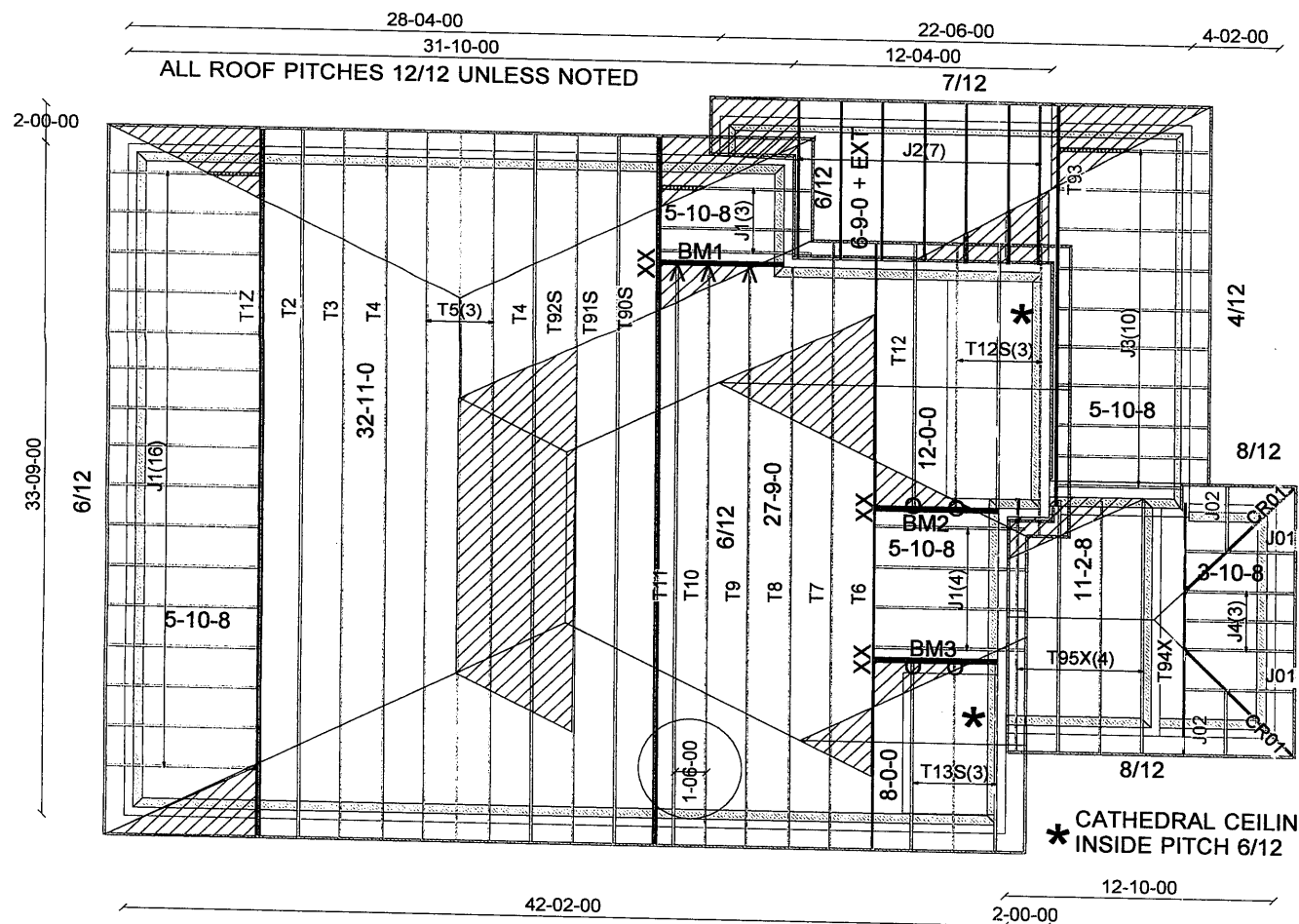




ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

311,575



Job Track: **44755**
Layout ID: **287697**
Plan Log: **94071**

Builder / Location:

BAYVIEW WELLINGTON HOMES / BRADFORD

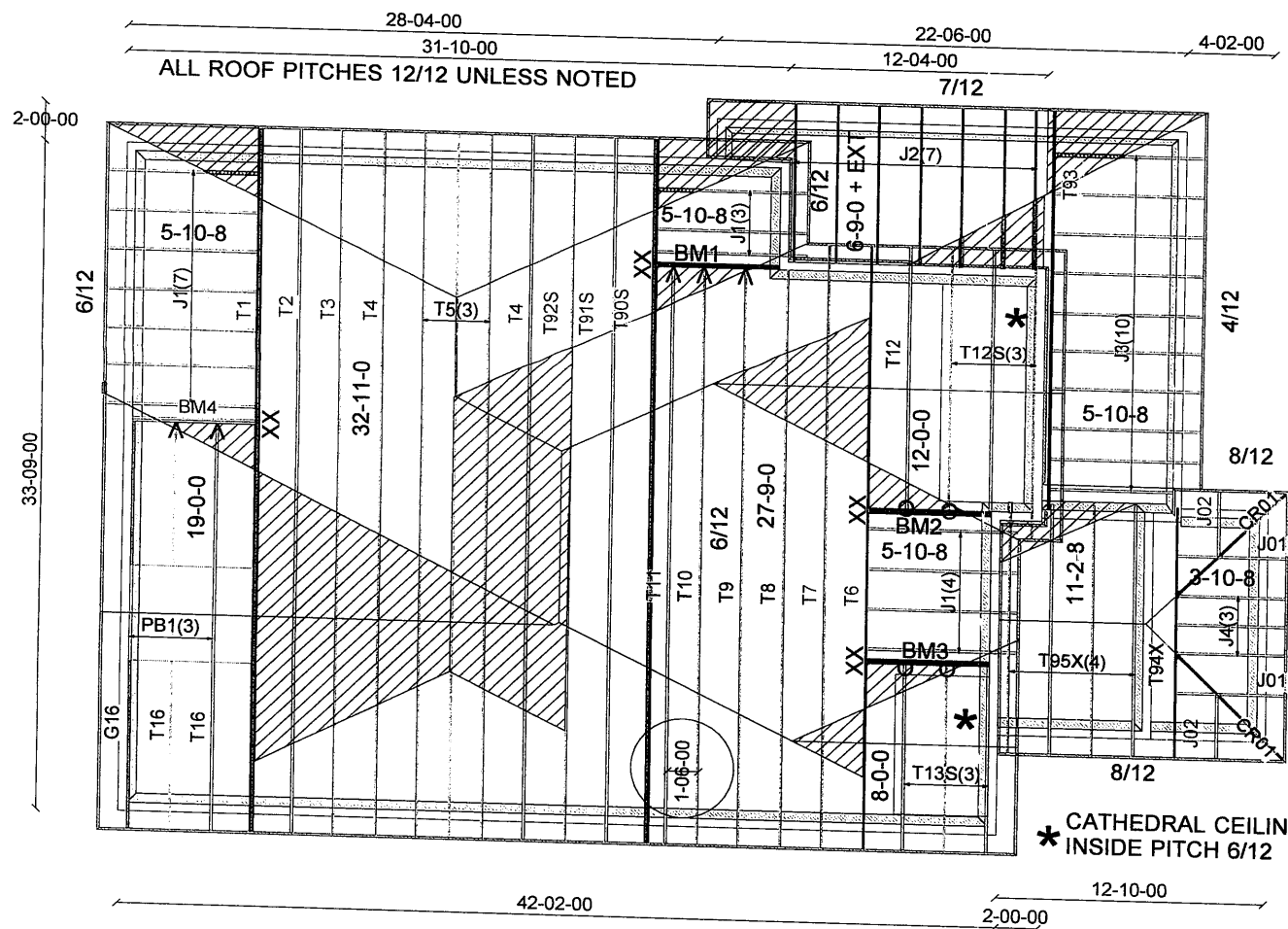
Project: **GREEN VALLEY ESTATES**

Date: **4/6/2018** Designer: **AC/S.V.E**

Model / Elevation:

S42-16 / A STD

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING



DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

T - 170639

BM1,BM2,BM3,BM4 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

* CATHEDRAL CEILING
INSIDE PITCH 6/12

m11,575



Job Track: **44755**
Layout ID: **287698**
Plan Log: **94071**

Builder / Location:

BAYVIEW WELLINGTON HOMES / BRADFORD

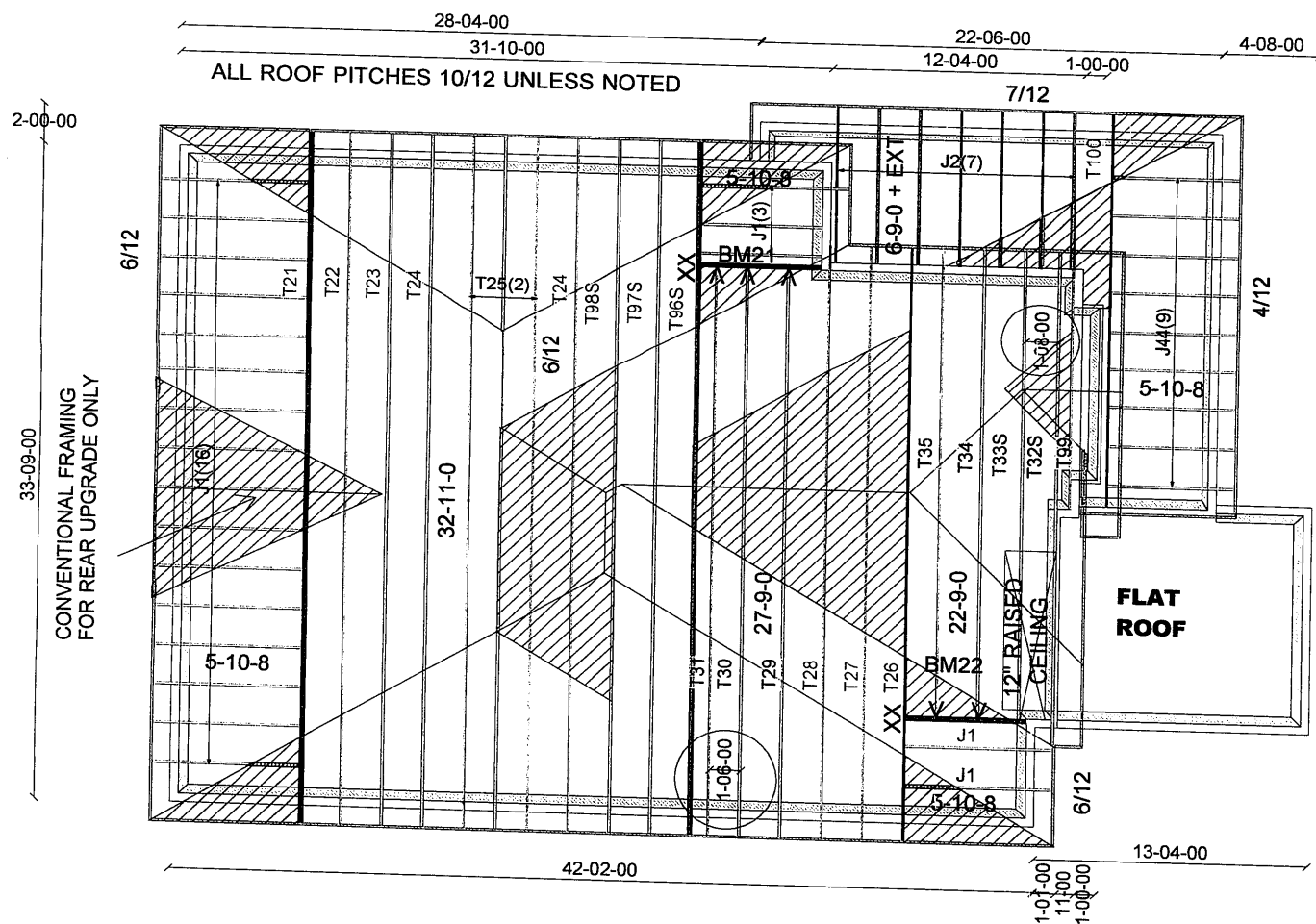
Project: **GREEN VALLEY ESTATES**

Date: **4/6/2018** Designer: **AC/S.V.E**

Model / Elevation:

S42-16 / A REAR UPGRADE

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)



DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

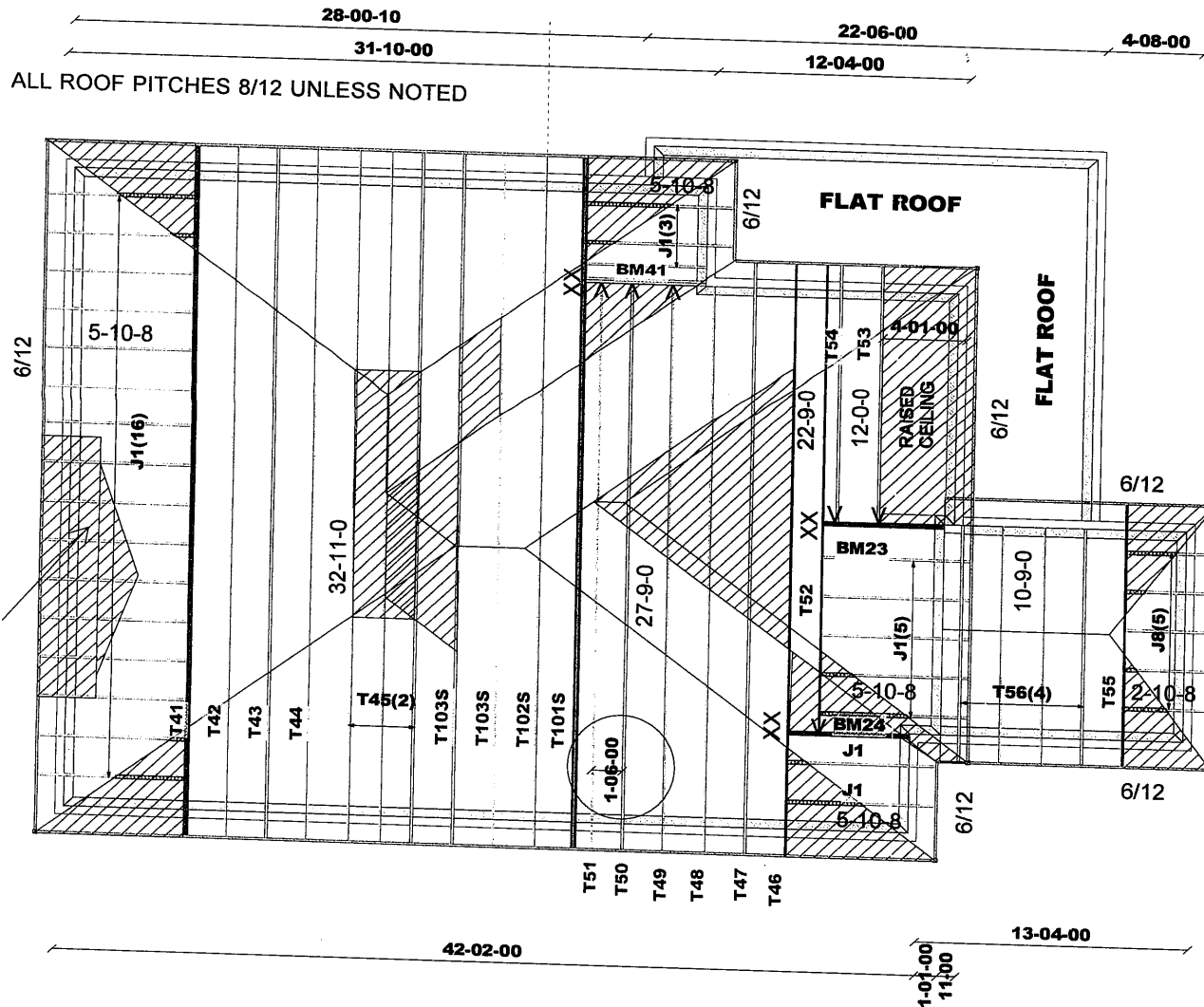
T - 170639

BM21,BM22 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

mil. 575

 TAMARACK LUMBER INC. ALFA LUMBER GROUP	Job Track: 44755	Builder / Location:		Model / Elevation:
	Layout ID: 287699	BAYVIEW WELLINGTON HOMES / BRADFORD		S42-16 / B STD OR REAR UPGRADE
	Plan Log: 94071	Project: GREEN VALLEY ESTATES	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
	Date: 4/6/2018	Designer: AC/S.V.E	Mitek ver 7.5.5	



ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

DESIGN CONFORMS
WITH O.B.C. 2012 PART 9
DESIGN LOADS:
SNOW LOAD 32.5 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

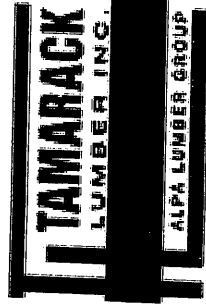
T - 170639

BM23, BM24 = 2-2X10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C. 2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4
SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

111,575

TAMARACK LUMBER INC. ALFA LUMBER GROUP	Job Track: 44755	Builder / Location:		Model / Elevation:
	Layout ID: 287661	BAYVIEW WELLINGTON HOMES / BRADFORD		S42-16 / C STD OR REAR UPGRADE
	Plan Log: 94071	Project: GREEN VALLEY EAST		
	Date: 4/6/2018	Designer: AC/S.V.E	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Mitek ver 7.5.0				



Delivery Shiplist

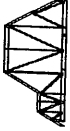
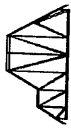
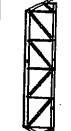
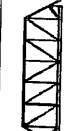
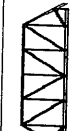
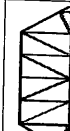
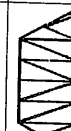
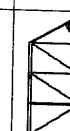
DATE	04/06/18
SALES REP	Mario

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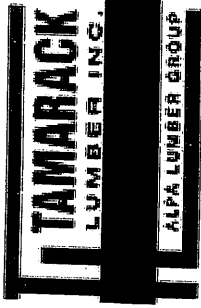
JOB TRACK: 44755	LAYOUT ID: 287697	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: A STD	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT						
	1 2 Ply	T90S ROOF	12.00 0.00	32-11-00	09-03-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		472.82 292.00		
	1	T91S ROOF	12.00 0.00	32-11-00	10-03-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		174.50 107.83		
	1	T92S ROOF	12.00 0.00	32-11-00	11-03-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		182.33 114.50		
	1 2 Ply	T1Z HIP GIRDER	12.00 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		382.00 241.34		
	1	T2 HIP	12.00 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		141.31 89.17		
	1	T3 HIP	12.00 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		142.77 92.00		
	2	T4 HIP	12.00 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		301.24 191.00		
	3	T5 HIP	12.00 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		498.15 312.00		
	1	T6 HIP GIRDER	12.00 0.00	27-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		169.33 105.34		
	1	T7 HIP	12.00 0.00	27-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		120.38 75.83		
	1	T8 HIP	12.00 0.00	27-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		120.47 77.17		
	1	T9 HIP	12.00 0.00	27-09-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		126.86 80.17		
	1	T10 HIP	12.00 0.00	27-09-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		140.80 88.17		
	1	T11 HIP	12.00 0.00	27-09-00	08-10-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		146.63 91.67		
	1	T12 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		63.57 41.33		
	3	T12S SCISSORS	12.00 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		186.90 118.50		
	3	T13S SCISSORS	12.00 6.00	08-00-00	05-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		127.02 81.99		
	1	T93 HALF HIP	7.00 0.00	19-00-00	02-03-07	2 X 6	2 X 6	01-03-08 00-00-00	00-08-03 02-03-07		96.08 59.33		

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Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK:44755	LAYOUT ID: 287697	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: A STD	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T94X HIP GIRDER	8.00 0.00	11-02-08	03-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-01-02	46.86 31.00		
	4	T95X COMMON	8.00 0.00	11-02-08	04-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-01-02	189.24 124.68		
	23	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	386.17 245.41		
	3	J4 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	38.43 24.00		
	2	J01 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4 2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	25.94 16.00		
	2	J02 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4 2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	18.34 11.34		
	7	J2 JACK TRUSS	7.00 0.00	06-09-00	05-00-11	2 X 4 2 X 6	01-03-08 00-03-00	00-08-03 05-00-11	200.13 126.00		
	10	J3 JACK-OPEN	4.00 0.00	05-10-08	02-03-07	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-03-07	154.20 93.30		

TOTAL # TRUSS= 79.00

TOTAL BFT OF ALL TRUSSES=

2931.07 BFT.

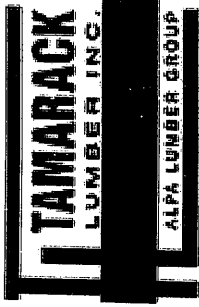
TOTAL WEIGHT OF ALL TRUSSES=

4652.47 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= 10.00



Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

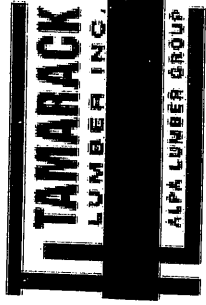
JOB TRACK:44755	LAYOUT ID: 287698	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: A REAR UPGRADE	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT						
	1 2 Ply	T90S ROOF	12.00 0.00	32-11-00	09-03-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		472.82 292.00		
	1	T91S ROOF	12.00 0.00	32-11-00	10-03-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		174.50 107.83		
	1	T92S ROOF	12.00 0.00	32-11-00	11-03-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		182.33 114.50		
	1 2 Ply	T1 HIP GIRDER	12.00 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		382.00 241.34		
	1	T2 HIP	12.00 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		141.31 89.17		
	1	T3 HIP	12.00 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		142.77 92.00		
	2	T4 HIP	12.00 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		301.24 191.00		
	3	T5 HIP	12.00 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		498.15 312.00		
	1	T6 HIP GIRDER	12.00 0.00	27-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-10-08 01-10-08		169.33 105.34		
	1	T7 HIP	12.00 0.00	27-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		120.38 75.83		
	1	T8 HIP	12.00 0.00	27-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		120.47 77.17		
	1	T9 HIP	12.00 0.00	27-09-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		126.86 80.17		
	1	T10 HIP	12.00 0.00	27-09-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		140.80 88.17		
	1	T11 HIP	12.00 0.00	27-09-00	08-10-04	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		146.63 91.67		
	2	T16 PIGGYBACK	12.00 0.00	19-00-00	09-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		197.60 125.34		
	1	G16 GABLE	12.00 0.00	19-00-00	09-00-00	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		112.00 71.67		
	1	T12 COMMON	12.00 0.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		63.57 41.33		
	3	T12S SCISSORS	12.00 6.00	12-00-00	07-10-08	2 X 4	2 X 4	01-03-08 01-03-08	01-10-08 01-10-08		186.90 118.50		

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Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

Page 2 of 2

JOB TRACK:44755	LAYOUT ID: 287698	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: A REAR UPGRADE	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	3		T13S SCISSORS	12.00 6.00	08-00-00	05-10-08	2 X 4 2 X 4	01-03-08 01-03-08	01-10-08 01-10-08	127.02 81.99		
	1		T93 HALF HIP	7.00 0.00	19-00-00	02-03-07	2 X 6 2 X 6	01-03-08 00-00-00	00-08-03 02-03-07	96.08 59.33		
	1		T94X HIP GIRDER	8.00 0.00	11-02-08	03-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-01-02	46.86 31.00		
	4		T95X COMMON	8.00 0.00	11-02-08	04-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 01-01-02	189.24 124.68		
	3		PB1 PIGGYBACK	12.00 0.00	03-07-10	02-03-00	2 X 4 2 X 4	00-00-00 00-00-00	00-05-03 00-05-03	39.21 27.00		
	14		J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	235.06 149.38		
	3		J4 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	38.43 24.00		
	2		J01 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4 2 X 4	01-03-08 02-01-01	01-04-13 00-03-08	25.94 16.00		
	2		J02 JACK-OPEN	8.00 0.00	01-09-07	02-07-02	2 X 4 2 X 4	01-03-08 00-01-01	01-04-13 00-03-08	18.34 11.34		
	7		J2 JACK TRUSS	7.00 0.00	06-09-00	05-00-11	2 X 4 2 X 6	01-03-08 00-03-00	00-08-03 05-00-11	200.13 126.00		
	10		J3 JACK-OPEN	4.00 0.00	05-10-08	02-03-07	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 02-03-07	154.20 93.30		

TOTAL # TRUSS= 76.00

HARDWARE

TOTAL BFT OF ALL TRUSSES=

3059.05 BFT.

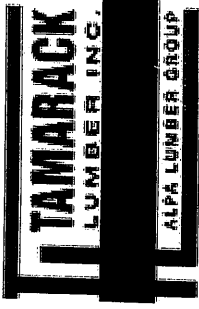
TOTAL WEIGHT OF ALL TRUSSES=

4850.17 LBS.

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
4	Hangers	HGUS26-2	
5	Hangers	LJS26DS	
4	Hangers	LUS24	

TOTAL # ITEMS= 13.00

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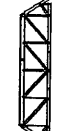
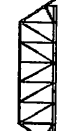
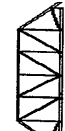
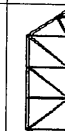
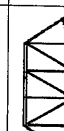
DATE	04/06/18
SALES REP	Mario

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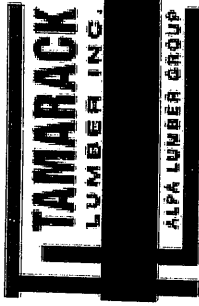
JOB TRACK:44755	LAYOUT ID: 287699	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: B STD OR REAR UPGRADE	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT						
	1 2 Ply	T96S ROOF	10.00 0.00	32-11-00	09-03-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11		468.70 284.68		
	1	T97S ROOF	10.00 0.00	32-11-00	11-07-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		171.59 110.33		
	1	T98S ROOF	10.00 0.00	32-11-00	11-03-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		176.47 111.33		
	1 2 Ply	T21 HIP GIRDER	10.00 0.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11		377.72 229.34		
	1	T22 HIP	10.00 0.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		140.24 90.17		
	1	T23 HIP	10.00 0.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		141.78 89.67		
	2	T24 HIP	10.00 0.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		310.60 197.66		
	3	T25 HIP	10.00 0.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		459.63 293.01		
	1	T26 HIP GIRDER	10.00 0.00	27-09-00	04-01-04	2 X 6	2 X 6	01-03-08 01-03-08	01-07-11 01-07-11		174.12 105.34		
	1	T27 HIP	10.00 0.00	27-09-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		113.37 72.33		
	1	T28 HIP	10.00 0.00	27-09-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		119.46 75.33		
	1	T29 HIP	10.00 0.00	27-09-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		131.54 82.00		
	1	T30 HIP	10.00 0.00	27-09-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		127.98 81.33		
	1	T31 HIP	10.00 0.00	27-09-00	08-10-04	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		132.50 84.83		
	1	T32S HIP	10.00 0.00	22-09-00	05-01-11	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-07-11		102.04 65.33		
	1	T33S HIP	10.00 0.00	22-09-00	06-06-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 00-07-11		111.95 71.00		
	1	T34 HIP	10.00 0.00	22-09-00	08-02-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		108.96 69.67		
	1	T35 HIP	10.00 0.00	22-09-00	09-10-06	2 X 4	2 X 4	01-03-08 01-03-08	01-07-11 01-07-11		116.68 74.67		

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Delivery Shiplist

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DATE	04/06/18
SALES REP	Mario

JOB TRACK:44755	LAYOUT ID: 287699	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: B STD OR REAR UPGRADE	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)											
PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOPBOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T99 COMMON	10.00 0.00	12-00-00	06-07-11	2 X 4 2 X 4	01-03-08 01-03-08	01-07-11 01-07-11	58.08 37.00		
	1	T100 HALF HIP	7.00 0.00	19-00-00	01-11-07	2 X 6 2 X 6	01-03-08 00-00-00	00-08-03 01-11-07	95.88 59.33		
	21	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	352.59 224.07		
	7	J2 JACK TRUSS	7.00 0.00	06-09-00	05-00-11	2 X 4 2 X 6	01-03-08 00-03-00	00-08-03 05-00-11	200.13 126.00		
	9	J44 JACK-OPEN	4.00 0.00	04-10-08	01-11-07	2 X 4 2 X 4	01-03-08 00-00-00	00-03-15 01-11-07	117.72 72.00		

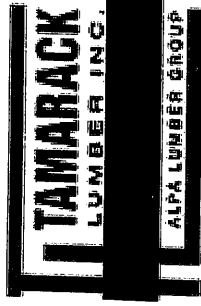
TOTAL # TRUSS= 62.00

TOTAL BFT OF ALL TRUSSES= 2706.42 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4309.73 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
2	Hangers	HGUS26-2	
5	Hangers	LJS26DS	

TOTAL # ITEMS= 7.00



Delivery Shiplist

DATE	04/06/18
SALES REP	Mario

JOB TRACK:44755	LAYOUT ID: 287661	LOCATION: BRADFORD
BUILDER: BAYVIEW WELLINGTON-GREEN VALLE	SUB-BUILDER:	
MODEL: S42-16	ELEVATION: C STD OR REAR UPGRADE	

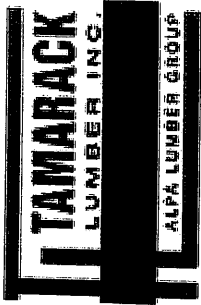
ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT		LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT						
	1	T101S ROOF	8.00	32-11-00	09-03-04	2 X 6	2 X 6	01-03-08	01-04-13	450.54			
	2 Ply		0.00					01-03-08	01-04-13	278.68			
	1	T102S ROOF	8.00	32-11-00	10-03-04	2 X 4	2 X 4	01-03-08	01-04-13	157.85			
			0.00					01-03-08	01-04-13	100.50			
	2	T103S ROOF	8.00	32-11-00	10-07-13	2 X 4	2 X 4	01-03-08	01-04-13	304.80			
			0.00					01-03-08	01-04-13	192.66			
	1	T41 HIP GIRDER	8.00	32-11-00	04-01-04	2 X 6	2 X 6	01-03-08	01-04-13	373.82			
	2 Ply		0.00					01-03-08	01-04-13	226.68			
	1	T42 HIP	8.00	32-11-00	05-01-04	2 X 4	2 X 4	01-03-08	01-04-13	133.34			
			0.00					01-03-08	01-04-13	83.17			
	1	T43 HIP	8.00	32-11-00	06-01-04	2 X 4	2 X 4	01-03-08	01-04-13	137.38			
			0.00					01-03-08	01-04-13	86.83			
	1	T44 HIP	8.00	32-11-00	07-01-04	2 X 4	2 X 4	01-03-08	01-04-13	144.42			
			0.00					01-03-08	01-04-13	91.50			
	3	T45 HIP	8.00	32-11-00	08-01-04	2 X 4	2 X 4	01-03-08	01-04-13	435.78			
			0.00					01-03-08	01-04-13	270.99			
	1	T46 HIP GIRDER	8.00	27-09-00	04-01-04	2 X 6	2 X 6	01-03-08	01-04-13	163.80			
			0.00					01-03-08	01-04-13	100.00			
	1	T47 HIP	8.00	27-09-00	05-01-04	2 X 4	2 X 4	01-03-08	01-04-13	112.41			
			0.00					01-03-08	01-04-13	69.83			
	1	T48 HIP	8.00	27-09-00	06-01-04	2 X 4	2 X 4	01-03-08	01-04-13	115.10			
			0.00					01-03-08	01-04-13	72.00			
	1	T49 HIP	8.00	27-09-00	07-01-04	2 X 4	2 X 4	01-03-08	01-04-13	120.65			
			0.00					01-03-08	01-04-13	76.17			
	1	T50 HIP	8.00	27-09-00	08-01-04	2 X 4	2 X 4	01-03-08	01-04-13	125.18			
			0.00					01-03-08	01-04-13	78.00			
	1	T51 HIP	8.00	27-09-00	08-10-04	2 X 4	2 X 4	01-03-08	01-04-13	123.78			
			0.00					01-03-08	01-04-13	78.33			
	1	T52 HIP GIRDER	8.00	22-09-00	04-01-04	2 X 6	2 X 6	01-03-08	01-04-13	143.91			
			0.00					01-03-08	01-04-13	90.67			
	1	T53 HALF HIP	8.00	12-00-00	02-09-00	2 X 4	2 X 6	01-03-08	01-04-13	58.88			
			0.00					00-00-00	02-09-00	34.67			
	1	T54 HALF HIP	8.00	12-00-00	03-09-00	2 X 4	2 X 4	01-03-08	01-04-13	50.93			
			0.00					00-00-00	03-09-00	33.33			
	1	T55 HIP GIRDER	6.00	10-09-00	03-03-13	2 X 4	2 X 4	01-03-08	01-02-00	46.65			
			0.00					01-03-08	01-02-00	31.00			

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JOB TRACK:44755 LAYOUT ID: 287661 LOCATION: BRADFORD

BUILDER: BAYVIEW WELLINGTON-GREEN VALLE SUB-BUILDER:

MODEL: S42-16 ELEVATION: C STD OR REAR UPGRADE

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOPBOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	4	T56 COMMON	6.00 0.00	10-09-00	03-10-04	2 X 4 2 X 4	01-03-08 01-03-08	01-02-00 01-02-00	167.96 109.32		
	26	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4 2 X 4	01-03-08 00-00-00	01-02-00 04-01-04	436.54 277.42		
	5	J8 JACK-OPEN	8.00 0.00	02-10-08	03-03-13	2 X 4 2 X 4	01-03-08 00-00-00	01-04-13 03-03-13	51.50 33.35		

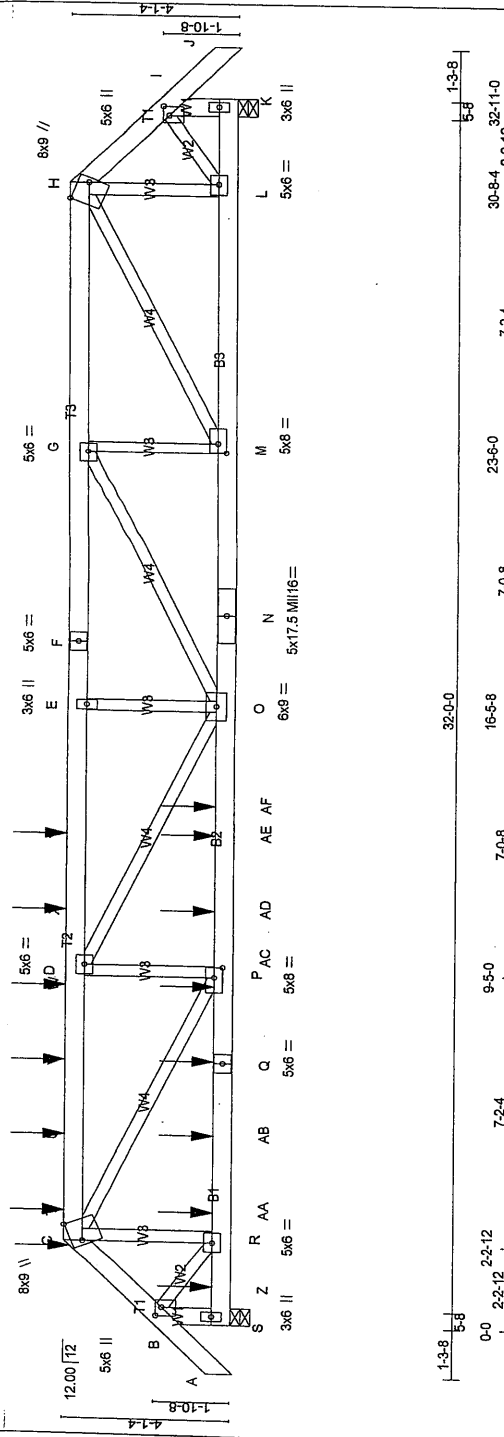
TOTAL # TRUSS= 58.00

TOTAL BFT OF ALL TRUSSES= 2415.10 BFT. TOTAL WEIGHT OF ALL TRUSSES= 3855.22 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
6	Hangers	LJS26DS	

TOTAL # ITEMS= 9.00



TOTAL WEIGHT = 2 X 191 = 382 LBS

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF
BOT CH. LL = 6.0 PSF
DL = 10.5 PSF
LL = 7.0 PSF
TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

*** NON-UNIFORM GIRDER ***
ADDITIONAL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

(65% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFLECTION (LL) = L/360 (1, 10")
CALCULATED DEFLECTION (LL) = L/989 (0.33")
ALLOWABLE DEFLECTION (TL) = L/360 (1, 10")
CALCULATED DEFLECTION (TL) = L/727 (0.54")

CSI: TC=0.571/10 (D-E-1), BC=0.721/10 (O-P-1), WB=0.641/10 (C-P-1), SS=0.451/10 (O-P-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COL=1.00 SHCAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

RUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (PSI) SHEAR (PLI) SECTION (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656
MI16 473 276 2341 1245 4454 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.87 (C) (INPUT = 0.90)
ISI GRIP = 0.87 (C) (INPUT = 1.00)

DWG NO. 2012-08-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
		GROSS REACTION		GROSS REACTION		DOWN		BRG	
		VERT		HORZ		UPLIFT		IN-XX	
JT	S	4598	0	4598	0	0	5.8	5.8	5.8
K	K	3545	0	3545	0	0	5.8	5.8	5.8

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
		COMBINED		LIVE		PERMILIVE			
		SNOW							
JT	S	3532	2161/0	642/0	0/0	0/0	0/0	828/0	0/0
K	K	2793	1674/0	485/0	0/0	0/0	0/0	633/0	0/0

BEARING MATERIAL TO BE SPF NO 2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATEROALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		MEMB.		MAX. FACTORED	
		FORCE		CS1 (LC)		LENGTH FR-TO		FORCE	
		(LBS)		(PLF)				(LBS)	
		FROM		TO				CS1 (LC)	
FR-TO	MEMB.	FR-TO	MEMB.	FR-TO	MEMB.	FR-TO	MEMB.	FR-TO	MEMB.
A-B	0/58	-112.4	-112.4	0.05 (1)	10.00	R-C	-1378/0	0.12 (1)	0.12 (1)
B-C	-3688/0	-112.4	-112.4	0.04 (1)	5.73	C-P	0/7193	0.64 (1)	0.64 (1)
C-T	-9062/0	-112.4	-112.4	0.51 (1)	3.63	P-D	-2082/0	0.18 (1)	0.18 (1)
T-U	-9062/0	-112.4	-112.4	0.51 (1)	3.63	D-O	0/1499	0.13 (1)	0.13 (1)
U-V	-9062/0	-112.4	-112.4	0.51 (1)	3.63	O-E	-776/0	0.07 (1)	0.07 (1)
V-W	-9062/0	-112.4	-112.4	0.51 (1)	3.63	E-G	0/3752	0.33 (1)	0.33 (1)
W-D	-9062/0	-112.4	-112.4	0.51 (1)	3.63	M-G	-2587/0	0.22 (1)	0.22 (1)
D-X	-10413/0	-112.4	-112.4	0.57 (1)	3.36	M-H	0/5568	0.48 (1)	0.48 (1)
X-Y	-10413/0	-112.4	-112.4	0.57 (1)	3.36	L-H	-817/0	0.07 (1)	0.07 (1)
Y-E	-10413/0	-112.4	-112.4	0.44 (1)	3.52	L-I	0/2443	0.22 (1)	0.22 (1)
E-F	-10413/0	-112.4	-112.4	0.44 (1)	3.52				
F-G	-10413/0	-112.4	-112.4	0.31 (1)	4.27				
G-H	-7081/0	-112.4	-112.4	0.04 (1)	6.25				
H-I	-3051/0	-112.4	-112.4	0.04 (1)	6.25				
I-J	0/58	0.0	0.0	0.17 (1)	10.00				
J-B	-3559/0	0.0	0.0	0.17 (1)	6.74				
K-I	-3657/0	0.0	0.0	0.14 (1)	7.35				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX+	FACE	DIR.	TYPE
JT	2-2-12	-25	-28	FRONT	VERT	DEAD
C	2-2-12	-156	-156	FRONT	VERT	SNOW
Q	7-0-4	-40	-70	FRONT	VERT	TOTAL
T	3-0-4	-147	-147	FRONT	VERT	TOTAL

LUMBER		DESOR.	
N.L.G.A. RULES		SPF	
CHORDS		SPF	
A - C	No.2	SPF	SPF
C - F	No.2	SPF	SPF
F - H	No.2	SPF	SPF
H - J	No.2	SPF	SPF
J - B	No.2	SPF	SPF
K - I	No.2	SPF	SPF
S - Q	2100F 1.8E	SPF	SPF
Q - N	2100F 1.8E	SPF	SPF
N - K	2100F 1.8E	SPF	SPF
ALL WEBS EXCEPT	No.2	SPF	SPF
DRY, SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(122.0)
C-F	12	SIDE(183.1)
F-H	12	TOP
H-J	12	TOP
J-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
S-Q	12	SIDE(0.0)
Q-N	12	SIDE(197.8)
N-K	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x4	6	
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.		
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.		
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.		

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)			
JT	TYPE	W	LEN
B	TMWV+p	5.0	6.0
C	TMWV+m	8.0	9.0
D	TMWV-l	8.0	9.0
E	TMWV+w	3.0	6.0
F	TS-l	5.0	6.0
G	TMWV-l	5.0	6.0
H	TMWV+m	8.0	9.0
I	TMWV+p	8.0	9.0
J	BMV1+p	5.0	6.0
K	BMWV-l	3.0	6.0
L	BMWV-l	5.0	6.0
M	BS-l	5.0	8.0
N	BS-l	5.0	8.0
O	BMWVW-l	5.0	8.0
P	BMWV-l	5.0	8.0
Q	BS-l	5.0	6.0
R	BMWV-l	5.0	6.0
S	BMV1+p	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1)- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 181.0 lbs FACTORED DOWN AT 22-1/2" 141.0 lbs FACTORED DOWN AT 3-0/4" 135.3 lbs FACTORED DOWN AT 5-0/4" 135.3 lbs FACTORED DOWN AT 7-0/4" 135.3 lbs FACTORED DOWN AT 9-0/4" 135.3 lbs FACTORED DOWN AT 11-0/4" 135.3 lbs FACTORED DOWN AT 13-0/4" ON TOP CHORD, AND 86.9 lbs FACTORED DOWN AT 1-0/4" 86.9 lbs FACTORED DOWN AT 3-0/4" 69.9 lbs FACTORED DOWN AT 5-0/4" 69.9 lbs FACTORED DOWN AT 7-0/4" 69.9 lbs FACTORED DOWN AT 9-0/4" 69.9 lbs FACTORED DOWN AT 11-0/4" 69.9 lbs FACTORED DOWN AT 13-0/4" AND 1909.7 lbs FACTORED DOWN AT 13-9/7 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

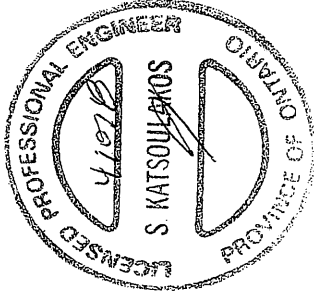
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 181.0 lbs FACTORED DOWN AT 2-2-12, 322.6 lbs FACTORED DOWN AT 30-8-4, 147.0 lbs FACTORED DOWN AT 30-4, 135.3 lbs FACTORED DOWN AT 50-4, 135.3 lbs FACTORED DOWN AT 7-0-4, 135.3 lbs FACTORED DOWN AT 9-0-4, 135.3 lbs FACTORED DOWN AT 11-0-4, 135.3 lbs FACTORED DOWN AT 13-0-4, 135.3 lbs FACTORED DOWN AT 14-10-4, 135.3 lbs FACTORED DOWN AT 16-10-4, 135.3 lbs FACTORED DOWN AT 20-10-4, 135.3 lbs FACTORED DOWN AT 24-10-4, 135.3 lbs FACTORED DOWN AT 26-10-4, 135.3 lbs FACTORED DOWN AT 28-10-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-0-4, 69.9 lbs FACTORED DOWN AT 3-0-4, 69.9 lbs FACTORED DOWN AT 5-0-4, 69.9 lbs FACTORED DOWN AT 7-0-4, 69.9 lbs FACTORED DOWN AT 9-0-4, 69.9 lbs FACTORED DOWN AT 11-0-4, 69.9 lbs FACTORED DOWN AT 13-0-4, 69.9 lbs FACTORED DOWN AT 14-10-4, 69.9 lbs FACTORED DOWN AT 16-10-4, 69.9 lbs FACTORED DOWN AT 18-10-4, 69.9 lbs FACTORED DOWN AT 20-10-4, 69.9 lbs FACTORED DOWN AT 22-10-4, 69.9 lbs FACTORED DOWN AT 24-10-4, 69.9 lbs FACTORED DOWN AT 26-10-4, AND 69.9 lbs FACTORED DOWN AT 28-10-4, AND 69.9 lbs FACTORED DOWN AT 30-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CS1 (LC)	MAX. LENGTH FR-TO	MEMB. MAX. FORCE (LBS)	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO	0 / 7283	-28.0	-28.0	0.24 (1)	10.00		
AP-AQ	0 / 7283	-28.0	-28.0	0.24 (1)	10.00		
AQ-M	0 / 7283	-28.0	-28.0	0.24 (1)	10.00		
M-AR	0 / 2378	-28.0	-28.0	0.12 (1)	10.00		
AR-AS	0 / 2378	-28.0	-28.0	0.12 (1)	10.00		
AS-AT	0 / 2378	-28.0	-28.0	0.12 (1)	10.00		
AT-L	0 / 2378	-28.0	-28.0	0.12 (1)	10.00		
L-AU	0 / 0	-28.0	-28.0	0.07 (2)	10.00		
AU-K	0 / 0	-28.0	-28.0	0.07 (2)	10.00		
FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC. LC1 MAX+ MAX+	FACE	DIR.	TYPE			
C	2-2-12 -23	FRONT	VERT	DEAD			
H	2-2-12 -156	FRONT	VERT	SNOW			
H	30-8-4 -25	FRONT	VERT	DEAD			
H	30-8-4 -142	FRONT	VERT	TOTAL			
H	30-8-4 -156	FRONT	VERT	SNOW			
N	18-10-4 -40	FRONT	VERT	TOTAL			
Q	7-0-4 -40	FRONT	VERT	TOTAL			
T	3-0-4 -147	FRONT	VERT	TOTAL			
U	5-0-4 -135	FRONT	VERT	TOTAL			
V	7-0-4 -135	FRONT	VERT	TOTAL			
W	9-0-4 -135	FRONT	VERT	TOTAL			
X	11-0-4 -135	FRONT	VERT	TOTAL			
Y	13-0-4 -135	FRONT	VERT	TOTAL			
Z	14-10-4 -135	FRONT	VERT	TOTAL			
AA	16-10-4 -135	FRONT	VERT	TOTAL			
AB	18-10-4 -135	FRONT	VERT	TOTAL			
AC	20-10-4 -135	FRONT	VERT	TOTAL			
AD	22-10-4 -135	FRONT	VERT	TOTAL			
AE	24-10-4 -135	FRONT	VERT	TOTAL			
AF	26-10-4 -135	FRONT	VERT	TOTAL			
AG	28-10-4 -135	FRONT	VERT	TOTAL			
AH	1-0-4 -40	FRONT	VERT	TOTAL			
AI	3-0-4 -40	FRONT	VERT	TOTAL			
AJ	5-0-4 -40	FRONT	VERT	TOTAL			
AK	7-0-4 -40	FRONT	VERT	TOTAL			
AL	9-0-4 -40	FRONT	VERT	TOTAL			
AM	11-0-4 -40	FRONT	VERT	TOTAL			
AN	13-0-4 -40	FRONT	VERT	TOTAL			
AO	15-10-4 -40	FRONT	VERT	TOTAL			
AP	17-10-4 -40	FRONT	VERT	TOTAL			
AQ	19-10-4 -40	FRONT	VERT	TOTAL			
AR	21-10-4 -40	FRONT	VERT	TOTAL			
AS	23-10-4 -40	FRONT	VERT	TOTAL			
AT	25-10-4 -40	FRONT	VERT	TOTAL			
AU	27-10-4 -40	FRONT	VERT	TOTAL			



162
DWG NO. TAM 18512-18
STRUCTURAL
COMPONENT ONLY

JOB NAME
287698

TRUSS NAME
T2

QUANTITY
1

PLY
1

JOB DESC.
OBC2012_6TCDL

TRUSS DESC.

DRWG NO.

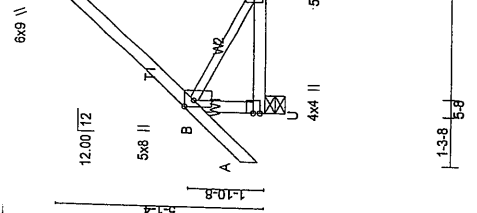
Tamarack Roof Truss, Burlington

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Tue Apr 10 09:47:23 2018 Page 1

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-1-3-8 0-0 3-2-12 3-2-12 8-7-0 5-3-0 13-10-0 5-3-0 19-1-0 5-3-0 24-4-0 5-4-4 29-8-4 3-2-12 3-2-12 1-3-8

Scale = 1/57.2



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	W	LEN	Y	X
A	C	F	2x4	DRY	No.2	5.0	8.0	2.75	2.00
B	C	F	2x4	DRY	No.2	6.0	8.0	2.75	2.00
C	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
D	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
E	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
F	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
G	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
H	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
I	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
J	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
K	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
L	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
M	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
N	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
O	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
P	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
Q	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
R	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
S	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
T	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
U	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
V	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
W	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
X	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
Y	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00
Z	F	I	2x4	DRY	No.2	4.0	4.0	2.00	2.00

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT	5.0	8.0	2.75	2.00
B	6.0	8.0	2.75	2.00
C	4.0	4.0	2.00	2.00
D	4.0	4.0	2.00	2.00
E	4.0	4.0	2.00	2.00
F	4.0	4.0	2.00	2.00
G	4.0	4.0	2.00	2.00
H	4.0	4.0	2.00	2.00
I	4.0	4.0	2.00	2.00
J	4.0	4.0	2.00	2.00
K	4.0	4.0	2.00	2.00
L	4.0	4.0	2.00	2.00
M	4.0	4.0	2.00	2.00
N	4.0	4.0	2.00	2.00
O	4.0	4.0	2.00	2.00
P	4.0	4.0	2.00	2.00
Q	4.0	4.0	2.00	2.00
R	4.0	4.0	2.00	2.00
S	4.0	4.0	2.00	2.00
T	4.0	4.0	2.00	2.00
U	4.0	4.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE

TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM		FACTORED		INPUT		RECORD	
		GROSS REACTION		GROSS REACTION		GROSS REACTION		BRG		BRG	
		VERT		HORZ		DOWN		UP		IN-SX	
JT	U	2468	0	2468	0	0	0	5.8	5.8	5.8	5.8
L	U	2468	0	2468	0	0	0	5.8	5.8	5.8	5.8

UNFACTORED REACTIONS				MAX. MIN. COMPONENT REACTIONS				SOIL			
1ST LCGSE		SNOW		LIVE		PERMILIVE		WIND		DEAD	
JT	U	1980	1160 / 0	346 / 0	346 / 0	0 / 0	0 / 0	0 / 0	445 / 0	445 / 0	445 / 0
L	U	1950	1160 / 0	346 / 0	346 / 0	0 / 0	0 / 0	0 / 0	445 / 0	445 / 0	445 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

UNFACTORED REACTIONS

TEST LOOSE	MAX	MIN	COMPONENT REACTIONS
JT	1950	1160	0
U	1950	1160	0
L	1950	1160	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	VERT. LOAD LC1	MAX	MEMB.	MAX. FACTORED	FORCE	MAX
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO
A-B	-112.4	-112.4	0.16	(1)	4.48	C-S	-515	(0)
B-C	-112.4	-112.4	0.27	(1)	3.30	S-D	-1374	(0)
C-D	-112.4	-112.4	0.68	(1)	2.87	D-Q	0	(1068)
D-E	-112.4	-112.4	0.80	(1)	3.08	Q-E	-573	(0)
E-F	-112.4	-112.4	0.85	(1)	2.86	P-G	-572	(0)
F-G	-112.4	-112.4	0.80	(1)	3.30	H-I	0	(1068)
G-H	-112.4	-112.4	0.68	(1)	4.48	N-H	-1373	(0)
H-I	-112.4	-112.4	0.27	(1)	10.00	M-I	0	(2282)
I-J	-112.4	-112.4	0.16	(1)	5.48	B-T	-515	(0)
J-K	0.0	0.0	0.27	(1)	5.48	M-J	0	(1575)
K-L	0.0	0.0	0.27	(1)	10.00	M-J	0	(1575)
L-M	-28.0	-28.0	0.14	(3)	10.00			
M-N	-28.0	-28.0	0.31	(1)	10.00			
N-O	-28.0	-28.0	0.57	(1)	10.00			
O-P	-28.0	-28.0	0.57	(1)	10.00			
P-Q	-28.0	-28.0	0.70	(1)	10.00			
Q-R	-28.0	-28.0	0.57	(1)	10.00			
R-S	-28.0	-28.0	0.31	(1)	10.00			
S-T	-28.0	-28.0	0.14	(3)	10.00			
T-U	-28.0	-28.0	0.14	(3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BOBC 2012, ABC 2014

- CSA 086-09

- TPIC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = L/360 (1.10")

CALCULATED VERT. DEF. (LL) = 1.098 (0.25")

ALLOWABLE DEF. (TL) = L/360 (1.10")

CALCULATED VERT. DEF. (TL) = 1.077 (0.40")

CSI: TC=0.801 (0.8+1), BC=0.701 (0.7+1), WB=0.537 (0.5+1), SS=0.281 (0.2+1)

DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIPDRY) SHEAR SECTION (PLI)

MAX MIN MAX MIN

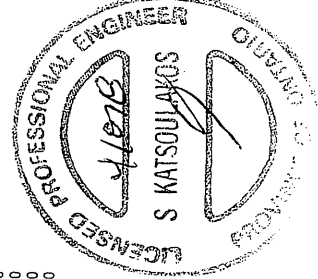
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

.JSI GRIP= 0.88 (C) (INPUT = 0.90)

.JSI METAL= 0.87 (O) (INPUT = 1.00)



DWG NO. TAM 18490.8
STRUCTURAL
COMPONENT ONLY

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. Inside the ring, the license number "41078" is on the left and the name "S. KATSOULAROS" is on the right, with a signature over it. To the right of the seal, there is a table with the following content:

TOTALS:	
TYPE	
TOTALS:	
TOTAL:	

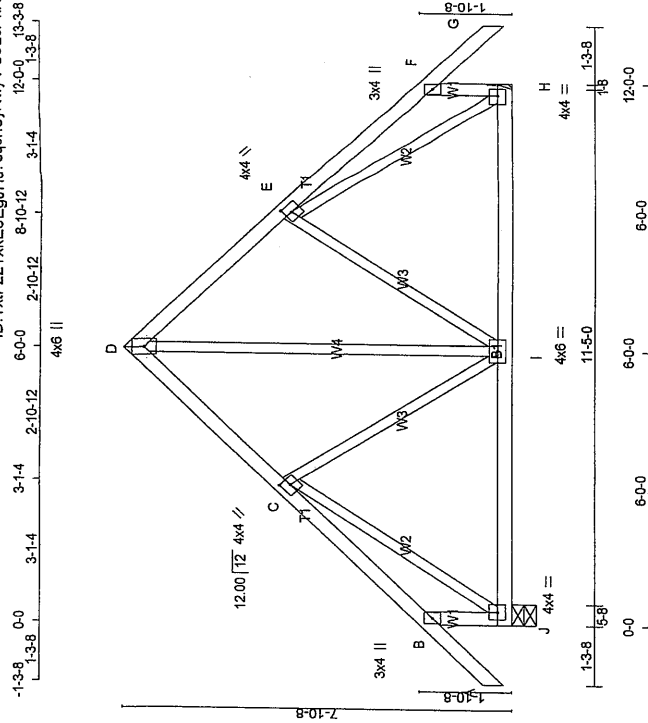
Below the table, the text "IR. PART" and "RT" are visible.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
287698	T12	1	1	OBC2012_6TCDL	

Tamarack Roof Truss, Burlington

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ID:Tx4F22TxkE6EqJhd7eqghSyRwy4-SolcPkRH5joZUYgzyBVkj7EYVmnusm61zCnDzSAIF

Scale = 1:47.1



LUMBER		DESCR.		LUMBER		W		LEN		Y		X	
N.L.G.A. RULES	SIZE	CHORDS	SPF	CHORDS	SPF	W	LEN	Y	X	W	LEN	Y	X
A - D	2x4	DRY	SPF	CHORDS	SPF	3.0	4.0	2.00	1.75	3.0	4.0	2.00	1.75
D - G	2x4	DRY	SPF	CHORDS	SPF	4.0	4.0	2.00	1.75	4.0	4.0	2.00	1.75
J - B	2x4	DRY	SPF	CHORDS	SPF	4.0	4.0	2.00	1.75	4.0	4.0	2.00	1.75
H - F	2x4	DRY	SPF	CHORDS	SPF	3.0	4.0	2.00	1.75	3.0	4.0	2.00	1.75
J - H	2x4	DRY	SPF	CHORDS	SPF	4.0	4.0	2.00	1.75	4.0	4.0	2.00	1.75
ALL WEBS	2x3	DRY	SPF	CHORDS	SPF	4.0	4.0	2.00	1.75	4.0	4.0	2.00	1.75
EXCEPT				CHORDS	SPF	4.0	4.0	2.00	1.75	4.0	4.0	2.00	1.75
DRY: SEASONED LUMBER.													

PLATES (table is in inches)		W		LEN		Y		X	
JT	TYPE	PLATES	W	LEN	Y	X	W	LEN	Y
B	TMW+P	MT20	3.0	4.0	2.00	1.75	3.0	4.0	2.00
C	TMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00
D	TMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00
E	TMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00
F	TMW+P	MT20	3.0	4.0	2.00	1.75	3.0	4.0	2.00
H	BMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00
I	BMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00
J	BMW+P	MT20	4.0	4.0	2.00	1.75	4.0	4.0	2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER		FACTORED		MAXIMUM		FACTORED		RECORD	
BEARINGS	FACTORED	GROSS REACTION	DOWN	HORIZ	UPLIFT	BRG	IN-SX	BRG	IN-SX
JT	999	0	999	0	0	5.8	5.8	5.8	5.8
H	999	0	999	0	0	5.8	5.8	5.8	5.8
MIN. SEAT SIZE: 1x									

UNFACTORED REACTIONS		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

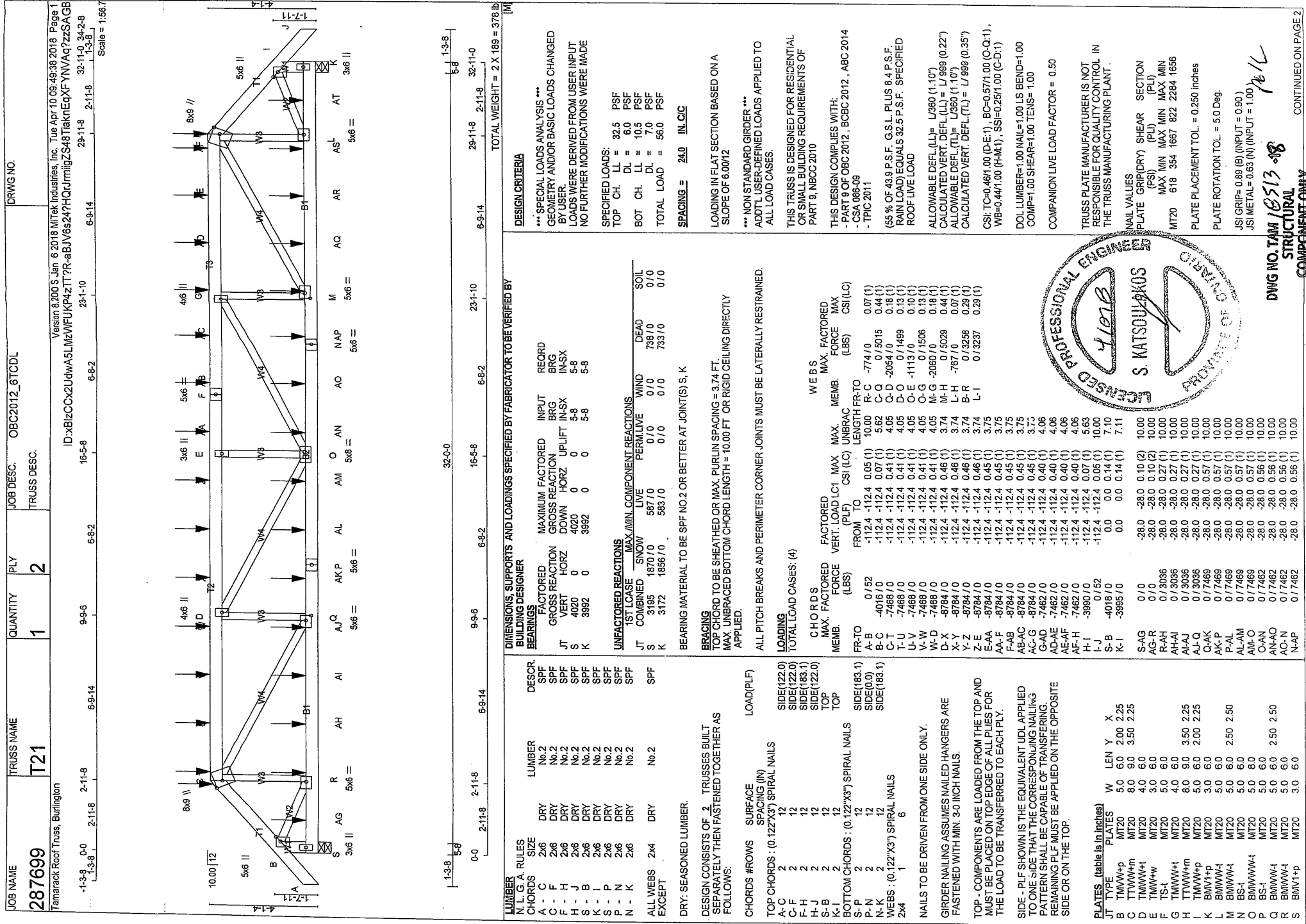
CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

WEBS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0

CHORDS		FACTORED		MAXIMUM		MINIMUM		COMPONENT REACTIONS		WIND		DEAD		SOIL	
1ST LCASE	SNOW	LIVE	PERM	LIVE	PERM	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD	WIND	DEAD
JT	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0	173/0	0/0	0/0	0/0	0/0
H	779	480/0	126/0	0/0	0/0	0/0	173/0	0/0	173/0	0/0</					



HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 238.0 lbs FACTORED DOWN AT 2'-11-8, 238.0 lbs FACTORED DOWN AT 28'-11-8, 169.3 lbs FACTORED DOWN AT 3'-4-4, 135.3 lbs FACTORED DOWN AT 5'-4-4, 135.3 lbs FACTORED DOWN AT 7'-4-4, 135.3 lbs FACTORED DOWN AT 9'-4-4, 135.3 lbs FACTORED DOWN AT 11'-4-4, 135.3 lbs FACTORED DOWN AT 13'-4-4, 135.3 lbs FACTORED DOWN AT 15'-4-4, 135.3 lbs FACTORED DOWN AT 17'-4-4, 135.3 lbs FACTORED DOWN AT 19'-4-4, 135.3 lbs FACTORED DOWN AT 21'-4-4, 135.3 lbs FACTORED DOWN AT 23'-4-4, 135.3 lbs FACTORED DOWN AT 25'-4-4, AND 135.3 lbs FACTORED DOWN AT 27'-4-4, AND 157.6 lbs FACTORED DOWN AT 29'-4-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1'-4-4, 69.9 lbs FACTORED DOWN AT 3'-4-4, 69.9 lbs FACTORED DOWN AT 5'-4-4, 69.9 lbs FACTORED DOWN AT 7'-4-4, 69.9 lbs FACTORED DOWN AT 9'-4-4, 69.9 lbs FACTORED DOWN AT 11'-4-4, 69.9 lbs FACTORED DOWN AT 13'-4-4, 69.9 lbs FACTORED DOWN AT 15'-4-4, 69.9 lbs FACTORED DOWN AT 17'-4-4, 69.9 lbs FACTORED DOWN AT 19'-4-4, 69.9 lbs FACTORED DOWN AT 21'-4-4, 69.9 lbs FACTORED DOWN AT 23'-4-4, 69.9 lbs FACTORED DOWN AT 25'-4-4, 69.9 lbs FACTORED DOWN AT 27'-4-4, AND 69.9 lbs FACTORED DOWN AT 29'-4-4, AND 69.9 lbs FACTORED DOWN AT 31'-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING TOTAL LOAD CASES: (4)										WEBS					
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
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										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										MEMB.		MAX. FACTORED FORCE (LBS)		MAX. CS (LC)	
										CHORDS		MAX. FACTORED FORCE (LBS)			

Tamarack Roof Truss, Burlington

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ID:xBzCc2UdwaSLmZwFUKP4ZTTR-3OHKc2mazhx?LuGN4hdmg88szPhc9wOXQxSAGAS

-13-8 0-0 3-4-10 3-4-10 3-2-2 6-6-11 5-0-0 11-6-12 4-10-12 4-10-12 26-4-5 3-2-2 29-6-6 3-4-10 3-4-10 3-2-8

Scale = 1:56



LATES (table is in inches)		W	LEN	Y	X
PLATES					
TMW+P	MT20	3.0	4.0		
TMW+T	MT20	5.0	6.0	2.50	1.75
TMW+M	MT20	6.0	9.0	Edge	1.75
TMW+M	MT20	4.0	4.0		
TMW+M	MT20	2.0	4.0		
TMW+M	MT20	2.0	4.0		
TMW+M	MT20	3.0	6.0		
TMW+M	MT20	4.0	4.0		
TMW+M	MT20	5.0	6.0	Edge	1.75
TMW+M	MT20	6.0	9.0	2.50	1.75
TMW+P	MT20	3.0	4.0		
TMW+M	MT20	5.0	6.0	2.00	3.00

D-E	-2592 / 0	-112.4	-112.4	0.54 (1)	3.74	S-E	-948 / 0	0.83 (1)
E-F	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57	E-Q	0 / 398	0.09 (1)
F-G	-2824 / 0	-112.4	-112.4	0.56 (1)	3.57	Q-F	-506 / 0	0.44 (1)
G-H	-2824 / 0	-112.4	-112.4	0.55 (1)	3.57	Q-H	0 / 392	0.09 (1)
H-I	-2592 / 0	-112.4	-112.4	0.54 (1)	3.74	O-H	-948 / 0	0.83 (1)
I-J	-2378 / 0	-112.4	-112.4	0.25 (1)	4.24	O-I	0 / 1327	0.30 (1)
J-K	0 / 24	-112.4	-112.4	0.17 (1)	10.00	N-I	0 / 204	0.05 (3)
K-L	0 / 50	-112.4	-112.4	0.15 (1)	10.00	N-J	0 / 201	0.05 (2)
U-B	-301 / 0	0.0	0.0	0.03 (1)	7.81	U-C	-2688 / 0	0.96 (1)
M-K	-301 / 0	0.0	0.0	0.03 (1)	7.81	J-M	-2688 / 0	0.96 (1)
U-T	0 / 1892	-28.0	-28.0	0.45 (2)	10.00			
T-S	0 / 1805	-28.0	-28.0	0.47 (2)	10.00			
S-R	0 / 2592	-28.0	-28.0	0.48 (1)	10.00			
R-Q	0 / 2592	-28.0	-28.0	0.48 (1)	10.00			
Q-P	0 / 2592	-28.0	-28.0	0.48 (1)	10.00			
P-O	0 / 2592	-28.0	-28.0	0.48 (1)	10.00			
O-N	0 / 1805	-28.0	-28.0	0.47 (2)	10.00			
N-M	0 / 1892	-28.0	-28.0	0.45 (2)	10.00			

REGISTERED PROFESSIONAL ENGINEER

41078

S KATSOURAKIS

PROFESSIONAL ENGINEER

THE TRUSS MANUFACTURING PLANT

MAX MIN MAX MIN MAX MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (J) (INPUT = 0.90)

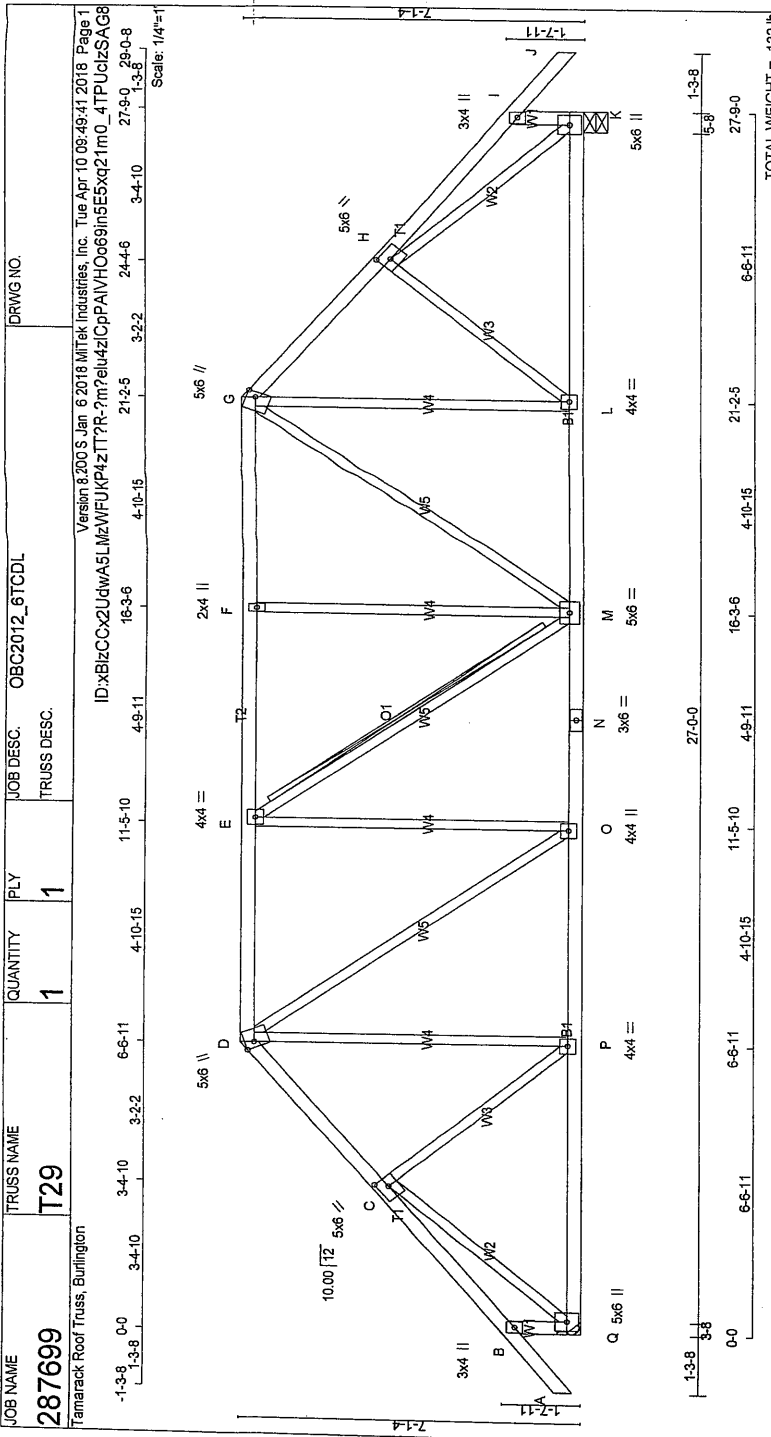
SSI METAL= 0.74 (R) (INPUT = 1.00)

DWG NO. TAM 18516-18

STRUCTURAL

COMPONENT ONLY

A circular professional engineer seal for the State of Oregon. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "STATE OF OREGON" at the bottom. The center of the seal features the name "S. KATSOULAS" and the license number "41817" written in a stylized, handwritten font.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED									
GROSS REACTION									
DOWN									
UP									
HANGER BY OTHERS									
MIN. SEAT SIZE: 3/8									
JT	2104	0	2104	0	0	5-8	5-8		
K	2104	0	2104	0	0	5-8	5-8		
UNFACTORED REACTIONS									
1ST LCASE									
COMBINED									
SNOW									
LIVE									
PERM. LIVE									
WIND									
DEAD									
SOIL									
JT	1660	981 / 0	291 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0	0 / 0
K	1660	981 / 0	291 / 0	0 / 0	0 / 0	0 / 0	377 / 0	0 / 0	0 / 0
BEARING MATERIAL TO BE SPFC NO.2 OR BETTER AT JOINT(S) K									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.									
2x3 DRY SPFC NO.2 T-BRACE AT E-M									
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 8" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS									
MAX. FACTORED									
FORCE									
(LBS)									
MEMB.									
FR-TO									
A-B	0 / 50	-112.4	-112.4	0.15 (1)	10.00	C-P	0 / 138	0.03 (3)	SOIL
B-C	0 / 25	-112.4	-112.4	0.17 (1)	10.00	P-D	0 / 232	0.06 (3)	
C-D	-1921 / 0	-112.4	-112.4	0.23 (1)	4.64	D-O	0 / 877	0.20 (1)	
D-E	-1968 / 0	-112.4	-112.4	0.44 (1)	4.32	O-E	-569 / 0	0.51 (1)	
E-F	-1966 / 0	-112.4	-112.4	0.44 (1)	4.32	E-M	-4 / 0	0.00 (1)	
F-G	-1966 / 0	-112.4	-112.4	0.43 (1)	4.34	M-F	-587 / 0	0.51 (1)	
G-H	-1921 / 0	-112.4	-112.4	0.23 (1)	4.64	M-G	0 / 873	0.20 (1)	
H-I	0 / 25	-112.4	-112.4	0.17 (1)	10.00	L-G	0 / 232	0.06 (3)	
I-J	0 / 50	-112.4	-112.4	0.15 (1)	10.00	L-H	0 / 139	0.03 (3)	
Q-B	-300 / 0	0.0	0.0	0.03 (1)	7.81	Q-C	-2223 / 0	0.96 (1)	
K-I	-300 / 0	0.0	0.0	0.03 (1)	7.81	H-K	-2223 / 0	0.96 (1)	
Q-P	0 / 1399	-28.0	-28.0	0.42 (2)	10.00				
P-O	0 / 1454	-28.0	-28.0	0.43 (2)	10.00				
O-N	0 / 1968	-28.0	-28.0	0.38 (1)	10.00				
N-M	0 / 1968	-28.0	-28.0	0.38 (1)	10.00				
M-L	0 / 1454	-28.0	-28.0	0.43 (2)	10.00				
L-K	0 / 1400	-28.0	-28.0	0.42 (2)	10.00				

4/10/18

PROFESSIONAL ENGINEER

S KATSOULAKOS

132.0

TOTAL WEIGHT = 132.0

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
BOT CH. LL = 6.0 PSF
DL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TFC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/860 (0.93")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL)= L/860 (0.93")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")
CSI: TC=0.44/1.00 (D-E-1), BC=0.43/1.00 (L-M-2), WB=0.96/1.00 (H-K-1), SSI=0.26/1.00 (D-E-1)
DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10
COMPANION LIVE LOAD FACTOR = 0.50
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.59 (N) (INPUT = 1.00)

DWG NO. TAA 18524-18

STRUCTURAL COMPONENT ONLY

287699

Tamarack Roof Truss, Burlington

TRUSS NAME

T30

QUANTITY

1

PLY

1

JOB DESC.

OBC 2012_6TCDL

DRWG NO.

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ID:XBIZCCX2UdwA5LMzMFUKP4zT7R-TyZdyE5b3VAG6S3TXW6OF_dMTL80mFC8I828ZSAG7

-1-3-8 0-0

3-11-13

3-9-5

7-9-2

6-1-6

13-10-8

6-1-6

19-11-14

3-9-5

23-9-3

27-9-0

29-0-8

1-3-8

5x6 //

10.00 [12] 5x6 //

3x4 II

1-1-2-1

1-7-11

3x4 II

5x6 //

1-3-8

0-0

7-9-2

13-10-8

6-1-6

27-0-0

19-11-14

6-1-6

27-9-0

1-3-8

CHORDS

N L G A RULES

SIZE

DESCR.

A - D

2x4

SPF

DRY

F - I

2x4

SPF

DRY

O - B

2x4

SPF

DRY

J - H

2x4

SPF

DRY

O - L

2x4

SPF

DRY

L - J

2x4

SPF

DRY

ALL WEBS

2x3

DRY

EXCEPT

DRY: SEASONED LUMBER.

LUMBER

LEN

Y

X

W

3.0

4.0

5.0

6.0

2.50

2.50

B

TMW+P

MT20

5.0

6.0

2.25

1.50

C

TTWW+I

MT20

2.0

4.0

D

TTWW+m

MT20

5.0

6.0

2.25

1.50

E

TTWW+w

MT20

5.0

6.0

2.50

2.50

F

TTWW+I

MT20

3.0

4.0

G

TMW+P

MT20

5.0

6.0

2.50

2.75

H

BMWW+I

MT20

4.0

4.0

J

BMWW+I

MT20

3.0

6.0

K

BS-I

MT20

3.0

6.0

L

BMWW+I

MT20

4.0

4.0

M

BMWW+I

MT20

4.0

4.0

N

BMWW+I

MT20

5.0

6.0

2.50

2.75

O

BMWW+I

MT20

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED

MAXIMUM FACTORED

RECORD

VERT

GROSS REACTION

DOWN

BRG

HORIZ

UPLIFT

IN-SX

BRG

MIN. SEAT SIZE: 3-8

5-6

1ST LCASE

MAX./MIN. COMPONENT REACTIONS

COMBINED

SNOW

LIVE

PERM LIVE

WIND

DEAD

SOIL

JT

1660

991/0

291/0

0/0

377/0

0/0

J

1660

991/0

291/0

0/0

377/0

0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x3 DRY SPF No.2 T-BRACE AT E-M

2x4 DRY SPF No.2 T-BRACE AT C-O, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MAX. FACTORED

FORCE

VERT. LOAD

LC1

MAX.

CS1 (LC)

UNBRAC

LENGTH

FR-TO

MEMB.

MAX. FACTORED

FORCE

WEBS

MAX. FACTORED

FORCE

A-B

0/50

-112.4

-112.4

0.15

(1)

10.00

C-N

-41/102

0.03

(1)

B-C

0/31

-112.4

-112.4

0.26

(1)

10.00

N-D

0/359

0.08

(2)

C-D

-1880/0

-112.4

-112.4

0.25

(1)

4.68

D-M

0/616

0.14

(1)

D-E

-1800/0

-112.4

-112.4

0.59

(1)

4.25

M-E

0/616

0.14

(1)

E-F

-1800/0

-112.4

-112.4

0.25

(1)

4.68

K-F

0/359

0.08

(2)

F-G

-1880/0

-112.4

-112.4

0.25

(1)

10.00

G-K

-41/102

0.03

(1)

G-H

0/31

-112.4

-112.4

0.15

(1)

10.00

C-C

-2223/0

0.60

(1)

H-I

0/50

0.0

0.0

0.03

(1)

7.81

G-J

-2223/0

0.60

(1)

I-J

-323/0

0.0

0.0

0.03

(1)

10.00

O-N

0/1443

-28.0

-28.0

0.54

(2)

N-M

0/1420

-28.0

-28.0

0.55

(2)

10.00

M-L

0/1420

-28.0

-28.0

0.55

(2)

L-K

0/1420

-28.0

-28.0

0.55

(2)

10.00

K-J

0/1443

-28.0

-28.0

0.54

(2)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.93")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/960 (0.93")

CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.59/1.00 (D-E:1), BC=0.55/1.00 (M-N:2), WB=0.83/1.00 (E-M:1), SS=0.33/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP/DRY SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)

JSI METAL= 0.55 (G) (INPUT = 1.00)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 56.0 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014

- CSA 086-09

- TPC 2011

(65 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/960 (0.93")

CALCULATED VERT. DEFL.(LL) = L/999 (0.16")

ALLOWABLE DEFL.(TL) = L/960 (0.93")

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DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP/DRY SHEAR SECTION (PSI) (PLI)

MAX MIN MAX MIN

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)

JSI METAL= 0.55 (G) (INPUT = 1.00)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

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JSI METAL

[illegible]

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H₁ U
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.

PLATES (table is in inches)		W		I		EN		Y		X	
IT	TYPE	PLATES									
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2012, CBC 2012, CBC 2014											

ITEM	DESCRIPTION	QUANTITY	UNIT	PRICE	TOTAL
1	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				
2	- CSA 086-09				
3	- TPIC 2011				

	LOADING	TOTAL LOAD CASES: (A)
C	5.0	4.0
D	6.0	4.0
TTWW+T	2.25	4.0
TTWW+M	1.50	4.0
MT20		
WT20		
3.0		
3.0		

(55% OF 439 P.S.F. G.S.F. PLUS 84 P.S.F. TOTAL LOAD CASES: (A))

	TOTAL LOAD ANGLES (°)		CHORDS		WEBS		RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD
	MAX	FACTORED	MAX	FACTORED	MAX	FACTORED	
E	4.0	4.0	2.25	1.50			
F	4.0	4.0					
G	4.0	4.0					
H	4.0	4.0					
I	4.0	4.0					
J	4.0	4.0					
K	4.0	4.0					
L	4.0	4.0					
M	4.0	4.0					
N	4.0	4.0					
O	4.0	4.0					
P	4.0	4.0					
Q	4.0	4.0					
R	4.0	4.0					
S	4.0	4.0					
T	4.0	4.0					
U	4.0	4.0					
V	4.0	4.0					
W	4.0	4.0					
X	4.0	4.0					
Y	4.0	4.0					
Z	4.0	4.0					

	MEMB.	FORCE	VERT. LOAD	MAX. FORCE	MAX. MEMB.	ALLOWABLE DEF. (LL) =
						CA/CIIATED VERT. DEF. (LL) = 1/999 (0.06")
H	1MBH1-H	2.0	4.0	MAX. FORCE (LBS)	MAX. MEMB. (LBS)	1/360 (0.76")
J	BMW4-W	2.0	4.0	MAX. FORCE (LBS)	MAX. MEMB. (LBS)	1/999 (0.06")

	FR-TO	FROM TO	LENGTH	FR-TO	ALLOWABLE DEF. (TL) =
	(LBO)	(LFL)	CM(LW)	CM(LW)	L/360 (0.76")
K	5.0	8.0	3.00	2.25	
L	3.0	4.0			

	A-B	C-D	E-F	G-H	I-J	K-L	M-N	O-P	Q-R	S-T	U-V	W-X	Y-Z	AA-AB	AC-AD	AE-AF	AG-AH	AI-AJ	AK-AL	AM-AN	AO-AP	AQ-AR	AS-AT	AV-AW	AX-AY	AZ-AA	AB-AC	AD-AE	AF-AG	AH-AI	AJ-AK	AL-AM	AN-AO	AP-AQ	AR-AS	AT-AV	AW-AX	AY-AZ	BA-BB	BC-BD	BE-BF	BG-BH	BI-BJ	BK-BL	BM-BN	BO-BP	BQ-BR	BS-BT	BU-BV	BW-BX	BY-BZ	CA-CB	CC-CD	CE-CF	CG-CH	CI-CJ	CK-CL	CM-CN	CO-CP	CQ-CR	CS-CT	CU-CV	CW-CX	CY-CZ	DA-DB	DC-DD	DE-DF	DG-DH	DI-DJ	DK-DL	DM-DN	DO-DP	DQ-DR	DS-DT	DU-DV	DW-DX	DY-DZ	EA-EB	EC-ED	EE-EF	EG-EH	EI-EJ	EK-EL	EM-EN	EO-EP	EQ-ER	ES-ET	EU-EV	EW-EX	EY-EZ	FA-FB	FC-FD	FE-FG	FH-FI	FJ-FK	FL-FM	FN-FO	FP-FQ	FR-FS	FT-FU	FV-FW	FX-FY	FZ-FA	GA-GB	GC-GD	GE-GF	GH-GI	GJ-GK	GL-GM	GN-GO	GP-GQ	GR-GS	GT-GU	GV-GW	GX-GY	GZ-GA	HA-HB	HC-HD	HE-HF	HG-HI	HJ-HK	HL-HM	HN-HO	HP-HT	HU-HV	HW-HX	HY-HZ	IA-IB	IC-ID	IE-IF	IG-IH	IJ-IK	IL-IM	IN-IO	IP-II	IR-IS	IT-IV	IW-IX	IY-IZ	JA-JB	JC-JD	JE-JF	JG-JH	JI-JJ	JK-JL	JM-JN	JO-JP	JQ-JR	JS-JT	JU-JV	JW-JX	JY-JZ	KA-KB	KC-KD	KE-KF	KG-KH	KI-KJ	KL-KM	KN-KO	KP-KQ	KR-KS	KT-KU	KV-KW	KX-KY	KZ-KA	LA-LB	LC-LD	LE-LF	LG-LH	LJ-LK	LL-LM	LN-LO	LP-LQ	LR-LS	LT-LU	LV-LW	LX-LY	LZ-LA	MA-MB	MC-MD	ME-MF	MG-MH	MJ-MK	ML-MM	MN-MO	MP-MQ	MR-MS	MT-MU	MV-MW	MX-MY	MZ-MA	NA-NB	NC-ND	NE-NF	NG-NH	NJ-NK	NL-NM	NO-NP	NQ-NS	NT-NU	NV-NW	NX-NY	NZ-NA	OA-OB	OC-OD	OE-OF	OG-OH	OJ-ok	OK-OL	OM-ON	OP-OQ	OR-OS	OT-OU	OV-OW	OX-OY	OZ-OA	PA-PB	PC-PD	PE-PF	PG-PH	PJ-PK	PL-PM	PN-PO	PP-PQ	PR-PS	PT-PU	PV-PW	PX-PY	PZ-PA	QA-QB	QC-QD	QE-QF	QG-QH	QJ-QK	QL-QM	QN-QO	QP-QR	QS-QT	QU-QV	QW-QX	QY-QZ	RA-RB	RC-RD	RE-RF	RG-RH	RJ-RK	RL-RM	RN-RO	RP-RQ	RR-RS	RT-RU	RV-RW	RX-RY	RZ-RA	SA-SB	SC-SD	SE-SF	SG-SH	SJ-SK	SL-SM	SN-SO	SP-SQ	SR-SS	ST-SU	SV-SW	SX-SY	SZ-SA	TA-TB	TC-TD	TE-TF	TG-TH	TJ-TK	TL-TM	TN-TO	TP-TQ	TR-TS	TT-TU	TV-TW	TX-TY	TZ-TA	UA-UB	UC-UD	UE-UF	UG-UH	UJ-UK	UL-UM	UN-UO	UP-UT	UR-US	UV-UW	UX-UY	UZ-UA	VA-VB	VC-VD	VE-VF	VG-VH	VJ-VK	VL-VM	VN-VO	VP-VQ	VR-VS	VT-VU	VV-VW	VX-VY	VZ-VA	WA-WB	WC-WD	WE-WF	WG-WH	WJ-WK	WL-WM	WN-WO	WP-WQ	WR-WS	WT-WU	WV-WW	WX-WY	WZ-WA	XA-XB	XC-XD	XE-XF	XG-XH	XJ-XK	XL-XM	XN-XO	XP-XQ	XR-XS	XT-XU	XV-XW	XX-XY	XZ-XA	YA-YB	YC-YD	YE-YF	YG-YH	YJ-YK	YL-YM	YN-YO	YP-YQ	YR-YS	YT-YU	YV-YW	YX-YY	YZ-YA	ZA-ZB	ZC-ZD	ZE-ZF	ZG-ZH	ZJ-ZK	ZL-ZM	ZN-ZO	ZP-ZQ	ZR-ZS	ZT-ZU	ZV-ZW	ZX-ZY	ZZ-ZA	AA-AA	BB-BB	CC-CC	DD-DD	EE-EE	FF-FF	GG-GG	HH-HH	II-II	JJ-JJ	KK-KK	LL-LL	MM-MM	NN-NN	OO-OO	PP-PP	QQ-QQ	RR-RR	SS-SS	TT-TT	UU-UU	VV-VV	WW-WW	XX-XX	YY-YY	ZZ-ZZ	AAA-AAA	BBB-BBB	CCC-CCC	DDD-DDD	EEE-EEE	FFF-FFF	GGG-GGG	HHH-HHH	III-III	JJJ-JJJ	KKK-KKK	LLL-LLL	MMM-MMM	NNN-NNN	OOO-OOO	PPP-PPP	QQQ-qqq	RRR-RRR	SSS-SSS	TTT-TTT	UUU-UUU	VVV-VVV	WWW-WWW	XXX-xxx	YYY-yyy	ZZZ-zzz	AAAA-AAAA	BBBB-BBBB	CCCC-CCCC	DDDD-DDDD	EEEE-EEEE	FFFF-FFFF	GGGG-GGGG	HHHH-HHHH	IIII-IIII	JJJJ-JJJJ	KKKK-KKKK	LLLL-LLLL	MMMM-MMMM	NNNN-NNNN	OOOO-OOOO	PPPP-pppp	RRRR-RRRR	SSSS-SSSS	TTTT-TTTT	UUUU-UUUU	VVVV-VVVV	WWWW-WWWW	XXXX-xxxx	YYYY-yyyy	ZZZZ-zzzz	AAAAA-AAAAA	BBBBB-BBBBB	CCCCC-CCCCC	DDDDD-DDDDD	EEEEE-EEEEE	FFFFFF-FFFFFF	GGGGG-GGGGG	HHHHH-HHHHH	IIIII-IIIII	JJJJJ-JJJJJ	KKKKK-KKKKK	LLLLL-LLLLL	MMMMM-MMMMM	NNNNN-NNNNN	OOOOO-OOOOO	PPPPP-ppppp	RRRRR-RRRRR	SSSSS-SSSSS	TTTTT-TTTTT	UUUUU-UUUUU	VVVVV-VVVVV	WWWWW-WWWWW	XXXXX-xxxxx	YYYYY-yyyyy	ZZZZZ-zzzzz	AAAAAa-AAAAAa	BBBBBa-BBBBBa	CCCCCa-CCCCCa	DDDDDa-DDDDDa	EEEEEd-EEEEEd	FFFFFf-FFFFFf	GGGGGg-GGGGGg	HHHHHg-HHHHHg	IIIIII-IIIIII	JJJJJj-JJJJJj	KKKKKk-KKKKKk	LLLLLl-LLLLLl	MMMMMm-MMMMMm	NNNNNm-NNNNNm	OOOOOo-OOOOOo	PPPPPp-PPPPPp	RRRRRp-RRRRRp	SSSSSp-SSSSSp	TTTTTt-TTTTTt	UUUUUu-UUUUUu	VVVVVv-VVVVVv	WWWWWw-WWWWWWw	XXXXXx-XXXXXx	YYYYYy-YYYYYy	ZZZZZz-zzzzzz	AAAAaa-AAAAaa	BBBBbb-BBBBbb	CCCCcc-CCCCcc	DDDDdd-DDDDdd</
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	MT20	5.0	6.0	C-D	-1500 / 0	-112.4	-112.4	0.13 (1)	5.25	D-M	U / 608	0.14 (1)	CSI: 1 C=0.46/1.00 (G-Q:1), BC=0.33/1.00 (H-T:1), WB=0.63/1.00 (C-O:1), SS=0.42/1.00 (H-Q:1)
○ BMWV1-t				D-E	-1468 / 0	-112.4	-112.4	0.23 (1)	5.17	M-E	-755 / 0	0.33 (1)	

Edge - INDICATES REFERENCE CORNER OF PLATE	E - F	-1654 / 0	-112.4	-112.4	0.24 (1)	4.93	M - K	0 / 1484	0.24 (1)
TOUCHES EDGE OF CHORD	F - G	-1660 / 0	-112.4	-112.4	0.14 (1)	5.04	E - K	0 / 1294	0.07 (1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10									

	COMP=1.10 SHEAR=1.10 TENS=1.10			
G-Q	-1798 / 0	-112.4	0.48 (1)	4.48 K-G
Q-H	-1886 / 0	-112.4	0.38 (1)	4.48 J-G
				0.1638
				0.14 (1)
				0.260
				0.06 (3)

	H-I	0/41	-112.4	-112.4	0.15 (1)	10.00	O-C	-1773/0	0.63 (1)
C-R	-285/0		0.0	0.0	0.03 (1)	7.81	P-Q	-99/491	0.00 (1)

COMPANION LIVE LOAD FACTOR = 0.50

[illegible]

C-N	-20.0	0.32 (2)	10.00
O-1091	-20.0	0.33 (2)	10.00
N-M	-28.0	0.44 (3)	10.00

W-L	0/34	-26.0	0.11 (3)	10.00
L-K	0/85	0.0	0.05 (1)	10.00

	N ¹⁵ VALUES			DATE	CRIPPER'S	SLIPPER	SECTION
K-F	-385/0	0.0	0.10 (1)	7/81			
K-J		28.0	-28.2	0.35 (1)	10.00		
				0/1353			

				PLATE	GRIP(JURY)	SHEAR	SECTION
				(PSI)	(PLI)	(PLI)	
J-P	0 / 1350	-28.0	-28.0	0.35 (1)	10.00		
P-H	0 / 1350	-28.0	-28.0	0.28 (1)	10.00		

	MAX	MIN	MAX	MIN
MT20	618	354	1667	822
			2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PIATE ROTATION TOL = 5.0 Deg.

IS/CAPID= 0.87 /C/INPI IT = 0.901

4/016

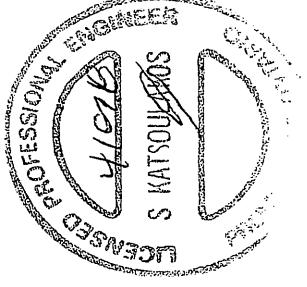
С. КАТОНИЛИДЗЕ

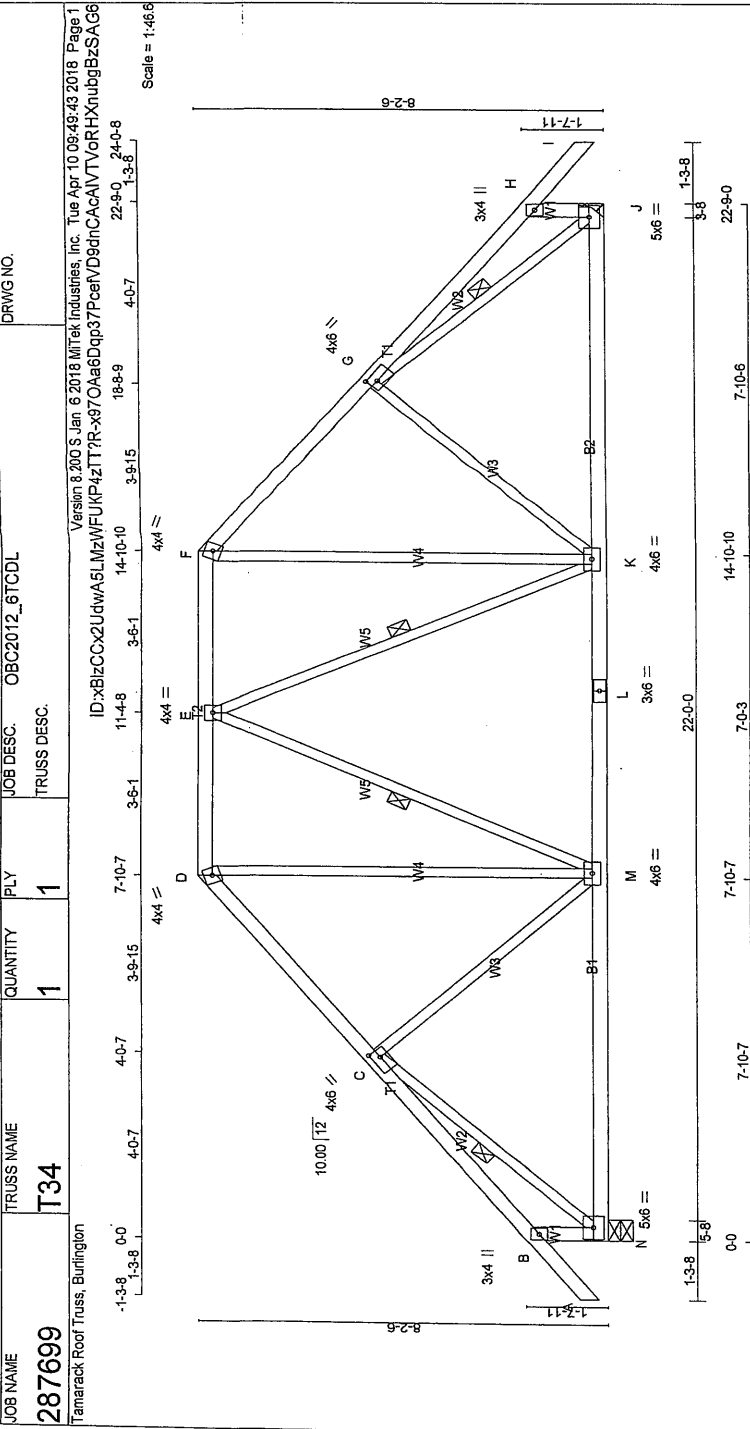
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DRUG NO. TAM / 03 25 18
SERIAL 12181

STRUCTURAL COMPONENT ONLY





TOTAL WEIGHT = 1091 LBS

[MIP]

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESOR.	LUMBER	W	LEN	Y	X
N.L.G.A. RULES					
CHORDS					
A - D	2x4	3.0	4.0	2.00	2.00
D - F	2x4	4.0	6.0	2.00	2.00
F - I	2x4	4.0	4.0		
N - B	2x4	4.0	4.0		
N - H	2x4	4.0	4.0		
N - L	2x4	4.0	6.0	2.00	2.00
N - J	2x4	4.0	6.0		
J	2x4	4.0	6.0		
ALL WEBS	2x3	3.0	4.0		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

PLATE TYPE	W	LEN	Y	X
B	3.0	4.0	2.00	2.00
C	4.0	6.0	2.00	2.00
D	4.0	4.0		
E	4.0	4.0		
F	4.0	4.0		
G	4.0	6.0	2.00	2.00
H	4.0	6.0		
I	4.0	6.0		
J	4.0	6.0		
K	4.0	6.0		
L	4.0	6.0		
M	4.0	6.0		
N	4.0	6.0		

BEARINGS

DESOR.	LUMBER	W	LEN	Y	X
N.L.G.A. RULES					
CHORDS					
A - D	2x4	3.0	4.0	2.00	2.00
D - F	2x4	4.0	6.0	2.00	2.00
F - I	2x4	4.0	4.0		
N - B	2x4	4.0	4.0		
N - H	2x4	4.0	4.0		
N - L	2x4	4.0	6.0	2.00	2.00
N - J	2x4	4.0	6.0		
J	2x4	4.0	6.0		
ALL WEBS	2x3	3.0	4.0		
EXCEPT					

UNFACTORED REACTIONS

1ST LCASE	MAX. MIN.	COMPONENT REACTIONS	SOIL
JT	1380	829 / 0	0 / 0
J	1380	829 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K, G-N, J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	MAX. FACTORED CS (LC)	MEMB. FORCE (LBS)	FACTORED FR-TO	MAX. FACTORED CS (LC)
FR-TO	FROM	TO	FROM	TO	FROM	TO	FROM	TO
A-B	0/50	-112.4	-112.4	0.15 (1)	10.00	C-M	-130 / 63	0.09 (1)
B-C	0/32	-112.4	-112.4	0.27 (1)	10.00	M-D	0 / 558	0.13 (1)
C-D	-1415 / 0	-112.4	-112.4	0.23 (1)	5.24	M-E	-255 / 0	0.13 (1)
D-E	-1068 / 0	-112.4	-112.4	0.17 (1)	5.91	E-K	-255 / 0	0.13 (1)
E-F	-1068 / 0	-112.4	-112.4	0.17 (1)	5.91	K-F	0 / 558	0.13 (1)
F-G	-1415 / 0	-112.4	-112.4	0.23 (1)	5.24	K-G	-130 / 63	0.08 (1)
G-H	0/32	-112.4	-112.4	0.27 (1)	10.00	N-C	-1757 / 0	0.48 (1)
H-I	0/50	-112.4	-112.4	0.15 (1)	10.00	G-J	-1757 / 0	0.48 (1)
N-B	-327 / 0	0.0	0.0	0.03 (1)	7.81			
J-H	-327 / 0	0.0	0.0	6.03 (1)	7.81			
N-M	0 / 1145	-28.0	-28.0	0.49 (2)	10.00			
M-L	0 / 1167	-28.0	-28.0	0.50 (2)	10.00			
L-K	0 / 1167	-28.0	-28.0	0.50 (2)	10.00			
K-J	0 / 1145	-28.0	-28.0	0.49 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
BOT CH. LL = 6.0 PSF
DL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 56.0 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TFC 2011

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAISED DECK EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/960 (0.76")
CALCULATED VERT. DEFL.(LL) = L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/960 (0.76")
CALCULATED VERT. DEFL.(TL) = L/980 (0.28")

CSI: TC=0.27/1.00 (B-C:1), BC=0.50/1.00 (K-M:2), WB=0.45/1.00 (C-N:1), SS=0.19/1.00 (M-N:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(CRY) SHEAR SECTION (PSI) (PLI)

MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

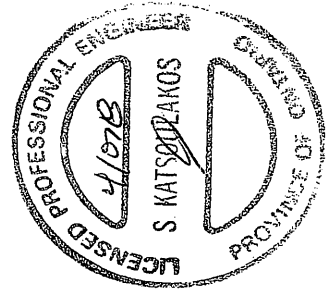
JSI GRIP= 0.86 (C) (INPUT = 0.80)
JSI METAL= 0.43 (C) (INPUT = 1.00)

DWG NO. TAM/13526-18
STRUCTURAL
COMPONENT ONLY

A circular professional engineer seal for the State of Ohio. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "STATE OF OHIO" at the bottom. The center of the seal features the license number "419674" on the left and the name "S. KATSOUMPAROS" on the right, both written in a stylized, handwritten font.

CONTINUED ON PAGE 2

DWG NO. TAM / 6533-18
STRUCTURAL



per
DWG NO. TAM 10533-08
STRUCTURAL
COMPONENT ONLY

JOB NAME 287661 Tamarack Roof Truss, Burlington	TRUSS NAME T41	QUANTITY 1	PLY 2	JOB DESC. OBC2012_6TCDL TRUSS DESC.	DRWG NO.
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Version 8.200 S Jan 6 2018 MITek Industries, Inc. Tue Apr 10 09:54:37 2018 Page 2
ID:YUVQg72w AGbWUXU7p19tZTqnH-KjXU6fep85Pho7AjZQaClq5Gp NmB28W69 8zSABW

LOADING TOTAL LOAD CASES: (4)									
CHORDS				WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO	FR-TO
N-AQ	0/7763	-28.0	-28.0	0.58 (1)	10.00				
AQ-M	0/7763	-28.0	-28.0	0.58 (1)	10.00				
M-AR	0/4046	-28.0	-28.0	0.32 (1)	10.00				
AR-AS	0/4046	-28.0	-28.0	0.32 (1)	10.00				
AS-AT	0/4046	-28.0	-28.0	0.32 (1)	10.00				
AT-L	0/4046	-28.0	-28.0	0.32 (1)	10.00				
L-AU	0/0	-28.0	-28.0	0.09 (3)	10.00				
AU-AV	0/0	-28.0	-28.0	0.09 (3)	10.00				
AV-K	0/0	-28.0	-28.0	0.09 (3)	10.00				

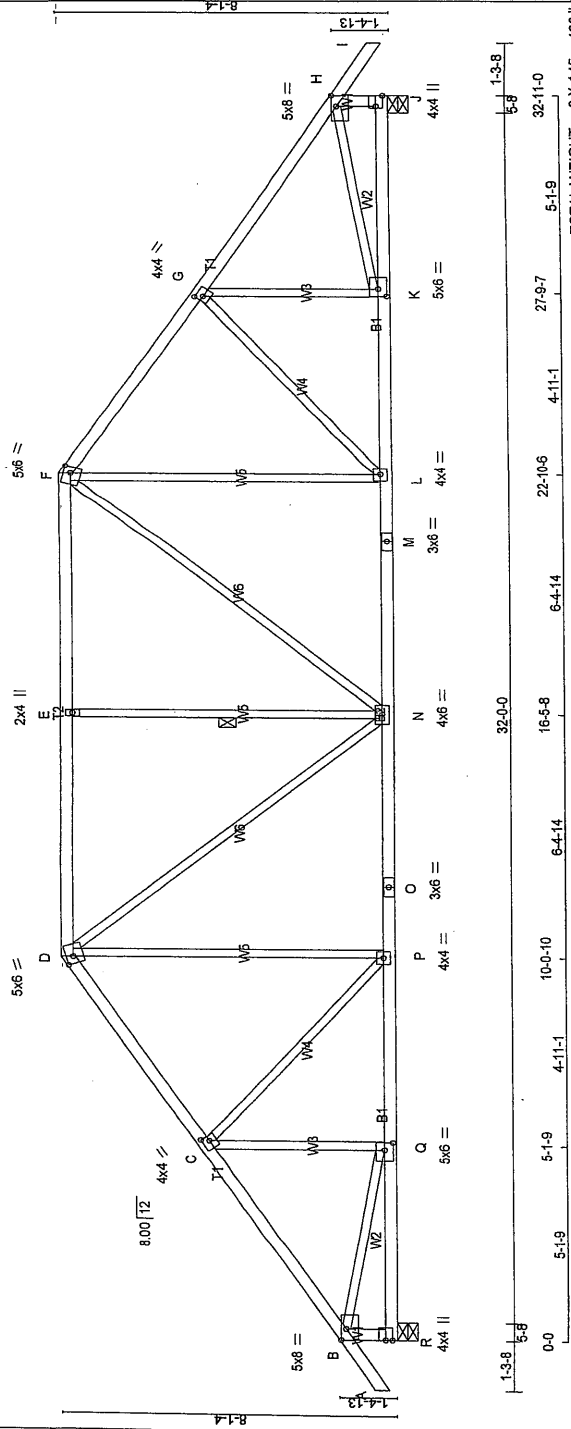
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC	LC1	MAX-	MAX+	FACE	DIR	TYPE		
C	4-0-11	-45	-50		FRONT	VERT	DEAD		
C	4-0-11	-282	-282		FRONT	VERT	DEAD		
H	28-10-6	-45	-50		FRONT	VERT	DEAD		
H	28-10-6	-282	-282		FRONT	VERT	DEAD		
T	5-4-4	-135	-135		FRONT	VERT	SNOW		
U	7-4-4	-135	-135		FRONT	VERT	TOTAL		
V	9-4-4	-135	-135		FRONT	VERT	TOTAL		
W	11-4-4	-135	-135		FRONT	VERT	TOTAL		
X	13-4-4	-135	-135		FRONT	VERT	TOTAL		
Y	15-4-4	-135	-135		FRONT	VERT	TOTAL		
Z	17-4-4	-135	-135		FRONT	VERT	TOTAL		
AA	19-4-4	-135	-135		FRONT	VERT	TOTAL		
AB	21-4-4	-135	-135		FRONT	VERT	TOTAL		
AC	23-4-4	-135	-135		FRONT	VERT	TOTAL		
AD	25-4-4	-135	-135		FRONT	VERT	TOTAL		
AE	27-4-4	-135	-135		FRONT	VERT	TOTAL		
AF	29-4-4	-164	-164		FRONT	VERT	TOTAL		
AG	3-4-4	-40	-70		FRONT	VERT	TOTAL		
AH	3-4-4	-40	-70		FRONT	VERT	TOTAL		
AI	5-4-4	-40	-70		FRONT	VERT	TOTAL		
AJ	7-4-4	-40	-70		FRONT	VERT	TOTAL		
AK	9-4-4	-40	-70		FRONT	VERT	TOTAL		
AL	11-4-4	-40	-70		FRONT	VERT	TOTAL		
AM	13-4-4	-40	-70		FRONT	VERT	TOTAL		
AN	15-4-4	-40	-70		FRONT	VERT	TOTAL		
AO	17-4-4	-40	-70		FRONT	VERT	TOTAL		
AP	19-4-4	-40	-70		FRONT	VERT	TOTAL		
AQ	21-4-4	-40	-70		FRONT	VERT	TOTAL		
AR	23-4-4	-40	-70		FRONT	VERT	TOTAL		
AS	25-4-4	-40	-70		FRONT	VERT	TOTAL		
AT	27-4-4	-40	-70		FRONT	VERT	TOTAL		
AU	29-4-4	-40	-70		FRONT	VERT	TOTAL		
AV	31-4-4	-40	-70		FRONT	VERT	TOTAL		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 4-0-11, 327.0 lbs FACTORED DOWN AT 28-10-6, 135.3 lbs FACTORED DOWN AT 5-4-4, 135.3 lbs FACTORED DOWN AT 7-4-4, 135.3 lbs FACTORED DOWN AT 9-4-4, 135.3 lbs FACTORED DOWN AT 11-4-4, 135.3 lbs FACTORED DOWN AT 13-4-4, 135.3 lbs FACTORED DOWN AT 15-4-4, 135.3 lbs FACTORED DOWN AT 17-4-4, 135.3 lbs FACTORED DOWN AT 19-4-4, 135.3 lbs FACTORED DOWN AT 21-4-4, 135.3 lbs FACTORED DOWN AT 23-4-4, 135.3 lbs FACTORED DOWN AT 25-4-4, AND 135.3 lbs FACTORED DOWN AT 27-4-4, AND 163.9 lbs FACTORED DOWN AT 29-4-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-4-4, 69.9 lbs FACTORED DOWN AT 3-4-4, 69.9 lbs FACTORED DOWN AT 5-4-4, 69.9 lbs FACTORED DOWN AT 7-4-4, 69.9 lbs FACTORED DOWN AT 9-4-4, 69.9 lbs FACTORED DOWN AT 11-4-4, 69.9 lbs FACTORED DOWN AT 13-4-4, 69.9 lbs FACTORED DOWN AT 15-4-4, 69.9 lbs FACTORED DOWN AT 17-4-4, 69.9 lbs FACTORED DOWN AT 19-4-4, 69.9 lbs FACTORED DOWN AT 21-4-4, 69.9 lbs FACTORED DOWN AT 23-4-4, 69.9 lbs FACTORED DOWN AT 25-4-4, AND 69.9 lbs FACTORED DOWN AT 27-4-4, AND 69.9 lbs FACTORED DOWN AT 29-4-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

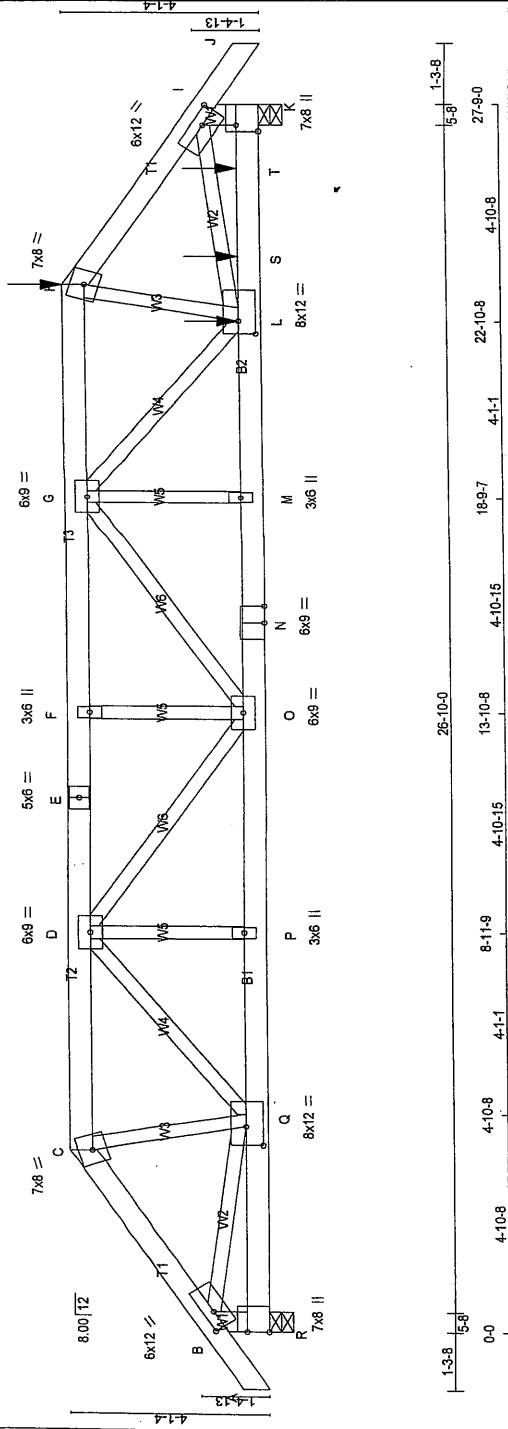
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LUMBER										DESIGN CRITERIA									
N.L.G.A. RULES										SPECIFIED LOADS:									
CHORDS										TOP CH. LL = 32.5 PSF									
A - D 2x4										BOT CH. LL = 10.5 PSF									
D - G 2x4										TOTAL LOAD = 56.0 PSF									
G - J 2x4										SPACING = 24.0 IN. O.C.									
R - B 2x4										LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12									
K - P 2x4										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010									
M - K 2x4										THIS DESIGN COMPLIES WITH:									
ALL WEBS 2x3										- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
EXCEPT										- CSA 086-09									
DRY: SEASONED LUMBER.										- TPC 2011									
										(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD), EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD									
										ALLOWABLE DEFL.(L/1) = L/260 (1.10")									
										ALLOWABLE DEFL.(L/1) = L/260 (1.10")									
										ALLOWABLE DEFL.(L/1) = L/260 (1.10")									
										CALCULATED VERT. DEFL.(L/1) = L/982 (0.41")									
										CSI: TC=0.46/1.00 (F-G-1), BC=0.89/1.00 (L-N-2), WB=0.78/1.00 (H-K-1), SS=0.28/1.00 (D-E-1)									
										DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10									
										COMPANION LIVE LOAD FACTOR = 0.50									
										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
										NAIL VALUES									
										PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)									
										MAX MIN MAX MIN MAX MIN									
										MT20 618 354 1667 822 2284 1656									
										PLATE PLACEMENT TOL. = 0.250 inches									
										PLATE ROTATION TOL. = 5.0 Deg.									
										JSI GRIP= 0.80 (O) (INPUT = 0.80) JSI METAL= 0.74 (H) (INPUT = 1.00)									
										DWG NO. TAM 18536-08 STRUCTURAL COMPONENT ONLY									
										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE MAINTAINED.									
										BUILDING DESIGNER									
										BEARINGS									
										REQRD									
										BRG									
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LUMBER										DESIGN CRITERIA									
N.L.G.A. RULES										SPECIFIED LOADS:									
CHORDS										TOP CH. LL = 32.5 PSF									
SIZE										DL = 6.0 PSF									
2x4										BOT CH. LL = 10.5 PSF									
D - F										DL = 7.0 PSF									
2x4										TOTAL LOAD = 56.0 PSF									
D - F										SPACING = 24.0 IN. OC									
2x4										LOADING IN FLAT SECTION BASED ON A									
2x4										SLOPE OF 6.00/12									
2x4										THIS TRUSS IS DESIGNED FOR RESIDENTIAL									
2x4										OR SMALL BUILDING REQUIREMENTS OF									
2x4										PART 9, NBCC 2010									
2x4										THIS DESIGN COMPLIES WITH:									
2x4										- PART 9 OF OBC 2012, BCBC 2012, ABC 2014									
2x4										- CSA 086-09									
2x4										- TPIC 2011									
2x4										(65% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.									
2x4										RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED									
2x4										ROOF LIVE LOAD									
2x4										ALLOWABLE DEF. (LL) = L/360 (1/10")									
2x4										CALCULATED VERT. DEF. (LL) = L/989 (0.14")									
2x4										ALLOWABLE DEF. (TL) = L/360 (1/10")									
2x4										CALCULATED VERT. DEF. (TL) = L/989 (0.23")									
2x4										CSI: TC=0.71/1.00 (E-F-1), BC=0.47/1.00 (K-L-1),									
2x4										WB=0.53/1.00 (H-K-1), SS=0.35/1.00 (E-F-1),									
2x4										DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.10									
2x4										COMP=1.10 SHEAR=1.10 TENS= 1.10									
2x4										COMPANION LIVE LOAD FACTOR = 0.50									
2x4										TRUSS PLATE MANUFACTURER IS NOT									
2x4										RESPONSIBLE FOR QUALITY CONTROL IN									
2x4										THE TRUSS MANUFACTURING PLANT.									
2x4										NAIL VALUES									
2x4										PLATE GRIP(DRY) SHEAR SECTION									
2x4										(PSI) (PLI)									
2x4										MAX MIN MAX MIN									
2x4										MT20 618 354 1667 822 2284 1656									
2x4										PLATE PLACEMENT TOL. = 0.250 inches									
2x4										PLATE ROTATION TOL. = 5.0 Deg.									
2x4										JSI GRIP= 0.88 (K) (INPUT = 0.90)									
2x4										JSI METAL= 0.59 (O) (INPUT = 1.00)									
2x4										DWG NO. TAM 18537-18									
2x4										STRUCTURAL									
2x4										COMPONENT ONLY									
2x4										4/18/18									
2x4										S KATSOUKIDAKIS									
2x4										LICENSED PROFESSIONAL ENGINEER									
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HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 327.0 lbs FACTORED DOWN AT 23-8.5 ON TOP CHORD, AND 3063.3 lbs FACTORED DOWN AT 22-10-8, AND 69.9 lbs FACTORED DOWN AT 24-4.4, AND 69.9 lbs FACTORED DOWN AT 26-4.4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.									
I-J	0/44	-112.4	-112.4	0.08 (1)	10.00	L-H	0/2748	0.48 (1)	ALLOWABLE DEF. (TL) = U/360 (0.32")
R-B	-2631/0	0.0	0.0	0.19 (1)	6.37	B-Q	0/2659	0.47 (1)	CALCULATED VERT. DEF. (TL) = U/999 (0.32")
K-I	-4882/0	0.0	0.0	0.34 (1)	4.84	L-I	0/5302	0.94 (1)	CSI: TC=0.42/1.00 (D-F:1), BC=0.38/1.00 (L-M:1), WB=0.94/1.00 (L-L:1), SS=0.22/1.00 (C-D:1)
R-Q	0/0	-28.0	-28.0	0.04 (3)	10.00				DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
P-P	0/4749	-28.0	-28.0	0.27 (1)	10.00				COMP=1.00 SHEAR=1.00 TENS=1.00
P-O	0/4749	-28.0	-28.0	0.29 (1)	10.00				COMPANION LIVE LOAD FACTOR = 0.50
C-N	0/6319	-28.0	-28.0	0.38 (1)	10.00				TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
N-M	0/6319	-28.0	-28.0	0.38 (1)	10.00				NAIL VALUES
M-L	0/6319	-28.0	-28.0	0.38 (1)	10.00				PLATE GRIP(ORY) SHEAR SECTION
L-S	0/0	-28.0	-28.0	0.09 (2)	10.00				(PSI) (PLI) (PLI)
S-T	0/0	-28.0	-28.0	0.09 (2)	10.00				MAX MIN MAX MIN MAX MIN
T-K	0/0	-28.0	-28.0	0.09 (2)	10.00				MT20 618 354 1687 822 2284 1656
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE		PLATE PLACEMENT TOL. = 0.250 inches
H	23-8.5	-45	-50	—	FRONT	VERT	DEAD		PLATE ROTATION TOL. = 5.0 Deg.
H	23-8.5	-282	-282	—	FRONT	VERT	SNOW		JSI GRIP= 0.89 (C) (INPUT = 0.90)
L	22-10-8	-3063	-3063	—	BACK	VERT	WIND		JSI METAL= 0.95 (B) (INPUT = 1.00)
S	24-4.4	-40	-70	—	BACK	VERT	TOTAL		
T	26-4.4	-40	-70	—	BACK	VERT	TOTAL		

DWG NO. TAM 1838-88
 STRUCTURAL
 COMPONENT ONLY

[illegible]

[illegible]

[illegible]

JOB NAME: 287661 TRUSS NAME: T50 JOB DESC: OBC2012_6TCDL QUANTITY: 1 PLY: 1 TRUSS DESC: TRUSS DESC.

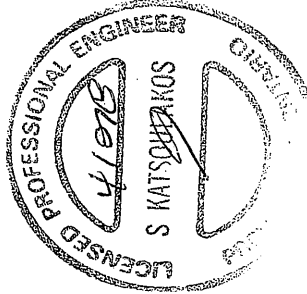
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Tue Apr 10 09:54:41 2018 Page 1
Tamarack Roof Truss, Burlington ID: YUV0g72vw_AGBwUxU7r3fzTqnH-DU72IT8fncAPQyovW62WVJtcmJap8384M7vzSABs

1-3-8 0-0 5-1-9 5-1-9 10-0-10 4-11-1 4-11-1 3-9-13 3-9-13 13-0-8 3-9-13 17-8-5 4-11-1 22-7-7 5-1-9 27-9-0 28-0-8 1-3-8

Scale = 1/4" = 1'-0"

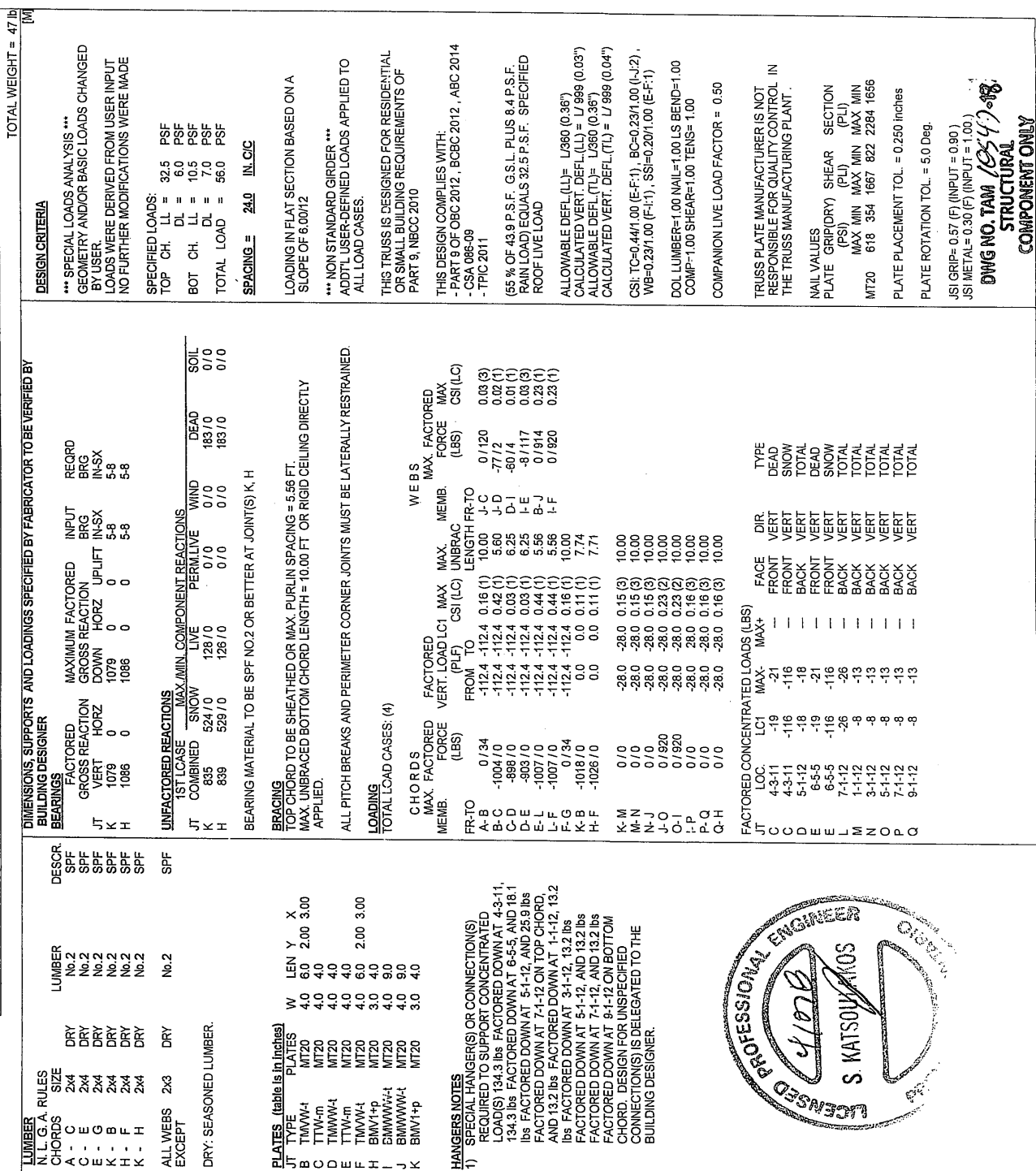
LUMBER									
N.L.G.A. RULES									
CHORDS									
DESOR.	LUMBER		W		LEN		Y		X
SPF	A - D	2x4	DRY	MIT20	5.0	6.0	1.75	2.75	5.0
SPF	A - E	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	E - H	2x4	DRY	MIT20	4.0	4.0	2.00	1.75	2.50
SPF	E - B	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	I - G	2x4	DRY	MIT20	5.0	6.0	1.75	2.75	5.0
SPF	I - L	2x4	DRY	MIT20	3.0	4.0	2.00	1.75	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - L	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0
SPF	L - I	2x4	DRY	MIT20	4.0	4.0	2.00	1.50	4.0

CONTINUED ON PAGE 2



DWG NO. TAM 10344-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 287661 Tamarack Roof Truss, Burlington	TRUSS NAME T52	QUANTITY 1	PLY 1	JOB DESC. OBC2012_6TCDL	DRWG NO.
Version 3.2005 Jan 6 2018 Mitek Industries Inc. Tue Apr 10 09:54:42 2018 Page 2 ID:YUVog72vv AGbWxU7i9tZTQnH-hghQwpmQhknZ77Ww0lPF3hJHYv2 bHopwgMzSABR					
FACTORED CONCENTRATED LOADS (LBS)		TYPE			
JT	LOC.	LC1	MAX-	MAX+	TOTAL
Y	21-10-4	-41	-72	—	
		FACE		DIR.	
		BACK		VERT	



287698

T90S

Tamarack Roof Truss, Burlington

Version 8,200 S Jan 6 2018 MITek Industries, Inc. Tue Apr 10 09:47:28 2018 Page 2

DRWG NO.

OB02012_6TCDL

JOB DESC.

OB02012_6TCDL

QUANTITY

PLY

1

2

ID:TXF22TXKE6EgJHd7eqqhSyRwy4-OBSMgPTXkK2HjsqI5IMEzpk4ZnKNLMgUPUHHyRWzSAID

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
Q	BMWV1	MT20	50	80	
R	BMWV1	MT20	30	60	
S	BMWV1	MT20	80	90	4.25 3.00
T	BMWV1	MT20	50	60	2.50 2.75
U	BMW1+p	MT20	30	60	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 181.0 lbs FACTORED DOWN AT 2-2, 12 AND 135.3 lbs FACTORED DOWN AT 3-3, 4 AND 135.3 lbs FACTORED DOWN AT 4-5, 2 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 1-3, 4, 69.9 lbs FACTORED DOWN AT 3-3, 4, AND 69.9 lbs FACTORED DOWN AT 4-5, 2, AND 4033.4 lbs FACTORED DOWN AT 50-7 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	1-3-4	-40	-70	-	BACK	VERT	TOTAL
Y	3-3-4	-40	-70	-	BACK	VERT	TOTAL
Z	4-5-2	-40	-70	-	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER

S. KATSOUKAKOS

PROVINCE OF ONTARIO

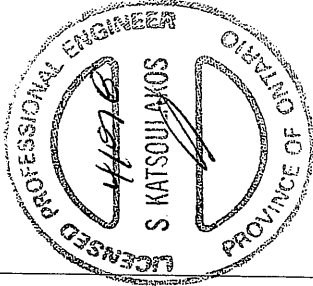
DWG NO. TAM/BS04-18

STRUCTURAL COMPONENT ONLY

18/2

JOB NAME	TRUSS NAME	JOB DESC.	PLY	QUANTITY	DRWG NO.
287698	T93	TRUSS DESC.	1	1	
Tamarack Roof Truss, Burlington					
Version 8.2005 Jan 6 2018 Mitek Industries, Inc. Tue Apr 10 09:47:29 2018 Page 2					
ID:TxIF22TxKE6EgJHd?eqohS/Rwy4-sN0k2lUAOeA8L0Pv4f4CMxdahkiY55XZixQVzvzSA[C					

FACTORED CONCENTRATED LOADS (LBS)					FACE	DIR.	TYPE
JT	LOC.	LC1	MAX-	MAX+			
P	6-11-4	-178	-178	-	BACK	VERT	TOTAL
Q	8-11-4	-178	-178	-	BACK	VERT	TOTAL
R	10-11-4	-178	-178	-	BACK	VERT	TOTAL
S	14-11-4	-178	-178	-	BACK	VERT	TOTAL
T	16-5-12	-178	-178	-	BACK	VERT	TOTAL
U	17-11-12	-178	-178	-	BACK	VERT	TOTAL
V	11-4	-95	-97	-	BACK	VERT	TOTAL
W	4-11-4	-94	-95	-	BACK	VERT	TOTAL
X	6-11-4	-94	-95	-	BACK	VERT	TOTAL
Y	8-11-4	-94	-95	-	BACK	VERT	TOTAL
Z	10-11-4	-94	-95	-	BACK	VERT	TOTAL
AA	12-11-4	-94	-95	-	BACK	VERT	TOTAL
AB	14-11-4	-94	-95	-	BACK	VERT	TOTAL
AC	16-5-12	-94	-95	-	BACK	VERT	TOTAL
AD	17-11-12	-94	-95	-	BACK	VERT	TOTAL



1622
DWG NO. TAM 18307-08
STRUCTURAL
COMPONENT ONLY

287698	T94X	1	1	TRUSS NAME	TRUSS DESC.	JOB DESC.	OBC2012_6TCDL	DRWG NO.
Tamarack Roof Truss, Burlington								
Version 8.200 S Jan 6 2018 Mittek Industries, Inc. Tue Apr 10 08:47:29 2018 Page 1								
ID: TxF22TxkE6EgJhd7eqatSYRwy4-sNOK2UUAOa8LDPv4lCMxdgKkpp5EMZxQVzyzSsAIC								
Scale = 1:24.2								

JOB NAME 287698	TRUSS NAME T95X	QUANTITY 4	PLY 1	JOB DESC. OBC2012_6TCDL	DRWG NO.																																																																																																																																																																																		
Tamarack Roof Truss, Burlington Version 8.200 S Jan 6 2018 Mittek Industries Inc. Tue Apr 10 09:47:29 2018 Page 1 ID: Txf22TxkE6EgJHd7eqofSyRwy4-sNOK2UAOeA8LPv4fCkMdkwkw6EgZjXQ/vzYSsAC Scale = 1:29.2																																																																																																																																																																																							
TOTAL WEIGHT = 4 X 47 = 189 lb [M/F]																																																																																																																																																																																							
DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF BOT CH. LL = 8.0 PSF DL = 10.5 PSF TOTAL LOAD = 50.0 PSF SPACING = 24.0 IN. O.C. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, CBC 2012, ABC 2014 - CSA 086.09 - TPC 2011 (55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEF. (LL) = L/360 (0.37") CALCULATED VERT. DEF. (LL) = L/999 (0.04") ALLOWABLE DEF. (TL) = L/360 (0.37") CALCULATED VERT. DEF. (TL) = L/999 (0.07") CSI: TC=0.15/1.00 (A-B-1), BC=0.32/1.00 (G-H-2), WB=0.23/1.00 (E-G-1), SG=0.15/1.00 (G-H-3) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PL) (PSI) MAX MIN MAX MIN MT20 618 354 1657 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.75 (C) (INPUT = 0.90) JSI METAL= 0.31 (E) (INPUT = 1.00)																																																																																																																																																																																							
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table> <tr> <th>DESOR.</th><th>FACTORED GROSS REACTION</th><th>MAXIMUM FACTORED GROSS REACTION</th><th>INPUT BRG UPLIFT IN-SX</th><th>RECORD BRG IN-SX</th></tr> <tr> <td>JT</td><td>941</td><td>0</td><td>0</td><td>5-8</td></tr> <tr> <td>I</td><td>941</td><td>0</td><td>0</td><td>5-8</td></tr> <tr> <td>G</td><td>787</td><td>0</td><td>0</td><td>3-8</td></tr> </table> UNFACTORED REACTIONS <table> <tr> <th>1ST LCASE</th><th>MAX. MIN.</th><th>COMPONENT REACTIONS</th><th>WIND</th><th>DEAD</th><th>SOIL</th></tr> <tr> <td>JT</td><td>733</td><td>453 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr> <tr> <td>G</td><td>627</td><td>364 / 0</td><td>0 / 0</td><td>0 / 0</td><td>0 / 0</td></tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS <table> <tr> <th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (LBS)</th><th>MAX. FACTORED VERT. LOAD (LBS)</th><th>MAX. FACTORED HORIZ. LOAD (LBS)</th><th>MAX. FACTORED MOMENT (LBS-FT)</th><th>MAX. FACTORED CSI (LC)</th></tr> <tr> <td>FR-TO</td><td>0 / 43</td><td>-112.4</td><td>-112.4</td><td>0.15 (1)</td><td>10.00</td><td>C-H -101 / 45</td></tr> <tr> <td>A-B</td><td>0 / 20</td><td>-112.4</td><td>-112.4</td><td>0.12 (1)</td><td>10.00</td><td>H-D 0 / 394</td></tr> <tr> <td>B-C</td><td>-610 / 0</td><td>-112.4</td><td>-112.4</td><td>0.10 (1)</td><td>6.25</td><td>H-E -196 / 13</td></tr> <tr> <td>C-D</td><td>-613 / 0</td><td>-112.4</td><td>-112.4</td><td>0.11 (1)</td><td>6.25</td><td>I-C -847 / 0</td></tr> <tr> <td>D-E</td><td>0 / 20</td><td>-112.4</td><td>-112.4</td><td>0.14 (1)</td><td>10.00</td><td>E-G -871 / 0</td></tr> <tr> <td>E-F</td><td>-286 / 0</td><td>0.0</td><td>0.0</td><td>0.03 (1)</td><td>7.81</td><td></td></tr> <tr> <td>G-F</td><td>-128 / 0</td><td>0.0</td><td>0.0</td><td>0.01 (1)</td><td>7.81</td><td></td></tr> <tr> <td>I-H</td><td>0 / 567</td><td>-28.0</td><td>-28.0</td><td>0.31 (2)</td><td>10.00</td><td></td></tr> <tr> <td>H-G</td><td>0 / 637</td><td>-28.0</td><td>-28.0</td><td>0.32 (2)</td><td>10.00</td><td></td></tr> </table> WEBS <table> <tr> <th>MEMB.</th><th>FACTORED FORCE (LBS)</th><th>FACTORED VERT. LOAD (LBS)</th><th>MAX. FACTORED VERT. LOAD (LBS)</th><th>MAX. FACTORED HORIZ. LOAD (LBS)</th><th>MAX. FACTORED MOMENT (LBS-FT)</th><th>MAX. FACTORED CSI (LC)</th></tr> <tr> <td>FR-TO</td><td>0 / 43</td><td>-112.4</td><td>-112.4</td><td>0.15 (1)</td><td>10.00</td><td>C-H -101 / 45</td></tr> <tr> <td>A-B</td><td>0 / 20</td><td>-112.4</td><td>-112.4</td><td>0.12 (1)</td><td>10.00</td><td>H-D 0 / 394</td></tr> <tr> <td>B-C</td><td>-610 / 0</td><td>-112.4</td><td>-112.4</td><td>0.10 (1)</td><td>6.25</td><td>H-E -196 / 13</td></tr> <tr> <td>C-D</td><td>-613 / 0</td><td>-112.4</td><td>-112.4</td><td>0.11 (1)</td><td>6.25</td><td>I-C -847 / 0</td></tr> <tr> <td>D-E</td><td>0 / 20</td><td>-112.4</td><td>-112.4</td><td>0.14 (1)</td><td>10.00</td><td>E-G -871 / 0</td></tr> <tr> <td>E-F</td><td>-286 / 0</td><td>0.0</td><td>0.0</td><td>0.03 (1)</td><td>7.81</td><td></td></tr> <tr> <td>G-F</td><td>-128 / 0</td><td>0.0</td><td>0.0</td><td>0.01 (1)</td><td>7.81</td><td></td></tr> <tr> <td>I-H</td><td>0 / 567</td><td>-28.0</td><td>-28.0</td><td>0.31 (2)</td><td>10.00</td><td></td></tr> <tr> <td>H-G</td><td>0 / 637</td><td>-28.0</td><td>-28.0</td><td>0.32 (2)</td><td>10.00</td><td></td></tr> </table>						DESOR.	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG UPLIFT IN-SX	RECORD BRG IN-SX	JT	941	0	0	5-8	I	941	0	0	5-8	G	787	0	0	3-8	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL	JT	733	453 / 0	0 / 0	0 / 0	0 / 0	G	627	364 / 0	0 / 0	0 / 0	0 / 0	MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)	MAX. FACTORED CSI (LC)	FR-TO	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-H -101 / 45	A-B	0 / 20	-112.4	-112.4	0.12 (1)	10.00	H-D 0 / 394	B-C	-610 / 0	-112.4	-112.4	0.10 (1)	6.25	H-E -196 / 13	C-D	-613 / 0	-112.4	-112.4	0.11 (1)	6.25	I-C -847 / 0	D-E	0 / 20	-112.4	-112.4	0.14 (1)	10.00	E-G -871 / 0	E-F	-286 / 0	0.0	0.0	0.03 (1)	7.81		G-F	-128 / 0	0.0	0.0	0.01 (1)	7.81		I-H	0 / 567	-28.0	-28.0	0.31 (2)	10.00		H-G	0 / 637	-28.0	-28.0	0.32 (2)	10.00		MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)	MAX. FACTORED CSI (LC)	FR-TO	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-H -101 / 45	A-B	0 / 20	-112.4	-112.4	0.12 (1)	10.00	H-D 0 / 394	B-C	-610 / 0	-112.4	-112.4	0.10 (1)	6.25	H-E -196 / 13	C-D	-613 / 0	-112.4	-112.4	0.11 (1)	6.25	I-C -847 / 0	D-E	0 / 20	-112.4	-112.4	0.14 (1)	10.00	E-G -871 / 0	E-F	-286 / 0	0.0	0.0	0.03 (1)	7.81		G-F	-128 / 0	0.0	0.0	0.01 (1)	7.81		I-H	0 / 567	-28.0	-28.0	0.31 (2)	10.00		H-G	0 / 637	-28.0	-28.0	0.32 (2)	10.00	
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G	787	0	0	3-8																																																																																																																																																																																			
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A-B	0 / 20	-112.4	-112.4	0.12 (1)	10.00	H-D 0 / 394																																																																																																																																																																																	
B-C	-610 / 0	-112.4	-112.4	0.10 (1)	6.25	H-E -196 / 13																																																																																																																																																																																	
C-D	-613 / 0	-112.4	-112.4	0.11 (1)	6.25	I-C -847 / 0																																																																																																																																																																																	
D-E	0 / 20	-112.4	-112.4	0.14 (1)	10.00	E-G -871 / 0																																																																																																																																																																																	
E-F	-286 / 0	0.0	0.0	0.03 (1)	7.81																																																																																																																																																																																		
G-F	-128 / 0	0.0	0.0	0.01 (1)	7.81																																																																																																																																																																																		
I-H	0 / 567	-28.0	-28.0	0.31 (2)	10.00																																																																																																																																																																																		
H-G	0 / 637	-28.0	-28.0	0.32 (2)	10.00																																																																																																																																																																																		

| LUMBER N. L. G. A. RULES | CHORDS | SIZE | DRY | W | LEN | Y | X | |----------|------|-----|-----|-----|---|------| | A - D | 2x4 | DRY | 3.0 | 4.0 | | | | D - F | 2x4 | DRY | 4.0 | 4.0 | | | | I - B | 2x4 | DRY | 4.0 | 4.0 | | 2.25 | | G - F | 2x4 | DRY | 4.0 | 4.0 | | 2.00 | | I - G | 2x4 | DRY | 3.0 | 4.0 | | | | I - G | 2x4 | DRY | 4.0 | 4.0 | | | | ALL WEBS | 2x3 | DRY | 4.0 | 6.0 | | | | EXCEPT | | | 4.0 | 6.0 | | | DRY: SEASONED LUMBER. PLATES (table is in inches) | JT TYPE | PLATES | W | LEN | Y | X | |---------|--------|------|-----|-----|---| | B | TMV-p | MT20 | 3.0 | 4.0 | | | C | TMW-i | MT20 | 4.0 | 4.0 | | | D | TTW-p | MT20 | 4.0 | 4.0 | | | E | TMW-i | MT20 | 4.0 | 4.0 | | | F | TMV-p | MT20 | 3.0 | 4.0 | | | G | BMV-i | MT20 | 4.0 | 4.0 | | | H | BMW-i | MT20 | 4.0 | 6.0 | | | I | BMV-i | MT20 | 4.0 | 4.0 | | | | | | | |
| DWG NO. TAM/18509-18 STRUCTURAL COMPONENT ONLY | | | | | |

JOB NAME	TRUSS NAME	JOB DESC.	OBC2012_6TCDL	DRWG NO.
287699	T96S	TRUSS DESC.		
Tamarack Roof Truss, Burlington				
QUANTITY	PLY	ID:xBzCCx2UdwA5LMzWfUKP4zTT?R-PLqmNwb78_1mDs3wgsKPlc9o3EAsQmRd9DdzSAG5		
1	2	Version 3.200 S Jan 6 2018 MTEK Industries, Inc. Tue Apr 10 09:49:44 2018 Page 2		

PLATES (table is in inches)

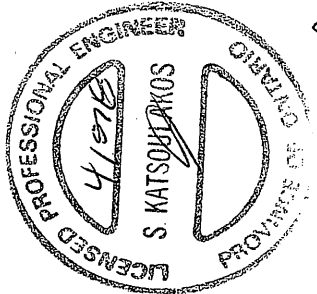
JT TYPE	PLATES	W	LEN	Y	X
Q BS-t	MT20	5.0	6.0		
R BMW+H	MT20	3.0	6.0		
S BMW+H	MT20	8.0	9.0		
T BMW-t	MT20	5.0	6.0	2.50	2.25
U-BMW+P	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 238.0 lbs FACTORED DOWN AT 2-11-8, AND 142.7 lbs FACTORED DOWN AT 2-11-6, AND 135.3 lbs FACTORED DOWN AT 4-5-2 ON TOP CHORD, AND 71.4 lbs FACTORED DOWN AT 11-4, 69.9 lbs FACTORED DOWN AT 2-11-4, AND 69.9 lbs FACTORED DOWN AT 4-5-2, AND 4031.5 lbs FACTORED DOWN AT 5-0-7 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
X	4-5-2	-40	-70	--	BACK	VERT	TOTAL



Part 1
DWG NO. TAM 1092B-18
STRUCTURAL
COMPONENT ONLY

PLATES (table is in inches)									
JT	TYPE	W	LEN	Y	X				
PLATES									
B	TMW+p	5.0	8.0	1.25	3.50				
C	TMW+p	6.0	9.0	1.25	3.50				
D	TMW+m	6.0	9.0	Edge	1.75				
E	TTW+h	6.0	9.0	Edge	6.0				
F	TMW+h	5.0	6.0	2.50	1.25				
G	TTW+n	5.0	6.0	Edge					
F	TMW+n	4.0	4.0	Edge					
G	TMW+h	5.0	6.0	Edge					
H	TTW+n	5.0	6.0	Edge					
I	TMW+n	4.0	4.0	2.50	1.25				
J	TMW+p	5.0	8.0	1.25	3.50				
K	TMW+p	5.0	8.0	2.00	3.50				
L	BMW+p	5.0	4.0	2.00	Edge				
M	BMW+h	5.0	6.0	2.50	2.75				
N	BMWWM-h	5.0	4.0	2.00	2.00				
O	BS-h	3.0	6.0	6.0	2.00				
P	BMWWM-h	3.0	6.0	2.25	1.50				
Q	BS-h	3.0	6.0	2.25	1.50				
R	BMWWM-h	3.0	6.0	2.25	4.50				
S	BMWWM-h	4.0	6.0	2.00	2.25				
T	BMW+p	4.0	4.0						

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= U/360 (1.10")

CALCULATED VERT. DEFL.(LL) = U/998 (0.22")

ALLOWABLE DEFL.(TL)= U/360 (1.10")

CALCULATED VERT. DEFL.(TL) = U/999 (0.37")

CSI: TC=0.531/0 (C-D-1) BC=0.571/0 (P-R-1), WB=0.621/0 (D-R-1), SS1=0.231/0 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-R, G-N, E-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING TOTAL LOAD CASES: (4)

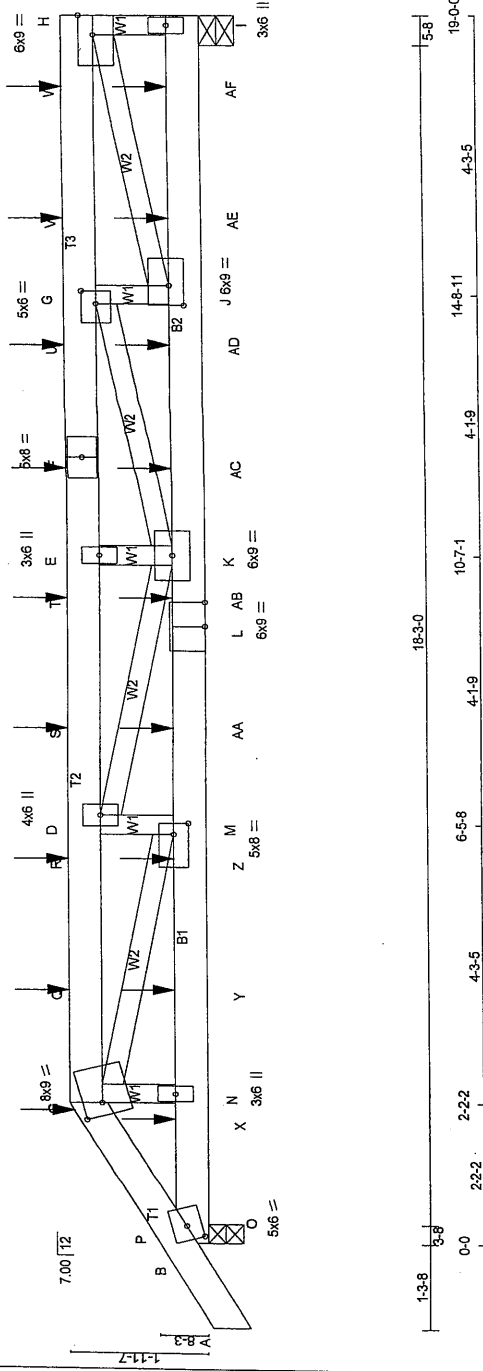
CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD/L1 (PLF)	MAX. UNBRACED LENGTH FR-TO (LBS)	MAX. FACTORED FORCE (LBS)	CS (LC)
FR-TO		FROM TO			
A-B	0.150	-112.4 -112.4	0.15 (1)	10.00	S-C -387 / 0
C-B	-23190 / 0	-112.4 -112.4	0.33 (1)	4.21	C-R 0 / 1969
D-C	-3199 / 0	-112.4 -112.4	0.53 (1)	3.41	R-D -3093 / 0
E-E	-4202 / 0	-112.4 -112.4	0.36 (1)	3.19	P-F 0 / 1283
D-F	-2514 / 0	-112.4 -112.4	0.24 (1)	4.16	P-G 0 / 179
F-G	-1916 / 0	-112.4 -112.4	0.19 (1)	4.71	N-H -630 / 0
F-G	-1663 / 0	-112.4 -112.4	0.18 (1)	4.99	G-N 0 / 1001
G-H	-1663 / 0	-112.4 -112.4	0.47 (1)	4.13	N-I -323 / 0
H-I	-2201 / 0	-112.4 -112.4	0.51 (1)	3.97	M-I -308 / 66
I-J	-2390 / 0	-112.4 -112.4	0.51 (1)	3.97	N-I -308 / 66

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NAIL VALUES		GRIP (DRY)		SHEAR		SECTION	
PLATE	(PSI)	MAX	MIN	MAX	MIN	(PL)	(PL)
MT20	618	354	1667	822	2284	1656	
PLATE PLACEMENT TOL. = 0.250 inches							
PLATE ROTATION TOL. = 5.0 Deg.							
JSI GRIP= 0.90 (N) (INPUT = 0.90.)							
JSI METAL= 0.90 (Q) (INPUT = 1.00.)							

DWG NO. TAM 18529 418
 STRUCTURAL
 COMPONENT ONLY

A circular professional engineer seal for the State of New York. The outer ring contains the text "LICENSED PROFESSIONAL ENGINEER" at the top and "STATE OF NEW YORK" at the bottom. Inside the ring, the name "S. KATSOUKIS" is written in a bold, sans-serif font. To the left of the name, the license number "41019" is handwritten. To the right of the name, there is a stylized graphic of a bridge or arch.

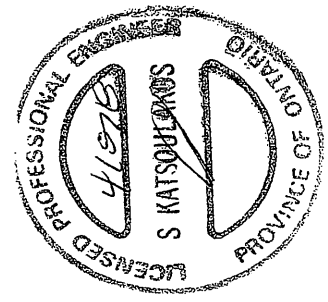
[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	OBC2012_6TCDL	DRWG NO.
287699	T100	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

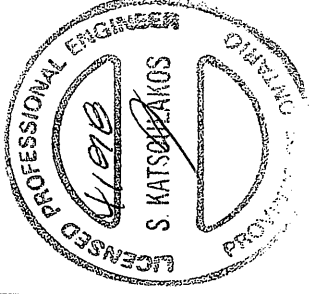
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Tue Apr 10 09:49:45 2018 Page 2
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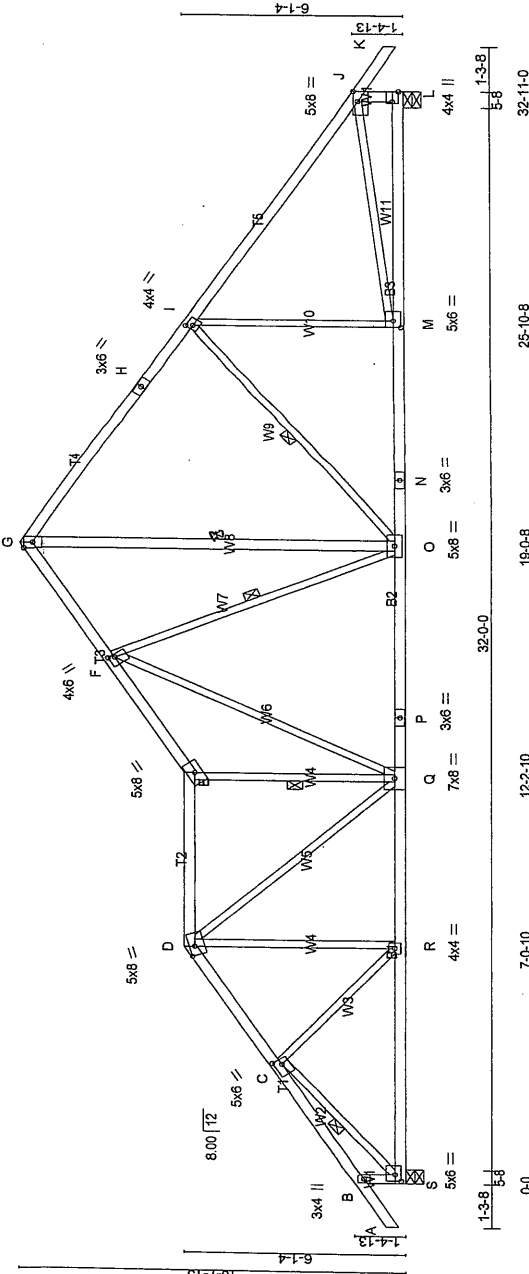
FACTORED CONCENTRATED LOADS (LBS)					TYPE
JT	LOC.	LC1	MAX-	MAX+	
S	7-11-4	-128	-128	—	TOTAL
T	9-11-4	-128	-128	—	TOTAL
U	13-11-4	-128	-128	—	TOTAL
-V-	15-11-4	-128	-128	—	TOTAL
W	17-11-4	-128	-128	—	TOTAL
X	1-11-4	-74	-74	—	TOTAL
Y	3-11-4	-74	-74	—	TOTAL
Z	5-11-4	-74	-74	—	TOTAL
AA	7-11-4	-74	-74	—	TOTAL
AB	9-11-4	-74	-74	—	TOTAL
AC	11-11-4	-74	-74	—	TOTAL
AD	13-11-4	-74	-74	—	TOTAL
AE	15-11-4	-74	-74	—	TOTAL
AF	17-11-4	-74	-74	—	TOTAL

FACE	DIR.
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT
BACK	VERT



22
DWG NO. TAM 18532-08
STRUCTURAL
COMPONENT ONLY

JOB NAME 287661	TRUSS NAME T101S	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	OBC2012_6TCDL	DRWG NO.																																										
Tamarack Roof Truss, Burlington																																																
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Tue Apr 10 09:54:43 2018 Page 2																																																
ID:YUVog72vv_AGbWuXU7isizTQntH-9Fp79kPA_sZPlak4DX_xTbefsOnr7RWSZTCozSABQ																																																
PLATES (table is in inches) <table><thead><tr><th>JT</th><th>TYPE</th><th>PLATES</th><th>W</th><th>LEN</th><th>Y</th><th>X</th></tr></thead><tbody><tr><td>P</td><td>BMWWMH</td><td>MT20</td><td>5.0</td><td>8.0</td><td></td><td></td></tr><tr><td>Q</td><td>BSH</td><td>MT20</td><td>5.0</td><td>6.0</td><td></td><td></td></tr><tr><td>R</td><td>BMWWMH</td><td>MT20</td><td>6.0</td><td>12.0</td><td>2.50</td><td>4.00</td></tr><tr><td>S</td><td>BMWWMH</td><td>MT20</td><td>8.0</td><td>9.0</td><td>4.50</td><td>3.25</td></tr><tr><td>T</td><td>BMVHP</td><td>MT20</td><td>3.0</td><td>6.0</td><td></td><td></td></tr></tbody></table> HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 141.8 lbs FACTORED DOWN AT 4-0-11, AND 327.0 lbs FACTORED DOWN AT 4-0-11 ON TOP CHORD, AND 68.9 lbs FACTORED DOWN AT 1-0-12, 68.9 lbs FACTORED DOWN AT 3-0-12, AND 68.9 lbs FACTORED DOWN AT 4-2-4, AND 4029.2 lbs FACTORED DOWN AT 5-0-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.							JT	TYPE	PLATES	W	LEN	Y	X	P	BMWWMH	MT20	5.0	8.0			Q	BSH	MT20	5.0	6.0			R	BMWWMH	MT20	6.0	12.0	2.50	4.00	S	BMWWMH	MT20	8.0	9.0	4.50	3.25	T	BMVHP	MT20	3.0	6.0		
JT	TYPE	PLATES	W	LEN	Y	X																																										
P	BMWWMH	MT20	5.0	8.0																																												
Q	BSH	MT20	5.0	6.0																																												
R	BMWWMH	MT20	6.0	12.0	2.50	4.00																																										
S	BMWWMH	MT20	8.0	9.0	4.50	3.25																																										
T	BMVHP	MT20	3.0	6.0																																												
 Per <i>[Signature]</i> DWG NO. TAM/0549-18 STRUCTURAL COMPONENT ONLY																																																



TOTAL WEIGHT = 2 X 152 = 305 [NIP]

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. DL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 96.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF'L (LL) = L/960 (1.10")
CALCULATED VERT. DEF'L (LL) = L/989 (0.23")
ALLOWABLE DEF'L (LL) = L/960 (1.10")
CALCULATED VERT. DEF'L (LL) = L/988 (0.40")

CSI: TC=1.00/1.00 (I-J-1), BC=0.57/1.00 (M-O-2), WB=0.75/1.00 (F-O-1), SS=0.32/1.00 (I-J-1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIPDRY SHEAR SECTION (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)

JSI METAL= 0.75 (P) (INPUT = 1.00)

DWG NO. TAM 1055 / 08
STRUCTURAL
COMPONENT ONLY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION		INPUT BRG. IN-SX	RECORD BRG. IN-SX
		VERT. DOWN	HORIZ. UP/LIFT		
JT	2466	0	0	5-8	5-8
L	2466	0	0	5-8	5-8

UNFACTORED REACTIONS	1ST LOOSE COMBINED	MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
JT	1948	1158 / 0	346 / 0	0 / 0	444 / 0	0 / 0
L	1948	1158 / 0	346 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q, F-O, I-O, C-S, G-D
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			FACTORED			MAX. FACTORED			WEBS		
	FR-TO	MAX. FORCE (LBS)	VERT. LOAD (LBS)	FROM TO	FACTORED (PLF)	CS1 (LC)	MAX. UNBRACED LENGTH (FT)	MEMB. FORCE (LBS)	MAX. FORCE (LBS)	CS1 (LC)	MAX. FORCE (LBS)	
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-R	0 / 197	0.04 (2)				
B-C	0 / 22	-112.4	-112.4	0.19 (1)	10.00	R-D	0 / 221	0.05 (3)				
C-D	-2707 / 0	-112.4	-112.4	0.24 (1)	4.03	D-Q	0 / 1248	0.28 (1)				
D-E	-3053 / 0	-112.4	-112.4	0.52 (1)	3.50	Q-E	-2481 / 0	0.62 (1)				
E-F	-3688 / 0	-112.4	-112.4	0.34 (1)	3.42	F-Q	0 / 1825	0.41 (1)				
F-G	-2200 / 0	-112.4	-112.4	0.23 (1)	4.40	F-O	-1428 / 0	0.75 (1)				
G-H	-2226 / 0	-112.4	-112.4	0.87 (1)	3.43	O-G	0 / 1976	0.32 (1)				
H-I	-2226 / 0	-112.4	-112.4	1.00 (1)	2.95	M-I	-151 / 2115	0.09 (1)				
I-J	-2772 / 0	-112.4	-112.4	0.15 (1)	10.00	S-C	-2987 / 0	0.58 (1)				
J-K	0 / 43	-112.4	-112.4	0.15 (1)	10.00	M-J	0 / 2377	0.53 (1)				
S-B	-313 / 0	0.0	0.0	0.03 (1)	7.81							
L-J	-2383 / 0	0.0	0.0	0.25 (1)	5.54							
S-R	0 / 2126	-28.0	-28.0	0.52 (2)	10.00							
R-Q	0 / 2232	-28.0	-28.0	0.55 (2)	10.00							
Q-P	0 / 2374	-28.0	-28.0	0.55 (1)	10.00							
P-O	0 / 2374	-28.0	-28.0	0.55 (1)	10.00							
O-N	0 / 2349	-28.0	-28.0	0.57 (2)	10.00							
N-M	0 / 2349	-28.0	-28.0	0.57 (2)	10.00							
M-L	0 / 0	-28.0	-28.0	0.34 (3)	10.00							

LUMBER		N.C.A. RULES		LUMBER		DESCR.	
CHORDS	SIZE	DRY	W	LEN	Y	X	
A - D	2x4	DRY	3.0	4.0			
D - E	2x4	DRY	5.0	6.0	2.50	2.00	
E - G	2x4	DRY	5.0	8.0	1.75	3.25	
G - H	2x4	DRY	5.0	8.0			
H - K	2x4	DRY	5.0	6.0	2.00	1.00	
K - B	2x4	DRY	4.0	6.0			
B - J	2x4	DRY	3.0	6.0			
J - L	2x4	DRY	4.0	4.0	2.00	1.50	
L - P	2x4	DRY	5.0	8.0	1.50	3.50	
P - N	2x4	DRY	4.0	4.0	2.00	Edge	
N - L	2x4	DRY	5.0	6.0	2.50		
ALL WEBS	2x3	DRY	3.0	6.0			
O - G	2x4	DRY	5.0	8.0			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.50	2.00
D	TTW+m	MT20	5.0	8.0	1.75	3.25
E	TTW-h	MT20	5.0	8.0		
F	TMVW+p	MT20	4.0	6.0	2.00	1.00
G	TTW-t	MT20	3.0	6.0		
H	TS-1	MT20	4.0	4.0	2.00	1.50
I	TMVW-1	MT20	5.0	8.0	1.50	3.50
J	TMV+p	MT20	4.0	4.0	2.00	Edge
L	BMV1+p	MT20	5.0	6.0	2.50	
M	BMVW-1	MT20	5.0	8.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-1	MT20	5.0	8.0		
P	BS-1	MT20	3.0	6.0		
Q	BMVW-1	MT20	7.0	8.0		
R	BMVW-1	MT20	4.0	4.0		
S	BMVW-1	MT20	5.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIWG NO. TAM 1055 / -18
STRUCTURAL
COMPONENT ONLY

**STRUCTURAL
COMPONENT ONLY**

LUS – Double Shear Joist Hangers



All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

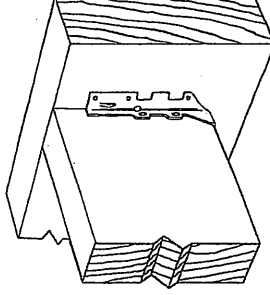
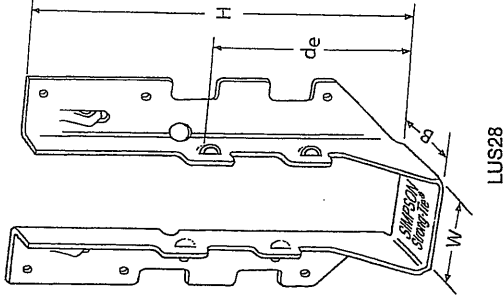
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire, $10d = 0.148"$ x $3"$ long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

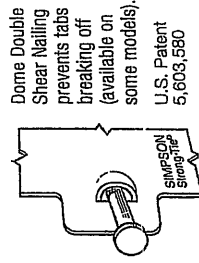
- These hangers cannot be modified



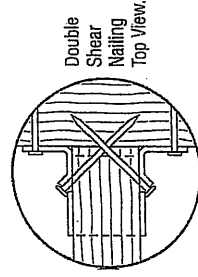
Typical LUS Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
						Face	Joist	D.Fir-L		S-P-F	
		W	H	B	d _g ¹			Uplift	Normal		
								(K _g =1.15)	(K _g =1.00)	(K _g =1.15)	Normal
LUS24	18	1 1/16	3 1/4	1 1/4	1 5/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/16	4 3/4	1 1/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/4	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/16	6 3/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 3/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/16	7 3/8	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

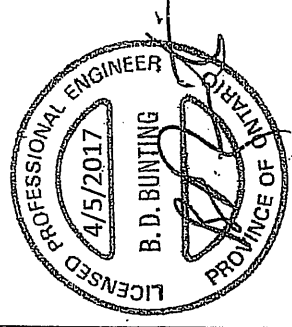
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,560



Double Shear Nailing
Top View.



LIMITED
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 2017.
The information is provided for general informational purposes only and is not intended to constitute a contract.
Contact Simpson Strong-Tie for current information and limited warranty. See strongtie.com
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T-SPEC1057 3/17, exp. 6/19

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HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

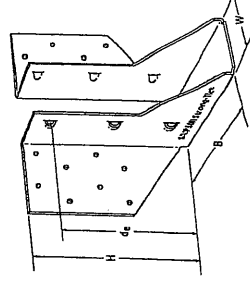
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

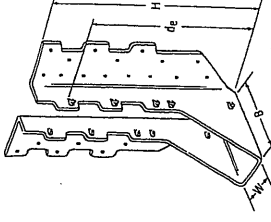
- Use all specified fasteners
- Nails: $16d = 0.162"$ dia. x $3\frac{1}{2}"$ long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

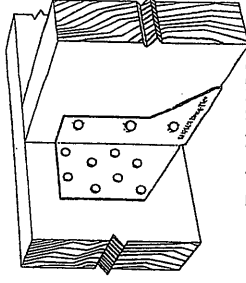
- See current catalogue for options



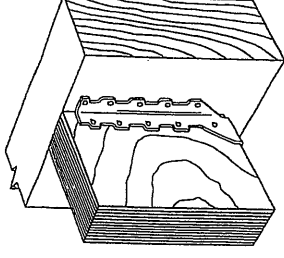
LJS26DS



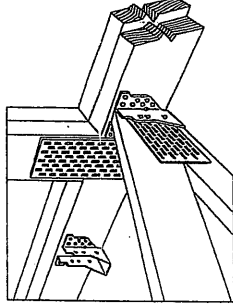
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



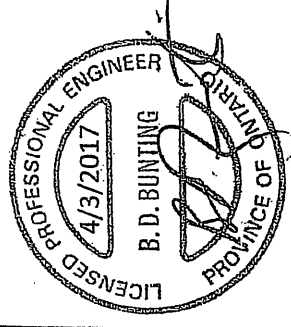
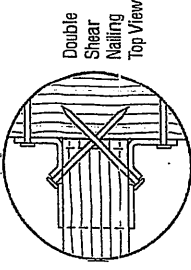
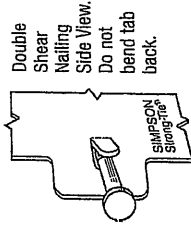
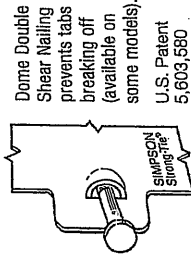
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d_e	Face	Joist	D.Fir-L ($K_F=1.15$)	Normal ($K_F=1.00$)	Uplift ($K_F=1.15$)	S-P-F Normal ($K_F=1.00$)
								lb.	lb.	lb.	lb.
LJS26DS	18	$1\frac{3}{16}$	5	3 $\frac{1}{2}$	4 $\frac{5}{16}$	(16) 16d	(6) 16d	2055	4285	1460	4115
HUS26	16	1 $\frac{1}{8}$	5 $\frac{1}{2}$	3	3 $\frac{1}{16}$	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 $\frac{1}{8}$	7 $\frac{1}{2}$	3	6 $\frac{1}{32}$	(22) 16d	(6) 16d	3605	5365	2675	4345
HUS210	16	1 $\frac{1}{8}$	9 $\frac{1}{2}$	3	7 $\frac{3}{32}$	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 $\frac{1}{16}$	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.



UNITED STATES DESIGN

This technical bulletin is effective July 1, 2019. It reflects information available as of March 1, 2018. This information is located periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and literature. © 2017 Simpson Strong-Tie Company, Inc.

1. SPEC HUS 17.37, 6/6/19

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HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

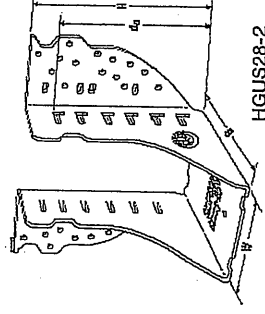
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

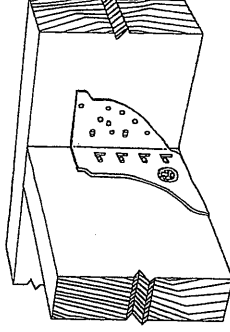
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

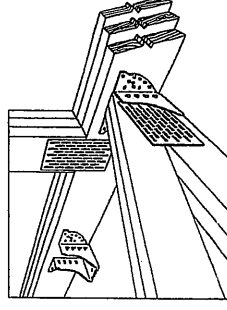
- See current catalogue for options



HGUS28-2



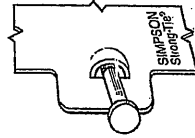
Typical HGUS Installation



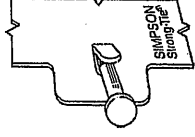
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners			Factored Resistance (lb.)		
		W	H	B	d _e ¹	Face	Joist		D.Fir-L Uplift (K _u =1.15)	Normal (K _u =1.00)	S-P-F Uplift (K _u =1.15)
HGUS26	12	1½	5¾	5	4½	(20) 16d	(8) 16d		2685	6625	2685
HGUS26-2	12	3¾	5¾	4	4½	(20) 16d	(8) 16d		4385	8950	3100
HGUS26-3	12	4¼	5¾	4	4½	(20) 16d	(8) 16d		4385	8950	3100
HGUS26-4	12	6¾	5¾	4	4½	(20) 16d	(8) 16d		4385	8950	3100
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d		3310	7675	3100
HGUS28-2	12	3¾	7¾	4	6½	(36) 16d	(12) 16d		6070	12980	4310
HGUS28-3	12	4¼	7¾	4	6½	(36) 16d	(12) 16d		6070	12980	4310
HGUS28-4	12	6¾	7¾	4	6½	(36) 16d	(12) 16d		6070	12980	4310
HGUS210-2	12	3¾	9¾	4	8½	(46) 16d	(16) 16d		6840	14645	4855
HGUS210-3	12	4¼	9¾	4	8½	(46) 16d	(16) 16d		6840	14645	4855
HGUS210-4	12	6¾	9¾	4	8½	(46) 16d	(16) 16d		6840	14645	4855
HGUS212-4	12	6¾	10¾	4	10½	(56) 16d	(20) 16d		7640	14995	5425
HGUS214-4	12	6¾	12¾	4	11½	(66) 16d	(22) 16d		10130	16400	7195

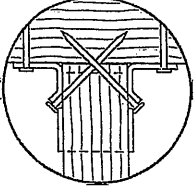
1. d_e is the distance from the seat of the hanger to the highest joist nail.



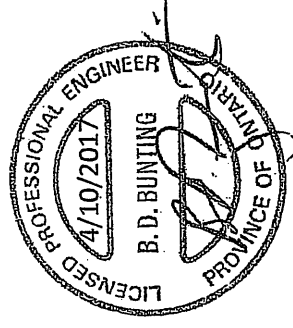
Dome Double Shear Nailing prevents tabs breaking off (available on some models),
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

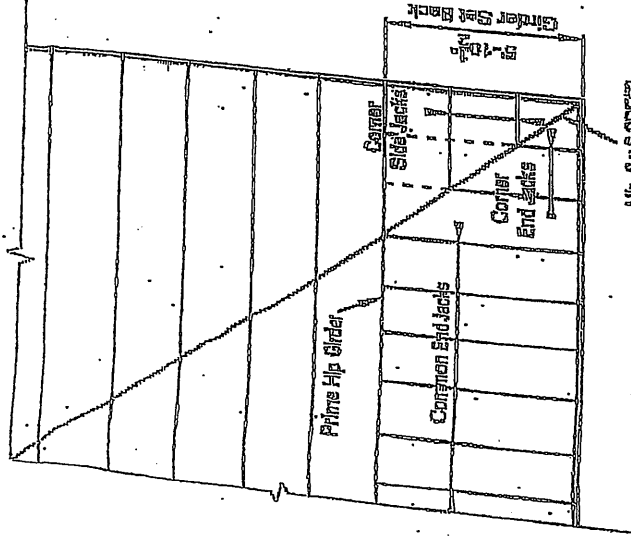
This technical bulletin is effective until June 30, 2019, or until superseded by a later revision. The information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty. See strongtie.com.

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HSPEC-HGUS17-0317 exp 6/19

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45° Hip End

Min. 2x6 SPF#2
Ridge Board

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

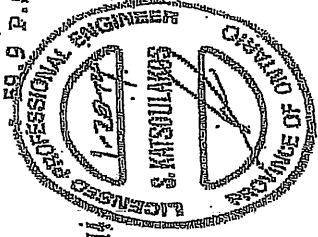
UNLESS OTHERWISE SHOWN

DESIGN LOADS:

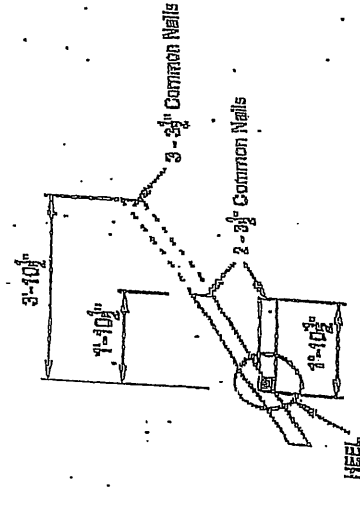
TOP CHORD LIVE LOAD = 39.4 P.S.F.
TOP CHORD DEAD LOAD = 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD = 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD = 7.0 P.S.F.

TOTAL LOAD

59.9 P.S.F.

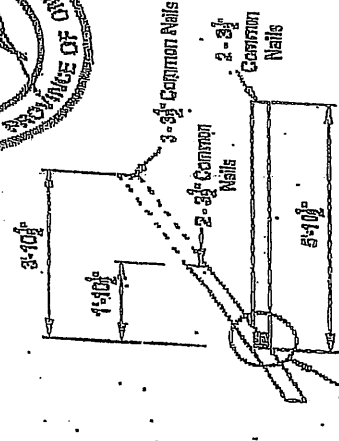


NO TAIL
STRAIGHT
CUT ONLY



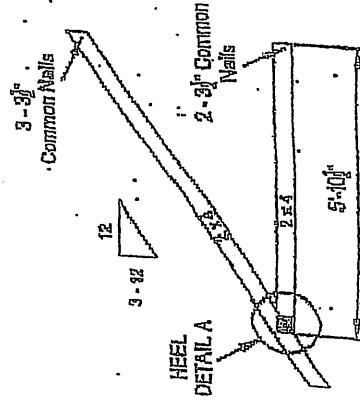
HEEL
DETAIL A

Corner Side Jacks



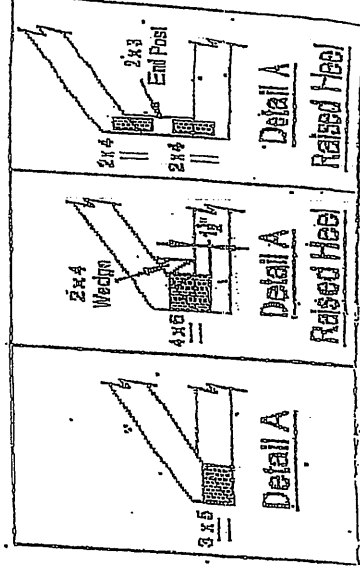
HEEL
DETAIL A

Corner End Jacks



HEEL
DETAIL A

Common End Jacks



DETAIL A

DETAIL A

DETAIL A

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)

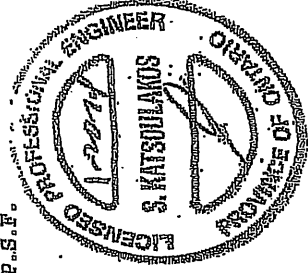
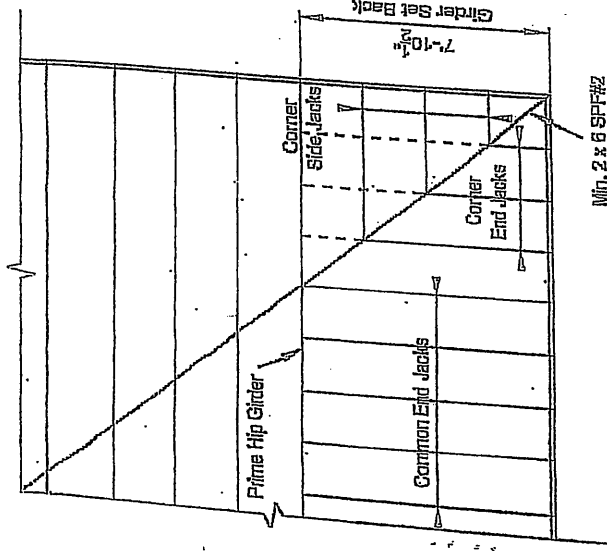
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

LUMBER SPECIFICATION

TOP CHORD : 2X4 SPF#2
BOTTOM CHORD : 2X4 SPF#2
WEBS : 2X3 SPF#2
UNLESS OTHERWISE SHOWN

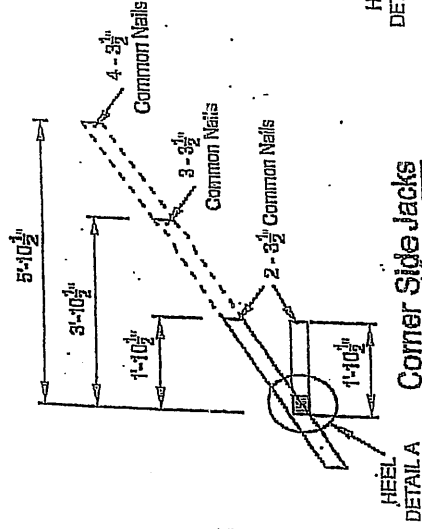
<u>DECKEN LOAD:</u>	
TOP CHORD LIVE LOAD	= 34.8 P.S.F.
TOP CHORD DEAD LOAD	= 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	= 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	= 7.0 P.S.F.
<u>TOTAL LOAD</u>	<u>= 44.8 P.S.F.</u>

DATE	DESCRIPTION	AMOUNT	TOTAL LOAD
10/1/20	...	...	...
10/2/20	...	...	...
10/3/20	...	...	...
10/4/20	...	...	...
10/5/20	...	...	...
10/6/20	...	...	...
10/7/20	...	...	...
10/8/20	...	...	...
10/9/20	...	...	...
10/10/20	...	...	...
10/11/20	...	...	...
10/12/20	...	...	...
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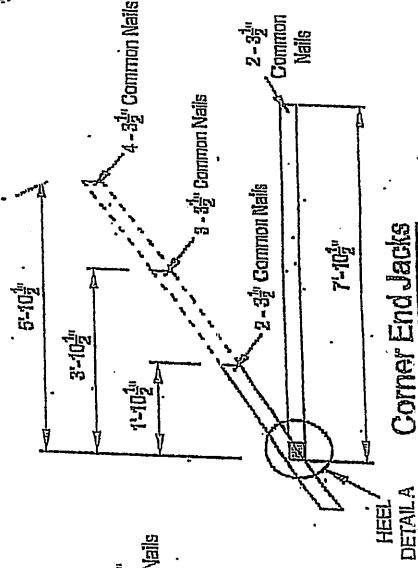
[illegible]

Min. 2 x 6 SPF#2
Ridge Board

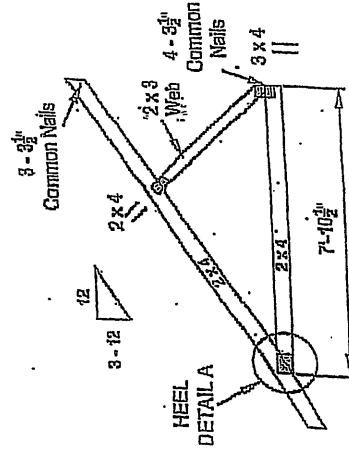
45° Hip End



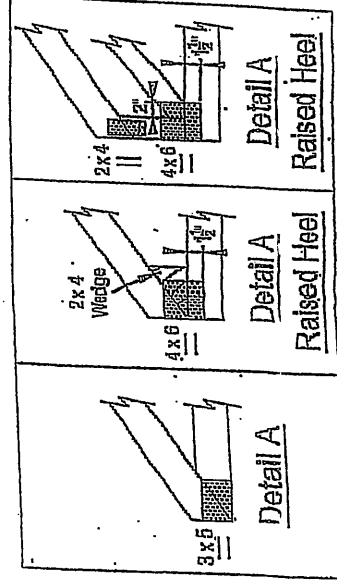
Corner Side Jacks



Corner End Jacks

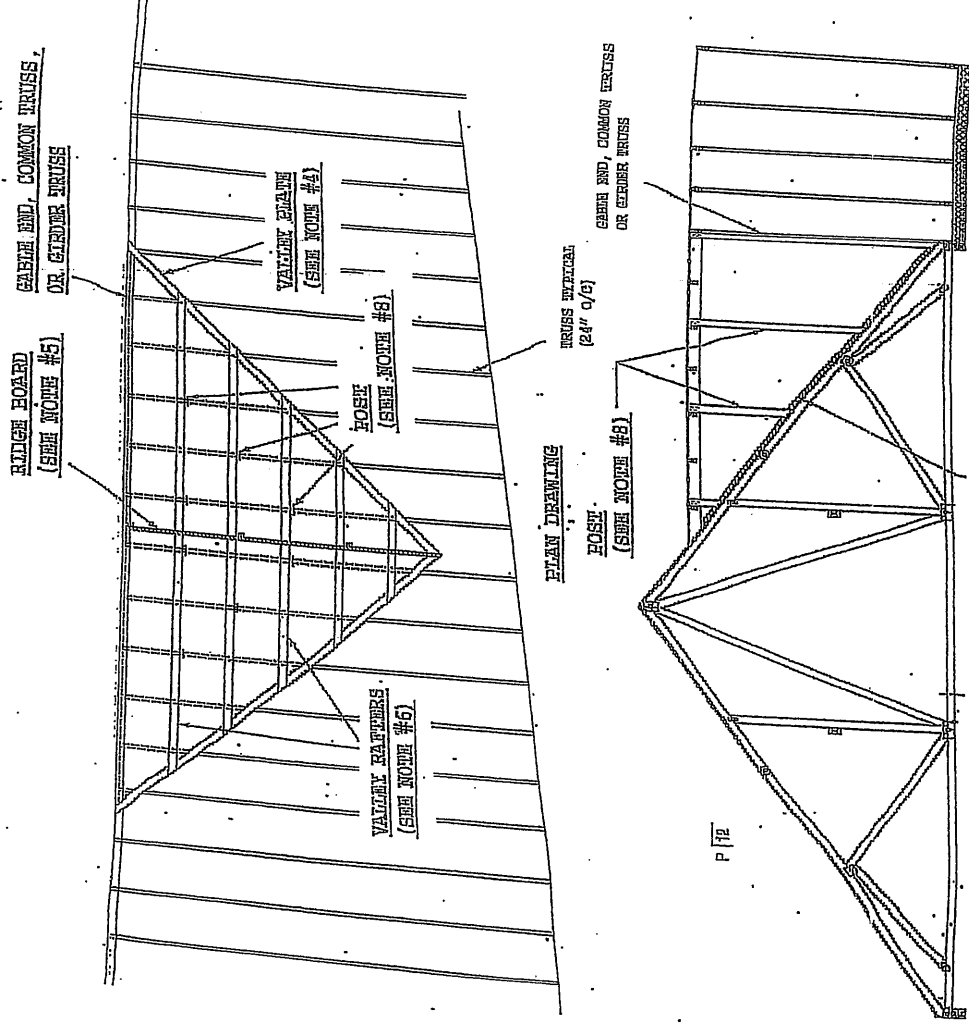


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FOUL TRUSS ENGINEERING PACKAGE)

CONVENTIONAL VALLEY FRAMING DETAIL

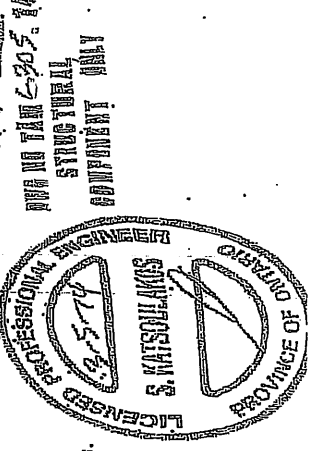


GENERAL SPECIFICATIONS:

- (1) MAKE THE MAIN TRUSSES BRACKET (CONSTRAINED), DEPLY SHEARING
- (2) BRACE TOP OF SUPPORTING (RARE) TRUSSES.
- (3) BRACE BOTTOM CHORD AND THE MEMBERS AS PER PRE-ENGINEERED TRUSS DETAILS.
- (4) DETAIL VALLEY RIDGE BY RUNNING A JOIST SERRING FROM THE UNDERSTANDING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEETING.
- (5) COMMON TRUSS TO 6 VALLEY PLANKS ON FEET. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" x 0.131") NAILS.
- (6) SET 2 x 5 1/2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 x 8 1/2 RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 x 4 POSTS SPACED 48" O/C. BRACE BOTTOM OF POST TO SET EVENLY ON THE SHEETING. FASTEN POST TO RIDGE WITH (4) 10d (3" x 0.131") NAILS. FASTEN POST TO ROOF SHEETING WITH (3) 10d (3" x 0.131") NAILS.
- (7) BRACE VALLEY RAFTERS FROM VALLEY PLANK TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE WITH (3) 16d (3.5" x 0.131") NAILS. FASTEN VALLEY RAFTER TO VALLEY PLANK WITH (4) 16d (3.5" x 0.131") NAILS.
- (8) SUPPORT THE VALLEY RAFTERS WITH 2 x 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSURE POSTS IN A BRACKETED FATHER AS SHOWN ON PLAN DRAWING. FASTEN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO JOIST WITH (4) 10d (3" x 0.131") NAILS. FASTEN POST TO VALLEY SHEETING WITH 2 x 4 1/2 SET OR BETTER. POSTS EXCEEDING 75' IN HEIGHT SHOULD BE INTERSPACED TO 4 x 4 SET, OR BETTER, OR BE PRE-ENGINEERED 10d (3" x 0.131") NAILS AT 6' O/C.
- (9) BRACE IN A MINIMUM 3/4" UNDER EDGE DISTANCE WHEN WALLING, NAIL SETTING SHOULD APPROXIMATE A MINIMUM 1-3/8" O/C OR MORE UNLESS NOTED OTHERWISE. AND CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT EDITION) AT ALL TIMES.

NOTES:

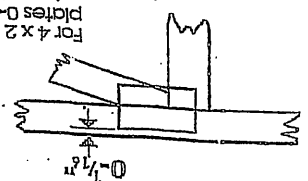
- (10) 48" O/C (MAXIMUM JOIST SPACING).
- (11) ROOF AREA LOAD = 24.3 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) REFER TO PRELIMINARY ONLY (ON-TO BUILDING CODE)
- (14) REFER TO PRELIMINARY ONLY (ON-TO BUILDING CODE)
- (15) WITH APPROVAL REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (16) FASTEN TRUSS BRACE (24" O/C MAX.)
- (17) WITH PRE-ENGINEERED TRUSS TRUSS CONNECTIONS TO BE MADE BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME
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Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated.
Dimensions are in ft-in-sixteenths or mm.
Apply plates to both sides of truss and fully embed teeth.
0-1/4"



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

Plate location details available in Mitek software or upon request.

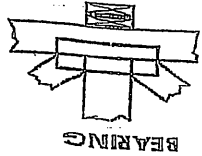
PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

4 x 4

LATERAL BRACING LOCATION

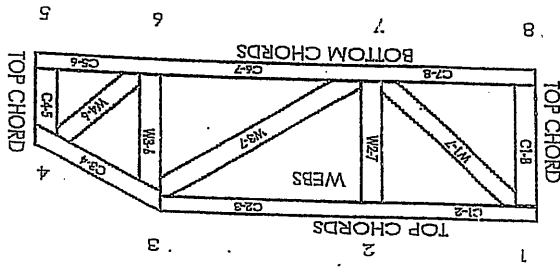
Indicated by symbol shown and/or output. Use T, L or Eliminator bracing if indicated.



Industry Standards:
Truss Design Procedures and Specifications For Light Metal Plate Connected Wood Trusses
Design Standard for Bracing Building Component Safety Information Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.
BCSI: DSB-89: Truss Design Procedures and Specifications

Numbering System

Dimensions shown in ft-in-sixteenths or mm (Drawings shown not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports: 11996-L, 10819-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet: MM-7473C rev. 10-08

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral bracing themselves may require bracing, or otherwise a T, L, or Eliminator bracing should be considered.
- Never exceed the design loadings shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPI.
- Design assumes trusses will be suitably protected from the environment in accord with TPI.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or pulins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPI quality criteria.

General Safety Notes
Failure to Follow Could Cause Property Damage or Personal Injury

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61
Glencoe, Ontario

NOL IMO

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

~~It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below.~~

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE ML-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpinc.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.

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