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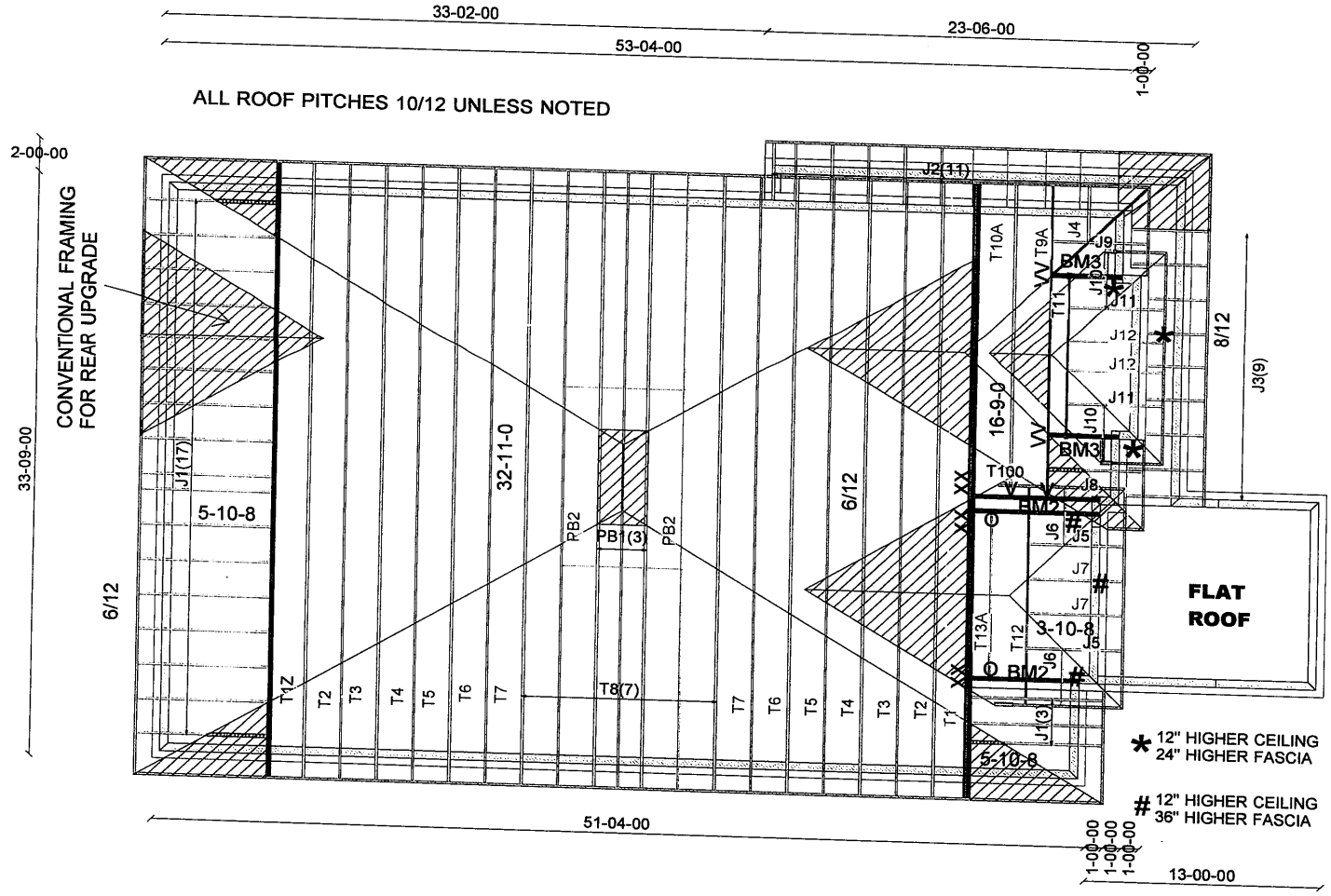
ALL ROOF PITCHES 10/12 UNLESS NOTED



* 12" HIGHER CEILING
24" HIGHER FASCIA

12" HIGHER CEILING
36" HIGHER FASCIA

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD [illegible] PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM2,BM3 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-15 / A STD OR REAR UPGRADE
	Layout ID: 296707	Project: GREEN VALLEY EAST	
	Plan Log: 94071	Date: 4/5/2018 Designer: AC/S.VE	

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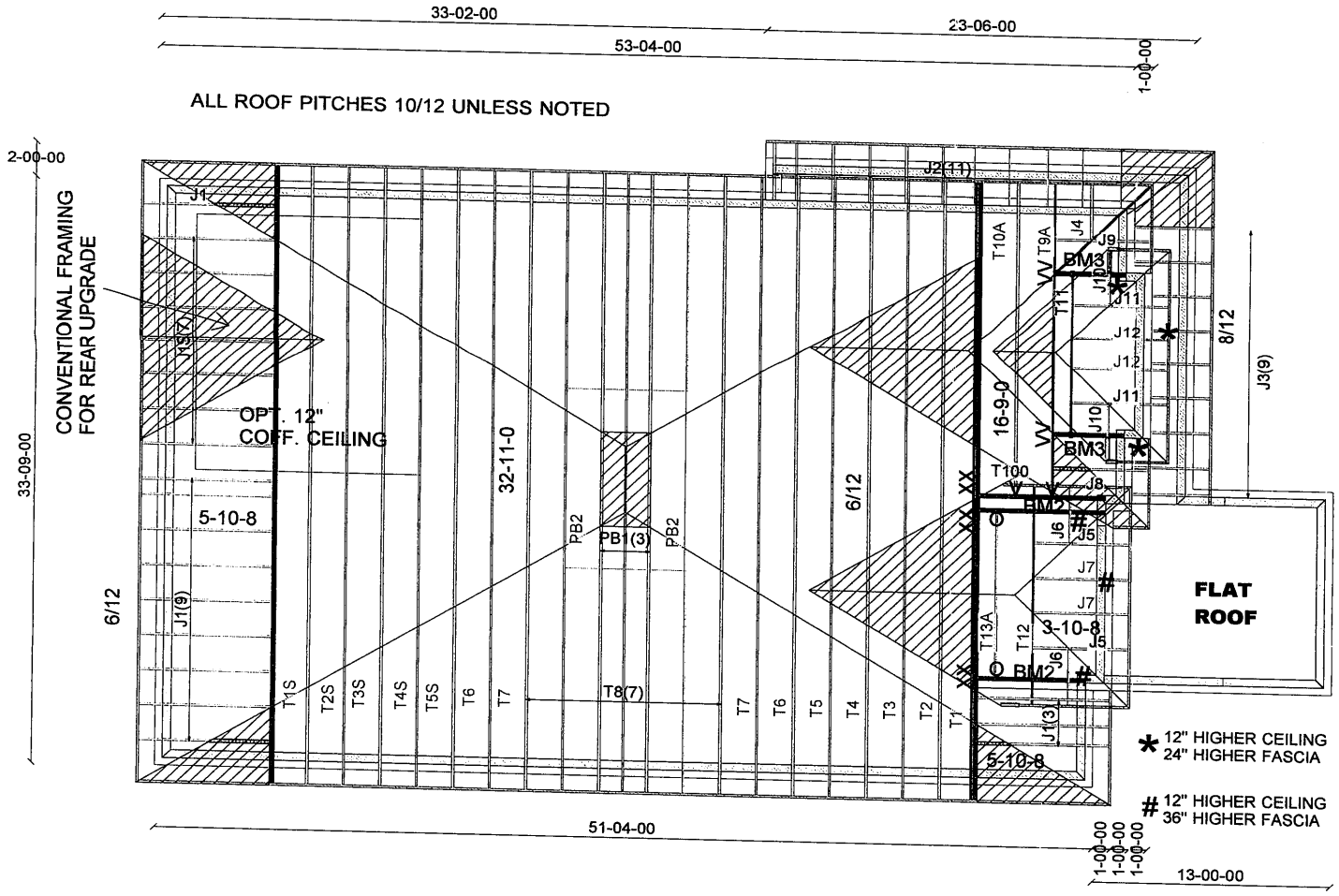
ALL ROOF PITCHES 10/12 UNLESS NOTED



* 12" HIGHER CEILING
24" HIGHER FASCIA

12" HIGHER CEILING
36" HIGHER FASCIA

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (VV)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM2,BM3 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / A STD OR REAR UPGRADE+OPT.COFF
Layout ID: 287689	Project: GREEN VALLEY ESTATES		
Plan Log: 94071	Date: 4/5/2018	Designer: AC/S.VE	

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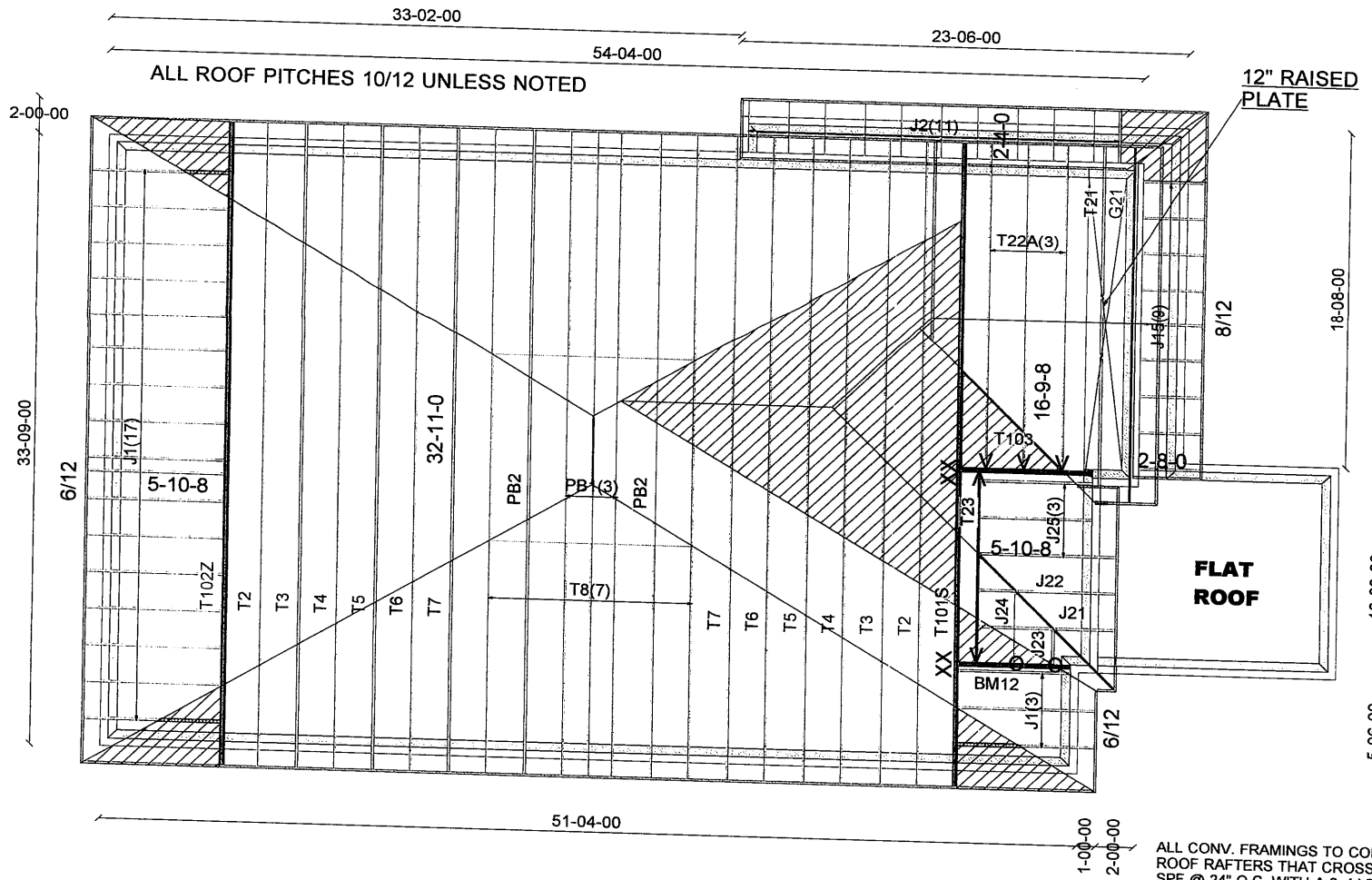

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM12 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
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m11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-15 / B STD
	Layout ID: 296708	Project: GREEN VALLEY EAST	
	Plan Log: 94071	Date: 4/5/2018 Designer: AC/S.V.E	

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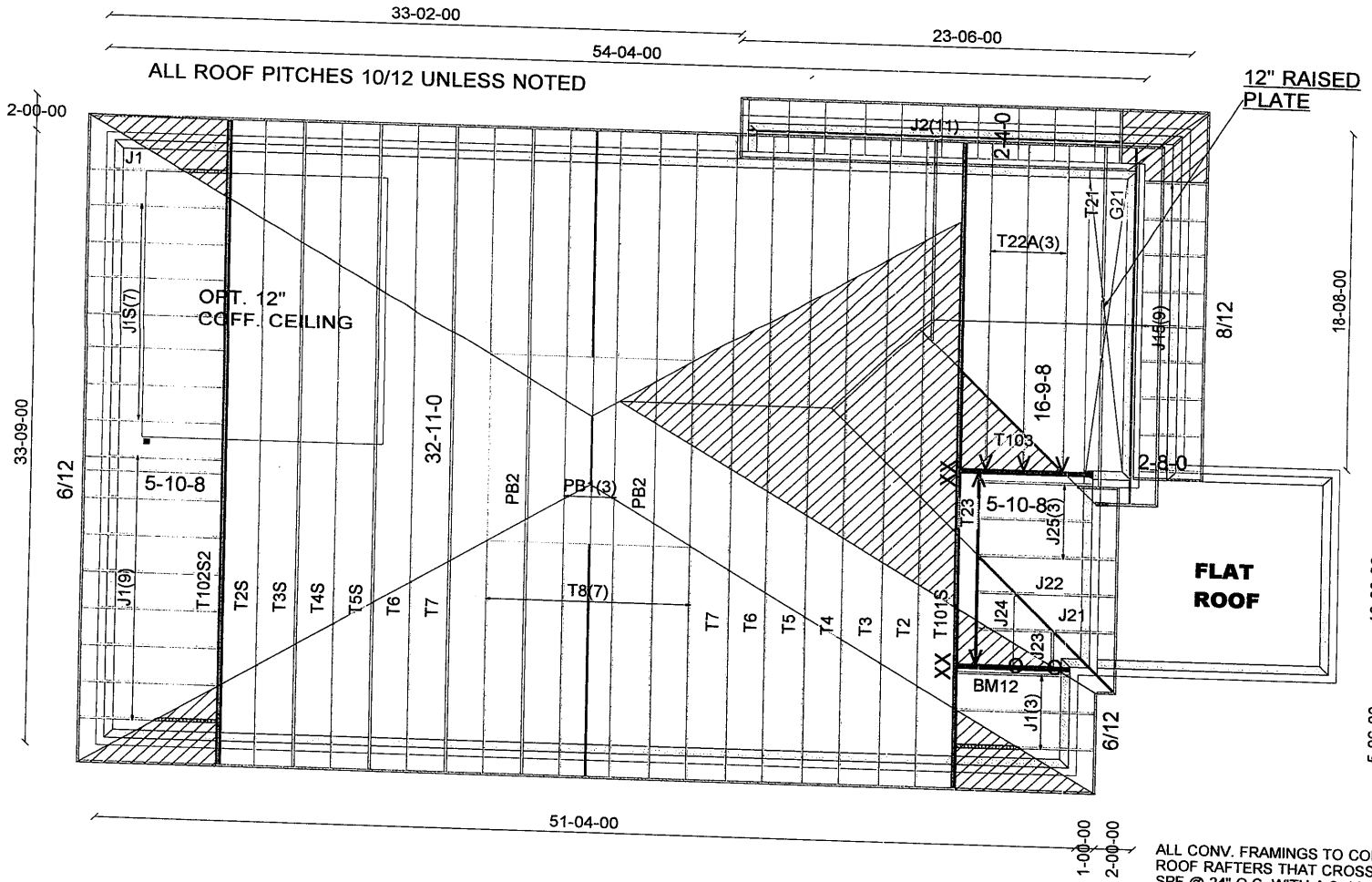

12" RAISED PLATE

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM12 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

m11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-15 / B STD+OPT. COFF.
	Layout ID: 287694	Project: GREEN VALLEY ESTATES	
	Plan Log: 94071	Date: 4/5/2018 Designer: AC/S.V.E	

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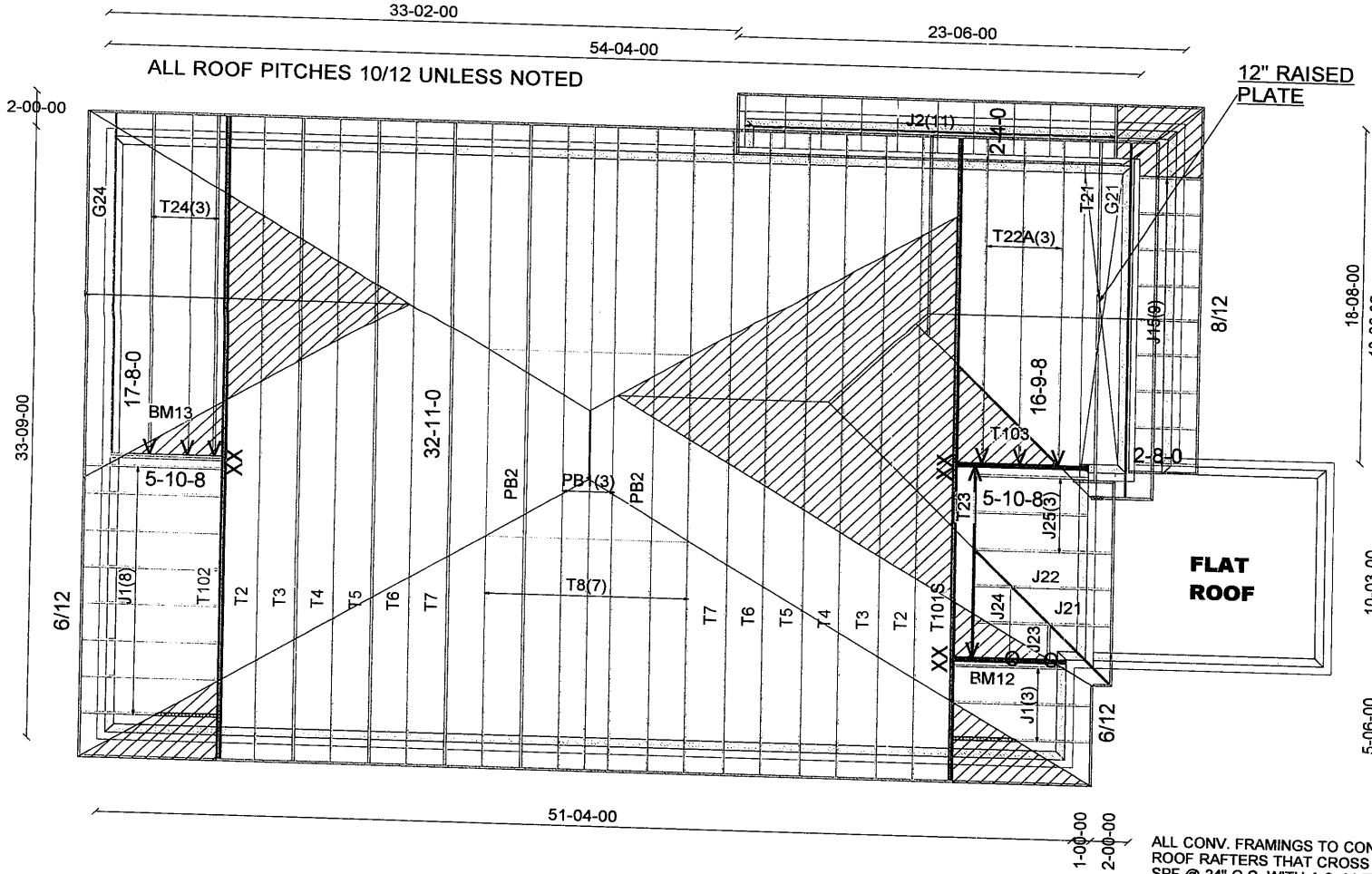

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM12,BM13 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / B REAR UPGRADE
Layout ID: 296709	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/5/2018	Designer: AC/S.V.E	

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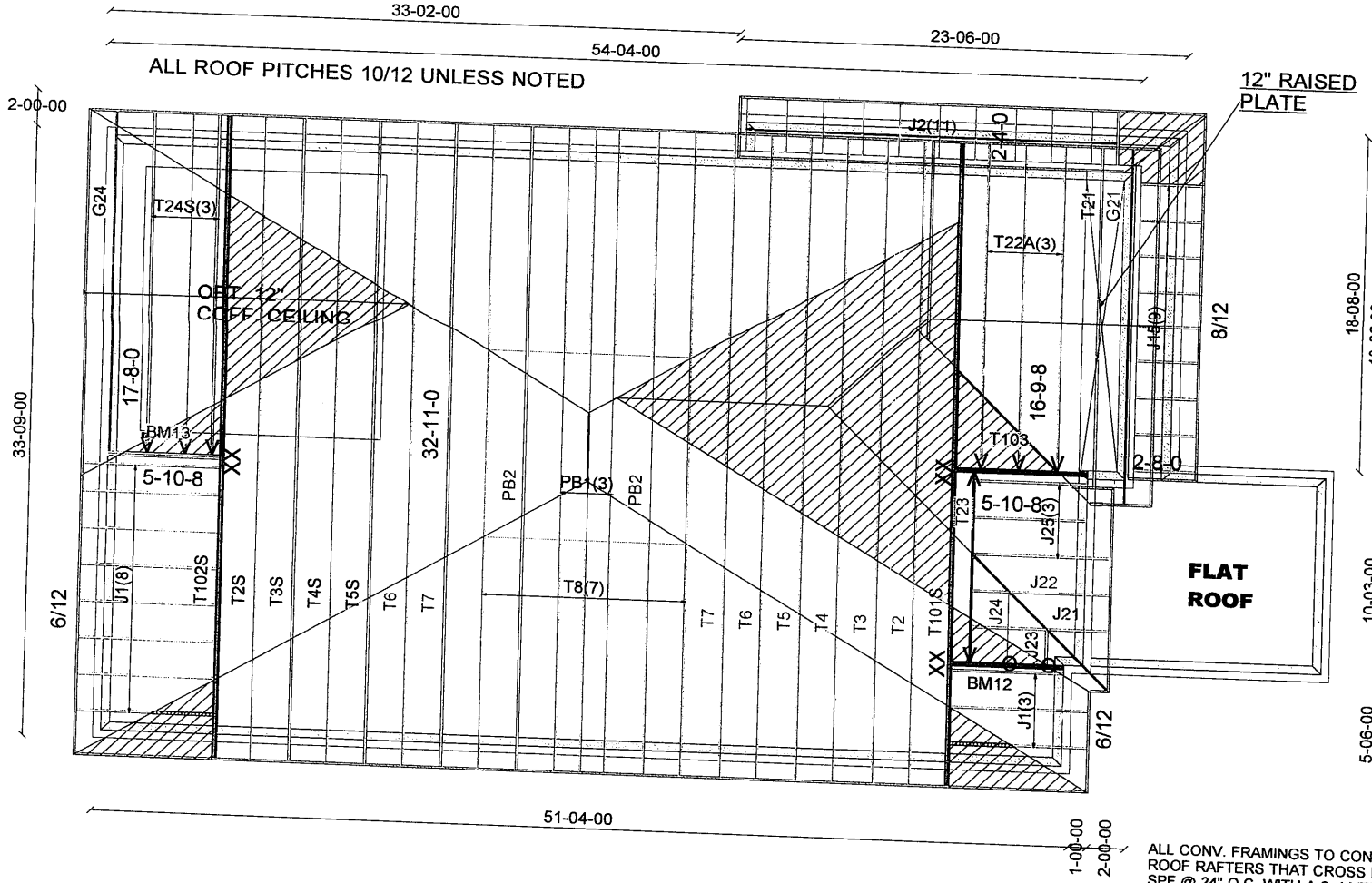

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
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M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD	Model / Elevation: S42-15 / B REAR UPGRADE+OPT. COFF.
	Layout ID: 287696	Project: GREEN VALLEY ESTATES	
	Plan Log: 94071	Date: 4/5/2018 Designer: AC	

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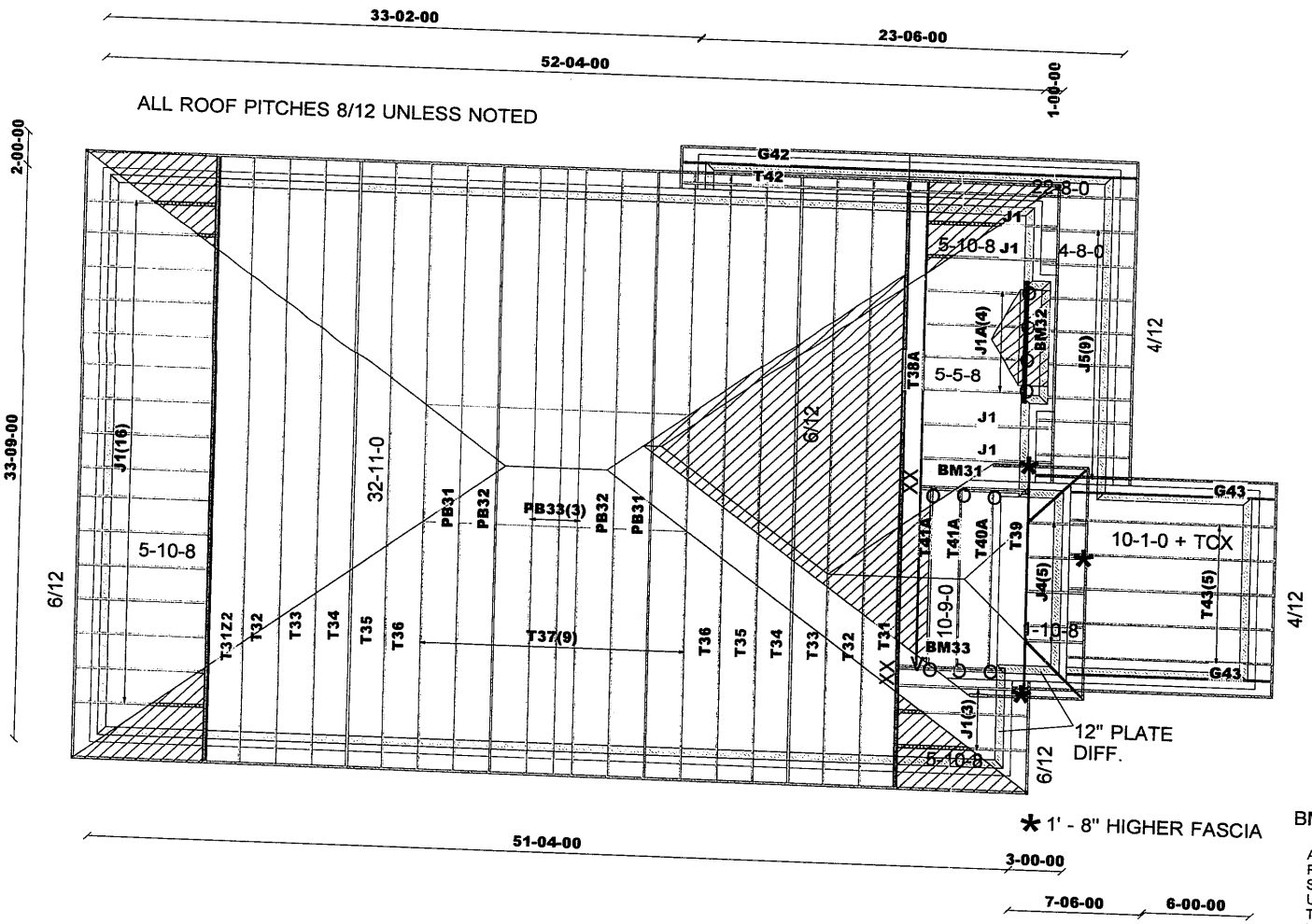
ALL ROOF PITCHES 8/12 UNLESS NOTED



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
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DENOTES CONV. FRAMING

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BC DEAD 7 PSF

T - 170639

* 1' - 8" HIGHER FASCIA

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M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / C STD
Layout ID: 296710	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/6/2018	Designer: AC/S.V.E	

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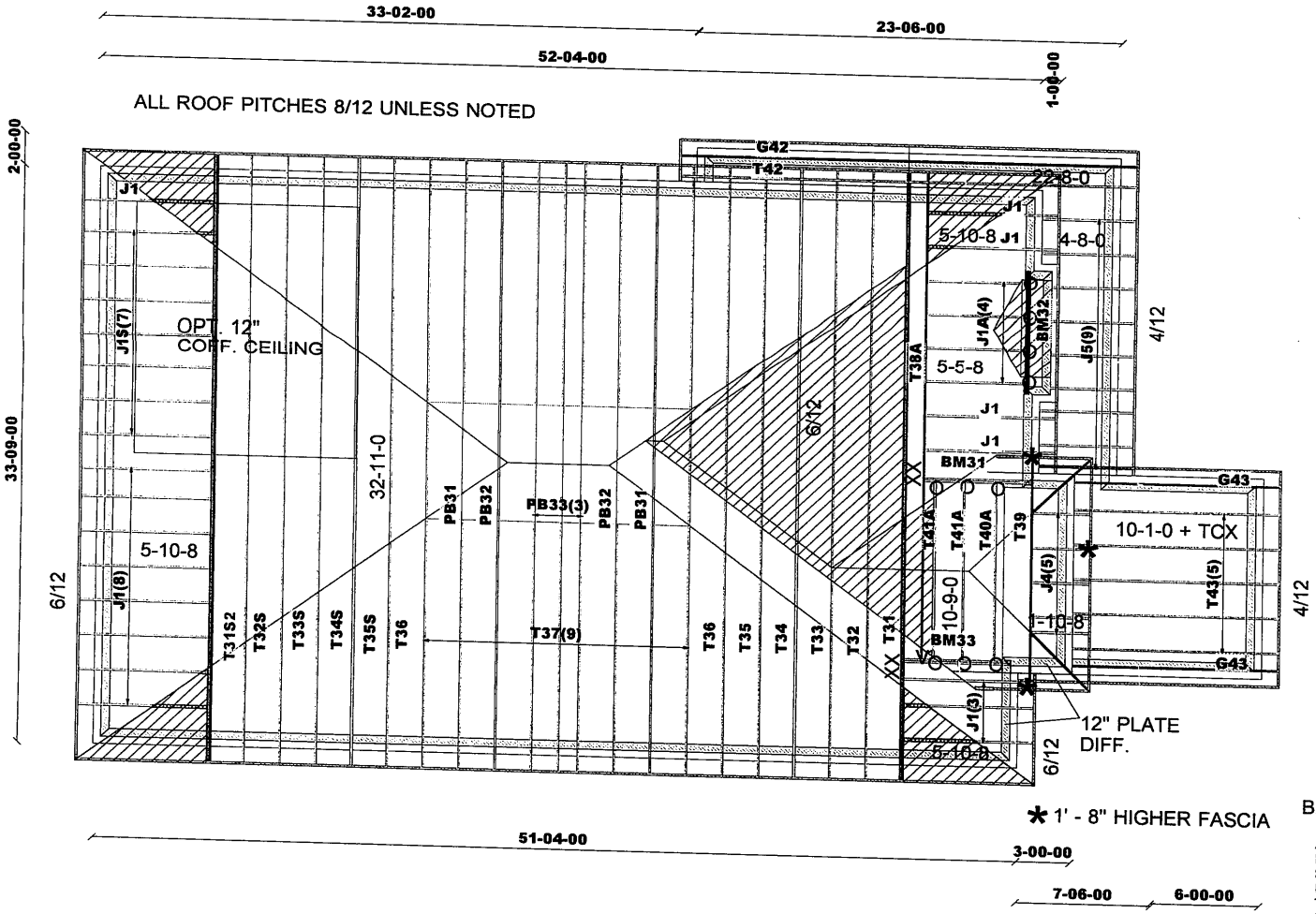

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

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T - 170639

BM31,BM32,BM33 = 2-2X10

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M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / C STD+OPT. COFF.
Layout ID: 287658	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/6/2018	Designer: AC/S.V.E	

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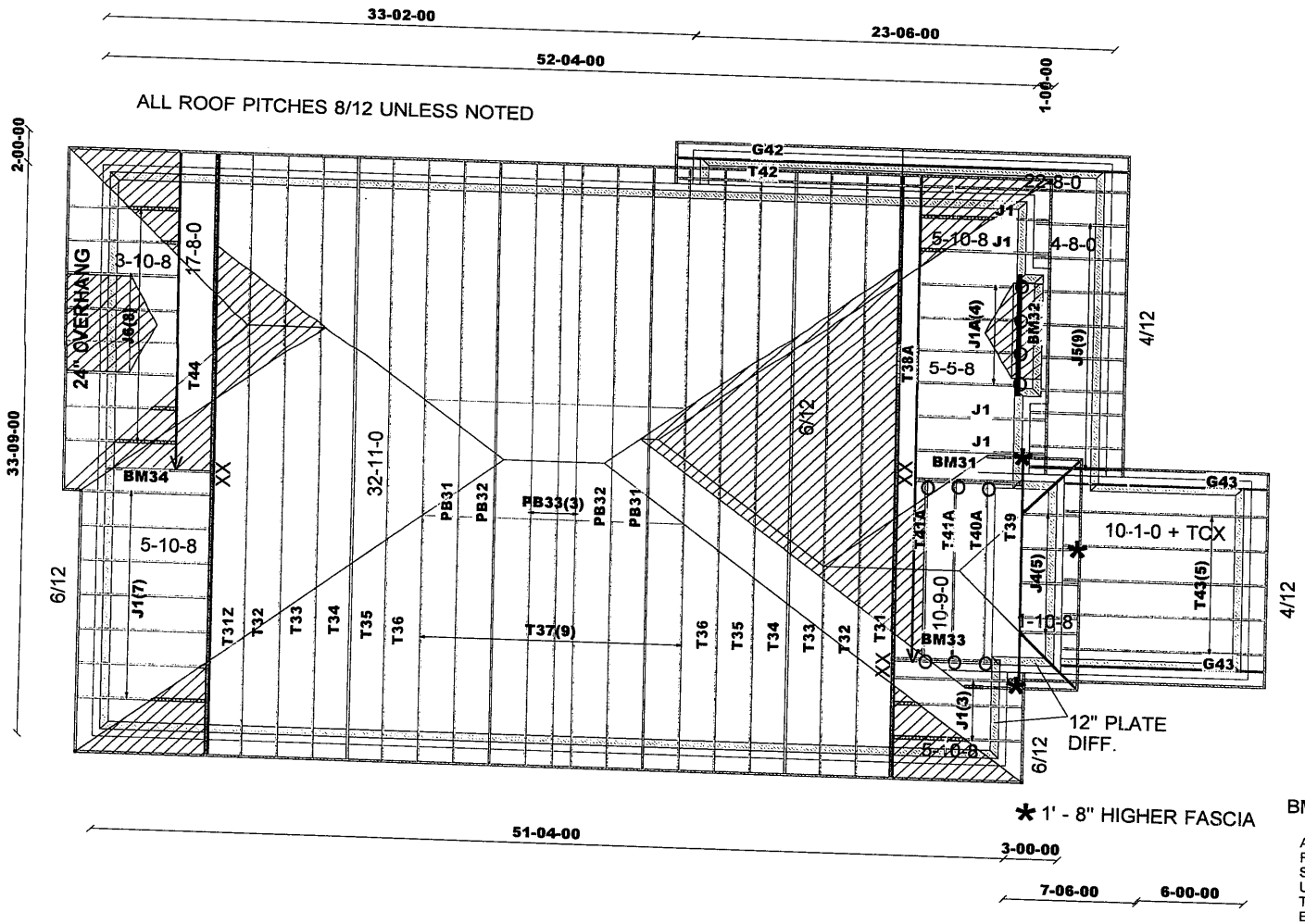

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
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TC DEAD 6 PSF
BC LIVE 10.5 PSF
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T - 170639

* 1' - 8" HIGHER FASCIA

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M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / C REAR UPGRADE
Layout ID: 296711	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/6/2018	Designer: AC/S.V.E	

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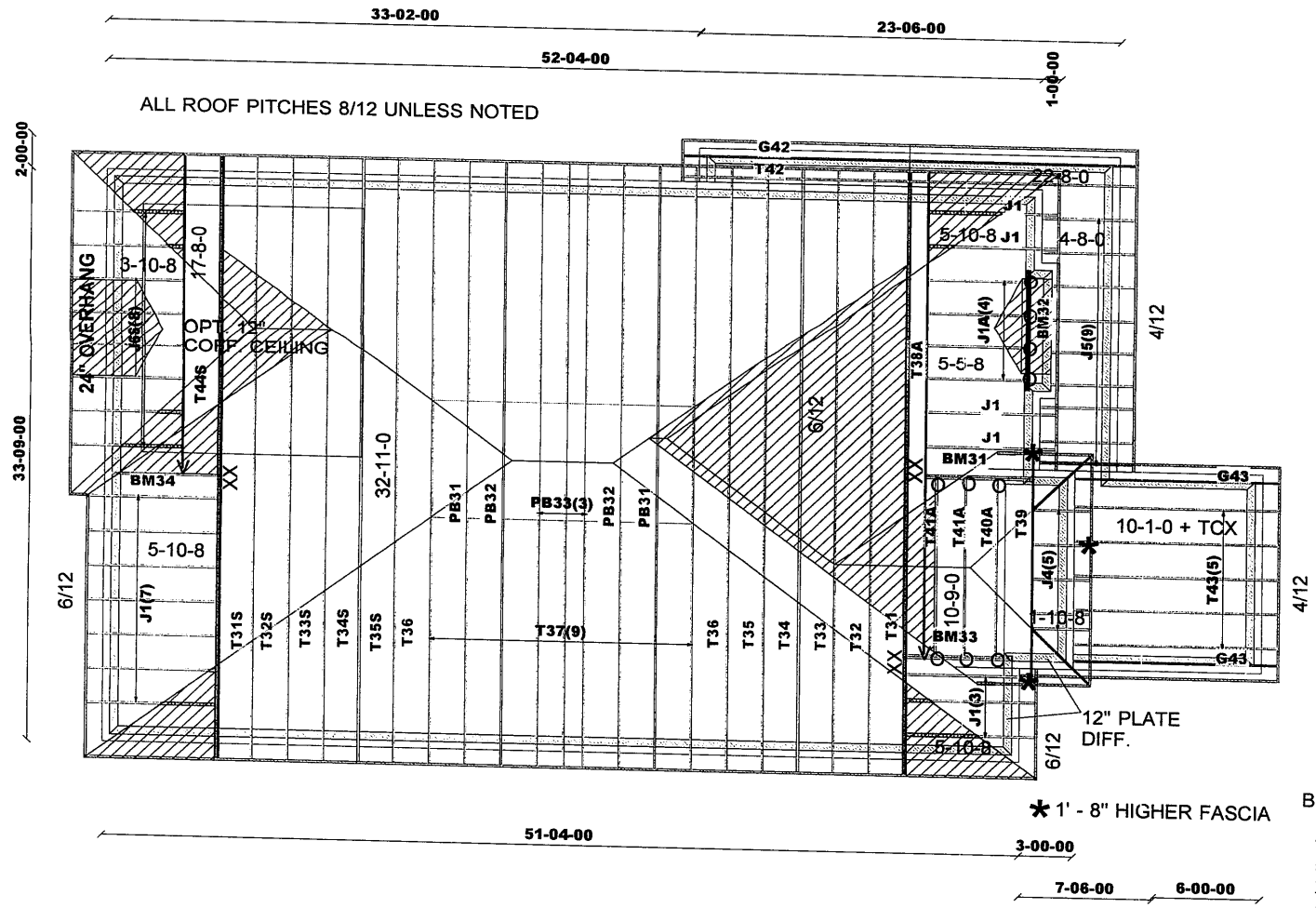

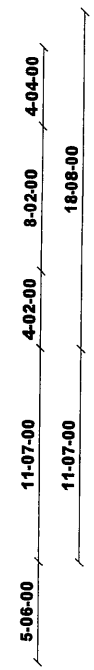

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:
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BM31,BM32,BM33,BM34 = 2-2X10

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M11,574

TAMARACK LUMBER INC. ALPA LUMBER GROUP

Job Track: 44755	Builder / Location: BAYVIEW WELLINGTON HOMES / BRADFORD		Model / Elevation: S42-15 / C REAR UPGRADE+OPT.COFF.
Layout ID: 287659	Project: GREEN VALLEY EAST		
Plan Log: 94071	Date: 4/6/2018	Designer: AC/S.V.E	

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Mitek ver 7.5.0

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

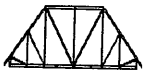
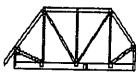
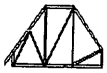
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287689 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** A STD OR REAR UPGRADE+OPT. COff.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 3 Ply	T1 / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	619.59 / 375.00		
	1 / 2 Ply	T1S / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	397.96 / 242.66		
	1	T2 / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.25 / 90.17		
	1	T2S / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	151.85 / 98.50		
	1	T3 / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.79 / 89.67		
	1	T3S / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	150.41 / 95.50		
	1	T4 / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	158.42 / 100.50		
	1	T4S / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	162.22 / 103.50		
	1	T5 / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	153.23 / 97.67		
	1	T5S / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	171.77 / 110.17		
	2	T6 / HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 / HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 / PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	1	T9A / HIP GIRDER	10.00 / 0.00	16-09-00	04-10-07	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 01-11-14	82.63 / 52.83		
	1	T10A / HIP	10.00 / 0.00	16-09-00	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-14	75.37 / 48.00		
	1	T11 / HIP GIRDER	10.00 / 0.00	09-08-00	05-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	02-07-11 / 02-07-11	54.10 / 35.33		
	1	T12 / HIP GIRDER	10.00 / 0.00	09-10-00	06-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	03-07-11 / 03-07-11	60.18 / 38.84		
	1	T13A / COMMON	10.00 / 0.00	09-04-00	08-08-14	2 X 4	2 X 4	00-00-00 / 00-00-00	04-10-03 / 04-10-03	51.72 / 34.17		

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

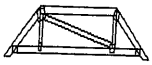
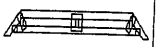
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287689 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** A STD OR REAR UPGRADE+OPT. COPY

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T100 / MONO TRUSS	6.00 / 0.00	06-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	00-08-00 / 04-01-04	59.88 / 37.34		
	3	PB1 / PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 / PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		
	13	J1 / JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	218.27 / 138.71		
	7	J1S / JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	149.31 / 109.69		
	2	J12 / JACK-OPEN	10.00 / 0.00	03-10-08	05-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 05-10-07	29.78 / 18.66		
	2	J10 / JACK-OPEN	10.00 / 0.00	01-10-08	04-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	02-07-11 / 00-03-08	19.02 / 13.34		
	2	J11 / JACK-OPEN	10.00 / 0.00	03-10-08	04-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	02-07-11 / 00-03-08	23.58 / 16.00		
	2	J7 / JACK-OPEN	10.00 / 0.00	03-10-08	06-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-07-11 / 06-10-07	32.06 / 20.00		
	2	J5 / JACK-OPEN	10.00 / 0.00	03-10-08	05-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	03-07-11 / 00-03-08	25.86 / 17.34		
	2	J6 / JACK-OPEN	10.00 / 0.00	01-10-08	05-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-07-11 / 00-06-10	21.56 / 14.66		
	1	J4 / JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	8.37 / 6.00		
	1	J8 / JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	13.75 / 8.67		
	1	J9 / JACK-OPEN	10.00 / 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	01-07-11 / 00-03-08	10.65 / 7.33		
	11	J2 / JACK-OPEN	10.00 / 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-07 / 00-00-00	00-07-05 / 02-06-10	102.30 / 69.63		
	9	J3 / JACK	8.00 / 0.00	03-08-00	02-09-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-07 / 02-09-13	105.21 / 72.00		

TOTAL # TRUSS= **91.00** TOTAL BFT OF ALL TRUSSES= **3467.18** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5468.61** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16

Site Copy

TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287689 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** A STD OR REAR UPGRADE+OPT. COFF.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LUS26-2	
2	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **9.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

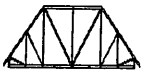
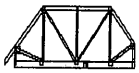
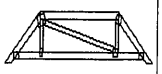
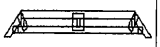
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296707 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** A STD OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T1Z HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	413.06 / 250.00		
	1 / 3 Ply	T1 HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	619.59 / 375.00		
	2	T2 HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.50 / 180.34		
	2	T3 HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.58 / 179.34		
	2	T4 HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	316.84 / 201.00		
	2	T5 HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.46 / 195.34		
	2	T6 HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	1	T9A HIP GIRDER	10.00 / 0.00	16-09-00	04-10-07	2 X 4	2 X 6	01-03-08 / 00-00-00	01-07-11 / 01-11-14	82.63 / 52.83		
	1	T10A HIP	10.00 / 0.00	16-09-00	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 01-11-14	75.37 / 48.00		
	1	T11 HIP GIRDER	10.00 / 0.00	09-08-00	05-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	02-07-11 / 02-07-11	54.10 / 35.33		
	1	T12 HIP GIRDER	10.00 / 0.00	09-10-00	06-10-07	2 X 4	2 X 4	01-03-08 / 01-03-08	03-07-11 / 03-07-11	60.18 / 38.84		
	1	T13A COMMON	10.00 / 0.00	09-04-00	08-08-14	2 X 4	2 X 4	00-00-00 / 00-00-00	04-10-03 / 04-10-03	51.72 / 34.17		
	1 / 2 Ply	T100 MONO TRUSS	6.00 / 0.00	06-10-08	04-01-04	2 X 4	2 X 6	00-00-00 / 00-00-00	00-08-00 / 04-01-04	59.88 / 37.34		
	3	PB1 PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		
	20	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	335.80 / 213.40		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296707 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** A STD OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	J4 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	01-07-11 / 00-03-08	8.37 / 6.00		
	1	J8 JACK-OPEN	10.00 / 0.00	03-10-08	04-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 04-10-07	13.75 / 8.67		
	1	J9 JACK-OPEN	10.00 / 0.00	03-10-08	03-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	01-07-11 / 00-03-08	10.65 / 7.33		
	2	J7 JACK-OPEN	10.00 / 0.00	03-10-08	06-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-07-11 / 06-10-07	32.06 / 20.00		
	2	J5 JACK-OPEN	10.00 / 0.00	03-10-08	05-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	03-07-11 / 00-03-08	25.86 / 17.34		
	2	J6 JACK-OPEN	10.00 / 0.00	01-10-08	05-02-07	2 X 4	2 X 4	01-03-08 / 00-00-00	03-07-11 / 00-06-10	21.56 / 14.66		
	2	J12 JACK-OPEN	10.00 / 0.00	03-10-08	05-10-07	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 05-10-07	29.78 / 18.66		
	2	J10 JACK-OPEN	10.00 / 0.00	01-10-08	04-01-09	2 X 4	2 X 4	01-03-08 / -00-01-01	02-07-11 / 00-03-08	19.02 / 13.34		
	2	J11 JACK-OPEN	10.00 / 0.00	03-10-08	04-01-09	2 X 4	2 X 4	01-03-08 / -02-01-01	02-07-11 / 00-03-08	23.58 / 16.00		
	11	J2 JACK-OPEN	10.00 / 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-07 / 00-00-00	00-07-05 / 02-06-10	102.30 / 69.63		
	9	J3 JACK	8.00 / 0.00	03-08-00	02-09-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-07 / 02-09-13	105.21 / 72.00		

TOTAL # TRUSS= **91.00** TOTAL BFT OF ALL TRUSSES= **3409.86** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5409.37** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LUS26-2	
2	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **9.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

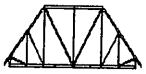
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287696 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T101S / ROOF	10.00 / 0.00	32-11-00	07-04-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	452.80 / 272.00		
	1 / 2 Ply	T102S / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	396.68 / 243.68		
	1	T2 / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.25 / 90.17		
	1	T2S / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	151.85 / 98.50		
	1	T3 / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.79 / 89.67		
	1	T3S / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	150.41 / 95.50		
	1	T4 / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	158.42 / 100.50		
	1	T4S / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	162.22 / 103.50		
	1	T5 / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	153.23 / 97.67		
	1	T5S / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	171.77 / 110.17		
	2	T6 / HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 / HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 / PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	3	T24S / ROOF	10.00 / 0.00	17-08-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	262.62 / 171.00		
	1	G24 / COMMON	10.00 / 0.00	17-08-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.60 / 59.83		
	1	T21 / COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	78.80 / 51.83		
	1	G21 / GABLE	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	86.73 / 56.83		
	3	T22A / COMMON	10.00 / 0.00	16-09-08	09-09-08	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 02-11-07	245.19 / 153.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

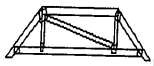
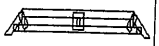
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287696 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	T23 HALF HIP	10.00 / 0.00	10-04-06	06-06-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 06-06-07	52.01 / 33.33		
	1 / 2 Ply	T103 MONO TRUSS	10.00 / 0.00	06-10-08	07-04-07	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 07-04-07	82.12 / 52.34		
	3	PB1 PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		
	11	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	184.69 / 117.37		
	3	J25 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	57.00 / 36.00		
	1	J21 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 04-01-01	01-07-11 / 00-03-08	17.58 / 11.67		
	1	J22 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 02-01-01	01-07-11 / 00-03-08	20.55 / 13.00		
	1	J23 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 / -00-01-01	01-07-11 / 00-03-08	6.45 / 4.67		
	1	J24 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 / 01-10-15	01-07-11 / 00-05-13	9.42 / 6.67		
	11	J2 JACK	10.00 / 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-07-05 / 02-06-10	102.41 / 69.63		
	9	J15 JACK	8.00 / 0.00	02-08-00	02-01-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-07 / 02-01-13	82.71 / 54.00		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3497.83** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5536.82** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	LUS24	
3	Hangers	HGUS26-2	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

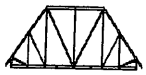
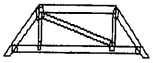
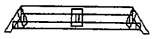
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296709 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-15 ELEVATION: B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T101S / ROOF	10.00 / 0.00	32-11-00	07-04-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	452.80 / 272.00		
	1 / 2 Ply	T102 / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.12 / 240.00		
	2	T2 / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.50 / 180.34		
	2	T3 / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.58 / 179.34		
	2	T4 / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	316.84 / 201.00		
	2	T5 / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.46 / 195.34		
	2	T6 / HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 / HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 / PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	3	T24 / COMMON	10.00 / 0.00	17-08-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	242.40 / 157.50		
	1	G24 / COMMON	10.00 / 0.00	17-08-00	09-00-00	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	91.60 / 60.50		
	1	T21 / COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	78.80 / 51.83		
	1	G21 / GABLE	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	86.73 / 56.83		
	3	T22A / COMMON	10.00 / 0.00	16-09-08	09-09-08	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 02-11-07	245.19 / 153.00		
	1	T23 / HALF HIP	10.00 / 0.00	10-04-06	06-06-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 06-06-07	52.01 / 33.33		
	1 / 2 Ply	T103 / MONO TRUSS	10.00 / 0.00	06-10-08	07-04-07	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 07-04-07	82.12 / 52.34		
	3	PB1 / PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 / PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		

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TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296709 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	11	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 00-04-11	184.69 117.37		
	3	J25 JACK-OPEN	10.00 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 06-06-07	57.00 36.00		
	1	J21 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 04-01-01	01-07-11 00-03-08	17.58 11.67		
	1	J22 JACK-OPEN	10.00 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	20.55 13.00		
	1	J23 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	01-07-11 00-03-08	6.45 4.67		
	1	J24 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 01-10-15	01-07-11 00-05-13	9.42 6.67		
	11	J2 JACK	10.00 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-08 00-00-00	00-07-05 02-06-10	102.41 69.63		
	9	J15 JACK	8.00 0.00	02-08-00	02-01-13	2 X 4	2 X 4	01-03-08 00-00-00	00-04-07 02-01-13	82.71 54.00		

TOTAL # TRUSS= **78.00** TOTAL BFT OF ALL TRUSSES= **3451.66** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5472.48** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LUS24	
8	Hangers	LJS26DS	

TOTAL # ITEMS= **13.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

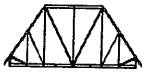
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287694 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T101S / ROOF	10.00 / 0.00	32-11-00	07-04-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	452.80 / 272.00		
	1 / 2 Ply	T102S2 / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	396.68 / 243.68		
	1	T2 / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	140.25 / 90.17		
	1	T2S / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	151.85 / 98.50		
	1	T3 / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	141.79 / 89.67		
	1	T3S / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	150.41 / 95.50		
	1	T4 / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	158.42 / 100.50		
	1	T4S / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	162.22 / 103.50		
	1	T5 / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	153.23 / 97.67		
	1	T5S / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	171.77 / 110.17		
	2	T6 / HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 / HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 / PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	3	T22A / COMMON	10.00 / 0.00	16-09-08	09-09-08	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 02-11-07	245.19 / 153.00		
	1	T21 / COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	78.80 / 51.83		
	1	G21 / GABLE	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	86.73 / 56.83		
	1	T23 / HALF HIP	10.00 / 0.00	10-04-06	06-06-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 06-06-07	52.01 / 33.33		
	1 / 2 Ply	T103 / MONO TRUSS	10.00 / 0.00	06-10-08	07-04-07	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 07-04-07	82.12 / 52.34		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

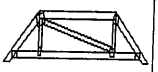
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287694 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B STD+OPT. COFF.

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	PB1 PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		
	13	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 00-04-11	218.27 / 138.71		
	7	J1S JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	152.53 / 112.00		
	3	J25 JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	57.00 / 36.00		
	1	J21 JACK-OPEN	10.00 / 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 / 04-01-01	01-07-11 / 00-03-08	17.58 / 11.67		
	1	J22 JACK-OPEN	10.00 / 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 / 02-01-01	01-07-11 / 00-03-08	20.55 / 13.00		
	1	J23 JACK-OPEN	10.00 / 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 / -00-01-01	01-07-11 / 00-03-08	6.45 / 4.67		
	1	J24 JACK-OPEN	10.00 / 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 / 01-10-15	01-07-11 / 00-05-13	9.42 / 6.67		
	11	J2 JACK	10.00 / 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-07-05 / 02-06-10	102.41 / 69.63		
	9	J15 JACK	8.00 / 0.00	02-08-00	02-01-13	2 X 4	2 X 4	01-03-08 / 00-00-00	00-04-07 / 02-01-13	82.71 / 54.00		

TOTAL # TRUSS= **83.00** TOTAL BFT OF ALL TRUSSES= **3400.34** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5368.71** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
2	Hangers	LUS24	
5	Hangers	LJS26DS	

TOTAL # ITEMS= **9.00**

Site Copy

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

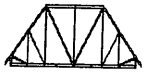
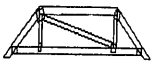
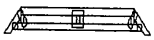
Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296708 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-15 ELEVATION: B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T101S / ROOF	10.00 / 0.00	32-11-00	07-04-07	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	452.80 / 272.00		
	1 / 2 Ply	T102Z / HIP GIRDER	10.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-07-11 / 01-07-11	395.12 / 240.00		
	2	T2 / HIP	10.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	280.50 / 180.34		
	2	T3 / HIP	10.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	283.58 / 179.34		
	2	T4 / HIP	10.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	316.84 / 201.00		
	2	T5 / HIP	10.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	306.46 / 195.34		
	2	T6 / HIP	10.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	320.48 / 202.34		
	2	T7 / HIP	10.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	333.52 / 210.66		
	7	T8 / PIGGYBACK	10.00 / 0.00	32-11-00	11-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	1287.44 / 800.31		
	3	T22A / COMMON	10.00 / 0.00	16-09-08	09-09-08	2 X 4	2 X 4	01-03-08 / 00-00-00	02-07-11 / 02-11-07	245.19 / 153.00		
	1	T21 / COMMON	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	78.80 / 51.83		
	1	G21 / GABLE	10.00 / 0.00	17-02-00	08-09-08	2 X 4	2 X 4	01-03-08 / 01-03-08	01-07-11 / 01-07-11	86.73 / 56.83		
	1	T23 / HALF HIP	10.00 / 0.00	10-04-06	06-06-07	2 X 4	2 X 4	00-00-00 / 00-00-00	01-07-11 / 06-06-07	52.01 / 33.33		
	1 / 2 Ply	T103 / MONO TRUSS	10.00 / 0.00	06-10-08	07-04-07	2 X 4	2 X 6	00-00-00 / 00-00-00	01-07-11 / 07-04-07	82.12 / 52.34		
	3	PB1 / PIGGYBACK	10.00 / 0.00	08-11-05	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	89.34 / 60.99		
	2	PB2 / PIGGYBACK	10.00 / 0.00	08-11-05	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-13 / 00-04-13	46.74 / 31.00		
	20	J1 / JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	335.80 / 213.40		
	3	J25 / JACK-OPEN	10.00 / 0.00	05-10-08	06-06-07	2 X 4	2 X 4	01-03-08 / 00-00-00	01-07-11 / 06-06-07	57.00 / 36.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

DATE	04/05/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296708 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** B STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	J21 JACK-OPEN	10.00 0.00	01-09-07	03-01-09	2 X 4	2 X 4	01-03-08 04-01-01	01-07-11 00-03-08	17.58 11.67		
	1	J22 JACK-OPEN	10.00 0.00	03-09-07	04-09-09	2 X 4	2 X 4	01-03-08 02-01-01	01-07-11 00-03-08	20.55 13.00		
	1	J23 JACK-OPEN	10.00 0.00	01-10-08	03-01-09	2 X 4	2 X 4	00-00-00 -00-01-01	01-07-11 00-03-08	6.45 4.67		
	1	J24 JACK-OPEN	10.00 0.00	01-10-08	04-09-09	2 X 4	2 X 4	00-00-00 01-10-15	01-07-11 00-05-13	9.42 6.67		
	11	J2 JACK	10.00 0.00	02-04-00	02-06-10	2 X 4	2 X 4	01-03-08 00-00-00	00-07-05 02-06-10	102.41 69.63		
	9	J15 JACK	8.00 0.00	02-08-00	02-01-13	2 X 4	2 X 4	01-03-08 00-00-00	00-04-07 02-01-13	82.71 54.00		

TOTAL # TRUSS= **83.00** TOTAL BFT OF ALL TRUSSES= **3329.69** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5289.59** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
2	Hangers	LUS24	

TOTAL # ITEMS= **9.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

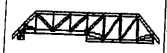
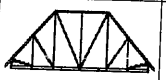
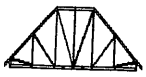
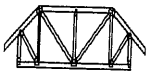
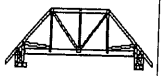
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 287659 LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-15 ELEVATION: C REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31 HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	398.18 / 248.02		
	1 / 2 Ply	T31S HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	402.26 / 251.34		
	1	T32 HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	133.34 / 83.17		
	1	T32S HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	143.45 / 91.50		
	1	T33 HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	137.38 / 86.83		
	1	T33S HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	150.49 / 96.17		
	1	T34 HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	144.42 / 91.50		
	1	T34S HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	157.48 / 100.00		
	1	T35 HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	145.26 / 90.33		
	1	T35S HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	155.48 / 96.83		
	2	T36 HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	303.72 / 189.00		
	9	T37 HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1420.56 / 893.97		
	1	T38A HIP GIRDER	8.00 / 0.00	27-02-00	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 03-02-13	159.95 / 98.67		
	1	T39 HIP GIRDER	8.00 / 0.00	10-09-00	03-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-00-13 / 02-00-13	50.27 / 32.50		
	1	T40A HIP	8.00 / 0.00	10-03-00	05-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	51.81 / 34.00		
	2	T41A COMMON	8.00 / 0.00	10-03-00	06-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	92.74 / 60.34		
	1	T44S HIP GIRDER	8.00 / 0.00	17-08-00	04-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	86.98 / 54.83		
	1	T42 COMMON	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	68.62 / 42.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

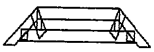
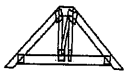
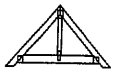
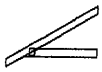
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287659 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C REAR UPGRADE+OPT. COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1	G42 GABLE	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	71.49 / 45.00		
	5	T43 MONOPITCH	4.00 / 0.00	10-01-00	03-10-11	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-10-11	182.30 / 120.00		
	2	G43 GABLE	4.00 / 0.00	10-01-00	03-08-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-08-04	67.12 / 45.34		
	2	PB31 PIGGYBACK	8.00 / 0.00	05-03-13	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	29.98 / 20.66		
	2	PB32 PIGGYBACK	8.00 / 0.00	05-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	36.82 / 26.00		
	3	PB33 PIGGYBACK	8.00 / 0.00	05-03-13	02-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	49.83 / 35.01		
	14	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	235.06 / 149.38		
	4	J1A JACK-OPEN	6.00 / 0.00	05-05-08	04-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-08 / 04-01-04	57.52 / 37.32		
	8	J6S JACK-OPEN	8.00 / 0.00	03-10-08	04-07-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 03-07-13	181.36 / 126.64		
	5	J4 JACK-OPEN	8.00 / 0.00	01-10-08	03-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	02-00-13 / 03-03-13	42.75 / 26.65		
	9	J5 JACK-OPEN	4.00 / 0.00	04-08-00	01-10-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-10-10	113.40 / 72.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3345.00** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5270.02** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
10	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

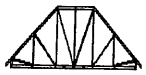
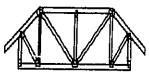
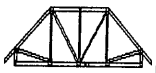
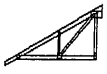
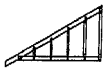
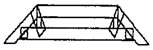
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296711 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31Z HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	391.56 / 244.02		
	1 / 2 Ply	T31 HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	398.18 / 248.02		
	2	T32 HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	266.68 / 166.34		
	2	T33 HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	274.76 / 173.66		
	2	T34 HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	288.84 / 183.00		
	2	T35 HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	290.52 / 180.66		
	2	T36 HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	303.72 / 189.00		
	9	T37 HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1420.56 / 893.97		
	1	T38A HIP GIRDER	8.00 / 0.00	27-02-00	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 03-02-13	159.95 / 98.67		
	1	T39 HIP GIRDER	8.00 / 0.00	10-09-00	03-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-00-13 / 02-00-13	50.27 / 32.50		
	1	T40A HIP	8.00 / 0.00	10-03-00	05-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	51.81 / 34.00		
	2	T41A COMMON	8.00 / 0.00	10-03-00	06-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	92.74 / 60.34		
	1	T44 HIP GIRDER	8.00 / 0.00	17-08-00	04-07-13	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	75.16 / 47.83		
	1	T42 COMMON	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	68.62 / 42.00		
	1	G42 GABLE	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	71.49 / 45.00		
	5	T43 MONOPITCH	4.00 / 0.00	10-01-00	03-10-11	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-10-11	182.30 / 120.00		
	2	G43 GABLE	4.00 / 0.00	10-01-00	03-08-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-08-04	67.12 / 45.34		
	2	PB31 PIGGYBACK	8.00 / 0.00	05-03-13	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	29.98 / 20.66		

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TAMARACK LUMBER INC.
[redacted]
ALPA LUMBER GROUP

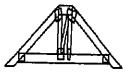
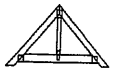
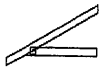
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296711 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	2	PB32 PIGGYBACK	8.00 / 0.00	05-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	36.82 / 26.00		
	3	PB33 PIGGYBACK	8.00 / 0.00	05-03-13	02-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	49.83 / 35.01		
	14	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	235.06 / 149.38		
	4	J1A JACK-OPEN	6.00 / 0.00	05-05-08	04-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-08 / 04-01-04	57.52 / 37.32		
	5	J4 JACK-OPEN	8.00 / 0.00	01-10-08	03-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	02-00-13 / 03-03-13	42.75 / 26.65		
	9	J5 JACK-OPEN	4.00 / 0.00	04-08-00	01-10-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-10-10	113.40 / 72.00		
	8	J6 JACK-OPEN	8.00 / 0.00	03-10-08	04-07-13	2 X 4	2 X 4	02-03-08 / 00-00-00	02-00-13 / 04-07-13	119.52 / 74.64		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3246.01** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5139.16** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
2	Hangers	LJS26DS	
10	Hangers	LUS24	

TOTAL # ITEMS= **15.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

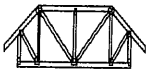
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287658 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C STD +OPT COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31S2 HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	402.26 / 251.34		
	1 / 2 Ply	T31 HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	398.18 / 248.02		
	1	T32 HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	133.34 / 83.17		
	1	T32S HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	143.45 / 91.50		
	1	T33 HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	137.38 / 86.83		
	1	T33S HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	150.49 / 96.17		
	1	T34 HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	144.42 / 91.50		
	1	T34S HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	157.48 / 100.00		
	1	T35 HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	145.26 / 90.33		
	1	T35S HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	155.48 / 96.83		
	2	T36 HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	303.72 / 189.00		
	9	T37 HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1420.56 / 893.97		
	1	T38A HIP GIRDER	8.00 / 0.00	27-02-00	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 03-02-13	159.95 / 98.67		
	1	T39 HIP GIRDER	8.00 / 0.00	10-09-00	03-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-00-13 / 02-00-13	50.27 / 32.50		
	1	T40A HIP	8.00 / 0.00	10-03-00	05-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	51.81 / 34.00		
	2	T41A COMMON	8.00 / 0.00	10-03-00	06-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	92.74 / 60.34		
	1	T42 COMMON	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	68.62 / 42.00		
	1	G42 GABLE	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	71.49 / 45.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

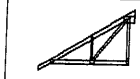
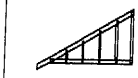
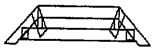
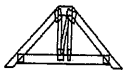
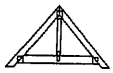
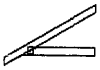
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 287658 **LOCATION:** BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C STD +OPT COFF

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T43 MONOPITCH	4.00 / 0.00	10-01-00	03-10-11	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-10-11	182.30 / 120.00		
	2	G43 GABLE	4.00 / 0.00	10-01-00	03-08-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-08-04	67.12 / 45.34		
	2	PB31 PIGGYBACK	8.00 / 0.00	05-03-13	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	29.98 / 20.66		
	2	PB32 PIGGYBACK	8.00 / 0.00	05-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	36.82 / 26.00		
	3	PB33 PIGGYBACK	8.00 / 0.00	05-03-13	02-01-12	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	49.83 / 35.01		
	16	J1 JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 04-01-04	268.64 / 170.72		
	4	J1A JACK-OPEN	6.00 / 0.00	05-05-08	04-01-04	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-08 / 04-01-04	57.52 / 37.32		
	7	J1S JACK-OPEN	6.00 / 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 / 00-00-00	01-02-00 / 03-01-04	152.53 / 112.00		
	5	J4 JACK-OPEN	8.00 / 0.00	01-10-08	03-03-13	2 X 4	2 X 4	01-03-08 / 00-00-00	02-00-13 / 03-03-13	42.75 / 26.65		
	9	J5 JACK-OPEN	4.00 / 0.00	04-08-00	01-10-10	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 01-10-10	113.40 / 72.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3296.87** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5187.79** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
1	Hangers	LJS26DS	
10	Hangers	LUS24	

TOTAL # ITEMS= **13.00**

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

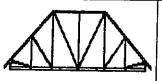
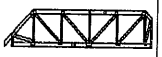
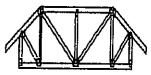
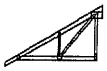
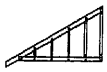
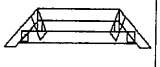
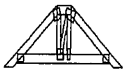
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 LAYOUT ID: 296710 LOCATION:
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:
MODEL: S42-15 ELEVATION: C STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 2 Ply	T31Z2 / HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	398.18 / 248.02		
	1 / 2 Ply	T31 / HIP GIRDER	8.00 / 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 / 01-03-08	01-04-13 / 01-04-13	398.18 / 248.02		
	2	T32 / HIP	8.00 / 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	266.68 / 166.34		
	2	T33 / HIP	8.00 / 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	274.76 / 173.66		
	2	T34 / HIP	8.00 / 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	288.84 / 183.00		
	2	T35 / HIP	8.00 / 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	290.52 / 180.66		
	2	T36 / HIP	8.00 / 0.00	32-11-00	09-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	303.72 / 189.00		
	9	T37 / HIP	8.00 / 0.00	32-11-00	10-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	01-04-13 / 01-04-13	1420.56 / 893.97		
	1	T38A / HIP GIRDER	8.00 / 0.00	27-02-00	04-01-04	2 X 6	2 X 6	01-03-08 / 00-00-00	01-04-13 / 03-02-13	159.95 / 98.67		
	1	T39 / HIP GIRDER	8.00 / 0.00	10-09-00	03-03-13	2 X 4	2 X 4	01-03-08 / 01-03-08	02-00-13 / 02-00-13	50.27 / 32.50		
	1	T40A / HIP	8.00 / 0.00	10-03-00	05-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	51.81 / 34.00		
	2	T41A / COMMON	8.00 / 0.00	10-03-00	06-07-13	2 X 4	2 X 4	00-00-00 / 00-00-00	03-02-13 / 03-02-13	92.74 / 60.34		
	1	T42 / COMMON	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	68.62 / 42.00		
	1	G42 / GABLE	4.00 / 0.00	22-08-00	04-01-04	2 X 4	2 X 4	01-03-08 / 01-03-08	00-03-15 / 00-03-15	71.49 / 45.00		
	5	T43 / MONOPITCH	4.00 / 0.00	10-01-00	03-10-11	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-10-11	182.30 / 120.00		
	2	G43 / GABLE	4.00 / 0.00	10-01-00	03-08-04	2 X 4	2 X 4	01-03-08 / 00-00-00	00-03-15 / 03-08-04	67.12 / 45.34		
	2	PB31 / PIGGYBACK	8.00 / 0.00	05-03-13	00-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	29.98 / 20.66		
	2	PB32 / PIGGYBACK	8.00 / 0.00	05-03-13	01-10-08	2 X 4	2 X 4	00-00-00 / 00-00-00	00-04-07 / 00-04-07	36.82 / 26.00		

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TAMARACK LUMBER INC.
ALPA LUMBER GROUP

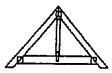
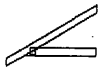
Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK: 44755 **LAYOUT ID:** 296710 **LOCATION:**
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE **SUB-BUILDER:**
MODEL: S42-15 **ELEVATION:** C STD

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	3	PB33 PIGGYBACK	8.00 0.00	05-03-13	02-01-12	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	49.83 35.01		
	23	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	386.17 245.41		
	4	J1A JACK-OPEN	6.00 0.00	05-05-08	04-01-04	2 X 4	2 X 4	00-00-00 00-00-00	01-04-08 04-01-04	57.52 37.32		
	5	J4 JACK-OPEN	8.00 0.00	01-10-08	03-03-13	2 X 4	2 X 4	01-03-08 00-00-00	02-00-13 03-03-13	42.75 26.65		
	9	J5 JACK-OPEN	4.00 0.00	04-08-00	01-10-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 01-10-10	113.40 72.00		

TOTAL # TRUSS= **85.00** TOTAL BFT OF ALL TRUSSES= **3223.57** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5102.21** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
1	Hangers	LJS26DS	
10	Hangers	LUS24	

TOTAL # ITEMS= **13.00**

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T31	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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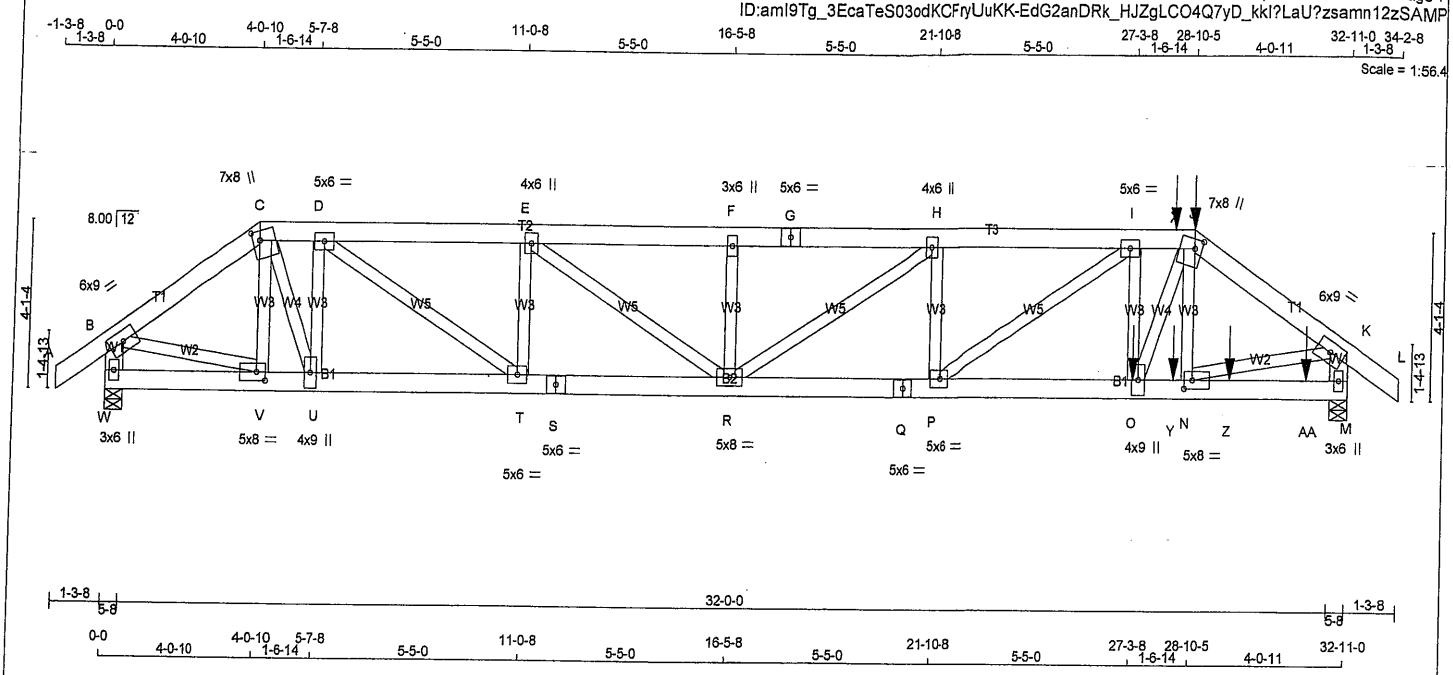

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	2100F 1.8E	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	TOP
G- J	2	12	SIDE(122.0)
J- L	2	12	SIDE(122.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	TOP
S- Q	2	12	TOP
Q- M	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
I- O	1	2	SIDE(908.4)
D- U	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0		
C	TTWW+m	MT20	7.0	8.0	2.75	2.25
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	2.75	2.25
K	TMVW-t	MT20	6.0	9.0		
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	2.50
O	BMWW+t	MT20	4.0	9.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3269	0	3269	0	0	5-8	5-8
M	6522	0	6522	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2576	1543 / 0	449 / 0	0 / 0	0 / 0	584 / 0	0 / 0
M	5135	3082 / 0	890 / 0	0 / 0	0 / 0	1163 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)			10.00
B- C	-3860 / 0	-112.4	-112.4	0.11 (1)			5.66
C- D	-4232 / 0	-112.4	-112.4	0.08 (1)			5.50
D- E	-7120 / 0	-112.4	-112.4	0.18 (1)			4.39
E- F	-8831 / 0	-112.4	-112.4	0.23 (1)			3.97
F- G	-8831 / 0	-112.4	-112.4	0.23 (1)			3.97
G- H	-8831 / 0	-112.4	-112.4	0.23 (1)			3.97
H- I	-9506 / 0	-112.4	-112.4	0.28 (1)			3.79
I- X	-8839 / 0	-112.4	-112.4	0.16 (1)			4.02
X- J	-8839 / 0	-112.4	-112.4	0.16 (1)			4.02
J- K	-8249 / 0	-112.4	-112.4	0.18 (1)			4.10
K- L	0 / 44	-112.4	-112.4	0.04 (1)			10.00
W- B	-3225 / 0	0.0	0.0	0.11 (1)			7.71
M- K	-6451 / 0	0.0	0.0	0.23 (1)			5.85
W- V	0 / 0	-28.0	-28.0	0.02 (2)			10.00
V- U	0 / 3181	-28.0	-28.0	0.10 (1)			10.00
U- T	0 / 4233	-28.0	-28.0	0.13 (1)			10.00
T- S	0 / 7120	-28.0	-28.0	0.22 (1)			10.00
S- R	0 / 7120	-28.0	-28.0	0.22 (1)			10.00
R- Q	0 / 9506	-28.0	-28.0	0.28 (1)			10.00
Q- P	0 / 9506	-28.0	-28.0	0.28 (1)			10.00
P- O	0 / 8837	-28.0	-28.0	0.30 (1)			10.00
O- Y	0 / 6806	-28.0	-28.0	0.24 (1)			10.00
Y- N	0 / 6806	-28.0	-28.0	0.24 (1)			10.00
N- Z	0 / 0	-28.0	-28.0	0.03 (2)			10.00
Z-AA	0 / 0	-28.0	-28.0	0.03 (2)			10.00
AA- M	0 / 0	-28.0	-28.0	0.03 (2)			10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-559 / 0	0.05 (1)
U- D	-2354 / 0	0.23 (1)
O- I	-706 / 0	0.20 (1)
O- J	0 / 5108	0.06 (1)
N- J	-980 / 0	0.45 (1)
B- V	0 / 3275	0.08 (1)
N- K	0 / 7000	0.29 (1)
P- I	0 / 806	0.62 (1)
D- T	0 / 3480	0.07 (1)
P- H	-218 / 69	0.31 (1)
T- E	-1788 / 0	0.02 (1)
R- H	-814 / 0	0.15 (1)
E- R	0 / 2063	0.21 (1)
R- F	-579 / 0	0.18 (1)
		0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	28-10-6	-44	-49	—	FRONT	VERT	DEAD
J	28-10-6	-282	-282	—	FRONT	VERT	SNOW
O	27-3-8	-4240	-4240	—	BACK	VERT	TOTAL
X	28-4-4	-163	-163	—	BACK	VERT	TOTAL
Y	28-4-4	-40	-70	—	BACK	VERT	TOTAL
Z	29-10-4	-40	-70	—	BACK	VERT	TOTAL
AA	31-10-4	-40	-70	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.28/1.00 (H-I:1) , BC=0.30/1.00 (O-P:1) , WB=0.62/1.00 (K-N:1) , SSI=0.12/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.73 (Q) (INPUT = 1.00)

PG 4/4

LICENSED PROFESSIONAL ENGINEER 41078 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 16460-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T31	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	4.0	9.0		
V	BMWW-t	MT20	5.0	8.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 325.8 lbs FACTORED DOWN AT 28-10-6, AND 163.0 lbs FACTORED DOWN AT 28-4-4 ON TOP CHORD, AND 4240.4 lbs FACTORED DOWN AT 27-3-8, 69.9 lbs FACTORED DOWN AT 28-4-4, AND 69.9 lbs FACTORED DOWN AT 29-10-4, AND 69.9 lbs FACTORED DOWN AT 31-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
41016
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18460-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296711	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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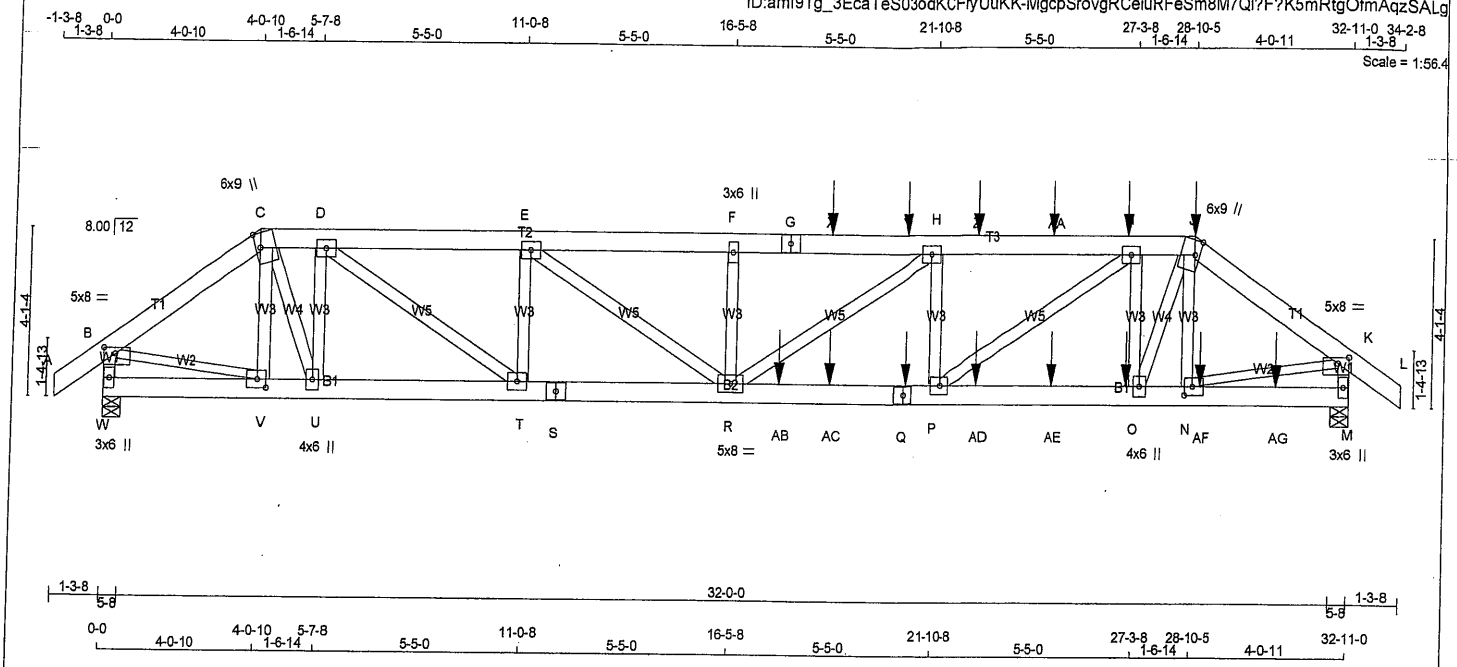

TOTAL WEIGHT = 2 X 196 = 392 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x6	DRY	2100F 1.8E	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - V	2x3	DRY	No.2	SPF
N - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- G	2	12	TOP
G- J	2	12	SIDE(183.1)
J- L	2	12	SIDE(122.0)
W- B	1	12	TOP
M- K	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	TOP
S- Q	2	12	SIDE(0.0)
Q- M	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D, E, H, I						
D	TMWW-t	MT20	5.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.25	1.50
K	TMVW-p	MT20	5.0	8.0	1.75	3.50
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.50
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3540	0	3540	0	0	5-8	5-8
M	4436	0	4436	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	2789	1670 / 0	486 / 0	0 / 0	0 / 0	633 / 0	0 / 0
M	3498	2089 / 0	611 / 0	0 / 0	0 / 0	798 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-4219 / 0	-112.4	-112.4	0.11 (1)	5.47
C- D	-4664 / 0	-112.4	-112.4	0.09 (1)	5.29
D- E	-7895 / 0	-112.4	-112.4	0.20 (1)	4.19
E- F	-10215 / 0	-112.4	-112.4	0.28 (1)	3.68
F- G	-10215 / 0	-112.4	-112.4	0.34 (1)	3.61
G- X	-10215 / 0	-112.4	-112.4	0.34 (1)	3.61
X- Y	-10215 / 0	-112.4	-112.4	0.34 (1)	3.61
Y- H	-10215 / 0	-112.4	-112.4	0.34 (1)	3.61
H- Z	-9253 / 0	-112.4	-112.4	0.30 (1)	3.81
Z-AA	-9253 / 0	-112.4	-112.4	0.30 (1)	3.81
AA- I	-9253 / 0	-112.4	-112.4	0.30 (1)	3.81
I- J	-5758 / 0	-112.4	-112.4	0.13 (1)	4.82
J- K	-5406 / 0	-112.4	-112.4	0.13 (1)	4.94
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00
W- B	-3488 / 0	0.0	0.0	0.19 (1)	6.26
M- K	-4361 / 0	0.0	0.0	0.24 (1)	5.70
W- V	0 / 0	-28.0	-28.0	0.02 (3)	10.00
V- U	0 / 3473	-28.0	-28.0	0.11 (1)	10.00
U- T	0 / 4665	-28.0	-28.0	0.16 (1)	10.00
T- S	0 / 7895	-28.0	-28.0	0.30 (1)	10.00
S- R	0 / 7895	-28.0	-28.0	0.30 (1)	10.00
R-AB	0 / 9253	-28.0	-28.0	0.49 (1)	10.00
AB-AC	0 / 9253	-28.0	-28.0	0.49 (1)	10.00
AC- Q	0 / 9253	-28.0	-28.0	0.49 (1)	10.00
Q- P	0 / 9253	-28.0	-28.0	0.49 (1)	10.00
P-AD	0 / 5759	-28.0	-28.0	0.22 (1)	10.00
AD-AE	0 / 5759	-28.0	-28.0	0.22 (1)	10.00
AE- O	0 / 5759	-28.0	-28.0	0.22 (1)	10.00
O- N	0 / 4457	-28.0	-28.0	0.14 (1)	10.00
N-AF	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AF-AG	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AG- M	0 / 0	-28.0	-28.0	0.03 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-692 / 0	0.06 (1)
C- U	0 / 2997	0.26 (1)
U- D	-2573 / 0	0.22 (1)
O- I	-3011 / 0	0.26 (1)
O- J	0 / 3273	0.29 (1)
N- J	-719 / 0	0.06 (1)
B- V	0 / 3580	0.44 (1)
N- K	0 / 4587	0.57 (1)
P- I	0 / 4212	0.37 (1)
D- T	0 / 3894	0.34 (1)
P- H	-1672 / 0	0.14 (1)
T- E	-2198 / 0	0.19 (1)
R- H	0 / 1159	0.10 (1)
E- R	0 / 2796	0.25 (1)
R- F	-642 / 0	0.05 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	27-0-12	-135	-135	---	FRONT	VERT	TOTAL
J	28-10-6	-44	-49	---	FRONT	VERT	DEAD
J	28-10-6	-141	-141	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 992 (0.40")

CSI: TC=0.34/1.00 (F-H:1) , BC=0.49/1.00 (P-R:1) , WB=0.57/1.00 (K-N:1) , SSI=0.45/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.72 (Q) (INPUT = 1.00) pb 12

LICENSED PROFESSIONAL ENGINEER 41078 S KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM/8485 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296711	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-MgcpSrovgRCeiuRFeSm8M7QI?F?K5mRtgOfmAqzSALg

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.50
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 467.1 lbs FACTORED DOWN AT 28-10-6, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, 135.3 lbs FACTORED DOWN AT 23-0-12, AND 135.3 lbs FACTORED DOWN AT 25-0-12, AND 135.3 lbs FACTORED DOWN AT 27-0-12 ON TOP CHORD, AND 1611.2 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-0-12, 69.9 lbs FACTORED DOWN AT 21-0-12, 69.9 lbs FACTORED DOWN AT 23-0-12, 69.9 lbs FACTORED DOWN AT 25-0-12, 69.9 lbs FACTORED DOWN AT 27-0-12, AND 69.9 lbs FACTORED DOWN AT 29-0-12, AND 69.9 lbs FACTORED DOWN AT 31-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
J	28-10-6	-282	-282	—	FRONT	VERT	SNOW
O	27-0-12	-40	-70	—	FRONT	VERT	TOTAL
Q	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
X	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
Y	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
Z	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AA	25-0-12	-135	-135	—	FRONT	VERT	TOTAL
AB	17-8-12	-1611	-1611	—	FRONT	VERT	TOTAL
AC	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
AD	23-0-12	-40	-70	—	FRONT	VERT	TOTAL
AE	25-0-12	-40	-70	—	FRONT	VERT	TOTAL
AF	29-0-12	-40	-70	—	FRONT	VERT	TOTAL
AG	31-0-12	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/9/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 16485-08
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296710	T31Z2	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 10 09:45:03 2018 Page 1
ID:amI9Tg_3EcaTeS03odKCFryUuKK-8HCYPqjamnnsqaEr5P7zR9amat?puAX38D3orQzSAKU

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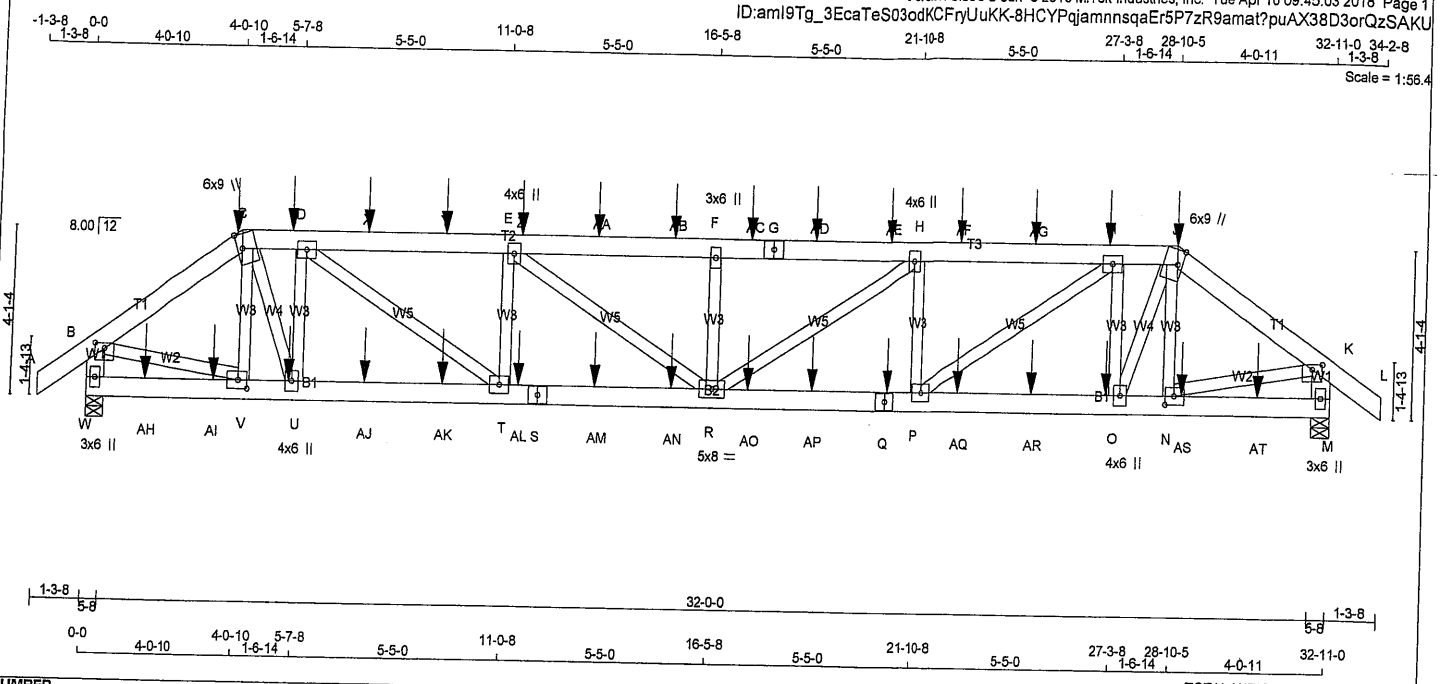

TOTAL WEIGHT = 2 X 199 = 398 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
W - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
W - S	2x6	DRY	2100F 1.8E	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - M	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- J	2	12	SIDE(183.1)
J- L	2	12	SIDE(122.0)
W- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- S	2	12	SIDE(197.8)
S- Q	2	12	SIDE(0.1)
Q- M	2	12	SIDE(197.8)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	6.0	9.0	4.25	1.50
D	TMWW-t	MT20	5.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TTWW+m	MT20	6.0	9.0	4.25	1.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	3869	0	3869	0	0	5-8	5-8
M	3953	0	3953	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3092	1782 / 0	588 / 0	0 / 0	0 / 0	722 / 0	0 / 0
M	3146	1831 / 0	583 / 0	0 / 0	0 / 0	732 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-4641 / 0	-112.4	-112.4	0.12 (1)	5.26
C- D	-5014 / 0	-112.4	-112.4	0.13 (1)	5.10
D- X	-7830 / 0	-112.4	-112.4	0.26 (1)	4.14
X- Y	-7830 / 0	-112.4	-112.4	0.26 (1)	4.14
Y- E	-7830 / 0	-112.4	-112.4	0.26 (1)	4.14
E- Z	-8750 / 0	-112.4	-112.4	0.28 (1)	3.92
Z-AA	-8750 / 0	-112.4	-112.4	0.28 (1)	3.92
AA-AB	-8750 / 0	-112.4	-112.4	0.28 (1)	3.92
AB- F	-8750 / 0	-112.4	-112.4	0.28 (1)	3.92
F-AC	-8750 / 0	-112.4	-112.4	0.29 (1)	3.92
AC- G	-8750 / 0	-112.4	-112.4	0.29 (1)	3.92
G-AD	-8750 / 0	-112.4	-112.4	0.29 (1)	3.92
AD-AE	-8750 / 0	-112.4	-112.4	0.29 (1)	3.92
AE- H	-8750 / 0	-112.4	-112.4	0.29 (1)	3.92
H-AF	-7872 / 0	-112.4	-112.4	0.26 (1)	4.12
AF-AG	-7872 / 0	-112.4	-112.4	0.26 (1)	4.12
AG- I	-7872 / 0	-112.4	-112.4	0.26 (1)	4.12
I- J	-5084 / 0	-112.4	-112.4	0.12 (1)	5.08
J- K	-4760 / 0	-112.4	-112.4	0.12 (1)	5.21
K- L	0 / 44	-112.4	-112.4	0.04 (1)	10.00
W- B	-3799 / 0	0.0	0.0	0.13 (1)	7.25
M- K	-3887 / 0	0.0	0.0	0.14 (1)	7.19
W-AH	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AH-AI	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AI- V	0 / 0	-28.0	-28.0	0.03 (3)	10.00
V- U	0 / 3826	-28.0	-28.0	0.12 (1)	10.00
U-AJ	0 / 5016	-28.0	-28.0	0.16 (1)	10.00
AJ-AK	0 / 5016	-28.0	-28.0	0.16 (1)	10.00
AK- T	0 / 5016	-28.0	-28.0	0.16 (1)	10.00
T-AL	0 / 7830	-28.0	-28.0	0.25 (1)	10.00
AL- S	0 / 7830	-28.0	-28.0	0.25 (1)	10.00
S-AM	0 / 7830	-28.0	-28.0	0.25 (1)	10.00
AM-AN	0 / 7830	-28.0	-28.0	0.25 (1)	10.00
AN- R	0 / 7830	-28.0	-28.0	0.25 (1)	10.00
R-AO	0 / 7872	-28.0	-28.0	0.25 (1)	10.00
AO-AP	0 / 7872	-28.0	-28.0	0.25 (1)	10.00
AP- Q	0 / 7872	-28.0	-28.0	0.25 (1)	10.00
Q- P	0 / 7872	-28.0	-28.0	0.25 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- C	-633 / 0	0.05 (1)
C- U	0 / 2992	0.26 (1)
U- D	-2606 / 0	0.22 (1)
O- I	-2544 / 0	0.22 (1)
O- J	0 / 2921	0.26 (1)
N- J	-656 / 0	0.06 (1)
B- V	0 / 3938	0.35 (1)
N- K	0 / 4039	0.36 (1)
P- I	0 / 3359	0.30 (1)
D- T	0 / 3393	0.30 (1)
P- H	-1612 / 0	0.14 (1)
T- E	-1633 / 0	0.14 (1)
R- H	0 / 1059	0.09 (1)
E- R	0 / 1109	0.10 (1)
R- F	-963 / 0	0.08 (1)

LICENSED PROFESSIONAL ENGINEER
41078
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18488-18
STRUCTURAL COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.29/1.00 (F-H:1) , BC=0.25/1.00 (P-R:1) , WB=0.36/1.00 (K-N:1) , SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (N) (INPUT = 0.90)
JSI METAL= 0.63 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296710	T31Z2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-cUmwcAjDX5vjRkp2f7eC_M7xKHL2ddnDNtoLNszSAKT

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	6.0		
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	5.0	6.0	2.50	2.75
W	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 238.0 lbs FACTORED DOWN AT 4-0-11, 379.3 lbs FACTORED DOWN AT 28-10-6, 135.3 lbs FACTORED DOWN AT 5-4-12, 135.3 lbs FACTORED DOWN AT 7-4-12, 135.3 lbs FACTORED DOWN AT 9-4-12, 135.3 lbs FACTORED DOWN AT 11-4-12, 135.3 lbs FACTORED DOWN AT 13-4-12, 135.3 lbs FACTORED DOWN AT 15-4-12, 135.3 lbs FACTORED DOWN AT 17-4-12, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, 135.3 lbs FACTORED DOWN AT 23-0-12, AND 135.3 lbs FACTORED DOWN AT 25-0-12, AND 135.3 lbs FACTORED DOWN AT 27-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-6-12, 69.9 lbs FACTORED DOWN AT 3-4-12, 69.9 lbs FACTORED DOWN AT 5-4-12, 69.9 lbs FACTORED DOWN AT 7-4-12, 69.9 lbs FACTORED DOWN AT 9-4-12, 69.9 lbs FACTORED DOWN AT 11-4-12, 69.9 lbs FACTORED DOWN AT 13-4-12, 69.9 lbs FACTORED DOWN AT 15-4-12, 69.9 lbs FACTORED DOWN AT 17-4-12, 69.9 lbs FACTORED DOWN AT 19-0-12, 69.9 lbs FACTORED DOWN AT 21-0-12, 69.9 lbs FACTORED DOWN AT 23-0-12, 69.9 lbs FACTORED DOWN AT 25-0-12, 69.9 lbs FACTORED DOWN AT 27-0-12, AND 69.9 lbs FACTORED DOWN AT 29-0-12, AND 69.9 lbs FACTORED DOWN AT 31-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-AQ	0 / 5086	-28.0	-28.0	0.17 (1)		10.00
AQ-AR	0 / 5086	-28.0	-28.0	0.17 (1)		10.00
AR- O	0 / 5086	-28.0	-28.0	0.17 (1)		10.00
O- N	0 / 3924	-28.0	-28.0	0.12 (1)		10.00
N-AS	0 / 0	-28.0	-28.0	0.03 (3)		10.00
AS-AT	0 / 0	-28.0	-28.0	0.03 (3)		10.00
AT- M	0 / 0	-28.0	-28.0	0.03 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-33	-36	—	FRONT	VERT	DEAD
C	4-0-11	-206	-206	—	FRONT	VERT	SNOW
D	5-4-12	-135	-135	—	FRONT	VERT	TOTAL
I	27-0-12	-135	-135	—	FRONT	VERT	TOTAL
J	28-10-6	-33	-36	—	FRONT	VERT	DEAD
J	28-10-6	-141	-141	—	FRONT	VERT	TOTAL
J	28-10-6	-206	-206	—	FRONT	VERT	SNOW
O	27-0-12	-40	-70	—	FRONT	VERT	TOTAL
Q	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
U	5-4-12	-40	-70	—	FRONT	VERT	TOTAL
X	7-4-12	-135	-135	—	FRONT	VERT	TOTAL
Y	9-4-12	-135	-135	—	FRONT	VERT	TOTAL
Z	11-4-12	-135	-135	—	FRONT	VERT	TOTAL
AA	13-4-12	-135	-135	—	FRONT	VERT	TOTAL
AB	15-4-12	-135	-135	—	FRONT	VERT	TOTAL
AC	17-4-12	-135	-135	—	FRONT	VERT	TOTAL
AD	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
AE	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
AF	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AG	25-0-12	-135	-135	—	FRONT	VERT	TOTAL
AH	1-6-12	-40	-70	—	FRONT	VERT	TOTAL
AI	3-4-12	-40	-70	—	FRONT	VERT	TOTAL
AJ	7-4-12	-40	-70	—	FRONT	VERT	TOTAL
AK	9-4-12	-40	-70	—	FRONT	VERT	TOTAL
AL	11-4-12	-40	-70	—	FRONT	VERT	TOTAL
AM	13-4-12	-40	-70	—	FRONT	VERT	TOTAL
AN	15-4-12	-40	-70	—	FRONT	VERT	TOTAL
AO	17-4-12	-40	-70	—	FRONT	VERT	TOTAL
AP	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
AQ	23-0-12	-40	-70	—	FRONT	VERT	TOTAL
AR	25-0-12	-40	-70	—	FRONT	VERT	TOTAL
AS	29-0-12	-40	-70	—	FRONT	VERT	TOTAL
AT	31-0-12	-40	-70	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4107B
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18400-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T31S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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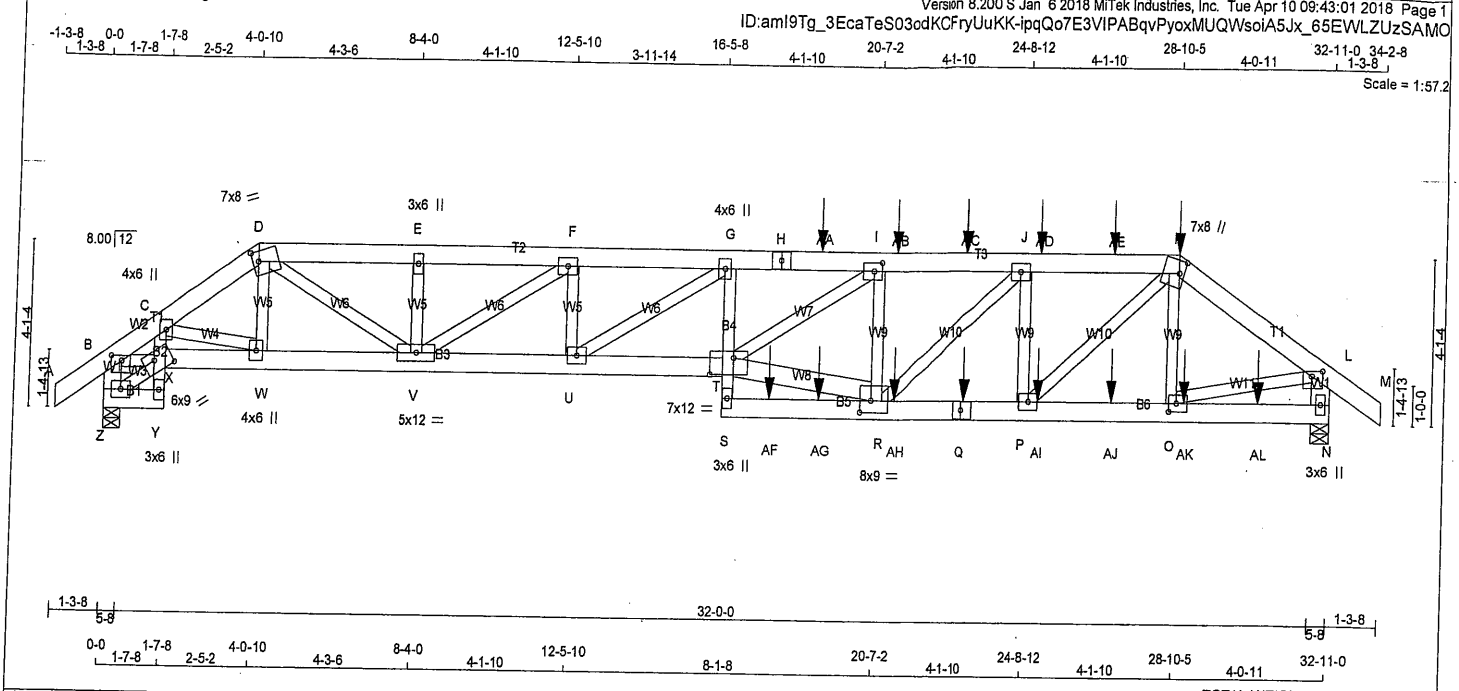

TOTAL WEIGHT = 2 X 201 = 402 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - Y	2x6	DRY	No.2	SPF
Y - C	2x4	DRY	No.2	SPF
X - T	2x6	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	TOP
D- H	2	12	TOP
H- K	2	12	SIDE(61.0)
K- M	2	12	SIDE(122.0)
Z- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- Y	2	12	TOP
X- T	2	12	TOP
S- Q	2	12	SIDE(0.0)
Q- N	2	12	SIDE(183.1)
Y- C	1	12	TOP
G- S	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.25
C	TMVW+p	MT20	4.0	6.0		
D	TTWW-m	MT20	7.0	8.0	3.00	1.75
E	TMW+w	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4431	0	4431	0	0	5-8	5-8
Z	3544	0	3544	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3494	2087 / 0	611 / 0	0 / 0	0 / 0	797 / 0	0 / 0
Z	2793	1673 / 0	486 / 0	0 / 0	0 / 0	634 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04 (1)	10.00
B- C	-5217 / 0	-112.4	-112.4	0.14 (1)	5.00
C- D	-5696 / 0	-112.4	-112.4	0.15 (1)	4.82
D- E	-8748 / 0	-112.4	-112.4	0.22 (1)	3.99
E- F	-8748 / 0	-112.4	-112.4	0.20 (1)	4.00
F- G	-11736 / 0	-112.4	-112.4	0.36 (1)	3.36
G- H	-13781 / 0	-112.4	-112.4	0.45 (1)	3.03
H-AA	-13781 / 0	-112.4	-112.4	0.45 (1)	3.03
AA- I	-13781 / 0	-112.4	-112.4	0.45 (1)	3.03
I-AB	-9696 / 0	-112.4	-112.4	0.24 (1)	3.80
AB-AC	-9696 / 0	-112.4	-112.4	0.24 (1)	3.80
AC- J	-9696 / 0	-112.4	-112.4	0.24 (1)	3.80
J-AD	-7589 / 0	-112.4	-112.4	0.22 (1)	4.23
AD-AE	-7589 / 0	-112.4	-112.4	0.22 (1)	4.23
AE- K	-7589 / 0	-112.4	-112.4	0.22 (1)	4.23
K- L	-5415 / 0	-112.4	-112.4	0.13 (1)	4.94
L- M	0 / 44	-112.4	-112.4	0.04 (1)	10.00
Z- B	-3421 / 0	0.0	0.0	0.12 (1)	7.54
N- L	-4368 / 0	0.0	0.0	0.15 (1)	6.87
Z- Y	0 / 176	-28.0	-28.0	0.01 (1)	10.00
Y- X	0 / 36	0.0	0.0	0.12 (1)	10.00
X- C	-396 / 0	0.0	0.0	0.11 (1)	7.81
X- W	0 / 4375	-28.0	-28.0	0.31 (1)	10.00
W- V	0 / 4785	-28.0	-28.0	0.35 (1)	10.00
V- U	0 / 11736	-28.0	-28.0	0.84 (1)	10.00
U- T	0 / 13898	-28.0	-28.0	0.97 (1)	10.00
S- T	0 / 1138	0.0	0.0	0.33 (1)	10.00
T- G	0 / 1040	0.0	0.0	0.32 (1)	10.00
S-AF	0 / 301	-28.0	-28.0	0.48 (1)	10.00
AF-AG	0 / 301	-28.0	-28.0	0.48 (1)	10.00
AG- R	0 / 301	-28.0	-28.0	0.48 (1)	10.00
R-AH	0 / 7589	-28.0	-28.0	0.65 (1)	10.00
AH- Q	0 / 7589	-28.0	-28.0	0.65 (1)	10.00
Q- P	0 / 7589	-28.0	-28.0	0.65 (1)	10.00
P-AI	0 / 4466	-28.0	-28.0	0.35 (1)	10.00
AI-AJ	0 / 4466	-28.0	-28.0	0.35 (1)	10.00
AJ- O	0 / 4466	-28.0	-28.0	0.35 (1)	10.00
O-AK	0 / 0	-28.0	-28.0	0.06 (3)	10.00
AK-AL	0 / 0	-28.0	-28.0	0.06 (3)	10.00
AL- N	0 / 0	-28.0	-28.0	0.06 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	0 / 423	0.04 (1)
W- D	0 / 289	0.03 (2)
T- R	0 / 9790	0.64 (1)
T- I	0 / 4764	0.42 (1)
R- I	-3425 / 0	0.29 (1)
R- J	0 / 2868	0.25 (1)
P- J	-2703 / 0	0.23 (1)
P- K	0 / 4121	0.36 (1)
O- K	-683 / 0	0.06 (1)
O- L	0 / 4595	0.41 (1)
U- G	-2566 / 0	0.35 (1)
D- V	0 / 4648	0.41 (1)
U- F	0 / 1381	0.12 (1)
V- E	-432 / 0	0.03 (1)
V- F	-3548 / 0	0.48 (1)
Z- X	-204 / 0	0.01 (1)
B- X	0 / 4230	0.37 (1)

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41016
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DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.35")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 685 (0.58")

CSI: TC=0.45/1.00 (G-I:1) , BC=0.97/1.00 (T-U:1) , WB=0.64/1.00 (R-T:1) , SSI=0.38/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 16U61-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T31S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-ipqQo7E3VIPABqvPyoxMUQWsoiA5Jx_65EWLZUzSAMO

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TTWW+m	MT20	7.0	8.0	3.75	1.50
L	TMVW-p	MT20	5.0	6.0	1.50	3.25
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	8.0	9.0	3.50	3.50
S	BMV+p	MT20	3.0	6.0		
T	BVMWW-t	MT20	7.0	12.0	5.00	7.50
U	BMWW-t	MT20	5.0	6.0		
V	BMWWW-t	MT20	5.0	12.0		
W	BMWW+t	MT20	4.0	6.0		
X	BVMWW+w	MT20	6.0	9.0	5.50	3.50
Y	BMV+p	MT20	3.0	6.0		
Z	BMVW1-t	MT20	5.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 467.1 lbs FACTORED DOWN AT 28-10-6, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, 135.3 lbs FACTORED DOWN AT 23-0-12, AND 135.3 lbs FACTORED DOWN AT 25-0-12, AND 135.3 lbs FACTORED DOWN AT 27-0-12 ON TOP CHORD, AND 1611.0 lbs FACTORED DOWN AT 17-8-12, 69.9 lbs FACTORED DOWN AT 19-0-12, 69.9 lbs FACTORED DOWN AT 21-0-12, 69.9 lbs FACTORED DOWN AT 23-0-12, 69.9 lbs FACTORED DOWN AT 25-0-12, 69.9 lbs FACTORED DOWN AT 27-0-12, AND 69.9 lbs FACTORED DOWN AT 29-0-12, AND 69.9 lbs FACTORED DOWN AT 31-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
K	28-10-6	-44	-49	—	FRONT	VERT	DEAD
K	28-10-6	-141	-141	—	FRONT	VERT	TOTAL
K	28-10-6	-282	-282	—	FRONT	VERT	SNOW
Q	23-0-12	-40	-70	—	FRONT	VERT	TOTAL
AA	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
AB	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
AC	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AD	25-0-12	-135	-135	—	FRONT	VERT	TOTAL
AE	27-0-12	-135	-135	—	FRONT	VERT	TOTAL
AF	17-8-12	-1611	-1611	—	FRONT	VERT	TOTAL
AG	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
AH	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
AI	25-0-12	-40	-70	—	FRONT	VERT	TOTAL
AJ	27-0-12	-40	-70	—	FRONT	VERT	TOTAL
AK	29-0-12	-40	-70	—	FRONT	VERT	TOTAL
AL	31-0-12	-40	-70	—	FRONT	VERT	TOTAL

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.78 (T) (INPUT = 1.00)

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41018
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DWG NO. TAM 18461-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287658	T31S2	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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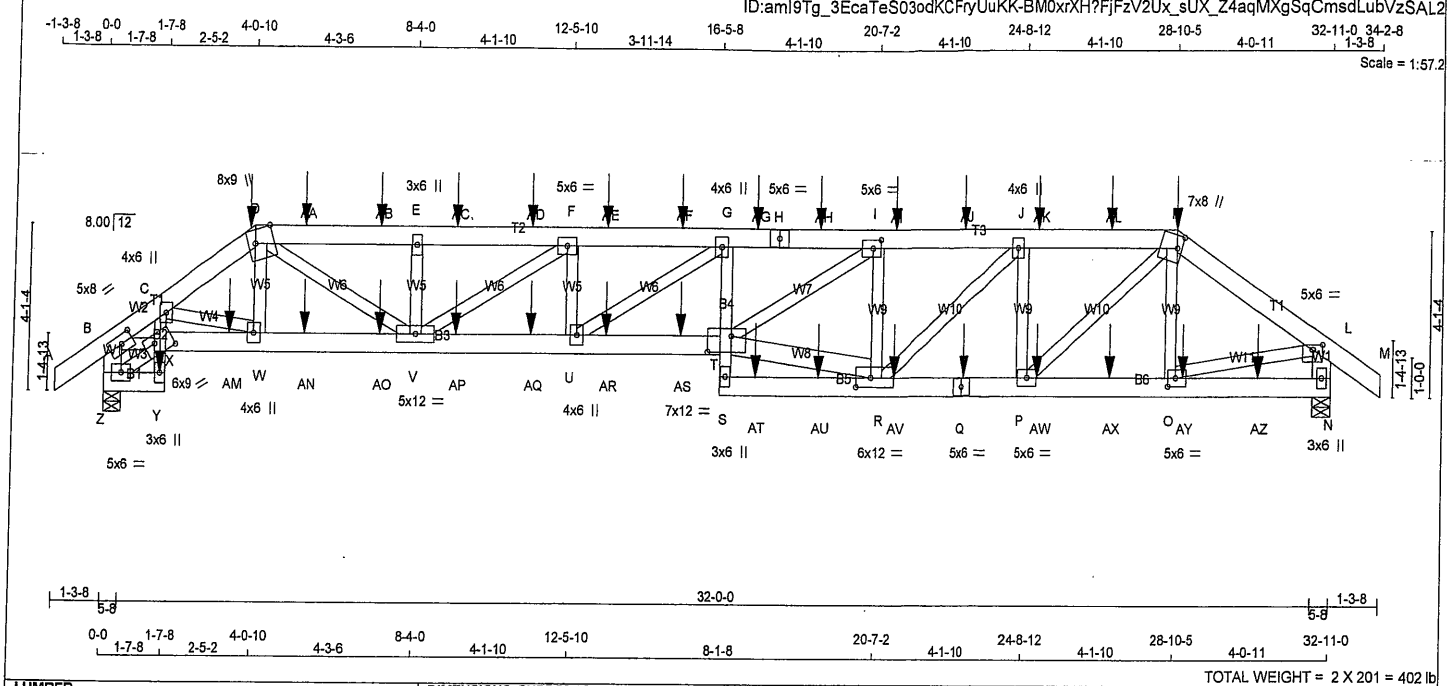

TOTAL WEIGHT = 2 X 201 = 402 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
D - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
K - M	2x6	DRY	No.2	SPF
Z - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - Y	2x6	DRY	No.2	SPF
Y - C	2x4	DRY	No.2	SPF
X - T	2x6	DRY	No.2	SPF
S - G	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
T - R	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	12	SIDE(122.0)
D- H	2	12	SIDE(61.0)
H- K	2	12	SIDE(61.0)
K- M	2	12	SIDE(122.0)
Z- B	2	12	TOP
N- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- Y	2	12	SIDE(122.0)
X- T	2	12	SIDE(0.0)
S- Q	2	12	SIDE(0.0)
Q- N	2	12	SIDE(183.1)
Y- C	1	12	TOP
G- S	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TMVW+p	MT20	4.0	6.0		
D	TTWW+m	MT20	8.0	9.0	Edge	5.75
E	TMW+w	MT20	3.0	6.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMVW+p	MT20	4.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.50
J	TMWW+t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	4195	0	4195	0	0	5-8	5-8
Z	4372	0	4372	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	3328	1955 / 0	604 / 0	0 / 0	0 / 0	768 / 0	0 / 0
Z	3478	2030 / 0	644 / 0	0 / 0	0 / 0	805 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, Z

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI	MAX (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.04	(1)	10.00
B- C	-6543 / 0	-112.4	-112.4	0.19	(1)	4.50
C- D	-7143 / 0	-112.4	-112.4	0.20	(1)	4.33
D-AA	-10154 / 0	-112.4	-112.4	0.31	(1)	3.64
AA-AB	-10154 / 0	-112.4	-112.4	0.31	(1)	3.64
AB- E	-10154 / 0	-112.4	-112.4	0.31	(1)	3.64
E-AC	-10154 / 0	-112.4	-112.4	0.27	(1)	3.68
AC-AD	-10154 / 0	-112.4	-112.4	0.27	(1)	3.68
AD- F	-10154 / 0	-112.4	-112.4	0.27	(1)	3.68
F-AE	-12465 / 0	-112.4	-112.4	0.41	(1)	3.22
AE-AF	-12465 / 0	-112.4	-112.4	0.41	(1)	3.22
AF- G	-12465 / 0	-112.4	-112.4	0.41	(1)	3.22
G-AG	-13010 / 0	-112.4	-112.4	0.41	(1)	3.16
AG- H	-13010 / 0	-112.4	-112.4	0.41	(1)	3.16
H-AH	-13010 / 0	-112.4	-112.4	0.41	(1)	3.16
AH- I	-13010 / 0	-112.4	-112.4	0.41	(1)	3.16
I-AI	-8796 / 0	-112.4	-112.4	0.21	(1)	3.99
AI-AJ	-8796 / 0	-112.4	-112.4	0.21	(1)	3.99
AJ- J	-8796 / 0	-112.4	-112.4	0.21	(1)	3.99
J-AK	-7083 / 0	-112.4	-112.4	0.20	(1)	4.36
AK-AL	-7083 / 0	-112.4	-112.4	0.20	(1)	4.36
AL- K	-7083 / 0	-112.4	-112.4	0.20	(1)	4.36
K- L	-5085 / 0	-112.4	-112.4	0.12	(1)	5.07
L- M	0 / 44	-112.4	-112.4	0.04	(1)	10.00
Z- B	-4223 / 0	0.0	0.0	0.15	(1)	6.96
N- L	-4125 / 0	0.0	0.0	0.15	(1)	7.02
Z- Y	0 / 222	-28.0	-28.0	0.02	(1)	10.00
Y- X	0 / 129	0.0	0.0	0.15	(1)	10.00
X- C	-439 / 0	0.0	0.0	0.14	(1)	7.81
X-AM	0 / 5493	-28.0	-28.0	0.41	(1)	10.00
AM- W	0 / 5493	-28.0	-28.0	0.41	(1)	10.00
W-AN	0 / 6013	-28.0	-28.0	0.46	(1)	10.00
AN-AO	0 / 6013	-28.0	-28.0	0.46	(1)	10.00
AO- V	0 / 6013	-28.0	-28.0	0.46	(1)	10.00
V-AP	0 / 12465	-28.0	-28.0	0.90	(1)	10.00
AP-AQ	0 / 12465	-28.0	-28.0	0.90	(1)	10.00
AQ- U	0 / 12465	-28.0	-28.0	0.90	(1)	10.00
U-AR	0 / 13120	-28.0	-28.0	0.94	(1)	10.00
AR-AS	0 / 13120	-28.0	-28.0	0.94	(1)	10.00
AS- T	0 / 13120	-28.0	-28.0	0.94	(1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	0 / 529	0.05 (1)
W- D	0 / 520	0.05 (2)
T- R	0 / 8881	0.58 (1)
T- I	0 / 4916	0.43 (1)
R- I	-3510 / 0	0.30 (1)
R- J	0 / 2331	0.21 (1)
P- J	-2337 / 0	0.20 (1)
P- K	0 / 3814	0.34 (1)
O- K	-663 / 0	0.06 (1)
O- L	0 / 4315	0.38 (1)
U- G	-778 / 0	0.11 (1)
D- V	0 / 4856	0.43 (1)
U- F	0 / 697	0.06 (1)
V- E	-739 / 0	0.05 (1)
V- F	-2744 / 0	0.38 (1)
Z- X	-256 / 0	0.01 (1)
R- X	0 / 5296	0.47 (1)

LICENSED PROFESSIONAL ENGINEER
4/08
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18487-B
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 706 (0.56")

CSI: TC=0.41/1.00 (F-G:1) , BC=0.94/1.00 (T-U:1) , WB=0.58/1.00 (R-T:1) , SSI=0.15/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287658	T31S2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-fYaJ2sld01Nq7C37YZ?mWmdlamtvBHRw5G4R7xzSAL1

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
K	TTWW+m	MT20	7.0	8.0	3.75	1.50
L	TMVW-p	MT20	5.0	6.0	1.50	3.00
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	6.0	12.0	2.75	4.75
S	BMV+p	MT20	3.0	6.0		
T	BVMWW-l	MT20	7.0	12.0	4.75	7.50
U	BMWW+t	MT20	4.0	6.0		
V	BMWWW-t	MT20	5.0	12.0		
W	BMWW+t	MT20	4.0	6.0		
X	BVMWW+w	MT20	6.0	9.0	5.75	3.50
Y	BMV+p	MT20	3.0	6.0		
Z	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 467.1 lbs FACTORED DOWN AT 28-10-6, 325.8 lbs FACTORED DOWN AT 4-0-11, 135.5 lbs FACTORED DOWN AT 5-4-12, 135.5 lbs FACTORED DOWN AT 7-4-12, 135.5 lbs FACTORED DOWN AT 9-4-12, 135.5 lbs FACTORED DOWN AT 11-4-12, 135.5 lbs FACTORED DOWN AT 13-4-12, 135.5 lbs FACTORED DOWN AT 15-4-12, 135.3 lbs FACTORED DOWN AT 17-4-12, 135.3 lbs FACTORED DOWN AT 19-0-12, 135.3 lbs FACTORED DOWN AT 21-0-12, 135.3 lbs FACTORED DOWN AT 23-0-12, AND 135.3 lbs FACTORED DOWN AT 25-0-12, AND 135.3 lbs FACTORED DOWN AT 27-0-12 ON TOP CHORD, AND 92.4 lbs FACTORED DOWN AT 1-5-12, 119.0 lbs FACTORED DOWN AT 3-4-12, 119.0 lbs FACTORED DOWN AT 5-4-12, 119.0 lbs FACTORED DOWN AT 7-4-12, 119.0 lbs FACTORED DOWN AT 9-4-12, 119.0 lbs FACTORED DOWN AT 11-4-12, 119.0 lbs FACTORED DOWN AT 13-4-12, 119.0 lbs FACTORED DOWN AT 15-4-12, 75.0 lbs FACTORED DOWN AT 17-4-12, 69.9 lbs FACTORED DOWN AT 19-0-12, 69.9 lbs FACTORED DOWN AT 21-0-12, 69.9 lbs FACTORED DOWN AT 23-0-12, 69.9 lbs FACTORED DOWN AT 25-0-12, 69.9 lbs FACTORED DOWN AT 27-0-12, AND 69.9 lbs FACTORED DOWN AT 29-0-12, AND 69.9 lbs FACTORED DOWN AT 31-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
S- T	0 / 165	0.0	0.0	0.23 (1)	10.00
T- G	-210 / 113	0.0	0.0	0.22 (1)	7.81
S-AT	0 / 285	-28.0	-28.0	0.07 (2)	10.00
AT-AU	0 / 285	-28.0	-28.0	0.07 (2)	10.00
AU- R	0 / 285	-28.0	-28.0	0.07 (2)	10.00
R-AV	0 / 7083	-28.0	-28.0	0.50 (1)	10.00
AV- Q	0 / 7083	-28.0	-28.0	0.50 (1)	10.00
Q- P	0 / 7083	-28.0	-28.0	0.50 (1)	10.00
P-AW	0 / 4193	-28.0	-28.0	0.31 (1)	10.00
AW-AX	0 / 4193	-28.0	-28.0	0.31 (1)	10.00
AX- O	0 / 4193	-28.0	-28.0	0.31 (1)	10.00
O-AY	0 / 0	-28.0	-28.0	0.05 (3)	10.00
AY-AZ	0 / 0	-28.0	-28.0	0.05 (3)	10.00
AZ- N	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-0-11	-44	-49	—	FRONT	VERT	DEAD
D	4-0-11	-282	-282	—	FRONT	VERT	SNOW
K	28-10-6	-44	-49	—	FRONT	VERT	DEAD
K	28-10-6	-141	-141	—	FRONT	VERT	TOTAL
K	28-10-6	-282	-282	—	FRONT	VERT	SNOW
Q	23-0-12	-40	-70	—	FRONT	VERT	TOTAL
Y	1-5-12	-53	-92	—	FRONT	VERT	TOTAL
AA	5-4-12	-135	-135	—	FRONT	VERT	TOTAL
AB	7-4-12	-135	-135	—	FRONT	VERT	TOTAL
AC	9-4-12	-135	-135	—	FRONT	VERT	TOTAL
AD	11-4-12	-135	-135	—	FRONT	VERT	TOTAL
AE	13-4-12	-135	-135	—	FRONT	VERT	TOTAL
AF	15-4-12	-135	-135	—	FRONT	VERT	TOTAL
AG	17-4-12	-135	-135	—	FRONT	VERT	TOTAL
AH	19-0-12	-135	-135	—	FRONT	VERT	TOTAL
AI	21-0-12	-135	-135	—	FRONT	VERT	TOTAL
AJ	23-0-12	-135	-135	—	FRONT	VERT	TOTAL
AK	25-0-12	-135	-135	—	FRONT	VERT	TOTAL
AL	27-0-12	-135	-135	—	FRONT	VERT	TOTAL
AM	3-4-12	-119	-119	—	FRONT	VERT	TOTAL
AN	5-4-12	-119	-119	—	FRONT	VERT	TOTAL
AO	7-4-12	-119	-119	—	FRONT	VERT	TOTAL
AP	9-4-12	-119	-119	—	FRONT	VERT	TOTAL
AQ	11-4-12	-119	-119	—	FRONT	VERT	TOTAL
AR	13-4-12	-119	-119	—	FRONT	VERT	TOTAL
AS	15-4-12	-119	-119	—	FRONT	VERT	TOTAL
AT	17-4-12	-43	-75	—	FRONT	VERT	TOTAL
AU	19-0-12	-40	-70	—	FRONT	VERT	TOTAL
AV	21-0-12	-40	-70	—	FRONT	VERT	TOTAL
AW	25-0-12	-40	-70	—	FRONT	VERT	TOTAL
AX	27-0-12	-40	-70	—	FRONT	VERT	TOTAL
AY	29-0-12	-40	-70	—	FRONT	VERT	TOTAL
AZ	31-0-12	-40	-70	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.77 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/07B
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PROVINCE OF ONTARIO

P62

DWG NO. TAM 18487-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T32	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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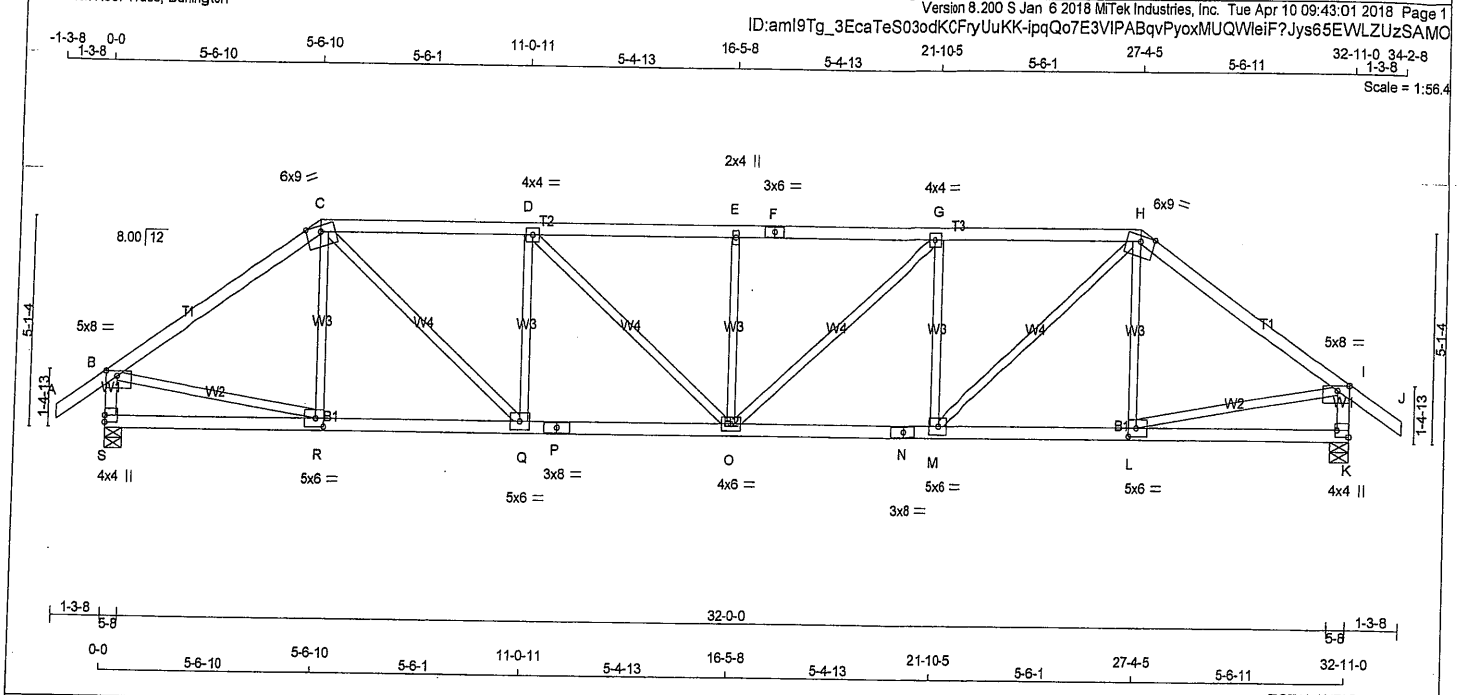

TOTAL WEIGHT = 133 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW-m	MT20	6.0	9.0	Edge	
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-m	MT20	6.0	9.0	Edge	
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.50
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.50
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2466	0	2466	0	0	5-8	5-8
K	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2707 / 0	-112.4	-112.4	0.91 (1)	3.22
C- D	-3607 / 0	-112.4	-112.4	0.80 (1)	2.95
D- E	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
E- F	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
F- G	-4007 / 0	-112.4	-112.4	0.86 (1)	2.75
G- H	-3607 / 0	-112.4	-112.4	0.80 (1)	2.95
H- I	-2707 / 0	-112.4	-112.4	0.91 (1)	3.22
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00
S- B	-2401 / 0	0.0	0.0	0.25 (1)	5.52
K- I	-2401 / 0	0.0	0.0	0.25 (1)	5.52
S- R	0 / 0	-28.0	-28.0	0.22 (3)	10.00
R- Q	0 / 2244	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
P O	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
O- N	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
N- M	0 / 3607	-28.0	-28.0	0.65 (1)	10.00
M- L	0 / 2244	-28.0	-28.0	0.48 (1)	10.00
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-252 / 109	0.10 (1)
L- H	-252 / 109	0.10 (1)
B- R	0 / 2290	0.52 (1)
L- I	0 / 2290	0.52 (1)
M- H	0 / 1809	0.41 (1)
C- Q	0 / 1809	0.41 (1)
M- G	-1045 / 0	0.40 (1)
Q- D	-1045 / 0	0.40 (1)
O- G	0 / 535	0.12 (1)
D- O	0 / 535	0.12 (1)
O- E	-560 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 990 (0.40")

CSI: TC=0.91/1.00 (H-I:1) , BC=0.65/1.00 (M-O:1) , WB=0.52/1.00 (I-L:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (R) (INPUT = 0.90)
USI METAL= 0.90 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4176
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 8462-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T32S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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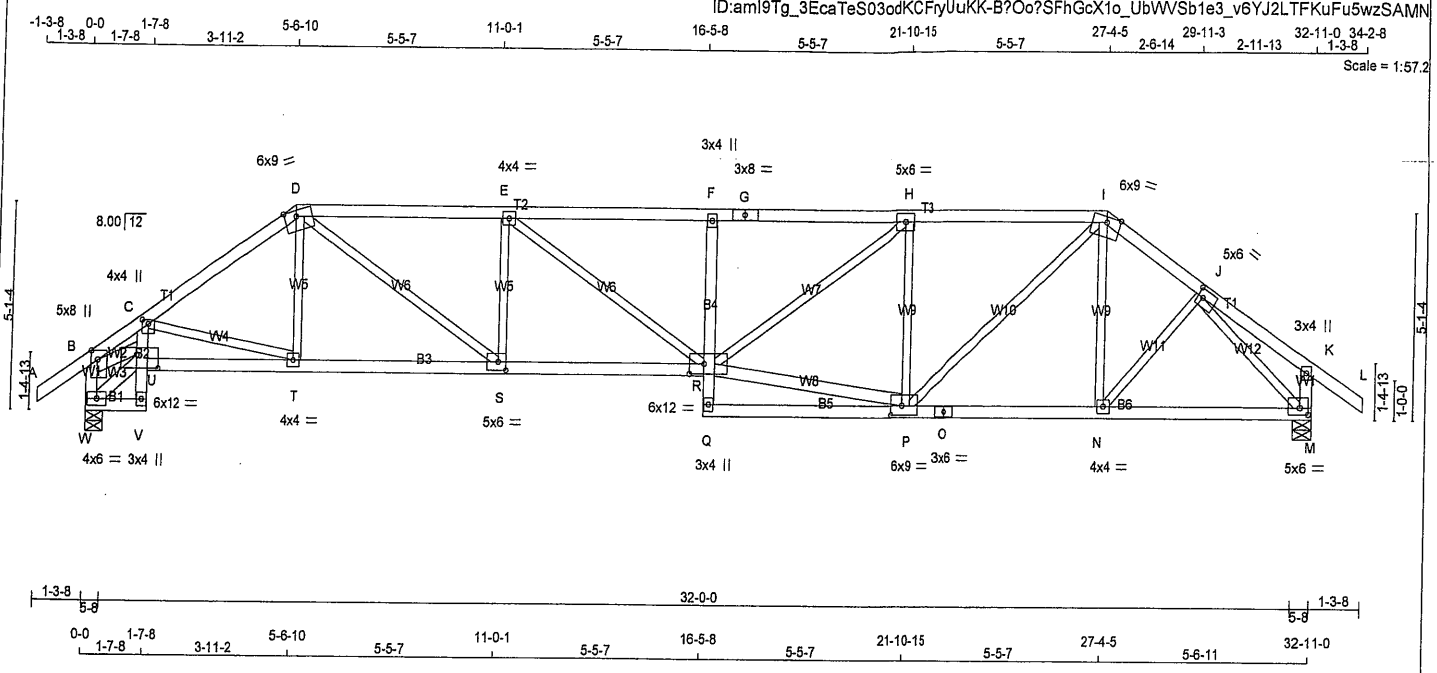

TOTAL WEIGHT = 143 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
W - U	2x4	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	2.75	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	6.0	9.0	1.75	4.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW-m	MT20	6.0	9.0	Edge	
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	6.0	9.0	2.75	3.75
Q	BMV+p	MT20	3.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	3.00	4.50
S	BMWW-t	MT20	5.0	6.0	2.50	2.50
T	BMWW-t	MT20	4.0	4.0		
U	BVMWW-l	MT20	6.0	12.0	4.00	7.00
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2462	0	2462	0	0	5-8	5-8
W	2469	0	2469	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
W	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-3530 / 0	-112.4	-112.4	0.27 (1)	3.51
C- D	-3382 / 0	-112.4	-112.4	0.47 (1)	3.44
D- E	-4549 / 0	-112.4	-112.4	0.65 (1)	3.30
E- F	-5026 / 0	-112.4	-112.4	0.69 (1)	3.12
F- G	-4989 / 0	-112.4	-112.4	0.66 (1)	3.15
G- H	-4989 / 0	-112.4	-112.4	0.66 (1)	3.15
H- I	-3576 / 0	-112.4	-112.4	0.55 (1)	3.74
I- J	-2713 / 0	-112.4	-112.4	0.18 (1)	4.07
J- K	0 / 13	-112.4	-112.4	0.12 (1)	10.00
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
W- B	-2363 / 0	0.0	0.0	0.24 (1)	5.55
M- K	-291 / 0	0.0	0.0	0.03 (1)	7.81
W- V	0 / 126	-28.0	-28.0	0.03 (2)	10.00
V- U	0 / 36	0.0	0.0	0.17 (1)	10.00
U- C	-179 / 4	0.0	0.0	0.17 (1)	7.81
U- T	0 / 3049	-28.0	-28.0	0.58 (1)	10.00
T- S	0 / 2805	-28.0	-28.0	0.54 (1)	10.00
S- R	0 / 4549	-28.0	-28.0	0.84 (1)	10.00
Q- R	0 / 108	0.0	0.0	0.20 (1)	10.00
R- F	-554 / 0	0.0	0.0	0.23 (1)	7.81
Q- P	0 / 143	-28.0	-28.0	0.20 (3)	10.00
P- O	0 / 2239	-28.0	-28.0	0.47 (1)	10.00
O- N	0 / 2239	-28.0	-28.0	0.47 (1)	10.00
N- M	0 / 2018	-28.0	-28.0	0.43 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-258 / 0	0.08 (1)
T- D	0 / 290	0.07 (2)
D- S	0 / 2112	0.48 (1)
S- E	-1029 / 0	0.26 (1)
E- R	0 / 581	0.13 (1)
P- H	-1678 / 0	0.65 (1)
P- I	0 / 1793	0.40 (1)
N- I	-69 / 120	0.03 (3)
W- U	-152 / 0	0.02 (1)
B- U	0 / 2935	0.47 (1)
R- P	0 / 3519	0.57 (1)
R- H	0 / 1706	0.38 (1)
N- J	0 / 325	0.07 (1)
J- M	-2916 / 0	0.81 (1)

LICENSED PROFESSIONAL ENGINEER
4/07
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 724 (0.55")

CSI: TC=0.69/1.00 (E-F:1) , BC=0.84/1.00 (R-S:1) , WB=0.81/1.00 (J-M:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.89 (J) (INPUT = 0.90)
CSI METAL= 0.72 (J) (INPUT = 1.00)

DWG NO. TAM 18463-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T33	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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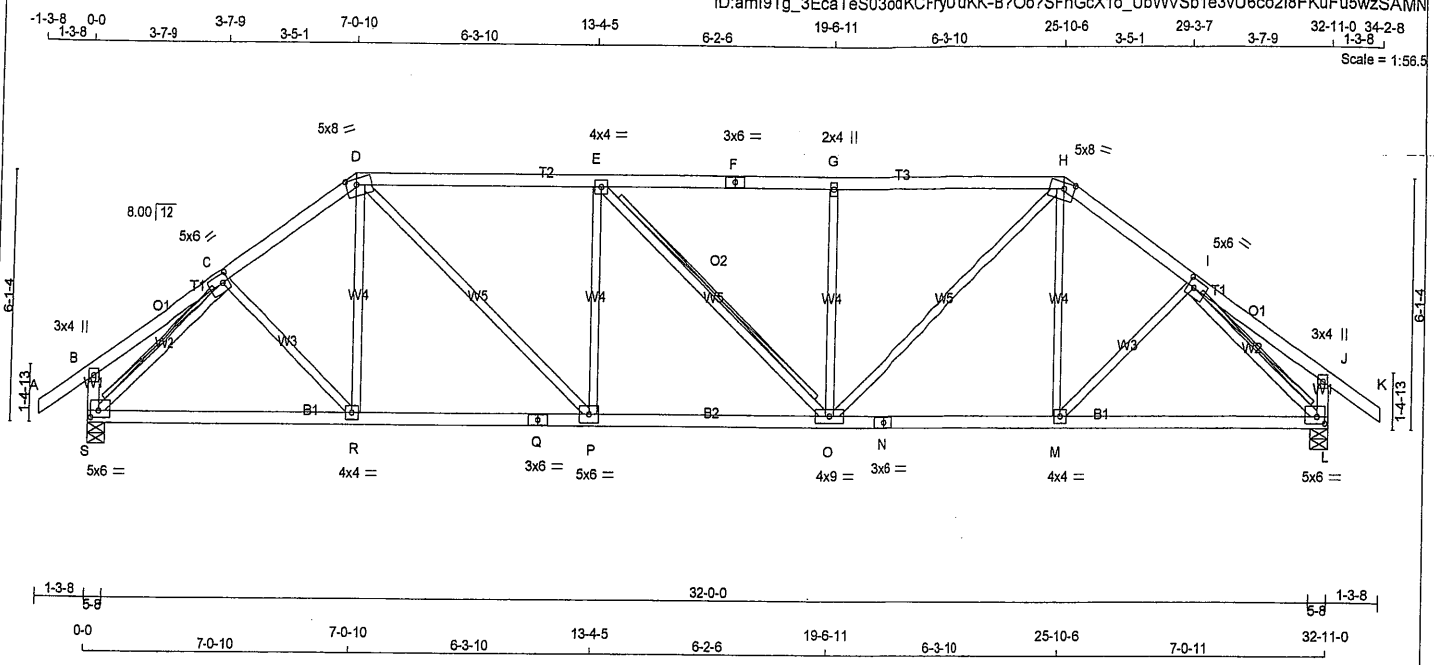

TOTAL WEIGHT = 137 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	1.75	3.25
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.00	2.50
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.00	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2466	0	2466	0	0	5-8	5-8
L	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
L	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-O, C-S, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 22	-112.4	-112.4	0.19 (1)	10.00
C- D	-2709 / 0	-112.4	-112.4	0.30 (1)	3.96
D- E	-3241 / 0	-112.4	-112.4	0.97 (1)	2.89
E- F	-3240 / 0	-112.4	-112.4	0.96 (1)	2.89
F- G	-3240 / 0	-112.4	-112.4	0.96 (1)	2.89
G- H	-3240 / 0	-112.4	-112.4	0.96 (1)	2.91
H- I	-2710 / 0	-112.4	-112.4	0.30 (1)	3.96
I- J	0 / 22	-112.4	-112.4	0.19 (1)	10.00
J- K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
S- B	-313 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-313 / 0	0.0	0.0	0.03 (1)	7.81
S- R	0 / 2127	-28.0	-28.0	0.55 (2)	10.00
R- Q	0 / 2234	-28.0	-28.0	0.57 (2)	10.00
Q- P	0 / 2234	-28.0	-28.0	0.57 (2)	10.00
P- O	0 / 3241	-28.0	-28.0	0.62 (1)	10.00
O- N	0 / 2234	-28.0	-28.0	0.58 (2)	10.00
N- M	0 / 2234	-28.0	-28.0	0.58 (2)	10.00
M- L	0 / 2127	-28.0	-28.0	0.55 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 198	0.04 (2)
R- D	0 / 250	0.06 (3)
D- P	0 / 1369	0.31 (1)
P- E	-762 / 0	0.45 (1)
E- O	-2 / 0	0.00 (1)
O- G	-762 / 0	0.45 (1)
O- H	0 / 1366	0.31 (1)
M- H	0 / 250	0.06 (3)
M- I	0 / 198	0.04 (2)
S- C	-2969 / 0	0.96 (1)
I- L	-2969 / 0	0.96 (1)

LICENSED PROFESSIONAL ENGINEER
41078
S KATSOULAROS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.33")

CSI: TC=0.97/1.00 (D-E:1) , BC=0.62/1.00 (O-P:1) , WB=0.96/1.00 (I-L:1) , SSI=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.74 (I) (INPUT = 1.00)

DWG NO. TAM 18464-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T33S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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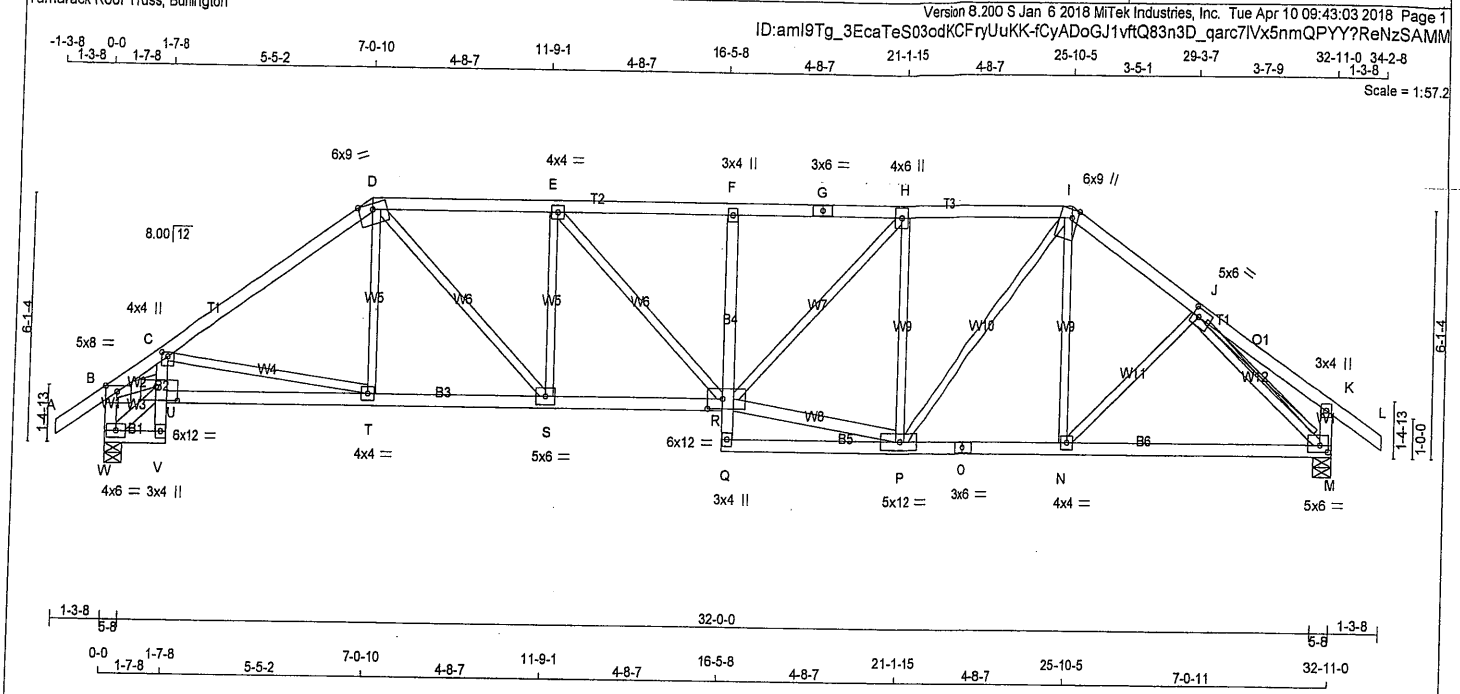

TOTAL WEIGHT = 150 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
Q - F	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
B - U	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	6.0	9.0	Edge	
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	2.50
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	5.0	12.0		
Q	BMV+p	MT20	3.0	4.0		
R	BVMWWW-l	MT20	6.0	12.0	3.00	4.75
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BVMWW-l	MT20	6.0	12.0	4.00	7.00
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2462	0	2462	0	0	5-8	5-8
W	2469	0	2469	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
W	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-3653 / 0	-112.4	-112.4	0.49 (1)	3.14
C- D	-3233 / 0	-112.4	-112.4	0.81 (1)	3.23
D- E	-3691 / 0	-112.4	-112.4	0.62 (1)	3.12
E- F	-3971 / 0	-112.4	-112.4	0.65 (1)	2.98
F- G	-3953 / 0	-112.4	-112.4	0.61 (1)	3.02
G- H	-3953 / 0	-112.4	-112.4	0.61 (1)	3.02
H- I	-3036 / 0	-112.4	-112.4	0.51 (1)	3.52
I- J	-2702 / 0	-112.4	-112.4	0.30 (1)	3.96
J- K	0 / 22	-112.4	-112.4	0.19 (1)	10.00
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
W- B	-2360 / 0	0.0	0.0	0.24 (1)	5.56
M- K	-312 / 0	0.0	0.0	0.03 (1)	7.81
W- V	0 / 131	-28.0	-28.0	0.03 (2)	10.00
V- U	0 / 36	0.0	0.0	0.18 (1)	10.00
U- C	-169 / 39	0.0	0.0	0.18 (1)	7.81
U- T	0 / 3210	-28.0	-28.0	0.63 (1)	10.00
T- S	0 / 2671	-28.0	-28.0	0.54 (1)	10.00
S- R	0 / 3692	-28.0	-28.0	0.68 (1)	10.00
Q- R	0 / 98	0.0	0.0	0.13 (1)	10.00
R- F	-477 / 0	0.0	0.0	0.18 (1)	7.81
Q- P	0 / 89	-28.0	-28.0	0.15 (3)	10.00
P- O	0 / 2227	-28.0	-28.0	0.55 (2)	10.00
O- N	0 / 2227	-28.0	-28.0	0.55 (2)	10.00
N- M	0 / 2122	-28.0	-28.0	0.52 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	-554 / 0	0.32 (1)
T- D	0 / 347	0.08 (2)
D- S	0 / 1442	0.32 (1)
S- E	-887 / 0	0.34 (1)
E- R	0 / 399	0.09 (1)
P- H	-1535 / 0	0.90 (1)
P- I	0 / 1295	0.29 (1)
N- I	0 / 223	0.05 (3)
N- J	0 / 197	0.04 (2)
J- M	-2962 / 0	0.96 (1)
W- U	-158 / 0	0.02 (1)
B- U	0 / 3092	0.50 (1)
R- P	0 / 3033	0.49 (1)
R- H	0 / 1308	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 983 (0.40")

CSI: TC=0.81/1.00 (C-D:1) , BC=0.68/1.00 (R-S:1) , WB=0.96/1.00 (J-M:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.74 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/07/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18465-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T34	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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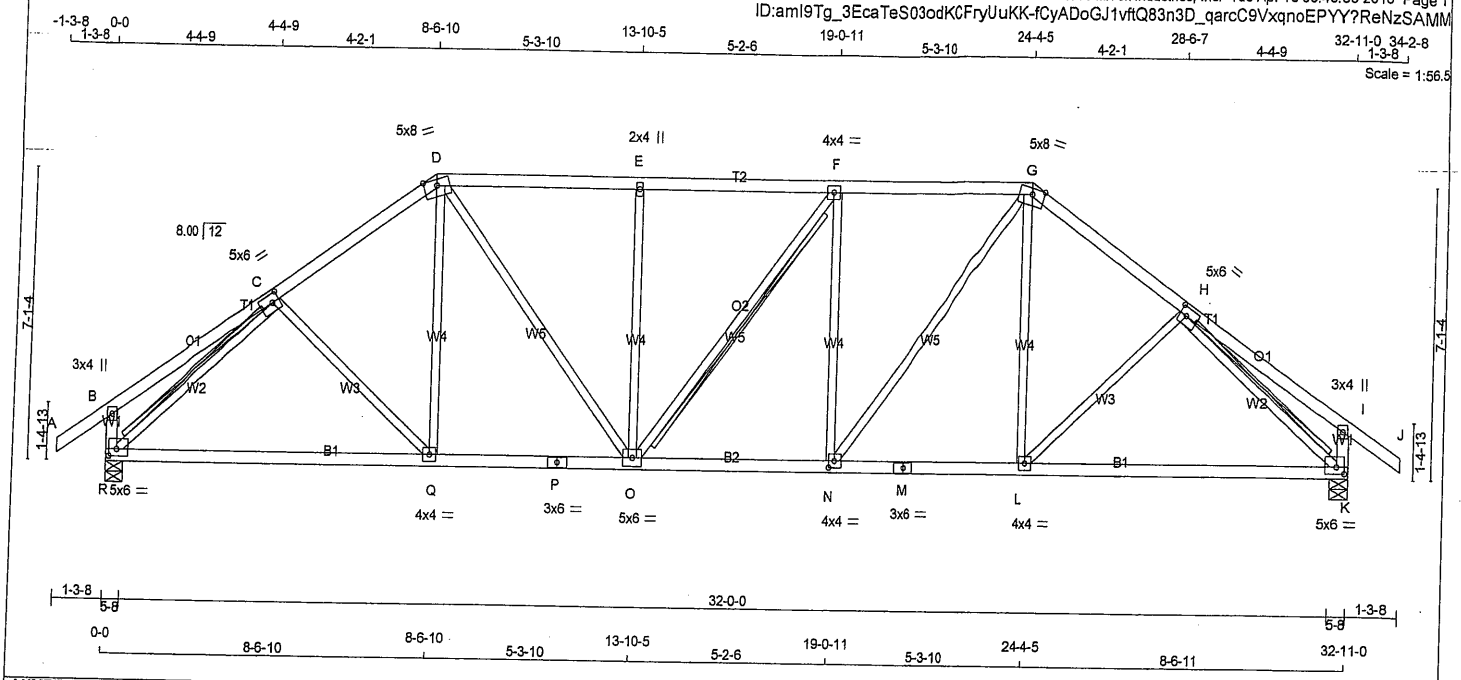

TOTAL WEIGHT = 144 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	
H	TMWW-t	MT20	5.0	6.0	2.50	2.25
I	TMV+p	MT20	3.0	4.0		
K	BMVW1-t	MT20	5.0	6.0	2.00	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWW-t	MT20	4.0	4.0	2.00	1.75
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
K	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
K	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-R, H-K
2x3 DRY SPF No.2 T-BRACE AT F-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 29	-112.4	-112.4	0.31 (1)	10.00
C- D	-2650 / 0	-112.4	-112.4	0.34 (1)	3.97
D- E	-2769 / 0	-112.4	-112.4	0.46 (1)	3.74
E- F	-2769 / 0	-112.4	-112.4	0.43 (1)	3.78
F- G	-2771 / 0	-112.4	-112.4	0.46 (1)	3.74
G- H	-2649 / 0	-112.4	-112.4	0.34 (1)	3.97
H- I	0 / 29	-112.4	-112.4	0.31 (1)	10.00
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00
R- B	-342 / 0	0.0	0.0	0.04 (1)	7.81
K- I	-342 / 0	0.0	0.0	0.04 (1)	7.81
R- Q	0 / 2200	-28.0	-28.0	0.68 (2)	10.00
Q- P	0 / 2181	-28.0	-28.0	0.69 (2)	10.00
P- O	0 / 2181	-28.0	-28.0	0.69 (2)	10.00
O- N	0 / 2771	-28.0	-28.0	0.54 (1)	10.00
N- M	0 / 2181	-28.0	-28.0	0.69 (2)	10.00
M- L	0 / 2181	-28.0	-28.0	0.69 (2)	10.00
L- K	0 / 2200	-28.0	-28.0	0.68 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- Q	-32 / 135	0.03 (3)
Q- D	0 / 362	0.08 (2)
L- G	0 / 361	0.08 (2)
L- H	-32 / 135	0.03 (3)
R- C	-2990 / 0	0.78 (1)
H- K	-2989 / 0	0.78 (1)
N- G	0 / 960	0.22 (1)
D- O	0 / 956	0.22 (1)
N- F	-638 / 0	0.56 (1)
O- E	-637 / 0	0.56 (1)
O- F	-3 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
LICENCE 41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 952 (0.41")

CSI: TC=0.46/1.00 (D-E:1) , BC=0.69/1.00 (O-Q:2) , WB=0.78/1.00 (C-R:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (N) (INPUT = 0.90)
JSI METAL= 0.74 (C) (INPUT = 1.00)

DWG NO. TAM 19466-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T34S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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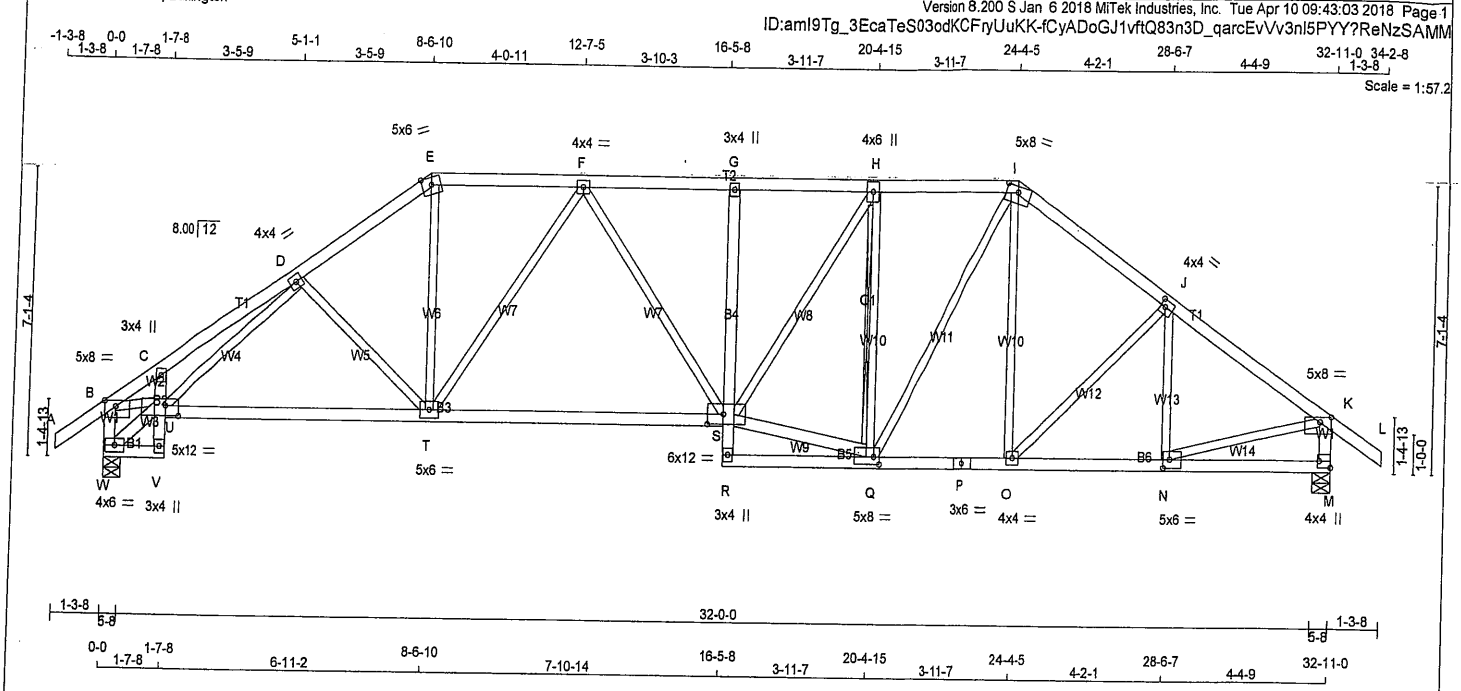

TOTAL WEIGHT = 157 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
R - G	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TMWW+t	MT20	4.0	6.0		
I	TTWW-m	MT20	5.0	8.0	1.75	4.00
J	TMWW-t	MT20	4.0	4.0	2.00	1.50
K	TMVW-p	MT20	5.0	8.0	1.50	3.50
M	BMV1+p	MT20	4.0	4.0	2.00	Edge
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.25	1.75
R	BMV+p	MT20	3.0	4.0		
S	BVMWWW-l	MT20	6.0	12.0	3.00	5.00
T	BMWWW-t	MT20	5.0	6.0		
U	BVMWWW-l	MT20	5.0	12.0	3.00	4.25
V	BMV+p	MT20	3.0	4.0		
W	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2462	0	2462	0	0	5-8	5-8
W	2469	0	2469	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
W	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT H-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-3420 / 0	-112.4	-112.4	0.23 (1)	3.63
C- D	-3473 / 0	-112.4	-112.4	0.33 (1)	3.53
D- E	-3079 / 0	-112.4	-112.4	0.29 (1)	3.76
E- F	-2559 / 0	-112.4	-112.4	0.28 (1)	4.09
F- G	-3280 / 0	-112.4	-112.4	0.32 (1)	3.63
G- H	-3269 / 0	-112.4	-112.4	0.31 (1)	3.64
H- I	-2649 / 0	-112.4	-112.4	0.27 (1)	4.03
I- J	-2621 / 0	-112.4	-112.4	0.34 (1)	3.98
J- K	-2683 / 0	-112.4	-112.4	0.35 (1)	3.94
K- L	0 / 43	-112.4	-112.4	0.15 (1)	10.00
W- B	-2355 / 0	0.0	0.0	0.24 (1)	5.57
M- K	-2408 / 0	0.0	0.0	0.25 (1)	5.51
W- V	0 / 137	-28.0	-28.0	0.03 (2)	10.00
V- U	0 / 36	0.0	0.0	0.19 (1)	10.00
U- C	-184 / 0	0.0	0.0	0.14 (1)	7.81
U- T	0 / 2870	-28.0	-28.0	0.78 (2)	10.00
T- S	0 / 3092	-28.0	-28.0	0.81 (2)	10.00
R- S	0 / 75	0.0	0.0	0.09 (1)	10.00
S- G	-400 / 0	0.0	0.0	0.18 (1)	7.81
R- Q	0 / 62	-28.0	-28.0	0.11 (3)	10.00
Q- P	0 / 2155	-28.0	-28.0	0.40 (1)	10.00
P- O	0 / 2155	-28.0	-28.0	0.40 (1)	10.00
O- N	0 / 2257	-28.0	-28.0	0.41 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- D	0 / 103	0.02 (3)
D- T	-455 / 0	0.19 (1)
T- E	0 / 1353	0.30 (1)
S- Q	0 / 2685	0.43 (1)
S- H	0 / 1110	0.25 (1)
Q- H	-1416 / 0	0.98 (1)
Q- I	0 / 995	0.22 (1)
O- I	0 / 250	0.06 (2)
O- J	-144 / 0	0.08 (1)
N- J	-429 / 0	0.12 (1)
N- K	0 / 2324	0.52 (1)
T- F	-946 / 0	0.90 (1)
F- S	0 / 332	0.07 (1)
W- U	-166 / 0	0.02 (1)
B- U	0 / 2801	0.63 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 903 (0.44")

CSI: TC=0.35/1.00 (J-K:1) , BC=0.81/1.00 (S-T:2) , WB=0.98/1.00 (H-Q:1) , SSI=0.21/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.57 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18467 -18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T35	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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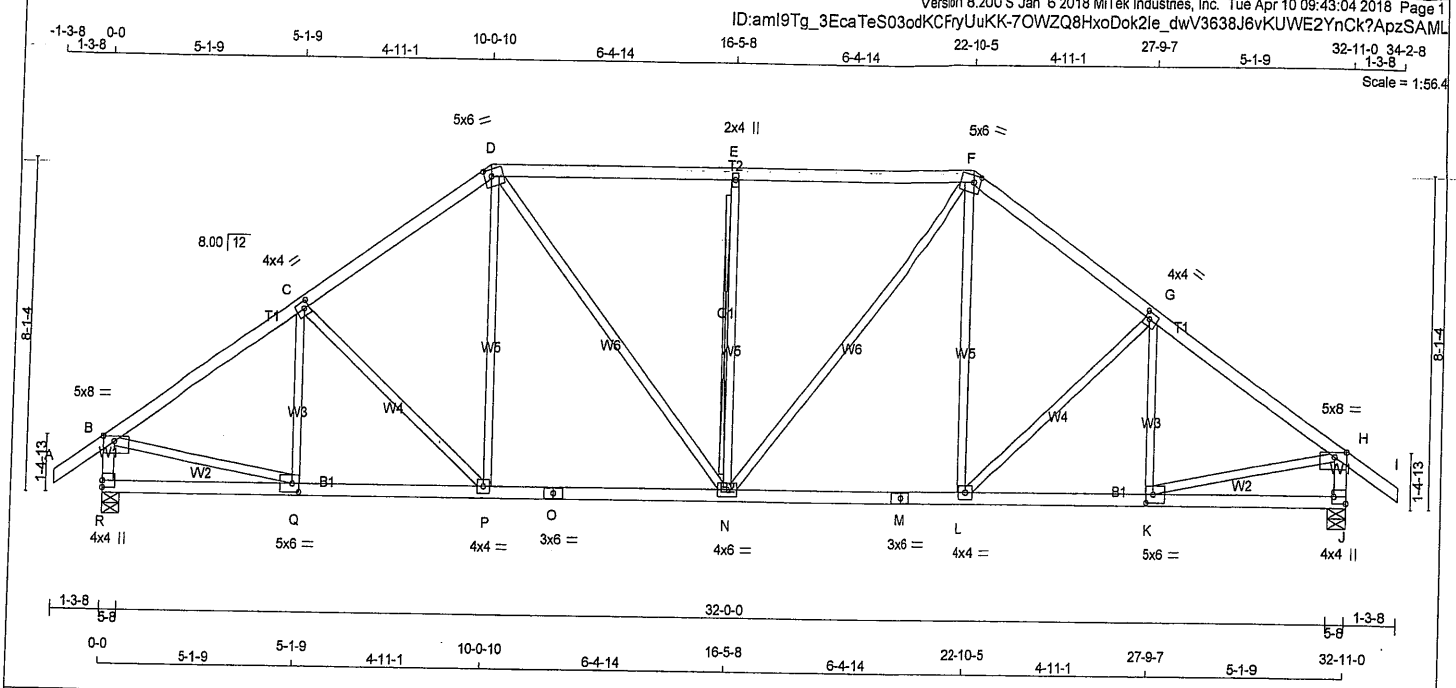

TOTAL WEIGHT = 145 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
J	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2739 / 0	-112.4	-112.4	0.49 (1)	3.75
C- D	-2536 / 0	-112.4	-112.4	0.46 (1)	3.91
D- E	-2517 / 0	-112.4	-112.4	0.71 (1)	3.50
E- F	-2517 / 0	-112.4	-112.4	0.71 (1)	3.50
F- G	-2536 / 0	-112.4	-112.4	0.46 (1)	3.91
G- H	-2739 / 0	-112.4	-112.4	0.49 (1)	3.75
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00
R- B	-2403 / 0	0.0	0.0	0.25 (1)	5.52
J- H	-2403 / 0	0.0	0.0	0.25 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.16 (3)	10.00
Q- P	0 / 2309	-28.0	-28.0	0.47 (1)	10.00
P- O	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
O- N	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
M- L	0 / 2081	-28.0	-28.0	0.47 (2)	10.00
L- K	0 / 2309	-28.0	-28.0	0.47 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.16 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-340 / 49	0.12 (1)
C- P	-313 / 0	0.26 (1)
P- D	0 / 406	0.09 (2)
D- N	0 / 688	0.15 (1)
N- E	-883 / 0	0.87 (1)
N- F	0 / 688	0.15 (1)
L- F	0 / 406	0.09 (2)
L- G	-313 / 0	0.26 (1)
K- G	-340 / 49	0.12 (1)
B- Q	0 / 2359	0.53 (1)
K- H	0 / 2359	0.53 (1)

LICENSED PROFESSIONAL ENGINEER
4/07/18
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PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.47/1.00 (K-L:1) , WB=0.87/1.00 (E-N:1) , SSI=0.35/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

DWG NO. TAM 18468-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T35S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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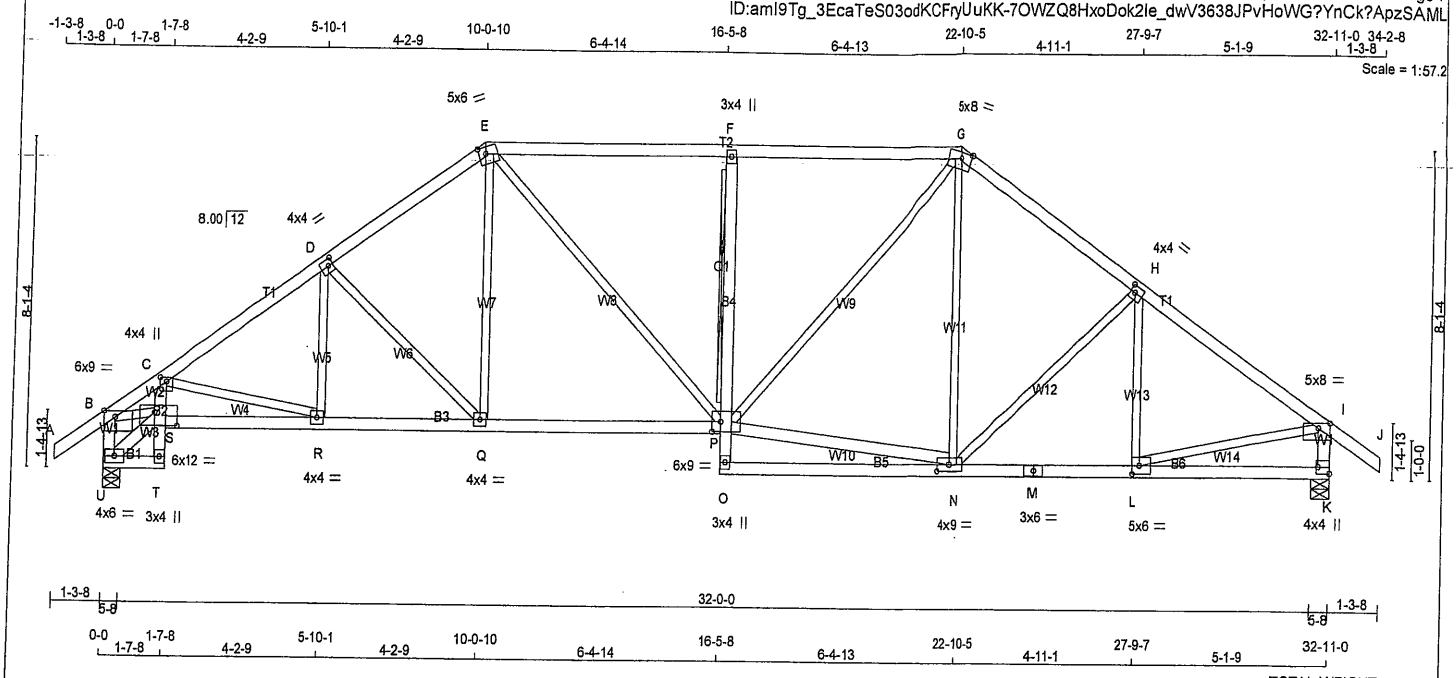

TOTAL WEIGHT = 155 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - C	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - F	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	3.50
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TMWW-t	MT20	4.0	4.0	2.00	1.50
E	TTWW-m	MT20	5.0	6.0	2.00	2.25
F	TMV+p	MT20	3.0	4.0		
G	TTWW-m	MT20	5.0	8.0	1.75	3.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.25
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0	2.00	4.25
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-l	MT20	6.0	9.0	3.00	2.75
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-l	MT20	6.0	12.0	4.00	7.25
T	BMV+p	MT20	3.0	4.0		
U	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	2462	0	2462	0	0	5-8	5-8
U	2469	0	2469	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1946	1156 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
U	1950	1160 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
B- C	-4025 / 0	-112.4	-112.4	0.27 (1)	3.32	
C- D	-3378 / 0	-112.4	-112.4	0.42 (1)	3.49	
D- E	-2886 / 0	-112.4	-112.4	0.37 (1)	3.79	
E- F	-2865 / 0	-112.4	-112.4	0.75 (1)	3.25	
F- G	-2858 / 0	-112.4	-112.4	0.73 (1)	3.26	
G- H	-2539 / 0	-112.4	-112.4	0.46 (1)	3.90	
H- I	-2733 / 0	-112.4	-112.4	0.49 (1)	3.76	
I- J	0 / 43	-112.4	-112.4	0.15 (1)	10.00	
U- B	-2319 / 0	0.0	0.0	0.24 (1)	5.60	
K- I	-2398 / 0	0.0	0.0	0.25 (1)	5.52	
U- T	0 / 163	-28.0	-28.0	0.04 (1)	10.00	
T- S	0 / 36	0.0	0.0	0.26 (1)	10.00	
S- C	0 / 297	0.0	0.0	0.31 (1)	10.00	
S- R	0 / 3528	-28.0	-28.0	0.65 (1)	10.00	
R- Q	0 / 2838	-28.0	-28.0	0.60 (1)	10.00	
Q- P	0 / 2380	-28.0	-28.0	0.52 (1)	10.00	
O- P	0 / 125	0.0	0.0	0.08 (1)	10.00	
P- F	-881 / 0	0.0	0.0	0.35 (1)	7.81	
O- N	0 / 51	-28.0	-28.0	0.25 (3)	10.00	
N- M	0 / 2304	-28.0	-28.0	0.51 (1)	10.00	
M- L	0 / 2304	-28.0	-28.0	0.51 (1)	10.00	
L- K	0 / 0	-28.0	-28.0	0.16 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	-708 / 0	0.24 (1)
R- D	0 / 214	0.05 (3)
D- Q	-634 / 0	0.38 (1)
Q- E	0 / 616	0.14 (1)
E- P	0 / 699	0.16 (1)
N- G	0 / 238	0.06 (3)
N- H	-305 / 0	0.26 (1)
L- H	-349 / 32	0.12 (1)
L- I	0 / 2354	0.53 (1)
U- S	-208 / 0	0.02 (1)
B- S	0 / 3317	0.75 (1)
P- N	0 / 2054	0.33 (1)
P- G	0 / 1152	0.26 (1)

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DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.75/1.00 (E-F:1) , BC=0.65/1.00 (R-S:1) , WB=0.75/1.00 (B-S:1) , SSI=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (S) (INPUT = 0.90)
JSI METAL= 0.64 (B) (INPUT = 1.00)

DWG NO. TAM 18469 -18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T36	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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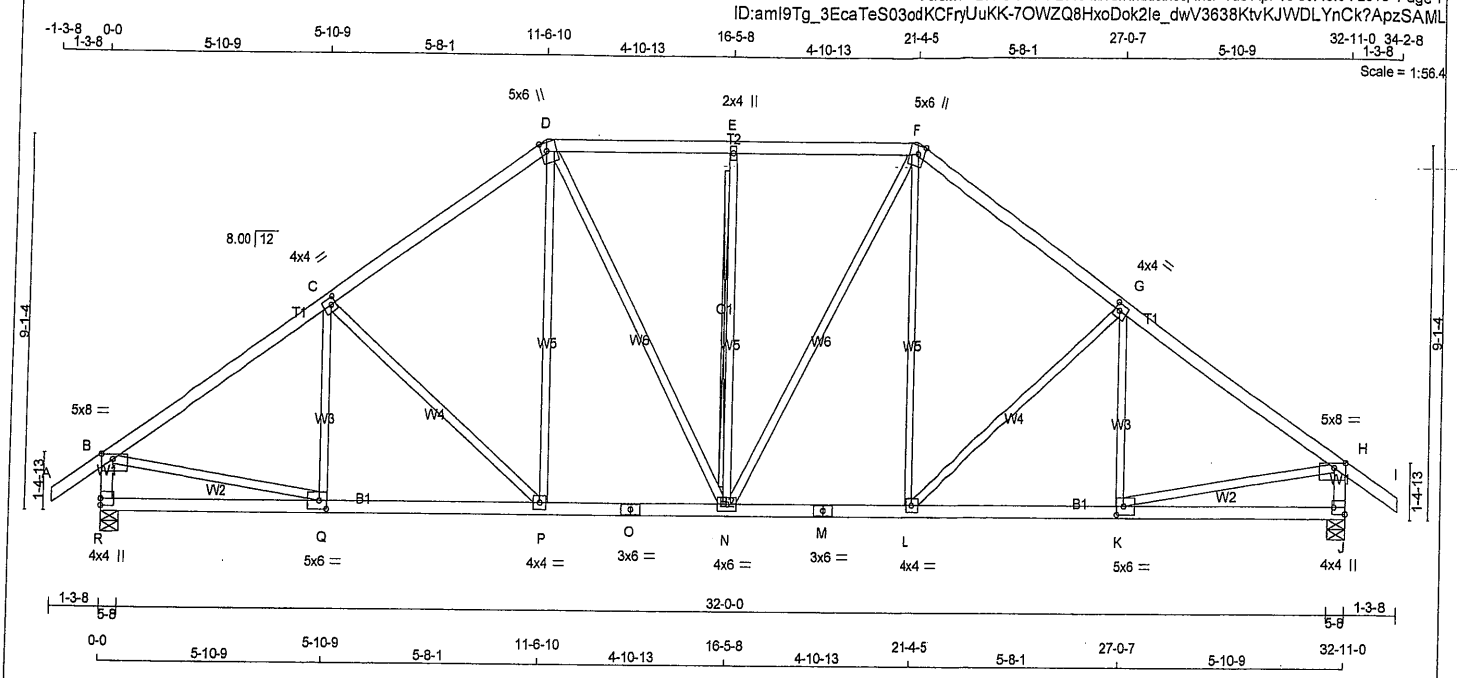

TOTAL WEIGHT = 2 X 152 = 304 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2466	0	2466	0	0	5-8	5-8
J	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
J	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2768 / 0	-112.4	-112.4	0.66 (1)	3.53
C- D	-2417 / 0	-112.4	-112.4	0.60 (1)	3.80
D- E	-2199 / 0	-112.4	-112.4	0.40 (1)	4.20
E- F	-2199 / 0	-112.4	-112.4	0.40 (1)	4.20
F- G	-2417 / 0	-112.4	-112.4	0.60 (1)	3.80
G- H	-2768 / 0	-112.4	-112.4	0.66 (1)	3.53
H- I	0 / 43	-112.4	-112.4	0.15 (1)	10.00
R- B	-2397 / 0	0.0	0.0	0.25 (1)	5.52
J- H	-2397 / 0	0.0	0.0	0.25 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.24 (3)	10.00
Q- P	0 / 2338	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
O- N	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
N- M	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
M- L	0 / 1978	-28.0	-28.0	0.40 (1)	10.00
L- K	0 / 2338	-28.0	-28.0	0.48 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-249 / 131	0.11 (1)
C- P	-484 / 0	0.57 (1)
P- D	0 / 466	0.10 (1)
D- N	0 / 455	0.10 (1)
N- E	-668 / 0	0.92 (1)
N- F	0 / 455	0.10 (1)
L- F	0 / 466	0.10 (1)
L- G	-484 / 0	0.57 (1)
K- G	-249 / 131	0.11 (1)
B- Q	0 / 2377	0.53 (1)
K- H	0 / 2377	0.53 (1)

LICENSED PROFESSIONAL ENGINEER
41078
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.66/1.00 (G-H:1) , BC=0.48/1.00 (K-L:1) , WB=0.92/1.00 (E-N:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 1.00)

DWG NO. TAM 18470-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T37	9	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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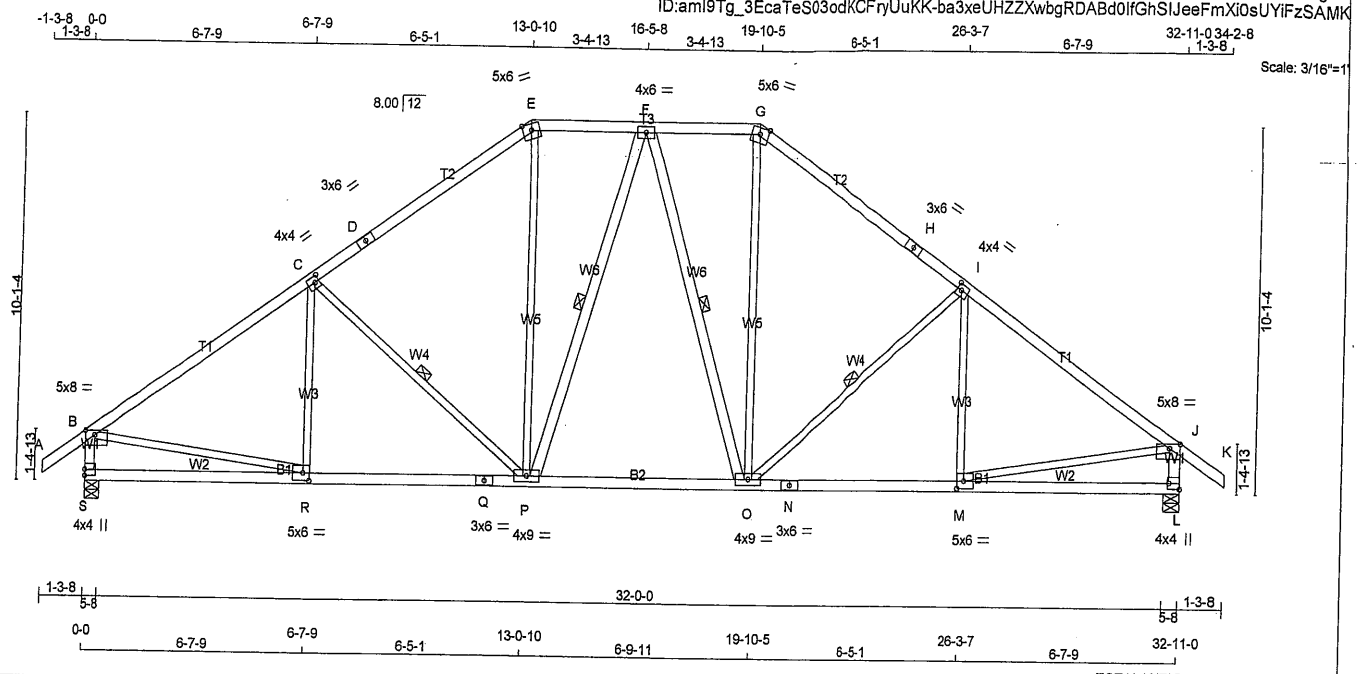

TOTAL WEIGHT = 9 X 158 = 1421 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - O	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	5.0	6.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	5.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	5.0	6.0	2.50	2.25
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.25
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2466	0	2466	0	0	5-8	5-8
L	2466	0	2466	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
L	1948	1158 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-P, F-O, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.15 (1)	10.00
B- C	-2776 / 0	-112.4	-112.4	0.87 (1)	3.20
C- D	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
D- E	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
E- F	-1871 / 0	-112.4	-112.4	0.18 (1)	4.75
F- G	-1871 / 0	-112.4	-112.4	0.18 (1)	4.75
G- H	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
H- I	-2291 / 0	-112.4	-112.4	0.77 (1)	3.61
I- J	-2776 / 0	-112.4	-112.4	0.87 (1)	3.20
J- K	0 / 43	-112.4	-112.4	0.15 (1)	10.00
S- B	-2389 / 0	0.0	0.0	0.25 (1)	5.53
L- J	-2389 / 0	0.0	0.0	0.25 (1)	5.53
S- R	0 / 0	-28.0	-28.0	0.31 (3)	10.00
R- Q	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
Q- P	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
P- O	0 / 1946	-28.0	-28.0	0.44 (1)	10.00
O- N	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
N- M	0 / 2350	-28.0	-28.0	0.54 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.31 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-181 / 188	0.09 (1)
C- P	-637 / 0	0.32 (1)
P- E	0 / 815	0.18 (1)
P- F	-235 / 0	0.13 (1)
F- O	-235 / 0	0.13 (1)
O- G	0 / 815	0.18 (1)
O- I	-637 / 0	0.32 (1)
M- I	-181 / 188	0.09 (1)
B- R	0 / 2381	0.54 (1)
M- J	0 / 2381	0.54 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.23")

CSI: TC=0.87/1.00 (I-J:1) , BC=0.54/1.00 (M-O:2) , WB=0.54/1.00 (J-M:1) , SSI=0.30/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (M) (INPUT = 0.90)
JSI METAL= 0.59 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/2018
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18471 -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T38A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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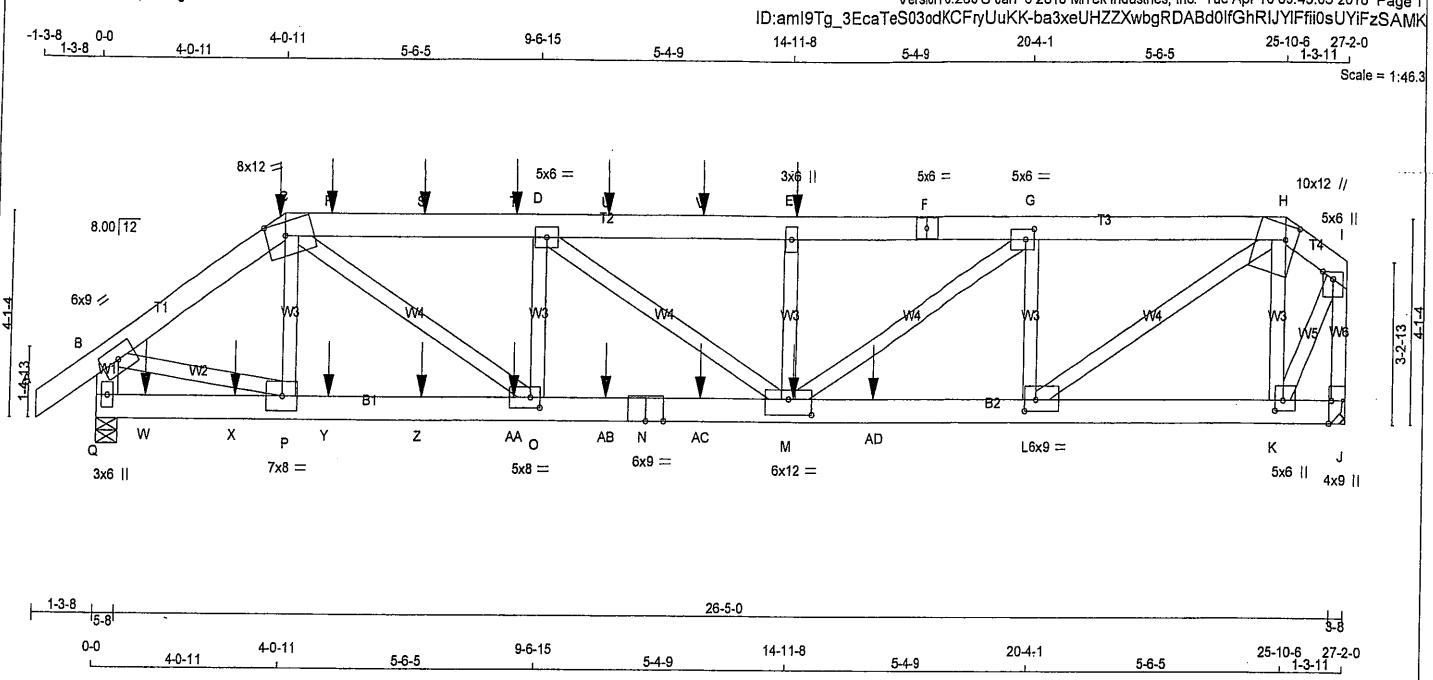

TOTAL WEIGHT = 160 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
N - J	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0		
C	TTWW-m	MT20	8.0	12.0	3.25	4.75
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	5.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.25
H	TTWW+m	MT20	10.0	12.0	3.50	2.75
I	TMVW+p	MT20	5.0	6.0	1.75	2.50
J	BMV1+t	MT20	4.0	9.0	Edge	0.50
K	BMWW+t	MT20	5.0	6.0	2.50	2.00
L	BMWW-t	MT20	6.0	9.0	2.75	3.00
M	BMWWW-t	MT20	6.0	12.0	3.75	6.00
N	BS-t	MT20	6.0	9.0		
O	BMWW-t	MT20	5.0	8.0	2.50	2.50
P	BMWW-t	MT20	7.0	8.0		
Q	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 4-0-11, 194.4 lbs FACTORED DOWN AT 5-0-12, 194.4 lbs FACTORED DOWN AT 7-0-12, 194.4 lbs FACTORED DOWN AT 9-0-12, 194.4 lbs FACTORED DOWN AT 11-0-12, AND 135.3 lbs FACTORED DOWN AT 13-0-12, AND 135.3 lbs FACTORED DOWN AT 15-0-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 84.7 lbs FACTORED DOWN AT 5-0-12, 84.7 lbs FACTORED DOWN AT 7-0-12, 84.7 lbs FACTORED DOWN AT 9-0-12, 84.7 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, AND 69.9 lbs FACTORED DOWN AT 15-0-12, AND 1677.9 lbs FACTORED DOWN AT 16-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3913	0	3913	0	0	5-8	5-8
J	3469	0	3469	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
Q	3093	1836 / 0	550 / 0	0 / 0	0 / 0	707 / 0	0 / 0
J	2744	1625 / 0	491 / 0	0 / 0	0 / 0	628 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 44	-112.4	-112.4	0.09 (1)	10.00
B- C	-4713 / 0	-112.4	-112.4	0.30 (1)	3.74
C- R	-7043 / 0	-112.4	-112.4	0.82 (1)	2.49
R- S	-7043 / 0	-112.4	-112.4	0.82 (1)	2.49
S- T	-7043 / 0	-112.4	-112.4	0.82 (1)	2.49
T- D	-7043 / 0	-112.4	-112.4	0.82 (1)	2.49
D- U	-8112 / 0	-112.4	-112.4	0.93 (1)	2.12
U- V	-8112 / 0	-112.4	-112.4	0.93 (1)	2.12
V- E	-8112 / 0	-112.4	-112.4	0.93 (1)	2.12
E- F	-8112 / 0	-112.4	-112.4	0.66 (1)	2.59
F- G	-8112 / 0	-112.4	-112.4	0.66 (1)	2.59
G- H	-5800 / 0	-112.4	-112.4	0.48 (1)	3.26
H- I	-1574 / 0	-112.4	-112.4	0.03 (1)	6.21
Q- B	-3852 / 0	0.0	0.0	0.27 (1)	5.41
J- I	-3407 / 0	0.0	0.0	0.55 (1)	4.61
Q- W	0 / 0	-28.0	-28.0	0.09 (2)	10.00
W- X	0 / 0	-28.0	-28.0	0.09 (2)	10.00
X- P	0 / 0	-28.0	-28.0	0.09 (2)	10.00
P- Y	0 / 3892	-28.0	-28.0	0.30 (1)	10.00
Y- Z	0 / 3892	-28.0	-28.0	0.30 (1)	10.00
Z-AA	0 / 3892	-28.0	-28.0	0.30 (1)	10.00
AA- O	0 / 3892	-28.0	-28.0	0.30 (1)	10.00
O-AB	0 / 7043	-28.0	-28.0	0.62 (1)	10.00
AB- N	0 / 7043	-28.0	-28.0	0.62 (1)	10.00
N-AC	0 / 7043	-28.0	-28.0	0.62 (1)	10.00
AC- M	0 / 7043	-28.0	-28.0	0.62 (1)	10.00
M-AD	0 / 5800	-28.0	-28.0	0.92 (1)	10.00
AD- L	0 / 5800	-28.0	-28.0	0.92 (1)	10.00
L- K	0 / 1199	-28.0	-28.0	0.25 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- C	-491 / 125	0.08 (1)
C- O	0 / 3769	0.67 (1)
O- D	-2004 / 0	0.34 (1)
D- M	0 / 1291	0.23 (1)
M- E	-775 / 0	0.13 (1)
M- G	0 / 2793	0.49 (1)
L- G	-2225 / 0	0.38 (1)
L- H	0 / 5502	0.97 (1)
K- H	-2492 / 0	0.43 (1)
B- P	0 / 4000	0.71 (1)
K- I	0 / 2769	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-45	-50	---	FRONT	VERT	DEAD
C	4-0-11	-282	-282	---	FRONT	VERT	SNOW
E	15-0-12	-135	-135	---	BACK	VERT	TOTAL
M	15-0-12	-40	-70	---	BACK	VERT	TOTAL
R	5-0-12	-194	-194	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 616 (0.53")

CSI: TC=0.93/1.00 (D-E:1) , BC=0.92/1.00 (L-M:1) , WB=0.97/1.00 (H-L:1) , SSI=0.80/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.86 (N) (INPUT = 1.00) pg 4

DWG NO. TAM 184772-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T38A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03cdKCFryUuKK-ba3xeUHZZXwbgRDABd0lfGhRIJYlFfii0sUYiFzSAMK

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	7-0-12	-194	-194	—	BACK	VERT	TOTAL
T	9-0-12	-194	-194	—	BACK	VERT	TOTAL
U	11-0-12	-194	-194	—	BACK	VERT	TOTAL
V	13-0-12	-135	-135	—	BACK	VERT	TOTAL
W	1-0-12	-40	-70	—	BACK	VERT	TOTAL
X	3-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	5-0-12	-48	-85	—	BACK	VERT	TOTAL
Z	7-0-12	-48	-85	—	BACK	VERT	TOTAL
AA	9-0-12	-48	-85	—	BACK	VERT	TOTAL
AB	11-0-12	-48	-85	—	BACK	VERT	TOTAL
AC	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AD	16-9-8	-1678	-1678	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4108
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10472-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T39	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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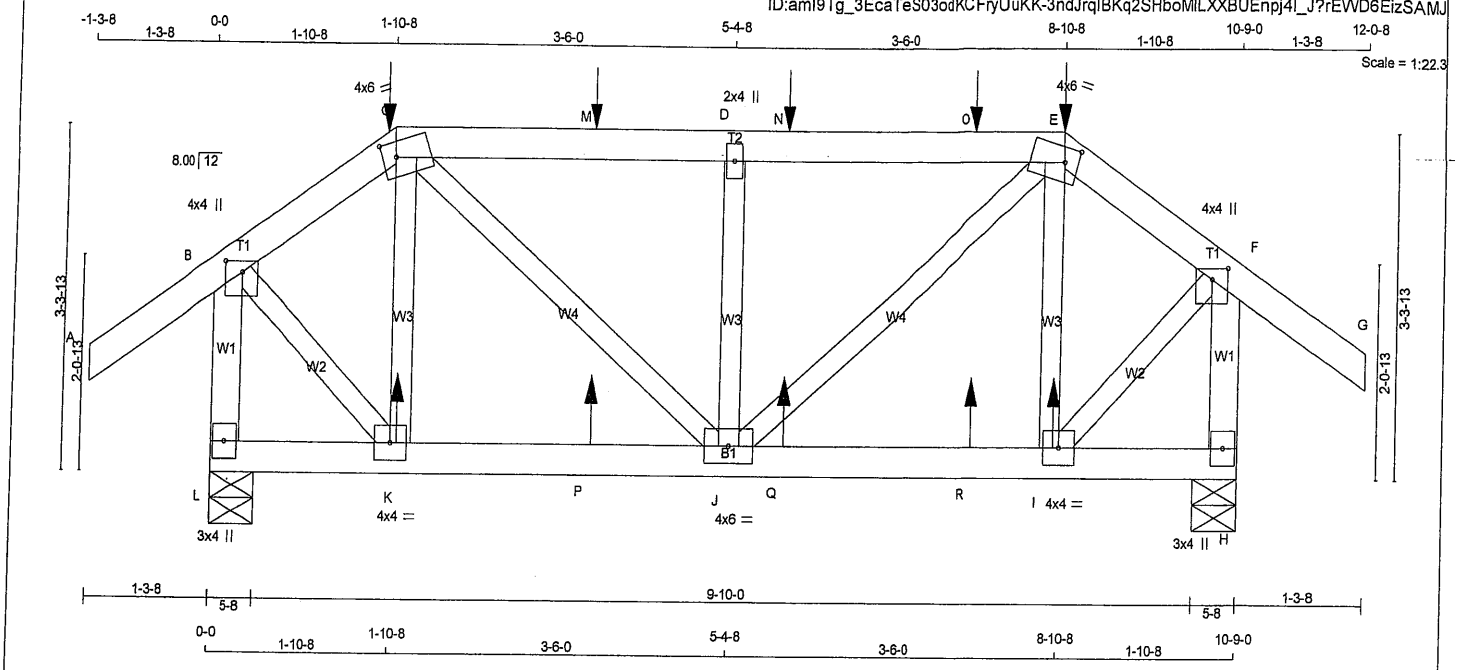

TOTAL WEIGHT = 50 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TTWW-m	MT20	4.0	6.0	1.75	1.75
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	1.75
F	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 86.6 lbs FACTORED DOWN AND 122.6 lbs FACTORED UP AT 1-10-8, 1.5 lbs FACTORED DOWN AND 134.4 lbs FACTORED UP AT 3-11-4, 1.5 lbs FACTORED DOWN AND 134.4 lbs FACTORED UP AT 5-11-4, AND 1.5 lbs FACTORED DOWN AND 131.3 lbs FACTORED UP AT 7-11-4, AND 86.6 lbs FACTORED DOWN AND 122.6 lbs FACTORED UP AT 8-10-8 ON TOP CHORD, AND 1.0 lbs FACTORED DOWN AND 12.0 lbs FACTORED UP AT 1-11-4, 1.0 lbs FACTORED DOWN AND 12.0 lbs FACTORED UP AT 3-11-4, 1.0 lbs FACTORED DOWN AND 12.0 lbs FACTORED UP AT 5-11-4, AND 1.0 lbs FACTORED DOWN AND 12.0 lbs FACTORED UP AT 7-11-4, AND 1.0 lbs FACTORED DOWN AND 12.0 lbs FACTORED UP AT 8-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	981	0	981	0	0	5-8	5-8
H	979	0	979	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	758	477 / 0	115 / 0	0 / 0	0 / 0	166 / 0	0 / 0
H	757	476 / 0	115 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
B- C	-534 / 0	-112.4	-112.4	0.16 (1)	6.25	
C- M	-728 / 0	-112.4	-112.4	0.25 (1)	6.25	
M- D	-728 / 0	-112.4	-112.4	0.25 (1)	6.25	
D- N	-728 / 0	-112.4	-112.4	0.25 (1)	6.25	
N- O	-728 / 0	-112.4	-112.4	0.25 (1)	6.25	
O- E	-728 / 0	-112.4	-112.4	0.25 (1)	6.25	
E- F	-532 / 0	-112.4	-112.4	0.16 (1)	6.25	
F- G	0 / 43	-112.4	-112.4	0.17 (1)	10.00	
L- B	-964 / 0	0.0	0.0	0.12 (1)	7.81	
H- F	-962 / 0	0.0	0.0	0.12 (1)	7.81	
L- K	0 / 0	-28.0	-28.0	0.07 (7)	10.00	
K- P	0 / 406	-28.0	-28.0	0.13 (7)	10.00	
P- J	0 / 406	-28.0	-28.0	0.13 (7)	10.00	
J- Q	0 / 405	-28.0	-28.0	0.13 (7)	10.00	
Q- R	0 / 405	-28.0	-28.0	0.13 (7)	10.00	
R- I	0 / 405	-28.0	-28.0	0.13 (7)	10.00	
I- H	0 / 0	-28.0	-28.0	0.07 (7)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	-287 / 0	0.06 (1)
C- J	0 / 424	0.11 (1)
J- D	-478 / 0	0.10 (1)
J- E	0 / 427	0.11 (1)
I- E	-290 / 0	0.06 (1)
B- K	0 / 555	0.14 (1)
I- F	0 / 553	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	1-10-8	-87	-87	134	BACK	VERT	TOTAL
E	8-10-8	-87	-87	134	BACK	VERT	TOTAL
I	8-9-12	7	1	12	BACK	VERT	TOTAL
K	1-11-4	7	1	12	BACK	VERT	TOTAL
M	3-11-4	-0	-1	134	BACK	VERT	TOTAL
N	5-11-4	-0	-1	134	BACK	VERT	TOTAL
O	7-11-4	-0	-1	131	BACK	VERT	TOTAL
P	3-11-4	7	1	12	BACK	VERT	TOTAL
Q	5-11-4	7	1	12	BACK	VERT	TOTAL
R	7-11-4	7	1	12	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.25/1.00 (C-D:1) , BC=0.13/1.00 (J-K:7) , WB=0.14/1.00 (B-K:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.16 (B) (INPUT = 1.00)

DWG NO. TAM 16473-B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T40A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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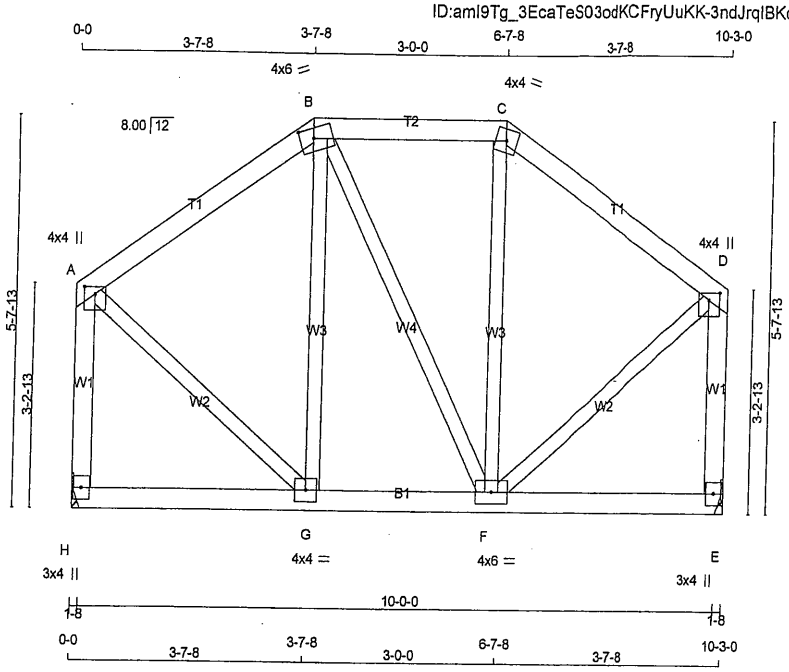

TOTAL WEIGHT = 52 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	720	0	720	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	720	0	720	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	574	333 / 0	108 / 0	0 / 0	0 / 0	133 / 0	0 / 0
E	574	333 / 0	108 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	-391 / 0	-112.4	-112.4	0.19 (1)	6.25
B- C	-321 / 0	-112.4	-112.4	0.13 (1)	6.25
C- D	-390 / 0	-112.4	-112.4	0.19 (1)	6.25
H- A	-676 / 0	0.0	0.0	0.12 (1)	7.81
E- D	-675 / 0	0.0	0.0	0.12 (1)	7.81
H- G	0 / 0	-28.0	-28.0	0.09 (3)	10.00
G- F	0 / 322	-28.0	-28.0	0.11 (2)	10.00
F- E	0 / 0	-28.0	-28.0	0.09 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-153 / 53	0.07 (1)
B- F	-1 / 0	0.00 (1)
F- C	-155 / 54	0.08 (1)
A- G	0 / 412	0.09 (1)
F- D	0 / 412	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.11/1.00 (F-G:2) , WB=0.09/1.00 (A-G:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (F) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/07/18
S KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM18474-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T41A	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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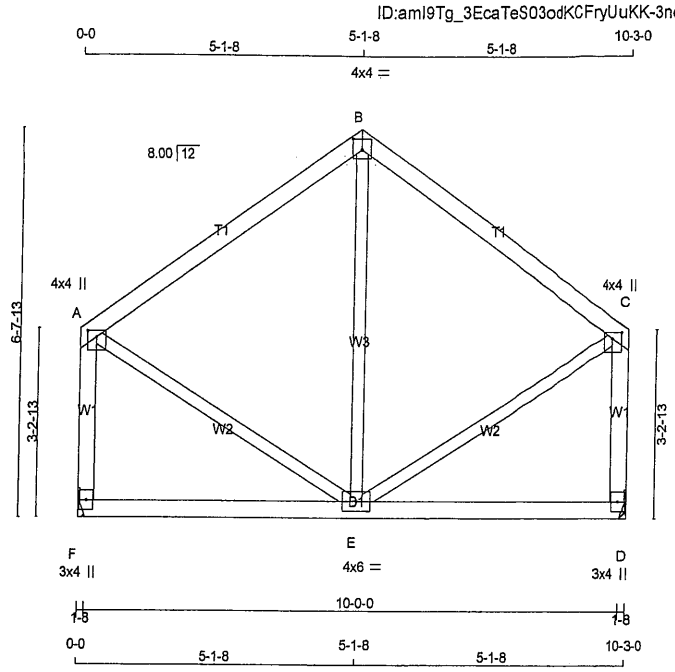

TOTAL WEIGHT = 2 X 46 = 93 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	720	0	720	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	720	0	720	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	574	333 / 0	108 / 0	0 / 0	0 / 0	133 / 0	0 / 0
D	574	333 / 0	108 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-367 / 0	-112.4	-112.4	0.38 (1)	6.25
B- C	-367 / 0	-112.4	-112.4	0.38 (1)	6.25
F- A	-665 / 0	0.0	0.0	0.12 (1)	7.81
D- C	-665 / 0	0.0	0.0	0.12 (1)	7.81
F- E	0 / 0	-28.0	-28.0	0.23 (3)	10.00
E- D	0 / 0	-28.0	-28.0	0.23 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-169 / 133	0.12 (1)
A- E	0 / 351	0.08 (1)
E- C	0 / 351	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS;

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.38/1.00 (A-B:1) , BC=0.23/1.00 (E-F:3) , WB=0.12/1.00 (B-E:1) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (A) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/10/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18475-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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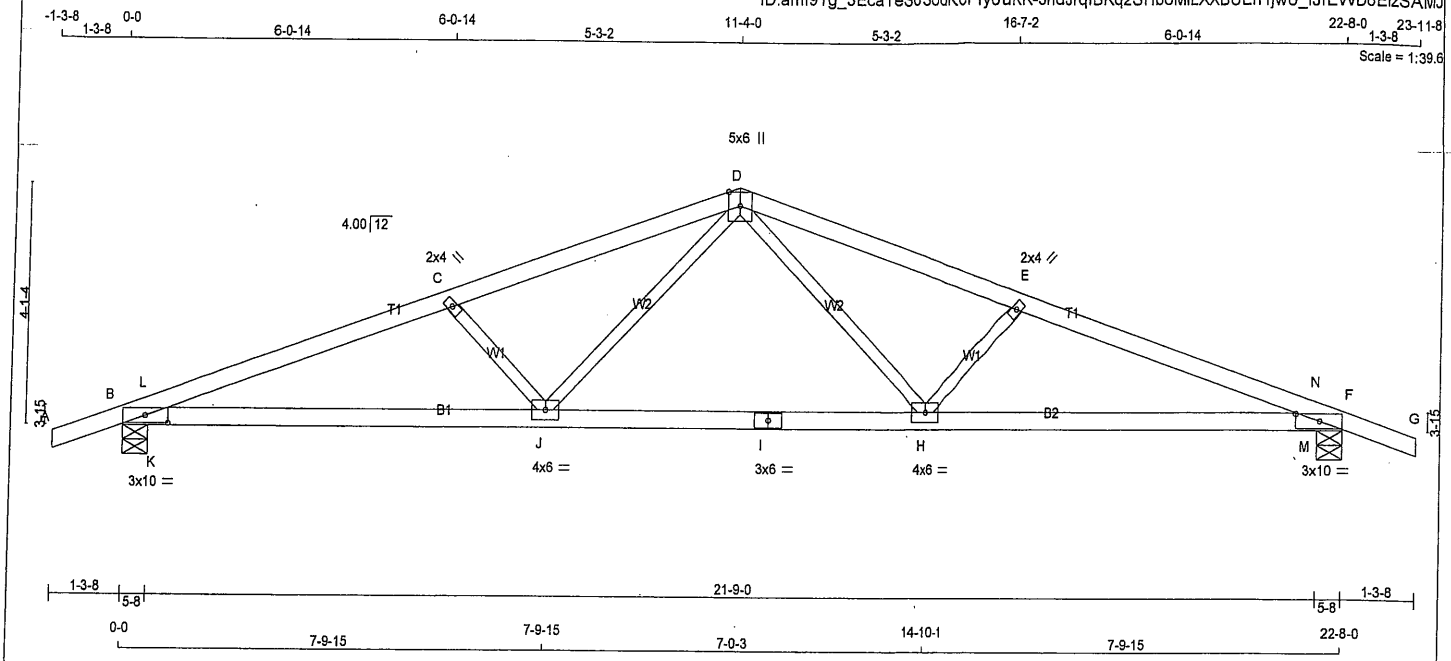

TOTAL WEIGHT = 69 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	10.0	1.50	5.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	5.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMB1-I	MT20	3.0	10.0	1.50	5.00
H	BMWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	1742	0	1742	0	0	5-8	5-8
F	1742	0	1742	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	1372	823 / 0	238 / 0	0 / 0	0 / 0	311 / 0	0 / 0
F	1372	823 / 0	238 / 0	0 / 0	0 / 0	311 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00
B- L	-3863 / 0	-112.4	-112.4	0.19 (2)	3.49
L- C	-3774 / 0	-112.4	-112.4	0.62 (1)	3.09
C- D	-3286 / 0	-112.4	-112.4	0.55 (1)	3.38
D- E	-3286 / 0	-112.4	-112.4	0.55 (1)	3.38
E- N	-3774 / 0	-112.4	-112.4	0.62 (1)	3.09
N- F	-3863 / 0	-112.4	-112.4	0.19 (2)	3.49
F- G	0 / 22	-112.4	-112.4	0.14 (1)	10.00
B- K	0 / 3601	-28.0	-28.0	0.70 (1)	10.00
K- J	0 / 3601	-28.0	-28.0	0.82 (1)	10.00
J- I	0 / 2359	-28.0	-28.0	0.56 (2)	10.00
I- H	0 / 2359	-28.0	-28.0	0.56 (2)	10.00
H- M	0 / 3601	-28.0	-28.0	0.82 (1)	10.00
M- F	0 / 3601	-28.0	-28.0	0.70 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- H	0 / 1090	0.25 (1)
H- E	-762 / 0	0.14 (1)
J- D	0 / 1090	0.25 (1)
C- J	-762 / 0	0.14 (1)
K- L	-72 / 277	0.00 (1)
M- N	-72 / 277	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.76")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (0.76")
CALCULATED VERT. DEFL.(TL) = L/ 678 (0.40")

CSI: TC=0.62/1.00 (E-N:1) , BC=0.82/1.00 (H-M:1) , WB=0.25/1.00 (D-H:1) , SSI=0.28/1.00 (C-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.66 (I) (INPUT = 1.00)

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PROVINCE OF ONTARIO

DWG NO. TAM / 18476-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	G42	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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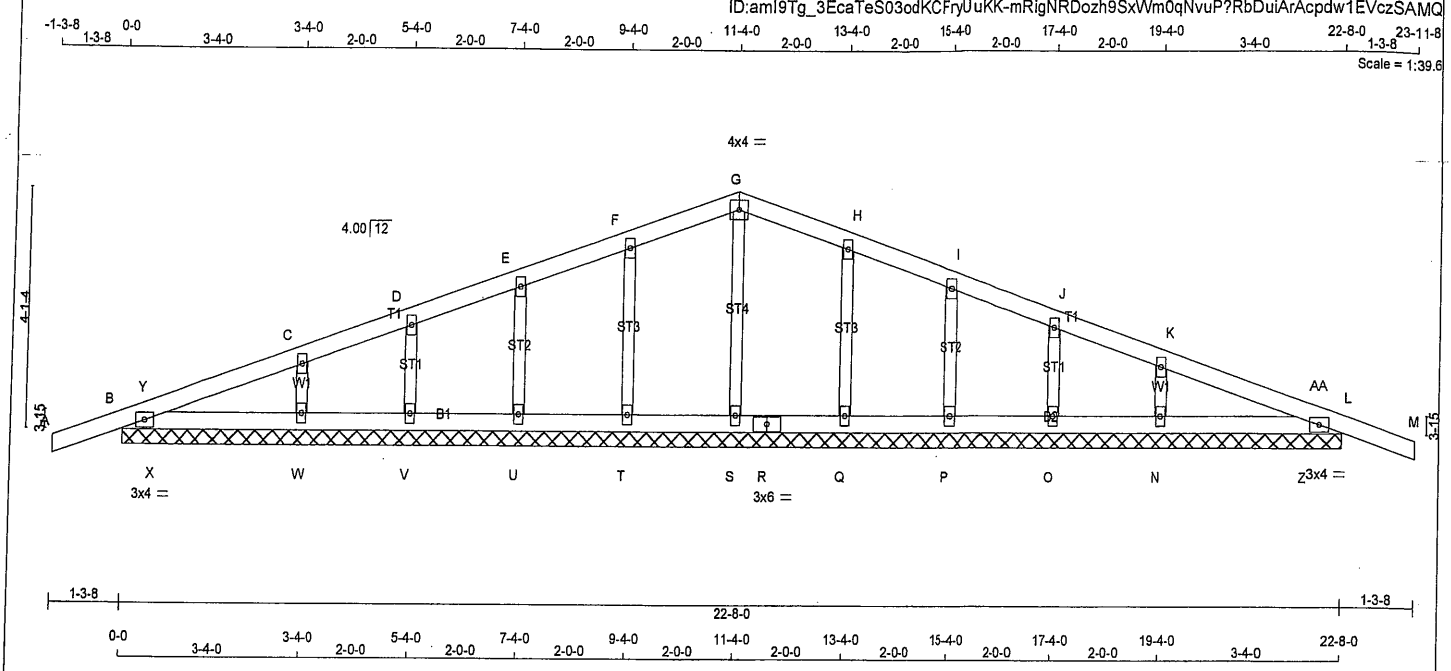

TOTAL WEIGHT = 71 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	
C, D, E, F, H, I, J, K					
C	TMW+w	MT20	2.0	4.0	
G	TTW-p	MT20	4.0	4.0	
L	TMB1-I	MT20	3.0	4.0	
N, O, P, Q, S, T, U, V, W					
N	BMW1+w	MT20	2.0	4.0	
R	BS-t	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00
B- Y	-52 / 0	-112.4	-112.4	0.04 (3)	6.25
Y- C	-26 / 0	-112.4	-112.4	0.12 (1)	6.25
C- D	-35 / 0	-112.4	-112.4	0.12 (1)	6.25
D- E	-18 / 0	-112.4	-112.4	0.05 (1)	6.25
E- F	-14 / 0	-112.4	-112.4	0.06 (1)	6.25
F- G	-19 / 0	-112.4	-112.4	0.06 (1)	6.25
G- H	-19 / 0	-112.4	-112.4	0.06 (1)	6.25
H- I	-14 / 0	-112.4	-112.4	0.06 (1)	6.25
I- J	-18 / 0	-112.4	-112.4	0.05 (1)	6.25
J- K	-35 / 0	-112.4	-112.4	0.12 (1)	6.25
K-AA	-26 / 0	-112.4	-112.4	0.12 (1)	6.25
AA- L	-52 / 0	-112.4	-112.4	0.04 (3)	6.25
L- M	0 / 22	-112.4	-112.4	0.14 (1)	10.00
B- X	0 / 37	-28.0	-28.0	0.10 (1)	10.00
X- W	0 / 37	-28.0	-28.0	0.10 (1)	10.00
W- V	0 / 24	-28.0	-28.0	0.07 (1)	10.00
V- U	0 / 19	-28.0	-28.0	0.03 (3)	10.00
U- T	0 / 15	-28.0	-28.0	0.03 (3)	10.00
T- S	0 / 11	-28.0	-28.0	0.02 (3)	10.00
S- R	0 / 11	-28.0	-28.0	0.02 (3)	10.00
R- Q	0 / 11	-28.0	-28.0	0.02 (3)	10.00
Q- P	0 / 15	-28.0	-28.0	0.03 (3)	10.00
P- O	0 / 19	-28.0	-28.0	0.03 (3)	10.00
O- N	0 / 24	-28.0	-28.0	0.07 (1)	10.00
N- Z	0 / 37	-28.0	-28.0	0.10 (1)	10.00
Z- L	0 / 37	-28.0	-28.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- G	-177 / 0	0.04 (1)
T- F	-253 / 0	0.05 (1)
U- E	-226 / 0	0.04 (1)
V- D	-184 / 0	0.03 (1)
Q- H	-253 / 0	0.05 (1)
P- I	-226 / 0	0.04 (1)
O- J	-184 / 0	0.03 (1)
W- C	-332 / 0	0.05 (1)
N- K	-332 / 0	0.05 (1)
X- Y	-110 / 42	0.00 (1)
Z-AA	-110 / 42	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (L-M:1) , BC=0.10/1.00 (N-Z:1) , WB=0.05/1.00 (F-T:1) , SSI=0.14/1.00 (K-AA:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (G) (INPUT = 0.90)
JSI METAL= 0.06 (W) (INPUT = 1.00)

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DWG NO. TAM 18479 -18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T43	5	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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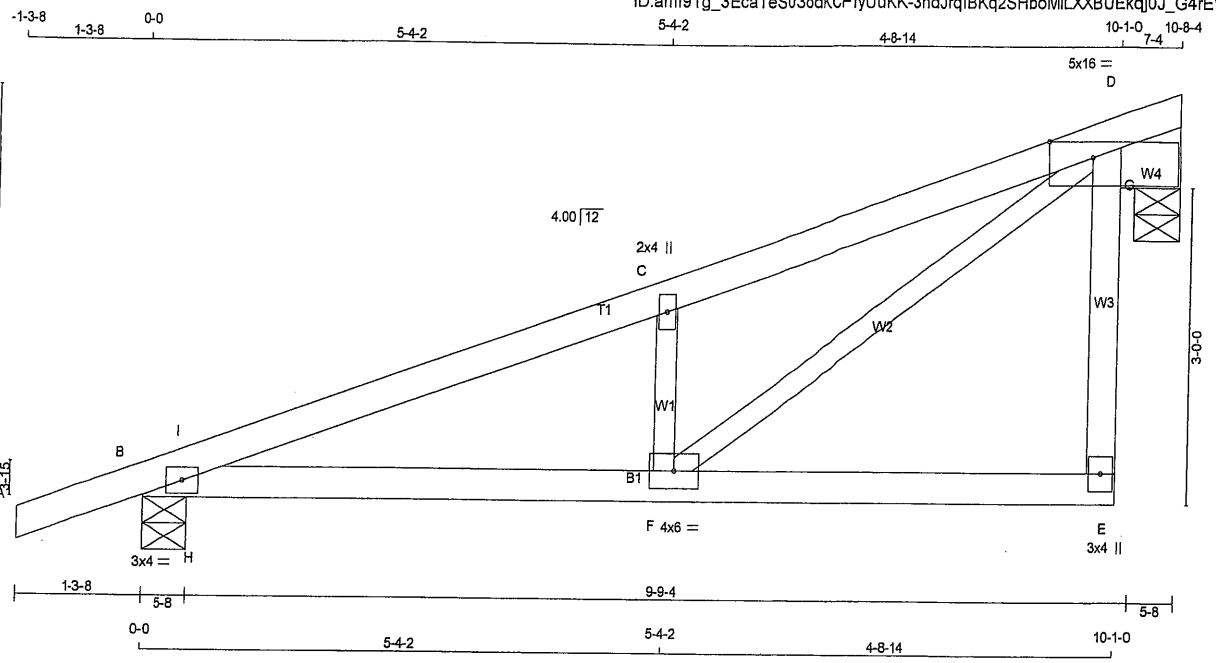

TOTAL WEIGHT = 5 X 36 = 182 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-i	MT20	3.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TMVWW1-p	MT20	5.0	16.0	Edge	5.50
E	BMV+p	MT20	3.0	4.0		
F	BMWW-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(D)	683	0	683	0	0	5-8	5-8
B	884	0	884	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(D)	544	316 / 0	102 / 0	0 / 0	0 / 0	126 / 0	0 / 0
B	688	426 / 0	110 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(D), B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00
B- I	-1355 / 0	-112.4	-112.4	0.08 (3)	5.57
I- C	-1314 / 0	-112.4	-112.4	0.43 (1)	5.07
C- D	-1334 / 0	-112.4	-112.4	0.44 (1)	5.03
E- G	0 / 90	0.0	0.0	0.30 (1)	10.00
G- D	0 / 90	0.0	0.0	0.30 (1)	10.00
B- H	0 / 1265	-28.0	-28.0	0.37 (1)	10.00
H- F	0 / 1265	-28.0	-28.0	0.39 (1)	10.00
F- E	0 / 76	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-648 / 0	0.10 (1)
F- D	0 / 1442	0.32 (1)
H- I	-172 / 91	0.00 (1)
D- J	-833 / 0	0.04 (1)
G- J	0 / 478	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.44/1.00 (C-D:1) , BC=0.39/1.00 (F-H:1) , WB=0.32/1.00 (D-F:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/9/18
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PROVINCE OF ONTARIO

DWG NO. TAM 18477 -18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	G43	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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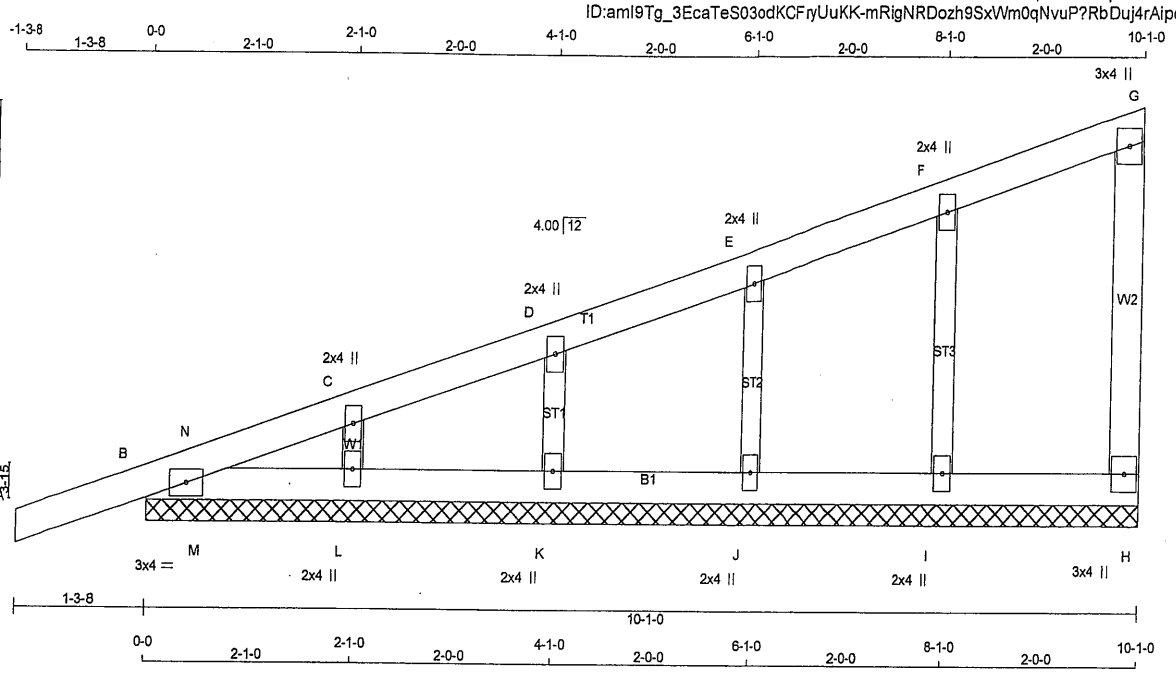

TOTAL WEIGHT = 2 X 34 = 67 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
B - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K, L						
I	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 22	-112.4	-112.4	0.14 (1)	10.00	
B- N	-43 / 0	-112.4	-112.4	0.02 (1)	6.25	
N- C	-21 / 0	-112.4	-112.4	0.06 (1)	6.25	
C- D	-18 / 0	-112.4	-112.4	0.06 (1)	6.25	
D- E	-10 / 0	-112.4	-112.4	0.05 (1)	10.00	
E- F	-3 / 0	-112.4	-112.4	0.06 (1)	10.00	
F- G	-7 / 0	-112.4	-112.4	0.06 (1)	10.00	
H- G	-92 / 0	0.0	0.0	0.02 (1)	7.81	
B- M	0 / 32	-28.0	-28.0	0.04 (1)	10.00	
M- L	0 / 32	-28.0	-28.0	0.04 (1)	10.00	
L- K	0 / 17	-28.0	-28.0	0.03 (2)	10.00	
K- J	0 / 9	-28.0	-28.0	0.02 (3)	10.00	
J- I	0 / 4	-28.0	-28.0	0.03 (3)	10.00	
I- H	0 / 0	-28.0	-28.0	0.03 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	-254 / 0	0.05 (1)
J- E	-217 / 0	0.03 (1)
K- D	-221 / 0	0.03 (1)
L- C	-231 / 0	0.03 (1)
M- N	-46 / 18	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.04/1.00 (B-M:1) , WB=0.05/1.00 (F-I:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.05 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41018
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 18480-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296711	T44	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:amI9Tg_3EcaTeS03odKCFryUuKK-MgcpSrovgRCeiuRFeSm8M7QhTF0X5pNtgOfmAqzSALg

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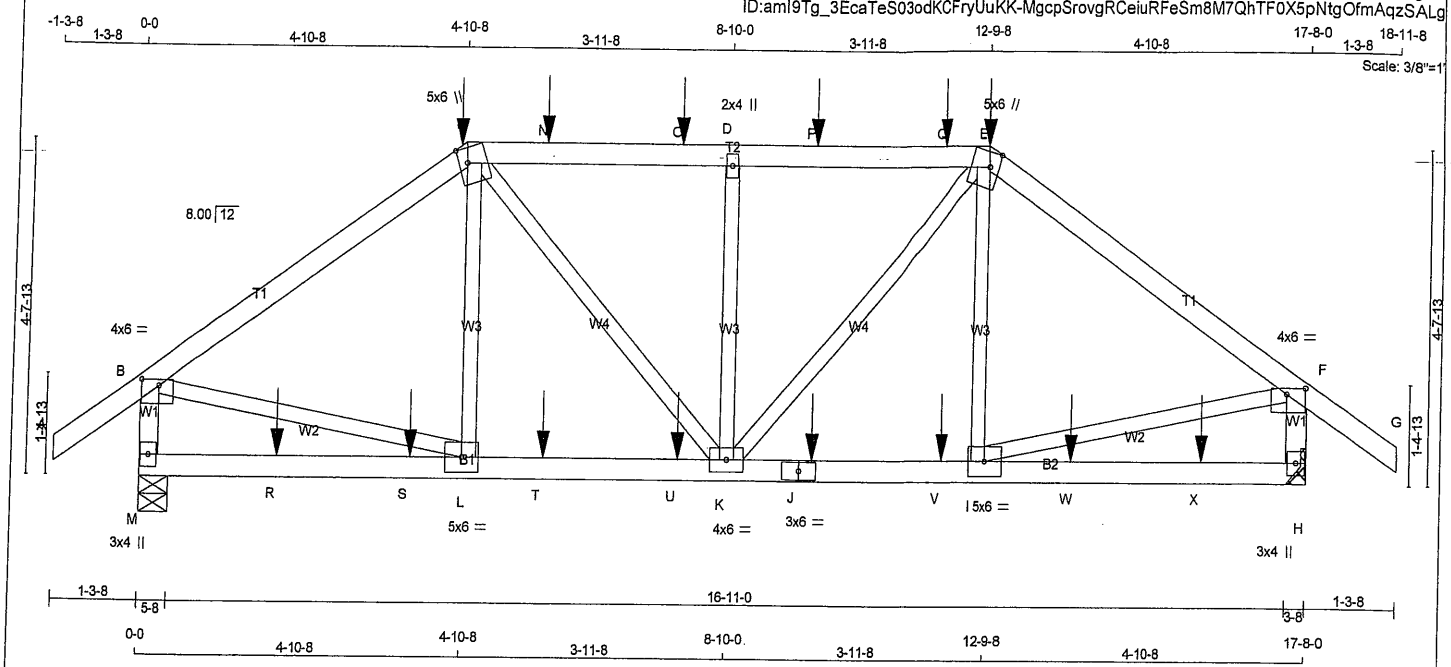

TOTAL WEIGHT = 75 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW+m	MT20	5.0	6.0	2.50	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.50	1.50
F	TMVW-p	MT20	4.0	6.0	1.00	3.25
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 232.3 lbs FACTORED DOWN AT 4-10-8, 232.3 lbs FACTORED DOWN AT 12-9-8, 51.3 lbs FACTORED DOWN AT 6-1-4, 51.3 lbs FACTORED DOWN AT 8-1-4, AND 51.3 lbs FACTORED DOWN AT 10-1-4, AND 68.8 lbs FACTORED DOWN AT 12-1-4 ON TOP CHORD, AND 35.8 lbs FACTORED DOWN AT 2-1-4, 35.8 lbs FACTORED DOWN AT 4-1-4, 35.8 lbs FACTORED DOWN AT 6-1-4, 35.8 lbs FACTORED DOWN AT 8-1-4, 35.8 lbs FACTORED DOWN AT 10-1-4, 35.8 lbs FACTORED DOWN AT 12-1-4, AND 35.8 lbs FACTORED DOWN AT 14-1-4, AND 35.8 lbs FACTORED DOWN AT 16-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1812	0	1812	0	0	5-8	5-8
H	1829	0	1829	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1425	858 / 0	245 / 0	0 / 0	0 / 0	322 / 0	0 / 0
H	1439	865 / 0	249 / 0	0 / 0	0 / 0	326 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.16 FT. .
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00
B- C	-1785 / 0	-112.4	-112.4	0.63 (1)	4.17
C- N	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
N- O	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
O- D	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
D- P	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
P- Q	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
Q- E	-1841 / 0	-112.4	-112.4	0.45 (1)	4.35
E- F	-1802 / 0	-112.4	-112.4	0.63 (1)	4.16
F- G	0 / 43	-112.4	-112.4	0.17 (1)	10.00
M- B	-1743 / 0	0.0	0.0	0.20 (1)	6.27
H- F	-1756 / 0	0.0	0.0	0.20 (1)	6.25
M- R	0 / 0	-28.0	-28.0	0.24 (3)	10.00
R- S	0 / 0	-28.0	-28.0	0.24 (3)	10.00
S- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00
L- T	0 / 1480	-28.0	-28.0	0.41 (2)	10.00
T- U	0 / 1480	-28.0	-28.0	0.41 (2)	10.00
U- K	0 / 1480	-28.0	-28.0	0.41 (2)	10.00
K- J	0 / 1494	-28.0	-28.0	0.41 (2)	10.00
J- V	0 / 1494	-28.0	-28.0	0.41 (2)	10.00
V- I	0 / 1494	-28.0	-28.0	0.41 (2)	10.00
I- W	0 / 0	-28.0	-28.0	0.24 (3)	10.00
W- X	0 / 0	-28.0	-28.0	0.24 (3)	10.00
X- H	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- C	-119 / 207	0.05 (3)
C- K	0 / 536	0.13 (1)
K- D	-665 / 0	0.22 (1)
K- E	0 / 515	0.13 (1)
I- E	-122 / 207	0.05 (3)
B- L	0 / 1517	0.38 (1)
I- F	0 / 1532	0.38 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-10-8	-31	-35	---	FRONT	VERT	DEAD
C	4-10-8	-201	-201	---	FRONT	VERT	SNOW
E	12-9-8	-31	-35	---	FRONT	VERT	DEAD
E	12-9-8	-201	-201	---	FRONT	VERT	SNOW
J	10-1-4	-20	-36	---	FRONT	VERT	TOTAL
N	6-1-4	-51	-51	---	FRONT	VERT	TOTAL
O	8-1-4	-51	-51	---	FRONT	VERT	TOTAL
P	10-1-4	-51	-51	---	FRONT	VERT	TOTAL
Q	12-1-4	-69	-69	---	FRONT	VERT	TOTAL
R	2-1-4	-20	-36	---	FRONT	VERT	TOTAL
S	4-1-4	-20	-36	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.63/1.00 (E-F:1) , BC=0.41/1.00 (I-K:2) , WB=0.38/1.00 (F-I:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.44 (J) (INPUT = 1.00)

DWG NO. TAM 18486-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296711	T44	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
T	6-1-4	-20	-36	—	FRONT	VERT	TOTAL
U	8-1-4	-20	-36	—	FRONT	VERT	TOTAL
V	12-1-4	-20	-36	—	FRONT	VERT	TOTAL
W	14-1-4	-20	-36	—	FRONT	VERT	TOTAL
X	16-1-4	-20	-36	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4198
S KATSOULAKOS
PROVINCE OF ONTARIO

P62/2

DWG NO. TAM 184186-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287659	T44S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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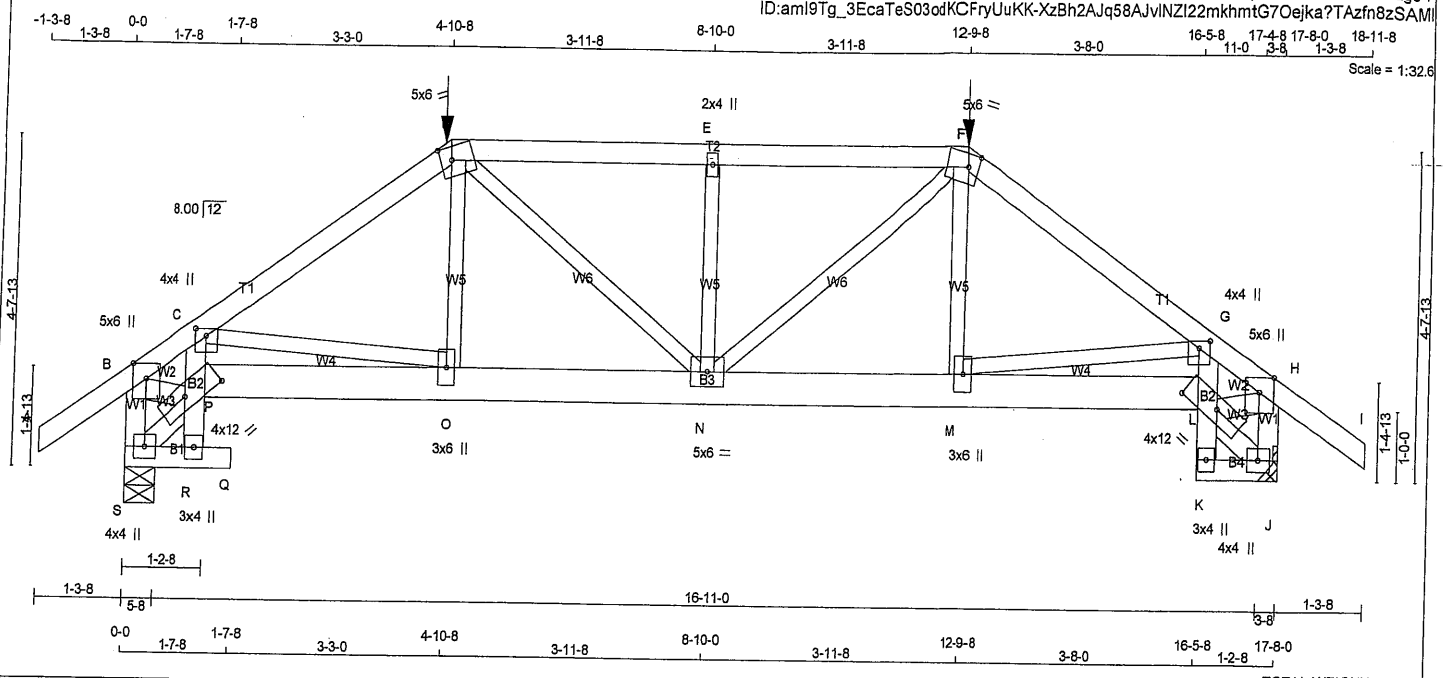

TOTAL WEIGHT = 87 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	TTWW-m	MT20	5.0	6.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW+p	MT20	4.0	4.0	1.25	2.00
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1+p	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWW+w	MT20	4.0	12.0	6.75	2.00
M	BMWW+t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	6.0		
P	BVMWW+w	MT20	4.0	12.0	6.75	2.00
R	BMV+p	MT20	3.0	4.0		
S	BMVW1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 217.0 lbs FACTORED DOWN AT 12-9-8, AND 217.0 lbs FACTORED DOWN AT 4-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	1877	0	1877	0	0	IN-SX	IN-SX
J	1858	0	1858	0	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1485	879 / 0	266 / 0	0 / 0	0 / 0	340 / 0	0 / 0
J	1462	879 / 0	252 / 0	0 / 0	0 / 0	331 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 43	-112.4	-112.4	0.17 (1)	10.00
B- C	-2014 / 0	-112.4	-112.4	0.23 (1)	4.49
C- D	-2394 / 0	-112.4	-112.4	0.33 (1)	4.09
D- E	-2483 / 0	-152.2	-152.2	0.52 (1)	3.74
E- F	-2483 / 0	-152.2	-152.2	0.52 (1)	3.74
F- G	-2391 / 0	-112.4	-112.4	0.33 (1)	4.10
G- H	-1992 / 0	-112.4	-112.4	0.23 (1)	4.52
H- I	0 / 43	-112.4	-112.4	0.17 (1)	10.00
S- B	-1788 / 0	0.0	0.0	0.20 (1)	6.20
J- H	-1770 / 0	0.0	0.0	0.20 (1)	6.23
S- R	0 / 93	-37.9	-37.9	0.03 (2)	10.00
R- Q	0 / 0	-37.9	-37.9	0.02 (3)	10.00
R- P	0 / 73	0.0	0.0	0.12 (1)	10.00
P- C	-560 / 0	0.0	0.0	0.11 (1)	7.81
P- O	0 / 1760	-37.9	-37.9	0.27 (1)	10.00
O- N	0 / 1982	-37.9	-37.9	0.32 (1)	10.00
N- M	0 / 1979	-37.9	-37.9	0.32 (1)	10.00
M- L	0 / 1741	-37.9	-37.9	0.27 (1)	10.00
K- L	0 / 35	0.0	0.0	0.11 (1)	10.00
L- G	-574 / 0	0.0	0.0	0.11 (1)	7.81
K- J	0 / 92	-37.9	-37.9	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- O	0 / 223	0.06 (1)
O- D	0 / 360	0.09 (2)
D- N	0 / 648	0.16 (1)
N- E	-719 / 0	0.16 (1)
N- F	0 / 652	0.16 (1)
M- F	0 / 353	0.09 (2)
M- G	0 / 239	0.06 (1)
S- P	-116 / 0	0.01 (1)
B- P	0 / 1745	0.31 (1)
L- J	-114 / 0	0.01 (1)
L- H	0 / 1726	0.31 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	4-10-8	-217	-217	---	FRONT	VERT	STOTAL
F	12-9-8	-217	-217	---	FRONT	VERT	TOTAL

DESIGN CRITERIA 1-2-8

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-10-8
END SETBACK = 3-10-8
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.52/1.00 (D-E:1) , BC=0.32/1.00 (N-O:1) , WB=0.31/1.00 (B-P:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER
4/9/18
STATHIS SOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18478-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	T44S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.38 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41978
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/18478-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T1	1	3	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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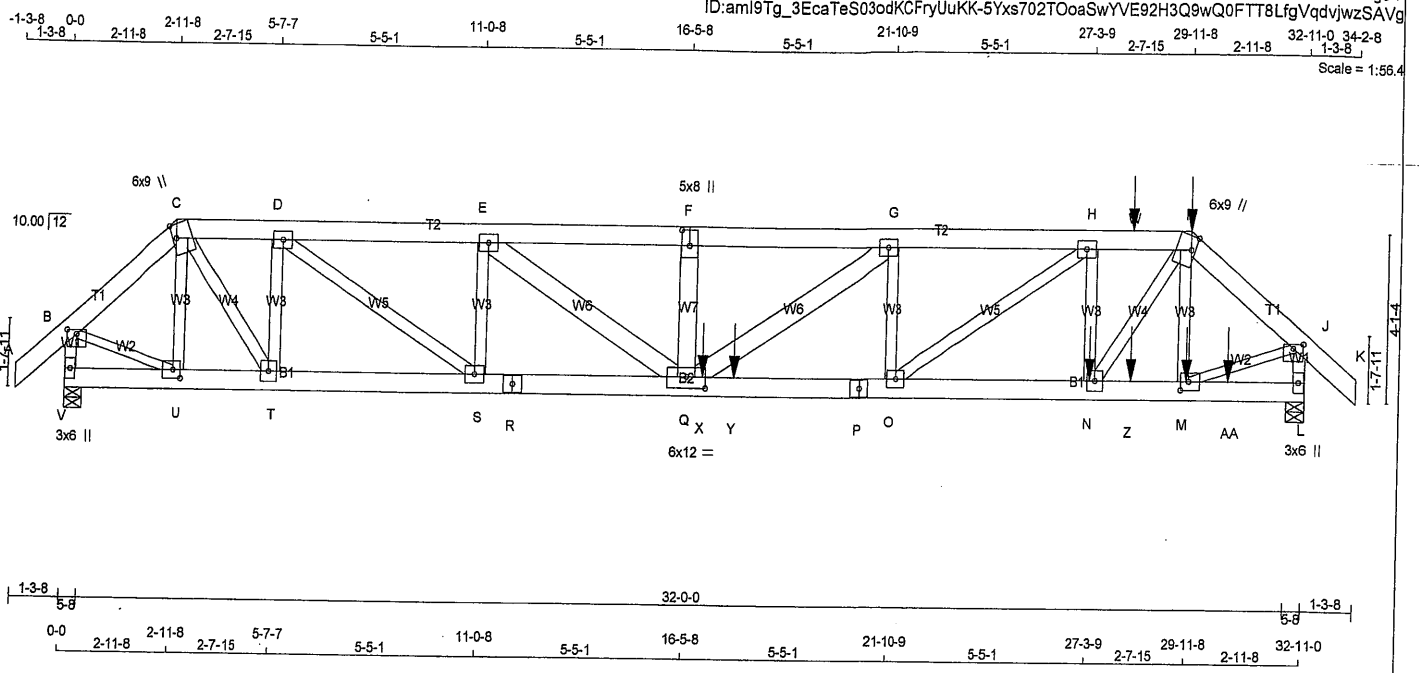

TOTAL WEIGHT = 3 X 207 = 620 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
E - Q	2x6	DRY	No.2	SPF
Q - F	2x6	DRY	No.2	SPF
Q - G	2x6	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	TOP
C- F	2	12	TOP
F- I	2	12	SIDE(91.5)
I- K	2	12	SIDE(183.1)
V- B	1	12	TOP
L- J	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- R	2	8	TOP
R- P	2	8	SIDE(689.4)
P- L	2	8	SIDE(457.7)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
F- Q	2	6	SIDE(213.3)
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D, E, G, H						
D	TMWW-t	MT20	5.0	6.0		
F	TSW+I	MT20	5.0	8.0	4.50	2.50
I	TTWW+m	MT20	6.0	9.0	4.00	1.25
J	TMVW-p	MT20	5.0	6.0	1.25	3.00
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4707	0	4707	0	0	5-8	5-8
L	6366	0	6366	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3690	2242 / 0	622 / 0	0 / 0	0 / 0	827 / 0	0 / 0
L	4987	3034 / 0	834 / 0	0 / 0	0 / 0	1118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD (PLF) TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.03 (1)	10.00	
B- C	-4722 / 0	-112.4	-112.4	0.04 (1)	6.21	
C- D	-6480 / 0	-112.4	-112.4	0.06 (1)	5.51	
D- E	-11355 / 0	-112.4	-112.4	0.15 (1)	4.35	
E- F	-15560 / 0	-112.4	-112.4	0.24 (1)	3.74	
F- G	-15560 / 0	-112.4	-112.4	0.24 (1)	3.73	
G- H	-13061 / 0	-112.4	-112.4	0.18 (1)	4.07	
H- W	-8535 / 0	-112.4	-112.4	0.07 (1)	4.94	
W- I	-8535 / 0	-112.4	-112.4	0.07 (1)	4.94	
I- J	-6535 / 0	-112.4	-112.4	0.05 (1)	5.49	
J- K	0 / 52	-112.4	-112.4	0.03 (1)	10.00	
V- B	-4670 / 0	0.0	0.0	0.16 (1)	6.61	
L- J	-6337 / 0	0.0	0.0	0.22 (1)	5.83	
V- U	0 / 0	-28.0	-28.0	0.01 (3)	10.00	
U- T	0 / 3556	-28.0	-28.0	0.17 (1)	10.00	
T- S	0 / 6480	-28.0	-28.0	0.34 (1)	10.00	
S- R	0 / 11355	-28.0	-28.0	0.65 (1)	10.00	
R- Q	0 / 11355	-28.0	-28.0	0.65 (1)	10.00	
Q- X	0 / 13061	-28.0	-28.0	0.88 (1)	10.00	
X- Y	0 / 13061	-28.0	-28.0	0.88 (1)	10.00	
Y- P	0 / 13061	-28.0	-28.0	0.88 (1)	10.00	
P- O	0 / 13061	-28.0	-28.0	0.88 (1)	10.00	
O- N	0 / 8535	-28.0	-28.0	0.43 (1)	10.00	
N- Z	0 / 4932	-28.0	-28.0	0.24 (1)	10.00	
Z- M	0 / 4932	-28.0	-28.0	0.24 (1)	10.00	
M-AA	0 / 0	-28.0	-28.0	0.02 (2)	10.00	
AA- L	0 / 0	-28.0	-28.0	0.02 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-1256 / 0	0.07 (1)
C- T	0 / 4927	0.26 (1)
T- D	-3718 / 0	0.20 (1)
D- S	0 / 5875	0.31 (1)
S- E	-3451 / 0	0.19 (1)
E- Q	0 / 5068	0.20 (1)
Q- F	-576 / 0	0.02 (1)
Q- G	0 / 3012	0.12 (1)
O- G	-2302 / 0	0.12 (1)
O- H	0 / 5454	0.29 (1)
N- H	-3519 / 0	0.19 (1)
N- I	0 / 6071	0.33 (1)
M- I	-1483 / 0	0.08 (1)
B- U	0 / 3832	0.29 (1)
M- J	0 / 5303	0.40 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
I	29-11-8	-142	-142	—	BACK	VERT	TOTAL
I	29-11-8	-33	-36	—	FRONT	VERT	DEAD
I	29-11-8	-206	-206	—	FRONT	VERT	SNOW
M	29-10-12	-40	-70	—	BACK	VERT	TOTAL
N	27-3-9	-1467	-1467	—	BACK	VERT	TOTAL
W	28-4-12	-135	-135	—	BACK	VERT	TOTAL
X	16-10-9	-2368	-2368	—	BACK	VERT	TOTAL
Y	17-8-7	-1657	-1657	—	BACK	VERT	TOTAL
Z	28-4-12	-40	-70	—	BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 975 (0.41")

CSI: TC=0.24/1.00 (F-G:1) , BC=0.88/1.00 (O-Q:1) , WB=0.40/1.00 (J-M:1) , SSI=0.73/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (U) (INPUT = 0.90)
JSI METAL= 0.66 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER 47078 S. KATSOULAKOS PROVINCE OF ONTARIO

DWG NO. TAM 18424-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287689	T1	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N	BMWW+t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	6.0	12.0	3.25	6.00
R	BS-t	MT20	5.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 135.3 lbs FACTORED DOWN AT 28-4-12, AND 142.4 lbs FACTORED DOWN AT 29-11-8, AND 238.0 lbs FACTORED DOWN AT 29-11-8 ON TOP CHORD, AND 2368.1 lbs FACTORED DOWN AT 16-10-9, 1656.9 lbs FACTORED DOWN AT 17-8-7, 1467.2 lbs FACTORED DOWN AT 27-3-9, 69.9 lbs FACTORED DOWN AT 28-4-12, AND 69.9 lbs FACTORED DOWN AT 29-10-12, AND 69.9 lbs FACTORED DOWN AT 30-11-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
AA	30-11-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/08
S. KATSOULAKOS
PROVINCE OF ONTARIO

P62/2

DWG NO. TAM 18424-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296707	T1Z	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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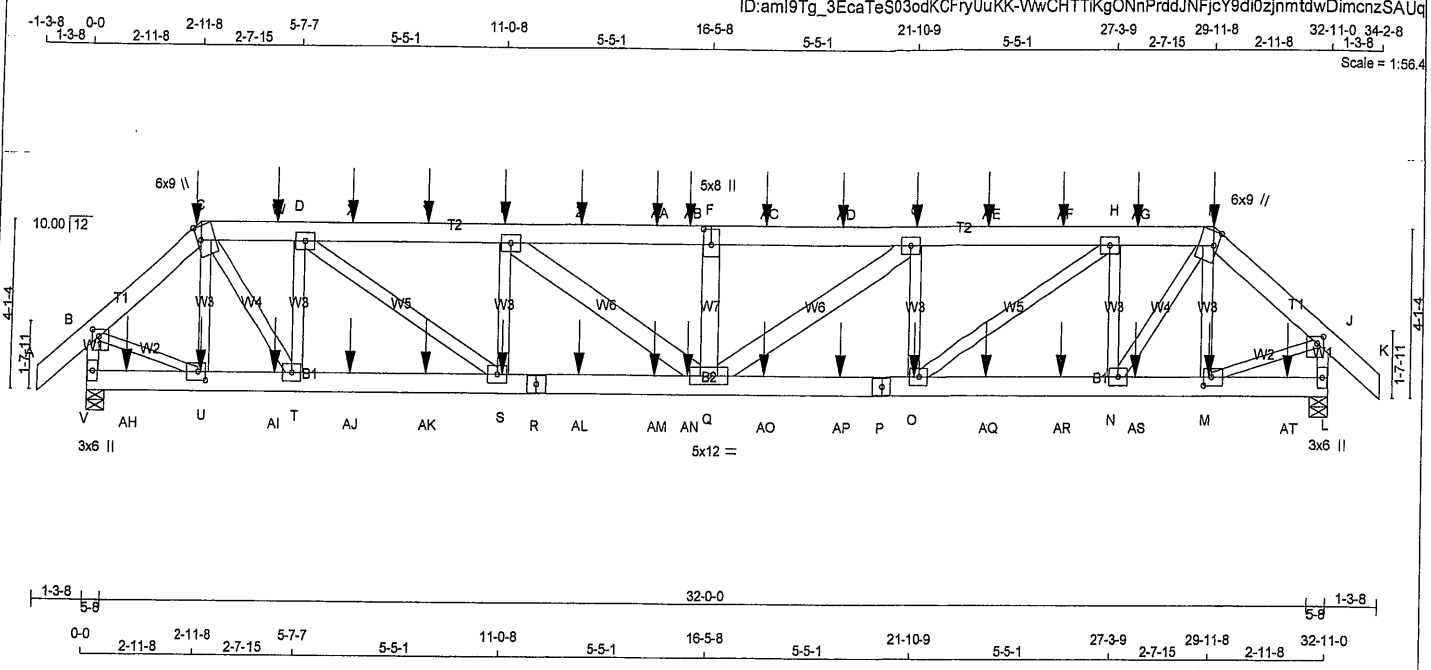

TOTAL WEIGHT = 2 X 207 = 413 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
E - Q	2x6	DRY	No.2	SPF
Q - F	2x6	DRY	No.2	SPF
Q - G	2x6	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(183.1)
F- I	2	12	SIDE(183.1)
I- K	2	12	SIDE(122.0)
V- B	1	12	TOP
L- J	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- R	2	12	SIDE(183.1)
R- P	2	12	SIDE(0.0)
P- L	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.00	2.00
C	TTWW+m	MT20	6.0	9.0	4.00	1.25
D, E, G, H						
D	TMWW-t	MT20	5.0	6.0		
F	TSW+l	MT20	5.0	8.0	4.50	2.50
I	TTWW+m	MT20	6.0	9.0	4.00	1.25
J	TMVW+p	MT20	5.0	6.0	2.00	2.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.50
N, O, S, T						
N	BMWW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4117	0	4117	0	0	5-8	5-8
L	4074	0	4074	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3271	1915 / 0	601 / 0	0 / 0	0 / 0	756 / 0	0 / 0
L	3241	1887 / 0	600 / 0	0 / 0	0 / 0	754 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-4052 / 0	-112.4	-112.4	0.07 (1)	5.60
C- W	-5160 / 0	-112.4	-112.4	0.15 (1)	5.02
W- D	-5160 / 0	-112.4	-112.4	0.15 (1)	5.02
D- X	-7999 / 0	-112.4	-112.4	0.25 (1)	4.12
X- Y	-7999 / 0	-112.4	-112.4	0.25 (1)	4.12
Y- E	-7999 / 0	-112.4	-112.4	0.25 (1)	4.12
E- Z	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
Z-AA	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
AA-AB	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
AB- F	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
F-AC	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
AC-AD	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
AD- G	-8966 / 0	-112.4	-112.4	0.29 (1)	3.88
G-AE	-7962 / 0	-112.4	-112.4	0.25 (1)	4.13
AE-AF	-7962 / 0	-112.4	-112.4	0.25 (1)	4.13
AF- H	-7962 / 0	-112.4	-112.4	0.25 (1)	4.13
H-AG	-5129 / 0	-112.4	-112.4	0.15 (1)	5.03
AG- I	-5129 / 0	-112.4	-112.4	0.15 (1)	5.03
I- J	-4005 / 0	-112.4	-112.4	0.07 (1)	5.63
J- K	0 / 52	-112.4	-112.4	0.05 (1)	10.00
V- B	-4054 / 0	0.0	0.0	0.23 (1)	5.88
L- J	-4011 / 0	0.0	0.0	0.22 (1)	5.91
V-AH	0 / 0	-28.0	-28.0	0.03 (3)	10.00
AH- U	0 / 0	-28.0	-28.0	0.03 (3)	10.00
U-AI	0 / 3056	-28.0	-28.0	0.22 (1)	10.00
AI- T	0 / 3056	-28.0	-28.0	0.22 (1)	10.00
T-AJ	0 / 5162	-28.0	-28.0	0.39 (1)	10.00
AJ-AK	0 / 5162	-28.0	-28.0	0.39 (1)	10.00
AK- S	0 / 5162	-28.0	-28.0	0.39 (1)	10.00
S- R	0 / 7999	-28.0	-28.0	0.58 (1)	10.00
R-AL	0 / 7999	-28.0	-28.0	0.58 (1)	10.00
AL-AM	0 / 7999	-28.0	-28.0	0.58 (1)	10.00
AM-AN	0 / 7999	-28.0	-28.0	0.58 (1)	10.00
AN- Q	0 / 7999	-28.0	-28.0	0.58 (1)	10.00
Q-AO	0 / 7962	-28.0	-28.0	0.58 (1)	10.00
AO-AP	0 / 7962	-28.0	-28.0	0.58 (1)	10.00
AP- P	0 / 7962	-28.0	-28.0	0.58 (1)	10.00
P- O	0 / 7962	-28.0	-28.0	0.58 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-976 / 0	0.08 (1)
C- T	0 / 3548	0.31 (1)
T- D	-2645 / 0	0.23 (1)
D- S	0 / 3420	0.30 (1)
S- E	-1657 / 0	0.14 (1)
E- Q	0 / 1165	0.08 (1)
Q- F	-1041 / 0	0.06 (1)
Q- G	0 / 1210	0.08 (1)
O- G	-1660 / 0	0.14 (1)
O- H	0 / 3413	0.30 (1)
N- H	-2649 / 0	0.23 (1)
N- I	0 / 3555	0.31 (1)
M- I	-963 / 0	0.08 (1)
B- U	0 / 3288	0.41 (1)
M- J	0 / 3250	0.40 (1)

LICENSED PROFESSIONAL ENGINEER
41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.34")

CSI: TC=0.29/1.00 (F-G:1) , BC=0.58/1.00 (Q-S:1) , WB=0.41/1.00 (B-U:1) , SSI=0.19/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

pb 14

DWG NO. TAM 18446-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296707	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:amI9Tg_3EcaTeS03odKCFryUuKK-WwCHTTiKgONnPrddJNFjcY9di0zjnmtdwDimcnzSAUo

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	12.0		
R	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.50
V	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.0 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-4, 135.3 lbs FACTORED DOWN AT 7-0-4, 135.3 lbs FACTORED DOWN AT 9-0-4, 135.3 lbs FACTORED DOWN AT 11-0-4, 135.3 lbs FACTORED DOWN AT 13-0-4, 135.3 lbs FACTORED DOWN AT 15-0-4, 135.3 lbs FACTORED DOWN AT 15-10-12, 135.3 lbs FACTORED DOWN AT 17-10-12, 135.3 lbs FACTORED DOWN AT 19-10-12, 135.3 lbs FACTORED DOWN AT 21-10-12, 135.3 lbs FACTORED DOWN AT 23-10-12, 135.3 lbs FACTORED DOWN AT 25-10-12, 135.3 lbs FACTORED DOWN AT 27-10-12, 142.3 lbs FACTORED DOWN AT 29-11-8, AND 238.0 lbs FACTORED DOWN AT 2-11-8, AND 238.0 lbs FACTORED DOWN AT 29-11-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, 69.9 lbs FACTORED DOWN AT 9-0-4, 69.9 lbs FACTORED DOWN AT 11-0-4, 69.9 lbs FACTORED DOWN AT 13-0-4, 69.9 lbs FACTORED DOWN AT 15-0-4, 69.9 lbs FACTORED DOWN AT 15-10-12, 69.9 lbs FACTORED DOWN AT 17-10-12, 69.9 lbs FACTORED DOWN AT 19-10-12, 69.9 lbs FACTORED DOWN AT 21-10-12, 69.9 lbs FACTORED DOWN AT 23-10-12, 69.9 lbs FACTORED DOWN AT 25-10-12, 69.9 lbs FACTORED DOWN AT 27-10-12, AND 69.9 lbs FACTORED DOWN AT 29-10-12, AND 69.9 lbs FACTORED DOWN AT 31-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
O-AQ	0 / 5130	-28.0	-28.0	0.39	(1)	10.00
AQ-AR	0 / 5130	-28.0	-28.0	0.39	(1)	10.00
AR- N	0 / 5130	-28.0	-28.0	0.39	(1)	10.00
N-AS	0 / 3020	-28.0	-28.0	0.22	(1)	10.00
AS- M	0 / 3020	-28.0	-28.0	0.22	(1)	10.00
M-AT	0 / 0	-28.0	-28.0	0.03	(3)	10.00
AT- L	0 / 0	-28.0	-28.0	0.03	(3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-188	-188	—	BACK	VERT	TOTAL
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
E	11-0-4	-135	-135	—	BACK	VERT	TOTAL
G	21-10-12	-135	-135	—	BACK	VERT	TOTAL
I	29-11-8	-142	-142	—	BACK	VERT	TOTAL
I	29-11-8	-33	-36	—	FRONT	VERT	DEAD
I	29-11-8	-206	-206	—	FRONT	VERT	SNOW
M	29-10-12	-40	-70	—	BACK	VERT	TOTAL
O	21-10-12	-40	-70	—	BACK	VERT	TOTAL
S	11-0-4	-40	-70	—	BACK	VERT	TOTAL
U	3-0-4	-40	-70	—	BACK	VERT	TOTAL
W	5-0-4	-135	-135	—	BACK	VERT	TOTAL
X	7-0-4	-135	-135	—	BACK	VERT	TOTAL
Y	9-0-4	-135	-135	—	BACK	VERT	TOTAL
Z	13-0-4	-135	-135	—	BACK	VERT	TOTAL
AA	15-0-4	-135	-135	—	BACK	VERT	TOTAL
AB	15-10-12	-135	-135	—	BACK	VERT	TOTAL
AC	17-10-12	-135	-135	—	BACK	VERT	TOTAL
AD	19-10-12	-135	-135	—	BACK	VERT	TOTAL
AE	23-10-12	-135	-135	—	BACK	VERT	TOTAL
AF	25-10-12	-135	-135	—	BACK	VERT	TOTAL
AG	27-10-12	-135	-135	—	BACK	VERT	TOTAL
AH	1-0-4	-40	-70	—	BACK	VERT	TOTAL
AI	5-0-4	-40	-70	—	BACK	VERT	TOTAL
AJ	7-0-4	-40	-70	—	BACK	VERT	TOTAL
AK	9-0-4	-40	-70	—	BACK	VERT	TOTAL
AL	13-0-4	-40	-70	—	BACK	VERT	TOTAL
AM	15-0-4	-40	-70	—	BACK	VERT	TOTAL
AN	15-10-12	-40	-70	—	BACK	VERT	TOTAL
AO	17-10-12	-40	-70	—	BACK	VERT	TOTAL
AP	19-10-12	-40	-70	—	BACK	VERT	TOTAL
AQ	23-10-12	-40	-70	—	BACK	VERT	TOTAL
AR	25-10-12	-40	-70	—	BACK	VERT	TOTAL
AS	27-10-12	-40	-70	—	BACK	VERT	TOTAL
AT	31-10-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4107B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18446-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T1S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 10 09:33:08 2018 Page 1
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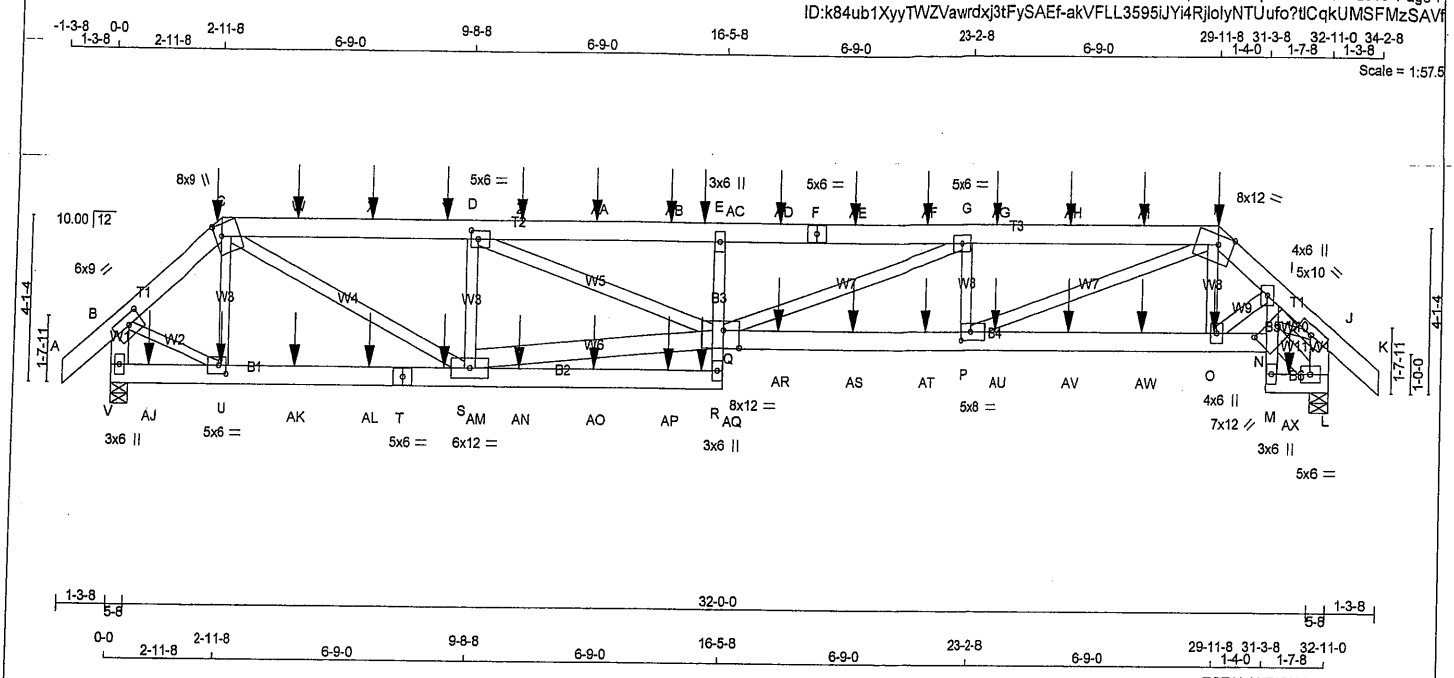

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
T - R	2x6	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
B - U	2x3	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
N - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- F	2	12	SIDE(61.0)
F- H	2	12	SIDE(61.0)
H- K	2	12	SIDE(122.0)
V- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- T	2	12	SIDE(183.1)
T- R	2	12	SIDE(0.0)
Q- N	2	12	SIDE(183.1)
M- L	2	12	SIDE(0.0)
R- E	1	12	TOP
I- M	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	9.0	2.75	4.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.00
D	TMWW-t	MT20	5.0	6.0	2.50	2.25
E	TMV+p	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4270	0	4270	0	0	5-8	5-8
L	4491	0	4491	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3394	1985 / 0	625 / 0	0 / 0	0 / 0	784 / 0	0 / 0
L	3574	2080 / 0	663 / 0	0 / 0	0 / 0	832 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00	
B- C	-4266 / 0	-112.4	-112.4	0.07 (1)	5.49	
C- W	-7928 / 0	-112.4	-112.4	0.45 (1)	3.90	
W- X	-7928 / 0	-112.4	-112.4	0.45 (1)	3.90	
X- Y	-7928 / 0	-112.4	-112.4	0.45 (1)	3.90	
Y- D	-7928 / 0	-112.4	-112.4	0.45 (1)	3.90	
D- Z	-13103 / 0	-112.4	-112.4	0.68 (1)	2.89	
Z-AA	-13103 / 0	-112.4	-112.4	0.68 (1)	2.89	
AA-AB	-13103 / 0	-112.4	-112.4	0.68 (1)	2.89	
AB-AC	-13103 / 0	-112.4	-112.4	0.68 (1)	2.89	
AC- E	-13103 / 0	-112.4	-112.4	0.68 (1)	2.89	
E-AD	-13259 / 0	-112.4	-112.4	0.63 (1)	2.95	
AD- F	-13259 / 0	-112.4	-112.4	0.63 (1)	2.95	
F-AE	-13259 / 0	-112.4	-112.4	0.63 (1)	2.95	
AE-AF	-13259 / 0	-112.4	-112.4	0.63 (1)	2.95	
AF- G	-13259 / 0	-112.4	-112.4	0.63 (1)	2.95	
G-AG	-11500 / 0	-112.4	-112.4	0.58 (1)	3.20	
AG-AH	-11500 / 0	-112.4	-112.4	0.58 (1)	3.20	
AH-AI	-11500 / 0	-112.4	-112.4	0.58 (1)	3.20	
AI- H	-11500 / 0	-112.4	-112.4	0.58 (1)	3.20	
H- I	-6068 / 0	-112.4	-112.4	0.24 (1)	4.58	
I- J	-6616 / 0	-112.4	-112.4	0.25 (1)	4.41	
J- K	0 / 52	-112.4	-112.4	0.05 (1)	10.00	
V- B	-4250 / 0	0.0	0.0	0.15 (1)	6.94	
L- J	-4326 / 0	0.0	0.0	0.16 (1)	6.89	
V-AJ	0 / 0	-28.0	-28.0	0.08 (2)	10.00	
AJ- U	0 / 0	-28.0	-28.0	0.08 (2)	10.00	
U-AK	0 / 3223	-28.0	-28.0	0.27 (2)	10.00	
AK-AL	0 / 3223	-28.0	-28.0	0.27 (2)	10.00	
AL- T	0 / 3223	-28.0	-28.0	0.27 (2)	10.00	
T-AM	0 / 3223	-28.0	-28.0	0.27 (2)	10.00	
AM- S	0 / 3223	-28.0	-28.0	0.27 (2)	10.00	
S-AN	0 / 412	-28.0	-28.0	0.15 (3)	10.00	
AN-AO	0 / 412	-28.0	-28.0	0.15 (3)	10.00	
AO-AP	0 / 412	-28.0	-28.0	0.15 (3)	10.00	
AP-AQ	0 / 412	-28.0	-28.0	0.15 (3)	10.00	
AQ- R	0 / 412	-28.0	-28.0	0.15 (3)	10.00	
R- Q	0 / 303	0.0	0.0	0.32 (1)	10.00	
Q- E	-1134 / 0	0.0	0.0	0.32 (1)	7.81	
Q-AR	0 / 11501	-28.0	-28.0	0.93 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-879 / 0	0.08 (1)
C- S	0 / 5366	0.47 (1)
S- D	-3387 / 0	0.29 (1)
S- Q	0 / 7750	0.50 (1)
D- Q	0 / 5413	0.48 (1)
Q- G	0 / 1888	0.17 (1)
P- G	-2021 / 0	0.13 (1)
P- H	0 / 7172	0.63 (1)
O- H	0 / 368	0.03 (3)
O- I	-341 / 0	0.02 (1)
B- U	0 / 3461	0.43 (1)
N- L	-201 / 0	0.01 (1)
N- J	0 / 4888	0.60 (1)

LICENSED PROFESSIONAL ENGINEER
41076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 972 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 592 (0.67")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.03/1.00 (P-Q:1) , WB=0.63/1.00 (H-P:1) , SSI=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

pg 4

DWG NO. TAM 18425-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T1S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:k84ub1XyyTWZVawrdxj3tFySAEf-akVFLL3595iJYi4RjloIyNTUufo?tlCqkUMSFMzSAVf

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TMWW-t	MT20	5.0	6.0		
H	TTWW-m	MT20	8.0	12.0	Edge	4.75
I	TMVW+p	MT20	4.0	6.0		
J	TMVW-t	MT20	5.0	10.0		
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	6.0		
N	BVMWW-w	MT20	7.0	12.0	2.75	7.50
O	BMWW+t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	3.25
Q	BVMWWW-l	MT20	8.0	12.0	5.25	5.00
R	BMV+p	MT20	3.0	6.0		
S	BMWWW-t	MT20	6.0	12.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 188.0 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-4, 135.3 lbs FACTORED DOWN AT 7-0-4, 135.3 lbs FACTORED DOWN AT 9-0-4, 135.3 lbs FACTORED DOWN AT 11-0-4, 135.3 lbs FACTORED DOWN AT 13-0-4, 135.3 lbs FACTORED DOWN AT 15-0-4, 135.3 lbs FACTORED DOWN AT 15-10-12, 138.8 lbs FACTORED DOWN AT 17-10-12, 138.8 lbs FACTORED DOWN AT 19-10-12, 138.8 lbs FACTORED DOWN AT 21-10-12, 138.8 lbs FACTORED DOWN AT 23-10-12, 138.8 lbs FACTORED DOWN AT 25-10-12, 138.8 lbs FACTORED DOWN AT 27-10-12, 145.8 lbs FACTORED DOWN AT 29-11-8, AND 238.0 lbs FACTORED DOWN AT 2-11-8, AND 238.0 lbs FACTORED DOWN AT 29-11-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, 69.9 lbs FACTORED DOWN AT 9-0-4, 69.9 lbs FACTORED DOWN AT 11-0-4, 69.9 lbs FACTORED DOWN AT 13-0-4, 69.9 lbs FACTORED DOWN AT 15-0-4, 84.2 lbs FACTORED DOWN AT 15-10-12, 115.6 lbs FACTORED DOWN AT 17-10-12, 115.6 lbs FACTORED DOWN AT 19-10-12, 115.6 lbs FACTORED DOWN AT 21-10-12, 115.6 lbs FACTORED DOWN AT 23-10-12, 115.6 lbs FACTORED DOWN AT 25-10-12, 115.6 lbs FACTORED DOWN AT 27-10-12, AND 115.6 lbs FACTORED DOWN AT 29-10-12, AND 83.2 lbs FACTORED DOWN AT 31-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING

TOTAL LOAD CASES: (4)

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.65 (D) (INPUT = 1.00)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
AR-AS	0 / 11501	-28.0	-28.0	0.93 (1)	10.00
AS-AT	0 / 11501	-28.0	-28.0	0.93 (1)	10.00
AT- P	0 / 11501	-28.0	-28.0	0.93 (1)	10.00
P-AU	0 / 4793	-28.0	-28.0	0.49 (1)	10.00
AU-AV	0 / 4793	-28.0	-28.0	0.49 (1)	10.00
AV-AW	0 / 4793	-28.0	-28.0	0.49 (1)	10.00
AW- O	0 / 4793	-28.0	-28.0	0.49 (1)	10.00
O- N	0 / 5054	-28.0	-28.0	0.50 (1)	10.00
M- N	0 / 94	0.0	0.0	0.15 (1)	10.00
N- I	0 / 794	0.0	0.0	0.21 (1)	10.00
M-AX	0 / 154	-28.0	-28.0	0.02 (2)	10.00
AX- L	0 / 154	-28.0	-28.0	0.02 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-188	-188	—	BACK	VERT	TOTAL
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
H	29-11-8	-146	-146	—	BACK	VERT	TOTAL
H	29-11-8	-33	-36	—	FRONT	VERT	DEAD
H	29-11-8	-206	-206	—	FRONT	VERT	SNOW
O	29-10-12	-116	-116	—	BACK	VERT	TOTAL
U	3-0-4	-40	-70	—	BACK	VERT	TOTAL
W	5-0-4	-135	-135	—	BACK	VERT	TOTAL
X	7-0-4	-135	-135	—	BACK	VERT	TOTAL
Y	9-0-4	-135	-135	—	BACK	VERT	TOTAL
Z	11-0-4	-135	-135	—	BACK	VERT	TOTAL
AA	13-0-4	-135	-135	—	BACK	VERT	TOTAL
AB	15-0-4	-135	-135	—	BACK	VERT	TOTAL
AC	15-10-12	-135	-135	—	BACK	VERT	TOTAL
AD	17-10-12	-139	-139	—	BACK	VERT	TOTAL
AE	19-10-12	-139	-139	—	BACK	VERT	TOTAL
AF	21-10-12	-139	-139	—	BACK	VERT	TOTAL
AG	23-10-12	-139	-139	—	BACK	VERT	TOTAL
AH	25-10-12	-139	-139	—	BACK	VERT	TOTAL
AI	27-10-12	-139	-139	—	BACK	VERT	TOTAL
AJ	1-0-4	-40	-70	—	BACK	VERT	TOTAL
AK	5-0-4	-40	-70	—	BACK	VERT	TOTAL
AL	7-0-4	-40	-70	—	BACK	VERT	TOTAL
AM	9-0-4	-40	-70	—	BACK	VERT	TOTAL
AN	11-0-4	-40	-70	—	BACK	VERT	TOTAL
AO	13-0-4	-40	-70	—	BACK	VERT	TOTAL
AP	15-0-4	-40	-70	—	BACK	VERT	TOTAL
AQ	15-10-12	-48	-84	—	BACK	VERT	TOTAL
AR	17-10-12	-116	-116	—	BACK	VERT	TOTAL
AS	19-10-12	-116	-116	—	BACK	VERT	TOTAL
AT	21-10-12	-116	-116	—	BACK	VERT	TOTAL
AU	23-10-12	-116	-116	—	BACK	VERT	TOTAL
AV	25-10-12	-116	-116	—	BACK	VERT	TOTAL
AW	27-10-12	-116	-116	—	BACK	VERT	TOTAL
AX	31-10-12	-48	-83	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4 1016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18425 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T2	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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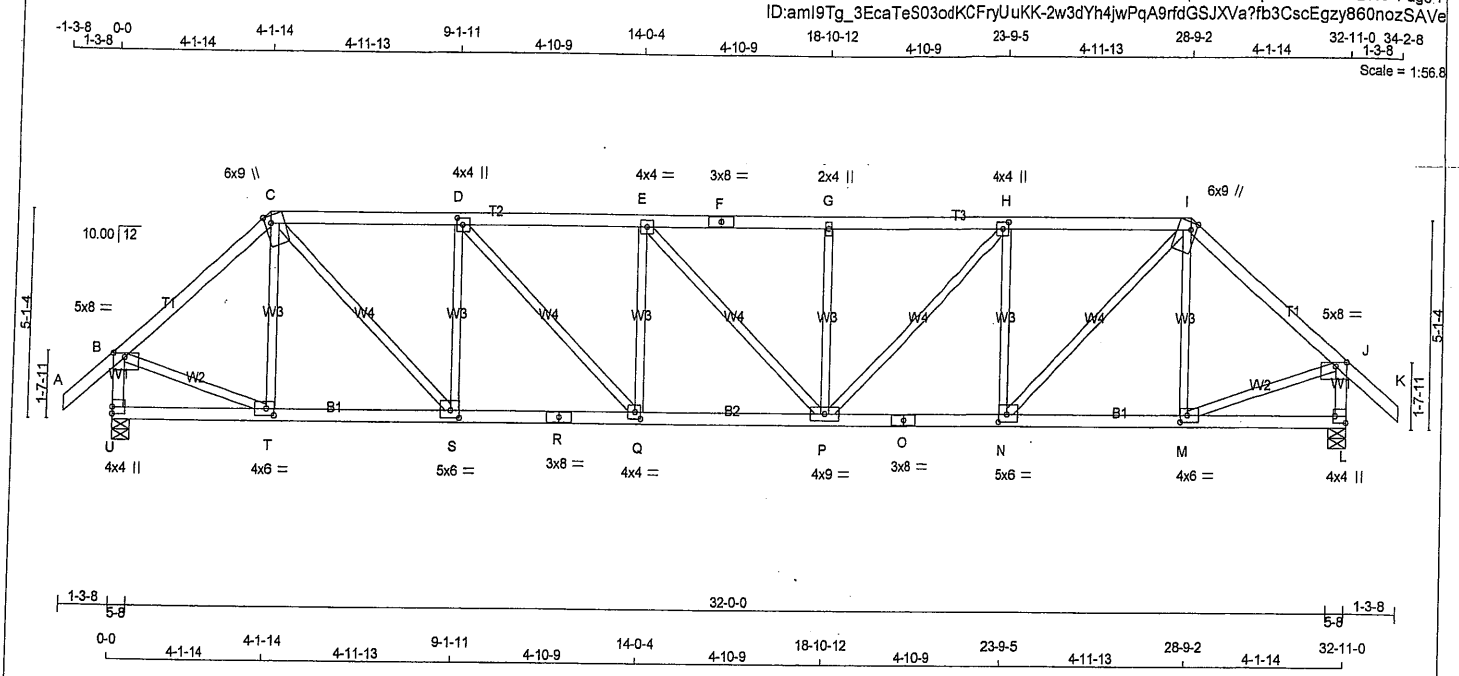

TOTAL WEIGHT = 140 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW+t	MT20	4.0	4.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	4.0	6.0	2.00	2.25
N	BMWW-t	MT20	5.0	6.0	2.25	2.75
O	BS-t	MT20	3.0	8.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.75
R	BS-t	MT20	3.0	8.0		
S	BMWW-t	MT20	5.0	6.0	2.25	2.75
T	BMWW-t	MT20	4.0	6.0	2.00	2.25
U	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2316 / 0	-112.4	-112.4	0.46 (1)	4.05
C- D	-3235 / 0	-112.4	-112.4	0.60 (1)	3.35
D- E	-3914 / 0	-112.4	-112.4	0.68 (1)	2.98
E- F	-3912 / 0	-112.4	-112.4	0.56 (1)	3.14
F- G	-3912 / 0	-112.4	-112.4	0.56 (1)	3.14
G- H	-3912 / 0	-112.4	-112.4	0.69 (1)	2.98
H- I	-3235 / 0	-112.4	-112.4	0.60 (1)	3.34
I- J	-2316 / 0	-112.4	-112.4	0.46 (1)	4.05
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
U- B	-2421 / 0	0.0	0.0	0.26 (1)	5.50
L- J	-2421 / 0	0.0	0.0	0.26 (1)	5.50
U- T	0 / 0	-28.0	-28.0	0.15 (3)	10.00
T- S	0 / 1766	-28.0	-28.0	0.37 (1)	10.00
S- R	0 / 3235	-28.0	-28.0	0.58 (1)	10.00
R- Q	0 / 3235	-28.0	-28.0	0.58 (1)	10.00
Q- P	0 / 3914	-28.0	-28.0	0.69 (1)	10.00
P- O	0 / 3236	-28.0	-28.0	0.58 (1)	10.00
O- N	0 / 3236	-28.0	-28.0	0.58 (1)	10.00
N- M	0 / 1766	-28.0	-28.0	0.36 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-398 / 0	0.15 (1)
C- S	0 / 2038	0.46 (1)
S- D	-1279 / 0	0.49 (1)
D- Q	0 / 954	0.21 (1)
Q- E	-532 / 0	0.20 (1)
E- P	-3 / 0	0.00 (1)
P- G	-531 / 0	0.20 (1)
P- H	0 / 950	0.21 (1)
N- H	-1278 / 0	0.49 (1)
N- I	0 / 2038	0.46 (1)
M- I	-398 / 0	0.15 (1)
B- T	0 / 1856	0.42 (1)
M- J	0 / 1856	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.39")

CSI: TC=0.69/1.00 (G-H:1) , BC=0.69/1.00 (P-Q:1) , WB=0.49/1.00 (D-S:1) , SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.90 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/07
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18426 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T2S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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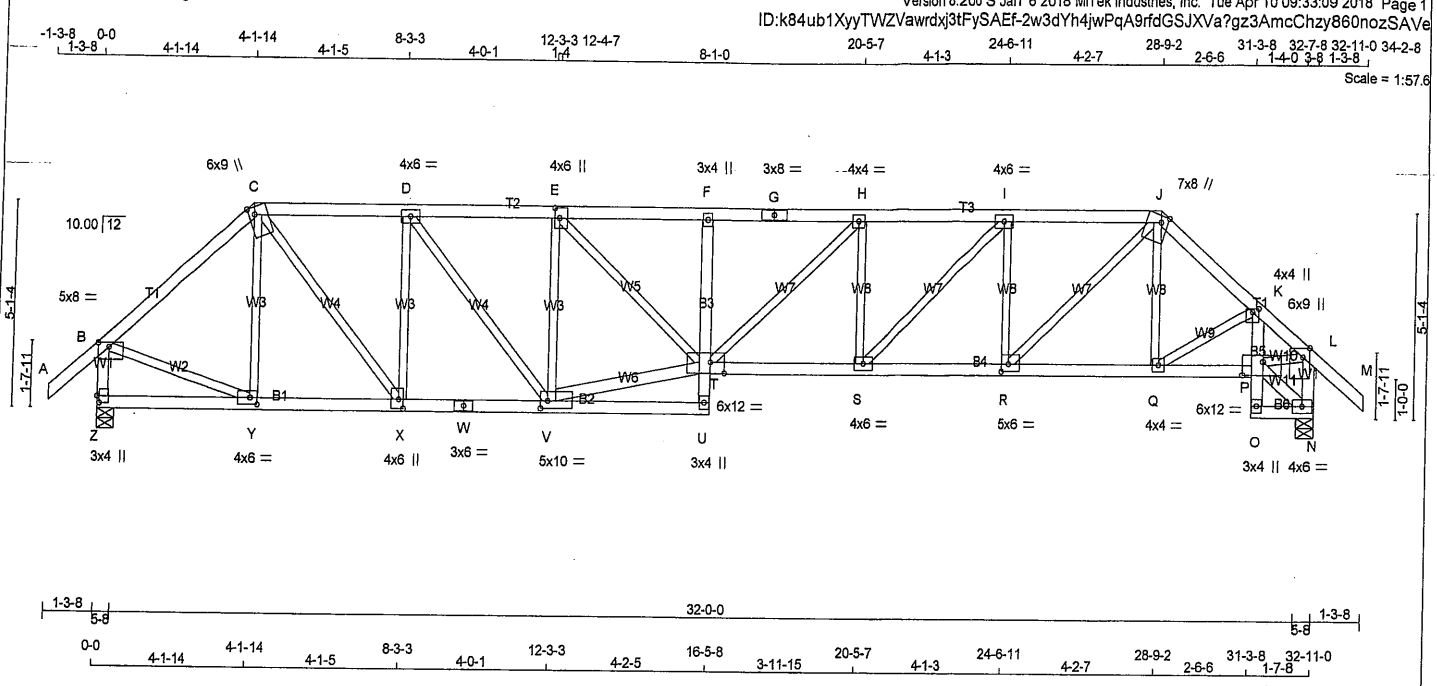

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
Z - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - W	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - F	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMWW+t	MT20	4.0	6.0	3.00	1.50
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.25
K	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	TMVW+p	MT20	6.0	9.0	2.75	2.00
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWW-l	MT20	6.0	12.0	4.00	6.25
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW-t	MT20	5.0	6.0	2.25	2.50
S	BMWW-t	MT20	4.0	6.0		
T	BVMWWW-l	MT20	6.0	12.0	3.25	4.50
U	BMV+p	MT20	3.0	4.0		
V	BMWWW-t	MT20	5.0	10.0	2.25	2.25
W	BS-t	MT20	3.0	6.0		
X	BMWW+t	MT20	4.0	6.0	2.75	1.50
Y	BMWW-t	MT20	4.0	6.0	2.00	2.25
Z	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	2464	0	2464	0	0	5-8	5-8
N	2470	0	2470	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
N	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2310 / 0	-112.4	-112.4	0.46 (1)	4.05
C- D	-3008 / 0	-112.4	-112.4	0.40 (1)	3.67
D- E	-3710 / 0	-112.4	-112.4	0.47 (1)	3.28
E- F	-4975 / 0	-112.4	-112.4	0.55 (1)	2.80
F- G	-5007 / 0	-112.4	-112.4	0.56 (1)	2.78
G- H	-5007 / 0	-112.4	-112.4	0.56 (1)	2.78
H- I	-4716 / 0	-112.4	-112.4	0.60 (1)	2.80
I- J	-3813 / 0	-112.4	-112.4	0.49 (1)	3.23
J- K	-2877 / 0	-112.4	-112.4	0.24 (1)	3.91
K- L	-2979 / 0	-112.4	-112.4	0.24 (1)	3.83
L- M	0 / 50	-112.4	-112.4	0.15 (1)	10.00
Z- B	-2416 / 0	0.0	0.0	0.26 (1)	5.50
N- L	-2389 / 0	0.0	0.0	0.25 (1)	5.53
Z- Y	0 / 0	-28.0	-28.0	0.13 (3)	10.00
Y- X	0 / 1762	-28.0	-28.0	0.36 (1)	10.00
X- W	0 / 3008	-28.0	-28.0	0.53 (1)	10.00
W- V	0 / 3008	-28.0	-28.0	0.53 (1)	10.00
V- U	0 / 125	-28.0	-28.0	0.12 (3)	10.00
U- T	0 / 82	0.0	0.0	0.18 (1)	10.00
T- F	-459 / 0	0.0	0.0	0.19 (1)	7.81
T- S	0 / 4716	-28.0	-28.0	0.83 (1)	10.00
S- R	0 / 3813	-28.0	-28.0	0.67 (1)	10.00
R- Q	0 / 2202	-28.0	-28.0	0.41 (1)	10.00
Q- P	0 / 2322	-28.0	-28.0	0.43 (1)	10.00
O- P	0 / 36	0.0	0.0	0.13 (1)	10.00
P- K	-72 / 28	0.0	0.0	0.12 (1)	7.81
O- N	0 / 76	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- C	-406 / 0	0.16 (1)
Q- J	0 / 151	0.04 (3)
Q- K	-134 / 0	0.03 (1)
B- Y	0 / 1852	0.42 (1)
P- N	-97 / 0	0.01 (1)
P- L	0 / 2262	0.51 (1)
R- J	0 / 2168	0.49 (1)
T- H	0 / 396	0.09 (1)
R- I	-1343 / 0	0.34 (1)
S- H	-719 / 0	0.18 (1)
S- I	0 / 1233	0.28 (1)
C- X	0 / 1915	0.43 (1)
V- E	-1610 / 0	0.62 (1)
X- D	-1356 / 0	0.52 (1)
D- V	0 / 1109	0.25 (1)
V- T	0 / 3713	0.60 (1)
E- T	0 / 1706	0.38 (1)

LICENSED PROFESSIONAL ENGINEER
4196
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 745 (0.53")

CSI: TC=0.60/1.00 (H-I:1) , BC=0.83/1.00 (S-T:1) , WB=0.62/1.00 (E-V:1) , SSI=0.22/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.88 (E) (INPUT = 0.90)
CSI METAL= 0.80 (W) (INPUT = 1.00)

DWG NO. TAM 18427 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T3	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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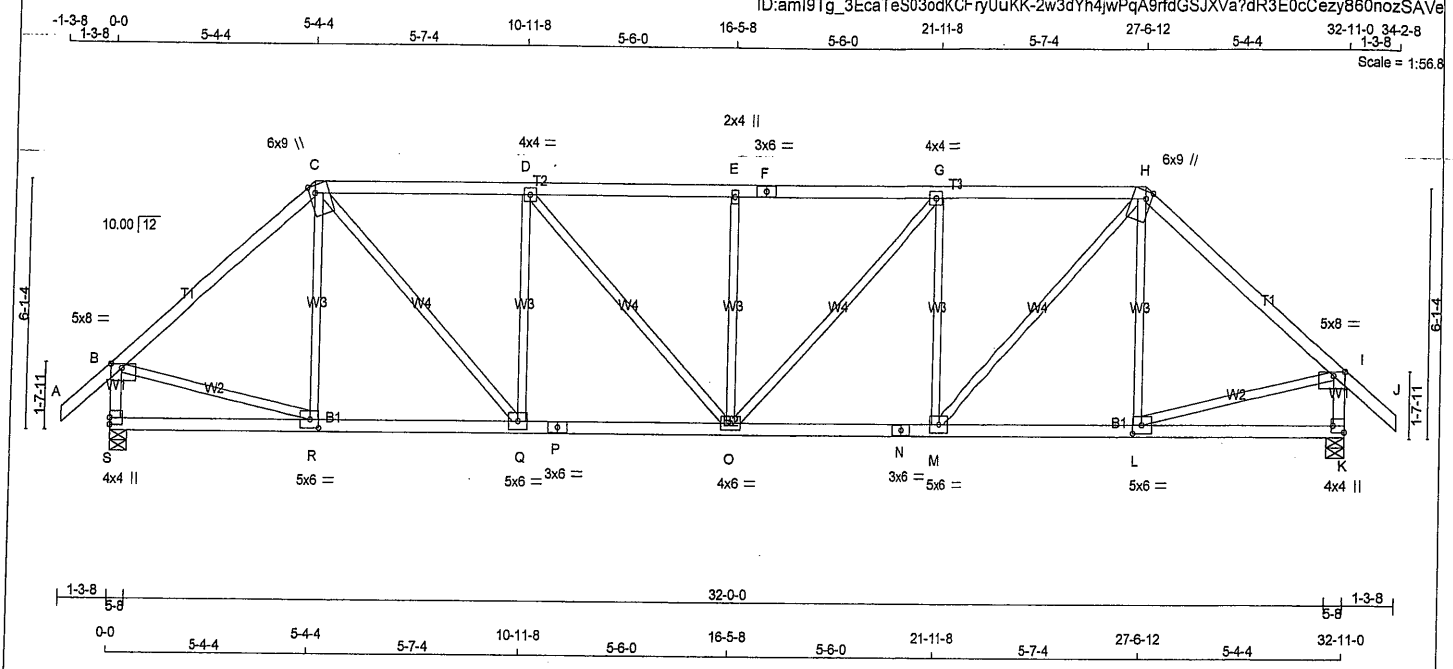

TOTAL WEIGHT = 142 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMVW-p	MT20	5.0	8.0	1.25	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2467	0	2467	0	0	5-8	5-8
K	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
K	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2362 / 0	-112.4	-112.4	0.82 (1)	3.58
C- D	-2978 / 0	-112.4	-112.4	0.75 (1)	3.28
D- E	-3320 / 0	-112.4	-112.4	0.79 (1)	3.06
E- F	-3320 / 0	-112.4	-112.4	0.79 (1)	3.06
F- G	-3320 / 0	-112.4	-112.4	0.79 (1)	3.06
G- H	-2978 / 0	-112.4	-112.4	0.75 (1)	3.28
H- I	-2362 / 0	-112.4	-112.4	0.82 (1)	3.58
I- J	0 / 50	-112.4	-112.4	0.15 (1)	10.00
S- B	-2406 / 0	0.0	0.0	0.26 (1)	5.52
K- I	-2406 / 0	0.0	0.0	0.26 (1)	5.52
S- R	0 / 0	-28.0	-28.0	0.22 (3)	10.00
R- Q	0 / 1806	-28.0	-28.0	0.41 (2)	10.00
Q- P	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
P- O	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
O- N	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
N- M	0 / 2978	-28.0	-28.0	0.55 (1)	10.00
M- L	0 / 1806	-28.0	-28.0	0.41 (2)	10.00
L- K	0 / 0	-28.0	-28.0	0.22 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-258 / 103	0.15 (1)
C- Q	0 / 1685	0.38 (1)
Q- D	-1062 / 0	0.62 (1)
D- O	0 / 498	0.11 (1)
O- E	-569 / 0	0.33 (1)
O- G	0 / 498	0.11 (1)
M- G	-1062 / 0	0.62 (1)
M- H	0 / 1685	0.38 (1)
L- H	-258 / 103	0.15 (1)
B- R	0 / 1862	0.42 (1)
L- I	0 / 1862	0.42 (1)

LICENSED PROFESSIONAL ENGINEER
4/04/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LCAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.82/1.00 (H-I:1) , BC=0.55/1.00 (M-O:1) , WB=0.62/1.00 (D-Q:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (P) (INPUT = 0.90)
JSI METAL= 0.75 (N) (INPUT = 1.00)

DWG NO. TAM 1842B -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T3S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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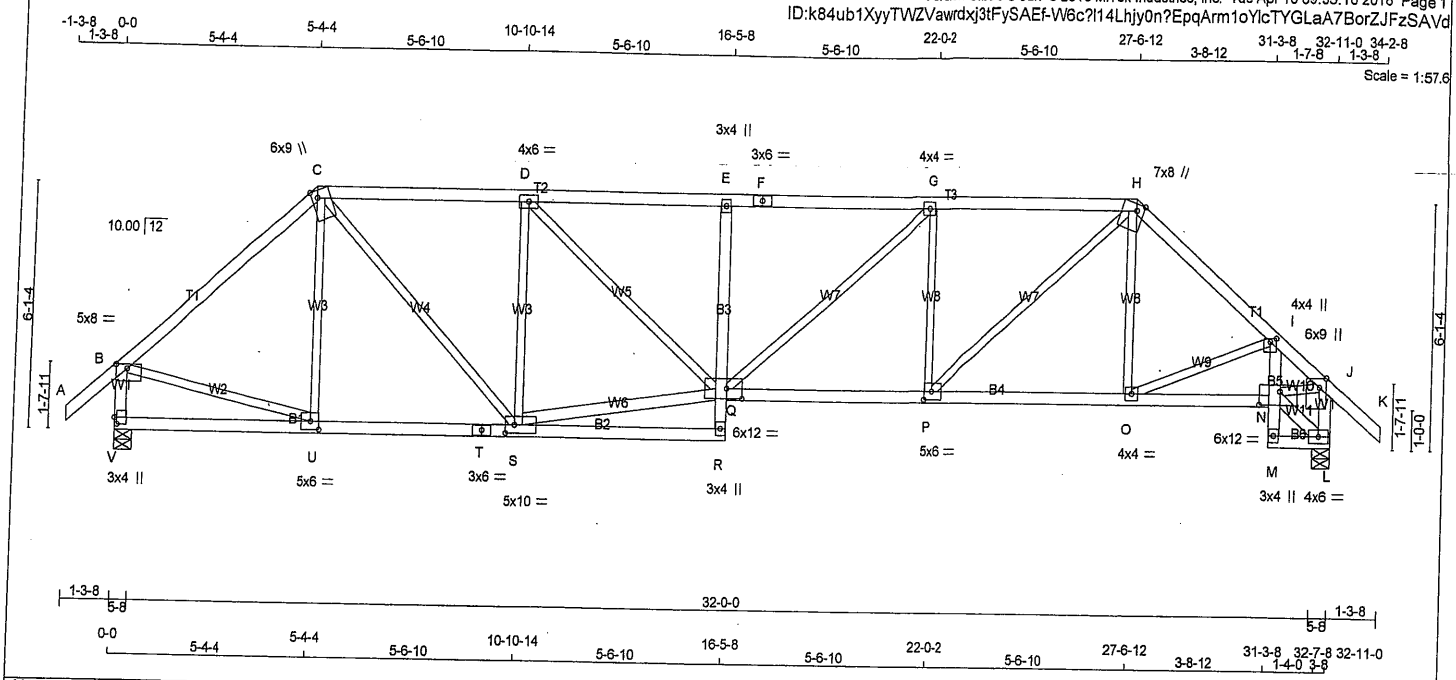

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	1.75
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	7.0	8.0	Edge	2.25
I	TMVW+p	MT20	4.0	4.0	1.00	2.00
J	TMVW+p	MT20	6.0	9.0	2.75	2.00
L	BMVW1-t	MT20	4.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-l	MT20	6.0	12.0	4.00	6.50
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BVMWWW-l	MT20	6.0	12.0	3.00	5.00
R	BMV+p	MT20	3.0	4.0		
S	BMWWW-t	MT20	5.0	10.0	2.50	3.00
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	4.0	2.00	0.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2464	0	2464	0	0	5-8	5-8
L	2470	0	2470	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
L	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2357 / 0	-112.4	-112.4	0.82 (1)	3.59
C- D	-2952 / 0	-112.4	-112.4	0.72 (1)	3.32
D- E	-3966 / 0	-112.4	-112.4	0.87 (1)	2.73
E- F	-3986 / 0	-112.4	-112.4	0.92 (1)	2.68
F- G	-3986 / 0	-112.4	-112.4	0.92 (1)	2.68
G- H	-3595 / 0	-112.4	-112.4	0.86 (1)	2.89
H- I	-2831 / 0	-112.4	-112.4	0.38 (1)	3.82
I- J	-3026 / 0	-112.4	-112.4	0.24 (1)	3.80
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
V- B	-2402 / 0	0.0	0.0	0.26 (1)	5.52
L- J	-2388 / 0	0.0	0.0	0.25 (1)	5.53
V- U	0 / 0	-28.0	-28.0	0.20 (3)	10.00
U- T	0 / 1802	-28.0	-28.0	0.40 (1)	10.00
T- S	0 / 1802	-28.0	-28.0	0.40 (1)	10.00
S- R	0 / 97	-28.0	-28.0	0.21 (3)	10.00
R- Q	0 / 109	0.0	0.0	0.14 (1)	10.00
Q- E	-567 / 0	0.0	0.0	0.22 (1)	7.81
Q- P	0 / 3595	-28.0	-28.0	0.68 (1)	10.00
P- O	0 / 2160	-28.0	-28.0	0.45 (1)	10.00
O- N	0 / 2388	-28.0	-28.0	0.47 (1)	10.00
M- N	0 / 36	0.0	0.0	0.13 (1)	10.00
N- I	-104 / 34	0.0	0.0	0.12 (1)	7.81
M- L	0 / 77	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-264 / 90	0.15 (1)
C- S	0 / 1674	0.38 (1)
S- D	-1577 / 0	0.92 (1)
Q- G	0 / 515	0.12 (1)
P- G	-1046 / 0	0.40 (1)
P- H	0 / 1882	0.42 (1)
O- H	0 / 232	0.05 (3)
O- I	-246 / 0	0.07 (1)
B- U	0 / 1858	0.42 (1)
S- Q	0 / 2922	0.47 (1)
D- Q	0 / 1330	0.30 (1)
N- L	-99 / 0	0.01 (1)
N- J	0 / 2329	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.24")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 974 (0.41")

CSI: TC=0.92/1.00 (E-G:1) , BC=0.68/1.00 (P-Q:1) , WB=0.92/1.00 (D-S:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (D) (INPUT = 0.90)
JSI METAL= 0.52 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/98
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 18429 -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287689	T4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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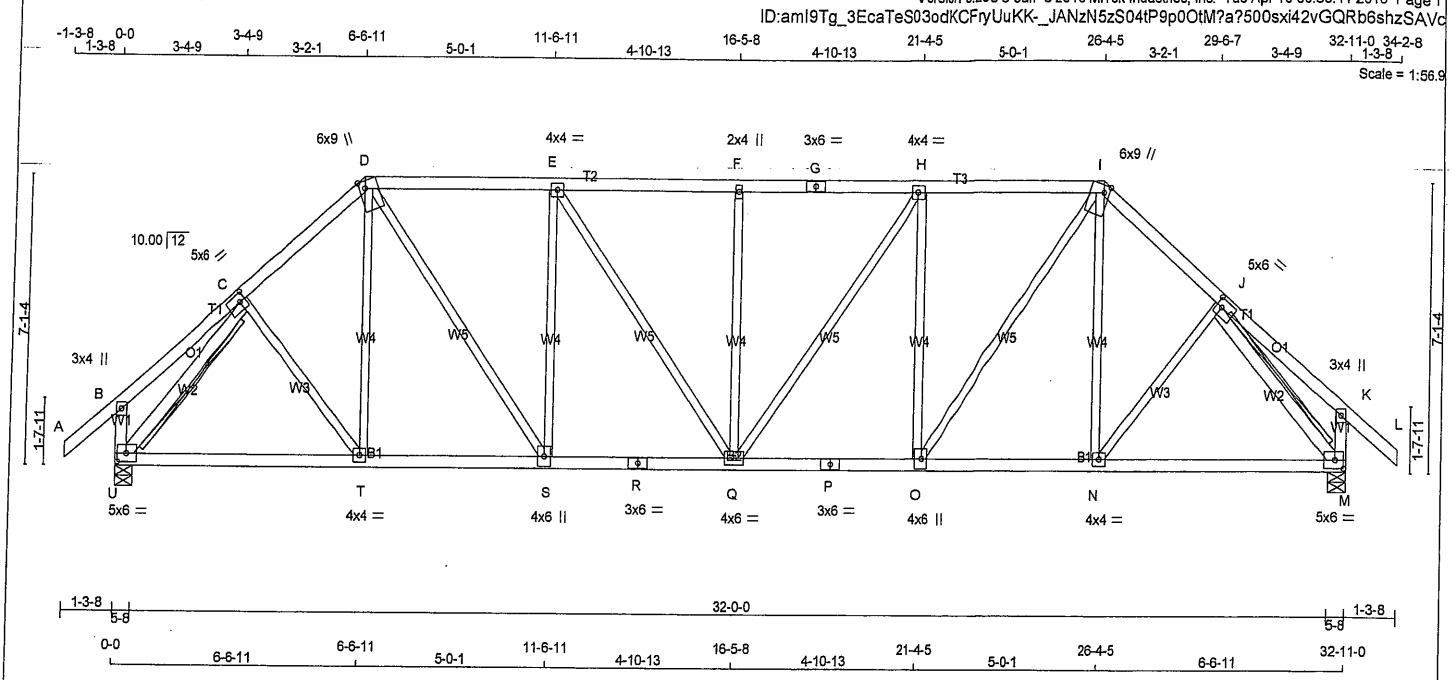

TOTAL WEIGHT = 158 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - C	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	1.75
J	TMWW-t	MT20	5.0	6.0	2.50	1.75
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW+t	MT20	4.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWWW-t	MT20	4.0	6.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW+t	MT20	4.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2467	0	2467	0	0	5-8	5-8
M	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
M	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT C-U, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 25	-112.4	-112.4	0.18 (1)	10.00
C- D	-2380 / 0	-112.4	-112.4	0.26 (1)	4.22
D- E	-2591 / 0	-112.4	-112.4	0.54 (1)	3.74
E- F	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
F- G	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
G- H	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57
H- I	-2591 / 0	-112.4	-112.4	0.54 (1)	3.74
I- J	-2380 / 0	-112.4	-112.4	0.26 (1)	4.22
J- K	0 / 25	-112.4	-112.4	0.18 (1)	10.00
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00
U- B	-299 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-299 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 1692	-28.0	-28.0	0.45 (2)	10.00
T- S	0 / 1805	-28.0	-28.0	0.47 (2)	10.00
S- R	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
R- Q	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 2592	-28.0	-28.0	0.48 (1)	10.00
O- N	0 / 1805	-28.0	-28.0	0.47 (2)	10.00
N- M	0 / 1692	-28.0	-28.0	0.45 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- T	0 / 200	0.05 (2)
T- D	0 / 205	0.05 (3)
D- S	0 / 1327	0.30 (1)
S- E	-948 / 0	0.83 (1)
E- Q	0 / 399	0.09 (1)
Q- F	-506 / 0	0.44 (1)
Q- H	0 / 399	0.09 (1)
O- H	-948 / 0	0.83 (1)
O- I	0 / 1327	0.30 (1)
N- I	0 / 205	0.05 (3)
N- J	0 / 200	0.05 (2)
U- C	-2690 / 0	0.74 (1)
J- M	-2690 / 0	0.74 (1)

LICENSED PROFESSIONAL ENGINEER
4 1076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.56/1.00 (F-H:1) , BC=0.48/1.00 (O-Q:1) , WB=0.83/1.00 (H-O:1) , SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (U) (INPUT = 0.90)
JSI METAL= 0.74 (R) (INPUT = 1.00)

DWG NO. TAM 18430-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T4S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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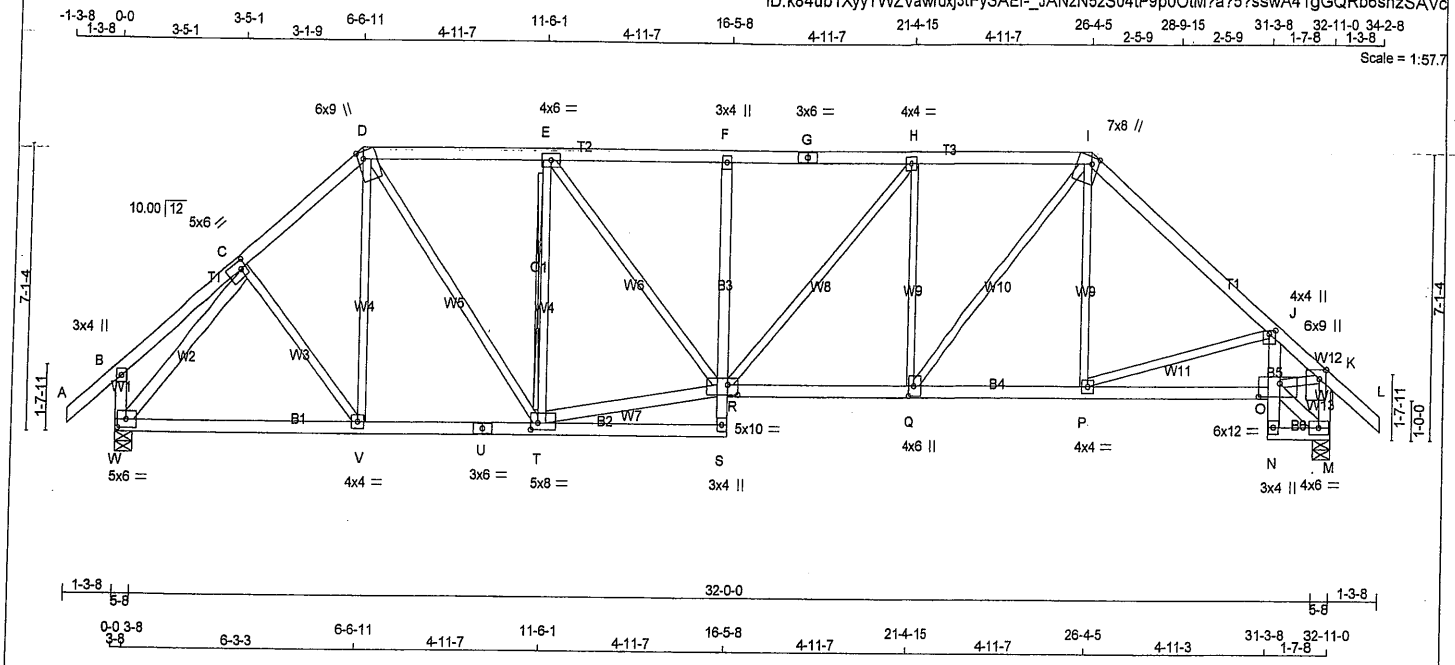

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
W - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
K	TMVW+p	MT20	6.0	9.0	2.75	2.00
M	BMVW1-t	MT20	4.0	6.0		
N	BMV+p	MT20	3.0	4.0		
O	BVMWW-I	MT20	6.0	12.0	4.00	6.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW+t	MT20	4.0	6.0	3.00	1.75
R	BVMWWW-I	MT20	5.0	10.0	3.00	3.25
S	BMV+p	MT20	3.0	4.0		
T	BMWWW-t	MT20	5.0	8.0	2.00	2.25
U	BS-t	MT20	3.0	6.0		
V	BMWW-t	MT20	4.0	4.0		
W	BMVW1-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2470	0	2470	0	0	5-8	5-8
W	2464	0	2464	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
W	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, W

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 25	-112.4	-112.4	0.18 (1)	10.00
C- D	-2376 / 0	-112.4	-112.4	0.26 (1)	4.23
D- E	-2571 / 0	-112.4	-112.4	0.52 (1)	3.78
E- F	-3282 / 0	-112.4	-112.4	0.59 (1)	3.31
F- G	-3294 / 0	-112.4	-112.4	0.64 (1)	3.27
G- H	-3294 / 0	-112.4	-112.4	0.64 (1)	3.27
H- I	-3023 / 0	-112.4	-112.4	0.59 (1)	3.43
I- J	-2756 / 0	-112.4	-112.4	0.61 (1)	3.68
J- K	-3091 / 0	-112.4	-112.4	0.41 (1)	3.54
K- L	0 / 50	-112.4	-112.4	0.15 (1)	10.00
W- B	-302 / 0	0.0	0.0	0.03 (1)	7.81
M- K	-2387 / 0	0.0	0.0	0.25 (1)	5.53
W- V	0 / 1694	-28.0	-28.0	0.44 (2)	10.00
V- U	0 / 1801	-28.0	-28.0	0.46 (2)	10.00
U- T	0 / 1801	-28.0	-28.0	0.46 (2)	10.00
T- S	0 / 67	-28.0	-28.0	0.15 (3)	10.00
S- R	0 / 101	0.0	0.0	0.10 (1)	10.00
R- F	-516 / 0	0.0	0.0	0.28 (1)	7.81
R- Q	0 / 3023	-28.0	-28.0	0.57 (1)	10.00
Q- P	0 / 2094	-28.0	-28.0	0.43 (1)	10.00
P- O	0 / 2482	-28.0	-28.0	0.50 (1)	10.00
N- O	0 / 36	0.0	0.0	0.13 (1)	10.00
O- J	-115 / 53	0.0	0.0	0.13 (1)	7.81
N- M	0 / 79	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- D	0 / 195	0.05 (3)
D- T	0 / 1319	0.30 (1)
T- E	-1455 / 0	0.51 (1)
R- H	0 / 410	0.09 (1)
Q- H	-940 / 0	0.55 (1)
Q- I	0 / 1430	0.32 (1)
P- I	0 / 281	0.06 (3)
O- M	-101 / 0	0.01 (1)
O- K	0 / 2422	0.55 (1)
T- R	0 / 2571	0.41 (1)
E- R	0 / 1094	0.25 (1)
C- V	0 / 196	0.04 (2)
W- C	-2686 / 0	0.84 (1)
P- J	-407 / 0	0.20 (1)

LICENSED PROFESSIONAL ENGINEER
4/07/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.64/1.00 (F-H:1) , BC=0.57/1.00 (Q-R:1) , WB=0.84/1.00 (C-W:1) , SSI=0.26/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.66 (C) (INPUT = 1.00)

DWG NO. TAM18431-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T5	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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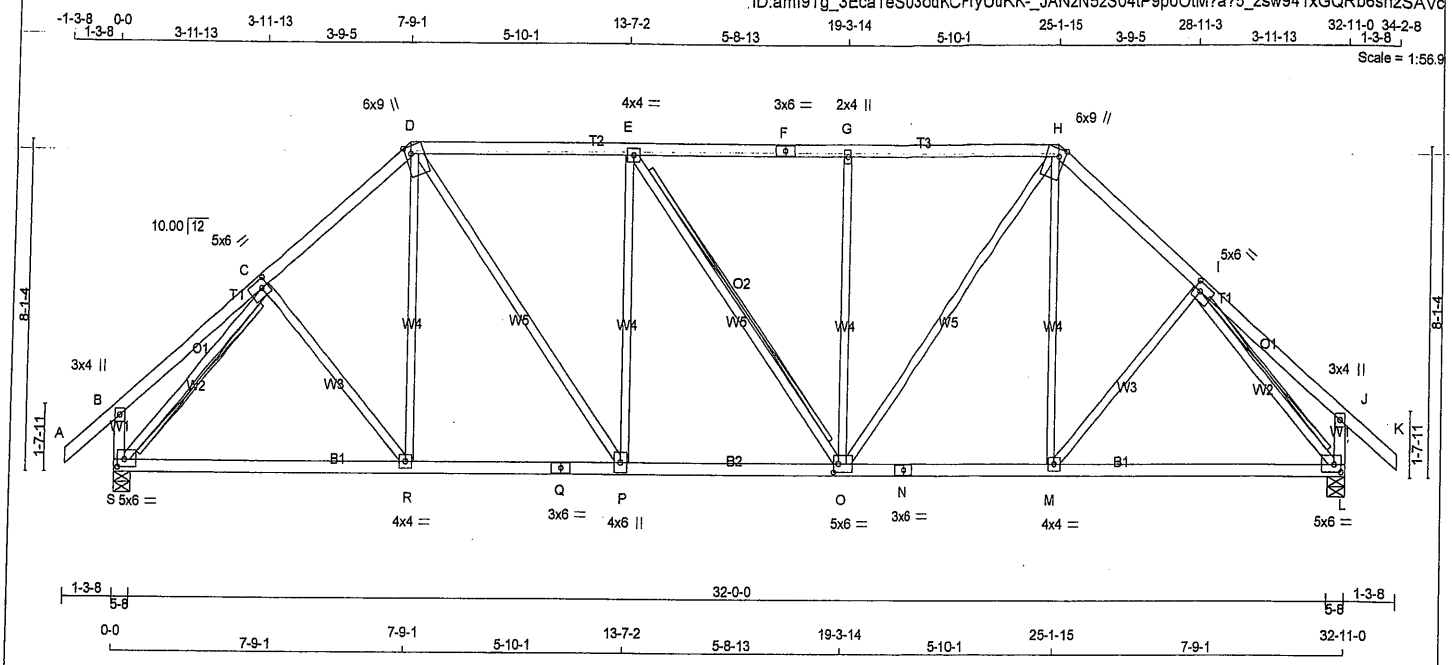

TOTAL WEIGHT = 153 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	1.75
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.25
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0	2.50	1.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-O, C-S, I-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
B- C	0 / 31	-112.4	-112.4	0.26 (1)	10.00	
C- D	-2350 / 0	-112.4	-112.4	0.36 (1)	4.14	
D- E	-2415 / 0	-112.4	-112.4	0.69 (1)	3.65	
E- F	-2413 / 0	-112.4	-112.4	0.69 (1)	3.65	
F- G	-2413 / 0	-112.4	-112.4	0.69 (1)	3.65	
G- H	-2413 / 0	-112.4	-112.4	0.68 (1)	3.67	
H- I	-2350 / 0	-112.4	-112.4	0.36 (1)	4.14	
I- J	0 / 31	-112.4	-112.4	0.26 (1)	10.00	
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
S- B	-324 / 0	0.0	0.0	0.03 (1)	7.81	
L- J	-324 / 0	0.0	0.0	0.03 (1)	7.81	
S- R	0 / 1750	-28.0	-28.0	0.56 (2)	10.00	
R- Q	0 / 1780	-28.0	-28.0	0.57 (2)	10.00	
Q- P	0 / 1780	-28.0	-28.0	0.57 (2)	10.00	
P- O	0 / 2415	-28.0	-28.0	0.47 (1)	10.00	
O- N	0 / 1780	-28.0	-28.0	0.58 (2)	10.00	
N- M	0 / 1780	-28.0	-28.0	0.58 (2)	10.00	
M- L	0 / 1751	-28.0	-28.0	0.56 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 139	0.03 (3)
R- D	0 / 304	0.07 (3)
D- P	0 / 1059	0.24 (1)
P- E	-703 / 0	0.89 (1)
E- O	-3 / 0	0.00 (1)
O- G	-702 / 0	0.89 (1)
O- H	0 / 1056	0.24 (1)
M- H	0 / 304	0.07 (3)
M- I	0 / 139	0.03 (3)
S- C	-2697 / 0	0.73 (1)
I- L	-2697 / 0	0.73 (1)

LICENSED PROFESSIONAL ENGINEER
41075
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.58/1.00 (M-O:2) , WB=0.89/1.00 (E-P:1) , SSI=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP (DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.66 (I) (INPUT = 1.00)

DWG NO. TAM 10432-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T5S	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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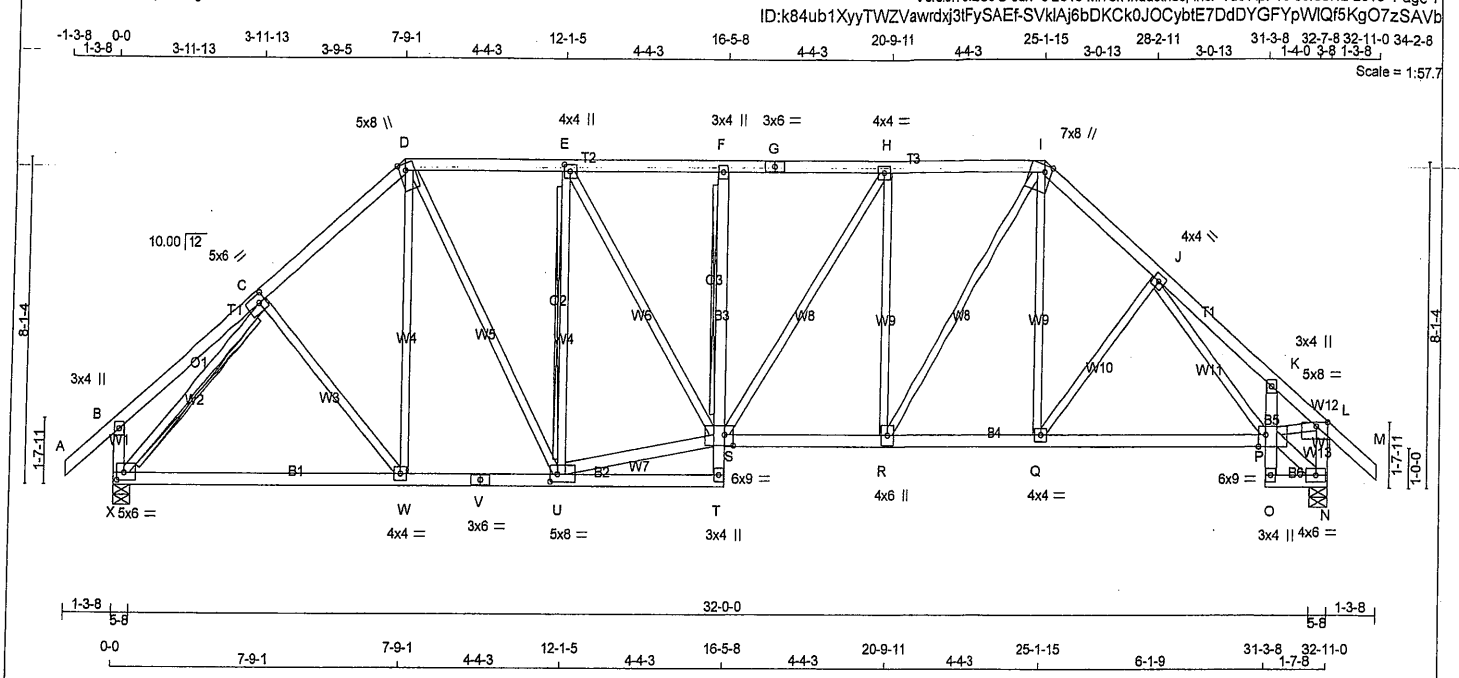

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
X - V	2x4	DRY	No.2	SPF
V - T	2x4	DRY	No.2	SPF
T - F	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, F, K						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	5.0	8.0	2.00	2.00
E	TMWW+t	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.25
J	TMWW-t	MT20	4.0	4.0		
L	TMVW-p	MT20	5.0	8.0	1.25	3.50
N	BMVW1-t	MT20	4.0	6.0		
O	BMV+p	MT20	3.0	4.0		
P	BVMWWW-l	MT20	6.0	9.0	Edge	2.25
Q	BMWW-t	MT20	4.0	4.0		
R	BMWW+t	MT20	4.0	6.0		
S	BVMWWW-l	MT20	6.0	9.0	3.25	2.75
T	BMV+p	MT20	3.0	4.0		
U	BMWWW-t	MT20	5.0	8.0	2.25	2.25
V	BS-t	MT20	3.0	6.0		
W	BMWW-t	MT20	4.0	4.0		
X	BMVW1-t	MT20	5.0	6.0	2.25	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2464	0	2464	0	0	5-8	5-8
N	2470	0	2470	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1947	1157 / 0	346 / 0	0 / 0	0 / 0	444 / 0	0 / 0
N	1951	1161 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT F-S
2x4 DRY SPF No.2 T-BRACE AT E-U, C-X

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 31	-112.4	-112.4	0.26 (1)	10.00
C- D	-2345 / 0	-112.4	-112.4	0.36 (1)	4.14
D- E	-2282 / 0	-112.4	-112.4	0.38 (1)	4.14
E- F	-2795 / 0	-112.4	-112.4	0.42 (1)	3.76
F- G	-2803 / 0	-112.4	-112.4	0.45 (1)	3.72
G- H	-2803 / 0	-112.4	-112.4	0.45 (1)	3.72
H- I	-2634 / 0	-112.4	-112.4	0.43 (1)	3.83
I- J	-2658 / 0	-112.4	-112.4	0.28 (1)	4.01
J- K	-2634 / 0	-112.4	-112.4	0.27 (1)	4.03
K- L	-2661 / 0	-112.4	-112.4	0.22 (1)	4.04
L- M	0 / 50	-112.4	-112.4	0.15 (1)	10.00
X- B	-324 / 0	0.0	0.0	0.03 (1)	7.81
N- L	-2399 / 0	0.0	0.0	0.26 (1)	5.52
X- W	0 / 1747	-28.0	-28.0	0.55 (2)	10.00
W- V	0 / 1776	-28.0	-28.0	0.57 (2)	10.00
V- U	0 / 1776	-28.0	-28.0	0.57 (2)	10.00
U- T	0 / 50	-28.0	-28.0	0.13 (3)	10.00
T- S	0 / 94	0.0	0.0	0.08 (1)	10.00
S- F	-442 / 0	0.0	0.0	0.14 (1)	7.81
S- R	0 / 2634	-28.0	-28.0	0.49 (1)	10.00
R- Q	0 / 2021	-28.0	-28.0	0.47 (2)	10.00
Q- P	0 / 2197	-28.0	-28.0	0.49 (1)	10.00
O- P	0 / 36	0.0	0.0	0.10 (1)	10.00
P- K	-162 / 0	0.0	0.0	0.10 (1)	7.81
O- N	0 / 75	-28.0	-28.0	0.03 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- W	0 / 139	0.03 (3)
W- D	0 / 289	0.07 (3)
D- U	0 / 1050	0.24 (1)
U- E	-1351 / 0	0.63 (1)
S- H	0 / 311	0.07 (1)
R- H	-819 / 0	0.72 (1)
R- I	0 / 1117	0.25 (1)
Q- I	0 / 415	0.09 (2)
Q- J	-290 / 0	0.13 (1)
J- P	-255 / 0	0.11 (1)
X- C	-2692 / 0	0.73 (1)
U- S	0 / 2304	0.37 (1)
E- S	0 / 957	0.22 (1)
P- N	-90 / 0	0.01 (1)
P- L	0 / 2046	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.45/1.00 (F-H:1) , BC=0.57/1.00 (U-W:2) , WB=0.73/1.00 (C-X:1) , SSI=0.24/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (X) (INPUT = 0.90)
JSI METAL= 0.66 (C) (INPUT = 1.00)

REGISTERED PROFESSIONAL ENGINEER
41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1843318
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T6	2	1	OBC2012_6TCDL TRUSS DESC.	

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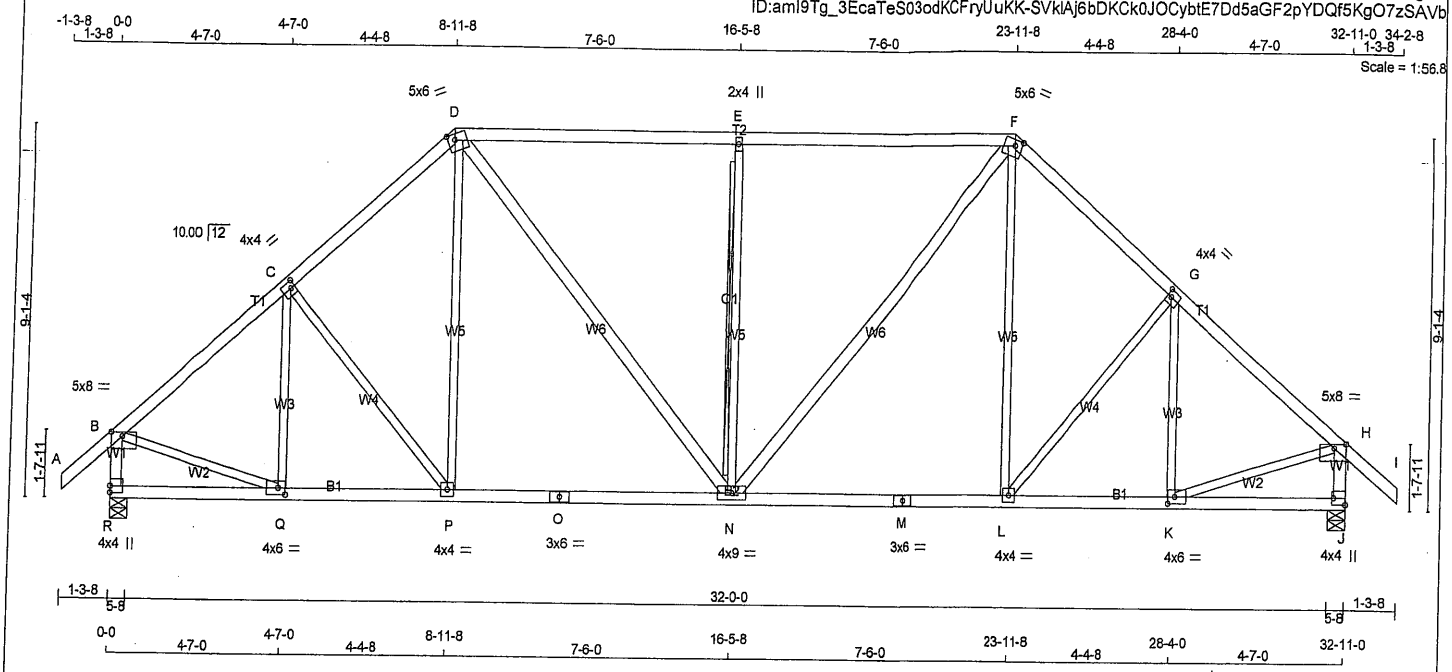

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	1.75	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	1.75	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	1.25	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	4.0	6.0	2.00	2.25
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	4.0	6.0	2.00	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2467	0	2467	0	0	5-8	5-8
J	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
J	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2354 / 0	-112.4	-112.4	0.37 (1)	4.14
C- D	-2286 / 0	-112.4	-112.4	0.36 (1)	4.20
D- E	-2262 / 0	-112.4	-112.4	0.96 (1)	2.91
E- F	-2262 / 0	-112.4	-112.4	0.96 (1)	2.91
F- G	-2286 / 0	-112.4	-112.4	0.36 (1)	4.20
G- H	-2354 / 0	-112.4	-112.4	0.37 (1)	4.14
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
R- B	-2409 / 0	0.0	0.0	0.26 (1)	5.51
J- H	-2409 / 0	0.0	0.0	0.26 (1)	5.51
R- Q	0 / 0	-28.0	-28.0	0.13 (3)	10.00
Q- P	0 / 1837	-28.0	-28.0	0.42 (1)	10.00
P- O	0 / 1726	-28.0	-28.0	0.53 (2)	10.00
O- N	0 / 1726	-28.0	-28.0	0.53 (2)	10.00
N- M	0 / 1726	-28.0	-28.0	0.53 (2)	10.00
M- L	0 / 1726	-28.0	-28.0	0.53 (2)	10.00
L- K	0 / 1837	-28.0	-28.0	0.42 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.13 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-403 / 0	0.18 (1)
C- P	-175 / 0	0.15 (1)
P- D	0 / 371	0.08 (2)
D- N	0 / 826	0.13 (1)
N- E	-1037 / 0	0.63 (1)
N- F	0 / 826	0.13 (1)
L- F	0 / 371	0.08 (2)
L- G	-175 / 0	0.15 (1)
K- G	-403 / 0	0.18 (1)
B- Q	0 / 1910	0.43 (1)
K- H	0 / 1910	0.43 (1)

LICENSED PROFESSIONAL ENGINEER
41076
S. KATSOULAROS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.96/1.00 (E-F:1) , BC=0.53/1.00 (L-N:2) , WB=0.63/1.00 (E-N:1) , SSI=0.41/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.59 (O) (INPUT = 1.00)

DWG NO. TAM 18434-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T7	2	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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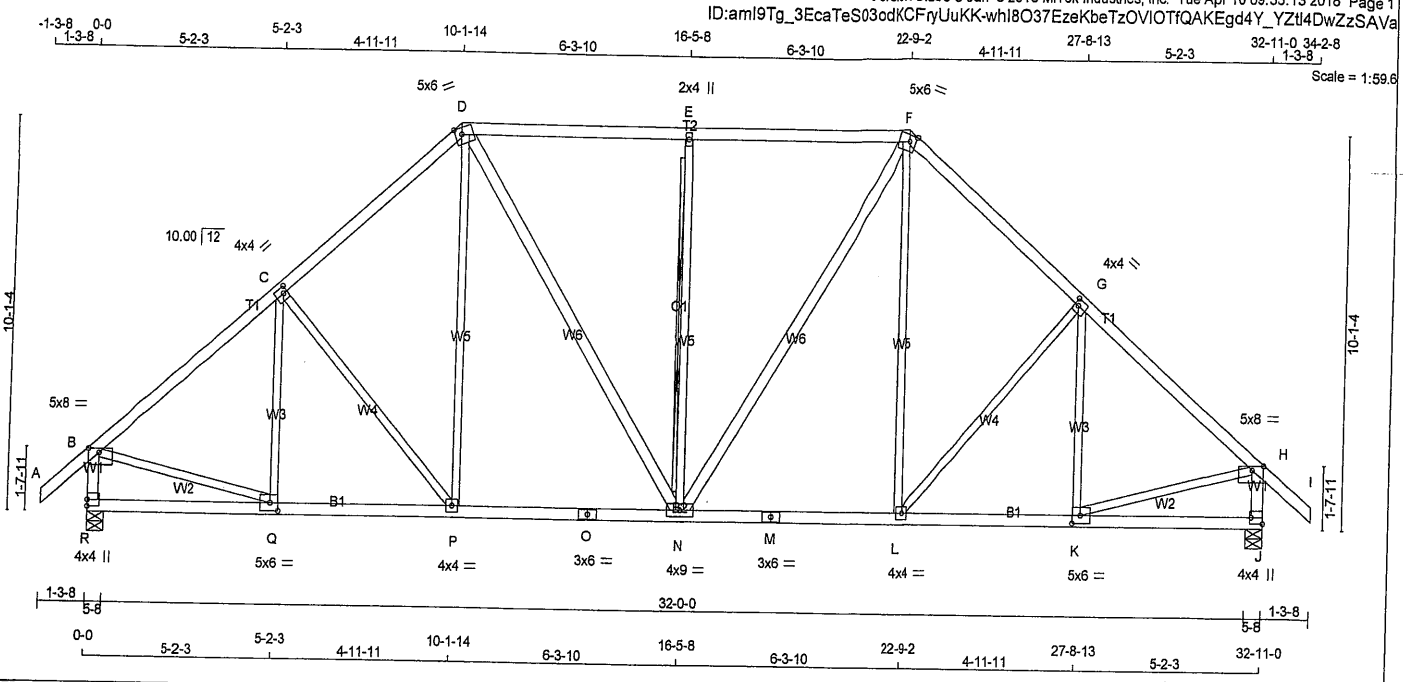

TOTAL WEIGHT = 2 X 167 = 333 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	6.0	2.00	2.25
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMVW-p	MT20	5.0	8.0	1.25	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2467	0	2467	0	0	5-8	5-8
J	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
J	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2387 / 0	-112.4	-112.4	0.49 (1)	3.99
C- D	-2213 / 0	-112.4	-112.4	0.46 (1)	4.14
D- E	-2003 / 0	-112.4	-112.4	0.64 (1)	3.97
E- F	-2003 / 0	-112.4	-112.4	0.64 (1)	3.97
F- G	-2213 / 0	-112.4	-112.4	0.46 (1)	4.14
G- H	-2387 / 0	-112.4	-112.4	0.49 (1)	3.99
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
R- B	-2405 / 0	0.0	0.0	0.26 (1)	5.52
J- H	-2405 / 0	0.0	0.0	0.26 (1)	5.52
R- Q	0 / 0	-28.0	-28.0	0.17 (3)	10.00
Q- P	0 / 1866	-28.0	-28.0	0.40 (1)	10.00
P- O	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
O- N	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
N- M	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
M- L	0 / 1666	-28.0	-28.0	0.42 (2)	10.00
L- K	0 / 1866	-28.0	-28.0	0.40 (1)	10.00
K- J	0 / 0	-28.0	-28.0	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- C	-318 / 62	0.18 (1)
C- P	-305 / 0	0.35 (1)
P- D	0 / 414	0.09 (2)
D- N	0 / 623	0.10 (1)
N- E	-867 / 0	0.69 (1)
N- F	0 / 623	0.10 (1)
L- F	0 / 414	0.09 (2)
L- G	-305 / 0	0.35 (1)
K- G	-318 / 62	0.18 (1)
B- Q	0 / 1925	0.43 (1)
K- H	0 / 1925	0.43 (1)

LICENSED PROFESSIONAL ENGINEER
4 1078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.64/1.00 (D-E:1) , BC=0.42/1.00 (L-N:2) , WB=0.69/1.00 (E-N:1) , SSI=0.34/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.52 (O) (INPUT = 1.00)

DWG NO. TAM 16435 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T8	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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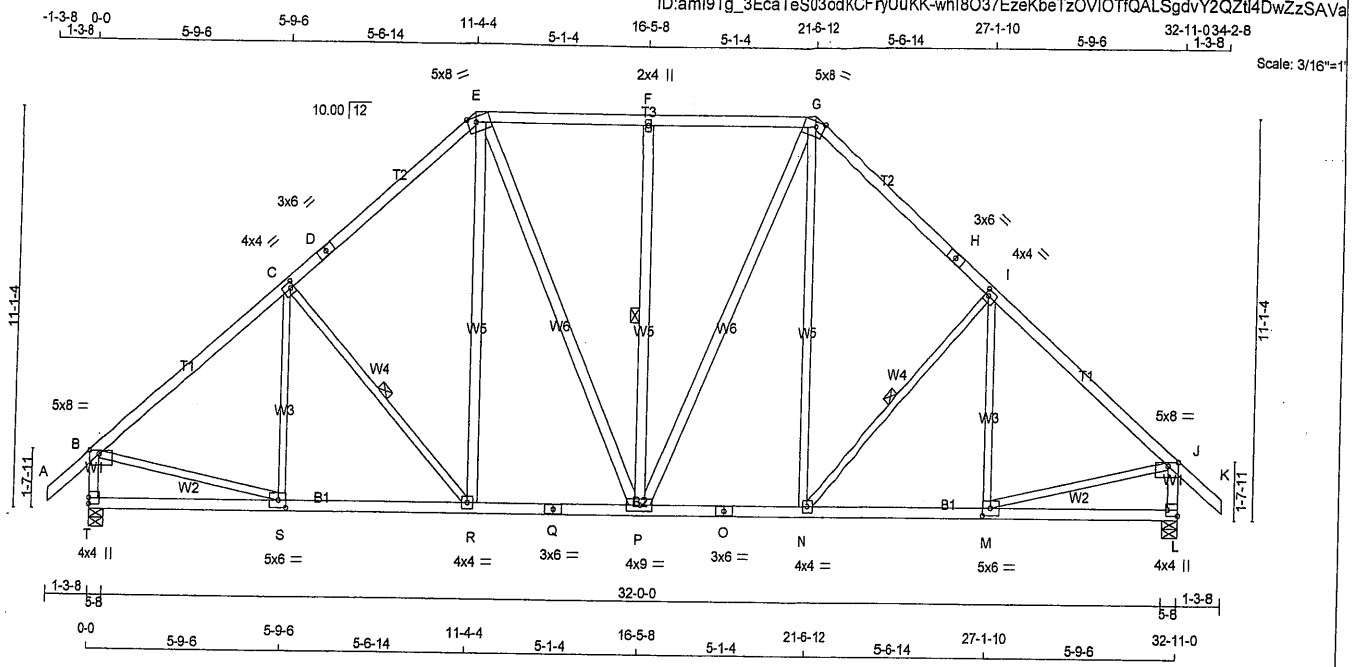

TOTAL WEIGHT = 7 X 184 = 1287 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
S - C	2x3	DRY	No.2	SPF
C - R	2x3	DRY	No.2	SPF
N - I	2x3	DRY	No.2	SPF
M - I	2x3	DRY	No.2	SPF
B - S	2x3	DRY	No.2	SPF
M - J	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	Edge	3.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.25
J	TMVW-p	MT20	5.0	8.0	1.25	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.75
T	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2467	0	2467	0	0	5-8	5-8
L	2467	0	2467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0
L	1949	1159 / 0	346 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10 00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-R, F-P, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-2405 / 0	-112.4	-112.4	0.63 (1)	3.81
C- D	-2129 / 0	-112.4	-112.4	0.58 (1)	4.06
D- E	-2129 / 0	-112.4	-112.4	0.58 (1)	4.06
E- F	-1796 / 0	-112.4	-112.4	0.41 (1)	4.54
F- G	-1796 / 0	-112.4	-112.4	0.41 (1)	4.54
G- H	-2129 / 0	-112.4	-112.4	0.58 (1)	4.06
H- I	-2129 / 0	-112.4	-112.4	0.58 (1)	4.06
I- J	-2405 / 0	-112.4	-112.4	0.63 (1)	3.81
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
T- B	-2400 / 0	0.0	0.0	0.26 (1)	5.52
L- J	-2400 / 0	0.0	0.0	0.26 (1)	5.52
T- S	0 / 0	-28.0	-28.0	0.24 (3)	10.00
S- R	0 / 1885	-28.0	-28.0	0.43 (2)	10.00
R- Q	0 / 1600	-28.0	-28.0	0.34 (1)	10.00
Q- P	0 / 1600	-28.0	-28.0	0.34 (1)	10.00
P- O	0 / 1600	-28.0	-28.0	0.34 (1)	10.00
O- N	0 / 1600	-28.0	-28.0	0.34 (1)	10.00
N- M	0 / 1885	-28.0	-28.0	0.43 (2)	10.00
M- L	0 / 0	-28.0	-28.0	0.24 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-246 / 128	0.17 (1)
C- R	-426 / 0	0.20 (1)
R- E	0 / 461	0.07 (2)
E- P	0 / 459	0.07 (1)
P- F	-696 / 0	0.44 (1)
P- G	0 / 459	0.07 (1)
N- G	0 / 461	0.07 (2)
N- I	-426 / 0	0.20 (1)
M- I	-246 / 128	0.17 (1)
B- S	0 / 1932	0.43 (1)
M- J	0 / 1932	0.43 (1)

LICENSED PROFESSIONAL ENGINEER
4 1070
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.63/1.00 (I-J:1) , BC=0.43/1.00 (R-S:2) , WB=0.44/1.00 (F-P:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (S) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)

DWG NO. TAM/18436 -18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T9A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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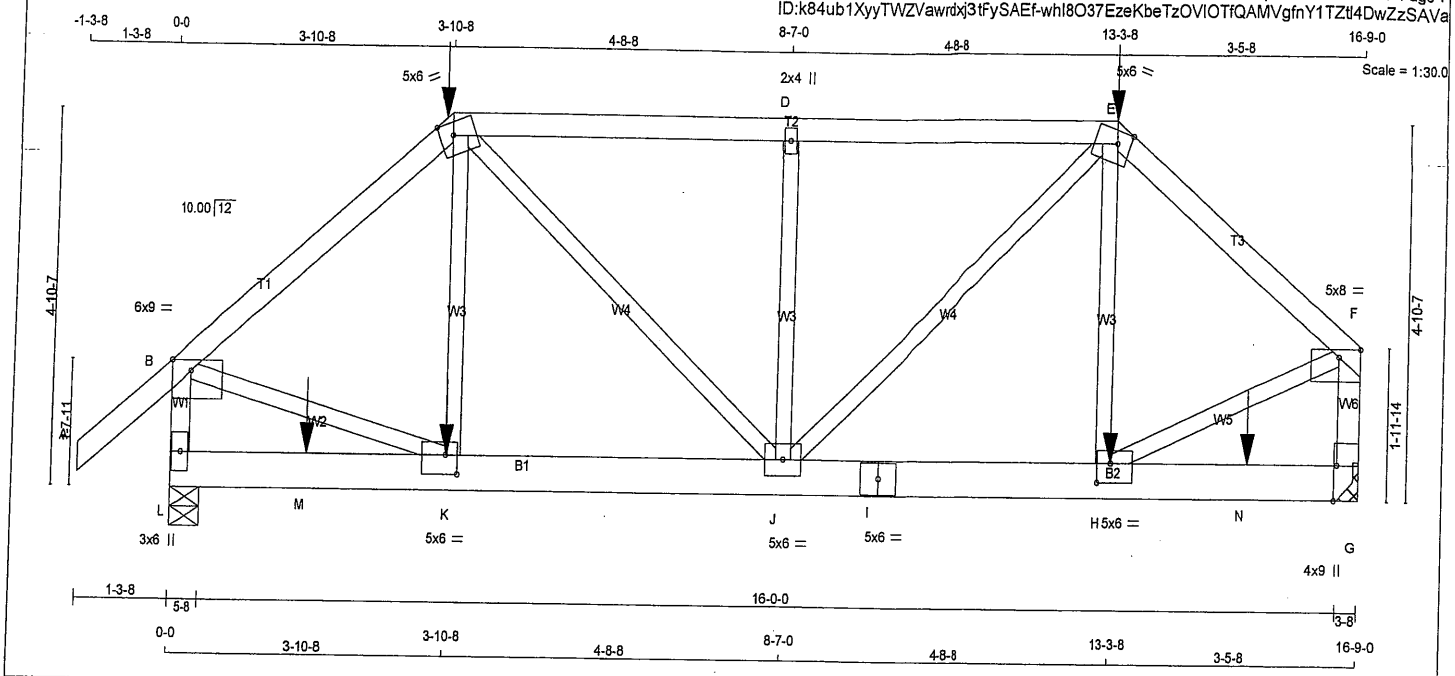

TOTAL WEIGHT = 83 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	3.50
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	5.0	6.0	2.00	2.25
F	TMVW-p	MT20	5.0	8.0	1.25	Edge
G	BMV1+t	MT20	4.0	9.0	Edge	0.50
H	BMWW-t	MT20	5.0	6.0	3.00	2.25
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	5.0	6.0	3.00	2.00
L	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 203.2 lbs FACTORED DOWN AT 13-3-8, AND 230.3 lbs FACTORED DOWN AT 3-10-8 ON TOP CHORD, AND 36.3 lbs FACTORED DOWN AT 1-11-4, 1138.0 lbs FACTORED DOWN AT 3-10-9, AND 1138.0 lbs FACTORED DOWN AT 13-3-7, AND 34.1 lbs FACTORED DOWN AT 15-2-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	2675	0	2675	0	0	5-8	5-8
G	2576	0	2576	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	2064	1311 / 0	310 / 0	0 / 0	0 / 0	443 / 0	0 / 0
G	2012	1237 / 0	330 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00
B- C	-2544 / 0	-112.4	-112.4	0.47 (1)	3.83
C- D	-2302 / 0	-112.4	-112.4	0.56 (1)	3.82
D- E	-2302 / 0	-112.4	-112.4	0.56 (1)	3.82
E- F	-2364 / 0	-112.4	-112.4	0.36 (1)	4.08
L- B	-2607 / 0	0.0	0.0	0.30 (1)	5.26
G- F	-2512 / 0	0.0	0.0	0.30 (1)	5.35
L- M	0 / 0	-28.0	-28.0	0.08 (2)	10.00
M- K	0 / 0	-28.0	-28.0	0.08 (2)	10.00
K- J	0 / 1964	-28.0	-28.0	0.31 (1)	10.00
J- I	0 / 1821	-28.0	-28.0	0.29 (1)	10.00
I- H	0 / 1821	-28.0	-28.0	0.29 (1)	10.00
H- N	0 / 0	-28.0	-28.0	0.06 (2)	10.00
N- G	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 652	0.16 (1)
C- J	0 / 466	0.12 (1)
J- D	-631 / 0	0.21 (1)
J- E	0 / 664	0.16 (1)
H- E	0 / 452	0.11 (1)
B- K	0 / 2039	0.50 (1)
H- F	0 / 1978	0.49 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-230	-230	---	BACK	VERT	TOTAL
E	13-3-8	-203	-203	---	FRONT	VERT	TOTAL
H	13-3-7	-1138	-1138	---	BACK	VERT	TOTAL
K	3-10-9	-1138	-1138	---	BACK	VERT	TOTAL
M	1-11-4	-15	-36	---	BACK	VERT	TOTAL
N	15-2-12	-20	-34	---	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
41016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (C-D:1) , BC=0.31/1.00 (J-K:1) , WB=0.50/1.00 (B-K:1) , SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.49 (K) (INPUT = 1.00)

DWG NO. TAM / B437-18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T10A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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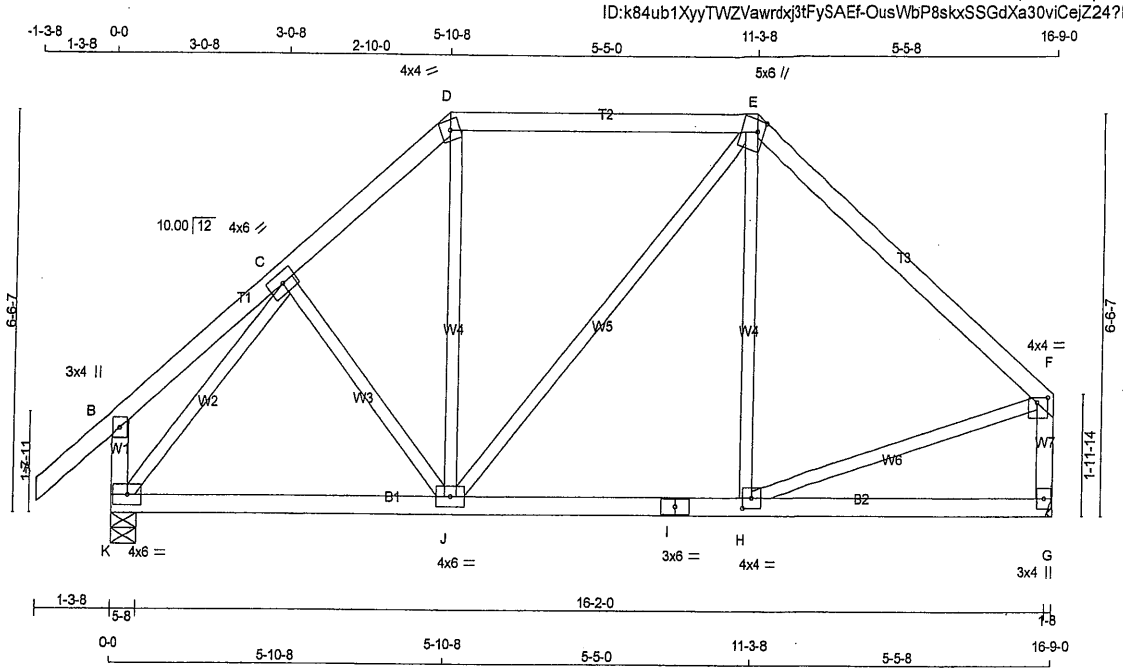

TOTAL WEIGHT = 75 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW-m	MT20	4.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-p	MT20	4.0	4.0	1.00	2.25
G	BMV1+p	MT20	3.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1332	0	1332	0	0	5-8	5-8
G	1176	0	1176	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1044	634 / 0	176 / 0	0 / 0	0 / 0	234 / 0	0 / 0
G	938	544 / 0	176 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 23	-112.4	-112.4	0.15 (1)	10.00
C- D	-979 / 0	-112.4	-112.4	0.12 (1)	6.17
D- E	-735 / 0	-112.4	-112.4	0.43 (1)	6.25
E- F	-925 / 0	-112.4	-112.4	0.45 (1)	5.80
K- B	-283 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-1112 / 0	0.0	0.0	0.13 (1)	7.51
K- J	0 / 764	-28.0	-28.0	0.29 (2)	10.00
J- I	0 / 709	-28.0	-28.0	0.30 (2)	10.00
I- H	0 / 709	-28.0	-28.0	0.30 (2)	10.00
H- G	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-49 / 67	0.02 (1)
J- D	0 / 250	0.06 (2)
J- E	0 / 41	0.01 (1)
H- E	-48 / 183	0.04 (3)
K- C	-1242 / 0	0.44 (1)
H- F	0 / 740	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.45/1.00 (E-F:1) , BC=0.30/1.00 (H-J:2) , WB=0.44/1.00 (C-K:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18438 -18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T11	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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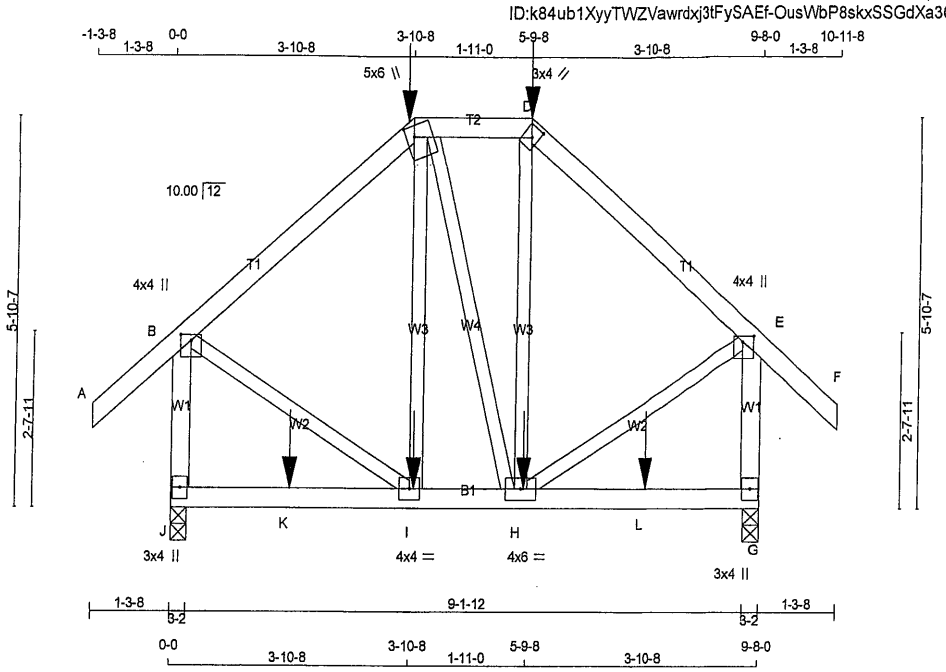

TOTAL WEIGHT = 54 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 195.0 lbs FACTORED DOWN AT 3-10-8, 195.0 lbs FACTORED DOWN AT 5-9-8, AND 58.3 lbs FACTORED DOWN AT 3-10-8, AND 58.3 lbs FACTORED DOWN AT 5-9-8 ON TOP CHORD, AND 38.5 lbs FACTORED DOWN AT 1-11-4, 37.4 lbs FACTORED DOWN AT 3-11-4, AND 37.4 lbs FACTORED DOWN AT 5-8-12, AND 38.5 lbs FACTORED DOWN AT 7-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1127	0	1127	0	0	3-2	3-2
G	1127	0	1127	0	0	3-2	3-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	872	551 / 0	134 / 0	0 / 0	0 / 0	187 / 0	0 / 0
G	872	551 / 0	134 / 0	0 / 0	0 / 0	187 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00	
B- C	-643 / 0	-112.4	-112.4	0.33 (1)	6.25	
C- D	-488 / 0	-112.4	-112.4	0.08 (1)	6.25	
D- E	-641 / 0	-112.4	-112.4	0.33 (1)	6.25	
E- F	0 / 50	-112.4	-112.4	0.17 (1)	10.00	
J- B	-1072 / 0	0.0	0.0	0.16 (1)	7.58	
G- E	-1071 / 0	0.0	0.0	0.16 (1)	7.59	
J- K	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
K- I	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
I- H	0 / 490	-28.0	-28.0	0.17 (2)	10.00	
H- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
L- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-162 / 95	0.09 (1)
C- H	-4 / 2	0.00 (1)
H- D	-167 / 98	0.09 (1)
B- I	0 / 569	0.14 (1)
H- E	0 / 568	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-15	-17	—	FRONT	VERT	DEAD
C	3-10-8	-58	-58	—	BACK	VERT	TOTAL
C	3-10-8	-180	-180	—	FRONT	VERT	SNOW
D	5-9-8	-15	-17	—	FRONT	VERT	DEAD
D	5-9-8	-58	-58	—	BACK	VERT	TOTAL
D	5-9-8	-180	-180	—	FRONT	VERT	SNOW
H	5-8-12	-21	-37	—	BACK	VERT	TOTAL
I	3-11-4	-21	-37	—	BACK	VERT	TOTAL
K	1-11-4	-17	-39	—	BACK	VERT	TOTAL
L	7-8-12	-17	-39	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4108
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.17/1.00 (H-I:2) , WB=0.14/1.00 (B-I:1) , SSI=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM 10439 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T12	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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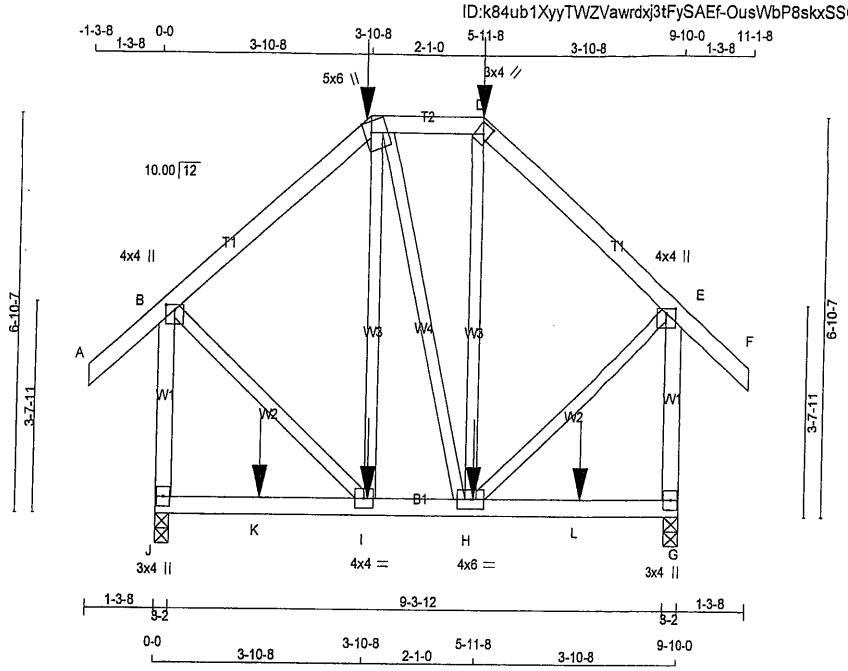

TOTAL WEIGHT = 60 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW+p	MT20	4.0	4.0	1.00	2.00
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 253.4 lbs FACTORED DOWN AT 3-10-8, AND 253.4 lbs FACTORED DOWN AT 5-11-8 ON TOP CHORD, AND 39.9 lbs FACTORED DOWN AT 1-11-4, 39.3 lbs FACTORED DOWN AT 3-11-4, AND 39.3 lbs FACTORED DOWN AT 5-10-12, AND 39.9 lbs FACTORED DOWN AT 7-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1141	0	1141	0	0	3-2	3-2
G	1141	0	1141	0	0	3-2	3-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	884	557 / 0	137 / 0	0 / 0	0 / 0	190 / 0	0 / 0
G	884	557 / 0	137 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.17 (1)	10.00	
B- C	-557 / 0	-112.4	-112.4	0.33 (1)	6.25	
C- D	-422 / 0	-112.4	-112.4	0.09 (1)	6.25	
D- E	-555 / 0	-112.4	-112.4	0.33 (1)	6.25	
E- F	0 / 50	-112.4	-112.4	0.17 (1)	10.00	
J- B	-1086 / 0	0.0	0.0	0.25 (1)	7.55	
G- E	-1084 / 0	0.0	0.0	0.25 (1)	7.55	
J- K	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
K- I	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
I- H	0 / 423	-28.0	-28.0	0.16 (2)	10.00	
H- L	0 / 0	-28.0	-28.0	0.15 (3)	10.00	
L- G	0 / 0	-28.0	-28.0	0.15 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-225 / 73	0.18 (1)
C- H	-5 / 2	0.00 (1)
H- D	-231 / 75	0.19 (1)
B- I	0 / 555	0.14 (1)
H- E	0 / 553	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-8	-15	-17	---	FRONT	VERT	DEAD
C	3-10-8	-58	-58	---	FRONT	VERT	TOTAL
C	3-10-8	-180	-180	---	FRONT	VERT	SNOW
D	5-11-8	-15	-17	---	FRONT	VERT	DEAD
D	5-11-8	-58	-58	---	FRONT	VERT	TOTAL
D	5-11-8	-180	-180	---	FRONT	VERT	SNOW
H	5-10-12	-22	-39	---	FRONT	VERT	TOTAL
I	3-11-4	-22	-39	---	FRONT	VERT	TOTAL
K	1-11-4	-19	-40	---	FRONT	VERT	TOTAL
L	7-10-12	-19	-40	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (B-C:1) , BC=0.16/1.00 (H-I:2) , WB=0.19/1.00 (D-H:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

DWG NO. TAM / 10440 -18

STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T13A	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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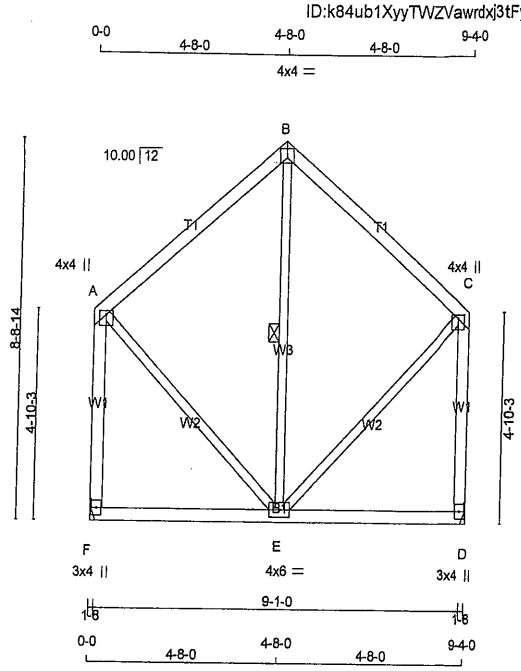

TOTAL WEIGHT = 52 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	1.50	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	655	0	655	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	655	0	655	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	522	303 / 0	98 / 0	0 / 0	0 / 0	121 / 0	0 / 0
D	522	303 / 0	98 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-248 / 0	-112.4	-112.4	0.31 (1)	6.25
B- C	-248 / 0	-112.4	-112.4	0.31 (1)	6.25
F- A	-605 / 0	0.0	0.0	0.25 (1)	7.81
D- C	-605 / 0	0.0	0.0	0.25 (1)	7.81
F- E	0 / 0	-28.0	-28.0	0.19 (3)	10.00
E- D	0 / 0	-28.0	-28.0	0.19 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-207 / 94	0.10 (1)
A- E	0 / 265	0.06 (1)
E- C	0 / 265	0.06 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.31/1.00 (A-B:1) , BC=0.19/1.00 (E-F:3) , WB=0.10/1.00 (B-E:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (C) (INPUT = 0.90)
JSI METAL= 0.11 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18441 -13
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T21	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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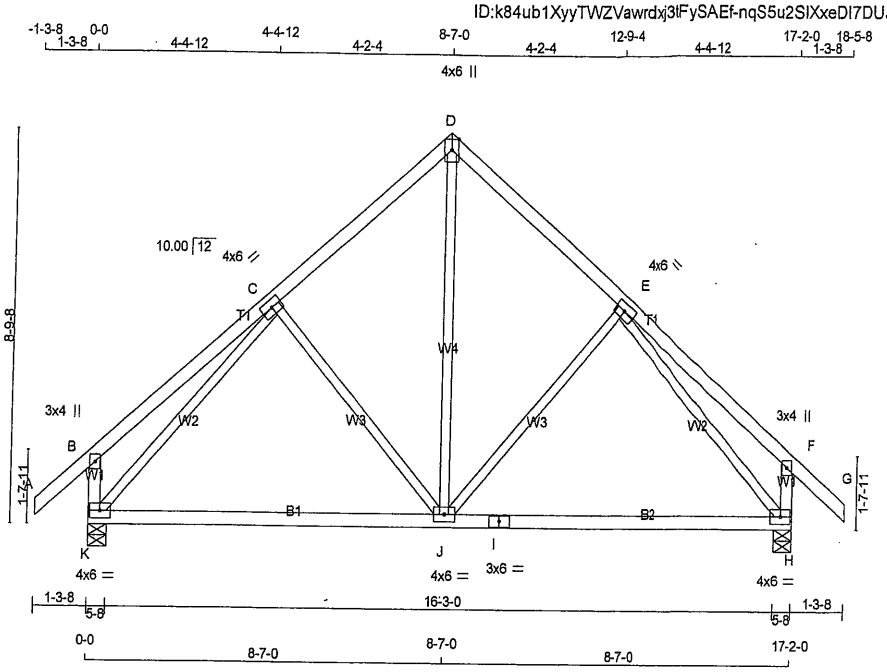

TOTAL WEIGHT = 79 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1361	0	1361	0	0	5-8	5-8
H	1361	0	1361	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1068	647 / 0	180 / 0	0 / 0	0 / 0	240 / 0	0 / 0
H	1068	647 / 0	180 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 37	-112.4	-112.4	0.34 (1)	10.00
C- D	-870 / 0	-112.4	-112.4	0.27 (1)	6.24
D- E	-870 / 0	-112.4	-112.4	0.27 (1)	6.24
E- F	0 / 37	-112.4	-112.4	0.34 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
K- B	-338 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-338 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 817	-28.0	-28.0	0.70 (3)	10.00
J- I	0 / 817	-28.0	-28.0	0.70 (3)	10.00
I- H	0 / 817	-28.0	-28.0	0.70 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 695	0.16 (1)
J- E	-266 / 16	0.20 (1)
C- J	-266 / 16	0.20 (1)
K- C	-1239 / 0	0.91 (1)
E- H	-1239 / 0	0.91 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 783 (0.26")

CSI: TC=0.34/1.00 (E-F:1) , BC=0.70/1.00 (J-K:3) , WB=0.91/1.00 (E-H:1) , SSI=0.22/1.00 (H-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4-107E
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 16447-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287696	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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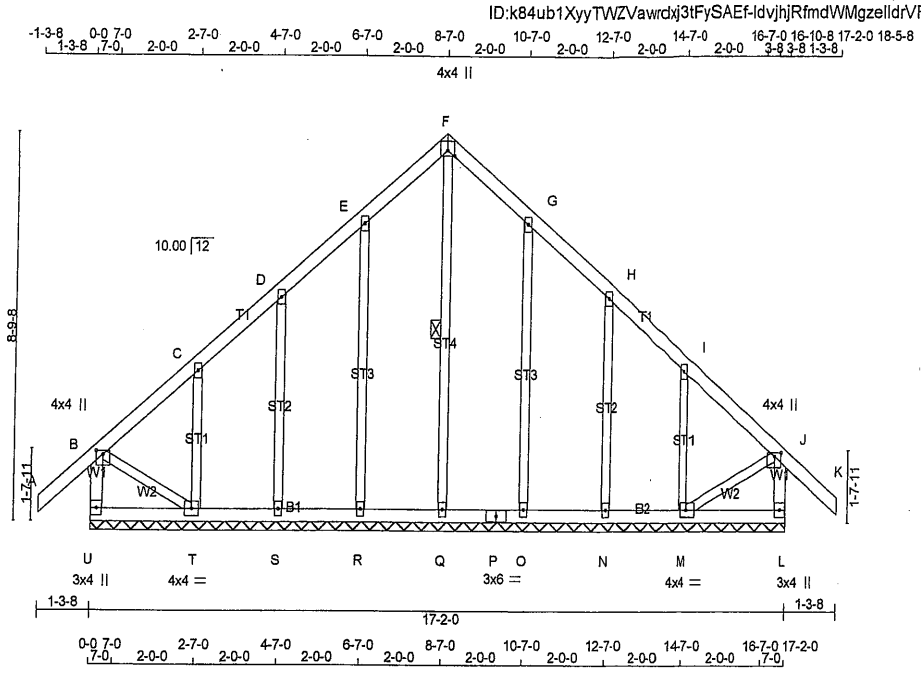

TOTAL WEIGHT = 87 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I						
C	TMW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	1.50	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, Q, R, S						
N	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	6.0		
T	BMWW1-t	MT20	4.0	4.0		
U	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-33 / 0	-112.4	-112.4	0.09 (1)	6.25
C- D	-51 / 0	-112.4	-112.4	0.09 (1)	6.25
D- E	-29 / 0	-112.4	-112.4	0.07 (1)	6.25
E- F	-45 / 0	-112.4	-112.4	0.07 (1)	6.25
F- G	-45 / 0	-112.4	-112.4	0.07 (1)	6.25
G- H	-29 / 0	-112.4	-112.4	0.07 (1)	6.25
H- I	-51 / 0	-112.4	-112.4	0.09 (1)	6.25
I- J	-33 / 0	-112.4	-112.4	0.09 (1)	6.25
J- K	0 / 50	-112.4	-112.4	0.15 (1)	10.00
U- B	-321 / 0	0.0	0.0	0.03 (1)	7.81
L- J	-321 / 0	0.0	0.0	0.03 (1)	7.81
U- T	0 / 0	-28.0	-28.0	0.04 (3)	10.00
T- S	0 / 30	-28.0	-28.0	0.05 (3)	10.00
S- R	0 / 27	-28.0	-28.0	0.03 (3)	10.00
R- Q	0 / 23	-28.0	-28.0	0.03 (3)	10.00
Q- P	0 / 23	-28.0	-28.0	0.03 (3)	10.00
P- O	0 / 23	-28.0	-28.0	0.03 (3)	10.00
O- N	0 / 27	-28.0	-28.0	0.03 (3)	10.00
N- M	0 / 30	-28.0	-28.0	0.05 (3)	10.00
M- L	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
Q- F	-151 / 0	0.07 (1)
R- E	-259 / 0	0.23 (1)
S- D	-195 / 0	0.09 (1)
T- C	-290 / 0	0.07 (1)
O- G	-259 / 0	0.23 (1)
N- H	-195 / 0	0.09 (1)
M- I	-290 / 0	0.07 (1)
B- T	0 / 43	0.01 (1)
M- J	0 / 43	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (J-K:1) , BC=0.05/1.00 (M-N:3) , WB=0.23/1.00 (G-O:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (F) (INPUT = 0.90)
JSI METAL= 0.08 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM/184584-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T22A	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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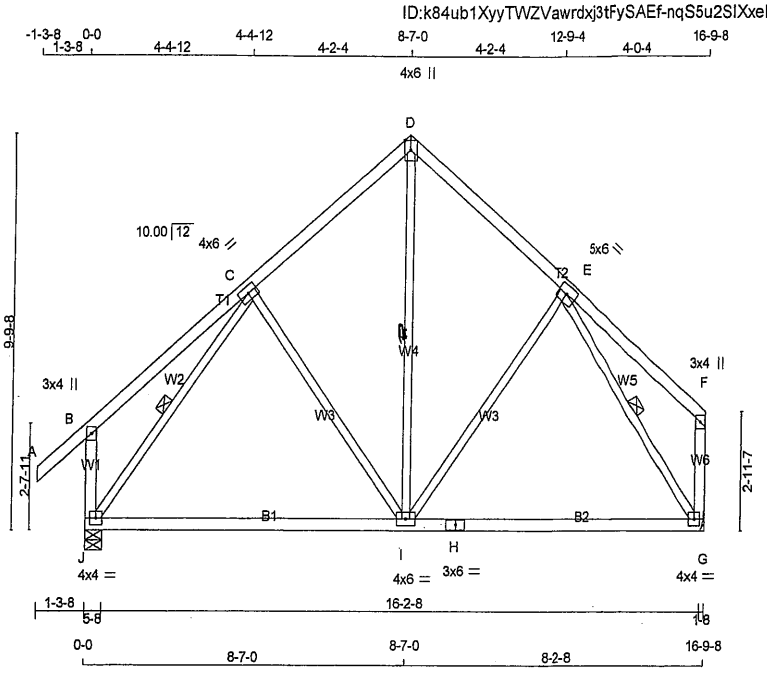

TOTAL WEIGHT = 3 X 82 = 245 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1335	0	1335	0	0	5-8	5-8
G	1179	0	1179	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1047	635 / 0	176 / 0	0 / 0	0 / 0	235 / 0	0 / 0
G	940	545 / 0	176 / 0	0 / 0	0 / 0	218 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-J, E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 37	-112.4	-112.4	0.34 (1)	10.00
C- D	-750 / 0	-112.4	-112.4	0.27 (1)	6.25
D- E	-747 / 0	-112.4	-112.4	0.25 (1)	6.25
E- F	0 / 37	-112.4	-112.4	0.31 (1)	10.00
J- B	-337 / 0	0.0	0.0	0.05 (1)	7.81
G- F	-160 / 0	0.0	0.0	0.03 (1)	7.81
J- I	0 / 658	-28.0	-28.0	0.66 (3)	10.00
I- H	0 / 619	-28.0	-28.0	0.65 (3)	10.00
H- G	0 / 619	-28.0	-28.0	0.65 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-185 / 47	0.19 (1)
I- D	0 / 537	0.12 (1)
I- E	-119 / 69	0.12 (1)
J- C	-1119 / 0	0.39 (1)
E- G	-1120 / 0	0.37 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 739 (0.27")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.66/1.00 (I-J:3) , WB=0.39/1.00 (C-J:1) , SSI=0.21/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (J) (INPUT = 0.90)
JSI METAL= 0.41 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4106
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 16446-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T23	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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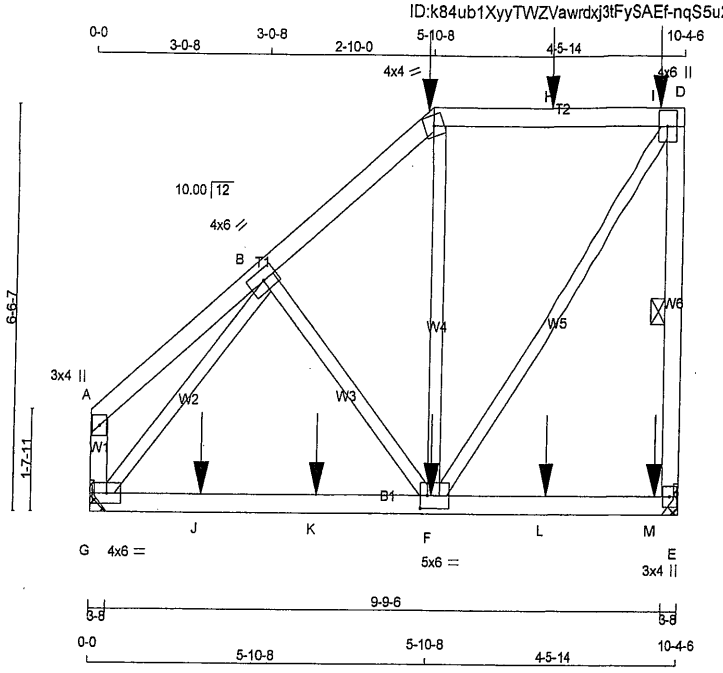

TOTAL WEIGHT = 52 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	6.0		
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	5.0	6.0	2.50	1.50
G	BMVW1-t	MT20	4.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 614.1 lbs FACTORED DOWN AT 5-10-8, AND 135.3 lbs FACTORED DOWN AT 7-11-4, AND 167.6 lbs FACTORED DOWN AT 9-11-4 ON TOP CHORD, AND 102.2 lbs FACTORED DOWN AT 1-11-4, 88.9 lbs FACTORED DOWN AT 3-11-4, 73.3 lbs FACTORED DOWN AT 5-11-4, AND 73.3 lbs FACTORED DOWN AT 7-11-4, AND 87.4 lbs FACTORED DOWN AT 9-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	1479	0	1479	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
G	1162	0	1162	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1170	692 / 0	209 / 0	0 / 0	0 / 0	270 / 0	0 / 0
G	936	527 / 0	186 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 24	-112.4	-112.4	0.17 (1)	10.00
B- C	-890 / 0	-112.4	-112.4	0.18 (1)	6.25
C- H	-654 / 0	-112.4	-112.4	0.72 (1)	5.69
H- I	-654 / 0	-112.4	-112.4	0.72 (1)	5.69
I- D	-654 / 0	-112.4	-112.4	0.72 (1)	5.69
E- D	-1395 / 0	0.0	0.0	0.29 (1)	5.48
G- A	-125 / 0	0.0	0.0	0.01 (1)	7.81
G- J	0 / 708	-28.0	-28.0	0.54 (2)	10.00
J- K	0 / 708	-28.0	-28.0	0.54 (2)	10.00
K- F	0 / 708	-28.0	-28.0	0.54 (2)	10.00
F- L	0 / 0	-28.0	-28.0	0.48 (3)	10.00
L- M	0 / 0	-28.0	-28.0	0.48 (3)	10.00
M- E	0 / 0	-28.0	-28.0	0.48 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- F	-71 / 53	0.03 (1)
F- C	-531 / 104	0.38 (1)
F- D	0 / 1139	0.28 (1)
G- B	-1152 / 0	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-614	-614	—	FRONT	VERT	TOTAL
F	5-11-4	-42	-73	—	FRONT	VERT	TOTAL
H	7-11-4	-135	-135	—	FRONT	VERT	TOTAL
I	9-11-4	-168	-168	—	FRONT	VERT	TOTAL
J	1-11-4	-84	-102	—	FRONT	VERT	TOTAL
K	3-11-4	-51	-89	—	FRONT	VERT	TOTAL
L	7-11-4	-42	-73	—	FRONT	VERT	TOTAL
M	9-11-4	-50	-87	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 777 (0.16")

CSI: TC=0.72/1.00 (C-D:1) , BC=0.54/1.00 (F-G:2) , WB=0.43/1.00 (B-G:1) , SSI=0.40/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

DWG NO. TAM 18449 -18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T24S	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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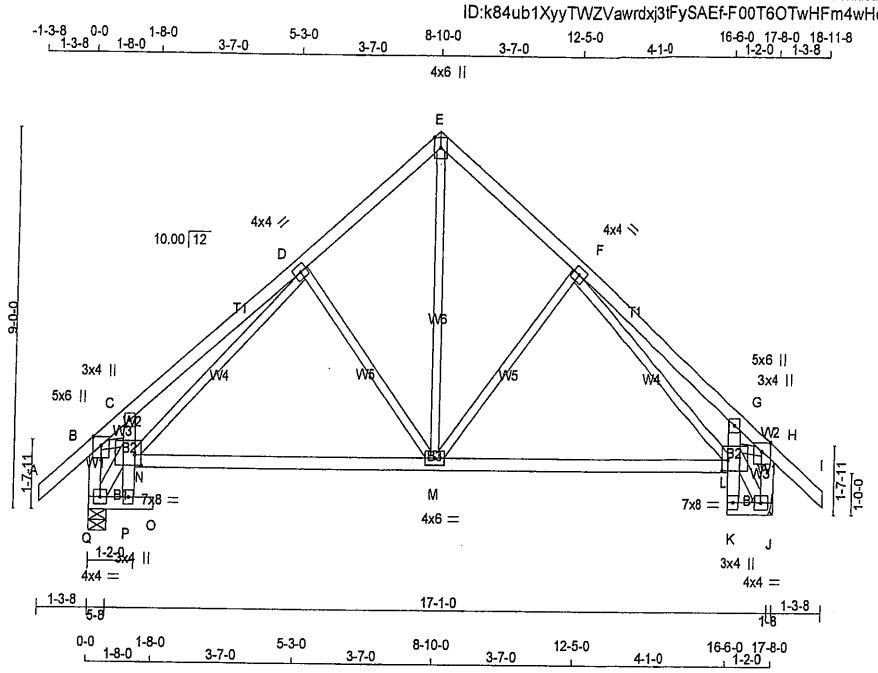

TOTAL WEIGHT = 3 X 88 = 263 lb
[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
P - C	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TMV+p	MT20	3.0	4.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMVW1-t	MT20	4.0	4.0		
K	BMV+p	MT20	3.0	4.0		
L	BVMWWW-l	MT20	7.0	8.0	3.00	2.00
M	BMWWW-t	MT20	4.0	6.0		
N	BVMWWW-l	MT20	7.0	8.0	3.00	2.00
P	BMV+p	MT20	3.0	4.0		
Q	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1413	0	1413	0	0	5-8	5-8
J	1397	0	1397	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1117	664 / 0	198 / 0	0 / 0	0 / 0	255 / 0	0 / 0
J	1097	664 / 0	186 / 0	0 / 0	0 / 0	247 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	-1307 / 0	-112.4	-112.4	0.13 (1)	5.52
C- D	-1304 / 0	-112.4	-112.4	0.23 (1)	5.43
D- E	-982 / 0	-112.4	-112.4	0.21 (1)	6.04
E- F	-982 / 0	-112.4	-112.4	0.21 (1)	6.04
F- G	-1288 / 0	-112.4	-112.4	0.22 (1)	5.45
G- H	-1290 / 0	-112.4	-112.4	0.13 (1)	5.55
H- I	0 / 50	-112.4	-112.4	0.15 (1)	10.00
Q- B	-1353 / 0	0.0	0.0	0.14 (1)	6.97
J- H	-1338 / 0	0.0	0.0	0.14 (1)	7.00
Q- P	0 / 46	-28.0	-28.0	0.01 (2)	10.00
P- O	0 / 0	-28.0	-28.0	0.01 (3)	10.00
P- N	0 / 57	0.0	0.0	0.07 (1)	10.00
N- C	-263 / 0	0.0	0.0	0.05 (1)	7.81
N- M	0 / 979	-28.0	-28.0	0.61 (3)	10.00
M- L	0 / 977	-28.0	-28.0	0.61 (3)	10.00
K- L	0 / 25	0.0	0.0	0.06 (1)	10.00
L- G	-264 / 0	0.0	0.0	0.05 (1)	7.81
K- J	0 / 46	-28.0	-28.0	0.01 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- M	-407 / 0	0.24 (1)
M- E	0 / 913	0.21 (1)
M- F	-405 / 0	0.24 (1)
N- D	0 / 93	0.02 (3)
F- L	0 / 63	0.02 (3)
Q- N	-65 / 0	0.01 (1)
B- N	0 / 1037	0.23 (1)
L- J	-64 / 0	0.01 (1)
L- H	0 / 1024	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CSI: TC=0.23/1.00 (C-D:1) , BC=0.61/1.00 (M-N:3) , WB=0.24/1.00 (D-M:1) , SSI=0.20/1.00 (L-M:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.82 (N) (INPUT = 0.90)
USI METAL= 0.24 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18450.18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296709	T24	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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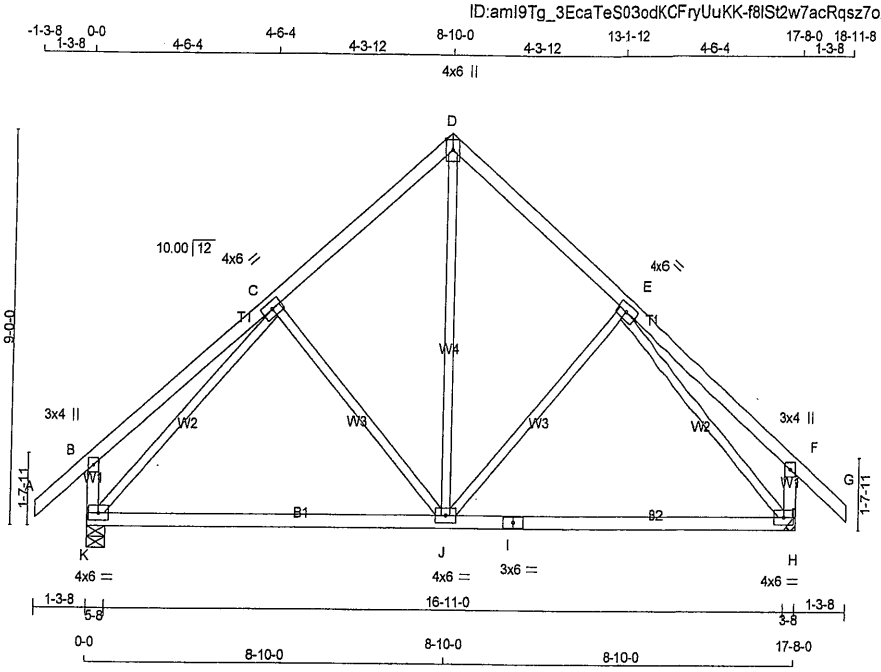

TOTAL WEIGHT = 3 X 81 = 242 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1396	0	1396	0	0	5-8	5-8
H	1396	0	1396	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1095	664 / 0	186 / 0	0 / 0	0 / 0	246 / 0	0 / 0
H	1095	664 / 0	186 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00
B- C	0 / 38	-112.4	-112.4	0.36 (1)	10.00
C- D	-900 / 0	-112.4	-112.4	0.28 (1)	6.14
D- E	-900 / 0	-112.4	-112.4	0.28 (1)	6.14
E- F	0 / 38	-112.4	-112.4	0.36 (1)	10.00
F- G	0 / 50	-112.4	-112.4	0.15 (1)	10.00
K- B	-344 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-344 / 0	0.0	0.0	0.04 (1)	7.81
K- J	0 / 848	-28.0	-28.0	0.74 (3)	10.00
J- I	0 / 848	-28.0	-28.0	0.74 (3)	10.00
I- H	0 / 848	-28.0	-28.0	0.74 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	0 / 721	0.16 (1)
J- E	-278 / 15	0.23 (1)
C- J	-278 / 15	0.23 (1)
K- C	-1280 / 0	1.00 (1)
E- H	-1280 / 0	1.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.59")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.59")
CALCULATED VERT. DEFL.(TL) = L/ 721 (0.29")

CSI: TC=0.36/1.00 (E-F:1) , BC=0.74/1.00 (H-J:3) , WB=1.00/1.00 (E-H:1) , SSI=0.22/1.00 (H-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (C) (INPUT = 0.90)
JSI METAL= 0.34 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEERS
41078
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM18456-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	G24	1	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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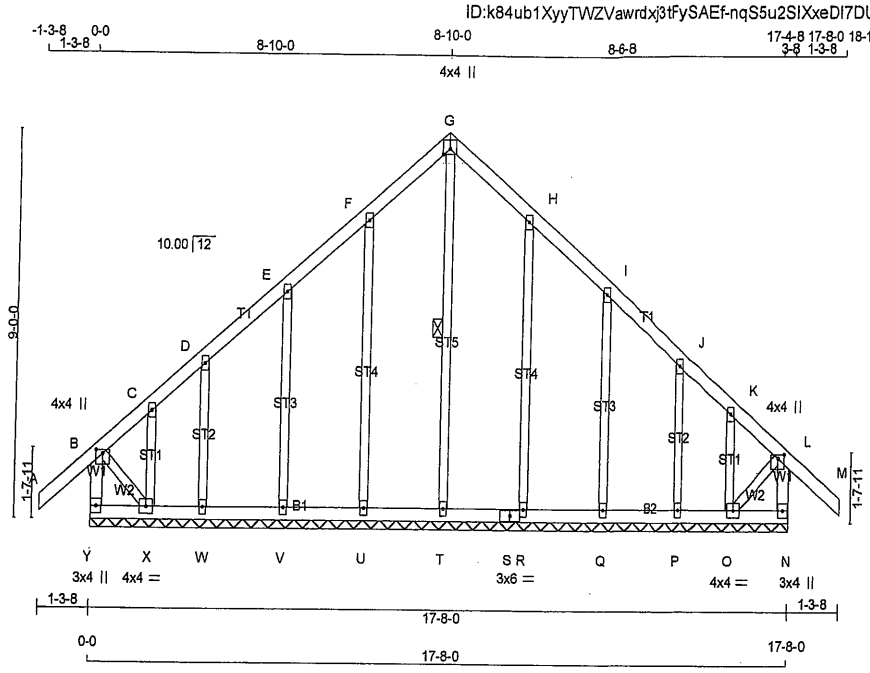

TOTAL WEIGHT = 92 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	1.50	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-365 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
B- C	-90 / 0	-112.4	-112.4	0.15 (1)	6.25	
C- D	-20 / 0	-112.4	-112.4	0.04 (1)	6.25	
D- E	-28 / 0	-112.4	-112.4	0.05 (1)	6.25	
E- F	-22 / 0	-112.4	-112.4	0.06 (1)	6.25	
F- G	-35 / 0	-112.4	-112.4	0.06 (1)	6.25	
G- H	-35 / 0	-112.4	-112.4	0.06 (1)	6.25	
H- I	-22 / 0	-112.4	-112.4	0.06 (1)	6.25	
I- J	-28 / 0	-112.4	-112.4	0.05 (1)	6.25	
J- K	-20 / 0	-112.4	-112.4	0.04 (1)	6.25	
K- L	-90 / 0	-112.4	-112.4	0.15 (1)	6.25	
L- M	0 / 50	-112.4	-112.4	0.15 (1)	10.00	
N- L	-365 / 0	0.0	0.0	0.04 (1)	7.81	
Y- X	0 / 0	-28.0	-28.0	0.01 (3)	10.00	
X- W	0 / 28	-28.0	-28.0	0.02 (2)	10.00	
W- V	0 / 23	-28.0	-28.0	0.03 (3)	10.00	
V- U	0 / 19	-28.0	-28.0	0.03 (3)	10.00	
U- T	0 / 16	-28.0	-28.0	0.02 (3)	10.00	
T- S	0 / 16	-28.0	-28.0	0.02 (3)	10.00	
S- R	0 / 16	-28.0	-28.0	0.02 (3)	10.00	
R- Q	0 / 19	-28.0	-28.0	0.03 (3)	10.00	
Q- P	0 / 23	-28.0	-28.0	0.03 (3)	10.00	
P- O	0 / 28	-28.0	-28.0	0.02 (2)	10.00	
O- N	0 / 0	-28.0	-28.0	0.01 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-164 / 0	0.08 (1)
U- F	-255 / 0	0.24 (1)
V- E	-219 / 0	0.11 (1)
W- D	-206 / 0	0.05 (1)
X- C	-47 / 0	0.01 (1)
R- H	-255 / 0	0.24 (1)
Q- I	-219 / 0	0.11 (1)
P- J	-206 / 0	0.05 (1)
O- K	-47 / 0	0.01 (1)
B- X	0 / 40	0.01 (1)
O- L	0 / 40	0.01 (1)

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (L-M:1) , BC=0.03/1.00 (V-W:3) , WB=0.24/1.00 (H-R:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (G) (INPUT = 0.90)
JSI METAL= 0.07 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4108
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18456-18
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	T100	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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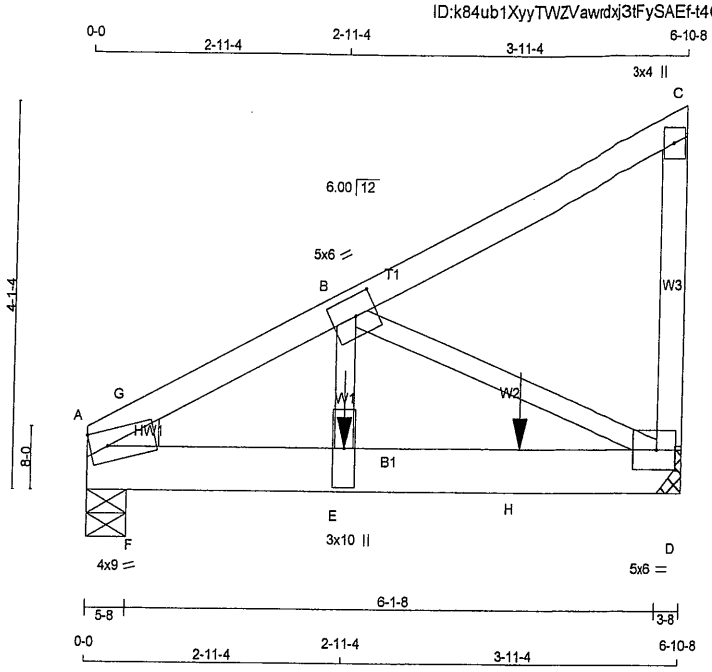

TOTAL WEIGHT = 2 X 30 = 60 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	2	6	SIDE(427.3)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	2	SIDE(237.6)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	4.0	9.0		Edge
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	5.0	6.0	2.50	2.75
E	BMW+w	MT20	3.0	10.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2548.4 lbs FACTORED DOWN AT 2-11-4, AND 1148.1 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
D	2396	0	2396	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8		
A	2266	0	2266	0	0	5-8	5-8	2x4 L

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	1878	1143 / 0	315 / 0	0 / 0	0 / 0	420 / 0	0 / 0
A	1771	1085 / 0	293 / 0	0 / 0	0 / 0	393 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- G	-4573 / 0	-112.4	-112.4	0.18 (1)	4.33
G- B	-3878 / 0	-112.4	-112.4	0.17 (1)	4.65
B- C	-3 / 4	-112.4	-112.4	0.15 (1)	10.00
D- C	-214 / 0	0.0	0.0	0.03 (1)	7.81
A- F	0 / 3447	-28.0	-28.0	0.40 (1)	10.00
F- E	0 / 3447	-28.0	-28.0	0.40 (1)	10.00
E- H	0 / 3447	-28.0	-28.0	0.67 (1)	10.00
H- D	0 / 3447	-28.0	-28.0	0.67 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2945	0.36 (1)
B- D	-3770 / 0	0.57 (1)
F- G	0 / 1067	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	2-11-4	-2548	-2548	—	FRONT	VERT	TOTAL
H	4-11-4	-1148	-1148	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.18/1.00 (A-G:1) , BC=0.67/1.00 (D-E:1) , WB=0.57/1.00 (B-D:1) , SSI=0.26/1.00 (A-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.59 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41010
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 13442 -18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T101S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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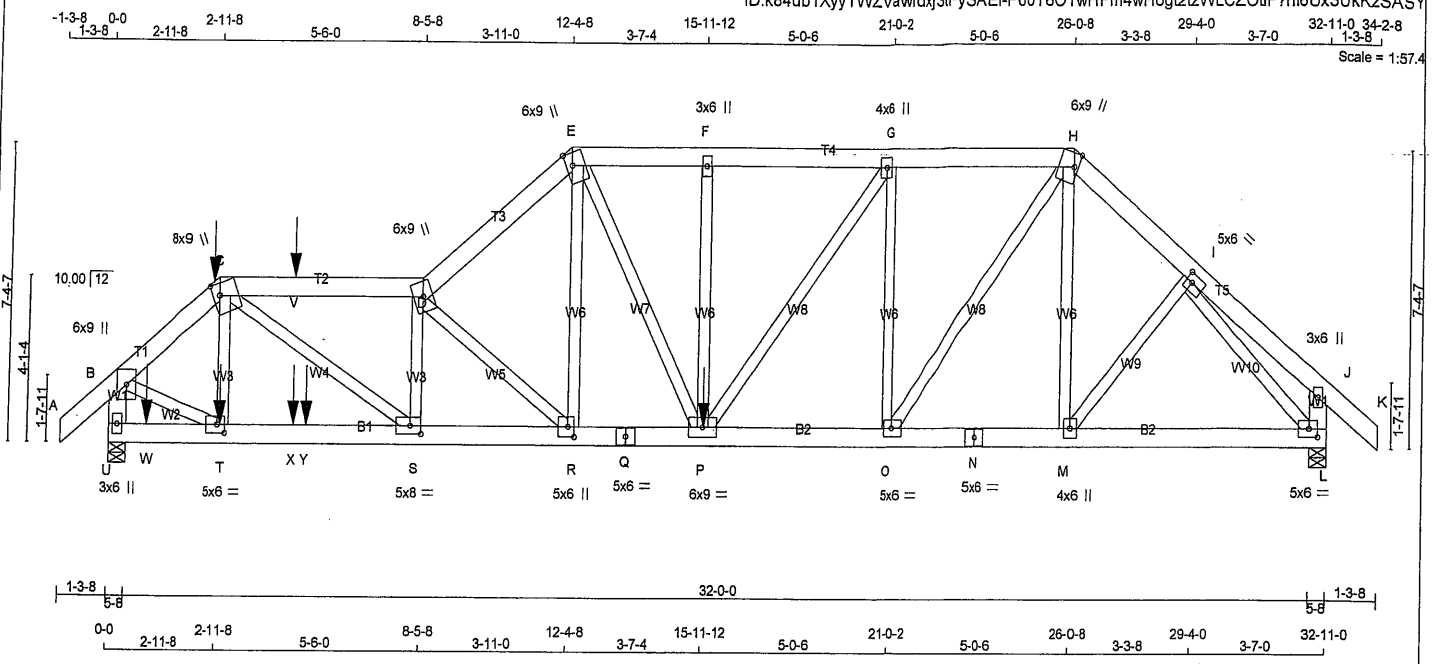

TOTAL WEIGHT = 2 X 226 = 453 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
D - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- D	2	12	SIDE(61.0)
D- E	2	12	TOP
E- H	2	12	TOP
H- K	2	12	TOP
U- B	2	12	TOP
L- J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
U- Q	2	12	SIDE(183.1)
Q- N	2	12	SIDE(183.1)
N- L	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
F- P	1	5	SIDE(727.7)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	6.0	9.0		
C	TTWW+m	MT20	8.0	9.0	3.50	2.00
D	TTWW+m	MT20	6.0	9.0		
E	TTWW+m	MT20	6.0	9.0	Edge	2.00
F	TMW+w	MT20	3.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	6.0	9.0	4.00	1.25
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	6.0		
L	BMVW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW+t	MT20	4.0	6.0		
N	BS-t	MT20	5.0	6.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	9.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	6079	0	6079	0	0	5-8	5-8
L	4471	0	4471	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	4773	2885 / 0	811 / 0	0 / 0	0 / 0	1077 / 0	0 / 0
L	3516	2117 / 0	604 / 0	0 / 0	0 / 0	795 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-6494 / 0	-112.4	-112.4	0.10 (1)	4.62
C- V	-10237 / 0	-112.4	-112.4	0.38 (1)	3.53
V- D	-10237 / 0	-112.4	-112.4	0.38 (1)	3.53
D- E	-8657 / 0	-112.4	-112.4	0.19 (1)	4.02
E- F	-7643 / 0	-112.4	-112.4	0.15 (1)	4.28
F- G	-7643 / 0	-112.4	-112.4	0.19 (1)	4.24
G- H	-6055 / 0	-112.4	-112.4	0.15 (1)	4.70
H- I	-5037 / 0	-112.4	-112.4	0.09 (1)	5.13
I- J	-7 / 4	-112.4	-112.4	0.06 (1)	10.00
J- K	0 / 52	-112.4	-112.4	0.05 (1)	10.00
U- B	-6299 / 0	0.0	0.0	0.23 (1)	5.91
L- J	-354 / 0	0.0	0.0	0.01 (1)	7.81
U- W	0 / 0	-28.0	-28.0	0.33 (1)	10.00
W- T	0 / 0	-28.0	-28.0	0.33 (1)	10.00
T- X	0 / 4945	-28.0	-28.0	0.82 (1)	10.00
X- Y	0 / 4945	-28.0	-28.0	0.82 (1)	10.00
Y- S	0 / 4945	-28.0	-28.0	0.82 (1)	10.00
S- R	0 / 10351	-28.0	-28.0	0.77 (1)	10.00
R- Q	0 / 6720	-28.0	-28.0	0.48 (1)	10.00
Q- P	0 / 6720	-28.0	-28.0	0.48 (1)	10.00
P- O	0 / 6055	-28.0	-28.0	0.43 (1)	10.00
O- N	0 / 3843	-28.0	-28.0	0.29 (1)	10.00
N- M	0 / 3843	-28.0	-28.0	0.29 (1)	10.00
M- L	0 / 3511	-28.0	-28.0	0.27 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- C	-364 / 114	0.03 (1)
C- S	0 / 6382	0.56 (1)
S- D	-2836 / 0	0.24 (1)
D- R	-4951 / 0	0.84 (1)
R- E	0 / 3295	0.29 (1)
E- P	0 / 1987	0.18 (1)
P- F	-370 / 0	0.11 (1)
P- G	0 / 2723	0.24 (1)
O- G	-2834 / 0	0.88 (1)
O- H	0 / 3712	0.33 (1)
M- H	-194 / 106	0.06 (1)
M- I	0 / 514	0.05 (1)
B- T	0 / 5270	0.47 (1)
I- L	-5348 / 0	0.85 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-188	-188	—	FRONT	VERT	TOTAL
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
P	15-11-15	-3491	-3491	—	FRONT	VERT	TOTAL
T	3-0-4	-40	-70	—	FRONT	VERT	TOTAL
V	5-0-4	-135	-135	—	FRONT	VERT	TOTAL
W	1-0-4	-40	-70	—	FRONT	VERT	TOTAL
X	5-0-4	-40	-70	—	FRONT	VERT	TOTAL
Y	5-4-7	-1432	-1432	—	FRONT	VERT	TOTAL

[M]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.82/1.00 (S-T:1) , WB=0.88/1.00 (G-O:1) , SSI=0.34/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.67 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4106
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18451-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	6.0	3.00	2.00
S	BMWW-t	MT20	5.0	8.0	2.50	3.50
T	BMWW-t	MT20	5.0	6.0	2.50	2.25
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 426.0 lbs FACTORED DOWN AT 2-11-8, AND 135.3 lbs FACTORED DOWN AT 5-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, AND 1431.8 lbs FACTORED DOWN AT 5-4-7, AND 3491.2 lbs FACTORED DOWN AT 15-11-15 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
41078
S. KATSOULAKOS
ONTARIO

DWG NO. TAM/18451-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296709	T102	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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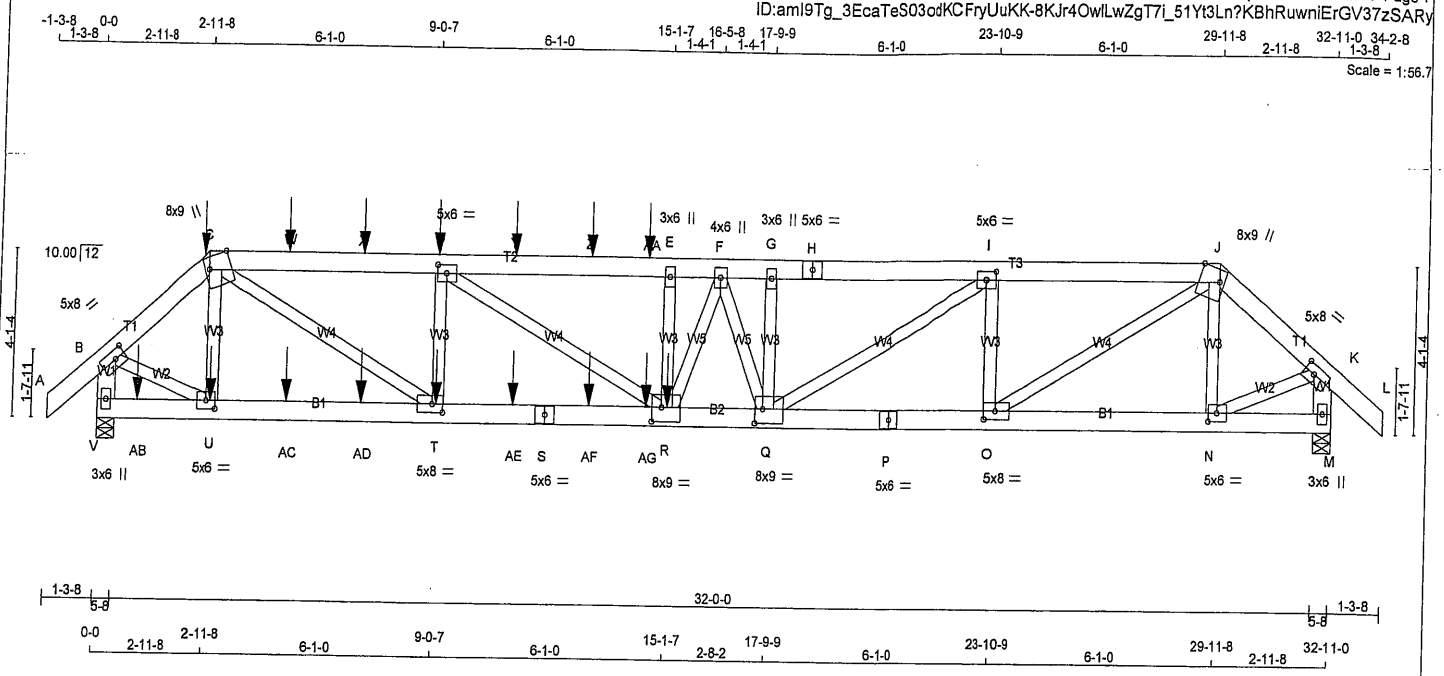

TOTAL WEIGHT = 2 X 198 = 395 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- H	2	12	SIDE(183.1)
H- J	2	12	TOP
J- L	2	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
E- R	1	2	SIDE(624.8)
G- Q	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	8.0	9.0	Edge	6.50
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	8.0	9.0	4.25	2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	5301	0	5301	0	0	5-8	5-8
M	4254	0	4254	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
V	4178	2497 / 0	729 / 0	0 / 0	0 / 0	953 / 0	0 / 0
M	3345	2014 / 0	575 / 0	0 / 0	0 / 0	756 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-5397 / 0	-112.4	-112.4	0.08 (1)	4.99
C- W	-10322 / 0	-112.4	-112.4	0.44 (1)	3.50
W- X	-10322 / 0	-112.4	-112.4	0.44 (1)	3.50
X- D	-10322 / 0	-112.4	-112.4	0.44 (1)	3.50
D- Y	-13841 / 0	-112.4	-112.4	0.60 (1)	2.91
Y- Z	-13841 / 0	-112.4	-112.4	0.60 (1)	2.91
Z-AA	-13841 / 0	-112.4	-112.4	0.60 (1)	2.91
AA- E	-13841 / 0	-112.4	-112.4	0.60 (1)	2.91
E- F	-13841 / 0	-112.4	-112.4	0.35 (1)	3.14
F- G	-12549 / 0	-112.4	-112.4	0.29 (1)	3.33
G- H	-12549 / 0	-112.4	-112.4	0.43 (1)	3.23
H- I	-12549 / 0	-112.4	-112.4	0.43 (1)	3.23
I- J	-8712 / 0	-112.4	-112.4	0.29 (1)	3.94
J- K	-4275 / 0	-112.4	-112.4	0.07 (1)	5.49
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
V- B	-5291 / 0	0.0	0.0	0.19 (1)	6.36
M- K	-4259 / 0	0.0	0.0	0.15 (1)	6.93
V-AB	0 / 0	-28.0	-28.0	0.08 (2)	10.00
AB- U	0 / 0	-28.0	-28.0	0.08 (2)	10.00
U-AC	0 / 4075	-28.0	-28.0	0.33 (1)	10.00
AC-AD	0 / 4075	-28.0	-28.0	0.33 (1)	10.00
AD- T	0 / 4075	-28.0	-28.0	0.33 (1)	10.00
T-AE	0 / 10322	-28.0	-28.0	0.79 (1)	10.00
AE- S	0 / 10322	-28.0	-28.0	0.79 (1)	10.00
S-AF	0 / 10322	-28.0	-28.0	0.79 (1)	10.00
AF-AG	0 / 10322	-28.0	-28.0	0.79 (1)	10.00
AG- R	0 / 10322	-28.0	-28.0	0.79 (1)	10.00
R- Q	0 / 13185	-28.0	-28.0	0.93 (1)	10.00
Q- P	0 / 8713	-28.0	-28.0	0.64 (1)	10.00
P- O	0 / 8713	-28.0	-28.0	0.64 (1)	10.00
O- N	0 / 3226	-28.0	-28.0	0.26 (1)	10.00
N- M	0 / 0	-28.0	-28.0	0.06 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-1171 / 0	0.10 (1)
C- T	0 / 7243	0.64 (1)
T- D	-3367 / 0	0.29 (1)
D- R	0 / 4116	0.36 (1)
R- E	-695 / 0	0.06 (1)
R- F	0 / 1905	0.17 (1)
F- Q	-1843 / 0	0.17 (1)
Q- G	-457 / 0	0.04 (1)
Q- I	0 / 4488	0.40 (1)
O- I	-3070 / 0	0.26 (1)
O- J	0 / 6361	0.56 (1)
N- J	-966 / 0	0.08 (1)
B- U	0 / 4380	0.39 (1)
N- K	0 / 3469	0.31 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-33	-36	---	FRONT	VERT	DEAD
C	2-11-8	-142	-142	---	BACK	VERT	TOTAL
C	2-11-8	-206	-206	---	FRONT	VERT	SNOW

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.31")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 775 (0.51")

CSI: TC=0.60/1.00 (D-E:1) , BC=0.93/1.00 (Q-R:1) , WB=0.64/1.00 (C-T:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (T) (INPUT = 0.90)
JSI METAL= 0.89 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4 1076
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18457 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
296709	T102	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	8.0	9.0	4.25	3.00
S	BS-t	MT20	5.0	6.0		
T	BMWW-t	MT20	5.0	8.0	2.50	3.25
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 238.0 lbs FACTORED DOWN AT 2-11-8, 142.0 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, 135.3 lbs FACTORED DOWN AT 11-0-12, AND 135.3 lbs FACTORED DOWN AT 13-0-12, AND 135.3 lbs FACTORED DOWN AT 14-6-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, AND 69.9 lbs FACTORED DOWN AT 14-6-12, AND 3096.9 lbs FACTORED DOWN AT 15-1-7 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	9-0-12	-135	-135	—	BACK	VERT	TOTAL
R	15-1-7	-3097	-3097	—	BACK	VERT	TOTAL
T	9-0-12	-40	-70	—	BACK	VERT	TOTAL
U	3-0-12	-40	-70	—	BACK	VERT	TOTAL
W	5-0-12	-135	-135	—	BACK	VERT	TOTAL
X	7-0-12	-135	-135	—	BACK	VERT	TOTAL
Y	11-0-12	-135	-135	—	BACK	VERT	TOTAL
Z	13-0-12	-135	-135	—	BACK	VERT	TOTAL
AA	14-6-12	-135	-135	—	BACK	VERT	TOTAL
AB	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AC	5-0-12	-40	-70	—	BACK	VERT	TOTAL
AD	7-0-12	-40	-70	—	BACK	VERT	TOTAL
AE	11-0-12	-40	-70	—	BACK	VERT	TOTAL
AF	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AG	14-6-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4198
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18457-18
STRUCTURAL
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296708	T102Z	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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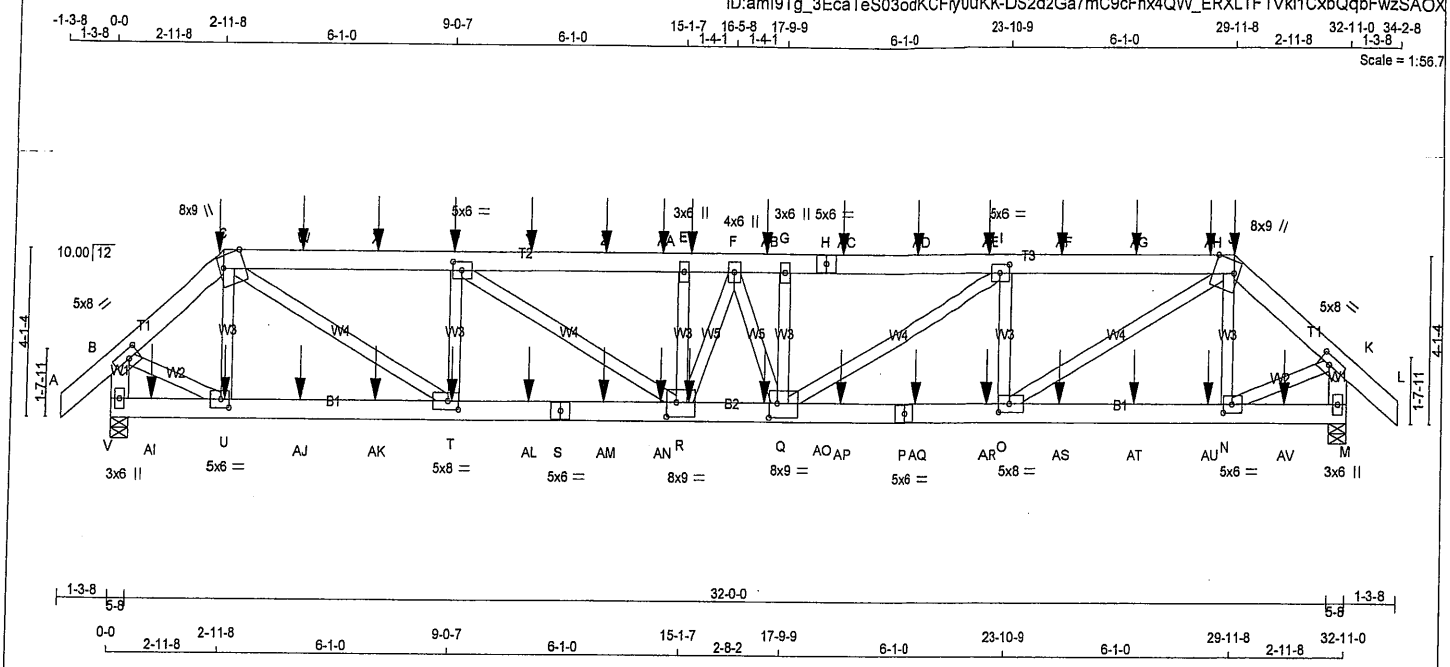

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- H	2	12	SIDE(183.1)
H- J	2	12	SIDE(61.0)
J- L	2	12	SIDE(122.0)
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(183.1)
P- M	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	3.0	6.0		
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.50	2.75
J	TTWW+m	MT20	8.0	9.0	Edge	6.50
K	TMVW-t	MT20	5.0	8.0	2.50	3.25
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.75
O	BMWW-t	MT20	5.0	8.0	2.50	3.25
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	8.0	9.0	4.25	2.50
R	BMWWW-t	MT20	8.0	9.0	4.25	3.00
S	BS-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4144	0	4144	0	0	5-8	5-8
M	4057	0	4057	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3293	1927 / 0	606 / 0	0 / 0	0 / 0	761 / 0	0 / 0
M	3225	1885 / 0	595 / 0	0 / 0	0 / 0	745 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-4131 / 0	-112.4	-112.4	0.07 (1)	5.56
C- W	-7363 / 0	-112.4	-112.4	0.34 (1)	4.16
W- X	-7363 / 0	-112.4	-112.4	0.34 (1)	4.16
X- D	-7363 / 0	-112.4	-112.4	0.34 (1)	4.16
D- Y	-8990 / 0	-112.4	-112.4	0.39 (1)	3.77
Y- Z	-8990 / 0	-112.4	-112.4	0.39 (1)	3.77
Z-AA	-8990 / 0	-112.4	-112.4	0.39 (1)	3.77
AA- E	-8990 / 0	-112.4	-112.4	0.39 (1)	3.77
E- F	-8990 / 0	-112.4	-112.4	0.22 (1)	3.93
F-AB	-8919 / 0	-112.4	-112.4	0.21 (1)	3.96
AB- G	-8919 / 0	-112.4	-112.4	0.21 (1)	3.96
G- H	-8919 / 0	-112.4	-112.4	0.40 (1)	3.77
H-AC	-8919 / 0	-112.4	-112.4	0.40 (1)	3.77
AC-AD	-8919 / 0	-112.4	-112.4	0.40 (1)	3.77
AD-AE	-8919 / 0	-112.4	-112.4	0.40 (1)	3.77
AE- I	-8919 / 0	-112.4	-112.4	0.40 (1)	3.77
I-AF	-7297 / 0	-112.4	-112.4	0.35 (1)	4.16
AF-AG	-7297 / 0	-112.4	-112.4	0.35 (1)	4.16
AG-AH	-7297 / 0	-112.4	-112.4	0.35 (1)	4.16
AH- J	-7297 / 0	-112.4	-112.4	0.35 (1)	4.16
J- K	-4047 / 0	-112.4	-112.4	0.07 (1)	5.60
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
V- B	-4126 / 0	0.0	0.0	0.15 (1)	7.02
M- K	-4049 / 0	0.0	0.0	0.15 (1)	7.07
V-AI	0 / 0	-28.0	-28.0	0.08 (2)	10.00
AI- U	0 / 0	-28.0	-28.0	0.08 (2)	10.00
U-AJ	0 / 3121	-28.0	-28.0	0.26 (1)	10.00
AJ-AK	0 / 3121	-28.0	-28.0	0.26 (1)	10.00
AK- T	0 / 3121	-28.0	-28.0	0.26 (1)	10.00
T-AL	0 / 7363	-28.0	-28.0	0.56 (1)	10.00
AL- S	0 / 7363	-28.0	-28.0	0.56 (1)	10.00
S-AM	0 / 7363	-28.0	-28.0	0.56 (1)	10.00
AM-AN	0 / 7363	-28.0	-28.0	0.56 (1)	10.00
AN- R	0 / 7363	-28.0	-28.0	0.56 (1)	10.00
R-AO	0 / 8920	-28.0	-28.0	0.60 (1)	10.00
AO- Q	0 / 8920	-28.0	-28.0	0.60 (1)	10.00
Q-AP	0 / 7297	-28.0	-28.0	0.55 (1)	10.00
AP- P	0 / 7297	-28.0	-28.0	0.55 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-837 / 0	0.07 (1)
C- T	0 / 4919	0.43 (1)
T- D	-2210 / 0	0.19 (1)
D- R	0 / 1903	0.17 (1)
R- E	-943 / 0	0.08 (1)
R- F	0 / 203	0.02 (1)
F- Q	-47 / 0	0.00 (3)
Q- G	-788 / 0	0.07 (1)
Q- I	0 / 1897	0.17 (1)
O- I	-2213 / 0	0.19 (1)
O- J	0 / 4915	0.43 (1)
N- J	-814 / 0	0.07 (1)
B- U	0 / 3352	0.30 (1)
N- K	0 / 3284	0.29 (1)

LICENSED PROFESSIONAL ENGINEER
4167
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.40/1.00 (G-I:1) , BC=0.60/1.00 (Q-R:1) , WB=0.43/1.00 (C-T:1) , SSI=0.21/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (B) (INPUT = 0.90)
JSI METAL= 0.63 (P) (INPUT = 1.00)

DWG NO. TAM 16459-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
296708	T102Z	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:aml9Tg_3EcaTeS03odKCFryUuKK-DS2d2Ga7mC9cFnx4QW_ERXLTFTVki1CxbQqbFwzSAOX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
T	BMWW-t	MT20	5.0	8.0	2.50	3.25
U	BMWW-t	MT20	5.0	6.0	2.50	2.75
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 238.0 lbs FACTORED DOWN AT 2-11-8, 238.0 lbs FACTORED DOWN AT 29-11-8, 185.6 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, 135.3 lbs FACTORED DOWN AT 11-0-12, 135.3 lbs FACTORED DOWN AT 13-0-12, 135.3 lbs FACTORED DOWN AT 14-6-12, 135.3 lbs FACTORED DOWN AT 15-3-12, 135.3 lbs FACTORED DOWN AT 17-3-12, 135.3 lbs FACTORED DOWN AT 19-3-12, 135.3 lbs FACTORED DOWN AT 21-3-12, 135.3 lbs FACTORED DOWN AT 23-3-12, 135.3 lbs FACTORED DOWN AT 25-3-12, AND 135.3 lbs FACTORED DOWN AT 27-3-12, AND 155.1 lbs FACTORED DOWN AT 29-3-12 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, 69.9 lbs FACTORED DOWN AT 14-6-12, 69.9 lbs FACTORED DOWN AT 15-3-12, 69.9 lbs FACTORED DOWN AT 17-3-12, 69.9 lbs FACTORED DOWN AT 19-3-12, 69.9 lbs FACTORED DOWN AT 21-3-12, 69.9 lbs FACTORED DOWN AT 23-3-12, 69.9 lbs FACTORED DOWN AT 25-3-12, 69.9 lbs FACTORED DOWN AT 27-3-12, AND 69.9 lbs FACTORED DOWN AT 29-3-12, AND 69.9 lbs FACTORED DOWN AT 31-3-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-AQ	0 / 7297	-28.0	-28.0	0.55 (1)	10.00	
AQ-AR	0 / 7297	-28.0	-28.0	0.55 (1)	10.00	
AR- O	0 / 7297	-28.0	-28.0	0.55 (1)	10.00	
O-AS	0 / 3058	-28.0	-28.0	0.26 (1)	10.00	
AS-AT	0 / 3058	-28.0	-28.0	0.26 (1)	10.00	
AT-AU	0 / 3058	-28.0	-28.0	0.26 (1)	10.00	
AU- N	0 / 3058	-28.0	-28.0	0.26 (1)	10.00	
N-AV	0 / 0	-28.0	-28.0	0.08 (2)	10.00	
AV- M	0 / 0	-28.0	-28.0	0.08 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-186	-186	—	BACK	VERT	TOTAL
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
D	9-0-12	-135	-135	—	BACK	VERT	TOTAL
E	15-3-12	-135	-135	—	BACK	VERT	TOTAL
J	29-11-8	-33	-36	—	FRONT	VERT	DEAD
J	29-11-8	-206	-206	—	FRONT	VERT	SNOW
R	15-3-12	-40	-70	—	BACK	VERT	TOTAL
T	9-0-12	-40	-70	—	BACK	VERT	TOTAL
U	3-0-12	-40	-70	—	BACK	VERT	TOTAL
W	5-0-12	-135	-135	—	BACK	VERT	TOTAL
X	7-0-12	-135	-135	—	BACK	VERT	TOTAL
Y	11-0-12	-135	-135	—	BACK	VERT	TOTAL
Z	13-0-12	-135	-135	—	BACK	VERT	TOTAL
AA	14-6-12	-135	-135	—	BACK	VERT	TOTAL
AB	17-3-12	-135	-135	—	BACK	VERT	TOTAL
AC	19-3-12	-135	-135	—	BACK	VERT	TOTAL
AD	21-3-12	-135	-135	—	BACK	VERT	TOTAL
AE	23-3-12	-135	-135	—	BACK	VERT	TOTAL
AF	25-3-12	-135	-135	—	BACK	VERT	TOTAL
AG	27-3-12	-135	-135	—	BACK	VERT	TOTAL
AH	29-3-12	-155	-155	—	BACK	VERT	TOTAL
AI	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AJ	5-0-12	-40	-70	—	BACK	VERT	TOTAL
AK	7-0-12	-40	-70	—	BACK	VERT	TOTAL
AL	11-0-12	-40	-70	—	BACK	VERT	TOTAL
AM	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AN	14-6-12	-40	-70	—	BACK	VERT	TOTAL
AO	17-3-12	-40	-70	—	BACK	VERT	TOTAL
AP	19-3-12	-40	-70	—	BACK	VERT	TOTAL
AQ	21-3-12	-40	-70	—	BACK	VERT	TOTAL
AR	23-3-12	-40	-70	—	BACK	VERT	TOTAL
AS	25-3-12	-40	-70	—	BACK	VERT	TOTAL
AT	27-3-12	-40	-70	—	BACK	VERT	TOTAL
AU	29-3-12	-40	-70	—	BACK	VERT	TOTAL
AV	31-3-12	-40	-70	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18459-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T102S	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 10 09:36:28 2018 Page 1
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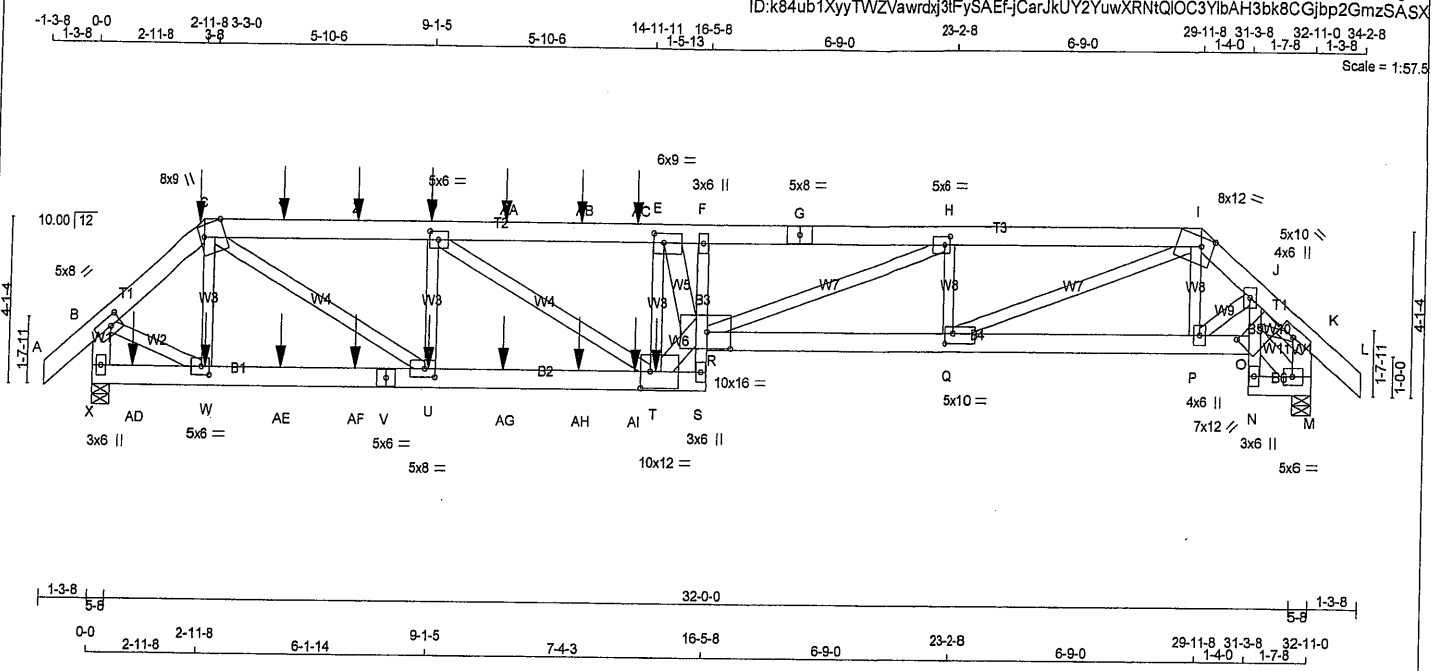

TOTAL WEIGHT = 2 X 200 = 399 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
E - R	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- I	2	12	TOP
I- L	2	12	TOP
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- X	2	12	SIDE(183.1)
V- S	2	12	SIDE(183.1)
R- O	2	12	TOP
N- M	2	12	TOP
S- F	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
E- T	1	2	SIDE(624.8)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.75
E	TMWW-t	MT20	6.0	9.0	2.75	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	5337	0	5337	0	0	5-8	5-8
M	4261	0	4261	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	4202	2521 / 0	729 / 0	0 / 0	0 / 0	952 / 0	0 / 0
M	3350	2019 / 0	575 / 0	0 / 0	0 / 0	757 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-5449 / 0	-112.4	-112.4	0.08 (1)	4.97
C- Y	-10316 / 0	-112.4	-112.4	0.45 (1)	3.49
Y- Z	-10316 / 0	-112.4	-112.4	0.45 (1)	3.49
Z- D	-10316 / 0	-112.4	-112.4	0.45 (1)	3.49
D-AA	-13977 / 0	-112.4	-112.4	0.62 (1)	2.87
AA-AB	-13977 / 0	-112.4	-112.4	0.62 (1)	2.87
AB-AC	-13977 / 0	-112.4	-112.4	0.62 (1)	2.87
AC- E	-13977 / 0	-112.4	-112.4	0.62 (1)	2.87
E- F	-18068 / 0	-112.4	-112.4	0.67 (1)	2.42
F- G	-18230 / 0	-112.4	-112.4	0.95 (1)	2.21
G- H	-18230 / 0	-112.4	-112.4	0.95 (1)	2.21
H- I	-12717 / 0	-112.4	-112.4	0.52 (1)	3.14
I- J	-5747 / 0	-112.4	-112.4	0.22 (1)	4.71
J- K	-6279 / 0	-112.4	-112.4	0.23 (1)	4.53
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
X- B	-5338 / 0	0.0	0.0	0.19 (1)	6.33
M- K	-4118 / 0	0.0	0.0	0.15 (1)	7.02
X-AD	0 / 0	-28.0	-28.0	0.09 (2)	10.00
AD- W	0 / 0	-28.0	-28.0	0.09 (2)	10.00
W-AE	0 / 4116	-28.0	-28.0	0.35 (1)	10.00
AE-AF	0 / 4116	-28.0	-28.0	0.35 (1)	10.00
AF- V	0 / 4116	-28.0	-28.0	0.35 (1)	10.00
V- U	0 / 4116	-28.0	-28.0	0.35 (1)	10.00
U-AG	0 / 10316	-28.0	-28.0	0.76 (1)	10.00
AG-AH	0 / 10316	-28.0	-28.0	0.76 (1)	10.00
AH-AI	0 / 10316	-28.0	-28.0	0.76 (1)	10.00
AI- T	0 / 10316	-28.0	-28.0	0.76 (1)	10.00
T- S	0 / 429	-28.0	-28.0	0.13 (2)	10.00
S- R	-222 / 0	0.0	0.0	0.32 (1)	7.81
R- F	0 / 109	0.0	0.0	0.33 (1)	10.00
R- Q	0 / 12717	-28.0	-28.0	0.94 (1)	10.00
Q- P	0 / 4525	-28.0	-28.0	0.44 (1)	10.00
P- O	0 / 4803	-28.0	-28.0	0.46 (1)	10.00
N- O	0 / 36	0.0	0.0	0.14 (1)	10.00
O- J	0 / 760	0.0	0.0	0.20 (1)	10.00
N- M	0 / 147	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-1149 / 0	0.10 (1)
T- E	-10037 / 0	0.86 (1)
T- R	0 / 17681	0.72 (1)
E- R	0 / 9953	0.65 (1)
R- H	0 / 5921	0.52 (1)
Q- H	-3044 / 0	0.20 (1)
Q- I	0 / 8760	0.77 (1)
P- I	-24 / 167	0.02 (3)
P- J	-340 / 0	0.02 (1)
B- W	0 / 4422	0.39 (1)
O- M	-191 / 0	0.01 (1)
O- K	0 / 4640	0.57 (1)
U- D	-3457 / 0	0.30 (1)
C- U	0 / 7189	0.64 (1)
D- T	0 / 4282	0.38 (1)

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAROS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPF OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 753 (0.52")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 465 (0.85")

CSI: TC=0.95/1.00 (F-H:1) , BC=0.94/1.00 (Q-R:1) , WB=0.86/1.00 (E-T:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 10452-18
STRUCTURAL COMPONENT

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287696	T102S	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:k84ub1XyyTWZVawrdxj3tFySAEf-jCarJkUY2YuwXRNtQlOC3YlbAH3bk8CGjbp2GmzSASX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
F	TMV+p	MT20	3.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0	2.50	2.00
I	TTWW-m	MT20	8.0	12.0	2.75	4.00
J	TMVW+p	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	10.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-w	MT20	7.0	12.0	2.75	7.50
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	10.0	3.00	2.75
R	BVMWWW-l	MT20	10.0	16.0	5.00	7.50
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	10.0	12.0	5.00	3.00
U	BMWW-t	MT20	5.0	8.0	2.50	3.25
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0	2.50	2.75
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 185.6 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, 135.3 lbs FACTORED DOWN AT 11-0-12, 135.3 lbs FACTORED DOWN AT 13-0-12, AND 135.3 lbs FACTORED DOWN AT 14-6-12, AND 238.0 lbs FACTORED DOWN AT 2-11-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, AND 69.9 lbs FACTORED DOWN AT 14-6-12, AND 3096.9 lbs FACTORED DOWN AT 15-1-7 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-186	-186	—	BACK	VERT	TOTAL
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
D	9-0-12	-135	-135	—	BACK	VERT	TOTAL
T	15-1-7	-3097	-3097	—	BACK	VERT	TOTAL
U	9-0-12	-40	-70	—	BACK	VERT	TOTAL
W	3-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	5-0-12	-135	-135	—	BACK	VERT	TOTAL
Z	7-0-12	-135	-135	—	BACK	VERT	TOTAL
AA	11-0-12	-135	-135	—	BACK	VERT	TOTAL
AB	13-0-12	-135	-135	—	BACK	VERT	TOTAL
AC	14-6-12	-135	-135	—	BACK	VERT	TOTAL
AD	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AE	5-0-12	-40	-70	—	BACK	VERT	TOTAL
AF	7-0-12	-40	-70	—	BACK	VERT	TOTAL
AG	11-0-12	-40	-70	—	BACK	VERT	TOTAL
AH	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AI	14-6-12	-40	-70	—	BACK	VERT	TOTAL

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.81 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
41978
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10452.18
STRUCTURAL
COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287694	T102S2	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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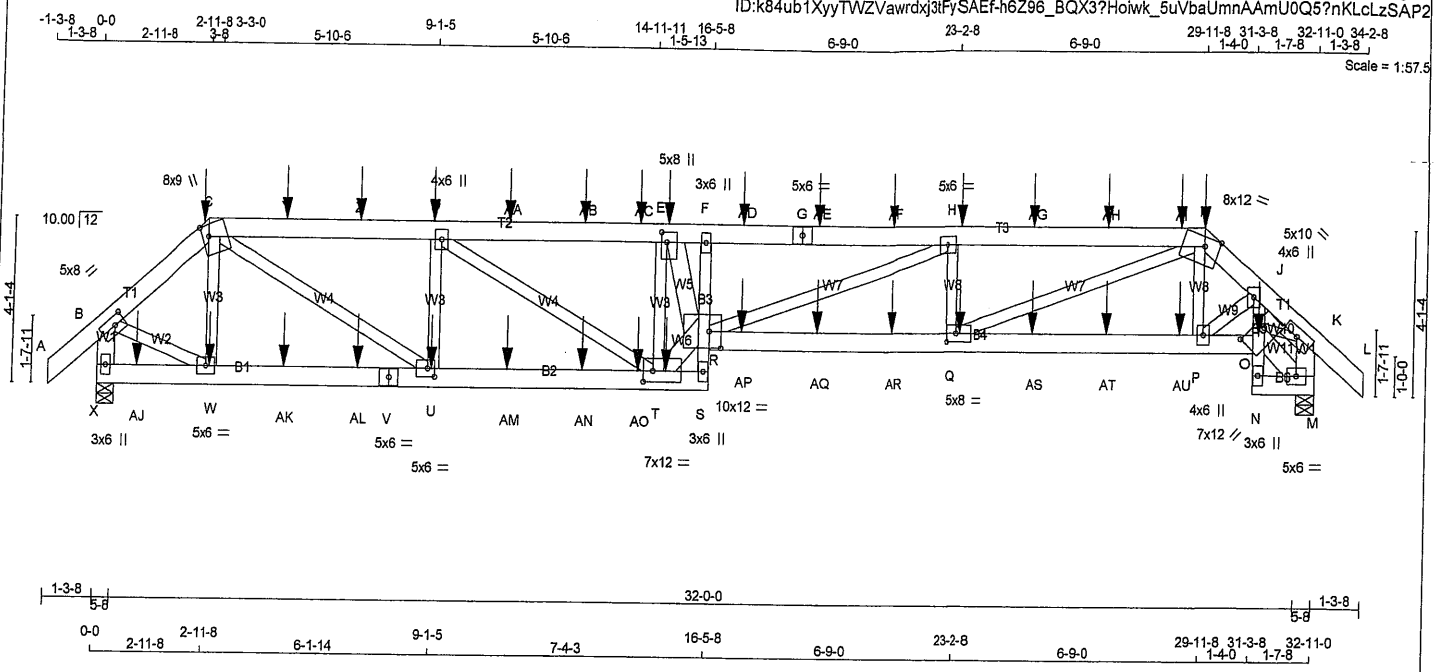

TOTAL WEIGHT = 2 X 200 = 399 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
I - L	2x6	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
X - V	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - F	2x4	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
T - R	2x6	DRY	1650F 1.5E	SPF
E - R	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
O - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	2	12	SIDE(122.0)
C- G	2	12	SIDE(183.1)
G- I	2	12	SIDE(183.1)
I- L	2	12	SIDE(122.0)
X- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- X	2	12	SIDE(183.1)
V- S	2	12	SIDE(183.1)
R- O	2	12	SIDE(183.1)
N- M	2	12	TOP
S- F	1	12	TOP
J- N	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.25
C	TTWW+m	MT20	8.0	9.0	3.50	2.25
D	TMWW+t	MT20	4.0	6.0		
E	TMWW+t	MT20	5.0	8.0	3.00	1.75
F	TMV+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	4265	0	4265	0	0	5-8	5-8
M	4459	0	4459	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	3394	1976 / 0	628 / 0	0 / 0	0 / 0	790 / 0	0 / 0
M	3547	2069 / 0	657 / 0	0 / 0	0 / 0	821 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.94 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 52	-112.4	-112.4	0.05 (1)	10.00
B- C	-4274 / 0	-112.4	-112.4	0.07 (1)	5.49
C- Y	-7722 / 0	-112.4	-112.4	0.36 (1)	4.05
Y- Z	-7722 / 0	-112.4	-112.4	0.36 (1)	4.05
Z- D	-7722 / 0	-112.4	-112.4	0.36 (1)	4.05
D-AA	-9721 / 0	-112.4	-112.4	0.43 (1)	3.60
AA-AB	-9721 / 0	-112.4	-112.4	0.43 (1)	3.60
AB-AC	-9721 / 0	-112.4	-112.4	0.43 (1)	3.60
AC- E	-9721 / 0	-112.4	-112.4	0.43 (1)	3.60
E- F	-13202 / 0	-112.4	-112.4	0.36 (1)	3.19
F-AD	-13331 / 0	-112.4	-112.4	0.64 (1)	2.94
AD- G	-13331 / 0	-112.4	-112.4	0.64 (1)	2.94
G-AE	-13331 / 0	-112.4	-112.4	0.64 (1)	2.94
AE-AF	-13331 / 0	-112.4	-112.4	0.64 (1)	2.94
AF- H	-13331 / 0	-112.4	-112.4	0.64 (1)	2.94
H-AG	-11591 / 0	-112.4	-112.4	0.59 (1)	3.18
AG-AH	-11591 / 0	-112.4	-112.4	0.59 (1)	3.18
AH-AI	-11591 / 0	-112.4	-112.4	0.59 (1)	3.18
AI- I	-11591 / 0	-112.4	-112.4	0.59 (1)	3.18
I- J	-6037 / 0	-112.4	-112.4	0.24 (1)	4.59
J- K	-6589 / 0	-112.4	-112.4	0.24 (1)	4.42
K- L	0 / 52	-112.4	-112.4	0.05 (1)	10.00
X- B	-4258 / 0	0.0	0.0	0.15 (1)	6.94
M- K	-4310 / 0	0.0	0.0	0.16 (1)	6.90
X-AJ	0 / 0	-28.0	-28.0	0.09 (2)	10.00
AJ- W	0 / 0	-28.0	-28.0	0.09 (2)	10.00
W-AK	0 / 3230	-28.0	-28.0	0.28 (1)	10.00
AK-AL	0 / 3230	-28.0	-28.0	0.28 (1)	10.00
AL- V	0 / 3230	-28.0	-28.0	0.28 (1)	10.00
V- U	0 / 3230	-28.0	-28.0	0.28 (1)	10.00
U-AM	0 / 7723	-28.0	-28.0	0.62 (1)	10.00
AM-AN	0 / 7723	-28.0	-28.0	0.62 (1)	10.00
AN-AO	0 / 7723	-28.0	-28.0	0.62 (1)	10.00
AO- T	0 / 7723	-28.0	-28.0	0.62 (1)	10.00
T- S	0 / 342	-28.0	-28.0	0.14 (2)	10.00
S- R	-253 / 0	0.0	0.0	0.26 (1)	7.81
R- F	-496 / 0	0.0	0.0	0.26 (1)	7.81
R-AP	0 / 11591	-28.0	-28.0	0.94 (1)	10.00
AP-AQ	0 / 11591	-28.0	-28.0	0.94 (1)	10.00
AQ-AR	0 / 11591	-28.0	-28.0	0.94 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- C	-844 / 0	0.07 (1)
T- E	-8536 / 0	0.73 (1)
T- R	0 / 12239	0.50 (1)
E- R	0 / 8470	0.55 (1)
R- H	0 / 1868	0.17 (1)
Q- H	-2050 / 0	0.13 (1)
Q- I	0 / 7295	0.64 (1)
P- I	0 / 358	0.03 (3)
P- J	-345 / 0	0.02 (1)
B- W	0 / 3468	0.31 (1)
O- M	-200 / 0	0.01 (1)
O- K	0 / 4868	0.60 (1)
U- D	-2450 / 0	0.21 (1)
C- U	0 / 5208	0.46 (1)
D- T	0 / 2337	0.21 (1)

LICENSED PROFESSIONAL ENGINEER
4/07
S. KATSOULADOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 972 (0.41")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 592 (0.67")

CSI: TC=0.64/1.00 (F-H:1) , BC=0.94/1.00 (Q-R:1) , WB=0.73/1.00 (E-T:1) , SSI=0.21/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 18456-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287694	T102S2	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TTWW-m	MT20	8.0	12.0	Edge	4.75
J	TMVW+p	MT20	4.0	6.0		
K	TMVW-t	MT20	5.0	10.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BMV+p	MT20	3.0	6.0		
O	BVMWW-w	MT20	7.0	12.0	2.75	7.50
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	3.25
R	BVMWWW-l	MT20	10.0	12.0	5.00	3.75
S	BMV+p	MT20	3.0	6.0		
T	BMWWW-t	MT20	7.0	12.0	3.25	3.00
U	BMWW-t	MT20	5.0	6.0	2.50	2.25
V	BS-t	MT20	5.0	6.0		
W	BMWW-t	MT20	5.0	6.0		
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 142.0 lbs FACTORED DOWN AT 2-11-8, 135.3 lbs FACTORED DOWN AT 5-0-12, 135.3 lbs FACTORED DOWN AT 7-0-12, 135.3 lbs FACTORED DOWN AT 9-0-12, 135.3 lbs FACTORED DOWN AT 11-0-12, 135.3 lbs FACTORED DOWN AT 13-0-12, 135.3 lbs FACTORED DOWN AT 14-6-12, 135.3 lbs FACTORED DOWN AT 15-3-12, 135.5 lbs FACTORED DOWN AT 17-3-12, 135.5 lbs FACTORED DOWN AT 19-3-12, 135.5 lbs FACTORED DOWN AT 21-3-12, 135.5 lbs FACTORED DOWN AT 23-3-12, 135.5 lbs FACTORED DOWN AT 25-3-12, 135.5 lbs FACTORED DOWN AT 27-3-12, 155.3 lbs FACTORED DOWN AT 29-3-12, AND 238.0 lbs FACTORED DOWN AT 2-11-8, AND 238.0 lbs FACTORED DOWN AT 29-11-8 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-12, 69.9 lbs FACTORED DOWN AT 3-0-12, 69.9 lbs FACTORED DOWN AT 5-0-12, 69.9 lbs FACTORED DOWN AT 7-0-12, 69.9 lbs FACTORED DOWN AT 9-0-12, 69.9 lbs FACTORED DOWN AT 11-0-12, 69.9 lbs FACTORED DOWN AT 13-0-12, 69.9 lbs FACTORED DOWN AT 14-6-12, 69.9 lbs FACTORED DOWN AT 15-3-12, 119.0 lbs FACTORED DOWN AT 17-3-12, 119.0 lbs FACTORED DOWN AT 19-3-12, 119.0 lbs FACTORED DOWN AT 21-3-12, 119.0 lbs FACTORED DOWN AT 23-3-12, 119.0 lbs FACTORED DOWN AT 25-3-12, 119.0 lbs FACTORED DOWN AT 27-3-12, AND 119.0 lbs FACTORED DOWN AT 29-3-12, AND 91.4 lbs FACTORED DOWN AT 31-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AR- Q	0 / 11591	-28.0	-28.0	0.94 (1)	10.00
Q-AS	0 / 4769	-28.0	-28.0	0.49 (1)	10.00
AS-AT	0 / 4769	-28.0	-28.0	0.49 (1)	10.00
AT-AU	0 / 4769	-28.0	-28.0	0.49 (1)	10.00
AU- P	0 / 4769	-28.0	-28.0	0.49 (1)	10.00
P- O	0 / 5032	-28.0	-28.0	0.51 (1)	10.00
N- O	0 / 36	0.0	0.0	0.14 (1)	10.00
O- J	0 / 798	0.0	0.0	0.21 (1)	10.00
N- M	0 / 154	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	2-11-8	-142	-142	—	BACK	VERT	TOTAL
C	2-11-8	-33	-36	—	FRONT	VERT	DEAD
C	2-11-8	-206	-206	—	FRONT	VERT	SNOW
D	9-0-12	-135	-135	—	BACK	VERT	TOTAL
E	15-3-12	-135	-135	—	BACK	VERT	TOTAL
H	23-3-12	-135	-135	—	BACK	VERT	TOTAL
I	29-11-8	-33	-36	—	FRONT	VERT	DEAD
I	29-11-8	-206	-206	—	FRONT	VERT	SNOW
O	31-5-4	-52	-91	—	BACK	VERT	TOTAL
Q	23-3-12	-119	-119	—	BACK	VERT	TOTAL
T	15-3-12	-40	-70	—	BACK	VERT	TOTAL
U	9-0-12	-40	-70	—	BACK	VERT	TOTAL
W	3-0-12	-40	-70	—	BACK	VERT	TOTAL
Y	5-0-12	-135	-135	—	BACK	VERT	TOTAL
Z	7-0-12	-135	-135	—	BACK	VERT	TOTAL
AA	11-0-12	-135	-135	—	BACK	VERT	TOTAL
AB	13-0-12	-135	-135	—	BACK	VERT	TOTAL
AC	14-6-12	-135	-135	—	BACK	VERT	TOTAL
AD	17-3-12	-135	-135	—	BACK	VERT	TOTAL
AE	19-3-12	-135	-135	—	BACK	VERT	TOTAL
AF	21-3-12	-135	-135	—	BACK	VERT	TOTAL
AG	25-3-12	-135	-135	—	BACK	VERT	TOTAL
AH	27-3-12	-135	-135	—	BACK	VERT	TOTAL
AI	29-3-12	-155	-155	—	BACK	VERT	TOTAL
AJ	1-0-12	-40	-70	—	BACK	VERT	TOTAL
AK	5-0-12	-40	-70	—	BACK	VERT	TOTAL
AL	7-0-12	-40	-70	—	BACK	VERT	TOTAL
AM	11-0-12	-40	-70	—	BACK	VERT	TOTAL
AN	13-0-12	-40	-70	—	BACK	VERT	TOTAL
AO	14-6-12	-40	-70	—	BACK	VERT	TOTAL
AP	17-3-12	-119	-119	—	BACK	VERT	TOTAL
AQ	19-3-12	-119	-119	—	BACK	VERT	TOTAL
AR	21-3-12	-119	-119	—	BACK	VERT	TOTAL
AS	25-3-12	-119	-119	—	BACK	VERT	TOTAL
AT	27-3-12	-119	-119	—	BACK	VERT	TOTAL
AU	29-3-12	-119	-119	—	BACK	VERT	TOTAL

JSI GRIP= 0.89 (Q) (INPUT = 0.90)
JSI METAL= 0.64 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
14107B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 1B45B-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287696	T103	1	2	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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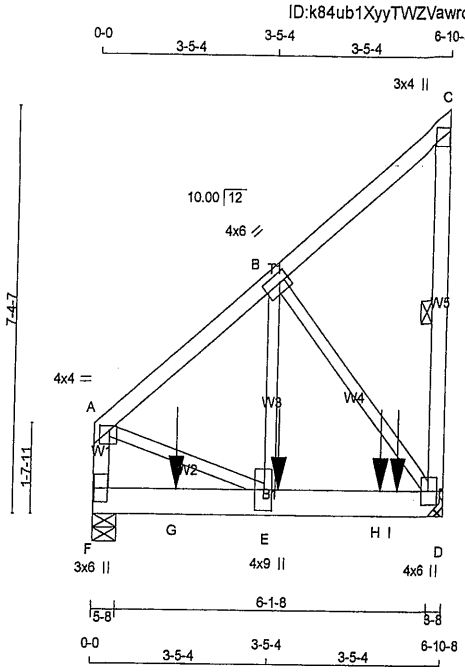

TOTAL WEIGHT = 2 X 41 = 82 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
F- A	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	6	SIDE(117.2)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.25
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	9.0	4.50	1.75
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1151.0 lbs FACTORED DOWN AT 1-7-4, 1151.0 lbs FACTORED DOWN AT 3-7-4, AND 1151.0 lbs FACTORED DOWN AT 5-7-4, AND 1452.1 lbs FACTORED DOWN AT 5-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	3519	0	3519	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
F	2351	0	2351	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2768	1665 / 0	475 / 0	0 / 0	0 / 0	627 / 0	0 / 0
F	1853	1109 / 0	323 / 0	0 / 0	0 / 0	421 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1910 / 0	-112.4	-112.4	0.13 (1)	6.19	
B- C	-27 / 0	-112.4	-112.4	0.11 (1)	6.25	
D- C	-151 / 0	0.0	0.0	0.02 (1)	6.25	
F- A	-1920 / 0	0.0	0.0	0.11 (1)	7.81	
F- G	0 / 0	-28.0	-28.0	0.29 (1)	10.00	
G- E	0 / 0	-28.0	-28.0	0.29 (1)	10.00	
E- H	0 / 1488	-28.0	-28.0	0.66 (1)	10.00	
H- I	0 / 1488	-28.0	-28.0	0.66 (1)	10.00	
I- D	0 / 1488	-28.0	-28.0	0.66 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	0 / 2538	0.31 (1)
B- D	-2311 / 0	0.52 (1)
A- E	0 / 1579	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	3-7-4	-1151	-1151	—	FRONT	VERT	TOTAL
G	1-7-4	-1151	-1151	—	FRONT	VERT	TOTAL
H	5-7-4	-1151	-1151	—	FRONT	VERT	TOTAL
I	5-11-4	-1452	-1452	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
4/10/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.23")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.23")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.66/1.00 (D-E:1) , WB=0.52/1.00 (B-D:1) , SSI=0.53/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.28 (E) (INPUT = 1.00)

DWG NO. TAM 18453-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	PB1	3	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

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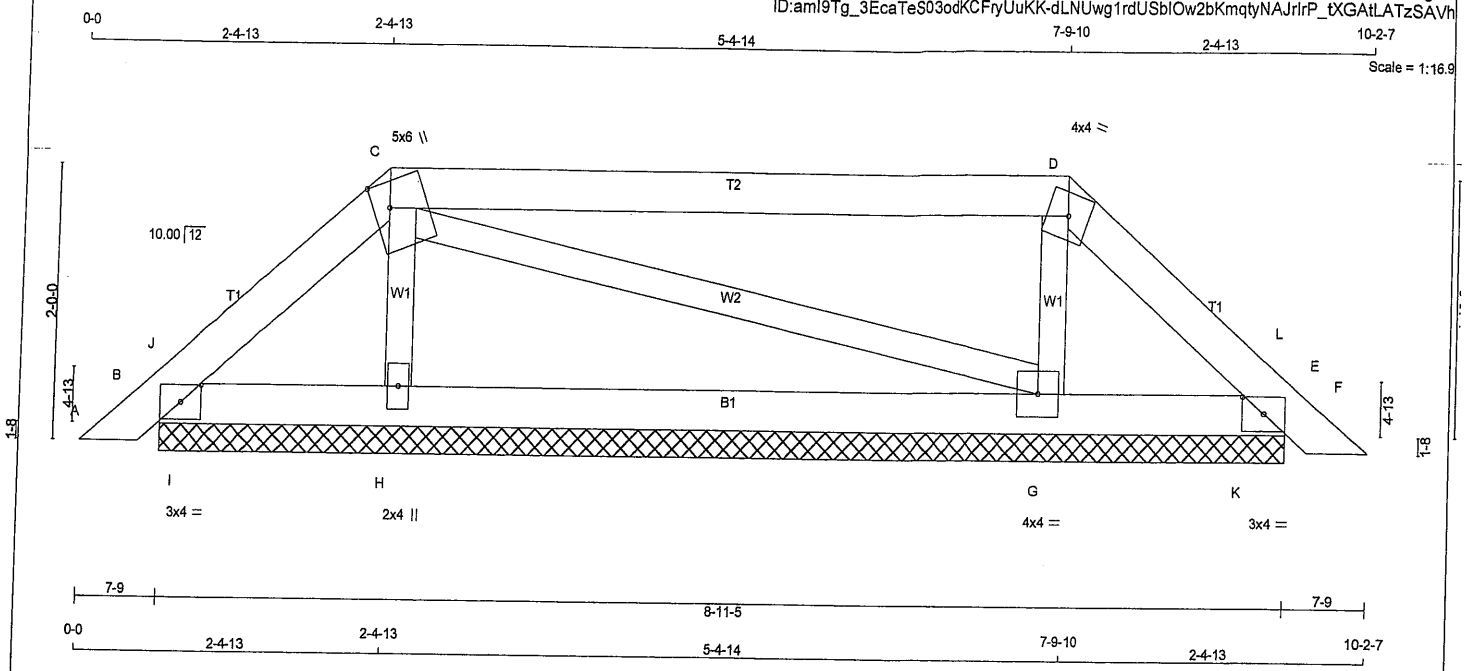

TOTAL WEIGHT = 3 X 30 = 89 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	223	0	223	0	0	8-11-5	8-11-5
E	212	0	212	0	0	8-11-5	8-11-5
H	461	0	461	0	0	8-11-5	8-11-5
G	480	0	480	0	0	8-11-5	8-11-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	150	132 / 0	0 / -4	0 / 0	0 / 0	22 / 0	0 / 0
E	142	126 / 0	0 / -4	0 / 0	0 / 0	20 / 0	0 / 0
H	389	191 / 0	98 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	403	202 / 0	98 / 0	0 / 0	0 / 0	103 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E. H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- J	-60 / 35	-112.4	-112.4	0.03 (3)	6.25
J- C	-106 / 0	-112.4	-112.4	0.03 (1)	6.25
C- D	-41 / 0	-112.4	-112.4	0.56 (1)	6.25
D- L	-89 / 0	-112.4	-112.4	0.03 (1)	6.25
L- E	-42 / 37	-112.4	-112.4	0.03 (3)	6.25
E- F	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- I	0 / 75	-28.0	-28.0	0.06 (1)	10.00
I- H	0 / 75	-28.0	-28.0	0.14 (3)	10.00
H- G	0 / 55	-28.0	-28.0	0.14 (3)	10.00
G- K	0 / 63	-28.0	-28.0	0.14 (3)	10.00
K- E	0 / 63	-28.0	-28.0	0.05 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-311 / 0	0.05 (1)
C- G	-14 / 0	0.01 (1)
G- D	-326 / 0	0.05 (1)
I- J	-162 / 0	0.00 (1)
K- L	-164 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.56/1.00 (C-D:1) , BC=0.14/1.00 (H-I:3) , WB=0.05/1.00 (D-G:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (D) (INPUT = 0.90)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM / 8447-18
STRUCTURAL
COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	PB2	2	1	OBC2012_6TCDL	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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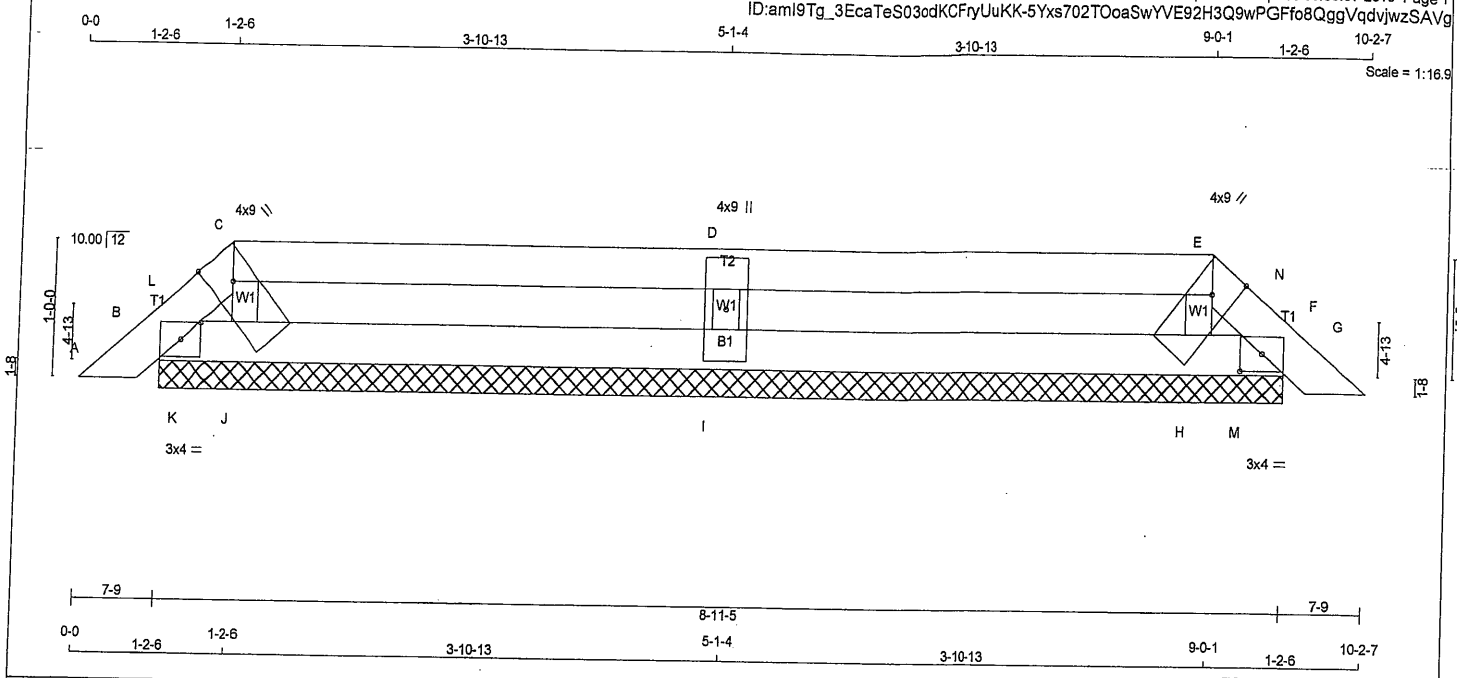

TOTAL WEIGHT = 2 X 23 = 47 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C, D, E, H, I, J						
C						
C	TTBW1*+h	MT20	4.0	9.0	Edge	
D	TMBMW1+I	MT20	4.0	9.0		
E	TTBW1*+h	MT20	4.0	9.0	Edge	
F	TMB1-I	MT20	3.0	4.0	1.50	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	115	0	115	0	-39	8-11-5	8-11-5
F	115	0	115	0	-39	8-11-5	8-11-5
J	244	0	244	0	0	8-11-5	8-11-5
I	659	0	659	0	0	8-11-5	8-11-5
H	244	0	244	0	0	8-11-5	8-11-5

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT F FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	60	86 / 0	0 / -25	0 / 0	0 / 0	0 / -1	0 / 0
F	60	86 / 0	0 / -25	0 / 0	0 / 0	0 / -1	0 / 0
J	225	81 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0
I	513	317 / 0	83 / 0	0 / 0	0 / 0	114 / 0	0 / 0
H	225	81 / 0	78 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- L	-48 / 32	-112.4	-112.4	0.02 (1)	6.25
L- C	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
C- D	-1 / 1	-112.4	-112.4	0.29 (1)	10.00
D- E	-1 / 1	-112.4	-112.4	0.29 (1)	10.00
E- N	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
N- F	-48 / 32	-112.4	-112.4	0.02 (1)	6.25
F- G	0 / 18	-112.4	-112.4	0.02 (1)	10.00
B- K	-3 / 24	-28.0	-28.0	0.01 (1)	10.00
K- J	-3 / 24	-28.0	-28.0	0.07 (3)	10.00
J- I	-1 / 1	-28.0	-28.0	0.09 (3)	10.00
I- H	-1 / 1	-28.0	-28.0	0.09 (3)	10.00
H- M	-3 / 24	-28.0	-28.0	0.07 (3)	10.00
M- F	-3 / 24	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-125 / 8	0.02 (1)
I- D	-548 / 0	0.08 (1)
H- E	-125 / 8	0.02 (1)
K- L	-79 / 0	0.00 (1)
M- N	-79 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
4016
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (C-D:1) , BC=0.09/1.00 (H-I:3) , WB=0.08/1.00 (D-I:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.16 (E) (INPUT = 0.90)
USI METAL= 0.05 (D) (INPUT = 1.00)

DWG NO. TAM 18445 -18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	PB31	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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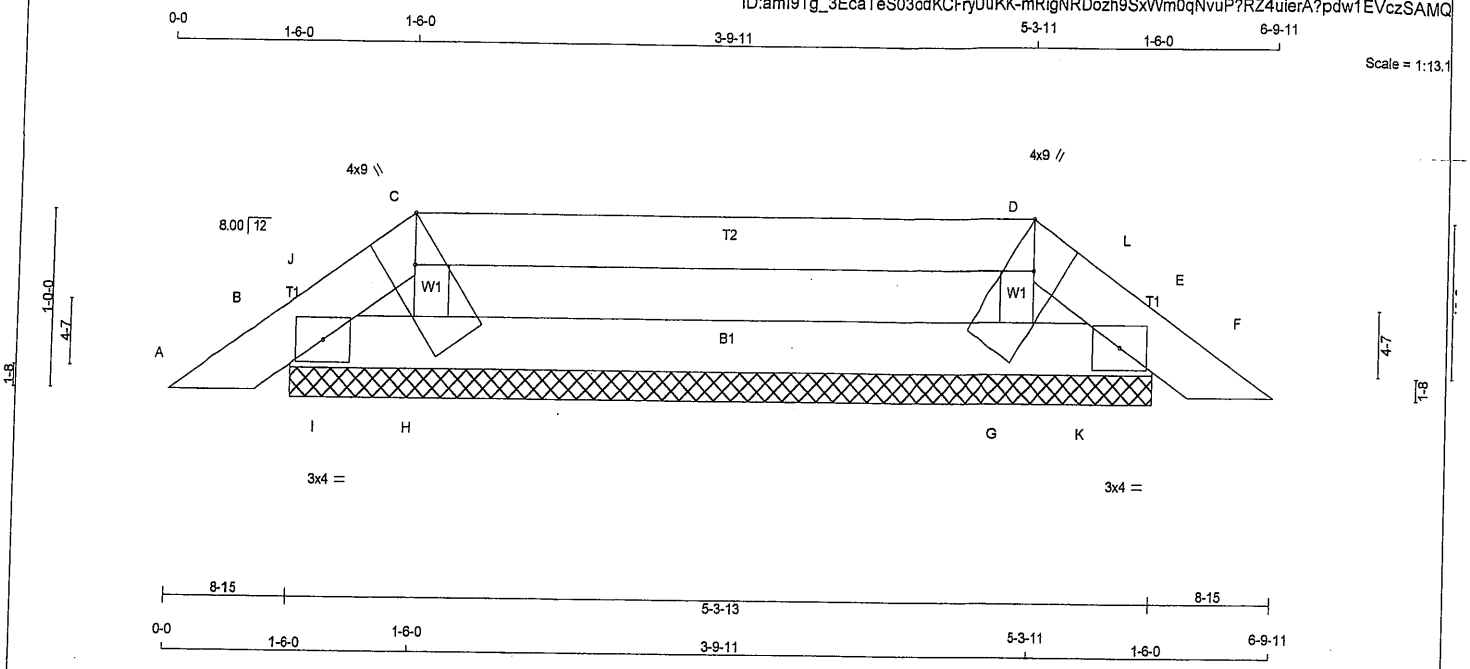

TOTAL WEIGHT = 2 X 15 = 30 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C, D, G, H						
C						
C	TTBW1*+h	MT20	4.0	9.0	Edge	2.00
D	TTBW1*+h	MT20	4.0	9.0	3.00	Edge
E	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	157	0	157	0	-12	5-3-13	5-3-13
E	157	0	157	0	-12	5-3-13	5-3-13
H	291	0	291	0	0	5-3-13	5-3-13
G	291	0	291	0	0	5-3-13	5-3-13

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	96	103 / 0	0 / -15	0 / 0	0 / 0	9 / 0	0 / 0
E	96	103 / 0	0 / -15	0 / 0	0 / 0	9 / 0	0 / 0
H	253	113 / 0	71 / 0	0 / 0	0 / 0	68 / 0	0 / 0
G	253	113 / 0	71 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- J	-68 / 19	-112.4	-112.4	0.03 (1)	6.25
J- C	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
C- D	0 / 2	-112.4	-112.4	0.28 (1)	10.00
D- L	-58 / 0	-112.4	-112.4	0.02 (2)	6.25
L- E	-68 / 19	-112.4	-112.4	0.03 (1)	6.25
E- F	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- I	0 / 34	-28.0	-28.0	0.01 (1)	10.00
I- H	0 / 34	-28.0	-28.0	0.07 (3)	10.00
H- G	-2 / 0	-28.0	-28.0	0.07 (3)	10.00
G- K	0 / 34	-28.0	-28.0	0.07 (3)	10.00
K- E	0 / 34	-28.0	-28.0	0.01 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-189 / 0	0.03 (1)
G- D	-189 / 0	0.03 (1)
I- J	-60 / 0	0.00 (1)
K- L	-60 / 0	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
4/09/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1) , BC=0.07/1.00 (G-H:3) , WB=0.03/1.00 (D-G:1) , SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.05 (C) (INPUT = 1.00)

DWG NO. TAM 18492-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	PB32	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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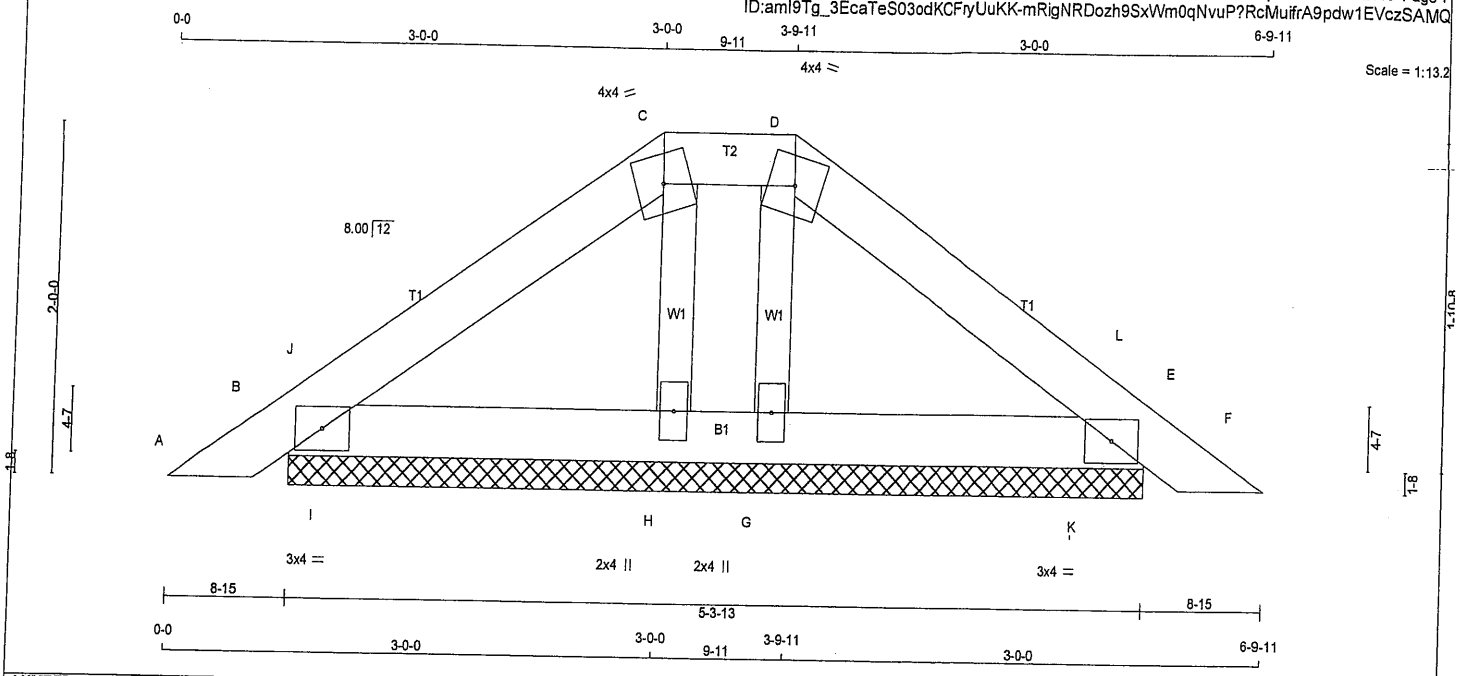

TOTAL WEIGHT = 2 X 17 = 35 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	260	0	260	0	0	5-3-13	5-3-13
E	260	0	260	0	0	5-3-13	5-3-13
H	188	0	188	0	0	5-3-13	5-3-13
G	188	0	188	0	0	5-3-13	5-3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	196	132 / 0	24 / 0	0 / 0	0 / 0	40 / 0	0 / 0
E	196	132 / 0	24 / 0	0 / 0	0 / 0	40 / 0	0 / 0
H	153	84 / 0	32 / 0	0 / 0	0 / 0	37 / 0	0 / 0
G	153	84 / 0	32 / 0	0 / 0	0 / 0	37 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- J	-54 / 0	-112.4	-112.4	0.01 (3)	6.25
J- C	-67 / 0	-112.4	-112.4	0.07 (1)	6.25
C- D	-46 / 0	-112.4	-112.4	0.01 (1)	6.25
D- L	-67 / 0	-112.4	-112.4	0.07 (1)	6.25
L- E	-54 / 0	-112.4	-112.4	0.01 (3)	6.25
E- F	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- I	0 / 54	-28.0	-28.0	0.07 (1)	10.00
I- H	0 / 54	-28.0	-28.0	0.07 (1)	10.00
H- G	0 / 46	-28.0	-28.0	0.05 (1)	10.00
G- K	0 / 54	-28.0	-28.0	0.07 (1)	10.00
K- E	0 / 54	-28.0	-28.0	0.07 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-115 / 0	0.02 (1)
G- D	-115 / 0	0.02 (1)
I- J	-148 / 23	0.00 (1)
K- L	-148 / 23	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (D-L:1) , BC=0.07/1.00 (G-K:1) , WB=0.02/1.00 (D-G:1) , SSI=0.12/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4108
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 10483.B
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	PB33	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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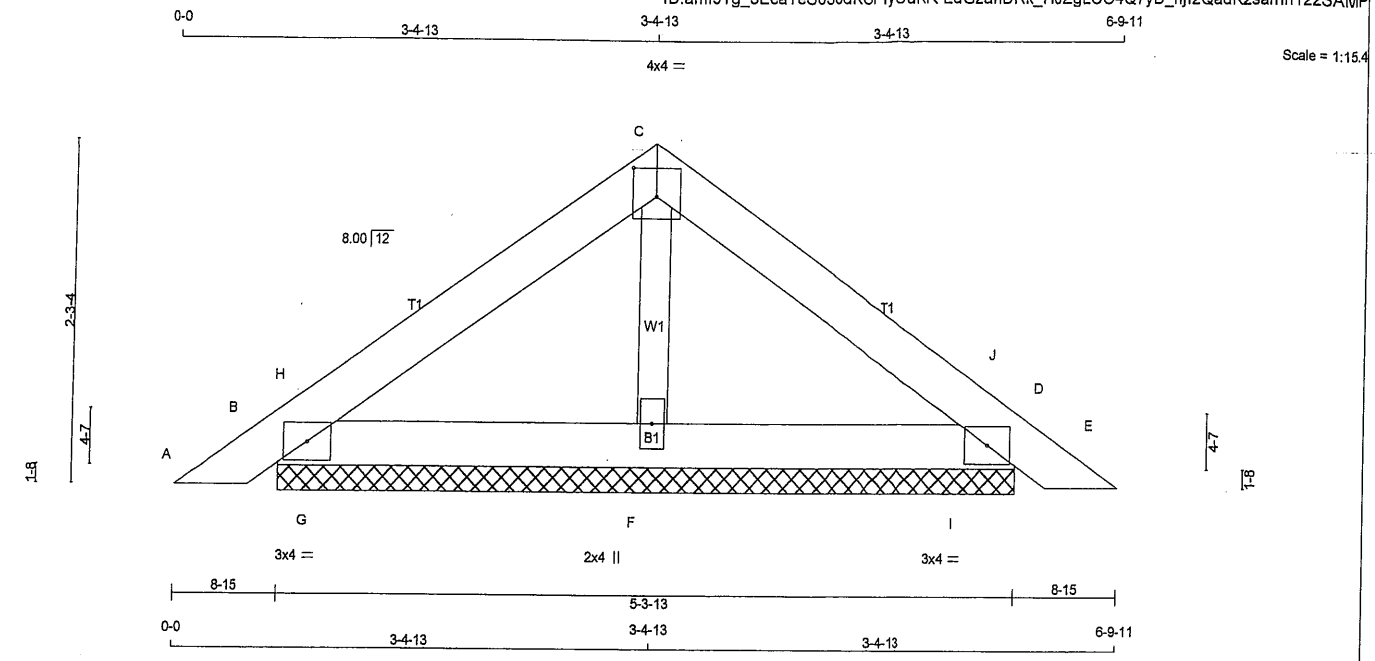

TOTAL WEIGHT = 3 X 17 = 50 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	292	0	292	0	0	5-3-13	5-3-13
D	292	0	292	0	0	5-3-13	5-3-13
F	313	0	313	0	0	5-3-13	5-3-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	218	149 / 0	25 / 0	0 / 0	0 / 0	44 / 0	0 / 0
D	218	149 / 0	25 / 0	0 / 0	0 / 0	44 / 0	0 / 0
F	261	133 / 0	62 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- H	-51 / 0	-112.4	-112.4	0.03 (1)	6.25
H- C	-97 / 0	-112.4	-112.4	0.09 (1)	6.25
C- J	-97 / 0	-112.4	-112.4	0.09 (1)	6.25
J- D	-51 / 0	-112.4	-112.4	0.03 (1)	6.25
D- E	0 / 19	-112.4	-112.4	0.04 (1)	10.00
B- G	0 / 76	-28.0	-28.0	0.10 (1)	10.00
G- F	0 / 76	-28.0	-28.0	0.10 (1)	10.00
F- I	0 / 76	-28.0	-28.0	0.10 (1)	10.00
I- D	0 / 76	-28.0	-28.0	0.10 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-144 / 0	0.02 (1)
G- H	-222 / 13	0.00 (1)
I- J	-222 / 13	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (C-J:1) , BC=0.10/1.00 (D-I:1) , WB=0.02/1.00 (C-F:1) , SSI=0.17/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4198
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18484-18
STRUCTURAL COMPONENT ONLY

Site Copy

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287689	J1S	7	1	OBC2012_6TCDL TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Tue Apr 10 09:33:06 2018 Page 1
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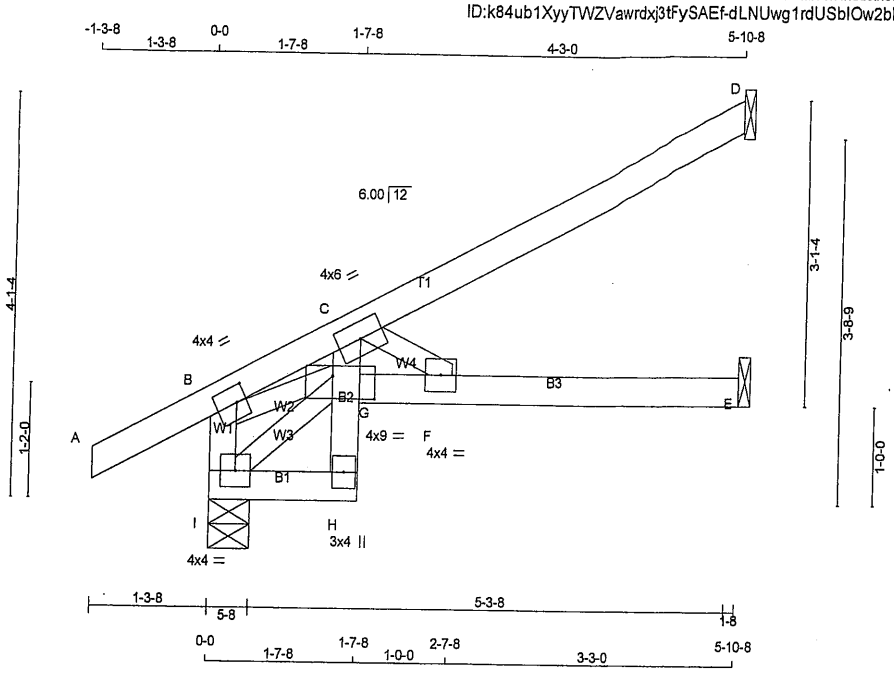

TOTAL WEIGHT = 7 X 21 = 149 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
C	TMVW-t	MT20	4.0	6.0		
F	BMW-t	MT20	4.0	4.0		
G	BVMWW-I	MT20	4.0	9.0	2.75	5.50
H	BMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	583	0	583	0	0	5-8	5-8
D	251	0	251	0	0	1-8	1-8
E	144	0	152	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	445	289 / 0	62 / 0	0 / 0	0 / 0	95 / 0	0 / 0
D	179	138 / 0	10 / 0	0 / 0	0 / 0	32 / 0	0 / 0
E	137	43 / 0	52 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
I- B	-513 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 34	-112.4	-112.4	0.15 (1)	10.00
B- C	-694 / 0	-112.4	-112.4	0.15 (1)	6.25
C- D	0 / 10	-112.4	-112.4	0.38 (1)	10.00
I- H	0 / 65	-28.0	-28.0	0.02 (2)	10.00
H- G	0 / 36	0.0	0.0	0.10 (1)	10.00
G- C	0 / 478	0.0	0.0	0.17 (1)	10.00
G- F	0 / 771	-28.0	-28.0	0.57 (1)	10.00
F- E	0 / 0	-28.0	-28.0	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- G	-81 / 0	0.01 (1)
B- G	0 / 596	0.13 (1)
C- F	-850 / 0	0.12 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 673 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 399 (0.18")

CSI: TC=0.38/1.00 (C-D:1) , BC=0.57/1.00 (F-G:1) , WB=0.13/1.00 (B-G:1) , SSI=0.26/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.30 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/09
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18443-18
STRUCTURAL COMPONENT ONLY

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. OBC2012_6TCDL	DRWG NO.
287659	J6S	8	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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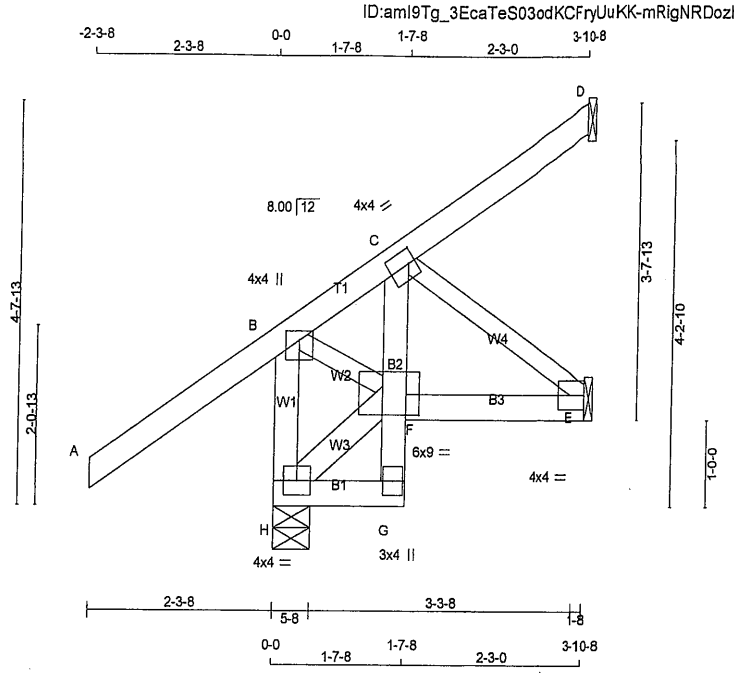

TOTAL WEIGHT = 8 X 23 = 181 lb
[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - C	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.00
E	BMW1-t	MT20	4.0	4.0	2.00	Edge
F	BVMWW-l	MT20	6.0	9.0	4.00	5.50
G	BMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	619	0	619	0	0	5-8	5-8
D	135	0	135	0	0	1-8	1-8
E	54	0	93	0	-73	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D , E

PROVIDE ANCHORAGE AT BEARING JOINT E FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	454	327 / 0	40 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	92	78 / 0	0 / 0	0 / 0	0 / 0	15 / 0	0 / 0
E	67	1 / -62	39 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
H- B	-593 / 0	0.0	0.0	0.07 (1)	7.81
A- B	0 / 74	-112.4	-112.4	0.44 (1)	10.00
B- C	-152 / 19	-112.4	-112.4	0.43 (5)	6.25
C- D	0 / 12	-112.4	-112.4	0.11 (1)	10.00
H- G	0 / 6	-28.0	-28.0	0.02 (3)	10.00
G- F	0 / 36	0.0	0.0	0.01 (1)	10.00
F- C	0 / 75	0.0	0.0	0.02 (2)	10.00
F- E	-130 / 54	-28.0	-28.0	0.06 (3)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- F	-8 / 0	0.00 (1)
B- F	-140 / 55	0.02 (5)
C- E	-67 / 162	0.04 (5)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	56.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.44/1.00 (A-B:1) , BC=0.06/1.00 (E-F:3) , WB=0.04/1.00 (C-E:5) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (B) (INPUT = 0.90)
JSI METAL= 0.08 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/07/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 18481 .18
STRUCTURAL
COMPONENT ONLY

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TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

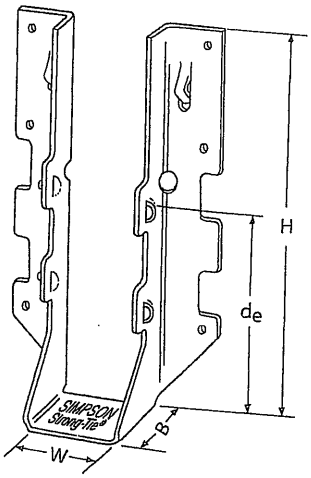
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

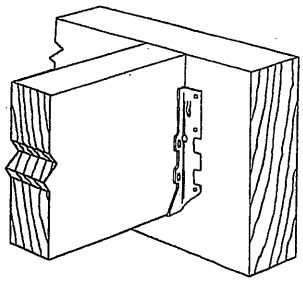
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



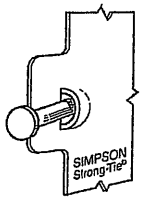
LUS28



Typical LUS Installation

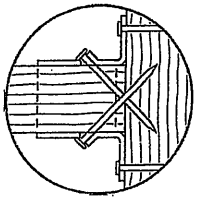
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

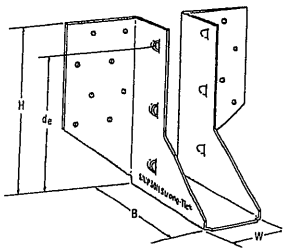
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

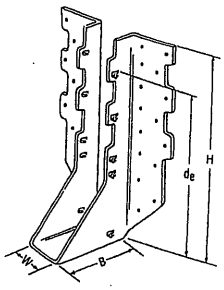
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

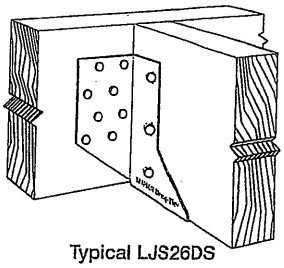
- See current catalogue for options



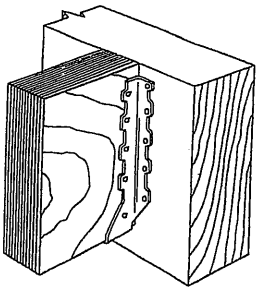
LJS26DS



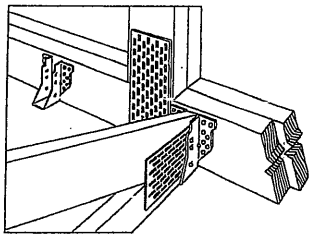
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



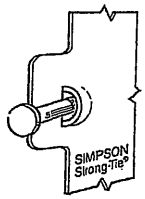
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

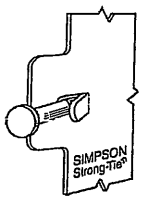
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

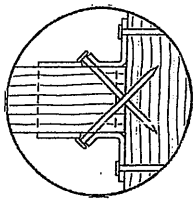


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

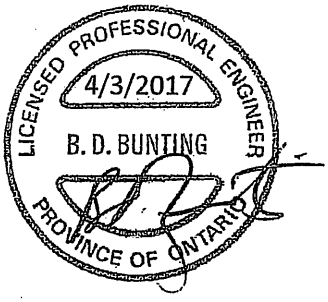
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically, and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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strongtie.com

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TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

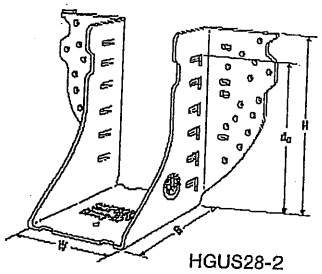
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

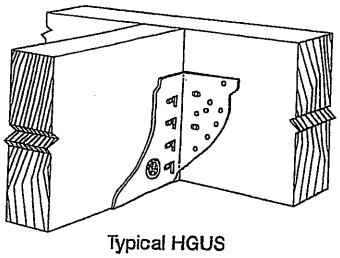
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

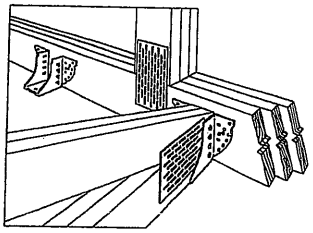
- See current catalogue for options



HGUS28-2



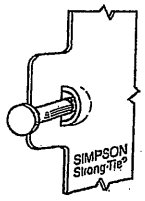
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

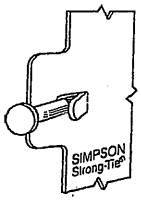
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

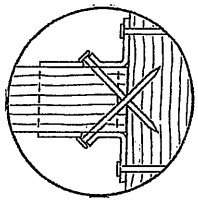


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS17 3/17 exp. 6/19

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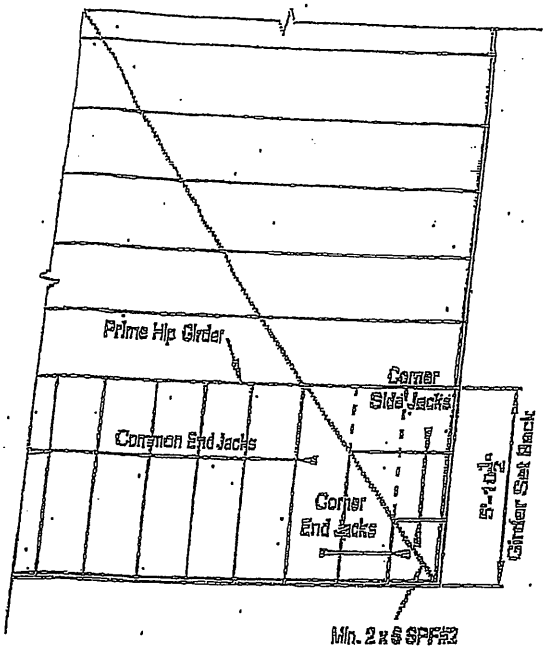
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	59.9 P.S.F.

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COMPONENT ONLY

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1-20-99
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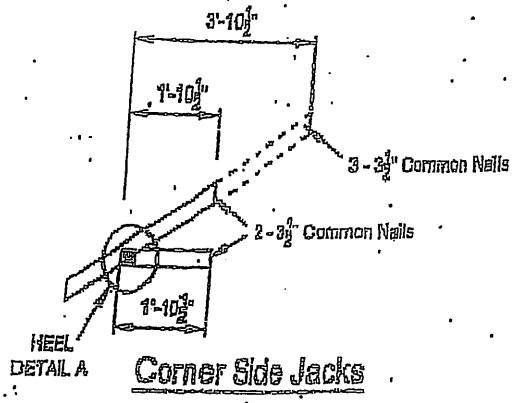
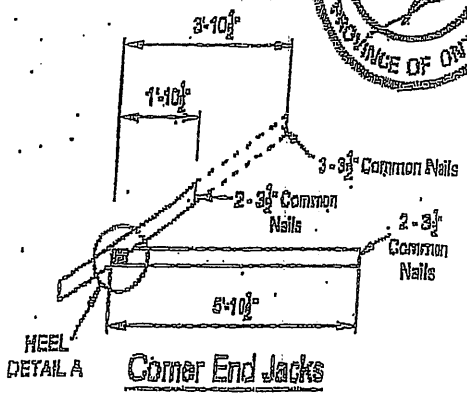
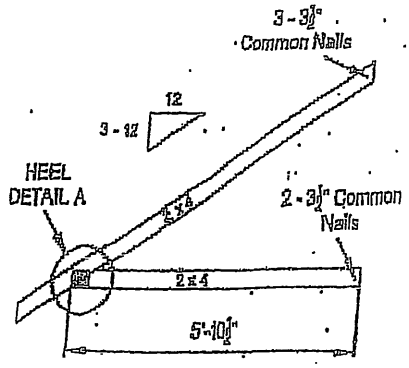


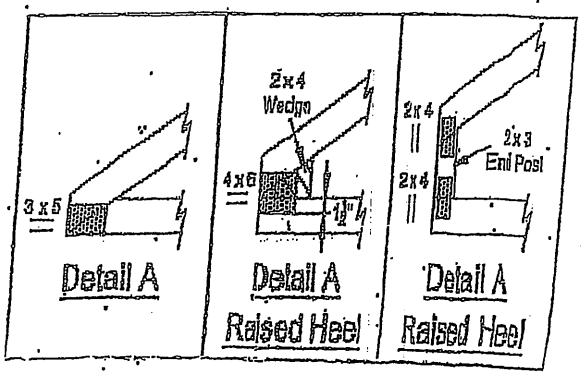
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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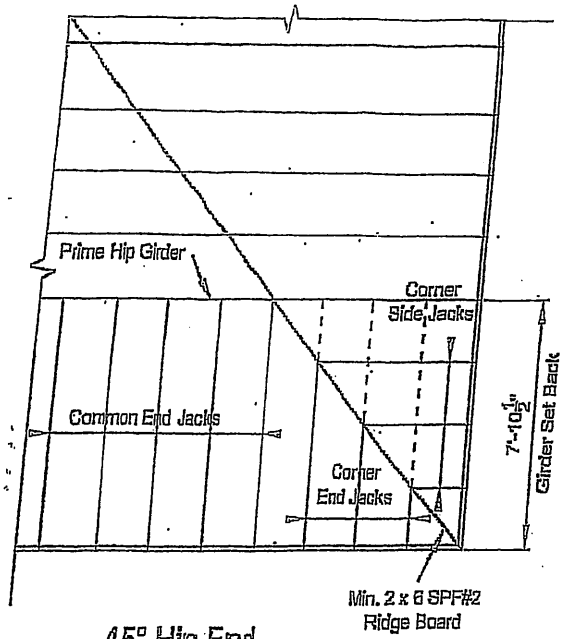
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2
BOTTOM CHORD : 2x4 SPF#2
WEBS : 2x3 SPF#2
UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOAD:

TOP CHORD LIVE LOAD	:	34.8 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	44.8 P.S.F.

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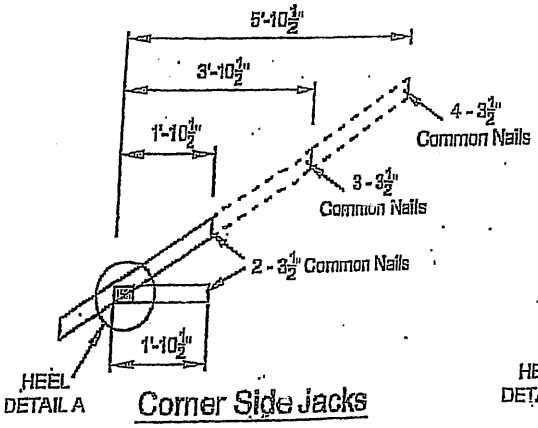
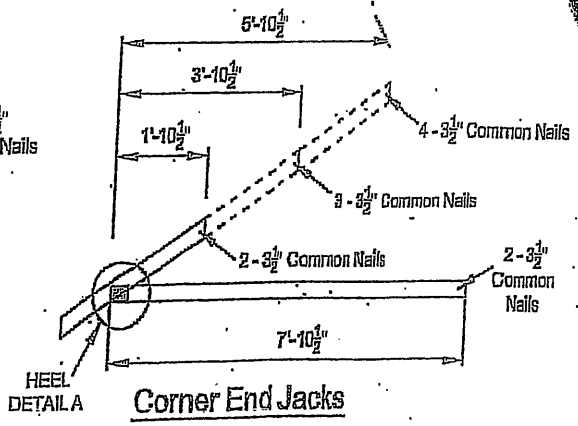
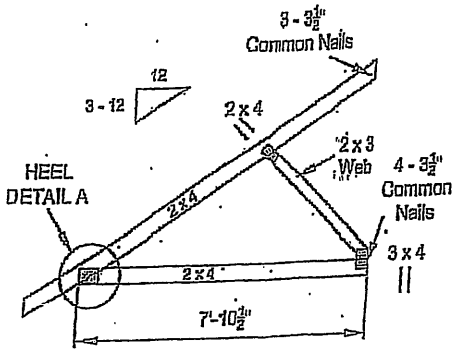


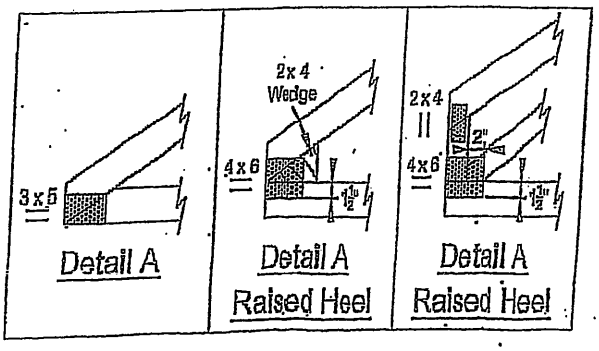
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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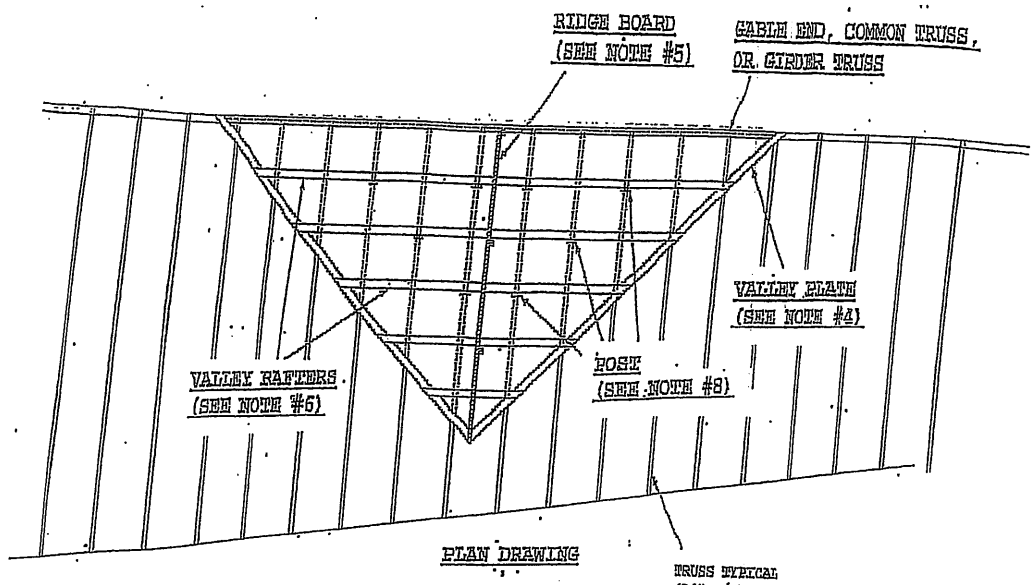
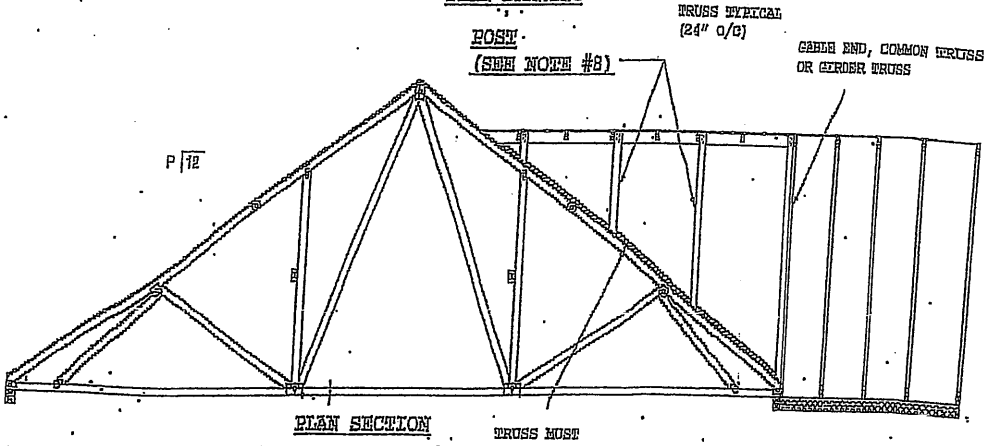
CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING, NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-14
S. KATSOULAKOS
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DWG NO TAM 6305.14
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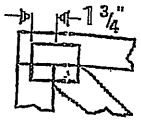
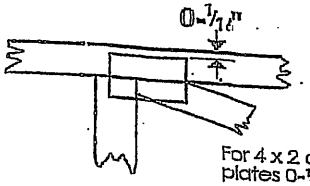
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

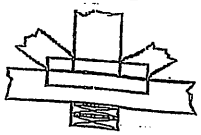
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

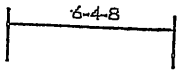
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

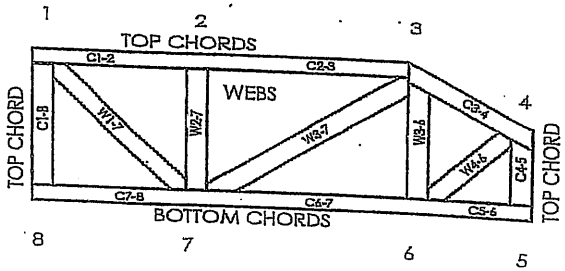
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-2079
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO T4M 5-1-92/14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering~~ Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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