

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Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK:44755	LAYOUT ID: 288739	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: C-OPT.COFF	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	8	C3 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	107.76 66.64		
	7	J03 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	124.81 74.69		
	6	J03A JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	01-04-13 04-03-13	132.12 91.98		
	4	J04 JACK-PARTIAL	8.00 0.00	07-00-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 05-03-13	89.76 59.32		

TOTAL # TRUSS= 68.00

TOTAL BFT OF ALL TRUSSES=

2814.14 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4474.44 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	HGUS28-3	
11	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	TBE6	

TOTAL # ITEMS= 20.00

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Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 288746 LOCATION: BRADFORD
 BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL SUB-BUILDER:
 MODEL: S38-8C ELEVATION: C STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T29S HALF HIP	8.00 0.00	16-05-00	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	70.72 43.67		
	1	T30 HIP	8.00 0.00	27-08-00	06-07-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	117.83 75.50		
	2	T31 HIP	8.00 0.00	27-08-00	07-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	248.40 156.00		
	2	T32 HIP	8.00 0.00	27-08-00	09-03-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	252.14 156.66		
	5	T33 COMMON	8.00 0.00	27-08-00	10-07-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	588.20 361.70		
	8	T33S COMMON	8.00 0.00	26-00-08	10-07-08	2 X 4	2 X 4	00-00-00 01-03-08	02-05-13 01-04-13	895.76 558.64		
	2 2 Ply	T34 COMMON	8.00 0.00	27-10-00	10-07-08	2 X 4	2 X 6	01-01-08 01-03-08	01-03-08 01-04-13	610.44 378.68		
	1 3 Ply	T41 FLAT GIRDER	0.00 0.00	17-05-00	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	259.83 165.00		
	1 2 Ply	T42 HIP GIRDER	8.00 0.00	28-11-08	06-07-13	2 X 4	2 X 6	00-00-00 01-03-08	00-06-08 01-04-13	305.70 190.00		
	5	T43 ROOF	4.00 0.00	04-07-00	02-01-12	2 X 4	2 X 4	01-03-08 00-10-08	00-03-15 01-10-04	84.75 56.65		
	4	T44 MONOPITCH	4.00 0.00	03-08-00	01-08-15	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-06-10	52.32 33.32		
	1 2 Ply	T290 HIP GIRDER	8.00 0.00	27-08-00	05-03-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	262.88 158.34		
	1 2 Ply	T400 JACK-CLOSED	8.00 0.00	09-00-08	06-07-13	2 X 4	2 X 6	00-00-00 00-00-00	00-07-08 06-07-13	98.22 62.34		
	13	J03 JACK-OPEN	8.00 0.00	05-10-08	05-03-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 05-03-13	231.79 138.71		
	4	J04 JACK-PARTIAL	8.00 0.00	07-00-08	05-03-13	2 X 4	2 X 4	00-00-00 00-00-00	00-07-08 05-03-13	89.76 59.32		
	2	C03 CORNER	5.66 0.00	10-00-02	00-00-00	2 X 6		00-00-00 00-00-00	00-04-13 00-06-13	40.42 24.00		
	8	C3 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4	2 X 4	01-03-08 04-01-01	01-04-13 00-03-08	107.76 66.64		

TOTAL # TRUSS= 68.00

TOTAL BFT OF ALL TRUSSES=

2685.17 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4316.92 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
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SITE COPY



Delivery Shiplist

DATE	03/05/18
SALES REP	Mario

JOB TRACK:44755	LAYOUT ID: 288746	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON/ GREEN VAYL	SUB-BUILDER:	
MODEL: S38-8C	ELEVATION: C STD	

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	HGUS28-3	
11	Hangers	LJS26DS	
4	Hangers	LUS24	
1	Hangers	TBE6	

TOTAL # ITEMS= 20.00

SITE COPY

JOB NAME 288713	TRUSS NAME T01	QUANTITY 1	PLY 2	JOB DESC. 44755 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

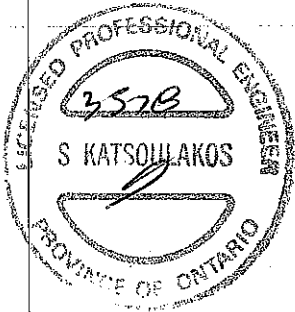
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 11:25:24 2018 Page 2
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWWh	MT20	5.0	6.0	3.25	1.50
W	BMVHp	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

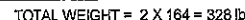
HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 128.3 lbs FACTORED DOWN AT 14-3-4, AND 126.3 lbs FACTORED DOWN AT 15-10-12 ON TOP CHORD, AND 2288.0 lbs FACTORED DOWN AT 1-1-8, 1537.7 lbs FACTORED DOWN AT 12-4-8, 67.8 lbs FACTORED DOWN AT 14-3-4, AND 67.8 lbs FACTORED DOWN AT 15-10-12, AND 1688.0 lbs FACTORED DOWN AT 17-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 11945
 STRUCTURAL
 COMPONENT ONLY

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Version 8.200 5 Jan 6 2018 MITek Industries, Inc. Mon Mar 5 11:29:04 2018 Page 1
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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288737	T01Z	1	2	TRUSS DESC.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
V	BMWW+H	MT20	5.0	6.0	3.25	1.25
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 126.3 lbs FACTORED DOWN AT 14-3-4, AND 126.3 lbs FACTORED DOWN AT 15-10-12 ON TOP CHORD, AND 2298.0 lbs FACTORED DOWN AT 1-1-8, 1676.2 lbs FACTORED DOWN AT 12-4-8, 67.8 lbs FACTORED DOWN AT 14-3-4, AND 67.8 lbs FACTORED DOWN AT 15-10-12, AND 1459.1 lbs FACTORED DOWN AT 17-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



1022
 DWG NO. TAM 11972-18
 STRUCTURAL
 COMPONENT ONLY

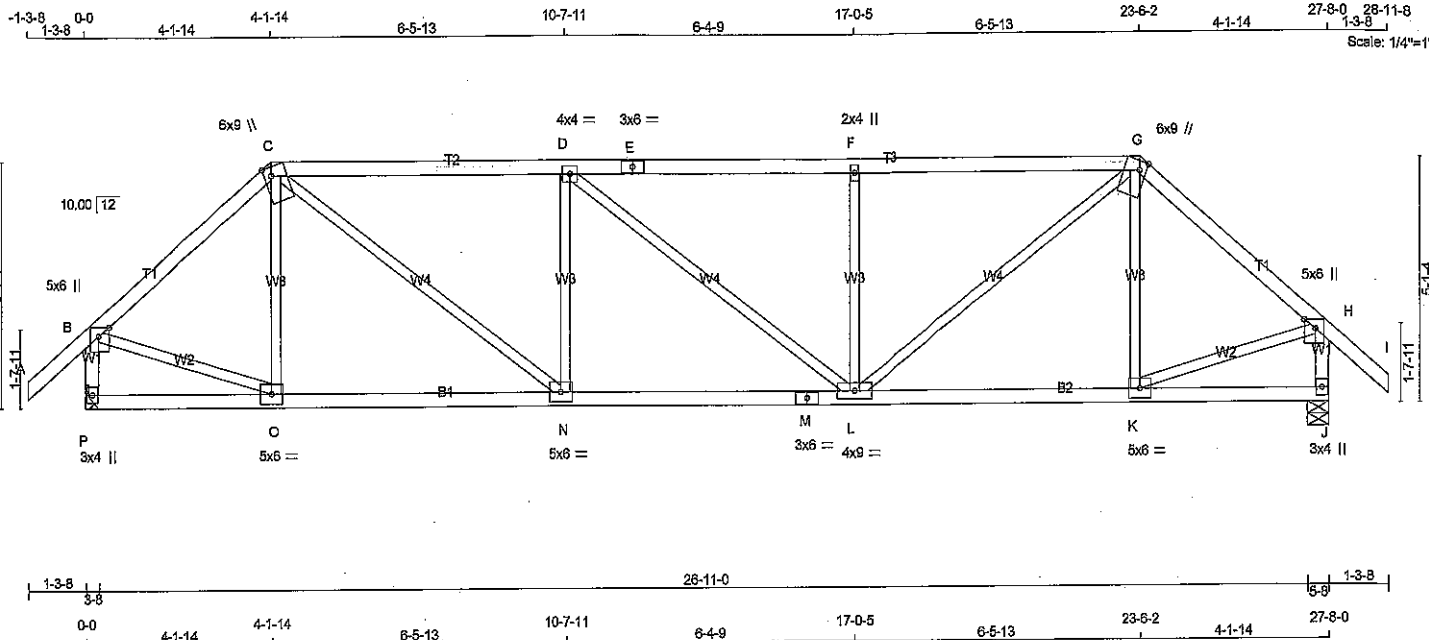
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T02	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 11:25:24 2018 Page 1

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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - E	2x4	DRY	No.2 SPF
E - G	2x4	DRY	No.2 SPF
G - I	2x4	DRY	No.2 SPF
P - B	2x4	DRY	No.2 SPF
J - H	2x4	DRY	No.2 SPF
P - M	2x4	DRY	No.2 SPF
M - J	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	5.0	6.0 Edge 2.75
C	TTWW+m	MT20	6.0	8.0 Edge 1.75
D	TMWW-t	MT20	4.0	4.0
E	TS-t	MT20	3.0	6.0
F	TMW+w	MT20	2.0	4.0
G	TTWW+m	MT20	6.0	9.0 Edge 1.75
H	TMVW+p	MT20	5.0	6.0 Edge 2.75
J	BMV1+p	MT20	3.0	4.0
K, N, O				
K	BMWW-t	MT20	5.0	6.0
L	BMWWW-t	MT20	4.0	8.0
M	BS-t	MT20	3.0	6.0
P	BMV1+p	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1977	0	1977	0	0	HANGER BY OTHERS
J	1977	0	1977	0	0	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	COMBINED	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J		988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM TO	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED WEBS FORCE (LBS)
A-B	0 / 47	-104.9 -104.9 0.14 (1)	10.00	O-C -256 / 101 0.10 (1)
B-C	-1795 / 0	-104.9 -104.9 0.39 (1)	4.57	C-N 0 / 1526 0.34 (1)
C-D	-2597 / 0	-104.9 -104.9 0.86 (1)	3.34	N-D -730 / 0 0.28 (1)
D-E	-2596 / 0	-104.9 -104.9 0.85 (1)	3.34	D-L -2 / 0 0.00 (1)
E-F	-2596 / 0	-104.9 -104.9 0.85 (1)	3.34	L-F -730 / 0 0.28 (1)
F-G	-2596 / 0	-104.9 -104.9 0.85 (1)	3.36	L-G 0 / 1524 0.34 (1)
G-H	-1795 / 0	-104.9 -104.9 0.39 (1)	4.57	K-G -256 / 101 0.10 (1)
H-I	0 / 47	-104.9 -104.9 0.14 (1)	10.00	B-O 0 / 1438 0.32 (1)
P-B	-1938 / 0	0.0 0.0 0.21 (1)	6.04	K-H 0 / 1438 0.32 (1)
J-H	-1938 / 0	0.0 0.0 0.21 (1)	6.04	
P-O	0 / 0	-27.5 -27.5 0.19 (3)	10.00	
O-N	0 / 1370	-27.5 -27.5 0.38 (2)	10.00	
N-M	0 / 2597	-27.5 -27.5 0.53 (1)	10.00	
M-L	0 / 2597	-27.5 -27.5 0.53 (1)	10.00	
L-K	0 / 1370	-27.5 -27.5 0.38 (2)	10.00	
K-J	0 / 0	-27.5 -27.5 0.19 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	DL =	32.5 PSF
	DL =	3.0 PSF
BOT CH.	DL =	10.0 PSF
	DL =	7.0 PSF
TOTAL LOAD	=	52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI: TC=0.88/1.00 (C-D:1), BC=0.53/1.00 (L-N:1),
WB=0.34/1.00 (C-N:1), SS=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822
	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

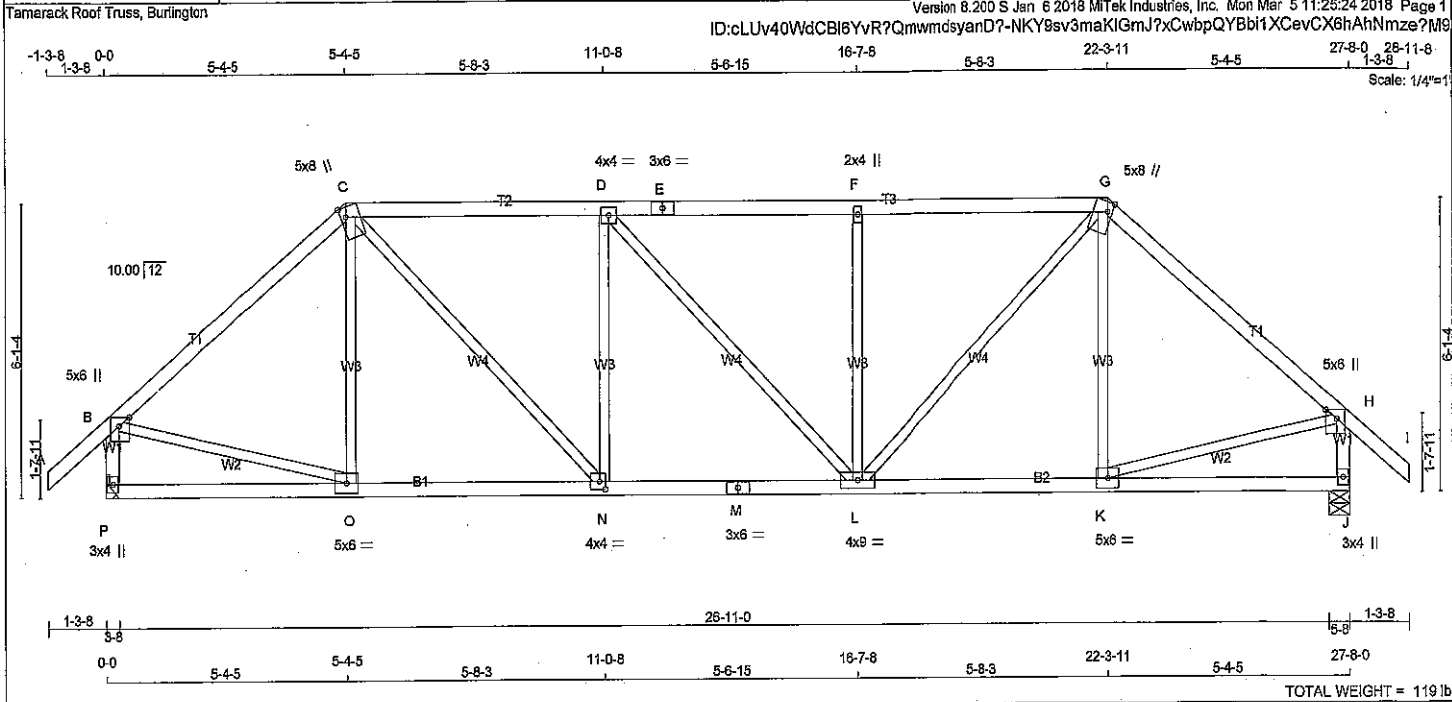
JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.64 (M) (INPUT = 1.00)



DWG NO. TAM 11946-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T03	1	1	TRUSS DESC.		



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - B	2x4	DRY No.2	SPF
B - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	8.0	Edge	1.25
D	TMWW-l	MT20	4.0	4.0		
E	TS-l	MT20	3.0	6.0		
F	TMVW+m	MT20	2.0	4.0		
G	TTWW+m	MT20	5.0	8.0	Edge	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-l	MT20	5.0	6.0		
L	BMWW-l	MT20	4.0	9.0		
M	BS-l	MT20	3.0	6.0		
N	BMWW-l	MT20	4.0	4.0	2.00	1.50
O	BMWW-l	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
P	1977	0	1977	0
J	1977	0	1977	0

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		FROM	TO	
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O-C	-159 / 154	0.09 (1)	
B-C	-1807 / 0	-104.9	-104.9	0.69 (1)	4.17	C-N	0 / 1105	0.25 (1)	
C-D	-2157 / 0	-104.9	-104.9	0.58 (1)	3.99	N-D	-639 / 0	0.37 (1)	
D-E	-2158 / 0	-104.9	-104.9	0.58 (1)	3.99	D-L	-2 / 0	0.00 (1)	
E-F	-2158 / 0	-104.9	-104.9	0.58 (1)	3.99	L-F	-636 / 0	0.37 (1)	
F-G	-2158 / 0	-104.9	-104.9	0.58 (1)	4.00	L-G	0 / 1103	0.25 (1)	
G-H	-1808 / 0	-104.9	-104.9	0.69 (1)	4.17	K-G	-158 / 154	0.09 (1)	
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	B-O	0 / 1425	0.32 (1)	
P-B	-1918 / 0	0.0	0.0	0.20 (1)	6.07	K-H	0 / 1425	0.32 (1)	
J-H	-1918 / 0	0.0	0.0	0.20 (1)	6.07				
P-O	0 / 0	-27.5	-27.5	0.20 (3)	10.00				
O-N	0 / 1383	-27.5	-27.5	0.35 (2)	10.00				
N-M	0 / 2157	-27.5	-27.5	0.43 (1)	10.00				
M-L	0 / 2157	-27.5	-27.5	0.43 (1)	10.00				
L-K	0 / 1383	-27.5	-27.5	0.35 (2)	10.00				
K-J	0 / 0	-27.5	-27.5	0.20 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.69/1.00 (G-H:1), BC=0.43/1.00 (L-N:1), WB=0.37/1.00 (D-N:1), SS=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

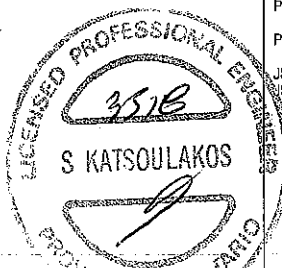
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.65 (M) (INPUT = 1.00)

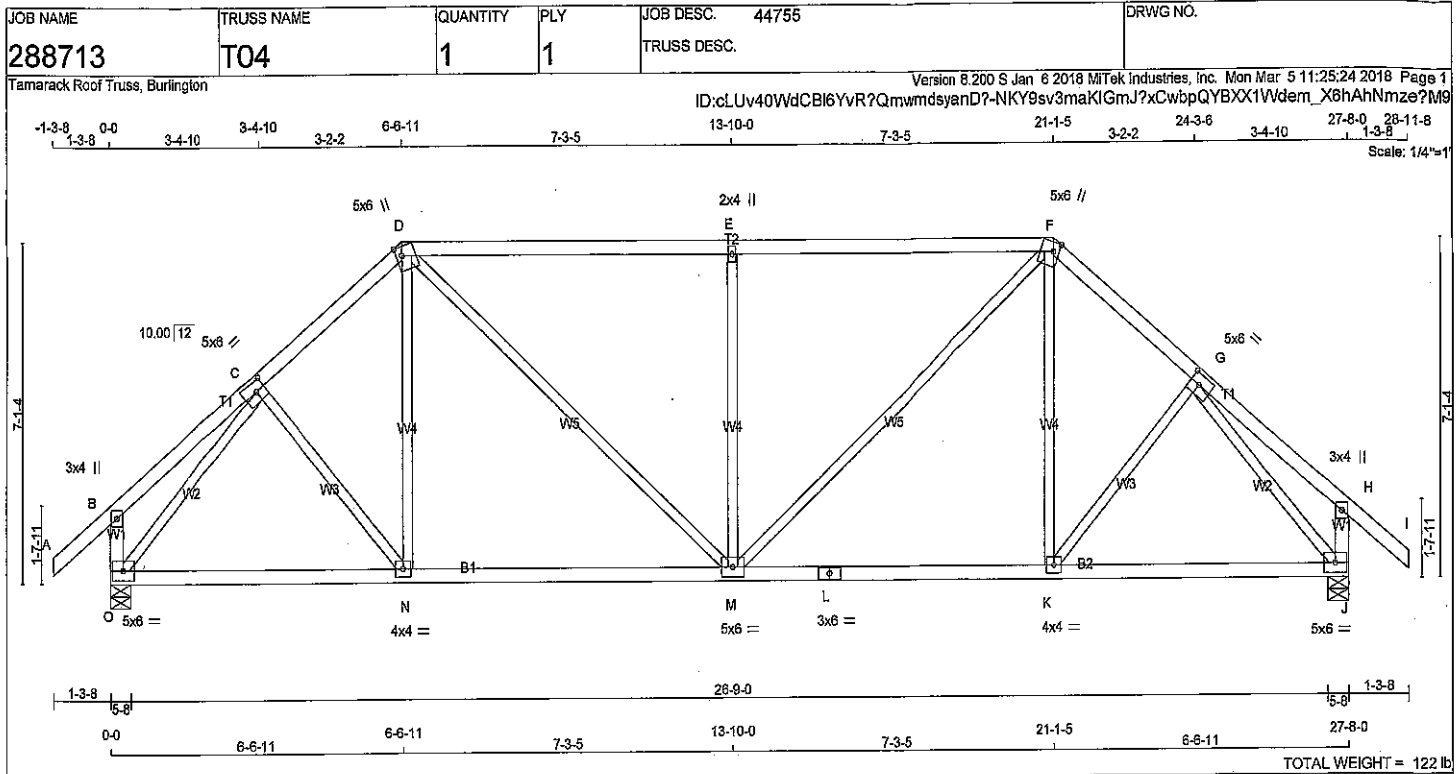
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DRWG NO. TAM 11947
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY 1650F 1.5E	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
O	1977	0	1977	0	5-8	5-8
J	1977	0	1977	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
O	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	C-N	0 / 135	0.03 (3)
B-C	0 / 23	-104.9 -104.9	0.16 (1)	10.00	N-D	0 / 268	0.06 (3)
C-D	-1810 / 0	-104.9 -104.9	0.21 (1)	4.77	D-M	0 / 825	0.19 (1)
D-E	-1974 / 0	-104.9 -104.9	0.85 (1)	3.77	M-E	-937 / 0	0.82 (1)
E-F	-1974 / 0	-104.9 -104.9	0.95 (1)	3.77	M-F	0 / 825	0.19 (1)
F-G	-1810 / 0	-104.9 -104.9	0.21 (1)	4.77	K-F	0 / 268	0.06 (3)
G-H	0 / 23	-104.9 -104.9	0.16 (1)	10.00	K-G	0 / 135	0.03 (3)
H-I	0 / 47	-104.9 -104.9	0.14 (1)	10.00	O-C	-2092 / 0	0.80 (1)
O-B	-280 / 0	0.0	0.03 (1)	7.81	G-J	-2092 / 0	0.80 (1)
J-H	-280 / 0	0.0	0.03 (1)	7.81			
O-N	0 / 1317	-27.5	-27.5 0.45 (2)	10.00			
N-M	0 / 1371	-27.5	-27.5 0.47 (2)	10.00			
M-L	0 / 1371	-27.5	-27.5 0.47 (2)	10.00			
L-K	0 / 1371	-27.5	-27.5 0.47 (2)	10.00			
K-J	0 / 1317	-27.5	-27.5 0.45 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.09")
ALLOWABLE DEFL.(TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.15")

CSI: TC=0.95/1.00 (E-F:1), BC=0.47/1.00 (K-M:2), WB=0.90/1.00 (G-J:1), SSI=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1657 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

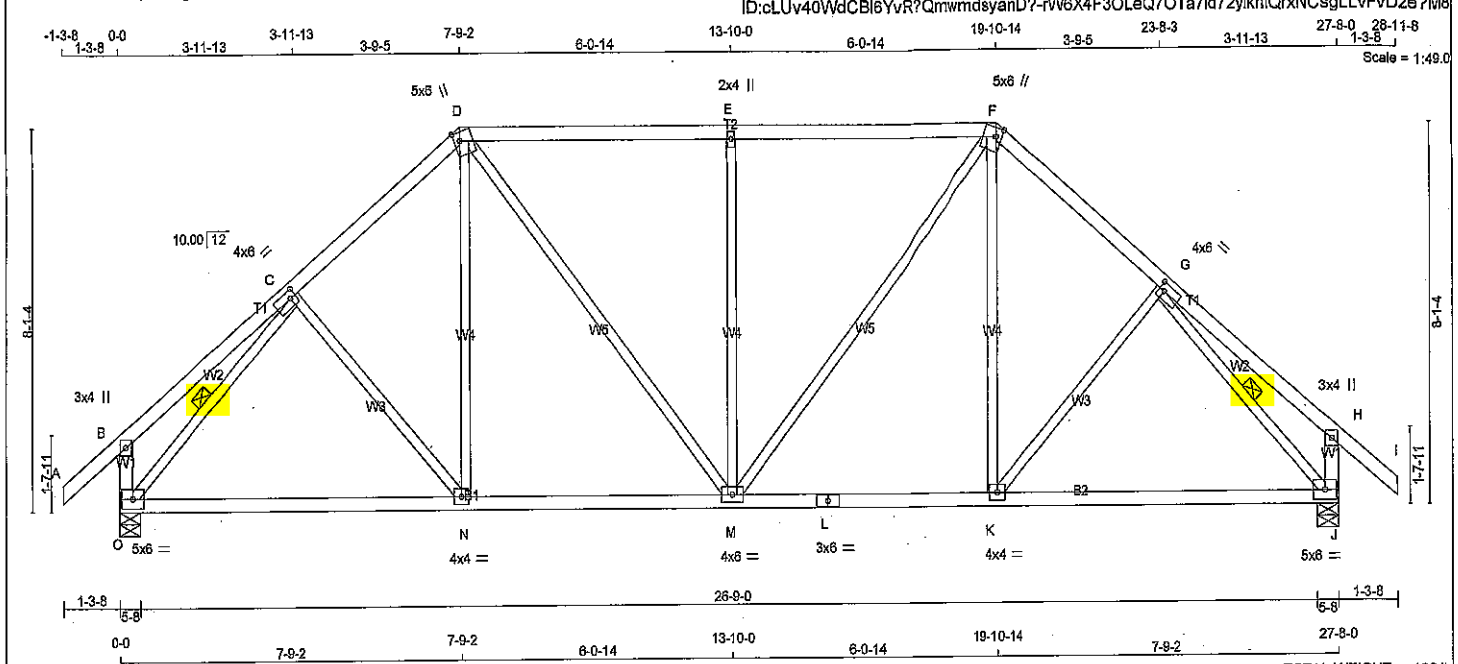
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.51 (C) (INPUT = 1.00)



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DWG NO. TAM 11948-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	6.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	6.0	2.00	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW-I	MT20	5.0	6.0		
K	BMWW-I	MT20	4.0	4.0		
L	BS-I	MT20	3.0	6.0		
M	BMWWW-I	MT20	4.0	6.0		
N	BMWW-I	MT20	4.0	4.0		
O	BMVW-I	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1977	0	1977	0	0	5-8	5-8
J	1977	0	1977	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
C	COMBINED	SNOW	LIVE
O	1550	988 / 0	277 / 0
J	1550	988 / 0	277 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (PLF)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (PLF)
FR-TO					FR-TO		
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	C-N	-36 / 106
B-C	0 / 29	-104.9	-104.9	0.24 (1)	10.00	N-D	0 / 344
C-D	-1768 / 0	-104.9	-104.9	0.23 (1)	4.82	D-M	0 / 572
D-E	-1686 / 0	-104.9	-104.9	0.54 (1)	4.46	M-E	-780 / 0
E-F	-1686 / 0	-104.9	-104.9	0.54 (1)	4.46	M-F	0 / 572
F-G	-1768 / 0	-104.9	-104.9	0.23 (1)	4.82	K-F	0 / 344
G-H	0 / 29	-104.9	-104.9	0.24 (1)	10.00	K-G	-36 / 106
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O-C	-2087 / 0
O-B	-302 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2087 / 0
J-H	-302 / 0	0.0	0.0	0.03 (1)	7.81		
O-N	0 / 1355	-27.5	-27.5	0.52 (2)	10.00		
N-M	0 / 1334	-27.5	-27.5	0.52 (2)	10.00		
M-L	0 / 1334	-27.5	-27.5	0.52 (2)	10.00		
L-K	0 / 1334	-27.5	-27.5	0.52 (2)	10.00		
K-J	0 / 1355	-27.5	-27.5	0.52 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL (LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL (LL) = $L/999$ (0.15")
ALLOWABLE DEFL (TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL (TL) = $L/999$ (0.25")

CSI: TC=0.54/1.00 (D-E:1), BC=0.52/1.00 (K-M:2), WB=0.99/1.00 (E-M:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1657	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

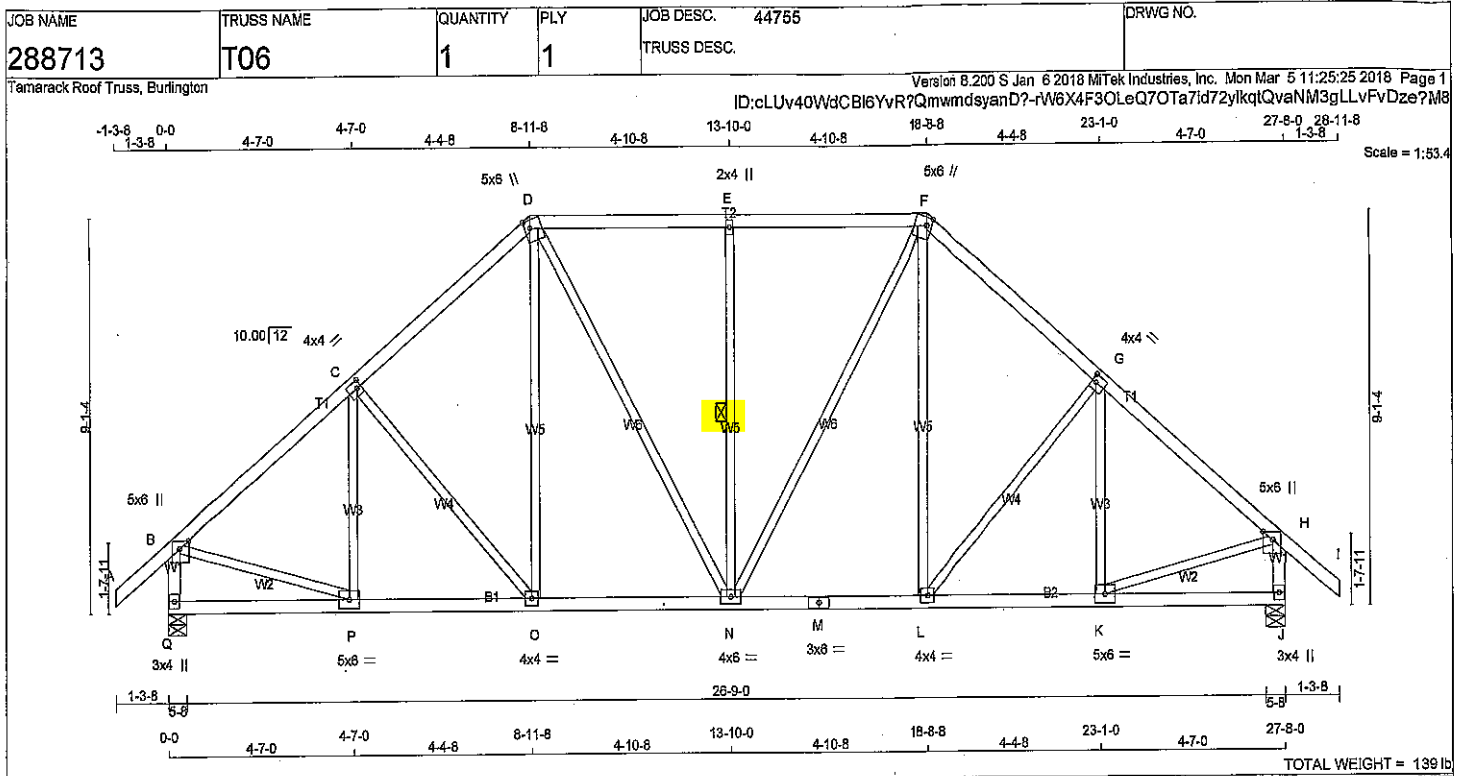
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (G) (INPUT = 0.90)
JSI METAL = 0.51 (G) (INPUT = 1.00)



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DWG NO. TAM 11949-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
Q - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
Q - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge	2.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQRD BRG
Q	1977	0	1977	0	0	5-8	5-8
J	1977	0	1977	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. MIN. SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 47	-104.9 -104.9 0.14 (1)	10.00	P-C -271 / 73 0.12 (1)
B-C	-1820 / 0	-104.9 -104.9 0.32 (1)	4.66	C-O -243 / 0 0.21 (1)
C-D	-1687 / 0	-104.9 -104.9 0.31 (1)	4.82	O-D 0 / 326 0.07 (2)
D-E	-1477 / 0	-104.9 -104.9 0.34 (1)	5.02	D-N 0 / 430 0.10 (1)
E-F	-1477 / 0	-104.9 -104.9 0.34 (1)	5.02	N-E -619 / 0 0.33 (1)
F-G	-1687 / 0	-104.9 -104.9 0.31 (1)	4.82	N-F 0 / 430 0.10 (1)
G-H	-1820 / 0	-104.9 -104.9 0.32 (1)	4.66	L-F 0 / 326 0.07 (2)
H-I	0 / 47	-104.9 -104.9 0.14 (1)	10.00	L-G -243 / 0 0.21 (1)
Q-B	-1924 / 0	0.0 0.0 0.20 (1)	6.06	K-G -271 / 73 0.12 (1)
J-H	-1924 / 0	0.0 0.0 0.20 (1)	6.06	B-P 0 / 1481 0.33 (1)
				K-H 0 / 1481 0.33 (1)
Q-P	0 / 0	-27.5 -27.5 0.13 (3)	10.00	
P-O	0 / 1425	-27.5 -27.5 0.28 (1)	10.00	
O-N	0 / 1268	-27.5 -27.5 0.27 (1)	10.00	
N-M	0 / 1268	-27.5 -27.5 0.27 (1)	10.00	
M-L	0 / 1268	-27.5 -27.5 0.27 (1)	10.00	
L-K	0 / 1425	-27.5 -27.5 0.29 (1)	10.00	
K-J	0 / 0	-27.5 -27.5 0.13 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.06")
ALLOWABLE DEFL. (TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.10")

CSI: TC=0.34/1.00 (D-E:1), BC=0.29/1.00 (K-L:1), WB=0.33/1.00 (E-N:1), SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

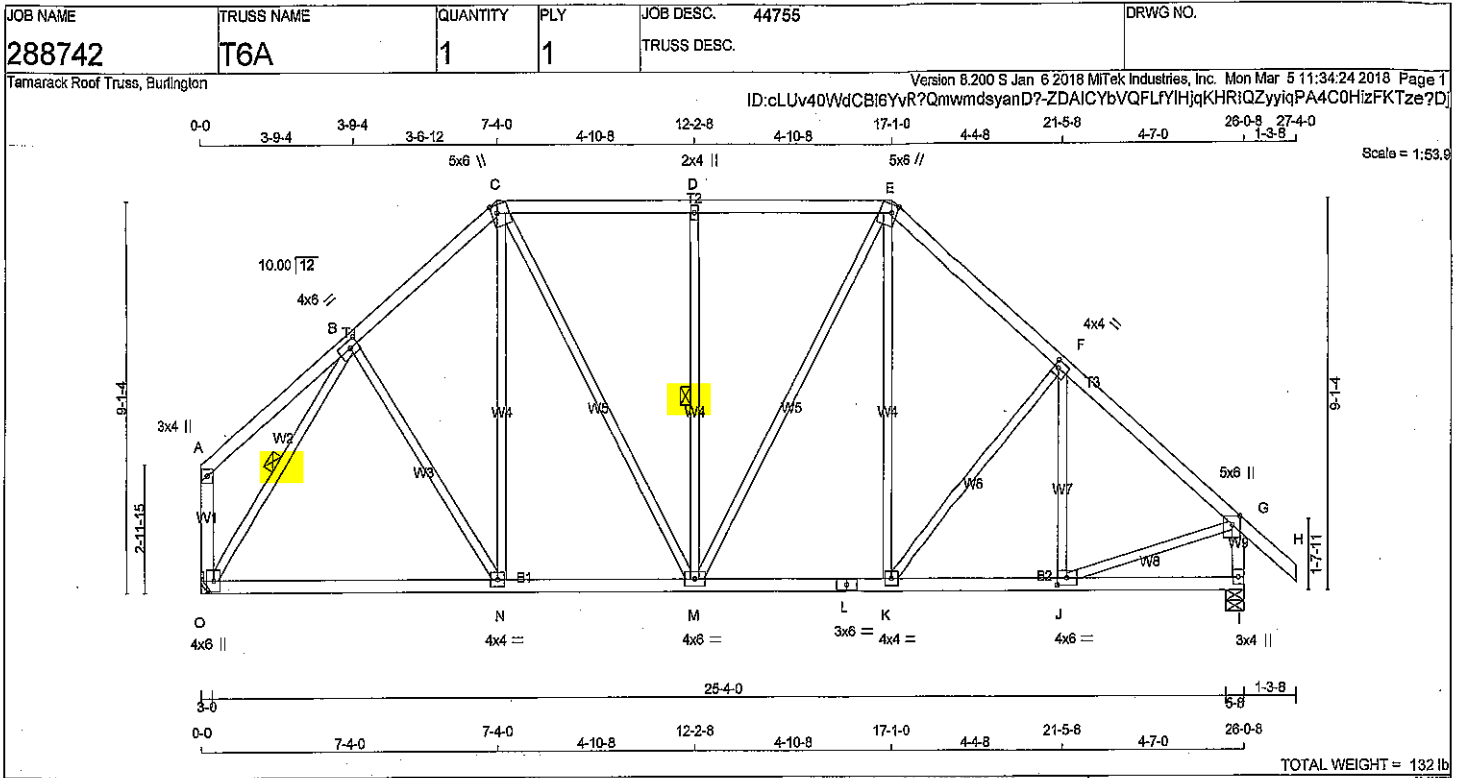
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.38 (M) (INPUT = 1.00)



DWG NO. TAM 11950-18
STRUCTURAL
COMPONENT ONLY

SITE COPY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
O - A	2x4	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - I	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	6.0	2.00	2.50
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW-w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.50	2.25
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	4.0	6.0	2.00	2.75
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-w	MT20	4.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1724	0	1724	0	0	5-8	5-8
I	1870	0	1870	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1367	848 / 0	260 / 0	0 / 0	0 / 0	260 / 0	0 / 0
I	1465	936 / 0	260 / 0	0 / 0	0 / 0	269 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, B-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 28	-104.9 -104.9	0.22 (1)	10.00	B-N	0 / 195	0.04 (2)
B-C	-1398 / 0	-104.9 -104.9	0.19 (1)	5.32	N-C	0 / 215	0.05 (3)
C-D	-1305 / 0	-104.9 -104.9	0.33 (1)	5.27	C-M	0 / 526	0.12 (1)
D-E	-1305 / 0	-104.9 -104.9	0.33 (1)	5.27	M-D	-621 / 0	0.34 (1)
E-F	-1545 / 0	-104.9 -104.9	0.30 (1)	4.99	M-E	0 / 300	0.07 (1)
F-G	-1894 / 0	-104.9 -104.9	0.31 (1)	4.80	X-E	0 / 349	0.08 (2)
G-H	0 / 47	-104.9 -104.9	0.14 (1)	10.00	K-F	-263 / 0	0.23 (1)
O-A	-150 / 0	0.0	0.02 (1)	7.81	J-F	-246 / 81	0.11 (1)
I-G	-1816 / 0	0.0	0.19 (1)	6.21	O-B	-1772 / 0	0.54 (1)
					J-G	0 / 1381	0.31 (1)
O-N	0 / 959	-27.5	-27.5	0.42 (2)	10.00		
N-M	0 / 1050	-27.5	-27.5	0.44 (2)	10.00		
M-L	0 / 1180	-27.5	-27.5	0.26 (1)	10.00		
L-K	0 / 1180	-27.5	-27.5	0.26 (1)	10.00		
K-J	0 / 1328	-27.5	-27.5	0.28 (1)	10.00		
J-I	0 / 0	-27.5	-27.5	0.13 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 3.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")

CALCULATED VERT. DEFL.(LL) = L/999 (0.12")

ALLOWABLE DEFL.(TL) = L/360 (0.87")

CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.33/1.00 (C-D:1), BC=0.44/1.00 (M-N:2), WB=0.54/1.00 (B-O:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1887
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)

JSI METAL= 0.42 (B) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 11905-18

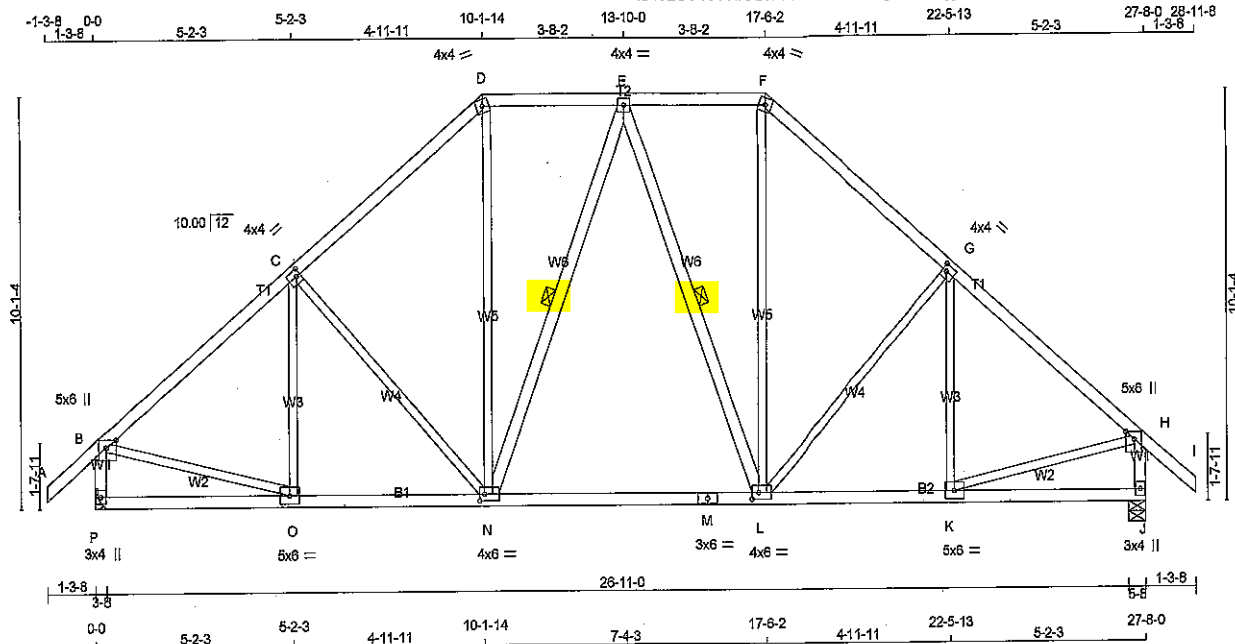
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T07	9	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 11:25:26 2018 Page 1

ID:cLUv40WdCB8YvR?QmwmDsynd7-JjgvHb416xZ_7c9JJLeHVzH_QqDR6nLqZ?foSfze?M7



TOTAL WEIGHT = 9 X 144 = 1300 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - E	2x4	DRY	No.2
E - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge 2.75	
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW+p	MT20	5.0	6.0	Edge 2.75	
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0		
L	BMVW-t	MT20	4.0	6.0	2.00	2.00
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	6.0	2.00	1.50
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	1977	0	1977	0	0	5-8	5-8
J	1977	0	1977	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	CS1 (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LBS)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 47	-104.9	-104.9	0.14 (1)	10.00	O-C	-219 / 104	0.12 (1)
B-C	-1834 / 0	-104.9	-104.9	0.42 (1)	4.53	C-N	-344 / 0	0.40 (1)
C-D	-1619 / 0	-104.9	-104.9	0.39 (1)	4.79	N-D	0 / 680	0.15 (1)
D-E	-1216 / 0	-104.9	-104.9	0.18 (1)	5.62	E-N	-243 / 0	0.14 (1)
E-F	-1216 / 0	-104.9	-104.9	0.18 (1)	5.62	E-L	-243 / 0	0.14 (1)
F-G	-1619 / 0	-104.9	-104.9	0.39 (1)	4.79	L-F	0 / 680	0.15 (1)
G-H	-1834 / 0	-104.9	-104.9	0.42 (1)	4.53	L-G	-344 / 0	0.40 (1)
H-I	0 / 47	-104.9	-104.9	0.14 (1)	10.00	K-G	-219 / 104	0.12 (1)
P-B	-1918 / 0	0.0	0.0	0.20 (1)	6.07	B-O	0 / 1484	0.33 (1)
J-H	-1918 / 0	0.0	0.0	0.20 (1)	6.07	K-H	0 / 1484	0.33 (1)
P-O	0 / 0	-27.5	-27.5	0.15 (2)	10.00			
O-N	0 / 1439	-27.5	-27.5	0.38 (2)	10.00			
N-M	0 / 1299	-27.5	-27.5	0.36 (2)	10.00			
M-L	0 / 1299	-27.5	-27.5	0.36 (2)	10.00			
L-K	0 / 1439	-27.5	-27.5	0.38 (2)	10.00			
K-J	0 / 0	-27.5	-27.5	0.15 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.82")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.10")
ALLOWABLE DEFL.(TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CSI: TC=0.42/1.00 (G-H:1), BC=0.38/1.00 (K-L:2), WB=0.40/1.00 (G-L:1), SS=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

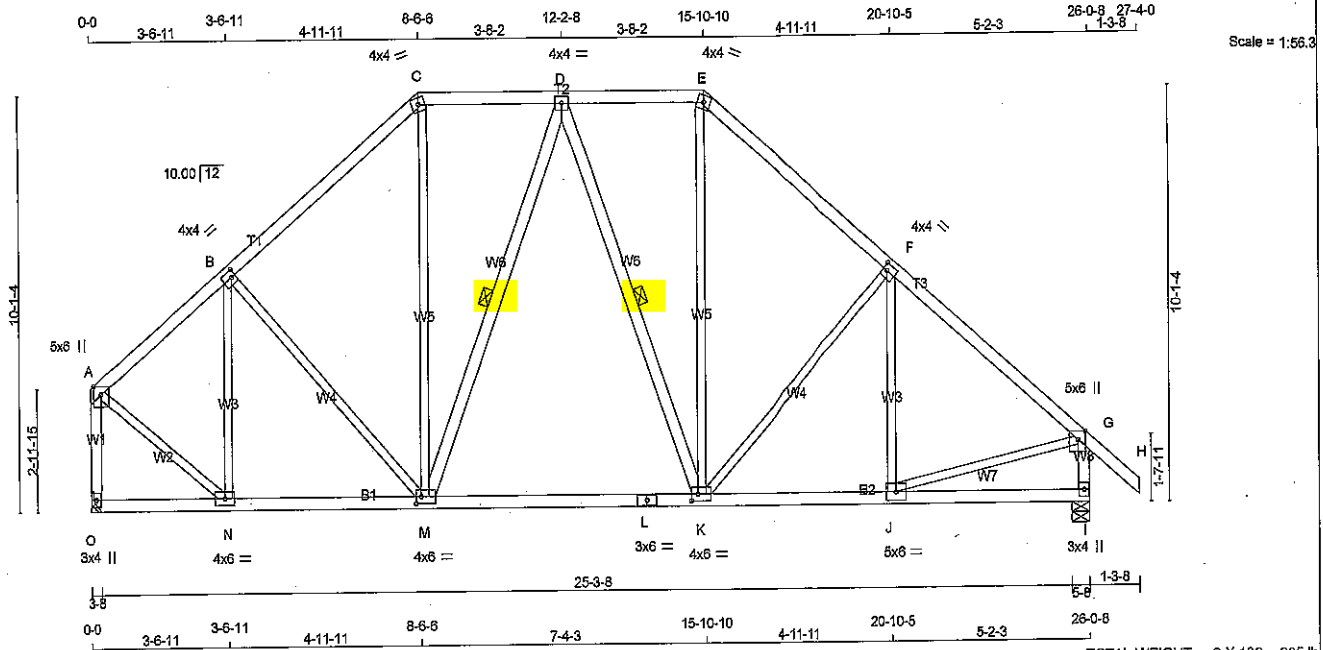
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.36 (H) (INPUT = 1.00)



DWG NO. TAM 11951-08
STRUCTURAL
COMPONENT ONLY

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - A	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
M - D	2x4	DRY	No.2
D - K	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.50	2.25
I	BMVt+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	6.0	2.00	2.00
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	6.0	2.00	1.50
N	BMVW-t	MT20	4.0	6.0		
O	BMVt+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1754	0	1754	0	0	HANGER BY OTHERS	
I	1896	0	1896	0	0	MIN. SEAT SIZE: 3-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1390	846 / 0	260 / 0	0 / 0	0 / 0	284 / 0	0 / 0
I	1486	938 / 0	260 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.84 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, D-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LENGTH (LC)	
FR-TO				FR-TO			
A-B	-1271 / 0	-104.9 -104.9	0.28 (1)	N-B	-840 / 0	0.35 (1)	
B-C	-1396 / 0	-104.9 -104.9	0.35 (1)	B-M	0 / 62	0.01 (2)	
C-D	-1051 / 0	-112.4 -112.4	0.19 (1)	M-C	0 / 498	0.11 (1)	
D-E	-1130 / 0	-112.4 -112.4	0.19 (1)	M-D	-378 / 0	0.22 (1)	
E-F	-1508 / 0	-104.9 -104.9	0.39 (1)	D-K	-147 / 17	0.09 (1)	
F-G	-1736 / 0	-104.9 -104.9	0.41 (1)	K-E	0 / 579	0.13 (1)	
G-H	0 / 147	-104.9 -104.9	0.14 (1)	K-F	-359 / 0	0.41 (1)	
O-A	-1714 / 0	0.0	0.0	J-F	-200 / 109	0.11 (1)	
I-G	-1834 / 0	0.0	0.0	A-N	0 / 1261	0.28 (1)	
				J-G	0 / 1407	0.32 (1)	
O-N	0 / 0	-27.5	-27.5	0.09 (3)			
N-M	0 / 1010	-27.5	-27.5	0.35 (2)			
M-L	0 / 1180	-27.5	-27.5	0.35 (2)			
L-K	0 / 1180	-27.5	-27.5	0.35 (2)			
K-J	0 / 1364	-27.5	-27.5	0.37 (2)			
J-I	0 / 0	-27.5	-27.5	0.15 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 10.00/12 AND -10.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CS1: TC=0.41/1.00 (F-G:1), BC=0.37/1.00 (J-K:2), WB=0.41/1.00 (F-K:1), SS=0.20/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

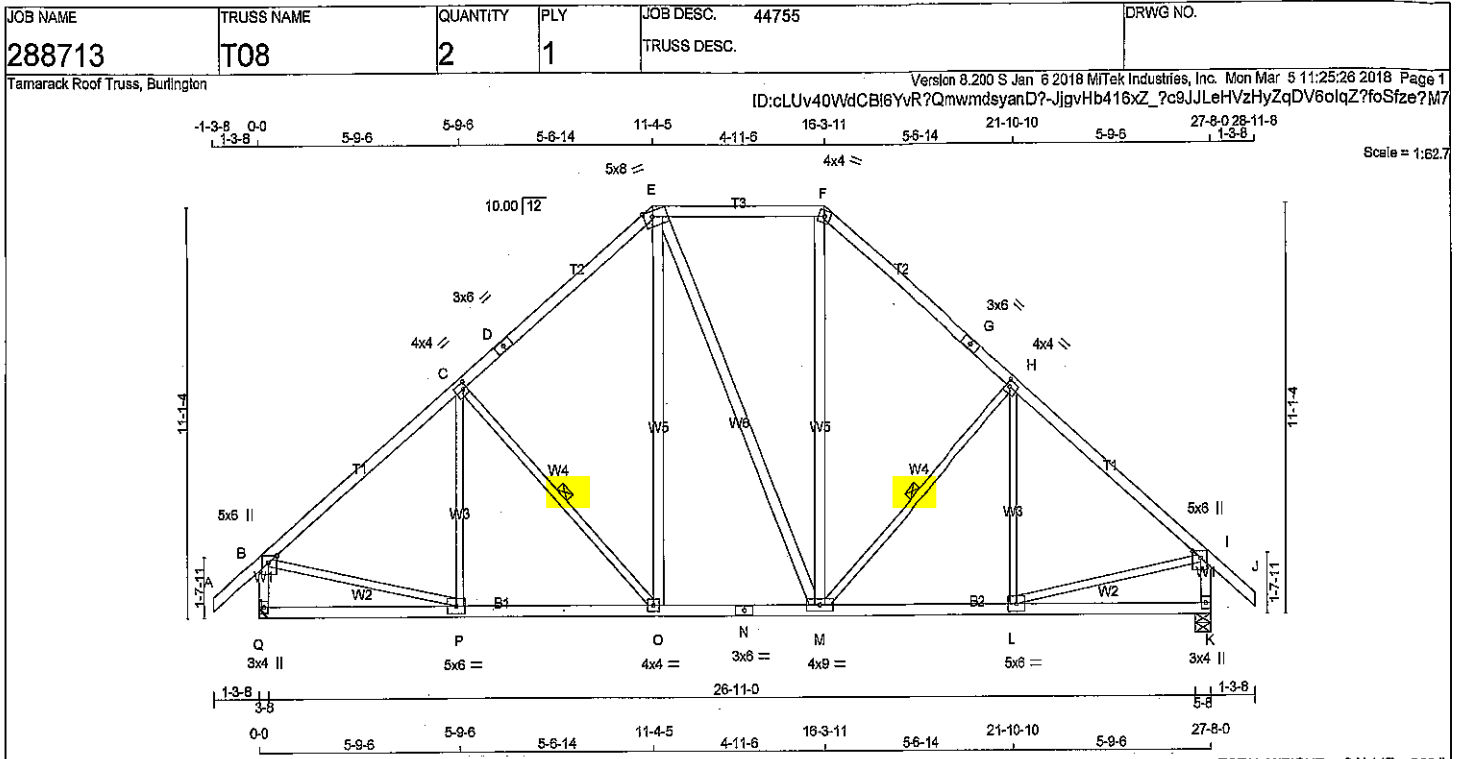
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.34 (J) (INPUT = 1.00)



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DWG NO. TAM 11973 -13
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
E - M	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TMVW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWV-m	MT20	5.0	8.0	Edge	3.00
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW+p	MT20	5.0	6.0	Edge	2.75
K	BMV1+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
Q	1977 0	1977 0	0 0	0 0
K	1977 0	1977 0	0 0	0 0

MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	1550	988 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	P-C	-148 / 179	0.10 (1)
B-C	-1842 / 0	-104.9 -104.9	0.53 (1)	4.37	C-O	-460 / 0	0.22 (1)
C-D	-1529 / 0	-104.9 -104.9	0.49 (1)	4.75	O-E	0 / 472	0.08 (1)
D-E	-1529 / 0	-104.9 -104.9	0.49 (1)	4.75	E-M	0 / 1	0.00 (1)
E-F	-1143 / 0	-104.9 -104.9	0.35 (1)	5.52	M-F	0 / 474	0.08 (1)
F-G	-1530 / 0	-104.9 -104.9	0.49 (1)	4.75	M-H	-459 / 0	0.22 (1)
G-H	-1530 / 0	-104.9 -104.9	0.49 (1)	4.75	L-H	-149 / 178	0.10 (1)
H-I	-1842 / 0	-104.9 -104.9	0.53 (1)	4.37	B-P	0 / 1487	0.33 (1)
I-J	0 / 47	-104.9 -104.9	0.14 (1)	10.00	L-I	0 / 1487	0.33 (1)
Q-B	-1913 / 0	0.0 0.0	0.20 (1)	6.07			
K-I	-1912 / 0	0.0 0.0	0.20 (1)	6.07			
Q-P	0 / 0	-27.5 -27.5	0.23 (3)	10.00			
P-O	0 / 1450	-27.5 -27.5	0.38 (2)	10.00			
O-N	0 / 1142	-27.5 -27.5	0.28 (1)	10.00			
N-M	0 / 1142	-27.5 -27.5	0.26 (1)	10.00			
M-L	0 / 1450	-27.5 -27.5	0.37 (2)	10.00			
L-K	0 / 0	-27.5 -27.5	0.23 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.92")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.92")
CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CSI: TC=0.53/1.00 (B-C:1), BC=0.38/1.00 (O-P:2), WB=0.33/1.00 (B-P:1), SS=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

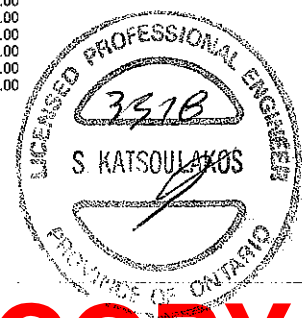
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

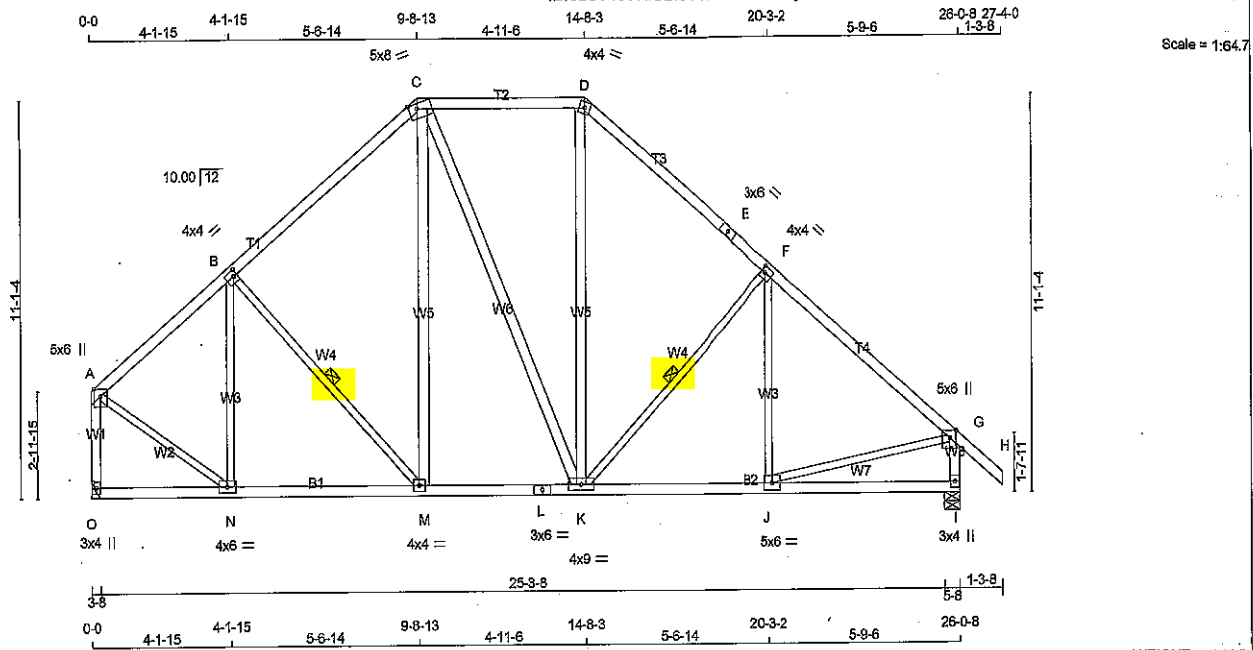
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.37 (B) (INPUT = 1.00)



SITE COPY

DRWG NO. TAM 11952-18
STRUCTURAL
COMPONENT ONLY



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TTW-m	MT20	5.0	6.0	Edge	3.00
D	TTW-m	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	5.0	6.0	2.50	2.25
I	BMV-t+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMVW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1724	0	1724	0	0	HANGER BY OTHERS	
I	1870	0	1870	0	0	MIN. SEAT SIZE: 3-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
O	COMBINED	SNOW	LIVE	PERM.LIVE
I	1367	846 / 0	260 / 0	0 / 0
I	1465	936 / 0	260 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)!

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-M, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-B	-1314 / 0	-104.9 -104.9	0.36 (1)	N-B	-517 / 0	0.35 (1)	
B-C	-1312 / 0	-104.9 -104.9	0.42 (1)	B-M	-102 / 0	0.05 (1)	
C-D	-1028 / 0	-104.9 -104.9	0.34 (1)	M-C	0 / 288	0.05 (2)	
D-E	-1381 / 0	-104.9 -104.9	0.48 (1)	C-K	0 / 117	0.02 (1)	
E-F	-1381 / 0	-104.9 -104.9	0.48 (1)	K-D	0 / 387	0.06 (2)	
F-G	-1710 / 0	-104.9 -104.9	0.52 (1)	K-F	-478 / 0	0.23 (1)	
G-H	0 / 47	-104.9 -104.9	0.14 (1)	J-F	-125 / 189	0.08 (1)	
O-A	-1682 / 0	0.0	0.0	A-N	0 / 1243	0.28 (1)	
I-G	-1605 / 0	0.0	0.0	J-G	0 / 1383	0.31 (1)	
O-N	0 / 0	-27.5	-27.5	0.16 (3)			
N-M	0 / 1047	-27.5	-27.5	0.27 (2)			
M-L	0 / 981	-27.5	-27.5	0.25 (2)			
L-K	0 / 981	-27.5	-27.5	0.25 (2)			
K-J	0 / 1349	-27.5	-27.5	0.36 (2)			
J-I	0 / 0	-27.5	-27.5	0.23 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.87")
CALCULATED VERT. DEFL.(LL)= L/999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.87")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.52/1.00 (F-G:1), BC=0.36/1.00 (J-K:2), WS=0.35/1.00 (B-N:1), SS=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

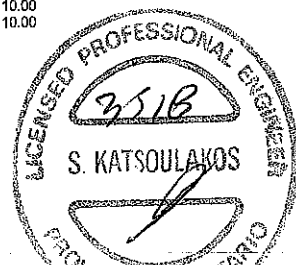
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (G) (INPUT = 0.90)
JSI METAL= 0.34 (G) (INPUT = 1.00)



SITE COPY

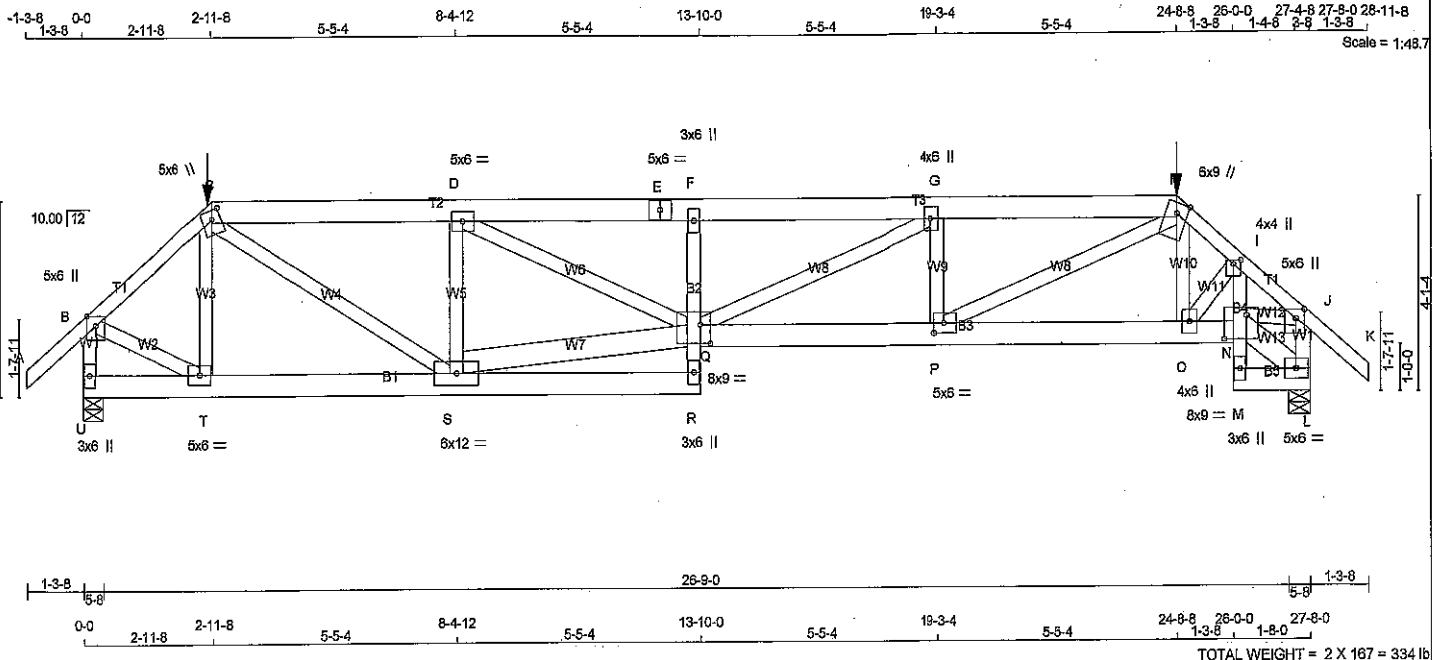
DRWG NO. TAM 11974-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T09	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:cLUv40WdCBi6Yv7QmwmDsyand7-JjgvHb416xZ_?c9JLeHVzH?_q9e6ndqZ?foSfze?M7



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - H	2x6	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - R	2x6	DRY No.2	SPF
R - F	2x4	DRY No.2	SPF
Q - N	2x6	DRY No.2	SPF
M - I	2x4	DRY No.2	SPF
M - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x4	DRY No.2	SPF
N - L	2x6	DRY No.2	SPF
S - Q	2x6	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(0.0)
H - K 1	12	SIDE(0.0)
U - B 1	12	TOP
L - J 1	12	TOP
C - E 2	12	SIDE(50.8)
E - H 2	12	SIDE(50.8)
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - R 2	12	SIDE(23.0)
Q - N 2	12	SIDE(23.0)
M - L 2	12	SIDE(23.0)
R - F 1	12	TOP
I - M 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	2.25
D	TMWV-l	MT20	5.0	6.0		
E	TS-l	MT20	5.0	6.0		
F	TMW+p	MT20	5.0	6.0		
G	TMWV+l	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	
I	TMWV+p	MT20	4.0	4.0	1.00	2.00
J	TMWV+p	MT20	5.0	6.0	2.25	2.00
L	BMWV-l	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	3674	0	0	5-8
L	3681	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
U	2804	1811 / 0	545 / 0	0 / 0	0 / 0	549 / 0	0 / 0	
L	2809	1815 / 0	545 / 0	0 / 0	0 / 0	549 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

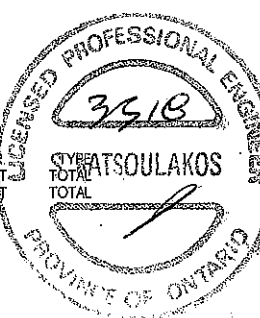
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 47	-104.9 -104.9	0.08 (1)	10.00	T-C	-885 / 0	0.08 (1)
B-C	-3430 / 0	-104.9 -104.9	0.13 (1)	4.94	C-S	0 / 3962	0.35 (1)
C-D	-5937 / 0	-206.6 -206.6	0.28 (1)	4.60	S-D	-3019 / 0	0.28 (1)
D-E	-9485 / 0	-206.6 -206.6	0.38 (1)	3.68	Q-G	0 / 1519	0.13 (1)
E-F	-9485 / 0	-206.6 -206.6	0.38 (1)	3.68	P-G	-1988 / 0	0.13 (1)
F-G	-8561 / 0	-206.6 -206.6	0.36 (1)	3.69	P-H	0 / 4996	0.44 (1)
G-H	-8190 / 0	-206.6 -206.6	0.32 (1)	3.99	O-H	0 / 310	0.03 (3)
H-I	-4751 / 0	-104.9 -104.9	0.14 (1)	4.31	O-I	-139 / 0	0.01 (1)
I-J	-4883 / 0	-104.9 -104.9	0.15 (1)	4.25	B-T	0 / 2892	0.25 (1)
J-K	0 / 47	-104.9 -104.9	0.08 (1)	10.00	N-L	-152 / 0	0.01 (1)
U-B	-3834 / 0	0.0 0.0	0.21 (1)	6.16	N-J	0 / 3891	0.33 (1)
L-J	-3547 / 0	0.0 0.0	0.20 (1)	6.23	S-Q	0 / 5895	0.38 (1)
					D-Q	0 / 3815	0.34 (1)
U-T	0 / 0	-54.1 -54.1	0.08 (2)	10.00			
T-S	0 / 2879	-54.1 -54.1	0.21 (1)	10.00			
S-R	0 / 255	-54.1 -54.1	0.11 (3)	10.00			
R-Q	0 / 209	0.0 0.0	0.20 (1)	10.00			
Q-P	-995 / 0	0.0 0.0	0.20 (1)	7.81			
Q-P	0 / 8189	-54.1 -54.1	0.62 (1)	10.00			
P-O	0 / 3703	-54.1 -54.1	0.33 (1)	10.00			
O-N	0 / 3805	-54.1 -54.1	0.34 (1)	10.00			
M-N	0 / 71	0.0 0.0	0.11 (1)	10.00			
N-I	0 / 176	0.0 0.0	0.12 (1)	10.00			
M-L	0 / 120	-54.1 -54.1	0.02 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR
C	2-11-8	-226	-226	-	FRONT	VERT
H	24-8-8	-226	-226	-	FRONT	VERT



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-11-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.22")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/984 (0.34")

CSI: TC=0.38/1.00 (D-F:1), BC=0.62/1.00 (P-Q:1), WB=0.44/1.00 (H-P:1), SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SITE COPY

DRWG NO. TAM 11953-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288713	T09	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:cLUv40WdCBi6YvR?Qmwmdsyand?-nvDHVx5fiFhrdmKVt29W1ApAkeVtrEztofOM 5ze?M6

PLATES (table is in inches)

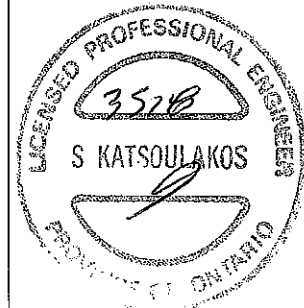
JT	TYPE	PLATES	W	LEN	Y	X
M	BMV+p	MT20	3.0	6.0		
N	BMWW-I	MT20	8.0	9.0	6.00	6.00
O	BMWW-I	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	2.50
Q	BMWWWW-I	MT20	8.0	9.0	4.75	2.75
R	BMV+p	MT20	3.0	6.0		
S	BMWWWW-t	MT20	6.0	12.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 226.3 lbs FACTORED DOWN AT 24-8-8,
AND 226.3 lbs FACTORED DOWN AT 2-11-8 ON
TOP CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

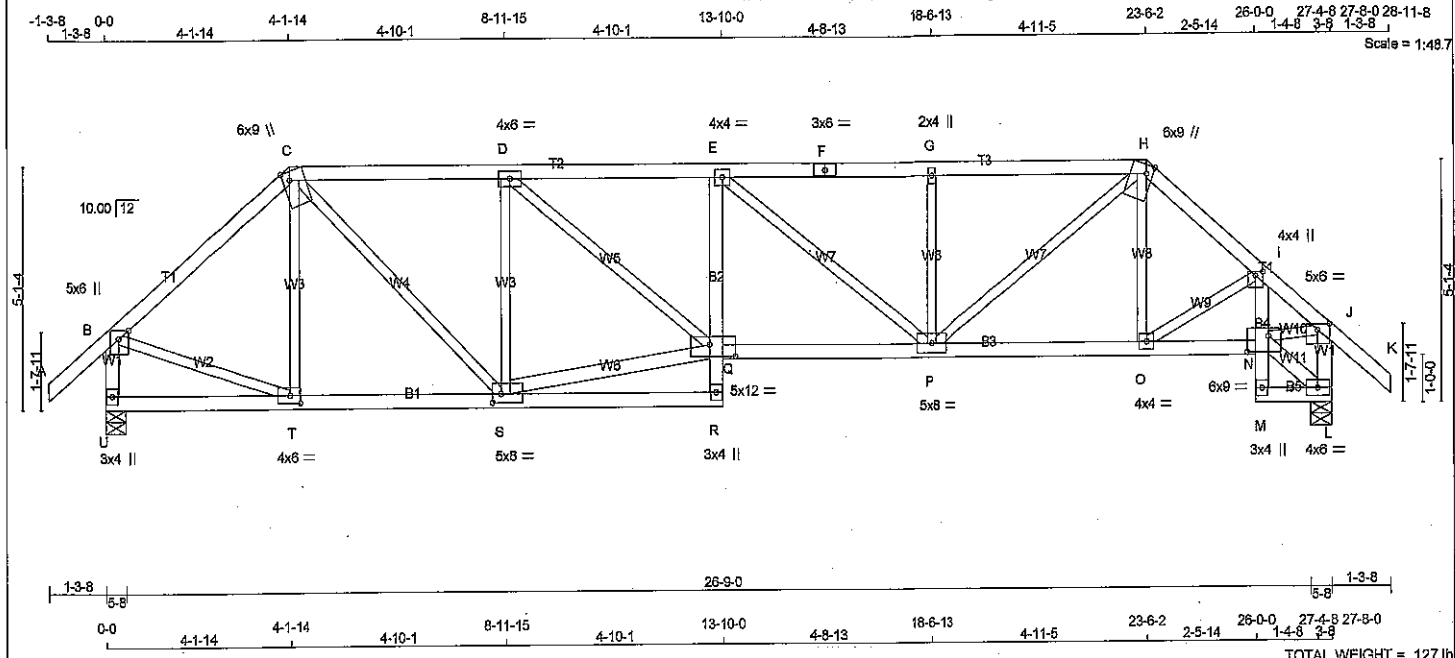
JSI GRIP= 0.88 (P) (INPUT = 0.90)
JSI METAL= 0.58 (P) (INPUT = 1.00)



DWG NO. TAM 11953-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 11:25:27 2018 Page 1
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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - F	2x4	DRY No.2	SPF
F - H	2x4	DRY No.2	SPF
H - K	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - E	2x4	DRY No.2	SPF
Q - N	2x4	DRY No.2	SPF
M - I	2x4	DRY No.2	SPF
M - L	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
S - Q	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
J	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	6.0	Edge 2.75
C	TTVW+m	MT20	6.0	9.0	Edge 1.75
D	TMVW-t	MT20	4.0	6.0	
E	TMVW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW+w	MT20	2.0	4.0	
H	TTVW+m	MT20	6.0	9.0	Edge 1.75
I	TMVW+p	MT20	4.0	4.0	1.00 2.00
J	TMVW+p	MT20	5.0	6.0	1.50 3.00
L	BMVW-t	MT20	4.0	6.0	
M	BMV+p	MT20	3.0	4.0	
N	BVMWV-t	MT20	6.0	9.0	4.00 5.75
O	BMVW-t	MT20	4.0	4.0	
P	BMVWV-t	MT20	5.0	8.0	
Q	BVMWV-t	MT20	5.0	12.0	3.00 7.00
R	BMV+p	MT20	3.0	4.0	
S	BMVWV-t	MT20	5.0	8.0	2.25 2.25
T	BMVW-t	MT20	4.0	6.0	2.00 2.75
U	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ UPLIFT	IN-SX
U	1974	0	1974	0	5-8
I	1981	0	1981	0	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1548	986 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
I	1553	991 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/47	-104.9 -104.9	0.14 (1)	10.00	T-C	-284/51	0.11 (1)		
B-C	-1783/0	-104.9 -104.9	0.39 (1)	4.58	C-S	0/1408	0.32 (1)		
C-D	-2351/0	-104.9 -104.9	0.45 (1)	4.02	S-D	-1347/0	0.52 (1)		
D-E	-3347/0	-104.9 -104.9	0.54 (1)	3.34	E-P	-477/0	0.32 (1)		
E-F	-2895/0	-104.9 -104.9	0.53 (1)	3.53	P-G	-575/0	0.15 (1)		
F-G	-2895/0	-104.9 -104.9	0.53 (1)	3.53	P-H	0/1633	0.37 (1)		
G-H	-2895/0	-104.9 -104.9	0.53 (1)	3.53	O-H	0/171	0.04 (3)		
H-I	-2220/0	-104.9 -104.9	0.18 (1)	4.43	O-I	-130/0	0.03 (1)		
I-J	-2332/0	-104.9 -104.9	0.19 (1)	4.31	B-T	0/1429	0.32 (1)		
J-K	0/47	-104.9 -104.9	0.14 (1)	10.00	N-L	-75/0	0.01 (1)		
U-B	-1827/0	0.0	0.0 0.20 (1)	6.06	N-J	0/1768	0.40 (1)		
L-J	-1815/0	0.0	0.0 0.20 (1)	6.07	S-Q	0/2336	0.38 (1)		
					D-Q	0/1253	0.28 (1)		
U-T	0/0	-27.5	-27.5 0.12 (3)	10.00					
T-S	0/1360	-27.5	-27.5 0.29 (1)	10.00					
S-R	0/89	-27.5	-27.5 0.15 (3)	10.00					
R-Q	0/92	0.0	0.0 0.13 (1)	10.00					
Q-E	-166/77	0.0	0.0 0.13 (1)	7.81					
Q-P	0/3371	-27.5	-27.5 0.82 (1)	10.00					
P-O	0/1699	-27.5	-27.5 0.34 (1)	10.00					
O-N	0/1812	-27.5	-27.5 0.35 (1)	10.00					
M-N	0/36	0.0	0.0 0.10 (1)	10.00					
N-I	-58/36	0.0	0.0 0.09 (1)	7.81					
M-L	0/59	-27.5	-27.5 0.02 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	3.0	PSF
BOT	CH.	LL =	10.0	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.92")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.18")
ALLOWABLE DEFL.(TL)= $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.29")

CSI: TC=0.54/1.00 (D-E:1), BC=0.62/1.00 (P-Q:1),
WB=0.52/1.00 (D-S:1), SSI=0.24/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.90 (Q) (INPUT = 0.90)
ISI METAL= 0.56 (Q) (INPUT = 1.00)

DWG NO. TAM 11954-18
STRUCTURAL
COMPONENT ONLY



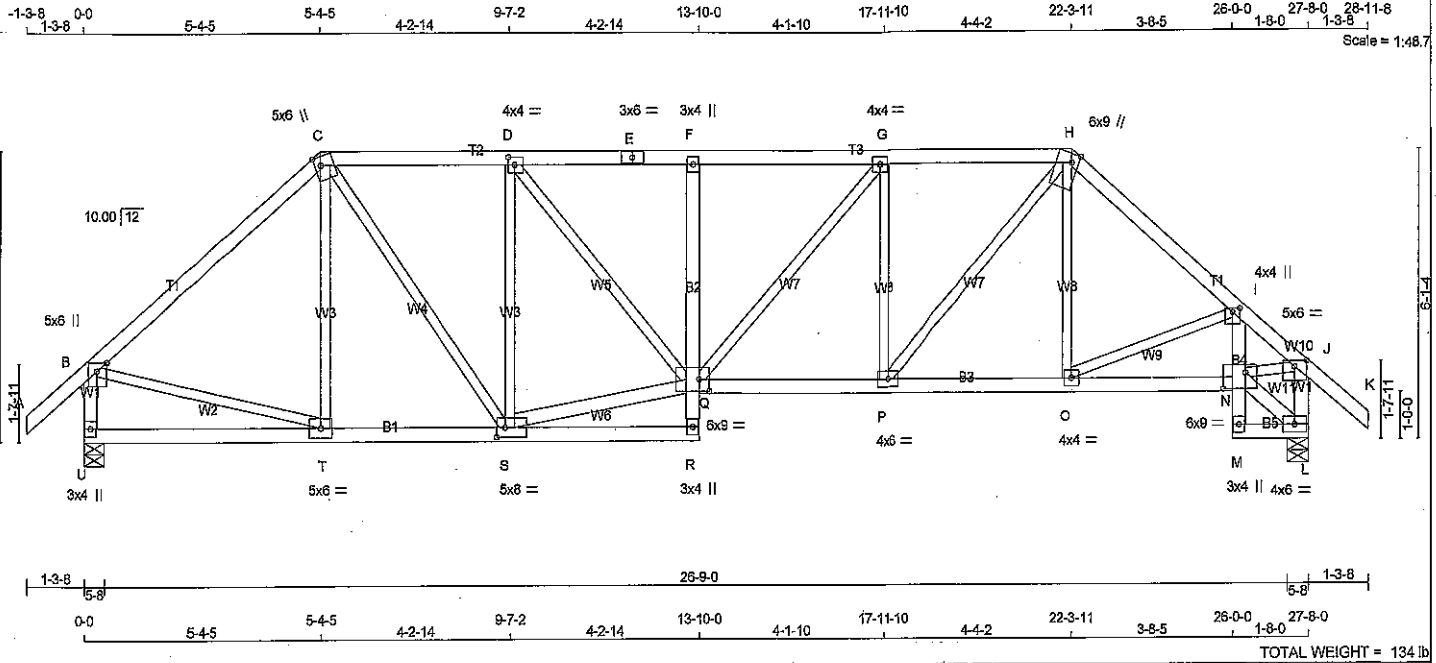
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T11	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: cLUv40WdCB6YvR7QmwmDsynd7-nvDHVx5fHrdmkV29WV1Ap5yEXW9dzofOM_Sze?M6



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - F	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
M - I	2x4	DRY	No.2
M - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - Q	2x4	DRY	No.2
N - L	2x4	DRY	No.2

DRY; SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	2.75
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW-l	MT20	4.0	4.0	2.00	1.75
E	TS-l	MT20	3.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TMVW-l	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW-p	MT20	5.0	6.0	1.50	3.00
L	BMVW1-l	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWVW-l	MT20	6.0	9.0	4.00	6.00
O	BMVW-l	MT20	4.0	4.0		
P	BMVW-l	MT20	4.0	6.0		
Q	BVMWVW-l	MT20	6.0	9.0	3.00	2.75
R	BMV+p	MT20	3.0	4.0		
S	BMVWVW-l	MT20	5.0	8.0	2.50	2.25
T	BMVW-l	MT20	5.0	6.0		
U	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
U	1974	0	1974	0	5-8
L	1981	0	1981	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1548	986 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
L	1553	991 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FACTORED FORCE (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 47	T-C	-177 / 123
B-C	-1800 / 0	C-S	0 / 1063
C-D	-1998 / 0	S-D	-1219 / 0
D-E	-2633 / 0	S-Q	0 / 2009
E-F	-2633 / 0	D-Q	0 / 960
F-G	-2645 / 0	Q-G	0 / 319
G-H	-2433 / 0	P-H	-744 / 0
H-I	-2160 / 0	I-H	0 / 1172
I-J	-2373 / 0	O-H	0 / 213
J-K	0 / 47	O-I	-244 / 0
U-B	-1911 / 0	B-T	0 / 1419
L-J	-1914 / 0	N-L	-76 / 0
		N-J	0 / 1828
U-T	0 / 0		
T-S	0 / 1377		
S-R	0 / 58		
R-Q	0 / 83		
Q-F	-401 / 0		
Q-P	0 / 2433		
P-O	0 / 1846		
O-N	0 / 1871		
M-N	0 / 36		
N-I	-71 / 55		
M-L	0 / 60		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.21")

CSI: TC=0.69/1.00 (B-C:1), BC=0.45/1.00 (P-Q:1),
WB=0.71/1.00 (D-S:1), SSI=0.21/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.86 (S) (INPUT = 0.90)
JSI METAL=0.46 (J) (INPUT = 1.00)



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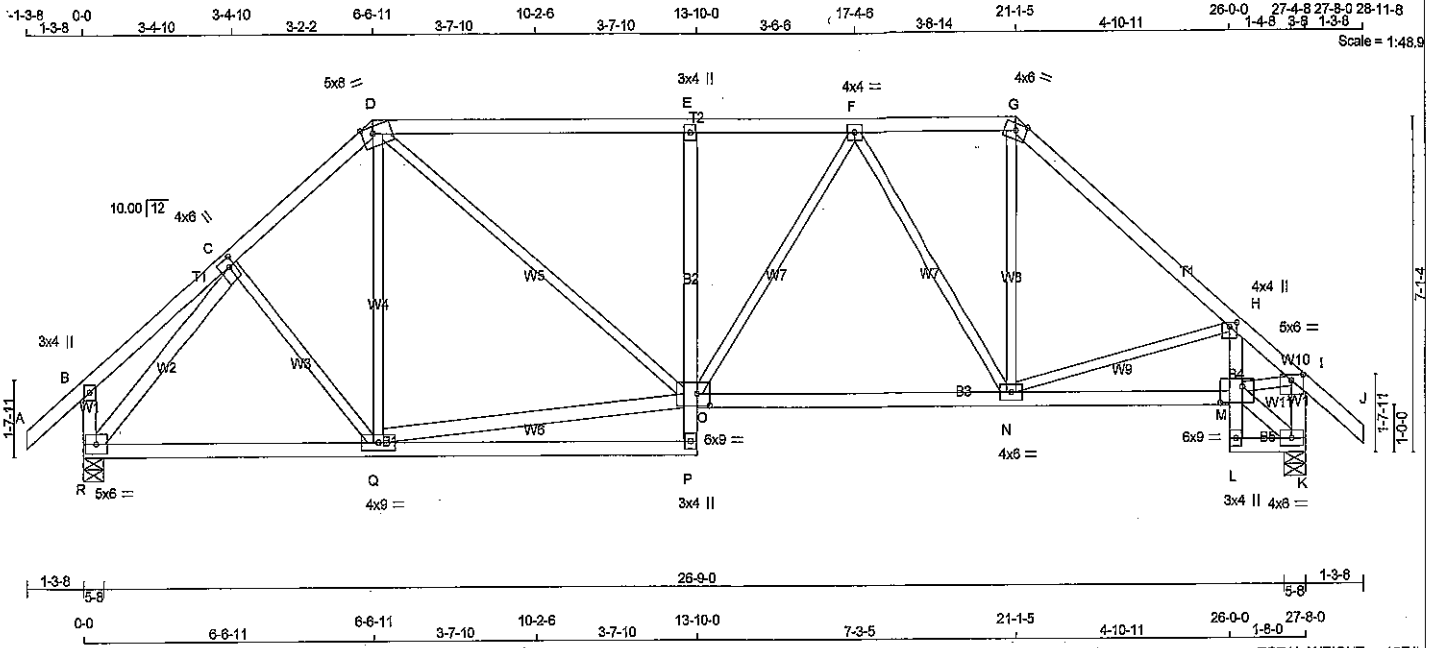
DRWG NO. TAM 11955-018
STRUCTURAL
COMPONENT ONLY

JOB NAME 288713	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. 44755	DRWG NO.
TRUSS DESC.					

Tamarack Roof Truss, Burlington

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ID:cLUv40WdCBi6YvR?Qmwmndsyand?F5ngIG6HeZpIFwJIRmglaOMCsesMadR71J8vWYze?M5



LUMBER			
N. L. G. A. RULES.			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
R - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - E	2x4	DRY	No.2
O - M	2x4	DRY	No.2
L - H	2x4	DRY	No.2
L - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
R - C	2x4	DRY	No.2
M - K	2x4	DRY	No.2
Q - O	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMVW+H	MT20	4.0	6.0 2.25 1.50
D	TTWV-m	MT20	5.0	8.0 Edge 3.00
E	TMV+p	MT20	3.0	4.0
F	TMVW-t	MT20	4.0	4.0
G	TTW-m	MT20	4.0	6.0 Edge
H	TMVW+p	MT20	4.0	4.0 1.00 2.00
I	TMVW-p	MT20	5.0	6.0 1.50 3.00
K	BMVW-t	MT20	4.0	6.0
L	BMV+p	MT20	3.0	4.0
M	BVMWV-t	MT20	6.0	9.0 4.00 6.00
N	BVMWV-t	MT20	4.0	8.0
O	BVMWV-t	MT20	6.0	9.0 3.00 3.50
P	BMV+p	MT20	3.0	4.0
Q	BMVW-t	MT20	4.0	9.0
R	BMVW-t	MT20	5.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
R	1974	0	1974	0
K	1981	0	1981	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1548	986 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	1553	991 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 47	-104.9 -104.9	0.14 (1)	10.00	C-Q	0 / 136	0.03 (3)
B-C	0 / 24	-104.9 -104.9	0.17 (1)	10.00	Q-D	-13 / 220	0.05 (3)
C-D	-1812 / 0	-104.9 -104.9	0.22 (1)	4.76	N-G	0 / 917	0.21 (1)
D-E	-2244 / 0	-104.9 -104.9	0.93 (1)	3.43	N-H	-379 / 0	0.18 (1)
E-F	-2252 / 0	-104.9 -104.9	0.74 (1)	3.43	R-C	-2098 / 0	0.64 (1)
F-G	-1616 / 0	-104.9 -104.9	0.26 (1)	4.93	M-K	-78 / 0	0.01 (1)
G-H	-2100 / 0	-104.9 -104.9	0.48 (1)	4.24	M-I	0 / 1909	0.43 (1)
H-I	-2430 / 0	-104.9 -104.9	0.35 (1)	4.03	O-F	0 / 440	0.10 (1)
I-J	0 / 47	-104.9 -104.9	0.14 (1)	10.00	F-N	-785 / 0	0.68 (1)
R-B	-278 / 0	0.0	0.0	0.03 (1)	Q-O	0 / 1334	0.21 (1)
K-I	-1913 / 0	0.0	0.0	0.20 (1)	D-O	0 / 1127	0.25 (1)
R-Q	0 / 1319	-27.5	-27.5	0.51 (2)			
Q-P	0 / 48	-27.5	-27.5	0.38 (3)			
P-O	0 / 130	0.0	0.0	0.08 (1)			
O-E	-740 / 0	0.0	0.0	0.44 (1)			
O-N	0 / 2017	-27.5	-27.5	0.54 (2)			
N-M	0 / 1952	-27.5	-27.5	0.53 (2)			
L-M	0 / 36	0.0	0.0	0.10 (1)			
M-H	-93 / 49	0.0	0.0	0.10 (1)			
L-K	0 / 62	-27.5	-27.5	0.03 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.26")

CSI: TC=0.93/1.00 (D-E:1), BC=0.54/1.00 (N-O:2), WB=0.68/1.00 (F-N:1), SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

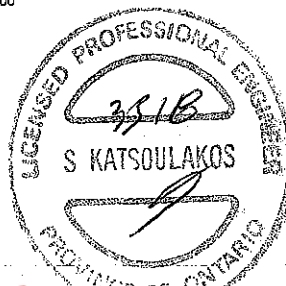
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2264 1656

PLATE PLACEMENT TOL. = 0.250 inches

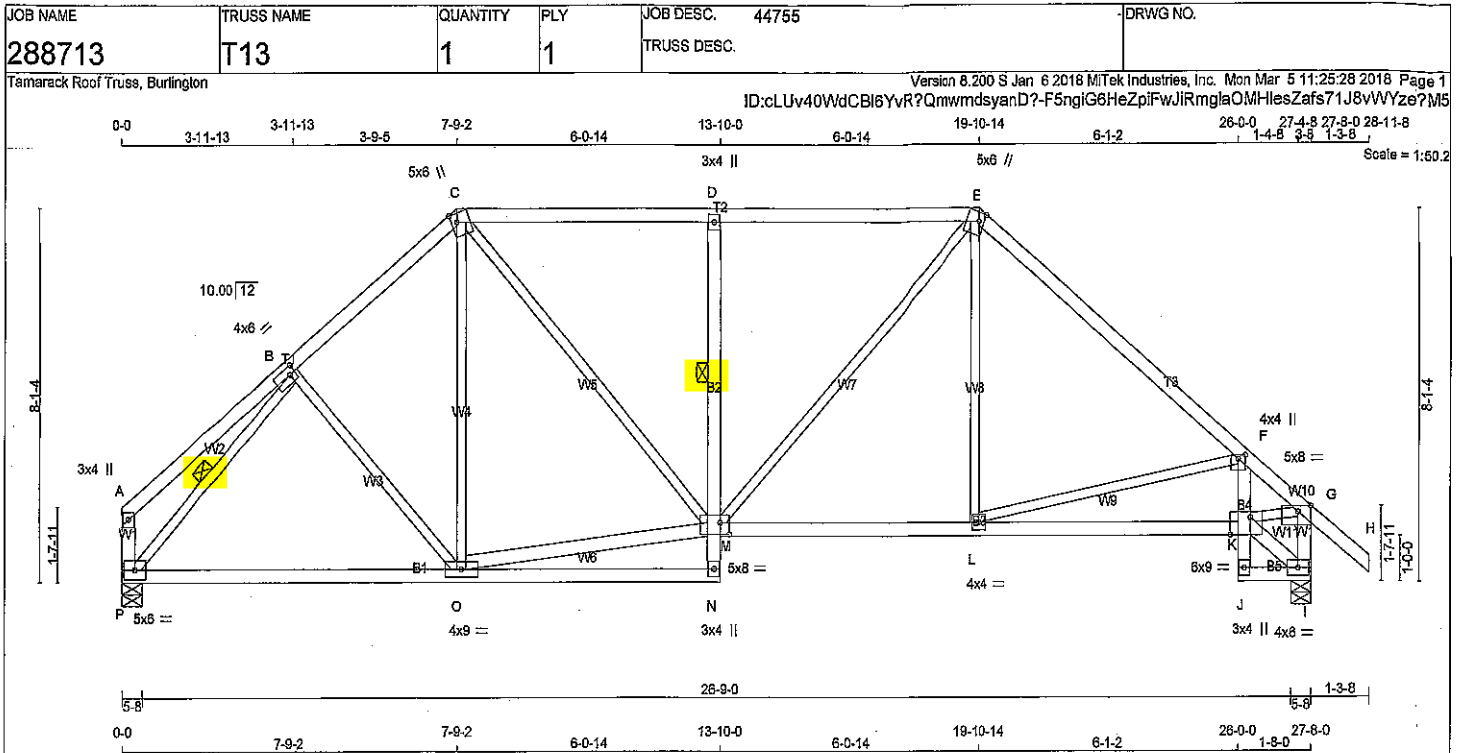
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (M) (INPUT = 0.90)
JSI METAL= 0.70 (C) (INPUT = 1.00)



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DWG NO. TAM 11956
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 134 LB
(M/F)

LUMBER			LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS	SIZE			
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
P - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - M	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	3.0	4.0
B	TMVWW-t	MT20	4.0	6.0 2.00 1.50
C	TTVWW+m	MT20	5.0	6.0 2.25 1.50
D	TMV+p	MT20	3.0	4.0
E	TTVWW+m	MT20	5.0	6.0 2.25 1.50
F	TMVWW+p	MT20	4.0	4.0 1.00 2.00
G	TMVWW-p	MT20	5.0	8.0 1.50 3.50
I	BMVWW-t	MT20	4.0	6.0
J	BMV+p	MT20	3.0	4.0
K	BMVWW-t	MT20	6.0	9.0 4.50 5.75
L	BMVWW-t	MT20	4.0	4.0
M	BMVWWWW-t	MT20	5.0	8.0 3.00 2.50
N	BMV+p	MT20	3.0	4.0
O	BMVWWWW-t	MT20	4.0	9.0
P	BMVWW-t	MT20	5.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
P	1828	0	1828	0	0	5-8	5-8		
I	1981	0	1981	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
P	1449	896 / 0	277 / 0	0 / 0	0 / 0	276 / 0	0 / 0		
I	1553	991 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-M, B-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.		MAX. FACTORED FORCE (LBS)
FR-TO			FROM TO		FR-TO		
A-B		0 / 29	-104.9 -104.9	0.24 (1)	B-C		-37 / 106
B-C		-1764 / 0	-104.9 -104.9	0.23 (1)	C-D		0 / 235
C-D		-1916 / 0	-104.9 -104.9	0.55 (1)	D-E		0 / 1318
D-E		-1921 / 0	-104.9 -104.9	0.56 (1)	E-F		0 / 862
E-F		-2008 / 0	-104.9 -104.9	0.82 (1)	F-G		0 / 800
F-G		-2508 / 0	-104.9 -104.9	0.45 (1)	G-H		0 / 404
G-H		0 / 47	-104.9 -104.9	0.14 (1)	H-I		-568 / 0
I-A		-159 / 0	0.0 0.0	0.02 (1)	A-B		-2085 / 0
I-G		-1911 / 0	0.0 0.0	0.20 (1)	B-C		-80 / 0
P-O		0 / 1353	-27.5 -27.5	0.53 (2)	K-I		0.01 (1)
O-N		0 / 32	-27.5 -27.5	0.40 (3)	K-G		0 / 2018
N-M		0 / 91	0.0 0.0	0.05 (1)			0.45 (1)
M-D		-778 / 0	0.0 0.0	0.08 (1)			
M-L		0 / 1516	-27.5 -27.5	0.47 (2)			
L-K		0 / 2063	-27.5 -27.5	0.52 (2)			
J-K		0 / 36	0.0 0.0	0.10 (1)			
K-F		-91 / 82	0.0 0.0	0.08 (1)			
J-I		0 / 64	-27.5 -27.5	0.03 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.82/1.00 (E-F:1), BC=0.53/1.00 (O-P:2), WB=0.52/1.00 (B-P:1), SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.51 (B) (INPUT = 1.00)

DWG NO. TAM 11957-08
STRUCTURAL
COMPONENT ONLY

SITE COPY

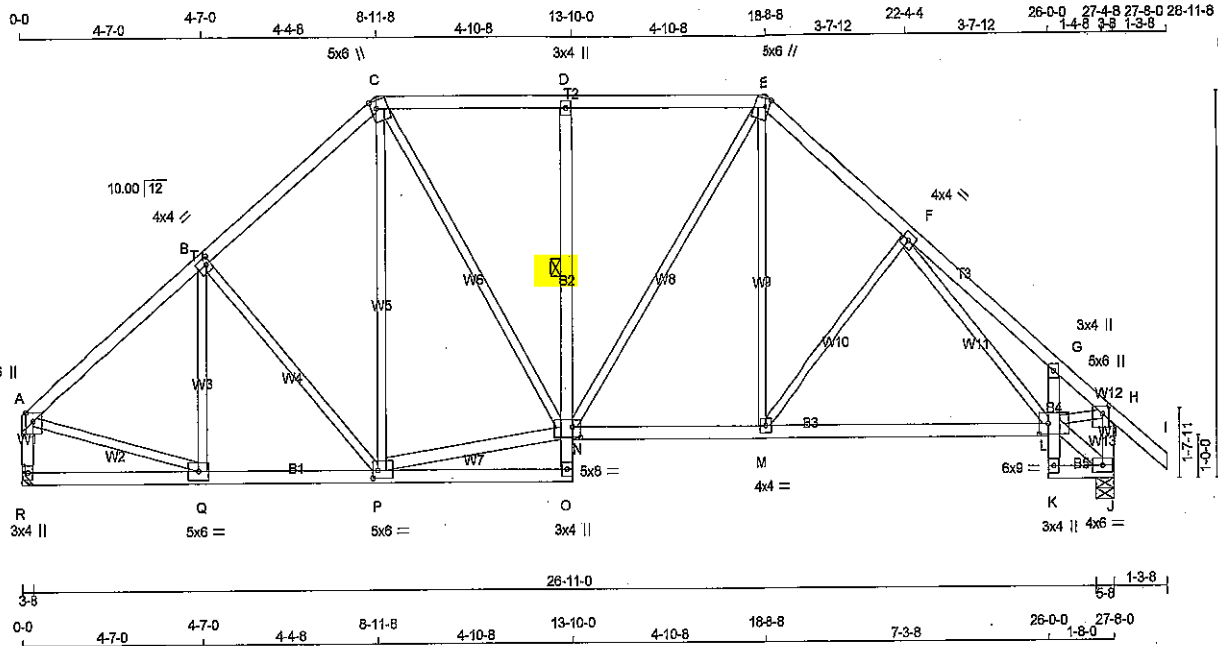
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: cLUv40WdCBi6YvR?QmwmDsyand?F5ngIG6HeZpiFwJIRmglaOMLzeskaJ71J8vWYze?M5



TOTAL WEIGHT = 144 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - I	2x4	DRY	No.2	SPF	
R - A	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
R - O	2x4	DRY	No.2	SPF	
O - D	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
K - G	2x4	DRY	No.2	SPF	
K - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
L - J	2x4	DRY	No.2	SPF	
P - N	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.50	2.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMV+p	MT20	3.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TMVW+p	MT20	5.0	6.0	2.25	2.00
J	BMVW-t	MT20	4.0	6.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-t	MT20	6.0	9.0	3.00	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BVMWW-t	MT20	5.0	8.0	3.00	2.50
O	BMV+p	MT20	3.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.25	1.50
Q	BMWW-t	MT20	5.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
R	1828	0	1828	0
J	1981	0	1981	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1449	896/0	277/0	0/0	0/0	276/0	0/0
J	1553	991/0	277/0	0/0	0/0	285/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX.	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	-1815/0	-104.9 -104.9	0.32 (1)	4.67	Q-B	-277/83	0.12 (1)
B-C	-1687/0	-104.9 -104.9	0.31 (1)	4.81	B-P	-240/0	0.21 (1)
C-D	-1650/0	-104.9 -104.9	0.35 (1)	4.79	P-C	0/198	0.04 (3)
D-E	-1653/0	-104.9 -104.9	0.35 (1)	4.79	N-E	0/402	0.09 (1)
E-F	-1894/0	-104.9 -104.9	0.23 (1)	4.69	M-E	0/498	0.11 (2)
F-G	-2085/0	-104.9 -104.9	0.24 (1)	4.50	M-F	-351/0	0.23 (1)
G-H	-2111/0	-104.9 -104.9	0.15 (1)	4.56	F-L	-85/83	0.04 (1)
H-I	0/47	-104.9 -104.9	0.14 (1)	10.00	A-Q	0/1477	0.33 (1)
R-A	-1774/0	0.0 0.0	0.19 (1)	6.27	L-J	-86/0	0.01 (1)
J-H	-1922/0	0.0 0.0	0.20 (1)	6.07	L-H	0/1626	0.37 (1)
					P-N	0/1270	0.20 (1)
					C-N	0/743	0.17 (1)
R-Q	0/0	-27.5 -27.5	0.12 (3)	10.00			
Q-P	0/1421	-27.5 -27.5	0.31 (1)	10.00			
P-O	0/22	-27.5 -27.5	0.14 (2)	10.00			
O-N	0/90	0.0 0.0	0.04 (1)	10.00			
N-D	-818/0	0.0 0.0	0.07 (1)	6.25			
N-M	0/1436	-27.5 -27.5	0.50 (2)	10.00			
M-L	0/1661	-27.5 -27.5	0.52 (2)	10.00			
K-L	0/55	0.0 0.0	0.08 (1)	10.00			
L-G	-198/0	0.0 0.0	0.08 (1)	7.81			
K-J	0/58	-27.5 -27.5	0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.35/1.00 (D-E:1), BC=0.52/1.00 (L-M:2), WB=0.37/1.00 (H-L:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (P) (INPUT = 0.90)
SI METAL= 0.41 (H) (INPUT = 1.00)



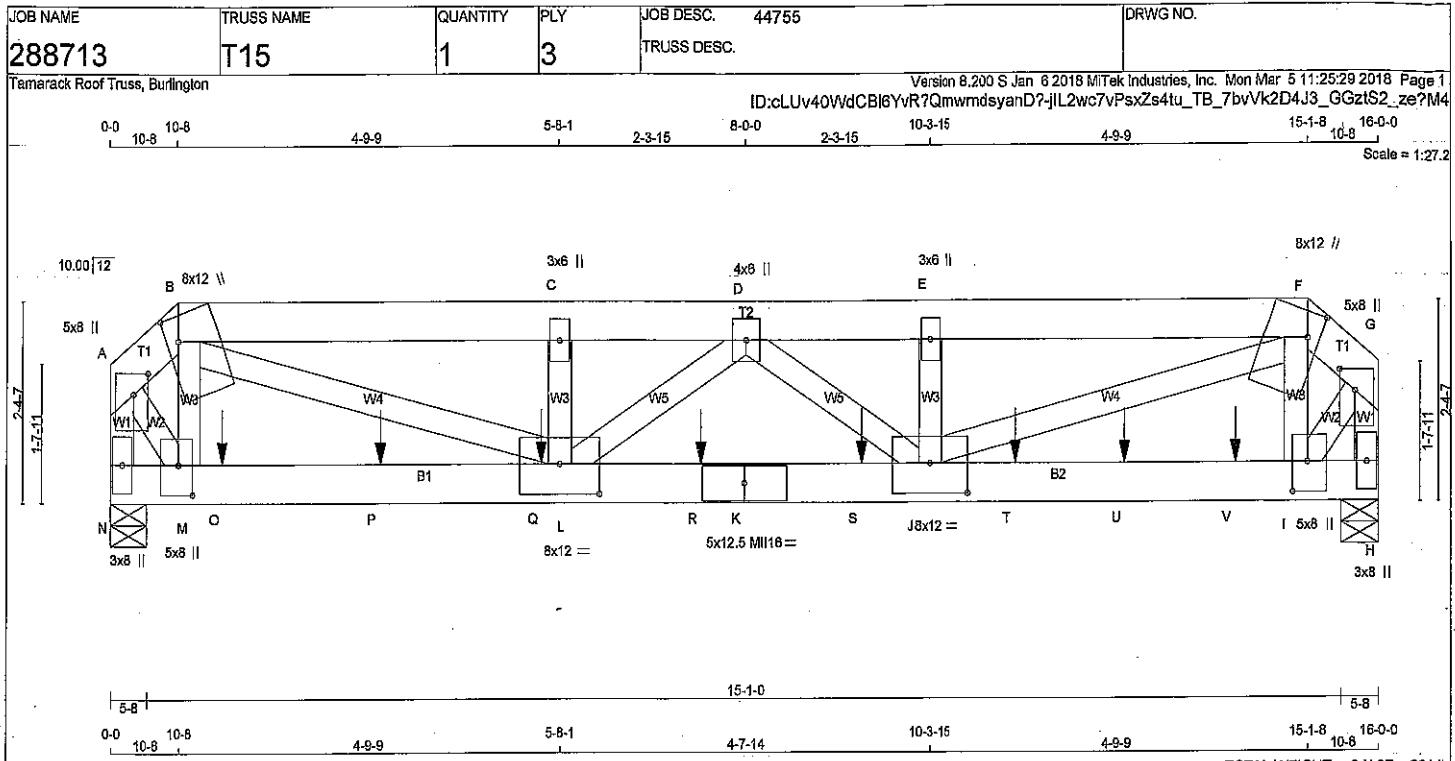
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DWG NO. TAM 11975-19
STRUCTURAL
COMPONENT ONLY

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LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x6	DRY No.2	SPF
B - F	2x6	DRY No.2	SPF
F - G	2x6	DRY No.2	SPF
N - A	2x4	DRY No.2	SPF
H - G	2x4	DRY No.2	SPF
N - K	2x6	DRY 2100F 1.8E	SPF
K - H	2x6	DRY 2100F 1.8E	SPF

ALL WEBS 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B 2	12	SIDE(6.9)
B - F 2	12	SIDE(9.2)
F - G 2	12	SIDE(6.9)
N - A 1	12	TOP
H - G 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
N - K 2	7	SIDE(508.6)
K - H 2	7	SIDE(1525.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	8.0	3.00	2.25
B	TTWW+m	MT20	8.0	12.0	3.50	1.75
C	TMVW+w	MT20	3.0	6.0		
D	TMVW+t	MT20	4.0	6.0		
E	TMVW+w	MT20	3.0	6.0		
F	TTWW+m	MT20	8.0	12.0	3.50	1.75
G	TMVW+p	MT20	5.0	8.0	3.00	2.25
H	BMV1+p	MT20	3.0	8.0		
I	BMVW+t	MT20	5.0	8.0	4.25	2.25
J	BMVWWW-t	MT20	8.0	12.0	4.25	6.00
K	BS-I	MI16	5.0	12.5		
L	BMVWWW-t	MT20	8.0	12.0	4.25	6.00
M	BMVW+t	MT20	5.0	8.0	4.25	2.25
N	BMV1+p	MT20	3.0	8.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	8627	0	8627	0	5-8	5-8
H	9161	0	9161	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	8740	4338 / 0	1176 / 0	0 / 0	0 / 0	1224 / 0	0 / 0
H	7143	4624 / 0	1230 / 0	0 / 0	0 / 0	1288 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD	LC1	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)			
FR-TO		FROM	TO			FR-TO					
A-B	-7297 / 0	-118.7	-118.7	0.03 (1)	5.29	M-B	-590 / 0	0.02 (1)			
B-C	-18012 / 0	-118.7	-118.7	0.36 (1)	3.34	B-L	0 / 13437	0.72 (1)			
C-D	-18020 / 0	-118.7	-118.7	0.32 (1)	3.36	L-C	0 / 211	0.01 (2)			
D-E	-18758 / 0	-118.7	-118.7	0.34 (1)	3.28	J-E	0 / 232	0.01 (2)			
E-F	-18748 / 0	-118.7	-118.7	0.38 (1)	3.26	J-F	0 / 13488	0.72 (1)			
F-G	-8157 / 0	-118.7	-118.7	0.03 (1)	5.06	I-F	-55 / 84	0.00 (3)			
N-A	-10836 / 0	0.0	0.0	0.37 (1)	4.55	A-M	0 / 8127	0.44 (1)			
H-G	-12106 / 0	0.0	0.0	0.41 (1)	4.29	I-G	0 / 9084	0.49 (1)			
						L-D	-1145 / 0	0.05 (1)			
						D-J	-166 / 0	0.01 (1)			
N-M	0 / 0	-27.5	-27.5	0.27 (1)	10.00						
M-O	0 / 5452	-27.5	-27.5	0.36 (1)	10.00						
O-P	0 / 5452	-27.5	-27.5	0.36 (1)	10.00						
P-Q	0 / 5452	-27.5	-27.5	0.36 (1)	10.00						
Q-L	0 / 5452	-27.5	-27.5	0.36 (1)	10.00						
L-R	0 / 18884	-27.5	-27.5	0.51 (1)	10.00						
R-K	0 / 18884	-27.5	-27.5	0.51 (1)	10.00						
K-S	0 / 18884	-27.5	-27.5	0.51 (1)	10.00						
S-J	0 / 18884	-27.5	-27.5	0.51 (1)	10.00						
J-T	0 / 6141	-27.5	-27.5	0.46 (1)	10.00						
T-U	0 / 6141	-27.5	-27.5	0.46 (1)	10.00						
U-V	0 / 6141	-27.5	-27.5	0.46 (1)	10.00						
V-I	0 / 6141	-27.5	-27.5	0.46 (1)	10.00						
I-H	0 / 0	-27.5	-27.5	0.37 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	TOTAL
O	1-5-4	-1801	-1801	---	BACK	VERT	TOTAL	
P	3-5-4	-1950	-1950	---	BACK	VERT	TOTAL	
Q	5-5-4	-1950	-1950	---	BACK	VERT	TOTAL	
R	7-5-4	-1950	-1950	---	BACK	VERT	TOTAL	
S	9-5-4	-1950	-1950	---	BACK	VERT	TOTAL	
T	11-5-4	-1950	-1950	---	BACK	VERT	TOTAL	
U	12-10-0	-1950	-1950	---	BACK	VERT	TOTAL	
V	14-2-12	-1950	-1950	---	BACK	VERT	TOTAL	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 32.5 PSF
 DL = 3.0 PSF
 BOT CH. LL = 10.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CS1dGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 2-5-0
 END DISTANCE = 16-0-0
 END SPAN CARRIED = 2-5-0
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF TOP CHORD.
 - ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
 CALCULATED VERT. DEFL.(LL) = L/982 (0.20")
 ALLOWABLE DEFL.(TL) = L/360 (0.53")
 CALCULATED VERT. DEFL.(TL) = L/638 (0.30")

CSI: TC=0.41/1.00 (G-H:1), BC=0.51/1.00 (J-L:1), WB=0.72/1.00 (F-J:1), SSI=0.74/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667	822	2284
MI16	473	276	2341	1245	4454

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
 JSI METAL= 0.69 (K) (INPUT = 1.00)

SITE COPY

DRWG NO. TAM 11959-88
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288713	T15	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:cLUv40WdCBi6YvR?Qmwmdsyand7-jIL2wc7vPssZs4tu TB 7bvVk2D4J3 GGztS2 ze?M4

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1800.8 lbs FACTORED DOWN AT 1-5-4, 1949.8 lbs FACTORED DOWN AT 3-5-4, 1949.8 lbs FACTORED DOWN AT 5-5-4, 1949.8 lbs FACTORED DOWN AT 7-5-4, 1949.8 lbs FACTORED DOWN AT 9-5-4, 1949.8 lbs FACTORED DOWN AT 11-5-4, AND 1949.8 lbs FACTORED DOWN AT 12-10-0, AND 1949.8 lbs FACTORED DOWN AT 14-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



SITE COPY

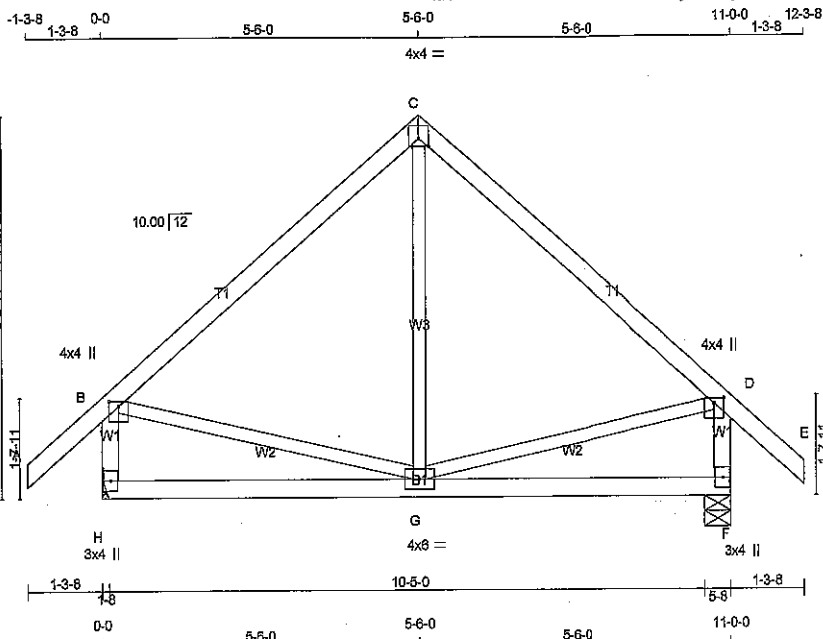
DWG NO. TAM 11959-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 288713	TRUSS NAME T16	QUANTITY 3	PLY 1	JOB DESC. 44755 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: cLUv40WdCBi6YvR7QmwmmsyanD7-JIL2wc7vPssZs4tu_TB_7bvVl2HFJD_GGztS2_ze?M4



Scale = 1:37.7

TOTAL WEIGHT = 3 X 48 = 147 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW+H	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	874	0	0/0	0/0
H	874	0	0/0	0/0
F	874	0	0/0	0/0

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	676	447 / 0	110 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	676	447 / 0	110 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	CS1 (LC)
A-B		0 / 47	-104.9	-104.9	0.14 (1)	10.00	6.25	G-C	0 / 244	0.06 (3)	
B-C		-465 / 0	-104.9	-104.9	0.41 (1)	6.25	B-G	0 / 357	0.08 (1)		
C-D		-465 / 0	-104.9	-104.9	0.41 (1)	6.25	G-D	0 / 357	0.08 (1)		
D-E		0 / 47	-104.9	-104.9	0.14 (1)	10.00					
H-B		-816 / 0	0.0	0.0	0.09 (1)	7.81					
F-D		-816 / 0	0.0	0.0	0.09 (1)	7.81					
H-G		0 / 0	-27.5	-27.5	0.24 (3)	10.00					
G-F		0 / 0	-27.5	-27.5	0.24 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	10.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.41/1.00 (C-D:1), BC=0.24/1.00 (F-G:3),
WB=0.08/1.00 (B-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

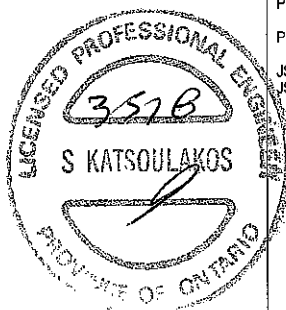
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.83 (C) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)



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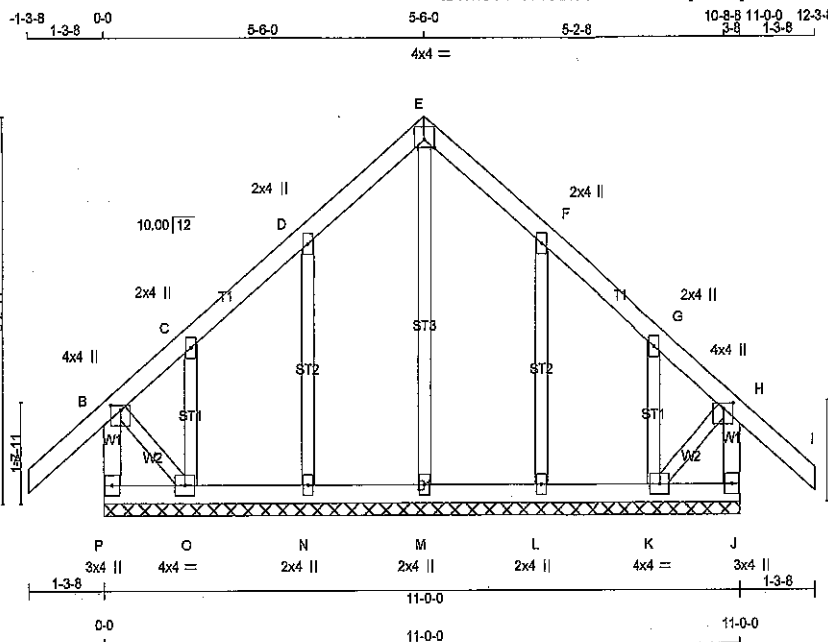
DRG NO. TAM 11960-8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T16G	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: cLUv40VdCBi6YvR?Qmwmdsyand7-jlL2wc7vPsxZs4tu_TB_7bvZv2KjJDpGGztS2_ze?M4



Scale = 1:37.3

TOTAL WEIGHT = 53 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE
P - B 2x4 DRY No.2
A - E 2x4 DRY No.2
E - I 2x4 DRY No.2
J - H 2x4 DRY No.2
P - J 2x4 DRY No.2

LUMBER

DESCR.

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO		FR-TO		
P-B	-314 / 0	0.0 0.0 0.03 (1)	7.81	M-E	-158 / 0	0.09 (1)
A-B	0 / 47	-104.9 -104.9 0.14 (1)	10.00	N-D	-254 / 0	0.08 (1)
B-C	-64 / 0	-104.9 -104.9 0.14 (1)	8.25	O-C	-90 / 0	0.02 (1)
C-D	-5 / 0	-104.9 -104.9 0.07 (1)	10.00	L-F	-254 / 0	0.08 (1)
D-E	-26 / 0	-104.9 -104.9 0.07 (1)	6.25	K-G	-90 / 0	0.02 (1)
E-F	-26 / 0	-104.9 -104.9 0.07 (1)	6.25	B-O	0 / 23	0.01 (1)
F-G	-5 / 0	-104.9 -104.9 0.07 (1)	10.00	K-H	0 / 23	0.01 (1)
G-H	-64 / 0	-104.9 -104.9 0.14 (1)	6.25			
H-I	0 / 47	-104.9 -104.9 0.14 (1)	10.00			
J-H	-314 / 0	0.0 0.0 0.03 (1)	7.81			
P-O	0 / 0	-27.5 -27.5 0.02 (3)	10.00			
O-N	0 / 14	-27.5 -27.5 0.02 (2)	10.00			
N-M	0 / 8	-27.5 -27.5 0.02 (2)	10.00			
M-L	0 / 8	-27.5 -27.5 0.02 (2)	10.00			
L-K	0 / 14	-27.5 -27.5 0.02 (2)	10.00			
K-J	0 / 0	-27.5 -27.5 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (H-I:1), BC=0.02/1.00 (N-O:2), WB=0.09/1.00 (E-M:1), SS=0.09/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

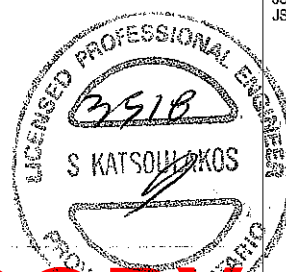
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 inches

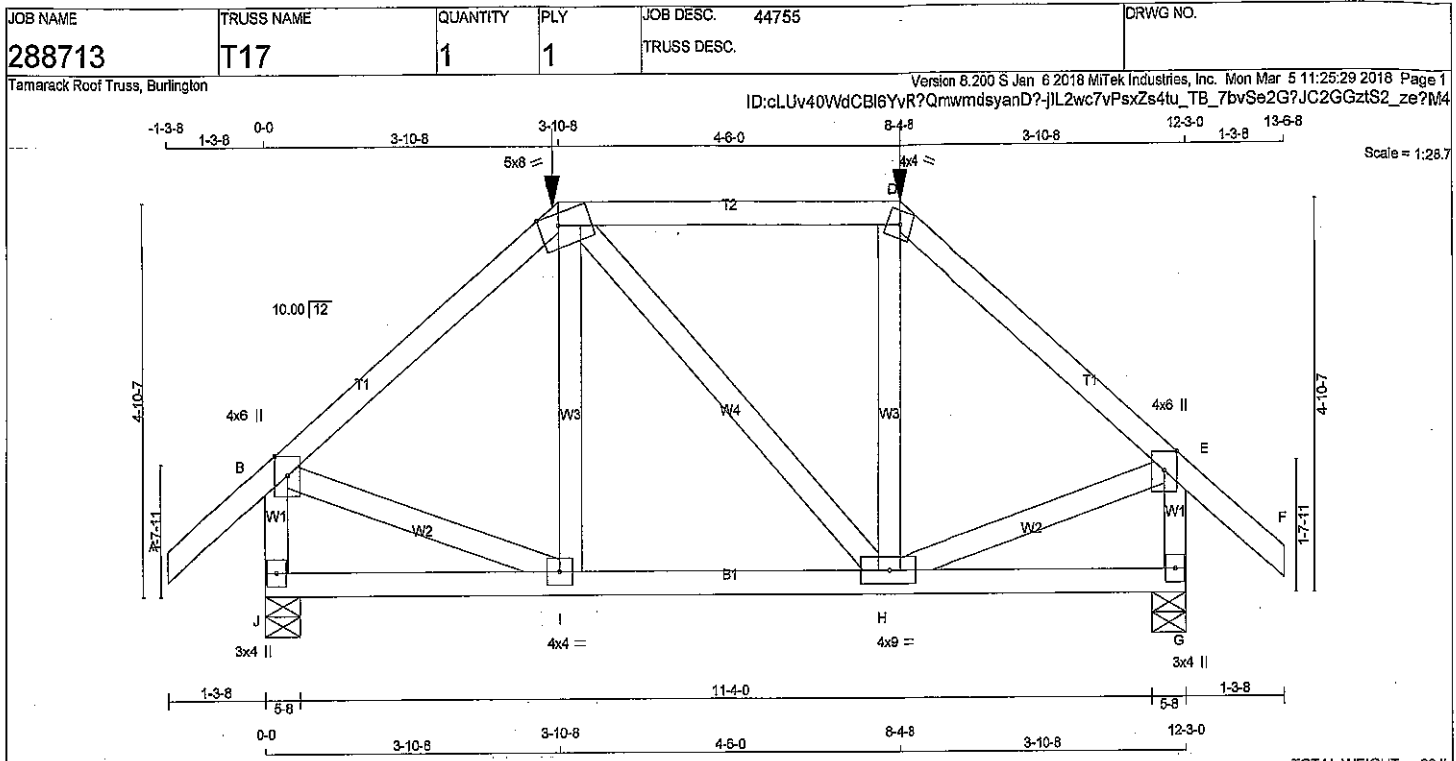
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



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DRWG NO. TAM 1196T-17
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
G - E	2x4	DRY No.2	SPF
J - G	2x4	DRY No.2	SPF

ALL WEBS 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVWW-t	MT20	4.0	9.0		
I	BMVWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 160.9 lbs FACTORED DOWN AT 8-4-8, AND 160.9 lbs FACTORED DOWN AT 3-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
J	1307	0	1307	0	5-8	5-8
G	1307	0	1307	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
J	1022	656 / 0	180 / 0	0 / 0	0 / 0	187 / 0	0 / 0
G	1022	656 / 0	180 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	CS1 (LC)			MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO		
A-B	0 / 47	-104.9	-104.9	0.16 (1)	10.00	I-C	-87 / 213	0.04 (3)
B-C	-1002 / 0	-104.9	-104.9	0.33 (1)	5.78	C-H	0 / 0	0.00 (1)
C-D	-765 / 0	-154.1	-154.1	0.61 (1)	5.78	H-D	-87 / 214	0.04 (3)
D-E	-1002 / 0	-104.9	-104.9	0.33 (1)	5.78	B-I	0 / 807	0.14 (1)
E-F	0 / 47	-104.9	-104.9	0.16 (1)	10.00	H-E	0 / 807	0.14 (1)
J-B	-1241 / 0	0.0	0.0	0.14 (1)	7.17			
G-E	-1241 / 0	0.0	0.0	0.14 (1)	7.17			
J-I	0 / 0	-40.4	-40.4	0.17 (3)	10.00			
I-H	0 / 765	-40.4	-40.4	0.28 (2)	10.00			
H-G	0 / 0	-40.4	-40.4	0.17 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+ FACE	DIR.	TYPE
C	3-10-8	-161	-161	---	FRONT	VERT TOTAL
D	8-4-8	-161	-161	---	FRONT	VERT TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 3.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 3-10-8

END SETBACK = 3-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")

CALCULATED VERT. DEFL. (LL) = L/999 (0.02")

ALLOWABLE DEFL. (TL) = L/360 (0.41")

CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.61/1.00 (C-D:1), BC=0.26/1.00 (H-I:2), WB=0.14/1.00 (B-I:1), SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354
	1667	822
	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

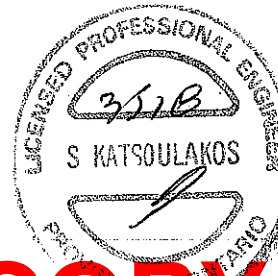
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.81 (B) (INPUT = 0.90)

JSI METAL = 0.29 (I) (INPUT = 1.00)

DWG NO. TAM 11962-88

STRUCTURAL COMPONENT ONLY



SITE COPY



DRY: SEASONED LUMBER.

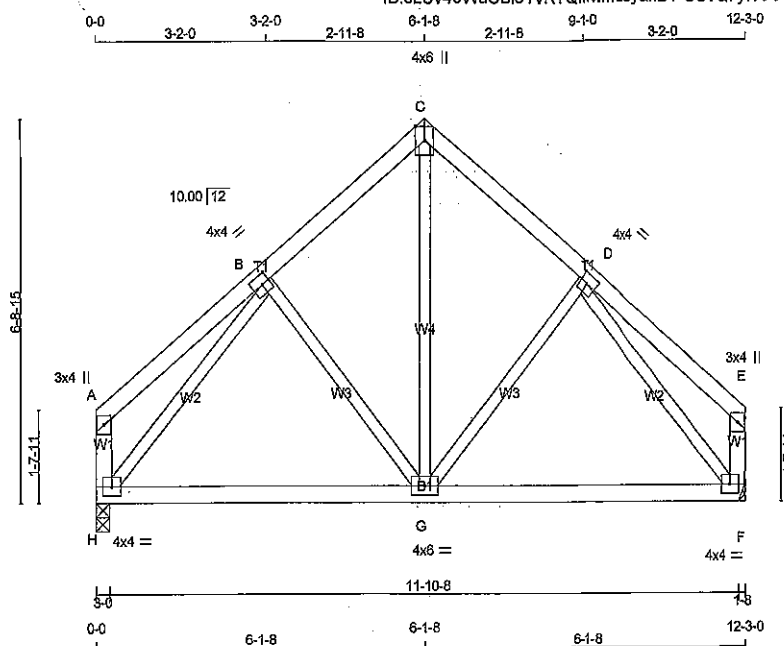
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JOB NAME 288713	TRUSS NAME T19	QUANTITY 3	PLY 1	JOB DESC. 44755 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 11:25:30 2018 Page 1

ID:cLUv40WdCBi6YyR?QmwmDsyand7-CUvQ7y7XAA3QUES4YBIDfpRkXRbx2dmPUdd0bQze?M3



Scale = 1:40.7

TOTAL WEIGHT = 3 X 55 = 165 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY No.2	SPF	
C - E	2x4	DRY No.2	SPF	
H - A	2x4	DRY No.2	SPF	
F - E	2x4	DRY No.2	SPF	
H - F	2x4	DRY No.2	SPF	
ALL WEBS	2x3	DRY No.2	SPF	
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW-t	MT20	4.0	4.0	2.00	1.75
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW-t	MT20	4.0	4.0		
G	BMVW-t	MT20	4.0	6.0		
H	BMVW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ
JT	811	0	811	0
H	811	0	811	0
F	811	0	811	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	643	398 / 0	122 / 0	0 / 0	0 / 0	122 / 0	0 / 0
H	643	398 / 0	122 / 0	0 / 0	0 / 0	122 / 0	0 / 0
F	643	398 / 0	122 / 0	0 / 0	0 / 0	122 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 23	-104.9 -104.9	0.15 (1)	10.00	G-C	0 / 424	0.10 (1)
B-C	-552 / 0	-104.9 -104.9	0.12 (1)	6.25	G-D	-135 / 37	0.08 (1)
C-D	-552 / 0	-104.9 -104.9	0.12 (1)	6.25	B-G	-135 / 37	0.08 (1)
D-E	0 / 23	-104.9 -104.9	0.15 (1)	10.00	H-B	-795 / 0	0.30 (1)
H-A	-123 / 0	0.0 0.0	0.01 (1)	7.81	D-F	-795 / 0	0.30 (1)
F-E	-123 / 0	0.0 0.0	0.01 (1)	7.81			
H-G	0 / 494	-27.5 -27.5	0.34 (2)	10.00			
G-F	0 / 494	-27.5 -27.5	0.34 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 32.5 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (A-B:1), BC=0.34/1.00 (G-H:2)
WB=0.30/1.00 (B-H:1), SS=0.14/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	618	354	1867	822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.78 (D) (INPUT = 0.90)
JSI METAL= 0.29 (D) (INPUT = 1.00)

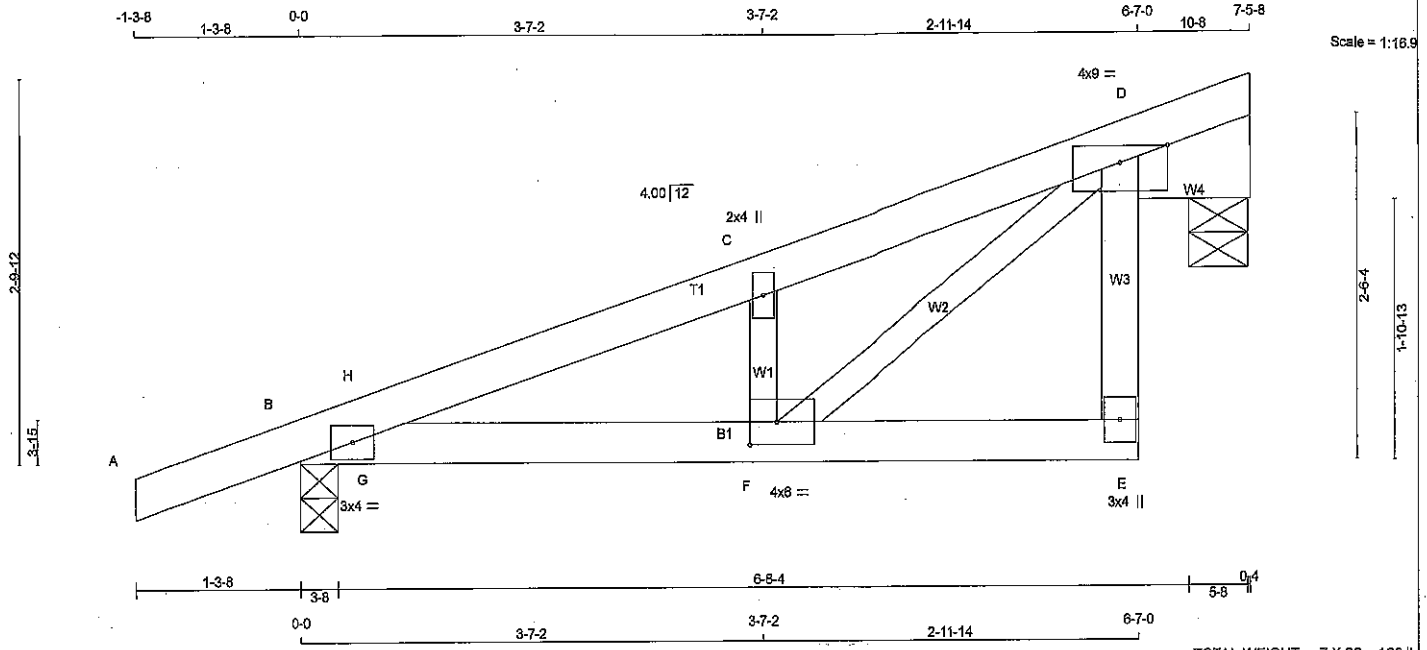


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DRWG NO. TAM 11964-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 288713	TRUSS NAME T20	QUANTITY 7	PLY 1	JOB DESC. 44755	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.200 S Jan 8 2018 Mitek Industries, Inc. Mon Mar 5 11:25:30 2018 Page 1
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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TM51-I	MT20	3.0	4.0	
C	TMVW-w	MT20	2.0	4.0	
D	TMVWW1-p	MT20	4.0	9.0	Edge
E	BMV-p	MT20	3.0	4.0	
F	BMWW-I	MT20	4.0	6.0	2.00 2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
D	436	0	436	0	5-8
B	576	0	576	0	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
D	345	214 / 0	66 / 0	0 / 0	66 / 0	0 / 0
B	440	301 / 0	66 / 0	0 / 0	74 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. MEMB. UNBRAC LENGTH FR-TO	FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX
FR-TO		FROM	TO				
A-B	0 / 21	-104.9	-104.9	0.13 (1)	10.00	F-C	-379 / 0
B-H	-738 / 0	-104.9	-104.9	0.03 (2)	6.25	F-D	0 / 628
H-C	-703 / 0	-104.9	-104.9	0.11 (1)	6.25	G-H	-81 / 55
C-D	-710 / 0	-104.9	-104.9	0.10 (1)	6.25		
E-D	0 / 57	0.0	0.0	0.01 (2)	10.00		
B-G	0 / 679	-27.5	-27.5	0.19 (1)	10.00		
G-F	0 / 679	-27.5	-27.5	0.20 (1)	10.00		
F-E	0 / 0	-27.5	-27.5	0.06 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.13/1.00 (A-B:1), BC=0.20/1.00 (F-G:1),
WB=0.19/1.00 (D-F:1), SSI=0.14/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX. MIN.	MAX. MIN.	MAX. MIN.	
MT20	618	354	1667 822 2284 1856

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

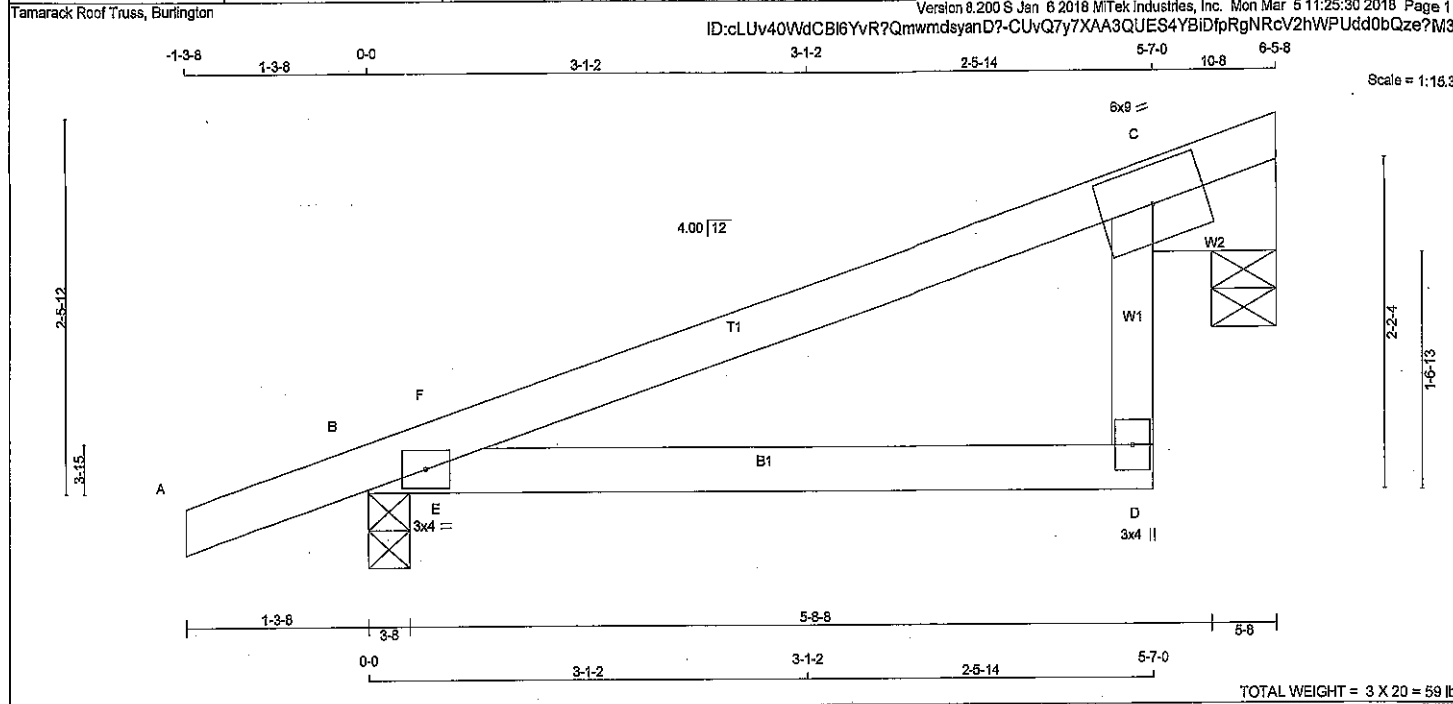
JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



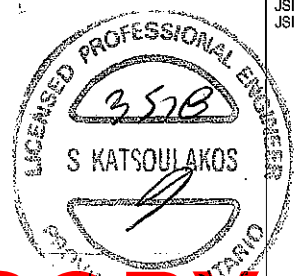
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DRWG NO. TAM 11965-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288713	T21	3	1	TRUSS DESC.		

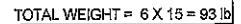


LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER A - C 2x4 DRY No.2 D - C 2x4 DRY No.2 B - D 2x4 DRY No.2 DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 3.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 089-09 - TPIC 2011 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.19") CALCULATED VERT. DEFL.(LL) = L/730 (0.09") ALLOWABLE DEFL.(TL)= L/360 (0.19") CALCULATED VERT. DEFL.(TL) = L/449 (0.15") CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.28/1.00 (B-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX. MIN MAX MIN MAX MIN MT20 616 354 1667 622 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.29 (B) (INPUT = 0.90) JSI METAL= 0.09 (B) (INPUT = 1.00)				BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT BRG REQD BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX C 370 0 370 0 0 5-8 5-8 B 510 0 510 0 0 3-8 3-8 BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL C 293 181/0 56/0 0/0 0/0 56/0 0/0 B 388 268/0 56/0 0/0 0/0 64/0 0/0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. UNBRACED LENGTH FR-TO TO FR-TO WEBS MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH FR-TO TO FR-TO A-B 0/21 -104.9 -104.9 0.13 (1) 10.00 E-F -318/152 0.00 (1) B-F -55/6 -104.9 -104.9 0.10 (3) 6.25 F-C 0/6 -104.9 -104.9 0.42 (1) 10.00 D-C 0/130 0.0 0.0 0.02 (2) 10.00 B-E 0/0 -27.5 -27.5 0.29 (1) 10.00 E-D 0/0 -27.5 -27.5 0.31 (1) 10.00			
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DRWG NO. TAM 11966-18
STRUCTURAL
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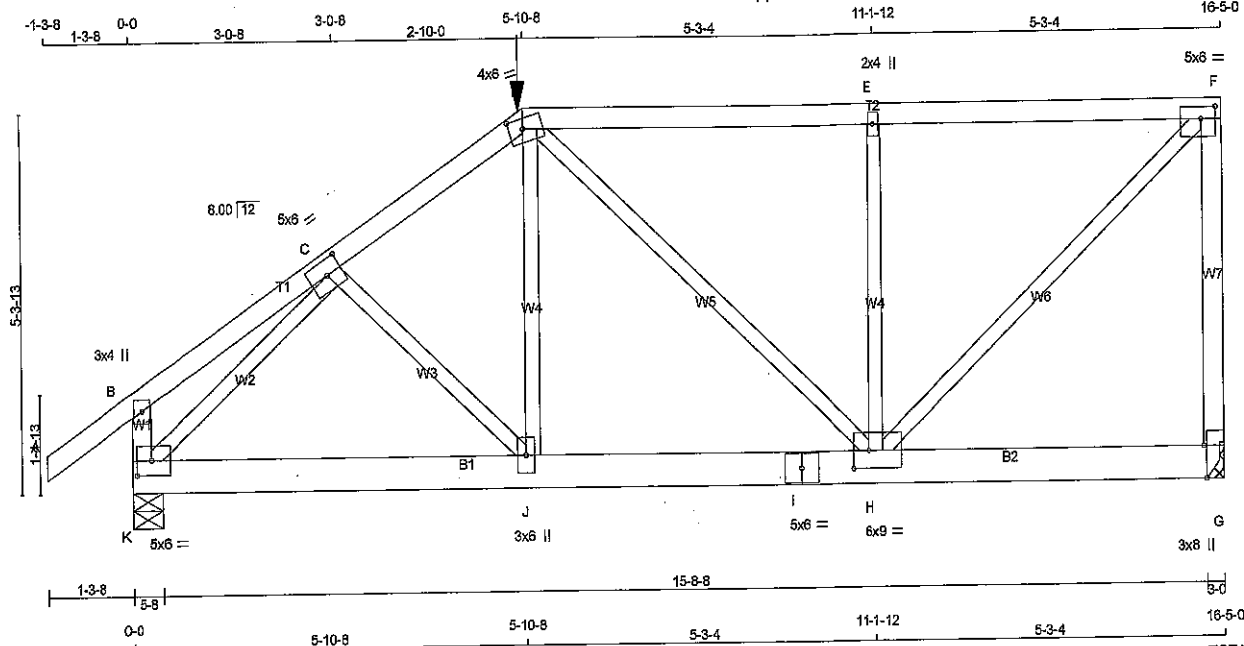
DWG NO. TAM 11976-17
STRUCTURAL
COMPONENT ONLY



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Tamarack Roof Truss, Burlington

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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF
A - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	1850F 1.5E	SPF		
G - F	2x4	DRY	No.2	SPF		
K - B	2x4	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
I - G	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	TTVW-w	MT20	4.0	6.0	1.75	2.50
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.00	2.50
G	BMV1-t	MT20	3.0	6.0	Edge	0.50
H	BMVWW-t	MT20	6.0	9.0	3.00	3.00
I	BS-t	MT20	5.0	6.0		
J	BMVWW-t	MT20	3.0	6.0		
K	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 449.5 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
G	2164	0	2164	0
K	2064	0	2064	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	1708	1069 / 0	318 / 0	0 / 0	0 / 0	0 / 0	321 / 0	0 / 0
K	1641	1007 / 0	318 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LCI (LC)	MAX. FACTORED CSI (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						FR-TO			
A-B	0 / 40		-104.9	-104.9	0.15 (1)	C-J	0 / 181	0.04 (2)	
B-C	0 / 16		-104.9	-104.9	0.13 (1)	J-D	0 / 463	0.11 (3)	
C-D	-2138 / 0		-104.9	-104.9	0.19 (1)	D-H	-72 / 0	0.07 (2)	
D-E	-1734 / 0		-203.3	-203.3	0.93 (1)	H-E	-1323 / 0	0.54 (1)	
E-F	-1734 / 0		-203.3	-203.3	0.94 (1)	H-F	0 / 2378	0.59 (1)	
G-F	-2034 / 0		0.0	0.0	0.94 (1)	K-C	-2338 / 0	0.68 (1)	
K-B	-289 / 0		0.0	0.0	0.03 (1)				
K-J	0 / 1647		-53.3	-53.3	0.34 (2)				
J-I	0 / 1767		-53.3	-53.3	0.35 (2)				
I-H	0 / 1767		-53.3	-53.3	0.35 (2)				
H-G	0 / 0		-53.3	-53.3	0.17 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-450	-450		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPPrimeHip
LEFT SETBACK = 5-10-8
RIGHT SETBACK = 0-0
END SETBACK = 5-10-8
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.55")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.55")
CALCULATED VERT. DEFL.(TL)= L/999 (0.09")

CSI: TC=0.94/1.00 (F-G:1), BC=0.35/1.00 (H-J:2), WB=0.68/1.00 (C-K:1), SSI=0.57/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

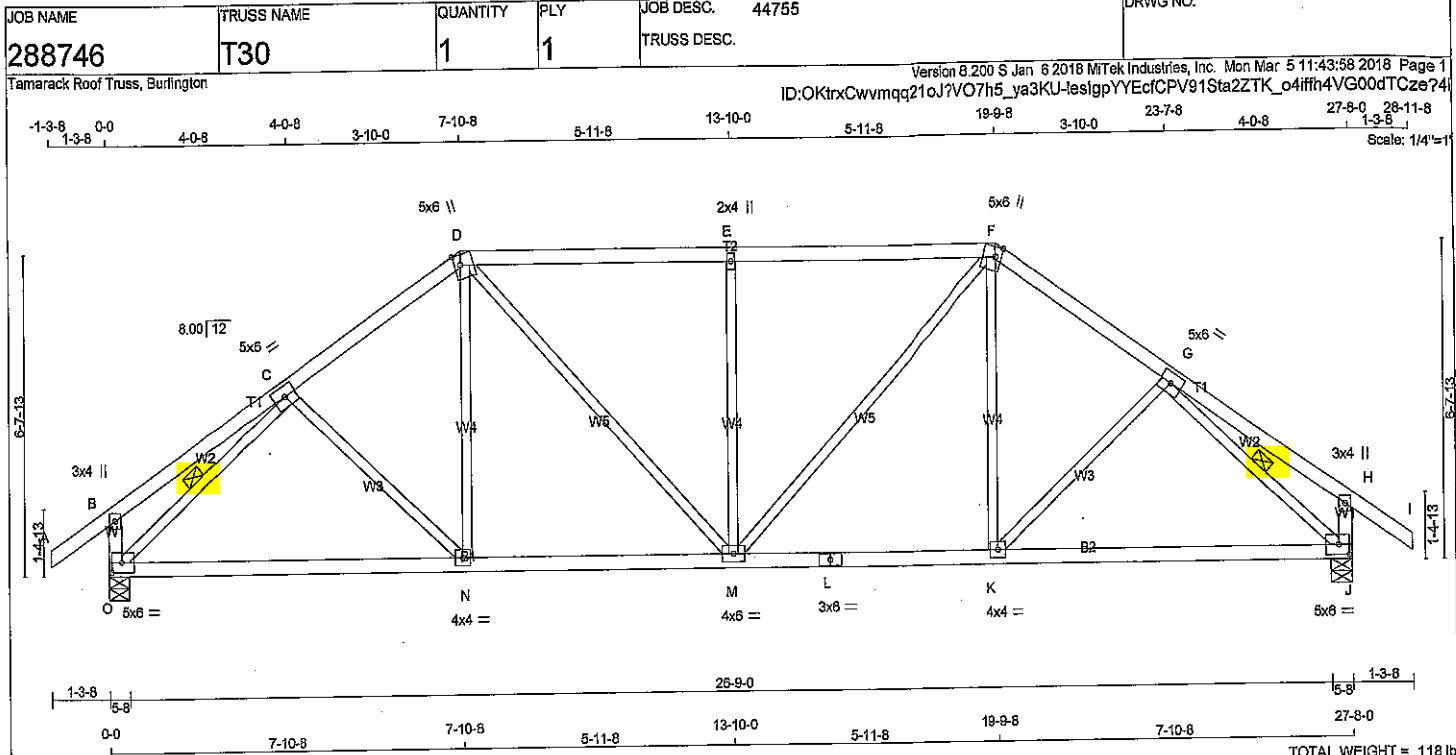
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.58 (C) (INPUT = 1.00)

DRWG NO. TAM 11 987-18
STRUCTURAL
COMPONENT ONLY



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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A - B	TMV+p	MT20	3.0	4.0
C - D	TMVW-i	MT20	5.0	6.0
D - E	TTVW+m	MT20	5.0	6.0 2.50 1.50
E - F	TMVW+w	MT20	2.0	4.0
F - G	TTVW+m	MT20	5.0	6.0 2.50 1.50
G - H	TMVW-i	MT20	5.0	6.0
H - I	TMV+p	MT20	3.0	4.0
J - K	BMVW-i	MT20	5.0	6.0 2.50 2.75
L - M	BS-i	MT20	4.0	6.0
M - N	BMVW-i	MT20	4.0	6.0
N - O	BMVW-i	MT20	4.0	6.0
O - P	BMVW-i	MT20	5.0	6.0 2.50 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	1976	0	1976	0	5-8
J	1976	0	1976	0	5-8

UNFACTORED REACTIONS		1ST CASE	MAX / MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM. LIVE
O	1549	988 / 0	277 / 0	0 / 0
J	1549	988 / 0	277 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 40	-104.9 -104.9	10.00
B-C	0 / 25	-104.9 -104.9	10.00
C-D	-2010 / 0	-104.9 -104.9	4.57
D-E	-2067 / 0	-104.9 -104.9	4.10
E-F	-2067 / 0	-104.9 -104.9	4.10
F-G	-2010 / 0	-104.9 -104.9	4.57
G-H	0 / 25	-104.9 -104.9	10.00
H-I	0 / 40	-104.9 -104.9	10.00
O-B	-305 / 0	0.0 0.0	7.81
J-H	-305 / 0	0.0 0.0	7.81
O-N	0 / 1679	-27.5 -27.5	10.00
N-M	0 / 1653	-27.5 -27.5	10.00
M-L	0 / 1653	-27.5 -27.5	10.00
L-K	0 / 1653	-27.5 -27.5	10.00
K-J	0 / 1679	-27.5 -27.5	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.28")

CSI: TC=0.54/1.00 (D-E:1), BC=0.57/1.00 (K-M:2), WB=0.56/1.00 (E-M:1), SI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

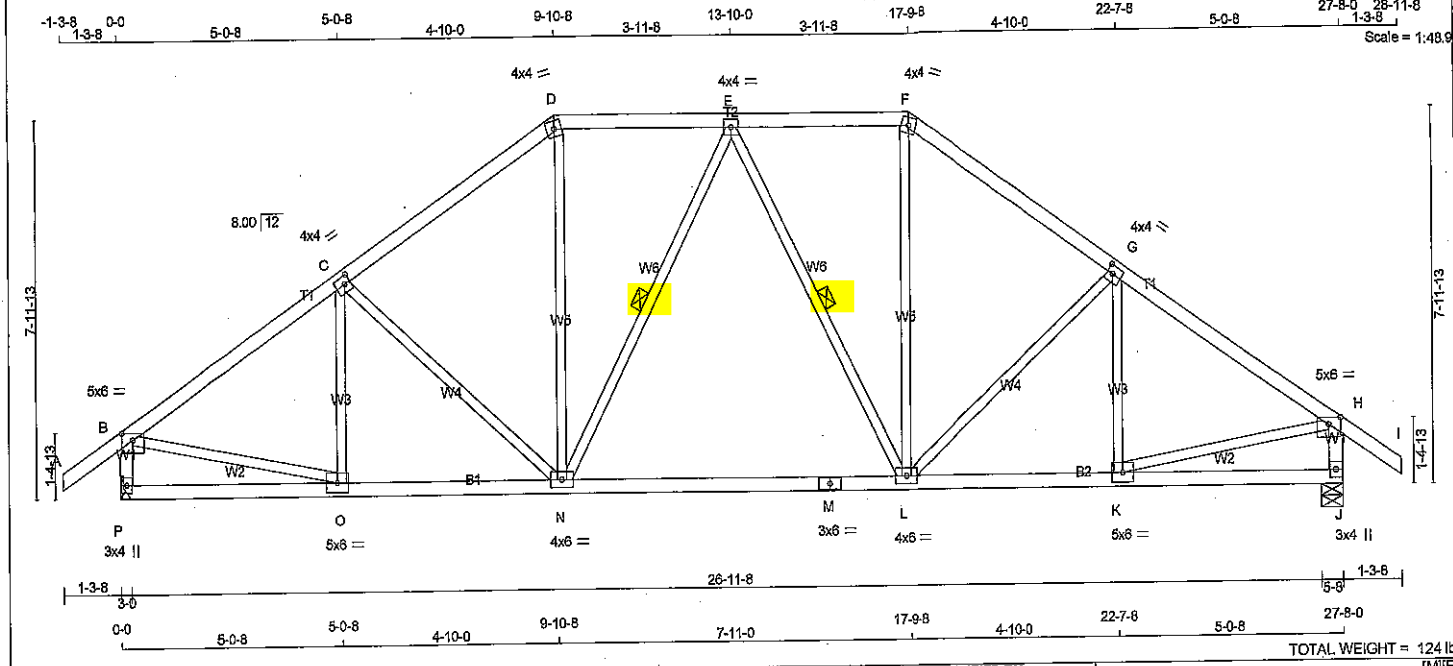
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.57 (C) (INPUT = 1.00)



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DWG NO. TAM 12005-18
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LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.75 3.00
C	TMVWV-t	MT20	4.0	4.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	
E	TMVWV-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.0	
G	TMVWV-t	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	6.0	1.75 3.00
J	BMV1+p	MT20	3.0	4.0	
K	BMVWV-t	MT20	5.0	6.0	
L	BMVWV-t	MT20	4.0	6.0	
M	BS-t	MT20	3.0	6.0	
N	BMVWV-t	MT20	4.0	6.0	
O	BMVWV-t	MT20	5.0	6.0	
P	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	HANGER BY OTHERS	MIN. SEAT SIZE: 3-0
P	1976	0	1976	0	0	5-8	5-8		
J	1976	0	1976	0	0	5-8	5-8		

UNFACTORED REACTIONS						
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
P	COMBINED	SNOW	LIVE	0/0	0/0	0/0
J	1549	988 / 0	277 / 0	0/0	285 / 0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.31 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 40	-104.9	-104.9	0.14 (1)	10.00	O-C	-250 / 78	0.08 (1)	
B-C	-2103 / 0	-104.9	-104.9	0.39 (1)	4.31	C-N	-339 / 0	0.27 (1)	
C-D	-1870 / 0	-104.9	-104.9	0.37 (1)	4.53	N-D	0 / 664	0.15 (1)	
D-E	-1534 / 0	-104.9	-104.9	0.22 (1)	5.09	E-L	-279 / 0	0.14 (1)	
E-F	-1534 / 0	-104.9	-104.9	0.22 (1)	5.09	E-L	-279 / 0	0.14 (1)	
F-G	-1870 / 0	-104.9	-104.9	0.37 (1)	4.53	L-F	0 / 664	0.15 (1)	
G-H	-2103 / 0	-104.9	-104.9	0.39 (1)	4.31	L-G	-339 / 0	0.27 (1)	
H-I	0 / 40	-104.9	-104.9	0.14 (1)	10.00	K-G	-250 / 78	0.08 (1)	
P-B	-1914 / 0	0.0	0.0	0.20 (1)	6.07	B-O	0 / 1817	0.41 (1)	
J-H	-1914 / 0	0.0	0.0	0.20 (1)	6.07	K-H	0 / 1817	0.41 (1)	
P-O	0 / 0	-27.5	-27.5	0.15 (2)	10.00				
O-N	0 / 1777	-27.5	-27.5	0.45 (2)	10.00				
N-M	0 / 1659	-27.5	-27.5	0.43 (2)	10.00				
M-L	0 / 1659	-27.5	-27.5	0.43 (2)	10.00				
L-K	0 / 1777	-27.5	-27.5	0.45 (2)	10.00				
K-J	0 / 0	-27.5	-27.5	0.15 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:		
TOP CH.	LL	= 32.5 PSF
	DL	= 3.0 PSF
BOT CH.	LL	= 10.0 PSF
	DL	= 7.0 PSF
TOTAL LOAD		= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.92")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.92")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.39/1.00 (B-C:1), BC=0.45/1.00 (N-O:2), WB=0.41/1.00 (B-O:1), SS=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1687	822	2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (O) (INPUT = 0.90)
JSI METAL= 0.48 (M) (INPUT = 1.00)

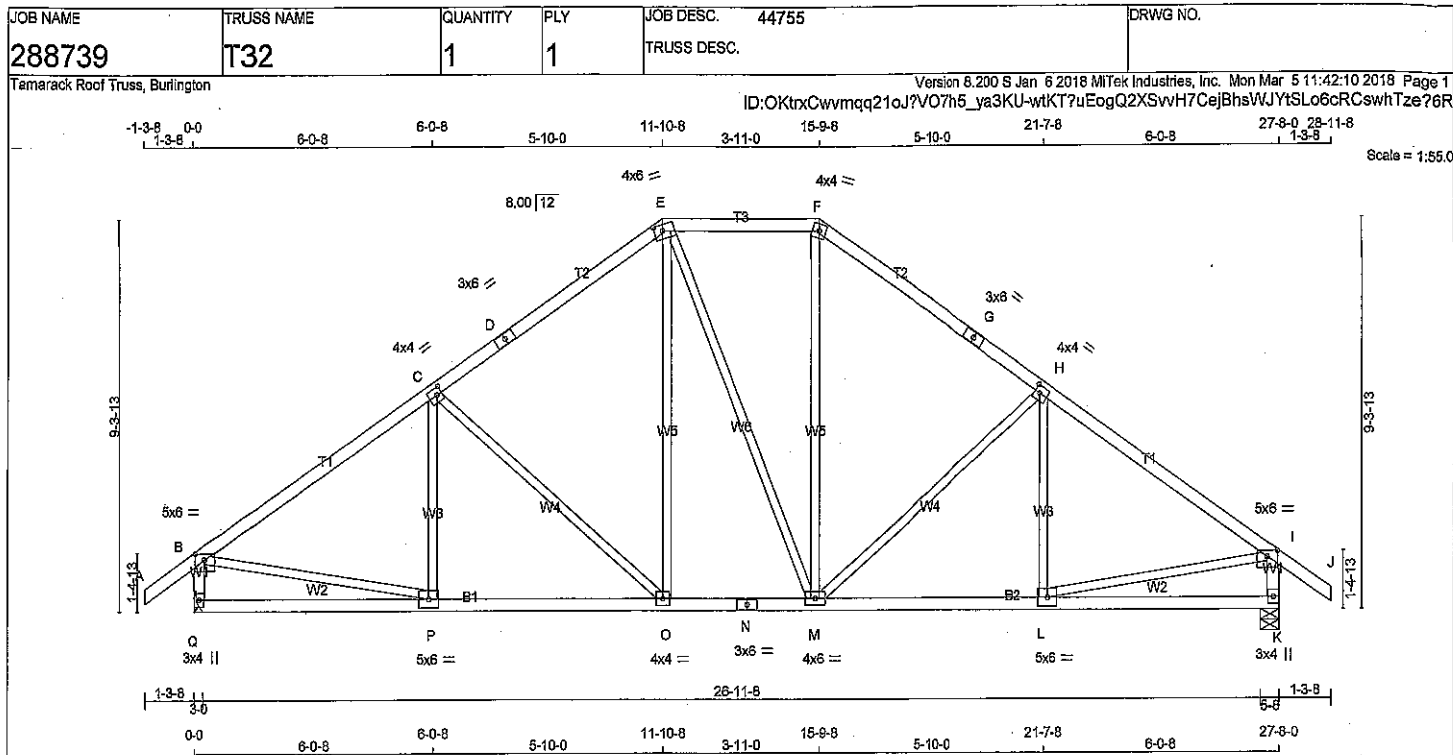
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DRWG NO. TAM 11988-18
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LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 126 lb									
N. L. G. A. RULES										BEARINGS										[M]F									
CHORDS SIZE																				DESIGN CRITERIA									
A - D 2x4 DRY No.2										FACTORED MAXIMUM FACTORED INPUT REQD										SPECIFIED LOADS:									
D - E 2x4 DRY No.2										GROSS REACTION GROSS REACTION BRG BRG										TOP CH. LL = 32.5 PSF									
E - F 2x4 DRY No.2										DOWN HORZ UPLIFT IN-SX IN-SX										DL = 3.0 PSF									
F - G 2x4 DRY No.2										HANGER BY OTHERS										BOT CH. LL = 10.0 PSF									
G - J 2x4 DRY No.2										MIN. SEAT SIZE: 3-0										DL = 7.0 PSF									
Q - B 2x4 DRY No.2																				TOTAL LOAD = 52.5 PSF									
K - I 2x4 DRY No.2																				SPACING = 24.0 IN. C/C									
Q - N 2x4 DRY No.2																													
N - K 2x4 DRY No.2																													
ALL WEBS 2x3 DRY No.2																				LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
EXCEPT																				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010									
DRY, SEASONED LUMBER.																				THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD									
PLATES (table is in inches)										UNFACTORED REACTIONS										ALLOWABLE DEFL.(LL)= L/360 (0.92")									
JT TYPE PLATES W LEN Y X										1ST LCASE MAX/MIN COMPONENT REACTIONS										CALCULATED VERT. DEFL.(LL) = L/999 (0.09")									
B TMVW-p MT20 5.0 6.0 1.75 3.00										JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL										ALLOWABLE DEFL.(TL)= L/360 (0.92")									
C TMWW-t MT20 4.0 4.0 2.00 1.50										Q 1549 988 / 0 277 / 0 0 / 0 0 / 0 285 / 0 0 / 0										CALCULATED VERT. DEFL.(TL) = L/999 (0.15")									
D TS-t MT20 3.0 6.0										K 1549 988 / 0 277 / 0 0 / 0 0 / 0 285 / 0 0 / 0										CSI: TC=0.59/1.00 (B-C:1), BC=0.44/1.00 (O-P:2), WB=0.70/1.00 (C-O:1), SS=0.25/1.00 (H-I:1)									
E TTWW-m MT20 4.0 6.0 1.75 2.50										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									
F TTW-m MT20 4.0 4.0										BRACING										COMP=1.10 SHEAR=1.10 TENS=1.10									
G TS-t MT20 3.0 6.0										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.										COMPANION LIVE LOAD FACTOR = 0.50									
H TMVW-t MT20 4.0 4.0 2.00 1.50										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
I TMVW-p MT20 5.0 6.0 1.75 3.00										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										NAIL VALUES									
K BMV1+p MT20 3.0 4.0										LOADING										PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)									
L BMWW-t MT20 5.0 6.0										TOTAL LOAD CASES: (4)										MAX MIN MAX MIN MAX MIN									
M BMWW-t MT20 4.0 6.0										CHORDS FACTORED WEBS FACTORED										MT20 618 354 1667 822 2284 1656									
N BS-t MT20 3.0 6.0										MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX.										PLATE PLACEMENT TOL = 0.250 inches									
O BMWW-t MT20 4.0 4.0										(LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC)										PLATE ROTATION TOL = 5.0 Deg.									
P BMWW-t MT20 5.0 6.0										FR-TO FROM TO										JSI GRIP= 0.85 (B) (INPUT = 0.90)									
Q BMV1+p MT20 3.0 4.0										A-B 0 / 40 -104.9 -104.9 0.14 (1) 10.00 P-C -133 / 199 0.06 (1)										JSI METAL= 0.45 (P) (INPUT = 1.00)									
										B-C -2122 / 0 -104.9 -104.9 0.59 (1) 4.03 C-O -557 / 0 0.70 (1)																			
										C-D -1700 / 0 -104.9 -104.9 0.54 (1) 4.47 O-E 0 / 485 0.11 (1)																			
										D-E -1700 / 0 -104.9 -104.9 0.54 (1) 4.47 E-M 0 / 3 0.00 (1)																			
										E-F -1385 / 0 -104.9 -104.9 0.23 (1) 5.29 M-F 0 / 488 0.11 (1)																			
										F-G -1702 / 0 -104.9 -104.9 0.54 (1) 4.47 M-H -555 / 0 0.70 (1)																			
										G-H -1702 / 0 -104.9 -104.9 0.54 (1) 4.47 L-H -136 / 197 0.08 (1)																			
										H-I -2121 / 0 -104.9 -104.9 0.59 (1) 4.03 B-P 0 / 1828 0.41 (1)																			
										I-J 0 / 40 -104.9 -104.9 0.14 (1) 10.00 L-I 0 / 1828 0.41 (1)																			
										Q-B -1908 / 0 0.0 0.0 0.20 (1) 6.08																			
										K-I -1908 / 0 0.0 0.0 0.20 (1) 6.08																			
										Q-P 0 / 0 -27.5 -27.5 0.25 (3) 10.00																			
										P-O 0 / 1800 -27.5 -27.5 0.44 (2) 10.00																			
										O-N 0 / 1384 -27.5 -27.5 0.30 (1) 10.00																			
										N-M 0 / 1384 -27.5 -27.5 0.30 (1) 10.00																			
										M-L 0 / 1799 -27.5 -27.5 0.43 (2) 10.00																			
										L-K 0 / 0 -27.5 -27.5 0.25 (3) 10.00																			

3318

S KATSOUKAKOS

PROVINCE OF ONTARIO

DWG NO. TAM 11989-18

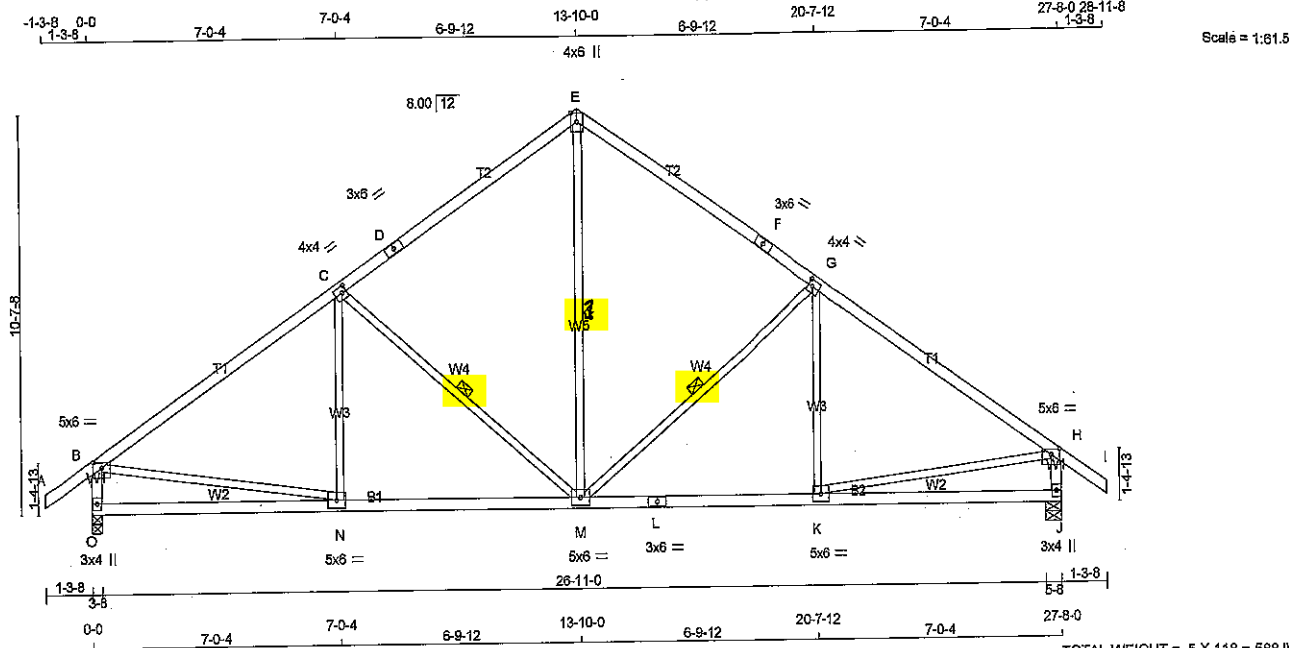
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DWG NO. TAM 11989-08
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LUMBER			
N.L.G.A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - O	2x4	DRY	No.2
O - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.75 3.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	6.0	
E	TTW+p	MT20	4.0	6.0	Edge
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	6.0	1.75 3.00
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	6.0	
L	BS-t	MT20	3.0	6.0	
M	BMVW-t	MT20	5.0	6.0	
N	BMVW-t	MT20	5.0	6.0	
O	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
O	1976	0	1976	0	0	3-8	3-8	3-8
J	1976	0	1976	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	PERM. LIVE			
O	1549	998 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1549	998 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M. *EMF*

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-104.9 -104.9	0.14 (1)	10.00	M-E	0 / 1117	0.25 (1)
B-C	-2108 / 0	-104.9 -104.9	0.83 (1)	3.64	M-G	-727 / 0	0.40 (1)
C-D	-1539 / 0	-104.9 -104.9	0.73 (1)	4.27	K-G	-83 / 281	0.05 (3)
D-E	-1539 / 0	-104.9 -104.9	0.73 (1)	4.27	C-M	-727 / 0	0.40 (1)
E-F	-1539 / 0	-104.9 -104.9	0.73 (1)	4.27	N-C	-83 / 281	0.05 (3)
F-G	-1539 / 0	-104.9 -104.9	0.73 (1)	4.27	B-N	0 / 1815	0.41 (1)
G-H	-2108 / 0	-104.9 -104.9	0.83 (1)	3.64	K-H	0 / 1815	0.41 (1)
H-I	0 / 40	-104.9 -104.9	0.14 (1)	10.00			
O-B	-1897 / 0	0.0 0.0	0.20 (1)	6.10			
J-H	-1897 / 0	0.0 0.0	0.20 (1)	6.10			
O-N	0 / 0	-27.5 -27.5	0.33 (3)	10.00			
N-M	0 / 1794	-27.5 -27.5	0.51 (2)	10.00			
M-L	0 / 1794	-27.5 -27.5	0.51 (2)	10.00			
L-K	0 / 1794	-27.5 -27.5	0.51 (2)	10.00			
K-J	0 / 0	-27.5 -27.5	0.33 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.18")

CSI: TC=0.83/1.00 (B-C:1), BC=0.51/1.00 (M-N:2), WB=0.41/1.00 (B-N:1), SS=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

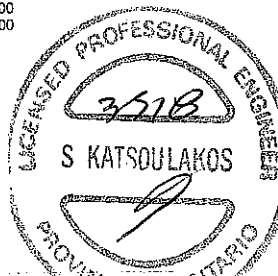
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

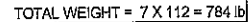
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.80)
JSI METAL= 0.52 (L) (INPUT = 1.00)



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DWG NO. TAM 11990-18
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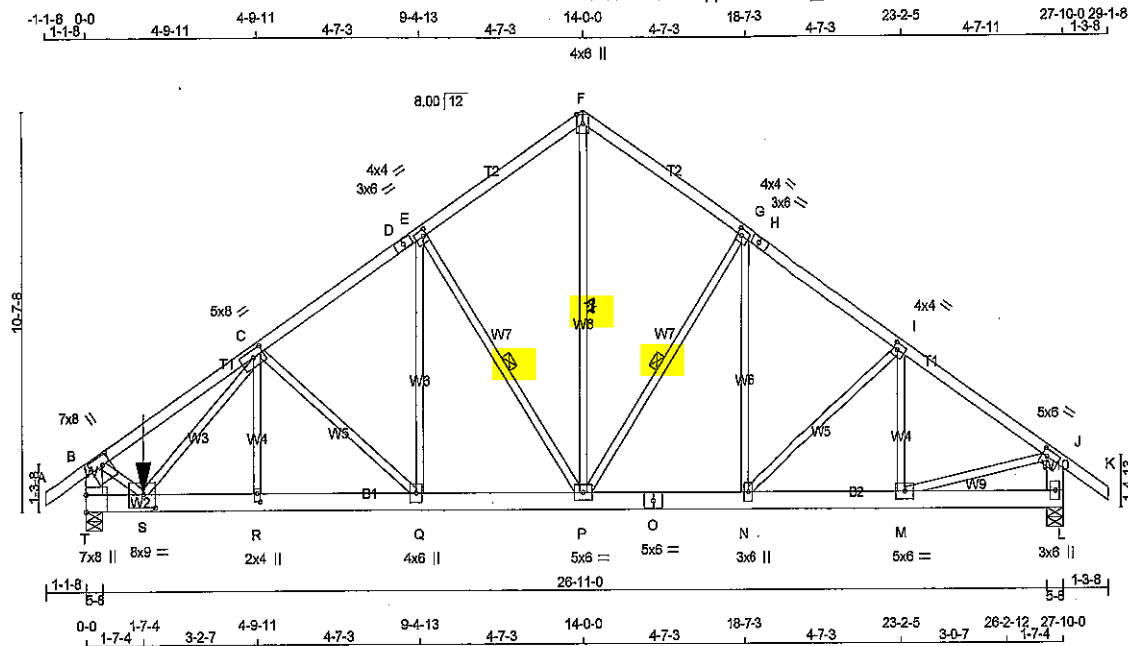
DWG NO. TAM 11991-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T34	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 11:42:11 2018 Page 1

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
H - K	2x4	DRY	No.2	SPF	
T - B	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
T - O	2x6	DRY	No.2	SPF	
O - L	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1 12	TOP	
D-F 1 12	TOP	
F-H 1 12	TOP	
H-K 1 12	TOP	
T-B 2 12	TOP	
L-J 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
T-O 2 12	SIDE(0.0)	
O-L 2 12	TOP	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHLF BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	7.0	8.0	3.00	2.75
C	TMWWW-t	MT20	5.0	8.0	2.25	3.75
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMWW-t	MT20	5.0	6.0	2.25	1.75
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	3.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	2.0	4.0	2.50	1.00
S	BMWW-t	MT20	8.0	9.0	4.25	4.00
T	BMV1-t	MT20	7.0	8.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	DOWN	HORZ	UPLIFT	INPUT BRG	REQD BRG
T	8503	0	9503	0	0	0	5-8	5-8	5-8
L	2448	0	2448	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	7459	4741 / 0	1341 / 0	0 / 0	0 / 0	0 / 0	1377 / 0	0 / 0	0 / 0
L	1920	1223 / 0	343 / 0	0 / 0	0 / 0	0 / 0	353 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, G-P, F-P

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORZ. LOAD (LC)	MAX. UNBRACED LENGTH (FR-TO)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FR-TO)
A-B	0 / 35	-104.9	-104.9	0.08 (1)	10.00	R-C	0 / 672	0.08 (1)
B-C	-8256 / 0	-104.9	-104.9	0.55 (1)	3.11	C-Q	-2298 / 0	0.76 (1)
C-D	-3368 / 0	-104.9	-104.9	0.23 (1)	4.88	Q-E	0 / 1497	0.19 (1)
D-E	-3368 / 0	-104.9	-104.9	0.23 (1)	4.88	E-P	-1690 / 0	0.42 (1)
E-F	-2297 / 0	-104.9	-104.9	0.19 (1)	5.69	P-F	0 / 2128	0.26 (1)
F-G	-2298 / 0	-104.9	-104.9	0.19 (1)	5.71	P-G	-595 / 0	0.15 (1)
G-H	-2659 / 0	-104.9	-104.9	0.20 (1)	5.38	N-G	0 / 258	0.03 (2)
H-I	-2659 / 0	-104.9	-104.9	0.20 (1)	5.38	N-I	-134 / 0	0.04 (1)
I-J	-2754 / 0	-104.9	-104.9	0.21 (1)	5.30	M-I	-374 / 35	0.05 (1)
J-K	0 / 40	-104.9	-104.9	0.08 (1)	10.00	B-S	0 / 7866	0.97 (1)
T-B	-8730 / 0	0.0	0.0	0.31 (1)	5.10	S-C	0 / 3846	0.48 (1)
L-J	-2381 / 0	0.0	0.0	0.09 (1)	7.81	M-J	0 / 2365	0.28 (1)

JT	LOC.	LC1	MAX.	MAX+	FACE
S	1-7-4	-7995	-7995	--	BACK

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE
S	1-7-4	-7995	-7995	--	BACK

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.55/1.00 (B-C:1), BC=0.74/1.00 (R-S:1), WB=0.97/1.00 (B-S:1), SSI=0.28/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

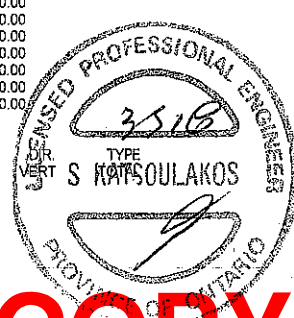
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.83 (B) (INPUT = 1.00)



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DRWG NO. TAM 11992-18
STRUCTURAL
COMPONENT ONLY

P61/2

CONTINUED ON PAGE 2

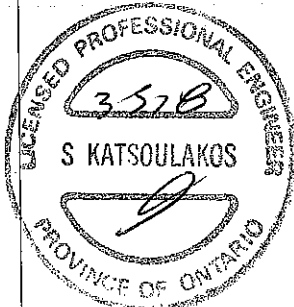
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T34	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Mon Mar 5 11:42:11 2018 Page 2
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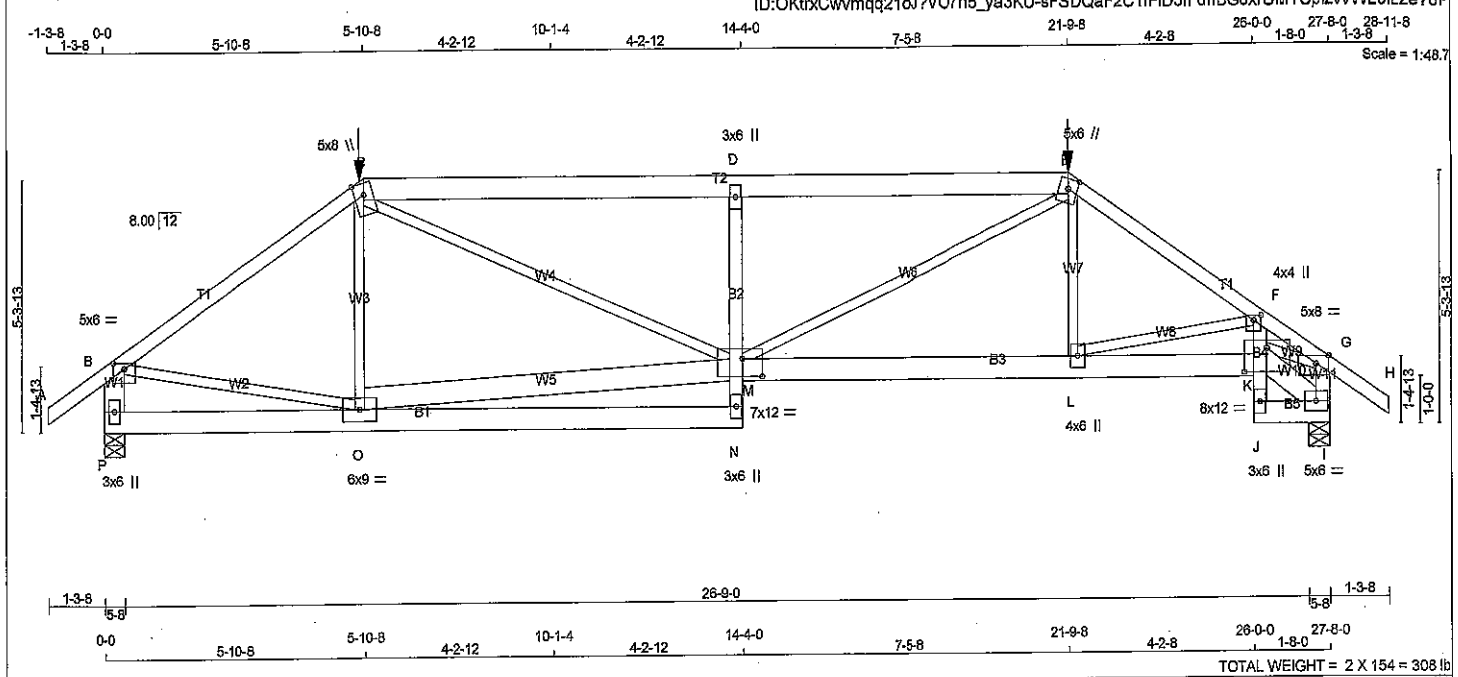
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 7994.6 lbs FACTORED DOWN AT 1-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



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DWG NO. TAM 11992-08
STRUCTURAL COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
P - B	2x3	DRY No.2	SPF
I - G	2x4	DRY No.2	SPF
P - N	2x6	DRY No.2	SPF
N - D	2x4	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
J - F	2x4	DRY No.2	SPF
J - I	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
O - M	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE (0.0)
E - H 1	12	SIDE (0.0)
I - G 1	12	TOP
C - E 2	12	SIDE (49.2)
P - B 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
P - N 2	12	SIDE (22.3)
M - K 2	12	SIDE (22.3)
J - I 2	12	SIDE (22.3)
N - D 1	12	TOP
F - J 1	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	8.0	Edge	
D	TMV+p	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	8.0	2.50	2.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	TMVW+p	MT20	5.0	8.0	Edge	3.25
I	BMVW1-t	MT20	5.0	6.0		
J	BMV+p	MT20	3.0	6.0		
K	BMVW1-t	MT20	8.0	12.0	6.00	6.00
L	BMVW1-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	7.0	12.0	4.50	5.25
N	BMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRO BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	3581	0	3581	0	5-8	5-8
I	3589	0	3589	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
P	2821	1748 / 0	536 / 0	0 / 0	537 / 0	0 / 0
I	2826	1753 / 0	536 / 0	0 / 0	537 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.53 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 40	-104.9 -104.9	0.08 (1)	10.00	O-C	-558 / 393	0.11 (1)
B-C	-4328 / 0	-104.9 -104.9	0.55 (1)	4.11	O-M	0 / 3468	0.23 (1)
C-D	-6904 / 0	-203.3 -203.3	0.68 (1)	3.80	C-M	0 / 3624	0.45 (1)
D-E	-6951 / 0	-203.3 -203.3	0.66 (1)	3.79	M-E	0 / 2737	0.34 (1)
E-F	-6424 / 0	-104.9 -104.9	0.34 (1)	3.89	L-E	0 / 777	0.10 (2)
F-G	-6594 / 0	-104.9 -104.9	0.21 (1)	3.86	L-F	-1377 / 0	0.23 (1)
G-H	0 / 40	-104.9 -104.9	0.08 (1)	10.00	B-O	0 / 3662	0.45 (1)
P-B	-3465 / 0	0.0 0.0	0.12 (1)	7.51	K-I	-379 / 0	0.01 (1)
I-G	-3297 / 0	0.0 0.0	0.18 (1)	6.41	K-G	0 / 5483	0.69 (1)
P-O	0 / 0	-53.3 -53.3	0.23 (3)	10.00			
O-N	0 / 180	-53.3 -53.3	0.23 (2)	10.00			
N-M	0 / 316	0.0 0.0	0.15 (1)	10.00			
M-D	-2016 / 0	0.0 0.0	0.24 (1)	7.53			
M-L	0 / 4513	-53.3 -53.3	0.39 (1)	10.00			
L-K	0 / 5868	-53.3 -53.3	0.48 (1)	10.00			
J-K	0 / 70	0.0 0.0	0.27 (1)	10.00			
K-F	0 / 721	0.0 0.0	0.33 (1)	10.00			
J-I	0 / 300	-53.3 -53.3	0.03 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR
C	5-10-8	-450	-450	---	FRONT	VERT
E	21-9-8	-450	-450	---	FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 1-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.82")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (0.82")
CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.68/1.00 (C-D:1), BC=0.48/1.00 (K-L:1),
WB=0.68/1.00 (G-K:1), SS=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWWV-I	MT20	6.0	9.0		
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

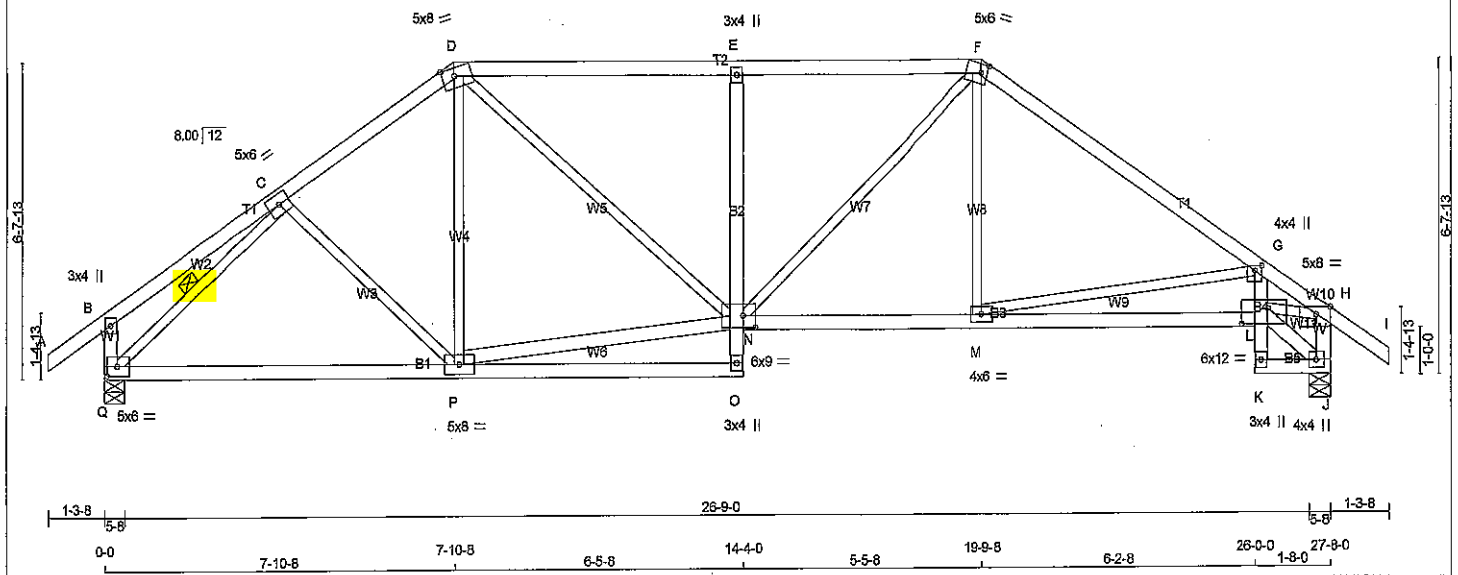
- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 448.5 lbs FACTORED DOWN AT 21-8-8, AND 449.5 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.53 (G) (INPUT = 1.00)



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DRWG NO. TAM 11993-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - O	2x4	DRY	No.2
O - E	2x4	DRY	No.2
N - L	2x4	DRY	No.2
K - G	2x4	DRY	No.2
K - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

P - N 2x4 DRY No.2

L - J 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTWW-m	MT20	5.0	8.0	2.00	3.25
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	1.75
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	TMVW-p	MT20	5.0	8.0	Edge	3.50
J	BMVW-t+p	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BMVW-t	MT20	6.0	12.0	4.00	7.00
M	BMVW-t	MT20	4.0	6.0		
N	BMVW-t	MT20	6.0	9.0	3.00	3.25
O	BMV+p	MT20	3.0	4.0		
P	BMVW-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REGR	IN-SX
Q	1972	0	1972	0	0	5-8	5-8	5-8
J	1880	0	1980	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1547	985 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
J	1552	990 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-104.9 -104.9	0.14 (1)	10.00	C-P	-41 / 120	0.03 (3)
B-C	0 / 25	-104.9 -104.9	0.25 (1)	10.00	P-D	0 / 231	0.05 (3)
C-D	-2010 / 0	-104.9 -104.9	0.24 (1)	4.57	P-N	0 / 1616	0.26 (1)
D-E	-2427 / 0	-104.9 -104.9	0.80 (1)	3.77	D-N	0 / 1021	0.23 (1)
E-F	-2437 / 0	-104.9 -104.9	0.57 (1)	3.78	N-F	0 / 684	0.15 (1)
F-G	-2357 / 0	-104.9 -104.9	0.69 (1)	3.72	M-F	0 / 413	0.09 (2)
G-H	-3409 / 0	-104.9 -104.9	0.45 (1)	3.37	M-G	-1154 / 0	0.91 (1)
H-I	0 / 40	-104.9 -104.9	0.14 (1)	10.00	Q-C	-2305 / 0	0.51 (1)
Q-B	-305 / 0	0.0 0.0	0.03 (1)	7.81	L-J	-172 / 0	0.02 (1)
J-H	-1854 / 0	0.0 0.0	0.19 (1)	6.16	L-H	0 / 2907	0.65 (1)
Q-P	0 / 1878	-27.5 -27.5	0.58 (2)	10.00			
P-O	0 / 53	-27.5 -27.5	0.42 (3)	10.00			
O-N	0 / 101	0.0 0.0	0.08 (1)	10.00			
N-E	-765 / 0	0.0 0.0	0.33 (1)	7.81			
N-M	0 / 1942	-27.5 -27.5	0.50 (2)	10.00			
M-L	0 / 3074	-27.5 -27.5	0.66 (1)	10.00			
K-L	0 / 36	0.0 0.0	0.22 (1)	10.00			
L-G	0 / 241	0.0 0.0	0.25 (1)	10.00			
K-J	0 / 137	-27.5 -27.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 3.0 PSF

BOT CH. LL = 10.0 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.92")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.15")

ALLOWABLE DEFL.(TL) = L/360 (0.92")

CALCULATED VERT. DEFL.(TL) = 1/999 (0.26")

CSI: TC=0.69/1.00 (F-G:1), BC=0.66/1.00 (L-M:1), WB=0.91/1.00 (G-M:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

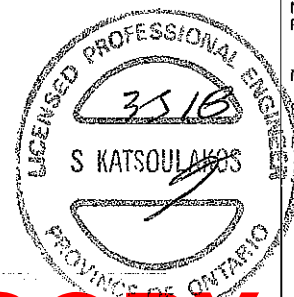
PLATE	GRIP(ORY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
MT20	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (J) (INPUT = 0.90)

SI METAL= 0.57 (C) (INPUT = 1.00)



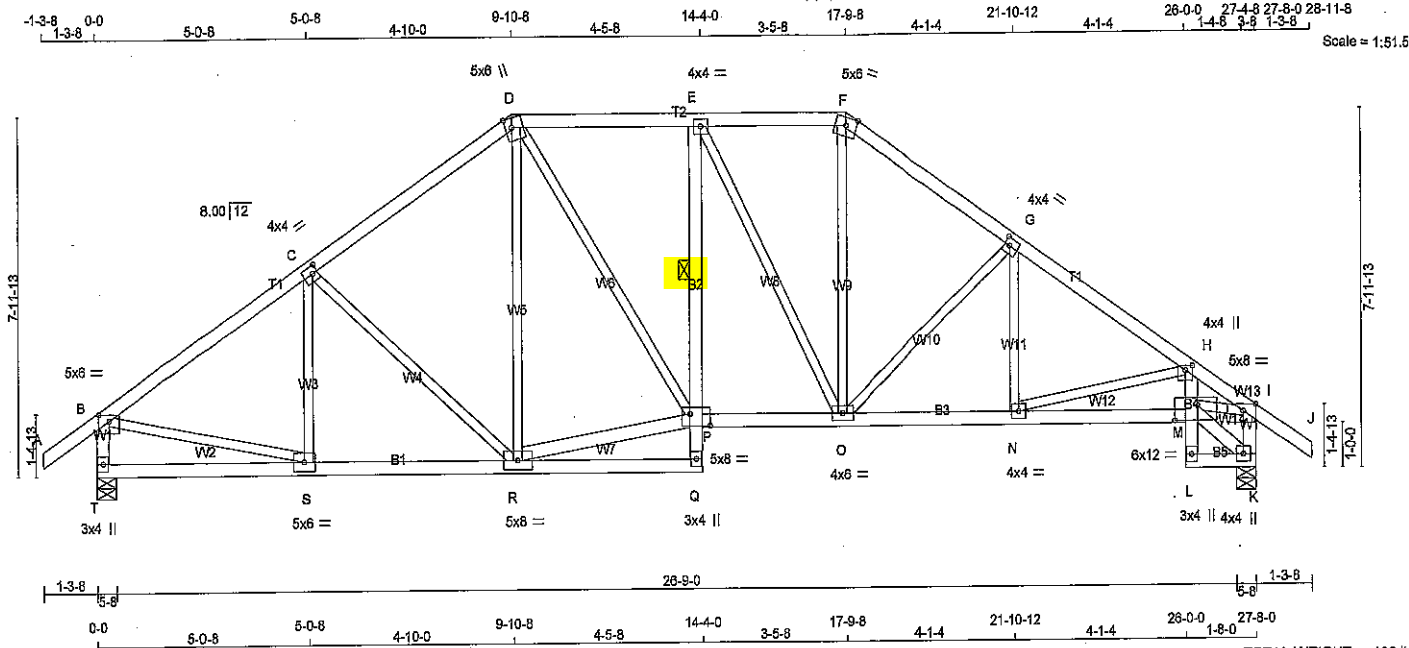
SITE COPY DWG NO. TAM 11994-18 STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T37	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:OKtrxCwvmqq21oJ?VO7h5_ya3KU-KR0bewGhzLQ6JNespKCQpKU4cmi5YA127A4aHnze?60



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - J	2x4	DRY No.2	SPF
T - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
T - Q	2x4	DRY No.2	SPF
Q - E	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
L - H	2x4	DRY No.2	SPF
L - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW+m	MT20	5.0	6.0	Edge	1.75
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	4.0	4.0	1.25	2.00
I	TMVW-p	MT20	5.0	6.0	Edge	3.50
K	BMVW1+p	MT20	4.0	4.0		
L	BMV+p	MT20	3.0	4.0		
M	BMVWW-I	MT20	6.0	12.0	4.00	6.50
N	BMVWW-t	MT20	4.0	4.0		
O	BMVWW-I	MT20	4.0	6.0		
P	BMVWW-I	MT20	5.0	8.0	3.25	5.75
Q	BMV+p	MT20	3.0	4.0		
R	BMVWW-t	MT20	5.0	6.0		
S	BMVWW-I	MT20	5.0	6.0		
T	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
T	1972	0	1972	0
K	1980	0	1980	0

UNFACTORED REACTIONS

	1ST LCASE	MAX / MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1547	985 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0
K	1552	990 / 0	277 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 40	-104.9 -104.9	0.14 (1)	S-C	-236 / 104	0.08 (1)	
B-C	-2101 / 0	-104.9 -104.9	0.40 (1)	C-R	-351 / 0	0.28 (1)	
C-D	-1861 / 0	-104.9 -104.9	0.38 (1)	R-D	0 / 170	0.04 (3)	
D-E	-1916 / 0	-104.9 -104.9	0.25 (1)	R-P	0 / 1532	0.25 (1)	
E-F	-1744 / 0	-104.9 -104.9	0.23 (1)	D-P	0 / 734	0.17 (1)	
F-G	-2113 / 0	-104.9 -104.9	0.28 (1)	E-O	-380 / 0	0.43 (1)	
G-H	-2584 / 0	-104.9 -104.9	0.32 (1)	O-F	0 / 852	0.19 (1)	
H-I	-3133 / 0	-104.9 -104.9	0.20 (1)	O-G	-804 / 0	0.34 (1)	
I-J	0 / 40	-104.9 -104.9	0.14 (1)	N-G	0 / 245	0.06 (2)	
J-T	-1913 / 0	0.0 0.0	0.20 (1)	N-H	-579 / 0	0.19 (1)	
K-I	-1863 / 0	0.0 0.0	0.19 (1)	B-S	0 / 1817	0.41 (1)	
				M-K	-157 / 0	0.02 (1)	
				M-I	0 / 2581	0.58 (1)	
T-S	0 / 0	-27.5 -27.5	0.16 (3)				
S-R	0 / 1776	-27.5 -27.5	0.37 (1)				
R-Q	0 / 27	-27.5 -27.5	0.13 (2)				
Q-P	0 / 80	0.0 0.0	0.05 (1)				
P-E	-160 / 75	0.0 0.0	0.04 (1)				
P-O	0 / 1921	-27.5 -27.5	0.38 (1)				
O-N	0 / 2173	-27.5 -27.5	0.40 (1)				
N-M	0 / 2735	-27.5 -27.5	0.50 (1)				
L-M	0 / 36	0.0 0.0	0.20 (1)				
M-H	0 / 225	0.0 0.0	0.23 (1)				
L-K	0 / 125	-27.5 -27.5	0.03 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.0/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.17")

CSI: TC=0.40/1.00 (B-C:1), BC=0.50/1.00 (M-N:1), WB=0.55/1.00 (I-M:1), SI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

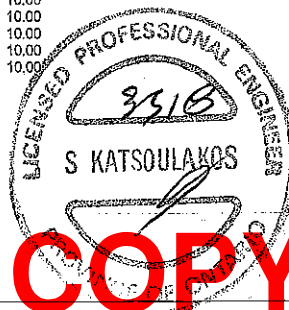
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (M) (INPUT = 0.90)
JSI METAL = 0.51 (I) (INPUT = 1.00)

DWG NO. TAM1/995
STRUCTURAL
COMPONENT ONLY

SITE COPY

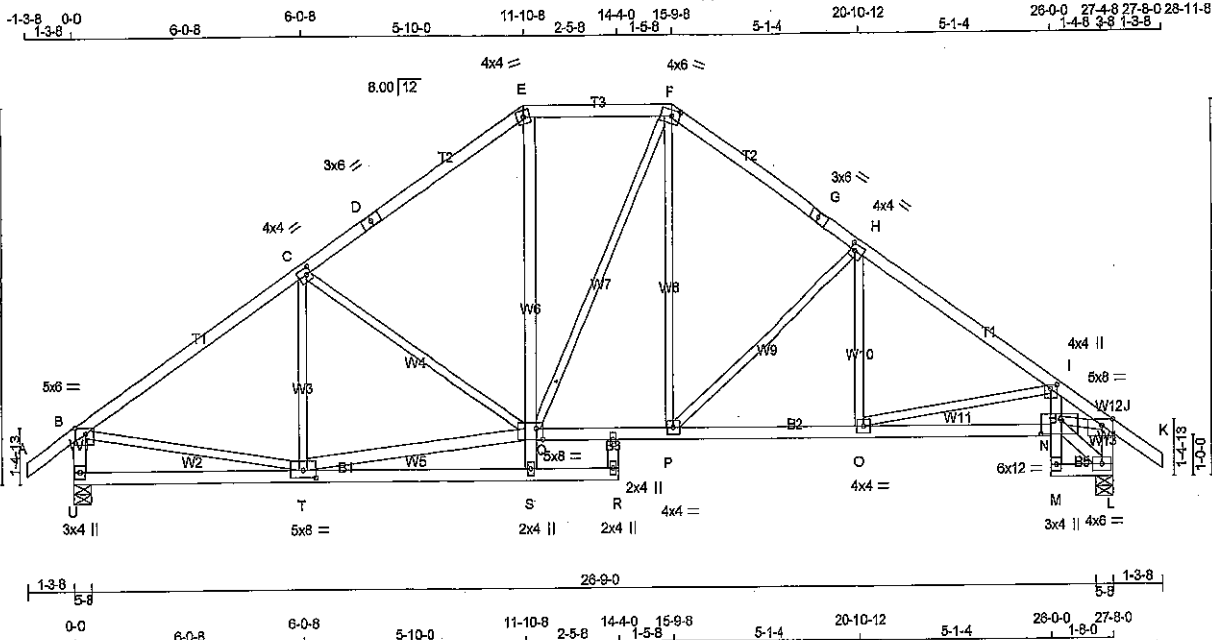


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T38	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:OKtrxCwvmqq21aJ7V07h5_ya3KU-KR0bewGhzLQ6JNespKCCqKU1TmsQY9527A4aHnze760



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
M - I	2x4	DRY	No.2
M - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - E	2x4	DRY	No.2
T - Q	2x4	DRY	No.2
N - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	3.00
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTVW-m	MT20	4.0	4.0		
F TTVW-m	MT20	4.0	6.0	1.75	2.50
G TS-t	MT20	3.0	6.0		
H TMVW-t	MT20	4.0	4.0	2.00	1.50
I TMVW-p	MT20	4.0	4.0	1.25	2.00
J TMVW-p	MT20	5.0	8.0	Edge	3.50
L BMVW-t	MT20	4.0	6.0		
M BMV-p	MT20	3.0	4.0		
N BVMW-t	MT20	6.0	12.0	4.00	6.75
O BVMW-t	MT20	4.0	4.0		
P BVMW-t	MT20	4.0	4.0		
Q BVMW-t	MT20	5.0	8.0	3.25	2.25
R NP-w	MT20	2.0	4.0		
S BVMW-w	MT20	2.0	4.0		
T BVMW-t	MT20	5.0	8.0	2.25	4.00
U BVM-t	MT20	3.0	4.0		
V NP-w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
U	2009	0	0	5-8
L	2007	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
U	1582	985 / 0	303 / 0	0 / 0	0 / 0	303 / 0	0 / 0
L	1586	990 / 0	297 / 0	0 / 0	0 / 0	299 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 40	-104.9	-104.9	0.14 (1)	T-C	-432 / 70	0.19 (1)
B-C	-2172 / 0	-104.9	-104.9	0.60 (1)	S-Q	0 / 218	0.06 (2)
C-D	-1951 / 0	-104.9	-104.9	0.56 (1)	Q-E	0 / 827	0.14 (1)
D-E	-1951 / 0	-104.9	-104.9	0.56 (1)	P-F	0 / 631	0.14 (1)
E-F	-1600 / 0	-104.9	-104.9	0.24 (1)	O-H	-756 / 0	0.69 (1)
F-G	-1940 / 0	-104.9	-104.9	0.41 (1)	G-I	0 / 313	0.07 (2)
G-H	-1940 / 0	-104.9	-104.9	0.41 (1)	O-I	-705 / 0	0.34 (1)
H-I	-2555 / 0	-104.9	-104.9	0.46 (1)	B-T	0 / 1871	0.42 (1)
I-J	-3234 / 0	-104.9	-104.9	0.21 (1)	Q-F	0 / 30	0.01 (2)
J-K	0 / 40	-104.9	-104.9	0.14 (1)	T-Q	0 / 1838	0.30 (1)
U-B	-1943 / 0	0.0	0.0	0.20 (1)	C-Q	-310 / 0	0.31 (1)
L-J	-1888 / 0	0.0	0.0	0.19 (1)	N-L	-162 / 0	0.02 (1)
					N-J	0 / 2662	0.60 (1)
U-T	0 / 0	-27.5	-27.5	0.28 (3)			
T-S	0 / 29	-27.5	-27.5	0.28 (3)			
S-R	0 / 0	-27.5	-27.5	0.17 (3)			
Q-P	0 / 1591	-27.5	-27.5	0.32 (1)			
P-O	0 / 2149	-27.5	-27.5	0.44 (1)			
O-N	0 / 2841	-27.5	-27.5	0.55 (1)			
M-N	0 / 36	0.0	0.0	0.21 (1)			
N-I	0 / 229	0.0	0.0	0.24 (1)			
M-L	0 / 129	-27.5	-27.5	0.03 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (0.92")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.19")

CSI: TC=0.60/1.00 (B-C:1), BC=0.55/1.00 (N-C:1), WB=0.59/1.00 (H-P:1), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

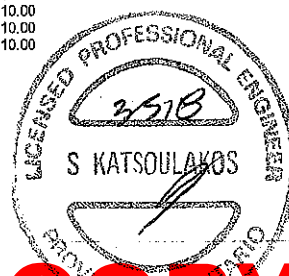
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

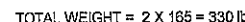
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)



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DRWG NO. TAM 11996-18
STRUCTURAL
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JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.61 (B) (INPUT = 1.00)

SITE COPY

DWG NO. TAM 11997-17
STRUCTURAL
COMPONENT ONLY

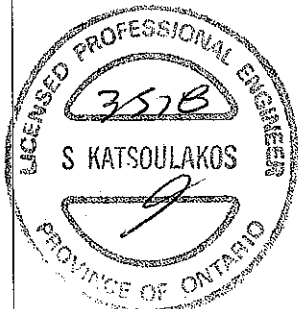
CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	TMWWH	MT20	4.0	4.0	2.00	1.50
J	TMWW+p	MT20	4.0	4.0	1.25	2.00
K	TMWW-p	MT20	5.0	6.0	2.25	2.50
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-w	MT20	8.0	9.0	4.00	6.75
P	BMWWH-t	MT20	3.0	6.0		
Q	BMWWH-t	MT20	3.0	6.0		
R	BWMWWWW-H	MT20	6.0	9.0	4.50	3.00
T	BMW+w	MT20	3.0	6.0		
U	BMWWWWH-t	MT20	8.0	9.0		
V	BMWWH-t	MT20	8.0	9.0		
W	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 7834.9 lbs FACTORED DOWN AT 1-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 11997-18
STRUCTURAL
COMPONENT ONLY

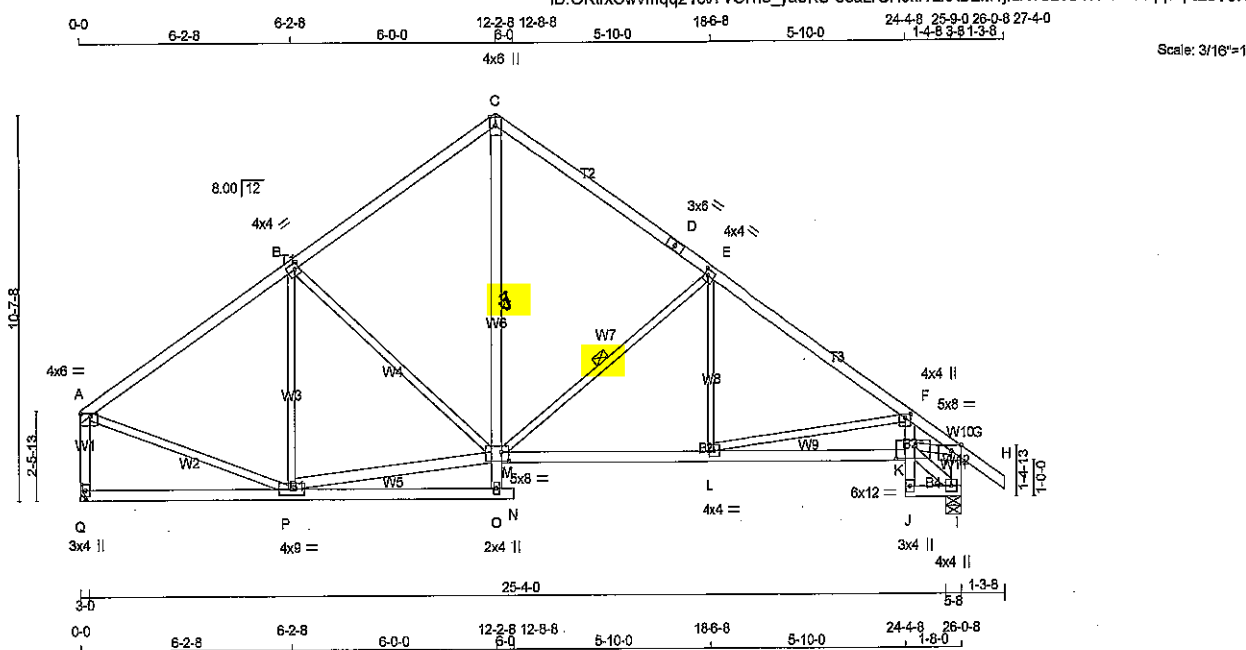
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T40	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 125 lb

LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF			
C - D	2x4	DRY	No.2	SPF			
D - H	2x4	DRY	No.2	SPF			
Q - A	2x4	DRY	No.2	SPF			
I - G	2x4	DRY	No.2	SPF			
Q - N	2x4	DRY	No.2	SPF			
M - K	2x4	DRY	No.2	SPF			
J - F	2x4	DRY	No.2	SPF			
J - I	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			
O - C	2x4	DRY	No.2	SPF			
P - M	2x4	DRY	No.2	SPF			
K - I	2x4	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.25
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TS-t	MT20	3.0	6.0		
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
I	BMVW-t+p	MT20	4.0	4.0		
J	BMV+p	MT20	3.0	4.0		
K	BVMW-t	MT20	6.0	12.0	4.00	6.50
L	BVMW-t	MT20	4.0	4.0		
M	BVMWVW-t	MT20	5.0	8.0	3.00	2.50
O	BMVW+p	MT20	2.0	4.0		
P	BMVW-t	MT20	4.0	9.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQ'D BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
Q	1728	0	1728	0	5-8	5-8
I	1878	0	1878	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.	LIVE			
Q	1373	843 / 0	286 / 0	0 / 0	0 / 0	284 / 0	0 / 0
I	1474	937 / 0	285 / 0	0 / 0	0 / 0	272 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)!

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX.	FORCE	VERT.	LOAD	MAX.	FORCE	MAX.	FORCE
FR-TO		(LBS)	FROM	TO	FR-TO		FROM	TO
A-B	-1618 / 0		-104.9	-104.9	0.58 (1)	4.50	P-B	-510 / 58
B-C	-1490 / 0		-104.9	-104.9	0.55 (1)	4.68	M-E	-917 / 0
C-D	-1492 / 0		-104.9	-104.9	0.57 (1)	4.66	L-E	0 / 399
D-E	-1492 / 0		-104.9	-104.9	0.57 (1)	4.66	L-F	-763 / 0
E-F	-2276 / 0		-104.9	-104.9	0.81 (1)	3.89	A-P	0 / 1467
F-G	-3030 / 0		-104.9	-104.9	0.21 (1)	3.85	O-M	0 / 136
G-H	0 / 40		-104.9	-104.9	0.14 (1)	10.00	M-C	0 / 1113
H-I	-1664 / 0		0.0	0.0	0.22 (1)	6.43	P-M	0 / 1388
I-G	-1765 / 0		0.0	0.0	0.18 (1)	6.28	B-M	-255 / 0
							K-I	-152 / 0
							K-G	0 / 2525
Q-P	0 / 0		-27.5	-27.5	0.29 (3)	10.00		
P-O	0 / 24		-27.5	-27.5	0.30 (3)	10.00		
O-N	0 / 0		-27.5	-27.5	0.01 (2)	10.00		
M-L	0 / 1822		-27.5	-27.5	0.50 (2)	10.00		
L-K	0 / 2673		-27.5	-27.5	0.59 (1)	10.00		
J-K	0 / 36		0.0	0.0	0.20 (1)	10.00		
K-F	0 / 221		0.0	0.0	0.22 (1)	10.00		
J-I	0 / 121		-27.5	-27.5	0.03 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.61/1.00 (E-F-1), BC=0.59/1.00 (K-L-1),
WB=0.57/1.00 (G-K-1), SSI=0.26/1.00 (C-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

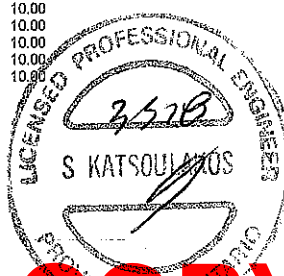
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

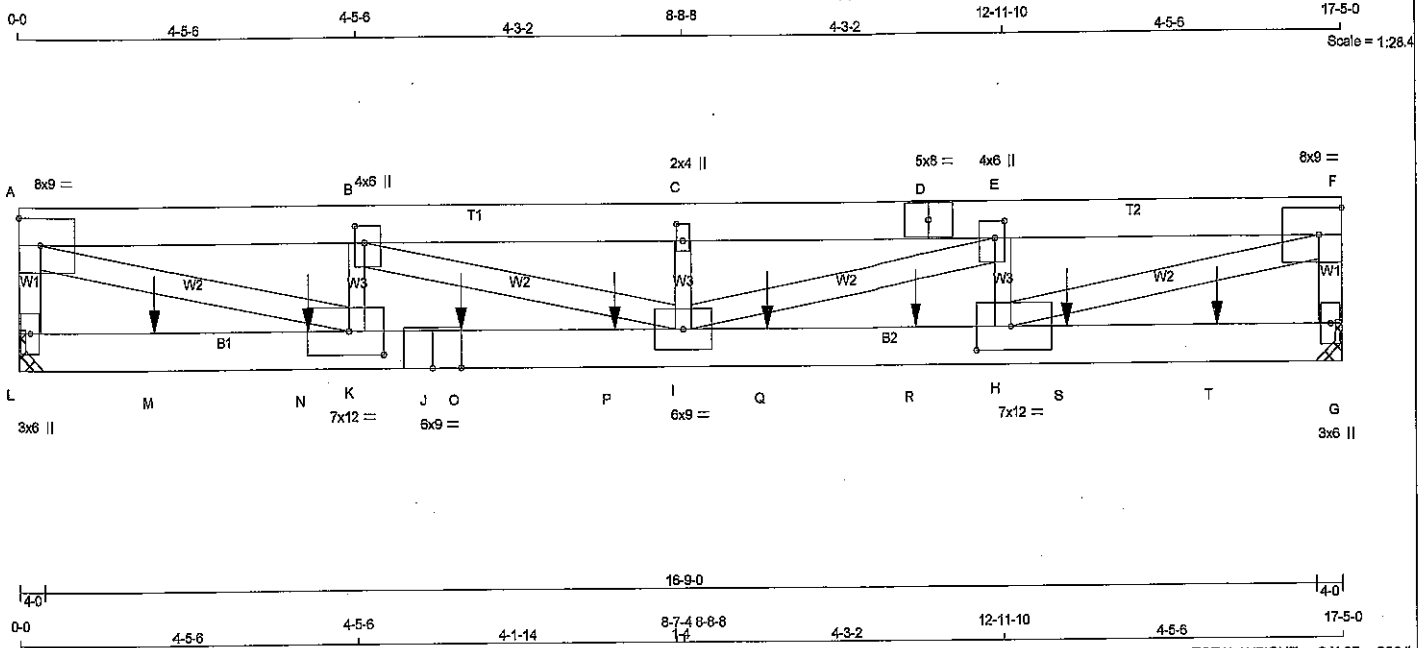
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.49 (G) (INPUT = 1.00)



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DRWG NO. TAM 1199B-18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY No.2	SPF
A - D	2x6	DRY 2100F 1.8E	SPF
D - F	2x6	DRY 2100F 1.8E	SPF
G - F	2x4	DRY No.2	SPF
L - J	2x6	DRY 2100F 1.8E	SPF
J - G	2x6	DRY 2100F 1.8E	SPF

ALL WEBS EXCEPT

CHORDS	SIZE	LUMBER	DESCR.
H - E	2x3	DRY No.2	SPF
K - B	2x3	DRY No.2	SPF
I - C	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS		
L-A 1	12	TOP
F-G 1	12	TOP
A-D 2	12	TOP
D-F 2	12	TOP
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS		
L-J 2	9	SIDE (443.5)
J-G 2	9	SIDE (442.7)
WEBS : (0.122'X3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	8.0	9.0		Edge
B	TMVW-t	MT20	4.0	6.0	2.50	1.50
C	TMVW-t	MT20	2.0	4.0	2.50	1.00
D	TS-t	MT20	5.0	8.0		
E	TMVW-t	MT20	4.0	6.0	2.50	1.50
F	TMVW-t	MT20	8.0	9.0		Edge
G	BMV1-p	MT20	3.0	6.0		
H	BMVW-t	MT20	7.0	12.0	3.50	5.50
I	BMVW-t	MT20	6.0	9.0		
J	BS-t	MT20	6.0	9.0		
K	BMVW-t	MT20	7.0	12.0	3.50	5.50
L	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQD BRG
L	7883	0	7883	0	0		HANGER BY OTHERS
G	8022	0	8022	0	0		MIN. SEAT SIZE: 40 HANGER BY OTHERS MIN. SEAT SIZE: 40

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	6186	3906 / 0	1129 / 0	0 / 0	0 / 0	1151 / 0	0 / 0
G	6308	3989 / 0	1148 / 0	0 / 0	0 / 0	1172 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO												
L-A	-6758 / 0	0.0	0.0	0.24 (1)	5.68	H-E	-2244 / 0	0.11 (1)				
A-B	-18340 / 0	-104.9	-104.9	0.16 (1)	4.30	K-B	-2265 / 0	0.11 (1)				
B-C	-23859 / 0	-104.9	-104.9	0.23 (1)	3.79	I-C	-495 / 0	0.02 (1)				
C-D	-23859 / 0	-104.9	-104.9	0.23 (1)	3.79	A-K	0 / 19411	0.75 (1)				
D-E	-23859 / 0	-104.9	-104.9	0.23 (1)	3.79	B-I	0 / 5869	0.23 (1)				
E-F	-18390 / 0	-104.9	-104.9	0.16 (1)	4.29	I-E	0 / 5816	0.23 (1)				
G-F	-6774 / 0	0.0	0.0	0.24 (1)	5.67	H-F	0 / 19464	0.76 (1)				
L-M	0 / 0	-27.5	-27.5	0.24 (1)	10.00							
M-N	0 / 0	-27.5	-27.5	0.24 (1)	10.00							
N-K	0 / 0	-27.5	-27.5	0.24 (1)	10.00							
K-J	0 / 18340	-27.5	-27.5	0.46 (1)	10.00							
J-O	0 / 18340	-27.5	-27.5	0.46 (1)	10.00							
O-P	0 / 18340	-27.5	-27.5	0.46 (1)	10.00							
P-I	0 / 18340	-27.5	-27.5	0.46 (1)	10.00							
I-Q	0 / 18390	-27.5	-27.5	0.46 (1)	10.00							
Q-R	0 / 18390	-27.5	-27.5	0.46 (1)	10.00							
R-H	0 / 18390	-27.5	-27.5	0.46 (1)	10.00							
H-S	0 / 0	-27.5	-27.5	0.24 (1)	10.00							
S-T	0 / 0	-27.5	-27.5	0.24 (1)	10.00							
T-G	0 / 0	-27.5	-27.5	0.24 (1)	10.00							

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
M	1-9-12	-1700	-1700		BACK	VERT	TOTAL
N	3-9-12	-1697	-1697		BACK	VERT	TOTAL
O	5-9-12	-1697	-1697		BACK	VERT	TOTAL
P	7-9-12	-1697	-1697		BACK	VERT	TOTAL
Q	9-9-12	-1697	-1697		BACK	VERT	TOTAL
R	11-9-12	-1697	-1697		BACK	VERT	TOTAL
S	13-9-12	-1697	-1697		BACK	VERT	TOTAL
T	15-9-12	-1697	-1697		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/781 (0.27")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/505 (0.41")

CSI: TC=0.24/1.00 (F-G:1), BC=0.48/1.00 (H:1), WB=0.76/1.00 (F-H:1), SSI=0.48/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL = 0.97 (A) (INPUT = 1.00)

DRWG NO. TAM-11999-08
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288739	T41	1	3	TRUSS DESC.	

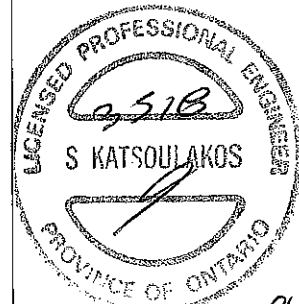
Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1700.1 lbs FACTORED DOWN AT 1-9-12, 1696.8 lbs FACTORED DOWN AT 3-9-12, 1696.8 lbs FACTORED DOWN AT 5-9-12, 1696.8 lbs FACTORED DOWN AT 7-9-12, 1696.8 lbs FACTORED DOWN AT 9-9-12, 1696.8 lbs FACTORED DOWN AT 11-9-12, AND 1696.8 lbs FACTORED DOWN AT 13-9-12, AND 1696.8 lbs FACTORED DOWN AT 15-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

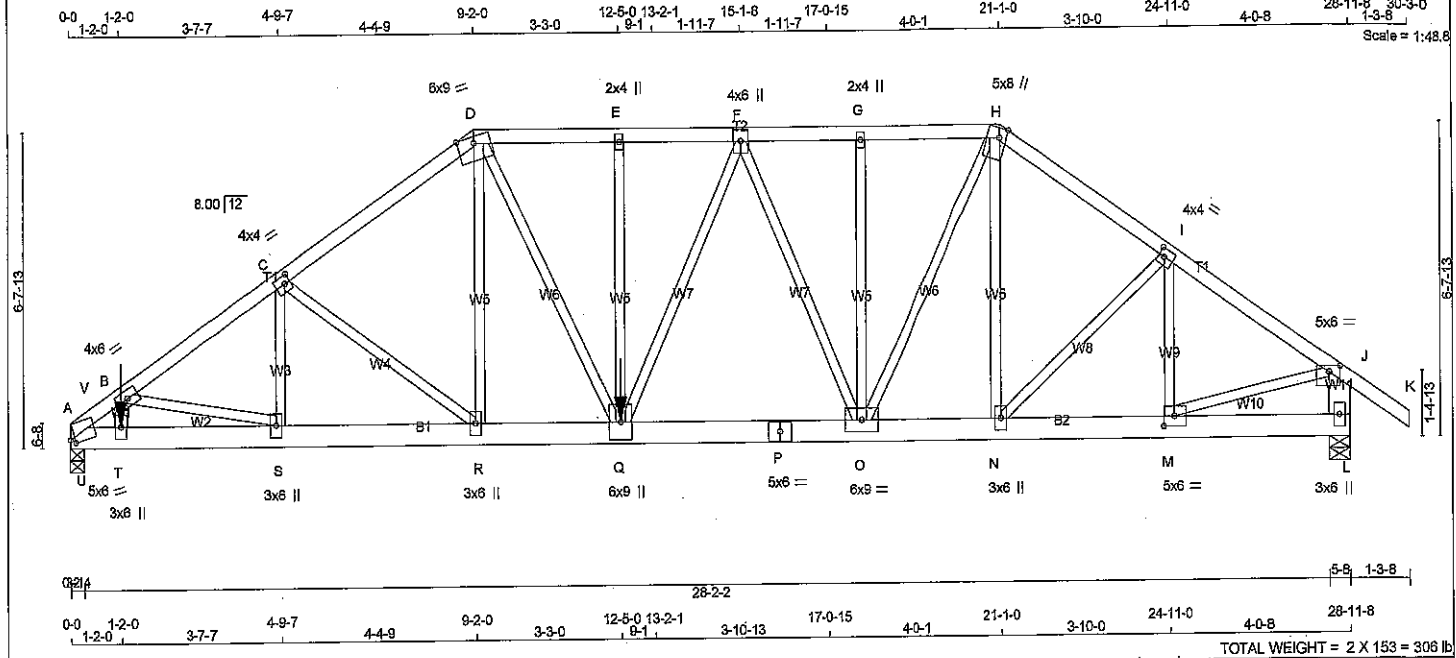


DWG NO. TAM 11999-18
STRUCTURAL
COMPONENT ONLY

SITE COPY

Tamarack Roof Truss, Burlington

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LUMBER						N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF							
A - D	2x4	DRY	No.2	SPF							
D - H	2x4	DRY	No.2	SPF							
H - K	2x4	DRY	No.2	SPF							
L - J	2x6	DRY	No.2	SPF							
A - P	2x6	DRY	No.2	SPF							
P - L	2x6	DRY	No.2	SPF							
ALL WEBS 2x3 DRY No.2 SPF											
EXCEPT											
T - B	2x4	DRY	No.2	SPF							
DRY: SEASONED LUMBER.											
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:											
CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)									
TOP CHORDS : (0.122"x3") SPIRAL NAILS											
A-D	12	TOP									
D-H	12	TOP									
H-K	12	TOP									
L-J	12	TOP									
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS											
A-P	12	SIDE(0.0)									
P-L	12	TOP									
WEBS : (0.122"x3") SPIRAL NAILS											
2x3	6										
2x4	6										
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.											
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/0 INCH NAILS.											
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.											
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.											
PLATES (table is in inches)											
JT	TYPE	PLATES	W	LEN	Y X						
A	TMB1-m	MT20	5.0	6.0	1.25 1.25						
B	TMMWW-t	MT20	4.0	8.0							
C	TMMWW-t	MT20	4.0	4.0	2.00 1.50						
D	TTWW-m	MT20	6.0	9.0	Edge						
E	TMMWW-t	MT20	2.0	4.0							
F	TMMWW-t	MT20	4.0	6.0							
G	TMMWW-t	MT20	2.0	4.0							
H	TTWW-m	MT20	5.0	8.0	Edge 1.75						
I	TMMWW-t	MT20	4.0	4.0	2.00 1.50						
J	TMMWW-p	MT20	5.0	6.0	1.50 3.00						
L	BMV1-p	MT20	3.0	6.0							
M	BMWW-t	MT20	5.0	6.0	2.50 2.75						
N, R, S											
N	BMWW-t	MT20	3.0	6.0							
O	BMWWWW-t	MT20	8.0	9.0							
P	BS-t	MT20	5.0	8.0							
Q	BMWWWW-t	MT20	6.0	9.0							
T	BMWW-w	MT20	3.0	6.0							
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.											

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
A	5911	0	5911	0	0	4-0 (3-14)	4-0		
L	3799	0	3799	0	0	5-8	5-8		
VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH									
UNFACTORED REACTIONS									
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS				DEAD	SOIL		
A	COMBINED	SNOW	LIVE	PERM. LIVE	WIND				
A	4835	2954 / 0	829 / 0	0 / 0	0 / 0	653 / 0	0 / 0		
L	2981	1896 / 0	535 / 0	0 / 0	0 / 0	550 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, L									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.90 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS		FACTORED		WEBS		FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)			
FR-TO		FROM TO		LENGTH FR-TO					
A-V	-9243 / 0	-104.9 -104.9	0.39 (1)	2.90	S-C	0 / 400	0.05 (1)		
V-B	-7976 / 0	-104.9 -104.9	0.33 (1)	3.20	C-R	-863 / 0	0.23 (1)		
B-C	-7275 / 0	-104.9 -104.9	0.37 (1)	3.37	R-D	0 / 726	0.09 (1)		
C-D	-6529 / 0	-104.9 -104.9	0.37 (1)	3.58	D-G	-318 / 0	0.11 (1)		
D-E	-6720 / 0	-104.9 -104.9	0.25 (1)	3.59	H-N	-48 / 82	0.02 (1)		
E-F	-8720 / 0	-104.9 -104.9	0.19 (1)	3.65	N-I	0 / 319	0.04 (1)		
F-G	-5190 / 0	-104.9 -104.9	0.15 (1)	4.15	M-I	-888 / 0	0.11 (1)		
G-H	-5190 / 0	-104.9 -104.9	0.17 (1)	4.13	M-J	0 / 3837	0.47 (1)		
H-I	-4790 / 0	-104.9 -104.9	0.23 (1)	4.22	Q-E	-271 / 11	0.09 (1)		
I-J	-4454 / 0	-104.9 -104.9	0.22 (1)	4.36	D-Q	0 / 2834	0.35 (1)		
J-K	0 / 40	-104.9 -104.9	0.08 (1)	10.00	T-B	0 / 580	0.05 (1)		
L-J	-3716 / 0	0.0 0.0	0.13 (1)	7.31	Q-H	0 / 2675	0.33 (1)		
					Q-F	0 / 1771	0.22 (1)		
A-U	0 / 6471	-27.5 -27.5	0.50 (1)	10.00	F-O	-2092 / 0	0.90 (1)		
U-T	0 / 6471	-27.5 -27.5	0.70 (1)	10.00	B-S	-387 / 0	0.05 (1)		
T-S	0 / 6471	-27.5 -27.5	0.70 (1)	10.00	U-V	0 / 1878	0.00 (1)		
S-R	0 / 6098	-27.5 -27.5	0.43 (1)	10.00					
R-Q	0 / 5415	-27.5 -27.5	0.42 (1)	10.00					
Q-P	0 / 6019	-27.5 -27.5	0.46 (1)	10.00					
P-O	0 / 6019	-27.5 -27.5	0.46 (1)	10.00					
O-N	0 / 3957	-27.5 -27.5	0.29 (1)	10.00					
N-M	0 / 3725	-27.5 -27.5	0.28 (1)	10.00					
M-L	0 / 0	-27.5 -27.5	0.05 (1)	10.00					
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	TYPE			
Q	12-5-0	-3877	-	-	FRONT	VERT			
T	1-2-0	-1854	-1854	-	BACK	VERT			

DESIGNED PROFESSIONAL ENGINEER

3378

S. SOULAKOS

PROVIDING SERVICE

DESIGN CRITERIA					
SPECIFIED LOADS:					
TOP CH.	LL	=	32.5	PSF	
	DL	=	3.0	PSF	
BOT CH.	LL	=	10.0	PSF	
	DL	=	7.0	PSF	
TOTAL LOAD		=	52.5	PSF	
SPACING = 24.0 IN. C/C					
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM					
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010					
THIS DESIGN COMPLIES WITH:					
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014					
- CSA 088-09					
- TPIC 2011					
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD					
ALLOWABLE DEFL.(LL)= L/360 (0.97")					
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")					
ALLOWABLE DEFL.(TL)= L/360 (0.97")					
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")					
CSI: TC=0.39/1.00 (A-V:1), BC=0.70/1.00 (S-T:1), WS=0.80/1.00 (F-O:1), SS=0.38/1.00 (T-U:1)					
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00					
COMP=1.00 SHEAR=1.00 TENS=1.00					
COMPANION LIVE LOAD FACTOR = 0.50					
AUTOSOLVE LEFT HEEL ONLY					
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.					
NAIL VALUES					
PLATE GRIP(DRY)	SHEAR	SECTION			
(PSI)	(PLI)	(PLI)			
MAX MIN	MAX MIN	MAX MIN			
MT20	618 354	1667 822	2284	1656	
PLATE PLACEMENT TOL. = 0.250 inches					
PLATE ROTATION TOL. = 5.0 Deg.					
JSI GRIP= 0.89 (H) (INPUT = 0.90)					
JSI METAL= 0.87 (A) (INPUT = 1.00)					

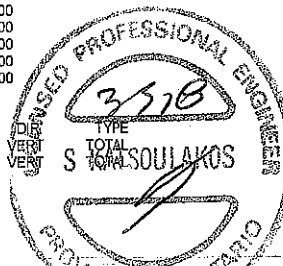
DWG NO: TAM 12000-18

STRUCTURAL COMPONENT ONLY

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Continued on Page 2

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DWG NO: TAM 12000-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T42	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1853.8 lbs FACTORED DOWN AT 1-2-0, AND 3877.2 lbs FACTORED DOWN AT 12-5-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



P672

DWG NO. TAM 12000-18
STRUCTURAL
COMPONENT ONLY

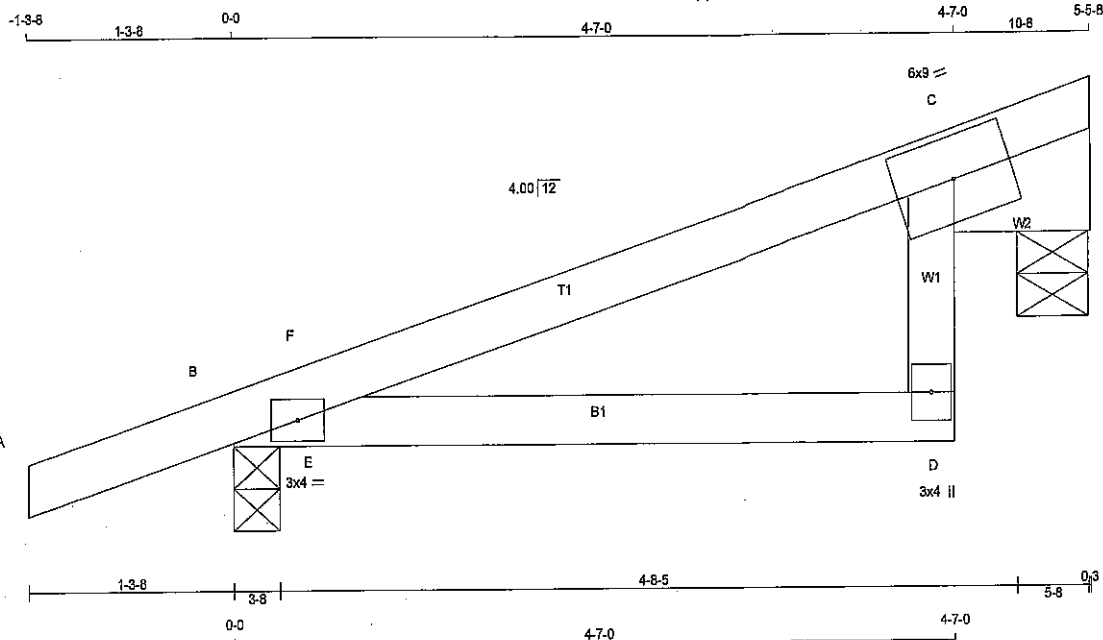
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288739	T43	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 17 = 85 lb

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE	LUMBER	DESCR.	SPF	
A - C	2x4	DRY.	No.2	SPF	
D - C	2x4	DRY.	No.2	SPF	
B - D	2x4	DRY.	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMBVW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

BEARINGS							
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	303	0	303	0	0	5-8	5-8
B	444	0	444	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	241	149 / 0	46 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	335	236 / 0	46 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		
A-B	0 / 21	10.00
B-F	-38 / 0	6.25
F-C	0 / 5	10.00
D-C	0 / 107	10.00
B-E	0 / 0	10.00
E-D	0 / 0	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/797$ (0.07")

CSI: TC=0.28/1.00 (C-F:1), BC=0.21/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.20/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

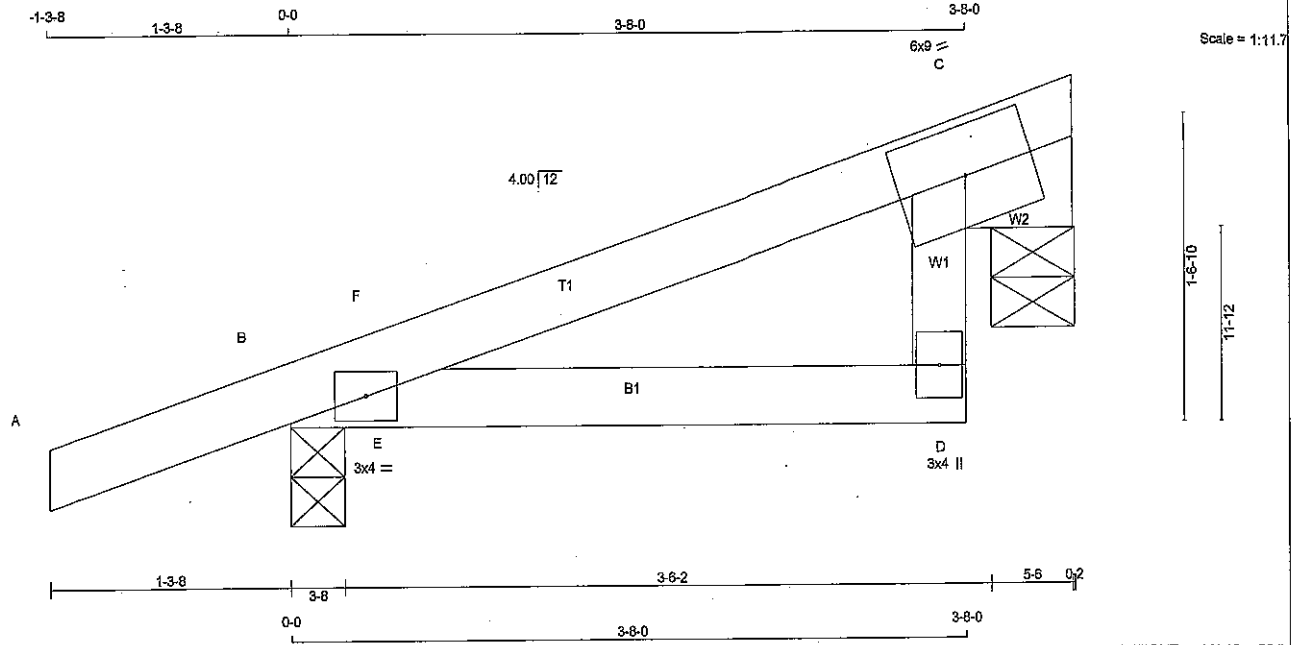
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



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DRWG NO. TAM 12001-18
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 D - C 2x4 DRY No.2 SPF
 B - D 2x4 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM51-H	MT20	3.0	4.0		
C	TMVW1-t	MT20	6.0	9.0		
D	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	243	0	243	0	5-8 (5-6)	5-8
B	383	0	383	0	3-8	3-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	192	119 / 0	37 / 0	0 / 0	0 / 0	37 / 0	0 / 0
B	287	206 / 0	37 / 0	0 / 0	0 / 0	45 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED	
		VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. CSI (LC)			FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO		
A- B	0 / 21	-104.9	-104.9	0.13 (1)	10.00	E- F	-152 / 72	0.00 (1)
B- F	-26 / 0	-104.9	-104.9	0.06 (3)	6.25			
F- C	0 / 5	-104.9	-104.9	0.18 (1)	10.00			
D- C	0 / 85	0.0	0.0	0.01 (2)	10.00			
B- E	0 / 0	-27.5	-27.5	0.14 (1)	10.00			
E- D	0 / 0	-27.5	-27.5	0.14 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
	32.5	PSF
BOT CH.	LL	PSF
	10.0	PSF
	7.0	PSF
TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TO=0.18/1.00 (C-F:1), BC=0.14/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.14/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687
	822	2284	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
 JSI METAL= 0.06 (B) (INPUT = 1.00)



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DRWG NO. TAM / 2002-8
 STRUCTURAL
 COMPONENT ONLY

TRUSS DESC.

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TOTAL WEIGHT = 2 X 168 = 337 lb

DWG NO. TAM 11-706.18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	44755	DRWG NO.
288742	T80	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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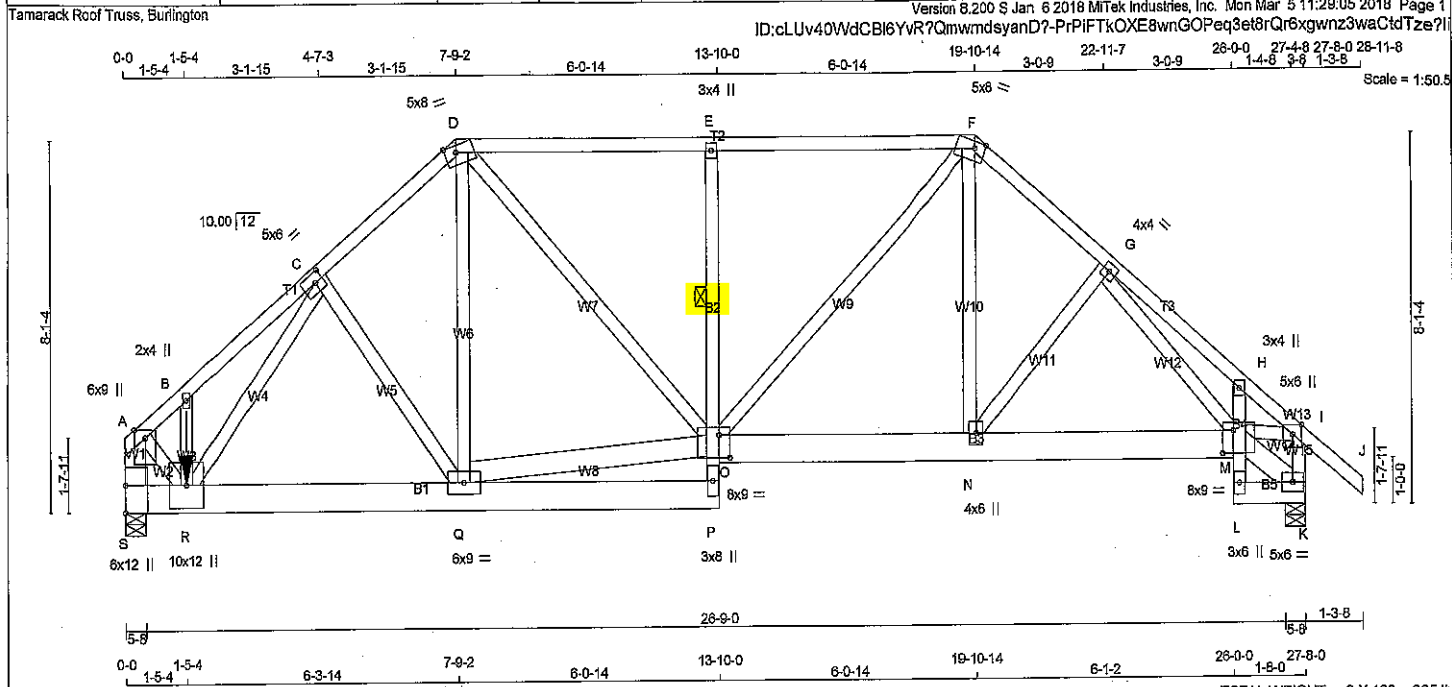
HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 8716.3 lbs FACTORED DOWN AT 1-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 11986-18
STRUCTURAL
COMPONENT ONLY

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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF D - F 2x4 DRY No.2 SPF F - J 2x4 DRY No.2 SPF S - A 2x6 DRY No.2 SPF K - I 2x4 DRY No.2 SPF S - P 2x8 DRY No.2 SPF P - E 2x4 DRY No.2 SPF O - M 2x8 DRY No.2 SPF L - H 2x4 DRY No.2 SPF L - K 2x6 DRY No.2 SPF ALL WEBS 2x4 DRY No.2 SPF EXCEPT Q - O 2x6 DRY No.2 SPF M - K 2x6 DRY No.2 SPF DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A - D 1 12 SIDE (18.0) D - F 1 12 TOP F - J 1 12 TOP K - I 1 12 TOP S - A 2 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS S - P 2 12 SIDE (0.0) O - M 2 12 TOP P - E 1 12 TOP L - L 1 12 TOP L - K 2 12 TOP WEBS: (0.122"x3") SPIRAL NAILS 2x4 1 6 2x6 2 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES. (table is in inches) JT TYPE PLATES W LEN Y X A TMVW+p MT20 8.0 9.0 Edge B TMW+w MT20 2.0 4.0 C TMW+W-t MT20 5.0 8.0 2.50 2.25 D TTWW-m MT20 5.0 8.0 Edge 3.00 E TMV+p MT20 3.0 4.0 F TTWW-m MT20 5.0 8.0 Edge 3.00 G TMW+W-t MT20 4.0 4.0 H TMV+p MT20 3.0 4.0 I TMW+W-p MT20 5.0 8.0 Edge K BMVW1-t MT20 5.0 6.0 L BMV+p MT20 3.0 6.0 M BVW+W+W-J MT20 8.0 9.0 6.00 3.00			
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REQRD JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX S 10153 0 10153 0 0 5-8 5-8 K 2435 0 2435 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST CASE MAX / MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL S 7980 5022 / 0 1436 / 0 0 / 0 0 / 0 1521 / 0 0 / 0 K 1909 1216 / 0 340 / 0 0 / 0 0 / 0 353 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O. END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD (PLF) FROM TO MAX. MAX. MEMB. MAX. FACTORED FORCE MAX. CSI (LC) MEMB. (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO FR-TO A-B -7288 / 0 -151.8 -151.8 0.27 (1) 3.43 R-B -58 / 58 0.01 (3) B-C -7134 / 0 -104.9 -104.9 0.33 (1) 3.44 R-C 0 / 3873 0.34 (1) C-D -3384 / 0 -104.9 -104.9 0.18 (1) 4.91 Q-C -1619 / 0 0.34 (1) D-E -2915 / 0 -104.9 -104.9 0.43 (1) 4.86 Q-D 0 / 1410 0.12 (1) E-F -2920 / 0 -104.9 -104.9 0.44 (1) 4.86 Q-O 0 / 2580 0.17 (1) F-G -2729 / 0 -104.9 -104.9 0.13 (1) 5.41 D-O 0 / 510 0.05 (1) G-H -2903 / 0 -104.9 -104.9 0.13 (1) 5.27 O-F 0 / 1230 0.11 (1) H-I -2829 / 0 -104.9 -104.9 0.10 (1) 5.29 N-F 0 / 471 0.04 (2) I-J 0 / 47 -104.9 -104.9 0.08 (1) 10.00 N-G -327 / 0 0.05 (1) S-A -9230 / 0 0.0 0.0 0.33 (1) 4.97 G-M -41 / 58 0.01 (1) K-I -2352 / 0 0.0 0.0 0.13 (1) 7.31 A-R 0 / 7078 0.63 (1) M-K -91 / 0 0.00 (1) M-I 0 / 2216 0.20 (1) S-R 0 / 0 -27.5 -27.5 0.32 (1) 10.00 R-Q 0 / 3490 -27.5 -27.5 0.51 (1) 10.00 Q-P 0 / 32 -27.5 -27.5 0.11 (1) 10.00 P-O 0 / 86 0.0 0.0 0.02 (1) 10.00 O-E -770 / 0 0.0 0.0 0.03 (1) 6.25 O-N 0 / 2079 -27.5 -27.5 0.14 (1) 10.00 N-M 0 / 2280 -27.5 -27.5 0.15 (1) 10.00 L-M 0 / 36 0.0 0.0 0.06 (1) 10.00 M-H -136 / 0 0.0 0.0 0.06 (1) 7.61 L-K 0 / 74 -27.5 -27.5 0.01 (2) 10.00			
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 3.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM GIRDER TYPE: CStdGirder START DISTANCE = 0-0 START SPAN CARRIED = 3-5-0 END DISTANCE = 1-5-4 END SPAN CARRIED = 3-5-0 END WALL WIDTH = 0-0 APPLIED TO FRONT SIDE OF TOP CHORD. - ADD'L LOADS BASED ON 55 % OF GSL. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.92") CALCULATED VERT. DEFL.(LL) = L/999 (0.08") ALLOWABLE DEFL.(TL) = L/360 (0.92") CALCULATED VERT. DEFL.(TL) = L/999 (0.10") CSI: TC=0.44/1.00 (E-F:1), BC=0.51/1.00 (Q-R:1), WB=0.63/1.00 (A-R:1), SSI=0.27/1.00 (R-S:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00 COMPANION LIVE LOAD FACTOR = 0.50 AUTOSOLVE HEELS OFF TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (Psi) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1687 822 2284 1858 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. p6/12			
FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX- (LBS)+ FACE OR VERT TOTAL R 1-5-4 -8716 -8716 -- FRONT 			

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DRWG NO. TAM 11977-038
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME 288737	TRUSS NAME T80A	QUANTITY 1	PLY 2	JOB DESC. 44755 TRUSS DESC.	DRWG NO. T22
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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWW+t	MT20	4.0	6.0		
O	BVMWWW-t	MT20	8.0	9.0	6.00	3.00
P	BMV+p	MT20	3.0	8.0		
Q	BMWWWW-t	MT20	6.0	9.0		
R	BMWWWW-t	MT20	10.0	12.0		
S	BMV1-t	MT20	6.0	12.0	7.25	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 8716.3 lbs FACTORED DOWN AT 1-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

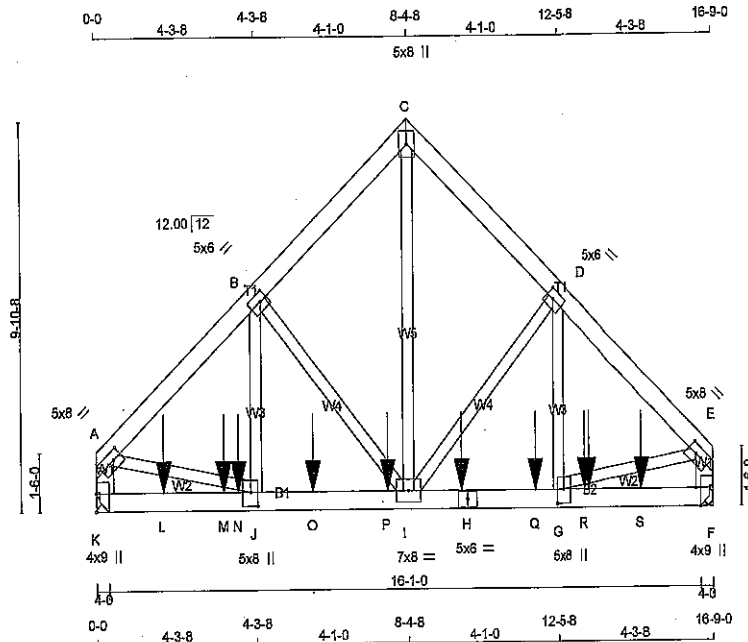
JSI GRIP= 0.87 (A) (INPUT = 0.80)
JSI METAL= 0.58 (A) (INPUT = 1.00)



DWG NO. TAM 11977-18
 STRUCTURAL
 COMPONENT ONLY

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Tamarack Roof Truss, Burlington



Scale = 1:58.6

TOTAL WEIGHT = 3 X 121 = 363 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2 SPF
C - E	2x6	DRY	No.2 SPF
K - A	2x6	DRY	No.2 SPF
F - E	2x6	DRY	No.2 SPF
K - H	2x6	DRY	No.2 SPF
H - F	2x6	DRY	No.2 SPF

ALL WEBS 2x4 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2	12	SIDE(13.7)
C-E 2	12	SIDE(13.7)
K-A 2	12	TOP
F-E 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K-H 2	7	SIDE(891.9)
H-F 2	7	SIDE(1350.9)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.50	3.00
B	TMVW-t	MT20	5.0	6.0	2.50	2.25
C	TTW+p	MT20	5.0	8.0		
D	TMVW-t	MT20	5.0	6.0	2.50	2.25
E	TMVW-t	MT20	5.0	8.0	2.50	3.00
F	BMV1-t	MT20	4.0	9.0	Edge	1.50
G	BMVW-t	MT20	5.0	8.0	4.25	2.25
H	BS-t	MT20	5.0	6.0		
I	BMVW-t	MT20	7.0	8.0		
J	BMVW-t	MT20	5.0	8.0	4.25	2.25
K	BMV1-t	MT20	4.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
K	8744	0	8744	0
F	9193	0	9193	0

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX. SNOW	MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	6871	4320 / 0	1233 / 0	0 / 0	0 / 0	1317 / 0	0 / 0
F	7224	4525 / 0	1291 / 0	0 / 0	0 / 0	1408 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.76 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	-8633 / 0	-133.9 -133.9	0.11 (1)	4.88	I-C	0 / 8171	0.44 (1)
B-C	-8164 / 0	-133.9 -133.9	0.08 (1)	3.58	I-D	-3440 / 0	0.62 (1)
C-D	-8162 / 0	-133.9 -133.9	0.08 (1)	5.58	G-D	0 / 4264	0.25 (1)
D-E	-9150 / 0	-133.9 -133.9	0.12 (1)	4.76	B-I	-2849 / 0	0.51 (1)
K-A	-7737 / 0	0.0 0.0	0.17 (1)	6.47	J-B	0 / 3427	0.18 (1)
F-E	-8184 / 0	0.0 0.0	0.18 (1)	6.32	A-J	0 / 6254	0.34 (1)
					G-E	0 / 8625	0.36 (1)
K-L	0 / 0	-27.5 -27.5	0.40 (1)	10.00			
L-M	0 / 0	-27.5 -27.5	0.40 (1)	10.00			
M-N	0 / 0	-27.5 -27.5	0.40 (1)	10.00			
N-J	0 / 0	-27.5 -27.5	0.40 (1)	10.00			
J-O	0 / 6107	-27.5 -27.5	0.60 (1)	10.00			
O-P	0 / 6107	-27.5 -27.5	0.60 (1)	10.00			
P-I	0 / 6107	-27.5 -27.5	0.60 (1)	10.00			
I-H	0 / 6471	-27.5 -27.5	0.68 (1)	10.00			
H-Q	0 / 6471	-27.5 -27.5	0.68 (1)	10.00			
Q-G	0 / 6471	-27.5 -27.5	0.68 (1)	10.00			
G-R	0 / 0	-27.5 -27.5	0.42 (1)	10.00			
R-S	0 / 0	-27.5 -27.5	0.42 (1)	10.00			
S-F	0 / 0	-27.5 -27.5	0.42 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
H	9-8-12	-1726	-1726		BACK	VERT	TOTAL
L	1-9-12	-1693	-1693		BACK	VERT	TOTAL
M	3-5-0	-743	-743		FRONT	VERT	TOTAL
N	3-9-12	-1726	-1726		BACK	VERT	TOTAL
O	5-9-12	-1697	-1697		BACK	VERT	TOTAL
P	7-9-12	-1726	-1726		BACK	VERT	TOTAL
Q	11-9-12	-1726	-1726		BACK	VERT	TOTAL
R	13-2-8	-1726	-1726		BACK	VERT	TOTAL
R	13-4-0	-743	-743		FRONT	VERT	TOTAL
S	14-9-12	-1726	-1726		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

GIRDER TYPE: CSdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-0-0
END DISTANCE = 16-9-0
END SPAN CARRIED = 3-0-0
END WALL WIDTH = 1-8
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/999 (0.10")

CSI: TC=0.18/1.00 (E-F:1), BC=0.68/1.00 (G-I:1), WB=0.62/1.00 (D-I:1), SS=0.68/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (A) (INPUT = 0.90)
JSI METAL= 0.58 (A) (INPUT = 1.00)

DRWG NO. TAM-11978-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME 288737	TRUSS NAME T81	QUANTITY 1	PLY 3	JOB DESC. 44755 TRUSS DESC.	DRWG NO. T23
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Tamarack Roof Truss, Burlington

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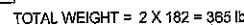
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 1693.0 lbs FACTORED DOWN AT
1-9-12, 743.3 lbs FACTORED DOWN AT 3-5-0,
1726.1 lbs FACTORED DOWN AT 3-9-12, 1696.8
lbs FACTORED DOWN AT 5-9-12, 1726.1 lbs
FACTORED DOWN AT 7-9-12, 1726.1 lbs
FACTORED DOWN AT 9-9-12, 1726.1 lbs
FACTORED DOWN AT 11-9-12, 1726.1 lbs
FACTORED DOWN AT 13-2-8, AND 743.3 lbs
FACTORED DOWN AT 13-4-0, AND 1726.1 lbs
FACTORED DOWN AT 14-9-12 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.



DWG NO. TAM11978-18
STRUCTURAL
COMPONENT ONLY

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CONTINUED ON PAGE 2

JOB NAME 288737	TRUSS NAME T82	QUANTITY 1	PLY 2	JOB DESC. 44755 TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

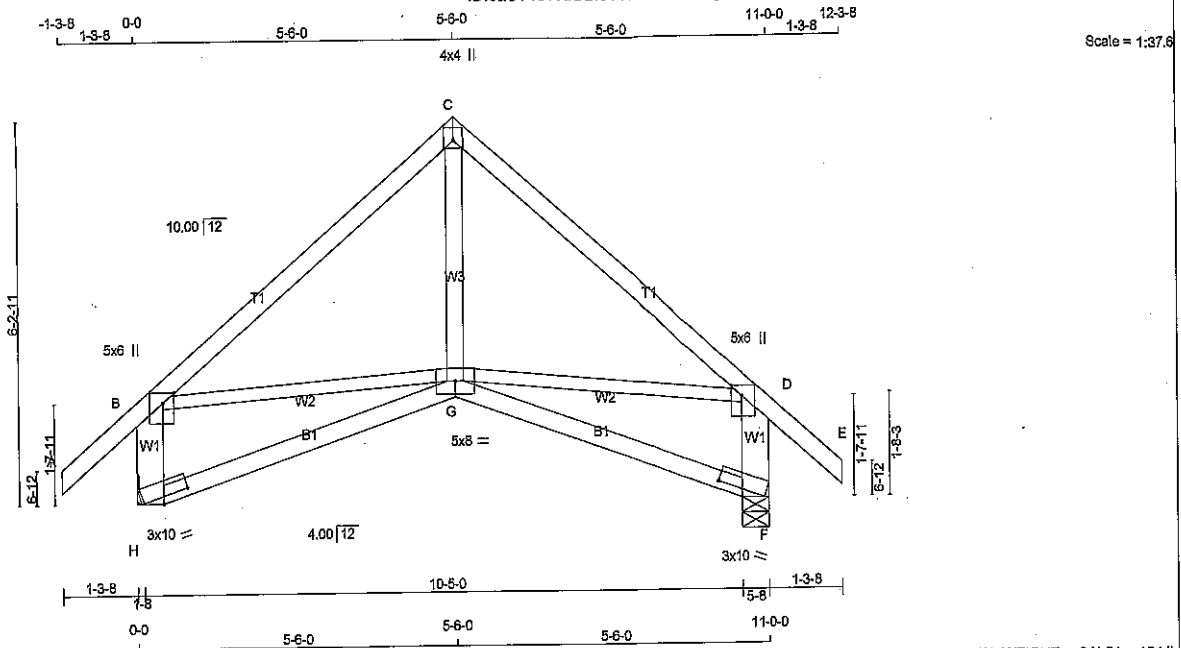
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 11:29:07 2018 Page 2
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HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 9185.2 lbs FACTORED DOWN AT 1-5-4
ON BOTTOM CHORD. DESIGN FOR
UNSPECIFIED CONNECTION(S) IS DELEGATED
TO THE BUILDING DESIGNER.



DWG NO. TAM 11979-18
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TOTAL WEIGHT = 3 X 51 = 154 lb [M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DRY
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x6	DRY	No.2
F - D	2x6	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS			
EXCEPT	SIZE	DRY	No.2
G - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMW+p	MT20	5.0	6.0 2.00 2.25
C	TTW+p	MT20	4.0	4.0 1.50 2.00
D	TMW+p	MT20	5.0	6.0 2.00 2.25
F	BVM1-I	MT20	3.0	10.0 1.50 5.75
G	BBWW+p	MT20	5.0	8.0
H	BVM1-I	MT20	3.0	10.0 1.50 5.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
H	874	0	874	0	0	5-8	5-8	5-8	5-8
F	874	0	874	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	876	447 / 0	110 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	876	447 / 0	110 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	MEMB.	MAX. FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX (LBS)
FR-TO		FROM	TO				
A-B	0 / 47	-104.9	-104.9 0.14 (1)	10.00	G-C	0 / 305	0.05 (2)
B-C	-621 / 0	-104.9	-104.9 0.41 (1)	6.25	B-G	0 / 478	0.11 (1)
C-D	-621 / 0	-104.9	-104.9 0.41 (1)	6.25	G-D	0 / 478	0.11 (1)
D-E	0 / 47	-104.9	-104.9 0.14 (1)	10.00			
H-B	-798 / 0	0.0	0.0 0.06 (1)	7.81			
F-D	-798 / 0	0.0	0.0 0.06 (1)	7.81			
H-G	0 / 0	-27.5	-27.5 0.25 (3)	10.00			
G-F	0 / 0	-27.5	-27.5 0.25 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.41/1.00 (C-D:1), BC=0.25/1.00 (F-G:3), WB=0.11/1.00 (D-G:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

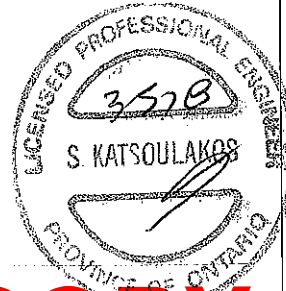
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

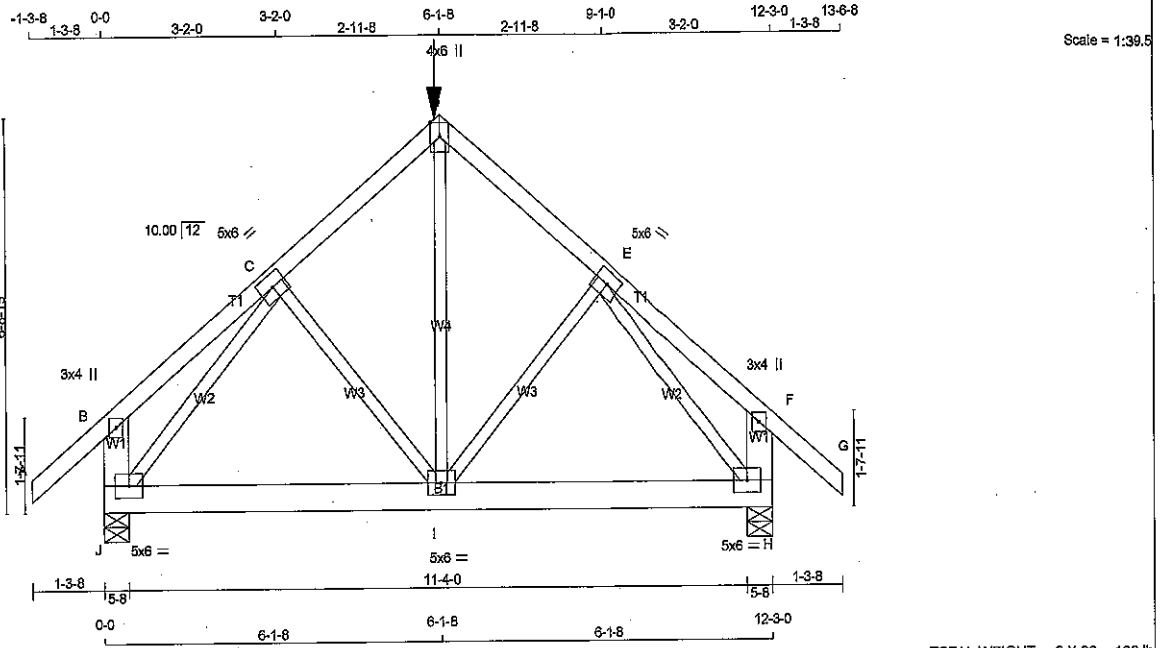
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)



DWG NO. TAM 11980-18
STRUCTURAL
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TOTAL WEIGHT = 2 X 68 = 136 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	SIDE(69.9)
D-G 1	12	SIDE(69.9)
J-B 2	12	TOP
H-F 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
J-H 2	12	SIDE(21.8)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 30 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWV+t	MT20	5.0	8.0		
D TTW+p	MT20	4.0	6.0	Edge	
E TMWV+t	MT20	5.0	6.0		
F TMV+p	MT20	3.0	4.0		
H BMWV+t	MT20	5.0	6.0		
I BMWV+t	MT20	5.0	6.0		
J BMWV+t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 990.8 lbs FACTORED DOWN AT 6'-1-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2722	0	5-8	5-8
JT HORZ	0	0	5-8	5-8
H VERT	2722	0	5-8	5-8
H HORZ	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	2151	1341 / 0	403 / 0	0 / 0	0 / 0	406 / 0	0 / 0
H	2151	1341 / 0	403 / 0	0 / 0	0 / 0	406 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.87 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 47	-104.9 -104.9	0.08 (1)	I-D	0 / 873	0.11 (2)	
B-C	0 / 63	-287.0 -287.0	0.23 (1)	I-E	-289 / 0	0.08 (1)	
C-D	-2028 / 0	-287.0 -287.0	0.24 (1)	C-I	-289 / 0	0.06 (1)	
D-E	-2028 / 0	-287.0 -287.0	0.24 (1)	J-C	-2704 / 0	0.48 (1)	
E-F	0 / 63	-287.0 -287.0	0.23 (1)	E-H	-2704 / 0	0.48 (1)	
F-G	0 / 47	-104.9 -104.9	0.08 (1)				
J-B	-482 / 0	0.0 0.0	0.02 (1)				
H-F	-482 / 0	0.0 0.0	0.02 (1)				
J-I	0 / 1701	-52.7 -52.7	0.20 (2)				
I-H	0 / 1701	-52.7 -52.7	0.20 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	LC1	MAX	MAX+	FACE	DIR	TYPE
D	6'-1-8	-991	-991		FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 32.5 PSF
DL	= 3.0 PSF
BOT CH. LL	= 10.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C

GIRDER TYPE: Cstd Girder
 START DISTANCE = 0-0
 START SPAN CARRIED = 7-6-0
 END DISTANCE = 12-3-0
 END SPAN CARRIED = 7-6-0
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF TOP CHORD.
 -ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
 SIDE SETBACK = 6-1-8
 END SETBACK = 6-1-8
 END WALL WIDTH = 5-8
 CORNER FRAMING TYPE: CONVENTIONAL
 END JACK TYPE: CONVENTIONAL
 APPLIED TO FRONT SIDE
 -ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.41")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.24/1.00 (C-D:1), BC=0.20/1.00 (I-J:2), WB=0.48/1.00 (C-J:1), GS=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DRWG NO. TAM 11981-08

STRUCTURAL

CONTINUED ON PAGE 2

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COMPONENT ONLY

JOB NAME 288737	TRUSS NAME T84	QUANTITY 1	PLY 2	JOB DESC. 44755 TRUSS DESC.	DRWG NO. T26
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Version 8.200 S Jan 6 2018 MITek Industries, Inc. Mon Mar 5 11:29:07 2018 Page 2
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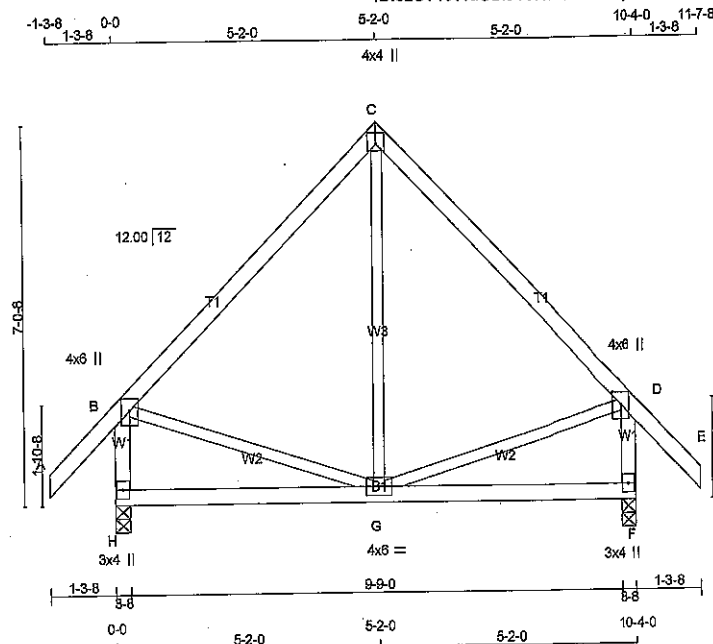
JSI GRIP= 0.86 (C) (INPUT = 0.90)
 JSI METAL= 0.33 (E) (INPUT = 1.00)



DWG NO. TAM 11981
 STRUCTURAL
 COMPONENT ONLY

SITE COPY

Scale = 1:42.8



TOTAL WEIGHT = 4 X 50 = 200 lb

LUMBER		LUMBER		DESCR
N. L. G. A.	RULES			
CHORDS	SIZE			
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
C	TMWV+p	MT20	4.0	6.0	2.75	2.00
D	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMWV+p	MT20	4.0	6.0	2.75	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVVVVW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

EDITING BEARINGS

	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION		UPLIFT	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ		IN-SX	IN-SX
H	831	0	831	0	0	3-8	3-8
E	831	0	831	0	0	3-8	3-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	641	426 / 0	103 / 0	0 / 0	0 / 0	112 / 0	0 / 0
F	641	426 / 0	103 / 0	0 / 0	0 / 0	112 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MAX.	MEMB.	FORCE	MAX.	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0/52	-104.9	-104.9	0.15 (1)	10.00	G-C	0/227	0.05 (3)	
B-C	-392/0	-104.9	-104.9	0.36 (1)	6.25	B-G	0/289	0.07 (1)	
C-D	-392/0	-104.9	-104.9	0.36 (1)	6.25	G-D	0/289	0.07 (1)	
D-E	0/52	-104.9	-104.9	0.15 (1)	10.00				
H-B	-778/0	0.0	0.0	0.09 (1)	7.81				
F-D	-778/0	0.0	0.0	0.09 (1)	7.81				
H-G	0/0	-27.5	-27.5	0.22 (3)	10.00				
G-F	0/0	-27.5	-27.5	0.22 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD =		52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , CBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.38/1.00 (B-C:1), BC=0.22/1.00 (G-H:3),
WB=0.07/1.00 (B-G:1), SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

	GRIP (DRY)		SHEAR		SECTION	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

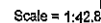
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)



DWG NO. TAM 11982-18
STRUCTURAL
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TOTAL WEIGHT = 56 lb

GABLE STUDS SPACED AT 2-0-0 OC.

UT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	6.0	2.75	2.00
C	D,F,G					
D	TMWw	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	1.50	2.00
F	TMWV+p	MT20	4.0	6.0	2.75	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWV1-t	MT20	4.0	4.0		
I	L,M,N					
J	BMW1+w	MT20	2.0	4.0		
K	BMWV1-t	MT20	4.0	4.0		
L	OP					
M	BMV1+p	MT20	3.0	4.0		

TOTAL LOAD CASES: (4)

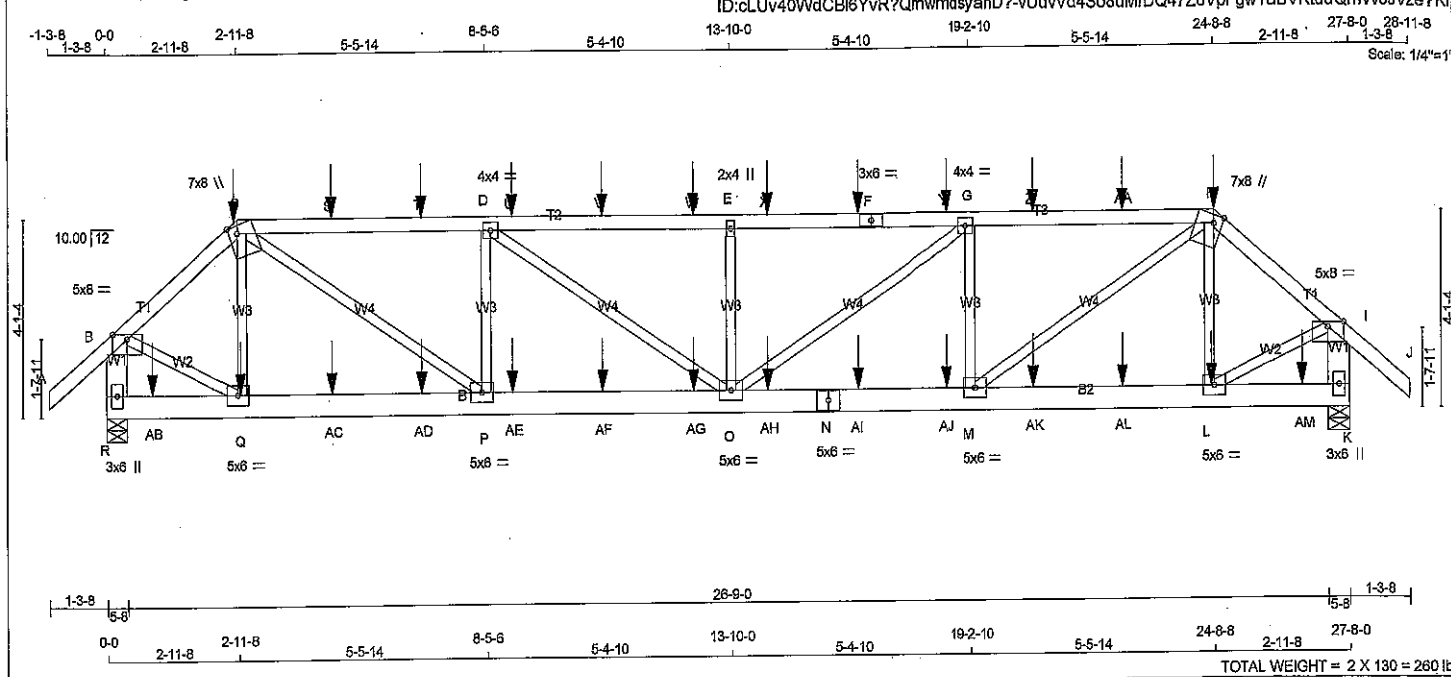
CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO				
P-B	-319 / 0	0.0	0.0	0.04 (1)	7.81	M-E	-163 / 0	0.13 (1)	
A-B	0 / 52	-104.9	-104.9	0.15 (1)	10.00	N-D	-253 / 0	0.09 (1)	
B-C	-79 / 0	-104.9	-104.9	0.14 (1)	6.25	O-C	-53 / 0	0.01 (1)	
C-D	-1 / 0	-104.9	-104.9	0.07 (1)	10.00	L-F	-253 / 0	0.09 (1)	
D-E	-24 / 0	-104.9	-104.9	0.07 (1)	6.25	K-G	-53 / 0	0.01 (1)	
E-F	-24 / 0	-104.9	-104.9	0.07 (1)	6.25	B-O	0 / 21	0.00 (1)	
F-G	-1 / 0	-104.9	-104.9	0.07 (1)	10.00	K-H	0 / 21	0.00 (1)	
G-H	-79 / 0	-104.9	-104.9	0.14 (1)	6.25				
H-I	0 / 52	-104.9	-104.9	0.15 (1)	10.00				
J-H	-319 / 0	0.0	0.0	0.04 (1)	7.81				
P-O	0 / 0	-27.5	-27.5	0.02 (3)	10.00				
O-N	0 / 11	-27.5	-27.5	0.02 (2)	10.00				
N-M	0 / 5	-27.5	-27.5	0.02 (3)	10.00				
M-L	0 / 5	-27.5	-27.5	0.02 (3)	10.00				
L-K	0 / 11	-27.5	-27.5	0.02 (2)	10.00				
K-J	0 / 0	-27.5	-27.5	0.02 (3)	10.00				

JSI GRIP= 0.32 (E) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



DWG NO. TAM 11983-18
STRUCTURAL
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LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA									
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS										*** SPECIAL LOADS ANALYSIS ***									
A - C	2x4	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED					INPUT					REQRD				
C - F	2x4	DRY	No.2	SPF	GROSS REACTION					GROSS REACTION					BRG					BRG				
F - H	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX						IN-SX							
H - J	2x4	DRY	No.2	SPF	R	3403	0	3403	0	0	5-8						5-8							
J - B	2x6	DRY	No.2	SPF	K	3608	0	3608	0	0	5-8						5-8							
K - I	2x6	DRY	No.2	SPF																				
R - N	2x6	DRY	No.2	SPF																				
N - K	2x6	DRY	No.2	SPF																				
ALL WEBS 2x3 DRY No.2 SPF															BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K									
EXCEPT																								
DRY: SEASONED LUMBER.																								
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:																								
CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)																								
TOP CHORDS : (0.122"x3") SPIRAL NAILS																								
A-C	1	12	SIDE(0.0)																					
C-F	1	12	SIDE(0.0)																					
F-H	1	12	SIDE(0.0)																					
H-J	1	12	SIDE(0.0)																					
R-B	2	12	TOP																					
K-I	2	12	TOP																					
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS																								
R-N	2	12	SIDE(0.0)																					
N-K	2	12	SIDE(0.0)																					
WEBS : (0.122"x3") SPIRAL NAILS																								
2x3	1	6																						
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.																								
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.																								
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.																								
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.																								
PLATES (table is in inches)																								
JT TYPE	PLATES	W	LEN Y X																					
B	TMVW+p	MT20	5.0 8.0 Edge																					
C	TTWW+m	MT20	7.0 8.0 Edge 2.25																					
D	TMVW+p	MT20	4.0 4.0																					
E	TMVW+p	MT20	2.0 4.0																					
F	TS-t	MT20	3.0 8.0																					
G	TMVW-t	MT20	4.0 4.0																					
H	TTWW+m	MT20	7.0 8.0 Edge 2.25																					
I	TMVW+p	MT20	5.0 8.0 Edge																					
K	BMV1+p	MT20	3.0 6.0																					
L	M, P, Q																							
L	BMVW-t	MT20	5.0 8.0																					
N	BS-t	MT20	5.0 6.0																					
O	BMVW-t	MT20	5.0 6.0																					
R	BMV1+p	MT20	3.0 6.0																					
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.																								

UNFACTORED REACTIONS									
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
R	2709	1654 / 0	529 / 0	0 / 0	0 / 0	526 / 0	0 / 0		
K	2885	1762 / 0	551 / 0	0 / 0	0 / 0	551 / 0	0 / 0		

BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									

LOADING									
TOTAL LOAD CASES: (4)									

CHORDS										WEBS									
MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX				
	(LBS)	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH		(LBS)	(LBS)	UNBRAC	LENGTH		(LBS)	(LBS)	UNBRAC				
FR-TO			FROM TO				FR-TO					FR-TO							
A-B	0 / 47		-104.9 -104.9	0.08 (1)	10.00		Q-C	-716 / 0	0.09 (1)			Q-C	-716 / 0	0.09 (1)					
B-C	-3246 / 0		-104.9 -104.9	0.13 (1)	5.05		C-P	0 / 3462	0.43 (1)			C-P	0 / 3462	0.43 (1)					
C-S	-5333 / 0		-104.9 -104.9	0.58 (1)	3.64		P-D	-1708 / 0	0.22 (1)			P-D	-1708 / 0	0.22 (1)					
S-T	-5333 / 0		-104.9 -104.9	0.58 (1)	3.64		D-O	0 / 1227	0.15 (1)			D-O	0 / 1227	0.15 (1)					
T-D	-5333 / 0		-104.9 -104.9	0.58 (1)	3.64		O-E	-908 / 0	0.11 (1)			O-E	-908 / 0	0.11 (1)					
D-U	-6342 / 0		-104.9 -104.9	0.65 (1)	3.30		O-G	0 / 890	0.11 (1)			O-G	0 / 890	0.11 (1)					
U-V	-6342 / 0		-104.9 -104.9	0.65 (1)	3.30		M-G	-1870 / 0	0.21 (1)			M-G	-1870 / 0	0.21 (1)					
V-W	-6342 / 0		-104.9 -104.9	0.65 (1)	3.30		M-H	0 / 3590	0.44 (1)			M-H	0 / 3590	0.44 (1)					
W-E	-6342 / 0		-104.9 -104.9	0.65 (1)	3.30		L-H	-713 / 0	0.09 (1)			L-H	-713 / 0	0.09 (1)					
E-X	-6342 / 0		-104.9 -104.9	0.72 (1)	3.19		B-Q	0 / 2674	0.33 (1)			B-Q	0 / 2674	0.33 (1)					
X-F	-6342 / 0		-104.9 -104.9	0.72 (1)	3.19		L-I	0 / 2859	0.35 (1)			L-I	0 / 2859	0.35 (1)					
F-Y	-6342 / 0		-104.9 -104.9	0.72 (1)	3.19														
Y-G	-6342 / 0		-104.9 -104.9	0.72 (1)	3.19														
G-Z	-5810 / 0		-104.9 -104.9	0.67 (1)	3.44														
Z-AA	-5810 / 0		-104.9 -104.9	0.67 (1)	3.44														
AA-H	-5810 / 0		-104.9 -104.9	0.67 (1)	3.44														
H-I	-3470 / 0		-104.9 -104.9	0.13 (1)	4.92														
I-J	0 / 47		-104.9 -104.9	0.08 (1)	10.00														
R-B	-3379 / 0		0.0 0.0	0.12 (1)	7.58														
K-I	-3591 / 0		0.0 0.0	0.13 (1)	7.40														
R-AB	0 / 0		-27.5 -27.5	0.06 (2)	10.00														
AB-Q	0 / 0		-27.5 -27.5	0.08 (2)	10.00														
Q-AC	0 / 2464		-27.5 -27.5	0.21 (1)	10.00														
AC-AD	0 / 2464		-27.5 -27.5	0.21 (1)	10.00														
AD-P	0 / 2464		-27.5 -27.5	0.21 (1)	10.00														
P-AE	0 / 5333		-27.5 -27.5	0.42 (1)	10.00														
AE-AF	0 / 5333		-27.5 -27.5	0.42 (1)	10.00														
AF-AG	0 / 5333		-27.5 -27.5	0.42 (1)	10.00														
AG-O	0 / 5333		-27.5 -27.5	0.42 (1)	10.00														
O-AH	0 / 5610		-27.5 -27.5	0.45 (1)	10.00														
AH-N	0 / 5610		-27.5 -27.5	0.45 (1)	10.00														
N-AJ	0 / 5610		-27.5 -27.5	0.45 (1)	10.00														
AJ-AJ	0 / 5610		-27.5 -27.5	0.45 (1)	10.00														
AJ-M	0 / 5610		-27.5 -27.5	0.45 (1)	10.00														
M-AK	0 / 2636		-27.5 -27.5	0.23 (1)	10.00														
AK-AL	0 / 2636		-27.5 -27.5	0.23 (1)	10.00														
AL-L	0 / 2636		-27.5 -27.5	0.23 (1)	10.00														
L-AM	0 / 0		-27.5 -27.5	0.07 (2)	10.00														
AM-K	0 / 0		-27.5 -27.5	0.07 (2)	10.00														

ALLOWABLE DEFL.(LL)= L/360 (0.92")												
CALCULATED VERT. DEFL.(LL)= L/999 (0.17")												
ALLOWABLE DEFL.(TL)= L/360 (0.92")												
CALCULATED VERT. DEFL.(TL)= L/999 (0.26")												
CSI: TC=0.72/1.00 (E-G:1), BC=0.45/1.00 (M-O:1)												
WB=0.44/1.00 (H-M:1), SS=0.28/1.00 (E-G:1)												
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00												
COMP=1.00 SHEAR=1.00 TENS= 1.00												
COMPANION LIVE LOAD FACTOR = 0.50												
AUTOSOLVE HEELS OFF												
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.												
VALUES												
PLATE GRIP(DRY)	SHEAR	SECTION										
(PSI)	(PLI)	(PLI)										
MAX MIN	MAX MIN	MAX MIN										
MT20	618	354	1687	822	2284	1658						
PLATE PLACEMENT TOL = 0.250 inches												
PLATE ROTATION TOL = 5.0 Deg.												
JSI GRIP=0.86 (M) (INPUT = 0.90)												
JSI METAL= 0.51 (N) (INPUT = 1.00)												

3578

S KATSOULAKOS

PROVINCE OF ONTARIO

DRWG NO. TAM 1974-18

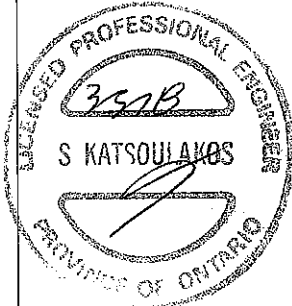
STRUCTURAL

CONTINUED ON PAGE 2

HANGERS NOTES

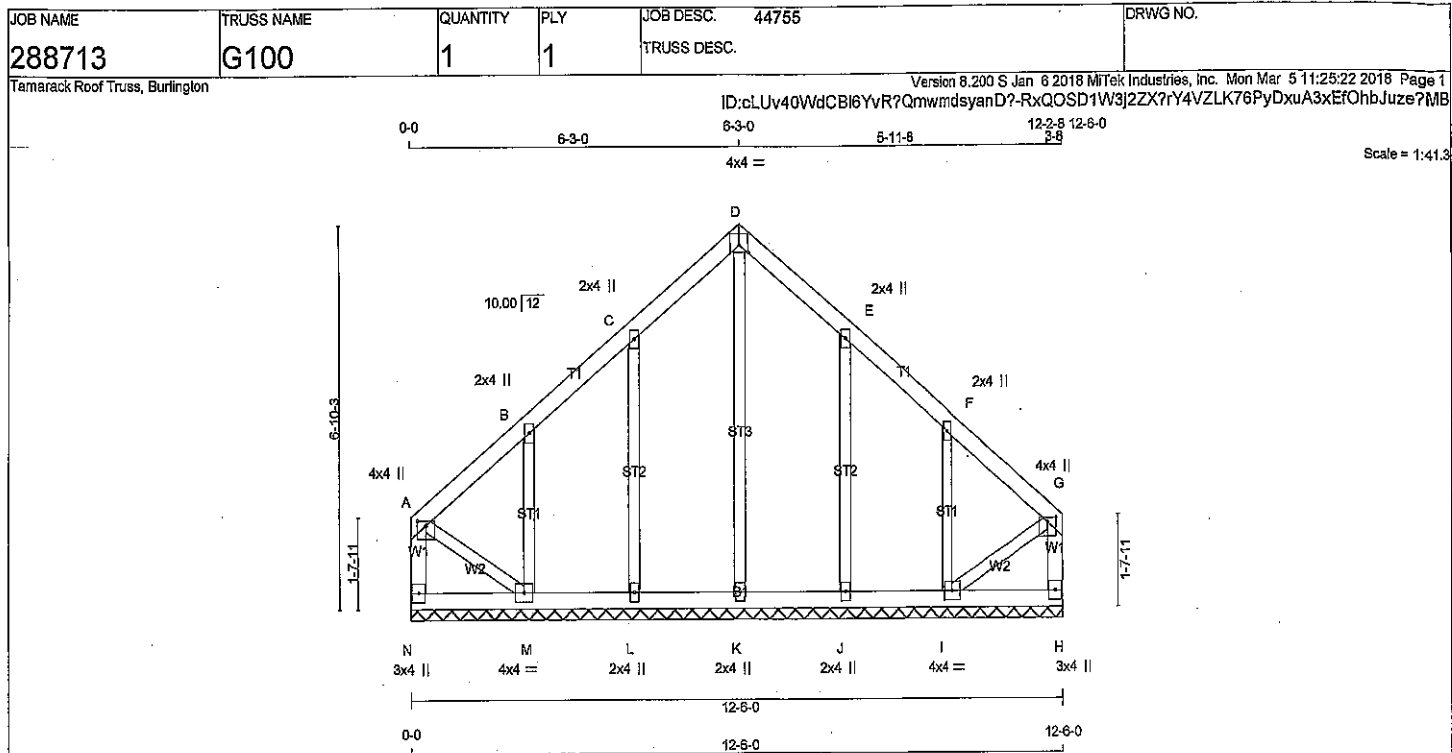
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 252.4 lbs FACTORED DOWN AT 2-11-8, 252.4 lbs FACTORED DOWN AT 24-8-8, 129.8 lbs FACTORED DOWN AT 2-11-8, 126.3 lbs FACTORED DOWN AT 5-0-4, 126.3 lbs FACTORED DOWN AT 7-0-4, 126.3 lbs FACTORED DOWN AT 9-0-4, 126.3 lbs FACTORED DOWN AT 11-0-4, 126.3 lbs FACTORED DOWN AT 13-0-4, 170.0 lbs FACTORED DOWN AT 14-7-12, 170.0 lbs FACTORED DOWN AT 16-7-12, 170.0 lbs FACTORED DOWN AT 18-7-12, 170.0 lbs FACTORED DOWN AT 20-7-12, AND 170.0 lbs FACTORED DOWN AT 22-7-12, AND 173.6 lbs FACTORED DOWN AT 24-8-8 ON TOP CHORD, AND 67.8 lbs FACTORED DOWN AT 1-0-4, 67.8 lbs FACTORED DOWN AT 3-0-4, 67.8 lbs FACTORED DOWN AT 5-0-4, 67.8 lbs FACTORED DOWN AT 7-0-4, 67.8 lbs FACTORED DOWN AT 9-0-4, 67.8 lbs FACTORED DOWN AT 11-0-4, 67.8 lbs FACTORED DOWN AT 13-0-4, 87.3 lbs FACTORED DOWN AT 14-7-12, 87.3 lbs FACTORED DOWN AT 16-7-12, 87.3 lbs FACTORED DOWN AT 18-7-12, 87.3 lbs FACTORED DOWN AT 20-7-12, 87.3 lbs FACTORED DOWN AT 22-7-12, AND 87.3 lbs FACTORED DOWN AT 24-7-12, AND 67.8 lbs FACTORED DOWN AT 26-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-252	-252	---	FRONT	VERT	TOTAL
C	2-11-8	-130	-130	---	BACK	VERT	TOTAL
F	16-7-12	-170	-170	---	BACK	VERT	TOTAL
H	24-8-8	-252	-252	---	FRONT	VERT	TOTAL
H	24-8-8	-174	-174	---	BACK	VERT	TOTAL
L	24-7-12	-76	-87	---	BACK	VERT	TOTAL
Q	3-0-4	-39	-68	---	BACK	VERT	TOTAL
S	5-0-4	-126	-126	---	BACK	VERT	TOTAL
T	7-0-4	-126	-126	---	BACK	VERT	TOTAL
U	9-0-4	-126	-126	---	BACK	VERT	TOTAL
V	11-0-4	-126	-126	---	BACK	VERT	TOTAL
W	13-0-4	-126	-126	---	BACK	VERT	TOTAL
X	14-7-12	-170	-170	---	BACK	VERT	TOTAL
Y	18-7-12	-170	-170	---	BACK	VERT	TOTAL
Z	20-7-12	-170	-170	---	BACK	VERT	TOTAL
AA	22-7-12	-170	-170	---	BACK	VERT	TOTAL
AB	1-0-4	-39	-68	---	BACK	VERT	TOTAL
AC	5-0-4	-39	-68	---	BACK	VERT	TOTAL
AD	7-0-4	-39	-68	---	BACK	VERT	TOTAL
AE	9-0-4	-39	-68	---	BACK	VERT	TOTAL
AF	11-0-4	-39	-68	---	BACK	VERT	TOTAL
AG	13-0-4	-39	-68	---	BACK	VERT	TOTAL
AH	14-7-12	-76	-87	---	BACK	VERT	TOTAL
AI	16-7-12	-76	-87	---	BACK	VERT	TOTAL
AJ	18-7-12	-76	-87	---	BACK	VERT	TOTAL
AK	20-7-12	-76	-87	---	BACK	VERT	TOTAL
AL	22-7-12	-76	-87	---	BACK	VERT	TOTAL
AM	26-7-12	-39	-68	---	BACK	VERT	TOTAL



DWG NO. TAM 11978-B
STRUCTURAL
COMPONENT ONLY

SITE COPY



TOTAL WEIGHT = 57 lb

LUMBER

N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR
N - A	2x4	DRY	No.2		SPF
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
N - H	2x4	DRY	No.2		SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, E, F					
B TMW+w	MT20	2.0	4.0		
D TTW-p	MT20	4.0	4.0	1.50	2.00
G TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1-t	MT20	4.0	4.0		
J, K, L					
J BMW1+w	MT20	2.0	4.0		
M BMVW1-t	MT20	4.0	4.0		
N BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
N-A	-146 / 0	0.0 0.0	0.02 (1)	7.81	K-D	-134 / 0	0.10 (1)
A-B	-37 / 0	-104.9 -104.9	0.06 (1)	6.25	L-C	-229 / 0	0.09 (1)
B-C	-42 / 0	-104.9 -104.9	0.06 (1)	6.25	M-B	-238 / 0	0.05 (1)
C-D	-48 / 0	-104.9 -104.9	0.06 (1)	6.25	J-E	-229 / 0	0.09 (1)
D-E	-48 / 0	-104.9 -104.9	0.06 (1)	6.25	I-F	-236 / 0	0.05 (1)
E-F	-42 / 0	-104.9 -104.9	0.06 (1)	6.25	A-M	0 / 45	0.01 (1)
F-G	-37 / 0	-104.9 -104.9	0.06 (1)	6.25	I-G	0 / 45	0.01 (1)
H-G	-146 / 0	0.0 0.0	0.02 (1)	7.81			
N-M	0 / 0	-27.5 -27.5	0.03 (2)	10.00			
M-L	0 / 31	-27.5 -27.5	0.03 (2)	10.00			
L-K	0 / 27	-27.5 -27.5	0.02 (2)	10.00			
K-J	0 / 27	-27.5 -27.5	0.02 (2)	10.00			
J-I	0 / 31	-27.5 -27.5	0.03 (2)	10.00			
I-H	0 / 0	-27.5 -27.5	0.03 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (E-F:1), BC=0.03/1.00 (I-J:2), WB=0.10/1.00 (D-K:1), SS=0.06/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

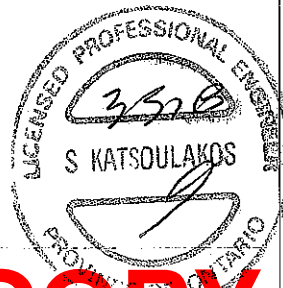
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

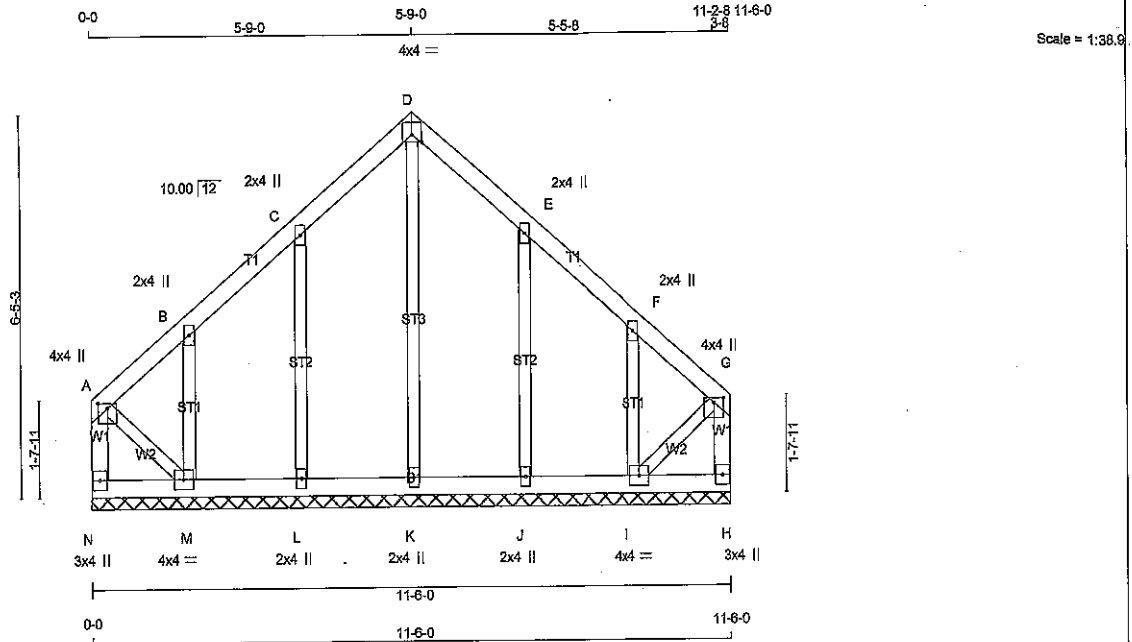
JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAM 11969-38
STRUCTURAL
COMPONENT ONLY

SITE COPY

JOB NAME 288737	TRUSS NAME G101	QUANTITY 1	PLY 1	JOB DESC. 44755	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 11:29:03 2018 Page 1	ID: cLUv40WdCBi6YvR?Qmwmndsyand?STHxqni80cuCXzF0XQ1AojmABJMkS0vmtGjmYaze?Ik



LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
N - A	2x4	DRY	No.2		SPF
A - D	2x4	DRY	No.2		SPF
D - G	2x4	DRY	No.2		SPF
H - G	2x4	DRY	No.2		SPF
N - H	2x4	DRY	No.2		SPF

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" O.C.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, E, F					
B TMVW+w	MT20	2.0	4.0		
D TTW-p	MT20	4.0	4.0	1.50	2.00
G TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1-t	MT20	4.0	4.0		
J, K, L					
J BMV1+w	MT20	2.0	4.0		
M BMVW1-t	MT20	4.0	4.0		
N BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC)	UNBRACED LENGTH (FR-TO)
FR-TO				FR-TO			
N-A	-127 / 0	0.0	0.0 0.01 (1)	7.81	K-D	-134 / 0	0.08 (1)
A-B	-37 / 0	-104.9	-104.9 0.04 (1)	8.25	L-C	-238 / 0	0.08 (1)
B-C	-35 / 0	-104.9	-104.9 0.08 (1)	8.25	M-B	-200 / 0	0.04 (1)
C-D	-47 / 0	-104.9	-104.9 0.08 (1)	6.25	J-E	-238 / 0	0.08 (1)
D-E	-47 / 0	-104.9	-104.9 0.08 (1)	6.25	I-F	-200 / 0	0.04 (1)
E-F	-36 / 0	-104.9	-104.9 0.08 (1)	6.25	A-M	0 / 47	0.01 (1)
F-G	-37 / 0	-104.9	-104.9 0.04 (1)	6.25	I-G	0 / 47	0.01 (1)
H-G	-127 / 0	0.0	0.0 0.01 (1)	7.81			
N-M	0 / 0	-27.5	-27.5 0.02 (2)	10.00			
M-L	0 / 31	-27.5	-27.5 0.03 (2)	10.00			
L-K	0 / 26	-27.5	-27.5 0.02 (2)	10.00			
K-J	0 / 26	-27.5	-27.5 0.02 (2)	10.00			
J-I	0 / 31	-27.5	-27.5 0.03 (2)	10.00			
I-H	0 / 0	-27.5	-27.5 0.02 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D:1), BC=0.03/1.00 (I-J:2), WB=0.09/1.00 (D-K:1), SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 822	2284 1656

PLATE PLACEMENT TOL = 0.250 inches

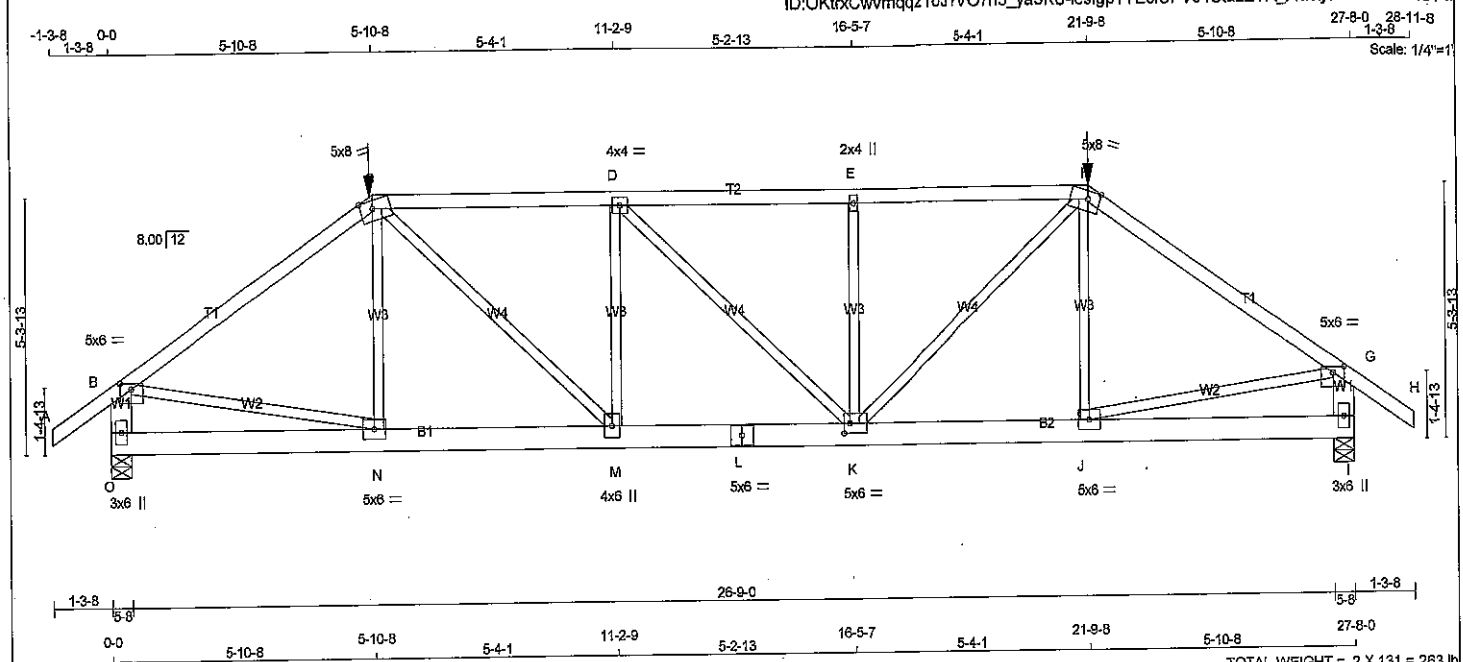
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.80)
JSI METAL= 0.08 (C) (INPUT = 1.00)



SITE COPY

DWG NO. TAM 1984-18
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 131 = 263 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - F	2x4	DRY	No.2 SPF
F - H	2x4	DRY	No.2 SPF
O - B	2x6	DRY	No.2 SPF
I - G	2x6	DRY	No.2 SPF
O - L	2x6	DRY	No.2 SPF
L - I	2x6	DRY	No.2 SPF
ALL WEBS EXCEPT	2x3	DRY	No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(0.0)
C-F 1	12	SIDE(44.8)
F-H 1	12	SIDE(0.0)
O-B 2	12	TOP
I-G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
O-L 2	12	SIDE(20.3)
L-I 2	12	SIDE(20.3)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p MT20	5.0	6.0	1.50	3.00
C	TTWW-m MT20	5.0	8.0	2.00	3.25
D	TMVW-t MT20	4.0	4.0		
E	TMVW-w MT20	2.0	4.0		
F	TTWW-m MT20	5.0	8.0	2.00	3.25
G	TMVW-p MT20	5.0	8.0	1.50	3.00
I	BMV1+p MT20	3.0	6.0		
J	BMVW-t MT20	5.0	6.0		
K	BMVWW-t MT20	5.0	6.0	2.50	1.50
L	BS-t MT20	5.0	8.0		
M	BMVW-t MT20	4.0	8.0		
N	BMVW-t MT20	5.0	6.0		
O	BMV1+p MT20	3.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	3464	0	3464	0	5-8	5-8
I	3464	0	3464	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX / MIN. COMPONENT REACTIONS		DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE		
O	2737	1707 / 0	513 / 0	0 / 0	517 / 0	0 / 0
I	2737	1707 / 0	513 / 0	0 / 0	517 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

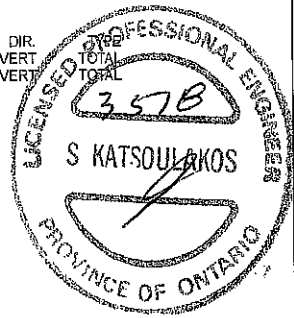
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		MAX. UNBRACED LENGTH	FR-TO	WEBS		FACTORED	
	MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)			MEMB.	FORCE (LBS)	MAX. (LBS)	CSI (LC)
A-B	0 / 40	-104.9	-104.9	0.08 (1)	10.00	N-C	-252 / 331	0.05 (1)		
B-C	-4145 / 0	-104.9	-104.9	0.53 (1)	4.19	C-M	0 / 2013	0.25 (1)		
C-D	-4919 / 0	-194.6	-194.6	0.56 (1)	3.77	M-D	-1104 / 0	0.22 (1)		
D-E	-4906 / 0	-194.6	-194.6	0.56 (1)	3.77	D-K	-18 / 0	0.01 (1)		
E-F	-4906 / 0	-194.6	-194.6	0.55 (1)	3.78	K-E	-1109 / 0	0.23 (1)		
F-G	-4145 / 0	-104.9	-104.9	0.53 (1)	4.19	K-F	0 / 1991	0.25 (1)		
G-H	0 / 40	-104.9	-104.9	0.08 (1)	10.00	J-F	-239 / 334	0.05 (1)		
O-B	-3332 / 0	0.0	0.0	0.12 (1)	7.61	B-N	0 / 3493	0.43 (1)		
I-G	-3334 / 0	0.0	0.0	0.12 (1)	7.61	J-G	0 / 3495	0.43 (1)		
O-N	0 / 0	-51.0	-51.0	0.10 (3)	10.00					
N-M	0 / 3440	-51.0	-51.0	0.27 (1)	10.00					
M-L	0 / 4919	-51.0	-51.0	0.38 (1)	10.00					
L-K	0 / 4919	-51.0	-51.0	0.38 (1)	10.00					
K-J	0 / 3442	-51.0	-51.0	0.27 (1)	10.00					
J-I	0 / 0	-51.0	-51.0	0.10 (3)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	5-10-8	-450	-450		FRONT	VERT
F	21-9-8	-450	-450		FRONT	VERT



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 3.0 PSF
BOT CH. LL = 10.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
-ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
-PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.82")
CALCULATED VERT. DEFL.(LL)= L/999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.82")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.58/1.00 (C-D:1), BC=0.38/1.00 (K-M:1),
WB=0.43/1.00 (G-J:1), SS=0.27/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

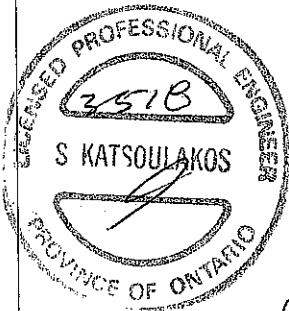
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (G) (INPUT = 0.90)
JSI METAL= 0.43 (L) (INPUT = 1.00)

SITE COPY DRWG NO. TAM 12006-18 STRUCTURAL COMPONENT ONLY

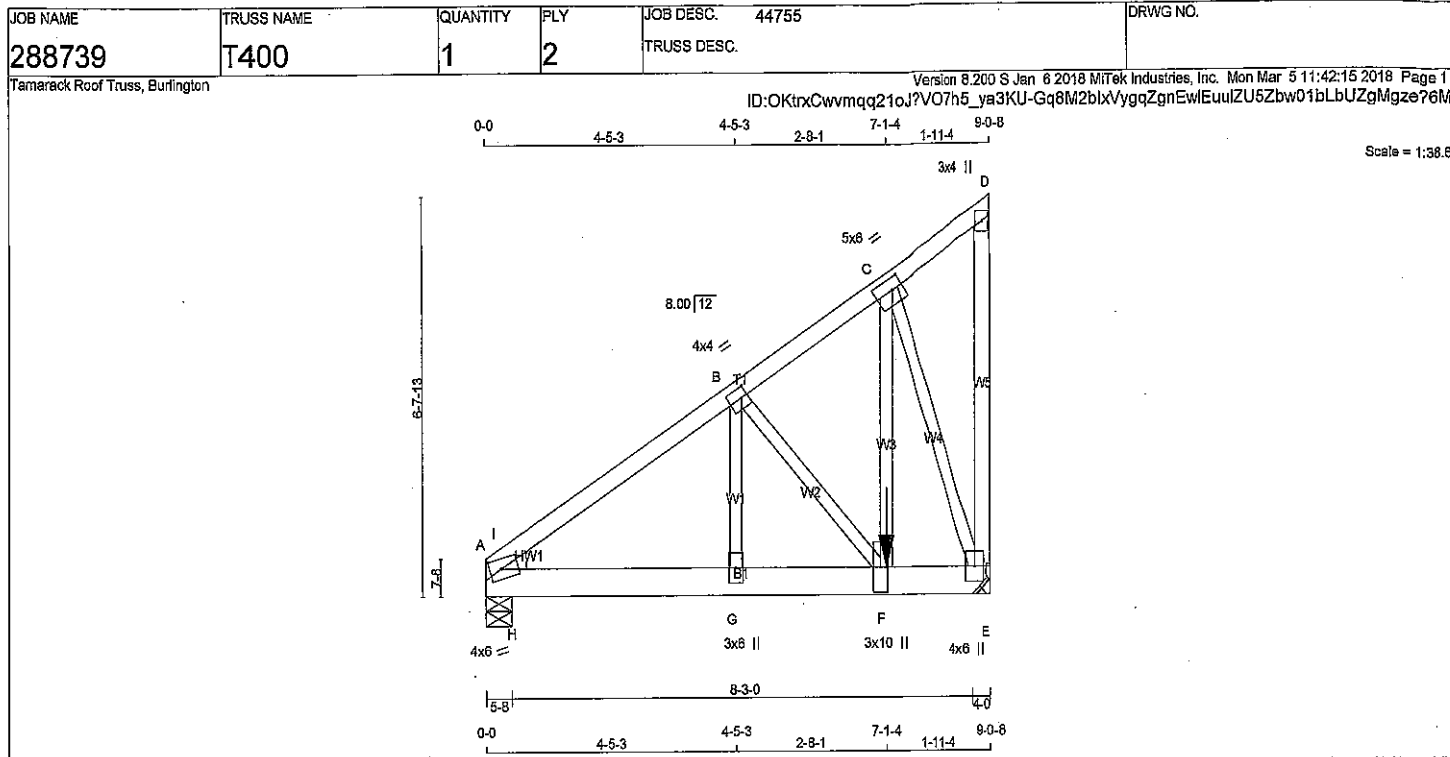
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. 44755	DRWG NO.
288746	T290	1	2	TRUSS DESC.	
Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Mar 5 11:43:58 2018 Page 2 ID:OKtrxCwvmgg21cJ7VO7h5_ya3KU-leslppYYEcCPV91Sta2ZTK_X4lclj3VG00dTCze74I					

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S)
 REQUIRED TO SUPPORT CONCENTRATED
 LOAD(S) 449.5 lbs FACTORED DOWN AT 21-9-8,
 AND 449.5 lbs FACTORED DOWN AT 5-10-8 ON
 TOP CHORD. DESIGN FOR UNSPECIFIED
 CONNECTION(S) IS DELEGATED TO THE
 BUILDING DESIGNER.



DWG NO. TAM 12006-18
 STRUCTURAL
 COMPONENT ONLY

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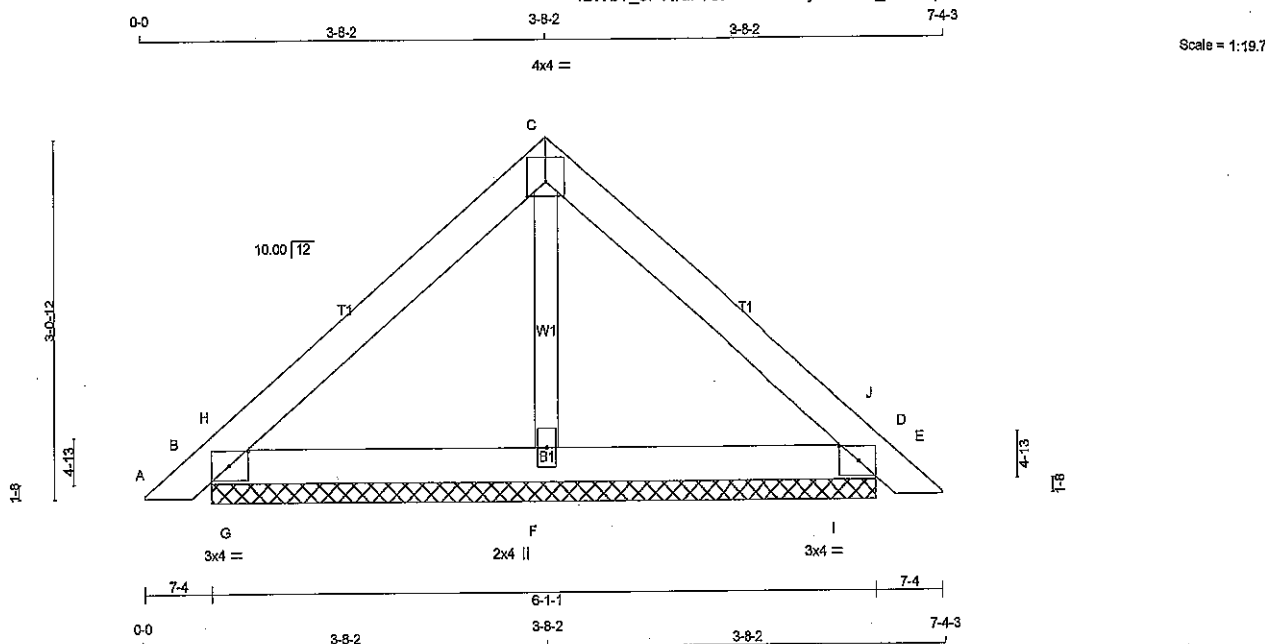
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - D 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF A - E 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF) TOP CHORDS: (0.122"x3") SPIRAL NAILS A-D 1 12 TOP D-E 1 12 TOP BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS A-E 2 12 SIDE (180.0) WEBS: (0.122"x3") SPIRAL NAILS 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMBH1-m MT20 4.0 8.0 2.00 2.50 B TMWW1 MT20 4.0 4.0 2.00 1.25 C TMWW1 MT20 5.0 8.0 2.25 2.25 D TMW1-p MT20 3.0 4.0 E BMWW1-p MT20 4.0 8.0 F BMWW1-p MT20 3.0 10.0 G BMW1-w MT20 3.0 6.0 HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2136.1 lbs FACTORED DOWN AT 7-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY .BUILDING DESIGNER BEARINGS FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT REGRD HEEL JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX WEDGE E 3905 0 3905 0 0 HANGER BY OTHERS MIN. SEAT SIZE: 4-0 5-8 5-8 2x4 L A 2684 0 2684 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL E 3080 1932 / 0 570 / 0 0 / 0 0 / 0 578 / 0 0 / 0 A 2123 1321 / 0 400 / 0 0 / 0 0 / 0 402 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX (PLF) CSI (LC) WEBS MAX. FACTORED FORCE (LBS) MAX. FACTORED FORCE (LBS) CSI (LC) FR-TO FROM TO LENGTH FR-TO A-I -3485 / 0 -104.9 -104.9 0.07 (1) 4.99 G-B 0 / 1708 0.21 (1) I-B -2879 / 0 -104.9 -104.9 0.14 (1) 5.29 F-C 0 / 13875 0.48 (1) B-C -1544 / 0 -104.9 -104.9 0.07 (1) 6.25 B-E -1745 / 0 0.26 (1) C-D -9 / 0 -104.9 -104.9 0.03 (1) 10.00 C-E -3503 / 0 0.81 (1) E-D -86 / 0 0.0 0.0 0.04 (1) 7.81 H-I 0 / 878 0.00 (1) A-H 0 / 2400 -387.5 -387.5 0.17 (1) 10.00 H-G 0 / 2400 -387.5 -387.5 0.35 (1) 10.00 G-F 0 / 2400 -387.5 -387.5 0.33 (1) 10.00 F-E 0 / 1276 -387.5 -387.5 0.23 (1) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT. LOC. LC1 MAX- MAX+ FACE DIR. TYPE F 7-1-4 -2136 -2136 - FRONT VERT TOTAL DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 3.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. C/C GIRDER TYPE: CSid Girder START DISTANCE = 0-0 START SPAN CARRIED = 13-0-0 END DISTANCE = 9-0-8 END SPAN CARRIED = 13-0-0 END WALL WIDTH = 1-8 APPLIED TO FRONT SIDE OF BOTTOM CHORD. -ADDT'L LOADS BASED ON 55 % OF GSL. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 086-09 - TPIC 2011 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL. (LL) = L/360 (0.30") CALCULATED VERT. DEFL. (LL) = L/999 (0.03") ALLOWABLE DEFL. (TL) = L/360 (0.30") CALCULATED VERT. DEFL. (TL) = L/999 (0.05") CSI: TC=0.14/1.00 (B-H), BC=0.35/1.00 (G-H), WB=0.81/1.00 (C-E-1), SSI=0.29/1.00 (G-H-1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1687 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (C) (INPUT = 0.90) JSI METAL= 0.36 (F) (INPUT = 1.00) DWG NO. TAM 12003.378 STRUCTURAL COMPONENT ONLY	
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Tamarack Roof Trusses, Burlington

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TOTAL WEIGHT = 5 X 20 = 98 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
B - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	314	0	314	0	0	6-1-1	6-1-1
D	314	0	314	0	0	6-1-1	6-1-1
F	288	0	288	0	0	6-1-1	6-1-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	234	170 / 0	28 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	234	170 / 0	28 / 0	0 / 0	0 / 0	35 / 0	0 / 0
F	245	123 / 0	65 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 16	-104.9 -104.9	0.02 (1)	10.00	F-C	-107 / 2	0.02 (1)
B-H	-58 / 0	-104.9 -104.9	0.06 (1)	6.25	G-H	-317 / 40	0.00 (1)
H-C	-132 / 0	-104.9 -104.9	0.11 (1)	6.25	I-J	-317 / 40	0.00 (1)
C-J	-132 / 0	-104.9 -104.9	0.11 (1)	6.25			
J-D	-58 / 0	-104.9 -104.9	0.06 (1)	6.25			
D-E	0 / 16	-104.9 -104.9	0.02 (1)	10.00			
B-G	0 / 94	-27.5 -27.5	0.12 (1)	10.00			
G-F	0 / 94	-27.5 -27.5	0.12 (1)	10.00			
F-I	0 / 94	-27.5 -27.5	0.12 (1)	10.00			
I-D	0 / 94	-27.5 -27.5	0.12 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
	DL = 3.0 PSF
BOT CH.	LL = 10.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (C-J:1), BC=0.12/1.00 (B-G:1), WB=0.02/1.00 (C-F:1), SS=0.24/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (D) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



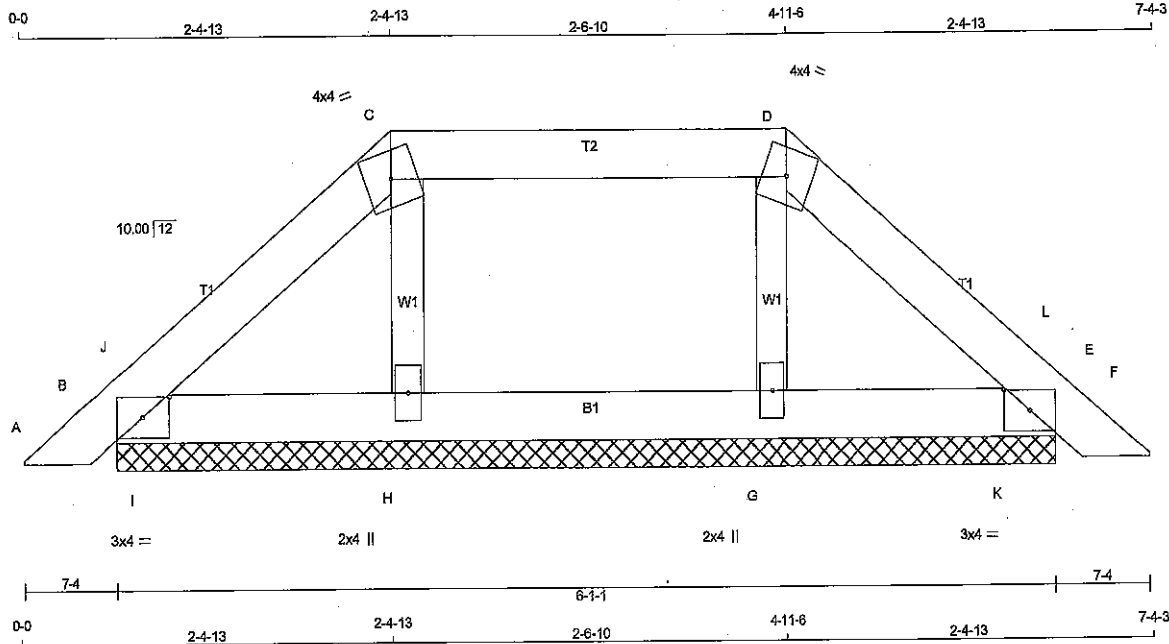
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STRUCTURAL
COMPONENT ONLY

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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 19 = 38 lb

LUMBER					DESCR.	
N. L. G. A. RULES						
CHORDS	SIZE		LUMBER			
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
B - E	2x4	DRY	No.2	SPF		
ALL WEBS 2x3 DRY					SPF	
DRY; SEASONED LUMBER.						

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.00
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	1.50 2.00
G	BMW1+w	MT20	2.0	4.0	
H	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	213	0	213	0	6-1-1	6-1-1
E	213	0	213	0	6-1-1	6-1-1
H	245	0	245	0	6-1-1	6-1-1
G	245	0	245	0	6-1-1	6-1-1

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
B	158	119/0	15/0	0/0	0/0	22/0	0/0
E	158	119/0	15/0	0/0	0/0	22/0	0/0
H	201	113/0	46/0	0/0	0/0	42/0	0/0
G	201	113/0	46/0	0/0	0/0	42/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/16	-104.9 -104.9 0.02 (1)	H-C	-165/0 0.02 (1)
B-J	-63/0	-104.9 -104.9 0.01 (1)	G-D	-165/0 0.02 (1)
J-C	-73/0	-104.9 -104.9 0.04 (1)	I-J	-108/15 0.00 (1)
C-D	-43/0	-104.9 -104.9 0.12 (1)	K-L	-108/15 0.00 (1)
D-L	-73/0	-104.9 -104.9 0.04 (1)		
L-E	-63/0	-104.9 -104.9 0.01 (1)		
E-F	0/16	-104.9 -104.9 0.02 (1)		
B-I	0/54	-27.5 -27.5 0.04 (1)		
I-H	0/54	-27.5 -27.5 0.04 (1)		
H-G	0/43	-27.5 -27.5 0.03 (2)		
G-K	0/54	-27.5 -27.5 0.04 (1)		
K-E	0/54	-27.5 -27.5 0.04 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.12/1.00 (C-D:1), BC=0.04/1.00 (B-I:1), WB=0.02/1.00 (D-G:1), SS=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
	(PSI)	(PLI)	(PLI)	
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

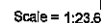
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.80)
JSI METAL= 0.04 (E) (INPUT = 1.00)



DRWG NO. TAM 1196B-18
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TOTAL WEIGHT = $6 \times 20 = 117$ lb

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	3.0	PSF
BOT CH.	LL =	10.0	PSF
	DL =	7.0	PSF
TOTAL LOAD =		52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/646$ (0.11")
ALLOWABLE DEFL.(TL)= $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/404$ (0.17")

CSI: TC=0.35/1.00 (B-C:1), BC=0.55/1.00 (E-F:1),
WB=0.11/1.00 (A-F:1), SSI=0.27/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

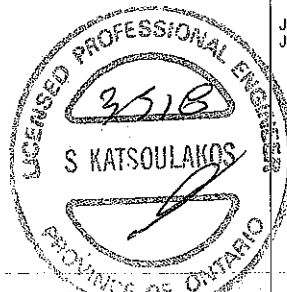
	GRIP (DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX. MIN.	MAX.	MIN.	MAX.	MIN.	MAX.	MIN.
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

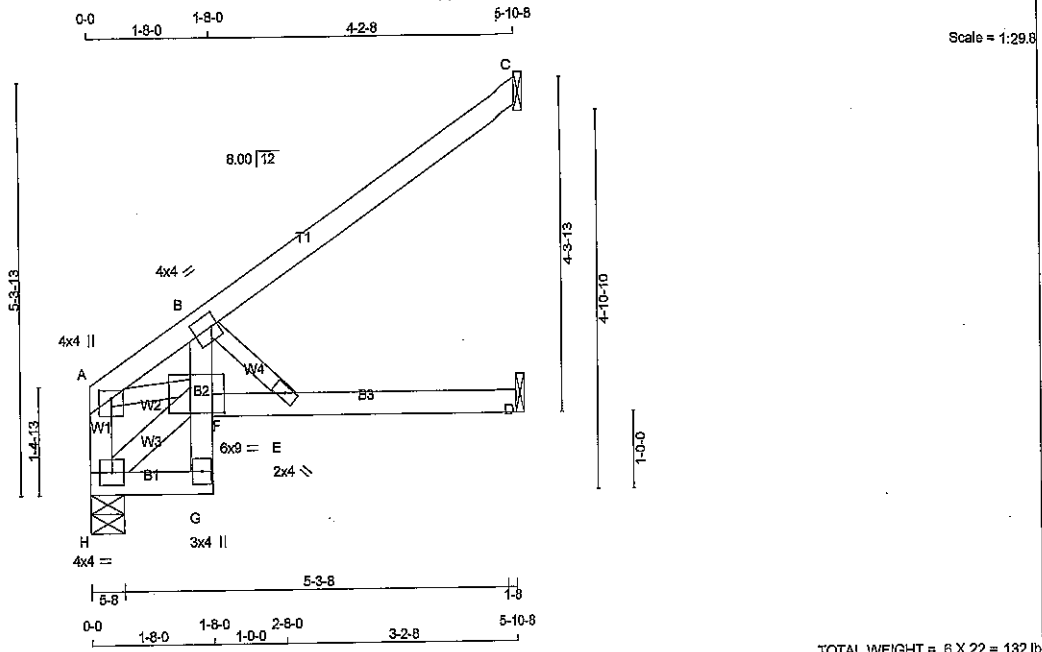
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.71 (A) (INPUT = 0.90)
JSI METAL= 0.24 (E) (INPUT = 1.00)

DWG. NO. TAM 11970-18
STRUCTURAL
COMPONENT ONLY



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TOTAL WEIGHT = 6 X 22 = 132 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
H - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS EXCEPT			
H - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMVW+w	MT20	2.0	4.0		
F	BMVW-w	MT20	6.0	8.0	4.00	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
H	389	0	389	0	0	5-8	5-8	5-8	5-8
C	229	0	229	0	0	1-8	1-8	1-8	1-8
D	160	0	160	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	308	191 / 0	59 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	162	135 / 0	8 / 0	0 / 0	0 / 0	18 / 0	0 / 0
D	147	56 / 0	50 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	MAX. FORCE (LBS)	MAX. (LC)	MEMB.
FR-TO		FROM	TO	FR-TO			
H-A	-350 / 0	0.0	0.0	0.04 (1)	7.81	H-F	-30 / 0
A-B	-528 / 0	-104.9	-104.9	0.11 (3)	6.25	A-F	0 / 438
B-C	0 / 11	-104.9	-104.9	0.34 (1)	10.00	B-E	-846 / 0
H-G	0 / 24	-27.5	-27.5	0.02 (2)	10.00		
G-F	0 / 36	0.0	0.0	0.04 (1)	10.00		
F-B	0 / 433	0.0	0.0	0.11 (1)	10.00		
F-E	0 / 518	-27.5	-27.5	0.59 (1)	10.00		
E-D	0 / 0	-27.5	-27.5	0.50 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 3.0	PSF
BOT CH.	LL = 10.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/599 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/375 (0.19")

CSI: TC=0.34/1.00 (B-C:1), BC=0.59/1.00 (E-F:1), WB=0.10/1.00 (A-F:1), SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.58 (A) (INPUT = 0.90)
JSI METAL= 0.38 (E) (INPUT = 1.00)



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DRWG NO. TAM 12004-08
STRUCTURAL
COMPONENT ONLY

TBE – Truss Bearing Enhancers

One size works with any number of girder plys. The TBE transfers load from the truss or girder to plates for bearing-limited conditions, and provides exceptional uplift capacity. Replaces nail-on scabs that provide lower load transfer, or in some cases, an additional ply when needed for bearing. The table lists factored resistances for TBE4 used on 2x4 and TBE6 used on 2x6 top plates. The tables give the different resistances calculated for TBE with and without wood bearing.

Material: 18 gauge

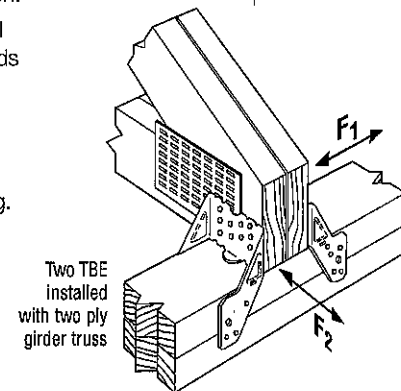
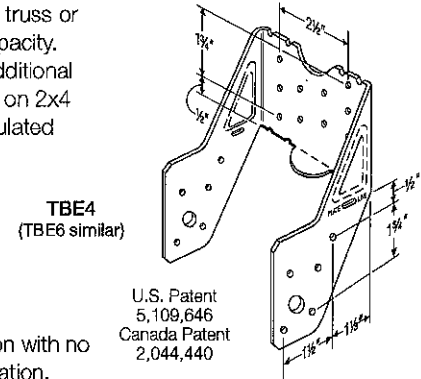
Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA 086-14.
- Factored uplift resistances have been increased 15% for short term load duration with no further increase allowed; reduce resistances by 15% for standard term load duration.
- Factored resistances are determined by nail shear calculations or tests of the metal connectors. The attached wood members must be designed to with stand the loads imposed by the nails.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia x 3" long common wire, 10d x 1½" = 0.148" dia x 1½" long.
- TBE must be installed in pairs.
- Top plate size is 2x4 for TBE4, 2x6 for TBE6. Use alternate installation for TBE4 and TBE6 on larger plates or pre-sheathed walls.
- Do not use TBEs in end-grain-bearing applications.
- See current catalogue for alternate installations.



Model No.	No. of Plys	Fasteners		TBE Only Factored Resistance				Combined TBE and Wood Bearing Factored Resistance	
		Truss	Plate	Uplift ($K_0 = 1.15$)	Normal ($K_0 = 1.00$)	Lateral ($K_0 = 1.15$)		Normal ² ($K_0 = 1.00$)	TBL ⁴
						F ₁	F ₂		
				lb.	lb.	lb.	lb.	lb.	in.
D.Fir-L									
TBE4	1	(20) 10d x 1 ½"	(20) 10d x 1 ½"	1605	3540	655	1415	7820	6.44
	2	(20) 10d	(20) 10d	1605	3660	655	1415	12180	5.00
	3	(20) 10d	(20) 10d	1605	3660	655	1415	16445	4.50
	4	(20) 10d	(20) 10d	1605	3660	655	1415	20705	4.25
TBE6	1	(20) 10d x 1 ½"	(20) 10d x 1 ½"	1760	3540	490	1745	10235	8.41
	2	(20) 10d	(20) 10d	1760	3860	490	1745	17250	7.09
	3	(20) 10d	(20) 10d	1760	3860	490	1745	23945	6.56
	4	(20) 10d	(20) 10d	1760	3860	490	1745	30640	6.29
S-P-F									
TBE4	1	(20) 10d x 1 ½"	(20) 10d x 1 ½"	1605	3220	615	1415	6445	6.99
	2	(20) 10d	(20) 10d	1605	3440	615	1415	9890	5.37
	3	(20) 10d	(20) 10d	1605	3440	615	1415	13120	4.74
	4	(20) 10d	(20) 10d	1605	3440	615	1415	16345	4.43
TBE6	1	(20) 10d x 1 ½"	(20) 10d x 1 ½"	1760	3220	490	1585	8290	8.99
	2	(20) 10d	(20) 10d	1760	3440	490	1585	13680	7.42
	3	(20) 10d	(20) 10d	1760	3440	490	1585	18750	6.78
	4	(20) 10d	(20) 10d	1760	3440	490	1585	23820	6.46

1. Factored resistances are for two TBEs only. Wood factored bearing resistance may be added as shown in the table.
2. Factored bearing resistances shown assume Q_v/A_b and Q_v/A_v = 812 psi (5.60 MPa) for D.Fir-L and 614 psi (4.24 MPa) for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 for required bearing reinforcement when compression loads are applied to both sides of truss member.
3. Use lower of top plate or wood truss species.
4. Total bearing length, TBL, equals the plate width plus simulated bearing length provided by the TBE. TBE4 = 3½" plate width; TBE6 = 5½" plate width.



This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECTBE17 3/17 exp. 6/19

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LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

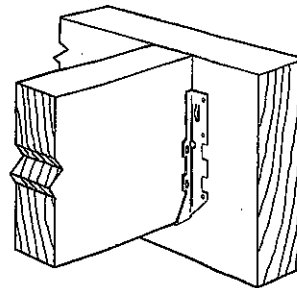
Finish: G90 galvanized

Design:

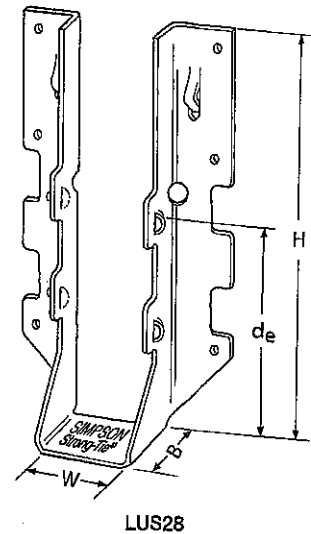
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



Typical LUS Installation



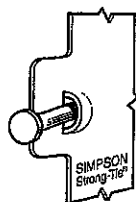
LUS28

Options:

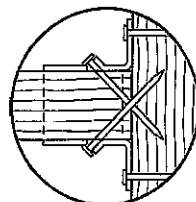
- These hangers cannot be modified

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	de	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1½	3½	1½	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4½	1½	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4½	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4½	4½	2	3½	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1½	3½	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4½	6¼	2	3½	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1½	3½	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4½	8½	2	5½	(8) 16d	(6) 16d	2580	3345	2320	2375

1. de is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC LUS17 3/17 exp. 6/19

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HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

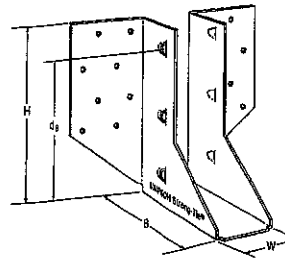
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

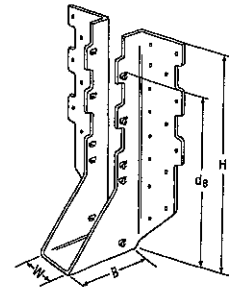
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

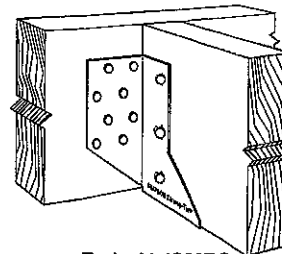
- See current catalogue for options



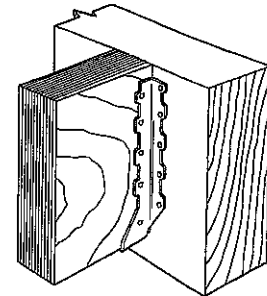
LJS26DS



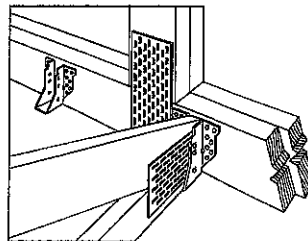
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



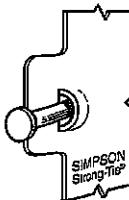
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

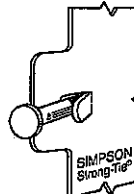
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3¼	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7¾	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

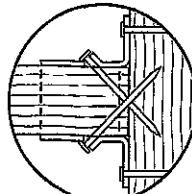


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHUS17 3/17 exp. 6/19

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HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

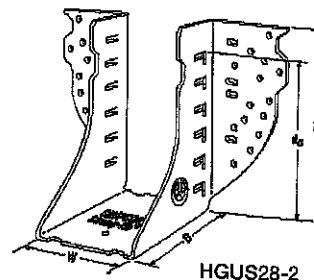
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

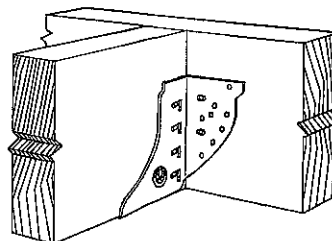
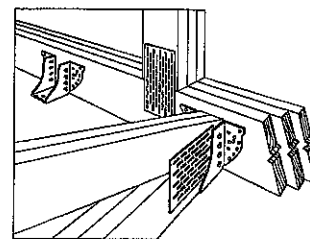
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options

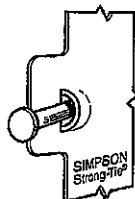


HGUS28-2

Typical HGUS
InstallationTypical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

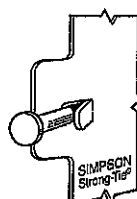
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 5/8	5 3/8	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/16	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/16	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_o is the distance from the seat of the hanger to the highest joist nail.

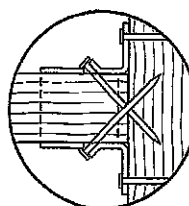


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC HGUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

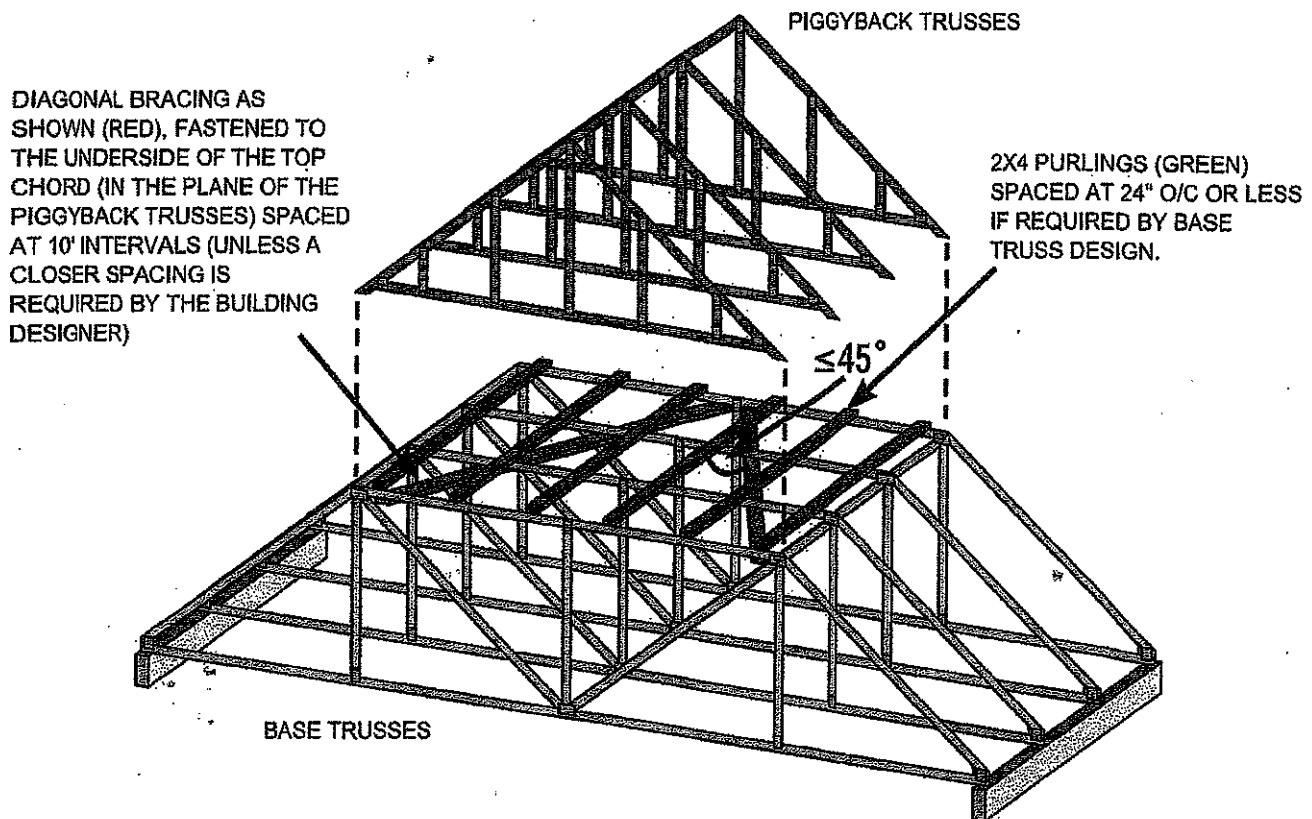
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Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

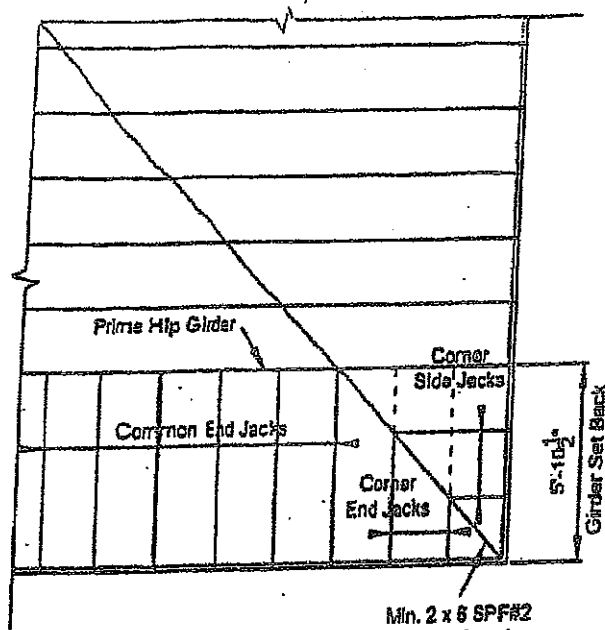
Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

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45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

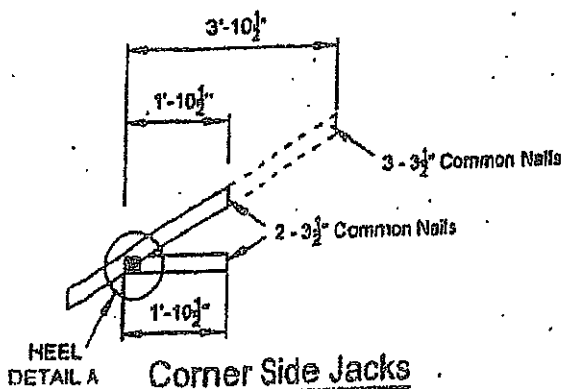
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

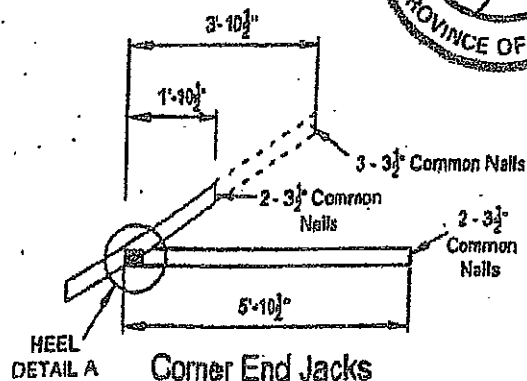
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

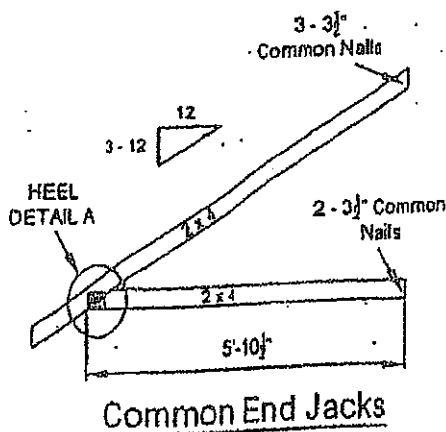
DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



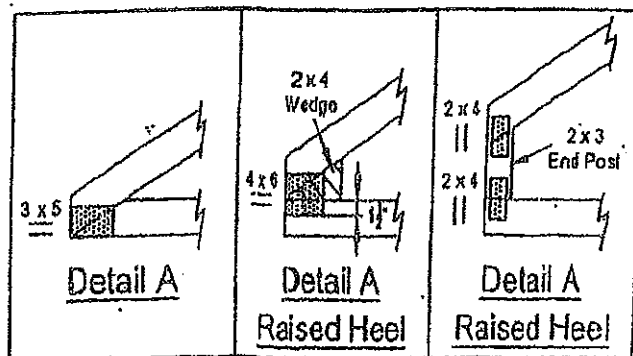
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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MICRO CITY ENGINEERING SERVICES INC.

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R.R. #1, P.O. BOX 81, GLENCOE, ONTARIO, N0L 1M0

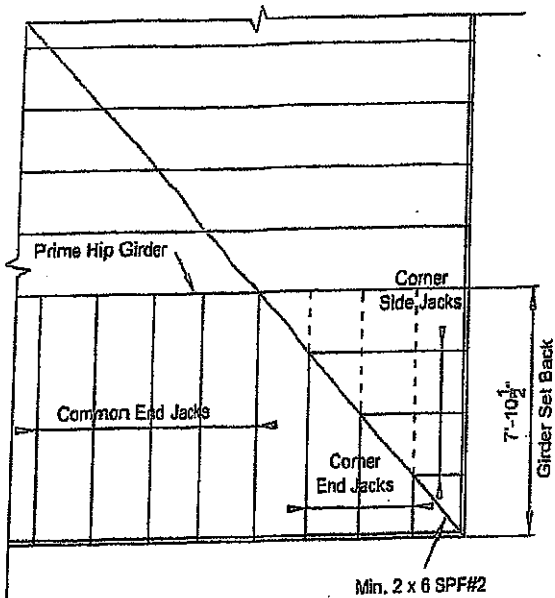
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

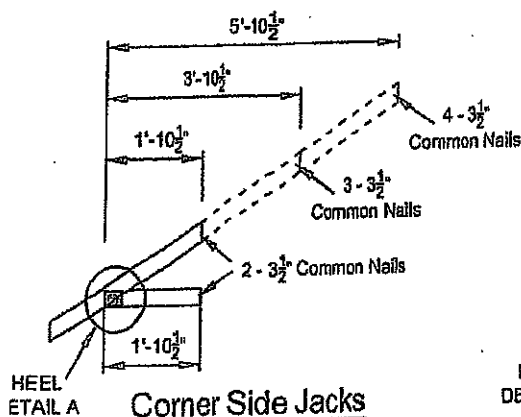
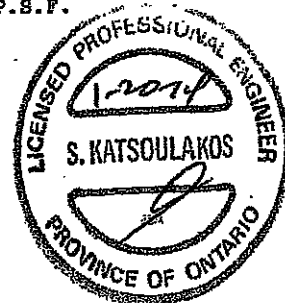
TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

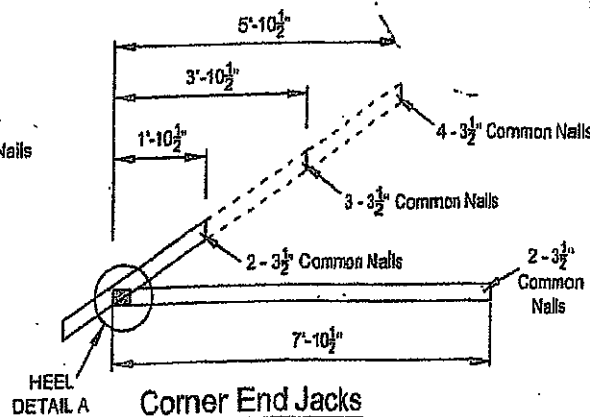


45° Hip End

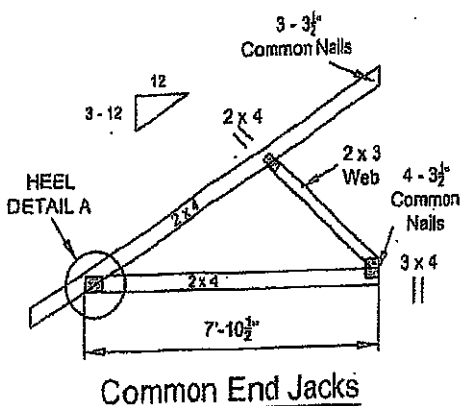
DWG NO TAM 3503 14
STRUCTURAL
COMPONENT ONLY



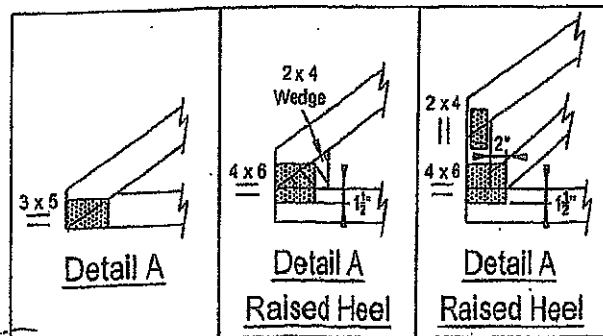
Corner Side Jacks



Corner End Jacks



Common End Jacks

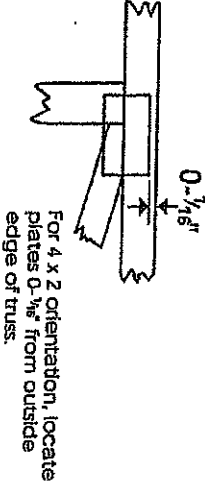


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

SITE COPY

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

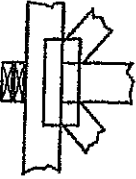
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use 1, 1 or Eliminator bracing if indicated.

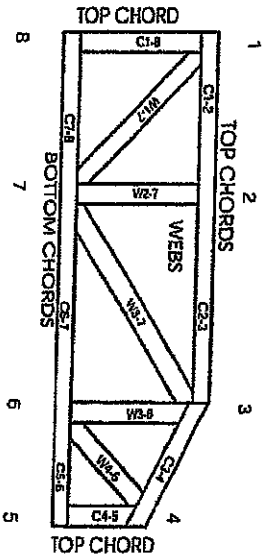
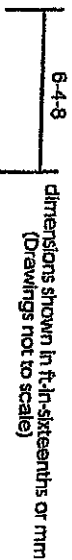
BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPI: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12891-R

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Mitek Engineering Reference Sheet Mtk-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative 1, 1 or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed rafter, knots and wane at joint locations are regulated by TPI/C.
7. Design assumes trusses will be suitably protected from the environment in accord with TPI/C.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPI/C Quality Criteria.

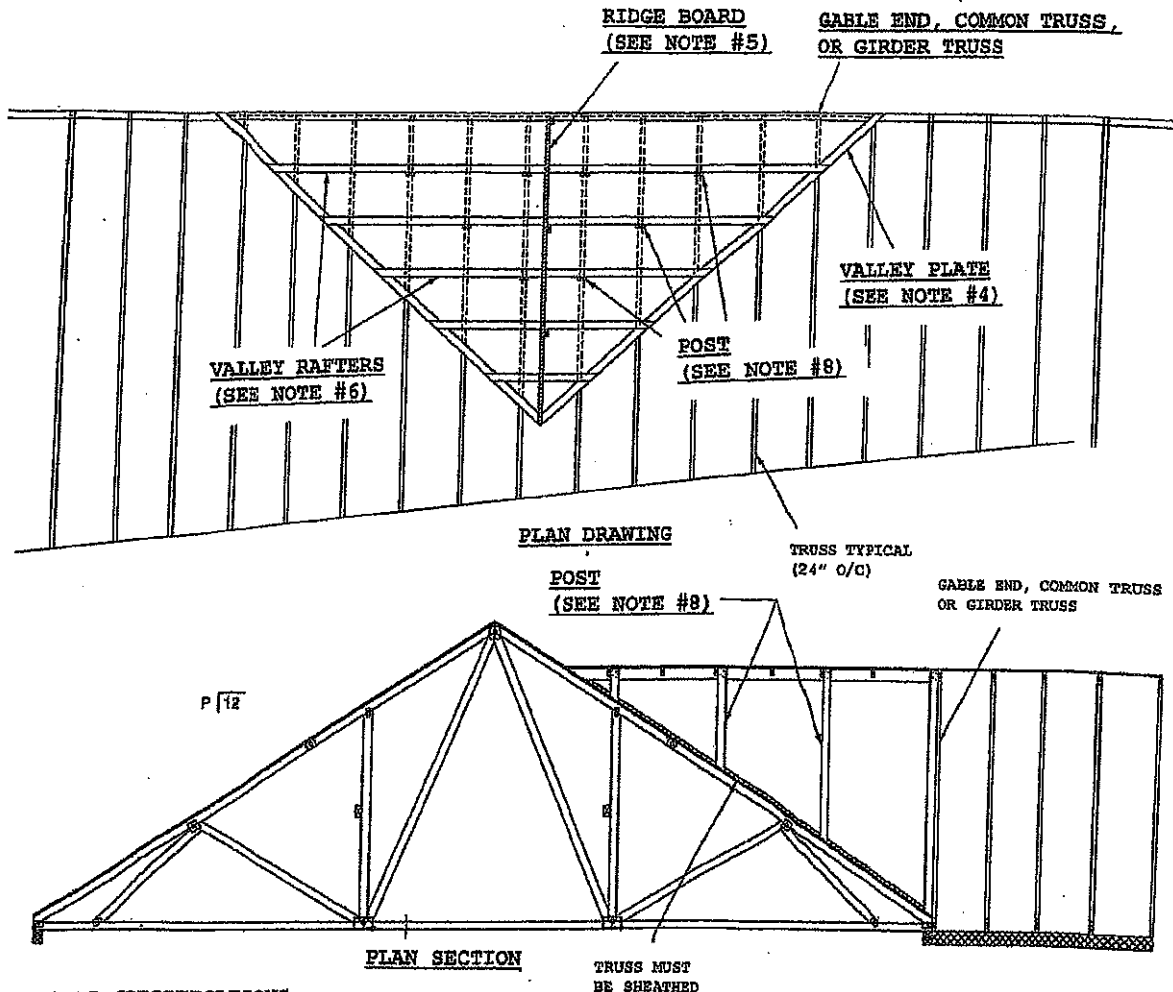
SITE COPY

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO TAM 6305.14
STRUCTURAL
COMPONENT ONLY

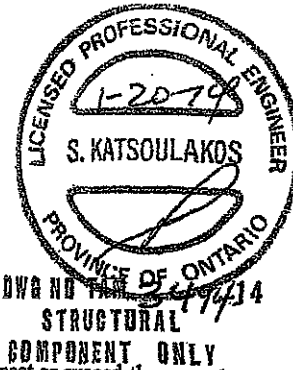
SITE COPY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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