

Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	1	17		
J1DJ	16-00-00	9 1/2" NI-40x	2	4		
J2	14-00-00	9 1/2" NI-40x	1	26		
J3	12-00-00	9 1/2" NI-40x	1	15		
J4	10-00-00	9 1/2" NI-40x	1	5		
J5	6-00-00	9 1/2" NI-40x	1	3		
J6	4-00-00	9 1/2" NI-40x	1	1		
J7	2-00-00	9 1/2" NI-40x	1	4		
B1	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B14L	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B2	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B3	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B4	6-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B5	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		

Connector Summary						
Qty Manuf Product						
9	H1	IUS2.56/9.5				
2	H1	IUS2.56/9.5				
7	H1	IUS2.56/9.5				

TOWN OF BRADFORD WEST GWILLIMBURY BUILDING DEPARTMENT PLANS EXAMINED ONTARIO BUILDING CODE APPLIES

DATE: 2018-10-22 INSPECTOR: BG

SITE COPY



FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY ESTATES

MODEL: SD25-4 SONOMA 4

ELEVATION: A,B,C

LOT: 154

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. **CERAMIC TILE APPLICATION AS PER** O.B.C 9.30.6.

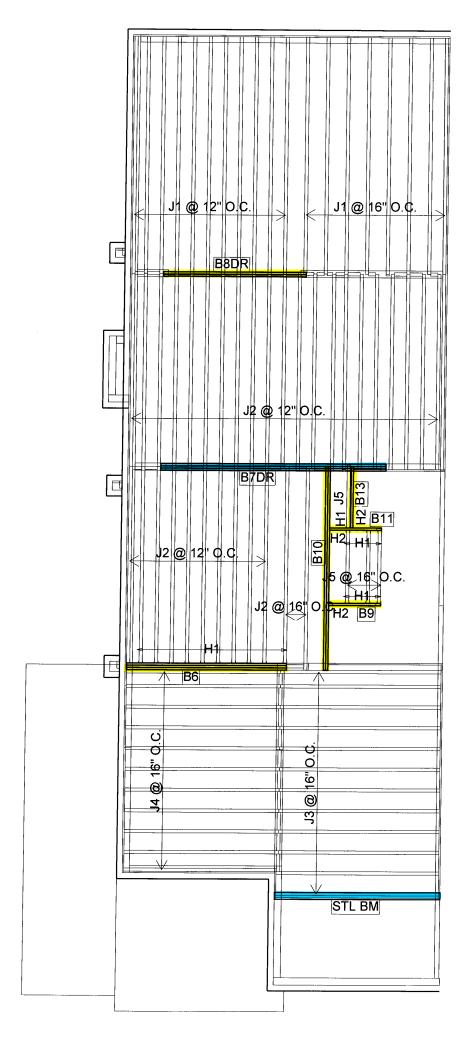
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

1st FLOOR



Products						
PlotID	Length	Product	Plies	Net Qty		
J1	16-00-00	9 1/2" NI-40x	1	19		
J2	14-00-00	9 1/2" NI-40x	1	33		
J3	12-00-00	9 1/2" NI-40x	1	12		
J4	10-00-00	9 1/2" NI-40x	1	11		
J5	6-00-00	9 1/2" NI-40x	1	4		
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3		
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2		
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1		
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3		

Connector Summary					
Qty Manuf Product					
7	H1	IUS2.56/9.5			
10	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			
2	H2	HUS1.81/10			





FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY ESTATES

MODEL: SD25-4 SONOMA 4

ELEVATION: A,B

LOT: 154

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

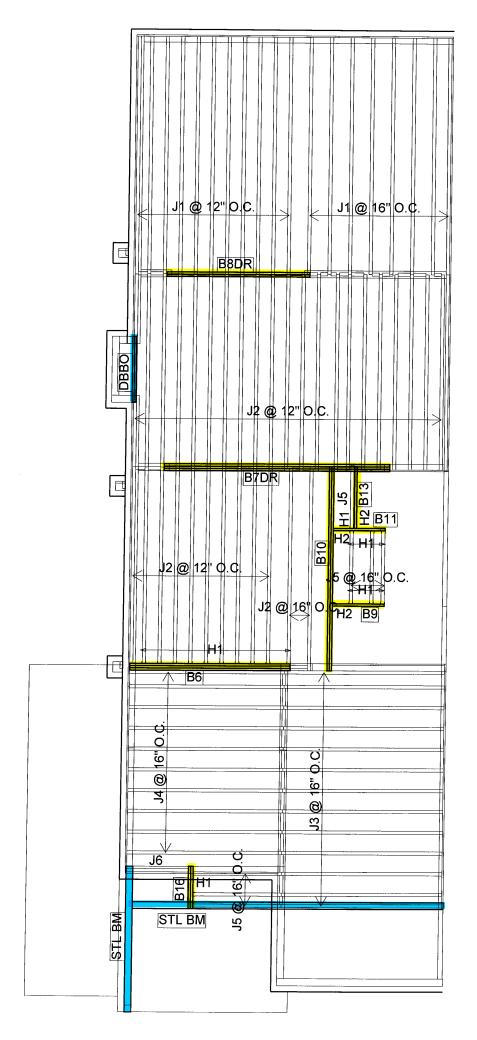
REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 fb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

2nd FLOOR



Products					
PlotID	Length	Product	Plies	Net Qty	
J1	16-00-00	9 1/2" NI-40x	1	19	
J2	14-00-00	9 1/2" NI-40x	1	33	
J3	12-00-00	9 1/2" NI-40x	1	13	
J4	10-00-00	9 1/2" NI-40x	1	10	
J5	6-00-00	9 1/2" NI-40x	1	7	
J6	4-00-00	9 1/2" NI-40x	1	1	
B10	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B6	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	3	3	
B8DR	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B11	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	
B13	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	
B9	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1	
B16	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2	
B7DR	16-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	3	3	

Connector Summary					
Qty Manuf Product					
7	H1	IUS2.56/9.5			
1	H1	IUS2.56/9.5			
10	H1	IUS2.56/9.5			
1	H2	HUS1.81/10			
2	H2	HUS1.81/10			



FROM PLAN DATED: NOV 2016

**BUILDER: BAYVIEW WELLINGTON** 

SITE: GREEN VALLEY ESTATES

MODEL: SD25-4 SONOMA 4

**ELEVATION:** C

LOT: 154

**CITY: BRADFORD** 

SALESMAN: M D DESIGNER: CZ REVISION:

NOTES:

REFER TO THE NORDIC **INSTALLATION GUIDE FOR PROPER** STORAGE AND INSTALLATION. **SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2** S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6 LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft

SUBFLOOR: 5/8" GLUED AND NAILED

DATE: 29/08/2017

2nd FLOOR

SITE COPY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1242)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:22

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

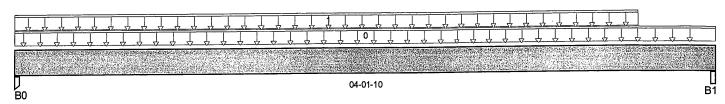
Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1242)

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Product Length = 04-01-10

Reaction Summary (Down / Uplift) (Ibs)							
Be aring	Live	De ad	Snow	Wind			
B0, 1-3/4"	485/0	252/0					
B1, 5-1/4"	553/0	287/0					

1.	ad Summary				Live	Dead	Snow Wind	Trib.
	g Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
ō	Us er Load	Unf. Lin. (lb/ft)	L 00-00-00	04-01-10	240	120		n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	03-08-06	12	6		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	917 ft-lbs	12,704 ft-lbs	7.2%	1	01-11-01
End Shear	534 lbs	5,785 lbs	9.2%	1	00-11-04
Total Load Defl.	L/999 (0.006")	n/a	n/a	4	01-11-01
Live Load Defl.	L/999 (0.004")	n/a	n/a	5	01-11-01
Max Defl.	0.006"	n/a	n/a	4	01-11-01
Span / Depth	4.6	n/a	n/a		00-00-00

Reari	ng Supports	Dim . (L x W)	Demand	De man d/ Re s istance Support	Demand/ Resistance Member	Material
B0	Post	1-3/4" x 1-3/4"	1,043 lbs	41.9%	27.9%	Unspecified
B1	Beam	5-1/4" x 1-3/4"	1,189 lbs	24.2%	10.6%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SY STEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

CONFORMS TO OBC 2012



DWG NO. TAM 9259 - 18 STRUCTURAL COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1241)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1241)

Specifier:

Designer: Company:

Misc:

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		] ] [ ] [ ] [ ] [ ] [ ] [ ] [ ]	
			And The Control of th
<b>⊠</b> B0		04-02-10	Д В1

Total Horizontal Product Length = 04-02-10

Reaction Summary (Down / Uplift) (Ibs)							
Be aring	Live	De ad	Snow	Wind			
B0, 4-3/8"	586/0	304/0		-			
B1, 1-3/4"	525/0	272/0	•				

10	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re f.	Start	En d	1.00	0.65	1.00	1.15	
Ō	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	04-02-10	240	120			n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	04-00-14	24	12			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,042 ft-lbs	12,704 ft-lbs	8.2%	1	02-02-10
End Shear	603 lbs	5,785 lbs	10.4%	1	01-01-14
Total Load Defl.	L/999 (0.008")	n/a	n/a	4	02-02-10
Live Load Defl.	L/999 (0.005")	n/a	n/a	5	02-02-10
îVlaxDefl.	0.008"	n/a	n/a	4	02-02-10
Span / Depth	4.8	n/a	n/a		00-00-00

					Demand/ Resistance	
Bearin	ng Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	1,259 lbs	30.8%	13.5%	Unspecified
B1	Post	1-3/4" x 1-3/4"	1,127 lbs	45.3%	30.2%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™. ALLJOIST®, BCRIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



TOWN OF OF OUR DWG NO. TAM 9260 - 18 STRUCTURAL COMPONENT ONLY

SITE COP



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1269)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

**Build 5033** Job Name:

Address: City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1269)

Specifier:

Designer: CZ

Company: Misc:

<b>V</b>	<b>1</b>	2/	3/
•	en de la companya de La companya de la co		
ј во		04-07-00	р В1

Total Horizonta	l Product I	Length:	= 04-07-00
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Reaction Summary (	Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 1-3/4"	442/0	232/0			
B1 3-1/2"	417/0	310/0			

				Live	Dead	Snow Wind	i rib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J4(i1346)	Conc. Pt. (lbs)	L 00-01-08	00-01-08	238	119		n/a
1 J4(i1334)	Conc. Pt. (lbs)	L 01-05-08	01-05-08	226	113		n/a
2 J5(i1303)	Conc. Pt. (lbs)	L 02-09-08	02-09-08	133	67		n/a
3 -	Conc. Pt. (lbs)	L 04-04-04	04-04-04	262	221		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	630 ft-lbs	12,704 ft-lbs	5%	1	01-05-08
End Shear	442 lbs	5,785 lbs	7.6%	1	00-11-04
Total Load Deff.	L/999 (0.006")	n/a	n/a	4	02-02-00
Live Load Defl.	Ľ/999 (0.004")	n/a	n/a	5	02-02-00
Max Defl.	0.006"	n/a	n/a	4	02-02-00
Span / Depth	5.4	n/a	n/a		00-00-00

Rearin	ng Supports	Dim.(LxW)	De man d	Resistance Support	Resistance Member	Material
B0	Post	1-3/4" x 1-3/4"	953 lbs	38.3%	25.5%	Un specified
B1	Post	3-1/2" x 1-3/4"	1,013 lbs	20.4%	13.6%	Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



S. KATSOUL

SITE COP

ONTHE STRUCTURAL -18 COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1580)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

**BC CALC® Design Report** 

Address: City, Province, Postal Code: BRADFORD,

Customer:

Build 5033

Job Name:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1580)

Specifier:

Designer: CZ Company:

Misc:

是是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们就是一个人,我们
Q4-08-08
80

Total Horizontal Product Length = 04-08-08

Reaction Summary (Dow Bearing	n / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 1-3/4"	26 / 0	25 / 0			
B1. 1-3/4"	26 / 0	25 / 0			

				Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15	
0 FC2 Floor Material	Unf. Lin. (lb/ft)	L 00-00-00	04-08-08	11	6			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	77 ft-lbs	12,704 ft-lbs	0.6%	1	02-04-04
End Shear	42 lbs	5,785 lbs	0.7%	1	00-11-04
Total Load Defl.	L/999 (0.001")	n/a	n/a	4	02-04-04
Live Load Defl.	L/999 (0")	n/a	n/a	5	02-04-04
Max Defl.	0.001"	n/a	n/a	4	02-04-04
Span / Depth	5.7	n/a	n/a		00-00-00

Do orin	a Supports	Dim. (L x W)	De man d	De mand/ Re sistance Su pport	De mand/ Resistance Member	Material
B0 B1	n <b>g Supports</b> Post Post	1-3/4" x 1-3/4" 1-3/4" x 1-3/4"	70 lbs 70 lbs	2.8% 2.8%	1.9% 1.9%	Un specified Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

**CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.





### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1343)

ort

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

\*

Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

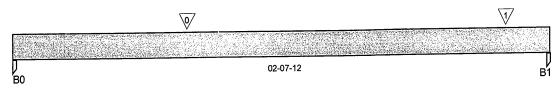
File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1343)

Specifier:

Designer: CZ

Company: Misc:



Total Horizontal Product Length = 02-07-12

Reaction Summary (	Down / Uplift) (lbs) Live	Dead	Snow	Wind	
B0, 3-1/2"	104/0	59 / 0			
B1 3-1/2"	318/0	259/0			

				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 J5(i1303)	Conc. Pt. (lbs)	L 00-10-04	00-10-04	133	67		n/a
1 -	Conc. Pt. (lbs)	L 02-05-02	02-05-02	289	238		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	141 ft-lbs	12,704 ft-lbs	1.1%	1	00-10-04
End Shear	141 lbs	5.785 lbs	2.4%	1	01-01-00
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-03-03
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-03-03
Max Defl.	0"	n/a	n/a	4	01-03-03
Span / Depth	2.8	n/a	n/a		00-00-00

Poorir	ng Supports	Dim . (L x W)	Demand	De man d/ Re sistance Support	Demand/ Resistance Member	Material
B0	Post	3-1/2" x 1-3/4"	229 lbs	4.6%	3.1%	Unspecified
B1	Post	3-1/2" x 1-3/4"	801 lbs	16.1%	10.7%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 9263 - 18
STRUCTURAL
COMPONENT ONLY



### Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i1554)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

Address: City, Province, Postal Code: BRADFORD,

Customer:

**Build 5033** 

Job Name:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

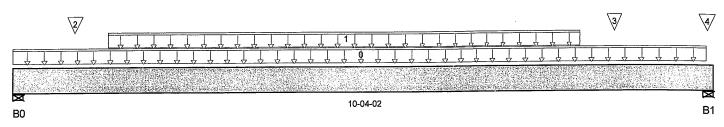
Description: Designs\Flush Beams\1st Floor\Flush Beams\86(i1554)

Specifier:

Designer: CZ

Company:

Misc:



### Total Horizontal Product Length = 10-04-02

Reaction Summary (Down	/ Uplift) (lbs) Live	Dead	Snow	Wind	
B0, 4-3/8"	1,212/0	679/0			
B1,7-1/4"	1,540 / 0	847/0			

10	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Ref	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	10-02-14	6	3			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	01-04-14	08-04-14	258	130			n/a
2	J2(i1450)	Conc. Pt. (lbs)	L	00-10-14	00-10-14	234	117			n/a
3	J2(i1463)	Conc. Pt. (lbs)	L	08-10-14	08-10-14	301	151			n/a
4	J2(i1618)	Conc. Pt. (lbs)	L	10-02-14	10-02-14	343	172			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	6,611 ft-lbs	39,636 ft-lbs	16.7%	1	04-10-14
End Shear	2,596 lbs	17,356 lbs	15%	1	08-11-06
Total Load Defl.	L/999 (0.101")	n/a	n/a	4	05-00-06
Live Load Defl.	L/999 (0.065")	n/a	n/a	5	05-00-06
Max Defl.	0.101"	n/a	n/a	4	05-00-06
Span / Depth	12	n/a	n/a		00-00-00

Bear	ing Supports	Dim . (L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate	4-3/8" x 5-1/4"	2,668 lbs	21.7%	9.5%	Unspecified
B1	Wall/Plate	7-1/4" x 5-1/4"	3,369 lbs	16.6%	7.3%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA 086. **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

SITE COPY



DWG NO. TAM 9/169 STRUCTURAL COMPONENT ONLY



### Triple 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B6(i1554)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

Build 5033

Job Name:

Address:

City, Province, Postal Code: BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\86(i1554

Specifier:

Designer: C

Company.

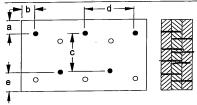
Misc:

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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**Connection Diagram** 



Calculated Side Load = 551.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Nailing schedule applies to both sides of the member.

4 rows

Connectors are: 16d

.5.

3.1/2" ARDOX SPIRAL

Nails

S. KATSOULAKOS S

DWG NO. TAM 9264 -18
STRUCTURAL
COMPONENT ONLY

SITE COPY



### Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7DR(i1792)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

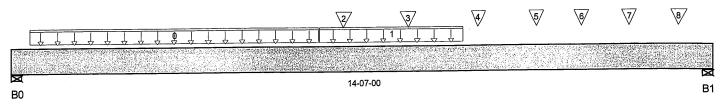
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B7D

Specifier:

Designer: CZ

Company:

Misc:



Total Horizontal Produc	t Length = 14-07-00
-------------------------	---------------------

Reaction Summary (Do			0	Wind	
Be aring	Live	De ad	Snow	yyiii u	
B0, 4"	3,503 / 0	1,901 / 0			
B1, 4"	3,198/0	1,794 / 0			

	ad Cumamami					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type Ref. Start E		En d	1.00	0.65	1.00 1.15			
0	Smoothed Load	Unf. Lin. (lb/ft)	L	00-04-08	06-04-08	504	252			n/a
1	Smoothed Load	Unf. Lin. (lb/ft)	L	06-04-08	09-04-08	253	126			n/a
2	J2(i1728)	Conc. Pt. (lbs)	L	06-10-08	06-10-08	294	* 147			n/a
3	J2(i1710)	Conc. Pt. (lbs)	L	08-02-08	08-02-08	336	168			n/a
4	-	Conc. Pt. (lbs)	L	09-08-05	09-08-05	618	310			n/a
5	_	Conc. Pt. (lbs)	L	10-11-01	10-11-01	848	498			n/a
6	J2(i1436)	Conc. Pt. (lbs)	L	11-10-08	11-10-08	220	110			n/a
7	J2(i1429)	Conc. Pt. (lbs)	L	12-10-08	12-10-08	251	125			n/a
8	J2(i1423)	Conc. Pt. (lbs)	L	13-10-08	13-10-08	258	129			n/a

Controls Summary	Factored ntrols Summary Demand		Demand / Resistance	Load Case	Location	
Pos. Moment	27,315 ft-lbs	60,415 ft-lbs	45.2%	1	06-10-08	
End Shear	7.113 lbs	21,696 lbs	32.8%	1	01-03-14	
Total Load Defl.	L/360 (0.468")	0.702"	66.7%	4	07-04-08	
Live Load Defl.	L/557 (0.303")	0.468"	64.7%	5	07-04-08	
Max Defl.	0.468"	n/a	n/a	4	07-04-08	
Span / Depth	14.2	n/a	n/a		00-00-00	

Beari	ing Supports	Dim . (L x W)	Demand	Resistance Support	Resistance Member	Material
B0 Wall/Plate B1 Wall/Plate		4" x 5-1/4"	7,630 lbs	44.7%	29.8%	Un specified
		4" x 5-1/4"	7,039 lbs	41.3%	27.5%	Un specified

Notes



DWG NO. TAM 9765-18 COMPONENT ONLY





### Triple 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B7DR(i1792)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\87

Designer: CZ Company.

City, Province, Postal Code: BRADFORD, Customer:

Build 5033

Job Name: Address:

Code reports:

CCMC 12472-R

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

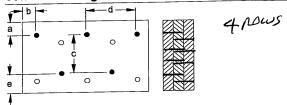
Calculations assume unbraced length of Top: 00-05-04, Bottom: 00-05-04. Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

**Connection Diagram** 



a minimum = 2" c = 6 - 7/8" b minimum = 3" e minimum = 34

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nailing schedule applies to both sides of the member.

Member has no side loads.

Connectors are: 16d A New Nails

3-1/2" ARDOX SPIRAL

### Disclosure

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DWG NO. TAM 9265. STRUCTURAL COMPONENT ONLY





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1694)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8D

Specifier:

CZ Designer:

Company:

Customer:

**Build 5033** 

Job Name:

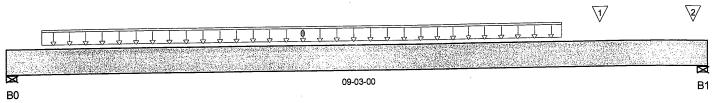
Address:

Code reports:

City, Province, Postal Code: BRADFORD,

CCMC 12472-R

Misc:



### Total Horizontal Product Length = 09-03-00

Reaction Summary (I	Down / Uplift) (lbs)	Do ad	Snow	Wind	
Be aring	Live	De ad	SHOW		
B0, 3-3/4"	2,329 / 0	1,211/0			
B1, 4-1/4"	2,755/0	1,425 / 0			•

				Live	Dead	Snow Wina	I FID.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
<u> </u>	Unf. Lin. (lb/ft)	1 00-05-08	07-04-04	556	278		n/a
0 Smoothed Load	• •	L 07-10-04			299		n/a
1 -	Conc. Pt. (lbs)				324		n/a
2 -	Conc. Pt. (lbs)	L 09-00-12	09-00-12	040	324		

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	11,250 ft-lbs	25,408 ft-lbs	44.3%	1	04-10-04
End Shear	4.626 lbs	11.571 lbs	40%	1	01-01-04
Total Load Defl.	L/485 (0.216")	0.435"	49.5%	4	04-09-00
Live Load Defl.	L/736 (0.142")	0.29"	48.9%	5	04-09-00
	0.216"	n/a	n/a	4	04-09-00
MaxDefl. Span / Depth	11	n/a	n/a		00-00-00

	. O	Dim . (L x W)	De man d	De man d/ Re sistance Support	De mand/ Resistance Member	Material
Bearin B0 B1	n <b>g Supports</b> Wall/Plate Wall/Plate	3-3/4" x 3-1/2" 4-1/4" x 3-1/2"	5,007 lbs 5,914 lbs	47% 49%	31.3% 32.6%	Un specified Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-03-03, Bottom: 00-03-03.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

O86. Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9







### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B8DR(i1694)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

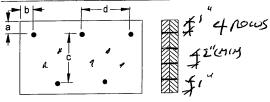
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B8

Specifier:

Designer: CZ Company.

Misc:

**Connection Diagram** 



c = 3 - 1/2" a minimum = 2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d Hoker Nails

3-1/2" ARDOX SPIRAL

### Di sclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG NO. TAM 926618 STRUCTURAL COMPONENT ONLY





### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1576)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

Job Name: Address: City, Province, Postal Code:BRADFORD,

Customer:

Build 5033

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\89(i1576)

Specifier:

Designer: CZ Company:

Misc:

	$\overline{\mathbb{V}}$	2/
03-04-00 B0		у В1

Total Horizonta	Product	Length	= 03-04-00
-----------------	---------	--------	------------

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind	
B0 B1, 3-1/2"	106/0 175/0	61 /0 96 /0			

				Live	Dead	Snow	wina	I FID.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00	1.15	
	Conc. Pt. (lbs)	L 01-02-04	01-02-04	133	67			n/a
0 J5(i1546) 1 J5(i1556)	Conc. Pt. (lbs)	L 02-06-04	02-06-04		54			n/a
2 J5(i1582)	Conc. Pt. (lbs)	L 03-02-12	03-02-12	41	20			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	250 ft-lbs	12,704 ft-lbs	2%	1	01-02-04
End Shear	229 lbs	5,785 lbs	4%	1	00-11-08
Total Load Defl.	L/999 (0.001")	n/a	n/a	4	01-07-01
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-07-01
Max Defl.	0.001"	n/a	n/a	4	01-07-01
Span / Depth	3.8	n/a	n/a		00-00-00

					Demand/ Resistance		
Beari	ng Supports	Dim.(L x W)	Demand	Support	Member	Material	
B0	Hanger	2" x 1-3/4"	235 lbs	n/a	5.5%	HUS1.81/10	
B1	Post	3-1/2" x 1-3/4"	383 lbs	7.7%	5.1%	Unspecified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

**CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

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DWG NO. TAM 9267 -18
STRUCTURAL COMPONENT ONLY

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Build 5033

Job Name:

Address:

### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1232)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1232)

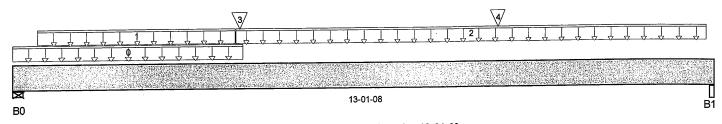
Specifier:

Designer:

Company: Misc:

Customer: CCMC 12472-R Code reports:

City, Province, Postal Code: BRADFORD,



Total Horizontal Product Length = 13-01-08

Reaction Summary (Down / Bearing	Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2"	1,260 / 0	703/0			
B1, 3-1/2"	604/0	376/0			

	- d C					Live	Dead	Snow	Wind	Trib.
	ad Summary g Description	Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
0	UserLoad	Unf. Lin. (lb/ft)	L	00-00-00	04-03-12	240	120			n/a
1	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-05-08	04-02-00	29	15			n/a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	04-02-00	12-11-12	53	26			n/a
2	B9(i1576)	Conc. Pt. (lbs)	Ĺ	04-02-14	04-02-14	103	59			n/a
4	B11(i1461)	Conc. Pt. (lbs)	L	09-01-02	09-01-02	148	88			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,537 ft-lbs	25,408 ft-lbs	21.8%	1	04-07-06
End Shear	2,061 lbs	11,571 lbs	17.8%	1	01-03-00
Total Load Defl.	L/675 (0.222")	0.625"	35.5%	4	06-04-14
Live Load Defl.	L/1,076 (0.139")	0.417"	33.5%	5	06-04-14
Max Defl.	0.222"	n/a	n/a	4	06-04-14
Span / Depth	15.8	n/a	n/a		00-00-00

Bearing Supports				Demand/ Resistance		
		Dim.(LxW)	Demand	Support	Member	Material
B0	Wall/Plate	5-1/2" x 3-1/2"	2,769 lbs	26.9%	11.8%	Unspecified
B1	Beam	3-1/2" x 3-1/2"	1,376 lbs	10.3%	9.2%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86. **CONFORMS TO OBC 2012** 

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9









Build 5033

Job Name:

Customer:

Address:

### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Fioor\...\B10(i1232)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i12\)

Specifier:

Designer: CZ

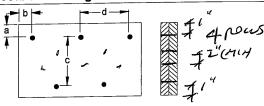
Company:

CCMC 12472-R Code reports:

City, Province, Postal Code: BRADFORD,

Misc:

### **Connection Diagram**



a minimum = 2" 6 b minimum = 3"

Calculated Side Load = 42.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Nails Connectors are: 3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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Page 2 of 2



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i1461)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

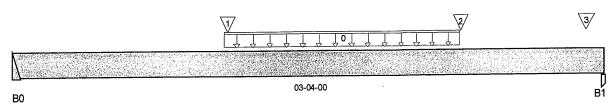
Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i1461)

Specifier:

Designer:

Company:

Misc:



Total Horizontal Product Length = 03-04-00

Reaction Summary (I	Down / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
BO	151/0	89 / 0			
B1, 3-1/2"	202/0	114/0	•		

ء ا	ad Cumman					Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description		Load Type	Ref	Start	En d	1.00	0.65	1.00	1.15	
0	FC3 Floor Material	Unf. Lin. (lb/ft)	L	01-02-04	02-06-04	3				n/a
1	-	Conc. Pt. (lbs)	L	01-02-08	01-02-08	204	112			n/a
2	J5(i1556)	Conc. Pt. (lbs)	L	02-06-04	02-06-04	105	53			n/a
3	J5(i1582)	Conc. Pt. (lbs)	L.	03-02-12	03-02-12	41	20			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	363 ft-1 bs	12,704 ft-lbs	2.9%	1	01-02-04
End Shear	333 lbs	5,785 lbs	5.8%	1	00-11-08
Total Load Defl.	L/999 (0.002")	n/a	n/a	4	01-06-09
Live Load Defl.	L/999 (0.001")	n/a	n/a	5	01-06-09
Max Defl.	0.002"	n/a	n/a	4	01-06-09
Span / Depth	3.8	n/a	n/a		00-00-00

Bearing Supports		<i>u</i>	<b>.</b>		Demand/ Resistance	Material	
		Dim . (L x W)	Demand	Support	Member		
B0	Hanger	2" x 1-3/4"	339 lbs	n/a	7.9%	HUS1.81/10	
B1	Post	3-1/2" x 1-3/4"	446 lbs	9%	6%	Unspecified	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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CONFORMS TO OBC

DWG NO. TAM 92STRUCTURAL COMPONENT ONLY



### Boise Cascade Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B13(i1675)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:24

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B13(i1675`

Specifier:

CZ Designer:

Company: Misc:

		l
		<u> </u>
B0	03-11-08	[] B1

Total Horizontal Product Length = 03-11-08

Reaction Summary (	Live	De ad	Snow	Wind			
B0	12 / 0	15 / 0					
B1, 3-1/2"	11 / 0	15 / 0					
Load Summary				Live	Dead	Snow Wind	Trib.

Land Oursenant						Live	Dead	Snow	Wind	i rib.
	ad Summary Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	<del></del>
	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-08-00	6	3			n/a

**CONFORMS TO OBC 2012** 

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	32 ft-lbs	12,704 ft-lbs	0.2%	1	01-11-00
End Shear	18 lbs	5,785 lbs	0.3%	1	00-11-08
Total Load Defl.	L/999 (0")	n/a	n/a	4	01-11-00
Live Load Defl.	L/999 (0")	n/a	n/a	5	01-11-00
Max Defl.	0"	n/a	n/a	4	01-11-00
Span / Depth	4.6	n/a	n/a		00-00-00

Rearin	ng Supports	Dim . (L x W)	Demand	De man d/ Re sistance Support	Demand/ Resistance Member	Material
B0	Hanger	2" x 1-3/4"	37 lbs	n/a	0.9%	HUS1.81/10
B1	Beam	3-1/2" x 1-3/4"	36 lbs	0.5%	0.5%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

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DWG NO. TAM 9270 STRUCTURAL COMPONENT ONLY





### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\...\B14L(i1364)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 09:54:23

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 EL C.mmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B14L(i136

Specifier:

Designer: CZ

Company. Misc:

Job Name: Address:

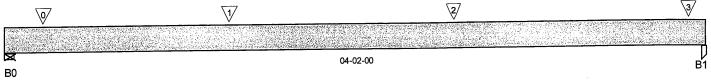
**Build 5033** 

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R



### Total Horizontal Product Length = 04-02-00

Reaction Summary (	Down / Uplift) ( lbs ) Live	De ad	Snow	Wind	
B0, 5-1/2"	255/0	153/0			
B1 3-1/2"	344/0	181/0			

				Live	Dead	Snow V	Vind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	End	1.00	0.65	1.00 1	1.15	
0 1(i649)	Conc. Pt. (lbs)	L 00-02-12	00-02-12		15			n/a
1 J4(i1390)	Conc. Pt. (lbs)	L 01-04-00		228	114			n/a
2 J4(i1388)	Conc. Pt. (lbs)	L 02-08-00	02-08-00	243	121			n/a
3 J4(i1386)	Conc. Pt. (lbs)	L 04-00-12	04-00-12	128	64			n/a

CONFORMS TO OBC 2012

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	592 ft-lbs	12,704 ft-lbs	4.7%	1	02-08-00
End Shear	547 lbs	5,785 lbs	9.5%	1	01-03-00
Total Load Defl.	L/999 (0.004")	n/a	n/a	4	02-02-00
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	02-02-00
Max Defl.	0.004"	n/a	n/a	4	02-02-00
Span / Depth	4.5	n/a	n/a		00-00-00

D	tu u Cummanta	Dim . (L x W)	De man d	De mand/ Resistance Support	Demand/ Resistance Member	Material
Be an B0 B1	ing Supports Wall/Plate Post	5-1/2" x 1-3/4" 3-1/2" x 1-3/4"	573 lbs 743 lbs	11.1% 14.9%	4.9% 9.9%	Un specified Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

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### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16(i2866)



Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 11:58:59

BC CALC® Design Report

Address: City, Province, Postal Code:BRADFORD,

Customer:

**Build 5033** 

Job Name:

Code reports:

CCMC 12472-R

File Name: SD25-4 SONOMA 4 CORNER EL C.mmdl

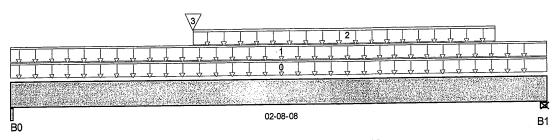
Description: Designs\Flush Beams\1st Floor\Flush Beams\B16(i2866)

Specifier:

Designer:

Company.

Misc:



Total Horizontal Product Length = 02-08-08

Reaction Summary (Down	/ Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/4"	179/0	273/0	193/0		
B1, 5-1/2"	130/0	251/0	196/0		

	1.0		L		Live	Dead	Snow	Wind	Trib.	
	ad Summary g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	User Load	Unf. Lin. (lb/ft)	L	00-00-00	02-08-08	33	130	72		n/a
1	LOWROOF	Unf. Lin. (lb/ft)	Ĺ	00-00-00	02-08-08	33	30	72		n/a
2	FC3 Floor Material	Unf. Lin. (lb/ft)	L	00-11-00	02-05-06	6				n/a
3	.15(i2808)	Conc. Pt. (lbs)	L	00-11-00	00-11-00	121	60			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	260 ft-1bs	25,408 ft-lbs	1%	1	01-02-01
End Shear	134 lbs	11,571 lbs	1.2%	1	01-02-12
Total Load Defl.	L/999 (0")	n/a	n/a	35	01-03-13
Live Load Defl.	L/999 (0")	n/a	n/a	51	01-03-13
Max Defl.	0" ` ´	n/a	n/a	35	01-03-13
Span / Depth	2.4	n/a	n/a		00-00-00

				De mand/ Resistance		
Bear	ing Supports	Dim.(LxW)	Demand	Support	Member	Material
B0	Beam	5-1/4" x 3-1/2"	720 lbs	7.3%	3.2%	Unspecified
B1	Wall/Plate	5-1/2" x 3-1/2"	674 lbs	6.6%	2.9%	Unspecified

Notes



COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B16(i2866)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

August 29, 2017 11:58:59

BC CALC® Design Report

File Name: SD25-4 SONOMA 4 CORNER EL C.mmdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B16(i286

Specifier:

Designer: CZ

Company:

Misc:

Build 5033

Job Name: Address:

City, Province, Postal Code:BRADFORD,

Customer:

Code reports:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

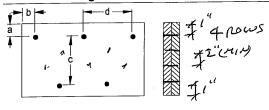
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA **CONFORMS TO OBC 2012** 

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### **Connection Diagram**



a minimum = 🎉 " b minimum = 3"

Calculated Side Load = 94.7 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

3-1/2" ARDOX SPIRAL

### Disclosure

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DWG NO. TAM 9/272 STRUCTURAL COMPONENT ONLY





Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			I	3are		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cen	tre Spacing				re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-7/0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19' <b>-</b> 9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	ın Blocking		Mid-S	Span Blocking a	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A
9-1/2"	N!-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A
11-7/8"	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20'-2"	19'-2"	N/A
	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A
11-7/0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A
	NI-60	24 <b>'-</b> 0"	22'-3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A
14"	NI-70	25' <b>-</b> 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A
	NI-80	25' <b>-</b> 7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A
16"	NI-70	27' <b>-</b> 9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26' <b>-1</b> "	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27' <b>-</b> 5"	26' <del>-</del> 2"	N/A

- 1. Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.
- 2. Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
- 3. Minimum bearing length shall be 1-3/4 inches for the end bearings.
- 4. Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.
- 6. Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







				Bare		1	1/2" Gy	psum Ceiling	
Depth	Series			tre Spacing				tre Spacing	
		12"	16"	19.2"	24"	12"	16"	<b>/</b> 19.2"	24"
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"
	NI-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
11 //0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19' <b>-</b> 5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"
	NI-60	21'-10"	20' <del>-</del> 2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23' <b>-</b> 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25'-9"	23'-10"	22' <del>-</del> 9"	21'-6"
	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"
	NI-70	20'-0"	· 18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10'
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"
	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17' <b>-</b> 8"
11-7/8"	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"
11 //0	NI-70	23'-4"	21' <del>-</del> 8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21'-5"	20'-0"
	NI-90x	24'-3"	22'-6"	21' <del>-</del> 6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23'-9"	22'-4"
	NI-80	26' <b>-</b> 6"	24'-7"	23'-5"	22'-2"	27'-1"	25' <b>-</b> 3"	24'-1"	22'-9"
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"
	NI-60	27 <b>'-</b> 3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"
16"	NI-70	28'-8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"
••	NI-80	29'-1"	27'-0"	25 <b>'-</b> 9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90x	29' <b>-</b> 11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27' <del>-</del> 2"	25'-8"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.
 Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA 086-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







			E	are		1	1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	<b>1</b> 5'-7"	15'-1"	N/A
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15' <b>-</b> 9"	15'-3"	N/A
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A
11-//0	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18' <b>-</b> 5"	N/A
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A
	NI-60	20'-5"	18' <b>-</b> 11"	18'-1"	N/A	21'-2"	19'-7"	18'-9"	N/A
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A
	NI-80	21'-11"	20' <b>-</b> 3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A
	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A
11-7/8"	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18'-5"	N/A
11-//0	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21' <del>-</del> 0"	N/A
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A
	NI-80	25'-7"	23' <del>-</del> 8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A
	NI-90x	26'-4"	24'-4"	23' <del>-</del> 3"	N/A	26'-10"	24'-11"	23'-9"	N/A
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A
16"	NI-70	27'-9"	25' <b>-</b> 8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A
10	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	2 <b>7'-</b> 5"	26'-2"	N/A

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			B	are			1/2" Gyp	sum Ceiling	
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"
	NI-70	18'-0"	16'-11"	16'-3"	<b>15'-6"</b>	18'-5"	17'-3"	16'-7"	15'-6"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"
	NI-40x	19'-4"	17'-11"	17'-3"	15' <b>-</b> 10"	19'-11"	18'-6"	17'-9"	15'-10"
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-1"
11-7/0	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"
	NI-80	23' <b>-</b> 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25' <b>-</b> 9"	23'-10"	22' <b>-</b> 9"	21'-6"
10	NI-80	25' <b>-</b> 6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15'-5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19'-3"	17'-9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10'
11 7/0"	NI-60	21'-9"	19'-8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/8"	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22' <del>-</del> 5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	N!-70	26'-1"	24'-3"	22'-9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25 <b>'-</b> 4"	24'-1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28'-8"	26' <b>-</b> 8"	25'-3"	23'-4"	29'-3"	26'-11"	25' <b>-</b> 3"	23'-4"
TO	NI-80	29'-1"	27'-0"	25' <b>-</b> 9"	23'-10"	29'-8"	27'-6"	25' <b>-1</b> 0"	23'-10"
	NI-90x	29'-11"	27' <b>-</b> 10"	26'-6"	24'-10"	30'-6"	28' <b>-</b> 5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

ries can result.

NSTALLATION GUIDE

**ENGINEERED** 

**₩00D** 

FOR RESIDENTIAL FLOORS



Once sheathed, do not over-stress I-joist with Never stack building unsheathed I-joists.

concentrated loads from building materials.

WARNING

Avoid Accidents by Following these Important Guidelines: -joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

- Brace and nail each Lipist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When Lipists are applied continuous blocking will be required at the interior support. over interior supports and a load-bearing wall is planned at that location,
- 2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the Lioists. Until this sheathing is applied, to prevent l-joist rollover or buckling. temporary bracing, often called struts, or temporary sheathing must be applied
- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each boy. Lap ends of adjoining bracing over at least two 1-joists.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only

can result in serious accidents. Follow these installation guidelines carefully, Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when require

Never install a damaged I-joist.

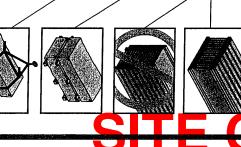
# STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- Store, stack, and handle I-joists vertically and level only.

- 3. Always stack and handle I-joists in the upright position only.
- 4. Do not store I-joists in direct contact with the ground and/or flatwise
- Bundled units should be kept intact until time of installation Protect I-joists from weather, and use spacers to separate bundles.
- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- ■Pick I-joists in bundles as shipped by the supplier.

Distributed by:

- ■Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5<sup>th</sup> points, using a spreader bar if necessary.
- 8. Do not handle Lipoists in a horizontal orientation
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST.





## MAXIMUM FLOOR SPANS

- 1. Maximum **clear** spans applicable to simple-span or or more of the adjacent span. For multiple-span applications, the end spans shall be 40% 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. limit states are based on the factored loads of 1.50L + ive load of 40 psf and dead load of 15 psf. The ultimate multiple-span residential floor construction with a design
- 2. Spans are based on a composite floor with glued-nailed Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive of gypsum and/or a row of blocking at mid-span. shall meet the requirements given in CGBS-71.26
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- 4. Bearing stiffeners are not required when 1-joists are used required for hangers. with the spans and spacings given in this table, except as
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- SI units conversion: 1 inch = 25.4 mm1 toot = 0.305 m

# MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS

SIMPLE AND MULTIPLE SPANS

				Joist Jo Depth Sei
				Joist Series 12"
21.9 22.1 22.6	1871); 200 200 2018 2018 2018	7.0 7.3 8.0 8.3 8.3 8.3	15/21 15/21 16/11 16/31	Simple On centre 16"
		15.5 16.5 16.7 17.4 17.6 17.6		
107	1525.55	184, 17.3 20.01 18.6 20.01 18.5 20.03 19.5 21.0 20.2 22.3 20.7 22.5 20.9		
		17-9 18-0 19-0 19-8 19-8 19-10		ple
214.10 23.0 23.4 23.9	119-4: 20-1: 21-2: 21-6: 21-10: 22-2:	1857 1948 1990 1991 1991	14.7" 15-5" 16-10" 17-0"	24"

CCMC EVALUATION REPORT 13032-R

 Hangers shown illustrate the three to support 1-joists. most commonly used metal hangers

I-JOIST HANGERS

- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based maximum spans. and load capacity based on the on the joist depth, flange width
- 4. Web stiffeners are required when the brace the top flange of the I-joist. sides of the hangers do not laterally







Face Mount

### WEB STIFFENERS

## A bearing stiffener is required in all

RECOMMENDATIONS:

the stiffener and the flange is at the top. reactions greater than shown in the Construction Guide (C101).The gap between i-joist properties table found of the I-joist engineered applications with factored

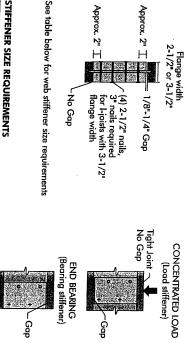
the I-joist is supported in a hanger and the stiffener and flange is at the top. support, the top flange. The gap between the sides of the hanger do not extend up to, and A bearing stiffener is required when

adjusted for other load durations as permitted by the code. The gap between the stiffener where a factored concentrated load greater than 2,370 lbs is applied to the top flange ■ A load stiffener is required at locations and the flange is at the bottom. standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever between supports, or in the case of a

SI units conversion: 1 inch = 25.4 mm

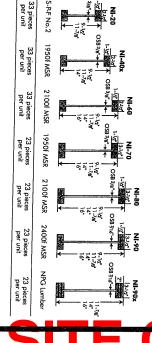
### FIGURE 2

## WEB STIFFENER INSTALLATION DETAILS



Tight Joint

## **NORDIC I-JOIST SERIES**



manufacturing process. Every phase of the operation, from totage to the finished product, reflects our commitment to quality. products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables North

lumber in their flanges, ensuring consistent quality, supejfor strength പ്രൂറ Nordic Engineered Wood I-joists use only finger-jointed back spruce longer span carrying capacity.

0490416

# INSTALLING NORDIC I-JOISTS

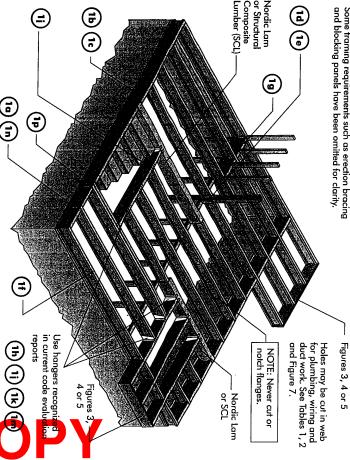
- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact your
- 2. Except for cutting to length, I-joist flanges should **never** be cut, drilled, or notched
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.

W. H.K

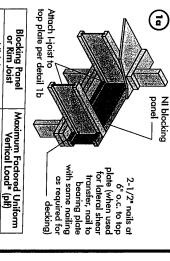
- 4. I-joists must be anchored securely to supports before floor sheathing is affached, and supports for multiple அளில் நார்க்கதாயை
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings かんちょう
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- 7. Leave a 1/16-inch gap between the I-joist end and a header.
- . Concentrated loads greater than those that can normally be expected in residential construction should only be applied to concentrated loads from the top of the Lioist. Or, attach the load to blocking that has been securely fastened to the the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all
- 9. Never install Lioists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or 1-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. Lipist blocking l-joist-compatible depth selected. panels or other engineered wood products – such as rim board – must be cut to fit between the I-joists, and an
- 13. Provide permanent lateral support of the bottom flange of all Lioists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered Lioists at the end support next to the cantilever extension. In the completed structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
- 14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to underlayment layer is installed. minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

# Some framing requirements such as erection bracing TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

FIGURE 1



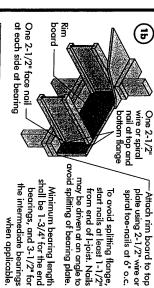
All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framily (unber assumed to be Spruce-Pine-Fir No. 2 or better, Individual components not shown to scale for clar



- 4	Г	
he uniform vertical load	NI Joists	or Rim Joist
*The uniform vertical load is limited to a joist depth of 16	3,300	Maximum Factored Uniform Vertical Load* (ptf)

inches or less and is based on standard term load duration

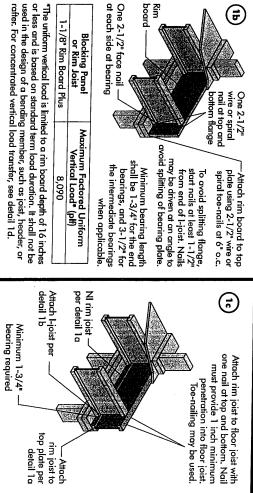
It shall not be used in the design of a bending member, such as joist, header, or rafter. For concentrated vertical

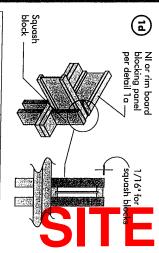


1-1/8" Rim Board Plus	Blocking Panel or Rim Joist
8,090	Maximum Factored Uniform Vertical Load* (pH)

used in the design of a bending member, such as joist, header, or

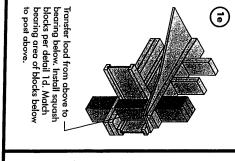
after. For concentrated vertical load transfer, see detail 1d

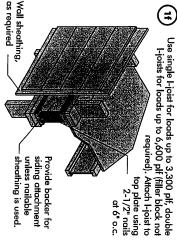




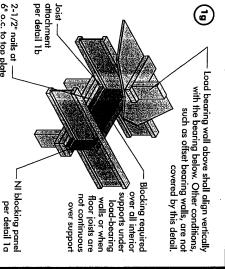
1-1/8" Rim Board Plus	2x Lumber		Pair of Squash Blocks	
4,300	5,500	3-1/2" wide	Pair of Squash Blocks (lbs)	Maximum Facto
6,600	8,500	5-1/2" wide	h Blocks (lbs)	Maximum Factored Vertical per

Provide lateral bracing per detail 1a, 1b, or 1c





Rim board may be used in lieu of Ljoists. Backer is not carried to the foundation. required when rim board is used. Bracing per code shall be





(T)

o.c. to top plate

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Nordic Lam or SCL

allowed past inside tace of wall or beam.

detail 1p \_

Filler block per

manufacturer's recommendations Top-mount hanger installed per \_

Install hanger per

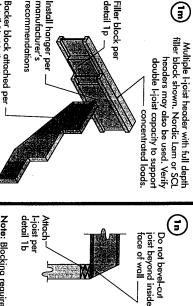
recommendations manutacturer's

support the top flange, bearing Note: Unless hanger sides laterally

Maximum support capacity = 1,620 lbs

clinch when possible.

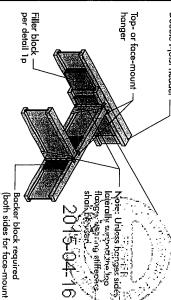
detail 1h. Nail with twelve 3" nails,



Note: Blocking required support, not shown at bearing for lateral

> Double I-joist header Use twelve 3" nails, clinched when possible. Maximum factored additional 3" nails through the webs and filler block where the Backer block (use if hanger load exceeds 360 lbs) resistance for hanger for this detail = 1,620 lbs. backer block will fit. Clinch. Install backer tight to top flange. Before installing a backer block to a double I-joist, drive three

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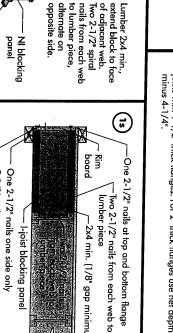


For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

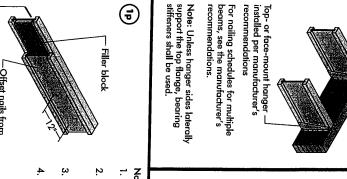
Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	ון	5-1/2"
3-1/2"	1-1/2"	7-1/4"

- better for solid sawn lumber and wood structural panels conformit to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- joists with 1-1/2" thick flanges. For 2" thick flanges use net depth For face-mount hangers use net joist depth minus 3-1/4" for minus 4-1/4".



- strap applied to underside of joist at blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joists. Optional: Minimum 1x4 inch

- In some local codes, blocking is prescriptively required in the first joist space (or first and second joist space) next to the starter joist. Where required, see local code requirements for spacing of the blocking One 2-1/2" nails one side only 2-1/2" nails at 6" o.c. –2x4 min. (1/8" gap minimun I-joist blocking panel
- All nails are common spiral in this detail



### Offset nails from opposite face by 6"

—1/8" to 1/4" gap between top flange and filler block

Notes:

1. Support back of I-joist web during nailing to prevent damage to web/flange connection.

DOUBLE I-JOIST CONSTRUCTION

€

FILLER BLOCK REQUIREMENTS FOR

- Filler block is required between joists for Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist
- tull length of span.

2-1/2"× 1-1/2"

11-7/8 9-1/2

2-1/8" × 10" 2-1/8" x 6" 2-1/8" × 8"

/8" x 12"

Nail joists together with two rows of 3" nails at 12 inches o.c. (clinched when possible) on each side of the double I-joist. are required. can be clinched, only two nails per toot Total of four nails per foot required. If nails

3-1/2" × 1-1/2"

3" x 10" သူ × ဇူ 3" × 6"

ფ× 12

9-1/2"

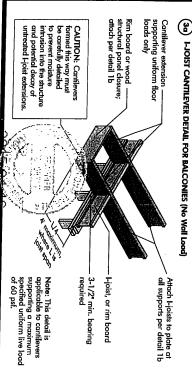
The maximum factored load that may be using this detail is 860 lbf/ft. Verify double applied to one side of the double joist

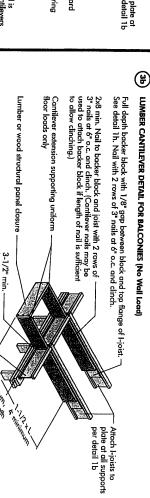
3-1/2" ×

11-7/8" 14" 16"

일 일 일 × × 7 1





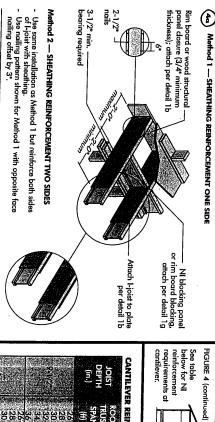


# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

cantilevers supporting a maximum specified uniform live load of 60 psf.

I-joist, or rim board bearing required

Note: This detail is applicable to



CANTILEVER REINFORCEMENT METHODS ALLOWED

Roof truss . span

cantilever ڳ ڏ

> truss Girder -Roof trusses

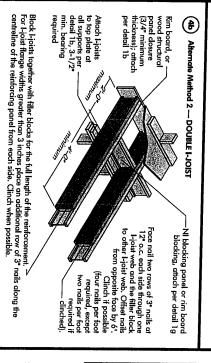
יי Jack trusses 72-0 maximum

> trusses running parallel to the cantilevered floor joists, the I-joist reinforcement For hip roofs with the jack

requirements for a span of 26 ft. shall be permitted to

Koot truss span

Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4\*) required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2\* nails at 6\* o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.



1. N = No re 1 = NI rei panel 2 = NI rei panel X = Try a a Adead load, plf wall loa maximum	6	Table 1	10.78 10.78 11.78		JOIST DEPTH (in.)
inforcement inforced with inforced with on one side on both sid beeper joist design load design load design floa do . 55 psf floo id. Wall load do . Mall windo	44 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	26 27 30 32 34 34 36	300 34 35 35 36 36 36 36 36 36 36 36 36 36 36 36 36	2 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	ROOF TRUSS SPAN (ff)
squired. //4" wood //4" wood //4" wood //4" wood /, or doubl closer sp nall be: 1: otal load, s based o	ZZZZZZZZ	ZZZZZZZ	ZZZŽZZŽ	ZZZZZ	
red. wood structural wood structural double I-joist. ser spacing. be: 15 psf roof load, and 80 sed on 3-0"	22222272	ZZZZZZZZ	ZZZZZZZ-	zz	= 30 psf, IOIST SPA 16
For la openii tional studs 3. Table meet live lo and a 12° o.	ZZZZZZZZ	ZZZZZZZ	zzz	222-1	DL = 15 CING (in 19.2
For larger openings, or multiple 3:0° wind openings spaced less than 6:0° o.c., and final joists beneath the opening's cripp studs may be required. Table applies to joists 12° to 24° o.c. than meet the floor span requirements for a life load of 40 psf and dead load of 15 and a live load deflection limit of L/480 12° o.c. requirements for lesser spacing.	zzzzzz	zzz	NDN1-	(XXXX)	psf ) 24
ppenings, or multipaced less than 6: beneath the oper be required. be required. be spoists 12* to ; oor span requirem 40 psf and dead and deflection lim puirements for less	ZZZZZZZŻ	<b>22</b> 222222	222222	22222	ROOF
or larger openings, or multiple 3:.0" width popenings spaced less than 6:.0" o.c., additional loists beneath the opening's cripple studs may be required.  Table applies to joists 1.2" to 24", o.c. that meet the floor span requirements for a desire look of 4.0 psf and dead load of 15 ps and a five load deflection limit of L/480, U 12" o.c. requirements for lesser spacing.	ZZZZZZZŻ	ZZZZZZZ		2 2 2	LOADING = 40 psf IOIST SP/
For larger openings, or multiple 3'-0" width peparings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required. 2" to 24" o.c., that labbe applies to joists 12" to 24" o.c., that meet the floor span requirements for a design into load of 40 psf and dead load of 15 psf, and a five load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.	ZZZZZZZ	Zzzz	×	××0000	3 (UNFAC , DL = 15 , CING (ir , 19,2
ام رو	NN	2277777	*********	××××*!	TORED) i psf i.) 74
For conventional roof ridge beam, the Roof obove is equivalent to the supporting wall an When the roof is fram the Roof Truss Span is distance between the struss is used.  Cantilevered joists sup	ZZZZZZZZ	ZZZZZZZZ	ZZZZZZ		5 E
For conventional roof construction using ridge beam, the Roof Truss Span column claore is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge beam to the ridge of the Roof Truss Span is equivalent to the distance between the supporting walls as truss is used.	-2222222	zzzzz		××2	= 50 psf JOIST SP,
nal roof construction using a the Roof Truss Span column volent to the distance between g well and the ridge beam, g well and the ridge beam. If is framed using a ridge board span is equivalent to the supporting walls as if a reen the supporting walls as if a		3 = z×	NNNN×	××××	DL = 18
Istruction using a ss Span column sustance between he ridge beam. using a ridge board, urvalent to the porting walls as if a	ל בייי ממממ×	×	××××××	××××	n.) 5 psf

4. For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the roof is framed using a ridge board, in the Roof Truss Span is equivalent to the Cantilevered joists supporting girder trusses or roof beams may require additional distance between the supporting walls as if a

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# RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively
- I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- 4. Whenever possible, field-cut holes should be centred on the middle of the web.
- between the top or bottom of the hole or opening and the adjacent I-joist flange. the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of
- Ģ 3/4 of the diameter of the maximum round hole permitted at that location. The sides of square holes or longest sides of rectangular holes should not exceed
- ٥. Where more than one hole is necessary, the distance between adjacent hole size of the largest square hole (or twice the length of the longest side of the longest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of edges shall exceed twice the diameter of the largest round hole or twice the Tables 1 and 2, respectively.
- A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 11. Limit three maximum size holes per span, of which one may be a duct chase
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them

### FIGURE 7

FIELD-CUT HOLE LOCATOR

See Table 1 bearing for minimum distance from Knockouts 2x diameter of larger See rule 12 diameter, length or hole 2x duct chase whichever is between top and bottom flange --all duct chase openings and holes Maintain minimum 1/8" space from bearing) Duct chase opening minimum distance (see Table 2 for

and may be ignored for purposes of calculating minimum distances A knockout is **NOT** considered a hole, may be utilized wherever it occurs

> are 1-1/2 inches in diameter, and are length of the I-joist. Where possible, it is preferable to use knockouts instead of electrical or small plumbing lines. They for the contractor's convenience to instal Knockouts are prescored holes provided ñeld-cut holes. spaced 15 inches on centre along the **Never** drill, cut or notch the flange, or



sharp saw. should be cut with a over-cut the web. Holes in webs

the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the holes is another good method to and then making the cuts between diameter hole in each of the four corners the corners is recommended. Starting the rectangular hole by drilling a 1-inch minimize damage to the I-joist For rectangular holes, avoid over-cutting

# LOCATION OF CIRCULAR HOLES IN JOIST WEBS Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

1 Ahova taki					Joist Depth
a many ha mand f		AND THE REAL PROPERTY.		92 26	Joist Series
					Min
		100 A A A A A A A A A A A A A A A A A A	00571 00524		m disian 5
	4 8 6 1 8 6 8 8 6 1 8				om insic Rour 6-1/4
I	7.8 7.8 8.0 6.5	522 200 200 200 200 200 200 200 200 200			ce of any ole diam 8 8
ı		0.60 837 888 1074 1078 1274 1078 1274	tive of the		
	1.6			<b>原 18</b> 次素	<b>₩</b>
			arthur gara		1.) 12 12-3/4
- U-12		esek.			Span Idjustment Factor

Above table may be used for Lipist spacing of 24 inches on centre or less.
 Hole location distance is measured from inside face of supports to centre of hole.
 Distances in this chart are based on uniformly loaded joists.

### OPTIONAL:

The above table is based on the Lipists used at their maximum span. If the Lipists are placed at less than their full maximum span (see Maximum Provincials). The minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced = Lactual × D

Where: Dreduced Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The jediced distance shall not be less than 6 inches from the face of the support to edge of the hole.

Lactual

¥ The actual measured span distance between the inside faces of supports (ft)

Span Adjustment Factor given in this table.

The minimum distance from the inside face of any support to centre of hole from this table Lactual is greater than 1, use 1 in the above calculation for Lactual

0.504

 $\circ$ 

TABLE 2

# DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

Depth	Series				Duct cho	use leng	th (in.)		ı	
	New Property	8	10	12	14	16	78	20	22	24
Y.		, 61 La	00.4 0000	0.0	8.1	6.6	63	9:6	7.J.	7.5 5.7
		514	545	500	6.3	6170		779	8-3	200 44:
			7.2	866	7.13	715	91.80	8 3	8.9	9.4
			7-8 7-4	9.0 9.0	8 6 0 6	9.0	913	0 0 . 00	10.3	
		7.6			ğ,	8-10	9.3 917	918	90	
		10.00	9.7	990	0.8	100			12.0	
		o 94	)010  014 <b>2</b>					1333 063	12.7	12.3
	1		00.8	52	6	12:1	2.6	10.2	12:7	
			10.9	1113	15 15 16 16 17 18 18 18 18 18 18 18 18 18 18 18 18 18	12.1	12/7	13-1	13-3	11

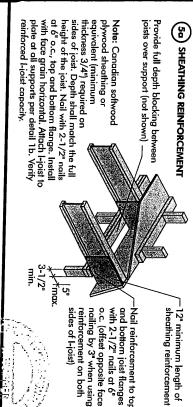
. Above table may be used for I-joist spacing of 24 inches on centre or less.

Duct chase opening location distance is measured from inside face of supports to centre of opening.

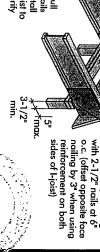
The above table is based on simple-span joists only. For other applications, contact your local distributor.

Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor.

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)



nailing by 3" when using o.c. (offset opposite face and bottom joist flanges with 2-1/2" nails at 6" Nail reinforcement to top



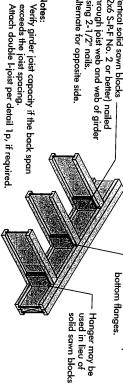
### (3/4" minimum thickness), attach per detail 1b. structural panel closure Rim board or wood between joists over support (not shown for clarity) Attach I-joist to plate at all 3-1/2" minimum I-joist supports per detail 1b. Provide full depth blocking SET-BACK DETAIL max. ςī Bearing walls girder joist per Attach joists to

**5**b

## (5c) SET-BACK CONNECTION

bearing required

through joist web and web of girder Vertical solid sawn blocks \_\_\_\_\_(2x6 S-P-F No. 2 or better) nailed Alternate for opposite side. using 2-1/2" nails.



nails, toe-nail at top and Nail joist end using 3"

detail 5c.

N = No reinforcement required.
 I = NI reinforced with 3/4 wood structural panel on one side only.
 2 = NI reinforced with 3/4 wood structural panel on both sides, or double I-joist.
 X = Try a deeper joist or doser spacing.

2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf maximum width window or door openings. wall load. Wall load is based on 3'-0"

FIGURE 5 (continued) cantilever. requirements at below for NI See table reinforcement Roof truss span L maximum 2-0 -5" maximur cantilever

3		truss	Girder 2	Roof trusses	
	AND DESCRIPTION OF THE PROPERTY OF THE PERSON OF THE PERSO	span	Roof tries	S es	
[1	The same of the sa	ī .	1		
- 5" maximum	cantilever	2'-0"	Jack trusses	— 13'–0" maximun	
_				E E	

the cantilevered floor joists, the I-joist reinforcement trusses running parallel to For hip roofs with the jack 26 ft. shall be permitted to requirements for a span of

# BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

For larger openings, or multiple 3'.0" width openings spaced less than 6'-0" o.c., additional joists beneath the opening's cripple studs may be required.
Table applies to joists 12" to 24" o.c. that meet

the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12° o.c. requirements for lesser spacing.

For conventional roof construction using a ridge beam, the Roof Truss Span column above is equivalent to the distance between the supporting wall and the ridge beam.
When the roof is framed using a ridge board, distance between the supporting walls as if a the Roof Truss Span is equivalent to the

truss is used.

Cantilevered joists supporting girder trusses or roof beams may require additional reintorcing.

# INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- 3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- 5. Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single 1-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on 1-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying
- 8. Tap the second row of panels into place, using a block to protect groove edges
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for paneis 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the

# FASTENERS FOR SHEATHING AND SUBFLOORING(1)

240	20	16:	Maximum Joist Spacing (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
2"	2"	2.	Common Wire or Spiral Nails
1-3/4"	1-3/4*	1-3/4*	il Size and Ty Ring Thread Nails or Screws
2	2"	2"	pe Staples
6ª	6"	6.	Maximum of Fast Edges
12"	12"	12"	n Spacing teners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- 2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimums shown.
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manutacturer.
- Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5.

### IMPORTANT NOTE:

Floor sheathing must be field glued to the L-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, L-joist spans must be verified with

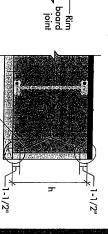
# RIM BOARD INSTALLATION DETAILS

## 8 ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

Rim board Joint Between Floor Joists

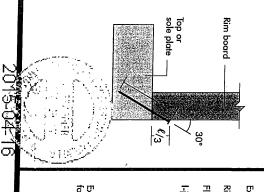
Rim board Joint at Corner





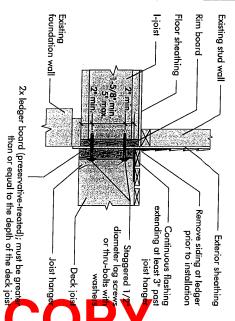
### (F TOE-NAIL CONNECTION AT RIM BOARD

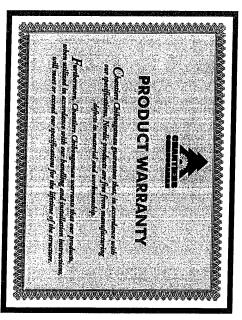
2-1/2" toe-nails at 6" o.c. (typical) —

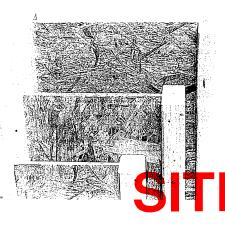


## (%) 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

Rim board joint







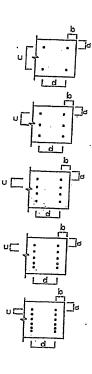
### MICRO CITY

### Engineering services inc.

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	LVL HEADER AND CONVENTIONAL					
		BER NAILING				
	DETAIL NUMBER	NUMBER OF ROWS	SPACING (INCHES o/c			
	. A	2	12			
	В	2	8			
	С	2	6			
	D	2	4			
4	1A	3	12			
J. Williams	1B	3	8			
	1C	3	. 6			
	1D	3:	4			
	2A	4	. 12			
	2B	4	8 ·			
L	2C	4	6			
Ŀ	2D	4	4			
	3A	5	12			
L	3B	5	8			
L	3C	5	6			
L	3D	5	4			
L	4A	6	12			
L	4B	6	8			
L	4C	6	6			
Ŀ	4D	6	4			



### NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
- (2) MINIMUM LUMBER END DISTANCE "b" = 2"
- (3) MINIMUM NAIL ROW SPACING "c" = 2"
- (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
- (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
- (6) DO NOT USE AIR-DRIVEN NAILS



DWG NO TANNIODI. 14

STRUCTURAL

GOMPONENT DNLY

TO BE USED DNLY

WITH BEAM CALCS

BEARING THE

STAMP BELOWS

PROVICE NATLING
DETAIL № ×/SEE
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