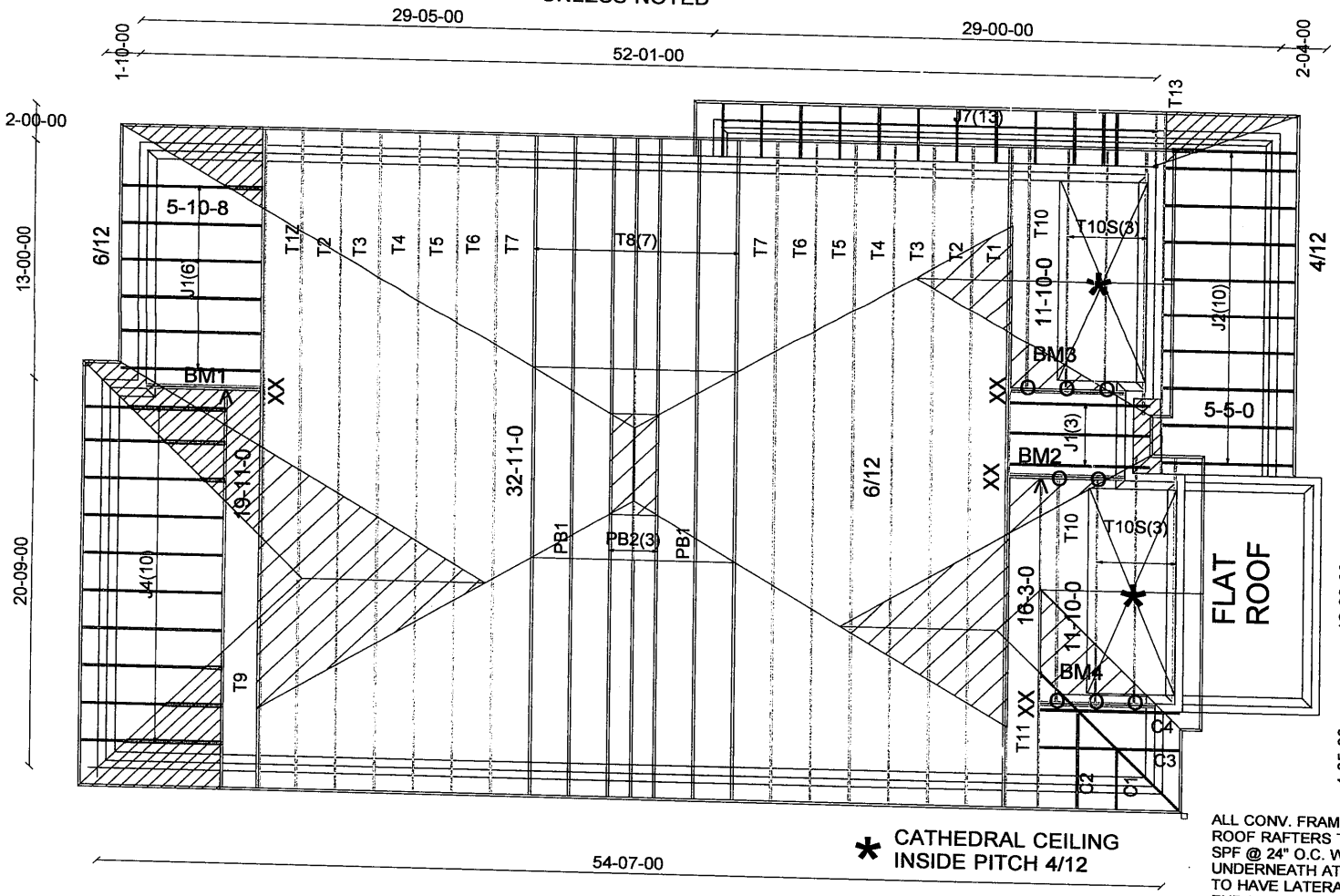
Site Copy

10/12 PITCHES (TYP.)
UNLESS NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3 BM4 = 2-2X10

* CATHEDRAL CEILING INSIDE PITCH 4/12

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder ::	Model :
	Layout ID: 296381	Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Elevation: S42-5 / A STD
	Plan Log: 94071	Project: GREEN VALLEY EAST	
		Date: 3/28/2018 Designer: AC.S.V.E	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

MiTek ver 7.5.0

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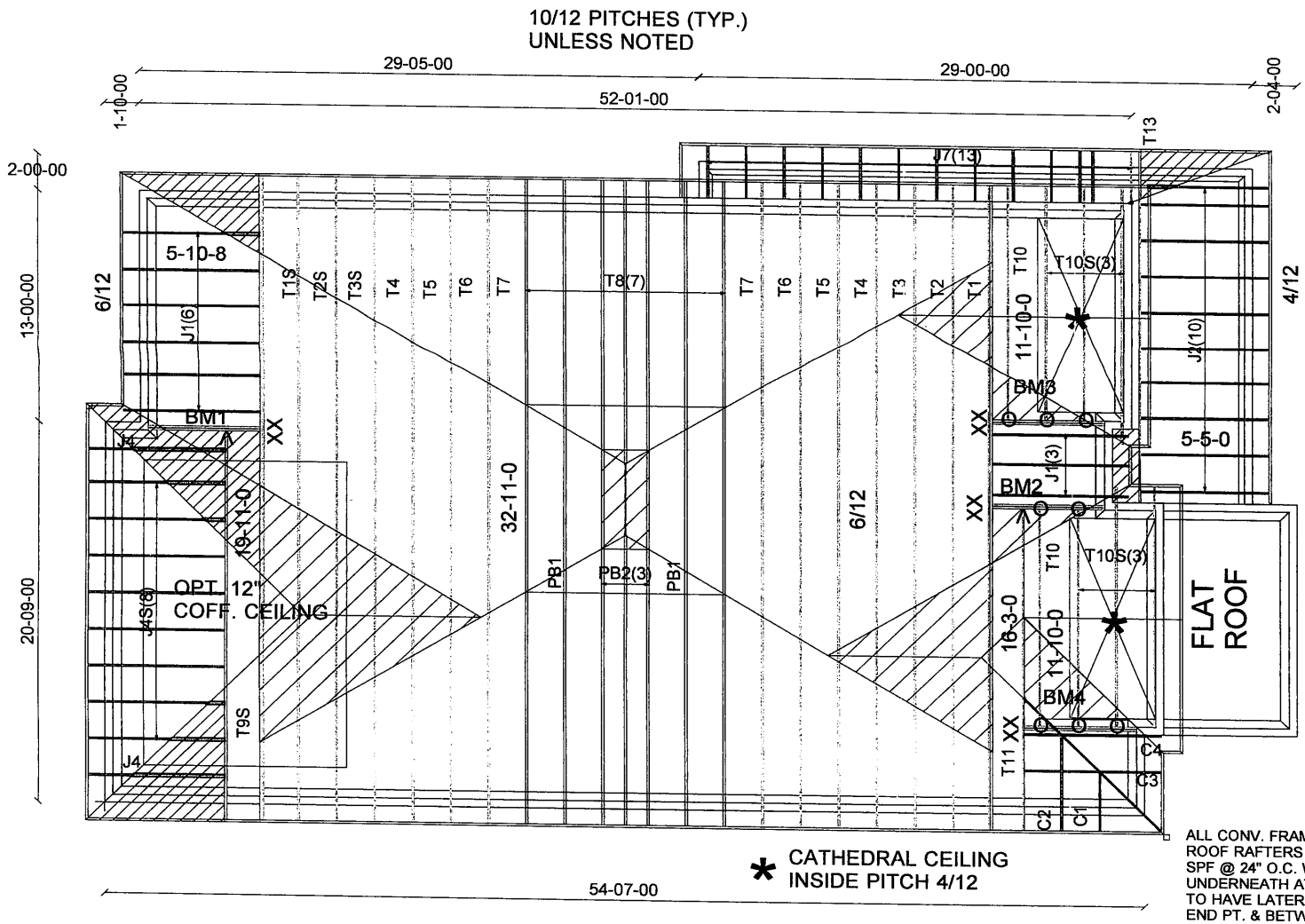

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3 BM4 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder :: BAYVIEW WELLINGTON HOMES / BRADFORD	Model : S42-5 / A STD +OPT. COFF.
	Layout ID: 287675	Location	Elevation
	Plan Log: 94071	Project: GREEN VALLEY ESTATES	
		Date: 3/28/2018 Designer: AC.S.V.E	

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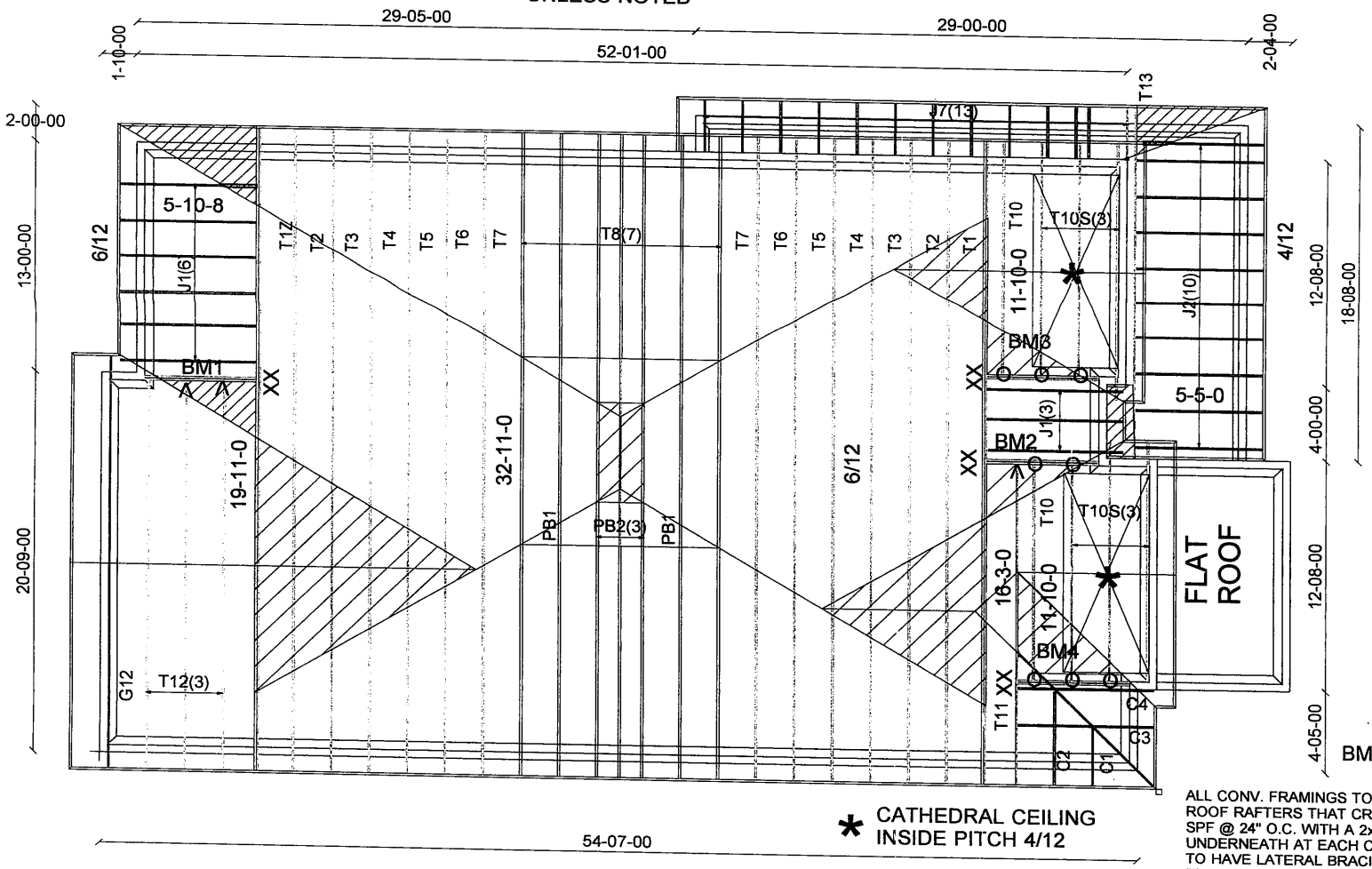
MiTek ver 7.5.0

Site Copy

10/12 PITCHES (TYP.)
UNLESS NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3 BM4 = 2-2X10

* CATHEDRAL CEILING INSIDE PITCH 4/12

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,510

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder ::	Model :
	Layout ID: 296382	Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Elevation: S42-5 / A REAR UPGRADE
	Plan Log: 94071	Project: GREEN VALLEY EAST	
		Date: 3/28/2018 Designer: AC.S.V.E	

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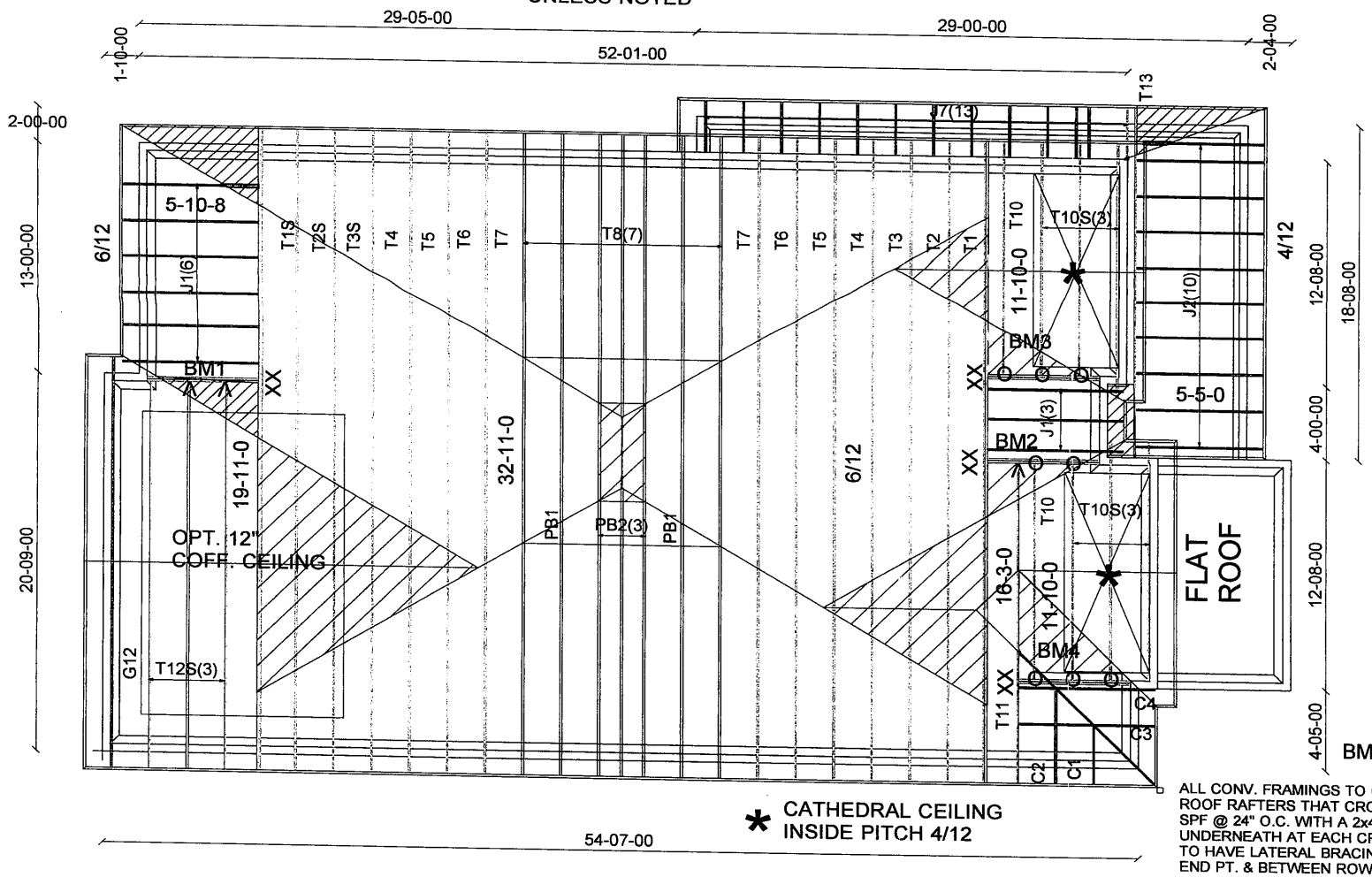
MiTek ver 7.5.0

Site Copy

10/12 PITCHES (TYP.)
UNLESS NOTED



* CATHEDRAL CEILING
INSIDE PITCH 4/12

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD [illegible] PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3 BM4 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder ::	Location
	Layout ID: **287678**	**BAYVIEW WELLINGTON HOMES / BRADFORD**	Model : S42-5 / A REAR UPGRADE +OPT. COFF.
	Plan Log: **94071**	Project: GREEN VALLEY ESTATES	Elevation
		Date: **3/28/2018** Designer: AC.S.V.E	

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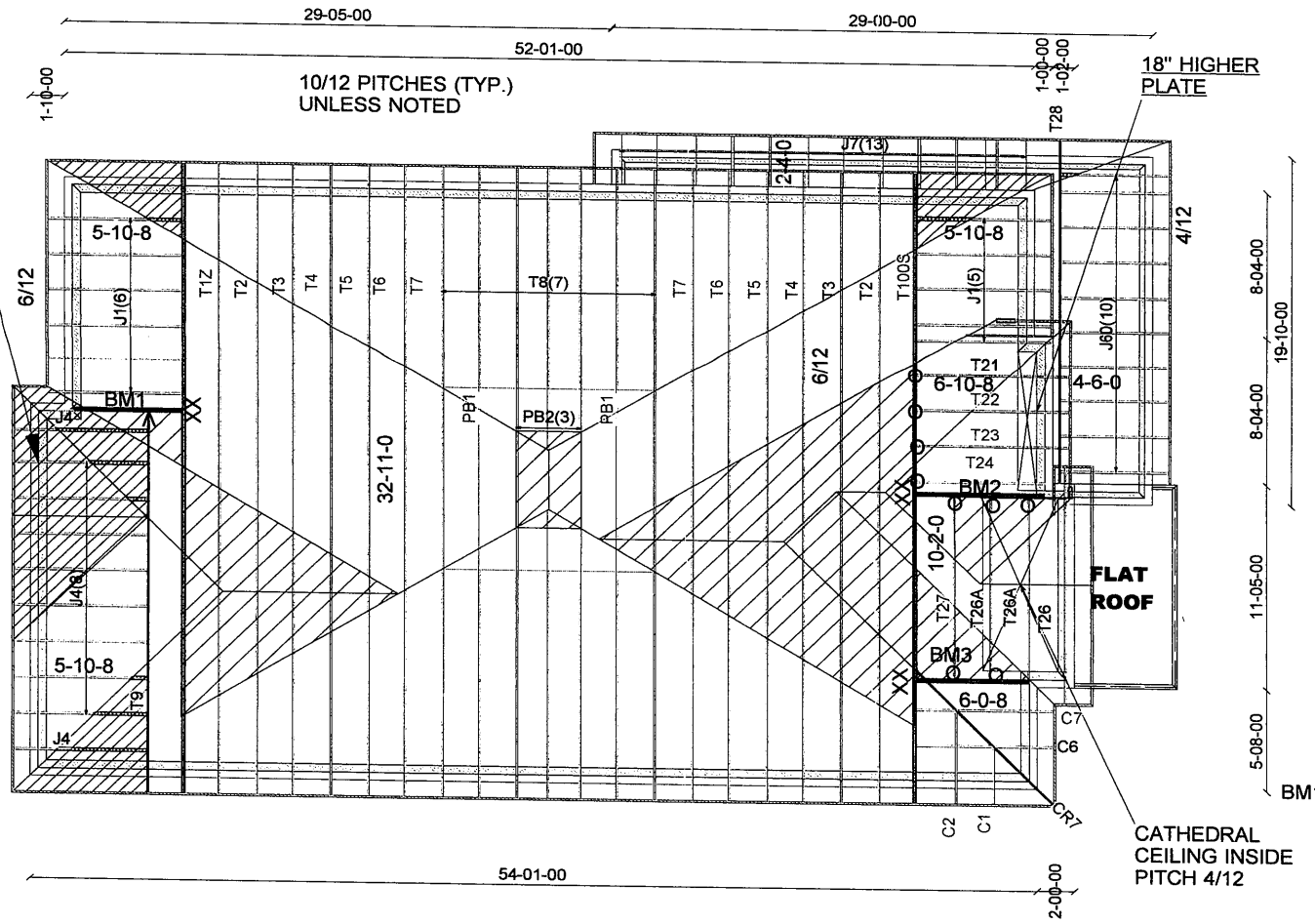
MiTek ver 7.5.0

Site Copy

CONVENTIONAL FRAMING FOR REAR UPGRADE ONLY

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1, BM2, BM3 = 2-2X10

CATHEDRAL CEILING INSIDE PITCH 4/12

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-5 / B STD OR REAR UPGRADE**
	Layout ID: **296383**	Project: **GREEN VALLEY EAST**	
	Plan Log: **94071**	Date: **3/28/2018** Designer: AC/S.V.E	

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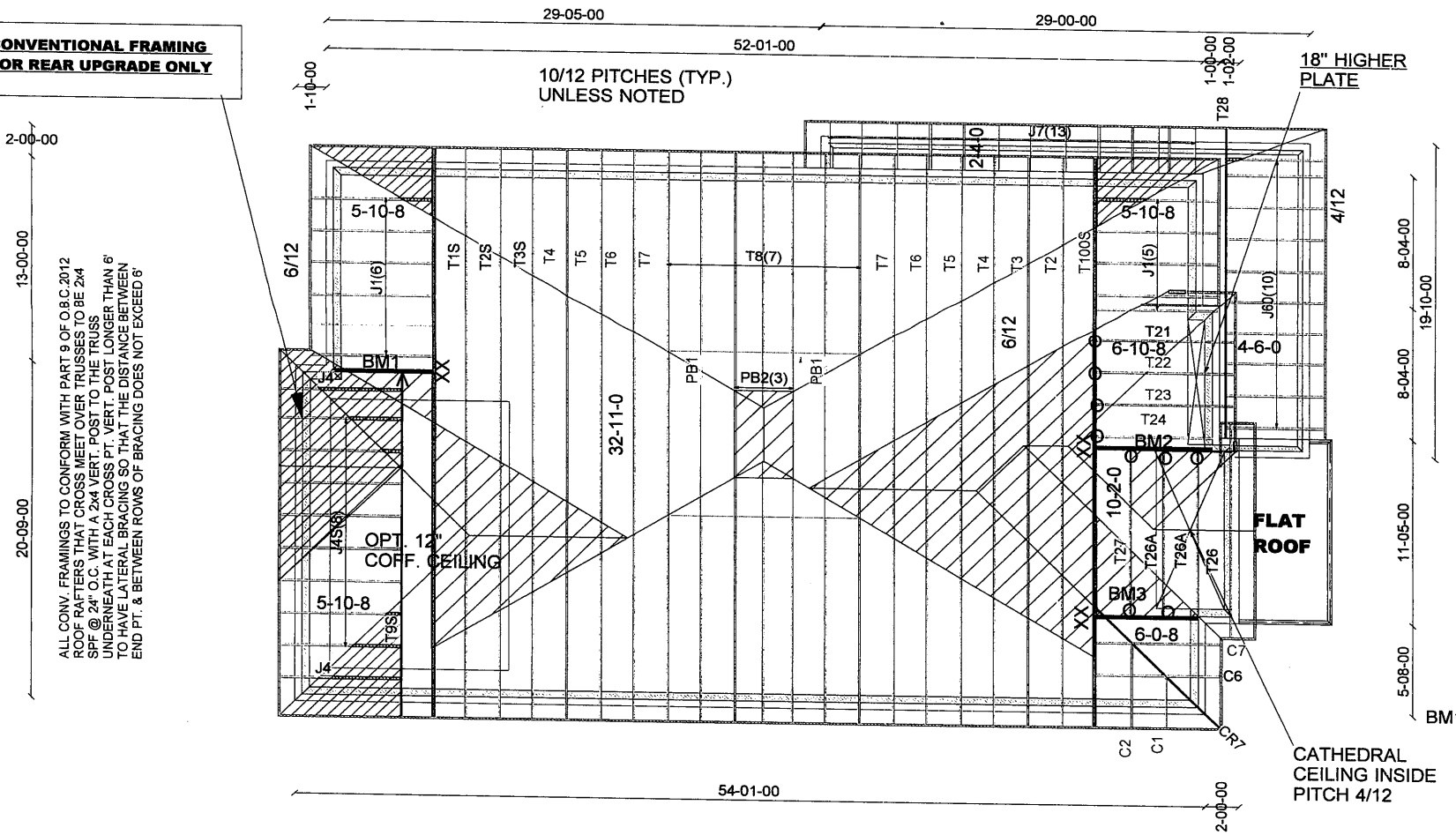
CONVENTIONAL FRAMING FOR REAR UPGRADE ONLY

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1, BM2, BM3 = 2-2X10

CATHEDRAL CEILING INSIDE PITCH 4/12

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-5 / B STD OR REAR UPGRADE +OPT. COFF.**
	Layout ID: **287680**	Project: **GREEN VALLEY ESTATES**	
	Plan Log: **94071**	Date: 3/28/2018 Designer: AC	

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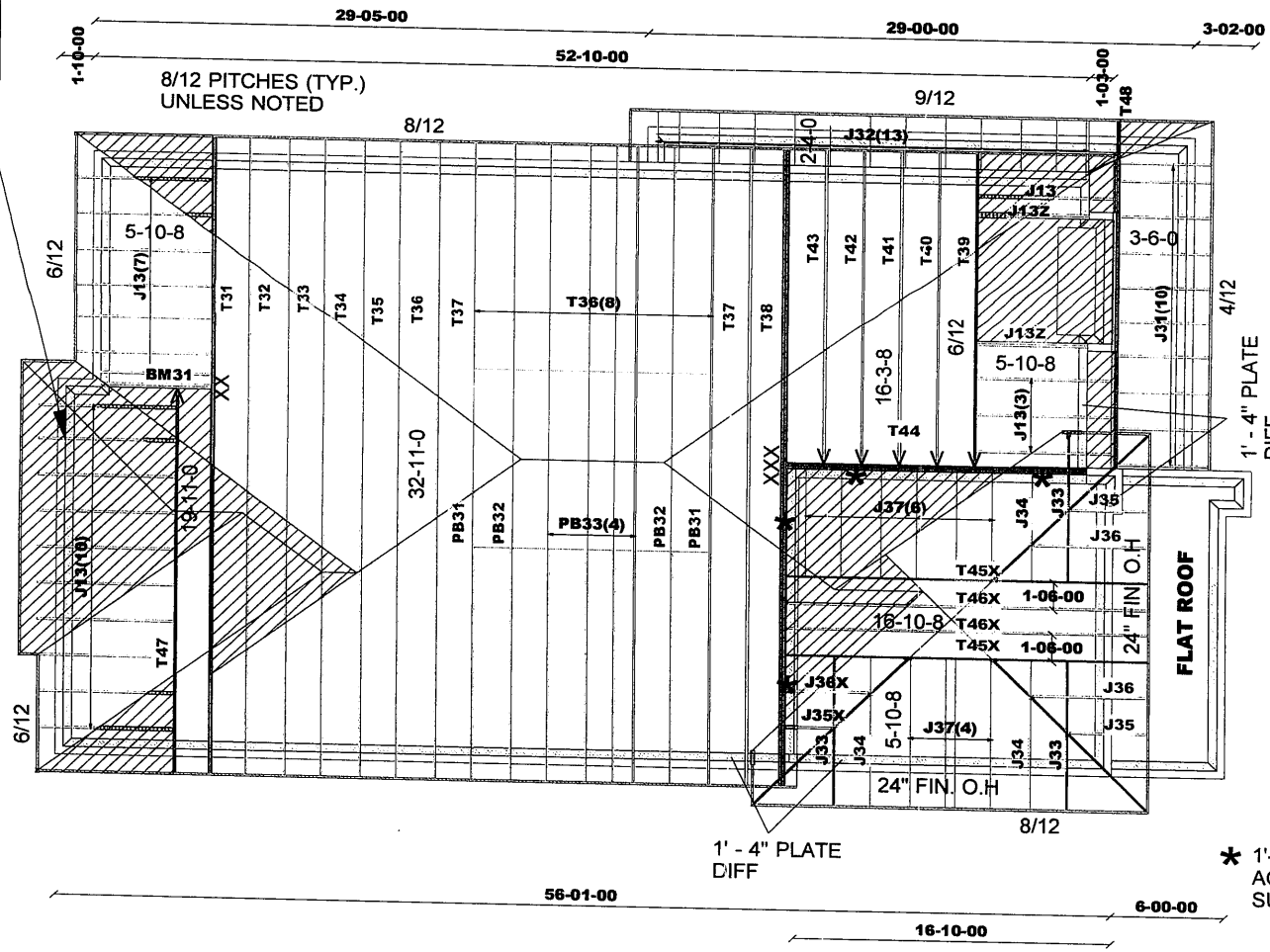
CONVENTIONAL FRAMING FOR REAR UPGRADE ONLY

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM31 = 2-2X10

* 1'-4"-0 BUILD UP KNEE WALLS AGAINST GIRDER TRS. TO SUPPORT TRS. ABOVE

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-5 / C STD OR REAR UPGRADE**
	Layout ID: **296384**	Project: **GREEN VALLEY EAST**	
	Plan Log: **94071**	Date: 3/28/2018 Designer: AC/S.V.E	

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CONVENTIONAL FRAMING FOR REAR UPGRADE ONLY

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
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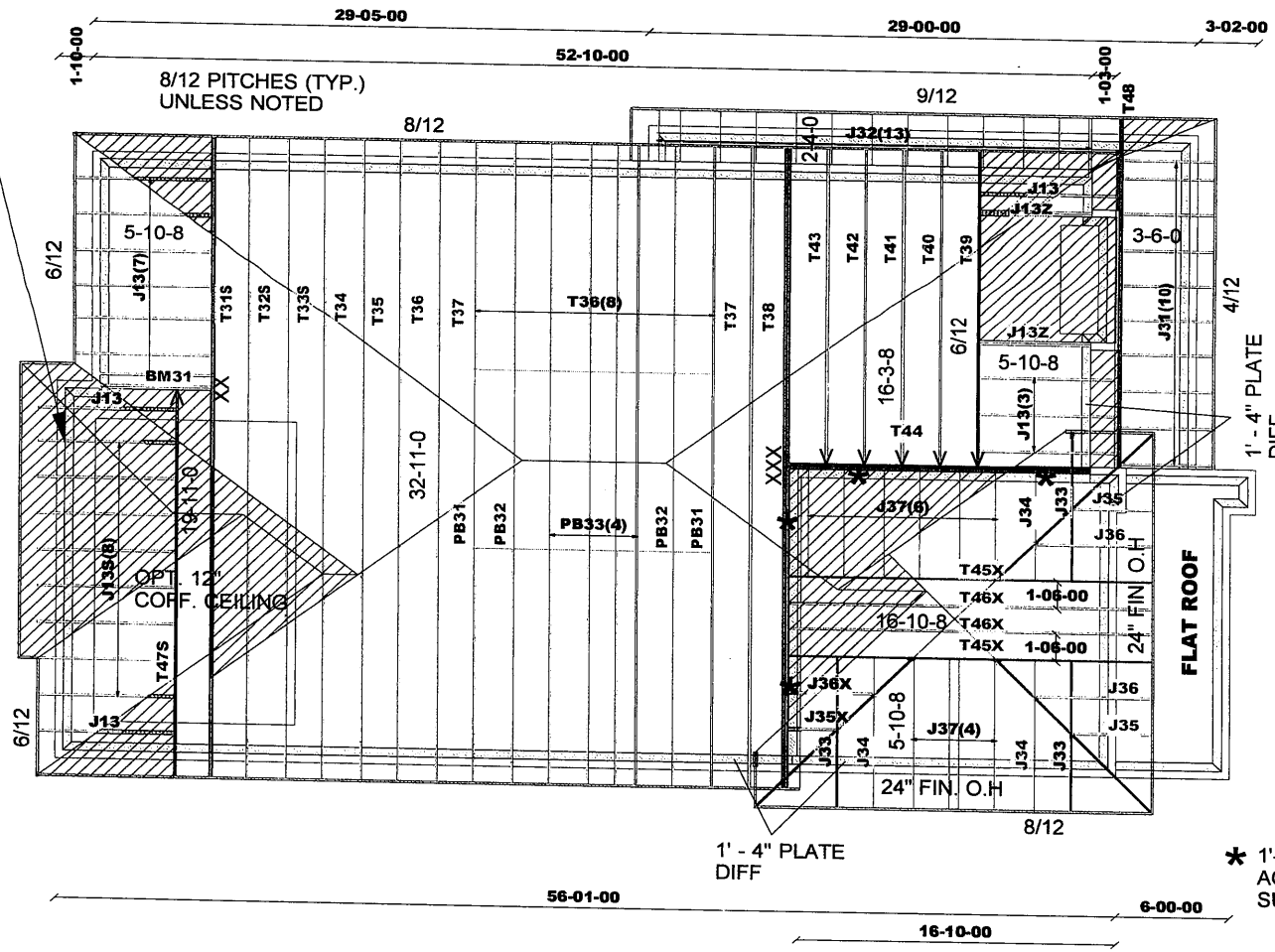

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)
HGUS26-3 - (XXX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM31 = 2-2X10

* 1'-4"-0 BUILD UP KNEE WALLS AGAINST GIRDER TRS. TO SUPPORT TRS. ABOVE

M11,570

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: **44755**	Builder / Location: **BAYVIEW WELLINGTON HOMES / BRADFORD**	Model / Elevation: **S42-5 / C STD OR REAR UPGRADE + OPT. COFF.**
	Layout ID: **287652**	Project: **GREEN VALLEY EAST**	
	Plan Log: **94071**	Date: 3/28/2018 Designer: AC/S.V.E	

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Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

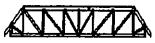
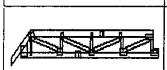
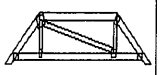
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296381 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: A STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	1 / 2 Ply	T1Z HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	2	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.48 / 180.34		
	2	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.56 / 179.34		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	1	T9 HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	108.60 / 68.00		
	1	T11 HIP GIRDER	10.00 / 0.00	16-03-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	110.03 / 69.67		
	2	T10 COMMON	10.00 / 0.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	114.82 / 74.00		
	6	T10S SCISSORS	10.00 / 4.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	340.68 / 220.98		
	1	T13 HALF HIP	10.00 / 0.00	18-03-00	02-01-10	2 X 6	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	100.21 / 64.33		
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		
	3	PB2 PIGGYBACK	10.00 / 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.31 / 60.99		
	9	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	151.11 / 96.03		
	10	J4 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	189.90 / 120.00		

Site Copy

DATE	03/28/18
SALES REP	Mario

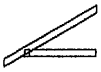
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 44755 **LAYOUT ID:** 296381 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** A STD.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	C1 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	8.36 / 6.00		
	1	C2 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	11.33 / 7.33		
	1	C3 JACK-OPEN	10.00 / 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -04-01-01	01-07-11 / 00-03-08	12.92 / 8.67		
	1	C4 JACK-OPEN	10.00 / 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / -02-01-01	01-07-11 / 00-03-08	15.89 / 10.00		
	10	J2 JACK-OPEN	4.00 / 0.00	05-05-00	02-01-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 02-01-10	143.50 / 93.30		
	13	J7 JACK	10.00 / 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-07 / 02-11-12	117.26 / 78.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3461.31** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5484.65** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
8	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

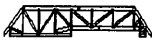
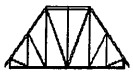
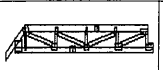
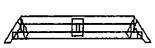
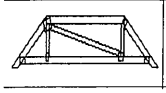
Delivery Shiplist

DATE	03/27/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287675 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** A STD+ OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	1 / 2 Ply	T1S HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	402.44 / 251.00		
	1	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.24 / 90.17		
	1	T2S HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	146.95 / 94.16		
	1	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.78 / 89.67		
	1	T3S HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	155.54 / 100.83		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	1	T9S HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	107.74 / 71.17		
	1	T11 HIP GIRDER	10.00 / 0.00	16-03-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	110.03 / 69.67		
	2	T10 COMMON	10.00 / 0.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	114.82 / 74.00		
	6	T10S SCISSORS	10.00 / 4.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	340.68 / 220.98		
	1	T13 HALF HIP	10.00 / 0.00	18-03-00	02-01-10	2 X 6	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	100.21 / 64.33		
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		
	3	PB2 PIGGYBACK	10.00 / 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.31 / 60.99		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

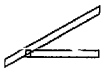
Delivery Shiplist

DATE	03/27/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287675 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** A STD+ OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	2	J4 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	37.98 24.00		
	8	J4S JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 05-06-07	209.28 148.00		
	1	C4 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	15.89 10.00		
	1	C1 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	8.36 6.00		
	1	C2 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	11.33 7.33		
	1	C3 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	12.92 8.67		
	10	J2 JACK-OPEN	4.00 0.00	05-05-00	02-01-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-10	143.50 93.30		
	13	J7 JACK-OPEN	10.00 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-00-07 02-11-12	117.26 78.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3541.95** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5568.46** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
8	Hangers	LUS24	

TOTAL # ITEMS= **14.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

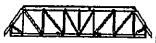
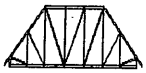
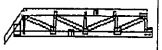
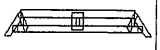
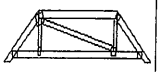
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296382 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	1 / 2 Ply	T1Z HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	2	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.48 / 180.34		
	2	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.56 / 179.34		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	3	T12 COMMON	10.00 / 0.00	19-11-00	09-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	287.10 / 179.01		
	1	G12 COMMON	10.00 / 0.00	19-11-00	09-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	103.99 / 66.67		
	1	T11 HIP GIRDER	10.00 / 0.00	16-03-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	110.03 / 69.67		
	2	T10 COMMON	10.00 / 0.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	114.82 / 74.00		
	6	T10S SCISSORS	10.00 / 4.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	340.68 / 220.98		
	1	T13 HALF HIP	10.00 / 0.00	18-03-00	02-01-10	2 X 6	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	100.21 / 64.33		
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		
	3	PB2 PIGGYBACK	10.00 / 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.31 / 60.99		
	9	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	151.11 / 96.03		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

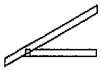
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296382 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: A REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	C1 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	8.36 6.00		
	1	C2 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	11.33 7.33		
	1	C3 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	12.92 8.67		
	1	C4 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	15.89 10.00		
	10	J2 JACK-OPEN	4.00 0.00	05-05-00	02-01-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-10	143.50 93.30		
	13	J7 JACK	10.00 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-00-07 02-11-12	117.26 78.00		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3518.99** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5577.24** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
8	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

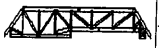
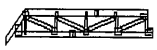
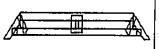
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287678 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: A REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1 HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	1 / 2 Ply	T1S HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	402.44 / 251.00		
	1	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.24 / 90.17		
	1	T2S HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	146.95 / 94.16		
	1	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.78 / 89.67		
	1	T3S HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	155.54 / 100.83		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	3	T12S ROOF	10.00 / 0.00	19-11-00	09-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	305.19 / 200.49		
	1	G12 COMMON	10.00 / 0.00	19-11-00	09-11-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	103.99 / 66.67		
	1	T11 HIP GIRDER	10.00 / 0.00	16-03-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	110.03 / 69.67		
	2	T10 COMMON	10.00 / 0.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	114.82 / 74.00		
	6	T10S SCISSORS	10.00 / 4.00	11-10-00	06-06-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	340.68 / 220.98		
	1	T13 HALF HIP	10.00 / 0.00	18-03-00	02-01-10	2 X 6	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	100.21 / 64.33		
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

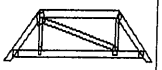
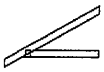
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287678 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** A REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	PB2 PIGGYBACK	10.00 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-13 00-04-13	89.31 60.99		
	9	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	151.11 96.03		
	1	C1 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -00-01-01	01-07-11 00-03-08	8.36 6.00		
	1	C2 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 01-10-15	01-07-11 00-05-13	11.33 7.33		
	1	C3 JACK-OPEN	10.00 0.00	05-10-08	03-01-09	2 X 4	2 X 4	01-03-08 -04-01-01	01-07-11 00-03-08	12.92 8.67		
	1	C4 JACK-OPEN	10.00 0.00	05-10-08	04-09-09	2 X 4	2 X 4	01-03-08 -02-01-01	01-07-11 00-03-08	15.89 10.00		
	10	J2 JACK-OPEN	4.00 0.00	05-05-00	02-01-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-01-10	143.50 93.30		
	13	J7 JACK	10.00 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 00-00-00	01-00-07 02-11-12	117.26 76.00		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3565.94** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5622.64** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
8	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

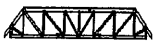
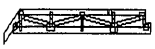
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296383 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** B STD OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T100S ROOF	10.00 / 0.00	32-11-00	08-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	456.40 / 272.66		
	1 / 2 Ply	T1Z HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.60 / 240.68		
	2	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.48 / 180.34		
	2	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.56 / 179.34		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	1	T9 HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	108.60 / 68.00		
	1	T26 SCISSOR	10.00 / 4.00	10-07-00	06-00-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	48.65 / 31.00		
	2	T26A SCISSOR	10.00 / 4.00	10-02-00	06-00-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-10-13	89.32 / 56.66		
	1	T27 HALF HIP	10.00 / 0.00	10-02-00	07-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 07-02-00	53.90 / 34.50		
	1	T21 HALF HIP	10.00 / 0.00	06-10-08	04-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-08-07	40.89 / 27.33		
	1	T22 HALF HIP	10.00 / 0.00	06-10-08	06-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-04-07	49.66 / 32.00		
	1	T23 HALF HIP	10.00 / 0.00	06-10-08	08-00-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-00-07	53.10 / 34.33		
	1	T24 JACK-CLOSED	10.00 / 0.00	06-10-08	08-10-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 08-10-07	47.23 / 31.50		
	1	T28 HALF HIP	10.00 / 0.00	18-03-00	01-09-15	2 X 8	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	98.06 / 64.33		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

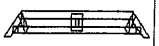
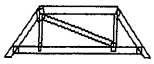
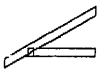
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296383 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** B STD OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		
	3	PB2 PIGGYBACK	10.00 / 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.31 / 60.99		
	11	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		
	10	J4 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	189.90 / 120.00		
	1	C1 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	9.99 / 7.00		
	1	C2 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	12.96 / 8.33		
	1	C6 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 04-03-01	01-07-11 / 00-03-08	17.90 / 12.33		
	1	C7 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 02-03-01	01-07-11 / 00-03-08	20.87 / 13.67		
	10	J60 JACK-OPEN	4.00 / 0.00	04-06-00	01-09-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-09-15	122.00 / 80.00		
	13	J7 JACK	10.00 / 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-07 / 02-11-12	117.26 / 78.00		

TOTAL # TRUSS= **86.00** TOTAL BFT OF ALL TRUSSES= **3393.33** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5385.82** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
1	Hangers	LJS26DS	
9	Hangers	LUS24	

TOTAL # ITEMS= **13.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

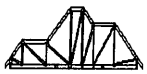
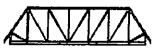
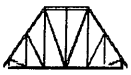
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287680 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** B STD OR REAR UPGRADE +OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T100S ROOF	10.00 / 0.00	32-11-00	08-10-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	456.40 / 272.66		
	1 / 2 Ply	T1S HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	402.44 / 251.00		
	1	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.24 / 90.17		
	1	T2S HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	146.95 / 94.16		
	1	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.78 / 89.67		
	1	T3S HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	155.54 / 100.83		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	310.60 / 197.66		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.42 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	331.06 / 208.00		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.46 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.23 / 800.31		
	1	T9S HIP GIRDER	10.00 / 0.00	19-11-00	06-06-07	2 X 4	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	107.74 / 71.17		
	1	T26 SCISSOR	10.00 / 4.00	10-07-00	06-00-09	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	48.65 / 31.00		
	2	T26A SCISSOR	10.00 / 4.00	10-02-00	06-00-09	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-10-13	89.32 / 56.66		
	1	T27 HALF HIP	10.00 / 0.00	10-02-00	07-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 07-02-00	53.90 / 34.50		
	1	T21 HALF HIP	10.00 / 0.00	06-10-08	04-08-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-08-07	40.89 / 27.33		
	1	T22 HALF HIP	10.00 / 0.00	06-10-08	06-04-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-04-07	49.66 / 32.00		
	1	T23 HALF HIP	10.00 / 0.00	06-10-08	08-00-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 08-00-07	53.10 / 34.33		

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TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

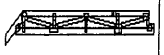
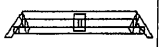
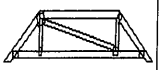
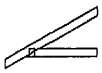
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287680 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** B STD OR REAR UPGRADE +OPT. COEF,

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T24 JACK-CLOSED	10.00 / 0.00	06-10-08	08-10-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 08-10-07	47.23 / 31.50		
	1	T28 HALF HIP	10.00 / 0.00	18-03-00	01-09-15	2 X 8	2 X 6	01-03-08 / 00-00-00	01-00-07 / 00-05-08	98.06 / 64.33		
	2	PB1 PIGGYBACK	10.00 / 0.00	08-11-04	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.72 / 31.00		
	3	PB2 PIGGYBACK	10.00 / 0.00	08-11-04	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.31 / 60.99		
	11	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	184.69 / 117.37		
	2	J4 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	37.98 / 24.00		
	8	J4S JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 05-06-07	209.28 / 148.00		
	1	C1 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 00-01-01	01-07-11 / 00-03-08	9.99 / 7.00		
	1	C2 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	01-03-08 / 01-10-15	01-07-11 / 00-05-13	12.96 / 8.33		
	1	C6 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 04-03-01	01-07-11 / 00-03-08	17.90 / 12.33		
	1	C7 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 02-03-01	01-07-11 / 00-03-08	20.87 / 13.67		
	10	J60 JACK-OPEN	4.00 / 0.00	04-06-00	01-09-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-09-15	122.00 / 80.00		
	13	J7 JACK	10.00 / 0.00	02-04-00	02-11-12	2 X 4	2 X 4	01-03-08 / 00-00-00	01-00-07 / 02-11-12	117.26 / 78.00		

TOTAL # TRUSS= **86.00** TOTAL BFT OF ALL TRUSSES= **3473.97** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5469.63** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
1	Hangers	LJS26DS	
9	Hangers	LUS24	

TOTAL # ITEMS= **13.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

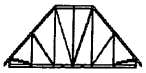
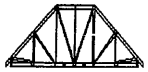
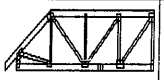
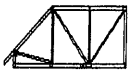
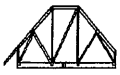
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296384 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: C STD. OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31 HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	388.80 / 240.00		
	1	T32 HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	133.34 / 83.17		
	1	T33 HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	145.05 / 92.33		
	1	T34 HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	148.27 / 94.50		
	1	T35 HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	145.26 / 90.33		
	9	T36 HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1366.74 / 850.50		
	2	T37 HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	315.68 / 198.66		
	1 / 3 Ply	T38 HIP GIRDER	8.00 / 0.00	32-11-00	09-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	669.66 / 404.01		
	1	T47 HIP GIRDER	8.00 / 0.00	19-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	98.73 / 60.33		
	1	T39 HALF HIP	8.00 / 0.00	16-03-08	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 04-01-04	90.89 / 57.17		
	1	T40 HALF HIP	8.00 / 0.00	16-03-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	69.38 / 43.67		
	1	T41 HALF HIP	8.00 / 0.00	16-03-08	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-04	76.01 / 48.00		
	1	T42 HALF HIP	8.00 / 0.00	16-03-08	07-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-01-04	77.99 / 49.67		
	1	T43 HALF HIP	8.00 / 0.00	16-03-08	08-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-01-04	82.99 / 52.83		
	1 / 3 Ply	T44 JACK-CLOSED	6.00 / 0.00	15-10-08	09-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 09-01-04	272.46 / 165.00		
	2	T45X HIP GIRDER	8.00 / 0.00	16-10-08	05-11-13	2 X 4	2 X 6	02-03-08 / 00-00-00	02-00-13 / 01-07-13	172.92 / 110.66		
	2	T46X HIP	8.00 / 0.00	16-10-08	06-11-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 01-07-13	163.14 / 104.00		
	1 / 2 Ply	T48 HALF HIP	9.00 / 0.00	18-03-00	01-05-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-00 / 00-03-08	128.74 / 83.66		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

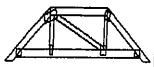
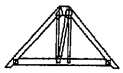
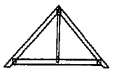
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296384 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** C STD. OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB31 PIGGYBACK	8.00 / 0.00	08-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	54.22 / 36.00		
	2	PB32 PIGGYBACK	8.00 / 0.00	08-03-13	02-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	57.50 / 39.00		
	4	PB33 PIGGYBACK	8.00 / 0.00	08-03-13	03-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	99.88 / 62.00		
	21	J13 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	352.59 / 224.07		
	2 / 2 Ply	J13Z JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-00-06 / 00-00-00	01-02-00 / 04-01-04	65.84 / 40.00		
	10	J37 JACK-OPEN	8.00 / 0.00	05-10-08	05-11-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 05-11-13	199.60 / 126.70		
	3	J33 JACK-OPEN	8.00 / 0.00	05-10-08	03-03-02	2 X 4	2 X 4	02-03-08 / -04-01-01	02-00-13 / 00-03-08	43.05 / 27.99		
	3	J34 JACK-OPEN	8.00 / 0.00	05-10-08	04-07-02	2 X 4	2 X 4	02-03-08 / -02-01-01	02-00-13 / 00-03-08	51.30 / 32.01		
	2	J35 JACK-OPEN	8.00 / 0.00	01-10-08	03-03-02	2 X 4	2 X 4	02-03-08 / -00-01-01	02-00-13 / 00-03-08	19.58 / 13.34		
	1	J35X JACK-OPEN	8.00 / 0.00	02-06-00	04-07-02	2 X 4	2 X 4	00-00-00 / -00-01-01	02-11-13 / 04-07-02	9.27 / 6.67		
	2	J36 JACK-OPEN	8.00 / 0.00	01-10-08	04-07-02	2 X 4	2 X 4	02-03-08 / 01-10-15	02-00-13 / 00-05-03	25.08 / 16.00		
	1	J36X JACK-OPEN	8.00 / 0.00	02-06-00	05-11-02	2 X 4	2 X 4	00-00-00 / 01-10-15	02-11-13 / 00-05-03	12.01 / 8.00		
	10	J31 JACK-OPEN	4.00 / 0.00	03-06-00	01-05-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-05-15	98.60 / 66.70		
	13	J32 JACK	9.00 / 0.00	02-04-00	02-08-00	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-00 / 02-08-00	112.06 / 78.00		

TOTAL # TRUSS= **113.00** TOTAL BFT OF ALL TRUSSES= **3604.97** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5746.63** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS26-3	
1	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

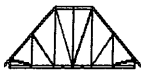
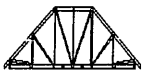
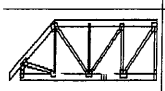
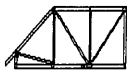
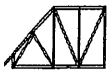
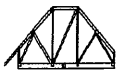
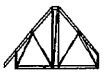
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287652 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** C STD.OR REAR UPGRADE 10 PT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31S HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	394.44 / 247.00		
	1	T32S HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	144.05 / 92.67		
	1	T33S HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	149.27 / 95.17		
	1	T34 HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	148.27 / 94.50		
	1	T35 HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	145.26 / 90.33		
	9	T36 HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1366.74 / 850.50		
	2	T37 HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	315.68 / 198.66		
	1 / 3 Ply	T38 HIP GIRDER	8.00 / 0.00	32-11-00	09-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	669.66 / 404.01		
	1	T47S HIP GIRDER	8.00 / 0.00	19-11-00	04-01-04	2 X 4	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	99.60 / 64.00		
	1	T39 HALF HIP	8.00 / 0.00	16-03-08	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 04-01-04	90.89 / 57.17		
	1	T40 HALF HIP	8.00 / 0.00	16-03-08	05-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 05-01-04	69.38 / 43.67		
	1	T41 HALF HIP	8.00 / 0.00	16-03-08	06-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 06-01-04	76.01 / 48.00		
	1	T42 HALF HIP	8.00 / 0.00	16-03-08	07-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 07-01-04	77.99 / 49.67		
	1	T43 HALF HIP	8.00 / 0.00	16-03-08	08-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-04-13 / 08-01-04	82.99 / 52.83		
	1 / 3 Ply	T44 JACK-CLOSED	6.00 / 0.00	15-10-08	09-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	01-02-00 / 09-01-04	272.46 / 165.00		
	2	T45X HIP GIRDER	8.00 / 0.00	16-10-08	05-11-13	2 X 4	2 X 6	02-03-08 / 00-00-00	02-00-13 / 01-07-13	172.92 / 110.66		
	2	T46X HIP	8.00 / 0.00	16-10-08	06-11-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 01-07-13	163.14 / 104.00		
	1 / 2 Ply	T48 HALF HIP	9.00 / 0.00	18-03-00	01-05-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-00 / 00-03-08	128.74 / 83.66		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

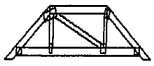
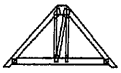
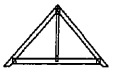
Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287652 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-5 ELEVATION: C STD.OR REAR UPGRADE + OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB31 PIGGYBACK	8.00 / 0.00	08-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	54.22 / 36.00		
	2	PB32 PIGGYBACK	8.00 / 0.00	08-03-13	02-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	57.50 / 39.00		
	4	PB33 PIGGYBACK	8.00 / 0.00	08-03-13	03-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	99.88 / 62.00		
	13	J13 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	218.27 / 138.71		
	2 / 2 Ply	J13Z JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-00-06 / 00-00-00	01-02-00 / 04-01-04	65.84 / 40.00		
	8	J13S JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 / 00-00-00	01-02-00 / 03-01-04	170.16 / 125.36		
	10	J37 JACK-OPEN	8.00 / 0.00	05-10-08	05-11-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 05-11-13	199.60 / 126.70		
	3	J33 JACK-OPEN	8.00 / 0.00	05-10-08	03-03-02	2 X 4	2 X 4	02-03-08 / -04-01-01	02-00-13 / 00-03-08	43.05 / 27.99		
	3	J34 JACK-OPEN	8.00 / 0.00	05-10-08	04-07-02	2 X 4	2 X 4	02-03-08 / -02-01-01	02-00-13 / 00-03-08	51.30 / 32.01		
	2	J35 JACK-OPEN	8.00 / 0.00	01-10-08	03-03-02	2 X 4	2 X 4	02-03-08 / -00-01-01	02-00-13 / 00-03-08	19.58 / 13.34		
	1	J35X JACK-OPEN	8.00 / 0.00	02-06-00	04-07-02	2 X 4	2 X 4	00-00-00 / -00-01-01	02-11-13 / 04-07-02	9.27 / 6.67		
	2	J36 JACK-OPEN	8.00 / 0.00	01-10-08	04-07-02	2 X 4	2 X 4	02-03-08 / 01-10-15	02-00-13 / 00-05-03	25.08 / 16.00		
	1	J36X JACK-OPEN	8.00 / 0.00	02-06-00	05-11-02	2 X 4	2 X 4	00-00-00 / 01-10-15	02-11-13 / 00-05-03	12.01 / 8.00		
	10	J31 JACK-OPEN	4.00 / 0.00	03-06-00	01-05-15	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-05-15	98.60 / 66.70		
	13	J32 JACK	9.00 / 0.00	02-04-00	02-08-00	2 X 4	2 X 4	01-03-08 / 00-00-00	00-11-00 / 02-08-00	112.06 / 78.00		

TOTAL # TRUSS= **113.00** TOTAL BFT OF ALL TRUSSES= **3667.98** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5803.91** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
1	Hangers	HGUS28-3	
1	Hangers	HGUS26-2	

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	03/28/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287652 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-5 **ELEVATION:** C STD.OR REAR UPGRADE to PT. COFF.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
6	Hangers	LJS26DS	

TOTAL # ITEMS= **8.00**

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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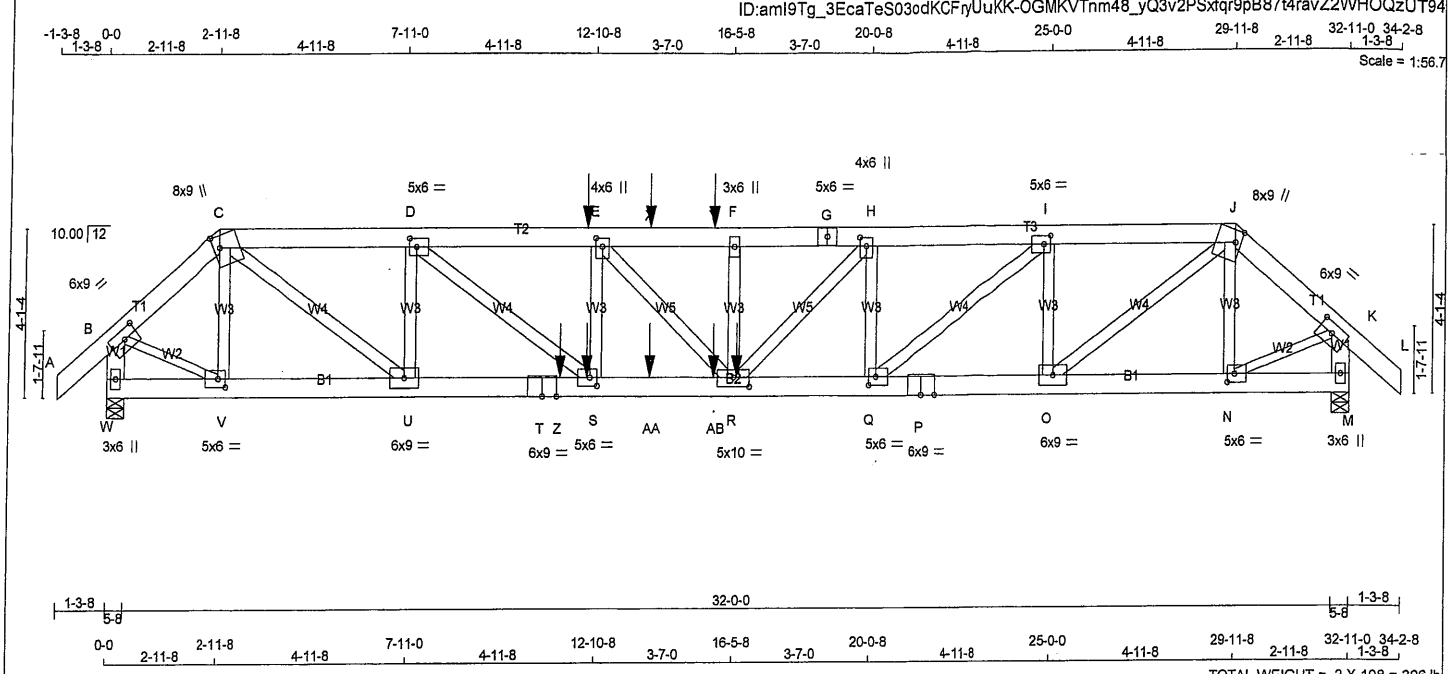

TOTAL WEIGHT = 2 X 198 = 396 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - V	2x3	DRY	No.2	SPF
N - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	SIDE(0.0)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	TOP
T- P	2	12	SIDE(0.0)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	8.0	9.0	3.50	1.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMWW+t	MT20	4.0	6.0	2.50	2.00
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0	2.50	2.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TTWW+m	MT20	8.0	9.0	3.50	1.75
K	TMVW-t	MT20	6.0	9.0	2.75	4.25
M	BMV1+p	MT20	3.0	6.0		
N, Q, S, V						
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	5241	0	5241	0	0	5-8	5-8
M	4664	0	4664	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	4090	2515 / 0	667 / 0	0 / 0	0 / 0	908 / 0	0 / 0
M	3645	2233 / 0	600 / 0	0 / 0	0 / 0	812 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M
BEARING SIZE FACTOR = 1.15 AT JNT(S) W, M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00	V- C	-1344 / 0	0.11 (1)
B- C	-5323 / 0	-112.4	-112.4	0.08 (1)	5.02	C- U	0 / 7364	0.65 (1)
C- D	-9994 / 0	-112.4	-112.4	0.28 (1)	3.71	U- D	-3851 / 0	0.33 (1)
D- E	-14408 / 0	-112.4	-112.4	0.47 (1)	2.98	D- S	0 / 5507	0.49 (1)
E- X	-14951 / 0	-112.4	-112.4	0.51 (1)	2.86	S- E	-1253 / 0	0.11 (1)
X- Y	-14951 / 0	-112.4	-112.4	0.51 (1)	2.86	Q- H	-2756 / 0	0.24 (1)
Y- F	-14951 / 0	-112.4	-112.4	0.51 (1)	2.86	Q- I	0 / 5019	0.44 (1)
F- G	-14951 / 0	-112.4	-112.4	0.46 (1)	2.92	O- I	-3582 / 0	0.31 (1)
G- H	-14951 / 0	-112.4	-112.4	0.46 (1)	2.92	O- J	0 / 6291	0.56 (1)
H- I	-12692 / 0	-112.4	-112.4	0.38 (1)	3.25	N- J	-1092 / 0	0.09 (1)
I- J	-8669 / 0	-112.4	-112.4	0.23 (1)	3.99	B- V	0 / 4318	0.53 (1)
J- K	-4717 / 0	-112.4	-112.4	0.07 (1)	5.27	N- K	0 / 3827	0.47 (1)
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00	R- F	-527 / 0	0.05 (1)
W- B	-5219 / 0	0.0	0.0	0.19 (1)	6.39	E- R	0 / 775	0.07 (1)
M- K	-4663 / 0	0.0	0.0	0.17 (1)	6.69	R- H	0 / 3223	0.28 (1)
W- V	0 / 0	-28.0	-28.0	0.02 (3)	10.00			
V- U	0 / 4012	-28.0	-28.0	0.14 (1)	10.00			
U- T	0 / 9994	-28.0	-28.0	0.47 (1)	10.00			
T- Z	0 / 9994	-28.0	-28.0	0.47 (1)	10.00			
Z- S	0 / 9994	-28.0	-28.0	0.47 (1)	10.00			
S-AA	0 / 14408	-28.0	-28.0	0.48 (1)	10.00			
AA-AB	0 / 14408	-28.0	-28.0	0.48 (1)	10.00			
AB- R	0 / 14408	-28.0	-28.0	0.48 (1)	10.00			
R- Q	0 / 12692	-28.0	-28.0	0.43 (1)	10.00			
Q- P	0 / 8669	-28.0	-28.0	0.26 (1)	10.00			
P- O	0 / 8669	-28.0	-28.0	0.26 (1)	10.00			
O- N	0 / 3559	-28.0	-28.0	0.12 (1)	10.00			
N- M	0 / 0	-28.0	-28.0	0.03 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	12-7-12	-135	-135	---	BACK	VERT	TOTAL
R	16-6-8	-2568	-2568	---	BACK	VERT	TOTAL
S	12-7-12	-40	-70	---	BACK	VERT	TOTAL
X	14-3-4	-135	-135	---	BACK	VERT	TOTAL
Y	15-11-4	-135	-135	---	BACK	VERT	TOTAL
Z	11-11-8	-1865	-1865	---	BACK	VERT	TOTAL
AA	14-3-4	-40	-70	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 773 (0.51")

CSI: TC=0.51/1.00 (E-F:1) , BC=0.48/1.00 (R-S:1) , WB=0.65/1.00 (C-U:1) , SSI=0.55/1.00 (S-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (D) (INPUT = 0.90)
USI METAL= 0.89 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4316
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pg 1/2

DWG NO. TAM 17158 -18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-t	MT20	6.0	9.0		
R	BMWWW-t	MT20	5.0	10.0	2.75	5.00
T	BS-t	MT20	6.0	9.0		
U	BMWW-t	MT20	6.0	9.0		
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 135.3 lbs FACTORED DOWN AT 12-7-12, AND 135.3 lbs FACTORED DOWN AT 14-3-4, AND 135.3 lbs FACTORED DOWN AT 15-11-4 ON TOP CHORD, AND 1865.4 lbs FACTORED DOWN AT 11-11-8, 69.9 lbs FACTORED DOWN AT 12-7-12, 69.9 lbs FACTORED DOWN AT 14-3-4, AND 69.9 lbs FACTORED DOWN AT 15-11-4, AND 2568.4 lbs FACTORED DOWN AT 16-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AB	15-11-4	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/3/18
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PROVINCE OF ONTARIO

DWG NO. TAM17158-B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296381	T1Z	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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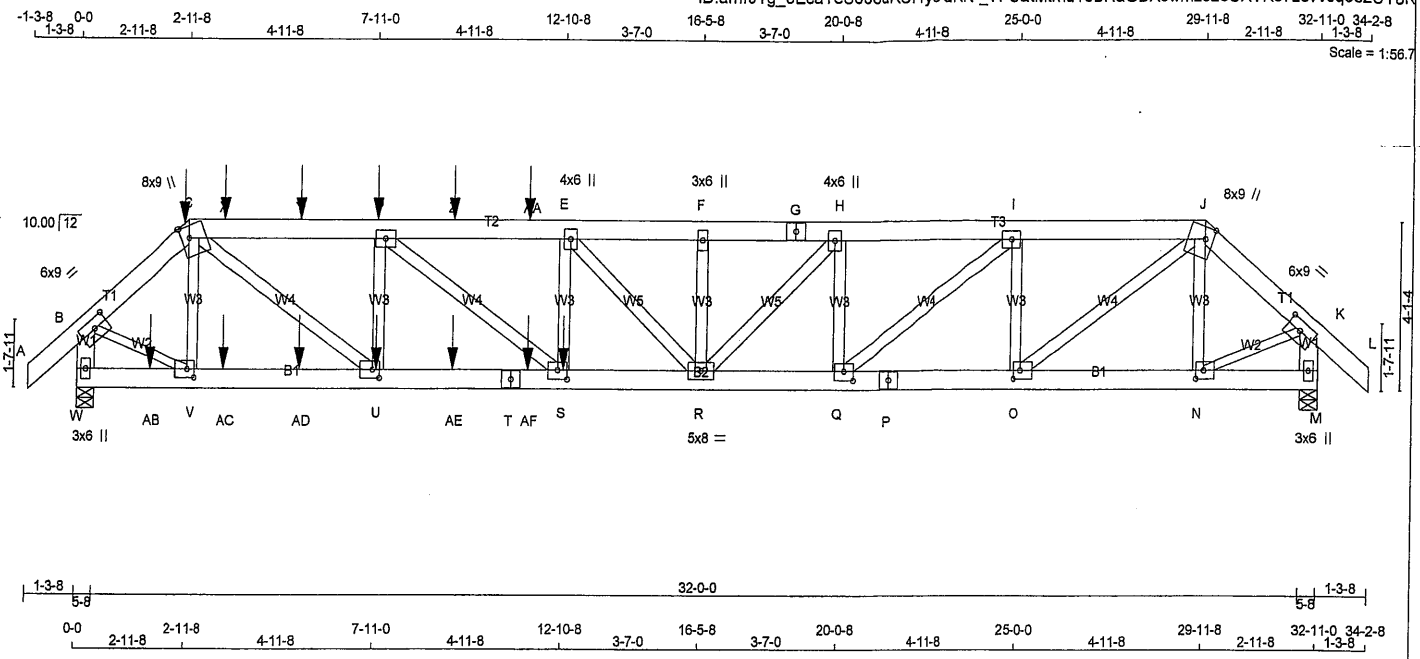

TOTAL WEIGHT = 2 X 198 = 396 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x6	DRY	2100F 1.8E	SPF
T - P	2x6	DRY	2100F 1.8E	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
B - V	2x3	DRY	No.2	SPF
N - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(0.0)
C- G	2	12	SIDE(0.0)
G- J	2	12	TOP
J- L	2	12	TOP
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- T	2	12	SIDE(0.0)
T- P	2	12	SIDE(0.0)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.50
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.50	2.50
K	TMVW-t	MT20	6.0	9.0	2.75	4.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW-t	MT20	5.0	6.0	2.50	2.25
P	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	4751	0	4751	0	0	5-8	5-8
M	3537	0	3537	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3748	2243 / 0	660 / 0	0 / 0	0 / 0	845 / 0	0 / 0
M	2793	1667 / 0	494 / 0	0 / 0	0 / 0	631 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BEARING SIZE FACTOR = 1.15 AT JNT(S) W, M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-4794 / 0	-112.4	-112.4	0.08 (1)	5.24
C- X	-8110 / 0	-112.4	-112.4	0.27 (1)	4.06
X- Y	-8110 / 0	-112.4	-112.4	0.27 (1)	4.06
Y- D	-8110 / 0	-112.4	-112.4	0.27 (1)	4.06
D- Z	-10780 / 0	-112.4	-112.4	0.35 (1)	3.52
Z-AA	-10780 / 0	-112.4	-112.4	0.35 (1)	3.52
AA- E	-10780 / 0	-112.4	-112.4	0.35 (1)	3.52
E- F	-9983 / 0	-112.4	-112.4	0.23 (1)	3.76
F- G	-9983 / 0	-112.4	-112.4	0.23 (1)	3.76
G- H	-9983 / 0	-112.4	-112.4	0.23 (1)	3.76
H- I	-8720 / 0	-112.4	-112.4	0.22 (1)	4.00
I- J	-6217 / 0	-112.4	-112.4	0.16 (1)	4.65
J- K	-3479 / 0	-112.4	-112.4	0.06 (1)	5.94
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
W- B	-4733 / 0	0.0	0.0	0.17 (1)	6.65
M- K	-3525 / 0	0.0	0.0	0.13 (1)	7.46
W-AB	0 / 0	-28.0	-28.0	0.03 (2)	10.00
AB- V	0 / 0	-28.0	-28.0	0.03 (2)	10.00
V-AC	0 / 3619	-28.0	-28.0	0.12 (1)	10.00
AC-AD	0 / 3619	-28.0	-28.0	0.12 (1)	10.00
AD- U	0 / 3619	-28.0	-28.0	0.12 (1)	10.00
U-AE	0 / 8110	-28.0	-28.0	0.27 (1)	10.00
AE- T	0 / 8110	-28.0	-28.0	0.27 (1)	10.00
T-AF	0 / 8110	-28.0	-28.0	0.27 (1)	10.00
AF- S	0 / 8110	-28.0	-28.0	0.27 (1)	10.00
S- R	0 / 10780	-28.0	-28.0	0.33 (1)	10.00
R- Q	0 / 8720	-28.0	-28.0	0.26 (1)	10.00
Q- P	0 / 6217	-28.0	-28.0	0.19 (1)	10.00
P- O	0 / 6217	-28.0	-28.0	0.19 (1)	10.00
O- N	0 / 2626	-28.0	-28.0	0.09 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-1055 / 0	0.09 (1)
C- U	0 / 5527	0.49 (1)
U- D	-2977 / 0	0.25 (1)
D- S	0 / 3332	0.29 (1)
S- E	0 / 292	0.03 (2)
Q- H	-1747 / 0	0.15 (1)
Q- I	0 / 3124	0.28 (1)
O- I	-2462 / 0	0.21 (1)
O- J	0 / 4420	0.39 (1)
N- J	-779 / 0	0.07 (1)
B- V	0 / 3890	0.48 (1)
N- K	0 / 2823	0.35 (1)
R- F	-360 / 0	0.03 (1)
E- R	-1137 / 0	0.17 (1)
R- H	0 / 1802	0.16 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-45	-50	---	FRONT	VERT	DEAD
C	2-11-8	-282	-282	---	FRONT	VERT	SNOW
D	7-11-4	-135	-135	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.36")

CSI: TC=0.35/1.00 (D-E:1) , BC=0.33/1.00 (R-S:1) , WB=0.49/1.00 (C-U:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.71 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4326 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17177 -18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296381	T1Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	5.0	6.0	2.75	3.00
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.75	3.00
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 2-11-8, 136.5 lbs FACTORED DOWN AT 3-11-4, 135.3 lbs FACTORED DOWN AT 5-11-4, 135.3 lbs FACTORED DOWN AT 7-11-4, AND 135.3 lbs FACTORED DOWN AT 9-11-4, AND 135.3 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, AND 69.9 lbs FACTORED DOWN AT 11-11-4, AND 2097.6 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	12-10-8	-2098	-2098	—	FRONT	VERT	TOTAL
U	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
X	3-11-4	-136	-136	—	FRONT	VERT	TOTAL
Y	5-11-4	-135	-135	—	FRONT	VERT	TOTAL
Z	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
AA	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
AB	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AC	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AD	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
AE	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AF	11-11-4	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4378
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17177-B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T1S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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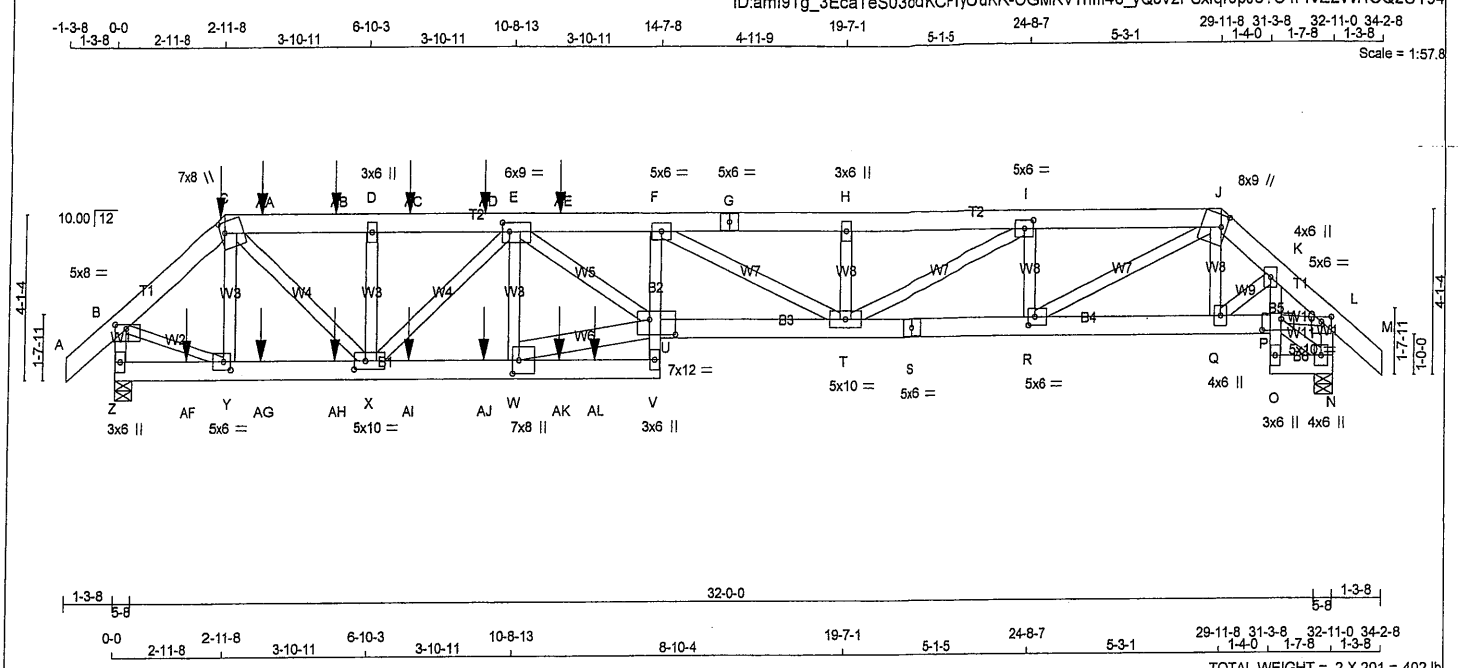

TOTAL WEIGHT = 2 X 201 = 402 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - V	2x6	DRY	No.2	SPF
V - F	2x4	DRY	No.2	SPF
U - S	2x6	DRY	1650F 1.5E	SPF
S - P	2x6	DRY	1650F 1.5E	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
W - U	2x6	DRY	No.2	SPF
B - Y	2x3	DRY	No.2	SPF
P - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(0.0)
C- G	2	12	SIDE(0.0)
G- J	2	12	TOP
J- M	2	12	TOP
Z- B	1	12	TOP
N- L	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- V	2	12	SIDE(0.0)
U- S	2	12	TOP
S- P	2	12	TOP
O- N	2	12	TOP
V- F	1	12	TOP
K- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	7.0	8.0	3.25	1.50
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	6.0	9.0	2.75	2.25
F	TMVW-t	MT20	5.0	6.0		
G	TS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	4889	0	4889	0	0	5-8	5-8
N	3637	0	3637	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	3851	2319 / 0	673 / 0	0 / 0	0 / 0	859 / 0	0 / 0
N	2867	1719 / 0	503 / 0	0 / 0	0 / 0	645 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-4948 / 0	-112.4	-112.4	0.08 (1)	5.18
C-AA	-7433 / 0	-112.4	-112.4	0.20 (1)	4.29
AA-AB	-7433 / 0	-112.4	-112.4	0.20 (1)	4.29
AB- D	-7433 / 0	-112.4	-112.4	0.20 (1)	4.29
D-AC	-7433 / 0	-112.4	-112.4	0.16 (1)	4.33
AC-AD	-7433 / 0	-112.4	-112.4	0.16 (1)	4.33
AD- E	-7433 / 0	-112.4	-112.4	0.16 (1)	4.33
E-AE	-15078 / 0	-112.4	-112.4	0.50 (1)	2.86
AE- F	-15078 / 0	-112.4	-112.4	0.50 (1)	2.86
F- G	-12712 / 0	-112.4	-112.4	0.46 (1)	3.16
G- H	-12712 / 0	-112.4	-112.4	0.46 (1)	3.16
H- I	-12712 / 0	-112.4	-112.4	0.39 (1)	3.24
I- J	-8961 / 0	-112.4	-112.4	0.27 (1)	3.90
J- K	-4812 / 0	-112.4	-112.4	0.13 (1)	5.16
K- L	-4663 / 0	-112.4	-112.4	0.13 (1)	5.23
L- M	0 / 52	-112.4	-112.4	0.05 (1)	10.00
Z- B	-4875 / 0	0.0	0.0	0.27 (1)	5.42
N- L	-3534 / 0	0.0	0.0	0.20 (1)	6.23
Z-AF	0 / 0	-28.0	-28.0	0.06 (2)	10.00
AF- Y	0 / 0	-28.0	-28.0	0.06 (2)	10.00
Y-AG	0 / 3738	-28.0	-28.0	0.35 (1)	10.00
AG-AH	0 / 3738	-28.0	-28.0	0.35 (1)	10.00
AH- X	0 / 3738	-28.0	-28.0	0.35 (1)	10.00
X-AI	0 / 10462	-28.0	-28.0	0.97 (1)	10.00
AI-AJ	0 / 10462	-28.0	-28.0	0.97 (1)	10.00
AJ- W	0 / 10462	-28.0	-28.0	0.97 (1)	10.00
W-AK	0 / 300	-28.0	-28.0	0.71 (1)	10.00
AK-AL	0 / 300	-28.0	-28.0	0.71 (1)	10.00
AL- V	0 / 300	-28.0	-28.0	0.71 (1)	10.00
V- U	0 / 1196	0.0	0.0	0.33 (1)	10.00
U- F	0 / 894	0.0	0.0	0.31 (1)	10.00
U- T	0 / 15195	-28.0	-28.0	0.67 (1)	10.00
T- S	0 / 8961	-28.0	-28.0	0.43 (1)	10.00
S- R	0 / 8961	-28.0	-28.0	0.43 (1)	10.00
R- Q	0 / 3749	-28.0	-28.0	0.21 (1)	10.00
Q- P	0 / 3542	-28.0	-28.0	0.20 (1)	10.00
O- P	0 / 36	0.0	0.0	0.09 (1)	10.00
P- K	-213 / 0	0.0	0.0	0.09 (1)	7.81
O- N	0 / 115	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-1035 / 0	0.09 (1)
C- X	0 / 5000	0.44 (1)
X- D	-686 / 0	0.06 (1)
X- E	-4189 / 0	0.66 (1)
W- E	-1094 / 0	0.09 (1)
W- U	0 / 10532	0.68 (1)
E- U	0 / 5639	0.50 (1)
Q- J	0 / 156	0.01 (3)
Q- K	0 / 264	0.02 (1)
B- Y	0 / 4014	0.50 (1)
R- J	0 / 5819	0.51 (1)
F- T	-2795 / 0	0.55 (1)
R- I	-2503 / 0	0.16 (1)
T- H	-620 / 0	0.04 (1)
T- I	0 / 4224	0.37 (1)
P- N	-142 / 0	0.01 (1)
P- L	0 / 3454	0.43 (1)

LICENSED PROFESSIONAL ENGINEER
4378
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 632 (0.62")

CSI: TC=0.50/1.00 (E-F:1) , BC=0.97/1.00 (W-X:1) , WB=0.68/1.00 (U-W:1) , SSI=0.43/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17159-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T1S	1	2	TRUSS DESC.	

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	8.0	9.0	3.50	1.75
K	TMVW+p	MT20	4.0	6.0		
L	TMVW-p	MT20	5.0	6.0	1.50	3.00
N	BMVW1+p	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWW-l	MT20	5.0	10.0	3.25	6.00
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BS-t	MT20	5.0	6.0		
T	BMWWW-t	MT20	5.0	10.0		
U	BVMWW-l	MT20	7.0	12.0	4.50	8.25
V	BMV+p	MT20	3.0	6.0		
W	BMWW+t	MT20	7.0	8.0	4.00	2.00
X	BMWWW-t	MT20	5.0	10.0	2.50	3.75
Y	BMWW-t	MT20	5.0	6.0	2.50	2.25
Z	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 314.5 lbs FACTORED DOWN AT 2-11-8, 136.5 lbs FACTORED DOWN AT 3-11-4, 135.3 lbs FACTORED DOWN AT 5-11-4, 135.3 lbs FACTORED DOWN AT 7-11-4, AND 135.3 lbs FACTORED DOWN AT 9-11-4, AND 135.3 lbs FACTORED DOWN AT 11-11-4 ON TOP CHORD; AND 69.9 lbs FACTORED DOWN AT 1-11-4, 69.9 lbs FACTORED DOWN AT 3-11-4, 69.9 lbs FACTORED DOWN AT 5-11-4, 69.9 lbs FACTORED DOWN AT 7-11-4, 69.9 lbs FACTORED DOWN AT 9-11-4, AND 69.9 lbs FACTORED DOWN AT 11-11-4, AND 2348.2 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-282	-282	—	FRONT	VERT	SNOW
AA	3-11-4	-136	-136	—	FRONT	VERT	TOTAL
AB	5-11-4	-135	-135	—	FRONT	VERT	TOTAL
AC	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
AD	9-11-4	-135	-135	—	FRONT	VERT	TOTAL
AE	11-11-4	-135	-135	—	FRONT	VERT	TOTAL
AF	1-11-4	-40	-70	—	FRONT	VERT	TOTAL
AG	3-11-4	-40	-70	—	FRONT	VERT	TOTAL
AH	5-11-4	-40	-70	—	FRONT	VERT	TOTAL
AI	7-11-4	-40	-70	—	FRONT	VERT	TOTAL
AJ	9-11-4	-40	-70	—	FRONT	VERT	TOTAL
AK	11-11-4	-40	-70	—	FRONT	VERT	TOTAL
AL	12-10-8	-2348	-2348	—	FRONT	VERT	TOTAL

JSI GRIP= 0.88 (X) (INPUT = 0.90)
JSI METAL= 0.95 (W) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17159 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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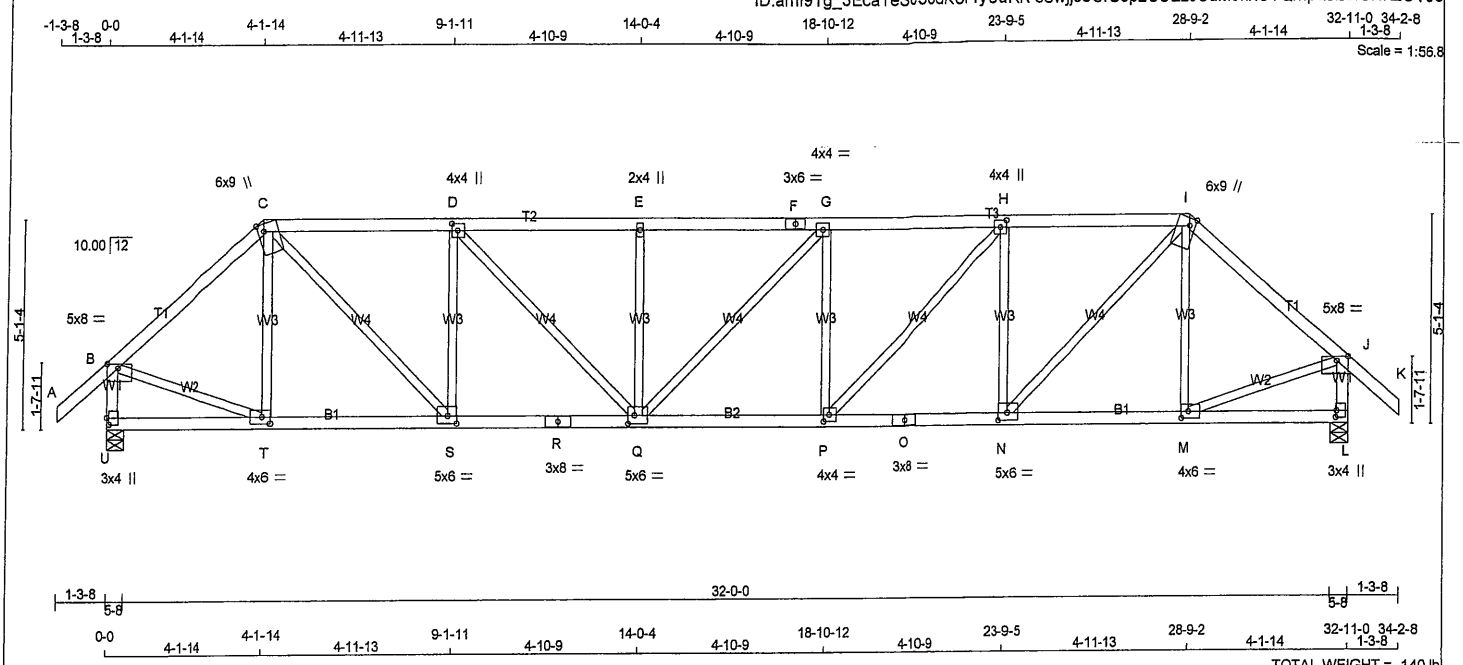

TOTAL WEIGHT = 140 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW+t	MT20	4.0	4.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW+t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMV1+p	MT20	3.0	4.0	2.00	0.25
M	BMWW-t	MT20	4.0	6.0	2.00	2.25
N	BMWW-t	MT20	5.0	6.0	2.25	2.75
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWWW-t	MT20	5.0	6.0	2.50	2.00
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.25	2.75
T	BMWW-t	MT20	4.0	6.0	2.00	2.25
U	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM (PLF)	TO (PLF)	VERT. LOAD LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)		10.00
B- C	-2317 / 0	-112.4	-112.4	0.46 (1)		4.04
C- D	-3236 / 0	-112.4	-112.4	0.60 (1)		3.34
D- E	-3912 / 0	-112.4	-112.4	0.68 (1)		2.98
E- F	-3912 / 0	-112.4	-112.4	0.56 (1)		3.14
F- G	-3912 / 0	-112.4	-112.4	0.56 (1)		3.14
G- H	-3914 / 0	-112.4	-112.4	0.68 (1)		2.98
H- I	-3235 / 0	-112.4	-112.4	0.60 (1)		3.35
I- J	-2317 / 0	-112.4	-112.4	0.46 (1)		4.04
J- K	0 / 50	-112.4	-112.4	0.15 (1)		10.00
U- B	-2421 / 0	0.0	0.0	0.26 (1)		5.50
L- J	-2421 / 0	0.0	0.0	0.26 (1)		5.50
U- T	0 / 0	-28.0	-28.0	0.15 (3)		10.00
T- S	0 / 1767	-28.0	-28.0	0.36 (1)		10.00
S- R	0 / 3236	-28.0	-28.0	0.58 (1)		10.00
R- Q	0 / 3236	-28.0	-28.0	0.58 (1)		10.00
Q- P	0 / 3914	-28.0	-28.0	0.69 (1)		10.00
P- O	0 / 3235	-28.0	-28.0	0.58 (1)		10.00
O- N	0 / 3235	-28.0	-28.0	0.58 (1)		10.00
N- M	0 / 1767	-28.0	-28.0	0.37 (1)		10.00
M- L	0 / 0	-28.0	-28.0	0.15 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-397 / 0	0.15 (1)
M- I	-397 / 0	0.15 (1)
B- T	0 / 1857	0.42 (1)
M- J	0 / 1857	0.42 (1)
N- I	0 / 2037	0.46 (1)
C- S	0 / 2038	0.46 (1)
N- H	-1279 / 0	0.49 (1)
S- D	-1278 / 0	0.49 (1)
P- H	0 / 953	0.21 (1)
D- Q	0 / 950	0.21 (1)
P- G	-532 / 0	0.20 (1)
Q- E	-531 / 0	0.20 (1)
Q- G	-3 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
4376
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.69/1.00 (P-Q:1) , WB=0.49/1.00 (H-N:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (S) (INPUT = 0.90)
JSI METAL= 0.90 (R) (INPUT = 1.00)

DWG NO. TAM 17160-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T2S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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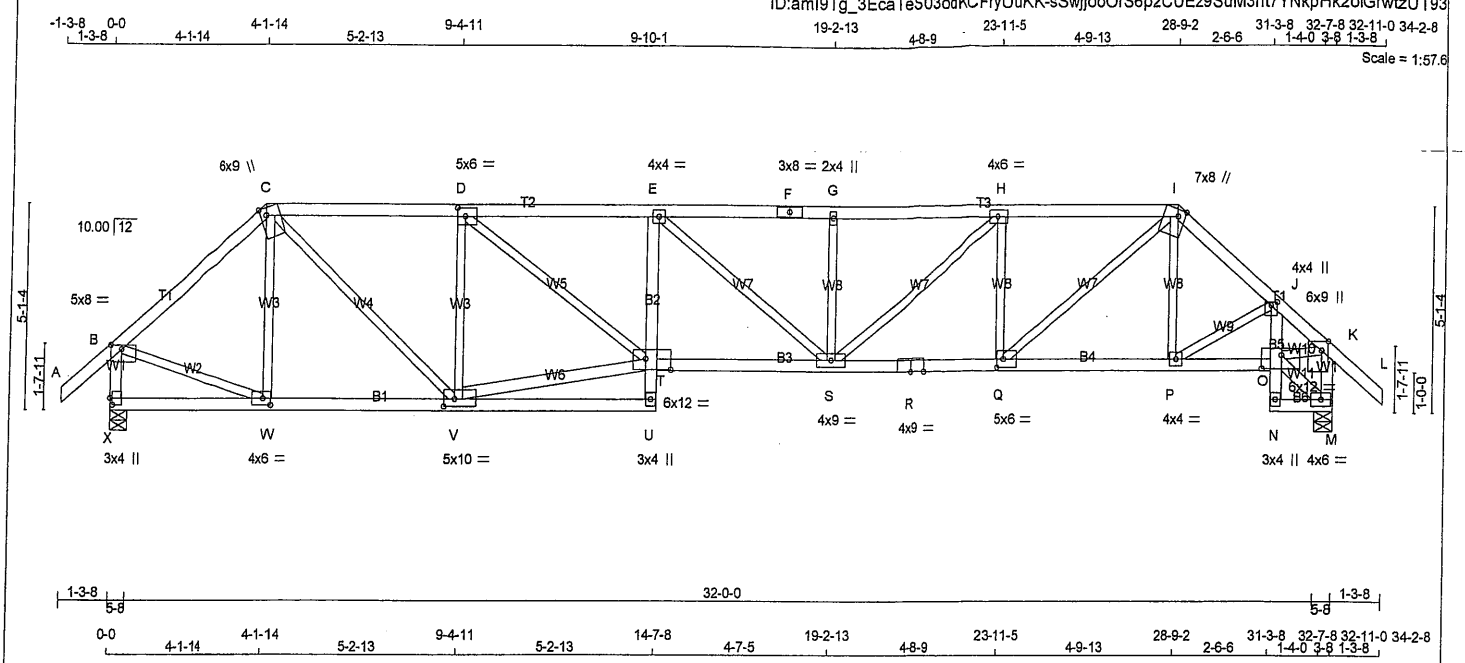

TOTAL WEIGHT = 147 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - E	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	6.0	9.0	2.75	2.00
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	4.00	6.25
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.00
R	BS-t	MT20	4.0	9.0		
S	BMWWW-t	MT20	4.0	9.0		
T	BVMWW-l	MT20	6.0	12.0	3.25	8.00
U	BMV+p	MT20	3.0	4.0		
V	BMWWW-t	MT20	5.0	10.0	2.25	3.25
W	BMWW-t	MT20	4.0	6.0	2.00	2.25
X	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2464	0	2464	0	0	5-8	5-8
M	2470	0	2470	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2313 / 0	-112.4	-112.4	0.46 (1)	4.05
C- D	-3273 / 0	-112.4	-112.4	0.68 (1)	3.23
D- E	-4944 / 0	-112.4	-112.4	0.95 (1)	2.39
E- F	-4885 / 0	-112.4	-112.4	0.67 (1)	2.73
F- G	-4885 / 0	-112.4	-112.4	0.67 (1)	2.73
G- H	-4885 / 0	-112.4	-112.4	0.78 (1)	2.59
H- I	-4008 / 0	-112.4	-112.4	0.65 (1)	2.99
I- J	-2882 / 0	-112.4	-112.4	0.24 (1)	3.91
J- K	-2982 / 0	-112.4	-112.4	0.24 (1)	3.83
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00
X- B	-2417 / 0	0.0	0.0	0.26 (1)	5.50
M- K	-2389 / 0	0.0	0.0	0.25 (1)	5.53
X- W	0 / 0	-28.0	-28.0	0.14 (3)	10.00
W- V	0 / 1763	-28.0	-28.0	0.37 (1)	10.00
V- U	0 / 135	-28.0	-28.0	0.20 (3)	10.00
U- T	0 / 102	0.0	0.0	0.20 (1)	10.00
T- E	-452 / 0	0.0	0.0	0.21 (1)	7.81
T- S	0 / 4981	-28.0	-28.0	0.89 (1)	10.00
S- R	0 / 4008	-28.0	-28.0	0.71 (1)	10.00
R- Q	0 / 4008	-28.0	-28.0	0.71 (1)	10.00
Q- P	0 / 2206	-28.0	-28.0	0.42 (1)	10.00
P- O	0 / 2325	-28.0	-28.0	0.44 (1)	10.00
N- O	0 / 36	0.0	0.0	0.13 (1)	10.00
O- J	-73 / 24	0.0	0.0	0.12 (1)	7.81
N- M	0 / 76	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-399 / 0	0.15 (1)
C- V	0 / 2061	0.46 (1)
V- D	-1874 / 0	0.72 (1)
P- I	0 / 168	0.04 (3)
P- J	-134 / 0	0.03 (1)
B- W	0 / 1853	0.42 (1)
V- T	0 / 3229	0.52 (1)
D- T	0 / 2047	0.46 (1)
Q- I	0 / 2294	0.52 (1)
E- S	-124 / 0	0.08 (1)
Q- H	-1292 / 0	0.33 (1)
S- G	-515 / 0	0.13 (1)
S- H	0 / 1127	0.25 (1)
O- M	-98 / 0	0.01 (1)
O- K	0 / 2264	0.51 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 699 (0.56")

CSI: TC=0.95/1.00 (D-E:1) , BC=0.89/1.00 (S-T:1) , WB=0.72/1.00 (D-V:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.80 (R) (INPUT = 1.00)

DWG NO. TAM 17161 -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T3	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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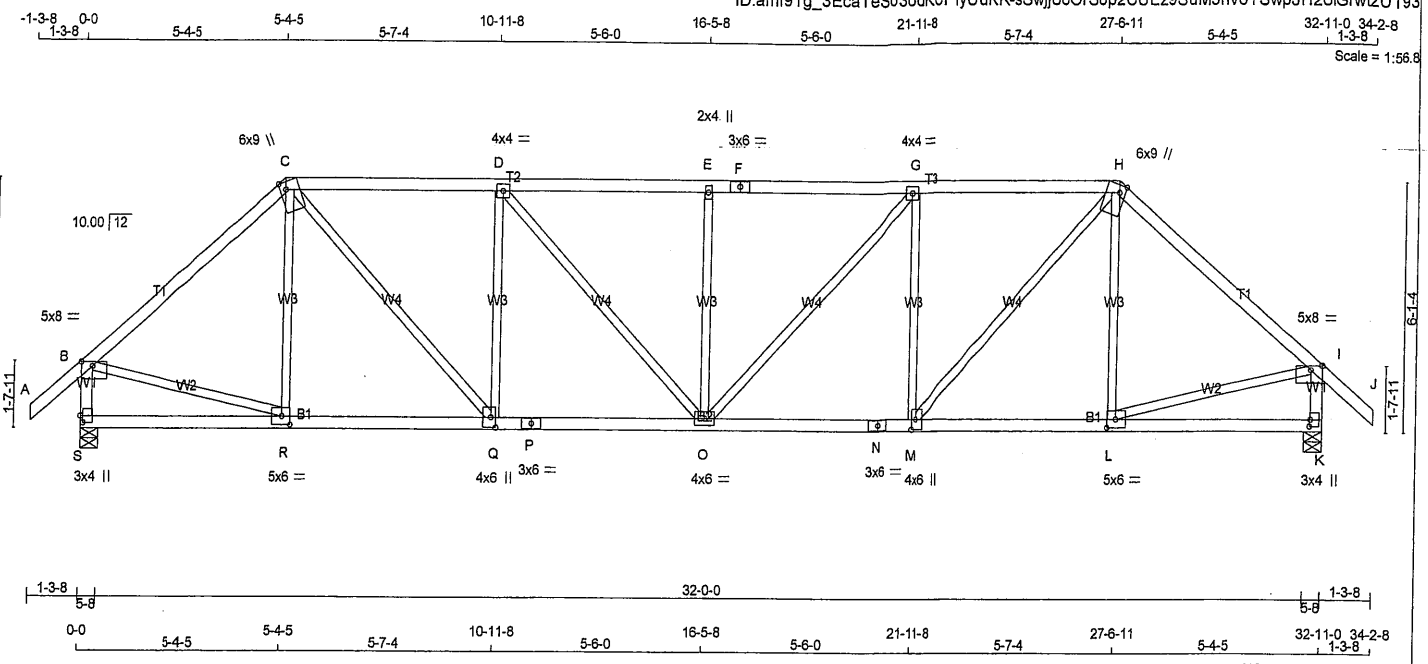

TOTAL WEIGHT = 142 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	1.25	3.50
K	BMV1+p	MT20	3.0	4.0	2.00	0.25
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	4.0	6.0	3.00	1.50
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW+t	MT20	4.0	6.0	3.00	1.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2467	0	2467	0	0	5-8	5-8
K	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
K	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2363 / 0	-112.4	-112.4	0.82 (1)	3.58
C- D	-2978 / 0	-112.4	-112.4	0.75 (1)	3.28
D- E	-3320 / 0	-112.4	-112.4	0.79 (1)	3.07
E- F	-3320 / 0	-112.4	-112.4	0.79 (1)	3.07
F- G	-3320 / 0	-112.4	-112.4	0.79 (1)	3.07
G- H	-2978 / 0	-112.4	-112.4	0.75 (1)	3.28
H- I	-2363 / 0	-112.4	-112.4	0.82 (1)	3.58
I- J	0 / 50	-112.4	-112.4	0.15 (1)	10.00
S- B	-2406 / 0	0.0	0.0	0.26 (1)	5.52
K- I	-2406 / 0	0.0	0.0	0.26 (1)	5.52
S- R	0 / 0	-28.0	-28.0	0.22 (3)	10.00
R- Q	0 / 1807	-28.0	-28.0	0.41 (2)	10.00
Q- P	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
P- O	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
O- N	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
N- M	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
M- L	0 / 1807	-28.0	-28.0	0.41 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-258 / 103	0.15 (1)
L- H	-258 / 103	0.15 (1)
B- R	0 / 1862	0.42 (1)
L- I	0 / 1862	0.42 (1)
M- H	0 / 1685	0.38 (1)
C- Q	0 / 1685	0.38 (1)
M- G	-1062 / 0	0.62 (1)
Q- D	-1062 / 0	0.62 (1)
O- G	0 / 497	0.11 (1)
D- O	0 / 497	0.11 (1)
O- E	-569 / 0	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.82/1.00 (B-C:1) , BC=0.55/1.00 (O-Q:1) , WB=0.62/1.00 (D-Q:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (P) (INPUT = 0.90)
JSI METAL= 0.75 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4318 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 171628
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T3S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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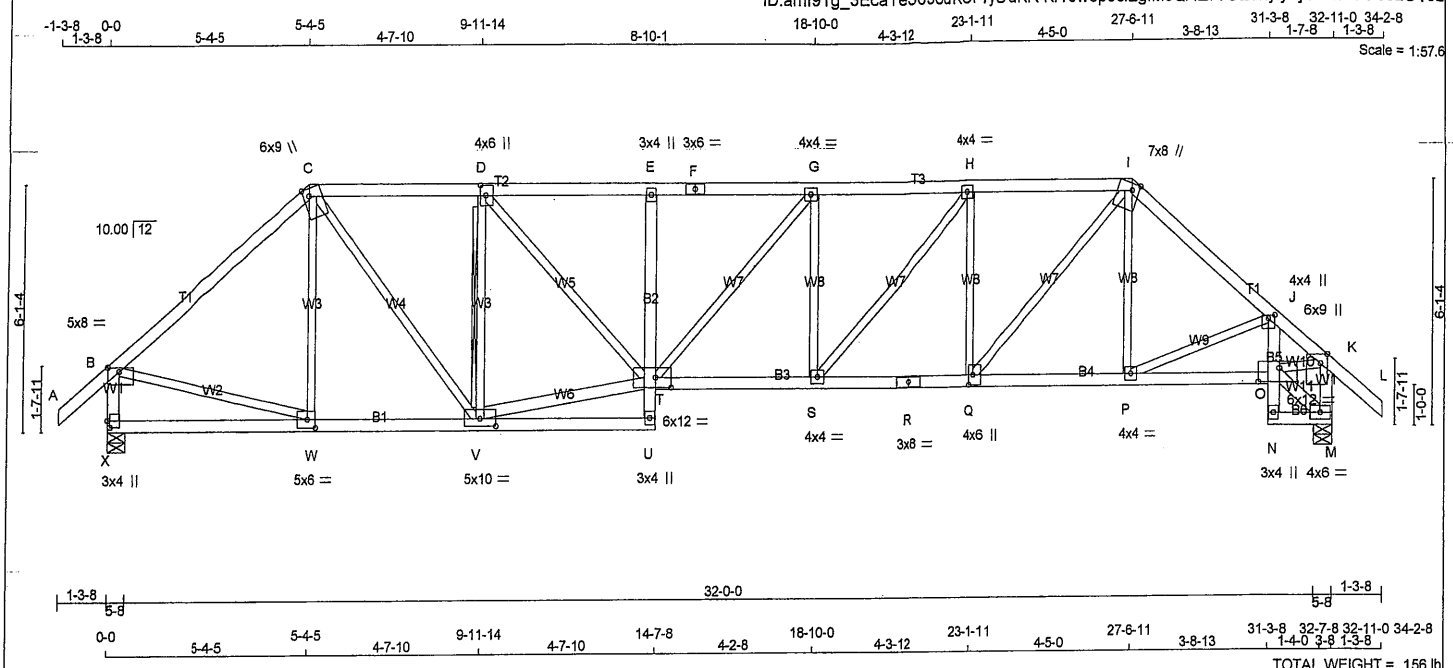

TOTAL WEIGHT = 156 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - E	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
V - T	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW+t	MT20	4.0	6.0	3.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	6.0	9.0	2.75	2.00
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	4.00	6.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW+t	MT20	4.0	6.0	3.00	1.50
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	4.0	4.0		
T	BVMWWW-l	MT20	6.0	12.0	3.00	5.00
U	BMV+p	MT20	3.0	4.0		
V	BMWWW-t	MT20	5.0	10.0	2.25	5.00
W	BMWW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2464	0	2464	0	0	5-8	5-8
M	2470	0	2470	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT D-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2356 / 0	-112.4	-112.4	0.82 (1)	3.59
C- D	-2796 / 0	-112.4	-112.4	0.48 (1)	3.70
D- E	-3897 / 0	-112.4	-112.4	0.59 (1)	3.06
E- F	-3916 / 0	-112.4	-112.4	0.45 (1)	3.23
F- G	-3916 / 0	-112.4	-112.4	0.45 (1)	3.23
G- H	-3896 / 0	-112.4	-112.4	0.54 (1)	3.13
H- I	-3333 / 0	-112.4	-112.4	0.46 (1)	3.43
I- J	-2829 / 0	-112.4	-112.4	0.38 (1)	3.82
J- K	-3029 / 0	-112.4	-112.4	0.24 (1)	3.80
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00
X- B	-2401 / 0	0.0	0.0	0.26 (1)	5.52
M- K	-2388 / 0	0.0	0.0	0.25 (1)	5.53
X- W	0 / 0	-28.0	-28.0	0.19 (3)	10.00
W- V	0 / 1801	-28.0	-28.0	0.39 (1)	10.00
V- U	0 / 88	-28.0	-28.0	0.13 (3)	10.00
U- T	0 / 92	0.0	0.0	0.13 (1)	10.00
T- E	-476 / 0	0.0	0.0	0.18 (1)	7.81
T- S	0 / 3896	-28.0	-28.0	0.70 (1)	10.00
S- R	0 / 3333	-28.0	-28.0	0.58 (1)	10.00
R- Q	0 / 3333	-28.0	-28.0	0.58 (1)	10.00
Q- P	0 / 2158	-28.0	-28.0	0.42 (1)	10.00
P- O	0 / 2391	-28.0	-28.0	0.45 (1)	10.00
N- O	0 / 36	0.0	0.0	0.13 (1)	10.00
O- J	-98 / 39	0.0	0.0	0.12 (1)	7.81
N- M	0 / 78	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-272 / 77	0.16 (1)
C- V	0 / 1606	0.36 (1)
V- D	-1731 / 0	0.82 (1)
P- I	0 / 214	0.05 (3)
P- J	-251 / 0	0.08 (1)
B- W	0 / 1857	0.42 (1)
V- T	0 / 2793	0.45 (1)
D- T	0 / 1566	0.36 (1)
O- M	-99 / 0	0.01 (1)
O- K	0 / 2332	0.52 (1)
Q- I	0 / 1734	0.39 (1)
T- G	-10 / 30	0.01 (3)
Q- H	-1161 / 0	0.45 (1)
S- G	-498 / 0	0.19 (1)
S- H	0 / 843	0.19 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.82/1.00 (B-C:1) , BC=0.70/1.00 (S-T:1) , WB=0.82/1.00 (D-V:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.90 (R) (INPUT = 1.00)

DWG NO. TAM 17163-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T4	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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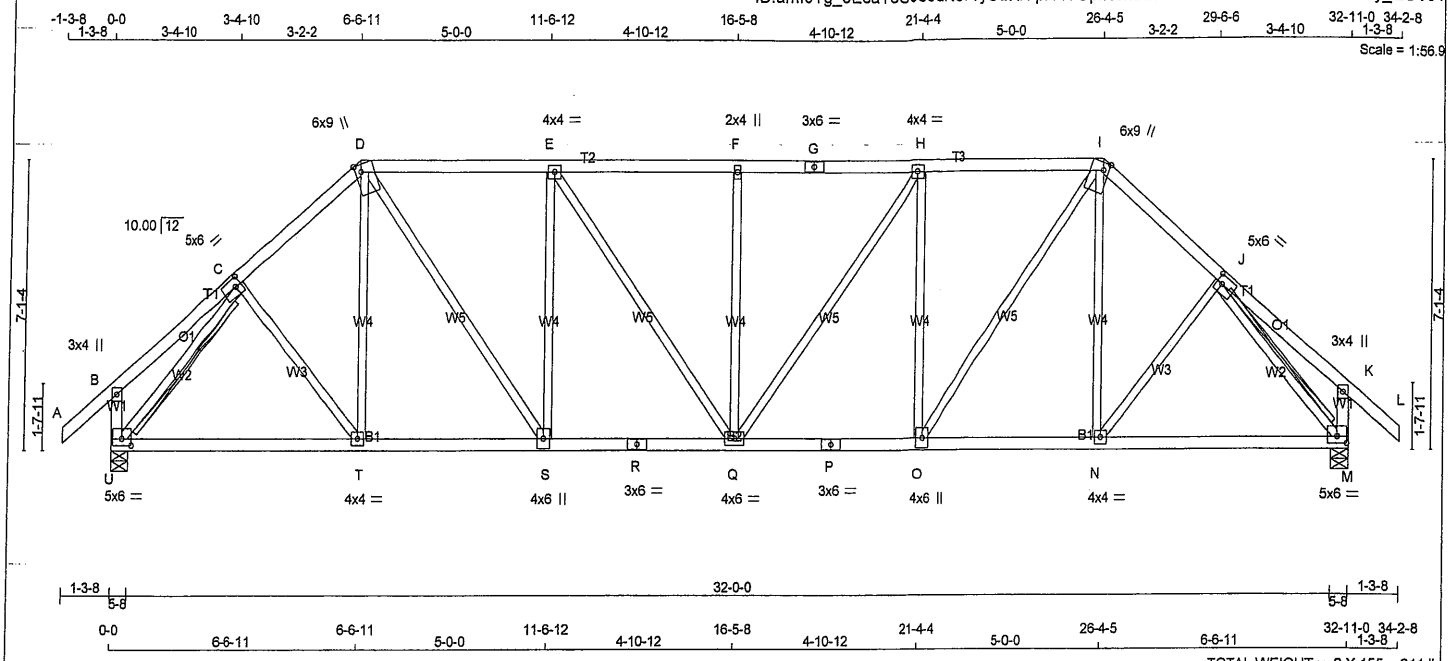

TOTAL WEIGHT = 2 X 155 = 311 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	3.00
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2467	0	2467	0	0	5-8	5-8
M	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
M	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 24	-112.4	-112.4	0.17 (1)	10.00
C- D	-2379 / 0	-112.4	-112.4	0.25 (1)	4.24
D- E	-2592 / 0	-112.4	-112.4	0.54 (1)	3.74
E- F	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
F- G	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
G- H	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
H- I	-2592 / 0	-112.4	-112.4	0.54 (1)	3.74
I- J	-2379 / 0	-112.4	-112.4	0.25 (1)	4.24
J- K	0 / 24	-112.4	-112.4	0.17 (1)	10.00
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00
U- B	-301 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-301 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 1692	-28.0	-28.0	0.45 (2)	10.00
T- S	0 / 1805	-28.0	-28.0	0.47 (2)	10.00
S- R	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
R- Q	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
O- N	0 / 1805	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 1692	-28.0	-28.0	0.45 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 201	0.05 (2)
T- D	0 / 204	0.05 (3)
D- S	0 / 1327	0.30 (1)
S- E	-948 / 0	0.83 (1)
E- Q	0 / 398	0.09 (1)
Q- F	-506 / 0	0.44 (1)
Q- H	0 / 398	0.09 (1)
O- H	-948 / 0	0.83 (1)
O- I	0 / 1327	0.30 (1)
N- I	0 / 204	0.05 (3)
N- J	0 / 201	0.05 (2)
U- C	-2688 / 0	0.96 (1)
J- M	-2688 / 0	0.96 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.56/1.00 (E-F:1) , BC=0.48/1.00 (Q-S:1) , WB=0.96/1.00 (C-U:1) , SSI=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.74 (R) (INPUT = 1.00)

DWG NO. TAM 17164 -18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T5	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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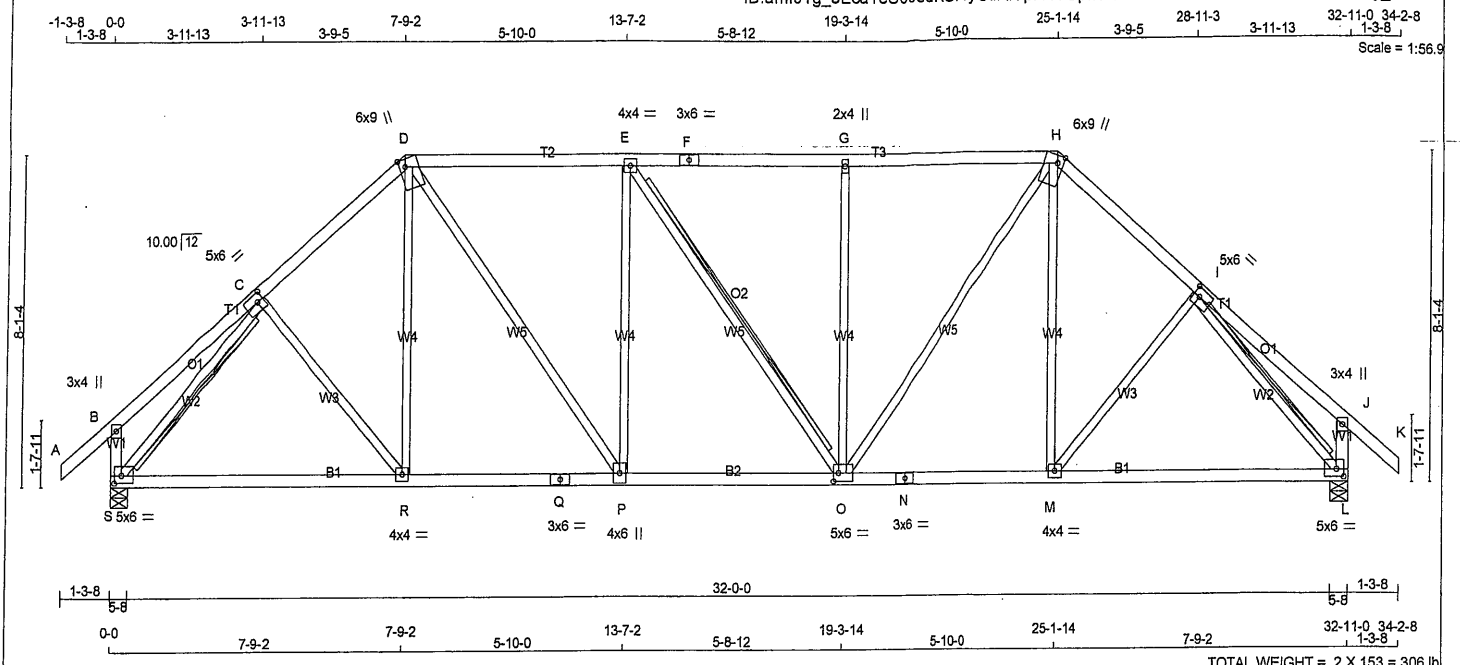

TOTAL WEIGHT = 2 X 153 = 306 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.25
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O, C-S, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 31	-112.4	-112.4	0.26 (1)	10.00
C- D	-2350 / 0	-112.4	-112.4	0.36 (1)	4.14
D- E	-2415 / 0	-112.4	-112.4	0.69 (1)	3.65
E- F	-2413 / 0	-112.4	-112.4	0.69 (1)	3.65
F- G	-2413 / 0	-112.4	-112.4	0.69 (1)	3.65
G- H	-2413 / 0	-112.4	-112.4	0.68 (1)	3.67
H- I	-2351 / 0	-112.4	-112.4	0.36 (1)	4.14
I- J	0 / 31	-112.4	-112.4	0.26 (1)	10.00
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
S- B	-324 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-324 / 0	0.0	0.0	0.03 (1)	7.81
S- R	0 / 1751	-28.0	-28.0	0.56 (2)	10.00
R- Q	0 / 1780	-28.0	-28.0	0.57 (2)	10.00
Q- P	0 / 1780	-28.0	-28.0	0.57 (2)	10.00
P- O	0 / 2415	-28.0	-28.0	0.47 (1)	10.00
O- N	0 / 1780	-28.0	-28.0	0.58 (2)	10.00
N- M	0 / 1780	-28.0	-28.0	0.58 (2)	10.00
M- L	0 / 1751	-28.0	-28.0	0.56 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 139	0.03 (3)
R- D	0 / 304	0.07 (3)
D- P	0 / 1059	0.24 (1)
P- E	-703 / 0	0.89 (1)
E- O	-3 / 0	0.00 (1)
O- G	-702 / 0	0.89 (1)
G- H	0 / 1056	0.24 (1)
M- H	0 / 305	0.07 (3)
M- I	0 / 139	0.03 (3)
S- C	-2698 / 0	0.73 (1)
I- L	-2698 / 0	0.73 (1)

LICENSED PROFESSIONAL ENGINEER
4376
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.58/1.00 (M-O:2) , WB=0.89/1.00 (E-P:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.66 (I) (INPUT = 1.00)

DWG NO. TAM 171654B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T6	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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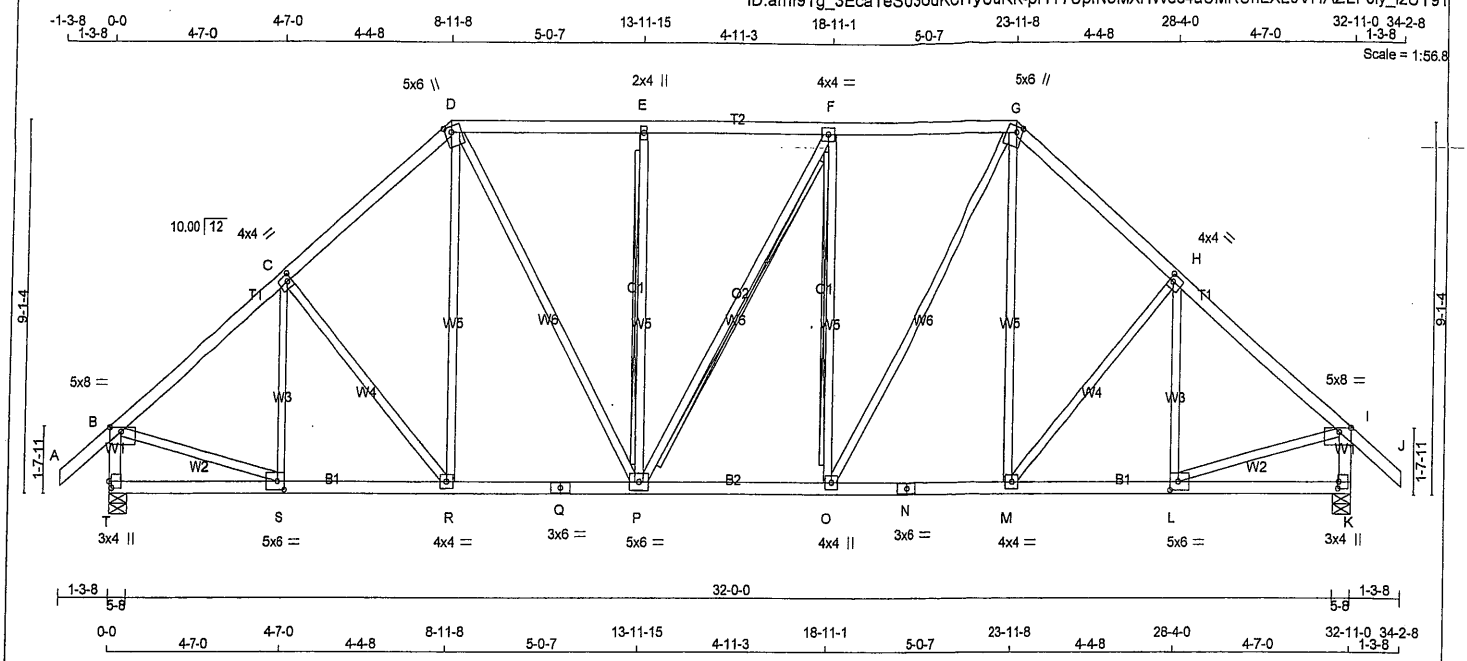

TOTAL WEIGHT = 2 X 166 = 331 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	1.75	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-p	MT20	5.0	8.0	1.25	3.50
K	BMV1+p	MT20	3.0	4.0	2.00	0.25
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW+t	MT20	4.0	4.0		
P	BMWWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMV1+p	MT20	3.0	4.0	2.00	0.75

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2467	0	2467	0	0	5-8	5-8
K	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
K	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-P, F-O
2x4 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2358 / 0	-112.4	-112.4	0.37 (1)	4.14
C- D	-2281 / 0	-112.4	-112.4	0.36 (1)	4.21
D- E	-2145 / 0	-112.4	-112.4	0.38 (1)	4.28
E- F	-2145 / 0	-112.4	-112.4	0.35 (1)	4.31
F- G	-2147 / 0	-112.4	-112.4	0.36 (1)	4.28
G- H	-2281 / 0	-112.4	-112.4	0.36 (1)	4.21
H- I	-2358 / 0	-112.4	-112.4	0.37 (1)	4.14
I- J	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-2412 / 0	0.0	0.0	0.26 (1)	5.51
K- I	-2412 / 0	0.0	0.0	0.26 (1)	5.51
T- S	0 / 0	-28.0	-28.0	0.14 (3)	10.00
S- R	0 / 1840	-28.0	-28.0	0.37 (1)	10.00
R- Q	0 / 1721	-28.0	-28.0	0.35 (1)	10.00
Q- P	0 / 1721	-28.0	-28.0	0.35 (1)	10.00
P- O	0 / 2147	-28.0	-28.0	0.42 (1)	10.00
O- N	0 / 1721	-28.0	-28.0	0.35 (1)	10.00
N- M	0 / 1721	-28.0	-28.0	0.35 (1)	10.00
M- L	0 / 1840	-28.0	-28.0	0.37 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-390 / 7	0.17 (1)
C- R	-185 / 0	0.16 (1)
R- D	0 / 315	0.07 (2)
D- P	0 / 852	0.19 (1)
P- E	-602 / 0	0.83 (1)
P- F	-4 / 0	0.00 (1)
O- F	-603 / 0	0.83 (1)
O- G	0 / 857	0.19 (1)
M- G	0 / 314	0.07 (2)
M- H	-185 / 0	0.16 (1)
L- H	-390 / 7	0.17 (1)
B- S	0 / 1913	0.43 (1)
L- I	0 / 1913	0.43 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.38/1.00 (D-E:1) , BC=0.42/1.00 (O-P:1) , WB=0.83/1.00 (F-O:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.50 (Q) (INPUT = 1.00)

DWG NO. TAM 171662
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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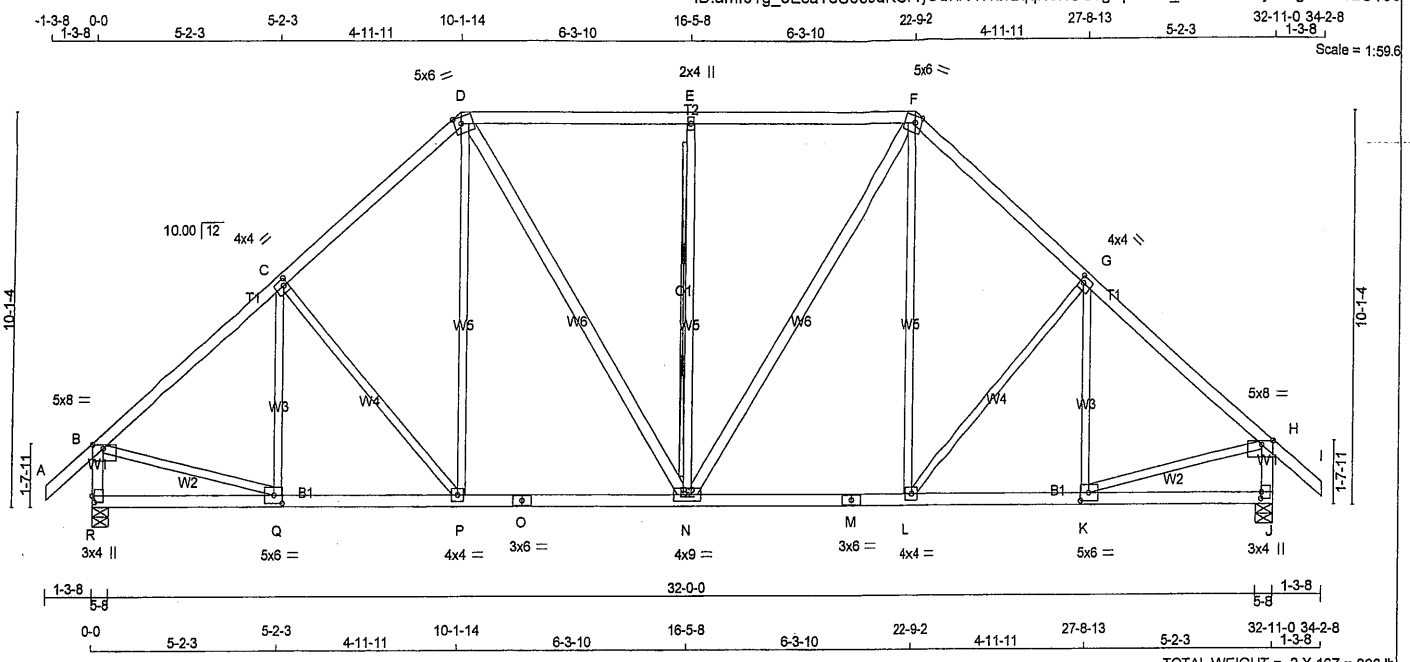

TOTAL WEIGHT = 2 X 167 = 333 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	1.25	3.50
J	BMV1+p	MT20	3.0	4.0	2.00	0.25
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	3.0	4.0	2.00	0.75

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2467	0	2467	0	0	5-8	5-8
J	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
J	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2387 / 0	-112.4	-112.4	0.49 (1)	3.99
C- D	-2213 / 0	-112.4	-112.4	0.46 (1)	4.14
D- E	-2003 / 0	-112.4	-112.4	0.64 (1)	3.97
E- F	-2003 / 0	-112.4	-112.4	0.64 (1)	3.97
F- G	-2213 / 0	-112.4	-112.4	0.46 (1)	4.14
G- H	-2387 / 0	-112.4	-112.4	0.49 (1)	3.99
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
R- B	-2405 / 0	0.0	0.0	0.26 (1)	5.52
J- H	-2405 / 0	0.0	0.0	0.26 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.17 (3)	10.00
Q- P	0 / 1867	-28.0	-28.0	0.40 (1)	10.00
P- O	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
O- N	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
N- M	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
M- L	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
L- K	0 / 1867	-28.0	-28.0	0.40 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-318 / 62	0.17 (1)
C- P	-306 / 0	0.35 (1)
P- D	0 / 414	0.09 (2)
D- N	0 / 623	0.10 (1)
N- E	-867 / 0	0.69 (1)
N- F	0 / 623	0.10 (1)
L- F	0 / 414	0.09 (2)
L- G	-306 / 0	0.35 (1)
K- G	-318 / 62	0.17 (1)
B- Q	0 / 1925	0.43 (1)
K- H	0 / 1925	0.43 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.42/1.00 (L-N:2) , WB=0.69/1.00 (E-N:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

DWG NO. TAM 17167-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T8	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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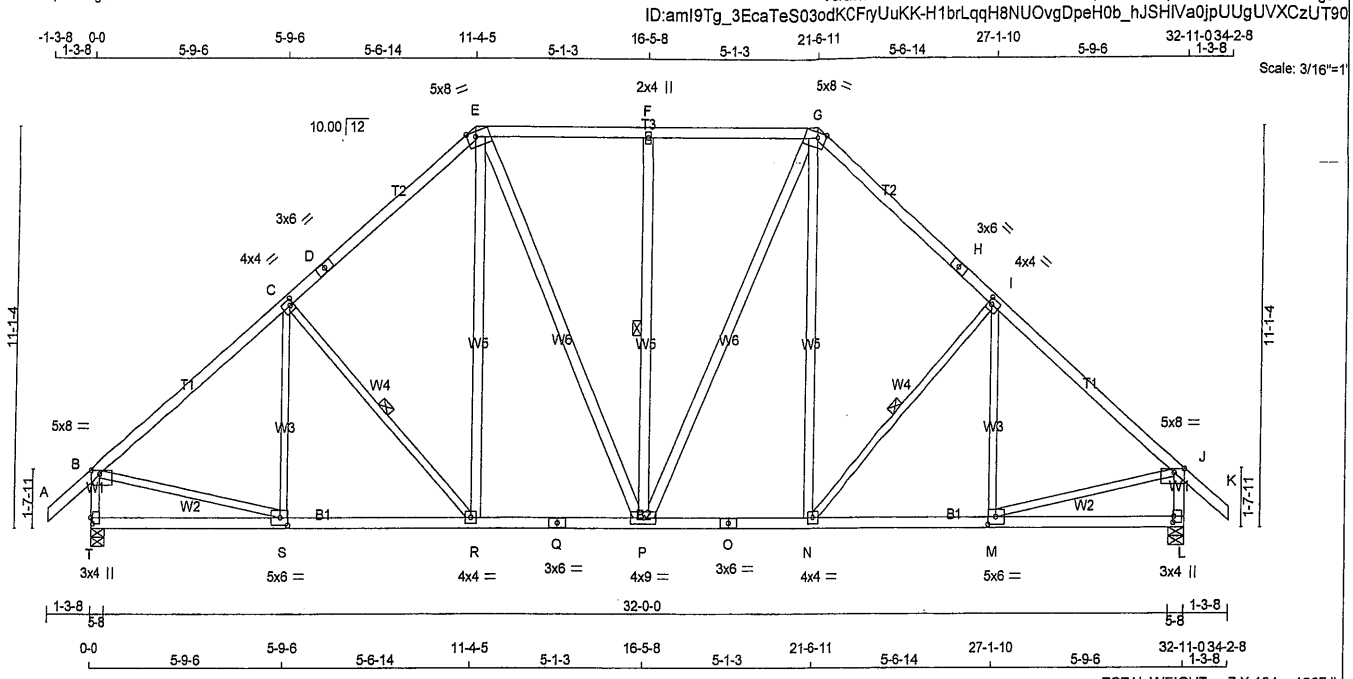

TOTAL WEIGHT = 7 X 184 = 1287 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMV1+p	MT20	3.0	4.0	2.00	0.25
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
B- C	-2405 / 0	-112.4	-112.4	0.63 (1)	3.81	
C- D	-2129 / 0	-112.4	-112.4	0.58 (1)	4.05	
D- E	-2129 / 0	-112.4	-112.4	0.58 (1)	4.05	
E- F	-1796 / 0	-112.4	-112.4	0.41 (1)	4.54	
F- G	-1796 / 0	-112.4	-112.4	0.41 (1)	4.54	
G- H	-2129 / 0	-112.4	-112.4	0.58 (1)	4.05	
H- I	-2129 / 0	-112.4	-112.4	0.58 (1)	4.05	
I- J	-2405 / 0	-112.4	-112.4	0.63 (1)	3.81	
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
T- B	-2400 / 0	0.0	0.0	0.26 (1)	5.52	
L- J	-2400 / 0	0.0	0.0	0.26 (1)	5.52	
T- S	0 / 0	-28.0	-28.0	0.24 (3)	10.00	
S- R	0 / 1885	-28.0	-28.0	0.43 (2)	10.00	
R- Q	0 / 1600	-28.0	-28.0	0.34 (1)	10.00	
Q- P	0 / 1600	-28.0	-28.0	0.34 (1)	10.00	
P- O	0 / 1600	-28.0	-28.0	0.34 (1)	10.00	
O- N	0 / 1600	-28.0	-28.0	0.34 (1)	10.00	
N- M	0 / 1885	-28.0	-28.0	0.43 (2)	10.00	
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-246 / 128	0.17 (1)
C- R	-427 / 0	0.20 (1)
R- E	0 / 461	0.07 (2)
E- P	0 / 459	0.07 (1)
P- F	-696 / 0	0.44 (1)
P- G	0 / 459	0.07 (1)
N- G	0 / 461	0.07 (2)
N- I	-427 / 0	0.20 (1)
M- I	-246 / 128	0.17 (1)
B- S	0 / 1933	0.43 (1)
M- J	0 / 1933	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.63/1.00 (B-C:1) , BC=0.43/1.00 (R-S:2) , WB=0.44/1.00 (F-P:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (S) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296381	T9	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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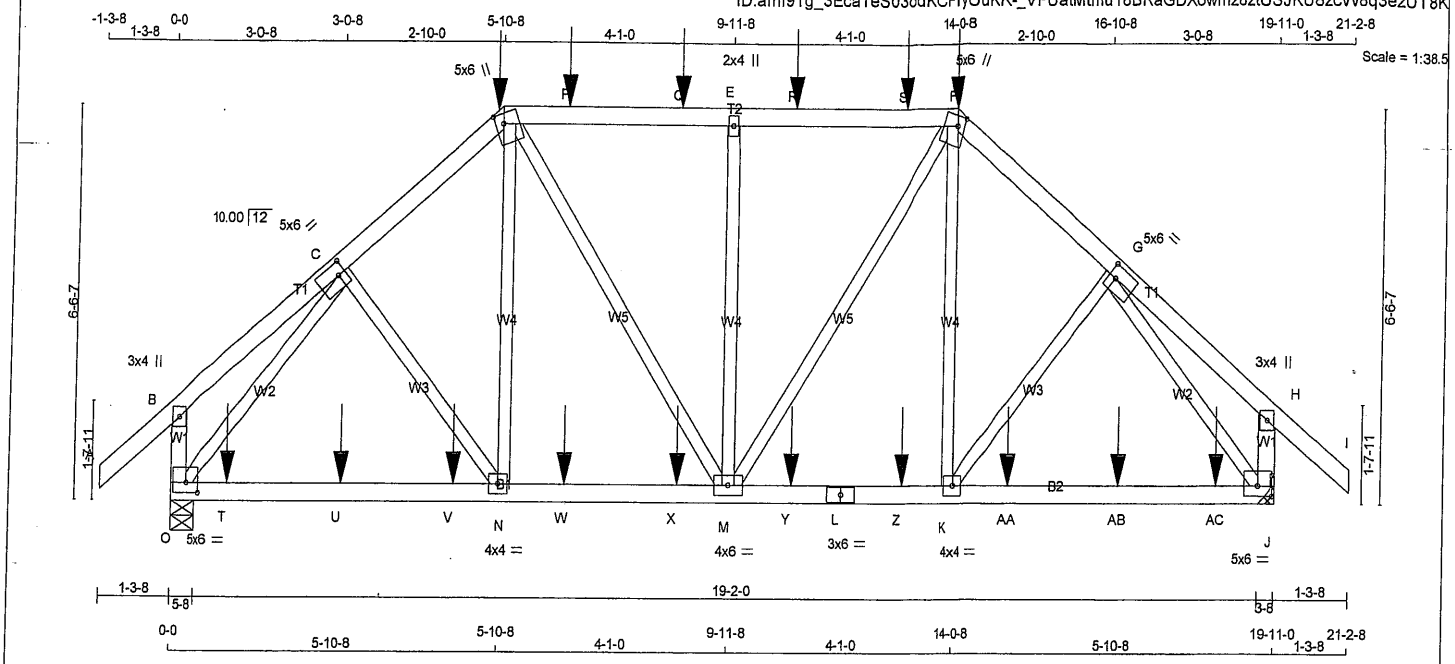

TOTAL WEIGHT = 97 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	1.75
G	TMWW-t	MT20	5.0	6.0	2.50	1.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0	2.00	3.00
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0	2.00	3.00

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 473.0 lbs FACTORED DOWN AT 5-10-8, 473.0 lbs FACTORED DOWN AT 14-0-8, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, AND 135.3 lbs FACTORED DOWN AT 11-0-12, AND 136.5 lbs FACTORED DOWN AT 13-0-12 ON TOP CHORD, AND 73.3 lbs FACTORED DOWN AT 1-0-12, 73.3 lbs FACTORED DOWN AT 3-0-12, 73.3 lbs FACTORED DOWN AT 5-0-12, 73.3 lbs FACTORED DOWN AT 7-0-12, 73.3 lbs FACTORED DOWN AT 9-0-12, 73.3 lbs FACTORED DOWN AT 11-0-12, 73.3 lbs FACTORED DOWN AT 13-0-12, 73.3 lbs FACTORED DOWN AT 15-0-12, AND 73.3 lbs FACTORED DOWN AT 17-0-12, AND 73.3 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	2503	0	2503	0	0	5-8	5-8
J	2513	0	2513	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1989	1164 / 0	365 / 0	0 / 0	0 / 0	460 / 0	0 / 0
J	1997	1167 / 0	368 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00
B- C	0 / 20	-112.4	-112.4	0.14 (1)	10.00
C- D	-2366 / 0	-112.4	-112.4	0.22 (1)	4.24
D- P	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
P- Q	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
Q- E	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
E- R	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
R- S	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
S- F	-2132 / 0	-112.4	-112.4	0.65 (1)	3.72
F- G	-2375 / 0	-112.4	-112.4	0.22 (1)	4.23
G- H	0 / 20	-112.4	-112.4	0.14 (1)	10.00
H- I	0 / 50	-112.4	-112.4	0.17 (1)	10.00
O- B	-289 / 0	0.0	0.0	0.03 (1)	7.81
J- H	-289 / 0	0.0	0.0	0.03 (1)	7.81
O- T	0 / 1636	-28.0	-28.0	0.62 (2)	10.00
T- U	0 / 1636	-28.0	-28.0	0.62 (2)	10.00
U- V	0 / 1636	-28.0	-28.0	0.62 (2)	10.00
V- N	0 / 1636	-28.0	-28.0	0.62 (2)	10.00
N- W	0 / 1798	-28.0	-28.0	0.65 (2)	10.00
W- X	0 / 1798	-28.0	-28.0	0.65 (2)	10.00
X- M	0 / 1798	-28.0	-28.0	0.65 (2)	10.00
M- Y	0 / 1805	-28.0	-28.0	0.66 (2)	10.00
Y- L	0 / 1805	-28.0	-28.0	0.66 (2)	10.00
L- Z	0 / 1805	-28.0	-28.0	0.66 (2)	10.00
Z- K	0 / 1805	-28.0	-28.0	0.66 (2)	10.00
K-AA	0 / 1641	-28.0	-28.0	0.63 (2)	10.00
AA-AB	0 / 1641	-28.0	-28.0	0.63 (2)	10.00
AB-AC	0 / 1641	-28.0	-28.0	0.63 (2)	10.00
AC- J	0 / 1641	-28.0	-28.0	0.63 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 253	0.06 (1)
N- D	0 / 390	0.10 (3)
D- M	0 / 609	0.15 (1)
M- E	-891 / 0	0.63 (1)
M- F	0 / 597	0.15 (1)
K- F	0 / 393	0.10 (3)
K- G	0 / 255	0.06 (1)
O- C	-2659 / 0	0.99 (1)
G- J	-2668 / 0	0.99 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-65	-73	—	FRONT	VERT	DEAD
D	5-10-8	-408	-408	—	FRONT	VERT	SNOW
F	14-0-8	-65	-73	—	FRONT	VERT	DEAD
F	14-0-8	-408	-408	—	FRONT	VERT	SNOW
P	7-0-12	-135	-135	—	BACK	VERT	TOTAL
Q	9-0-12	-135	-135	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
43B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.65/1.00 (E-F:1) , BC=0.66/1.00 (K-M:2) , WB=0.99/1.00 (G-J:1) , SSI=0.40/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 0.5 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.65 (C) (INPUT = 1.00) pb

DWG NO. TAM 1217 B-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296381	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	11-0-12	-135	-135	—	BACK	VERT	TOTAL
S	13-0-12	-137	-137	—	BACK	VERT	TOTAL
T	1-0-12	-42	-73	—	BACK	VERT	TOTAL
U	3-0-12	-42	-73	—	BACK	VERT	TOTAL
V	5-0-12	-42	-73	—	BACK	VERT	TOTAL
W	7-0-12	-42	-73	—	BACK	VERT	TOTAL
X	9-0-12	-42	-73	—	BACK	VERT	TOTAL
Y	11-0-12	-42	-73	—	BACK	VERT	TOTAL
Z	13-0-12	-42	-73	—	BACK	VERT	TOTAL
AA	15-0-12	-42	-73	—	BACK	VERT	TOTAL
AB	17-0-12	-42	-73	—	BACK	VERT	TOTAL
AC	18-10-4	-42	-73	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4378
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17178-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T9S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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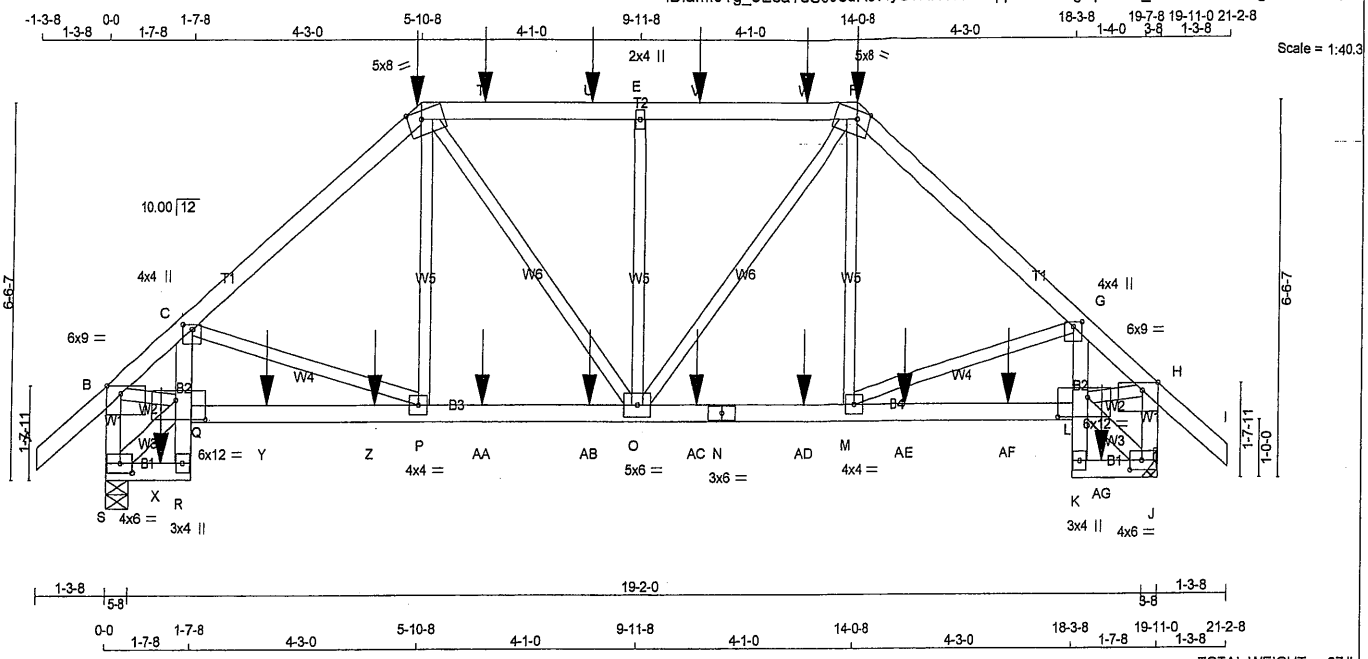

TOTAL WEIGHT = 97 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - Q	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMVW-p	MT20	6.0	9.0	Edge	3.50
J	BMVW1-t	MT20	4.0	6.0	2.00	2.75
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	12.0	4.00	6.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWW-l	MT20	6.0	12.0	4.00	6.75
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	6.0	2.00	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 473.0 lbs FACTORED DOWN AT 5-10-8, 473.0 lbs FACTORED DOWN AT 14-0-8, 114.8 lbs FACTORED DOWN AT 7-0-12, 114.8 lbs FACTORED DOWN AT 9-0-12, AND 114.8 lbs FACTORED DOWN AT 11-0-12, AND 116.0 lbs FACTORED DOWN AT 13-0-12 ON TOP CHORD, AND 87.6 lbs FACTORED DOWN AT 1-0-12, 138.9 lbs FACTORED DOWN AT 3-0-12, 138.9 lbs FACTORED DOWN AT 5-0-12, 138.9 lbs FACTORED DOWN AT 7-0-12, 138.9 lbs FACTORED DOWN AT 9-0-12, 138.9 lbs FACTORED DOWN AT 11-0-12, 138.9 lbs FACTORED DOWN AT 13-0-12, 138.9 lbs FACTORED DOWN AT 15-0-12, AND 138.9 lbs FACTORED DOWN AT 17-0-12, AND 87.6 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2855	0	2855	0	0	5-8	5-8
J	2872	0	2872	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2252	1343 / 0	396 / 0	0 / 0	0 / 0	514 / 0	0 / 0
J	2266	1351 / 0	399 / 0	0 / 0	0 / 0	517 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00
B- C	-3558 / 0	-112.4	-112.4	0.36 (1)	3.34
C- D	-3207 / 0	-112.4	-112.4	0.58 (1)	3.38
D- T	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
T- U	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
U- E	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
E- V	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
V- W	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
W- F	-2910 / 0	-112.4	-112.4	0.68 (1)	3.20
F- G	-3218 / 0	-112.4	-112.4	0.59 (1)	3.38
G- H	-3581 / 0	-112.4	-112.4	0.36 (1)	3.33
H- I	0 / 50	-112.4	-112.4	0.17 (1)	10.00
S- B	-2747 / 0	0.0	0.0	0.32 (1)	5.13
J- H	-2764 / 0	0.0	0.0	0.32 (1)	5.11
S- X	0 / 92	-28.0	-28.0	0.07 (2)	10.00
X- R	0 / 92	-28.0	-28.0	0.07 (2)	10.00
R- Q	0 / 99	0.0	0.0	0.17 (1)	10.00
Q- C	-12 / 155	0.0	0.0	0.16 (1)	7.81
Q- Y	0 / 2821	-28.0	-28.0	0.80 (1)	10.00
Y- Z	0 / 2821	-28.0	-28.0	0.80 (1)	10.00
Z- P	0 / 2821	-28.0	-28.0	0.80 (1)	10.00
P-AA	0 / 2452	-28.0	-28.0	0.73 (1)	10.00
AA-AB	0 / 2452	-28.0	-28.0	0.73 (1)	10.00
AB- O	0 / 2452	-28.0	-28.0	0.73 (1)	10.00
O-AC	0 / 2461	-28.0	-28.0	0.74 (1)	10.00
AC- N	0 / 2461	-28.0	-28.0	0.74 (1)	10.00
N-AD	0 / 2461	-28.0	-28.0	0.74 (1)	10.00
AD- M	0 / 2461	-28.0	-28.0	0.74 (1)	10.00
M-AE	0 / 2839	-28.0	-28.0	0.81 (1)	10.00
AE-AF	0 / 2839	-28.0	-28.0	0.81 (1)	10.00
AF- L	0 / 2839	-28.0	-28.0	0.81 (1)	10.00
K- L	0 / 99	0.0	0.0	0.17 (1)	10.00
L- G	0 / 163	0.0	0.0	0.16 (1)	10.00
K-AG	0 / 92	-28.0	-28.0	0.07 (2)	10.00
AG- J	0 / 92	-28.0	-28.0	0.07 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- P	-400 / 0	0.15 (1)
P- D	0 / 559	0.14 (2)
D- O	0 / 743	0.18 (1)
O- E	-828 / 0	0.39 (1)
O- F	0 / 729	0.18 (1)
M- F	0 / 563	0.14 (2)
M- G	-411 / 0	0.16 (1)
S- Q	-117 / 0	0.01 (1)
B- Q	0 / 2753	0.68 (1)
L- J	-118 / 0	0.01 (1)
L- H	0 / 2770	0.69 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.31/1.00 (L-M:1) , WB=0.69/1.00 (H-L:1) , SSI=0.37/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17169-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T9S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-65	-73	—	FRONT	VERT	DEAD
D	5-10-8	-408	-408	—	FRONT	VERT	SNOW
F	14-0-8	-65	-73	—	FRONT	VERT	DEAD
F	14-0-8	-408	-408	—	FRONT	VERT	SNOW
T	7-0-12	-115	-115	—	BACK	VERT	TOTAL
U	9-0-12	-115	-115	—	BACK	VERT	TOTAL
V	11-0-12	-115	-115	—	BACK	VERT	TOTAL
W	13-0-12	-116	-116	—	BACK	VERT	TOTAL
X	1-0-12	-50	-88	—	BACK	VERT	TOTAL
Y	3-0-12	-139	-139	—	BACK	VERT	TOTAL
Z	5-0-12	-139	-139	—	BACK	VERT	TOTAL
AA	7-0-12	-139	-139	—	BACK	VERT	TOTAL
AB	9-0-12	-139	-139	—	BACK	VERT	TOTAL
AC	11-0-12	-139	-139	—	BACK	VERT	TOTAL
AD	13-0-12	-139	-139	—	BACK	VERT	TOTAL
AE	15-0-12	-139	-139	—	BACK	VERT	TOTAL
AF	17-0-12	-139	-139	—	BACK	VERT	TOTAL
AG	18-10-4	-50	-88	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL= 0.74 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17169-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T10	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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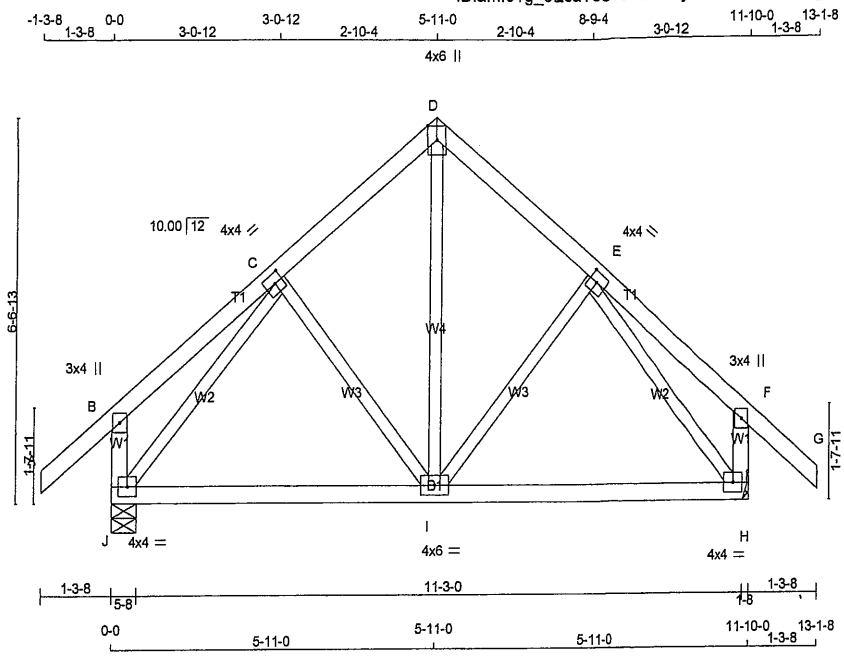

TOTAL WEIGHT = 2 X 57 = 115 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	987	0	987	0	0	5-8	5-8
H	987	0	987	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	769	474 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	769	474 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 24	-112.4	-112.4	0.15 (1)	10.00
C- D	-560 / 0	-112.4	-112.4	0.12 (1)	6.25
D- E	-560 / 0	-112.4	-112.4	0.12 (1)	6.25
E- F	0 / 24	-112.4	-112.4	0.15 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
J- B	-283 / 0	0.0	0.0	0.03 (1)	7.81
H- F	-283 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 500	-28.0	-28.0	0.34 (3)	10.00
I- H	0 / 500	-28.0	-28.0	0.34 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 424	0.10 (1)
I- E	-141 / 28	0.05 (1)
C- I	-141 / 28	0.05 (1)
J- C	-812 / 0	0.29 (1)
E- H	-812 / 0	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.34/1.00 (H-I:3) , WB=0.29/1.00 (C-J:1) , SSI=0.15/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (E) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17170-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T10S	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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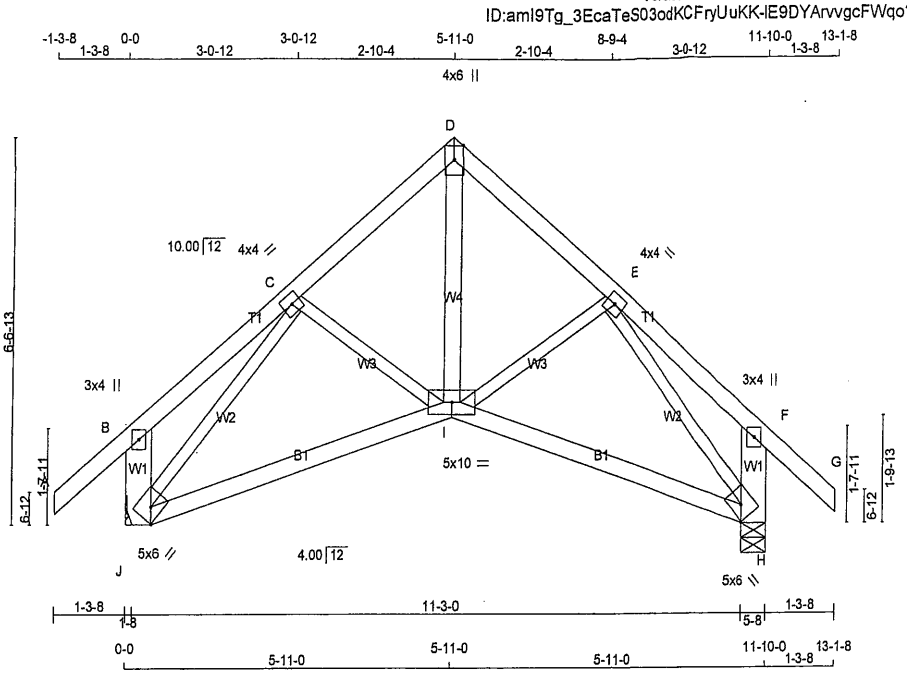

TOTAL WEIGHT = 6 X 58 = 348 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BVMW1+w	MT20	5.0	6.0	2.75	Edge
I	BBWWW-p	MT20	5.0	10.0		
J	BVMW1+w	MT20	5.0	6.0	2.75	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	987	0	987	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	987	0	987	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	769	474 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0
H	769	474 / 0	124 / 0	0 / 0	0 / 0	170 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H
BEARING SIZE FACTOR = 1.15 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.14 (1)	10.00
C- D	-748 / 0	-112.4	-112.4	0.11 (1)	6.25
D- E	-748 / 0	-112.4	-112.4	0.11 (1)	6.25
E- F	0 / 22	-112.4	-112.4	0.14 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
J- B	-286 / 0	0.0	0.0	0.02 (1)	7.81
H- F	-286 / 0	0.0	0.0	0.02 (1)	7.81
J- I	0 / 656	-28.0	-28.0	0.35 (2)	10.00
I- H	0 / 656	-28.0	-28.0	0.35 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- D	0 / 665	0.11 (1)
I- E	-77 / 45	0.02 (1)
C- I	-77 / 45	0.02 (1)
J- C	-1034 / 0	0.36 (1)
E- H	-1034 / 0	0.36 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 943 (0.15")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.35/1.00 (I-J:2) , WB=0.36/1.00 (E-H:1) , SSI=0.12/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN *THE TRUSS MANUFACTURING PLANT* .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/3/18
S. KATSOULAKIS
PROVINCE OF ONTARIO

DWG NO. TAM 17171 -18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T11	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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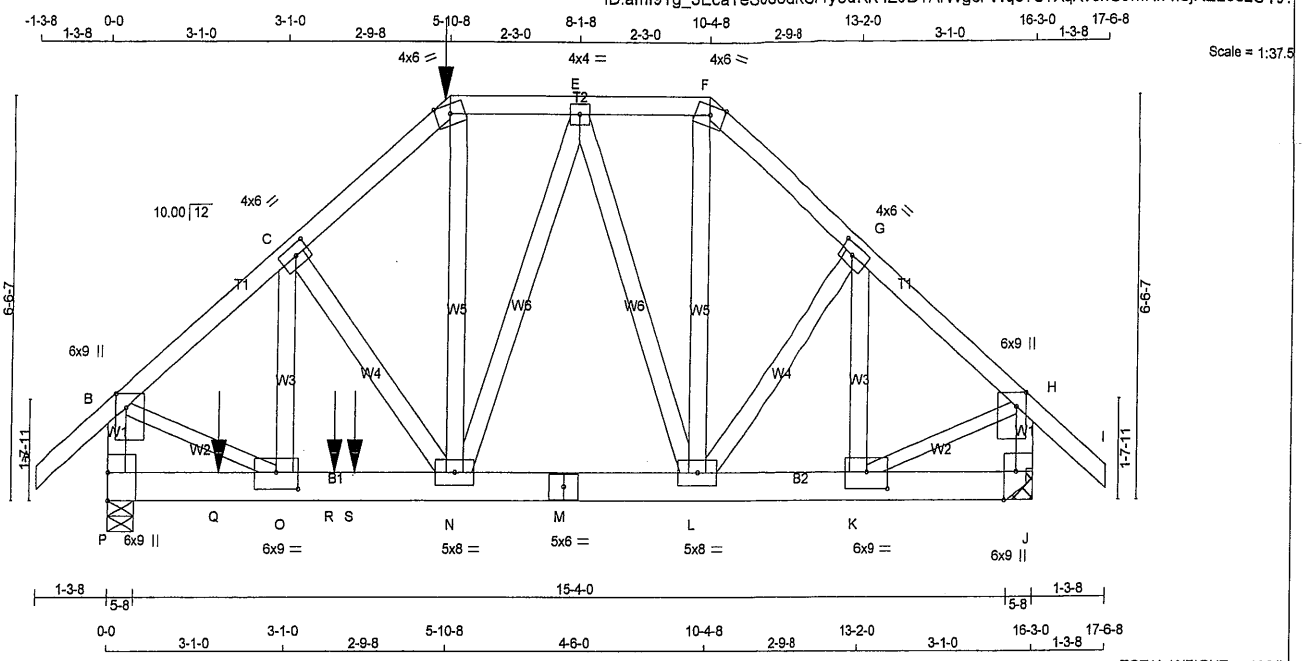

TOTAL WEIGHT = 108 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - M	2x6	DRY	1650F 1.5E	SPF
M - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - O	2x3	DRY	No.2	SPF
K - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0	2.75	2.00
C	TMWW-t	MT20	4.0	6.0	2.00	2.75
D	TTW-m	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	6.0	Edge	
G	TMWW-t	MT20	4.0	6.0	2.00	2.75
H	TMVW+p	MT20	6.0	9.0	2.75	2.00
J	BMV1+t	MT20	6.0	9.0	Edge	2.50
K	BMWW-t	MT20	6.0	9.0	3.25	4.50
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	6.0	9.0	3.25	4.50
P	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 421.3 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD, AND 77.2 lbs FACTORED DOWN AT 1-11-4, AND 73.3 lbs FACTORED DOWN AT 3-11-4, AND 1865.5 lbs FACTORED DOWN AT 4-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	3005	0	3005	0	0	5-8	5-8
J	1957	0	1957	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2343	1447 / 0	379 / 0	0 / 0	0 / 0	517 / 0	0 / 0
J	1520	947 / 0	240 / 0	0 / 0	0 / 0	333 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BEARING SIZE FACTOR = 1.15 AT JNT(S) P (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.84 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00		O- C	0 / 447	0.08 (1)
B- C	-2894 / 0	-112.4	-112.4	0.27 (1)	3.84		C- N	-657 / 0	0.18 (1)
C- D	-2419 / 0	-112.4	-112.4	0.23 (1)	4.19		N- D	0 / 872	0.15 (1)
D- E	-1856 / 0	-112.4	-112.4	0.12 (1)	4.79		N- E	0 / 661	0.12 (1)
E- F	-1340 / 0	-112.4	-112.4	0.10 (1)	5.47		E- L	-934 / 0	0.50 (1)
F- G	-1748 / 0	-112.4	-112.4	0.20 (1)	4.81		L- F	0 / 869	0.15 (1)
G- H	-1684 / 0	-112.4	-112.4	0.20 (1)	4.89		L- G	0 / 15	0.00 (1)
H- I	0 / 50	-112.4	-112.4	0.17 (1)	10.00		K- G	-431 / 0	0.08 (1)
P- B	-3049 / 0	0.0	0.0	0.35 (1)	4.88		B- O	0 / 2406	0.60 (1)
J- H	-1906 / 0	0.0	0.0	0.22 (1)	6.04		K- H	0 / 1409	0.35 (1)
P- Q	0 / 0	-28.0	-28.0	0.16 (1)	10.00				
Q- O	0 / 0	-28.0	-28.0	0.16 (1)	10.00				
O- R	0 / 2238	-28.0	-28.0	0.72 (1)	10.00				
R- S	0 / 2238	-28.0	-28.0	0.72 (1)	10.00				
S- N	0 / 2238	-28.0	-28.0	0.72 (1)	10.00				
N- M	0 / 1642	-28.0	-28.0	0.30 (1)	10.00				
M- L	0 / 1642	-28.0	-28.0	0.30 (1)	10.00				
L- K	0 / 1310	-28.0	-28.0	0.14 (1)	10.00				
K- J	0 / 0	-28.0	-28.0	0.03 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
D	5-10-8	-421	-421	---	FRONT	VERT	TOTAL
Q	1-11-4	-40	-77	---	FRONT	VERT	TOTAL
R	3-11-4	-42	-73	---	FRONT	VERT	TOTAL
S	4-3-8	-1865	-1865	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35/1.00 (B-P:1) , BC=0.72/1.00 (N-O:1) , WB=0.60/1.00 (B-O:1) , SSI=0.77/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.54 (H) (INPUT = 1.00)

DWG NO. TAM 17172-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287678	T12S	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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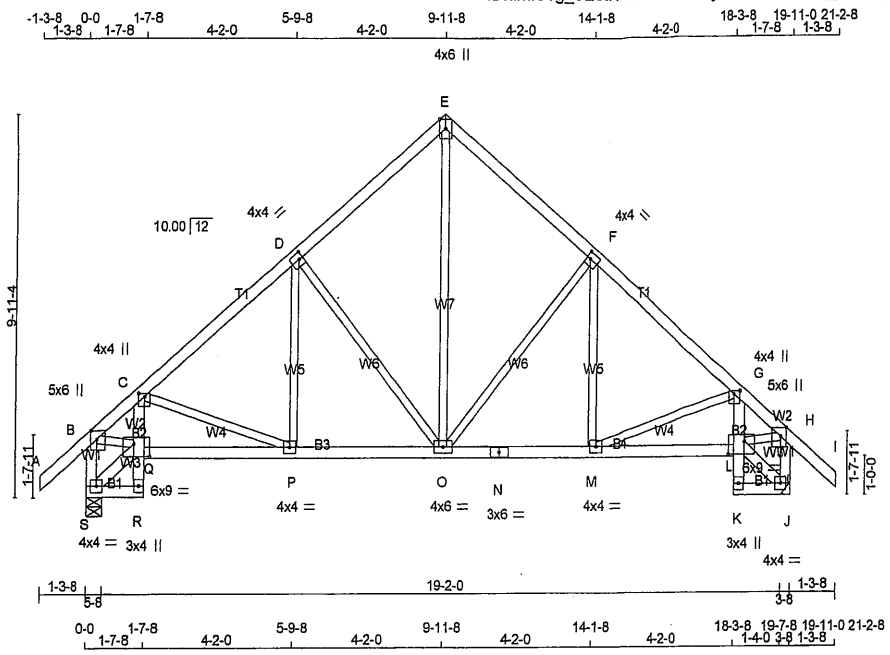

TOTAL WEIGHT = 3 X 102 = 305 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - Q	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.75	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	TMVW+p	MT20	5.0	6.0	2.75	2.00
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW-l	MT20	6.0	9.0	4.00	5.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BVMWW-l	MT20	6.0	9.0	4.00	5.50
R	BMV+p	MT20	3.0	4.0		
S	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1554	0	1554	0	0	5-8	5-8
J	1554	0	1554	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1221	737 / 0	209 / 0	0 / 0	0 / 0	276 / 0	0 / 0
J	1221	737 / 0	209 / 0	0 / 0	0 / 0	276 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1761 / 0	-112.4	-112.4	0.14 (1)	4.91
C- D	-1476 / 0	-112.4	-112.4	0.27 (1)	5.13
D- E	-1077 / 0	-112.4	-112.4	0.25 (1)	5.79
E- F	-1077 / 0	-112.4	-112.4	0.25 (1)	5.79
F- G	-1476 / 0	-112.4	-112.4	0.27 (1)	5.13
G- H	-1761 / 0	-112.4	-112.4	0.14 (1)	4.91
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
S- B	-1497 / 0	0.0	0.0	0.16 (1)	6.71
J- H	-1497 / 0	0.0	0.0	0.16 (1)	6.71
S- R	0 / 46	-28.0	-28.0	0.02 (2)	10.00
R- Q	0 / 36	0.0	0.0	0.08 (1)	10.00
Q- C	-28 / 76	0.0	0.0	0.06 (2)	7.81
Q- P	0 / 1382	-28.0	-28.0	0.28 (1)	10.00
P- O	0 / 1150	-28.0	-28.0	0.24 (2)	10.00
O- N	0 / 1150	-28.0	-28.0	0.24 (2)	10.00
N- M	0 / 1150	-28.0	-28.0	0.24 (2)	10.00
M- L	0 / 1382	-28.0	-28.0	0.28 (1)	10.00
K- L	0 / 36	0.0	0.0	0.08 (1)	10.00
L- G	-28 / 76	0.0	0.0	0.06 (2)	7.81
K- J	0 / 46	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- E	0 / 973	0.22 (1)
O- F	-554 / 0	0.44 (1)
M- F	0 / 251	0.06 (3)
M- G	-246 / 0	0.09 (1)
D- O	-554 / 0	0.44 (1)
P- D	0 / 251	0.06 (3)
C- P	-246 / 0	0.09 (1)
S- Q	-59 / 0	0.01 (1)
B- Q	0 / 1347	0.30 (1)
L- J	-59 / 0	0.01 (1)
L- H	0 / 1347	0.30 (1)

LICENSED PROFESSIONAL ENGINEER
4376
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.28/1.00 (P-Q:1) , WB=0.44/1.00 (D-O:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.33 (N) (INPUT = 1.00)

DWG NO. TAM 17179-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296382	T12	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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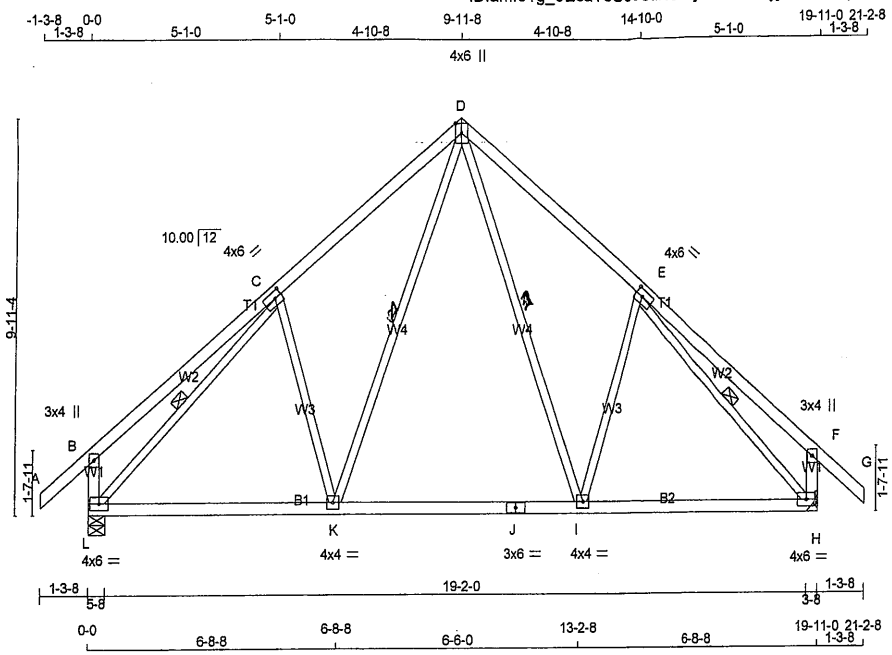

TOTAL WEIGHT = 3 X 96 = 287 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.25
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1554	0	1554	0	0	5-8	5-8
H	1554	0	1554	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1221	737 / 0	209 / 0	0 / 0	0 / 0	276 / 0	0 / 0
H	1221	737 / 0	209 / 0	0 / 0	0 / 0	276 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H. D-I, C-I

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 43	-112.4	-112.4	0.46 (1)	10.00
C- D	-1222 / 0	-112.4	-112.4	0.38 (1)	5.35
D- E	-1222 / 0	-112.4	-112.4	0.38 (1)	5.35
E- F	0 / 43	-112.4	-112.4	0.46 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
L- B	-366 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-366 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 1013	-28.0	-28.0	0.39 (2)	10.00
K- J	0 / 721	-28.0	-28.0	0.36 (2)	10.00
J- I	0 / 721	-28.0	-28.0	0.36 (2)	10.00
I- H	0 / 1013	-28.0	-28.0	0.39 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 570	0.13 (1)
I- E	-353 / 20	0.20 (1)
K- D	0 / 570	0.13 (1)
C- K	-353 / 20	0.20 (1)
L- C	-1505 / 0	0.54 (1)
E- H	-1505 / 0	0.54 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.39/1.00 (K-L:2) , WB=0.54/1.00 (C-L:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.41 (C) (INPUT = 1.00)

DWG NO. TAM 17181 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287678	G12	1	1	TRUSS DESC.	

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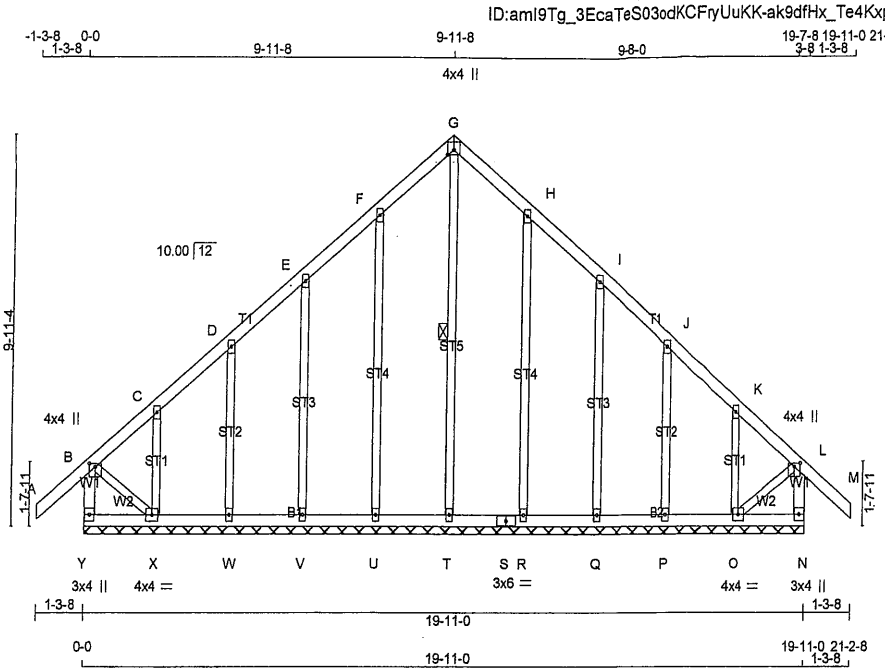

TOTAL WEIGHT = 104 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-362 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-77 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-32 / 0	-112.4	-112.4	0.06 (1)	6.25
D- E	-36 / 0	-112.4	-112.4	0.06 (1)	6.25
E- F	-25 / 0	-112.4	-112.4	0.06 (1)	6.25
F- G	-40 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-40 / 0	-112.4	-112.4	0.06 (1)	6.25
H- I	-25 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-36 / 0	-112.4	-112.4	0.06 (1)	6.25
J- K	-32 / 0	-112.4	-112.4	0.06 (1)	6.25
K- L	-77 / 0	-112.4	-112.4	0.15 (1)	6.25
L- M	0 / 50	-112.4	-112.4	0.15 (1)	10.00
N- L	-362 / 0	0.0	0.0	0.04 (1)	7.81
Y- X	0 / 0	-28.0	-28.0	0.03 (3)	10.00
X- W	0 / 31	-28.0	-28.0	0.03 (3)	10.00
W- V	0 / 26	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 22	-28.0	-28.0	0.02 (3)	10.00
U- T	0 / 19	-28.0	-28.0	0.02 (3)	10.00
T- S	0 / 19	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 19	-28.0	-28.0	0.02 (3)	10.00
R- Q	0 / 22	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 26	-28.0	-28.0	0.02 (3)	10.00
P- O	0 / 31	-28.0	-28.0	0.03 (3)	10.00
O- N	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-158 / 0	0.10 (1)
U- F	-256 / 0	0.34 (1)
V- E	-213 / 0	0.15 (1)
W- D	-236 / 0	0.08 (1)
X- C	-152 / 0	0.03 (1)
R- H	-256 / 0	0.34 (1)
Q- I	-213 / 0	0.15 (1)
P- J	-236 / 0	0.08 (1)
O- K	-152 / 0	0.03 (1)
B- X	0 / 43	0.01 (1)
O- L	0 / 43	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (L-M:1) , BC=0.03/1.00 (W-X:3) , WB=0.34/1.00 (F-U:1) , SSI=0.09/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (G) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4308
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PROVINCE OF ONTARIO

DWG NO. TAM 17180-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	T13	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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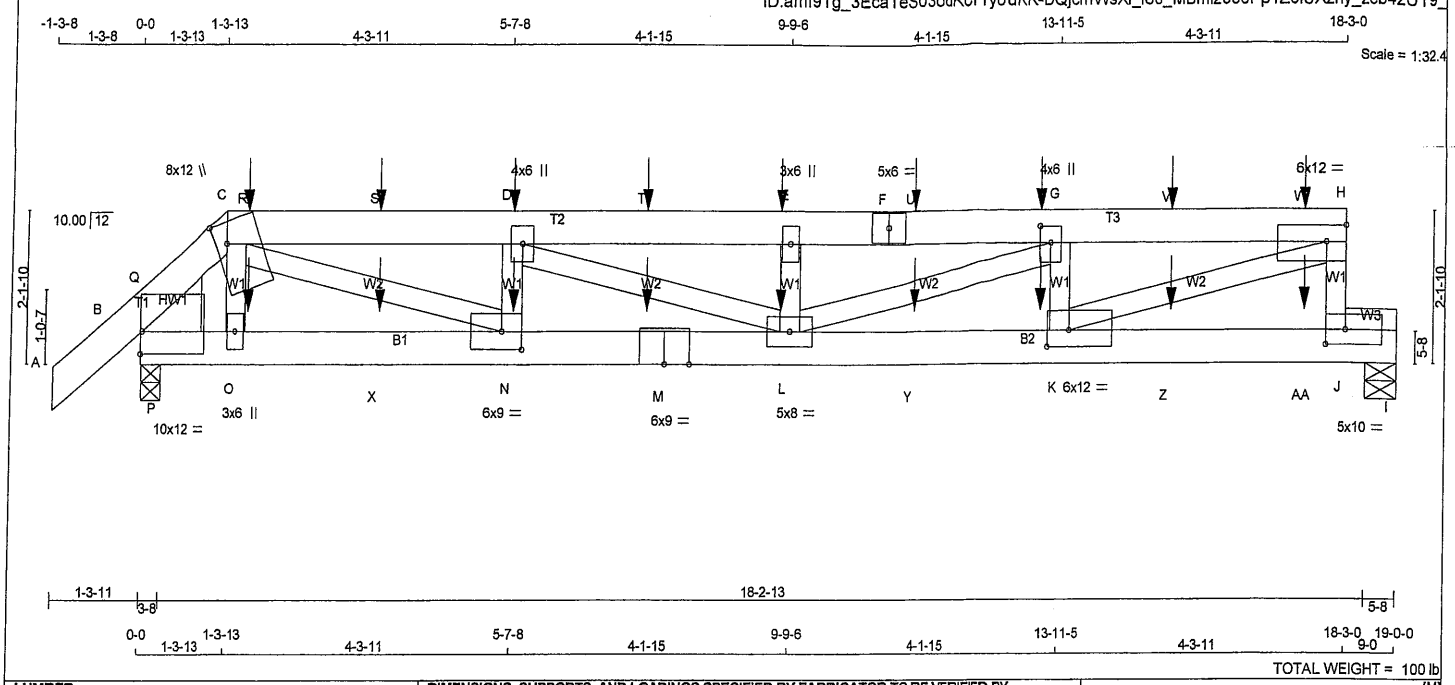

TOTAL WEIGHT = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
B - M	2x6	DRY	2100F 1.8E	SPF
M - I	2x6	DRY	2100F 1.8E	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - N	2x4	DRY	1650F 1.5E	SPF
D - L	2x4	DRY	1650F 1.5E	SPF
L - G	2x4	DRY	1650F 1.5E	SPF
K - H	2x4	DRY	1650F 1.5E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	10.0	12.0	3.75	Edge
C	TTWW+m	MT20	8.0	12.0	3.50	2.00
D	TMWW+t	MT20	4.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0	2.75	2.00
H	TMWW-t	MT20	6.0	12.0	2.75	Edge
J	BMWW-t	MT20	5.0	10.0	2.50	3.50
K	BMWW-t	MT20	6.0	12.0	2.75	4.00
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	6.0	9.0		
N	BMWW-t	MT20	6.0	9.0	3.00	3.50
O	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 192.9 lbs FACTORED DOWN AT 1-7-12, 155.2 lbs FACTORED DOWN AT 3-7-12, 155.2 lbs FACTORED DOWN AT 5-7-12, 155.2 lbs FACTORED DOWN AT 7-7-12, 155.2 lbs FACTORED DOWN AT 9-7-12, 155.2 lbs FACTORED DOWN AT 11-7-12, 155.2 lbs FACTORED DOWN AT 13-7-12, AND 155.2 lbs FACTORED DOWN AT 15-7-12, AND 177.4 lbs FACTORED DOWN AT 17-7-12 ON TOP CHORD, AND 84.7 lbs FACTORED DOWN AT 1-7-12, 84.7 lbs FACTORED DOWN AT 3-7-12, 84.7 lbs FACTORED DOWN AT 5-7-12, 84.7 lbs FACTORED DOWN AT 7-7-12, 84.7 lbs FACTORED DOWN AT 9-7-12, 84.7 lbs FACTORED DOWN AT 11-7-12, 84.7 lbs FACTORED DOWN AT 13-7-12, AND 84.7 lbs FACTORED DOWN AT 15-7-12, AND 84.7 lbs FACTORED DOWN AT 17-7-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	2594	0	2594	0	0	3-8	3-8	2x10 L
I	2371	0	2371	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	2045	1223 / 0	358 / 0	0 / 0	0 / 0	464 / 0	0 / 0
I	1898	1089 / 0	364 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I
BEARING SIZE FACTOR = 1.15 AT JNT(S) B, I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 35	-112.4	-112.4	0.09 (1)	10.00	
B- Q	-3923 / 0	-112.4	-112.4	0.49 (1)	3.75	
Q- C	-2869 / 0	-112.4	-112.4	0.45 (1)	4.36	
C- R	-5807 / 0	-112.4	-112.4	0.50 (1)	3.19	
R- S	-5807 / 0	-112.4	-112.4	0.50 (1)	3.19	
S- D	-5807 / 0	-112.4	-112.4	0.50 (1)	3.19	
D- T	-6845 / 0	-112.4	-112.4	0.53 (1)	2.91	
T- E	-6845 / 0	-112.4	-112.4	0.53 (1)	2.91	
E- F	-6845 / 0	-112.4	-112.4	0.55 (1)	2.88	
F- U	-6845 / 0	-112.4	-112.4	0.55 (1)	2.88	
U- G	-6845 / 0	-112.4	-112.4	0.55 (1)	2.88	
G- V	-5721 / 0	-112.4	-112.4	0.51 (1)	3.19	
V- W	-5721 / 0	-112.4	-112.4	0.51 (1)	3.19	
W- H	-5721 / 0	-112.4	-112.4	0.51 (1)	3.19	
J- H	-2658 / 0	0.0	0.0	0.31 (1)	5.21	
B- P	0 / 1902	-28.0	-28.0	0.22 (1)	10.00	
P- O	0 / 1902	-28.0	-28.0	0.22 (1)	10.00	
O- X	0 / 1915	-28.0	-28.0	0.17 (1)	10.00	
X- N	0 / 1915	-28.0	-28.0	0.17 (1)	10.00	
N- M	0 / 5807	-28.0	-28.0	0.45 (1)	10.00	
M- L	0 / 5807	-28.0	-28.0	0.45 (1)	10.00	
L- Y	0 / 5721	-28.0	-28.0	0.41 (1)	10.00	
Y- K	0 / 5721	-28.0	-28.0	0.41 (1)	10.00	
K- Z	0 / 0	-28.0	-28.0	0.71 (1)	10.00	
Z-AA	0 / 0	-28.0	-28.0	0.71 (1)	10.00	
AA- J	0 / 0	-28.0	-28.0	0.71 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.67 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 238	0.04 (3)
C- N	0 / 4165	0.53 (1)
N- D	-1244 / 0	0.14 (1)
D- L	0 / 1119	0.14 (1)
L- E	-735 / 0	0.08 (1)
L- G	0 / 1211	0.16 (1)
K- G	-1315 / 0	0.15 (1)
K- H	0 / 6139	0.79 (1)
P- Q	0 / 1217	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-7-12	-155	-155	—	BACK	VERT	TOTAL
E	9-7-12	-155	-155	—	BACK	VERT	TOTAL
G	13-7-12	-155	-155	—	BACK	VERT	TOTAL
K	13-7-12	-85	-85	—	BACK	VERT	TOTAL
L	9-7-12	-85	-85	—	BACK	VERT	TOTAL
M	7-7-12	-85	-85	—	BACK	VERT	TOTAL
N	5-7-12	-85	-85	—	BACK	VERT	TOTAL
O	1-7-12	-85	-85	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 821 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 503 (0.45")

CSI: TC=0.55/1.00 (E-G:1) , BC=0.71/1.00 (J-K:1) , WB=0.79/1.00 (H-K:1) , SSI=0.96/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.96 (H) (INPUT = 1.00)

P614

DWG NO. TAM 17173-B
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	T13	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	1-7-12	-193	-193	—	BACK	VERT	TOTAL
S	3-7-12	-155	-155	—	BACK	VERT	TOTAL
T	7-7-12	-155	-155	—	BACK	VERT	TOTAL
U	11-7-12	-155	-155	—	BACK	VERT	TOTAL
V	15-7-12	-155	-155	—	BACK	VERT	TOTAL
W	17-7-12	-177	-177	—	BACK	VERT	TOTAL
X	3-7-12	-85	-85	—	BACK	VERT	TOTAL
Y	11-7-12	-85	-85	—	BACK	VERT	TOTAL
Z	15-7-12	-85	-85	—	BACK	VERT	TOTAL
AA	17-7-12	-85	-85	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
43106
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17173-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T21	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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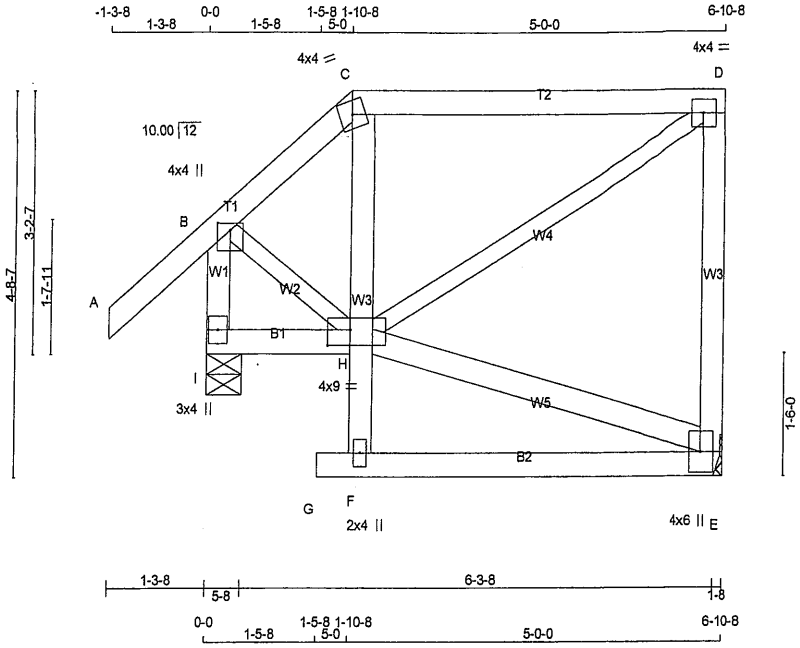

TOTAL WEIGHT = 41 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
B - H	2x3	DRY	No.2	SPF
H - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	BMVW1+p	MT20	4.0	6.0		
F	BMW+w	MT20	2.0	4.0		
H	BWMWWW*-I	MT20	4.0	9.0	2.25	3.50
I	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	472	0	472	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	665	0	665	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	380	214 / 0	76 / 0	0 / 0	0 / 0	90 / 0	0 / 0
I	516	322 / 0	80 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-316 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-204 / 0	-112.4	-112.4	0.48 (1)	6.25
E- D	-402 / 0	0.0	0.0	0.14 (1)	7.81
I- B	-637 / 0	0.0	0.0	0.07 (1)	7.81
I- H	0 / 0	-28.0	-28.0	0.04 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.01 (3)	10.00
F- E	0 / 5	-28.0	-28.0	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- H	0 / 147	0.03 (2)
H- C	-150 / 52	0.01 (3)
B- H	0 / 254	0.06 (1)
H- E	-6 / 0	0.00 (1)
H- D	0 / 237	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.48/1.00 (C-D:1) , BC=0.21/1.00 (E-F:3) , WB=0.06/1.00 (B-H:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
438
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17182-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T22	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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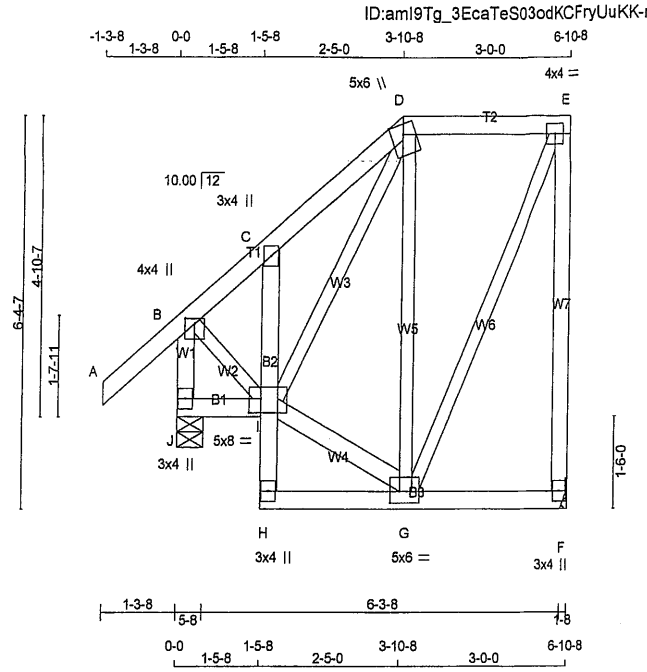

TOTAL WEIGHT = 50 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.25	2.00
H	BMV+p	MT20	3.0	4.0		
I	BVMWWW-l	MT20	5.0	8.0	2.75	2.50
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	467	0	467	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
J	654	0	654	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	374	214 / 0	72 / 0	0 / 0	0 / 0	88 / 0	0 / 0
J	502	322 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
B- C	-301 / 0	-112.4	-112.4	0.11 (1)	6.25	
C- D	-262 / 0	-112.4	-112.4	0.08 (1)	6.25	
D- E	-125 / 0	-112.4	-112.4	0.13 (1)	6.25	
F- E	-434 / 0	0.0	0.0	0.35 (1)	7.81	
J- B	-632 / 0	0.0	0.0	0.07 (1)	7.81	
J- I	0 / 0	-28.0	-28.0	0.02 (3)	10.00	
H- I	0 / 43	0.0	0.0	0.02 (1)	10.00	
I- C	-171 / 0	0.0	0.0	0.02 (1)	7.81	
H- G	0 / 9	-28.0	-28.0	0.06 (3)	10.00	
G- F	0 / 0	-28.0	-28.0	0.06 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	0 / 144	0.02 (1)
I- D	0 / 142	0.03 (1)
G- D	-253 / 0	0.17 (1)
G- E	0 / 294	0.07 (1)
B- I	0 / 266	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.35/1.00 (E-F:1) , BC=0.06/1.00 (G-H:3) , WB=0.17/1.00 (D-G:1) , SSI=0.13/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
43/B
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17/03-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287680	T23	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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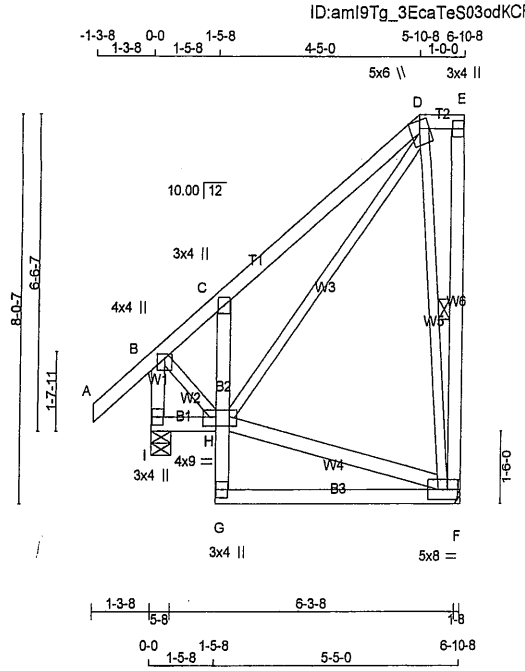

TOTAL WEIGHT = 53 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+m	MT20	5.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	BMVWW1-t	MT20	5.0	8.0		
G	BMV+p	MT20	3.0	4.0		
H	BVMWWW-l	MT20	4.0	9.0	2.25	3.50
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	467	0	467	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	654	0	654	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	374	214 / 0	72 / 0	0 / 0	0 / 0	88 / 0	0 / 0
I	502	322 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-316 / 0	-112.4	-112.4	0.19 (1)	6.25
C- D	-343 / 0	-112.4	-112.4	0.25 (1)	6.25
D- E	0 / 0	-112.4	-112.4	0.02 (1)	10.00
F- E	-56 / 0	0.0	0.0	0.02 (1)	6.25
I- B	-632 / 0	0.0	0.0	0.07 (1)	7.81
I- H	0 / 0	-28.0	-28.0	0.02 (3)	10.00
G- H	0 / 129	0.0	0.0	0.04 (1)	10.00
H- C	-401 / 0	0.0	0.0	0.03 (1)	7.81
G- F	0 / 15	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
B- H	0 / 343	0.08 (1)
D- F	-348 / 0	0.44 (1)
H- F	0 / 31	0.00 (1)
H- D	0 / 350	0.08 (1)

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 962 (0.09")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.25/1.00 (F-G:3) , WB=0.44/1.00 (D-F:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (H) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17184-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287680	T24	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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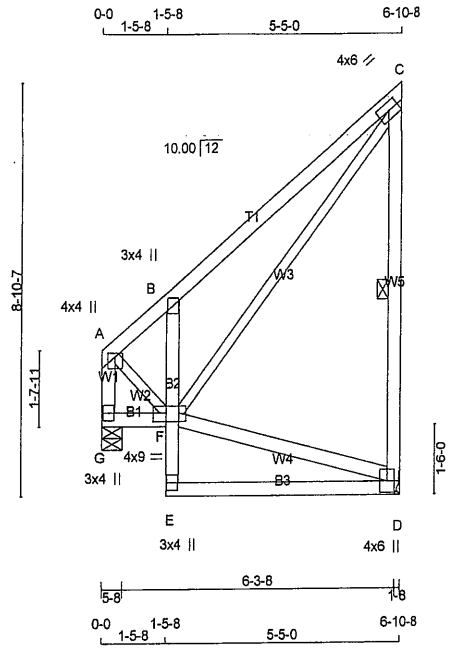

TOTAL WEIGHT = 47 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	6.0	2.00	3.00
D	BMVW1+p	MT20	4.0	6.0		
E	BMV+p	MT20	3.0	4.0		
F	BVMWWW-I	MT20	4.0	9.0	2.25	3.50
G	BMV1+p	MT20	3.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	483	0	483	0	0	5-8	5-8
D	483	0	483	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	385	223 / 0	72 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	385	223 / 0	72 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
G- A	-460 / 0	0.0	0.0	0.05 (1)	7.81
A- B	-327 / 0	-112.4	-112.4	0.32 (1)	6.25
B- C	-445 / 0	-112.4	-112.4	0.37 (1)	6.25
D- C	-402 / 0	0.0	0.0	0.14 (1)	6.25
G- F	0 / 0	-28.0	-28.0	0.02 (3)	10.00
E- F	0 / 129	0.0	0.0	0.06 (1)	10.00
F- B	-615 / 0	0.0	0.0	0.05 (1)	7.81
E- D	0 / 23	-28.0	-28.0	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- F	0 / 451	0.10 (1)
F- D	-24 / 0	0.01 (1)
F- C	0 / 524	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 948 (0.09")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.24/1.00 (D-E:3) , WB=0.12/1.00 (C-F:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (C) (INPUT = 0.90)
JSI METAL= 0.14 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
43B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 171B5-38
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T26	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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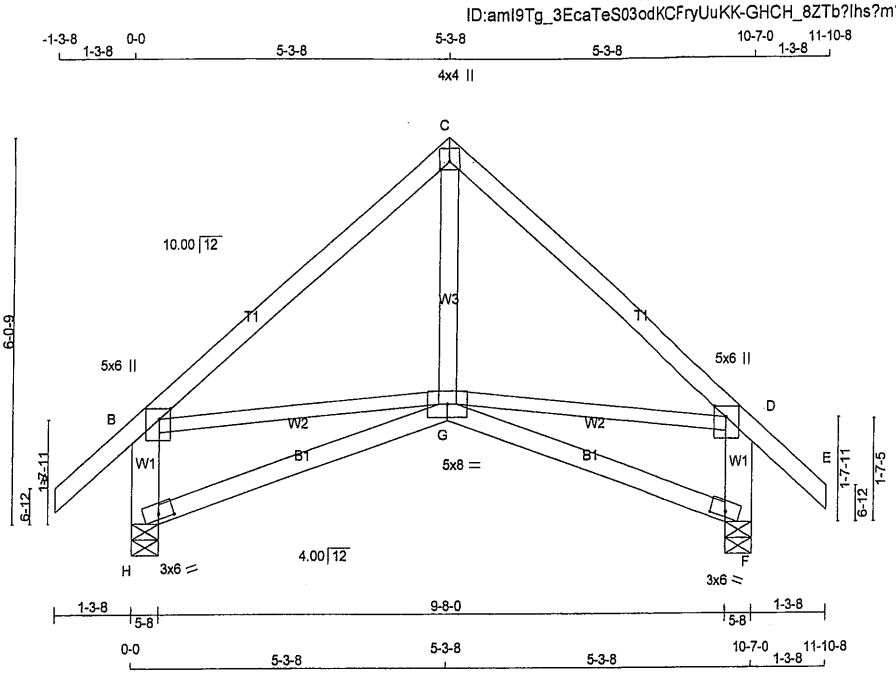

TOTAL WEIGHT = 50 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
G - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	5.0	6.0	2.00	2.25
F	BVM1-I	MT20	3.0	6.0	1.00	3.00
G	BBWWW-p	MT20	5.0	8.0		
H	BVM1-I	MT20	3.0	6.0	1.00	3.00

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	899	0	899	0	0	5-8	5-8
F	899	0	899	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	699	434 / 0	111 / 0	0 / 0	0 / 0	154 / 0	0 / 0
F	699	434 / 0	111 / 0	0 / 0	0 / 0	154 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
BEARING SIZE FACTOR = 1.15 AT JNT(S) H, F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-625 / 0	-112.4	-112.4	0.41 (1)	6.25
C- D	-625 / 0	-112.4	-112.4	0.41 (1)	6.25
D- E	0 / 50	-112.4	-112.4	0.15 (1)	10.00
H- B	-825 / 0	0.0	0.0	0.06 (1)	7.81
F- D	-825 / 0	0.0	0.0	0.06 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.25 (3)	10.00
G- F	0 / 0	-28.0	-28.0	0.25 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	0 / 299	0.05 (3)
B- G	0 / 481	0.11 (1)
G- D	0 / 481	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.41/1.00 (C-D:1) , BC=0.25/1.00 (F-G:3) , WB=0.11/1.00 (B-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	318	354	1667	822	2284	1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (C) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4328
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17186-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T26A	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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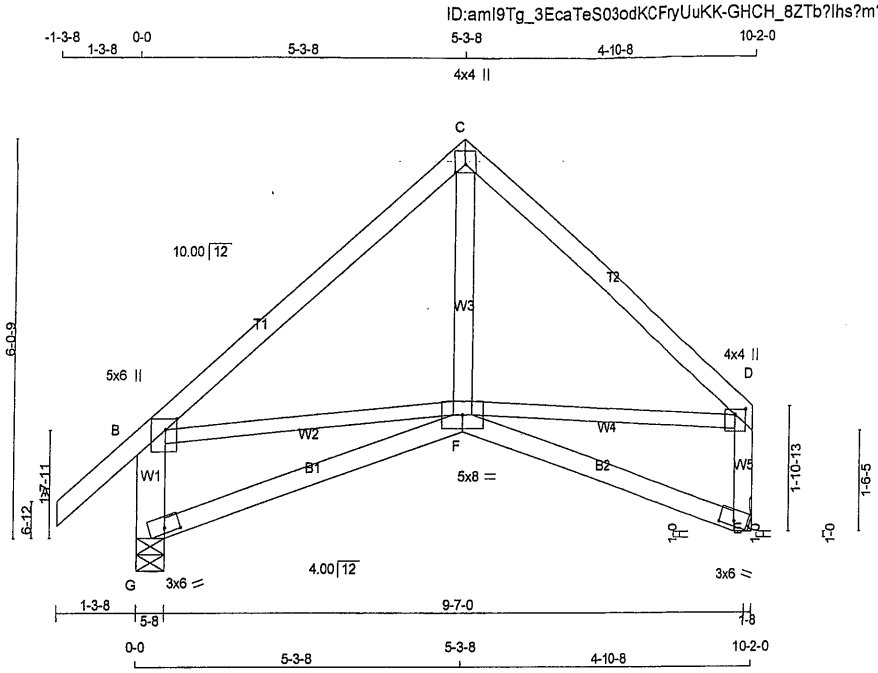

TOTAL WEIGHT = 2 X 46 = 92 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
B - F	2x3	DRY	No.2	SPF
F - D	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.25
C	TTW+p	MT20	4.0	4.0	1.50	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BVM1-I	MT20	3.0	6.0	1.00	3.00
F	BBWWW-p	MT20	5.0	8.0		
G	BVM1-I	MT20	3.0	6.0	1.00	3.00

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	870	0	870	0	0	5-8	5-8
E	714	0	714	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	676	420 / 0	107 / 0	0 / 0	0 / 0	149 / 0	0 / 0
E	569	330 / 0	107 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BEARING SIZE FACTOR = 1.15 AT JNT(S) G (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-576 / 0	-112.4	-112.4	0.41 (1)	6.25
C- D	-576 / 0	-112.4	-112.4	0.35 (1)	6.25
G- B	-795 / 0	0.0	0.0	0.06 (1)	7.81
E- D	-646 / 0	0.0	0.0	0.08 (1)	7.81
G- F	0 / 0	-28.0	-28.0	0.25 (3)	10.00
F- E	0 / 0	-28.0	-28.0	0.21 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	0 / 265	0.04 (3)
B- F	0 / 443	0.10 (1)
F- D	0 / 442	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.25/1.00 (F-G:3) , WB=0.10/1.00 (B-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1067	822	2294	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (C) (INPUT = 0.90)
JSI METAL= 0.20 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
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S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17187-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287680	T27	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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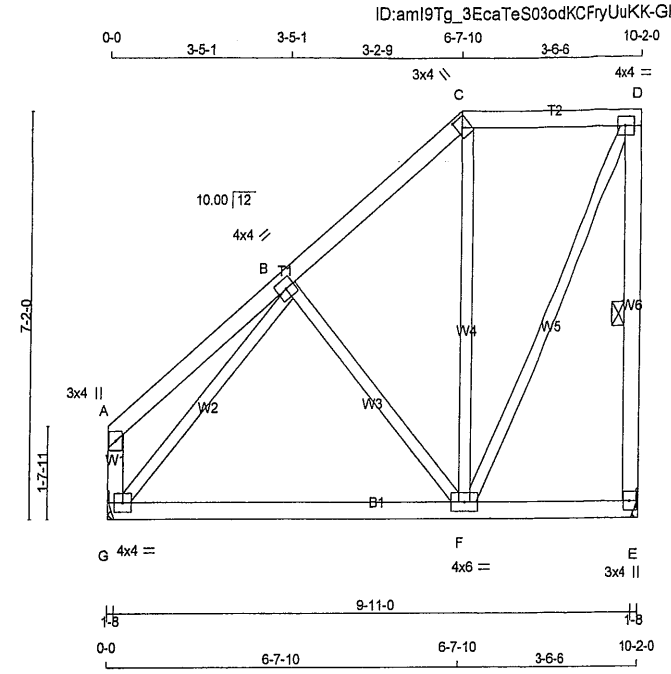

TOTAL WEIGHT = 54 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	4.0		
C	TTW+h	MT20	3.0	4.0	2.00	1.25
D	TMVW-t	MT20	4.0	4.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	714	0	714	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	714	0	714	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	569	330 / 0	107 / 0	0 / 0	0 / 0	132 / 0	0 / 0
G	569	330 / 0	107 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 29	-112.4	-112.4	0.20 (1)	10.00
B- C	-357 / 0	-112.4	-112.4	0.16 (1)	6.25
C- D	-250 / 0	-112.4	-112.4	0.18 (1)	6.25
E- D	-698 / 0	0.0	0.0	0.16 (1)	6.25
G- A	-140 / 0	0.0	0.0	0.01 (1)	7.81
G- F	0 / 403	-28.0	-28.0	0.32 (3)	10.00
F- E	0 / 0	-28.0	-28.0	0.29 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	-233 / 0	0.11 (1)
F- C	-133 / 67	0.12 (1)
F- D	0 / 560	0.13 (1)
G- B	-639 / 0	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 922 (0.13")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.32/1.00 (F-G:3) , WB=0.28/1.00 (B-G:1) , SSI=0.16/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (F) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM 17166-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T28	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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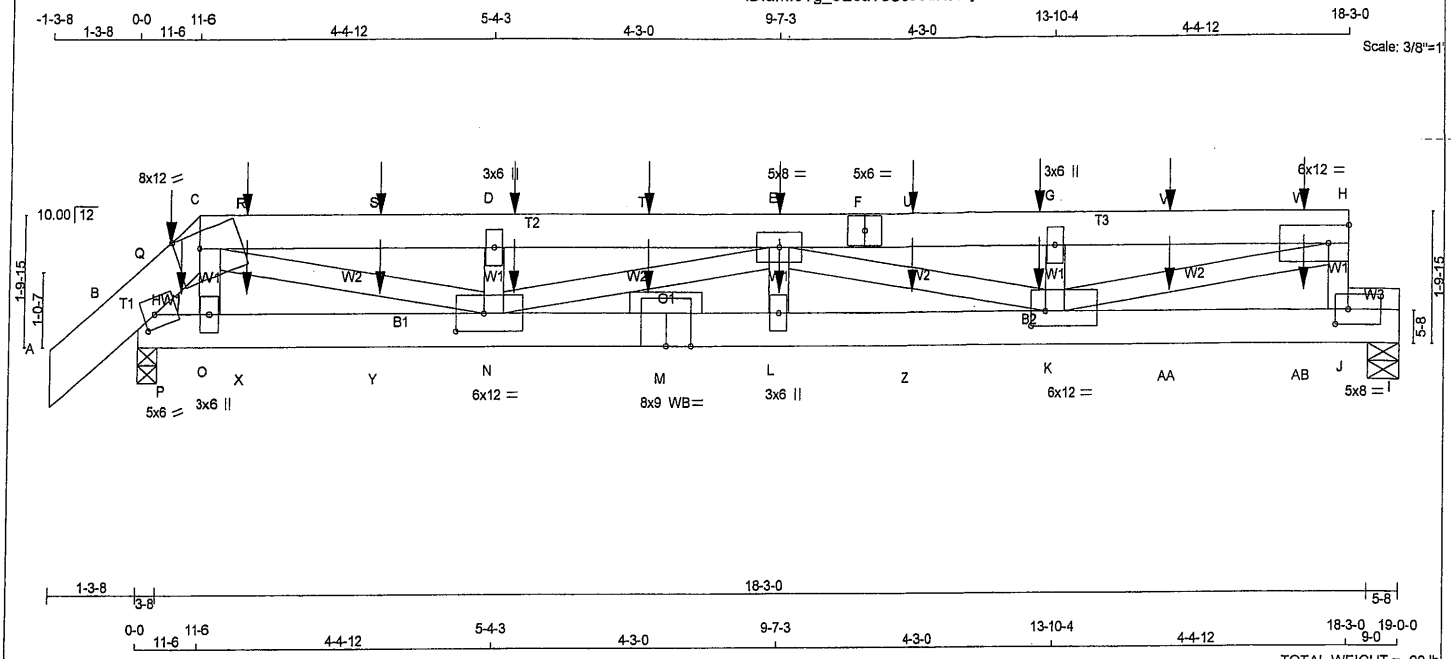

TOTAL WEIGHT = 99 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x8	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
B - M	2x6	DRY	2100F 1.8E	SPF
M - I	2x6	DRY	2100F 1.8E	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
C - N	2x4	DRY	2100F 1.8E	SPF
N - E	2x4	DRY	2100F 1.8E	SPF
E - K	2x4	DRY	2100F 1.8E	SPF
K - H	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	5.0	6.0	2.25	2.00
C	TTWW-m	MT20	8.0	12.0	Edge	4.75
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	5.0	8.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TMVW-t	MT20	6.0	12.0		Edge
J	BMVW-t	MT20	5.0	8.0	2.50	2.25
K	BMWWW-t	MT20	6.0	12.0	2.50	2.50
L	BMW+w	MT20	3.0	6.0		
M	BS-t	MT20	8.0	9.0		
N	BMWWW-t	MT20	6.0	12.0	3.00	5.00
O	BMW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 164.9 lbs FACTORED DOWN AT 7-12, 125.7 lbs FACTORED DOWN AT 1-7-12, 108.6 lbs FACTORED DOWN AT 3-7-12, 108.6 lbs FACTORED DOWN AT 5-7-12, 108.6 lbs FACTORED DOWN AT 7-7-12, 108.6 lbs FACTORED DOWN AT 9-7-4, 108.6 lbs FACTORED DOWN AT 11-7-4, 108.6 lbs FACTORED DOWN AT 13-7-4, AND 108.6 lbs FACTORED DOWN AT 15-7-4, AND 128.5 lbs FACTORED DOWN AT 17-7-4 ON TOP CHORD, AND 71.9 lbs FACTORED DOWN AT 7-12, 67.0 lbs FACTORED DOWN AT 1-7-12, 67.0 lbs FACTORED DOWN AT 3-7-12, 67.0 lbs FACTORED DOWN AT 5-7-12, 67.0 lbs FACTORED DOWN AT 7-7-12, 67.0 lbs FACTORED DOWN AT 9-7-4, 67.0 lbs FACTORED DOWN AT 11-7-4, 67.0 lbs FACTORED DOWN AT 13-7-4, AND 67.0 lbs FACTORED DOWN AT 15-7-4, AND 67.0 lbs FACTORED DOWN AT 17-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	2533	0	2533	0	0	3-8	3-8	2x4 L
I	2071	0	2071	0	0	5-8	5-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1994	1199 / 0	345 / 0	0 / 0	0 / 0	450 / 0	0 / 0
I	1659	950 / 0	320 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, I
BEARING SIZE FACTOR = 1.15 AT JNT(S) B, I (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 40	-112.4	-112.4	0.07 (1)	10.00	
B- Q	-3096 / 0	-112.4	-112.4	0.19 (1)	5.15	
Q- C	-2906 / 0	-112.4	-112.4	0.19 (1)	5.28	
C- R	-5923 / 0	-112.4	-112.4	0.51 (1)	3.14	
R- S	-5923 / 0	-112.4	-112.4	0.51 (1)	3.14	
S- D	-5923 / 0	-112.4	-112.4	0.51 (1)	3.14	
D- T	-5921 / 0	-112.4	-112.4	0.46 (1)	3.20	
T- E	-5921 / 0	-112.4	-112.4	0.46 (1)	3.20	
E- F	-6032 / 0	-112.4	-112.4	0.49 (1)	3.14	
F- U	-6032 / 0	-112.4	-112.4	0.49 (1)	3.14	
U- G	-6032 / 0	-112.4	-112.4	0.49 (1)	3.14	
G- V	-6032 / 0	-112.4	-112.4	0.54 (1)	3.07	
V- W	-6032 / 0	-112.4	-112.4	0.54 (1)	3.07	
W- H	-6032 / 0	-112.4	-112.4	0.54 (1)	3.07	
J- H	-2279 / 0	0.0	0.0	0.26 (1)	5.58	
B- P	0 / 1729	-28.0	-28.0	0.13 (1)	10.00	
P- O	0 / 1729	-28.0	-28.0	0.23 (1)	10.00	
O- X	0 / 1804	-28.0	-28.0	0.23 (1)	10.00	
X- Y	0 / 1804	-28.0	-28.0	0.23 (1)	10.00	
Y- N	0 / 1804	-28.0	-28.0	0.23 (1)	10.00	
N- M	0 / 7394	-28.0	-28.0	0.54 (1)	10.00	
M- L	0 / 7394	-28.0	-28.0	0.54 (1)	10.00	
L- Z	0 / 7394	-28.0	-28.0	0.48 (1)	10.00	
Z- K	0 / 7394	-28.0	-28.0	0.48 (1)	10.00	
K-AA	0 / 0	-28.0	-28.0	0.62 (1)	10.00	
AA-AB	0 / 0	-28.0	-28.0	0.62 (1)	10.00	
AB- J	0 / 0	-28.0	-28.0	0.62 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.58 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	0 / 661	0.12 (1)
C- N	0 / 4291	0.35 (1)
N- D	-641 / 0	0.07 (1)
N- E	-1547 / 0	0.22 (1)
L- E	0 / 290	0.05 (2)
E- K	-1431 / 0	0.21 (1)
K- G	-721 / 0	0.08 (1)
K- H	0 / 6318	0.52 (1)
P- Q	-22 / 1	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-7-12	-109	-109	—	BACK	VERT	TOTAL
E	9-7-4	-109	-109	—	BACK	VERT	TOTAL
G	13-7-4	-109	-109	—	BACK	VERT	TOTAL
K	13-7-4	-67	-67	—	BACK	VERT	TOTAL
L	9-7-4	-67	-67	—	BACK	VERT	TOTAL
M	7-7-12	-67	-67	—	BACK	VERT	TOTAL
N	5-7-12	-67	-67	—	BACK	VERT	TOTAL

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 730 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 447 (0.51")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.62/1.00 (J-K:1) , WB=0.52/1.00 (H-K:1) , SSI=0.84/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 *COMP=1.00 SHEAR=1.00 TENS= 1.00*

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.99 (M) (INPUT = 1.00)

P6

DWG NO. TAM 17189-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287680	T28	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	7-12	-72	-72	—	BACK	VERT	TOTAL
Q	7-12	-165	-165	—	BACK	VERT	TOTAL
R	1-7-12	-126	-126	—	BACK	VERT	TOTAL
S	3-7-12	-109	-109	—	BACK	VERT	TOTAL
T	7-7-12	-109	-109	—	BACK	VERT	TOTAL
U	11-7-4	-109	-109	—	BACK	VERT	TOTAL
V	15-7-4	-109	-109	—	BACK	VERT	TOTAL
W	17-7-4	-128	-128	—	BACK	VERT	TOTAL
X	1-7-12	-67	-67	—	BACK	VERT	TOTAL
Y	3-7-12	-67	-67	—	BACK	VERT	TOTAL
Z	11-7-4	-67	-67	—	BACK	VERT	TOTAL
AA	15-7-4	-67	-67	—	BACK	VERT	TOTAL
AB	17-7-4	-67	-67	—	BACK	VERT	TOTAL

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PROVINCE OF ONTARIO

DWG NO. TAM 17189-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296384	T31	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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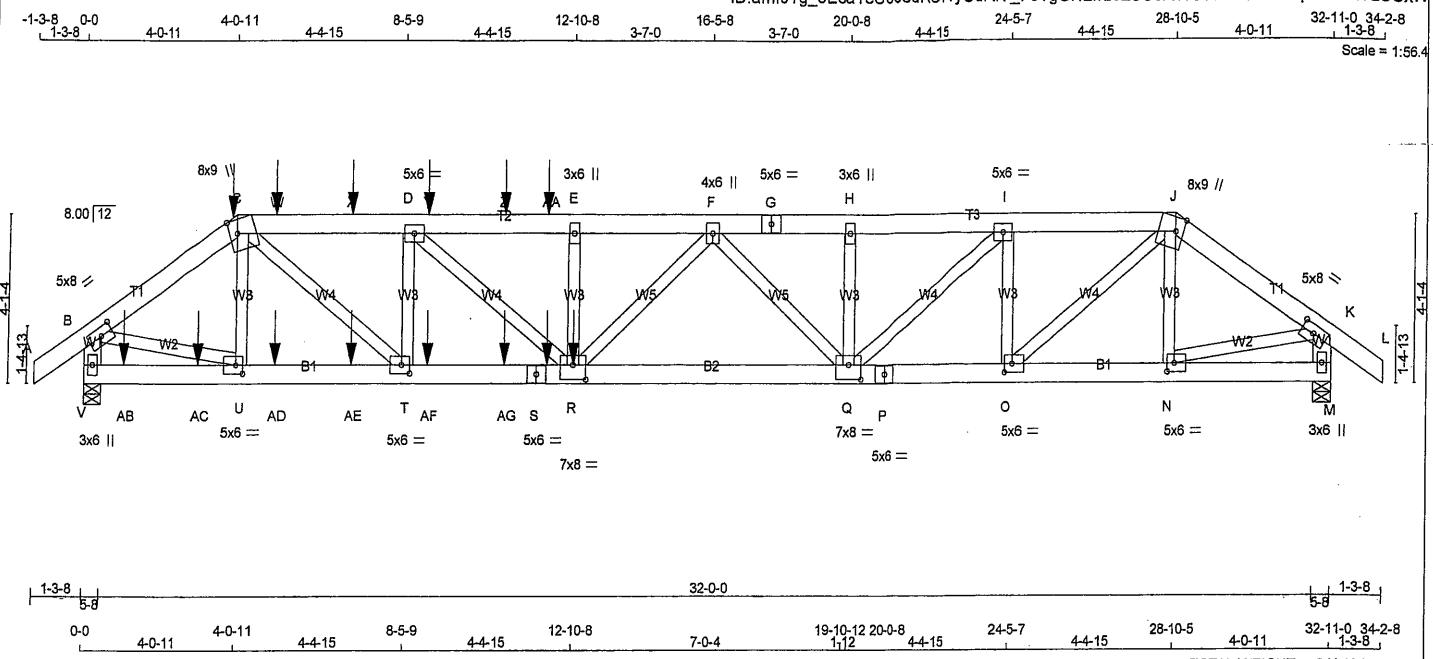

TOTAL WEIGHT = 2 X 194 = 389 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
J- L	2	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(0.0)
S- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- R	1	5	SIDE(363.6)
H- Q	1	5	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.50
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMW+w	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	Edge	2.50
K	TMVW-t	MT20	5.0	8.0	2.50	3.75
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	7.0	8.0	4.25	4.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4748	0	4748	0	0	5-8	5-8
M	3572	0	3572	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3762	2223 / 0	680 / 0	0 / 0	0 / 0	858 / 0	0 / 0
M	2820	1683 / 0	498 / 0	0 / 0	0 / 0	639 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-5822 / 0	-112.4	-112.4	0.13 (1)	4.79
C- W	-8632 / 0	-112.4	-112.4	0.25 (1)	3.98
W- X	-8632 / 0	-112.4	-112.4	0.25 (1)	3.98
X- D	-8632 / 0	-112.4	-112.4	0.25 (1)	3.98
D- Y	-11004 / 0	-112.4	-112.4	0.34 (1)	3.48
Y- Z	-11004 / 0	-112.4	-112.4	0.34 (1)	3.48
Z-AA	-11004 / 0	-112.4	-112.4	0.34 (1)	3.48
AA- E	-11004 / 0	-112.4	-112.4	0.34 (1)	3.48
E- F	-11004 / 0	-112.4	-112.4	0.26 (1)	3.56
F- G	-8865 / 0	-112.4	-112.4	0.19 (1)	3.99
G- H	-8865 / 0	-112.4	-112.4	0.19 (1)	3.99
H- I	-8865 / 0	-112.4	-112.4	0.20 (1)	3.98
I- J	-6611 / 0	-112.4	-112.4	0.16 (1)	4.54
J- K	-4269 / 0	-112.4	-112.4	0.11 (1)	5.44
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00
V- B	-4667 / 0	0.0	0.0	0.16 (1)	6.68
M- K	-3526 / 0	0.0	0.0	0.12 (1)	7.46
V-AB	0 / 0	-28.0	-28.0	0.06 (3)	10.00
AB-AC	0 / 0	-28.0	-28.0	0.06 (3)	10.00
AC- U	0 / 0	-28.0	-28.0	0.06 (3)	10.00
U-AD	0 / 4800	-28.0	-28.0	0.35 (1)	10.00
AD-AE	0 / 4800	-28.0	-28.0	0.35 (1)	10.00
AE- T	0 / 4800	-28.0	-28.0	0.35 (1)	10.00
T-AF	0 / 8632	-28.0	-28.0	0.63 (1)	10.00
AF-AG	0 / 8632	-28.0	-28.0	0.63 (1)	10.00
AG- S	0 / 8632	-28.0	-28.0	0.63 (1)	10.00
S- R	0 / 8632	-28.0	-28.0	0.63 (1)	10.00
R- Q	0 / 10137	-28.0	-28.0	0.74 (1)	10.00
Q- P	0 / 6611	-28.0	-28.0	0.46 (1)	10.00
P- O	0 / 6611	-28.0	-28.0	0.46 (1)	10.00
O- N	0 / 3519	-28.0	-28.0	0.26 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-786 / 0	0.07 (1)
C- T	0 / 4927	0.44 (1)
T- D	-2894 / 0	0.25 (1)
D- R	0 / 3098	0.27 (1)
R- E	-581 / 0	0.05 (1)
Q- H	-417 / 0	0.04 (1)
Q- I	0 / 2943	0.26 (1)
O- I	-2413 / 0	0.21 (1)
O- J	0 / 3976	0.35 (1)
N- J	-601 / 0	0.05 (1)
B- U	0 / 4940	0.44 (1)
N- K	0 / 3623	0.32 (1)
R- F	0 / 1238	0.11 (1)
F- Q	-1815 / 0	0.28 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-45	-50	---	FRONT	VERT	DEAD
C	4-0-11	-282	-282	---	FRONT	VERT	SNOW
R	12-10-8	-2096	-2096	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER 4316 S KATSOULAKOS PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.34/1.00 (D-E:1) , BC=0.74/1.00 (Q-R:1) , WB=0.44/1.00 (B-U:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (U) (INPUT = 0.90)
JSI METAL= 0.71 (P) (INPUT = 1.00)

DWG NO. TAM 17191-18
STRUCTURAL COMPONENT ONLY

pb 4

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296384	T31	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	7.0	8.0	4.25	4.00
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.50
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, 135.3 lbs FACTORED DOWN AT 11-0-12, AND 135.3 lbs FACTORED DOWN AT 12-2-4, AND 327.0 lbs FACTORED DOWN AT 4-0-11 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, AND 69.9 lbs FACTORED DOWN AT 12-2-4, AND 2095.6 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	12-2-4	-40	-70	—	FRONT	VERT	TOTAL
W	5-0-12	-135	-135	—	FRONT	VERT	TOTAL
X	7-0-12	-135	-135	—	FRONT	VERT	TOTAL
Y	9-0-12	-135	-135	—	FRONT	VERT	TOTAL
Z	11-0-12	-135	-135	—	FRONT	VERT	TOTAL
AA	12-2-4	-135	-135	—	FRONT	VERT	TOTAL
AB	1-0-12	-40	-70	—	FRONT	VERT	TOTAL
AC	3-0-12	-40	-70	—	FRONT	VERT	TOTAL
AD	5-0-12	-40	-70	—	FRONT	VERT	TOTAL
AE	7-0-12	-40	-70	—	FRONT	VERT	TOTAL
AF	9-0-12	-40	-70	—	FRONT	VERT	TOTAL
AG	11-0-12	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
431B
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17191-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T31S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 3 10:55:07 2018 Page 1
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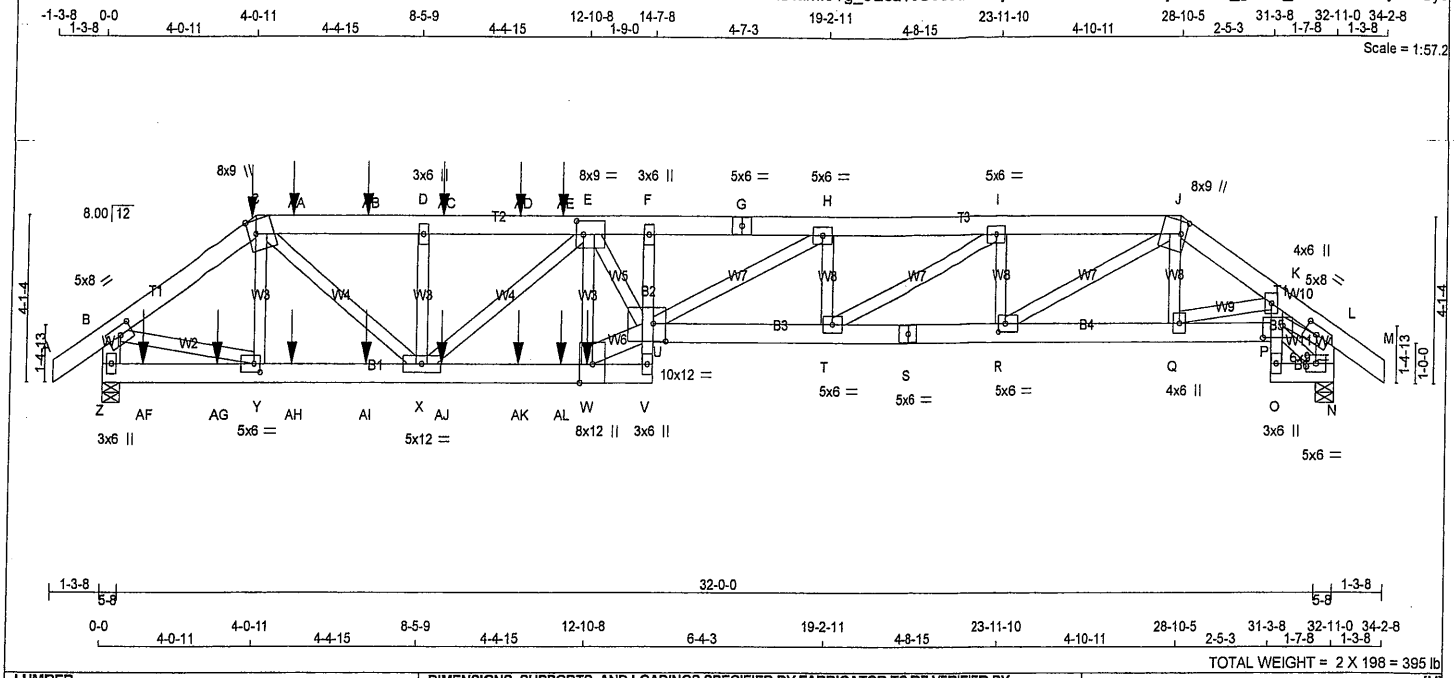

TOTAL WEIGHT = 2 X 198 = 395 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - M	2x6	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - V	2x6	DRY	No.2	SPF
V - F	2x4	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - U	2x6	DRY	No.2	SPF
P - L	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(61.0)
G- J	2	12	TOP
J- M	2	12	TOP
Z- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- V	2	12	SIDE(183.1)
U- S	2	12	TOP
S- P	2	12	TOP
O- N	2	12	TOP
V- F	1	12	TOP
K- O	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
E- W	1	4	SIDE(420.5)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW+m	MT20	8.0	9.0	Edge	2.50
D	TMW+w	MT20	3.0	6.0		
E	TMWWW-t	MT20	8.0	9.0	4.00	2.25
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	4877	0	4877	0	0	5-8	5-8
N	3661	0	3661	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	3863	2285 / 0	697 / 0	0 / 0	0 / 0	881 / 0	0 / 0
N	2889	1726 / 0	509 / 0	0 / 0	0 / 0	654 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.31 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD (PLF) TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00	
B- C	-6007 / 0	-112.4	-112.4	0.14 (1)	4.73	
C-AA	-8891 / 0	-112.4	-112.4	0.26 (1)	3.91	
AA-AB	-8891 / 0	-112.4	-112.4	0.26 (1)	3.91	
AB- D	-8891 / 0	-112.4	-112.4	0.26 (1)	3.91	
D-AC	-8891 / 0	-112.4	-112.4	0.24 (1)	3.93	
AC-AD	-8891 / 0	-112.4	-112.4	0.24 (1)	3.93	
AD-AE	-8891 / 0	-112.4	-112.4	0.24 (1)	3.93	
AE- E	-8891 / 0	-112.4	-112.4	0.24 (1)	3.93	
E- F	-15084 / 0	-112.4	-112.4	0.51 (1)	2.83	
F- G	-15193 / 0	-112.4	-112.4	0.60 (1)	2.75	
G- H	-15193 / 0	-112.4	-112.4	0.60 (1)	2.75	
H- I	-13068 / 0	-112.4	-112.4	0.38 (1)	3.19	
I- J	-9659 / 0	-112.4	-112.4	0.28 (1)	3.77	
J- K	-5896 / 0	-112.4	-112.4	0.22 (1)	4.66	
K- L	-5949 / 0	-112.4	-112.4	0.22 (1)	4.64	
L- M	0 / 44	-112.4	-112.4	0.04 (1)	10.00	
Z- B	-4803 / 0	0.0	0.0	0.17 (1)	6.61	
N- L	-3480 / 0	0.0	0.0	0.12 (1)	7.49	
Z-AF	0 / 0	-28.0	-28.0	0.06 (3)	10.00	
AF-AG	0 / 0	-28.0	-28.0	0.06 (3)	10.00	
AG- Y	0 / 0	-28.0	-28.0	0.06 (3)	10.00	
Y-AH	0 / 4954	-28.0	-28.0	0.39 (1)	10.00	
AH-AI	0 / 4954	-28.0	-28.0	0.39 (1)	10.00	
AI- X	0 / 4954	-28.0	-28.0	0.39 (1)	10.00	
X-AJ	0 / 11570	-28.0	-28.0	0.81 (1)	10.00	
AJ-AK	0 / 11570	-28.0	-28.0	0.81 (1)	10.00	
AK-AL	0 / 11570	-28.0	-28.0	0.81 (1)	10.00	
AL- W	0 / 11570	-28.0	-28.0	0.81 (1)	10.00	
W- V	0 / 290	-28.0	-28.0	0.08 (1)	10.00	
V- U	-81 / 0	0.0	0.0	0.22 (1)	7.81	
U- F	0 / 121	0.0	0.0	0.22 (1)	10.00	
U- T	0 / 13068	-28.0	-28.0	0.92 (1)	10.00	
T- S	0 / 9659	-28.0	-28.0	0.71 (1)	10.00	
S- R	0 / 9659	-28.0	-28.0	0.71 (1)	10.00	
R- Q	0 / 4982	-28.0	-28.0	0.37 (1)	10.00	
Q- P	0 / 5039	-28.0	-28.0	0.37 (1)	10.00	
O- P	0 / 36	0.0	0.0	0.18 (1)	10.00	
P- K	0 / 188	0.0	0.0	0.19 (1)	10.00	
O- N	0 / 238	-28.0	-28.0	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-776 / 0	0.07 (1)
C- X	0 / 5062	0.45 (1)
X- D	-649 / 0	0.07 (1)
X- E	-3499 / 0	0.64 (1)
W- E	-4396 / 0	0.38 (1)
W- U	0 / 13292	0.86 (1)
E- U	0 / 6778	0.60 (1)
Q- J	0 / 272	0.02 (2)
Q- K	-73 / 0	0.01 (1)
B- Y	0 / 5097	0.45 (1)
R- J	0 / 5306	0.47 (1)
U- H	0 / 2433	0.22 (1)
R- I	-2411 / 0	0.16 (1)
T- H	-1821 / 0	0.12 (1)
T- I	0 / 3904	0.35 (1)
P- N	-267 / 0	0.02 (1)
P- L	0 / 4781	0.59 (1)

LICENSED PROFESSIONAL ENGINEER
43B
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.39")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 623 (0.63")

CSI: TC=0.60/1.00 (H-H:1) , BC=0.92/1.00 (T-U:1) , WB=0.86/1.00 (U-W:1) , SSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 17195-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T31S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-NUrTTDIoze0Rjr?6SW2_gbi5U_GrEHrbJKwtytzUSyo

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	8.0	9.0	3.75	1.75
K	TMVW+p	MT20	4.0	6.0		
L	TMVW-t	MT20	5.0	8.0	2.50	3.75
N	BMVW1-t	MT20	5.0	6.0		
O	BMV+p	MT20	3.0	6.0		
P	BVMWW-l	MT20	6.0	9.0	3.50	6.00
Q	BMWW+t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BVMWWW-l	MT20	10.0	12.0	5.25	4.00
V	BMV+p	MT20	3.0	6.0		
W	BMWW+t	MT20	8.0	12.0	Edge	
X	BMWWW-t	MT20	5.0	12.0		
Y	BMWW-t	MT20	5.0	6.0	2.50	2.00
Z	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 4-0-11, 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, AND 135.3 lbs FACTORED DOWN AT 11-0-12, AND 135.3 lbs FACTORED DOWN AT 12-2-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, AND 69.9 lbs FACTORED DOWN AT 12-2-4, AND 2313.8 lbs FACTORED DOWN AT 12-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-45	-50	—	FRONT	VERT	DEAD
C	4-0-11	-282	-282	—	FRONT	VERT	SNOW
W	12-10-8	-2314	-2314	—	FRONT	VERT	TOTAL
AA	5-0-12	-135	-135	—	FRONT	VERT	TOTAL
AB	7-0-12	-135	-135	—	FRONT	VERT	TOTAL
AC	9-0-12	-135	-135	—	FRONT	VERT	TOTAL
AD	11-0-12	-135	-135	—	FRONT	VERT	TOTAL
AE	12-2-4	-135	-135	—	FRONT	VERT	TOTAL
AF	1-0-12	-40	-70	—	FRONT	VERT	TOTAL
AG	3-0-12	-40	-70	—	FRONT	VERT	TOTAL
AH	5-0-12	-40	-70	—	FRONT	VERT	TOTAL
AI	7-0-12	-40	-70	—	FRONT	VERT	TOTAL
AJ	9-0-12	-40	-70	—	FRONT	VERT	TOTAL
AK	11-0-12	-40	-70	—	FRONT	VERT	TOTAL
AL	12-2-4	-40	-70	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.92 (W) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4328
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17195-B
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296384	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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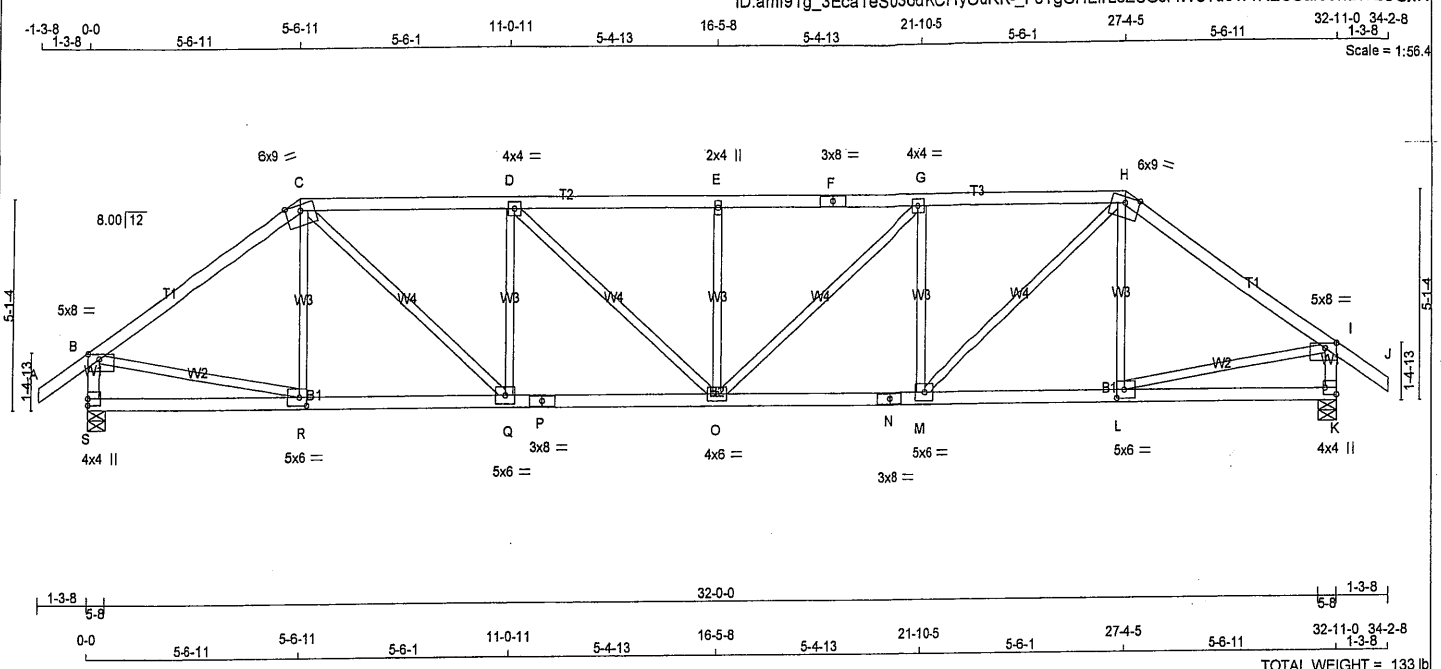

TOTAL WEIGHT = 133 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2466	0	2466	0	0	5-8	5-8
K	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2707 / 0	-112.4	-112.4	0.91 (1)	3.22
C- D	-3607 / 0	-112.4	-112.4	0.80 (1)	2.95
D- E	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
E- F	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
F- G	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
G- H	-3607 / 0	-112.4	-112.4	0.80 (1)	2.95
H- I	-2707 / 0	-112.4	-112.4	0.91 (1)	3.22
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00
S- B	-2401 / 0	0.0	0.0	0.25 (1)	5.52
K- I	-2401 / 0	0.0	0.0	0.25 (1)	5.52
S- R	0 / 0	-28.0	-28.0	0.22 (3)	10.00
R- Q	0 / 2244	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
P- O	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
O- N	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
N- M	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
M- L	0 / 2244	-28.0	-28.0	0.48 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-252 / 109	0.10 (1)
C- Q	0 / 1809	0.41 (1)
Q- D	-1045 / 0	0.40 (1)
D- O	0 / 535	0.12 (1)
O- E	-560 / 0	0.22 (1)
O- G	0 / 535	0.12 (1)
M- G	-1045 / 0	0.40 (1)
M- H	0 / 1809	0.41 (1)
L- H	-252 / 109	0.10 (1)
B- R	0 / 2290	0.52 (1)
L- I	0 / 2290	0.52 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 990 (0.40")

CSI: TC=0.91/1.00 (H-I:1) , BC=0.65/1.00 (M-O:1) , WB=0.52/1.00 (I-L:1) , SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.90 (N) (INPUT = 1.00)

DWG NO. TAM 171918
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T32S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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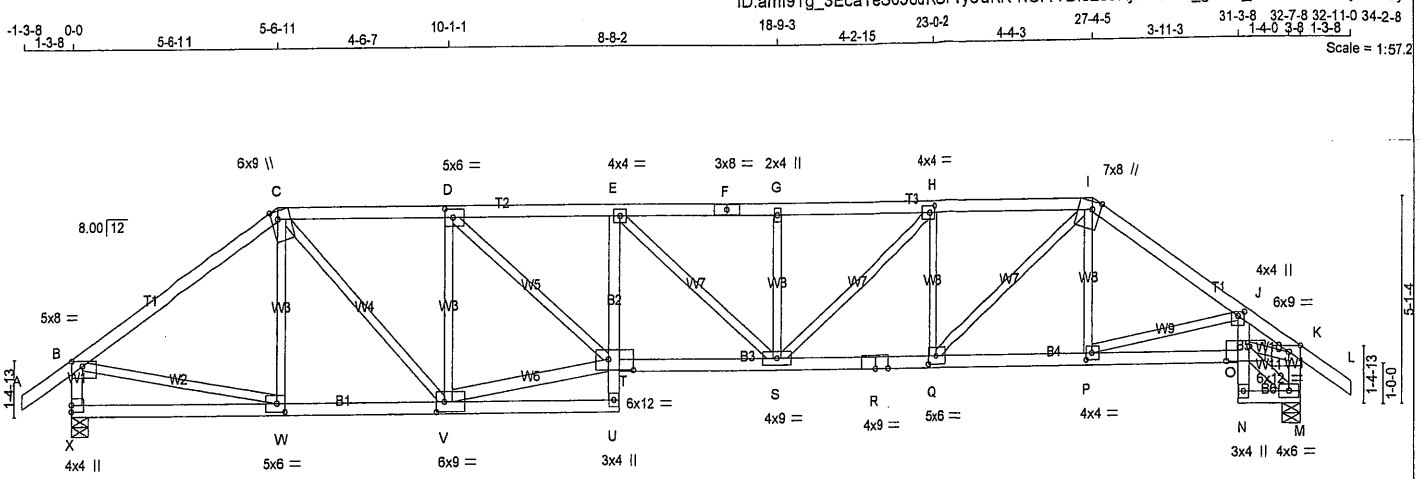

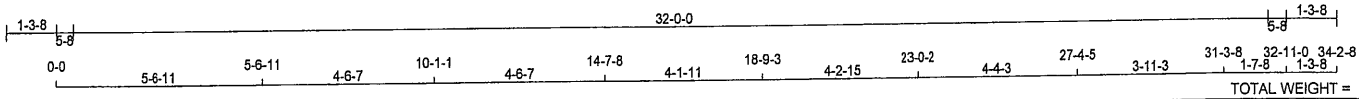

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - U	2x4	DRY	No.2	SPF
U - E	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-p	MT20	6.0	9.0	Edge	3.50
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	6.0	12.0	4.25	7.00
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BS-t	MT20	4.0	9.0		
S	BMWWW-t	MT20	4.0	9.0		
T	BVMWW-l	MT20	6.0	12.0	3.25	8.00
U	BMV+p	MT20	3.0	4.0		
V	BMWWW-t	MT20	6.0	9.0	3.00	2.50
W	BMWW-t	MT20	5.0	6.0	2.50	2.50
X	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2462	0	2462	0	0	5-8	5-8
M	2469	0	2469	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2699 / 0	-112.4	-112.4	0.91 (1)	3.22
C- D	-3391 / 0	-112.4	-112.4	0.52 (1)	3.36
D- E	-4940 / 0	-112.4	-112.4	0.71 (1)	2.61
E- F	-4917 / 0	-112.4	-112.4	0.56 (1)	2.80
F- G	-4917 / 0	-112.4	-112.4	0.56 (1)	2.80
G- H	-4917 / 0	-112.4	-112.4	0.65 (1)	2.69
H- I	-4233 / 0	-112.4	-112.4	0.56 (1)	3.00
I- J	-3373 / 0	-112.4	-112.4	0.50 (1)	3.39
J- K	-4075 / 0	-112.4	-112.4	0.32 (1)	3.23
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
X- B	-2396 / 0	0.0	0.0	0.25 (1)	5.52
M- K	-2317 / 0	0.0	0.0	0.24 (1)	5.61
X- W	0 / 0	-28.0	-28.0	0.20 (3)	10.00
W- V	0 / 2237	-28.0	-28.0	0.46 (1)	10.00
V- U	0 / 125	-28.0	-28.0	0.13 (3)	10.00
U- T	0 / 92	0.0	0.0	0.18 (1)	10.00
T- E	-414 / 0	0.0	0.0	0.19 (1)	7.81
T- S	0 / 4974	-28.0	-28.0	0.87 (1)	10.00
S- R	0 / 4234	-28.0	-28.0	0.74 (1)	10.00
R- Q	0 / 4234	-28.0	-28.0	0.74 (1)	10.00
Q- P	0 / 2803	-28.0	-28.0	0.52 (1)	10.00
P- O	0 / 3589	-28.0	-28.0	0.65 (1)	10.00
N- O	0 / 36	0.0	0.0	0.27 (1)	10.00
O- J	0 / 296	0.0	0.0	0.31 (1)	10.00
N- M	0 / 165	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-267 / 84	0.10 (1)
C- V	0 / 1693	0.38 (1)
V- D	-1874 / 0	0.72 (1)
P- I	0 / 265	0.06 (2)
P- J	-815 / 0	0.25 (1)
B- W	0 / 2284	0.51 (1)
Q- I	0 / 1899	0.43 (1)
E- S	-77 / 0	0.04 (1)
Q- H	-1140 / 0	0.29 (1)
S- G	-468 / 0	0.12 (1)
S- H	0 / 918	0.21 (1)
V- T	0 / 3379	0.54 (1)
D- T	0 / 2010	0.45 (1)
O- M	-211 / 0	0.02 (1)
O- K	0 / 3376	0.76 (1)

LICENSED PROFESSIONAL ENGINEER
4376
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 690 (0.57")

CSI: TC=0.91/1.00 (B-C:1) , BC=0.87/1.00 (S-T:1) , WB=0.76/1.00 (K-O:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.83 (R) (INPUT = 1.00)

DWG NO. TAM 17196-B8
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T33S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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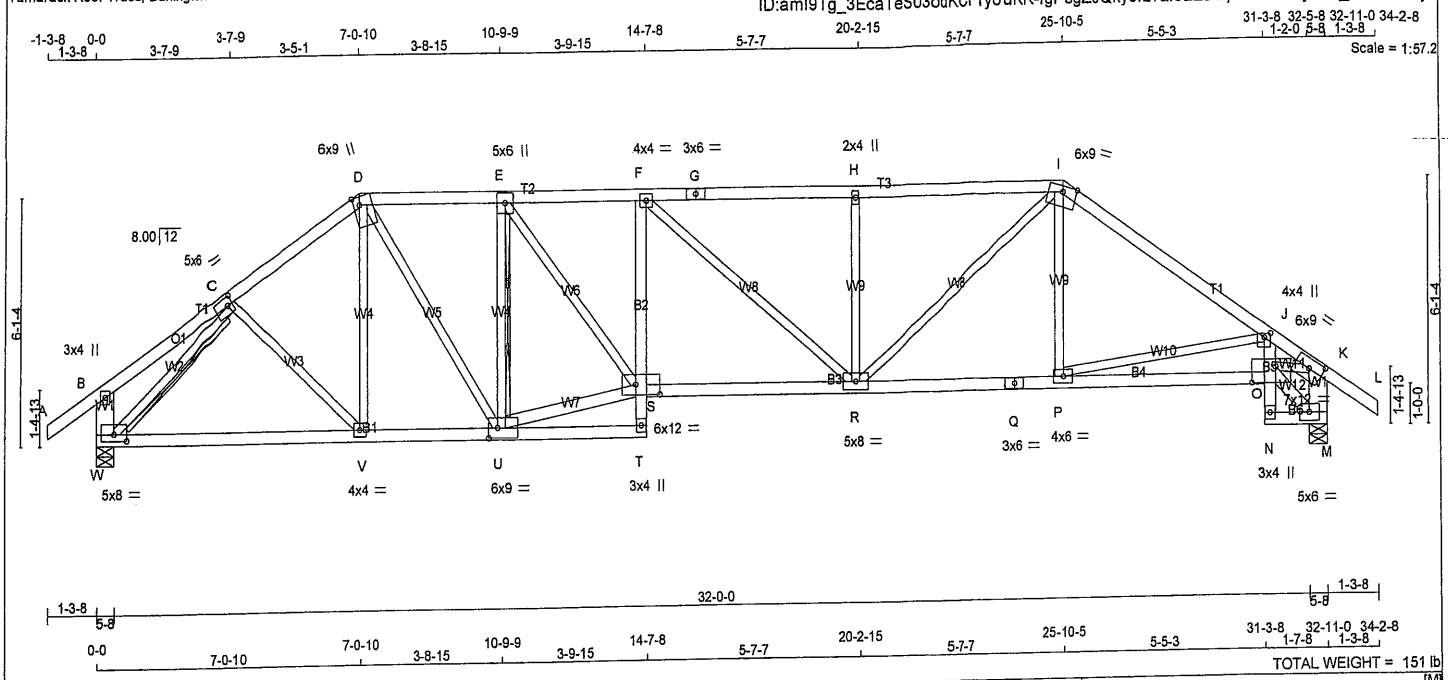

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - F	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW+t	MT20	5.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMVW+p	MT20	4.0	4.0	1.25	2.00
K	TMVW-t	MT20	6.0	9.0	2.75	4.25
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-l	MT20	7.0	12.0	4.75	7.25
P	BMWW-t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BVMWW-l	MT20	6.0	12.0	3.00	7.75
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	6.0	9.0	3.00	2.75
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	8.0	2.00	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2462	0	2462	0	0	5-8	5-8
M	2469	0	2469	0	0	5-8	5-8

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-W, E-U

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.19 (1)	10.00
C- D	-2702 / 0	-112.4	-112.4	0.30 (1)	3.96
D- E	-2893 / 0	-112.4	-112.4	0.31 (1)	3.83
E- F	-3932 / 0	-112.4	-112.4	0.46 (1)	3.17
F- G	-3834 / 0	-112.4	-112.4	0.93 (1)	2.70
G- H	-3834 / 0	-112.4	-112.4	0.93 (1)	2.70
H- I	-3834 / 0	-112.4	-112.4	0.94 (1)	2.70
I- J	-3229 / 0	-112.4	-112.4	0.86 (1)	3.16
J- K	-4423 / 0	-112.4	-112.4	0.47 (1)	2.91
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
W- B	-313 / 0	0.0	0.0	0.02 (1)	7.81
M- K	-2302 / 0	0.0	0.0	0.15 (1)	6.76
W- V	0 / 2122	-28.0	-28.0	0.53 (2)	10.00
V- U	0 / 2228	-28.0	-28.0	0.55 (2)	10.00
U- T	0 / 81	-28.0	-28.0	0.11 (3)	10.00
T- S	0 / 84	0.0	0.0	0.12 (1)	10.00
S- F	-404 / 0	0.0	0.0	0.16 (1)	7.81
S- R	0 / 3949	-28.0	-28.0	0.75 (1)	10.00
R- Q	0 / 2674	-28.0	-28.0	0.55 (1)	10.00
Q- P	0 / 2674	-28.0	-28.0	0.55 (1)	10.00
P- O	0 / 3965	-28.0	-28.0	0.76 (1)	10.00
N- O	0 / 36	0.0	0.0	0.30 (1)	10.00
O- J	0 / 439	0.0	0.0	0.37 (1)	10.00
N- M	0 / 177	-28.0	-28.0	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- V	0 / 197	0.04 (2)
V- D	0 / 222	0.05 (3)
F- R	-151 / 0	0.16 (1)
R- H	-733 / 0	0.28 (1)
R- I	0 / 1514	0.34 (1)
P- I	0 / 355	0.08 (2)
P- J	-1317 / 0	0.76 (1)
W- C	-2962 / 0	0.92 (1)
O- M	-230 / 0	0.02 (1)
O- K	0 / 3723	0.84 (1)
U- E	-1751 / 0	0.83 (1)
D- U	0 / 1242	0.28 (1)
U- S	0 / 2937	0.47 (1)
E- S	0 / 1672	0.38 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 860 (0.46")

CSI: TC=0.94/1.00 (H-I:1) , BC=0.76/1.00 (O-P:1) , WB=0.92/1.00 (C-W:1) , SSI=0.30/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.76 (K) (INPUT = 1.00)

DWG NO. TAM 17197-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296384	T33	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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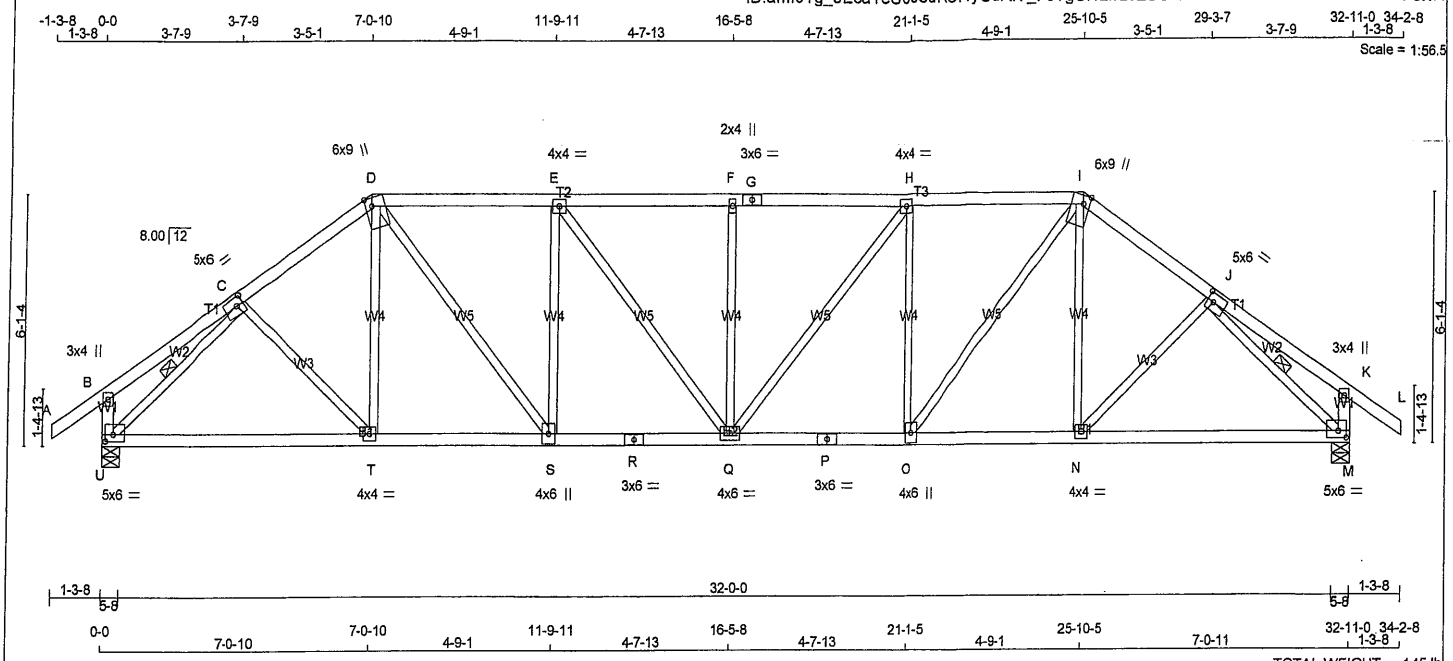

TOTAL WEIGHT = 145 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2466	0	2466	0	0	5-8	5-8
M	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
M	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-U, J-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.19 (1)	10.00
C- D	-2707 / 0	-112.4	-112.4	0.30 (1)	3.96
D- E	-3058 / 0	-112.4	-112.4	0.53 (1)	3.50
E- F	-3306 / 0	-112.4	-112.4	0.55 (1)	3.34
F- G	-3306 / 0	-112.4	-112.4	0.55 (1)	3.34
G- H	-3306 / 0	-112.4	-112.4	0.55 (1)	3.34
H- I	-3058 / 0	-112.4	-112.4	0.53 (1)	3.50
I- J	-2707 / 0	-112.4	-112.4	0.30 (1)	3.96
J- K	0 / 22	-112.4	-112.4	0.19 (1)	10.00
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
U- B	-313 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-313 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 2126	-28.0	-28.0	0.53 (2)	10.00
T- S	0 / 2232	-28.0	-28.0	0.56 (2)	10.00
S- R	0 / 3058	-28.0	-28.0	0.55 (1)	10.00
R- Q	0 / 3058	-28.0	-28.0	0.55 (1)	10.00
Q- P	0 / 3058	-28.0	-28.0	0.55 (1)	10.00
P- O	0 / 3058	-28.0	-28.0	0.55 (1)	10.00
O- N	0 / 2232	-28.0	-28.0	0.56 (2)	10.00
N- M	0 / 2126	-28.0	-28.0	0.53 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 197	0.04 (2)
T- D	0 / 233	0.06 (3)
D- S	0 / 1303	0.29 (1)
S- E	-903 / 0	0.53 (1)
E- Q	0 / 396	0.09 (1)
Q- F	-481 / 0	0.28 (1)
Q- H	0 / 396	0.09 (1)
O- H	-903 / 0	0.53 (1)
O- I	0 / 1303	0.29 (1)
N- I	0 / 233	0.06 (3)
N- J	0 / 197	0.04 (2)
U- C	-2967 / 0	0.59 (1)
J- M	-2967 / 0	0.59 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.55/1.00 (E-F:1) , BC=0.56/1.00 (N-O:2) , WB=0.59/1.00 (J-M:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.86 (R) (INPUT = 1.00)

DWG NO. TAM 12193-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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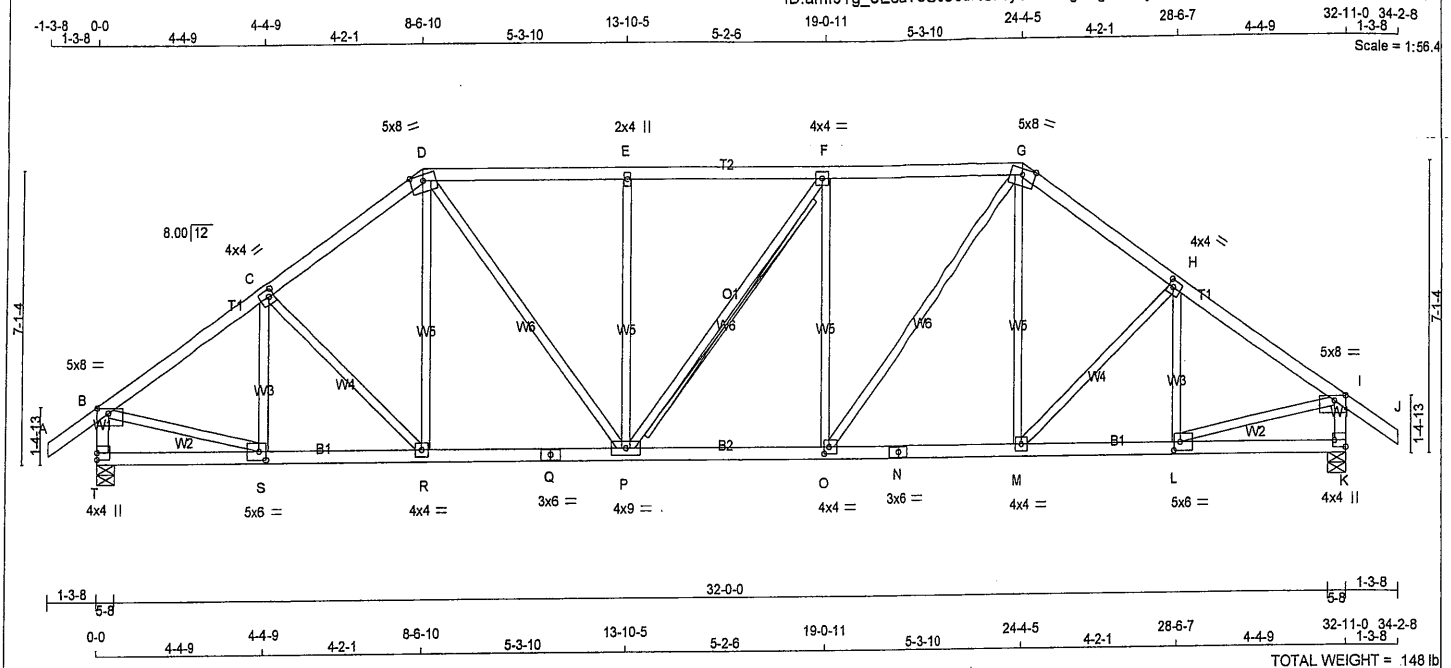

TOTAL WEIGHT = 148 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	4.00
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.50
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2466	0	2466	0	0	5-8	5-8
K	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No 2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)		10.00
B- C	-2686 / 0	-112.4	-112.4	0.35 (1)		3.94
C- D	-2630 / 0	-112.4	-112.4	0.34 (1)		3.97
D- E	-2773 / 0	-112.4	-112.4	0.46 (1)		3.74
E- F	-2773 / 0	-112.4	-112.4	0.43 (1)		3.78
F- G	-2775 / 0	-112.4	-112.4	0.46 (1)		3.74
G- H	-2629 / 0	-112.4	-112.4	0.34 (1)		3.97
H- I	-2686 / 0	-112.4	-112.4	0.35 (1)		3.94
I- J	0 / 43	-112.4	-112.4	0.15 (1)		10.00
T- B	-2410 / 0	0.0	0.0	0.25 (1)		5.51
K- I	-2410 / 0	0.0	0.0	0.25 (1)		5.51
T- S	0 / 0	-28.0	-28.0	0.12 (3)		10.00
S- R	0 / 2259	-28.0	-28.0	0.44 (1)		10.00
R- Q	0 / 2162	-28.0	-28.0	0.42 (1)		10.00
Q- P	0 / 2162	-28.0	-28.0	0.42 (1)		10.00
P- O	0 / 2775	-28.0	-28.0	0.52 (1)		10.00
O- N	0 / 2162	-28.0	-28.0	0.43 (1)		10.00
N- M	0 / 2162	-28.0	-28.0	0.43 (1)		10.00
M- L	0 / 2259	-28.0	-28.0	0.44 (1)		10.00
L- K	0 / 0	-28.0	-28.0	0.12 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-437 / 0	0.12 (1)
C- R	-138 / 0	0.08 (1)
R- D	0 / 289	0.07 (2)
D- P	0 / 993	0.22 (1)
P- E	-637 / 0	0.56 (1)
P- F	-3 / 0	0.00 (1)
O- F	-638 / 0	0.56 (1)
O- G	0 / 996	0.22 (1)
M- G	0 / 288	0.06 (2)
M- H	-138 / 0	0.08 (1)
L- H	-436 / 0	0.12 (1)
B- S	0 / 2327	0.52 (1)
L- I	0 / 2327	0.52 (1)

LICENSED PROFESSIONAL ENGINEER
4/3/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.52/1.00 (O-P:1) , WB=0.56/1.00 (F-O:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (G) (INPUT = 0.90)
JSI METAL= 0.62 (Q) (INPUT = 1.00)

DWG NO. TAM 17198-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T35	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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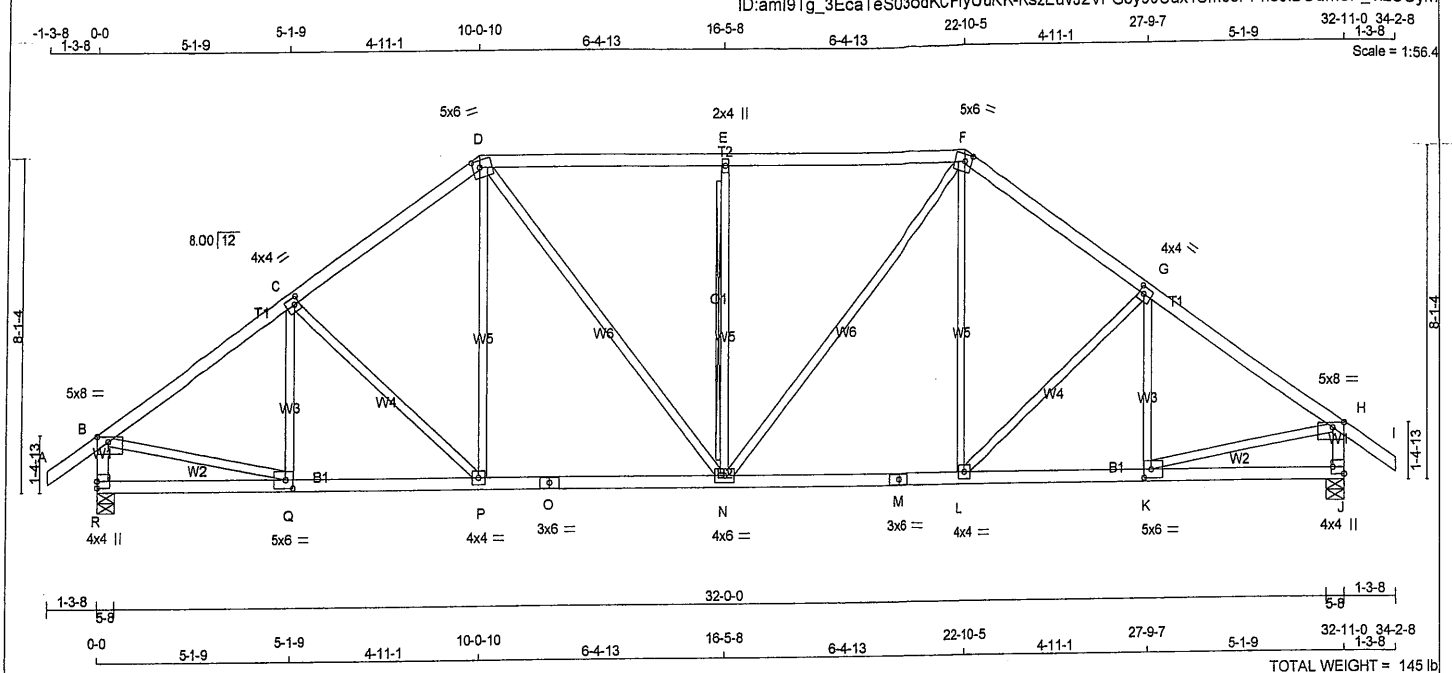

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
F	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
J	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2739 / 0	-112.4	-112.4	0.49 (1)	3.75
C- D	-2536 / 0	-112.4	-112.4	0.46 (1)	3.91
D- E	-2517 / 0	-112.4	-112.4	0.71 (1)	3.50
E- F	-2517 / 0	-112.4	-112.4	0.71 (1)	3.50
F- G	-2536 / 0	-112.4	-112.4	0.46 (1)	3.91
G- H	-2739 / 0	-112.4	-112.4	0.49 (1)	3.75
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00
R- B	-2403 / 0	0.0	0.0	0.25 (1)	5.52
J- H	-2403 / 0	0.0	0.0	0.25 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.16 (3)	10.00
Q- P	0 / 2309	-28.0	-28.0	0.47 (1)	10.00
P- O	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
O- N	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
M- L	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
L- K	0 / 2309	-28.0	-28.0	0.47 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-340 / 49	0.12 (1)
C- P	-313 / 0	0.26 (1)
P- D	0 / 406	0.09 (2)
D- N	0 / 688	0.15 (1)
N- E	-883 / 0	0.87 (1)
N- F	0 / 688	0.15 (1)
L- F	0 / 406	0.09 (2)
L- G	-313 / 0	0.26 (1)
K- G	-340 / 49	0.12 (1)
B- Q	0 / 2359	0.53 (1)
K- H	0 / 2359	0.53 (1)

LICENSED PROFESSIONAL ENGINEER
43318
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.47/1.00 (K-L:1) , WB=0.87/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

DWG NO. TAM 17199-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T36	9	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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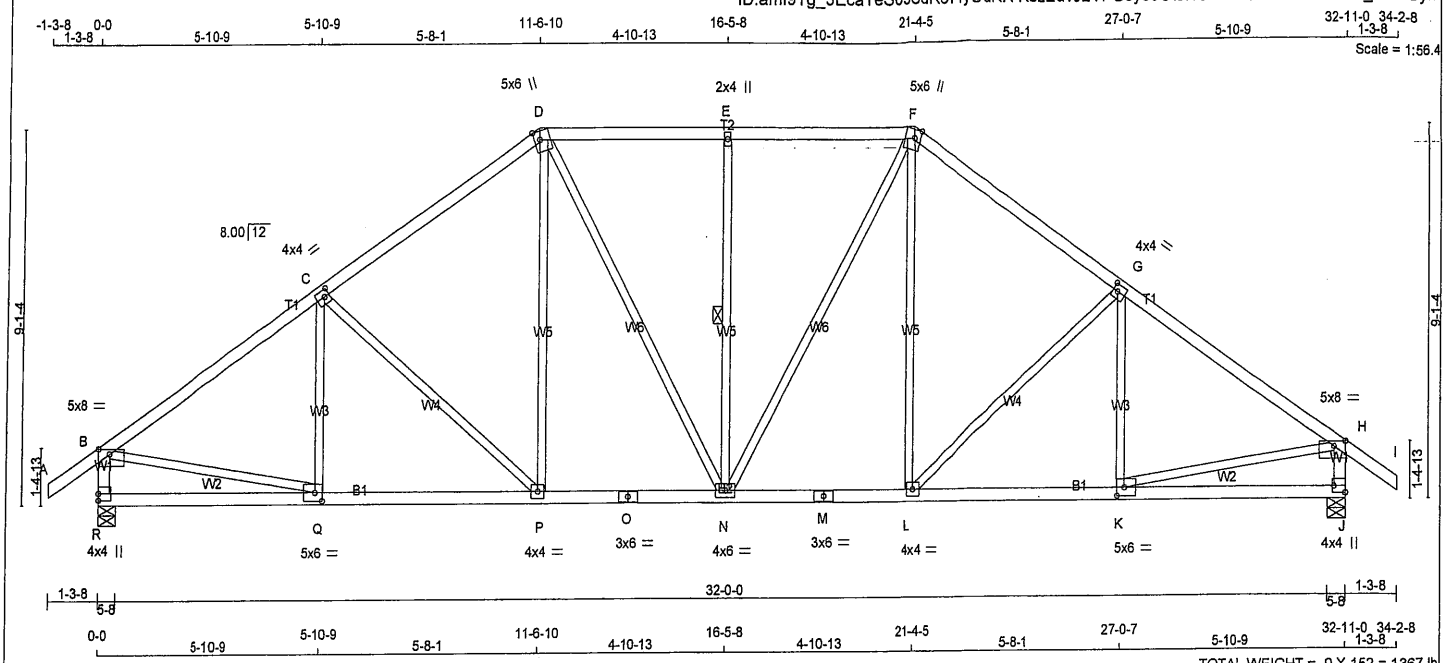

TOTAL WEIGHT = 9 X 152 = 1367 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
J	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2768 / 0	-112.4	-112.4	0.66 (1)	3.53
C- D	-2417 / 0	-112.4	-112.4	0.60 (1)	3.80
D- E	-2199 / 0	-112.4	-112.4	0.40 (1)	4.20
E- F	-2199 / 0	-112.4	-112.4	0.40 (1)	4.20
F- G	-2417 / 0	-112.4	-112.4	0.60 (1)	3.80
G- H	-2768 / 0	-112.4	-112.4	0.66 (1)	3.53
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00
R- B	-2397 / 0	0.0	0.0	0.25 (1)	5.52
J- H	-2397 / 0	0.0	0.0	0.25 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.24 (3)	10.00
Q- P	0 / 2338	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
O- N	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
N- M	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
M- L	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
L- K	0 / 2338	-28.0	-28.0	0.48 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-249 / 131	0.11 (1)
C- P	-484 / 0	0.57 (1)
P- D	0 / 466	0.10 (1)
D- N	0 / 455	0.10 (1)
N- E	-668 / 0	0.36 (1)
N- F	0 / 455	0.10 (1)
L- F	0 / 466	0.10 (1)
L- G	-484 / 0	0.57 (1)
K- G	-249 / 131	0.11 (1)
B- Q	0 / 2377	0.53 (1)
K- H	0 / 2377	0.53 (1)

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.66/1.00 (G-H:1) , BC=0.48/1.00 (K-L:1) , WB=0.57/1.00 (G-L:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.58 (Q) (INPUT = 1.00)

DWG NO. TAM 1720018
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T37	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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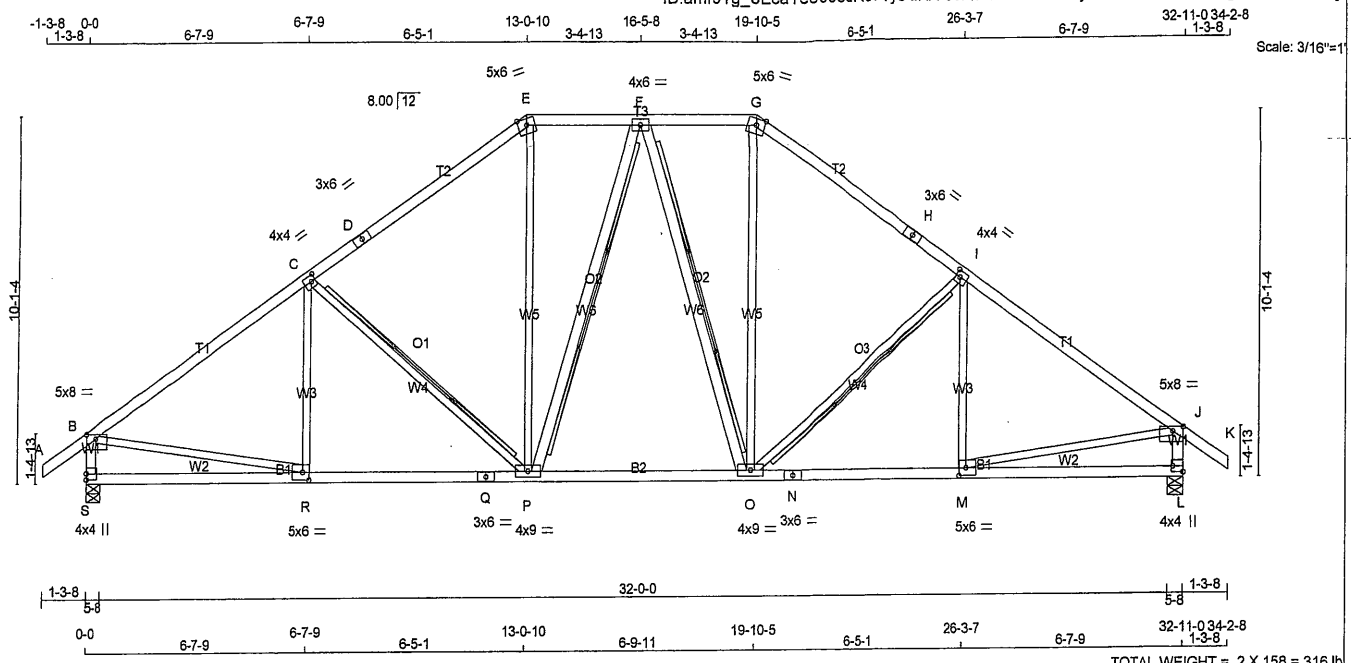

TOTAL WEIGHT = 2 X 158 = 316 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	5.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2466	0	2466	0	0	5-8	5-8
L	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
L	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-P, I-O
2x4 DRY SPF No.2 T-BRACE AT F-P, F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2776 / 0	-112.4	-112.4	0.87 (1)	3.20
C- D	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
D- E	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
E- F	-1871 / 0	-112.4	-112.4	0.18 (1)	4.75
F- G	-1871 / 0	-112.4	-112.4	0.18 (1)	4.75
G- H	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
H- I	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
I- J	-2776 / 0	-112.4	-112.4	0.87 (1)	3.20
J- K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
S- B	-2389 / 0	0.0	0.0	0.25 (1)	5.53
L- J	-2389 / 0	0.0	0.0	0.25 (1)	5.53
S- R	0 / 0	-28.0	-28.0	0.31 (3)	10.00
R- Q	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
Q- P	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
P- O	0 / 1946	-28.0	-28.0	0.44 (1)	10.00
O- N	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
N- M	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-181 / 188	0.09 (1)
C- P	-637 / 0	0.79 (1)
P- E	0 / 815	0.18 (1)
P- F	-235 / 0	0.18 (1)
F- O	-235 / 0	0.18 (1)
O- G	0 / 815	0.18 (1)
O- I	-637 / 0	0.79 (1)
M- I	-181 / 188	0.09 (1)
B- R	0 / 2381	0.54 (1)
M- J	0 / 2381	0.54 (1)

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4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.87/1.00 (I-J:1) , BC=0.54/1.00 (M-O:2) , WB=0.79/1.00 (I-O:1) , SSI=0.30/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.59 (Q) (INPUT = 1.00)

DWG NO. TAM 17201 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T38	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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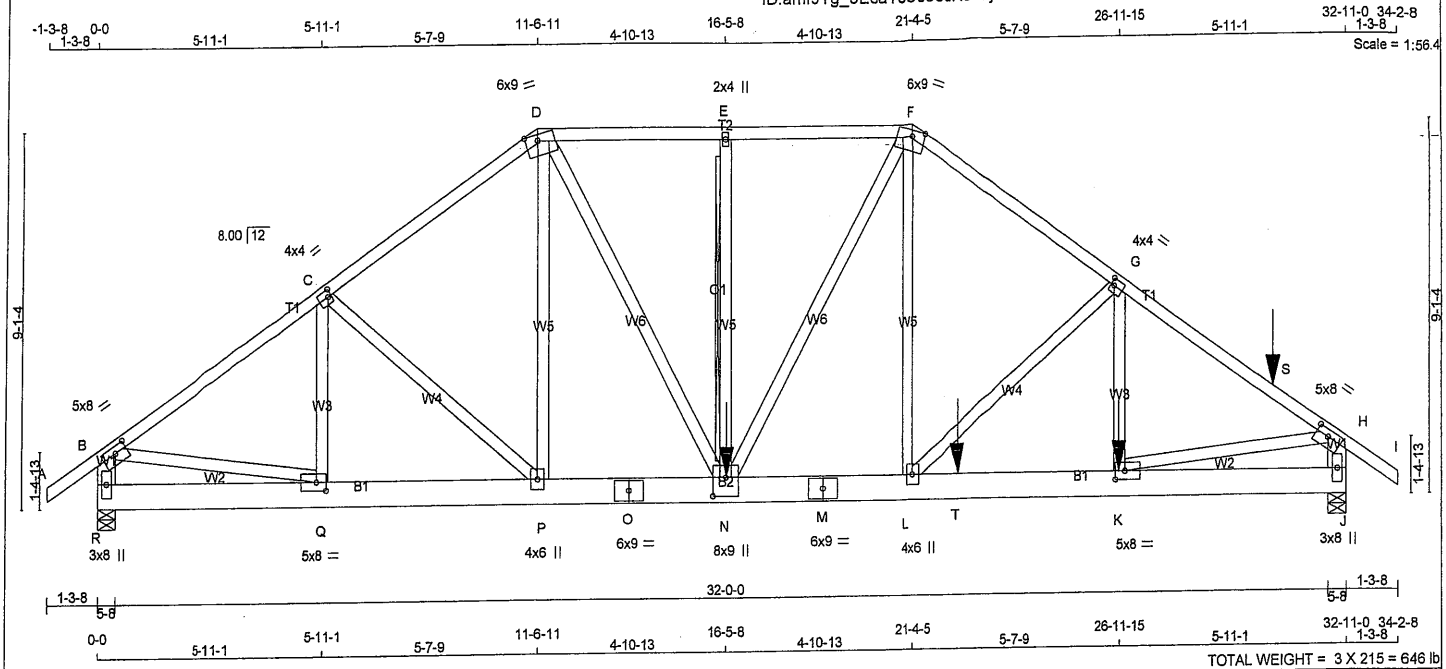

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
R - O	2x8	DRY	No.2	SPF
O - M	2x8	DRY	No.2	SPF
M - J	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
R- B	2	12	TOP
J- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R- O	2	6	TOP
O- M	2	6	SIDE(640.9)
M- J	2	6	SIDE(603.5)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- N	2	5	SIDE(1429.5)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.00	3.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.00
D	TTWW-m	MT20	6.0	9.0	1.75	3.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	6.0	9.0	1.75	3.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.00
H	TMVW-t	MT20	5.0	8.0	2.00	3.75
J	BMV1+p	MT20	3.0	8.0		
K	BMWW-t	MT20	5.0	8.0	2.50	3.00
L	BMWW+t	MT20	4.0	6.0		
M	BS-t	MT20	6.0	9.0		
N	BMWWW+t	MT20	8.0	9.0	5.50	4.00
O	BS-t	MT20	6.0	9.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	3.00
R	BMV1+p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	6886	0	6886	0	0	5-8	5-8
J	9260	0	9260	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	5388	3124 / 0	787 / 0	157 / 0	0 / 0	1320 / 0	0 / 0
J	7190	4070 / 0	834 / 0	479 / 0	0 / 0	1808 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.05 (1)	10.00	
B- C	-9187 / 0	-112.4	-112.4	0.49 (1)	3.72	
C- D	-9620 / 0	-112.4	-112.4	0.48 (1)	3.63	
D- E	-10547 / 0	-112.4	-112.4	0.31 (1)	3.55	
E- F	-10547 / 0	-112.4	-112.4	0.31 (1)	3.55	
F- G	-11190 / 0	-112.4	-112.4	0.58 (1)	3.34	
G- S	-12564 / 0	-112.4	-112.4	0.73 (1)	3.13	
S- H	-12564 / 0	-112.4	-112.4	0.73 (1)	3.13	
H- I	0 / 43	-112.4	-112.4	0.05 (1)	10.00	
R- B	-6731 / 0	0.0	0.0	0.14 (1)	6.83	
J- H	-9076 / 0	0.0	0.0	0.20 (1)	6.06	
R- Q	0 / 0	-28.0	-28.0	0.06 (1)	10.00	
Q- P	0 / 7677	-28.0	-28.0	0.31 (1)	10.00	
P- O	0 / 7965	-28.0	-28.0	0.37 (1)	10.00	
O- N	0 / 7965	-28.0	-28.0	0.37 (1)	10.00	
N- M	0 / 9324	-53.0	-53.0	0.49 (1)	10.00	
M- L	0 / 9324	-53.0	-53.0	0.49 (1)	10.00	
L- T	0 / 10482	-53.0	-53.0	0.56 (1)	10.00	
T- K	0 / 10482	-53.0	-53.0	0.56 (1)	10.00	
K- J	0 / 0	-53.0	-53.0	0.05 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-1244 / 0	0.11 (1)
C- P	0 / 370	0.02 (1)
P- D	0 / 332	0.02 (3)
D- N	0 / 5232	0.28 (1)
N- E	-598 / 0	0.05 (1)
N- F	0 / 2478	0.13 (1)
L- F	0 / 3552	0.10 (1)
L- G	-1593 / 0	0.41 (1)
K- G	0 / 1002	0.05 (1)
B- Q	0 / 7770	0.42 (1)
K- H	0 / 10609	0.57 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
K	26-11-12	-374	-419	---	FRONT	VERT	DEAD
K	26-11-12	-318	-477	---	FRONT	VERT	PERM.LIVE
K	26-11-12	-1400	-1400	---	FRONT	VERT	SNOW
N	16-5-12	-6567	-6567	---	BACK	VERT	TOTAL
S	30-11-12	-55	-108	---	TOP	VERT	TOTAL
T	22-7-4	-374	-419	---	FRONT	VERT	DEAD
T	22-7-4	-318	-477	---	FRONT	VERT	PERM.LIVE

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.73/1.00 (G-H:1) , BC=0.56/1.00 (K-L:1) , WB=0.57/1.00 (H-K:1) , SSI=0.35/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.80 (B) (INPUT = 1.00)

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DWG NO. TAM 172028
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T38	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 107.5 lbs FACTORED DOWN AT 30-11-12 ON TOP CHORD, AND 2091.3 lbs FACTORED DOWN AT 22-7-4, AND 2091.3 lbs FACTORED DOWN AT 26-11-12, AND 6566.6 lbs FACTORED DOWN AT 16-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	22-7-4	-1400	-1400	—	FRONT	VERT	SNOW

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DWG NO. TAM 17202-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T39	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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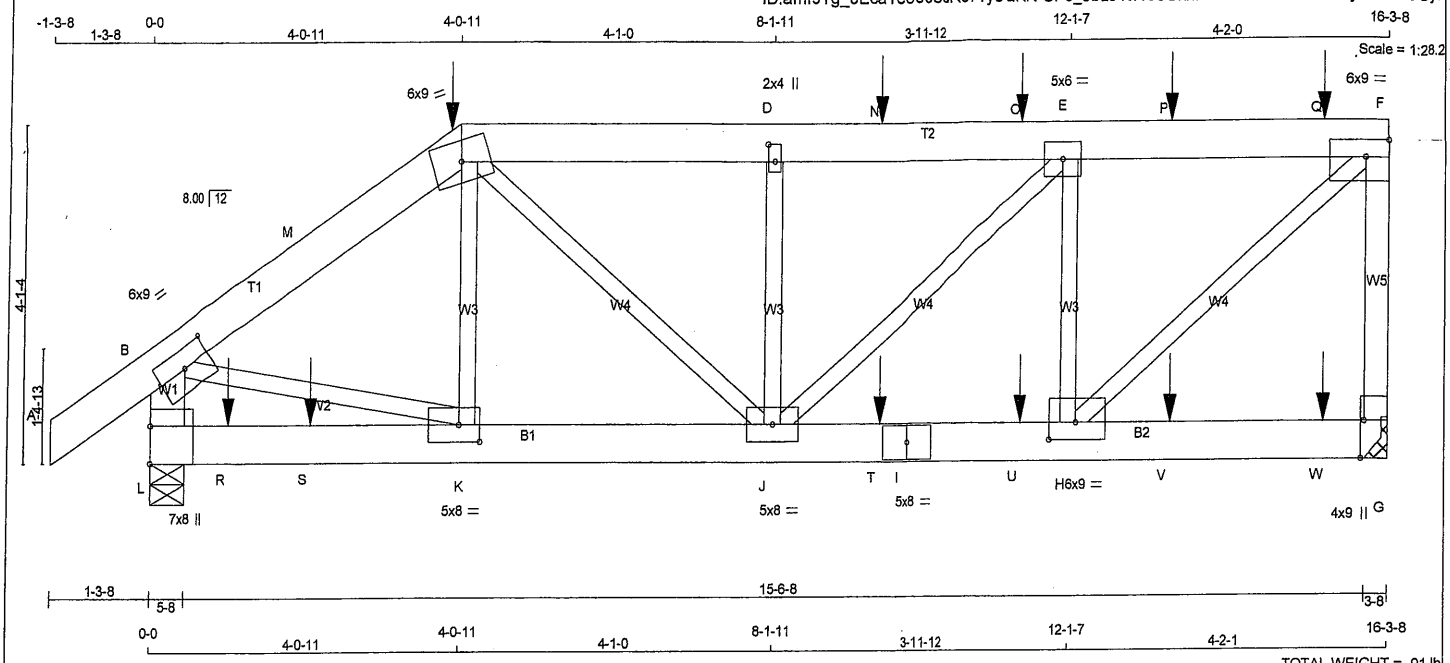

TOTAL WEIGHT = 91 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.50
C	TTWW-m	MT20	6.0	9.0		
D	TMW+w	MT20	2.0	4.0	2.50	1.00
E	TMWW-t	MT20	5.0	6.0		
F	TMVW-t	MT20	6.0	9.0	2.50	Edge
G	BMV1+t	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	6.0	9.0	2.50	4.25
I	BS-t	MT20	5.0	8.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BMWW-t	MT20	5.0	8.0	2.50	3.25
L	BMV1+t	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 385.0 lbs FACTORED DOWN AT 4-0-11, 135.3 lbs FACTORED DOWN AT 9-6-8, 135.3 lbs FACTORED DOWN AT 11-5-12, AND 135.3 lbs FACTORED DOWN AT 13-5-12, AND 145.9 lbs FACTORED DOWN AT 15-5-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 526.4 lbs FACTORED DOWN AT 2-1-8, 526.4 lbs FACTORED DOWN AT 9-6-8, 69.9 lbs FACTORED DOWN AT 11-5-12, AND 69.9 lbs FACTORED DOWN AT 13-5-12, AND 74.5 lbs FACTORED DOWN AT 15-5-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	2708	0	2708	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
L	3460	0	3460	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	2161	1267 / 0	413 / 0	0 / 0	0 / 0	480 / 0	0 / 0
L	2744	1645 / 0	508 / 0	0 / 0	0 / 0	591 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.09 (1)	10.00	
B- M	-3600 / 0	-112.4	-112.4	0.38 (1)	4.10	
M- C	-3600 / 0	-281.2	-281.2	0.38 (1)	4.10	
C- D	-3770 / 0	-281.2	-281.2	0.41 (1)	3.97	
D- N	-3770 / 0	-281.2	-281.2	0.32 (1)	4.10	
N- O	-3770 / 0	-112.4	-112.4	0.32 (1)	4.10	
O- E	-3770 / 0	-112.4	-112.4	0.32 (1)	4.10	
E- P	-2633 / 0	-112.4	-112.4	0.25 (1)	4.85	
P- Q	-2633 / 0	-112.4	-112.4	0.25 (1)	4.85	
Q- F	-2633 / 0	-112.4	-112.4	0.25 (1)	4.85	
G- F	-2638 / 0	0.0	0.0	0.61 (1)	5.23	
L- B	-3152 / 0	0.0	0.0	0.22 (1)	5.91	
L- R	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
R- S	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
S- K	0 / 0	-70.8	-70.8	0.40 (1)	10.00	
K- J	0 / 3027	-70.8	-70.8	0.57 (1)	10.00	
J- T	0 / 2633	-70.8	-70.8	0.69 (1)	10.00	
T- I	0 / 2633	-28.0	-28.0	0.69 (1)	10.00	
I- U	0 / 2633	-28.0	-28.0	0.69 (1)	10.00	
U- H	0 / 2633	-28.0	-28.0	0.69 (1)	10.00	
H- V	0 / 0	-28.0	-28.0	0.15 (2)	10.00	
V- W	0 / 0	-28.0	-28.0	0.15 (2)	10.00	
W- G	0 / 0	-28.0	-28.0	0.15 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-4 / 450	0.11 (3)
B- K	0 / 3096	0.77 (1)
H- F	0 / 3497	0.87 (1)
C- J	0 / 994	0.25 (1)
H- E	-1865 / 0	0.45 (1)
J- D	-1175 / 0	0.28 (1)
J- E	0 / 1542	0.38 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-55	-62	—	FRONT	VERT	DEAD
C	4-0-11	-330	-330	—	FRONT	VERT	SNOW
N	9-6-8	-135	-135	—	BACK	VERT	TOTAL
O	11-5-12	-135	-135	—	BACK	VERT	TOTAL
P	13-5-12	-135	-135	—	BACK	VERT	TOTAL
Q	15-5-12	-146	-146	—	BACK	VERT	TOTAL
R	1-0-12	-40	-70	—	BACK	VERT	TOTAL
S	2-1-8	-526	-526	—	BACK	VERT	TOTAL
T	9-6-8	-526	-526	—	BACK	VERT	TOTAL
U	11-5-12	-40	-70	—	BACK	VERT	TOTAL
V	13-5-12	-40	-70	—	BACK	VERT	TOTAL
W	15-5-12	-43	-75	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.61/1.00 (F-G:1) , BC=0.69/1.00 (H-J:1) , WB=0.87/1.00 (F-H:1) , SSI=0.43/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.72 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17203-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T40	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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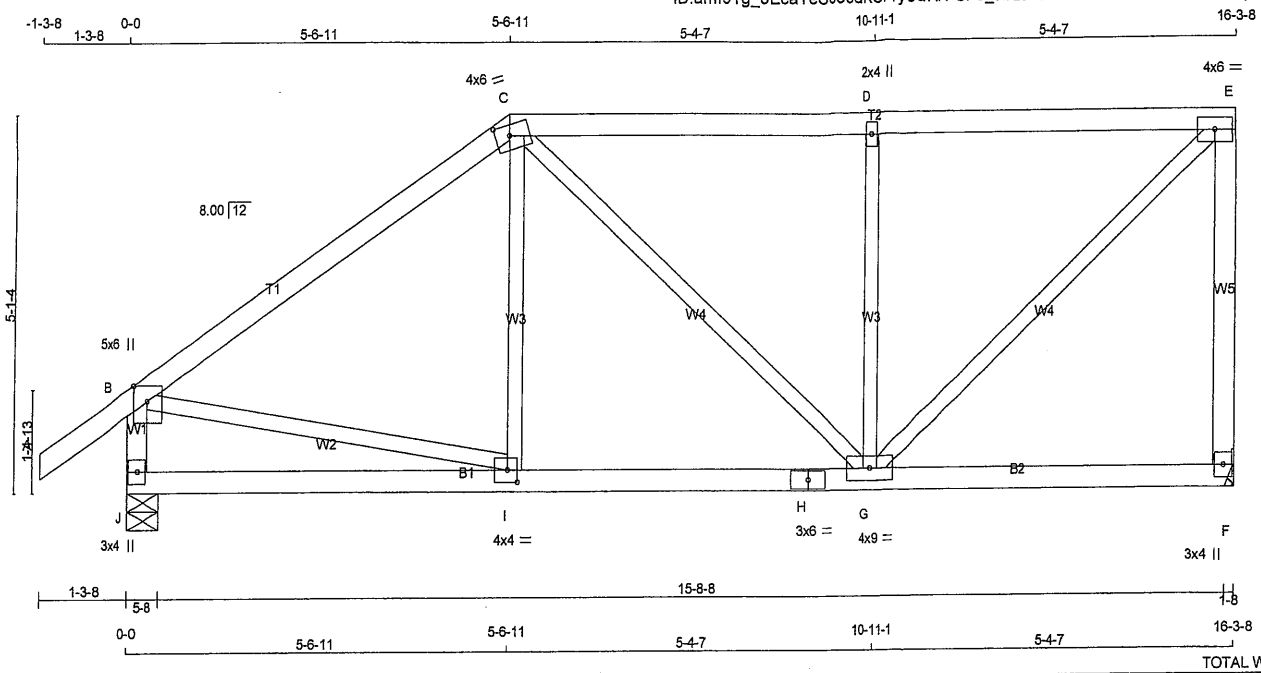

TOTAL WEIGHT = 69 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1144	0	1144	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
J	1298	0	1298	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	912	529 / 0	171 / 0	0 / 0	0 / 0	212 / 0	0 / 0
J	1018	618 / 0	171 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1072 / 0	-112.4	-112.4	0.67 (1)	5.02
C- D	-953 / 0	-112.4	-112.4	0.57 (1)	5.42
D- E	-954 / 0	-112.4	-112.4	0.57 (1)	5.42
F- E	-1080 / 0	0.0	0.0	0.48 (1)	7.60
J- B	-1233 / 0	0.0	0.0	0.13 (1)	7.23
J- I	0 / 0	-28.0	-28.0	0.21 (3)	10.00
I- H	0 / 891	-28.0	-28.0	0.30 (2)	10.00
H- G	0 / 891	-28.0	-28.0	0.30 (2)	10.00
G- F	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	0 / 213	0.05 (3)
C- G	-3 / 84	0.02 (1)
G- D	-746 / 0	0.29 (1)
G- E	0 / 1276	0.29 (1)
B- I	0 / 907	0.20 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.30/1.00 (G-I:2) , WB=0.29/1.00 (E-G:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 0.5 Deg.

JSI GRIP= 0.85 (I) (INPUT = 0.90)
JSI METAL= 0.33 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17204-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T41	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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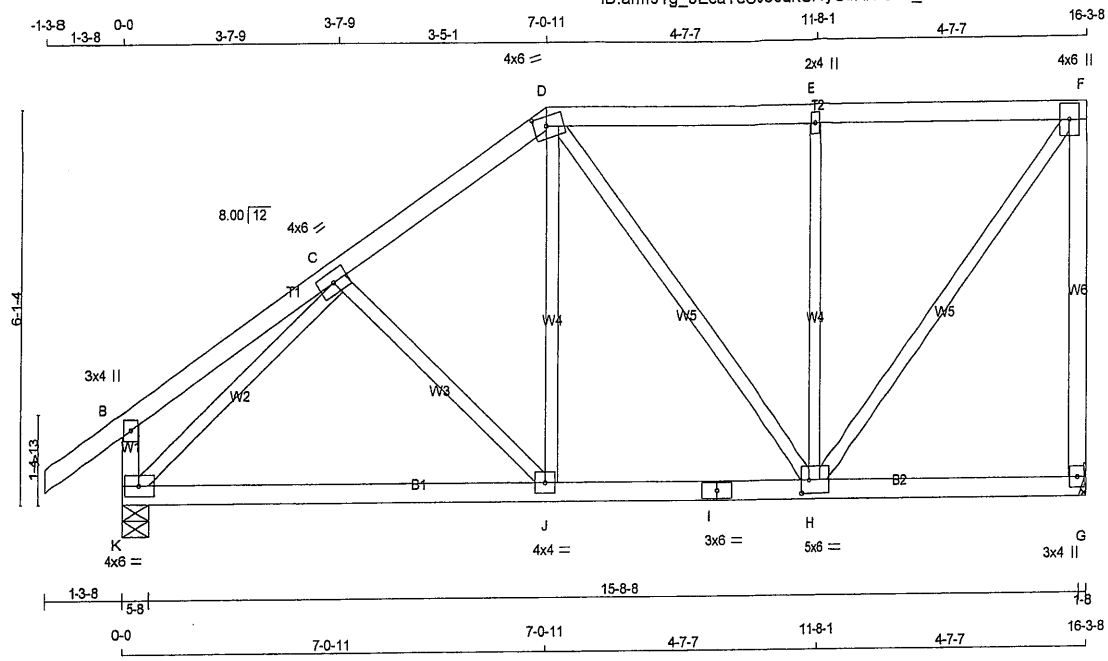

TOTAL WEIGHT = 76 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1144	0	1144	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
K	1298	0	1298	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	912	529 / 0	171 / 0	0 / 0	0 / 0	212 / 0	0 / 0
K	1018	618 / 0	171 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 25	-112.4	-112.4	0.22 (1)	10.00
C- D	-995 / 0	-112.4	-112.4	0.23 (1)	5.96
D- E	-709 / 0	-112.4	-112.4	0.41 (1)	6.25
E- F	-710 / 0	-112.4	-112.4	0.41 (1)	6.25
G- F	-1086 / 0	0.0	0.0	0.78 (1)	7.58
K- B	-308 / 0	0.0	0.0	0.03 (1)	7.81
K- J	0 / 932	-28.0	-28.0	0.39 (2)	10.00
J- I	0 / 812	-28.0	-28.0	0.39 (2)	10.00
I- H	0 / 812	-28.0	-28.0	0.39 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-173 / 41	0.07 (1)
J- D	0 / 363	0.08 (2)
D- H	-165 / 0	0.17 (1)
H- E	-641 / 0	0.38 (1)
H- F	0 / 1134	0.26 (1)
K- C	-1301 / 0	0.50 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.78/1.00 (F-G:1) , BC=0.39/1.00 (J-K:2) , WB=0.50/1.00 (C-K:1) , SSI=0.25/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.31 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4378
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 172057B
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T42	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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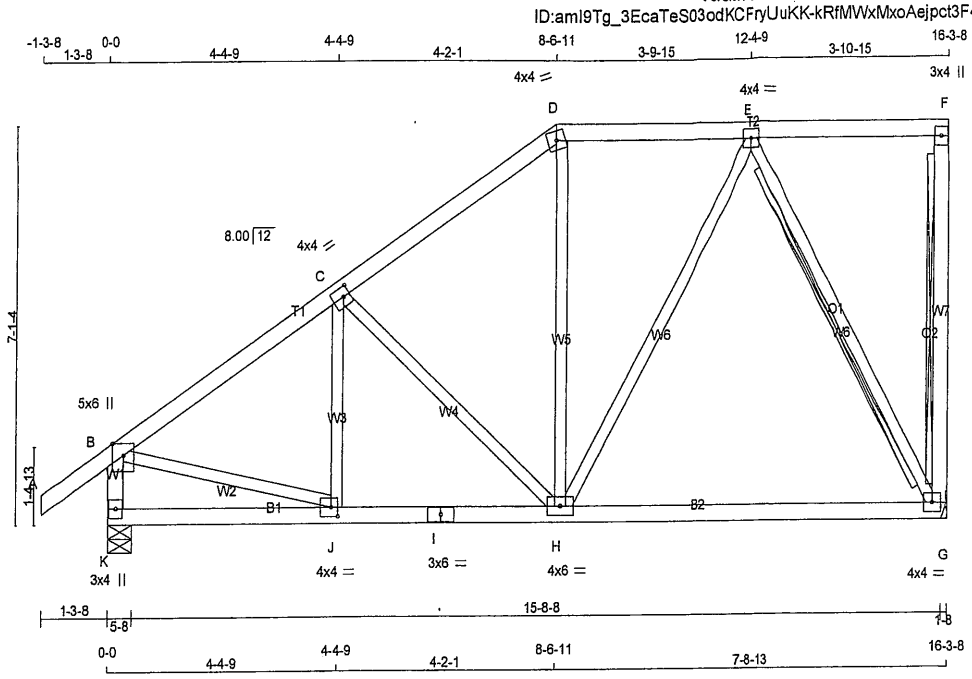

TOTAL WEIGHT = 78 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	1144	0	1144	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
K	1298	0	1298	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	912	529 / 0	171 / 0	0 / 0	0 / 0	212 / 0	0 / 0
K	1018	618 / 0	171 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-G, E-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1148 / 0	-112.4	-112.4	0.28 (1)	5.62
C- D	-875 / 0	-112.4	-112.4	0.27 (1)	6.22
D- E	-701 / 0	-112.4	-112.4	0.22 (1)	6.25
E- F	0 / 0	-112.4	-112.4	0.28 (1)	10.00
G- F	-168 / 0	0.0	0.0	0.09 (1)	7.81
K- B	-1241 / 0	0.0	0.0	0.13 (1)	7.21
K- J	0 / 0	-28.0	-28.0	0.14 (3)	10.00
J- I	0 / 981	-28.0	-28.0	0.46 (2)	10.00
I- H	0 / 981	-28.0	-28.0	0.46 (2)	10.00
H- G	0 / 510	-28.0	-28.0	0.40 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-147 / 65	0.04 (1)
C- H	-386 / 0	0.23 (1)
H- D	0 / 191	0.04 (3)
B- J	0 / 1011	0.23 (1)
H- E	0 / 400	0.09 (1)
E- G	-1024 / 0	0.99 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 883 (0.22")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.46/1.00 (H-J:2) , WB=0.99/1.00 (E-G:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.37 (J) (INPUT = 1.00)

DWG NO. TAM 17206-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T43	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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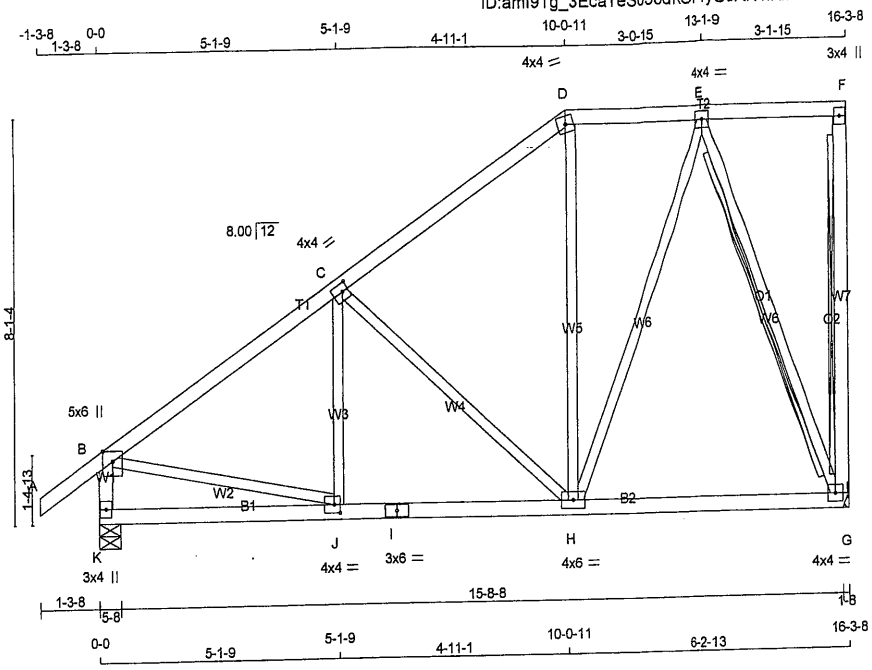

TOTAL WEIGHT = 83 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.50
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1144	0	1144	0	0	IN-SX HANGER BY OTHERS MIN. SEAT SIZE: 1-8	IN-SX
K	1298	0	1298	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	912	529 / 0	171 / 0	0 / 0	0 / 0	212 / 0	0 / 0
K	1018	618 / 0	171 / 0	0 / 0	0 / 0	228 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-G
2x4 DRY SPF No.2 T-BRACE AT E-G

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-1143 / 0	-112.4	-112.4	0.38 (1)	5.47
C- D	-726 / 0	-112.4	-112.4	0.37 (1)	6.25
D- E	-571 / 0	-112.4	-112.4	0.14 (1)	6.25
E- F	0 / 0	-112.4	-112.4	0.18 (1)	10.00
G- F	-137 / 0	0.0	0.0	0.11 (1)	7.81
K- B	-1237 / 0	0.0	0.0	0.13 (1)	7.22
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00
J- I	0 / 982	-28.0	-28.0	0.34 (2)	10.00
I- H	0 / 982	-28.0	-28.0	0.34 (2)	10.00
H- G	0 / 379	-28.0	-28.0	0.27 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-62 / 166	0.04 (3)
C- H	-552 / 0	0.46 (1)
H- D	0 / 137	0.03 (3)
B- J	0 / 1004	0.23 (1)
H- E	0 / 541	0.12 (1)
E- G	-1011 / 0	0.54 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.34/1.00 (H-J:2) , WB=0.54/1.00 (E-G:1) , SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.84 (J) (INPUT = 0.90)
USI METAL= 0.37 (J) (INPUT = 1.00)

DWG NO. TAM 17207-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T44	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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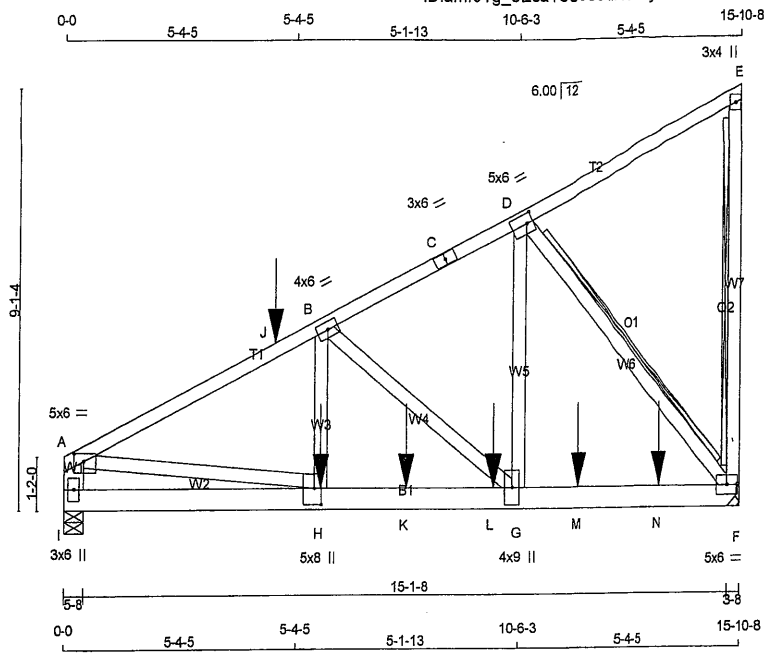

TOTAL WEIGHT = 3 X 91 = 272 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
I- A	2	12	TOP
A- C	1	11	SIDE(172.2)
C- E	1	12	TOP
E- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- F	2	9	SIDE(366.5)
WEBS : (0.122"X3") SPIRAL NAILS			
B- H	2	4	SIDE(623.3)
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. *REMAINING PLF MUST BE APPLIED ON THE OPPOSITE* SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.00	2.75
B	TMWW-t	MT20	4.0	6.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	6.0	2.50	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	5.0	6.0		
G	BMWW+t	MT20	4.0	9.0		
H	BMWW+t	MT20	5.0	8.0	4.25	2.00
I	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 660.1 lbs FACTORED DOWN AT 4-11-4 ON TOP CHORD, AND 2679.8 lbs FACTORED DOWN AT 5-11-4, 1115.9 lbs FACTORED DOWN AT 7-11-4, 1115.9 lbs FACTORED DOWN AT 9-11-4, AND 1115.9 lbs FACTORED DOWN AT 11-11-4, AND 1115.9 lbs FACTORED DOWN AT 13-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	5832	0	5832	0	0	5-8	5-8
F	6595	0	6595	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	4613	2655 / 0	796 / 0	0 / 0	0 / 0	1162 / 0	0 / 0
F	5212	3012 / 0	898 / 0	0 / 0	0 / 0	1303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F, D-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- A	-5367 / 0	0.0	0.0	0.11 (1)	7.45	
A- J	-8755 / 0	-112.4	-112.4	0.51 (1)	3.73	
J- B	-8755 / 0	-112.4	-112.4	0.51 (1)	3.73	
B- C	-5214 / 0	-112.4	-112.4	0.32 (1)	4.72	
C- D	-5214 / 0	-112.4	-112.4	0.32 (1)	4.72	
D- E	-18 / 0	-112.4	-112.4	0.14 (1)	6.25	
F- E	-262 / 0	0.0	0.0	0.02 (1)	7.81	
I- H	0 / 0	-178.8	-178.8	0.13 (1)	10.00	
H- K	0 / 7955	-178.8	-178.8	0.75 (1)	10.00	
K- L	0 / 7955	-178.8	-178.8	0.75 (1)	10.00	
L- G	0 / 7955	-178.8	-178.8	0.75 (1)	10.00	
G- M	0 / 4634	-178.8	-178.8	0.62 (1)	10.00	
M- N	0 / 4634	-178.8	-178.8	0.62 (1)	10.00	
N- F	0 / 4634	-178.8	-178.8	0.62 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- H	0 / 8022	0.43 (1)
H- B	0 / 3071	0.16 (1)
B- G	-4291 / 0	0.59 (1)
G- D	0 / 7005	0.38 (1)
D- F	-6975 / 0	0.54 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	5-11-4	-2680	-2680	—	FRONT	VERT	TOTAL
J	4-11-4	-660	-660	—	BACK	VERT	TOTAL
K	7-11-4	-1116	-1116	—	FRONT	VERT	TOTAL
L	9-11-4	-1116	-1116	—	FRONT	VERT	TOTAL
M	11-11-4	-1116	-1116	—	FRONT	VERT	TOTAL
N	13-11-4	-1116	-1116	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
43318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 15-10-8
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 3-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED *ROOF LIVE LOAD*

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.51/1.00 (A-B:1) , BC=0.75/1.00 (G-H:1) , WB=0.59/1.00 (B-G:1) , SSI=0.45/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 172088

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T44	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.69 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17 208-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T45X	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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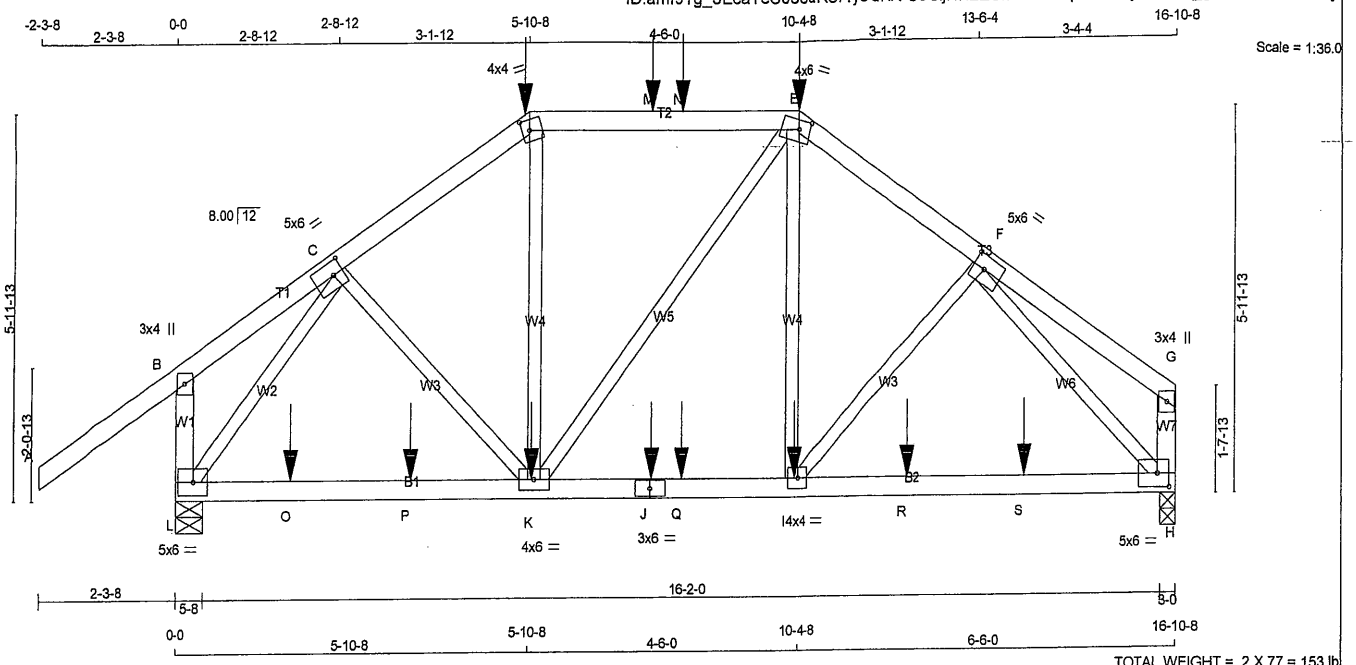

TOTAL WEIGHT = 2 X 77 = 153 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTW-m	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	4.0	6.0	1.75	2.50
F	TMWW-t	MT20	5.0	6.0	2.50	2.25
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0	2.50	2.25
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 541.1 lbs FACTORED DOWN AT 5-10-8, 135.4 lbs FACTORED DOWN AT 7-11-4, AND 135.4 lbs FACTORED DOWN AT 8-5-4, AND 605.5 lbs FACTORED DOWN AT 10-4-8 ON TOP CHORD, AND 91.1 lbs FACTORED DOWN AT 1-11-4, 75.9 lbs FACTORED DOWN AT 3-11-4, 75.9 lbs FACTORED DOWN AT 5-11-4, 75.9 lbs FACTORED DOWN AT 7-11-4, 75.9 lbs FACTORED DOWN AT 8-5-4, 75.9 lbs FACTORED DOWN AT 10-3-12, AND 75.9 lbs FACTORED DOWN AT 12-3-12, AND 91.1 lbs FACTORED DOWN AT 14-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2396	0	2396	0	0	5-8	5-8
H	2044	0	2044	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1888	1129 / 0	329 / 0	0 / 0	0 / 0	430 / 0	0 / 0
H	1640	933 / 0	318 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 74	-112.4	-112.4	0.49 (1)	10.00
B- C	-61 / 0	-112.4	-112.4	0.47 (1)	6.25
C- D	-2042 / 0	-112.4	-112.4	0.27 (1)	4.44
D- M	-1691 / 0	-112.4	-112.4	0.93 (1)	3.08
M- N	-1691 / 0	-112.4	-112.4	0.93 (1)	3.08
N- E	-1691 / 0	-112.4	-112.4	0.93 (1)	3.08
E- F	-2175 / 0	-112.4	-112.4	0.26 (1)	4.35
F- G	0 / 20	-112.4	-112.4	0.18 (1)	10.00
L- B	-523 / 0	0.0	0.0	0.07 (1)	7.81
H- G	-146 / 0	0.0	0.0	0.02 (1)	7.81
L- O	0 / 1315	-28.0	-28.0	0.58 (2)	10.00
O- P	0 / 1315	-28.0	-28.0	0.58 (2)	10.00
P- K	0 / 1315	-28.0	-28.0	0.58 (2)	10.00
K- J	0 / 1793	-28.0	-28.0	0.68 (2)	10.00
J- Q	0 / 1793	-28.0	-28.0	0.68 (2)	10.00
Q- I	0 / 1793	-28.0	-28.0	0.68 (2)	10.00
I- R	0 / 1653	-28.0	-28.0	0.70 (2)	10.00
R- S	0 / 1653	-28.0	-28.0	0.70 (2)	10.00
S- H	0 / 1653	-28.0	-28.0	0.70 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- K	0 / 542	0.13 (1)
K- D	0 / 352	0.09 (3)
K- E	-166 / 0	0.16 (1)
I- E	0 / 427	0.11 (3)
I- F	0 / 221	0.05 (2)
L- C	-2193 / 0	0.70 (1)
F- H	-2429 / 0	0.91 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	5-10-8	-541	-541	---	BACK	VERT	TOTAL
E	10-4-8	-605	-605	---	BACK	VERT	TOTAL
I	10-3-12	-43	-76	---	BACK	VERT	TOTAL
J	7-11-4	-43	-76	---	BACK	VERT	TOTAL
K	5-11-4	-43	-76	---	BACK	VERT	TOTAL
M	7-11-4	-135	-135	---	BACK	VERT	TOTAL
N	8-5-4	-135	-135	---	BACK	VERT	TOTAL
O	1-11-4	-63	-91	---	BACK	VERT	TOTAL
P	3-11-4	-43	-76	---	BACK	VERT	TOTAL
Q	8-5-4	-43	-76	---	BACK	VERT	TOTAL
R	12-3-12	-43	-76	---	BACK	VERT	TOTAL
S	14-3-12	-63	-91	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 836 (0.24")

CSI: TC=0.93/1.00 (D-E:1) , BC=0.70/1.00 (H-I:2) , WB=0.91/1.00 (F-H:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.60 (F) (INPUT = 1.00)

DWG NO. TAM 17209-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T46X	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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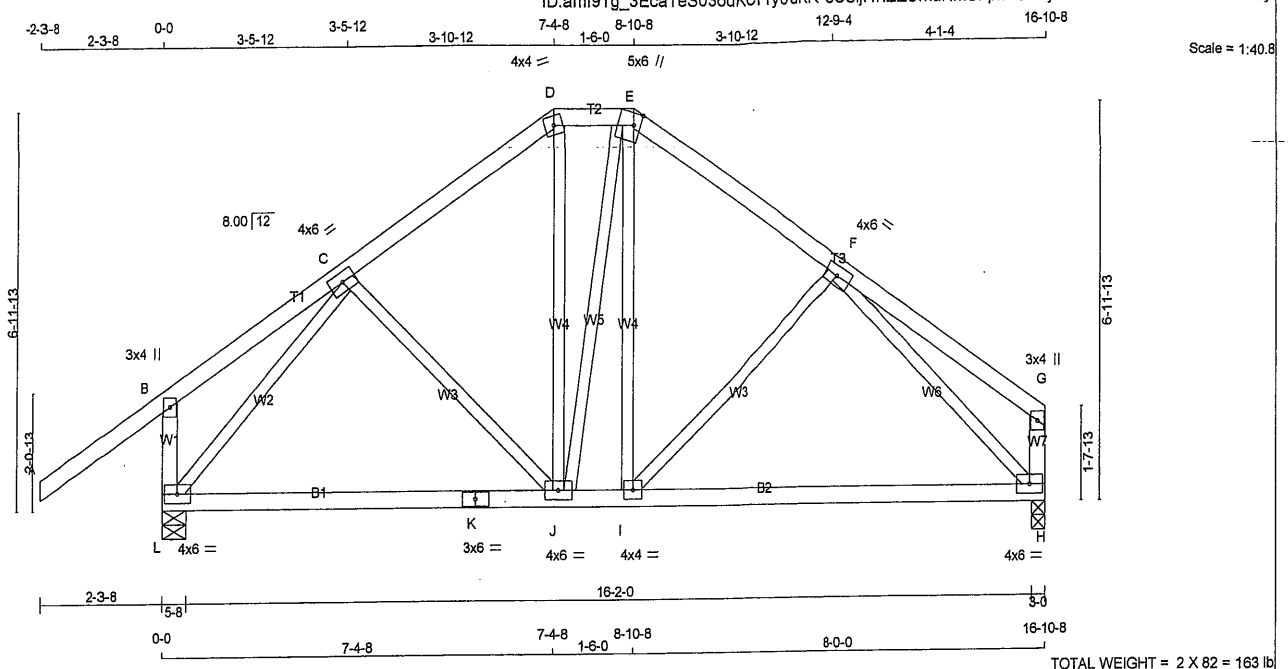

TOTAL WEIGHT = 2 X 82 = 163 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMWW-t	MT20	4.0	6.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1452	0	1452	0	0	5-8	5-8
H	1185	0	1185	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1127	702 / 0	177 / 0	0 / 0	0 / 0	248 / 0	0 / 0
H	945	548 / 0	177 / 0	0 / 0	0 / 0	219 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 74	-112.4	-112.4	0.44 (1)	10.00
B- C	0 / 30	-112.4	-112.4	0.25 (1)	10.00
C- D	-936 / 0	-112.4	-112.4	0.21 (1)	6.16
D- E	-761 / 0	-112.4	-112.4	0.04 (1)	6.25
E- F	-954 / 0	-112.4	-112.4	0.23 (1)	6.09
F- G	0 / 29	-112.4	-112.4	0.29 (1)	10.00
L- B	-402 / 0	0.0	0.0	0.05 (1)	7.81
H- G	-173 / 0	0.0	0.0	0.02 (1)	7.81
L- K	0 / 814	-28.0	-28.0	0.45 (2)	10.00
K- J	0 / 814	-28.0	-28.0	0.45 (2)	10.00
J- I	0 / 773	-28.0	-28.0	0.53 (3)	10.00
I- H	0 / 921	-28.0	-28.0	0.52 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-80 / 82	0.04 (1)
J- D	0 / 252	0.06 (2)
J- E	-67 / 0	0.06 (2)
I- E	0 / 329	0.07 (2)
I- F	-213 / 39	0.12 (1)
L- C	-1263 / 0	0.55 (1)
F- H	-1302 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 769 (0.26")

CSI: TC=0.44/1.00 (A-B:1) , BC=0.53/1.00 (I-J:3) , WB=0.68/1.00 (F-H:1) , SSI=0.20/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.31 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4310
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17210-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296384	T47	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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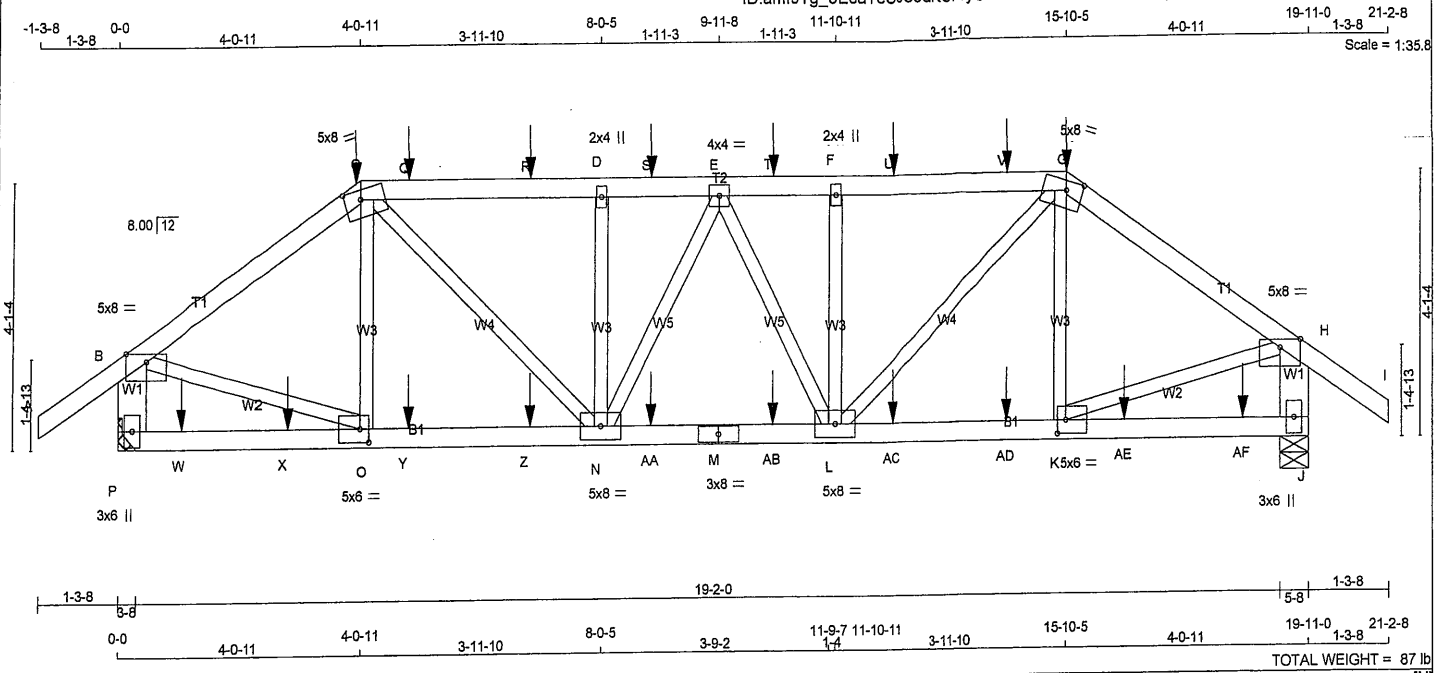

TOTAL WEIGHT = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW-m	MT20	5.0	8.0	1.75	3.25
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.25
H	TMVW-p	MT20	5.0	8.0	Edge	
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0	2.50	1.75
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWW-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	1.75
P	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) *REQUIRED TO SUPPORT CONCENTRATED* LOAD(S) 327.0 lbs FACTORED DOWN AT 4-0-11, 327.0 lbs FACTORED DOWN AT 15-10-6, 146.6 lbs FACTORED DOWN AT 4-10-4, 135.3 lbs FACTORED DOWN AT 6-10-4, 135.3 lbs FACTORED DOWN AT 8-10-4, 135.3 lbs FACTORED DOWN AT 10-10-4, AND 135.3 lbs FACTORED DOWN AT 12-10-4, AND 135.3 lbs FACTORED DOWN AT 14-10-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 2-10-4, 69.9 lbs FACTORED DOWN AT 4-10-4, 69.9 lbs FACTORED DOWN AT 6-10-4, 69.9 lbs FACTORED DOWN AT 8-10-4, 69.9 lbs FACTORED DOWN AT 10-10-4, 69.9 lbs FACTORED DOWN AT 12-10-4, 69.9 lbs FACTORED DOWN AT 14-10-4, AND 69.9 lbs FACTORED DOWN AT 16-10-4, AND 69.9 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2500	0	2500	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
J	2482	0	2482	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1983	1166 / 0	360 / 0	0 / 0	0 / 0	457 / 0	0 / 0
J	1969	1158 / 0	358 / 0	0 / 0	0 / 0	454 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00
B- C	-2636 / 0	-112.4	-112.4	0.50 (1)	3.71
C- Q	-3148 / 0	-112.4	-112.4	0.55 (1)	3.32
Q- R	-3148 / 0	-112.4	-112.4	0.55 (1)	3.32
R- D	-3148 / 0	-112.4	-112.4	0.55 (1)	3.32
D- S	-3149 / 0	-112.4	-112.4	0.50 (1)	3.32
S- E	-3149 / 0	-112.4	-112.4	0.50 (1)	3.32
E- T	-3142 / 0	-112.4	-112.4	0.50 (1)	3.33
T- F	-3142 / 0	-112.4	-112.4	0.50 (1)	3.33
F- U	-3142 / 0	-112.4	-112.4	0.55 (1)	3.32
U- V	-3142 / 0	-112.4	-112.4	0.55 (1)	3.32
V- G	-3142 / 0	-112.4	-112.4	0.55 (1)	3.32
G- H	-2615 / 0	-112.4	-112.4	0.50 (1)	3.73
H- I	0 / 43	-112.4	-112.4	0.17 (1)	10.00
P- B	-2418 / 0	0.0	0.0	0.18 (1)	6.59
J- H	-2403 / 0	0.0	0.0	0.17 (1)	6.61
P- W	0 / 0	-28.0	-28.0	0.23 (3)	10.00
W- X	0 / 0	-28.0	-28.0	0.23 (3)	10.00
X- O	0 / 0	-28.0	-28.0	0.23 (3)	10.00
O- Y	0 / 2180	-28.0	-28.0	0.50 (1)	10.00
Y- Z	0 / 2180	-28.0	-28.0	0.50 (1)	10.00
Z- N	0 / 2180	-28.0	-28.0	0.50 (1)	10.00
N-AA	0 / 3164	-28.0	-28.0	0.64 (1)	10.00
AA- M	0 / 3164	-28.0	-28.0	0.64 (1)	10.00
M-AB	0 / 3164	-28.0	-28.0	0.64 (1)	10.00
AB- L	0 / 3164	-28.0	-28.0	0.64 (1)	10.00
L-AC	0 / 2163	-28.0	-28.0	0.50 (1)	10.00
AC-AD	0 / 2163	-28.0	-28.0	0.50 (1)	10.00
AD- K	0 / 2163	-28.0	-28.0	0.50 (1)	10.00
K-AE	0 / 0	-28.0	-28.0	0.23 (3)	10.00
AE-AF	0 / 0	-28.0	-28.0	0.23 (3)	10.00
AF- J	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- C	-344 / 135	0.09 (1)
C- N	0 / 1340	0.33 (1)
N- D	-722 / 0	0.19 (1)
L- F	-719 / 0	0.19 (1)
L- G	0 / 1355	0.34 (1)
K- G	-342 / 131	0.09 (1)
B- O	0 / 2262	0.56 (1)
K- H	0 / 2245	0.56 (1)
N- E	-34 / 0	0.01 (1)
E- L	-48 / 0	0.02 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-45	-50	—	FRONT	VERT	DEAD
C	4-0-11	-282	-282	—	FRONT	VERT	SNOW
G	15-10-6	-45	-50	—	FRONT	VERT	DEAD

LICENSED PROFESSIONAL ENGINEER
4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.55/1.00 (F-G:1) , BC=0.64/1.00 (L-N:1) , WB=0.56/1.00 (B-O:1) , SSI=0.36/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.90 (M) (INPUT = 1.00)

DWG NO. TAM 17194-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296384	T47	1	1	TRUSS DESC.	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	15-10-6	-282	-282	—	FRONT	VERT	SNOW
Q	4-10-4	-147	-147	—	FRONT	VERT	TOTAL
R	6-10-4	-135	-135	—	FRONT	VERT	TOTAL
S	8-10-4	-135	-135	—	FRONT	VERT	TOTAL
T	10-10-4	-135	-135	—	FRONT	VERT	TOTAL
U	12-10-4	-135	-135	—	FRONT	VERT	TOTAL
V	14-10-4	-135	-135	—	FRONT	VERT	TOTAL
W	1-0-12	-40	-70	—	FRONT	VERT	TOTAL
X	2-10-4	-40	-70	—	FRONT	VERT	TOTAL
Y	4-10-4	-40	-70	—	FRONT	VERT	TOTAL
Z	6-10-4	-40	-70	—	FRONT	VERT	TOTAL
AA	8-10-4	-40	-70	—	FRONT	VERT	TOTAL
AB	10-10-4	-40	-70	—	FRONT	VERT	TOTAL
AC	12-10-4	-40	-70	—	FRONT	VERT	TOTAL
AD	14-10-4	-40	-70	—	FRONT	VERT	TOTAL
AE	16-10-4	-40	-70	—	FRONT	VERT	TOTAL
AF	18-10-4	-40	-70	—	FRONT	VERT	TOTAL

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4318
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DWG NO. TAM 17194-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T47S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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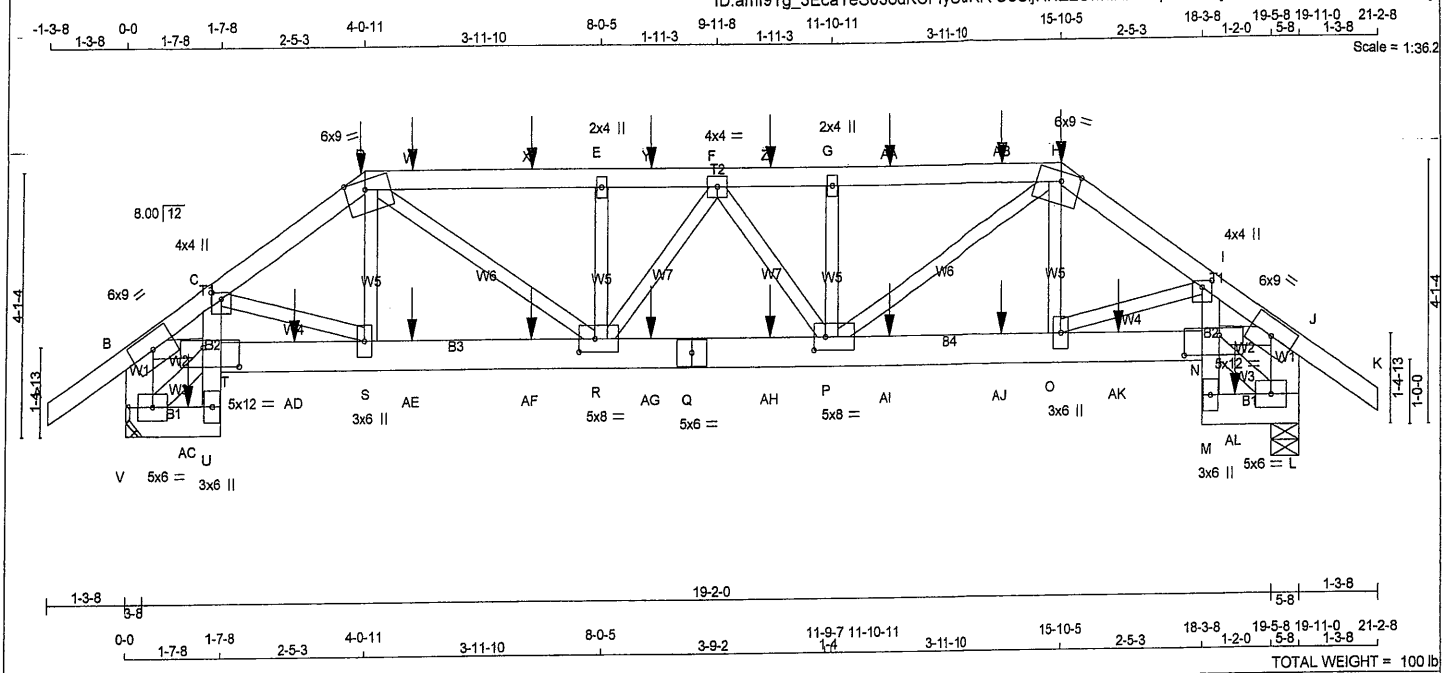

TOTAL WEIGHT = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
T - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
V - T	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0		
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	6.0	9.0	1.75	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	6.0	9.0	1.75	4.00
I	TMVW+p	MT20	4.0	4.0	1.25	2.00
J	TMVW-t	MT20	6.0	9.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-l	MT20	5.0	12.0	3.50	7.25
O	BMWW+t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.25
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0	2.50	3.25
S	BMWW+t	MT20	3.0	6.0		
T	BVMWW-l	MT20	5.0	12.0	3.50	7.25
U	BMV+p	MT20	3.0	6.0		
V	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2774	0	2774	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
L	2639	0	2639	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	2226	1278 / 0	438 / 0	0 / 0	0 / 0	511 / 0	0 / 0
L	2208	1155 / 0	546 / 0	0 / 0	0 / 0	507 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)		10.00
B- C	-4525 / 0	-112.4	-112.4	0.41 (1)		2.91
C- D	-4055 / 0	-112.4	-112.4	0.42 (1)		3.09
D- W	-4851 / 0	-112.4	-112.4	0.78 (1)		2.36
W- X	-4851 / 0	-112.4	-112.4	0.78 (1)		2.36
X- E	-4851 / 0	-112.4	-112.4	0.78 (1)		2.36
E- Y	-4851 / 0	-112.4	-112.4	0.64 (1)		2.45
Y- F	-4851 / 0	-112.4	-112.4	0.64 (1)		2.45
F- Z	-4785 / 0	-112.4	-112.4	0.63 (1)		2.49
Z- G	-4785 / 0	-112.4	-112.4	0.63 (1)		2.49
G-AA	-4785 / 0	-112.4	-112.4	0.78 (1)		2.38
AA-AB	-4785 / 0	-112.4	-112.4	0.78 (1)		2.38
AB- H	-4785 / 0	-112.4	-112.4	0.78 (1)		2.38
H- I	-3829 / 0	-112.4	-112.4	0.39 (1)		3.22
I- J	-4277 / 0	-112.4	-112.4	0.39 (1)		3.03
J- K	0 / 43	-112.4	-112.4	0.17 (1)		10.00
V- B	-2612 / 0	0.0	0.0	0.19 (1)		6.39
L- J	-2484 / 0	0.0	0.0	0.18 (1)		6.52
V-AC	0 / 174	-28.0	-28.0	0.04 (2)		10.00
AC- U	0 / 174	-28.0	-28.0	0.04 (2)		10.00
U- T	0 / 97	0.0	0.0	0.29 (1)		10.00
T- C	0 / 318	0.0	0.0	0.34 (1)		10.00
T-AD	0 / 3921	-28.0	-28.0	0.59 (1)		10.00
AD- S	0 / 3921	-28.0	-28.0	0.59 (1)		10.00
S-AE	0 / 3394	-28.0	-28.0	0.55 (1)		10.00
AE-AF	0 / 3394	-28.0	-28.0	0.55 (1)		10.00
AF- R	0 / 3394	-28.0	-28.0	0.55 (1)		10.00
R-AG	0 / 4851	-28.0	-28.0	0.77 (1)		10.00
AG- Q	0 / 4851	-28.0	-28.0	0.77 (1)		10.00
Q-AH	0 / 4851	-28.0	-28.0	0.77 (1)		10.00
AH- P	0 / 4851	-28.0	-28.0	0.77 (1)		10.00
P-AI	0 / 3205	-28.0	-28.0	0.52 (1)		10.00
AI-AJ	0 / 3205	-28.0	-28.0	0.52 (1)		10.00
AJ- O	0 / 3205	-28.0	-28.0	0.52 (1)		10.00
O-AK	0 / 3705	-28.0	-28.0	0.57 (1)		10.00
AK- N	0 / 3705	-28.0	-28.0	0.57 (1)		10.00
M- N	0 / 97	0.0	0.0	0.28 (1)		10.00
N- I	0 / 320	0.0	0.0	0.32 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
C- S	-579 / 0	0.11 (1)
S- D	0 / 465	0.11 (2)
D- R	0 / 1764	0.44 (1)
R- E	-705 / 0	0.13 (1)
P- G	-701 / 0	0.13 (1)
P- H	0 / 1914	0.47 (1)
O- H	0 / 452	0.11 (2)
O- I	-550 / 0	0.11 (1)
V- T	-216 / 0	0.02 (1)
B- T	0 / 3717	0.66 (1)
N- L	-204 / 0	0.02 (1)
N- J	0 / 3513	0.62 (1)
R- F	-91 / 0	0.02 (3)
F- P	-115 / 48	0.03 (1)

LICENSED PROFESSIONAL ENGINEER
4310
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DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 796 (0.30")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.77/1.00 (P-R:1) , WB=0.66/1.00 (B-T:1) , SSI=0.36/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 172-11 -18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 4-0-11, 327.0 lbs FACTORED DOWN AT 15-10-6, 148.8 lbs FACTORED DOWN AT 4-10-4, 137.6 lbs FACTORED DOWN AT 6-10-4, 137.6 lbs FACTORED DOWN AT 8-10-4, 137.6 lbs FACTORED DOWN AT 10-10-4, AND 137.6 lbs FACTORED DOWN AT 12-10-4, AND 137.6 lbs FACTORED DOWN AT 14-10-4 ON TOP CHORD, AND 84.2 lbs FACTORED DOWN AT 1-0-12, 113.5 lbs FACTORED DOWN AT 2-10-4, 113.5 lbs FACTORED DOWN AT 4-10-4, 113.5 lbs FACTORED DOWN AT 6-10-4, 113.5 lbs FACTORED DOWN AT 8-10-4, 113.5 lbs FACTORED DOWN AT 10-10-4, 113.5 lbs FACTORED DOWN AT 12-10-4, 113.5 lbs FACTORED DOWN AT 14-10-4, AND 113.5 lbs FACTORED DOWN AT 16-10-4, AND 84.2 lbs FACTORED DOWN AT 18-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
M-AL	0 / 165	-28.0	-28.0	0.04 (2)	10.00
AL- L	0 / 165	-28.0	-28.0	0.04 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-0-11	-45	-50	—	FRONT	VERT	DEAD
D	4-0-11	-282	-282	—	FRONT	VERT	SNOW
H	15-10-6	-45	-50	—	FRONT	VERT	DEAD
H	15-10-6	-94	-282	—	FRONT	VERT	LIVE
W	4-10-4	-149	-149	—	FRONT	VERT	TOTAL
X	6-10-4	-138	-138	—	FRONT	VERT	TOTAL
Y	8-10-4	-138	-138	—	FRONT	VERT	TOTAL
Z	10-10-4	-138	-138	—	FRONT	VERT	TOTAL
AA	12-10-4	-138	-138	—	FRONT	VERT	TOTAL
AB	14-10-4	-138	-138	—	FRONT	VERT	TOTAL
AC	1-0-12	-48	-84	—	FRONT	VERT	TOTAL
AD	2-10-4	-114	-114	—	FRONT	VERT	TOTAL
AE	4-10-4	-114	-114	—	FRONT	VERT	TOTAL
AF	6-10-4	-114	-114	—	FRONT	VERT	TOTAL
AG	8-10-4	-114	-114	—	FRONT	VERT	TOTAL
AH	10-10-4	-114	-114	—	FRONT	VERT	TOTAL
AI	12-10-4	-114	-114	—	FRONT	VERT	TOTAL
AJ	14-10-4	-114	-114	—	FRONT	VERT	TOTAL
AK	16-10-4	-114	-114	—	FRONT	VERT	TOTAL
AL	18-10-4	-48	-84	—	FRONT	VERT	TOTAL

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.88 (Q) (INPUT = 1.00)

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4320
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DWG NO. TAM 17211-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287652	T48	1	2	OBC2012_6TCDL TRUSS DESC.	

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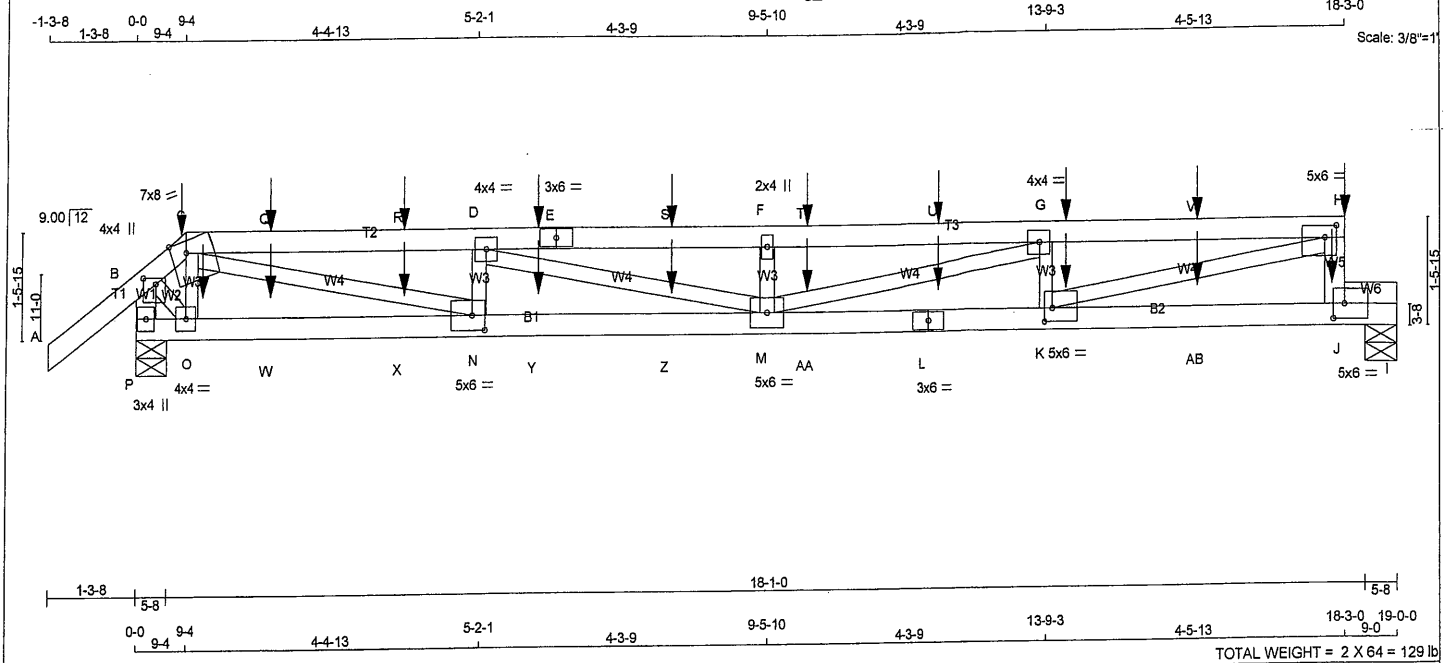

TOTAL WEIGHT = 2 X 64 = 129 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- F	1	12	SIDE(61.0)
E- H	1	12	SIDE(61.0)
H- J	1	4	SIDE(39.5)
P- B	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- L	1	12	SIDE(61.0)
L- I	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
O- C	1	4	SIDE(28.7)
K- G	1	4	SIDE(27.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.25
C	TTWW-m	MT20	7.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMVW-t	MT20	5.0	6.0	2.00	2.00
J	BMVW-t	MT20	5.0	6.0	2.50	2.00
K	BMWW-t	MT20	5.0	6.0	2.25	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.25
O	BMWW-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	2047	0	2047	0	0	5-8	5-8
I	1797	0	1797	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1614	963 / 0	281 / 0	0 / 0	0 / 0	370 / 0	0 / 0
I	1438	826 / 0	276 / 0	0 / 0	0 / 0	337 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 47	-112.4	-112.4	0.08 (1)	10.00	
B- C	-1759 / 0	-112.4	-112.4	0.09 (1)	6.25	
C- Q	-5816 / 0	-112.4	-112.4	0.35 (1)	3.75	
Q- R	-5816 / 0	-112.4	-112.4	0.35 (1)	3.75	
R- D	-5816 / 0	-112.4	-112.4	0.35 (1)	3.75	
D- E	-7233 / 0	-112.4	-112.4	0.38 (1)	3.38	
E- S	-7233 / 0	-112.4	-112.4	0.38 (1)	3.38	
S- F	-7233 / 0	-112.4	-112.4	0.38 (1)	3.38	
F- T	-7233 / 0	-112.4	-112.4	0.39 (1)	3.37	
T- U	-7233 / 0	-112.4	-112.4	0.39 (1)	3.37	
U- G	-7233 / 0	-112.4	-112.4	0.39 (1)	3.37	
G- V	-6147 / 0	-112.4	-112.4	0.39 (1)	3.61	
V- H	-6147 / 0	-112.4	-112.4	0.39 (1)	3.61	
J- H	-2006 / 0	0.0	0.0	0.11 (1)	7.78	
P- B	-2257 / 0	0.0	0.0	0.12 (1)	7.44	
P- O	0 / 0	-28.0	-28.0	0.15 (1)	10.00	
O- W	0 / 1284	-28.0	-28.0	0.26 (1)	10.00	
W- X	0 / 1284	-28.0	-28.0	0.26 (1)	10.00	
X- N	0 / 1284	-28.0	-28.0	0.26 (1)	10.00	
N- Y	0 / 5816	-28.0	-28.0	0.67 (1)	10.00	
Y- Z	0 / 5816	-28.0	-28.0	0.67 (1)	10.00	
Z- M	0 / 5816	-28.0	-28.0	0.67 (1)	10.00	
M-AA	0 / 6147	-28.0	-28.0	0.77 (1)	10.00	
AA- L	0 / 6147	-28.0	-28.0	0.77 (1)	10.00	
L- K	0 / 6147	-28.0	-28.0	0.77 (1)	10.00	
K-AB	0 / 0	-28.0	-28.0	1.00 (1)	10.00	
AB- J	0 / 0	-28.0	-28.0	1.00 (1)	10.00	
J- I	0 / 0	-28.0	-28.0	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
O- C	-452 / 0	0.04 (1)
K- H	0 / 6364	0.79 (1)
C- N	0 / 4694	0.58 (1)
K- G	-961 / 0	0.07 (1)
N- D	-1040 / 0	0.08 (1)
M- G	0 / 1128	0.14 (1)
D- M	0 / 1471	0.18 (1)
M- F	-585 / 0	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	9-4	-63	-63	---	BACK	VERT	TOTAL
E	6-0-12	-58	-58	---	BACK	VERT	TOTAL
G	14-0-12	-58	-58	---	BACK	VERT	TOTAL
H	18-3-0	-103	-103	---	BACK	VERT	TOTAL
J	18-0-12	-49	-49	---	BACK	VERT	TOTAL
K	14-0-12	-48	-48	---	BACK	VERT	TOTAL
L	12-0-12	-48	-48	---	BACK	VERT	TOTAL
O	1-0-12	-48	-48	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 670 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 410 (0.56")

CSI: TC=0.39/1.00 (G-H:1) , BC=1.00/1.00 (J-K:1) , WB=0.79/1.00 (H-K:1) , SSI=0.39/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.77 (H) (INPUT = 1.00)

DWG NO. TAM 172248
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	T48	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 63.0 lbs FACTORED DOWN AT 9-4, 57.7 lbs FACTORED DOWN AT 2-0-12, 57.7 lbs FACTORED DOWN AT 4-0-12, 57.7 lbs FACTORED DOWN AT 6-0-12, 57.7 lbs FACTORED DOWN AT 8-0-12, 57.7 lbs FACTORED DOWN AT 10-0-12, 57.7 lbs FACTORED DOWN AT 12-0-12, 57.7 lbs FACTORED DOWN AT 14-0-12, AND 57.7 lbs FACTORED DOWN AT 16-0-12, AND 103.3 lbs FACTORED DOWN AT 18-3-0 ON TOP CHORD, AND 47.6 lbs FACTORED DOWN AT 1-0-12, 47.6 lbs FACTORED DOWN AT 2-0-12, 47.6 lbs FACTORED DOWN AT 4-0-12, 47.6 lbs FACTORED DOWN AT 6-0-12, 47.6 lbs FACTORED DOWN AT 8-0-12, 47.6 lbs FACTORED DOWN AT 10-0-12, 47.6 lbs FACTORED DOWN AT 12-0-12, 47.6 lbs FACTORED DOWN AT 14-0-12, AND 47.6 lbs FACTORED DOWN AT 16-0-12, AND 48.5 lbs FACTORED DOWN AT 18-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Q	2-0-12	-58	-58	—	BACK	VERT	TOTAL
R	4-0-12	-58	-58	—	BACK	VERT	TOTAL
S	8-0-12	-58	-58	—	BACK	VERT	TOTAL
T	10-0-12	-58	-58	—	BACK	VERT	TOTAL
U	12-0-12	-58	-58	—	BACK	VERT	TOTAL
V	16-0-12	-58	-58	—	BACK	VERT	TOTAL
W	2-0-12	-48	-48	—	BACK	VERT	TOTAL
X	4-0-12	-48	-48	—	BACK	VERT	TOTAL
Y	6-0-12	-48	-48	—	BACK	VERT	TOTAL
Z	8-0-12	-48	-48	—	BACK	VERT	TOTAL
AA	10-0-12	-48	-48	—	BACK	VERT	TOTAL
AB	16-0-12	-48	-48	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 172128
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T100S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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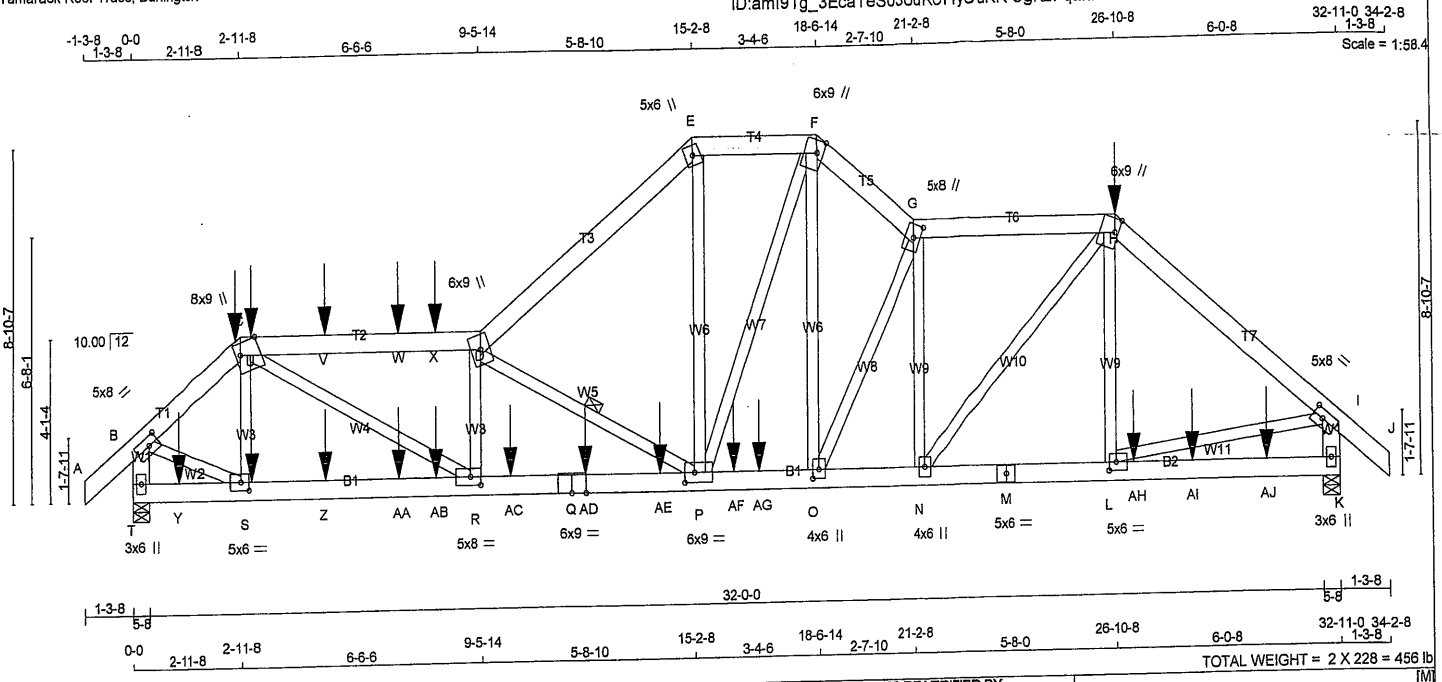

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - F	2x6	DRY	No.2	SPF
F - G	2x6	DRY	No.2	SPF
G - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
M - K	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- D	2	12	SIDE(122.0)
D- E	2	12	TOP
E- F	2	12	TOP
F- G	2	12	TOP
G- H	2	12	SIDE(61.0)
H- J	2	12	SIDE(122.0)
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	SIDE(197.8)
Q- M	2	12	SIDE(0.0)
M- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TTWW+m	MT20	6.0	9.0		
E	TTW+m	MT20	5.0	6.0		
F	TTWW+m	MT20	6.0	9.0	Edge	2.00
G	TTWW+m	MT20	5.0	8.0	4.00	2.25
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMVW-t	MT20	5.0	8.0	2.50	3.25
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	5310	0	5310	0	0	5-8	5-8
K	5459	0	5459	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	4169	2522 / 0	709 / 0	0 / 0	0 / 0	939 / 0	0 / 0
K	4280	2599 / 0	720 / 0	0 / 0	0 / 0	960 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) T, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-5439 / 0	-112.4	-112.4	0.08 (1)	4.98
C- U	-10356 / 0	-112.4	-112.4	0.59 (1)	3.27
U- V	-10356 / 0	-112.4	-112.4	0.59 (1)	3.27
V- W	-10356 / 0	-112.4	-112.4	0.59 (1)	3.27
W- X	-10356 / 0	-112.4	-112.4	0.59 (1)	3.27
X- D	-10356 / 0	-112.4	-112.4	0.59 (1)	3.27
D- E	-7506 / 0	-112.4	-112.4	0.29 (1)	4.19
E- F	-5841 / 0	-112.4	-112.4	0.10 (1)	4.82
F- G	-7110 / 0	-112.4	-112.4	0.10 (1)	4.45
G- H	-6631 / 0	-112.4	-112.4	0.24 (1)	4.44
H- I	-6119 / 0	-112.4	-112.4	0.28 (1)	4.57
I- J	0 / 52	-112.4	-112.4	0.05 (1)	10.00
T- B	-5326 / 0	0.0	0.0	0.19 (1)	6.34
K- I	-5277 / 0	0.0	0.0	0.19 (1)	6.36
T- Y	0 / 0	-28.0	-28.0	0.06 (2)	10.00
Y- S	0 / 0	-28.0	-28.0	0.06 (2)	10.00
S- Z	0 / 4109	-28.0	-28.0	0.16 (1)	10.00
Z-AA	0 / 4109	-28.0	-28.0	0.16 (1)	10.00
AA-AB	0 / 4109	-28.0	-28.0	0.16 (1)	10.00
AB- R	0 / 4109	-28.0	-28.0	0.16 (1)	10.00
R-AC	0 / 10471	-28.0	-28.0	0.50 (1)	10.00
AC- Q	0 / 10471	-28.0	-28.0	0.50 (1)	10.00
Q-AD	0 / 10471	-28.0	-28.0	0.50 (1)	10.00
AD-AE	0 / 10471	-28.0	-28.0	0.50 (1)	10.00
AE- P	0 / 10471	-28.0	-28.0	0.50 (1)	10.00
P-AF	0 / 5521	-28.0	-28.0	0.37 (1)	10.00
AF-AG	0 / 5521	-28.0	-28.0	0.37 (1)	10.00
AG- O	0 / 5521	-28.0	-28.0	0.37 (1)	10.00
O- N	0 / 6685	-28.0	-28.0	0.23 (1)	10.00
N- M	0 / 4702	-28.0	-28.0	0.22 (1)	10.00
M- L	0 / 4702	-28.0	-28.0	0.22 (1)	10.00
L-AH	0 / 0	-28.0	-28.0	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-1122 / 0	0.10 (1)
C- R	0 / 7177	0.63 (1)
R- D	-2855 / 0	0.24 (1)
D- P	-5525 / 0	0.65 (1)
P- E	0 / 4295	0.38 (1)
P- F	0 / 892	0.08 (1)
O- F	0 / 3383	0.30 (1)
O- G	-2981 / 0	0.85 (1)
N- G	-2301 / 0	0.54 (1)
N- H	0 / 2895	0.26 (1)
L- H	0 / 658	0.06 (2)
B- S	0 / 4412	0.39 (1)
L- I	0 / 4763	0.42 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.59/1.00 (C-D:1) , BC=0.50/1.00 (P-R:1) , WB=0.85/1.00 (G-O:1) , SSI=0.43/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.86 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4318 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17190-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287680	T100S	1	2	OBC2012_6TCDL TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMWW+t	MT20	4.0	6.0		
O	BMWW+t	MT20	4.0	6.0	2.75	2.00
P	BMWWW-t	MT20	6.0	9.0	3.00	3.25
Q	BS-t _	MT20	6.0	9.0		
R	BMWW-t	MT20	5.0	8.0	2.50	3.25
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 241.0 lbs FACTORED DOWN AT 2-11-8, 174.0 lbs FACTORED DOWN AT 3-3-4, 135.3 lbs FACTORED DOWN AT 5-3-4, 135.3 lbs FACTORED DOWN AT 7-3-4, AND 135.3 lbs FACTORED DOWN AT 8-3-4, AND 500.1 lbs FACTORED DOWN AT 26-10-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-3-4, 69.9 lbs FACTORED DOWN AT 3-3-4, 69.9 lbs FACTORED DOWN AT 5-3-4, 69.9 lbs FACTORED DOWN AT 7-3-4, 69.9 lbs FACTORED DOWN AT 8-3-4, 443.5 lbs FACTORED DOWN AT 10-3-4, 439.0 lbs FACTORED DOWN AT 12-3-4, 439.0 lbs FACTORED DOWN AT 14-3-4, 454.7 lbs FACTORED DOWN AT 16-3-4, 1275.6 lbs FACTORED DOWN AT 16-11-8, 1144.7 lbs FACTORED DOWN AT 27-4-8, AND 92.8 lbs FACTORED DOWN AT 28-11-12, AND 92.8 lbs FACTORED DOWN AT 30-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AH-AI	0 / 0	-28.0	-28.0	0.10 (2)	10.00
AI-AJ	0 / 0	-28.0	-28.0	0.10 (2)	10.00
AJ- K	0 / 0	-28.0	-28.0	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-33	-36	---	FRONT	VERT	DEAD
C	2-11-8	-209	-209	---	FRONT	VERT	SNOW
H	26-10-8	-500	-500	---	BACK	VERT	TOTAL
S	3-3-4	-40	-70	---	BACK	VERT	TOTAL
U	3-3-4	-174	-174	---	BACK	VERT	TOTAL
V	5-3-4	-135	-135	---	BACK	VERT	TOTAL
W	7-3-4	-135	-135	---	BACK	VERT	TOTAL
X	8-3-4	-135	-135	---	BACK	VERT	TOTAL
Y	1-3-4	-40	-70	---	BACK	VERT	TOTAL
Z	5-3-4	-40	-70	---	BACK	VERT	TOTAL
AA	7-3-4	-40	-70	---	BACK	VERT	TOTAL
AB	8-3-4	-40	-70	---	BACK	VERT	TOTAL
AC	10-3-4	-444	-444	---	BACK	VERT	TOTAL
AD	12-3-4	-439	-439	---	BACK	VERT	TOTAL
AE	14-3-4	-439	-439	---	BACK	VERT	TOTAL
AF	16-3-4	-455	-455	---	BACK	VERT	TOTAL
AG	16-11-8	-1276	-1276	---	BACK	VERT	TOTAL
AH	27-4-8	-1145	-1145	---	BACK	VERT	TOTAL
AI	28-11-12	-53	-93	---	BACK	VERT	TOTAL
AJ	30-11-12	-53	-93	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM V1190-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287675	PB1	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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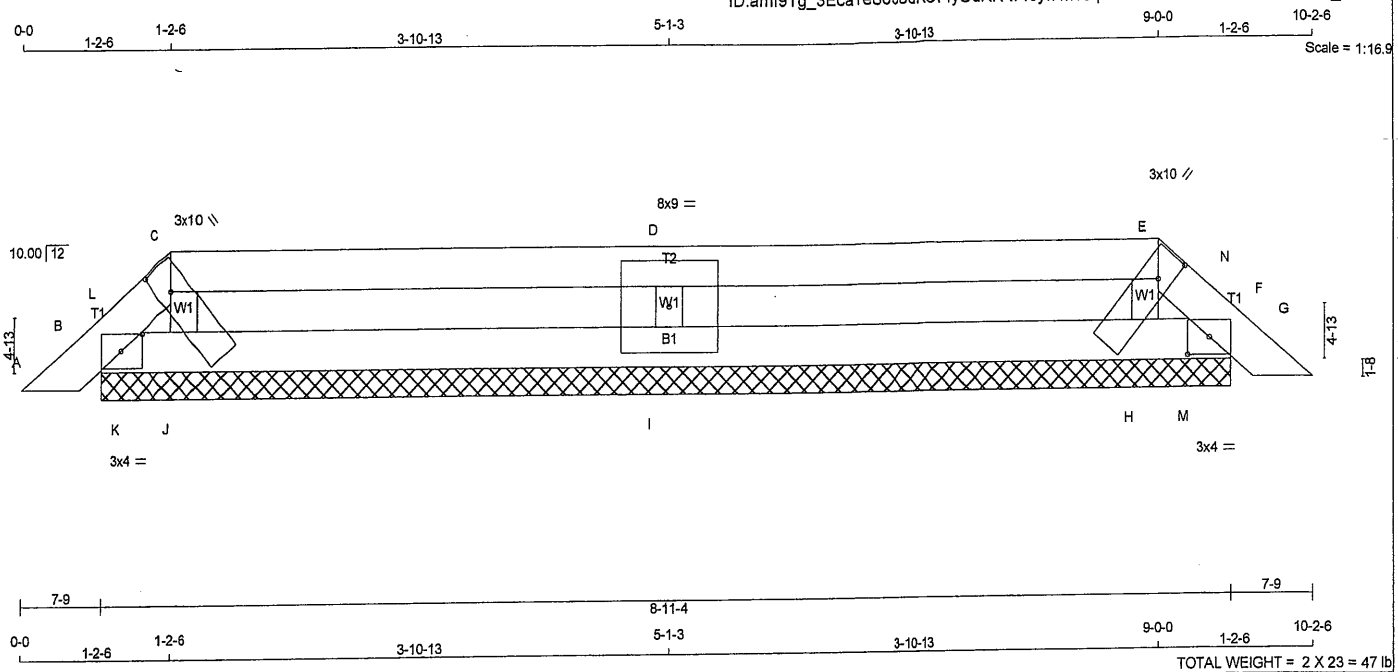

TOTAL WEIGHT = 2 X 23 = 47 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C, D, E, H, I, J						
C						
C	TTBW1*+h	MT20	3.0	10.0	2.50	1.25
D	TMBMW1-I	MT20	8.0	9.0		
E	TTBW1*+h	MT20	3.0	10.0	2.50	1.25
F	TMB1-I	MT20	3.0	4.0	1.50	2.00

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	115	0	115	0	-39	8-11-4	8-11-4
F	115	0	115	0	-39	8-11-4	8-11-4
J	244	0	244	0	0	8-11-4	8-11-4
I	658	0	658	0	0	8-11-4	8-11-4
H	244	0	244	0	0	8-11-4	8-11-4

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	60	86 / 0	0 / -25	0 / 0	0 / 0	0 / -1	0 / 0
F	60	86 / 0	0 / -25	0 / 0	0 / 0	0 / -1	0 / 0
J	225	81 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0
I	513	316 / 0	83 / 0	0 / 0	0 / 0	114 / 0	0 / 0
H	225	81 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- L	-48 / 32	-112.4	-112.4	0.02 (1)	6.25
L- C	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
C- D	-1 / 1	-112.4	-112.4	0.29 (1)	10.00
D- E	-1 / 1	-112.4	-112.4	0.29 (1)	10.00
E- N	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
N- F	-48 / 32	-112.4	-112.4	0.02 (1)	6.25
F- G	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- K	-3 / 24	-28.0	-28.0	0.01 (1)	10.00
K- J	-3 / 24	-28.0	-28.0	0.07 (3)	10.00
J- I	-1 / 1	-28.0	-28.0	0.09 (3)	10.00
I- H	-1 / 1	-28.0	-28.0	0.09 (3)	10.00
H- M	-3 / 24	-28.0	-28.0	0.07 (3)	10.00
M- F	-3 / 24	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-125 / 8	0.02 (1)
I- D	-548 / 0	0.08 (1)
H- E	-125 / 8	0.02 (1)
K- L	-79 / 0	0.00 (1)
M- N	-79 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
4318
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.09/1.00 (I-J:3) , WB=0.08/1.00 (D-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

DWG NO. TAM 17175-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	PB2	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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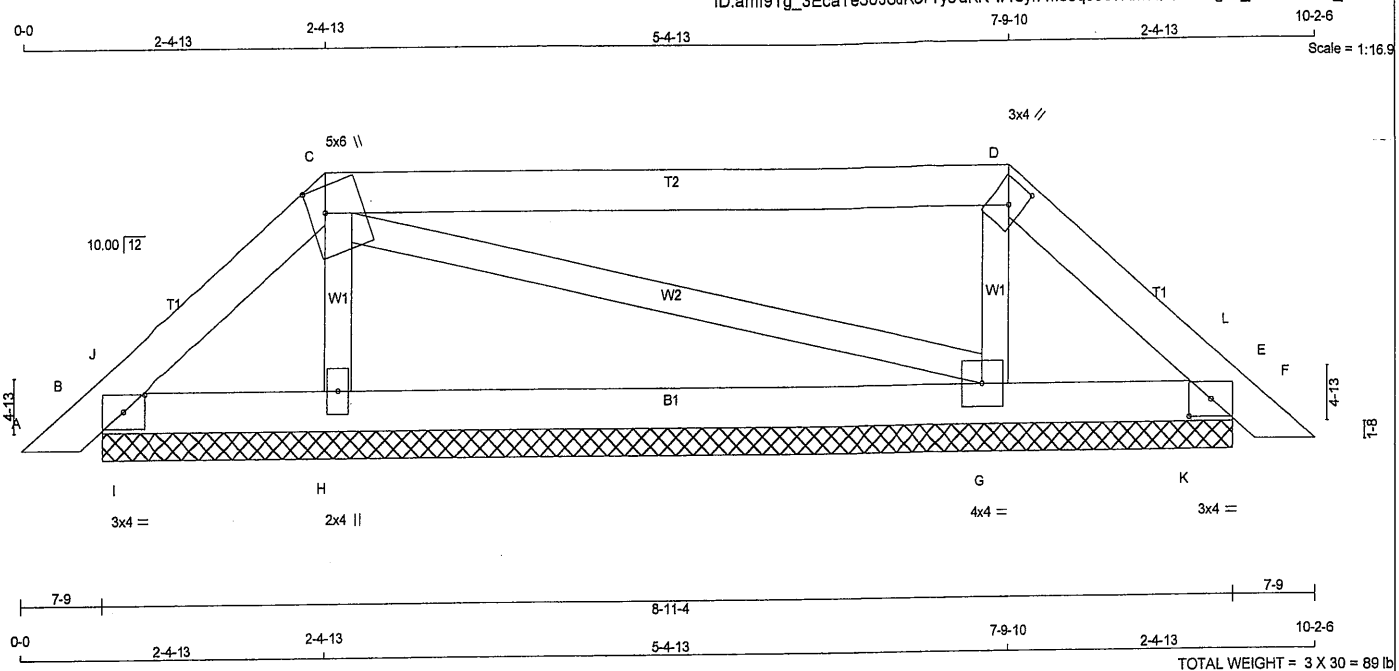

TOTAL WEIGHT = 3 X 30 = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	223	0	223	0	0	8-11-4	8-11-4
E	212	0	212	0	0	8-11-4	8-11-4
H	461	0	461	0	0	8-11-4	8-11-4
G	480	0	480	0	0	8-11-4	8-11-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	150	132 / 0	0 / -4	0 / 0	0 / 0	22 / 0	0 / 0
E	142	126 / 0	0 / -4	0 / 0	0 / 0	21 / 0	0 / 0
H	389	191 / 0	98 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	402	202 / 0	98 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- J	-60 / 35	-112.4	-112.4	0.03 (3)	6.25
J- C	-106 / 0	-112.4	-112.4	0.03 (1)	6.25
C- D	-41 / 0	-112.4	-112.4	0.56 (1)	6.25
D- L	-89 / 0	-112.4	-112.4	0.03 (1)	6.25
L- E	-42 / 37	-112.4	-112.4	0.03 (3)	6.25
E- F	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- I	0 / 75	-28.0	-28.0	0.06 (1)	10.00
I- H	0 / 75	-28.0	-28.0	0.14 (3)	10.00
H- G	0 / 55	-28.0	-28.0	0.14 (3)	10.00
G- K	0 / 63	-28.0	-28.0	0.14 (3)	10.00
K- E	0 / 63	-28.0	-28.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-311 / 0	0.05 (1)
C- G	-14 / 0	0.01 (1)
G- D	-326 / 0	0.05 (1)
I- J	-162 / 0	0.00 (1)
K- L	-164 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-D:1) , BC=0.14/1.00 (H-I:3) , WB=0.05/1.00 (D-G:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4316
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1717b-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	PB31	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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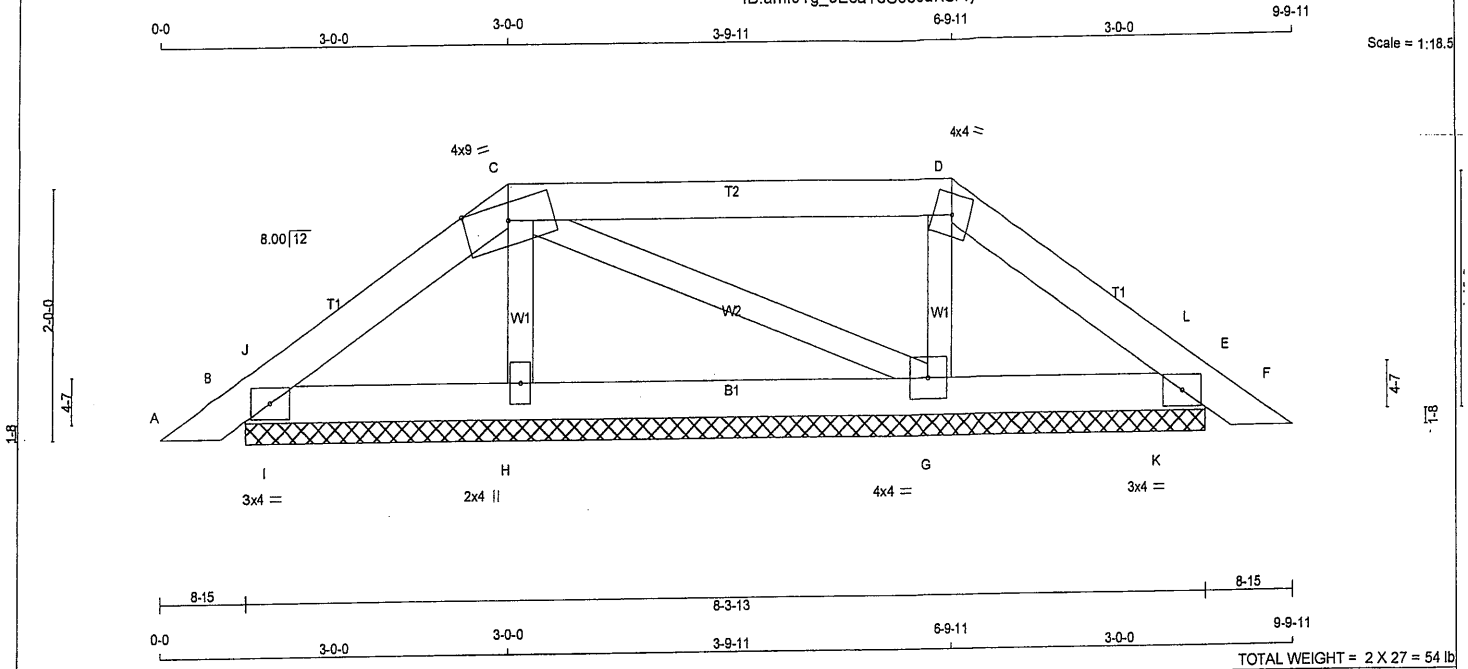


TOTAL WEIGHT = 2 X 27 = 54 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW-m	MT20	4.0	9.0	Edge	
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	273	0	273	0	0	8-3-13	8-3-13
E	260	0	260	0	0	8-3-13	8-3-13
H	377	0	377	0	0	8-3-13	8-3-13
G	407	0	407	0	0	8-3-13	8-3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	200	145 / 0	17 / 0	0 / 0	0 / 0	38 / 0	0 / 0
E	191	137 / 0	17 / 0	0 / 0	0 / 0	37 / 0	0 / 0
H	311	164 / 0	70 / 0	0 / 0	0 / 0	77 / 0	0 / 0
G	331	181 / 0	70 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

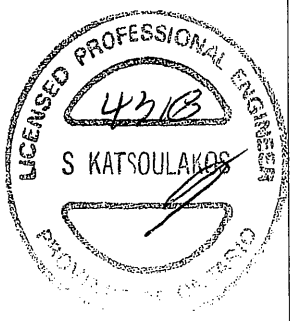
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00	
B- J	-77 / 0	-112.4	-112.4	0.01 (1)	6.25	
J- C	-94 / 0	-112.4	-112.4	0.07 (1)	6.25	
C- D	-38 / 0	-112.4	-112.4	0.28 (1)	6.25	
D- L	-71 / 0	-112.4	-112.4	0.07 (1)	6.25	
L- E	-54 / 0	-112.4	-112.4	0.01 (1)	6.25	
E- F	0 / 19	-112.4	-112.4	0.04 (1)	10.00	
B- I	0 / 75	-28.0	-28.0	0.07 (1)	10.00	
I- H	0 / 75	-28.0	-28.0	0.07 (1)	10.00	
H- G	0 / 58	-28.0	-28.0	0.07 (2)	10.00	
G- K	0 / 57	-28.0	-28.0	0.07 (2)	10.00	
K- E	0 / 57	-28.0	-28.0	0.07 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-260 / 0	0.04 (1)
C- G	-22 / 0	0.01 (1)
G- D	-281 / 0	0.04 (1)
I- J	-152 / 0	0.00 (1)
K- L	-154 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1) , BC=0.07/1.00 (B-I:1) , WB=0.04/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 17214-B8
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	PB32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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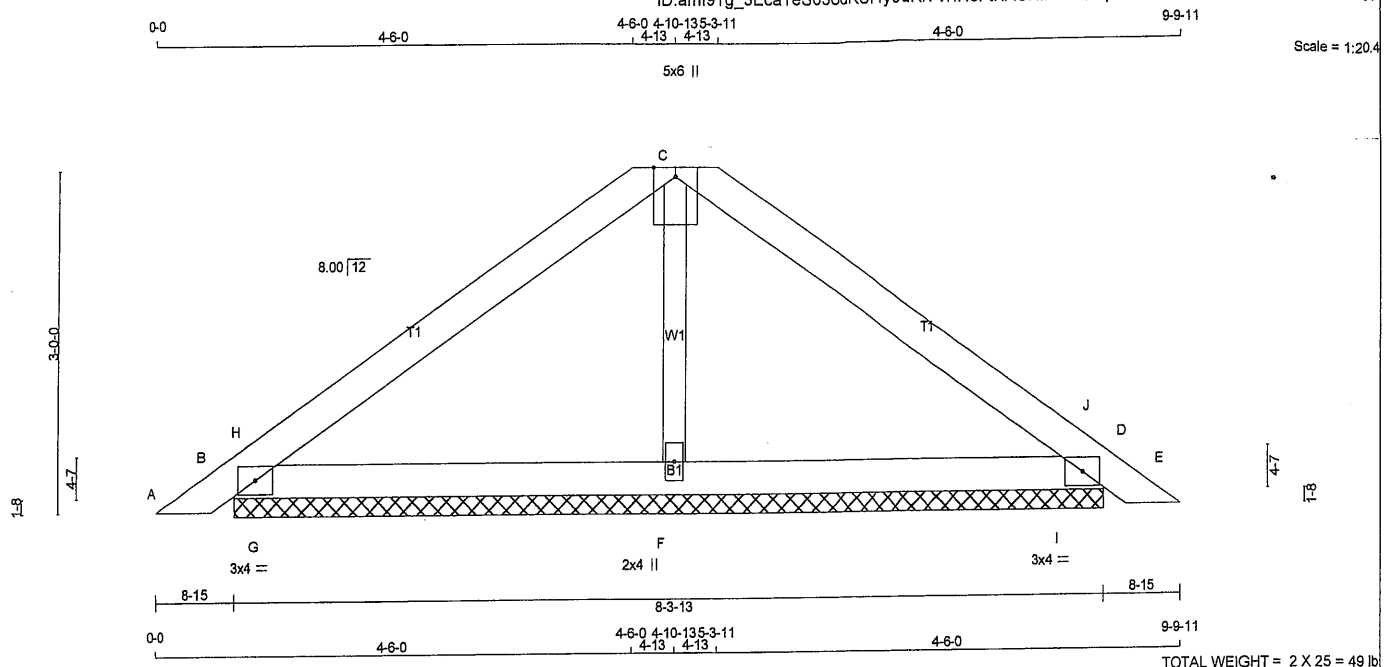

TOTAL WEIGHT = 2 X 25 = 49 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TMTMW+p	MT20	5.0	6.0	Edge	
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	437	0	437	0	0	8-3-13	8-3-13
D	437	0	437	0	0	8-3-13	8-3-13
F	444	0	444	0	0	8-3-13	8-3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	330	221 / 0	41 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	330	221 / 0	41 / 0	0 / 0	0 / 0	68 / 0	0 / 0
F	373	185 / 0	92 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- H	-87 / 0	-112.4	-112.4	0.11 (1)	6.25
H- C	-194 / 0	-112.4	-112.4	0.23 (1)	6.25
C- J	-194 / 0	-112.4	-112.4	0.23 (1)	6.25
J- D	-87 / 0	-112.4	-112.4	0.11 (1)	6.25
D- E	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- G	0 / 152	-28.0	-28.0	0.22 (1)	10.00
G- F	0 / 152	-28.0	-28.0	0.22 (1)	10.00
F- I	0 / 152	-28.0	-28.0	0.22 (1)	10.00
I- D	0 / 152	-28.0	-28.0	0.22 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-189 / 0	0.03 (1)
G- H	-498 / 30	0.00 (1)
I- J	-498 / 30	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-H:1) , BC=0.22/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.38/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4318
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17215-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	PB33	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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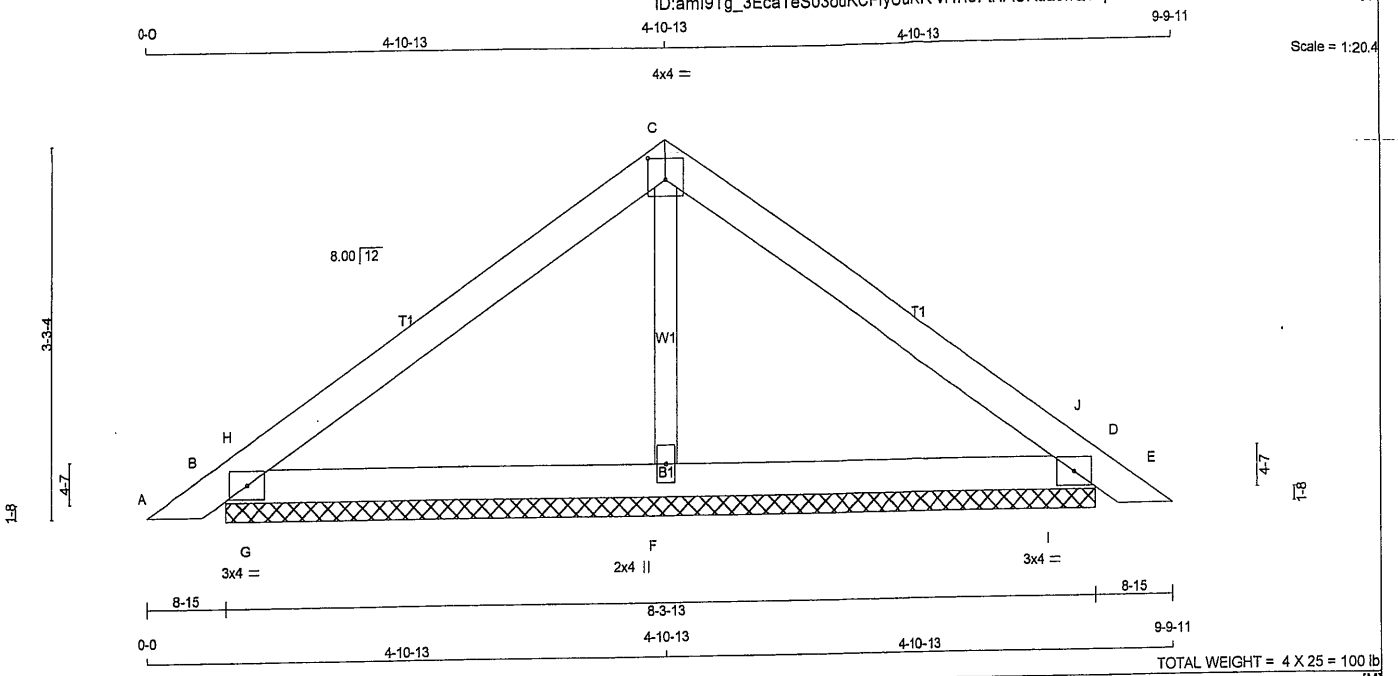

TOTAL WEIGHT = 4 X 25 = 100 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	437	0	437	0	0	8-3-13	8-3-13
D	437	0	437	0	0	8-3-13	8-3-13
F	444	0	444	0	0	8-3-13	8-3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	330	221 / 0	41 / 0	0 / 0	0 / 0	68 / 0	0 / 0
D	330	221 / 0	41 / 0	0 / 0	0 / 0	68 / 0	0 / 0
F	373	185 / 0	92 / 0	0 / 0	0 / 0	96 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00	
B- H	-87 / 0	-112.4	-112.4	0.11 (1)	6.25	
H- C	-194 / 0	-112.4	-112.4	0.23 (1)	6.25	
C- J	-194 / 0	-112.4	-112.4	0.23 (1)	6.25	
J- D	-87 / 0	-112.4	-112.4	0.11 (1)	6.25	
D- E	0 / 19	-112.4	-112.4	0.04 (1)	10.00	
B- G	0 / 152	-28.0	-28.0	0.22 (1)	10.00	
G- F	0 / 152	-28.0	-28.0	0.22 (1)	10.00	
F- I	0 / 152	-28.0	-28.0	0.22 (1)	10.00	
I- D	0 / 152	-28.0	-28.0	0.22 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-189 / 0	0.03 (1)
G- H	-498 / 30	0.00 (1)
I- J	-498 / 30	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (C-H:1) , BC=0.22/1.00 (R-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.38/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.36 (D) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 4318 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 17216-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287675	J4S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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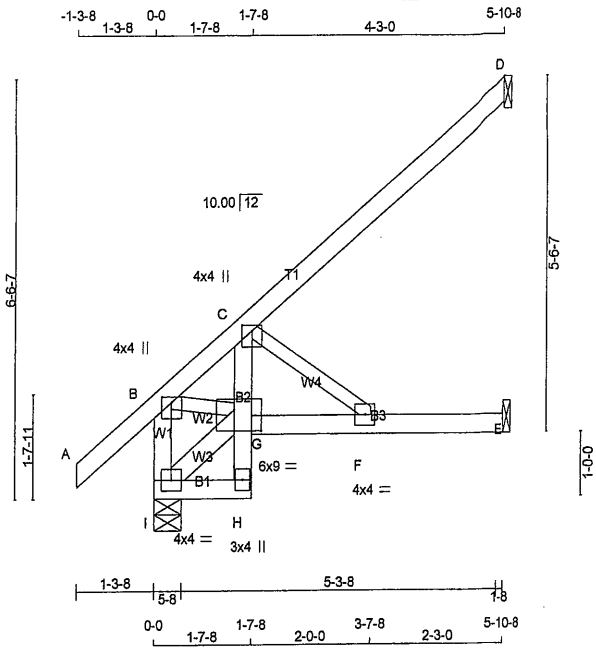

TOTAL WEIGHT = 8 X 26 = 209 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-l	MT20	6.0	9.0	4.00	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	587	0	587	0	0	5-8	5-8
D	227	0	227	0	0	1-8	1-8
E	167	0	169	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	448	291 / 0	62 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	160	127 / 0	6 / 0	0 / 0	0 / 0	27 / 0	0 / 0
E	156	54 / 0	56 / 0	0 / 0	0 / 0	47 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-555 / 0	0.0	0.0	0.06 (1)	7.81
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-436 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	-13 / 7	-112.4	-112.4	0.31 (1)	6.25
I- H	0 / 14	-28.0	-28.0	0.02 (3)	10.00
H- G	0 / 36	0.0	0.0	0.03 (1)	10.00
G- C	0 / 207	0.0	0.0	0.05 (1)	10.00
G- F	0 / 354	-28.0	-28.0	0.48 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.43 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-17 / 0	0.00 (1)
B- G	0 / 329	0.07 (1)
C- F	-425 / 0	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 839 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 488 (0.14")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.48/1.00 (F-G:1) , WB=0.07/1.00 (C-F:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17174-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287652	J13S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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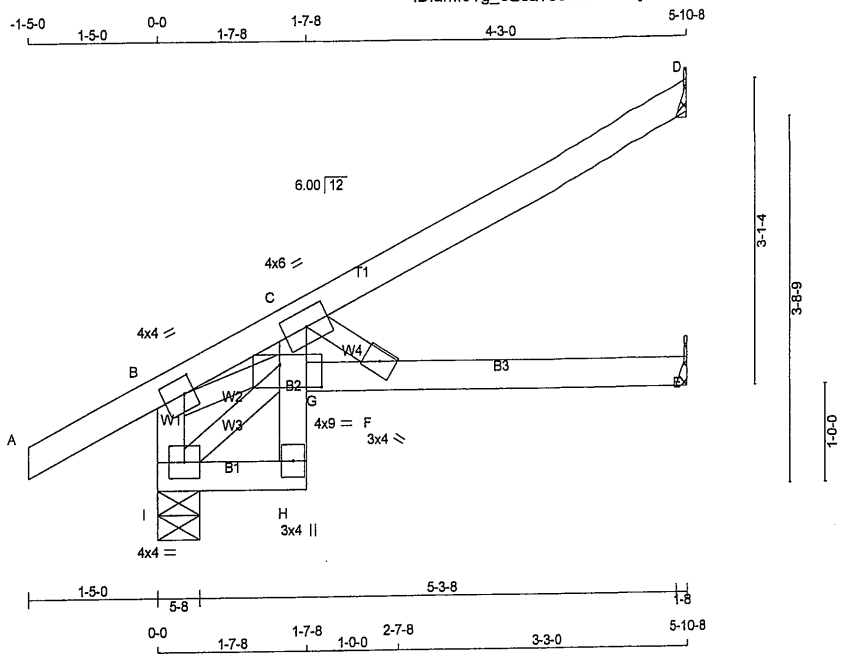


TOTAL WEIGHT = 8 X 21 = 172 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
F	BMW+w	MT20	3.0	4.0		
G	BVMWW-l	MT20	4.0	9.0	2.75	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	600	0	600	0	0	5-8	5-8
D	252	0	252	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	140	0	151	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	457	299 / 0	62 / 0	0 / 0	0 / 0	96 / 0	0 / 0
D	180	138 / 0	10 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	135	41 / 0	52 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-532 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 37	-112.4	-112.4	0.17 (1)	10.00
B- C	-669 / 0	-112.4	-112.4	0.17 (1)	6.25
C- D	0 / 10	-112.4	-112.4	0.38 (1)	10.00
I- H	0 / 63	-28.0	-28.0	0.02 (2)	10.00
H- G	0 / 36	0.0	0.0	0.10 (1)	10.00
G- C	0 / 494	0.0	0.0	0.17 (1)	10.00
G- F	0 / 743	-28.0	-28.0	0.55 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.43 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- F	-830 / 0	0.12 (1)
I- G	-78 / 0	0.01 (1)
B- G	0 / 568	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 685 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 405 (0.17")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.55/1.00 (F-G:1) , WB=0.13/1.00 (B-G:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (B) (INPUT = 0.90)
JSI METAL= 0.32 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4316
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17213-18
STRUCTURAL
COMPONENT ONLY

Site Copy

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

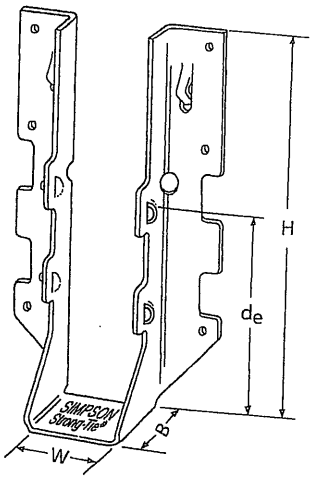
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

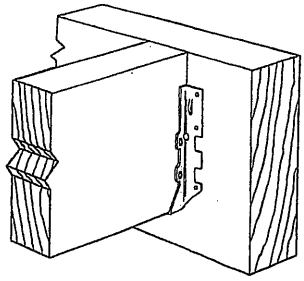
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



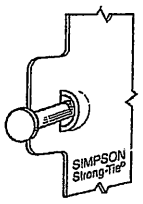
LUS28



Typical LUS Installation

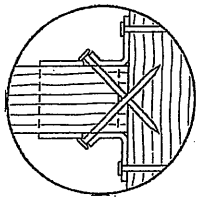
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

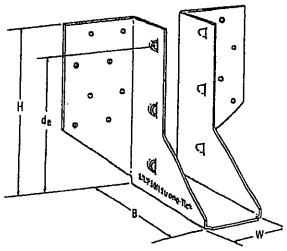
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

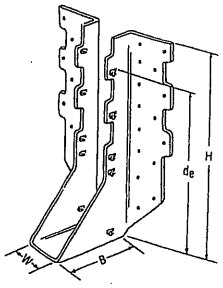
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

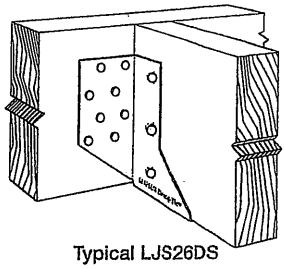
- See current catalogue for options



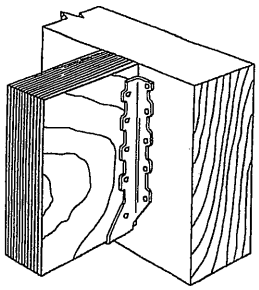
LJS26DS



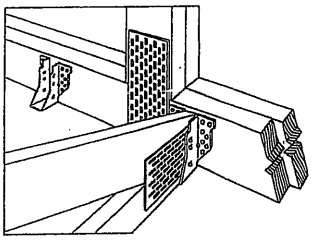
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



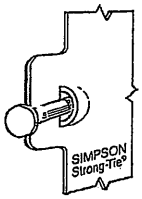
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

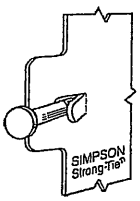
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

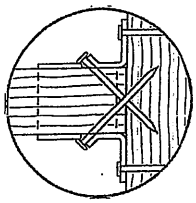


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

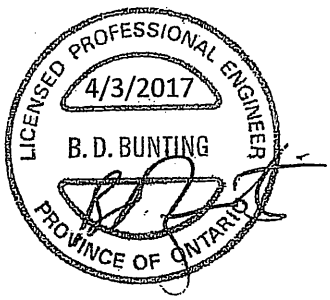
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

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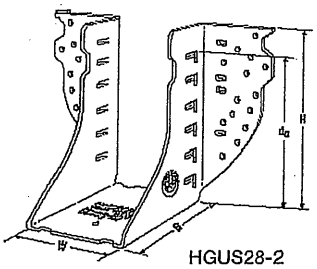
TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



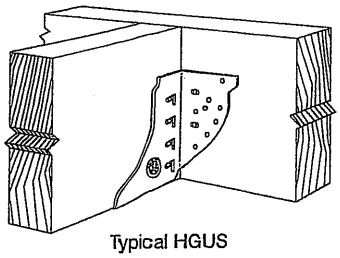
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

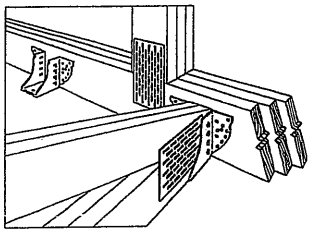

Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

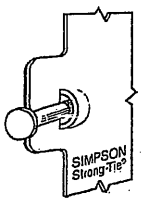
Options:

- See current catalogue for options


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

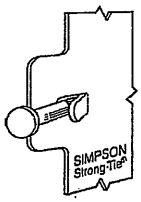
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

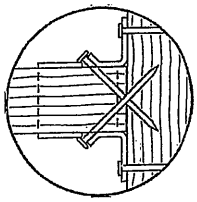


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LIMIT STATES DESIGN

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ENGINEERING SERVICES INC.

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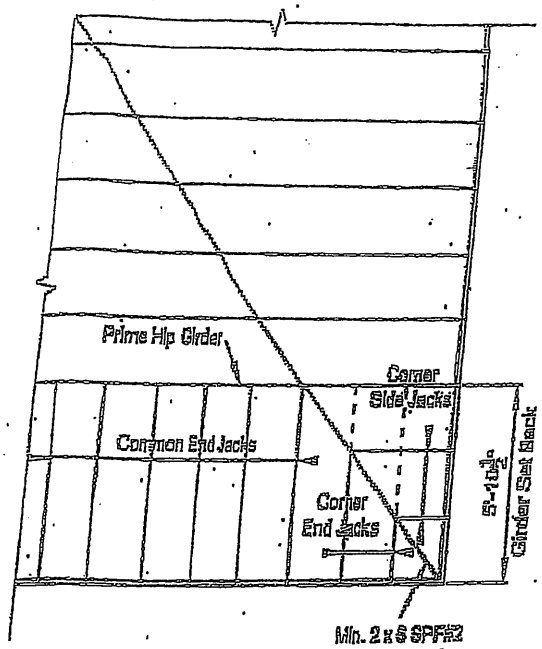


45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

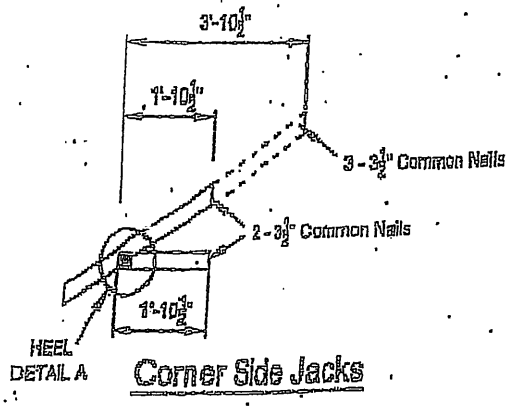
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

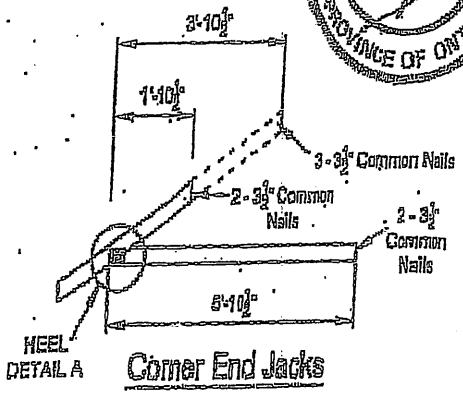
TOP CHORD LIVE LOAD	:	39.4 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F
TOTAL LOAD		59.9 P.S.F.

DWG NO T1M 2415.14
STRUCTURAL
COMPONENT ONLY

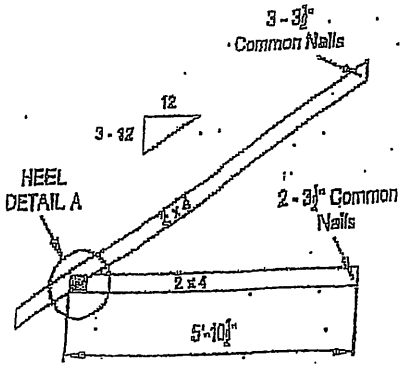
LICENSED PROFESSIONAL ENGINEER
1-2011
S. KATSOULAKOS
PROVINCE OF ONTARIO



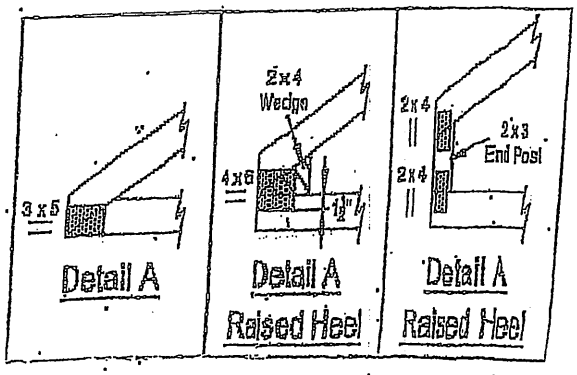
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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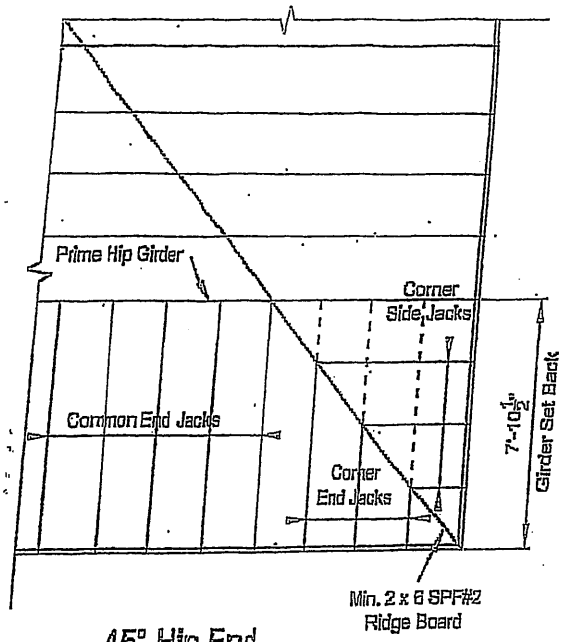
TEL: (519) 287 - 2242
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOAD:

TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

DWG NO TAM 3503 14
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1-2014
S. KATSOULAKOS
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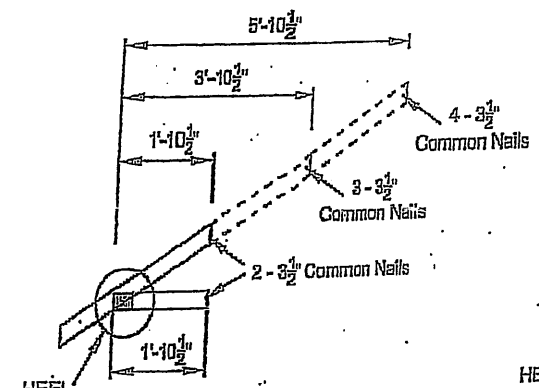
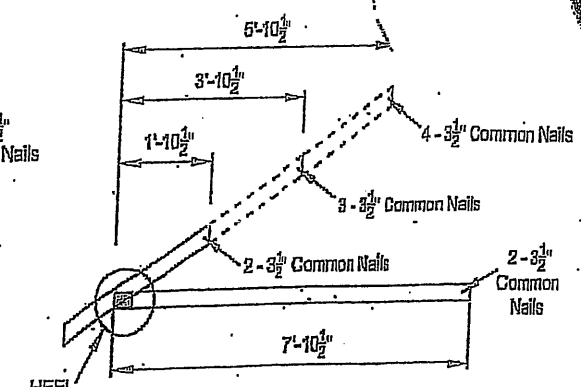
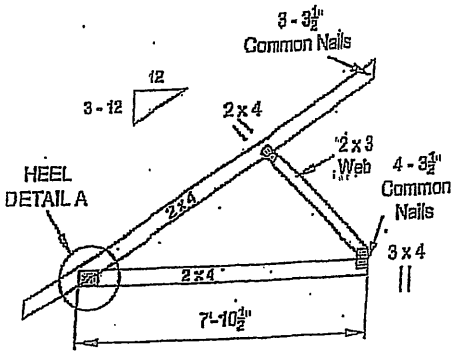


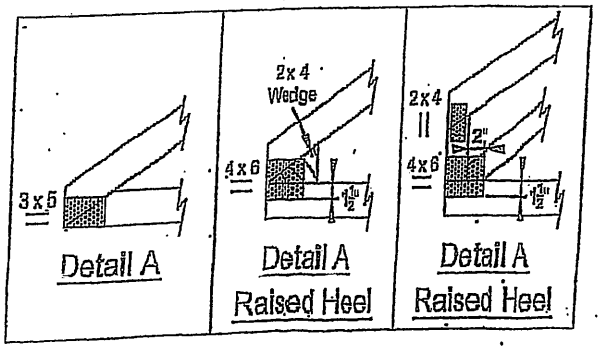
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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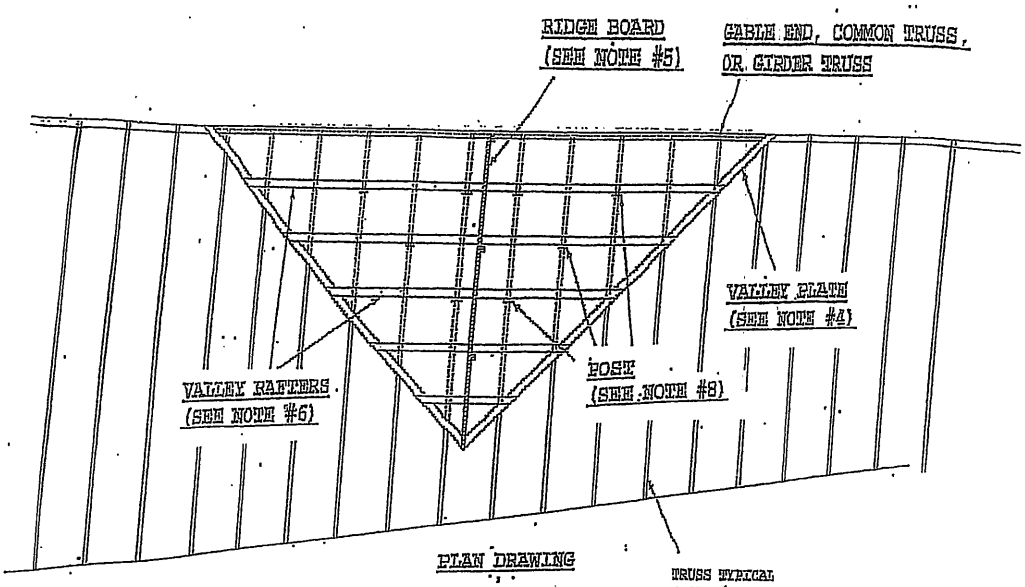
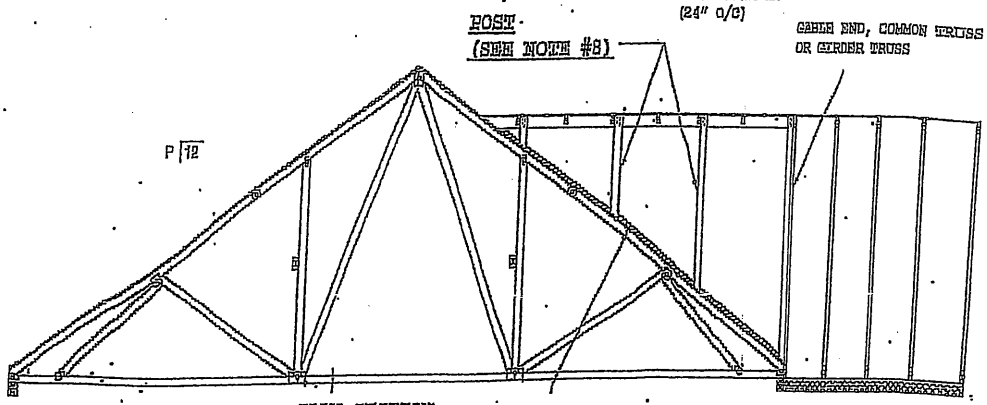
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

TRUSS MUST BE SHEATHED

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO TAM 6305.14
STRUCTURAL COMPONENT ONLY

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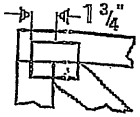
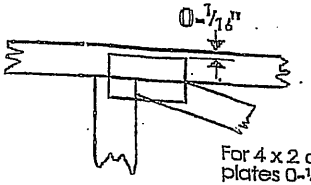
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

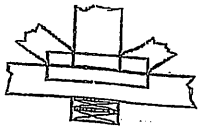
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

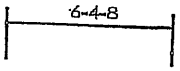
Industry Standards:

TPIC: Truss Design Procedures and Specifications

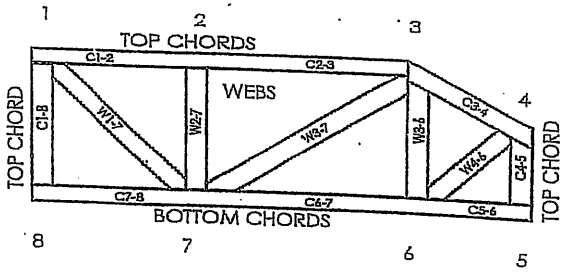
DSB-89: For Light Metal Plate Connected Wood Trusses

BCSI: *Design Standard for Bracing.*
Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS
CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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POWER TO PERFORM.™

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loadingshown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Site Copy

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2079
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO T4M-[illegible]-94/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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