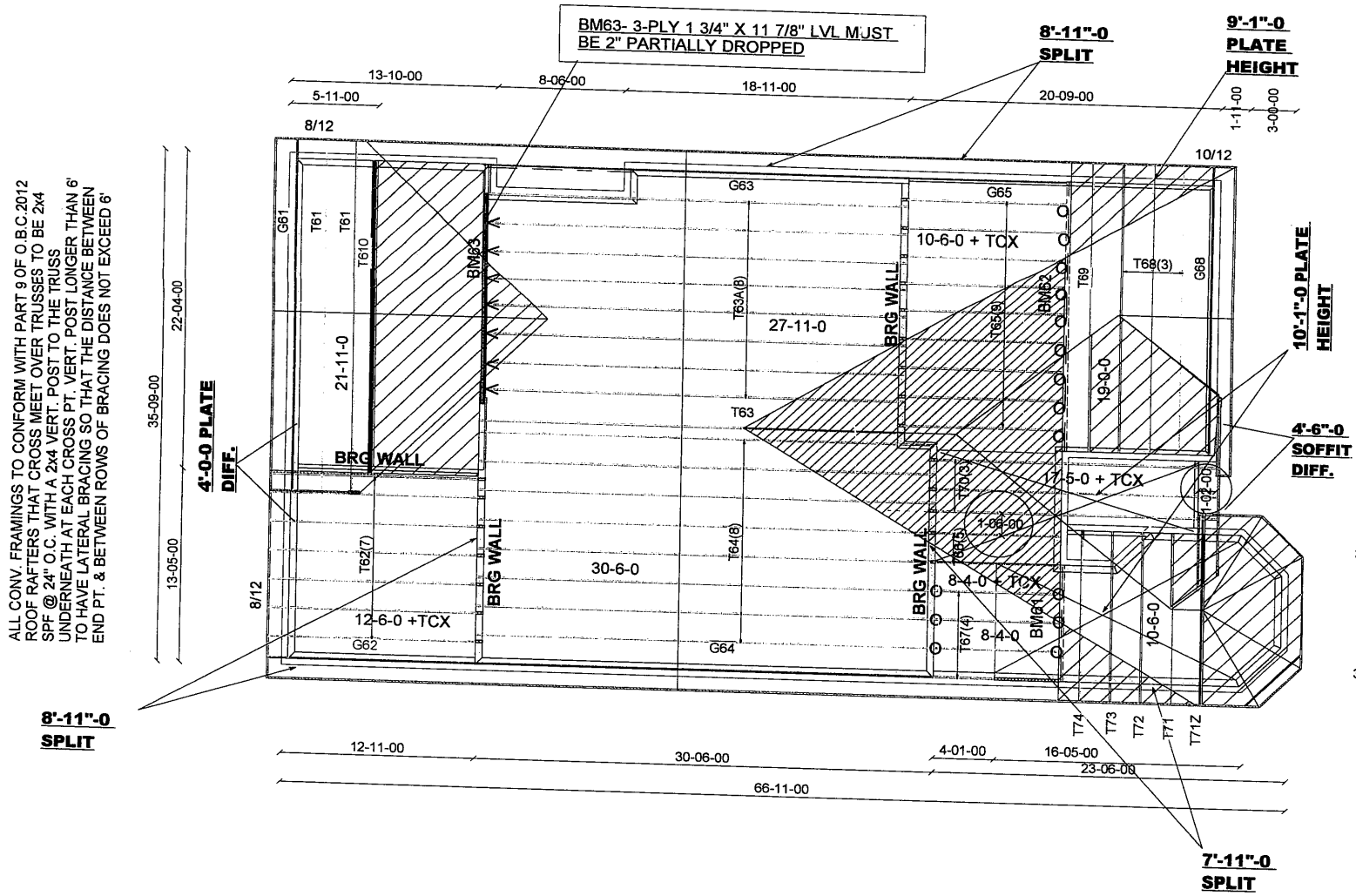


ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

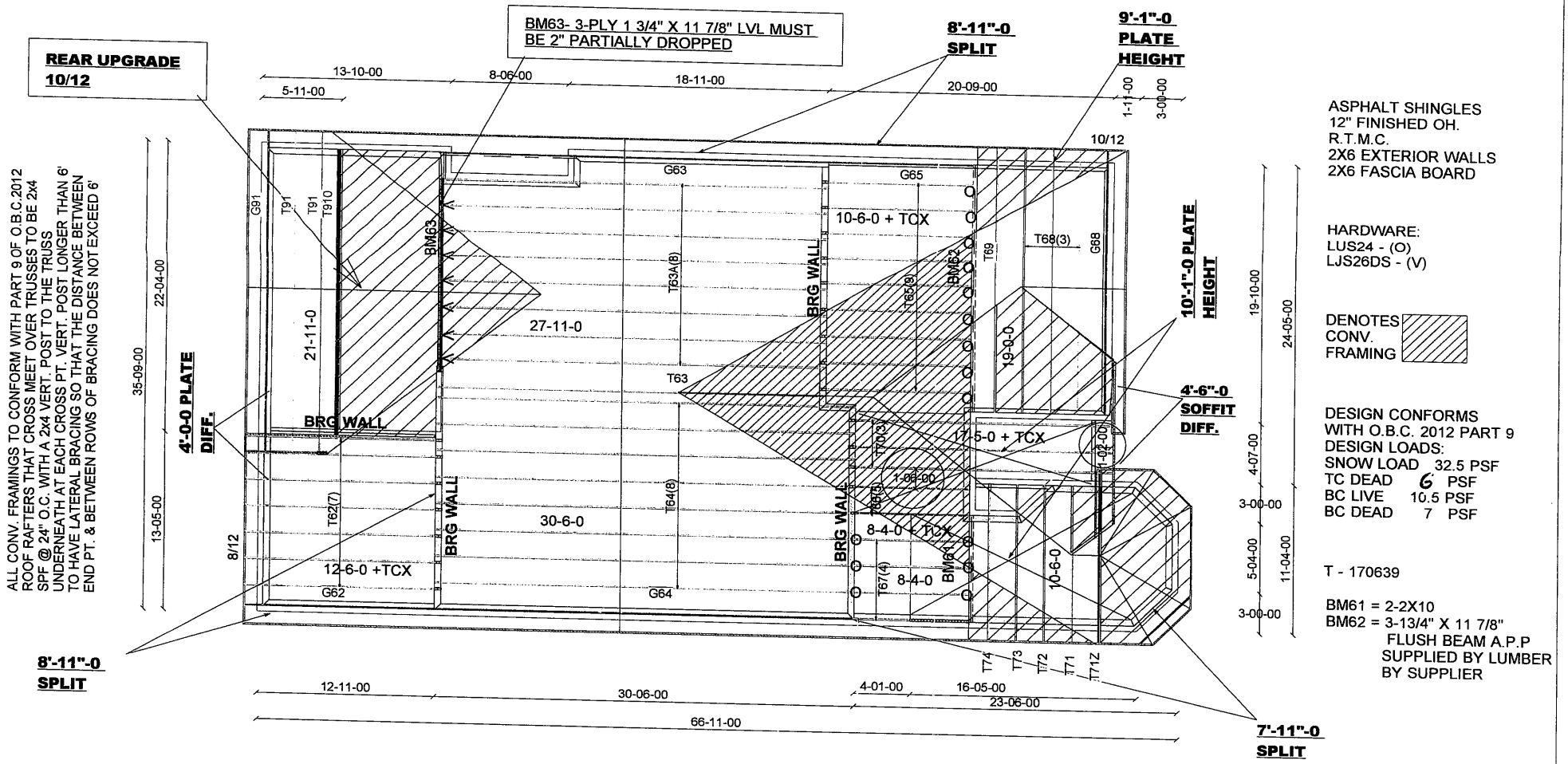
T - 170639

BM61 = 2-2X10
BM62 = 3-13/4" X 11 7/8" FLUSH BEAM A.P.P SUPPLIED BY LUMBER BY SUPPLIER

M11,568

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B-LOFT / A
	Layout ID: 287667	Project: GREEN VALLEY ESTATES	
	Plan Log: 94071	Date: 3/23/2018 Designer: AC/S.V.E	

Mitek ver 7.5.0



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM61 = 2-2X10
BM62 = 3-13/4" X 11 7/8" FLUSH BEAM A.P.P SUPPLIED BY LUMBER BY SUPPLIER

MU, 5108

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track:	44755
Layout ID:	289222
Plan Log:	94071

Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD

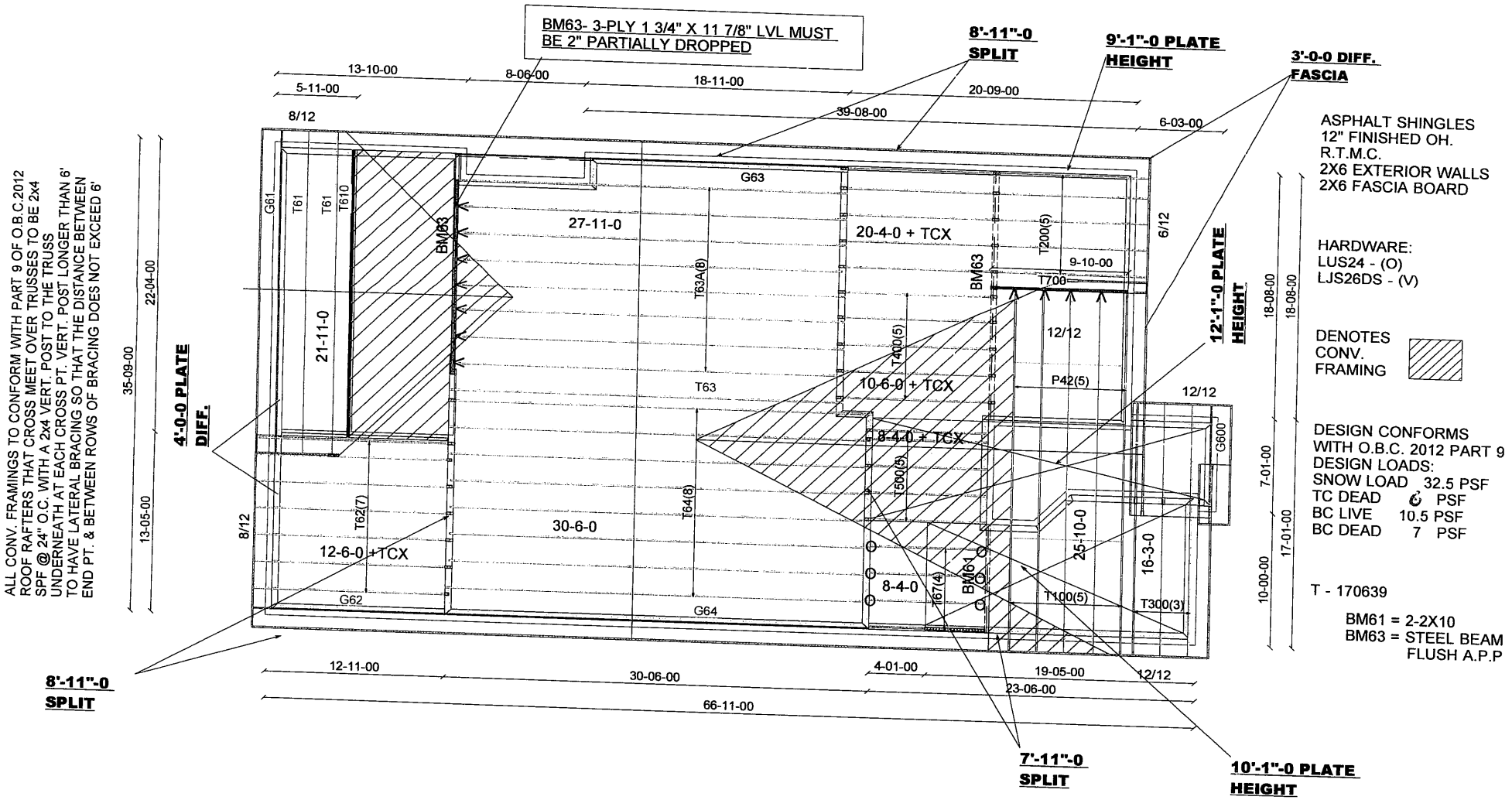
Project: GREEN VALLEY ESTATES

Date: 3/23/2018 Designer: AC/S.V.E

Model / Elevation: S42-1B-LOFT / A -REAR UPGRADE

Mitek ver 7.5.0

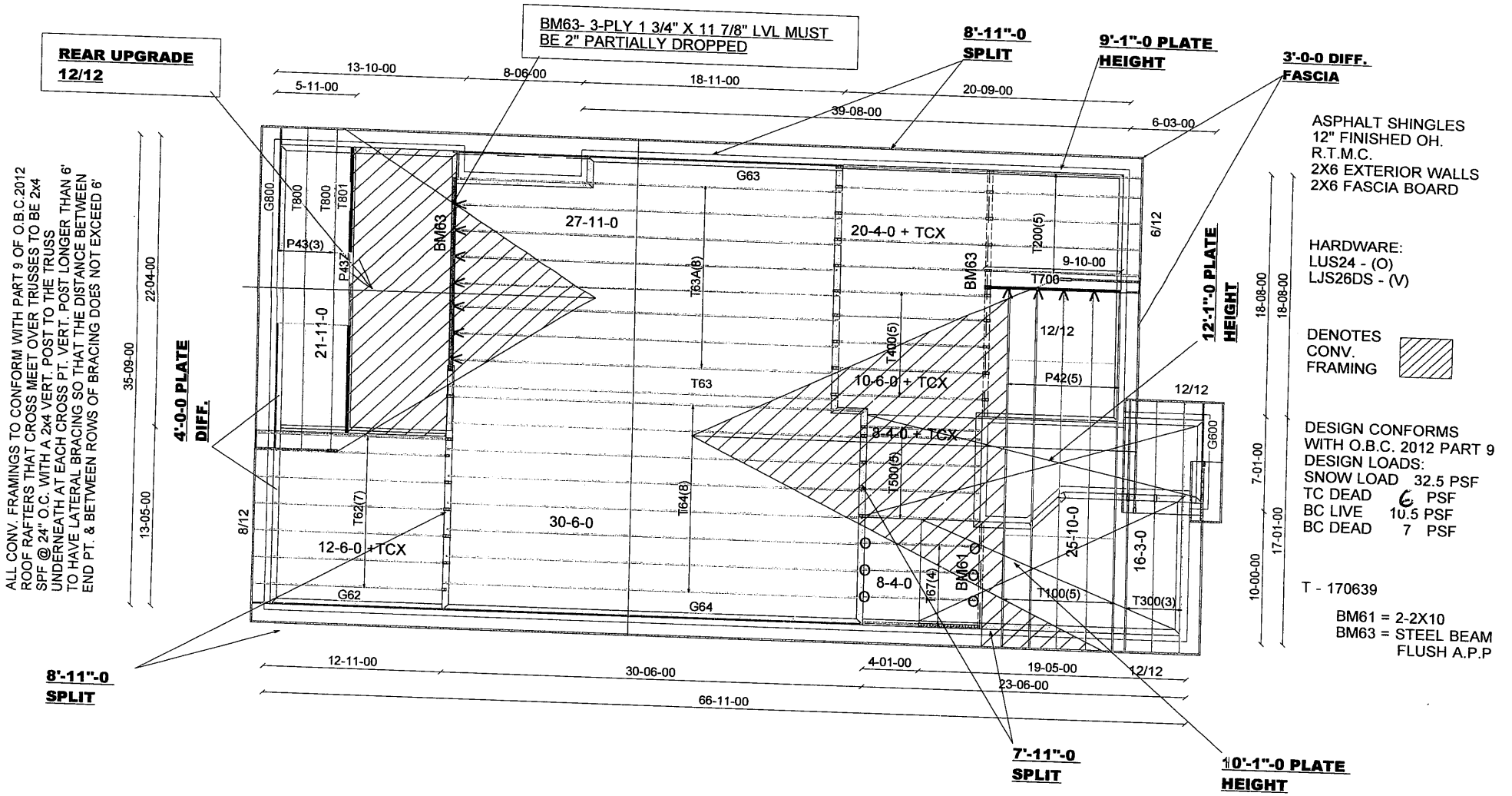
Site Copy



N11,5168

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B-LOFT / B
	Layout ID: 287668	Project: GREEN VALLEY ESTATES	
	Plan Log: 94071	Date: 3/23/2018 Designer: AC/S.V.E	

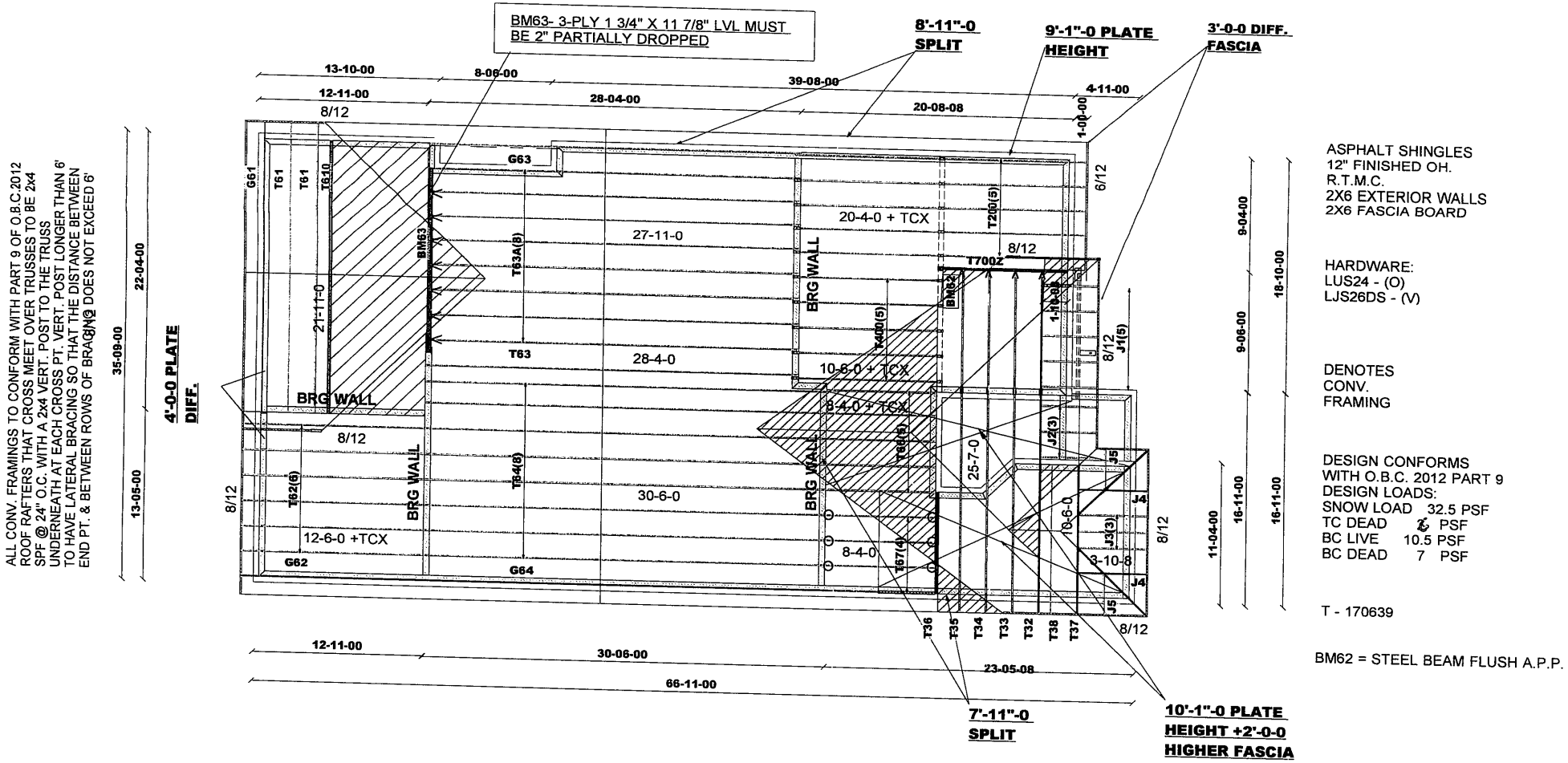
Mitek ver 7.5.0



N11, 5168

TAMARACK LUMBER INC. ASPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B-LOFT / B -REAR UPGRADE
	Layout ID: 289223	Project: GREEN VALLEY ESTATES	
	Plan Log: 94071	Date: 3/23/2018 Designer: AC/S.V.E	

Mitek ver 7.5.0



ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 8 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

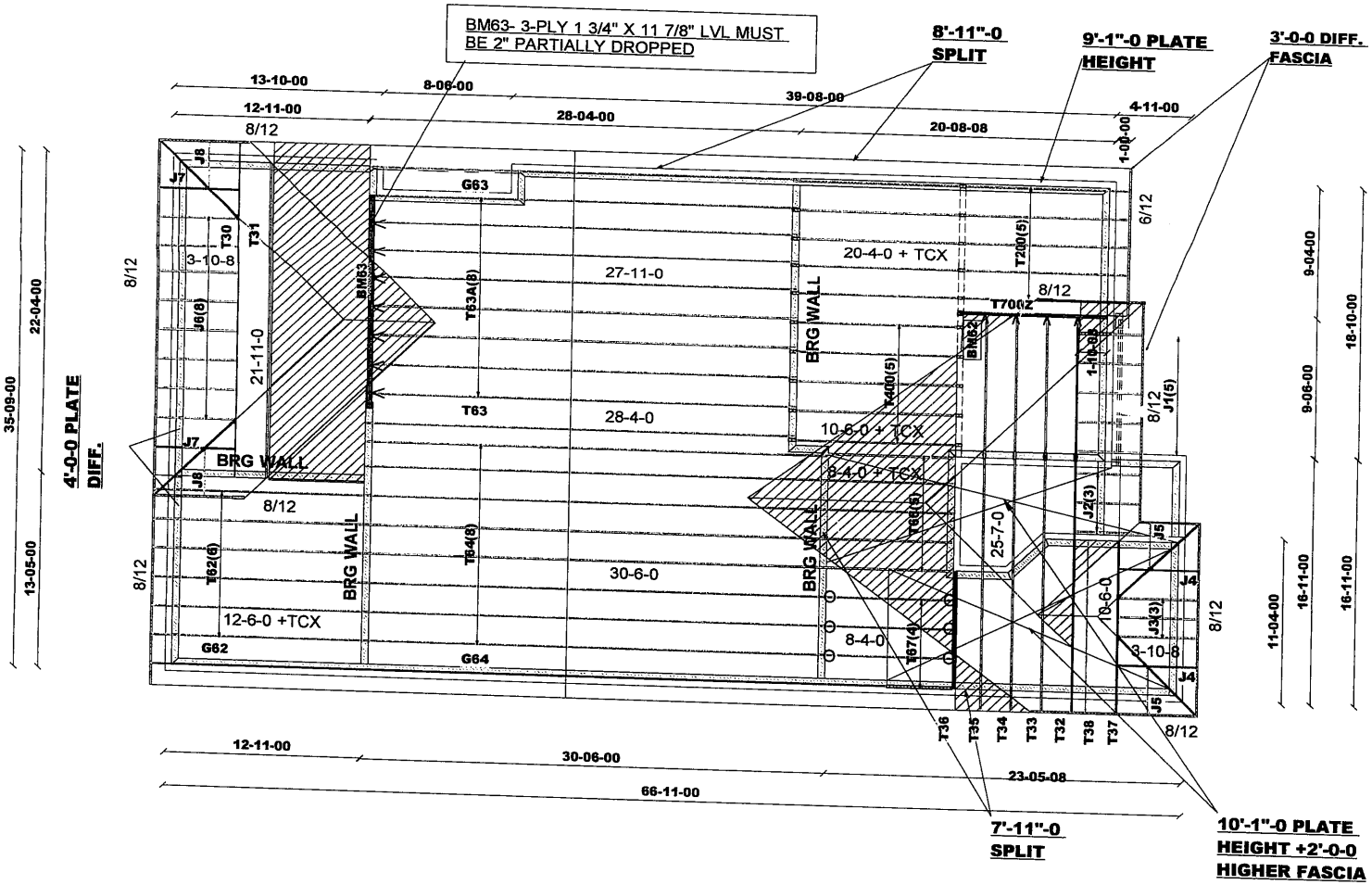
BM62 = STEEL BEAM FLUSH A.P.P.

N11,5108

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B LOFT / C
	Layout ID: 287648	Project: GREEN VALLEY EAST	
	Plan Log: 94071	Date: 3/26/2018 Designer: AC/S.V.E	

Mitek ver 7.5.0

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 10 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM62 = STEEL BEAM FLUSH A.P.P.

111508

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-1B LOFT / C REAR UPGRADE
	Layout ID: 289225	Project: GREEN VALLEY EAST	
	Plan Log: 94071	Date: 3/26/2018 Designer: AC/S.V.E	

Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287667 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B-LOFT ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	T64 ROOF	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	1001.92 622.64		
	1	G64 GABLE	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	138.90 89.83		
	1	T63 ROOF	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	115.44 70.83		
	1	G63 GABLE	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	133.43 85.50		
	8	T63A ROOF	6.00 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 01-01-03	903.36 562.64		
	1 2 Ply	T610 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 6	00-00-00 00-00-00	01-04-14 01-04-14	238.14 144.00		
	2	T61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	190.02 117.00		
	1	G61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	103.32 65.50		
	7	T62 MONOPITCH	8.00 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 10-00-08	476.56 299.81		
	1	G62 GABLE	8.00 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 09-08-14	67.76 42.83		
	3	T68 ROOF	10.00 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	273.33 175.50		
	1	G68 GABLE	10.00 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	98.26 63.50		
	1	T69 HALF HIP	10.00 0.00	19-00-00	10-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-08-14	113.79 71.33		
	3	T70 HALF HIP	8.00 0.00	17-10-08	11-02-11	2 X 4	2 X 4	01-03-08 00-00-00	04-10-14 11-02-11	405.99 254.01		
	4	T67 MONOPITCH	6.00 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 10-03-00	247.76 159.32		
	2	T66 MONOPITCH	6.00 0.00	08-07-00	09-04-08	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 09-04-08	110.26 72.00		
	9	T65 MONOPITCH	6.00 0.00	10-11-08	11-06-12	2 X 4	2 X 4	00-00-00 00-00-00	06-01-00 11-06-12	625.50 394.47		
	1	G65 GABLE	6.00 0.00	10-06-00	11-04-00	2 X 4	2 X 4	00-00-00 00-00-00	06-01-00 11-04-00	97.42 61.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287667 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B-LOFT ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T71 COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	52.11 / 34.00		
	1	T71Z COMMON	10.00 / 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	52.11 / 34.00		
	1	T72 HALF HIP	10.00 / 0.00	10-06-00	07-03-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 07-03-09	57.20 / 36.33		
	1	T73 HALF HIP	10.00 / 0.00	10-06-00	08-07-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-07-09	62.66 / 40.83		
	1	T74 MONOPITCH	10.00 / 0.00	10-06-00	09-11-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 09-11-12	56.28 / 35.50		

TOTAL # TRUSS= **61.00** TOTAL BFT OF ALL TRUSSES= **3532.70** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5621.52** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	
15	Hangers	LUS24	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **25.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289222 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	T64 ROOF	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	1001.92 622.64		
	1	G64 GABLE	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	138.90 89.83		
	1	T63 ROOF	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	115.44 70.83		
	1	G63 GABLE	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	133.43 85.50		
	8	T63A ROOF	6.00 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 01-01-03	903.36 562.64		
	2	T91 COMMON	10.00 0.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	222.40 137.34		
	1	G91 GABLE	10.00 0.00	21-11-00	10-09-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	120.67 77.33		
	1 2 Ply	T910 COMMON	10.00 0.00	21-11-00	10-09-04	2 X 4	2 X 6	00-00-00 00-00-00	01-07-11 01-07-11	259.02 158.66		
	7	T62 MONOPITCH	8.00 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 10-00-08	476.56 299.81		
	1	G62 GABLE	8.00 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 09-08-14	67.76 42.83		
	3	T68 ROOF	10.00 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	273.33 175.50		
	1	G68 GABLE	10.00 0.00	19-00-00	09-06-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	98.26 63.50		
	1	T69 HALF HIP	10.00 0.00	19-00-00	10-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 10-08-14	113.79 71.33		
	3	T70 HALF HIP	8.00 0.00	17-10-08	11-02-11	2 X 4	2 X 4	01-03-08 00-00-00	04-10-14 11-02-11	405.99 254.01		
	4	T67 MONOPITCH	6.00 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 10-03-00	247.76 159.32		
	2	T66 MONOPITCH	6.00 0.00	08-07-00	09-04-08	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 09-04-08	110.26 72.00		
	9	T65 MONOPITCH	6.00 0.00	10-11-08	11-06-12	2 X 4	2 X 4	00-00-00 00-00-00	06-01-00 11-06-12	674.55 421.47		
	1	G65 GABLE	6.00 0.00	10-06-00	11-04-00	2 X 4	2 X 4	00-00-00 00-00-00	06-01-00 11-04-00	97.42 61.33		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 289222 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B LOFT **ELEVATION:** A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T71 COMMON	10.00 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	52.11 34.00		
	1	T71Z COMMON	10.00 0.00	10-06-00	06-00-03	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	52.11 34.00		
	1	T72 HALF HIP	10.00 0.00	10-06-00	07-03-09	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 07-03-09	57.20 36.33		
	1	T73 HALF HIP	10.00 0.00	10-06-00	08-07-09	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 08-07-09	62.66 40.83		
	1	T74 MONOPITCH	10.00 0.00	10-06-00	09-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 09-11-12	56.28 35.50		

TOTAL # TRUSS= **61.00** TOTAL BFT OF ALL TRUSSES= **3606.53** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5741.18** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
7	Hangers	LJS26DS	
15	Hangers	LUS24	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **25.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287668 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: 42-1B-LOFT ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	8	T64 ROOF	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	1001.92 622.64		
	1	G64 GABLE	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	138.90 89.83		
	1	T63 ROOF	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	115.44 70.83		
	1	G63 GABLE	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	133.43 85.50		
	8	T63A ROOF	6.00 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 01-01-03	903.36 562.64		
	2	T61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	190.02 117.00		
	1	G61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	103.32 65.50		
	1 2 Ply	T610 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 6	00-00-00 00-00-00	01-04-14 01-04-14	238.14 144.00		
	7	T62 MONOPITCH	8.00 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 10-00-08	476.56 299.81		
	1	G62 GABLE	8.00 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 09-08-14	67.76 42.83		
	5	T100 PIGGYBACK	12.00 0.00	25-10-00	10-00-00	2 X 4	2 X 4	01-03-08 00-10-08	01-10-08 04-05-08	807.80 515.00		
	5	T200 MONO TRUSS	6.00 0.00	20-09-08	11-06-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 11-06-12	669.50 414.15		
	3	T300 ROOF	12.00 0.00	16-03-00	11-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	287.64 183.51		
	5	T400 MONOPITCH	6.00 0.00	11-05-00	11-06-12	2 X 4	2 X 4	00-00-00 00-00-00	05-10-04 11-06-12	445.00 273.35		
	5	T500 MONOPITCH	6.00 0.00	08-09-08	07-05-12	2 X 4	2 X 4	00-00-00 00-00-00	03-01-00 07-05-12	237.00 151.65		
	1	G600 COMMON	12.00 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	35.50 24.17		
	1 2 Ply	T700 MONOPITCH	6.00 0.00	09-10-00	06-01-00	2 X 4	2 X 4	00-00-00 00-00-00	01-02-00 06-01-00	100.44 64.66		
	4	T67 MONOPITCH	6.00 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 10-03-00	247.76 159.32		

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287668 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: 42-1B-LOFT ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	P42 PIGGYBACK	12.00 0.00	12-00-10	06-05-08	2 X 4	2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	230.90 145.00		

TOTAL # TRUSS= **67.00** TOTAL BFT OF ALL TRUSSES= **4031.39** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6430.39** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
6	Hangers	LUS24	
11	Hangers	LJS26DS	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **20.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289223 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	8	T64 ROOF	6.00 / 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 00-09-14	1001.92 / 622.64		
	1	G64 GABLE	6.00 / 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 00-09-14	138.90 / 89.83		
	1	T63 ROOF	6.00 / 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	02-05-00 / 00-09-14	115.44 / 70.83		
	1	G63 GABLE	6.00 / 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	02-05-00 / 00-09-14	133.43 / 85.50		
	8	T63A ROOF	6.00 / 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 / 00-00-00	02-05-00 / 01-01-03	903.36 / 562.64		
	2	T800 PIGGYBACK	12.00 / 0.00	21-11-00	10-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	229.78 / 142.66		
	1	G800 GABLE	12.00 / 0.00	21-11-00	10-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	127.82 / 81.67		
	1 / 2 Ply	T801 PIGGYBACK	12.00 / 0.00	21-11-00	10-00-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-10-08 / 01-10-08	300.08 / 182.68		
	7	T62 MONOPITCH	8.00 / 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 10-00-08	476.56 / 299.81		
	1	G62 GABLE	8.00 / 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-14 / 09-08-14	67.76 / 42.83		
	5	T100 PIGGYBACK	12.00 / 0.00	25-10-00	10-00-00	2 X 4	2 X 4	01-03-08 / 00-10-08	01-10-08 / 04-05-08	807.80 / 515.00		
	5	T200 MONO TRUSS	6.00 / 0.00	20-09-08	11-06-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 11-06-12	669.50 / 414.15		
	3	T300 ROOF	12.00 / 0.00	16-03-00	11-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	287.64 / 183.51		
	5	T400 MONOPITCH	6.00 / 0.00	11-05-00	11-06-12	2 X 4	2 X 4	00-00-00 / 00-00-00	05-10-04 / 11-06-12	445.00 / 273.35		
	5	T500 MONOPITCH	6.00 / 0.00	08-09-08	07-05-12	2 X 4	2 X 4	00-00-00 / 00-00-00	03-01-00 / 07-05-12	237.00 / 151.65		
	1	G600 COMMON	12.00 / 0.00	06-03-00	05-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-10-08 / 01-10-08	35.50 / 24.17		
	1 / 2 Ply	T700 MONOPITCH	6.00 / 0.00	09-10-00	06-01-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-02-00 / 06-01-00	100.44 / 64.66		
	4	T67 MONOPITCH	6.00 / 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 / 00-00-00	05-01-00 / 10-03-00	247.76 / 159.32		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/23/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 289223 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-1B LOFT **ELEVATION:** B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	P42 PIGGYBACK	12.00 / 0.00	12-00-10	06-05-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	230.90 / 145.00		
	3	P43 PIGGYBACK	12.00 / 0.00	04-06-10	02-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	47.88 / 34.50		
	1 / 2 Ply	P43Z PIGGYBACK	12.00 / 0.00	04-06-10	02-08-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-05-03 / 00-05-03	31.92 / 23.00		

TOTAL # TRUSS= **72.00** TOTAL BFT OF ALL TRUSSES= **4169.40** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6636.39** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
6	Hangers	LUS24	
11	Hangers	LJS26DS	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **20.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

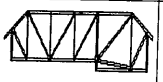
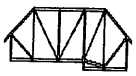
Delivery Shiplist

DATE	03/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287648 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	T64 ROOF	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	1001.92 622.64		
	1	G64 GABLE	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	138.90 89.83		
	1	T63 ROOF	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	115.44 70.83		
	1	G63 GABLE	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	133.43 85.50		
	8	T63A ROOF	6.00 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 01-01-03	903.36 562.64		
	2	T61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	190.02 117.00		
	1	G61 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	103.32 65.50		
	1 2 Ply	T610 COMMON	8.00 0.00	21-11-00	08-08-09	2 X 4	2 X 6	00-00-00 00-00-00	01-04-14 01-04-14	238.14 144.00		
	6	T62 MONOPITCH	8.00 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 10-00-08	408.48 256.98		
	1	G62 GABLE	8.00 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 09-08-14	67.76 42.83		
	5	T200 MONO TRUSS	6.00 0.00	20-09-08	11-06-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 11-06-12	669.50 414.15		
	1	T32 HIP GIRDER	8.00 0.00	25-07-00	06-03-14	2 X 4	2 X 4	01-03-08 00-10-08	03-04-14 04-03-08	129.47 80.84		
	1	T33 HIP	8.00 0.00	25-07-00	07-07-14	2 X 4	2 X 4	01-03-08 00-10-08	03-04-14 04-03-08	128.88 82.50		
	1	T34 HIP	8.00 0.00	25-07-00	08-11-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 04-03-08	144.00 90.33		
	1	T35 HIP	8.00 0.00	25-07-00	10-03-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 04-03-08	142.36 89.67		
	1 2 Ply	T36 MONOPITCH	8.00 0.00	08-01-00	08-09-09	2 X 4	2 X 6	00-00-00 00-00-00	03-04-14 08-09-09	102.92 64.00		
	1	T37 HIP GIRDER	8.00 0.00	10-06-00	05-11-14	2 X 4	2 X 4	01-03-08 01-03-08	03-04-14 03-04-14	61.84 39.67		
	1	T38 COMMON	8.00 0.00	10-06-00	06-10-14	2 X 4	2 X 4	01-03-08 01-03-08	03-04-14 03-04-14	51.46 32.83		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287648 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: C

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T66 MONOPITCH	6.00 0.00	08-07-00	09-04-08	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 09-04-08	275.65 180.00		
	4	T67 MONOPITCH	6.00 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 10-03-00	247.76 159.32		
	5	T400 MONOPITCH	6.00 0.00	11-05-00	11-06-12	2 X 4	2 X 4	00-00-00 00-00-00	05-10-04 11-06-12	445.00 273.35		
	1 2 Ply	T700Z MONOPITCH	6.00 0.00	09-10-00	06-01-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-01-00	115.54 72.66		
	5	J1 JACK-OPEN	8.00 0.00	01-10-08	06-03-14	2 X 4	2 X 4	02-03-08 00-00-00	05-00-14 06-03-14	66.75 43.35		
	3	J2 JACK-OPEN	8.00 0.00	01-10-08	05-03-14	2 X 4	2 X 4	02-03-08 00-00-00	04-00-14 05-03-14	36.60 24.00		
	3	J3 JACK-OPEN	8.00 0.00	03-10-08	05-11-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 05-11-14	45.30 27.99		
	2	J4 JACK-OPEN	8.00 0.00	01-09-07	04-07-03	2 X 4	2 X 4	01-03-08 02-01-01	03-04-14 00-03-08	24.46 14.66		
	2	J5 JACK-OPEN	8.00 0.00	01-09-07	04-07-03	2 X 4	2 X 4	01-03-08 00-01-01	03-04-14 00-03-08	19.90 12.00		

TOTAL # TRUSS= **75.00** TOTAL BFT OF ALL TRUSSES= **3759.07** BFT. TOTAL WEIGHT OF ALL TRUSSES= **6008.16** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
11	Hangers	LJS26DS	
6	Hangers	LUS24	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **20.00**

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

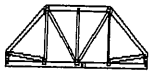
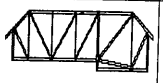
Delivery Shiplist

DATE	03/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289225 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: C REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	8	T64 ROOF	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	1001.92 622.64		
	1	G64 GABLE	6.00 0.00	30-06-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 00-09-14	138.90 89.83		
	1	T63 ROOF	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	115.44 70.83		
	1	G63 GABLE	6.00 0.00	28-04-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 00-09-14	133.43 85.50		
	8	T63A ROOF	6.00 0.00	27-11-00	09-09-15	2 X 4	2 X 4	00-00-00 00-00-00	02-05-00 01-01-03	903.36 562.64		
	1	T30 HIP GIRDER	8.00 0.00	21-11-00	03-11-14	2 X 4	2 X 4	01-03-08 01-03-08	01-04-14 01-04-14	90.83 58.34		
	1 2 Ply	T31 HIP	8.00 0.00	21-11-00	05-04-09	2 X 6	2 X 6	00-00-00 00-00-00	01-04-14 01-04-14	257.88 154.66		
	6	T62 MONOPITCH	8.00 0.00	12-11-08	10-00-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 10-00-08	408.48 256.98		
	1	G62 GABLE	8.00 0.00	12-06-00	09-08-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 09-08-14	67.76 42.83		
	5	T200 MONO TRUSS	6.00 0.00	20-09-08	11-06-12	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 11-06-12	669.50 414.15		
	1	T32 HIP GIRDER	8.00 0.00	25-07-00	06-03-14	2 X 4	2 X 4	01-03-08 00-10-08	03-04-14 04-03-08	129.47 80.84		
	1	T33 HIP	8.00 0.00	25-07-00	07-07-14	2 X 4	2 X 4	01-03-08 00-10-08	03-04-14 04-03-08	128.88 82.50		
	1	T34 HIP	8.00 0.00	25-07-00	08-11-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 04-03-08	144.00 90.33		
	1	T35 HIP	8.00 0.00	25-07-00	10-03-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 04-03-08	142.36 89.67		
	1 2 Ply	T36 MONOPITCH	8.00 0.00	08-01-00	08-09-09	2 X 4	2 X 6	00-00-00 00-00-00	03-04-14 08-09-09	102.92 64.00		
	1	T37 HIP GIRDER	8.00 0.00	10-06-00	05-11-14	2 X 4	2 X 4	01-03-08 01-03-08	03-04-14 03-04-14	61.84 39.67		
	1	T38 COMMON	8.00 0.00	10-06-00	06-10-14	2 X 4	2 X 4	01-03-08 01-03-08	03-04-14 03-04-14	51.46 32.83		
	5	T66 MONOPITCH	6.00 0.00	08-07-00	09-04-08	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 09-04-08	275.65 180.00		

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/26/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 289225 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-1B LOFT ELEVATION: C REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	4	T67 MONOPITCH	6.00 0.00	08-04-00	10-03-00	2 X 4	2 X 4	00-00-00 00-00-00	05-01-00 10-03-00	247.76 159.32		
	5	T400 MONOPITCH	6.00 0.00	11-05-00	11-06-12	2 X 4	2 X 4	00-00-00 00-00-00	05-10-04 11-06-12	445.00 273.35		
	1 2 Ply	T700Z MONOPITCH	6.00 0.00	09-10-00	06-01-00	2 X 4	2 X 6	00-00-00 00-00-00	01-02-00 06-01-00	115.54 72.66		
	5	J1 JACK-OPEN	8.00 0.00	01-10-08	06-03-14	2 X 4	2 X 4	02-03-08 00-00-00	05-00-14 06-03-14	66.75 43.35		
	3	J2 JACK-OPEN	8.00 0.00	01-10-08	05-03-14	2 X 4	2 X 4	02-03-08 00-00-00	04-00-14 05-03-14	36.60 24.00		
	3	J3 JACK-OPEN	8.00 0.00	03-10-08	05-11-14	2 X 4	2 X 4	01-03-08 00-00-00	03-04-14 05-11-14	45.30 27.99		
	2	J4 JACK-OPEN	8.00 0.00	01-09-07	04-07-03	2 X 4	2 X 4	01-03-08 02-01-01	03-04-14 00-03-08	24.46 14.66		
	2	J5 JACK-OPEN	8.00 0.00	01-09-07	04-07-03	2 X 4	2 X 4	01-03-08 00-01-01	03-04-14 00-03-08	19.90 12.00		
	8	J6 JACK-OPEN	8.00 0.00	03-10-08	03-11-14	2 X 4	2 X 4	01-03-08 00-00-00	01-04-14 03-11-14	102.56 64.00		
	2	J7 JACK-OPEN	8.00 0.00	01-09-07	02-07-03	2 X 4	2 X 4	01-03-08 02-01-01	01-04-14 00-03-08	19.90 12.00		
	2	J8 JACK-OPEN	8.00 0.00	01-09-07	02-07-03	2 X 4	2 X 4	01-03-08 00-01-01	01-04-14 00-03-08	15.34 9.34		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3730.91** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5963.19** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
11	Hangers	LJS26DS	
6	Hangers	LUS24	
3	Beam	1 3/4" x 11 7/8" LVL	16-00-00

TOTAL # ITEMS= **20.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T30	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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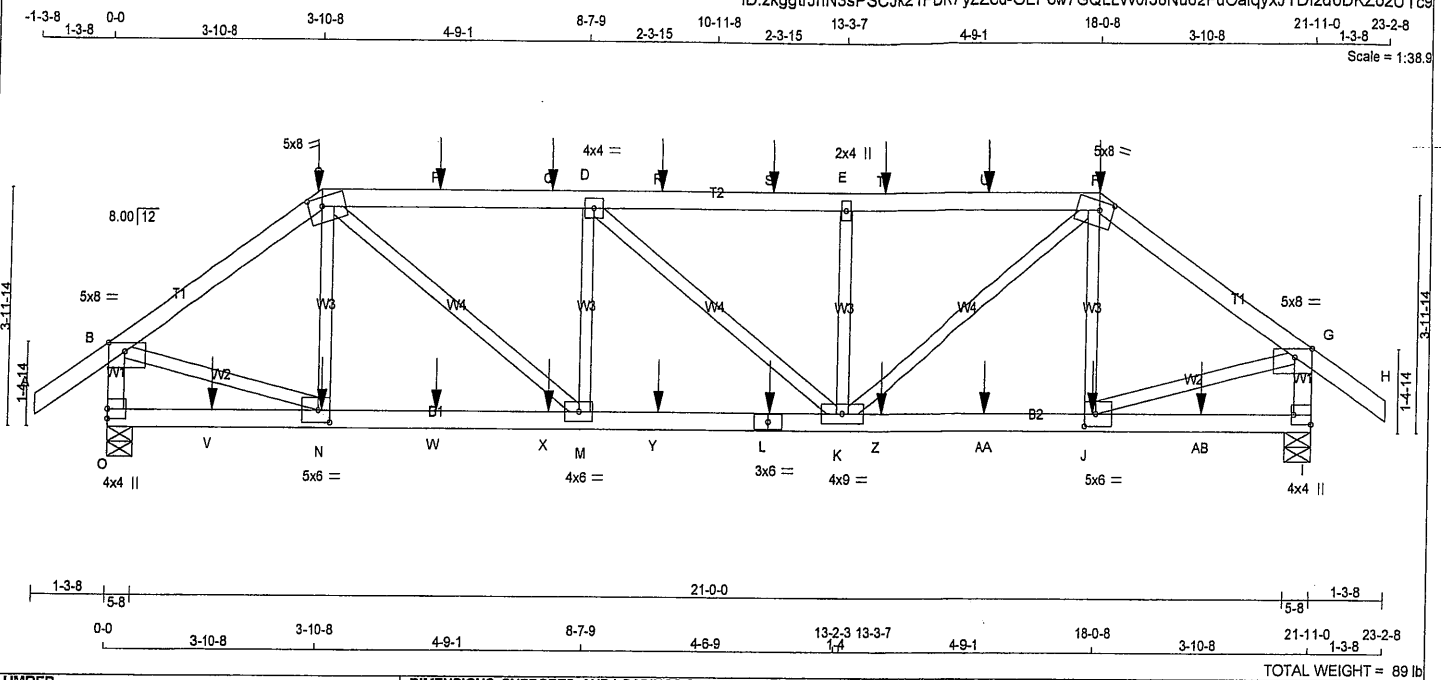

TOTAL WEIGHT = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TTWW-m	MT20	5.0	8.0	1.75	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	1.75	3.00
G	TMVW-p	MT20	5.0	8.0	1.75	3.50
I	BMV1+p	MT20	4.0	4.0	2.00	Edge
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 306.1 lbs FACTORED DOWN AT 3-10-8, 51.1 lbs FACTORED DOWN AT 5-11-12, 51.1 lbs FACTORED DOWN AT 7-11-12, 51.1 lbs FACTORED DOWN AT 9-11-12, 51.1 lbs FACTORED DOWN AT 11-11-12, 51.1 lbs FACTORED DOWN AT 13-11-12, AND 51.1 lbs FACTORED DOWN AT 15-11-12, AND 306.1 lbs FACTORED DOWN AT 18-0-8 ON TOP CHORD, AND 35.5 lbs FACTORED DOWN AT 1-11-4, 33.0 lbs FACTORED DOWN AT 3-11-4, 33.0 lbs FACTORED DOWN AT 5-11-12, 33.0 lbs FACTORED DOWN AT 7-11-12, 33.0 lbs FACTORED DOWN AT 9-11-12, 33.0 lbs FACTORED DOWN AT 11-11-12, 33.0 lbs FACTORED DOWN AT 13-11-12, 33.0 lbs FACTORED DOWN AT 15-11-12, AND 33.0 lbs FACTORED DOWN AT 17-11-12, AND 35.5 lbs FACTORED DOWN AT 19-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2242	0	2242	0	0	5-8	5-8
I	2243	0	2243	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1768	1057 / 0	309 / 0	0 / 0	0 / 0	403 / 0	0 / 0
I	1768	1057 / 0	309 / 0	0 / 0	0 / 0	403 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
B- C	-2325 / 0	-112.4	-112.4	0.43 (1)	4.00	
C- P	-3043 / 0	-112.4	-112.4	0.64 (1)	3.28	
P- Q	-3043 / 0	-112.4	-112.4	0.64 (1)	3.28	
Q- D	-3043 / 0	-112.4	-112.4	0.64 (1)	3.28	
D- R	-3041 / 0	-112.4	-112.4	0.63 (1)	3.28	
R- S	-3041 / 0	-112.4	-112.4	0.63 (1)	3.28	
S- E	-3041 / 0	-112.4	-112.4	0.63 (1)	3.28	
E- T	-3041 / 0	-112.4	-112.4	0.63 (1)	3.29	
T- U	-3041 / 0	-112.4	-112.4	0.63 (1)	3.29	
U- F	-3041 / 0	-112.4	-112.4	0.63 (1)	3.29	
F- G	-2327 / 0	-112.4	-112.4	0.43 (1)	4.00	
G- H	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
O- B	-2194 / 0	0.0	0.0	0.25 (1)	5.68	
I- G	-2195 / 0	0.0	0.0	0.25 (1)	5.68	
O- V	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
V- N	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
N- W	0 / 1922	-28.0	-28.0	0.43 (1)	10.00	
W- X	0 / 1922	-28.0	-28.0	0.43 (1)	10.00	
X- M	0 / 1922	-28.0	-28.0	0.43 (1)	10.00	
M- Y	0 / 3044	-28.0	-28.0	0.62 (1)	10.00	
Y- L	0 / 3044	-28.0	-28.0	0.62 (1)	10.00	
L- K	0 / 3044	-28.0	-28.0	0.62 (1)	10.00	
K- Z	0 / 1923	-28.0	-28.0	0.43 (1)	10.00	
Z-AA	0 / 1923	-28.0	-28.0	0.43 (1)	10.00	
AA- J	0 / 1923	-28.0	-28.0	0.43 (1)	10.00	
J-AB	0 / 0	-28.0	-28.0	0.18 (3)	10.00	
AB- I	0 / 0	-28.0	-28.0	0.18 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-350 / 75	0.09 (1)
M- D	0 / 1419	0.35 (1)
K- E	-696 / 0	0.18 (1)
K- F	-695 / 0	0.18 (1)
J- F	0 / 1415	0.35 (1)
B- N	-348 / 75	0.09 (1)
J- G	0 / 2002	0.50 (1)
D- K	0 / 2003	0.50 (1)
	-3 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-306	-306	---	FRONT	VERT	TOTAL
F	18-0-8	-306	-306	---	FRONT	VERT	TOTAL
J	17-11-12	-19	-33	---	FRONT	VERT	TOTAL
L	11-11-12	-19	-33	---	FRONT	VERT	TOTAL
N	3-11-4	-19	-33	---	FRONT	VERT	TOTAL
P	5-11-12	-51	-51	---	FRONT	VERT	TOTAL
Q	7-11-12	-51	-51	---	FRONT	VERT	TOTAL
R	9-11-12	-51	-51	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.64/1.00 (C-D:1) , BC=0.62/1.00 (K-M:1) , WB=0.50/1.00 (G-J:1) , SSI=0.34/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.84 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS 4318 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM17334-19
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T30	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	11-11-12	-51	-51	—	FRONT	VERT	TOTAL
T	13-11-12	-51	-51	—	FRONT	VERT	TOTAL
U	15-11-12	-51	-51	—	FRONT	VERT	TOTAL
V	1-11-4	-15	-36	—	FRONT	VERT	TOTAL
W	5-11-12	-19	-33	—	FRONT	VERT	TOTAL
X	7-11-12	-19	-33	—	FRONT	VERT	TOTAL
Y	9-11-12	-19	-33	—	FRONT	VERT	TOTAL
Z	13-11-12	-19	-33	—	FRONT	VERT	TOTAL
AA	15-11-12	-19	-33	—	FRONT	VERT	TOTAL
AB	19-11-12	-15	-36	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17334-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
289225	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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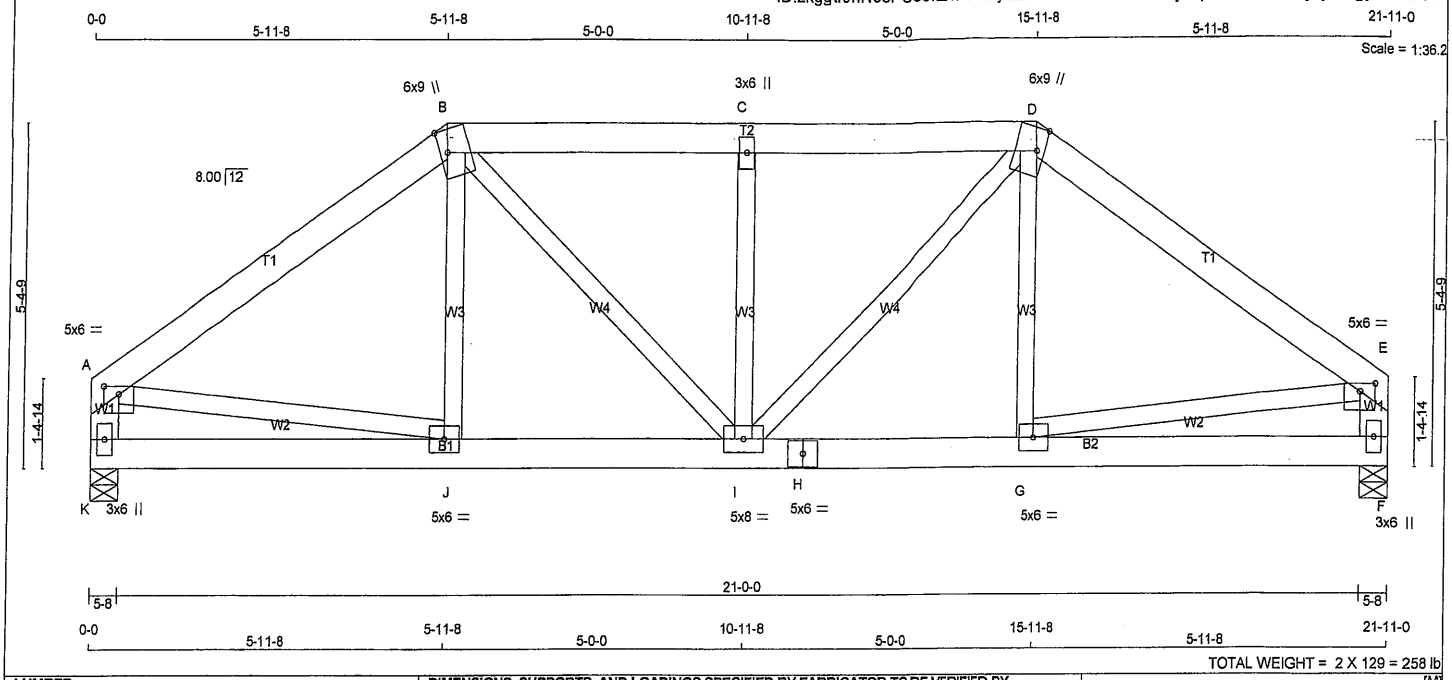

TOTAL WEIGHT = 2 X 129 = 258 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x6	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	2	12	TOP
B- D	2	12	TOP
D- E	2	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(76.8)
H- F	2	12	SIDE(76.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TTWW+m	MT20	6.0	9.0	4.25	1.50
C	TMW+w	MT20	3.0	6.0		
D	TTWW+m	MT20	6.0	9.0	4.25	1.50
E	TMVW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	3222	0	3222	0	0	5-8	5-8
F	3222	0	3222	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2569	1490 / 0	482 / 0	0 / 0	0 / 0	597 / 0	0 / 0
F	2569	1490 / 0	482 / 0	0 / 0	0 / 0	597 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3577 / 0	-112.4	-112.4	0.21 (1)		5.69
B- C	-3678 / 0	-112.4	-112.4	0.12 (1)		5.75
C- D	-3678 / 0	-112.4	-112.4	0.12 (1)		5.75
D- E	-3577 / 0	-112.4	-112.4	0.21 (1)		5.69
K- A	-2766 / 0	0.0	0.0	0.10 (1)		7.81
F- E	-2766 / 0	0.0	0.0	0.10 (1)		7.81
K- J	0 / 0	-181.6	-181.6	0.22 (1)		10.00
J- I	0 / 2993	-181.6	-181.6	0.42 (1)		10.00
I- H	0 / 2993	-181.6	-181.6	0.42 (1)		10.00
H- G	0 / 2993	-181.6	-181.6	0.42 (1)		10.00
G- F	0 / 0	-181.6	-181.6	0.22 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	0 / 743	0.07 (1)
B- I	0 / 959	0.08 (1)
I- C	-636 / 0	0.09 (1)
I- D	0 / 959	0.08 (1)
G- D	0 / 743	0.07 (1)
A- J	0 / 3003	0.27 (1)
G- E	0 / 3003	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 6-10-0
END DISTANCE = 21-11-0
END SPAN CARRIED = 6-10-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.21/1.00 (D-E:1) , BC=0.42/1.00 (G-I:1) , WB=0.27/1.00 (E-G:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (G) (INPUT = 0.90)
JSI METAL= 0.37 (G) (INPUT = 1.00)

DWG NO. TAM 17335-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T32	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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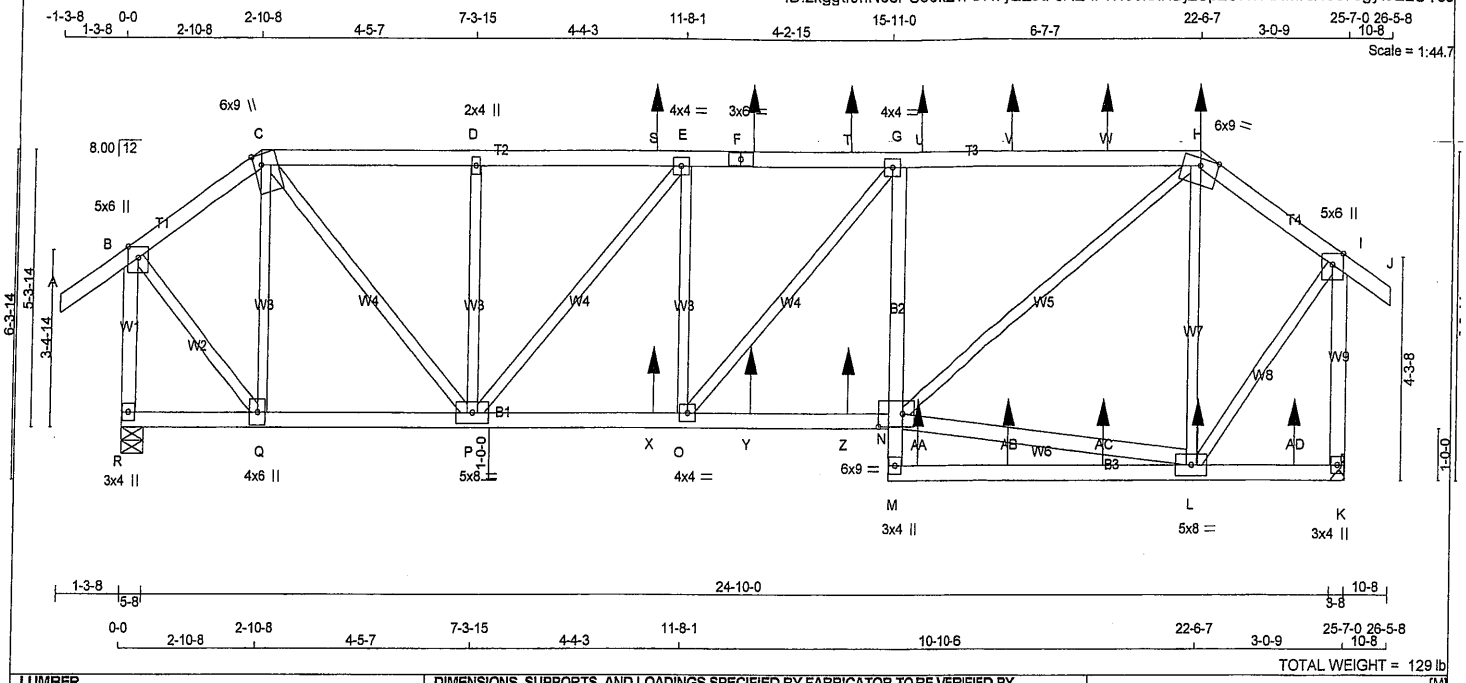

TOTAL WEIGHT = 129 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
M - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMVW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW+p	MT20	5.0	6.0	Edge	
K	BMV1+p	MT20	3.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	9.0	3.00	6.00
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BMWW+t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	1860	0	1860	0	0	5-8	5-8
K	1726	0	1726	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1472	891 / 0	270 / 0	0 / 0	0 / 0	311 / 0	0 / 0
K	1377	831 / 0	271 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00
B- C	-1071 / 0	-112.4	-112.4	0.19 (1)	5.83
C- D	-1786 / 0	-112.4	-112.4	0.42 (1)	4.48
D- S	-1786 / 0	-112.4	-112.4	0.42 (1)	4.48
S- E	-1786 / 0	-112.4	-112.4	0.42 (1)	4.48
E- F	-2083 / 0	-112.4	-112.4	0.69 (1)	3.70
F- T	-2083 / 0	-112.4	-112.4	0.69 (1)	3.70
T- G	-2083 / 0	-112.4	-112.4	0.69 (1)	3.70
G- U	-1989 / 0	-112.4	-112.4	0.77 (1)	3.78
U- V	-1989 / 0	-112.4	-112.4	0.77 (1)	3.78
V- W	-1989 / 0	-112.4	-112.4	0.77 (1)	3.78
W- H	-1989 / 0	-112.4	-112.4	0.77 (1)	3.78
H- I	-886 / 0	-112.4	-112.4	0.21 (1)	6.23
I- J	0 / 30	-112.4	-112.4	0.08 (1)	10.00
R- B	-1833 / 0	0.0	0.0	0.38 (1)	6.13
K- I	-1719 / 0	0.0	0.0	0.55 (1)	6.30
R- Q	0 / 0	-28.0	-28.0	0.12 (7)	10.00
Q- P	0 / 872	-28.0	-28.0	0.22 (1)	10.00
P- X	0 / 2083	-28.0	-28.0	0.42 (1)	10.00
X- O	0 / 2083	-28.0	-28.0	0.42 (1)	10.00
O- Y	0 / 1998	-28.0	-28.0	0.40 (1)	10.00
Y- Z	0 / 1998	-28.0	-28.0	0.40 (1)	10.00
Z- N	0 / 1998	-28.0	-28.0	0.40 (1)	10.00
M- N	0 / 133	0.0	0.0	0.08 (1)	10.00
N- G	-772 / 0	0.0	0.0	0.27 (1)	7.81
M-AA	0 / 44	-28.0	-28.0	0.33 (7)	10.00
AA-AB	0 / 44	-28.0	-28.0	0.33 (7)	10.00
AB-AC	0 / 44	-28.0	-28.0	0.33 (7)	10.00
AC- L	0 / 44	-28.0	-28.0	0.33 (7)	10.00
L-AD	0 / 0	-28.0	-28.0	0.32 (7)	10.00
AD- K	0 / 0	-28.0	-28.0	0.32 (7)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-799 / 0	0.35 (1)
N- L	0 / 694	0.12 (1)
N- H	0 / 1592	0.39 (1)
L- H	-922 / 0	0.60 (1)
B- Q	0 / 1279	0.32 (1)
L- I	0 / 1199	0.30 (1)
O- G	0 / 384	0.10 (5)
C- P	0 / 1377	0.34 (1)
O- E	-199 / 146	0.09 (5)
P- D	-570 / 0	0.25 (1)
P- E	-455 / 243	0.37 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	13-0-12	20	1	241	FRONT	VERT	TOTAL
H	22-6-7	13	1	238	FRONT	VERT	TOTAL
L	22-6-12	16	1	19	FRONT	VERT	TOTAL
S	11-0-12	20	1	241	FRONT	VERT	TOTAL
T	15-0-12	20	1	241	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.77/1.00 (G-H:1) , BC=0.42/1.00 (O-P:1) , WB=0.60/1.00 (H-L:1) , SSI=0.33/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (Q) (INPUT = 0.90)
JSI METAL= 0.32 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/3/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17336-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
289225	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:zkggtrJnN3sPSCJk21FDR7yZZod-sRzA7TH36fetHDjZSpZURc7tRMKiHes7sgyt5EzUTc8

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1.0 lbs FACTORED DOWN AND 241.5 lbs FACTORED UP AT 11-0-12, 1.0 lbs FACTORED DOWN AND 241.5 lbs FACTORED UP AT 13-0-12, 1.0 lbs FACTORED DOWN AND 241.5 lbs FACTORED UP AT 15-0-12, 1.0 lbs FACTORED DOWN AND 245.5 lbs FACTORED UP AT 16-6-12, 1.0 lbs FACTORED DOWN AND 245.5 lbs FACTORED UP AT 18-6-12, AND 1.0 lbs FACTORED DOWN AND 245.5 lbs FACTORED UP AT 20-6-12, AND 1.0 lbs FACTORED DOWN AND 238.2 lbs FACTORED UP AT 22-6-7 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 22.0 lbs FACTORED UP AT 11-0-12, 1.0 lbs FACTORED DOWN AND 22.0 lbs FACTORED UP AT 13-0-12, 1.0 lbs FACTORED DOWN AND 22.0 lbs FACTORED UP AT 15-0-12, 5.6 lbs FACTORED DOWN AND 18.1 lbs FACTORED UP AT 16-6-12, 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 18-6-12, 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 20-6-12, AND 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 22-6-12, AND 1.0 lbs FACTORED DOWN AND 18.5 lbs FACTORED UP AT 24-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	16-6-12	21	1	246	FRONT	VERT	TOTAL
V	18-6-12	21	1	246	FRONT	VERT	TOTAL
W	20-6-12	21	1	246	FRONT	VERT	TOTAL
X	11-0-12	19	1	22	FRONT	VERT	TOTAL
Y	13-0-12	19	1	22	FRONT	VERT	TOTAL
Z	15-0-12	19	1	22	FRONT	VERT	TOTAL
AA	16-6-12	14	-6	18	FRONT	VERT	TOTAL
AB	18-6-12	16	1	19	FRONT	VERT	TOTAL
AC	20-6-12	16	1	19	FRONT	VERT	TOTAL
AD	24-6-12	16	1	19	FRONT	VERT	TOTAL

DWG NO. TAM17336-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T33	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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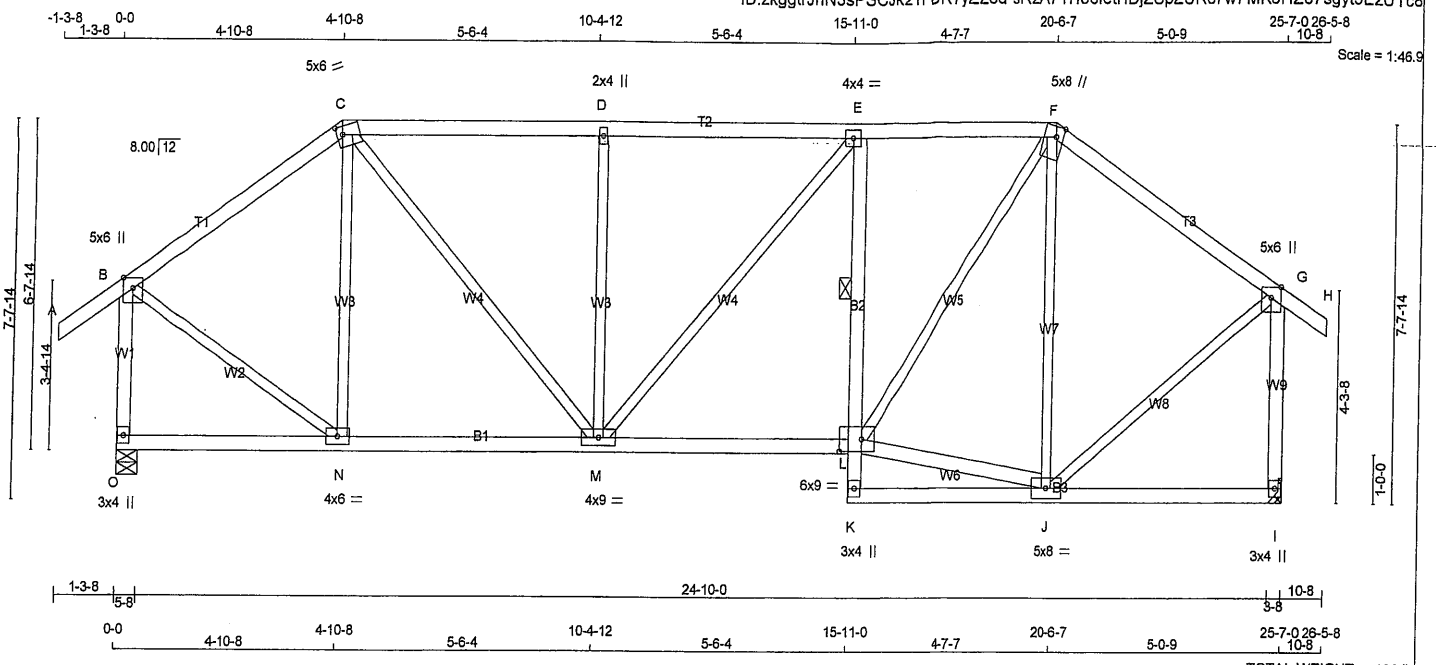


TOTAL WEIGHT = 129 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	1.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-i	MT20	4.0	4.0		
F	TTWW+m	MT20	5.0	8.0	Edge	1.75
G	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	9.0	3.00	5.75
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1951	0	1951	0	0	5-8	5-8
I	1904	0	1904	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1538	920 / 0	269 / 0	0 / 0	0 / 0	349 / 0	0 / 0
I	1506	893 / 0	269 / 0	0 / 0	0 / 0	344 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-L.

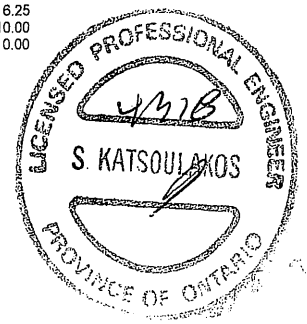
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1370 / 0	-112.4	-112.4	0.53 (1)	4.84
C- D	-1815 / 0	-112.4	-112.4	0.60 (1)	4.21
D- E	-1815 / 0	-112.4	-112.4	0.60 (1)	4.21
E- F	-1728 / 0	-112.4	-112.4	0.41 (1)	4.58
F- G	-1207 / 0	-112.4	-112.4	0.56 (1)	5.03
G- H	0 / 30	-112.4	-112.4	0.07 (1)	10.00
O- B	-1896 / 0	0.0	0.0	0.37 (1)	6.10
I- G	-1849 / 0	0.0	0.0	0.57 (1)	6.16
O- N	0 / 0	-26.0	-28.0	0.19 (3)	10.00
N- M	0 / 1129	-28.0	-28.0	0.33 (2)	10.00
M- L	0 / 1733	-28.0	-28.0	0.39 (2)	10.00
K- L	0 / 78	0.0	0.0	0.05 (1)	10.00
L- E	-690 / 0	0.0	0.0	0.07 (1)	6.25
K- J	0 / 32	-28.0	-28.0	0.20 (3)	10.00
J- I	0 / 0	-28.0	-28.0	0.20 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-543 / 0	0.40 (1)
C- M	0 / 1034	0.23 (1)
M- D	-707 / 0	0.52 (1)
M- E	0 / 125	0.03 (1)
L- J	0 / 994	0.16 (1)
L- F	0 / 1260	0.28 (1)
J- F	-841 / 0	0.91 (1)
B- N	0 / 1339	0.30 (1)
J- G	0 / 1274	0.29 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.60/1.00 (C-D:1) , BC=0.39/1.00 (L-M:2) , WB=0.91/1.00 (F-J:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

DWG NO. TAM 17337-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T34	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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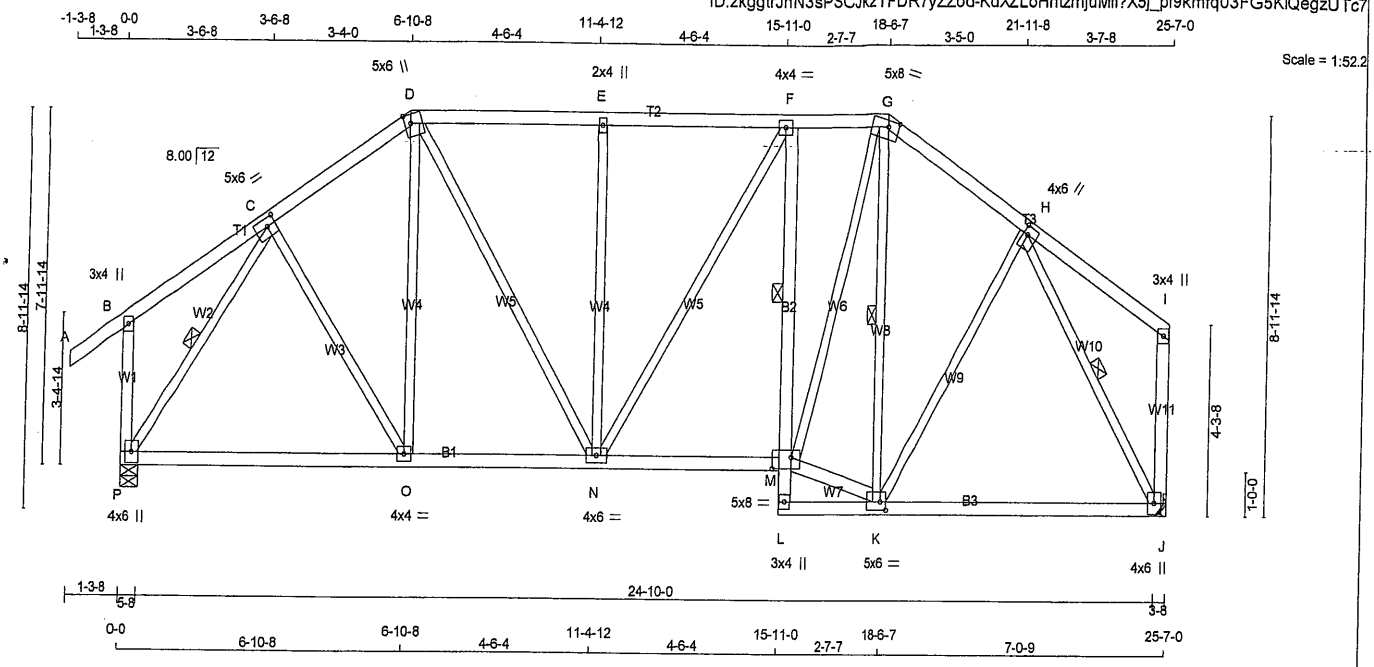

TOTAL WEIGHT = 144 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
L - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMWW+t	MT20	4.0	6.0	2.25	1.25
I	TMV+p	MT20	3.0	4.0		
J	BMVW1+p	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0	2.25	1.50
L	BMV+p	MT20	3.0	4.0		
M	BVMWW-l	MT20	5.0	8.0	3.00	5.50
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
P	1951	0	1951	0	0	IN-SX	IN-SX
J	1796	0	1796	0	0	5-8 HANGER BY OTHERS MIN. SEAT SIZE: 3-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1538	920 / 0	269 / 0	0 / 0	0 / 0	349 / 0	0 / 0
J	1432	831 / 0	269 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-M, G-K, C-P, H-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 24	-112.4	-112.4	0.21 (1)	10.00
C- D	-1461 / 0	-112.4	-112.4	0.18 (1)	5.24
D- E	-1513 / 0	-112.4	-112.4	0.29 (1)	5.03
E- F	-1514 / 0	-112.4	-112.4	0.29 (1)	5.03
F- G	-1406 / 0	-112.4	-112.4	0.15 (1)	5.35
G- H	-1319 / 0	-112.4	-112.4	0.19 (1)	5.43
H- I	0 / 26	-112.4	-112.4	0.23 (1)	10.00
P- B	-303 / 0	0.0	0.0	0.06 (1)	7.81
J- I	-150 / 0	0.0	0.0	0.05 (1)	7.81
P- O	0 / 999	-28.0	-28.0	0.39 (2)	10.00
O- N	0 / 1192	-28.0	-28.0	0.42 (2)	10.00
N- M	0 / 1410	-28.0	-28.0	0.31 (1)	10.00
L- M	-40 / 0	0.0	0.0	0.03 (1)	7.81
M- F	-581 / 0	0.0	0.0	0.07 (1)	6.25
L- K	0 / 29	-28.0	-28.0	0.34 (3)	10.00
K- J	0 / 870	-28.0	-28.0	0.42 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	0 / 352	0.08 (1)
O- D	-95 / 125	0.12 (1)
D- N	0 / 623	0.14 (1)
N- E	-594 / 0	0.72 (1)
N- F	0 / 202	0.05 (1)
M- K	0 / 1127	0.18 (1)
M- G	0 / 1095	0.25 (1)
K- G	-575 / 0	0.30 (1)
K- H	0 / 412	0.09 (1)
P- C	-1859 / 0	0.52 (1)
H- J	-1793 / 0	0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.42/1.00 (N-O:2) , WB=0.72/1.00 (E-N:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.54 (H) (INPUT = 1.00)

DWG NO. TAM 17336-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T35	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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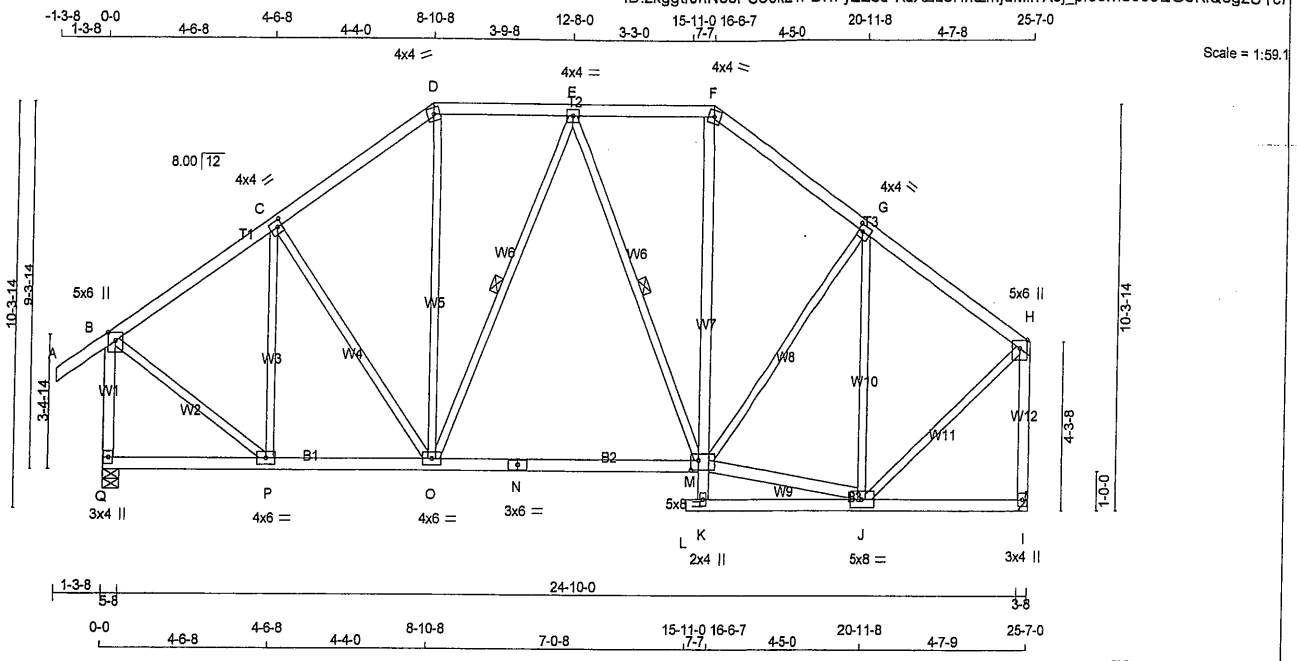


TOTAL WEIGHT = 142 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
I	BMV1+p	MT20	3.0	4.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMW+w	MT20	2.0	4.0		
M	BWMWWW*-I	MT20	5.0	8.0	3.00	2.50
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	1955	0	1955	0	0	5-8	5-8
I	1805	0	1805	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1544	920 / 0	272 / 0	0 / 0	0 / 0	351 / 0	0 / 0
I	1443	831 / 0	275 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O, E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

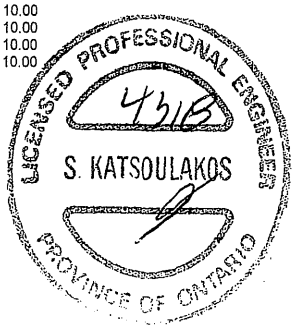
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1349 / 0	-112.4	-112.4	0.31 (1)	5.25
C- D	-1447 / 0	-112.4	-112.4	0.31 (1)	5.10
D- E	-1179 / 0	-112.4	-112.4	0.22 (1)	5.64
E- F	-1179 / 0	-112.4	-112.4	0.22 (1)	5.64
F- G	-1444 / 0	-112.4	-112.4	0.32 (1)	5.09
G- H	-1182 / 0	-112.4	-112.4	0.31 (1)	5.51
Q- B	-1898 / 0	0.0	0.0	0.37 (1)	6.10
I- H	-1756 / 0	0.0	0.0	0.54 (1)	6.29
Q- P	0 / 0	-28.0	-28.0	0.14 (3)	10.00
P- O	0 / 1149	-28.0	-28.0	0.45 (2)	10.00
O- N	0 / 1286	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 1286	-28.0	-28.0	0.47 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.01 (3)	10.00
K- J	0 / 18	-28.0	-28.0	0.18 (3)	10.00
J- I	0 / 0	-28.0	-28.0	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-672 / 0	0.45 (1)
C- O	0 / 67	0.02 (3)
O- D	0 / 437	0.10 (1)
K- M	0 / 110	0.04 (1)
M- F	0 / 426	0.09 (1)
J- G	-940 / 0	0.92 (1)
B- P	0 / 1389	0.31 (1)
J- H	0 / 1342	0.30 (1)
O- E	-284 / 0	0.19 (1)
E- M	-280 / 0	0.19 (1)
M- J	0 / 1026	0.16 (1)
M- G	0 / 248	0.06 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.54/1.00 (H-I:1) , BC=0.47/1.00 (M-O:2) , WB=0.92/1.00 (G-J:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (P) (INPUT = 0.90)
JSI METAL= 0.51 (N) (INPUT = 1.00)

DWG NO. TAM 17339-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T36	1	2	OBC2012_6TCDL TRUSS DESC.	

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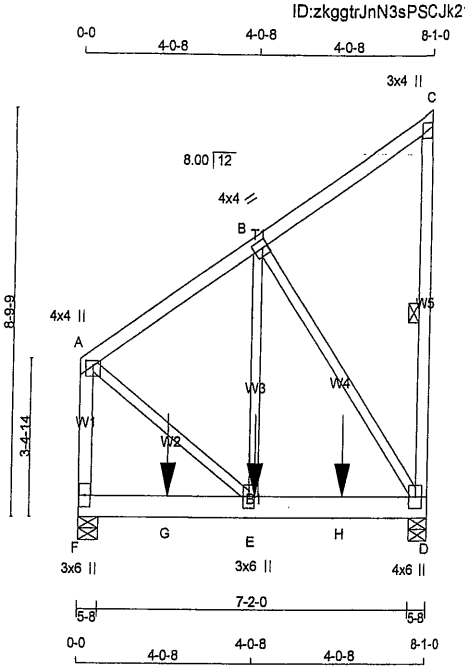


TOTAL WEIGHT = 2 X 51 = 103 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	3.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 557.1 lbs FACTORED DOWN AT 2-0-12, AND 557.1 lbs FACTORED DOWN AT 4-0-12, AND 557.1 lbs FACTORED DOWN AT 6-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1406	0	1406	0	0	5-8	5-8
F	1400	0	1400	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1102	670 / 0	185 / 0	0 / 0	0 / 0	247 / 0	0 / 0
F	1097	667 / 0	184 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

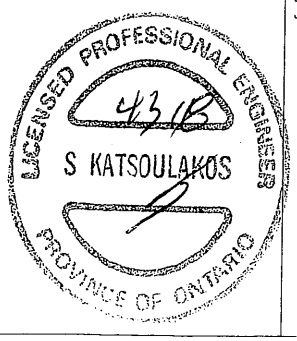
LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-817 / 0	-112.4	-112.4	0.17 (1)	6.25		E- B	0 / 899	0.11 (1)
B- C	-30 / 0	-112.4	-112.4	0.16 (1)	6.25		B- D	-1217 / 0	0.53 (1)
D- C	-173 / 0	0.0	0.0	0.03 (1)	6.25		A- E	0 / 878	0.11 (1)
F- A	-1166 / 0	0.0	0.0	0.11 (1)	7.81				
F- G	0 / 0	-28.0	-28.0	0.16 (1)	10.00				
G- E	0 / 0	-28.0	-28.0	0.16 (1)	10.00				
E- H	0 / 705	-28.0	-28.0	0.20 (1)	10.00				
H- D	0 / 705	-28.0	-28.0	0.20 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	4-0-12	-557	-557	—	BACK	VERT	TOTAL
G	2-0-12	-557	-557	—	BACK	VERT	TOTAL
H	6-0-12	-557	-557	—	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.20/1.00 (D-E:1) , WB=0.53/1.00 (B-D:1) , SSI=0.14/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (E) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

DWG NO. TAM 12340-18.
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T37	1	1	OBC2012_6TCDL TRUSS DESC.	

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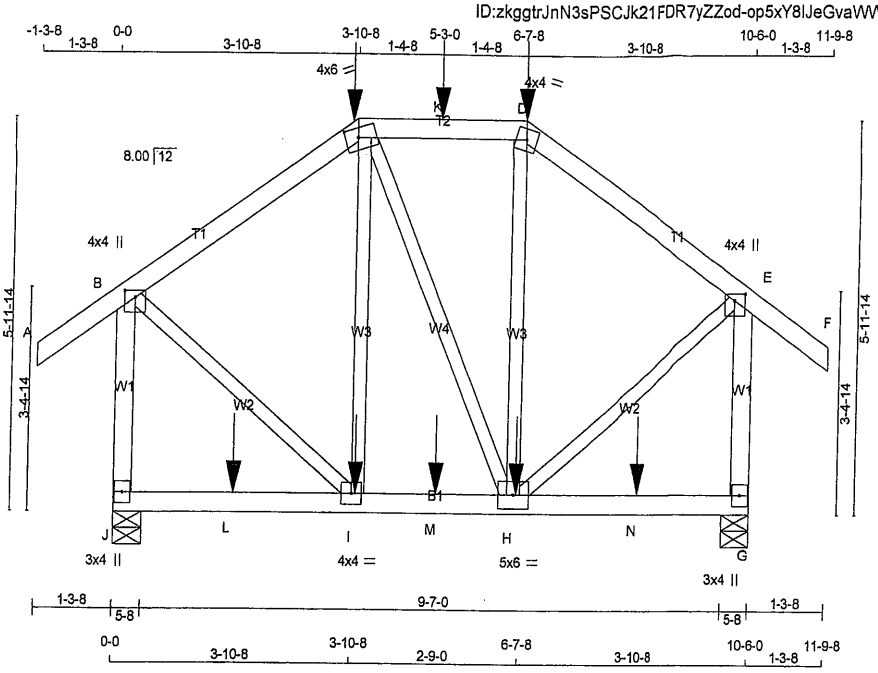

TOTAL WEIGHT = 58 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 306.4 lbs FACTORED DOWN AT 3-10-8, AND 51.4 lbs FACTORED DOWN AT 5-3-0, AND 303.3 lbs FACTORED DOWN AT 6-7-8 ON TOP CHORD, AND 39.6 lbs FACTORED DOWN AT 1-11-4, 38.9 lbs FACTORED DOWN AT 3-11-4, 38.9 lbs FACTORED DOWN AT 5-3-0, AND 38.9 lbs FACTORED DOWN AT 6-6-12, AND 39.6 lbs FACTORED DOWN AT 8-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1275	0	1275	0	0	5-8	5-8
G	1274	0	1274	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	991	614 / 0	157 / 0	0 / 0	0 / 0	221 / 0	0 / 0
G	990	614 / 0	155 / 0	0 / 0	0 / 0	221 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
B- C	-706 / 0	-112.4	-112.4	0.33 (1)	6.25	
C- K	-579 / 0	-112.4	-112.4	0.22 (1)	6.25	
K- D	-579 / 0	-112.4	-112.4	0.22 (1)	6.25	
D- E	-704 / 0	-112.4	-112.4	0.33 (1)	6.25	
E- F	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
J- B	-1220 / 0	0.0	0.0	0.25 (1)	7.22	
G- E	-1217 / 0	0.0	0.0	0.25 (1)	7.23	
J- L	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
L- I	0 / 0	-28.0	-28.0	0.16 (3)	10.00	
I- M	0 / 581	-28.0	-28.0	0.19 (2)	10.00	
M- H	0 / 581	-28.0	-28.0	0.19 (2)	10.00	
H- N	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
N- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-307 / 62	0.18 (1)
H- D	-310 / 67	0.18 (1)
B- I	0 / 742	0.18 (1)
H- E	0 / 740	0.18 (1)
C- H	-5 / 0	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-306	-306	---	FRONT	VERT	TOTAL
D	6-7-8	-303	-303	---	FRONT	VERT	TOTAL
H	6-6-12	-22	-39	---	FRONT	VERT	TOTAL
I	3-11-4	-22	-39	---	FRONT	VERT	TOTAL
K	5-3-0	-51	-51	---	FRONT	VERT	TOTAL
L	1-11-4	-19	-40	---	FRONT	VERT	TOTAL
M	5-3-0	-22	-39	---	FRONT	VERT	TOTAL
N	8-6-12	-19	-40	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.19/1.00 (H-I:2) , WB=0.18/1.00 (B-I:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.22 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17341-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289225	T38	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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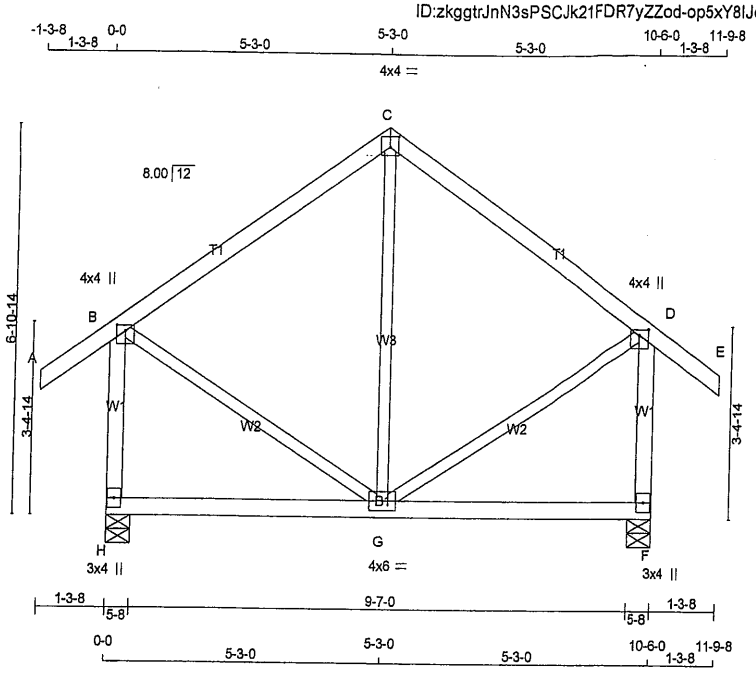


TOTAL WEIGHT = 51 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	892	0	892	0	0	5-8	5-8
F	892	0	892	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	693	430 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0
F	693	430 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	G- C	-180 / 133	0.14 (1)
B- C	-370 / 0	-112.4	-112.4	0.40 (1)	6.25	B- G	0 / 357	0.08 (1)
C- D	-370 / 0	-112.4	-112.4	0.40 (1)	6.25	G- D	0 / 357	0.08 (1)
D- E	0 / 43	-112.4	-112.4	0.15 (1)	10.00			
H- B	-835 / 0	0.0	0.0	0.16 (1)	7.81			
F- D	-835 / 0	0.0	0.0	0.16 (1)	7.81			
H- G	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
G- F	0 / 0	-28.0	-28.0	0.24 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.40/1.00 (C-D:1) , BC=0.24/1.00 (F-G:3) , WB=0.14/1.00 (C-G:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

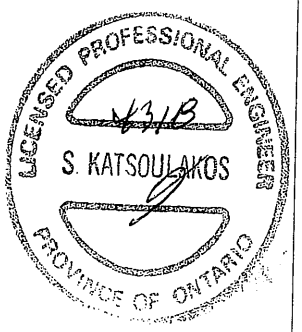
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (B) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)



DWG NO. TAM 17342-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T61	2	1	OBC2012_6TCDL TRUSS DESC.	

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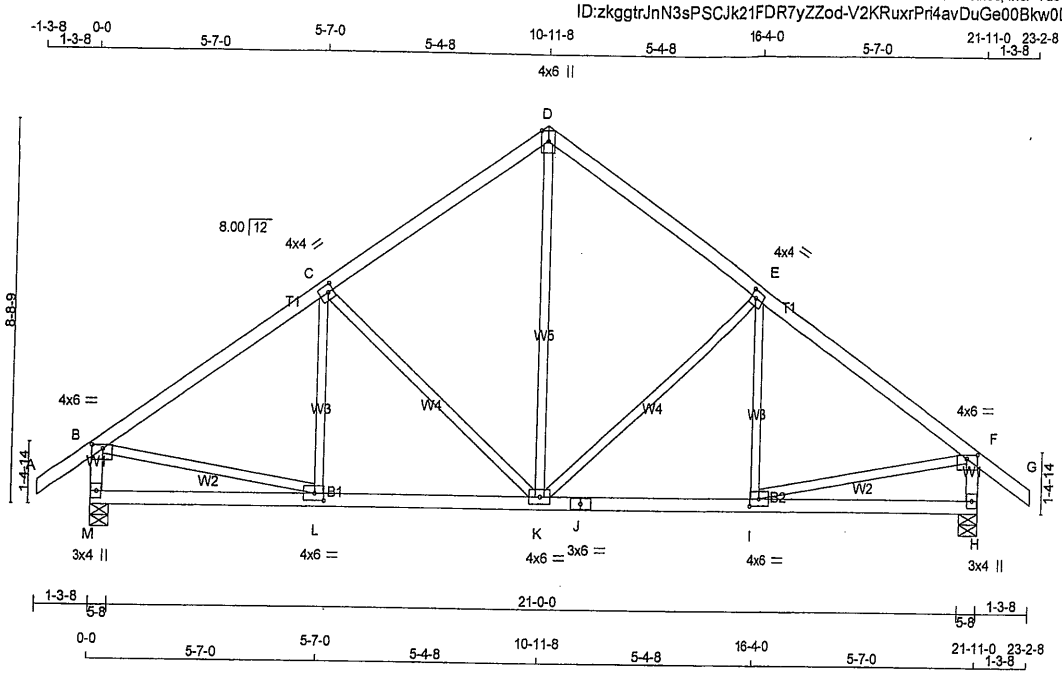

TOTAL WEIGHT = 2 X 95 = 190 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.50
F	TMVW-p	MT20	4.0	6.0	1.00	3.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.75
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0	2.00	2.75
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1693	0	1693	0	0	IN-SX	IN-SX
H	1693	0	1693	0	0	5-8	5-8
						5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1332	801 / 0	230 / 0	0 / 0	0 / 0	301 / 0	0 / 0
H	1332	801 / 0	230 / 0	0 / 0	0 / 0	301 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1680 / 0	-112.4	-112.4	0.49 (1)	4.57
C- D	-1258 / 0	-112.4	-112.4	0.46 (1)	5.16
D- E	-1258 / 0	-112.4	-112.4	0.46 (1)	5.16
E- F	-1680 / 0	-112.4	-112.4	0.49 (1)	4.57
F- G	0 / 43	-112.4	-112.4	0.15 (1)	10.00
M- B	-1628 / 0	0.0	0.0	0.17 (1)	6.48
H- F	-1628 / 0	0.0	0.0	0.17 (1)	6.48
M- L	0 / 0	-28.0	-28.0	0.22 (3)	10.00
L- K	0 / 1432	-28.0	-28.0	0.36 (2)	10.00
K- J	0 / 1432	-28.0	-28.0	0.36 (2)	10.00
J- I	0 / 1432	-28.0	-28.0	0.36 (2)	10.00
I- H	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 896	0.20 (1)
K- E	-563 / 0	0.57 (1)
I- E	-109 / 179	0.04 (3)
C- K	-563 / 0	0.57 (1)
L- C	-109 / 179	0.04 (3)
B- L	0 / 1459	0.33 (1)
I- F	0 / 1459	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.49/1.00 (E-F:1) , BC=0.36/1.00 (I-K:2) , WB=0.57/1.00 (E-K:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1037	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.40 (B) (INPUT = 1.00)

DWG NO. TAM 17295-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G61	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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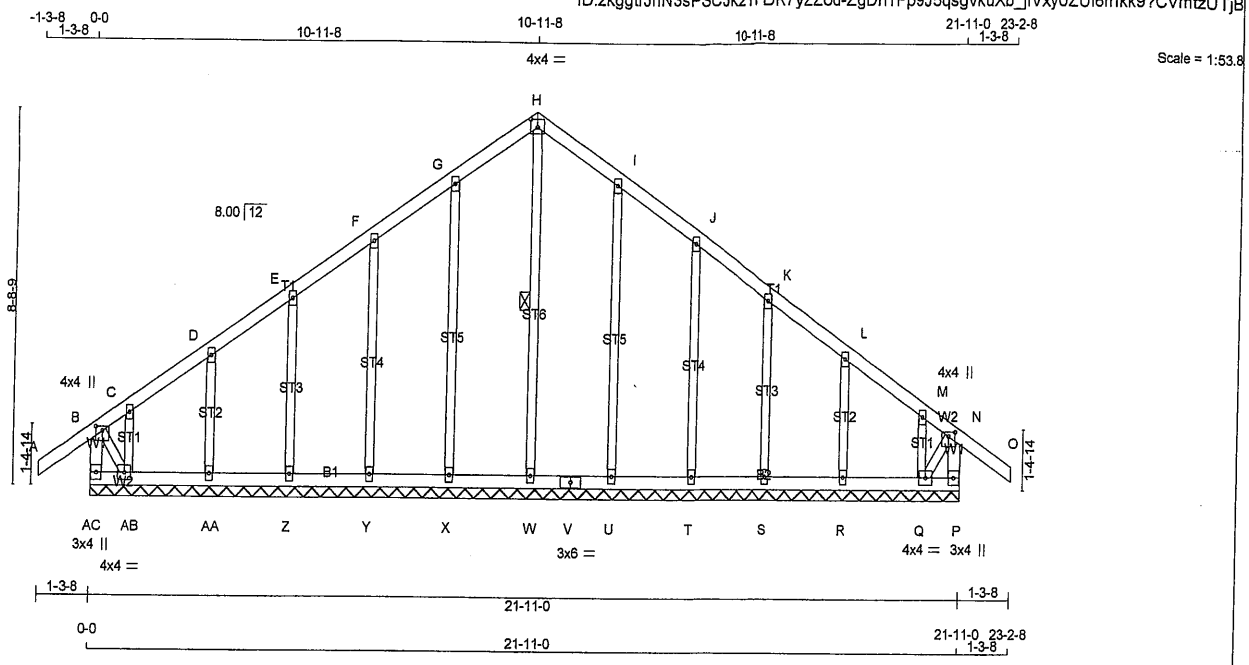


TOTAL WEIGHT = 103 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AC- B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AC- V	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, F, G, I, J, K, L, M						
C	TMW+w	MT20	2.0	4.0		
H	TTW-p	MT20	4.0	4.0	2.25	2.00
N	TMVW+p	MT20	4.0	4.0	1.25	2.00
P	BMV1+p	MT20	3.0	4.0		
Q	BMWW1-t	MT20	4.0	4.0		
R, S, T, U, W, X, Y, Z, AA						
R	BMW1+w	MT20	2.0	4.0		
V	BS-t	MT20	3.0	6.0		
AB	BMWW1-t	MT20	4.0	4.0		
AC	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

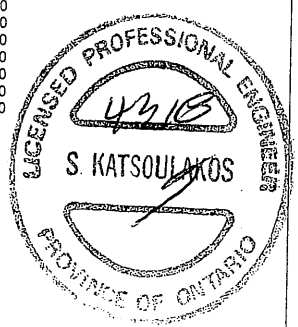
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AC- B	-355 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-90 / 0	-112.4	-112.4	0.14 (1)	6.25
C- D	-23 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	-24 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-18 / 0	-112.4	-112.4	0.05 (1)	6.25
F- G	-11 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-23 / 0	-112.4	-112.4	0.06 (1)	6.25
H- I	-23 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-11 / 0	-112.4	-112.4	0.06 (1)	6.25
J- K	-18 / 0	-112.4	-112.4	0.05 (1)	6.25
K- L	-24 / 0	-112.4	-112.4	0.06 (1)	6.25
L- M	-23 / 0	-112.4	-112.4	0.06 (1)	6.25
M- N	-90 / 0	-112.4	-112.4	0.14 (1)	6.25
N- O	0 / 43	-112.4	-112.4	0.15 (1)	10.00
P- N	-355 / 0	0.0	0.0	0.04 (1)	7.81
AC-AB	0 / 0	-28.0	-28.0	0.02 (3)	10.00
AB-AA	0 / 25	-28.0	-28.0	0.03 (3)	10.00
AA- Z	0 / 19	-28.0	-28.0	0.03 (3)	10.00
Z- Y	0 / 15	-28.0	-28.0	0.02 (3)	10.00
Y- X	0 / 12	-28.0	-28.0	0.02 (3)	10.00
X- W	0 / 9	-28.0	-28.0	0.02 (3)	10.00
W- V	0 / 9	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 9	-28.0	-28.0	0.02 (3)	10.00
U- T	0 / 12	-28.0	-28.0	0.02 (3)	10.00
T- S	0 / 15	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 19	-28.0	-28.0	0.03 (3)	10.00
R- Q	0 / 25	-28.0	-28.0	0.03 (3)	10.00
Q- P	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- H	-177 / 0	0.08 (1)
X- G	-256 / 0	0.25 (1)
Y- F	-217 / 0	0.12 (1)
Z- E	-220 / 0	0.07 (1)
AA- D	-236 / 0	0.05 (1)
AB- C	-45 / 0	0.01 (1)
U- I	-256 / 0	0.25 (1)
T- J	-217 / 0	0.12 (1)
S- K	-220 / 0	0.07 (1)
R- L	-236 / 0	0.05 (1)
Q- M	-45 / 0	0.01 (1)
B-AB	0 / 42	0.01 (1)
Q- N	0 / 42	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.03/1.00 (AA-AB:3) , WB=0.25/1.00 (G-X:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1037	822	2234	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (AB) (INPUT = 0.90)
JSI METAL= 0.06 (I) (INPUT = 1.00)

DWG NO. TAM 173/2 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T62	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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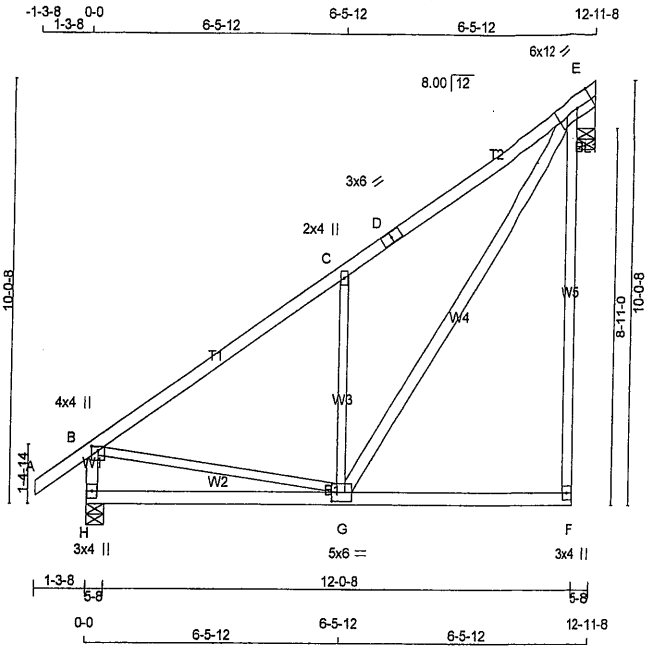

TOTAL WEIGHT = 7 X 68 = 477 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMVWK1-t	MT20	6.0	12.0	Edge	5.00
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	878	0	878	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
H	1032	0	1032	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	700	406 / 0	131 / 0	0 / 0	0 / 0	163 / 0	0 / 0
H	805	495 / 0	131 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) E (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-718 / 0	-112.4	-112.4	0.59 (1)	6.04
C- D	-781 / 0	-112.4	-112.4	0.58 (1)	5.90
D- E	-781 / 0	-112.4	-112.4	0.58 (1)	5.90
F- E	0 / 106	0.0	0.0	0.02 (3)	10.00
H- B	-959 / 0	0.0	0.0	0.10 (1)	7.81
H- G	*0 / 0*	*-28.0*	*-28.0*	*0.34 (3)*	*10.00*
G- F	0 / 0	-28.0	-28.0	0.34 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-857 / 0	0.43 (1)
G- E	0 / 1142	0.18 (1)
B- G	0 / 644	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.59/1.00 (B-C:1) , BC=0.34/1.00 (G-H:3) , WB=0.43/1.00 (C-G:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)

DWG NO. TAM 17296-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G62	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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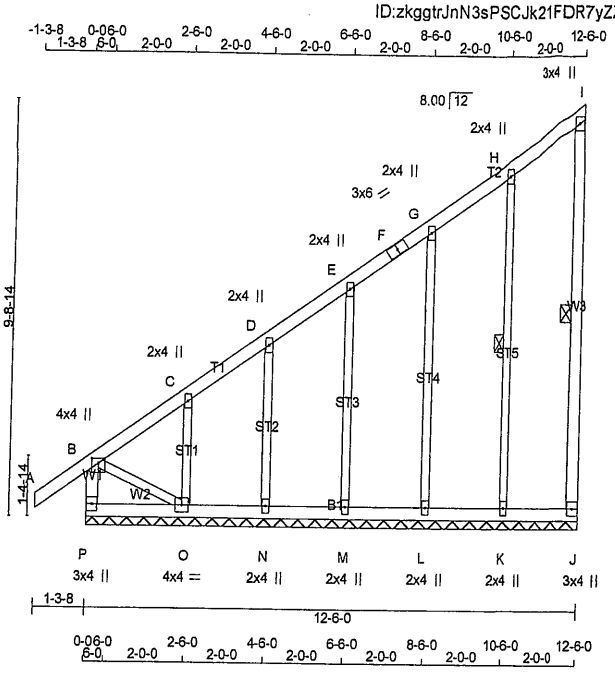

Scale = 1:54.3

TOTAL WEIGHT = 68 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TMV+p	MT20	3.0	4.0		
J	DMV1+p	MT20	3.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-11 / 0	-112.4	-112.4	0.08 (1)	6.25
C- D	-24 / 0	-112.4	-112.4	0.08 (1)	6.25
D- E	-9 / 0	-112.4	-112.4	0.05 (1)	10.00
E- F	-7 / 0	-112.4	-112.4	0.05 (1)	10.00
F- G	-7 / 0	-112.4	-112.4	0.05 (1)	10.00
G- H	-1 / 0	-112.4	-112.4	0.06 (1)	10.00
H- I	-12 / 0	-112.4	-112.4	0.06 (1)	6.25
J- I	-96 / 0	0.0	0.0	0.04 (1)	6.25
P- B	-288 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-28.0	-28.0	0.04 (3)	10.00
O- N	0 / 13	-28.0	-28.0	0.04 (3)	10.00
N- M	0 / 9	-28.0	-28.0	0.03 (3)	10.00
M- L	0 / 5	-28.0	-28.0	0.03 (3)	10.00
L- K	0 / 3	-28.0	-28.0	0.03 (3)	10.00
K- J	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- H	-252 / 0	0.11 (1)
L- G	-218 / 0	0.19 (1)
M- E	-227 / 0	0.11 (1)
N- D	-204 / 0	0.06 (1)
O- C	-284 / 0	0.05 (1)
B- O	0 / 23	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.04/1.00 (N-O:3) , WB=0.19/1.00 (G-L:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17313 -T1
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T63	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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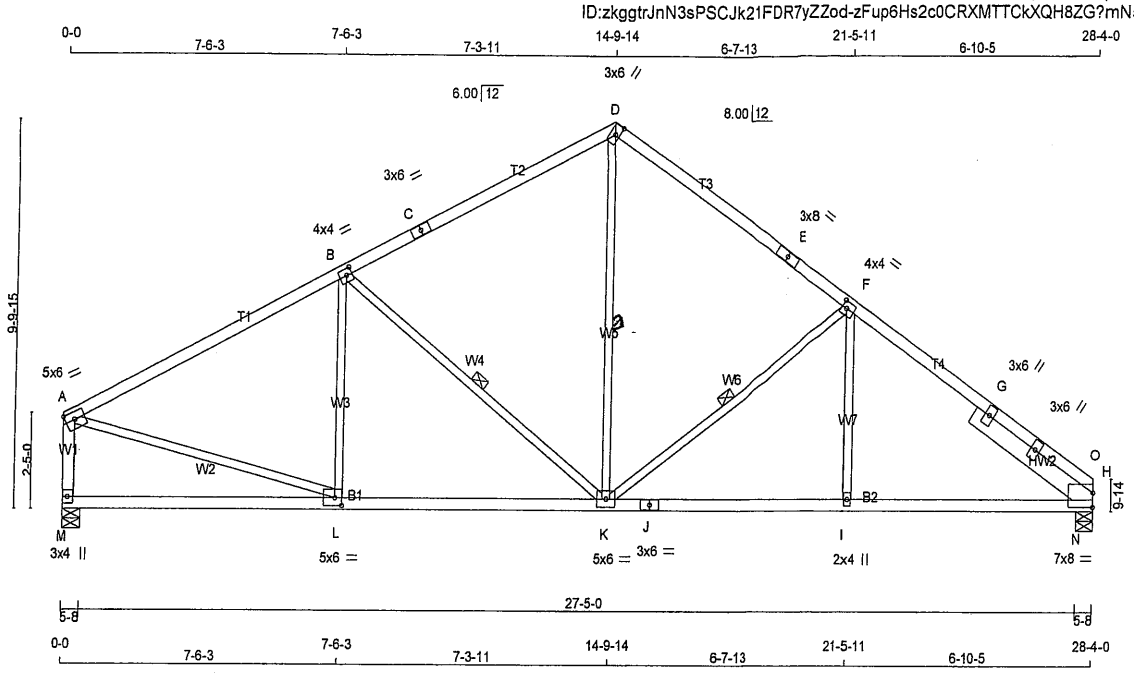


TOTAL WEIGHT = 115 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TS-t	MT20	3.0	6.0		
D	TTW+h	MT20	3.0	6.0	3.00	1.25
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMBMR1-I	MT20	7.0	8.0	4.50	
H	RT+t	MT20	3.0	6.0		
H	RT+t	MT20	3.0	6.0		
I	BMW+w	MT20	2.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1989	0	1989	0	0	5-8	5-8
H	1989	0	1989	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1586	920 / 0	298 / 0	0 / 0	0 / 0	368 / 0	0 / 0
H	1586	920 / 0	298 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

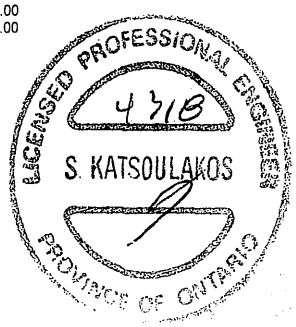
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-K, F-K. 0-16

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2220 / 0	-112.4	-112.4	1.00 (1)	2.95
B- C	-1717 / 0	-112.4	-112.4	0.90 (1)	3.54
C- D	-1717 / 0	-112.4	-112.4	0.90 (1)	3.54
D- E	-1819 / 0	-112.4	-112.4	0.71 (1)	4.06
E- F	-1819 / 0	-112.4	-112.4	0.71 (1)	4.06
F- G	-1967 / 0	-112.4	-112.4	0.59 (1)	4.19
G- O	-1967 / 0	-112.4	-112.4	0.59 (1)	4.19
O- H	-2493 / 0	-112.4	-112.4	0.10 (1)	4.31
M- A	-1904 / 0	0.0	0.0	0.24 (1)	6.09
M- L	0 / 0	-28.0	-28.0	0.46 (3)	10.00
L- K	0 / 2026	-28.0	-28.0	0.60 (2)	10.00
K- J	0 / 2139	-28.0	-28.0	0.49 (1)	10.00
J- I	0 / 2139	-28.0	-28.0	0.49 (1)	10.00
I- N	0 / 2139	-28.0	-28.0	0.47 (1)	10.00
N- H	0 / 1214	-28.0	-28.0	0.38 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-338 / 153	0.21 (1)
B- K	-684 / 0	0.41 (1)
K- D	0 / 1119	0.25 (1)
K- F	-820 / 0	0.40 (1)
I- F	0 / 320	0.08 (3)
A- L	0 / 2104	0.47 (1)
N- O	0 / 1207	0.00 (1)
N- G	-1285 / 0	0.16 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=1.00/1.00 (A-B:1) , BC=0.60/1.00 (K-L:2) , WB=0.47/1.00 (A-L:1) , SSI=0.36/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.64 (H) (INPUT = 1.00)

DWG NO. TAM 17297 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G63	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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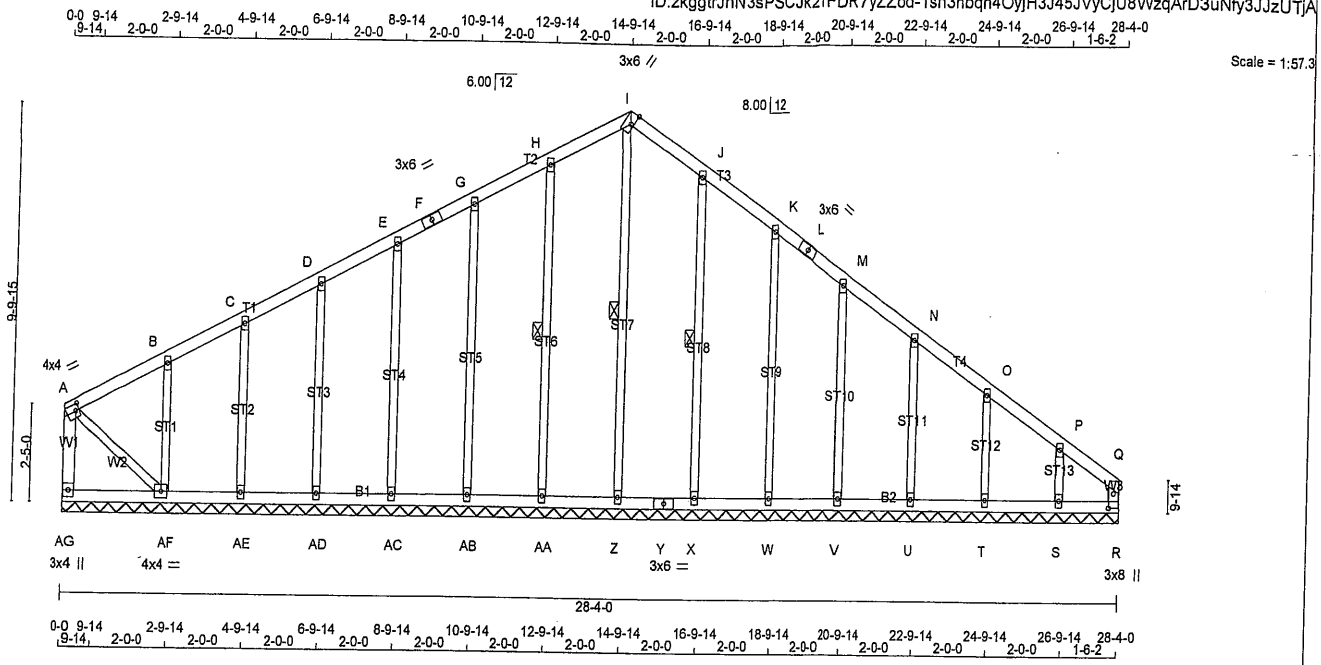


TOTAL WEIGHT = 133 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
AG- A	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
AG- Y	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, G, H, J, K, M, N, O, P						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW+h	MT20	3.0	6.0	Edge	0.75
L	TS-t	MT20	3.0	6.0		
Q						
R						
R	TMBMV1+p	MT20	3.0	8.0	4.25	1.50
S, T, U, V, W, X, Z, AA, AB, AC, AD, AE						
S	BMW1+w	MT20	2.0	4.0		
Y	BS-t	MT20	3.0	6.0		
AF	BMWW1-t	MT20	4.0	4.0		
AG	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-Z, H-AA, J-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

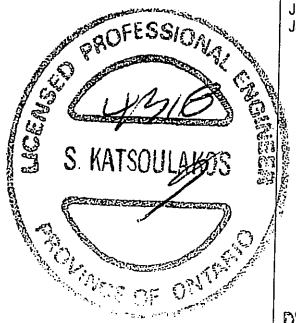
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29		-112.4	-112.4	0.10	(1)	10.00
B- C	0 / 14		-112.4	-112.4	0.10	(1)	10.00
C- D	0 / 28		-112.4	-112.4	0.06	(1)	10.00
D- E	0 / 29		-112.4	-112.4	0.06	(1)	10.00
E- F	0 / 32		-112.4	-112.4	0.06	(1)	10.00
F- G	0 / 32		-112.4	-112.4	0.06	(1)	10.00
G- H	0 / 37		-112.4	-112.4	0.07	(1)	10.00
H- I	0 / 26		-112.4	-112.4	0.07	(1)	10.00
I- J	0 / 24		-112.4	-112.4	0.07	(1)	10.00
J- K	0 / 36		-112.4	-112.4	0.07	(1)	10.00
K- L	0 / 30		-112.4	-112.4	0.06	(1)	10.00
L- M	0 / 30		-112.4	-112.4	0.06	(1)	10.00
M- N	0 / 26		-112.4	-112.4	0.06	(1)	10.00
N- O	0 / 21		-112.4	-112.4	0.06	(1)	10.00
O- P	0 / 15		-112.4	-112.4	0.05	(1)	10.00
P- Q	0 / 12		-112.4	-112.4	0.05	(1)	10.00
AG- A	-103 / 0		0.0	0.0	0.01	(1)	7.81
R- Q	-64 / 0		0.0	0.0	0.01	(1)	7.81
AG-AF	0 / 0		-28.0	-28.0	0.05	(3)	10.00
AF-AE	-21 / 0		-28.0	-28.0	0.05	(3)	6.25
AE-AD	-24 / 0		-28.0	-28.0	0.03	(3)	6.25
AD-AC	-26 / 0		-28.0	-28.0	0.03	(3)	6.25
AC-AB	-29 / 0		-28.0	-28.0	0.02	(3)	6.25
AB-AA	-31 / 0		-28.0	-28.0	0.02	(3)	6.25
AA- Z	-33 / 0		-28.0	-28.0	0.02	(3)	6.25
Z- Y	-30 / 0		-28.0	-28.0	0.02	(3)	6.25
Y- X	-30 / 0		-28.0	-28.0	0.02	(3)	6.25
X- W	-28 / 0		-28.0	-28.0	0.02	(3)	6.25
W- V	-25 / 0		-28.0	-28.0	0.02	(3)	6.25
V- U	-22 / 0		-28.0	-28.0	0.02	(3)	6.25
U- T	-18 / 0		-28.0	-28.0	0.02	(3)	6.25
T- S	-11 / 0		-28.0	-28.0	0.02	(3)	6.25
S- R	0 / 0		-28.0	-28.0	0.02	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- I	-223 / 0	0.14 (1)
AA- H	-264 / 0	0.13 (1)
AB- G	-216 / 0	0.24 (1)
AC- E	-224 / 0	0.17 (1)
AD- D	-228 / 0	0.12 (1)
AE- C	-199 / 0	0.07 (1)
AF- B	-306 / 0	0.07 (1)
X- J	-249 / 0	0.12 (1)
W- K	-218 / 0	0.20 (1)
V- M	-224 / 0	0.12 (1)
U- N	-221 / 0	0.07 (1)
T- O	-224 / 0	0.04 (1)
S- P	-211 / 0	0.03 (1)
A-AF	-19 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (A-B:1) , BC=0.05/1.00 (AE-AF:3) , WB=0.24/1.00 (G-AB:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (A) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 173/14 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T63A	8	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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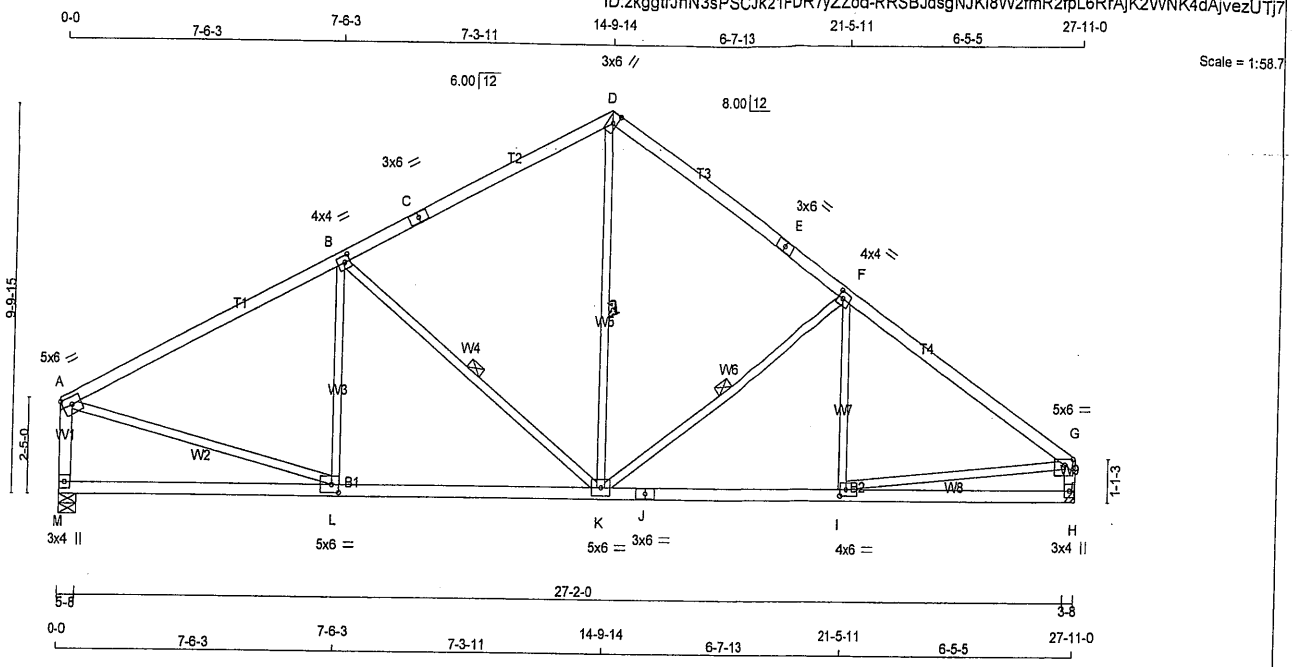

TOTAL WEIGHT = 8 X 113 = 903 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.25	Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TS-t	MT20	3.0	6.0		
D	TTW+h	MT20	3.0	6.0	3.00	1.25
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-p	MT20	5.0	6.0	1.75	2.75
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0	2.00	2.00
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1960	0	1960	0	0	5-8	5-8
H	1960	0	1960	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0
H	1563	907 / 0	293 / 0	0 / 0	0 / 0	363 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, F-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2178 / 0	-112.4	-112.4	0.99 (1)	2.99	L- B	-326 / 160	0.20 (1)
B- C	-1666 / 0	-112.4	-112.4	0.90 (1)	3.59	B- K	-696 / 0	0.42 (1)
C- D	-1666 / 0	-112.4	-112.4	0.90 (1)	3.59	K- D	0 / 1089	0.25 (1)
D- E	-1777 / 0	-112.4	-112.4	0.73 (1)	4.04	K- F	-767 / 0	0.37 (1)
E- F	-1777 / 0	-112.4	-112.4	0.73 (1)	4.04	I- F	-46 / 250	0.06 (3)
F- G	-2414 / 0	-112.4	-112.4	0.79 (1)	3.50	A- L	0 / 2065	0.46 (1)
M- A	-1875 / 0	0.0	0.0	0.24 (1)	6.13	I- G	0 / 2066	0.46 (1)
H- G	-1886 / 0	0.0	0.0	0.19 (1)	6.11			
M- L	0 / 0	-28.0	-28.0	0.40 (3)	10.00			
L- K	0 / 1988	-28.0	-28.0	0.60 (2)	10.00			
K- J	0 / 2051	-28.0	-28.0	0.51 (2)	10.00			
J- I	0 / 2051	-28.0	-28.0	0.51 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.30 (3)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.99/1.00 (A-B:1) , BC=0.60/1.00 (K-L:2) , WB=0.46/1.00 (G-I:1) , SSI=0.36/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.59 (A) (INPUT = 1.00)

DWG NO. TAM 17298-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T64	8	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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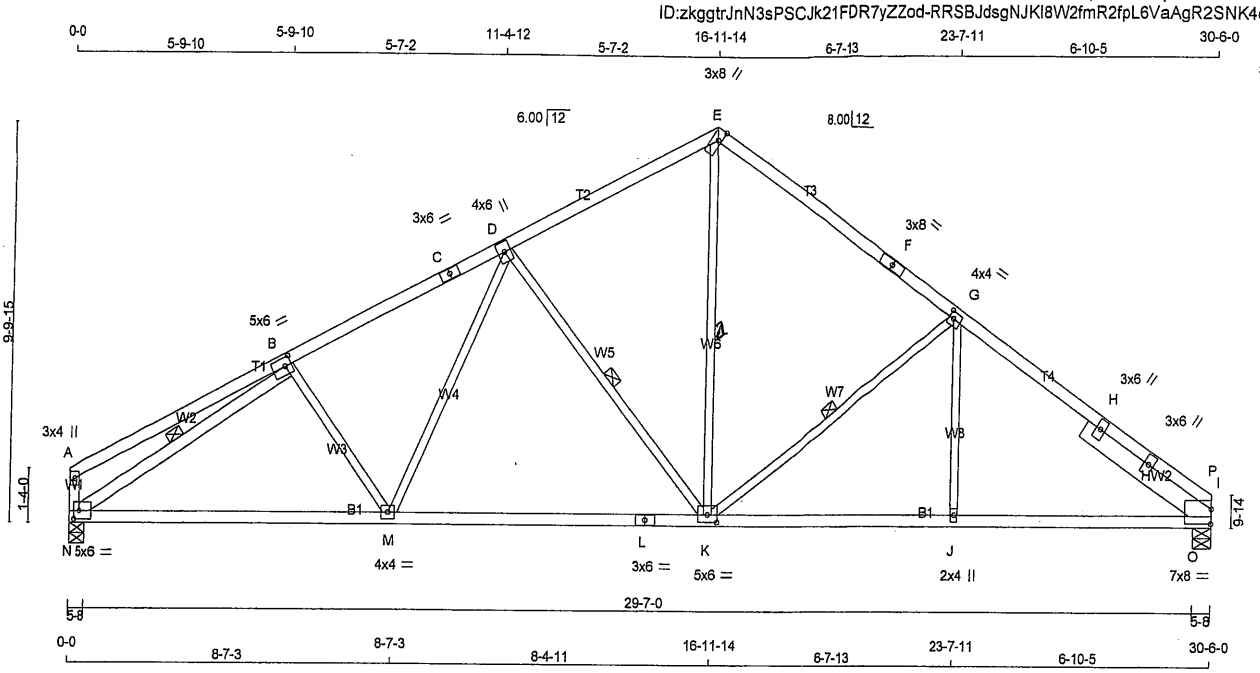

TOTAL WEIGHT = 8 X 125 = 1002 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW2	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0	2.50	2.00
C	TS-t	MT20	3.0	6.0		
D	TMWW+t	MT20	4.0	6.0		
E	TTW+h	MT20	3.0	8.0	Edge	1.00
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMBMR1-I	MT20	7.0	8.0	4.50	
I	RT+t	MT20	3.0	6.0		
I	RT+t	MT20	3.0	6.0		
J	BMW+w	MT20	2.0	4.0		
K	BMWWW-t	MT20	5.0	6.0	2.25	3.00
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMVW1-t	MT20	5.0	6.0	2.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE *TOUCHES EDGE OF CHORD.*

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	2142	0	2142	0	0	5-8	5-8
I	2142	0	2142	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1707	991 / 0	320 / 0	0 / 0	0 / 0	397 / 0	0 / 0
I	1707	991 / 0	320 / 0	0 / 0	0 / 0	397 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K, G-K, B-N. ENG

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 27	-112.4	-112.4	0.47 (1)		10.00
B- C	-2797 / 0	-112.4	-112.4	0.52 (1)		3.70
C- D	-2797 / 0	-112.4	-112.4	0.52 (1)		3.70
D- E	-1947 / 0	-112.4	-112.4	0.47 (1)		4.33
E- F	-2080 / 0	-112.4	-112.4	0.75 (1)		3.79
F- G	-2080 / 0	-112.4	-112.4	0.75 (1)		3.79
G- H	-2142 / 0	-112.4	-112.4	0.61 (1)		4.03
H- P	-2142 / 0	-112.4	-112.4	0.61 (1)		4.03
P- I	-2719 / 0	-112.4	-112.4	0.10 (1)		4.16
N- A	-260 / 0	0.0	0.0	0.03 (1)		7.81
N- M	0 / 2623	-28.0	-28.0	0.78 (2)		10.00
M- L	0 / 2319	-28.0	-28.0	0.75 (2)		10.00
L- K	0 / 2319	-28.0	-28.0	0.75 (2)		10.00
K- J	0 / 2339	-28.0	-28.0	0.56 (2)		10.00
J- O	0 / 2339	-28.0	-28.0	0.53 (1)		10.00
O- I	0 / 1308	-28.0	-28.0	0.41 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- M	-205 / 104	0.08 (1)
M- D	0 / 496	0.11 (2)
D- K	-950 / 0	0.49 (1)
K- E	0 / 1428	0.32 (1)
K- G	-797 / 0	0.39 (1)
J- G	0 / 309	0.07 (3)
N- B	-3173 / 0	0.72 (1)
O- P	0 / 1369	0.00 (1)
O- H	-1432 / 0	0.17 (1)

LICENSED PROFESSIONAL ENGINEER
43118
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1:02")
CALCULATED VERT. DEFL.(TL) = L/ 990 (0.37")

CSI: TC=0.75/1.00 (E-G:1) , BC=0.78/1.00 (M-N:2) , WB=0.72/1.00 (B-N:1) , SSI=0.37/1.00 (I-O:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.78 (B) (INPUT = 1.00)

DWG NO. TAM 17299 -B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G64	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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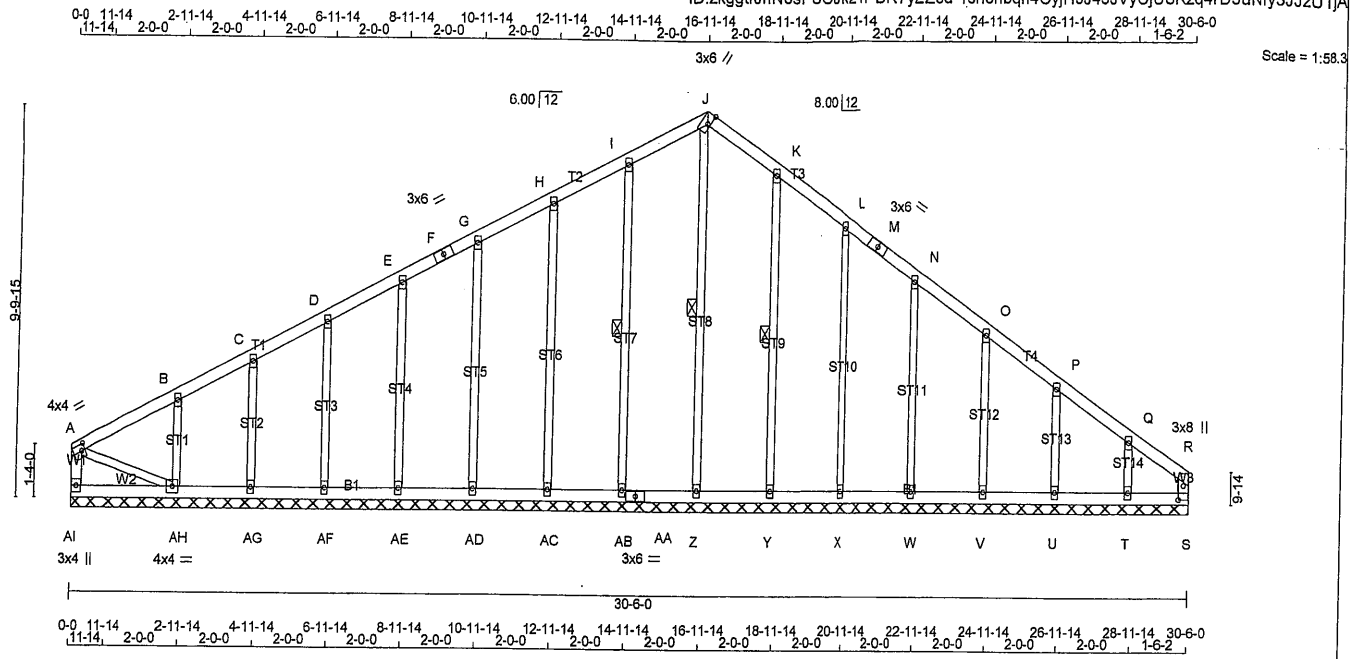


TOTAL WEIGHT = 139 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
M - R	2x4	DRY	No.2	SPF
AI - A	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
AI - AA	2x4	DRY	No.2	SPF
AA- S	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

ALL GABLE WEBS
2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, G, H, I, K, L, N, O, P, Q						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
J	TTW+h	MT20	3.0	6.0	Edge	0.75
M	TS-t	MT20	3.0	6.0		
R						
R	TMBMV1+p	MT20	3.0	8.0	4.25	1.50
S						
T, U, V, W, X, Y, Z, AB, AC, AD, AE, AF, AG						
T	BMW1+w	MT20	2.0	4.0		
AA	BS-t	MT20	3.0	6.0		
AH	BMWV1-t	MT20	4.0	4.0		
AI	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE *TOUCHES EDGE OF CHORD.*

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

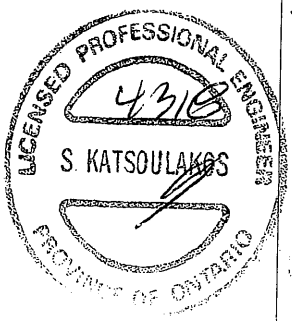
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF J-Z, I-AB, K-Y.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.11 (1)		10.00
B- C	0 / 7	-112.4	-112.4	0.11 (1)		10.00
C- D	0 / 25	-112.4	-112.4	0.06 (1)		10.00
D- E	0 / 26	-112.4	-112.4	0.06 (1)		10.00
E- F	0 / 30	-112.4	-112.4	0.06 (1)		10.00
F- G	0 / 30	-112.4	-112.4	0.06 (1)		10.00
G- H	0 / 32	-112.4	-112.4	0.06 (1)		10.00
H- I	0 / 37	-112.4	-112.4	0.07 (1)		10.00
I- J	0 / 26	-112.4	-112.4	0.07 (1)		10.00
J- K	0 / 24	-112.4	-112.4	0.07 (1)		10.00
K- L	0 / 36	-112.4	-112.4	0.07 (1)		10.00
L- M	0 / 30	-112.4	-112.4	0.06 (1)		10.00
M- N	0 / 30	-112.4	-112.4	0.06 (1)		10.00
N- O	0 / 26	-112.4	-112.4	0.06 (1)		10.00
O- P	0 / 21	-112.4	-112.4	0.06 (1)		10.00
P- Q	0 / 15	-112.4	-112.4	0.05 (1)		10.00
Q- R	0 / 12	-112.4	-112.4	0.05 (1)		10.00
AI- A	-123 / 0	0.0	0.0	0.01 (1)		7.81
S- R	-64 / 0	0.0	0.0	0.01 (1)		7.81
AI-AH	0 / 0	-28.0	-28.0	0.05 (3)		10.00
AH-AG	-17 / 0	-28.0	-28.0	0.05 (3)		6.25
AG-AF	-20 / 0	-28.0	-28.0	0.03 (3)		6.25
AF-AE	-24 / 0	-28.0	-28.0	0.03 (3)		6.25
AE-AD	-26 / 0	-28.0	-28.0	0.02 (3)		6.25
AD-AC	-29 / 0	-28.0	-28.0	0.02 (3)		6.25
AC-AB	-31 / 0	-28.0	-28.0	0.02 (3)		6.25
AB-AA	-33 / 0	-28.0	-28.0	0.02 (3)		6.25
AA- Z	-33 / 0	-28.0	-28.0	0.02 (3)		6.25
Z- Y	-30 / 0	-28.0	-28.0	0.02 (3)		6.25
Y- X	-28 / 0	-28.0	-28.0	0.02 (3)		6.25
X- W	-25 / 0	-28.0	-28.0	0.02 (3)		6.25
W- V	-22 / 0	-28.0	-28.0	0.02 (3)		6.25
V- U	-18 / 0	-28.0	-28.0	0.02 (3)		6.25
U- T	-11 / 0	-28.0	-28.0	0.02 (3)		6.25
T- S	0 / 0	-28.0	-28.0	0.02 (3)		10.00

WEBS MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
Z- J	-223 / 0	0.14 (1)
AB- I	-264 / 0	0.13 (1)
AC- H	-215 / 0	0.24 (1)
AD- G	-225 / 0	0.17 (1)
AE- E	-222 / 0	0.11 (1)
AF- D	-230 / 0	0.08 (1)
AG- C	-191 / 0	0.04 (1)
AH- B	-322 / 0	0.05 (1)
Y- K	-249 / 0	0.12 (1)
X- L	-218 / 0	0.20 (1)
W- N	-224 / 0	0.12 (1)
V- O	-221 / 0	0.07 (1)
U- P	-224 / 0	0.04 (1)
T- Q	-211 / 0	0.03 (1)
A-AH	-9 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.11/1.00 (A-B:1) , BC=0.05/1.00 (AG-AH:3) , WB=0.24/1.00 (H-AC:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (A) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 17315 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T65	9	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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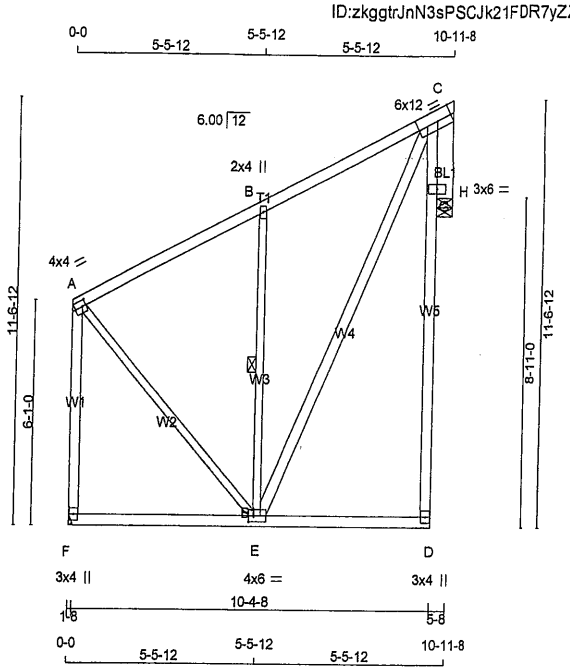


TOTAL WEIGHT = 9 X 75 = 675 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	6.0	12.0	Edge	5.00
D	BMV+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0	2.00	1.50
F	BMV1+p	MT20	3.0	4.0		
G	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H(C)	722	0	722	0	0	5-8	5-8
							(** SEE "BEARING NOTE" **)
F	753	0	753	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	575	334 / 0	108 / 0	0 / 0	0 / 0	134 / 0	0 / 0
F	600	348 / 0	113 / 0	0 / 0	0 / 0	139 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C)
BEARING SIZE FACTOR = 1.15 AT JNT(S) H(C) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

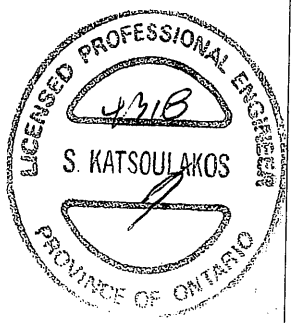
1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-298 / 0	-112.4	-112.4	0.40 (1)	6.25	E- B	-731 / 0	0.37 (1)
B- C	-356 / 0	-112.4	-112.4	0.40 (1)	6.25	E- C	0 / 661	0.11 (1)
D- G	0 / 90	0.0	0.0	0.19 (1)	10.00	A- E	0 / 428	0.10 (1)
G- C	0 / 90	0.0	0.0	0.19 (1)	10.00	C- H	-725 / 0	0.07 (1)
F- A	-692 / 0	0.0	0.0	0.52 (1)	7.81	G- H	0 / 73	0.00 (1)
F- E	0 / 0	-28.0	-28.0	0.24 (3)	10.00			
E- D	0 / 15	-28.0	-28.0	0.24 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.52/1.00 (A-F:1) , BC=0.24/1.00 (D-E:3) , WB=0.37/1.00 (B-E:1) , SSI=0.26/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	322	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (E) (INPUT = 0.90)
JSI METAL= 0.17 (E) (INPUT = 1.00)

DWG NO. TAM 17300 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G65	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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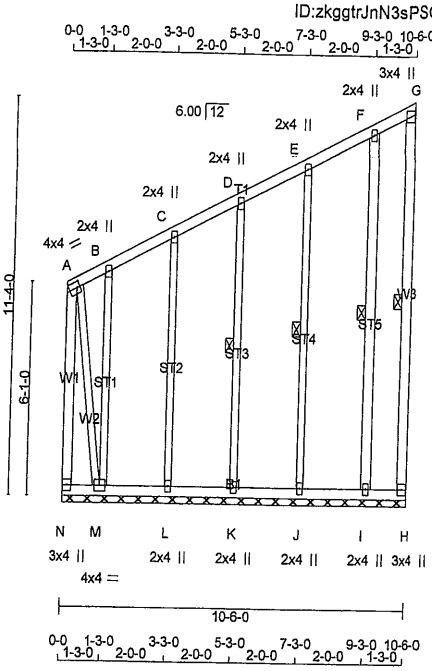

TOTAL WEIGHT = 85 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - F	2x4	DRY	No.2	SPF
ALL GABLE WEBS				
	2x3	DRY	No.2	SPF
EXCEPT				
ST6	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B, C, D, E, F						
B	TMW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-K, E-J, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 7	-112.4	-112.4	0.04 (1)	10.00
B- C	-4 / 0	-112.4	-112.4	0.05 (1)	10.00
C- D	-6 / 0	-112.4	-112.4	0.05 (1)	10.00
D- E	-3 / 0	-112.4	-112.4	0.05 (1)	10.00
E- F	-3 / 0	-112.4	-112.4	0.05 (1)	10.00
F- G	-10 / 0	-112.4	-112.4	0.04 (1)	10.00
H- G	-52 / 0	0.0	0.0	0.04 (1)	6.25
N- A	-61 / 0	0.0	0.0	0.05 (1)	7.81
N- M	0 / 0	-28.0	-28.0	0.02 (3)	10.00
M- L	0 / 7	-28.0	-28.0	0.03 (3)	10.00
L- K	0 / 5	-28.0	-28.0	0.03 (3)	10.00
K- J	0 / 3	-28.0	-28.0	0.02 (3)	10.00
J- I	0 / 1	-28.0	-28.0	0.02 (3)	10.00
I- H	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	-223 / 0	0.11 (1)
L- C	-232 / 0	0.26 (1)
M- B	-196 / 0	0.15 (1)
J- E	-228 / 0	0.14 (1)
I- F	-203 / 0	0.12 (1)
A- M	0 / 15	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1) , BC=0.03/1.00 (L-M:3) , WB=0.26/1.00 (C-L:1) , SSI=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (C) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17316-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T66	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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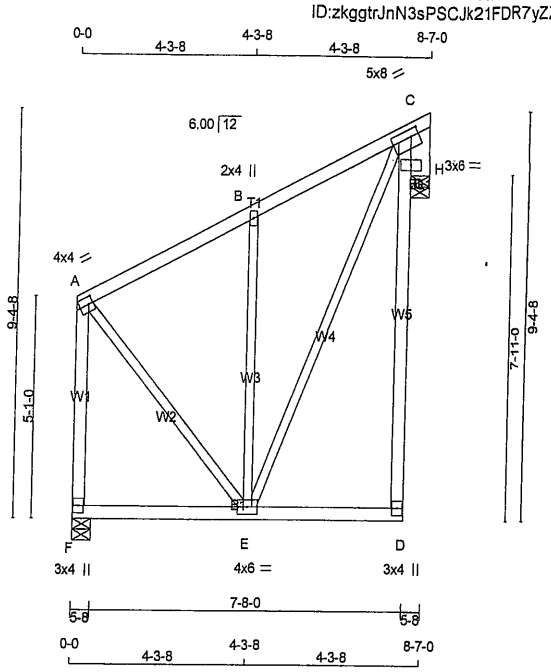

Scale = 1:53.3

TOTAL WEIGHT = 2 X 55 = 110 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	5.0	8.0	2.00	2.25
D	BMV+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	KP-p	MT20	3.0	6.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H(C)	555	0	555	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
F	586	0	586	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	442	257 / 0	83 / 0	0 / 0	0 / 0	103 / 0	0 / 0
F	467	271 / 0	88 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H(C) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-220 / 0	-112.4	-112.4	0.23 (1)	6.25	E- B	-562 / 0	0.52 (1)
B- C	-263 / 0	-112.4	-112.4	0.23 (1)	6.25	E- C	0 / 498	0.11 (1)
D- G	0 / 69	0.0	0.0	0.16 (1)	10.00	A- E	0 / 326	0.07 (1)
G- C	0 / 69	0.0	0.0	0.16 (1)	10.00	C- H	-567 / 0	0.04 (1)
F- A	-537 / 0	0.0	0.0	0.25 (1)	7.81	G- H	0 / 119	0.00 (1)
F- E	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
E- D	0 / 14	-28.0	-28.0	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.25/1.00 (A-F:1) , BC=0.14/1.00 (D-E:3) , WB=0.52/1.00 (B-E:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1367	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

DWG NO. TAM 17301-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T67	4	1	OBC2012_6TCDL TRUSS DESC.	

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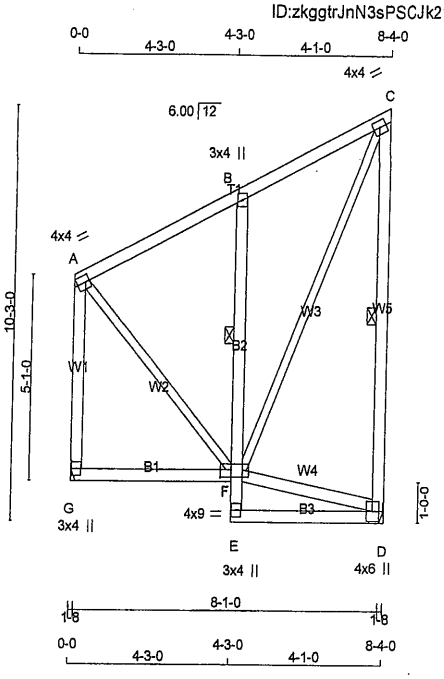


TOTAL WEIGHT = 4 X 62 = 248 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
F - C	2x3	DRY	No.2	SPF
A - F	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMV+p	MT20	3.0	4.0		
F	BVMWWW-I	MT20	4.0	9.0	2.25	3.50
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
D	585	0	585	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	585	0	585	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	466	271 / 0	87 / 0	0 / 0	0 / 0	108 / 0	0 / 0
G	466	271 / 0	87 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D, B-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-211 / 0	-112.4	-112.4	0.24 (1)	6.25	F- D	-11 / 0	0.00 (1)
B- C	-254 / 0	-112.4	-112.4	0.24 (1)	6.25	F- C	0 / 505	0.11 (1)
D- C	-527 / 0	0.0	0.0	0.27 (1)	6.25	A- F	0 / 311	0.07 (1)
G- A	-524 / 0	0.0	0.0	0.24 (1)	7.81			
G- F	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
E- F	0 / 96	0.0	0.0	0.02 (2)	10.00			
F- B	-577 / 0	0.0	0.0	0.04 (1)	6.25			
E- D	0 / 11	-28.0	-28.0	0.14 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.17/1.00 (F-G:3) , WB=0.11/1.00 (C-F:1) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

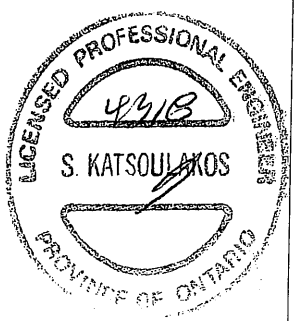
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1087	822	2234	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (F) (INPUT = 0.90)
JSI METAL= 0.15 (F) (INPUT = 1.00)



DWG NO. TAM 17302-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T68	3	1	OBC2012_6TCDL TRUSS DESC.	

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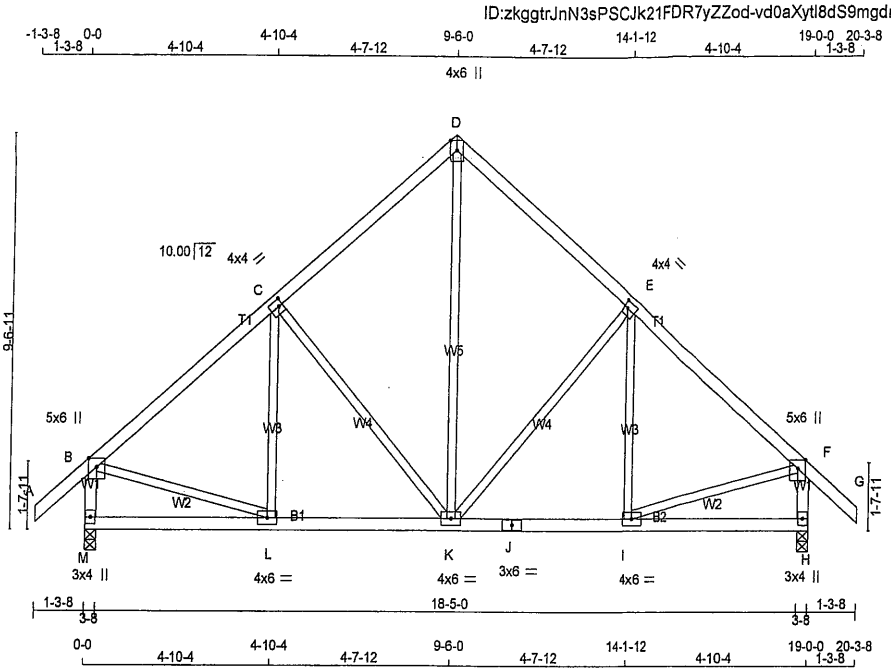

TOTAL WEIGHT = 3 X 91 = 273 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMVW+p	MT20	5.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1490	0	1490	0	0	3-8	3-8
H	1490	0	1490	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1170	707 / 0	200 / 0	0 / 0	0 / 0	264 / 0	0 / 0
H	1170	707 / 0	200 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	K- D	0 / 762	0.17 (1)
B- C	-1221 / 0	-112.4	-112.4	0.35 (1)	5.39	K- E	-423 / 0	0.41 (1)
C- D	-942 / 0	-112.4	-112.4	0.34 (1)	5.96	I- E	-112 / 147	0.06 (1)
D- E	-942 / 0	-112.4	-112.4	0.34 (1)	5.96	C- K	-423 / 0	0.41 (1)
E- F	-1221 / 0	-112.4	-112.4	0.35 (1)	5.39	L- C	-112 / 147	0.06 (1)
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00	B- L	0 / 1003	0.23 (1)
M- B	-1434 / 0	0.0	0.0	0.15 (1)	6.82	I- F	0 / 1003	0.23 (1)
H- F	-1434 / 0	0.0	0.0	0.15 (1)	6.82			
M- L	0 / 0	-28.0	-28.0	0.16 (3)	10.00			
L- K	0 / 969	-28.0	-28.0	0.26 (2)	10.00			
K- J	0 / 969	-28.0	-28.0	0.26 (2)	10.00			
J- I	0 / 969	-28.0	-28.0	0.26 (2)	10.00			
I- H	0 / 0	-28.0	-28.0	0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.26/1.00 (K-L:2) , WB=0.41/1.00 (C-K:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (I) (INPUT = 0.90)
JSI METAL= 0.29 (J) (INPUT = 1.00)

DWG NO. TAM17303-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	G68	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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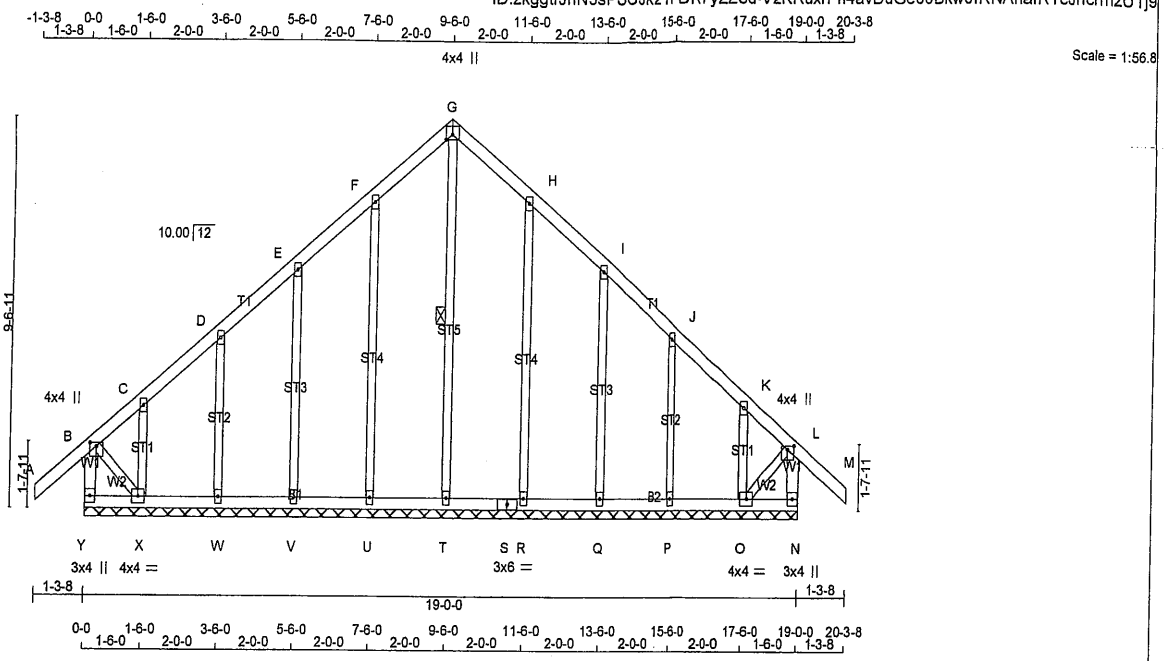


TOTAL WEIGHT = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWV1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-86 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-28 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	-33 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
F- G	-36 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-36 / 0	-112.4	-112.4	0.06 (1)	6.25
H- I	-22 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-33 / 0	-112.4	-112.4	0.06 (1)	6.25
J- K	-28 / 0	-112.4	-112.4	0.06 (1)	6.25
K- L	-86 / 0	-112.4	-112.4	0.15 (1)	6.25
L- M	0 / 50	-112.4	-112.4	0.15 (1)	10.00
Y- B	-358 / 0	0.0	0.0	0.04 (1)	7.81
N- L	-358 / 0	0.0	0.0	0.04 (1)	7.81
Y- X	0 / 0	-28.0	-28.0	0.02 (3)	10.00
X- W	0 / 29	-28.0	-28.0	0.02 (3)	10.00
W- V	0 / 23	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 20	-28.0	28.0	0.02 (3)	10.00
U- T	0 / 17	-28.0	-28.0	0.02 (3)	10.00
T- S	0 / 17	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 17	-28.0	-28.0	0.02 (3)	10.00
R- Q	0 / 20	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 23	-28.0	-28.0	0.02 (3)	10.00
P- O	0 / 29	-28.0	-28.0	0.02 (3)	10.00
O- N	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-162 / 0	0.10 (1)
U- F	-256 / 0	0.30 (1)
V- E	-212 / 0	0.13 (1)
W- D	-240 / 0	0.07 (1)
X- C	-105 / 0	0.02 (1)
R- H	-256 / 0	0.30 (1)
Q- I	-212 / 0	0.13 (1)
P- J	-240 / 0	0.07 (1)
O- K	-105 / 0	0.02 (1)
B- X	0 / 43	0.01 (1)
O- L	0 / 43	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (A-B:1) , BC=0.02/1.00 (W-X:3) , WB=0.30/1.00 (H-R:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

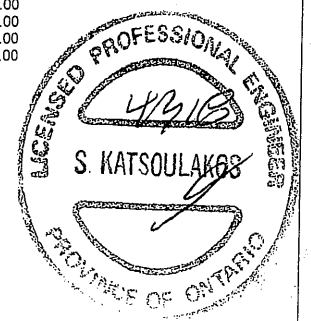
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (G) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)



DWG NO. TAM 12317 -T8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T69	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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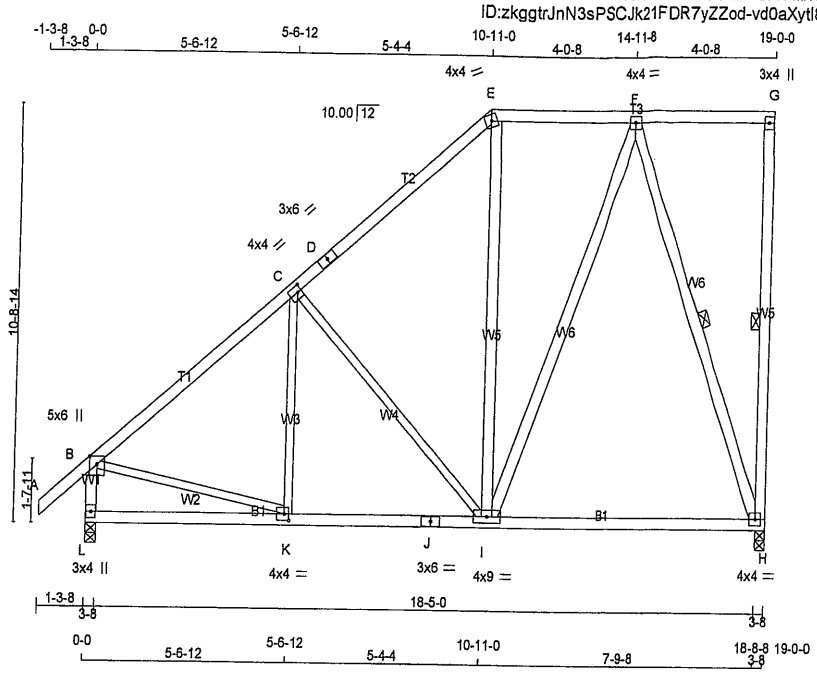

TOTAL WEIGHT = 114 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1334	0	1334	0	0	3-8	3-8
L	1490	0	1490	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1064	617 / 0	200 / 0	0 / 0	0 / 0	247 / 0	0 / 0
L	1170	707 / 0	200 / 0	0 / 0	0 / 0	264 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1205 / 0	-112.4	-112.4	0.47 (1)	5.22
C- D	-835 / 0	-112.4	-112.4	0.45 (1)	6.03
D- E	-835 / 0	-112.4	-112.4	0.45 (1)	6.03
E- F	-604 / 0	-112.4	-112.4	0.24 (1)	6.25
F- G	0 / 0	-112.4	-112.4	0.31 (1)	10.00
H- G	-172 / 0	0.0	0.0	0.10 (1)	6.25
L- B	-1422 / 0	0.0	0.0	0.15 (1)	6.84
L- K	0 / 0	-28.0	-28.0	0.20 (3)	10.00
K- J	0 / 962	-28.0	-28.0	0.48 (2)	10.00
J- I	0 / 962	-28.0	-28.0	0.48 (2)	10.00
I- H	0 / 414	-28.0	-28.0	0.43 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-79 / 160	0.05 (1)
C- I	-530 / 0	0.73 (1)
I- E	0 / 190	0.03 (3)
B- K	0 / 989	0.22 (1)
I- F	0 / 543	0.09 (1)
F- H	-1148 / 0	0.79 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 887 (0.26")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.48/1.00 (I-K:2) , WB=0.79/1.00 (F-H:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (K) (INPUT = 0.90)
JSI METAL= 0.36 (K) (INPUT = 1.00)

DWG NO. TAM 17304-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T70	3	1	OBC2012_6TCDL TRUSS DESC.	

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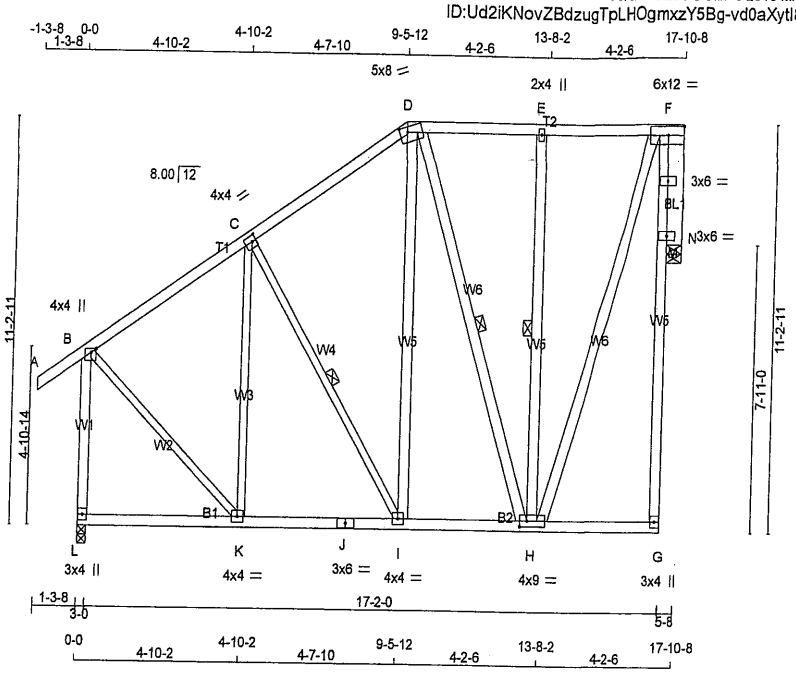

TOTAL WEIGHT = 3 X 135 = 406 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - C	2x3	DRY	No.2	SPF
C - I	2x3	DRY	No.2	SPF
B - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	1.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	2.75
E	TMW+w	MT20	2.0	4.0		
F	TMVWK1-t	MT20	6.0	12.0		Edge
G	BMV+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0	2.00	2.50
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		
M	KP-p	MT20	3.0	6.0		
M	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N(F)	1207	0	1207	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
L	1393	0	1393	0	0	3-0	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N(F)	962	558 / 0	181 / 0	0 / 0	0 / 0	223 / 0	0 / 0
L	1093	662 / 0	185 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N(F), L
BEARING SIZE FACTOR = 1.15 AT JNT(S) N(F) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-I, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-692 / 0	-112.4	-112.4	0.33 (1)	6.25
C- D	-636 / 0	-112.4	-112.4	0.33 (1)	6.25
D- E	-385 / 0	-112.4	-112.4	0.22 (1)	6.25
E- F	-385 / 0	-112.4	-112.4	0.22 (1)	6.25
G- M	0 / 78	0.0	0.0	0.27 (1)	10.00
M- F	0 / 78	0.0	0.0	0.27 (1)	10.00
L- B	-1339 / 0	0.0	0.0	0.57 (1)	7.00
L- K	0 / 0	-28.0	-28.0	0.17 (3)	10.00
K- J	0 / 605	-28.0	-28.0	0.23 (2)	10.00
J- I	0 / 605	-28.0	-28.0	0.23 (2)	10.00
I- H	0 / 501	-28.0	-28.0	0.16 (2)	10.00
H- G	0 / 25	-28.0	-28.0	0.11 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-417 / 14	0.54 (1)
C- I	-206 / 0	0.12 (1)
I- D	0 / 309	0.05 (2)
D- H	-340 / 0	0.26 (1)
H- E	-549 / 0	0.36 (1)
H- F	0 / 1054	0.17 (1)
B- K	0 / 833	0.19 (1)
F- N	-1210 / 0	0.17 (1)
M- N	0 / 87	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (B-L:1) , BC=0.23/1.00 (I-K:2) , WB=0.54/1.00 (C-K:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (B) (INPUT = 0.90)
JSI METAL= 0.23 (K) (INPUT = 1.00)

DWG NO. TAM 17305-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T71	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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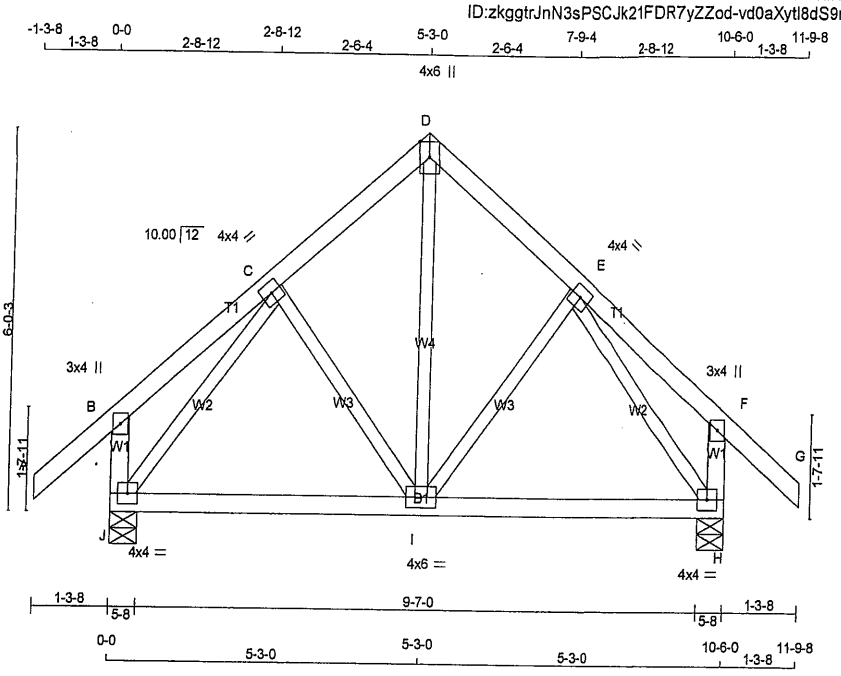

TOTAL WEIGHT = 52 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	893	0	893	0	0	5-8	5-8
H	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	694	431 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	694	431 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 21	-112.4	-112.4	0.12 (1)	10.00
C- D	-484 / 0	-112.4	-112.4	0.09 (1)	6.25
D- E	-484 / 0	-112.4	-112.4	0.09 (1)	6.25
E- F	0 / 21	-112.4	-112.4	0.12 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
J- B	-269 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-269 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 424	-28.0	-28.0	0.27 (3)	10.00
I- H	0 / 424	-28.0	-28.0	0.27 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 358	0.08 (1)
I- E	-112 / 30	0.04 (1)
C- I	-112 / 30	0.04 (1)
J- C	-708 / 0	0.21 (1)
E- H	-708 / 0	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.27/1.00 (H-I:3) , WB=0.21/1.00 (E-H:1) , SSI=0.13/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

DWG NO. TAM 17306-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T71Z	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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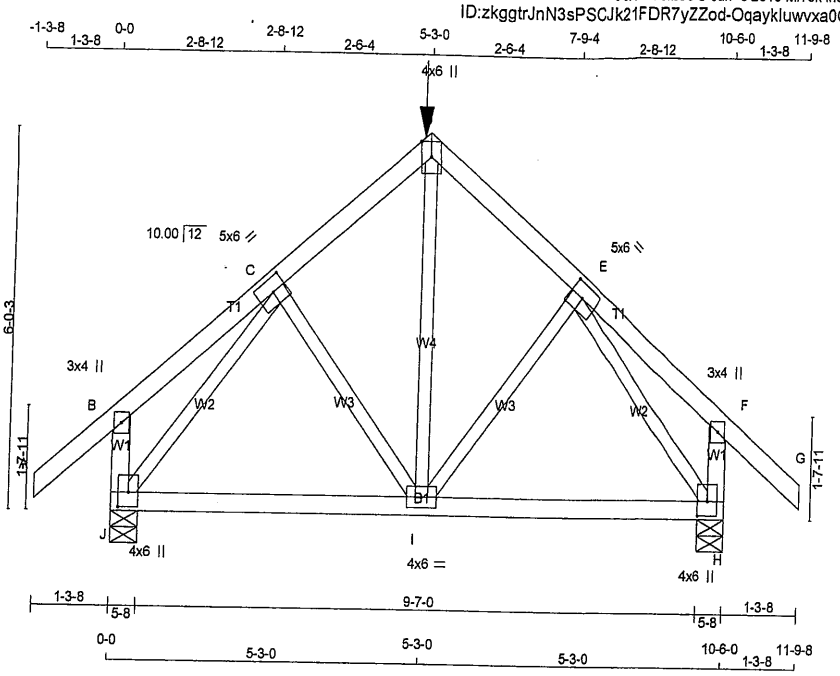

Scale = 1:36.5

TOTAL WEIGHT = 52 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0	2.50	2.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1+p	MT20	4.0	6.0	2.75	2.00
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1+p	MT20	4.0	6.0	2.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 737.8 lbs FACTORED DOWN AT 5-3-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1364	0	1364	0	0	5-8	5-8
H	1364	0	1364	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1075	644 / 0	187 / 0	0 / 0	0 / 0	244 / 0	0 / 0
H	1075	644 / 0	187 / 0	0 / 0	0 / 0	244 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.17	(1)	10.00	I- D	0 / 432	0.11 (3)
B- C	0 / 19	-112.4	-112.4	0.12	(1)	10.00	I- E	0 / 78	0.02 (3)
C- D	-1002 / 0	-112.4	-112.4	0.13	(1)	6.07	C- I	0 / 78	0.02 (3)
D- E	-1002 / 0	-112.4	-112.4	0.13	(1)	6.07	J- C	-1245 / 0	0.40 (1)
E- F	0 / 19	-112.4	-112.4	0.12	(1)	10.00	E- H	-1245 / 0	0.40 (1)
F- G	0 / 50	-112.4	-112.4	0.17	(1)	10.00			
J- B	-272 / 0	0.0	0.0	0.03	(1)	7.81			
H- F	-272 / 0	0.0	0.0	0.03	(1)	7.81			
J- I	0 / 746	-47.5	-47.5	0.50	(3)	10.00			
I- H	0 / 746	-47.5	-47.5	0.50	(3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-3-0	-738	-738	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-3-0
END SETBACK = 5-3-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.50/1.00 (I-J:3) , WB=0.40/1.00 (E-H:1) , SSI=0.25/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17307-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287667	T72	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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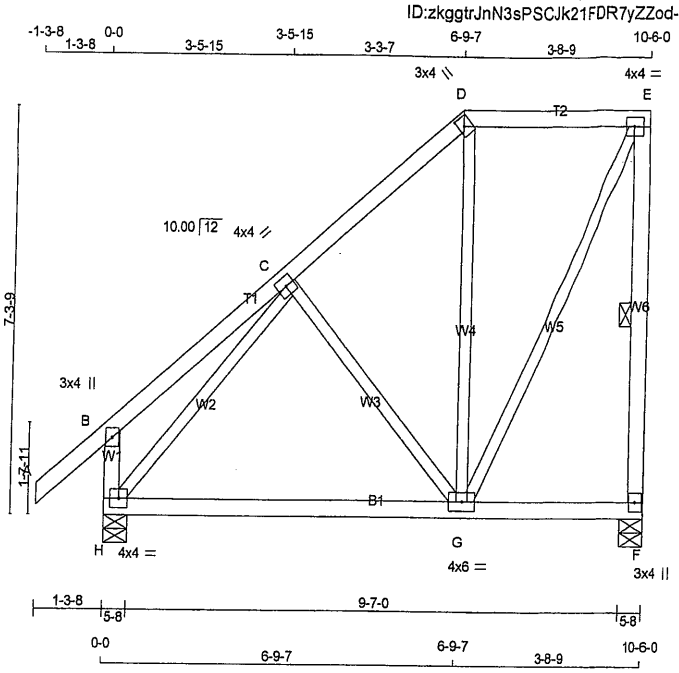

Scale = 1:41.6

TOTAL WEIGHT = 57 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	737	0	737	0	0	5-8	5-8
H	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	588	341 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
H	694	431 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 29	-112.4	-112.4	0.21 (1)	10.00
C- D	-376 / 0	-112.4	-112.4	0.16 (1)	6.25
D- E	-264 / 0	-112.4	-112.4	0.20 (1)	6.25
F- E	-719 / 0	0.0	0.0	0.17 (1)	6.25
H- B	-299 / 0	0.0	0.0	0.03 (1)	7.81
H- G	0 / 421	-28.0	-28.0	0.33 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-239 / 0	0.12 (1)
G- D	-135 / 71	0.13 (1)
G- E	0 / 575	0.13 (1)
H- C	-665 / 0	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 875 (0.14")

CSI: TC=0.21/1.00 (B-C:1) , BC=0.33/1.00 (G-H:3) , WB=0.30/1.00 (C-H:1) , SSI=0.17/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (G) (INPUT = 0.90)
JSI METAL= 0.24 (C) (INPUT = 1.00)

DWG NO. TAM 17306-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T73	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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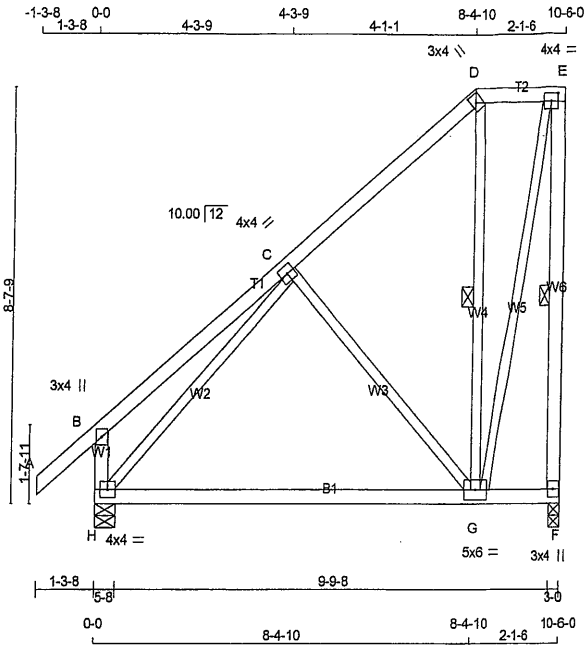

Scale: 1/4"=1'

TOTAL WEIGHT = 63 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	4.0	2.00	1.75
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0		
H	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	737	0	737	0	0	3-0	3-0
H	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	588	341 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
H	694	431 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	0 / 37	-112.4	-112.4	0.33 (1)		10.00
C- D	-255 / 0	-112.4	-112.4	0.25 (1)		6.25
D- E	-165 / 0	-112.4	-112.4	0.08 (1)		6.25
F- E	-803 / 0	0.0	0.0	0.27 (1)		6.25
H- B	-332 / 0	0.0	0.0	0.04 (1)		7.81
H- G	0 / 405	-28.0	-28.0	0.53 (3)		10.00
G- F	0 / 0	-28.0	-28.0	0.51 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- G	-354 / 0	0.26 (1)
G- D	-160 / 48	0.08 (1)
G- E	0 / 705	0.16 (1)
H- C	-616 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 684 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 410 (0.31")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.53/1.00 (G-H:3) , WB=0.43/1.00 (C-H:1) , SSI=0.21/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (G) (INPUT = 0.90)
JSI METAL= 0.22 (C) (INPUT = 1.00)

DWG NO. TAM 17309-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T74	1	1	OBC2012_6TCDL TRUSS DESC.	

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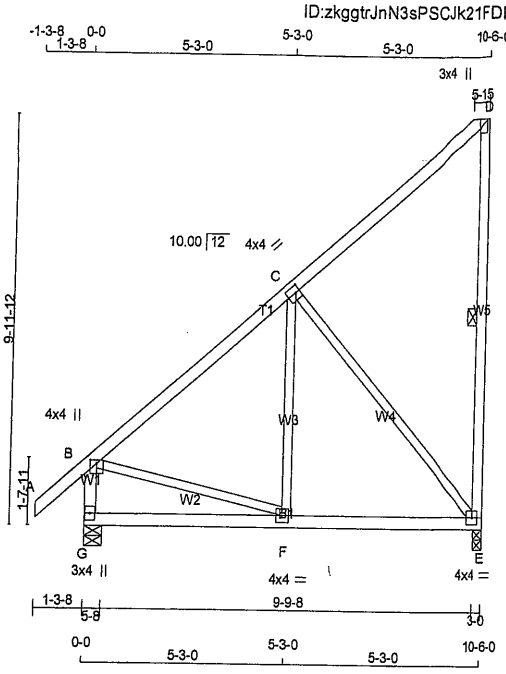

Scale = 1:56.5

TOTAL WEIGHT = 56 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMV+p	MT20	3.0	4.0	Edge	0.25
E	BMVW1-t	MT20	4.0	4.0		
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	737	0	737	0	0	3-0	3-0
G	893	0	893	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	588	341 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
G	694	431 / 0	110 / 0	0 / 0	0 / 0	153 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-505 / 0	-112.4	-112.4	0.41 (1)	6.25
C- D	-47 / 0	-112.4	-112.4	0.40 (1)	6.25
E- D	-222 / 0	0.0	0.0	0.12 (1)	6.25
G- B	-836 / 0	0.0	0.0	0.09 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.24 (3)	10.00
F- E	0 / 424	-28.0	-28.0	0.27 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 272	0.07 (3)
C- E	-624 / 0	0.73 (1)
B- F	0 / 437	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.27/1.00 (E-F:3) , WB=0.73/1.00 (C-E:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

DWG NO. TAM 173/0 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289222	T91	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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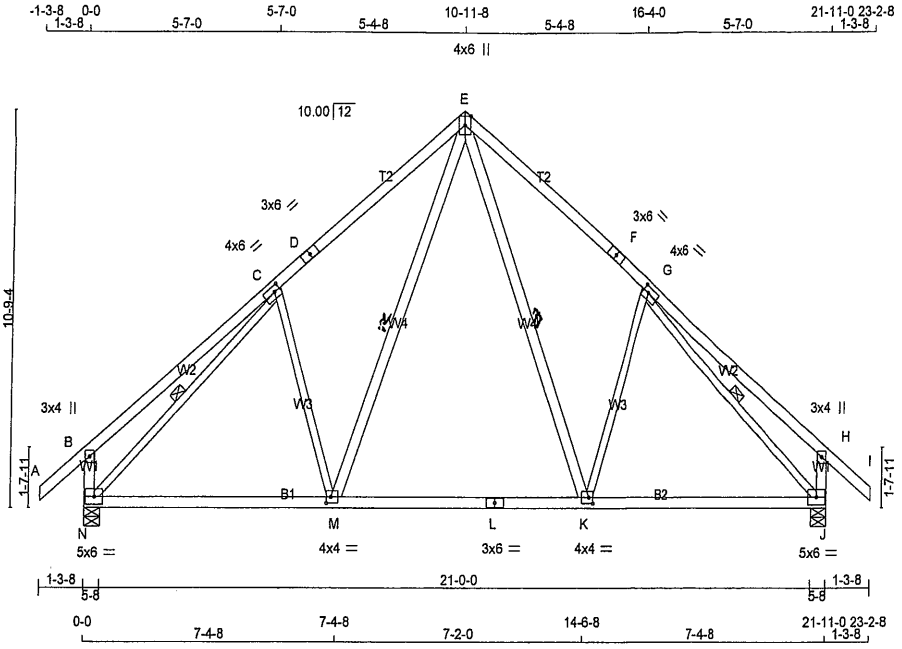


TOTAL WEIGHT = 2 X 111 = 222 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
K - G	2x3	DRY	No.2	SPF
C - M	2x3	DRY	No.2	SPF
N - C	2x3	DRY	No.2	SPF
G - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TS-t	MT20	3.0	6.0		
E	TTWW+p	MT20	4.0	6.0	Edge	
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	6.0	2.00	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.50
N	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1695	0	1695	0	0	5-8	5-8
J	1695	0	1695	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1333	802 / 0	230 / 0	0 / 0	0 / 0	302 / 0	0 / 0
J	1333	802 / 0	230 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

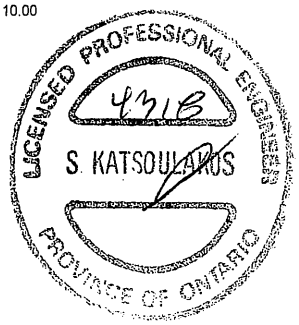
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J. EMIE-K

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 48	-112.4	-112.4	0.56 (1)	10.00
C- D	-1369 / 0	-112.4	-112.4	0.48 (1)	4.97
D- E	-1369 / 0	-112.4	-112.4	0.48 (1)	4.97
E- F	-1369 / 0	-112.4	-112.4	0.48 (1)	4.97
F- G	-1369 / 0	-112.4	-112.4	0.48 (1)	4.97
G- H	0 / 48	-112.4	-112.4	0.56 (1)	10.00
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
N- B	-387 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-387 / 0	0.0	0.0	0.04 (1)	7.81
N- M	0 / 1141	-28.0	-28.0	0.47 (2)	10.00
M- L	0 / 804	-28.0	-28.0	0.44 (2)	10.00
L- K	0 / 804	-28.0	-28.0	0.44 (2)	10.00
K- J	0 / 1141	-28.0	-28.0	0.47 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- K	0 / 645	0.10 (1)
K- G	-406 / 14	0.28 (1)
M- E	0 / 645	0.10 (1)
C- M	-406 / 14	0.28 (1)
N- C	-1674 / 0	0.71 (1)
G- J	-1674 / 0	0.71 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.19")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.47/1.00 (M-N:2) , WB=0.71/1.00 (G-J:1) ; SSI=0.23/1.00 (C-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1367	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

DWG NO. TAM 1731B-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289222	G91	1	1	OBC2012_6TCDL TRUSS DESC.	

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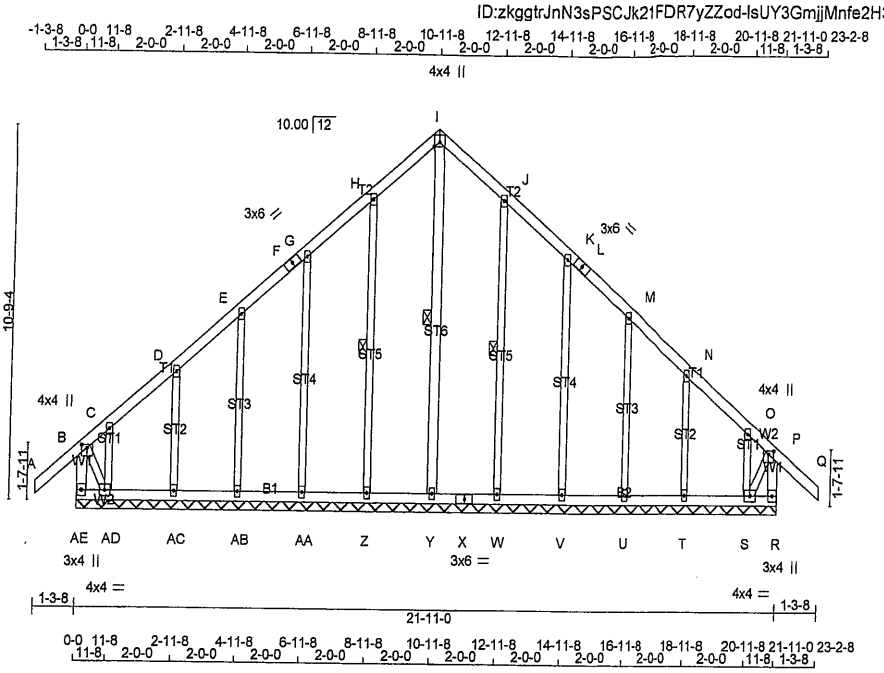

TOTAL WEIGHT = 121 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
L - Q	2x4	DRY	No.2	SPF
AE- B	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
AE- X	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Y - I	2x4	DRY	No.2	SPF
ALL GABLE WEBS EXCEPT	2x3	DRY	No.2	SPF
ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, J, K, M, N, O						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
I	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TS-t	MT20	3.0	6.0		
P	TMVW+p	MT20	4.0	4.0	1.00	2.00
R	BMV1+p	MT20	3.0	4.0		
S	BMWW1-t	MT20	4.0	4.0		
T, U, V, W, Y, Z, AA, AB, AC						
T	BMW1+w	MT20	2.0	4.0		
X	BS-t	MT20	3.0	6.0		
AD	BMWW1-t	MT20	4.0	4.0		
AE	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-Y, H-Z, J-W.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-94 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-16 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	-18 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
F- G	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
G- H	-5 / 0	-112.4	-112.4	0.06 (1)	10.00
H- I	-18 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-18 / 0	-112.4	-112.4	0.06 (1)	6.25
J- K	-5 / 0	-112.4	-112.4	0.06 (1)	10.00
K- L	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
L- M	-12 / 0	-112.4	-112.4	0.05 (1)	6.25
M- N	-18 / 0	-112.4	-112.4	0.06 (1)	6.25
N- O	-16 / 0	-112.4	-112.4	0.06 (1)	6.25
O- P	-94 / 0	-112.4	-112.4	0.15 (1)	6.25
P- Q	0 / 50	-112.4	-112.4	0.15 (1)	10.00
AE- B	-354 / 0	0.0	0.0	0.04 (1)	7.81
R- P	-354 / 0	0.0	0.0	0.04 (1)	7.81
AE-AD	0 / 0	-28.0	-28.0	0.02 (3)	10.00
AD-AC	0 / 19	-28.0	-28.0	0.03 (3)	10.00
AC-AB	0 / 13	-28.0	-28.0	0.03 (3)	10.00
AB-AA	0 / 9	-28.0	-28.0	0.02 (3)	10.00
AA- Z	0 / 6	-28.0	-28.0	0.02 (3)	10.00
Z- Y	0 / 3	-28.0	-28.0	0.02 (3)	10.00
Y- X	0 / 3	-28.0	-28.0	0.02 (3)	10.00
X- W	0 / 3	-28.0	-28.0	0.02 (3)	10.00
W- V	0 / 6	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 9	-28.0	-28.0	0.02 (3)	10.00
U- T	0 / 13	-28.0	-28.0	0.03 (3)	10.00
T- S	0 / 19	-28.0	-28.0	0.03 (3)	10.00
S- R	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- I	-186 / 0	0.11 (1)
Z- H	-255 / 0	0.14 (1)
AA- G	-218 / 0	0.22 (1)
AB- E	-220 / 0	0.11 (1)
AC- D	-235 / 0	0.06 (1)
AD- C	-42 / 0	0.01 (1)
W- J	-255 / 0	0.14 (1)
V- K	-218 / 0	0.22 (1)
U- M	-220 / 0	0.11 (1)
T- N	-235 / 0	0.06 (1)
S- O	-42 / 0	0.01 (1)
B-AD	0 / 35	0.01 (1)
S- P	0 / 35	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (P-Q:1) , BC=0.03/1.00 (AC-AD:3) , WB=0.22/1.00 (K-V:1) , SSI=0.09/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT *RESPONSIBLE FOR QUALITY CONTROL IN* THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (I) (INPUT = 0.90)
JSI METAL= 0.07 (J) (INPUT = 1.00)

DWG NO. TAM 17320-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T100	5	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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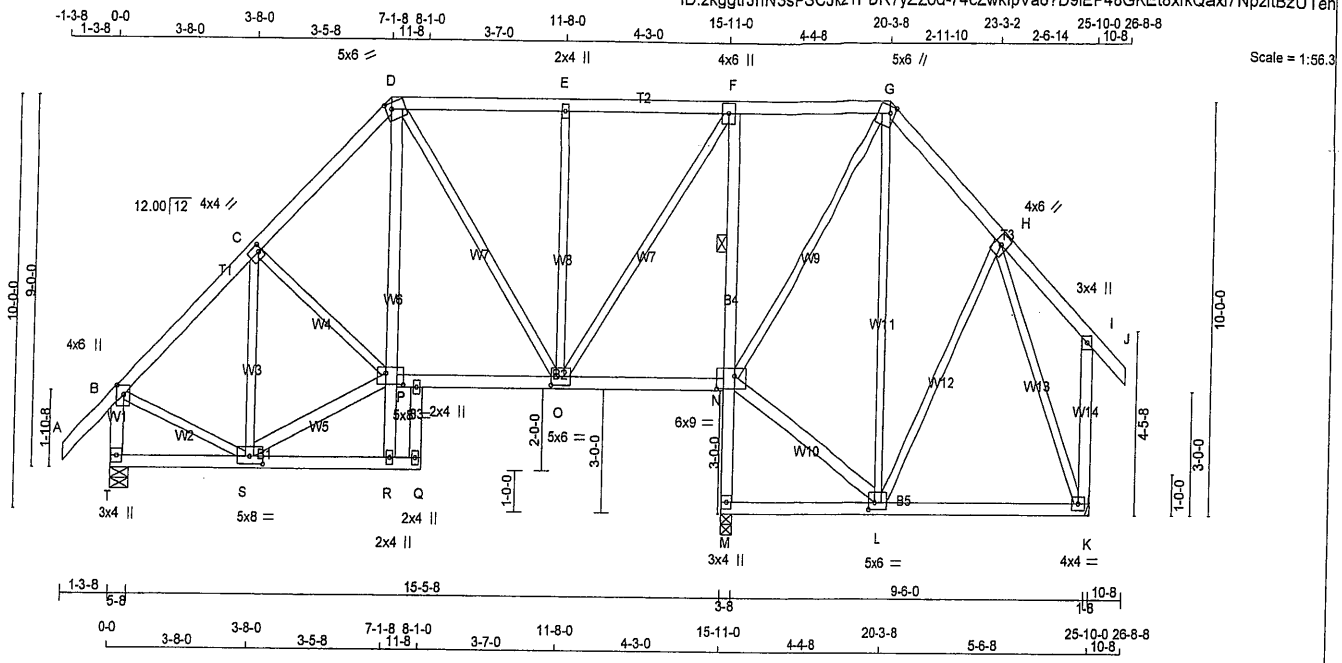

TOTAL WEIGHT = 5 X 162 = 808 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
M - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - D	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	5.0	6.0	1.75	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMWW+t	MT20	4.0	6.0		
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	4.0	4.0		
L	BMWWW-t	MT20	5.0	6.0	2.00	2.00
M	BMV1+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	9.0	3.75	5.50
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BWMWW*-I	MT20	5.0	8.0	3.25	5.50
Q	NP+w	MT20	2.0	4.0		
R	BMW+w	MT20	2.0	4.0		
S	BMWWW-t	MT20	5.0	8.0	2.25	4.00
T	BMV1+p	MT20	3.0	4.0		
U	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1304	0	1304	0	0	5-8	5-8
M	1805	0	1805	0	0	3-8	3-8
K	808	0	808	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1032	611 / 0	185 / 0	0 / 0	0 / 0	236 / 0	0 / 0
M	1432	842 / 0	261 / 0	0 / 0	0 / 0	329 / 0	0 / 0
K	639	379 / 0	114 / 0	0 / 0	0 / 0	146 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 5.55 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-112.4	-112.4	0.16 (1)	10.00
B- C	-906 / 0	-112.4	-112.4	0.25 (1)	6.15
C- D	-1014 / 0	-112.4	-112.4	0.26 (1)	5.89
D- E	-596 / 0	-112.4	-112.4	0.30 (1)	6.25
E- F	-597 / 0	-112.4	-112.4	0.30 (1)	6.25
F- G	-51 / 0	-112.4	-112.4	0.29 (1)	6.25
G- H	-286 / 0	-112.4	-112.4	0.15 (1)	6.25
H- I	0 / 31	-112.4	-112.4	0.15 (1)	10.00
I- J	0 / 39	-112.4	-112.4	0.08 (1)	10.00
T- B	-1266 / 0	0.0	0.0	0.14 (1)	7.16
K- I	-201 / 0	0.0	0.0	0.07 (1)	7.81
T- S	0 / 0	-28.0	-28.0	0.11 (3)	10.00
S- R	0 / 5	-28.0	-28.0	0.11 (3)	10.00
R- Q	0 / 0	-28.0	-28.0	0.02 (3)	10.00
P- O	0 / 702	-28.0	-28.0	0.23 (2)	10.00
O- N	0 / 51	-28.0	-28.0	0.17 (3)	10.00
M- N	-1766 / 0	0.0	0.0	0.04 (1)	6.28
N- F	-1369 / 0	0.0	0.0	0.10 (1)	5.55
M- L	0 / 0	-28.0	-28.0	0.22 (3)	10.00
L- K	0 / 200	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- P	0 / 111	0.03 (2)
P- D	0 / 494	0.09 (1)
N- L	0 / 231	0.04 (1)
N- G	-248 / 0	0.31 (1)
L- G	0 / 164	0.04 (3)
O- E	-546 / 0	0.46 (1)
D- O	-188 / 0	0.25 (1)
O- F	0 / 997	0.22 (1)
L- H	-36 / 64	0.04 (1)
H- K	-579 / 0	0.53 (1)
S- C	-538 / 0	0.25 (1)
B- S	0 / 723	0.16 (1)
S- P	0 / 770	0.12 (1)
C- P	0 / 23	0.01 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.23/1.00 (O-P:2) , WB=0.53/1.00 (H-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.24 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4316 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17326-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T200	5	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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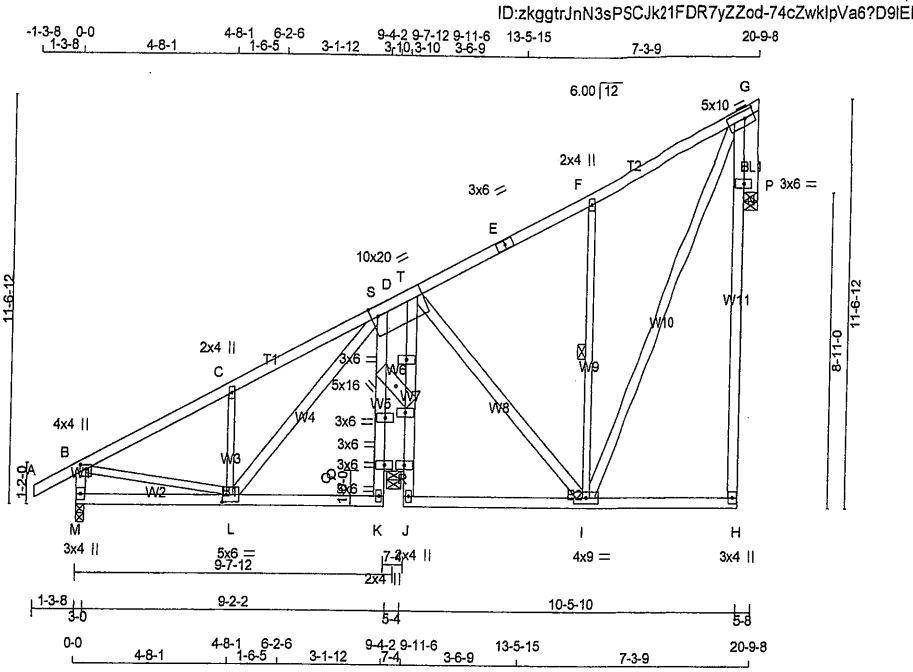

TOTAL WEIGHT = 5 X 134 = 669 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - F	2x3	DRY	No.2	SPF
L - C	2x3	DRY	No.2	SPF
B - L	2x3	DRY	No.2	SPF
D - O	2 - 2x4	DRY	No.2	SPF
D - O	2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.50	2.00
C	TMW+w	MT20	2.0	4.0		
D	TMWQQWW-t	MT20	10.0	20.0	Edge	2.25
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMVWK1-t	MT20	5.0	10.0	2.50	3.75
H	BMV+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMW+w	MT20	2.0	4.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	1.50
M	BMV1+p	MT20	3.0	4.0		
N	KP-p	MT20	3.0	6.0		
O						
O, O, O, O, O						
O	QP+t	MT20	3.0	6.0		
O	WMQMQM1*+r	MT20	5.0	16.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P(G)	667	0	667	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
M	754	0	754	0	0	3-0	3-0
O	1562	0	1562	0	0	5-4	5-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P(G)	534	307 / 0	102 / 0	0 / 0	0 / 0	125 / 0	0 / 0
M	586	364 / 0	92 / 0	0 / 0	0 / 0	129 / 0	0 / 0
O	1230	737 / 0	214 / 0	0 / 0	0 / 0	279 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P(G), M, O
BEARING SIZE FACTOR = 1.15 AT JNT(S) P(G), O (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-537 / 0	-112.4	-112.4	0.35 (1)	6.25
C- S	-537 / 0	-112.4	-112.4	0.35 (1)	6.25
S- D	0 / 114	-112.4	-112.4	0.23 (1)	10.00
D- T	0 / 225	-112.4	-112.4	0.33 (1)	10.00
T- E	-272 / 0	-112.4	-112.4	0.42 (1)	6.25
E- F	-272 / 0	-112.4	-112.4	0.42 (1)	6.25
F- G	-301 / 0	-112.4	-112.4	0.42 (1)	6.25
H- N	0 / 86	0.0	0.0	0.17 (1)	10.00
N- G	0 / 86	0.0	0.0	0.17 (1)	10.00
M- B	-700 / 0	0.0	0.0	0.07 (1)	7.81
J- I	-27 / 0	-28.0	-28.0	0.23 (3)	6.25
I- H	0 / 14	-28.0	-28.0	0.23 (3)	10.00
M- L	0 / 0	-28.0	-28.0	0.18 (3)	10.00
L- K	-47 / 0	-28.0	-28.0	0.18 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	-652 / 0	0.34 (1)
I- G	0 / 575	0.09 (1)
T- I	0 / 410	0.07 (1)
L- C	-576 / 0	0.12 (1)
B- L	0 / 510	0.11 (1)
L- S	0 / 825	0.13 (1)
G- P	-671 / 0	0.06 (1)
N- F	0 / 66	0.00 (1)
D- O	-95 / 197	0.01 (3)
K- Q	0 / 81	0.06 (1)
S- Q	0 / 81	0.06 (1)
J- R	0 / 103	0.04 (2)
T- R	0 / 103	0.04 (2)
S- O	-713 / 0	0.00 (1)
T- O	-1053 / 0	0.00 (1)
Q- O	-56 / 0	0.00 (1)
R- O	-32 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.42/1.00 (F-T:1) , BC=0.23/1.00 (H-I:3) , WB=0.34/1.00 (F-I:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (D) (INPUT = 0.90)
JSI METAL= 0.26 (E) (INPUT = 1.00)

DWG NO. TAM17327-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T300	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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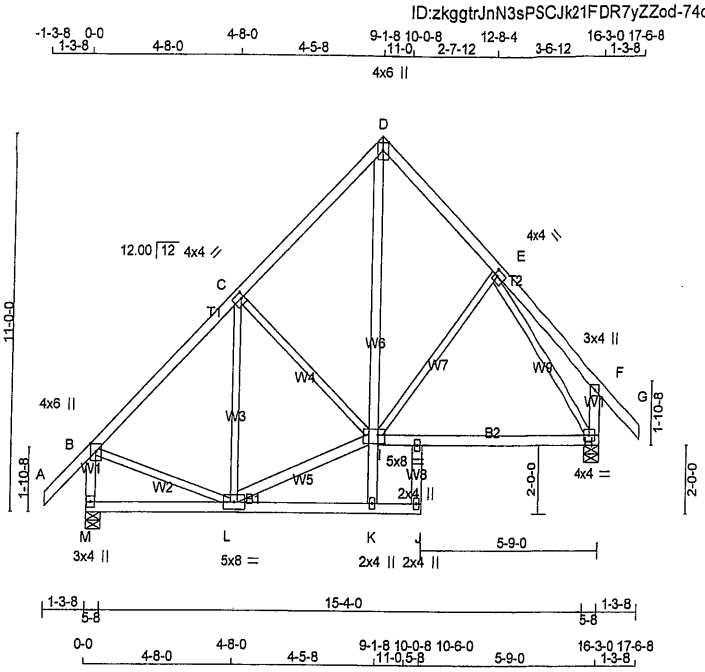

TOTAL WEIGHT = 3 X 96 = 288 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - D	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTW+p	MT20	4.0	6.0	2.75	2.00
E	TMWW-t	MT20	4.0	4.0	2.00	1.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BWMWWW*-I	MT20	5.0	8.0	3.00	2.50
J	NP+w	MT20	2.0	4.0		
K	BMW+w	MT20	2.0	4.0		
L	BMWWW-t	MT20	5.0	8.0		
M	BMV1+p	MT20	3.0	4.0		
N	NP+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1317	0	1317	0	0	5-8	5-8
H	1321	0	1321	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1041	618 / 0	185 / 0	0 / 0	0 / 0	237 / 0	0 / 0
H	1046	618 / 0	188 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 55	-112.4	-112.4	0.16 (1)	10.00	L- C	-381 / 16	0.26 (1)
B- C	-918 / 0	-112.4	-112.4	0.32 (1)	6.04	K- I	0 / 158	0.03 (2)
C- D	-829 / 0	-112.4	-112.4	0.31 (1)	6.25	I- D	0 / 747	0.13 (1)
D- E	-800 / 0	-112.4	-112.4	0.18 (1)	6.25	B- L	0 / 718	0.16 (1)
E- F	0 / 34	-112.4	-112.4	0.23 (1)	10.00	I- E	-149 / 35	0.10 (1)
F- G	0 / 55	-112.4	-112.4	0.16 (1)	10.00	E- H	-1117 / 0	0.69 (1)
M- B	-1268 / 0	0.0	0.0	0.14 (1)	7.15	L- I	0 / 750	0.12 (1)
H- F	-301 / 0	0.0	0.0	0.03 (1)	7.81	C- I	-189 / 0	0.12 (1)
M- L	0 / 0	-28.0	-28.0	0.17 (3)	10.00			
L- K	0 / 5	-28.0	-28.0	0.17 (3)	10.00			
K- J	0 / 0	-28.0	-28.0	0.08 (3)	10.00			
I- H	0 / 635	-28.0	-28.0	0.51 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 627 (0.31")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.51/1.00 (H-I:3) , WB=0.69/1.00 (E-H:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.42 (E) (INPUT = 1.00)

DWG NO. TAM 17328-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T400	5	1	OBC2012_6TCDL TRUSS DESC.	

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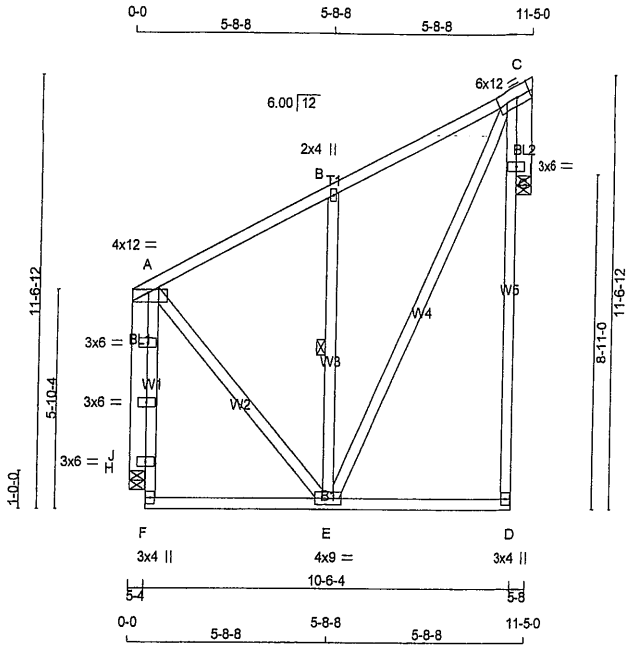

TOTAL WEIGHT = 5 X 89 = 445 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL2	2x6	DRY	No.2	SPF
BL1	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVWK1-p	MT20	4.0	12.0	1.00	Edge
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	6.0	12.0	Edge	5.00
D	BMV+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	9.0		
F	BMV+p	MT20	3.0	4.0		
G, H, H						
G	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I(C)	738	0	738	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
J(A)	737	0	737	0	0	5-4	5-4 (** SEE "BEARING NOTE" **)

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I(C)	588	341 / 0	110 / 0	0 / 0	0 / 0	137 / 0	0 / 0
J(A)	587	341 / 0	110 / 0	0 / 0	0 / 0	136 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I(C), J(A)
BEARING SIZE FACTOR = 1.15 AT JNT(S) I(C), J(A) (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
A- B	-313 / 0	-112.4	-112.4	0.40 (1)	6.25	E- B	-731 / 0	0.26 (1)	
B- C	-371 / 0	-112.4	-112.4	0.40 (1)	6.25	E- C	0 / 671	0.11 (1)	
D- G	0 / 97	0.0	0.0	0.19 (1)	10.00	A- E	0 / 415	0.07 (1)	
G- C	0 / 97	0.0	0.0	0.19 (1)	10.00	C- I	-742 / 0	0.07 (1)	
F- H	0 / 97	0.0	0.0	0.04 (1)	10.00	A- J	-737 / 0	0.25 (1)	
H- A	0 / 97	0.0	0.0	0.04 (1)	10.00	G- I	0 / 75	0.00 (1)	
						H- J	0 / 36	0.00 (1)	
F- E	0 / 31	-28.0	-28.0	0.24 (3)	10.00				
E- D	0 / 15	-28.0	-28.0	0.24 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.24/1.00 (E-F:3) , WB=0.26/1.00 (B-E:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	613	354	1607	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.59 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM 12329-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T500	5	1	OBC2012_6TCDL TRUSS DESC.	

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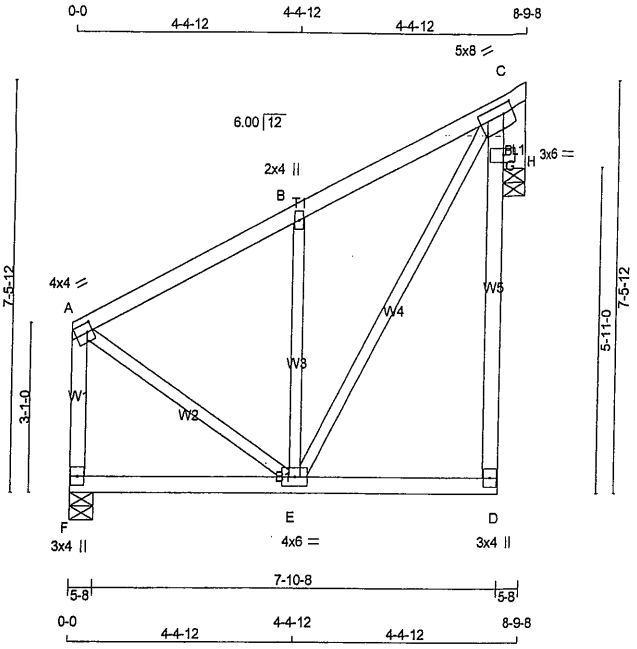

Scale = 1:42.2

TOTAL WEIGHT = 5 X 47 = 237 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	2.00	1.25
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	5.0	8.0	2.00	2.25
D	BMV+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	KP-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H(C)	569	0	569	0	0	5-8	5-8 (** SEE "BEARING NOTE" **)
F	601	0	601	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	454	263 / 0	85 / 0	0 / 0	0 / 0	105 / 0	0 / 0
F	479	278 / 0	90 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H(C) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-332 / 0	-112.4	-112.4	0.24	(1)	6.25	E- B	-576 / 0	0.24 (1)
B- C	-374 / 0	-112.4	-112.4	0.24	(1)	6.25	E- C	0 / 593	0.13 (1)
D- G	0 / 71	0.0	0.0	0.15	(1)	10.00	A- E	0 / 377	0.08 (1)
G- C	0 / 71	0.0	0.0	0.15	(1)	10.00	C- H	-580 / 0	0.04 (1)
F- A	-551 / 0	0.0	0.0	0.09	(1)	7.81	G- H	0 / 111	0.00 (1)
F- E	0 / 0	-28.0	-28.0	0.15	(3)	10.00			
E- D	0 / 19	-28.0	-28.0	0.15	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.15/1.00 (D-E:3) , WB=0.24/1.00 (B-E:1) , SSI=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	922	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.17 (A) (INPUT = 1.00)

DWG NO. TAM 12330-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	G600	1	1	OBC2012_6TCDL TRUSS DESC.	

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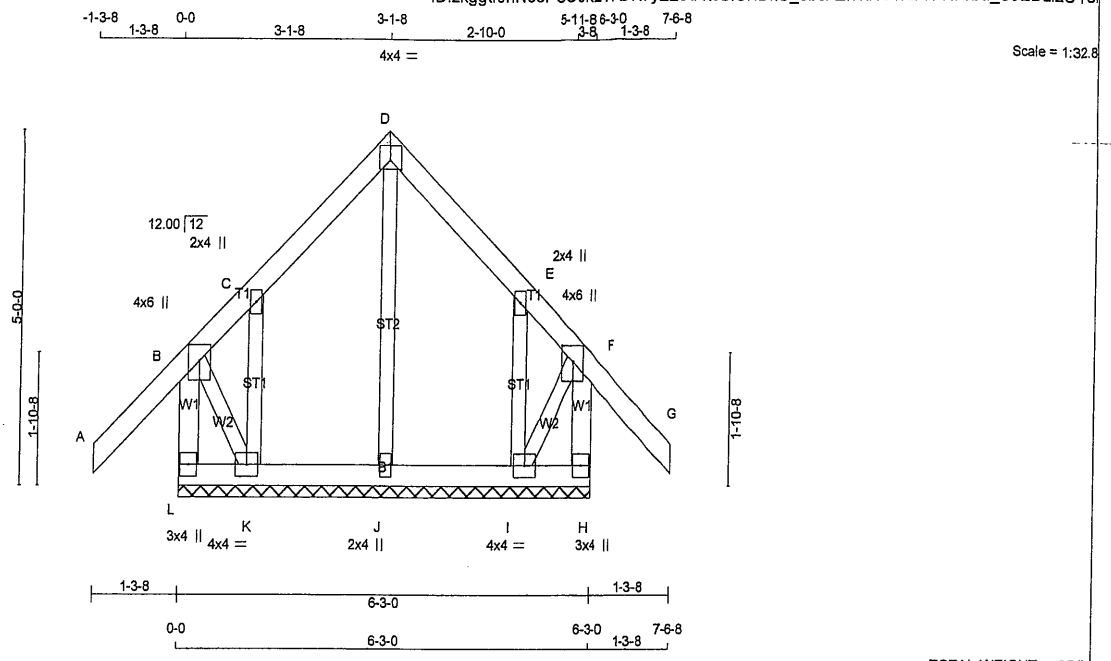

TOTAL WEIGHT = 35 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
L- B	-305 / 0	0.0	0.0	0.03 (1)	7.81	J- D	-203 / 0	0.07 (1)
A- B	0 / 55	-112.4	-112.4	0.16 (1)	10.00	K- C	-110 / 0	0.02 (1)
B- C	-63 / 0	-112.4	-112.4	0.15 (1)	6.25	I- E	-110 / 0	0.02 (1)
C- D	-13 / 0	-112.4	-112.4	0.06 (1)	6.25	B- K	0 / 10	0.00 (1)
D- E	-13 / 0	-112.4	-112.4	0.06 (1)	6.25	I- F	0 / 10	0.00 (1)
E- F	-63 / 0	-112.4	-112.4	0.15 (1)	6.25			
F- G	0 / 55	-112.4	-112.4	0.16 (1)	10.00			
H- F	-305 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	0 / 0	-28.0	-28.0	0.02 (3)	10.00			
K- J	0 / 2	-28.0	-28.0	0.03 (3)	10.00			
J- I	0 / 2	-28.0	-28.0	0.03 (3)	10.00			
I- H	0 / 0	-28.0	-28.0	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (F-G:1) , BC=0.03/1.00 (I-J:3) , WB=0.07/1.00 (D-J:1) , SSI=0.09/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.05 (F) (INPUT = 1.00)

DWG NO. TAM 172332-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287667	T610	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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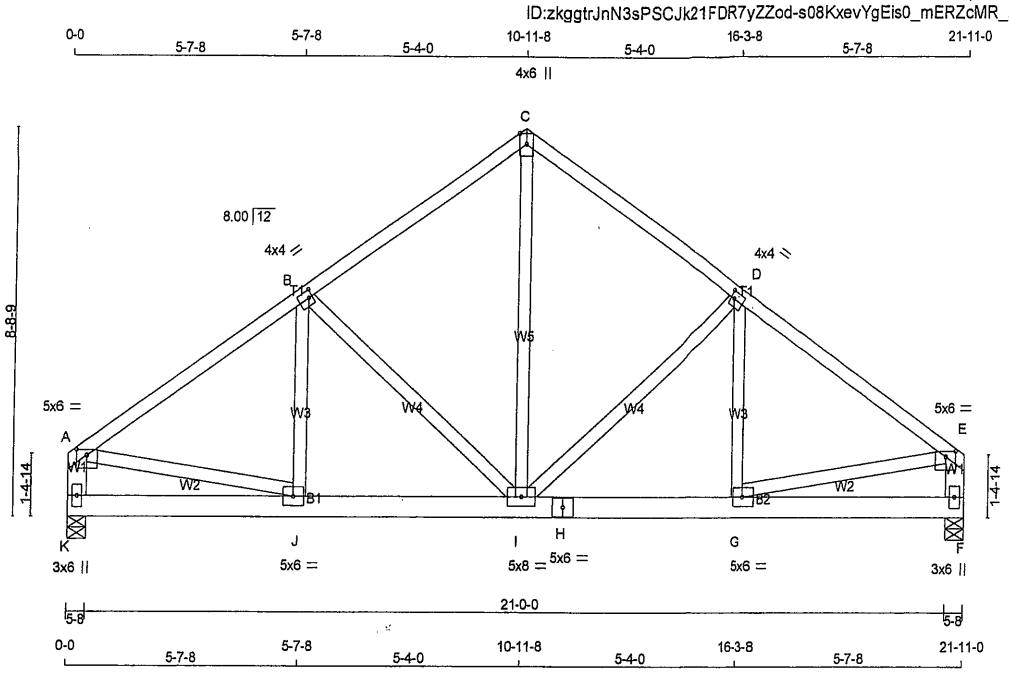

TOTAL WEIGHT = 2 X 119 = 238 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(82.6)
H- F	2	12	SIDE(82.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TMVW-p	MT20	5.0	6.0	1.50	3.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	5.0	6.0		
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	3350	0	3350	0	0	5-8	5-8
F	3350	0	3350	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0
F	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3732 / 0	-112.4	-112.4	0.43 (1)	4.47
B- C	-2754 / 0	-112.4	-112.4	0.37 (1)	5.10
C- D	-2754 / 0	-112.4	-112.4	0.37 (1)	5.10
D- E	-3732 / 0	-112.4	-112.4	0.43 (1)	4.47
K- A	-2897 / 0	0.0	0.0	0.10 (1)	7.81
F- E	-2897 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-193.3	-193.3	0.20 (1)	10.00
J- I	0 / 3138	-193.3	-193.3	0.39 (1)	10.00
I- H	0 / 3138	-193.3	-193.3	0.39 (1)	10.00
H- G	0 / 3138	-193.3	-193.3	0.39 (1)	10.00
G- F	0 / 0	-193.3	-193.3	0.20 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 2558	0.23 (1)
I- D	-1179 / 0	0.40 (1)
G- D	0 / 612	0.05 (1)
B- I	-1179 / 0	0.40 (1)
J- B	0 / 612	0.05 (1)
A- J	0 / 3187	0.28 (1)
G- E	0 / 3187	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 7-2-0
END DISTANCE = 21-11-0
END SPAN CARRIED = 7-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.39/1.00 (I-J:1) , WB=0.40/1.00 (B-I:1) , SSI=0.21/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.39 (J) (INPUT = 1.00)

DWG NO. TAM 12311 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	T700	1	2	OBC2012_6TCDL TRUSS DESC.	

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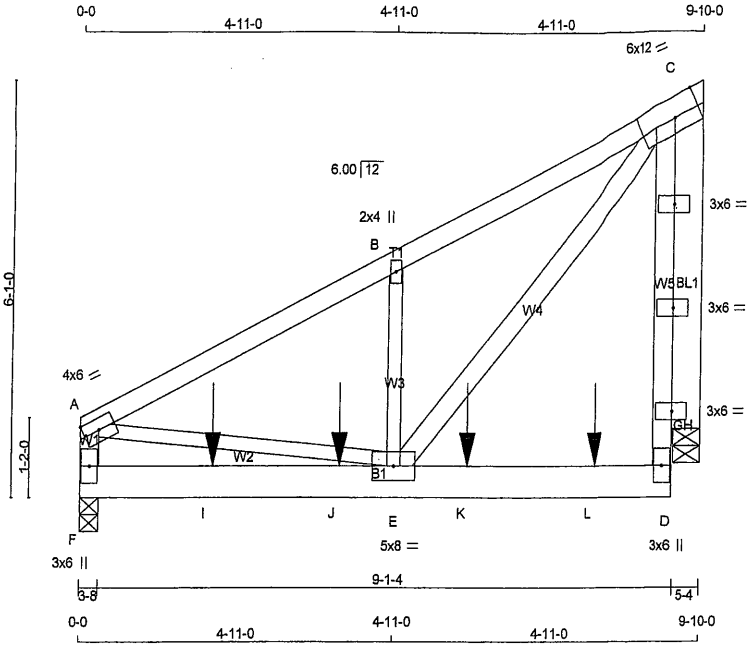

Scale = 1:34.0

TOTAL WEIGHT = 2 X 58 = 116 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	6.0	12.0	Edge	5.00
D	BMV+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		
G, G, G						
G	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H(C)	2284	0	2284	0	0	5-4	5-4 (** SEE "BEARING NOTE" **)
F	2151	0	2151	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	1784	1095 / 0	292 / 0	0 / 0	0 / 0	397 / 0	0 / 0
F	1681	1030 / 0	277 / 0	0 / 0	0 / 0	375 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H(C) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-2453 / 0	-112.4	-112.4	0.23 (1)	5.50	E- B	-622 / 0	0.07 (1)
B- C	-2467 / 0	-112.4	-112.4	0.23 (1)	5.49	E- C	0 / 3339	0.30 (1)
D- G	0 / 570	0.0	0.0	0.11 (1)	10.00	A- E	0 / 2242	0.28 (1)
G- C	0 / 570	0.0	0.0	0.11 (1)	10.00	C- H	-2287 / 0	0.36 (1)
F- A	-1670 / 0	0.0	0.0	0.09 (1)	7.81	G- H	0 / 117	0.00 (1)
F- I	0 / 0	-28.0	-28.0	0.36 (1)	10.00			
I- J	0 / 0	-28.0	-28.0	0.36 (1)	10.00			
J- E	0 / 0	-28.0	-28.0	0.36 (1)	10.00			
E- K	0 / 100	-28.0	-28.0	0.37 (1)	10.00			
K- L	0 / 100	-28.0	-28.0	0.37 (1)	10.00			
L- D	0 / 100	-28.0	-28.0	0.37 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	2-0-12	-780	-780	---	BACK	VERT	TOTAL
J	4-0-12	-780	-780	---	BACK	VERT	TOTAL
K	6-0-12	-780	-780	---	BACK	VERT	TOTAL
L	8-0-12	-780	-780	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.37/1.00 (D-E:1) , WB=0.36/1.00 (C-H:1) , SSI=0.40/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	610	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.32 (A) (INPUT = 1.00)

P6 1/2

DWG NO. TAM I 7331 -18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287668	T700	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 779.8 lbs FACTORED DOWN AT 2-0-12, 779.8 lbs FACTORED DOWN AT 4-0-12, AND 779.8 lbs FACTORED DOWN AT 6-0-12, AND 779.8 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM17331-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
289225	T700Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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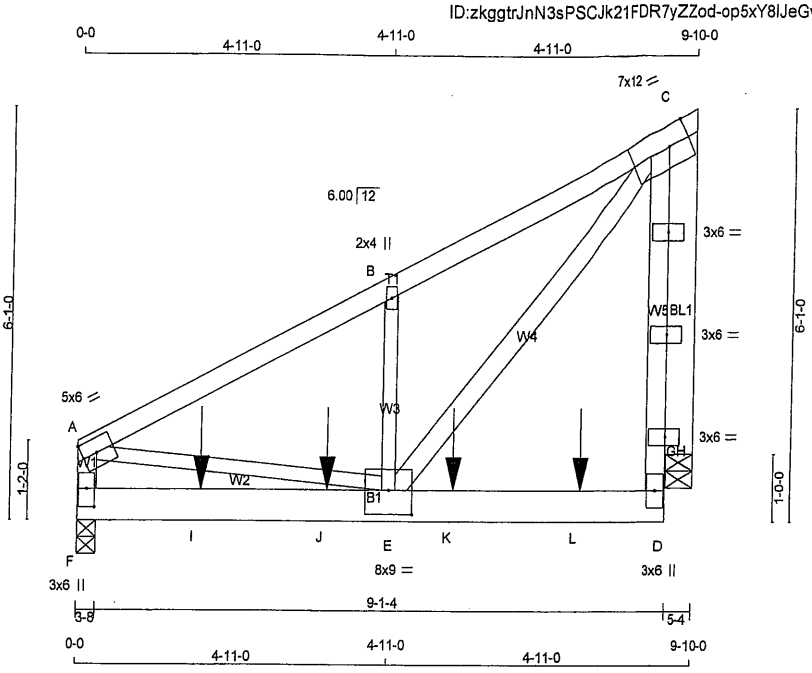

TOTAL WEIGHT = 2 X 58 = 116 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMW+w	MT20	2.0	4.0		
C	TMVWK1-t	MT20	7.0	12.0	Edge	4.00
D	BMV+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	8.0	9.0	4.25	4.50
F	BMV1+p	MT20	3.0	6.0	3.25	1.50
G, G, G						
G	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H(C)	4311	0	4311	0	0	5-4	5-4 (** SEE "BEARING NOTE" **)
F	4124	0	4124	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H(C)	3409	2029 / 0	610 / 0	0 / 0	0 / 0	770 / 0	0 / 0
F	3257	1966 / 0	586 / 0	0 / 0	0 / 0	706 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H(C), F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H(C) (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-4803 / 0	-112.4	-112.4	0.30 (1)	4.15	E- B	-602 / 0	0.07 (1)
B- C	-4810 / 0	-112.4	-112.4	0.27 (1)	4.17	E- C	0 / 6561	0.58 (1)
D- G	0 / 1119	0.0	0.0	0.21 (1)	10.00	A- E	0 / 4365	0.54 (1)
G- C	0 / 1119	0.0	0.0	0.21 (1)	10.00	C- H	-4317 / 0	0.68 (1)
F- A	-3055 / 0	0.0	0.0	0.17 (1)	6.61	G- H	0 / 220	0.00 (1)
F- I	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
I- J	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
J- E	0 / 0	-28.0	-28.0	0.82 (1)	10.00			
E- K	0 / 189	-28.0	-28.0	0.83 (1)	10.00			
K- L	0 / 189	-28.0	-28.0	0.83 (1)	10.00			
L- D	0 / 189	-28.0	-28.0	0.83 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	1-11-4	-1698	-1698	—	BACK	VERT	TOTAL
J	3-11-4	-1876	-1876	—	BACK	VERT	TOTAL
K	5-11-4	-1768	-1768	—	BACK	VERT	TOTAL
L	7-11-4	-1777	-1777	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 4310 S. KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	32.5	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL	LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 747 (0.15")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.83/1.00 (D-E:1) , WB=0.68/1.00 (C-H:1) , SSI=0.88/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.60 (A) (INPUT = 1.00)

DWG NO. TAM17343-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
289225	T700Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1697.8 lbs FACTORED DOWN AT 1-11-4, 1875.8 lbs FACTORED DOWN AT 3-11-4, AND 1768.3 lbs FACTORED DOWN AT 5-11-4, AND 1776.9 lbs FACTORED DOWN AT 7-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DWG NO. TAM 17343-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289223	T800	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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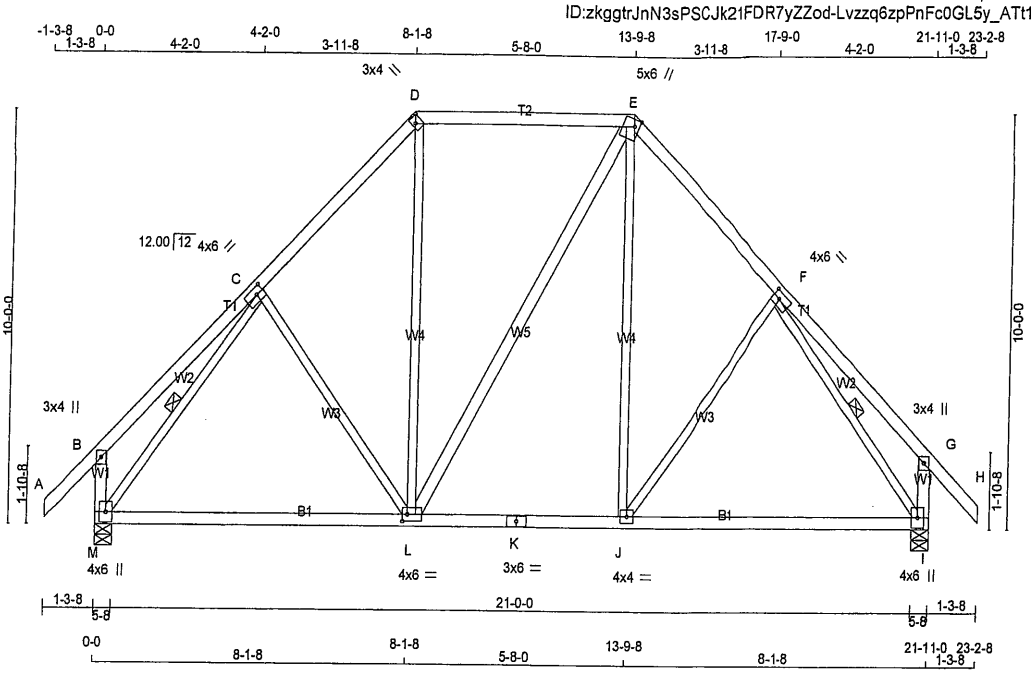


TOTAL WEIGHT = 2 X 115 = 230 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTW+h	MT20	3.0	4.0	Edge	2.00
E	TTWW+m	MT20	5.0	6.0	2.00	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.25
G	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWWW-t	MT20	4.0	6.0	2.00	1.50
M	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1696	0	1696	0	0	5-8	5-8
I	1696	0	1696	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1334	802 / 0	230 / 0	0 / 0	0 / 0	302 / 0	0 / 0
I	1334	802 / 0	230 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

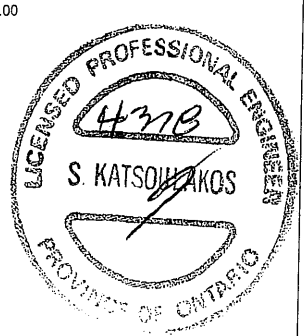
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-112.4	-112.4	0.16 (1)	10.00
B- C	0 / 38	-112.4	-112.4	0.30 (1)	10.00
C- D	-1209 / 0	-112.4	-112.4	0.25 (1)	5.55
D- E	-829 / 0	-112.4	-112.4	0.47 (1)	5.98
E- F	-1209 / 0	-112.4	-112.4	0.25 (1)	5.55
F- G	0 / 38	-112.4	-112.4	0.30 (1)	10.00
G- H	0 / 55	-112.4	-112.4	0.16 (1)	10.00
M- B	-329 / 0	0.0	0.0	0.04 (1)	7.81
I- G	-329 / 0	0.0	0.0	0.04 (1)	7.81
M- L	0 / 921	-28.0	-28.0	0.49 (2)	10.00
L- K	0 / 829	-28.0	-28.0	0.46 (2)	10.00
K- J	0 / 829	-28.0	-28.0	0.46 (2)	10.00
J- I	0 / 920	-28.0	-28.0	0.49 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- L	-157 / 46	0.15 (1)
L- D	0 / 377	0.08 (2)
L- E	0 / 0	0.00 (1)
J- E	0 / 376	0.08 (2)
J- F	-157 / 46	0.15 (1)
M- C	-1568 / 0	0.50 (1)
F- I	-1568 / 0	0.50 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 801 (0.33")

CSI: TC=0.47/1.00 (D-E:1) , BC=0.49/1.00 (I-J:2) , WB=0.50/1.00 (C-M:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.39 (C) (INPUT = 1.00)

DWG NO. TAM 17321-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289223	G800	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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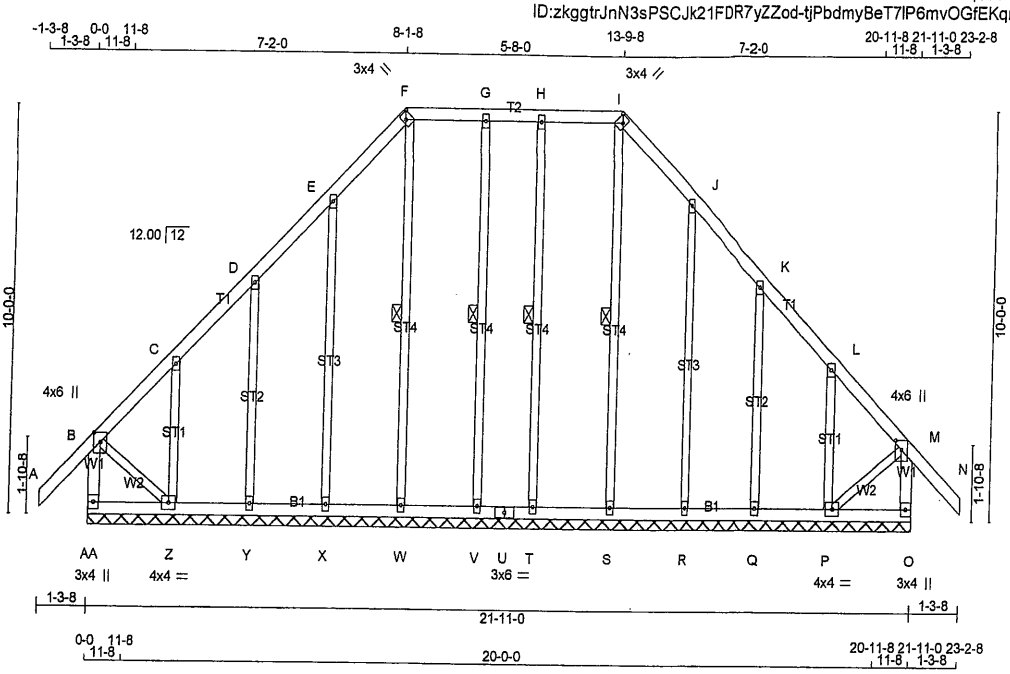


TOTAL WEIGHT = 127 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - N	2x4	DRY	No.2	SPF
AA- B	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
AA- U	2x4	DRY	No.2	SPF
U - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY- SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C, D, E, G, H, J, K, L						
C	TMW+w	MT20	2.0	4.0		
F	TTW+h	MT20	3.0	4.0	Edge	2.00
I	TTW+h	MT20	3.0	4.0	Edge	2.00
M	TMVW+p	MT20	4.0	6.0	2.75	2.00
O	BMV1+p	MT20	3.0	4.0		
P	BMWW1-t	MT20	4.0	4.0		
Q, R, S, T, V, W, X, Y						
Q	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	6.0		
Z	BMWW1-t	MT20	4.0	4.0		
AA	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-W, G-V, I-S, H-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

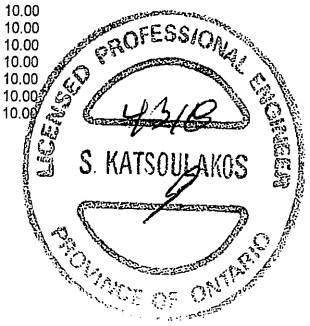
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 55	-112.4	-112.4	0.16 (1)	10.00
B- C	-43 / 0	-112.4	-112.4	0.07 (1)	6.25
C- D	-56 / 0	-112.4	-112.4	0.07 (1)	6.25
D- E	-40 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-56 / 0	-112.4	-112.4	0.06 (1)	6.25
F- G	-26 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-26 / 0	-112.4	-112.4	0.05 (1)	6.25
H- I	-26 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-56 / 0	-112.4	-112.4	0.06 (1)	6.25
J- K	-40 / 0	-112.4	-112.4	0.06 (1)	6.25
K- L	-56 / 0	-112.4	-112.4	0.07 (1)	6.25
L- M	-43 / 0	-112.4	-112.4	0.07 (1)	6.25
M- N	0 / 55	-112.4	-112.4	0.16 (1)	10.00
AA- B	-324 / 0	0.0	0.0	0.04 (1)	7.81
O- M	-324 / 0	0.0	0.0	0.04 (1)	7.81
AA- Z	0 / 0	-28.0	-28.0	0.04 (3)	10.00
Z- Y	0 / 35	-28.0	-28.0	0.04 (3)	10.00
Y- X	0 / 31	-28.0	-28.0	0.02 (3)	10.00
X- W	0 / 28	-28.0	-28.0	0.03 (3)	10.00
W- V	0 / 26	-28.0	-28.0	0.03 (3)	10.00
V- U	0 / 26	-28.0	-28.0	0.02 (2)	10.00
U- T	0 / 26	-28.0	-28.0	0.02 (2)	10.00
T- S	0 / 26	-28.0	-28.0	0.03 (3)	10.00
S- R	0 / 28	-28.0	-28.0	0.03 (3)	10.00
R- Q	0 / 31	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 35	-28.0	-28.0	0.04 (3)	10.00
P- O	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- F	-162 / 0	0.11 (1)
X- E	-250 / 0	0.31 (1)
Y- D	-207 / 0	0.12 (1)
Z- C	-257 / 0	0.07 (1)
V- G	-218 / 0	0.15 (1)
S- I	-162 / 0	0.11 (1)
R- J	-250 / 0	0.31 (1)
Q- K	-207 / 0	0.12 (1)
P- L	-257 / 0	0.07 (1)
T- H	-218 / 0	0.15 (1)
B- Z	0 / 51	0.01 (1)
P- M	0 / 51	0.01 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.16/1.00 (M-N:1) , BC=0.04/1.00 (Y-Z:3) , WB=0.31/1.00 (J-R:1) , SSI=0.11/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (F) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

DWG NO. TAM 17323-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289223	T801	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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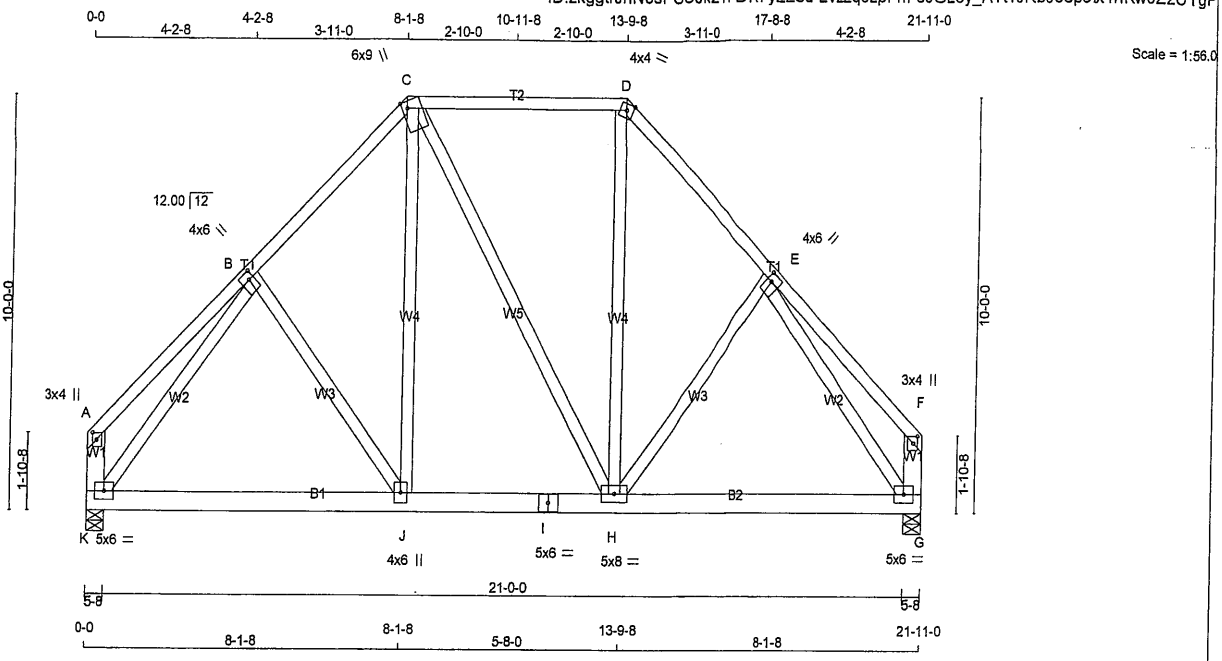

TOTAL WEIGHT = 2 X 140 = 280 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
K- A	2	12	TOP
G- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- I	2	12	SIDE(82.6)
I- G	2	12	SIDE(82.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	2.00	1.25
B	TMWW+t	MT20	4.0	6.0	2.25	1.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TTW-m	MT20	4.0	4.0	Edge	
E	TMWW+t	MT20	4.0	6.0	2.25	1.50
F	TMV+p	MT20	3.0	4.0	2.00	1.25
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	5.0	6.0		
J	BMWW+t	MT20	4.0	6.0		
K	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	3350	0	3350	0	0	5-8	5-8
G	3350	0	3350	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0
G	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 36	-112.4	-112.4	0.16 (1)	10.00		B- J	0 / 145	0.01 (3)
B- C	-2700 / 0	-112.4	-112.4	0.20 (1)	5.35		J- C	0 / 1387	0.12 (1)
C- D	-1902 / 0	-112.4	-112.4	0.37 (1)	5.82		C- H	0 / 7	0.00 (1)
D- E	-2705 / 0	-112.4	-112.4	0.20 (1)	5.35		H- D	0 / 1397	0.12 (1)
E- F	0 / 36	-112.4	-112.4	0.16 (1)	10.00		H- E	0 / 145	0.01 (3)
K- A	-176 / 0	0.0	0.0	0.01 (1)	7.81		K- B	-3102 / 0	0.89 (1)
G- F	-176 / 0	0.0	0.0	0.01 (1)	7.81		E- G	-3107 / 0	0.90 (1)
K- J	0 / 1841	-193.3	-193.3	0.56 (1)	10.00				
J- I	0 / 1898	-193.3	-193.3	0.51 (1)	10.00				
I- H	0 / 1898	-193.3	-193.3	0.51 (1)	10.00				
H- G	0 / 1844	-193.3	-193.3	0.55 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 7-2-0
END DISTANCE = 21-11-0
END SPAN CARRIED = 7-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.56/1.00 (J-K:1) , WB=0.90/1.00 (E-G:1) , SSI=0.31/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (E) (INPUT = 0.90)
JSI METAL= 0.52 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 173222-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289222	T910	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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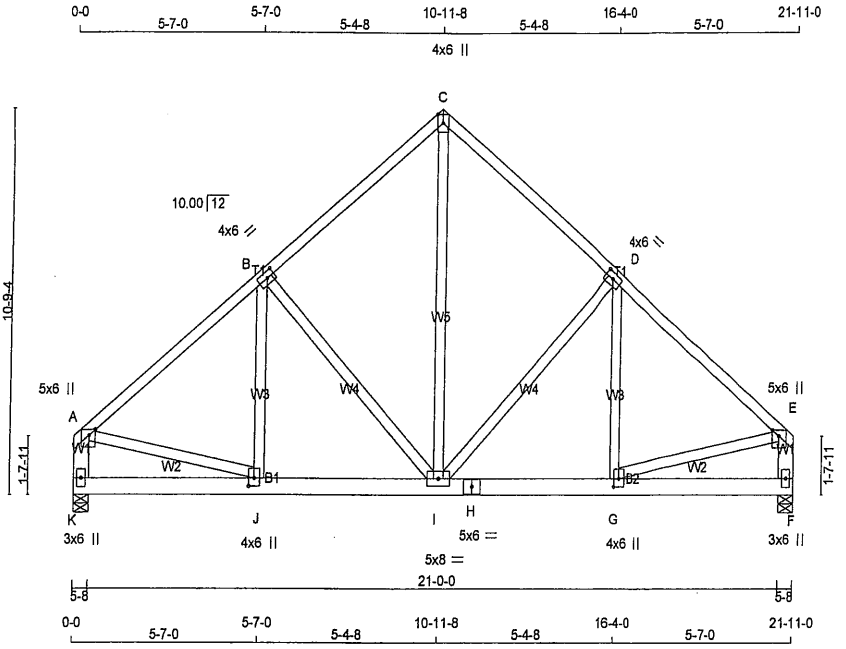

TOTAL WEIGHT = 2 X 130 = 259 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
K- A	2	12	TOP
F- E	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
K- H	2	12	SIDE(82.6)
H- F	2	12	SIDE(82.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.75
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0	2.00	2.75
E	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWW+t	MT20	4.0	6.0	2.75	2.00
H	BS-t	MT20	5.0	6.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW+t	MT20	4.0	6.0	2.75	2.00
K	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	3350	0	3350	0	0	5-8	5-8
F	3350	0	3350	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0
F	2671	1550 / 0	501 / 0	0 / 0	0 / 0	620 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3249 / 0	-112.4	-112.4	0.43 (1)	4.75
B- C	-2405 / 0	-112.4	-112.4	0.37 (1)	5.37
C- D	-2405 / 0	-112.4	-112.4	0.37 (1)	5.37
D- E	-3249 / 0	-112.4	-112.4	0.43 (1)	4.75
K- A	-2906 / 0	0.0	0.0	0.11 (1)	7.81
F- E	-2906 / 0	0.0	0.0	0.11 (1)	7.81
K- J	0 / 0	-193.3	-193.3	0.20 (1)	10.00
J- I	0 / 2531	-193.3	-193.3	0.36 (1)	10.00
I- H	0 / 2531	-193.3	-193.3	0.36 (1)	10.00
H- G	0 / 2531	-193.3	-193.3	0.36 (1)	10.00
G- F	0 / 0	-193.3	-193.3	0.20 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 2563	0.23 (1)
I- D	-1067 / 0	0.49 (1)
G- D	0 / 625	0.06 (1)
B- I	-1067 / 0	0.49 (1)
J- B	0 / 625	0.06 (1)
A- J	0 / 2591	0.23 (1)
G- E	0 / 2591	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 7-2-0
END DISTANCE = 21-11-0
END SPAN CARRIED = 7-2-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.43/1.00 (A-B:1) , BC=0.36/1.00 (I-J:1) , WB=0.49/1.00 (B-I:1) , SSI=0.21/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.41 (G) (INPUT = 1.00)

DWG NO. TAM 123/9 -18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287668	P42	5	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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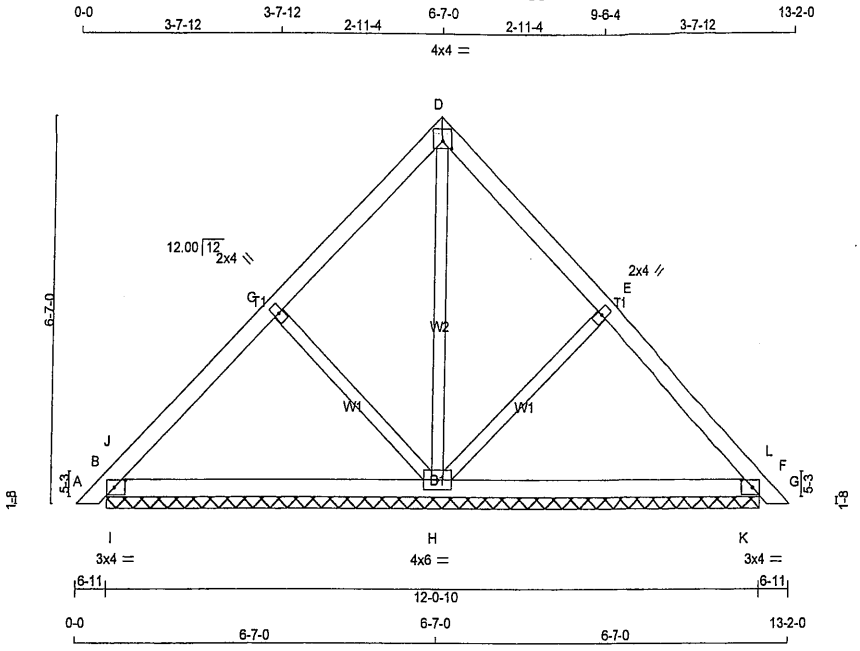

TOTAL WEIGHT = 5 X 46 = 231 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	1.50	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	4.0	1.50	2.50
H	BMWWW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	459	0	459	0	0	12-0-10	12-0-10
F	459	0	459	0	0	12-0-10	12-0-10
H	877	0	877	0	0	12-0-10	12-0-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	357	221 / 0	57 / 0	0 / 0	0 / 0	79 / 0	0 / 0
F	357	221 / 0	57 / 0	0 / 0	0 / 0	79 / 0	0 / 0
H	706	399 / 0	140 / 0	0 / 0	0 / 0	167 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-112.4	-112.4	0.02 (1)	10.00	H- D	-251 / 5	0.16 (1)
B- J	-488 / 0	-112.4	-112.4	0.11 (3)	6.25	H- E	-300 / 0	0.09 (1)
J- C	-265 / 0	-112.4	-112.4	0.18 (1)	6.25	C- H	-300 / 0	0.09 (1)
C- D	-35 / 0	-112.4	-112.4	0.17 (1)	6.25	I- J	0 / 420	0.00 (1)
D- E	-35 / 0	-112.4	-112.4	0.17 (1)	6.25	K- L	0 / 420	0.00 (1)
E- L	-265 / 0	-112.4	-112.4	0.18 (1)	6.25			
L- F	-488 / 0	-112.4	-112.4	0.11 (3)	6.25			
F- G	0 / 16	-112.4	-112.4	0.02 (1)	10.00			
B- I	0 / 215	-28.0	-28.0	0.10 (3)	10.00			
I- H	0 / 215	-28.0	-28.0	0.27 (2)	10.00			
H- K	0 / 215	-28.0	-28.0	0.27 (2)	10.00			
K- F	0 / 215	-28.0	-28.0	0.10 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-J:1) , BC=0.27/1.00 (H-I:2) , WB=0.16/1.00 (D-H:1) , SSI=0.24/1.00 (F-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

DWG NO. TAM 17333-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289223	P43	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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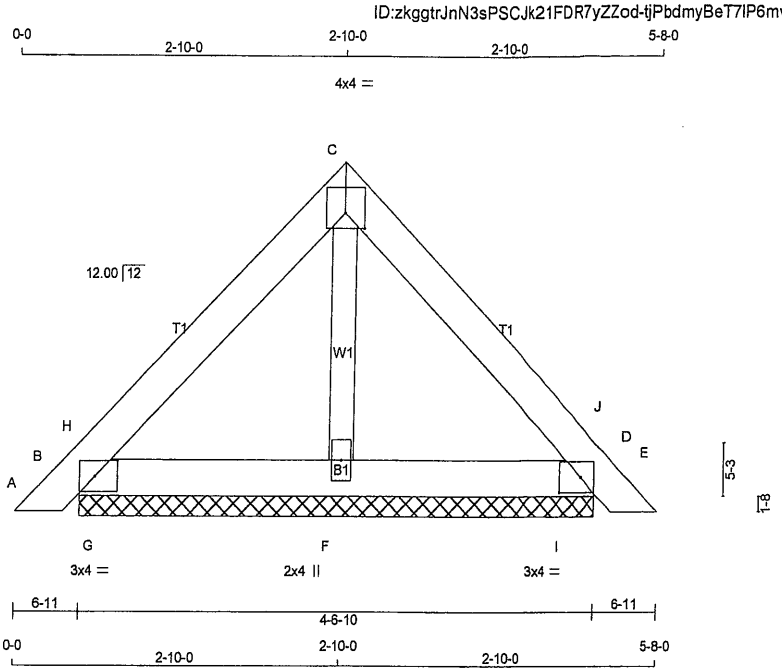

TOTAL WEIGHT = 3 X 16 = 48 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	262	0	262	0	0	4-6-10	4-6-10
D	262	0	262	0	0	4-6-10	4-6-10
F	217	0	217	0	0	4-6-10	4-6-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	196	135 / 0	22 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	196	135 / 0	22 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	188	85 / 0	52 / 0	0 / 0	0 / 0	51 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 16	-112.4	-112.4	0.02 (1)	10.00	F- C	-75 / 0	0.01 (1)
B- H	-40 / 0	-112.4	-112.4	0.03 (1)	6.25	G- H	-232 / 8	0.00 (1)
H- C	-103 / 0	-112.4	-112.4	0.07 (1)	6.25	I- J	-232 / 8	0.00 (1)
C- J	-103 / 0	-112.4	-112.4	0.07 (1)	6.25			
J- D	-40 / 0	-112.4	-112.4	0.03 (1)	6.25			
D- E	0 / 16	-112.4	-112.4	0.02 (1)	10.00			
B- G	0 / 67	-28.0	-28.0	0.07 (1)	10.00			
G- F	0 / 67	-28.0	-28.0	0.07 (1)	10.00			
F- I	0 / 67	-28.0	-28.0	0.07 (1)	10.00			
I- D	0 / 67	-28.0	-28.0	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (C-J:1) , BC=0.07/1.00 (F-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.18/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM 17324-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
289223	P43Z	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 3 10:06:27 2018 Page 1
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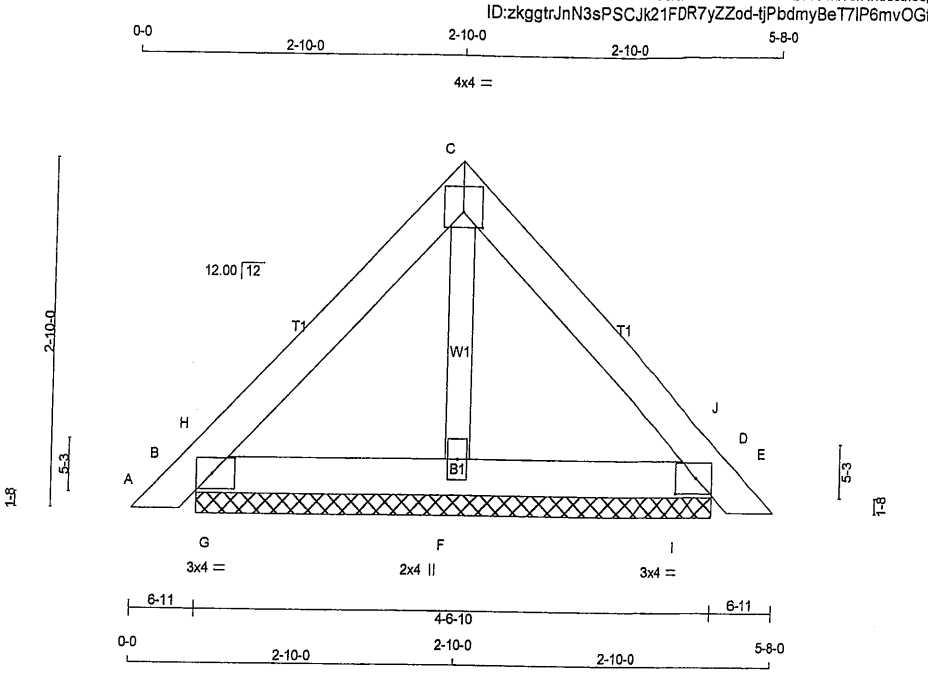


TOTAL WEIGHT = 2 X 16 = 32 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- D	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.50
C	TTW-p	MT20	4.0	4.0	1.50	2.00
D	TMB1-I	MT20	3.0	4.0	1.50	2.50
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	264	0	264	0	0	4-6-10	4-6-10
D	264	0	264	0	0	4-6-10	4-6-10
F	214	0	214	0	0	4-6-10	4-6-10

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	197	136 / 0	21 / 0	0 / 0	0 / 0	39 / 0	0 / 0
D	197	136 / 0	21 / 0	0 / 0	0 / 0	39 / 0	0 / 0
F	186	83 / 0	53 / 0	0 / 0	0 / 0	50 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 16	-112.4	-112.4	0.01 (1)	10.00
B- H	-41 / 0	-112.4	-112.4	0.02 (1)	6.25
H- C	-104 / 0	-112.4	-112.4	0.03 (1)	6.25
C- J	-104 / 0	-112.4	-112.4	0.03 (1)	6.25
J- D	-41 / 0	-112.4	-112.4	0.02 (1)	6.25
D- E	0 / 16	-112.4	-112.4	0.01 (1)	10.00
B- G	0 / 68	-28.0	-28.0	0.04 (1)	10.00
G- F	0 / 68	-28.0	-28.0	0.04 (1)	10.00
F- I	0 / 68	-28.0	-28.0	0.04 (1)	10.00
I- D	0 / 68	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-74 / 0	0.01 (1)
G- H	-224 / 8	0.00 (1)
I- J	-224 / 8	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-J:1) , BC=0.04/1.00 (F-I:1) , WB=0.01/1.00 (C-F:1) , SSI=0.09/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

DWG NO. TAM17325-18
STRUCTURAL COMPONENT ONLY

Boise Cascade

Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

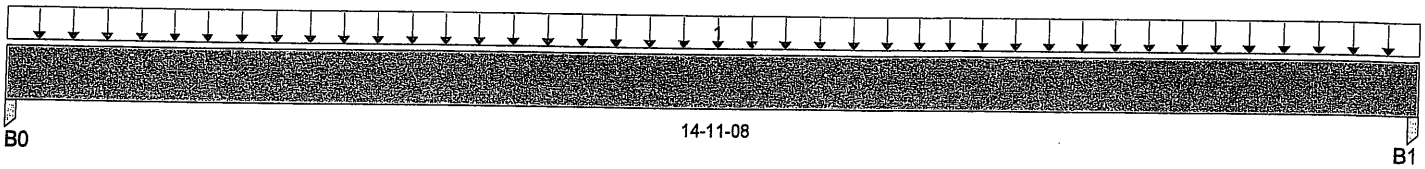
BC CALC® Design Report

Dry | 1 span | No cantilevers | 0/12 slope (deg)

April 2, 2018 10:28:03

Build 6536

Job Name:	BAYVIEW WELLINGTON HOMES	File Name:	S42-1BLOFT
Address:	S42-1B-LOFT . (BM63)	Description:	Designs\FB01
City, Province, Postal Code:	BRADFORD,	Specifier:	
Customer:		Designer:	SUNNY
Code reports:	CCMC 12472-R	Company:	
		Misc:	



Total Horizontal Product Length = 14-11-08

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 5-1/2"	1,459 / 0	1,859 / 0	4,376 / 0	
B1, 5-1/2"	1,459 / 0	1,859 / 0	4,376 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	TYING IN SPAN	Unf. Area (lb/ft^2)	L	00-00-00	14-11-08	11	13	33		17-08-12

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	32,256 ft-lbs	55,212 ft-lbs	58.4%	5	07-05-12
End Shear	7,755 lbs	21,696 lbs	35.7%	5	01-05-06
Total Load Defl.	L/295 (0.576")	0.708"	81.3%	13	07-05-12
Live Load Defl.	L/403 (0.422")	0.472"	89.4%	17	07-05-12
Max Defl.	0.576"	1"	57.6%	13	07-05-12
Span / Depth	14.3	n/a	n/a		00-00-00

Bearing Supports

		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	5-1/2" x 5-1/4"	9,616 lbs	n/a	27.3%	Unspecified
B1	Post	5-1/2" x 5-1/4"	9,616 lbs	n/a	27.3%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9

CONFORMS TO OBC 2012

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17372-18
STRUCTURAL
COMPONENT ONLY

Boise Cascade

Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP

BC CALC® Design Report

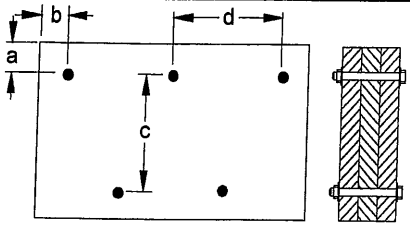
Dry | 1 span | No cantilevers | 0/12 slope (deg)

April 2, 2018 10:28:03

Build 6536
Job Name: BAYVIEW WELLINGTON HOMES
Address: S42-1B-LOFT (BM63)
City, Province, Postal Code: BRADFORD,
Customer:
Code reports: CCMC 12472-R

File Name: S42-1BLOFT
Description: Designs\FB01
Specifier:
Designer: SUNNY
Company:
Misc:

Connection Diagram



a minimum = 2½" c = 6-7/8"
b minimum = 2-1/2" d = 12"

Bolts are assumed to be Grade A307 or Grade 2 or higher.
Member has no side loads.
Connectors are: 1/2 in. Staggered Through Bolt

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12344 -18
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

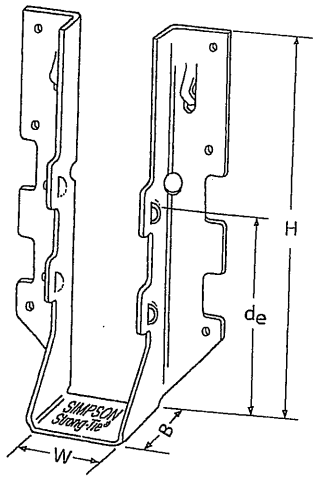
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

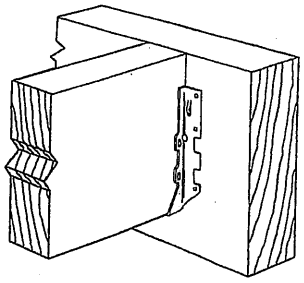
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

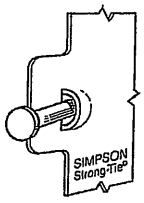


LUS28


Typical LUS Installation

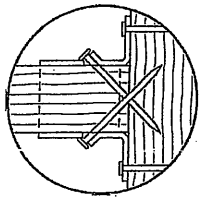
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

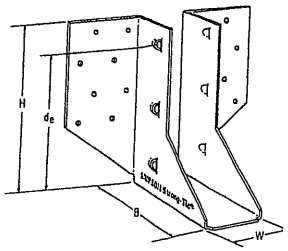
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

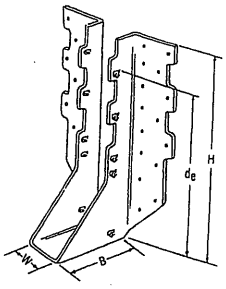
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

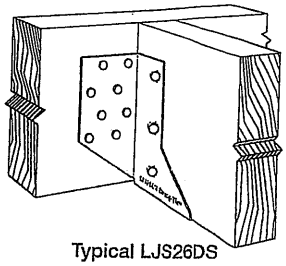
- See current catalogue for options



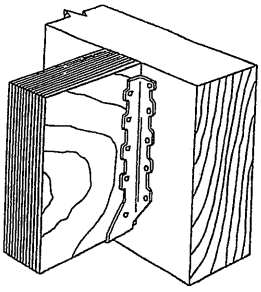
LJS26DS



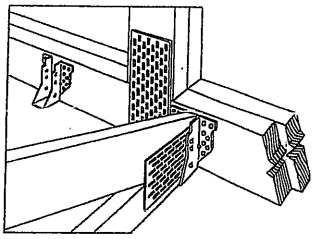
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



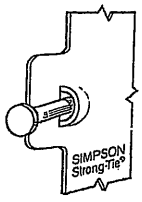
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

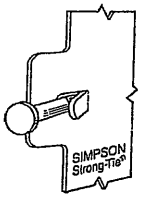
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

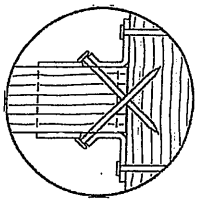


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

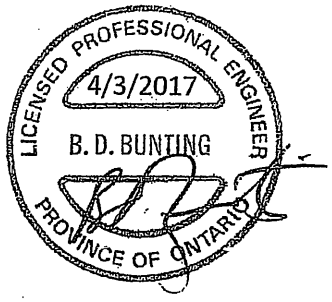
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

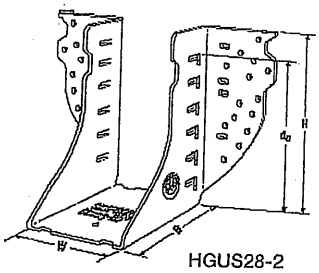
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

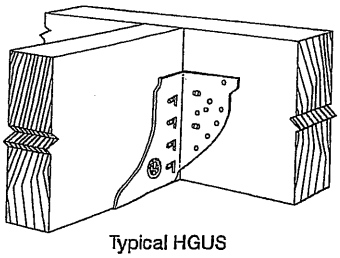
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HGUS28-2

Installation:

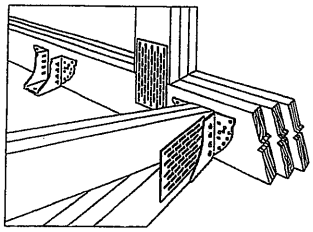
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HGUS Installation

Options:

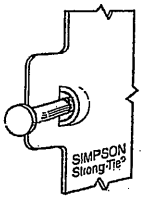
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

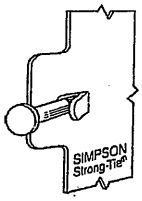
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4⅝₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4¹⁵⁄₁₆	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6⁵⁄₁₆	5⁷⁄₁₆	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4¹⁵⁄₁₆	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6⁵⁄₁₆	7³⁄₁₆	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4¹⁵⁄₁₆	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6⁵⁄₁₆	9³⁄₁₆	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6⁵⁄₁₆	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6⁵⁄₁₆	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

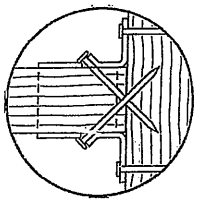


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

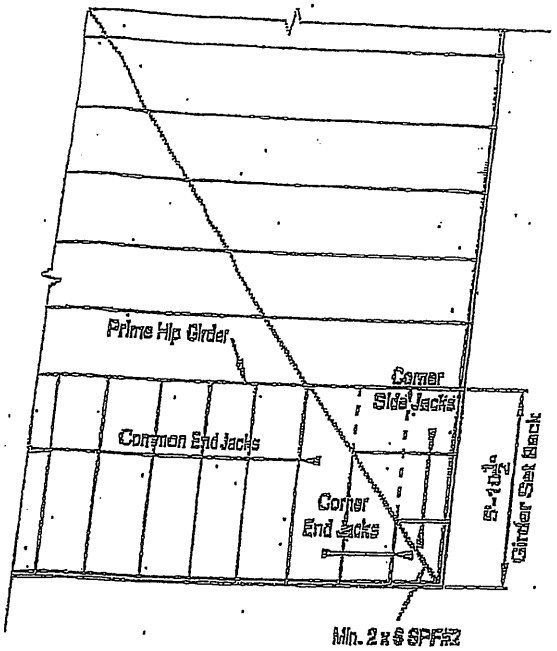
This technical bulletin is effective until June 30, 2019 and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHGUS17 3/17 exp 6/19

(800) 999-5099
strongtie.com

ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

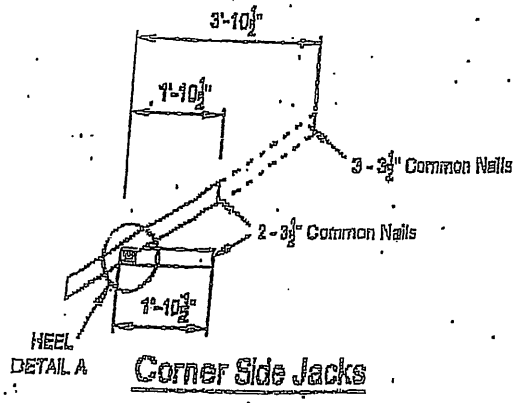
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

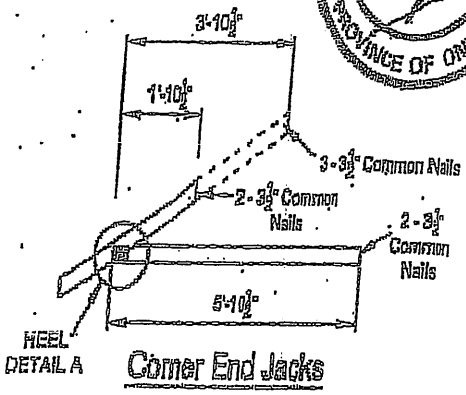
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	59.9 P.S.F.

DWG NO T1M 2475.14
STRUCTURAL
COMPONENT ONLY

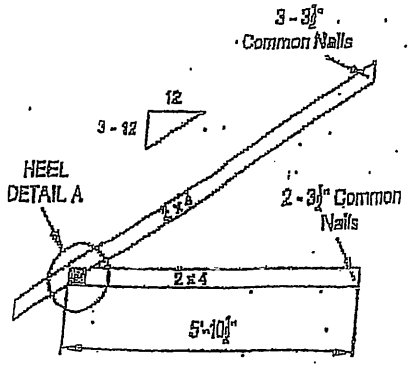
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



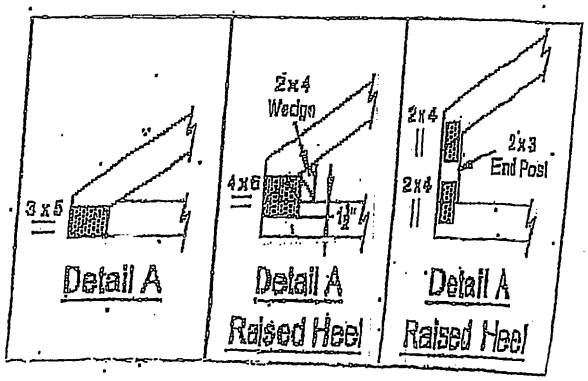
Corner Side Jacks



Corner End Jacks



Common End Jacks

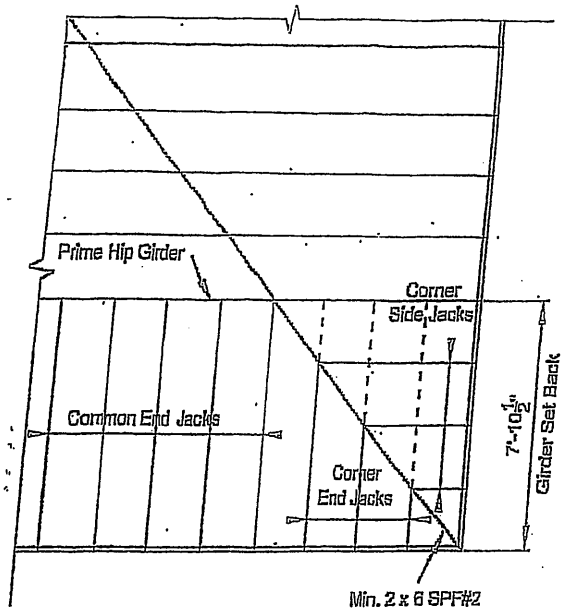


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN



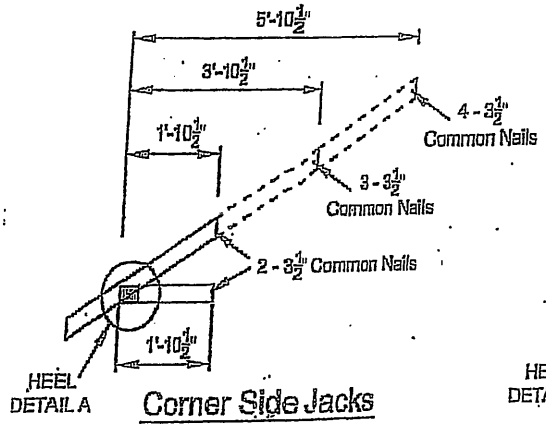
45° Hip End

DESIGN LOAD:

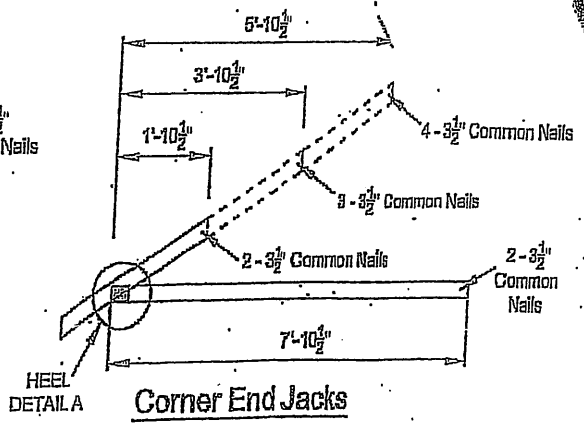
TOP CHORD LIVE LOAD	: 34.8 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F.
TOTAL LOAD	: 44.8 P.S.F.

DWG NO T&M 3503.14
STRUCTURAL
COMPONENT ONLY

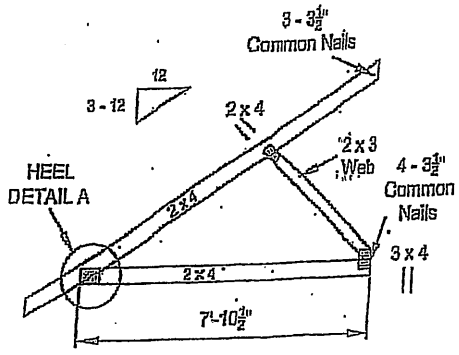
LICENSED PROFESSIONAL ENGINEER
1-2014
S. KATSOULAKOS
PROVINCE OF ONTARIO



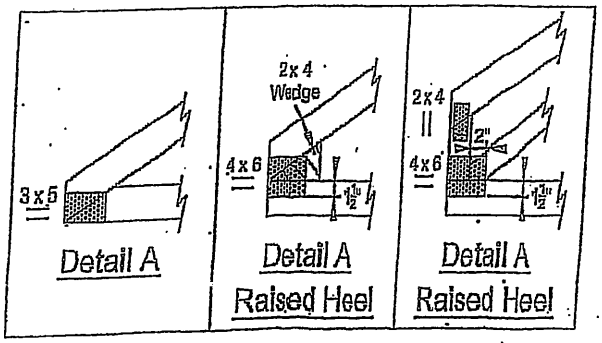
Corner Side Jacks



Corner End Jacks



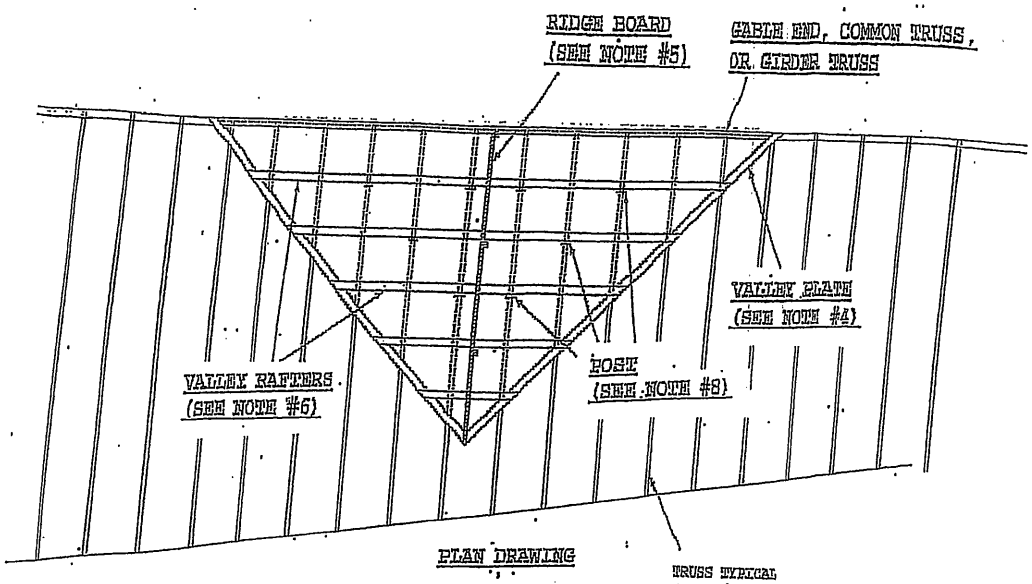
Common End Jacks



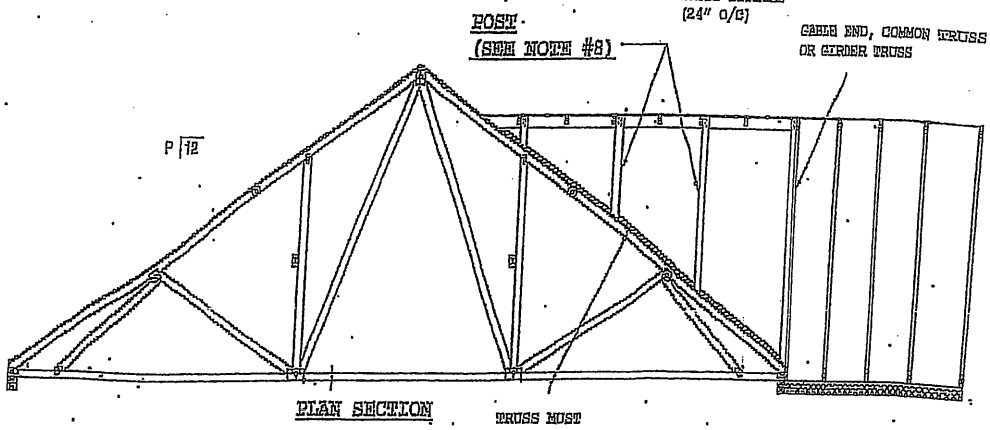
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

ENGINEERING SERVICES INC.
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING, NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

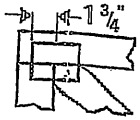
(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

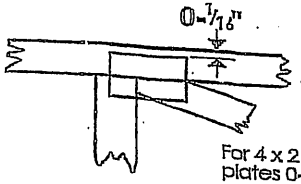
DWG NO T&M 6305.14
STRUCTURAL COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.*



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.*

This symbol indicates the required direction of slots in connector plates.*

*Plate location details available in MiTek software or upon request.

PLATE SIZE

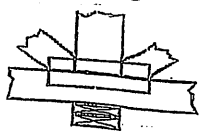
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

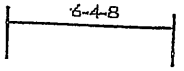
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

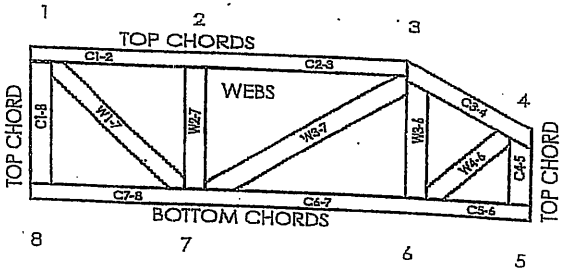
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2079
S. KATSOULAKOS
PROVINCE OF ONTARIO
DWG NO [illegible] 7414
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.