

REVIEWED

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS28-2 -(SS)
LJS26DS -(V)
LUS24 -(O)

NOTE:

S - STUCCO

H3 -3'-1"-0 HIGHER PLATE

ALL B-2-2X10 (FLUSH)

1'-4"-0 RAISED FASCIA

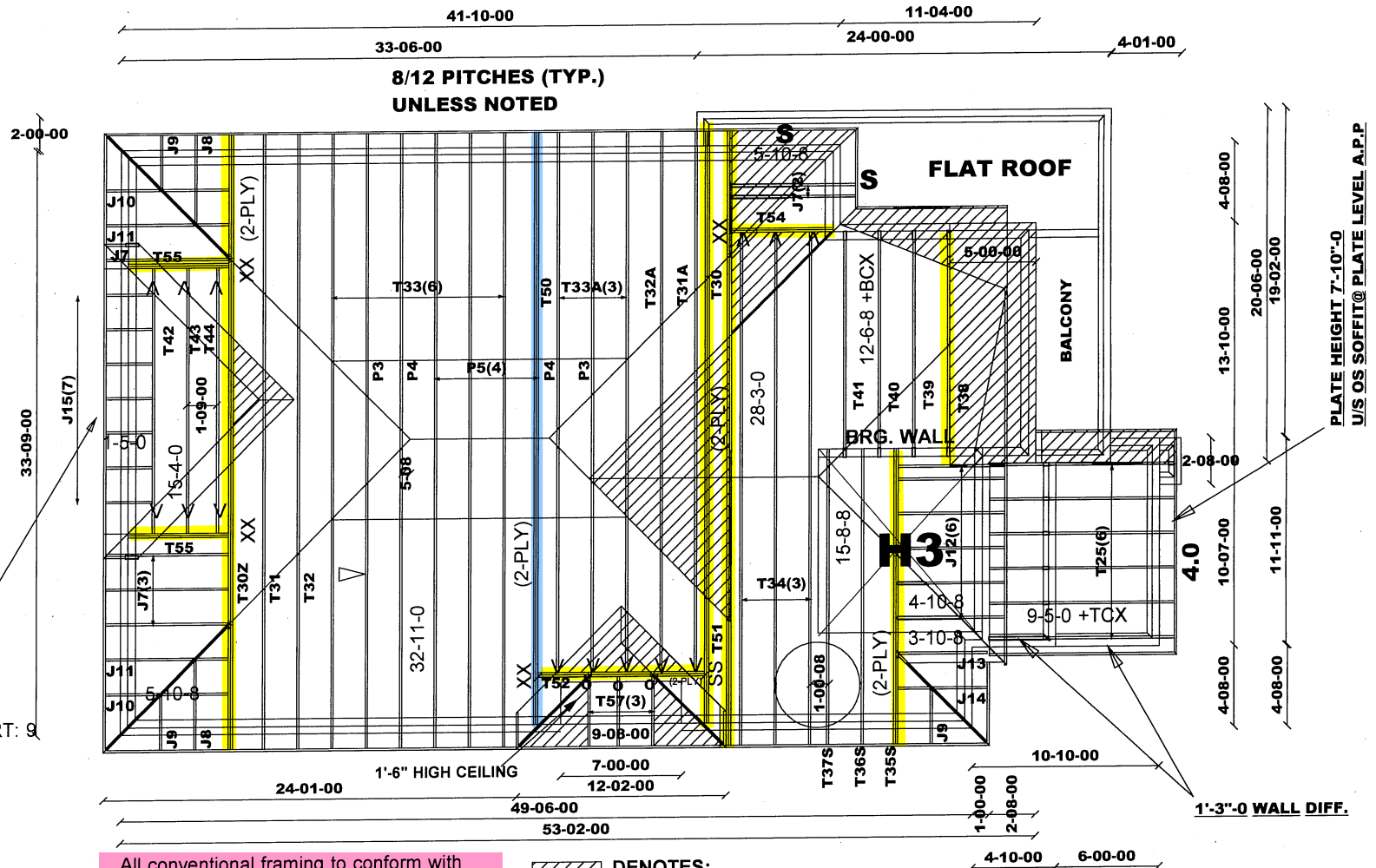
DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.9 psf | Sr = 8.4 psf

DESIGN LOADS:

TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

TOWN OF BRADFORD WEST GWILLIMBURY
BUILDING DEPARTMENT
PLANS EXAMINED
ONTARIO BUILDING CODE APPLIES
DATE: 2022-07-12

INSPECTOR: SE



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24\"/>

 **DENOTES:**
CONVENTIONAL
FRAMING



Job Track: **52917**
Plan Log: **206241**
Layout ID: **426687**

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

Project: **GREEN VALLEY EAST 2022**

Date: 2022-06-06 Sales: Rick DiCiano Designer: ND

Model / Elevation:

S42-17 / C REAR & SIDE UPG. LOT 109

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

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REVIEWED

ASPHALT SHINGLES
12" FINISH O.H.
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
HGUS28-2 - (SS)
LJS26DS -(V)
LUS24 - (O)

NOTE:

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DESIGN CONFORMS WITH OBC
2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 43.9 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 32.5 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

 **DENOTES:**
CONVENTIONAL
FRAMING

Job Track:	52917
Plan Log:	206241
Layout ID:	426688

Builder / Location:

BAYVIEW WELLINGTON / BRADFORD

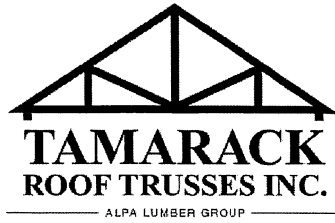
Project: **GREEN VALLEY EAST 2022**

Date: 2022-06-06	Sales: Rick DiCiano	Designer: ND
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Model / Elevation:

S42-17 / C REAR & SIDE UPG. + OPT.COF. LOT 109

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REVIEWED**DELIVERY SHIPLIST**

Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST 2022
 Location: BRADFORD
 Model: S42-17
 Lot #:
 Elevation: C REAR & SIDE UPG. LOT 109

Job Track: 52917
 PlanLog: 206241
 Layout ID: 426687
 Ref #: 13780
 Page: 1 of 3
 Date: 06-06-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	T25 Monopitch	4 /12	9-05-00	4-03-15	2 x 4	1-03-08	11-03 2-09-14	265.48 186.00		
	1 2-ply	T30 Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.97 238.33		
	1 2-ply	T30Z Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	383.97 238.33		
	1	T31 Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	139.59 88.83		
	1	T31A Hip	8 /12	29-11-00	6-07-13	2 x 4	1-03-08	1-04-13 3-04-13	129.73 82.83		
	1	T32 Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	144.5 90.33		
	1	T32A Hip	8 /12	29-11-00	7-11-13	2 x 4	1-03-08	1-04-13 3-04-13	133.7 83.83		
	6	T33 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	877.84 544.00		
	3	T33A Piggyback Base	8 /12	29-11-00	9-03-13	2 x 4	1-03-08	1-04-13 3-04-13	439.16 273.00		
	3	T34 Common	8 /12	28-03-00	10-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	369.6 226.00		
	1 2-ply	T35S Half Hip Girder	8 /12	15-08-08	7-08-15	2 x 6 2 x 4	1-03-08	1-04-13 4-07-13	175.26 112.00		
	1	T36S Half Hip	8 /12	15-08-08	9-00-13	2 x 4	1-03-08	1-04-13 5-11-13	82.23 53.00		
	1	T37S Hip	8 /12	15-08-08	10-04-13	2 x 4	1-03-08	1-04-13 6-08-02	93 59.33		
	1	T38 Half Hip Girder	8 /12	12-06-08	5-09-02	2 x 4 2 x 6	5-08	1-04-13 5-09-02	68.33 42.83		

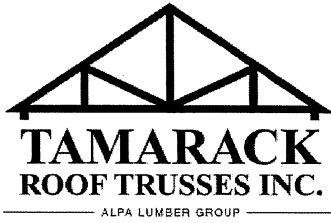
REVIEWED**DELIVERY SHIPLIST**

Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST 2022
 Location: BRADFORD
 Model: S42-17
 Lot #:
 Elevation: C REAR & SIDE UPG. LOT 109

Job Track: 52917
 PlanLog: 206241
 Layout ID: 426687
 Ref #: 13780
 Page: 2 of 3
 Date: 06-06-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T39 Half Hip	8 / 12	12-06-08	7-01-02	2 x 4	5-08	1-04-13 7-01-02	60.7 38.17		
	1	T40 Half Hip	8 / 12	12-06-08	8-05-02	2 x 4	5-08	1-04-13 8-05-02	68.41 42.33		
	1	T41 Roof Special	8 / 12	12-06-08	9-09-02	2 x 4	5-08	1-04-13 9-09-02	61.97 38.67		
	1	T42 Hip Girder	8 / 12	15-04-00	3-08-02	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	73.13 48.67		
	1	T43 Hip	8 / 12	15-04-00	5-00-02	2 x 4		2-08-13 2-08-13	68.98 44.83		
	1	T44 Hip	8 / 12	15-04-00	6-02-02	2 x 4		2-08-13 2-08-13	66.78 43.33		
	1 2-ply	T50 Piggyback Base Girder	8 / 12	32-11-00	9-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	371.99 231.67		
	1 2-ply	T51 Hip Girder	8 / 12	32-11-00	6-02-02	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	342.69 210.67		
	1 2-ply	T52 Hip Girder	8 / 12	9-08-00	4-07-08	2 x 4 2 x 6		2-08-03 2-08-03	106.99 70.00		
	1 2-ply	T54 Half Hip Girder	8 / 12	5-10-08	4-04-02	2 x 4 2 x 6		1-04-13 4-04-02	66.87 43.00		
	2 2-ply	T55 Jack-Open Girder	8 / 12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	129.04 82.67		
	3	T57 Monopitch	8 / 12	2-09-00	4-07-08	2 x 4	1-03-08	1-03-08 3-01-08	48.88 34.50		
	2	P3 Piggyback	8 / 12	9-02-00	1-04-00	2 x 4			49.97 33.67		
	2	P4 Piggyback	8 / 12	9-02-00	2-08-00	2 x 4			51.89 33.67		

REVIEWED**DELIVERY SHIPLIST**

Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
 Project: GREEN VALLEY EAST 2022
 Location: BRADFORD
 Model: S42-17
 Lot #:
 Elevation: C REAR & SIDE UPG. LOT 109

Job Track: 52917
 PlanLog: 206241
 Layout ID: 426687
 Ref #: 13780
 Page: 3 of 3
 Date: 06-06-2022
 Designer:
 Sales Rep: Rick DiCiano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	P5 Piggyback	8 /12	9-02-00	3-00-11	2 x 4			92.72 59.33		
	6	J7 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	141.47 88.00		
	2	J8 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	23.82 15.33		
	3	J9 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	27.51 17.00		
	2	J10 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	27.34 16.67		
	2	J11 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	32.99 20.67		
	6	J12 Jack-Open	8 /12	4-10-08	4-07-13	2 x 4	1-03-08	1-04-13 4-07-13	101.08 62.00		
	1	J13 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	14.34 9.00		
	1	J14 Jack-Open Girder	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	11.39 7.00		
	7	J15 Jack-Open	8 /12	1-05-00	3-08-02	2 x 4	1-03-08	2-08-13 3-08-02	71.7 52.50		

TOTAL # TRUSS= 90

TOTAL BFT OF ALL TRUSSES= 3661.99

BFT.

TOTAL WEIGHT OF ALL TRSSES 5799.04 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LUS24	
14	Hardware	LJS26DS	
4	Hardware	HGUS26-2	
1	Hardware	HGUS28-2	

TOTAL NUMBER OF ITEMS= 22

DELIVERY SHIPLIST



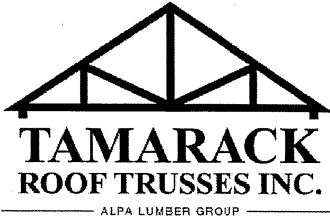
Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
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 Location: BRADFORD
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 Elevation: C REAR & SIDE UPG. + OPT.COF.

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PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	6	T25 Monopitch	4 /12	9-05-00	4-03-15	2 x 4	1-03-08	11-03 2-09-14	265.48 186.00		
	1 2-ply	T30 Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.71 235.00		
	1 2-ply	T30S Hip Girder	8 /12	32-11-00	5-03-15	2 x 6 2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	382.19 237.33		
	1 2-ply	T30Z Hip Girder	8 /12	32-11-00	5-03-15	2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	378.71 235.00		
	1	T31A Hip	8 /12	29-11-00	6-07-13	2 x 4	1-03-08	1-04-13 3-04-13	129.73 82.83		
	1	T31S Hip	8 /12	32-11-00	6-07-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	155.32 100.83		
	1	T32A Hip	8 /12	29-11-00	7-11-13	2 x 4	1-03-08	1-04-13 3-04-13	133.7 83.83		
	1	T32S Hip	8 /12	32-11-00	7-11-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	161.71 103.33		
	6	T33 Hip	8 /12	32-11-00	9-03-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	877.84 544.00		
	3	T33A Piggyback Base	8 /12	29-11-00	9-03-13	2 x 4	1-03-08	1-04-13 3-04-13	439.16 273.00		
	3	T34 Common	8 /12	28-03-00	10-09-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	369.6 226.00		
	1 2-ply	T35S Half Hip Girder	8 /12	15-08-08	7-08-13	2 x 4	1-03-08	1-04-13 4-07-13	152.29 99.33		
	1	T36S Half Hip	8 /12	15-08-08	9-00-13	2 x 4	1-03-08	1-04-13 5-11-13	82.23 53.00		
	1	T37S Hip	8 /12	15-08-08	10-04-13	2 x 4	1-03-08	1-04-13 6-08-02	93 59.33		

DELIVERY SHIPLIST

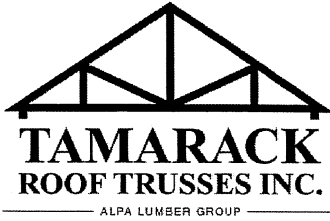


Lumber Yard: TAMARACK LUMBER
 Builder: BAYVIEW WELLINGTON
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 Location: BRADFORD
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Roof Trusses

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	1	T38 Half Hip Girder	8 /12	12-06-08	5-09-02	2 x 4 2 x 6	5-08	1-04-13 5-09-02	64.73 42.00		
	1	T39 Half Hip	8 /12	12-06-08	7-01-02	2 x 4	5-08	1-04-13 7-01-02	60.7 38.17		
	1	T40 Half Hip	8 /12	12-06-08	8-05-02	2 x 4	5-08	1-04-13 8-05-02	68.41 42.33		
	1	T41 Roof Special	8 /12	12-06-08	9-09-02	2 x 4	5-08	1-04-13 9-09-02	61.97 38.67		
	1	T42 Hip Girder	8 /12	15-04-00	3-08-02	2 x 4	1-03-08 1-03-08	2-08-13 2-08-13	73.13 48.67		
	1	T43S Hip	8 /12	15-04-00	5-00-02	2 x 4		2-08-13 1-08-13	73.09 49.17		
	1	T44S Hip	8 /12	15-04-00	6-02-02	2 x 4		2-08-13 1-08-13	74.58 48.50		
	1 2-ply	T50 Piggyback Base Girder	8 /12	32-11-00	9-03-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	371.99 231.67		
	1 2-ply	T51 Hip Girder	8 /12	32-11-00	6-02-02	2 x 4 2 x 8	1-03-08 1-03-08	1-04-13 1-04-13	376.11 233.33		
	1 2-ply	T52 Hip Girder	8 /12	9-08-00	4-07-08	2 x 4 2 x 6		2-08-03 2-08-03	106.99 70.00		
	1 2-ply	T54 Half Hip Girder	8 /12	5-10-08	4-04-02	2 x 4 2 x 6		1-04-13 4-04-02	66.87 43.00		
	1 2-ply	T55 Jack-Open Girder	8 /12	5-10-08	5-03-13	2 x 4 2 x 6		1-04-13 5-03-13	64.52 41.33		
	1 2-ply	T55S Monopitch Girder	8 /12	5-10-08	5-03-13	2 x 4		1-04-13 4-03-13	61.75 41.33		
	1	T56 Half Hip Girder	8 /12	5-10-08	2-02-13	2 x 4	1-03-08	1-04-13 2-02-13	24.71 16.50		

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 Project: GREEN VALLEY EAST 2022
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 Elevation: C REAR & SIDE UPG. + OPT.COF.

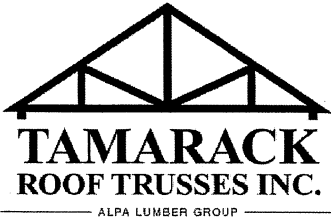
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	3	T57 Monopitch	8 /12	2-09-00	4-07-08	2 x 4	1-03-08	1-03-08 3-01-08	48.88 34.50		
	3	J7 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	70.74 44.00		
	4	J7S Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 4-03-13	89.72 58.67		
	1	J8 Jack-Open	8 /12	1-10-08	3-11-02	2 x 4	1-03-08 1-10-15	1-04-13 2-07-13	11.91 7.67		
	2	J9 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	18.34 11.33		
	1	J10 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 4-01-01	1-04-13 2-07-02	13.67 8.33		
	1	J11 Jack-Open	8 /12	3-09-07	3-11-02	2 x 4	1-03-08 2-01-01	1-04-13 3-11-02	16.5 10.33		
	6	J12 Jack-Open	8 /12	4-10-08	4-07-13	2 x 4	1-03-08	1-04-13 4-07-13	101.08 62.00		
	1	J13 Jack-Open	8 /12	3-10-08	3-11-13	2 x 4	1-03-08	1-04-13 3-11-13	14.34 9.00		
	1	J14 Jack-Open	8 /12	1-09-07	2-07-02	2 x 4	1-03-08 2-01-01	1-04-13 2-07-02	11.39 7.00		
	7	J15 Jack-Open	8 /12	1-05-00	3-08-02	2 x 4	1-03-08	2-08-13 3-08-02	71.7 52.50		
	3	J16 Jack-Open	8 /12	1-03-00	2-02-13	2 x 4	1-03-08	1-04-13 2-02-13	21.95 17.00		
	2	P3 Piggyback	8 /12	9-02-00	1-04-00	2 x 4			49.97 33.67		
	2	P4 Piggyback	8 /12	9-02-00	2-08-00	2 x 4			51.89 33.67		

REVIEWED


DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
Builder: BAYVIEW WELLINGTON
Project: GREEN VALLEY EAST 2022
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Lot #:
Elevation: C REAR & SIDE UPG. + OPT.COF.

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Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	P5 Piggyback	8 /12	9-02-00	3-00-11	2 x 4			92.72 59.33		

TOTAL # TRUSS= 93 TOTAL BFT OF ALL TRUSSES= 3952.48 BFT. TOTAL WEIGHT OF ALL TRSSES 6234.06 LBS

HARDWARE

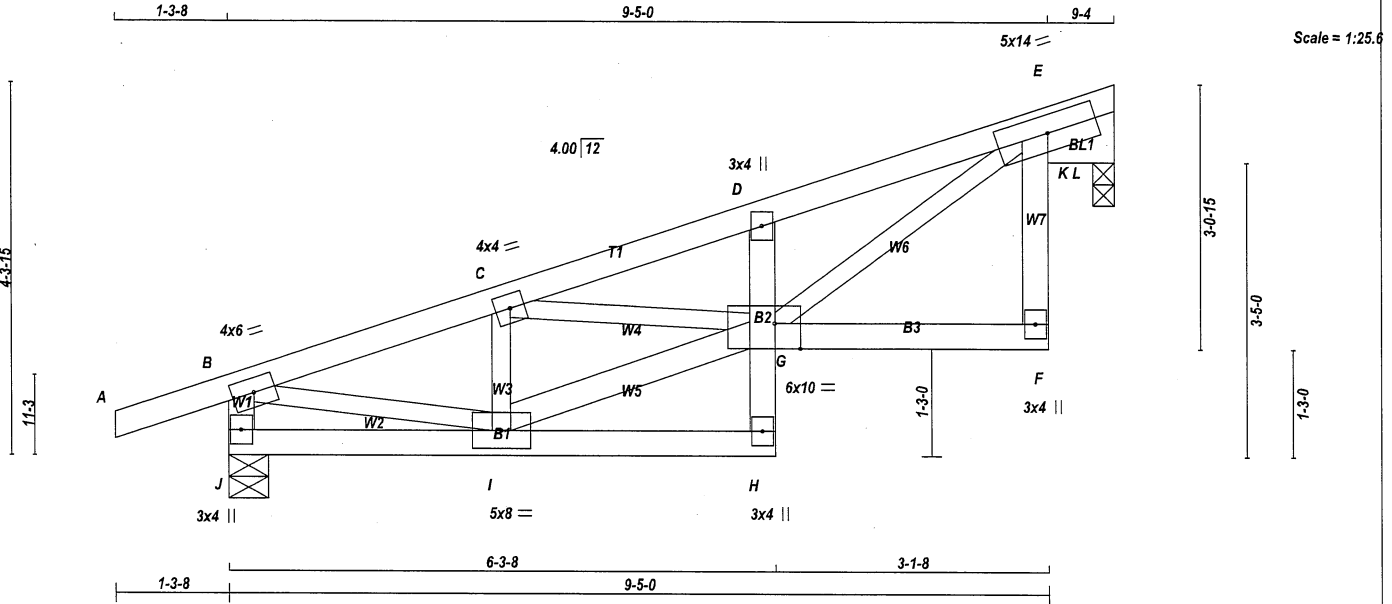
QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS24	
14	Hardware	LJS28DS	
4	Hardware	HGUS26-2	
1	Hardware	HGUS28-2	

TOTAL NUMBER OF ITEMS= 23

JOB NAME 426687	TRUSS NAME T25	QUANTITY 6	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:10 2022 Page 1
ID:cR3hXPouHfKvTr1GMcM3ucymxZo-jjfdkol9I?ukk3lw7QecoWaiAAGsH20dGynPw_zANRd



Scale = 1:25.6

TOTAL WEIGHT = 6 X 44 = 265 lb [M/F]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x10	DRY	No.2	SPF
ALL WEBS				
I - G	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	
C	TMVW-t	MT20	4.0	4.0	
D	TMV+p	MT20	3.0	4.0	
E	TMVWK1-t	MT20	5.0	14.0	
F	BMV+p	MT20	3.0	4.0	
G	BMVWWW-I	MT20	6.0	10.0	Edge 3.50
H	BMV+p	MT20	3.0	4.0	
I	BMVWWW-t	MT20	5.0	8.0	
J	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L(E)	577	0	577	0	0	3-0	3-0
J	806	0	806	0	0	5-8	5-8

UNFACTORED REACTIONS							
1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L(E)	404	286 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
J	563	412 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L(E), J
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)
FR-TO					FR-TO				
A-B	0 / 24	-112.4	-112.4 0.14 (1)	10.00	G-E	0 / 1247	0.28 (1)		
B-C	-938 / 0	-112.4	-112.4 0.13 (1)	6.25	I-C	-480 / 0	0.07 (1)		
C-D	-1175 / 0	-112.4	-112.4 0.11 (1)	5.77	I-G	0 / 931	0.15 (1)		
D-E	-1204 / 0	-112.4	-112.4 0.16 (1)	5.66	C-G	0 / 217	0.05 (1)		
F-K	0 / 34	0.0	0.0 0.41 (1)	10.00	B-I	0 / 917	0.21 (1)		
K-E	0 / 34	0.0	0.0 0.41 (1)	10.00	E-L	-941 / 0	0.04 (1)		
J-B	-778 / 0	0.0	0.0 0.08 (1)	7.81	K-L	0 / 744	0.00 (1)		
J-I	0 / 0	-18.5	-18.5 0.04 (4)	10.00					
I-H	0 / 39	-18.5	-18.5 0.04 (4)	10.00					
H-G	0 / 27	0.0	0.0 0.07 (1)	10.00					
G-D	-376 / 0	0.0	0.0 0.05 (1)	7.81					
G-F	0 / 148	-18.5	-18.5 0.07 (4)	10.00					

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF
SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015
THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF CBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014
(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD
ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")
CSI: TC=0.41/1.00 (F-K:1), BC=0.07/1.00 (G-H:1), WB=0.28/1.00 (E-G:1), SSI=0.20/1.00 (E-K:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE LEFT HEEL ONLY
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.65 (E) (INPUT = 0.90)
JSI METAL= 0.25 (E) (INPUT = 1.00)



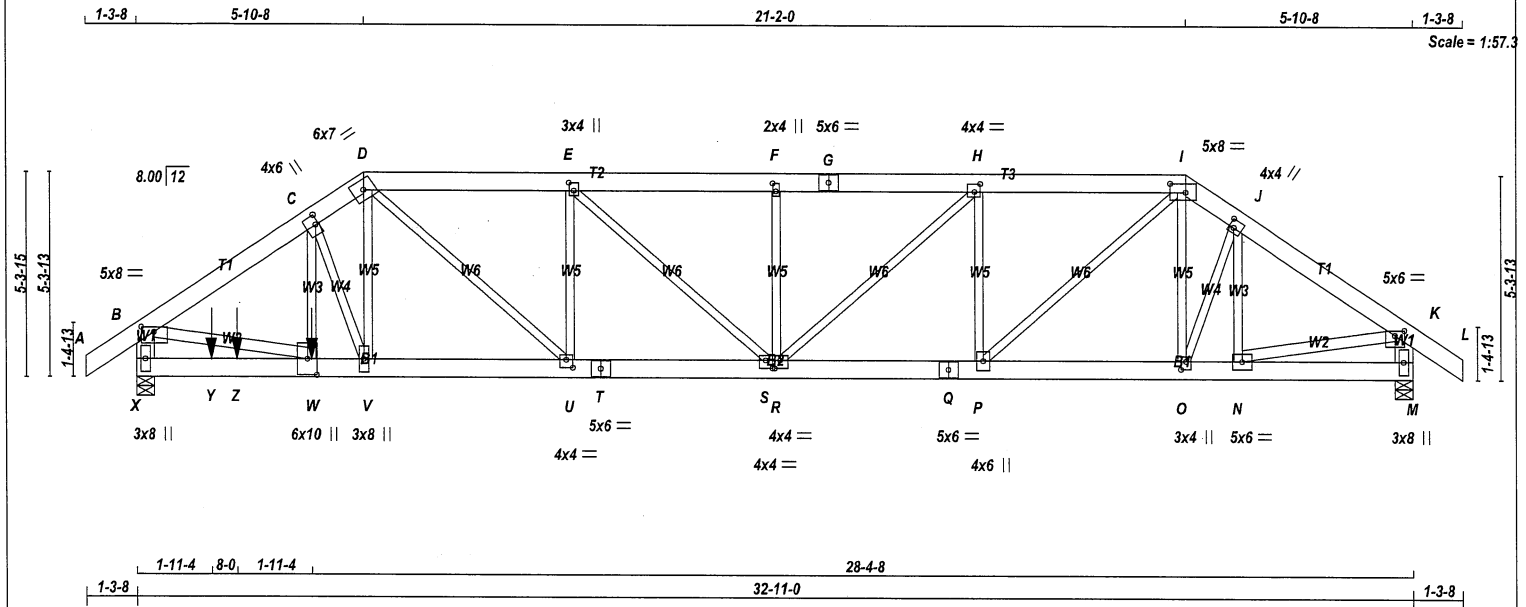
Structural component only
DWG# T-2213203

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T30	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-BvD?y8Jn2J0bLd6h79rLk7zSaVm0ynmVcWzSQzANRc



TOTAL WEIGHT = 2 X 192 = 384 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x6	DRY	No.2
D - G	2x6	DRY	No.2
G - I	2x6	DRY	No.2
I - L	2x6	DRY	No.2
L - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
X - T	2x6	DRY	No.2
T - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - W	2x4	DRY	No.2
N - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D	2	12
D-G	2	12
G-I	2	12
I-L	2	12
X-B	2	12
M-K	2	12
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-T	2	12
T-Q	2	12
Q-M	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
C-W	1	2
2x3	1	6
2x4	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX
X	6194	0	0	6194	0	0	0	5-8	5-8
M	2910	0	0	2910	0	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	4327	3140 / 0	0 / 0	0 / 0	0 / 0	1187 / 0	0
M	2035	1466 / 0	0 / 0	0 / 0	0 / 0	568 / 0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					W E B S				
MAX. FACTORED		FACTORED		MAX. FACTORED					
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 44	-112.4 -112.4	0.04 (1)	10.00	W-C	0 / 2659	0.33 (1)		
B-C	-7604 / 0	-112.4 -112.4	0.25 (1)	4.17	C-V	-2431 / 0	0.35 (1)		
C-D	-6515 / 0	-112.4 -112.4	0.14 (1)	4.56	V-D	0 / 2781	0.34 (1)		
D-E	-5987 / 0	-112.4 -112.4	0.17 (1)	4.72	O-I	0 / 90	0.01 (1)		
E-F	-5696 / 0	-112.4 -112.4	0.16 (1)	4.82	O-J	0 / 77	0.01 (1)		
F-G	-5696 / 0	-112.4 -112.4	0.16 (1)	4.82	N-J	-601 / 0	0.08 (1)		
G-H	-5696 / 0	-112.4 -112.4	0.16 (1)	4.82	B-W	0 / 6423	0.57 (1)		
H-I	-4699 / 0	-112.4 -112.4	0.14 (1)	5.22	N-K	0 / 2918	0.26 (1)		
I-J	-3558 / 0	-112.4 -112.4	0.07 (1)	5.88	P-I	0 / 2443	0.30 (1)		
J-K	-3424 / 0	-112.4 -112.4	0.10 (1)	5.93	D-U	0 / 625	0.08 (1)		
K-L	0 / 44	-112.4 -112.4	0.04 (1)	10.00	P-H	-1570 / 0	0.30 (1)		
X-B	-5914 / 0	0.0 0.0	0.21 (1)	6.07	U-E	-361 / 0	0.07 (1)		
M-K	-2859 / 0	0.0 0.0	0.10 (1)	7.81	R-H	0 / 1362	0.17 (1)		
					E-S	-404 / 0	0.19 (1)		
					R-F	-552 / 0	0.10 (1)		
X-Y	0 / 0	-18.5 -18.5	0.21 (1)	10.00					
Y-Z	0 / 0	-18.5 -18.5	0.21 (1)	10.00					
Z-W	0 / 0	-18.5 -18.5	0.21 (1)	10.00					
W-V	0 / 6301	-18.5 -18.5	0.54 (1)	10.00					
V-U	0 / 5525	-18.5 -18.5	0.43 (1)	10.00					
U-T	0 / 5987	-18.5 -18.5	0.43 (1)	10.00					
T-S	0 / 5987	-18.5 -18.5	0.43 (1)	10.00					
S-R	0 / 5696	-18.5 -18.5	0.40 (1)	10.00					
R-Q	0 / 4700	-18.5 -18.5	0.33 (1)	10.00					
Q-P	0 / 4700	-18.5 -18.5	0.33 (1)	10.00					
P-O	0 / 2891	-18.5 -18.5	0.22 (1)	10.00					
O-N	0 / 2863	-18.5 -18.5	0.21 (1)	10.00					
N-M	0 / 0	-18.5 -18.5	0.03 (4)	10.00					

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	4-6-8	-2897	-2897		BACK	VERT	TOTAL		C1
Y	1-11-4	-113	-113		BACK	VERT	TOTAL		C1
Z	2-7-4	-113	-113		BACK	VERT	TOTAL		C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.25/1.00 (B-C-1), BC=0.54/1.00 (V-W-1), WB=0.57/1.00 (B-W-1), SSI=0.13/1.00 (V-W-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.95 (W) (INPUT = 1.00)



Structural component only
DWG# T-2213204

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T30	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID: msZCRikkNthSDqhrbA0REzSQw2-BvD?y8Jn2J0bLDt6h79rLk7zSaVm0ynmVcWzSQzANRc

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	4.00
C	TMWW+t	MT20	4.0	6.0	3.00	1.00
D	TTWW-h	MT20	6.0	7.0		
E	TMWW+t	MT20	3.0	4.0	2.50	1.50
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.50	1.75
I	TTWW-l	MT20	5.0	8.0	2.75	4.75
J	TMWW+t	MT20	4.0	4.0	2.50	1.50
K	TMVW-p	MT20	5.0	6.0	1.50	3.00
M	BMV1+p	MT20	3.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMWW+t	MT20	3.0	4.0	2.50	1.50
P	BMWW+t	MT20	4.0	6.0		
Q	BS-l	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	1.75
S	BMW-t	MT20	4.0	4.0	2.50	2.00
T	BS-l	MT20	5.0	6.0		
U	BMWW-t	MT20	4.0	4.0	2.50	2.00
V	BMWW+t	MT20	3.0	8.0		
W	BMWW+t	MT20	6.0	10.0	5.00	2.75
X	BMV1+p	MT20	3.0	8.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

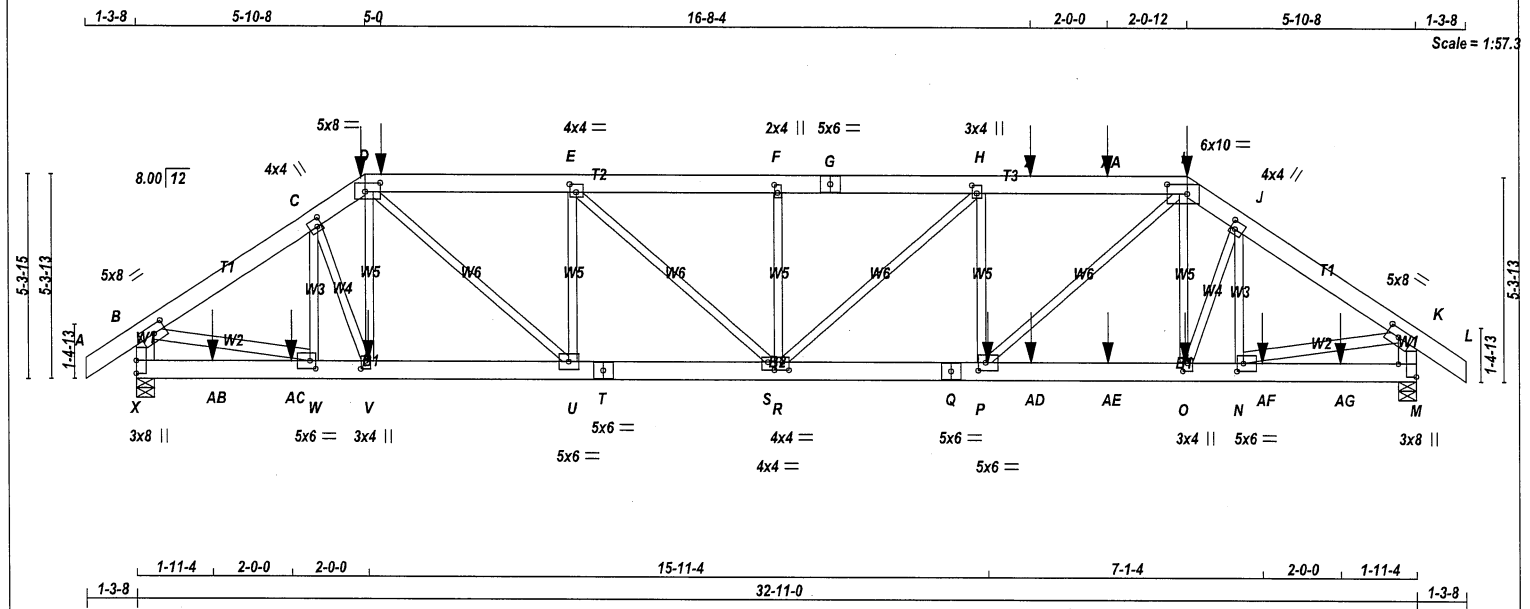


Structural component only
DWG# T-2213204

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T30Z	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T30Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:12 2022 Page 2

ID: msZCRjkkNthSDqhrbA0REzSQw2-g5mO9TKQpd8SzNSJFrg4t58zpPIPVvjGGW?tzANRb

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TMWW-t	MT20	4.0	4.0	2.50	1.50
D	TTWW-t	MT20	5.0	8.0	2.75	4.75
E	TMWW-t	MT20	4.0	4.0	2.50	2.00
F	TMW-w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	3.0	4.0	2.50	1.50
I	TTWW-t	MT20	6.0	10.0	3.00	6.25
J	TMWW-t	MT20	4.0	4.0	2.50	1.50
K	TMVW-t	MT20	5.0	8.0	2.50	3.75
M	BMV1+p	MT20	3.0	8.0	4.00	Edge
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW-t	MT20	3.0	4.0	2.50	1.50
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	4.0	4.0	2.50	2.00
S	BMW-t	MT20	4.0	4.0	2.50	2.00
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW-t	MT20	3.0	4.0	2.50	1.50
W	BMWW-t	MT20	5.0	6.0	2.50	1.75
X	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	6-3-8	-1438	-1438	---	FRONT	VERT	TOTAL	---	C1
Z	22-11-12	-53	-53	---	FRONT	VERT	TOTAL	---	C1
AA	24-11-12	-53	-53	---	FRONT	VERT	TOTAL	---	C1
AB	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AC	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AD	22-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AE	24-11-12	-113	-113	---	FRONT	VERT	TOTAL	---	C1
AF	28-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AG	30-11-12	-29	-29	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS
1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

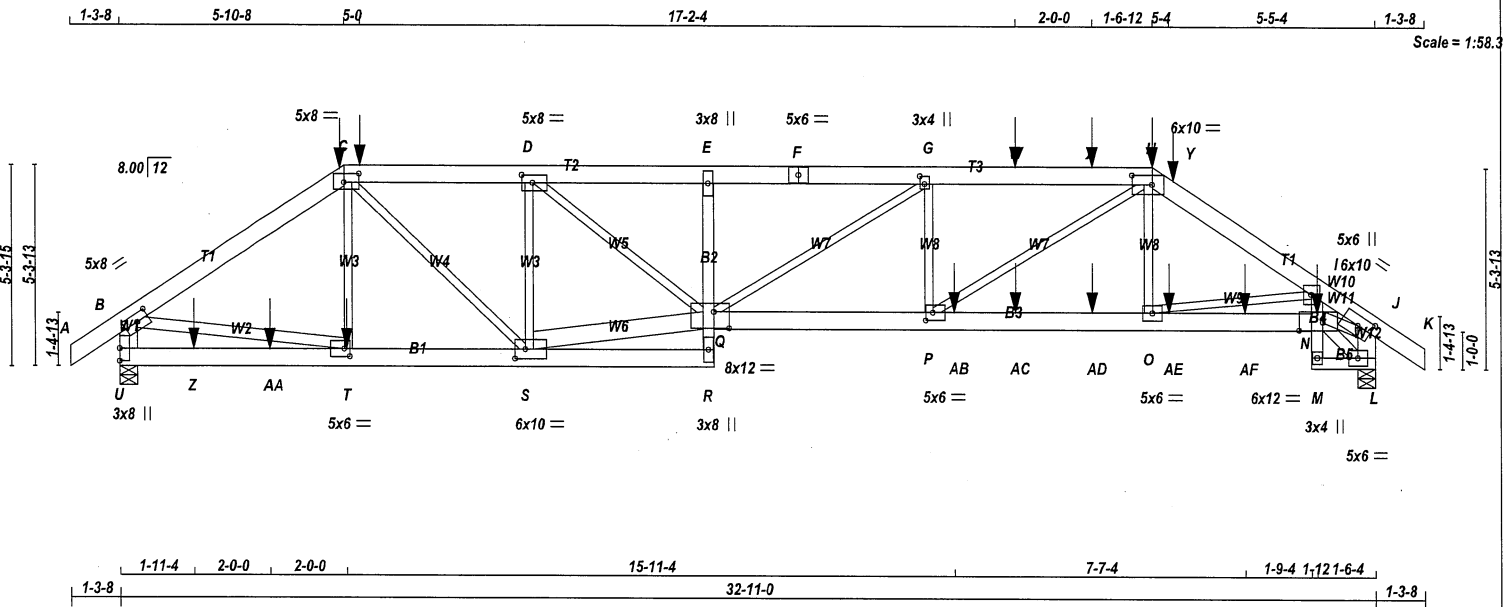


Structural component only
DWG# T-2213205

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426688	T30S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-nhCYp0L5xjTcMXgTYJEjtqef66xjSJwbCAhAtxAMui



TOTAL WEIGHT = 2 X 191 = 382 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x6	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - K	2x6	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x6	DRY	2100F 1.8E	SPF
R - E	2x4	DRY	No.2	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
M - I	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
N - J	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 2	12	SIDE(122.0)
C - F 2	12	SIDE(122.0)
F - H 2	12	SIDE(61.0)
H - K 2	12	SIDE(122.0)
U - B 2	12	TOP
L - J 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
U - R 2	12	SIDE(197.8)
Q - N 2	12	SIDE(65.9)
R - E 1	12	TOP
I - M 1	12	SIDE(23.0)
M - L 1	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
T - C 1	6	SIDE(425.6)
2x3 1	6	
2x4 1	6	
2x6 2	6	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
U	5679	0	5-8	5-8
L	5680	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
U	3974 2843 / 0 0 / 0 0 / 0 1131 / 0 0 / 0
L	3972 2861 / 0 0 / 0 0 / 0 1111 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 44	-112.4 -112.4 0.04 (1)	T-C	-636 / 0
B-C	-7480 / 0	-112.4 -112.4 0.29 (1)	C-S	0 / 2466
C-V	-7935 / 0	-112.4 -112.4 0.44 (1)	S-D	-3296 / 0
V-D	-7935 / 0	-112.4 -112.4 0.44 (1)	S-Q	0 / 7937
D-E	-10948 / 0	-112.4 -112.4 0.36 (1)	D-Q	0 / 3835
E-F	-11004 / 0	-112.4 -112.4 0.35 (1)	Q-H	0 / 870
F-G	-11004 / 0	-112.4 -112.4 0.35 (1)	Q-I	-2933 / 0
G-W	-11493 / 0	-112.4 -112.4 0.46 (1)	B-T	0 / 6282
W-X	-11493 / 0	-112.4 -112.4 0.46 (1)	N-L	-479 / 0
X-H	-11493 / 0	-112.4 -112.4 0.46 (1)	N-J	0 / 9898
H-Y	-8867 / 0	-112.4 -112.4 0.50 (1)	P-H	0 / 4759
Y-I	-8867 / 0	-112.4 -112.4 0.50 (1)	Q-G	-589 / 0
I-J	-12033 / 0	-112.4 -112.4 0.54 (1)	P-G	-512 / 0
J-K	0 / 44	-112.4 -112.4 0.04 (1)		
U-B	-5587 / 0	0.0 0.0 0.20 (1)		
L-J	-5354 / 0	0.0 0.0 0.19 (1)		
U-Z	0 / 0	-18.5 -18.5 0.04 (4)		
Z-AA	0 / 0	-18.5 -18.5 0.04 (4)		
AA-T	0 / 0	-18.5 -18.5 0.04 (4)		
T-S	0 / 6198	-18.5 -18.5 0.17 (1)		
S-R	0 / 214	-18.5 -18.5 0.02 (1)		
R-Q	0 / 47	-18.5 -18.5 0.16 (1)		
Q-E	-431 / 0	0.0 0.0 0.17 (1)		
Q-P	0 / 11493	-18.5 -18.5 0.36 (1)		
P-AB	0 / 7517	-18.5 -18.5 0.40 (1)		
AB-AC	0 / 7517	-18.5 -18.5 0.40 (1)		
AC-AD	0 / 7517	-18.5 -18.5 0.40 (1)		
AD-O	0 / 7517	-18.5 -18.5 0.40 (1)		
O-AE	0 / 10372	-18.5 -18.5 0.39 (1)		
AE-AF	0 / 10372	-18.5 -18.5 0.39 (1)		
AF-N	0 / 10372	-18.5 -18.5 0.39 (1)		
M-N	0 / 16	0.0 0.0 0.36 (1)		
N-I	0 / 2603	0.0 0.0 0.58 (1)		
M-L	0 / 363	-18.5 -18.5 0.04 (1)		

CHORDS	MAX. FACTORED	FACTORED	W E B S	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	MEMB.	FORCE
	(LBS)	(PLF)		(LBS)
FR-TO		FROM TO	FR-TO	
A-B	0 / 44	-112.4 -112.4 0.04 (1)	T-C	-636 / 0
B-C	-7480 / 0	-112.4 -112.4 0.29 (1)	C-S	0 / 2466
C-V	-7935 / 0	-112.4 -112.4 0.44 (1)	S-D	-3296 / 0
V-D	-7935 / 0	-112.4 -112.4 0.44 (1)	S-Q	0 / 7937
D-E	-10948 / 0	-112.4 -112.4 0.36 (1)	D-Q	0 / 3835
E-F	-11004 / 0	-112.4 -112.4 0.35 (1)	Q-H	0 / 870
F-G	-11004 / 0	-112.4 -112.4 0.35 (1)	Q-I	-2933 / 0
G-W	-11493 / 0	-112.4 -112.4 0.46 (1)	B-T	0 / 6282
W-X	-11493 / 0	-112.4 -112.4 0.46 (1)	N-L	-479 / 0
X-H	-11493 / 0	-112.4 -112.4 0.46 (1)	N-J	0 / 9898
H-Y	-8867 / 0	-112.4 -112.4 0.50 (1)	P-H	0 / 4759
Y-I	-8867 / 0	-112.4 -112.4 0.50 (1)	Q-G	-589 / 0
I-J	-12033 / 0	-112.4 -112.4 0.54 (1)	P-G	-512 / 0
J-K	0 / 44	-112.4 -112.4 0.04 (1)		
U-B	-5587 / 0	0.0 0.0 0.20 (1)		
L-J	-5354 / 0	0.0 0.0 0.19 (1)		
U-Z	0 / 0	-18.5 -18.5 0.04 (4)		
Z-AA	0 / 0	-18.5 -18.5 0.04 (4)		
AA-T	0 / 0	-18.5 -18.5 0.04 (4)		
T-S	0 / 6198	-18.5 -18.5 0.17 (1)		
S-R	0 / 214	-18.5 -18.5 0.02 (1)		
R-Q	0 / 47	-18.5 -18.5 0.16 (1)		
Q-E	-431 / 0	0.0 0.0 0.17 (1)		
Q-P	0 / 11493	-18.5 -18.5 0.36 (1)		
P-AB	0 / 7517	-18.5 -18.5 0.40 (1)		
AB-AC	0 / 7517	-18.5 -18.5 0.40 (1)		
AC-AD	0 / 7517	-18.5 -18.5 0.40 (1)		
AD-O	0 / 7517	-18.5 -18.5 0.40 (1)		
O-AE	0 / 10372	-18.5 -18.5 0.39 (1)		
AE-AF	0 / 10372	-18.5 -18.5 0.39 (1)		
AF-N	0 / 10372	-18.5 -18.5 0.39 (1)		
M-N	0 / 16	0.0 0.0 0.36 (1)		
N-I	0 / 2603	0.0 0.0 0.58 (1)		
M-L	0 / 363	-18.5 -18.5 0.04 (1)		

LOC.	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	5-10-8	-395	-395	FRONT	VERT	TOTAL	C1
C	27-0-8	-337	-337	FRONT	VERT	TOTAL	C1
H	31-4-12	-235	-235	FRONT	VERT	TOTAL	C1
N	5-11-4	-113	-113	FRONT	VERT	TOTAL	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 32.5	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 0.0	PSF
	DL	= 7.4	PSF
TOTAL LOAD		= 45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.10")
CALCULATED VERT. DEFL. (LL) = L/999 (0.25")
ALLOWABLE DEFL. (TL) = L/360 (1.10")
CALCULATED VERT. DEFL. (TL) = L/922 (0.43")

CSI: TC=0.54/1.00 (I-J:1), BC=0.58/1.00 (I-N:1), WB=0.63/1.00 (D-S:1), SSI=0.70/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.



Structural component only
DWG# T-2213231

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426688	T30S	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TTWW-I	MT20	5.0	8.0	2.75	4.75
D	TMWW-t	MT20	5.0	8.0	2.50	3.50
E	TMV+p	MT20	3.0	8.0		
F	TS-I	MT20	5.0	6.0		
G	TMWW+t	MT20	3.0	4.0	2.50	1.50
H	TTWW-I	MT20	6.0	10.0	3.00	6.25
I	TMVW+p	MT20	5.0	6.0		
J	TMVW-t	MT20	6.0	10.0	3.00	4.50
L	BMVW1-t	MT20	5.0	6.0		
M	BMV+p	MT20	3.0	4.0		
N	BVMWW-I	MT20	6.0	12.0	2.50	7.50
O	BMWW-t	MT20	5.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.25
Q	BVMWWW-I	MT20	8.0	12.0	5.25	4.75
R	BMV+p	MT20	3.0	8.0		
S	BMWWW-t	MT20	6.0	10.0	3.00	3.25
T	BMWW-t	MT20	5.0	6.0	2.50	1.75
U	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	6-3-8	-1469	-1469	---	FRONT	VERT	TOTAL	---	C1
W	23-5-12	-88	-88	---	FRONT	VERT	TOTAL	---	C1
X	25-5-12	-88	-88	---	FRONT	VERT	TOTAL	---	C1
Y	27-5-12	-110	-110	---	FRONT	VERT	TOTAL	---	C1
Z	1-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AA	3-11-4	-29	-29	---	FRONT	VERT	TOTAL	---	C1
AB	21-10-8	-1505	-1505	---	FRONT	VERT	TOTAL	---	C1
AC	23-5-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AD	25-5-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AE	27-5-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1
AF	29-5-12	-78	-78	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

JSI GRIP= 0.89 (B) (INPUT = 0.90)

JSI METAL= 0.87 (J) (INPUT = 1.00)



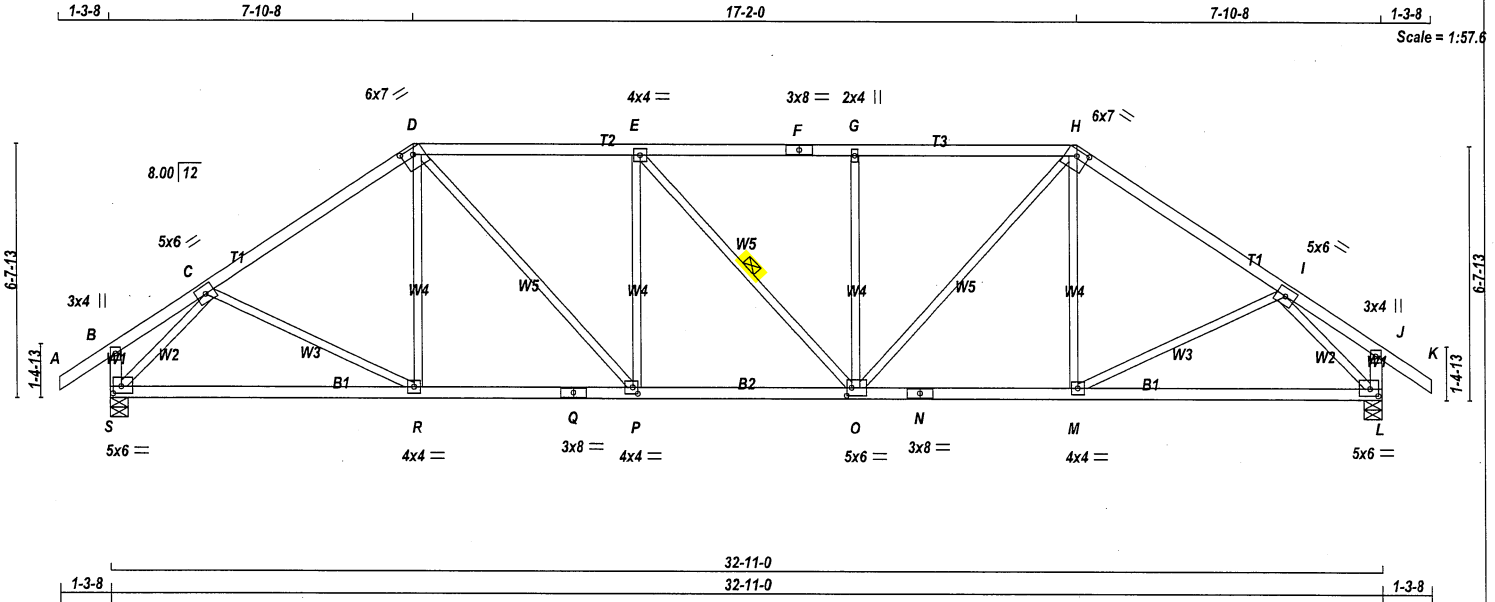
Structural component only

DWG# T-2213231

JOB NAME 426687	TRUSS NAME T31	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 140 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
S - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
S - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0		
D	TTWW-h	MT20	6.0	7.0	1.75	3.50
E	TMVW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMVW-w	MT20	2.0	4.0		
H	TTWW-h	MT20	6.0	7.0	1.75	3.50
I	TMVW-t	MT20	5.0	6.0		
J	TMV+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0	2.25	2.50
M	BMVW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMVW-t	MT20	5.0	6.0	2.50	1.50
P	BMVW-t	MT20	4.0	4.0	2.00	1.50
Q	BS-t	MT20	3.0	8.0		
R	BMVW-t	MT20	4.0	4.0		
S	BMVW-t	MT20	5.0	6.0	2.25	2.50

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
S	2309	0	2309	0	0	5-8	5-8		
L	2309	0	2309	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

C H O R D S		FACTORED		MAX. FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	LC1
FR-TO		FROM	TO	CSI (LC)	LENGTH	FR-TO			
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-R	0 / 197	0.04 (1)	
B-C	0 / 62	-112.4	-112.4	0.38 (1)	10.00	R-D	0 / 117	0.04 (1)	
C-D	-2496 / 0	-112.4	-112.4	0.60 (1)	3.81	D-P	0 / 1064	0.24 (1)	
D-E	-2765 / 0	-112.4	-112.4	0.71 (1)	3.43	P-E	-695 / 0	0.51 (1)	
E-F	-2765 / 0	-112.4	-112.4	0.71 (1)	3.43	E-O	-3 / 0	0.00 (1)	
F-G	-2765 / 0	-112.4	-112.4	0.71 (1)	3.43	O-G	-694 / 0	0.51 (1)	
G-H	-2765 / 0	-112.4	-112.4	0.71 (1)	3.44	H-H	0 / 1061	0.24 (1)	
H-I	-2496 / 0	-112.4	-112.4	0.60 (1)	3.81	M-H	0 / 117	0.04 (1)	
I-J	0 / 62	-112.4	-112.4	0.38 (1)	10.00	M-I	0 / 197	0.04 (1)	
J-K	0 / 43	-112.4	-112.4	0.15 (1)	10.00	S-C	-2789 / 0	0.66 (1)	
S-B	-183 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-2789 / 0	0.66 (1)	
L-J	-183 / 0	0.0	0.0	0.02 (1)	7.81				
S-R	0 / 1874	-18.5	-18.5	0.44 (1)	10.00				
R-Q	0 / 2052	-18.5	-18.5	0.47 (1)	10.00				
Q-P	0 / 2052	-18.5	-18.5	0.47 (1)	10.00				
P-O	0 / 2767	-18.5	-18.5	0.51 (1)	10.00				
O-N	0 / 2052	-18.5	-18.5	0.47 (1)	10.00				
N-M	0 / 2052	-18.5	-18.5	0.47 (1)	10.00				
M-L	0 / 1874	-18.5	-18.5	0.44 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CSI: TC=0.71/1.00 (D-E:1), BC=0.51/1.00 (O-P:1), WB=0.66/1.00 (H-L:1), SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.64 (I) (INPUT = 1.00)

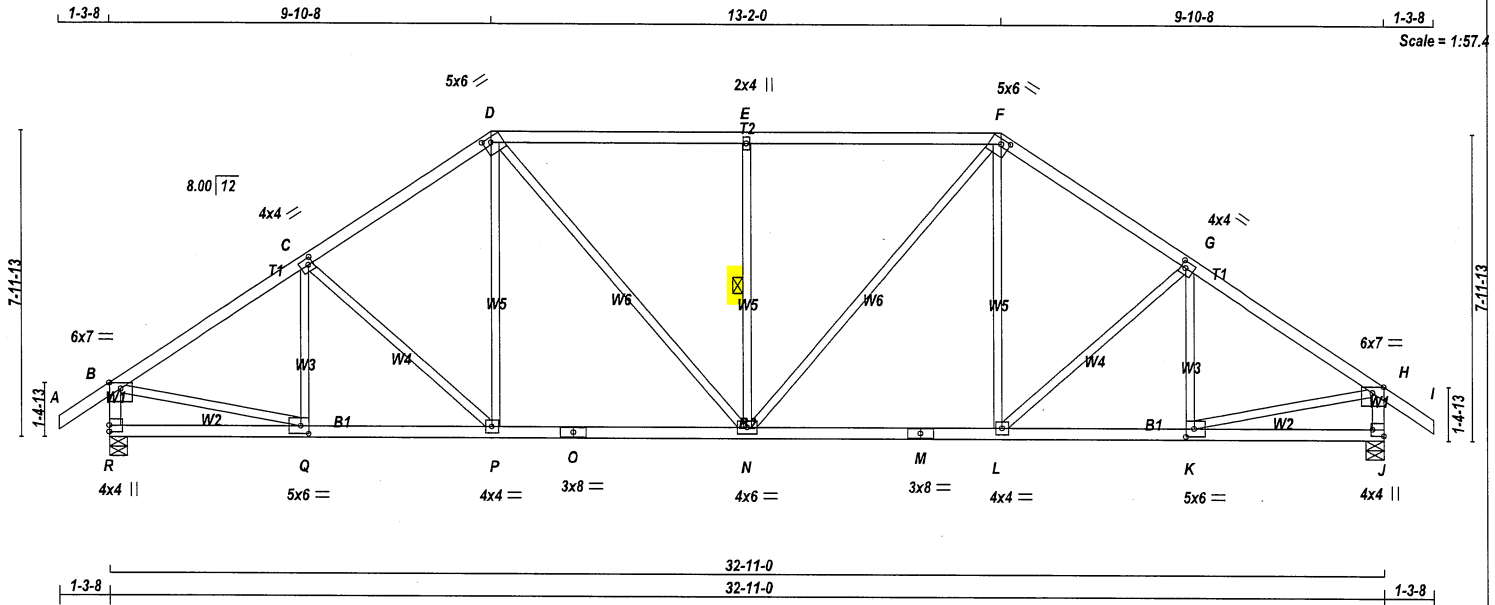


Structural component only
DWG# T-2213206

JOB NAME 426687	TRUSS NAME T32	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-cUu8a9LgLEACgchMGiYyML6nYNDKfCBald3lzANRZ



TOTAL WEIGHT = 145 lb
[M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-h	MT20	5.0	6.0	1.50	2.50
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-h	MT20	5.0	6.0	1.50	2.50
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	6.0	7.0	Edge	
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-w	MT20	4.0	4.0		
O	BS-t	MT20	3.0	8.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
R	2309	0	2309	0	0	5-8	5-8		
J	2309	0	2309	0	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0
J	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN
THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)	Q-C	-370 / 0	0.12 (1)	
B-C	-2547 / 0	-112.4 -112.4	0.45 (1)	C-P	-276 / 0	0.22 (1)	
C-D	-2376 / 0	-112.4 -112.4	0.43 (1)	P-D	0 / 296	0.07 (1)	
D-E	-2390 / 0	-112.4 -112.4	0.73 (1)	D-N	0 / 679	0.15 (1)	
E-F	-2390 / 0	-112.4 -112.4	0.73 (1)	N-E	-909 / 0	0.37 (1)	
F-G	-2376 / 0	-112.4 -112.4	0.43 (1)	N-F	0 / 679	0.15 (1)	
G-H	-2547 / 0	-112.4 -112.4	0.45 (1)	L-F	0 / 296	0.07 (1)	
H-I	0 / 43	-112.4 -112.4	0.15 (1)	L-G	-276 / 0	0.22 (1)	
R-B	-2267 / 0	0.0 0.0	0.23 (1)	K-G	-370 / 0	0.12 (1)	
J-H	-2267 / 0	0.0 0.0	0.23 (1)	B-Q	0 / 2198	0.49 (1)	
				K-H	0 / 2198	0.49 (1)	
R-Q	0 / 0	-18.5 -18.5	0.10 (4)				
Q-P	0 / 2149	-18.5 -18.5	0.42 (1)				
P-O	0 / 1948	-18.5 -18.5	0.39 (1)				
O-N	0 / 1948	-18.5 -18.5	0.39 (1)				
N-M	0 / 1948	-18.5 -18.5	0.39 (1)				
M-L	0 / 1948	-18.5 -18.5	0.39 (1)				
L-K	0 / 2149	-18.5 -18.5	0.42 (1)				
K-J	0 / 0	-18.5 -18.5	0.10 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F.
RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED
ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.21")

CSI: TC=0.73/1.00 (D-E:1), BC=0.42/1.00 (K-L:1),
WB=0.49/1.00 (H-K:1), SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN THE
TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (K) (INPUT = 0.90)
JSI METAL= 0.62 (O) (INPUT = 1.00)

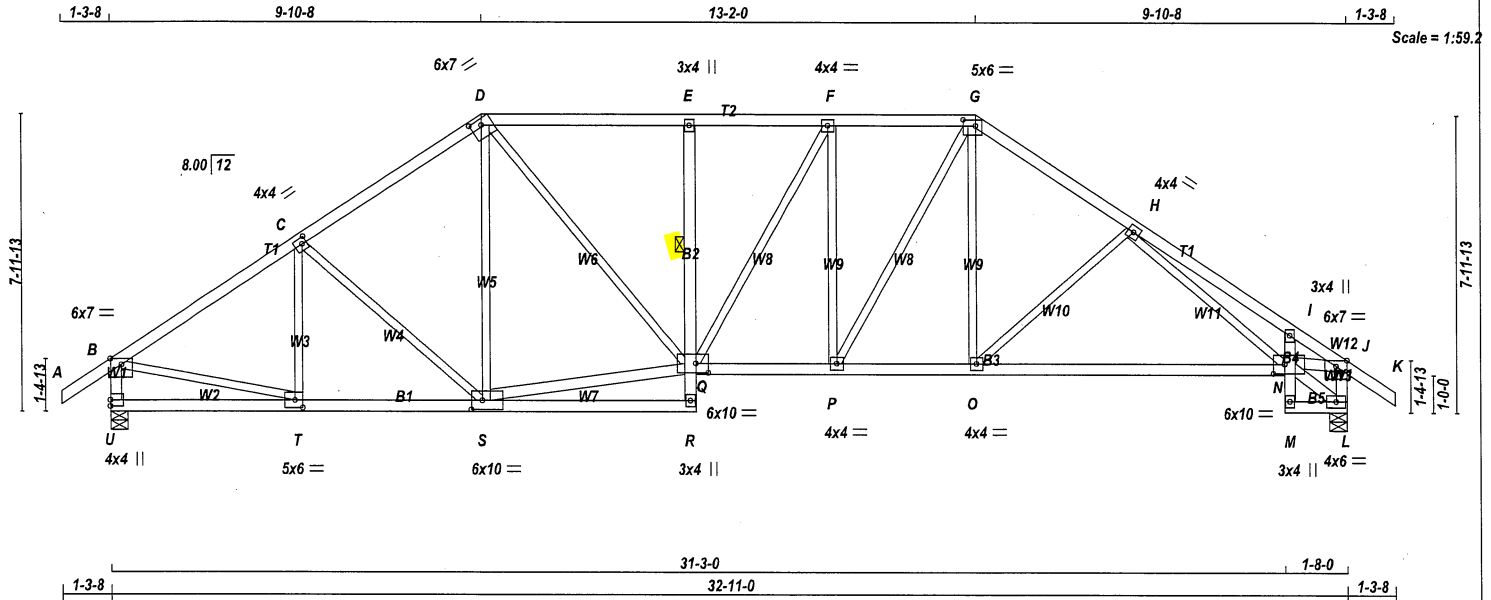


Structural component only
DWG# T-2213208

JOB NAME 426688	TRUSS NAME T32S	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:11:57 2022 Page 1
ID: _msZCRjkkNthSDqhrbA0REzSQw2-VpENZWuf4tVMVRUG2ZI4i3MBs0g4WnYFXPvYIGzANIW



TOTAL WEIGHT = 162 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - K	2x4	DRY	No.2
U - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
U - R	2x4	DRY	No.2
R - E	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
M - I	2x4	DRY	No.2
M - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
S - Q	2x4	DRY	No.2
N - L	2x4	DRY	No.2
N - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTWW-h	MT20	6.0	7.0	1.75	3.50
E	TMV-p	MT20	3.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW-i	MT20	5.0	6.0	2.00	4.00
H	TMVW-t	MT20	4.0	4.0		
I	TMV-p	MT20	3.0	4.0		
J	TMVW-p	MT20	6.0	7.0	Edge	
L	BMVW-t	MT20	4.0	6.0		
M	BMV-p	MT20	3.0	4.0		
N	BMVW-i	MT20	6.0	10.0	3.00	3.50
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-i	MT20	6.0	10.0	3.00	4.00
R	BMV-p	MT20	3.0	4.0		
S	BMVW-t	MT20	6.0	10.0	3.00	3.50
T	BMVW-t	MT20	5.0	6.0	2.50	2.50
U	BMV1-p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT	2306	0	2306	0
U	2312	0	2312	0
L	2312	0	2312	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
JT	1614	1156 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0	
U	1618	1160 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	T-C	-372 / 0
B-C	-2543 / 0	-112.4	-112.4	0.45 (1)	3.91	C-S	-275 / 0
C-D	-2375 / 0	-112.4	-112.4	0.43 (1)	4.05	S-D	-43 / 69
D-E	-2669 / 0	-112.4	-112.4	0.51 (1)	3.74	S-Q	0 / 1928
E-F	-2676 / 0	-112.4	-112.4	0.39 (1)	3.86	D-Q	0 / 1136
F-G	-2583 / 0	-112.4	-112.4	0.27 (1)	4.07	O-G	0 / 515
G-H	-2715 / 0	-112.4	-112.4	0.35 (1)	3.91	O-H	-537 / 0
H-I	-3276 / 0	-112.4	-112.4	0.40 (1)	3.56	H-N	0 / 165
I-J	-3240 / 0	-112.4	-112.4	0.22 (1)	3.74	B-T	0 / 2194
J-K	0 / 43	-112.4	-112.4	0.15 (1)	10.00	P-G	0 / 695
U-B	-2263 / 0	0.0	0.0	0.23 (1)	5.66	Q-F	0 / 188
L-J	-2224 / 0	0.0	0.0	0.23 (1)	5.71	P-F	-565 / 0
					N-L	-135 / 0	
					N-J	0 / 2665	
U-T	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
T-S	0 / 2145	-18.5	-18.5	0.42 (1)	10.00		
S-R	0 / 45	-18.5	-18.5	0.13 (4)	10.00		
R-Q	0 / 46	0.0	0.0	0.07 (1)	10.00		
Q-E	-618 / 0	0.0	0.0	0.08 (1)	6.25		
Q-P	0 / 2584	-18.5	-18.5	0.46 (1)	10.00		
P-O	0 / 2238	-18.5	-18.5	0.52 (1)	10.00		
O-N	0 / 2631	-18.5	-18.5	0.58 (1)	10.00		
M-N	0 / 16	0.0	0.0	0.16 (1)	10.00		
N-I	-240 / 0	0.0	0.0	0.12 (1)	7.81		
M-L	0 / 113	-18.5	-18.5	0.03 (1)	10.00		

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.10")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.14")
ALLOWABLE DEFL. (TL) = $L/360$ (1.10")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.29")

CSI: TC=0.51/1.00 (D-E:1), BC=0.58/1.00 (N-O:1),
WB=0.49/1.00 (B-T:1), SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

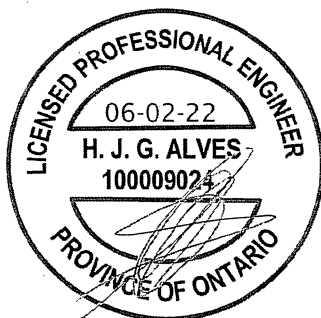
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.50 (N) (INPUT = 1.00)

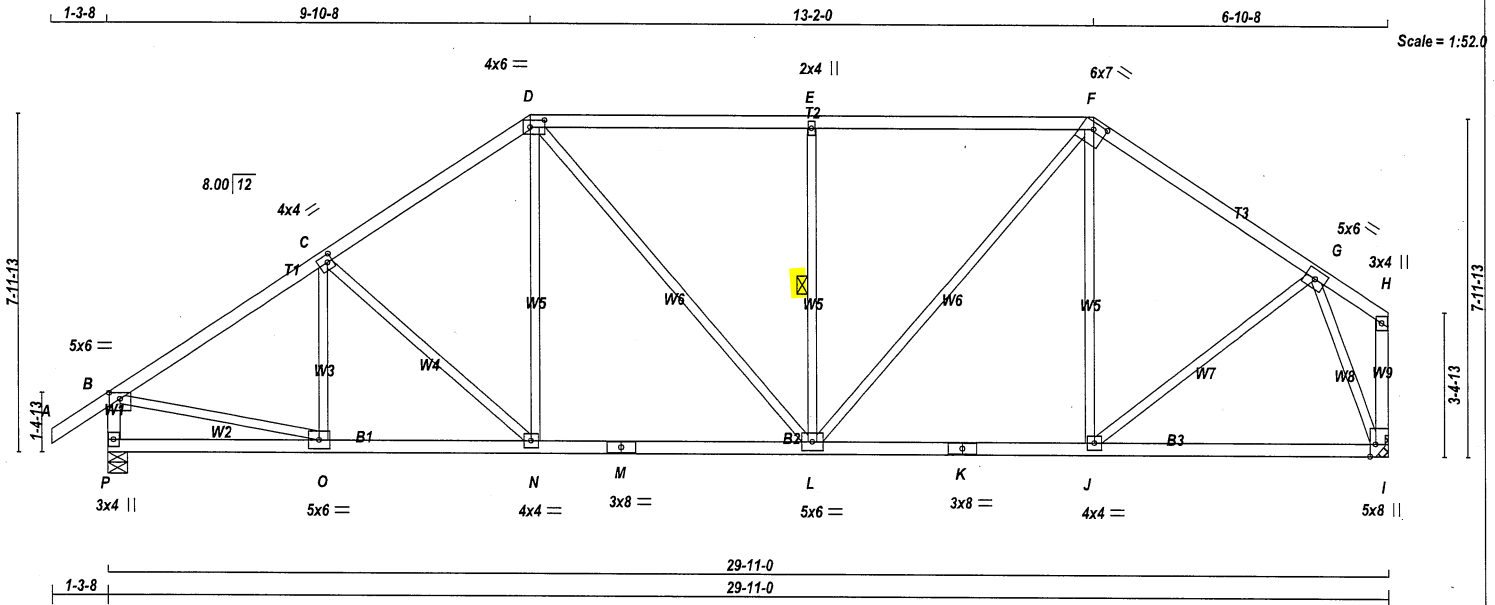


Structural component only
DWG# T-2213233

JOB NAME 426687	TRUSS NAME T32A	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:15 2022 Page 1
ID: _msZCRjkkNthSDqhrbA0REzSQw2-4gSWoVMI6YW1qqBuwzEnVaHXQB7ykIMPEUAbCzANRY



TOTAL WEIGHT = 134 lb
[M][F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TMW-w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	1.75	3.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV-p	MT20	3.0	4.0		
I	BMVW1+t	MT20	5.0	8.0	Edge	1.50
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER								
BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
P	2113	0	2113	0	0	5-8	5-8	
I	1958	0	1958	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1478	1061 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0
I	1372	972 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	O-C	-322 / 0
B-C	-2280 / 0	-112.4	-112.4	0.43 (1)	4.12	C-N	-317 / 0
C-D	-2073 / 0	-112.4	-112.4	0.41 (1)	4.30	N-D	0 / 325
D-E	-1968 / 0	-112.4	-112.4	0.70 (1)	3.88	D-L	0 / 419
E-F	-1968 / 0	-112.4	-112.4	0.70 (1)	3.88	L-E	-911 / 0
F-G	-1675 / 0	-112.4	-112.4	0.40 (1)	4.71	F-F	0 / 930
G-H	0 / 92	-112.4	-112.4	0.39 (1)	10.00	J-F	-314 / 3
P-B	-2071 / 0	0.0	0.0	0.21 (1)	5.88	J-G	0 / 740
I-H	0 / 72	0.0	0.0	0.01 (1)	10.00	B-O	0 / 1970
						G-I	-2130 / 0
P-O	0 / 0	-18.5	-18.5	0.10 (4)	10.00		
O-N	0 / 1927	-18.5	-18.5	0.38 (1)	10.00		
N-M	0 / 1695	-18.5	-18.5	0.35 (1)	10.00		
M-L	0 / 1695	-18.5	-18.5	0.35 (1)	10.00		
L-K	0 / 1363	-18.5	-18.5	0.34 (1)	10.00		
K-J	0 / 1363	-18.5	-18.5	0.34 (1)	10.00		
J-I	0 / 787	-18.5	-18.5	0.26 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.00")
CALCULATED VERT. DEFL.(LL)= L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.00")
CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CSI: TC=0.70/1.00 (E-F-1), BC=0.38/1.00 (N-O-1), WB=0.70/1.00 (G-I-1), SSI=0.36/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (O) (INPUT = 0.90)
JSI METAL= 0.55 (M) (INPUT = 1.00)

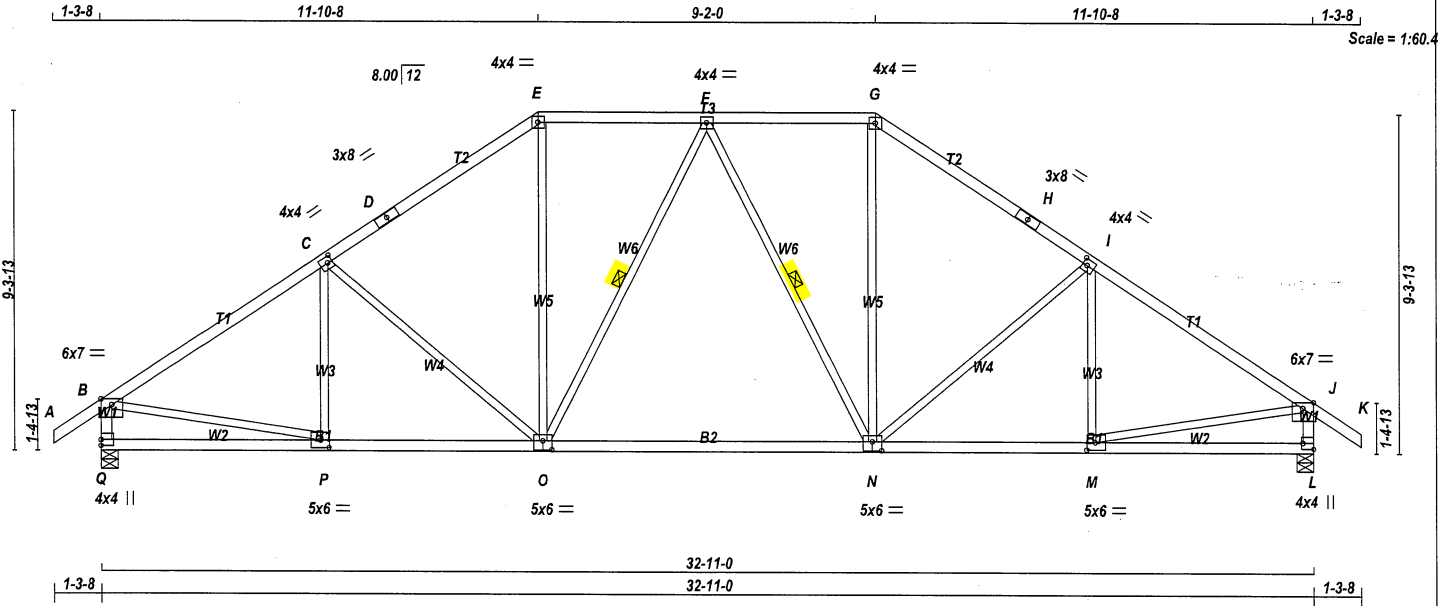


Structural component only
DWG# T-2213209

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T33	10	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-4gSWoVMI6YW1qqBuwzEnVaHXIBsEylsMPEUAbCzANRY



LUMBER				DESCR.			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER				
A - D	2x4	DRY	No.2	SPF			
D - E	2x4	DRY	No.2	SPF			
E - G	2x4	DRY	No.2	SPF			
G - H	2x4	DRY	No.2	SPF			
H - K	2x4	DRY	No.2	SPF			
Q - B	2x4	DRY	No.2	SPF			
L - J	2x4	DRY	No.2	SPF			
Q - O	2x4	DRY	No.2	SPF			
O - N	2x4	DRY	No.2	SPF			
N - L	2x4	DRY	No.2	SPF			
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-t	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	8.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	6.0	7.0	Edge	
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BSWWW-t	MT20	5.0	6.0	3.00	3.00
O	BSWWW-t	MT20	5.0	6.0	3.00	3.00
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

Structural component only
DWG# T-2213210

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED		MAXIMUM FACTORED		INPUT		REQD		
	GROSS REACTION		GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
Q	2309	0	2309	0	0	5-8	5-8		
L	2309	0	2309	0	0	5-8	5-8		
UNFACTORED REACTIONS									
	1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Q	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0		
L	1616	1158 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-O, F-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 43	-112.4	-112.4 0.15 (1)	10.00	P-C	-260 / 41	0.11 (1)
B-C	-2584 / 0	-112.4	-112.4 0.68 (1)	3.60	C-O	-496 / 0	0.63 (1)
C-D	-2224 / 0	-112.4	-112.4 0.62 (1)	3.91	O-E	0 / 763	0.17 (1)
D-E	-2224 / 0	-112.4	-112.4 0.62 (1)	3.91	O-F	-348 / 0	0.26 (1)
E-F	-1820 / 0	-112.4	-112.4 0.33 (1)	4.63	F-N	-348 / 0	0.26 (1)
F-G	-1820 / 0	-112.4	-112.4 0.33 (1)	4.63	N-G	0 / 763	0.17 (1)
G-H	-2224 / 0	-112.4	-112.4 0.62 (1)	3.91	N-I	-496 / 0	0.63 (1)
H-I	-2224 / 0	-112.4	-112.4 0.62 (1)	3.91	M-I	-260 / 41	0.11 (1)
I-J	-2584 / 0	-112.4	-112.4 0.68 (1)	3.60	B-P	0 / 2222	0.50 (1)
J-K	0 / 43	-112.4	-112.4 0.15 (1)	10.00	M-J	0 / 2222	0.50 (1)
Q-B	-2264 / 0	0.0	0.0 0.23 (1)	5.66			
L-J	-2264 / 0	0.0	0.0 0.23 (1)	5.66			
Q-P	0 / 0	-18.5	-18.5 0.18 (4)	10.00			
P-O	0 / 2187	-18.5	-18.5 0.44 (1)	10.00			
O-N	0 / 1975	-18.5	-18.5 0.57 (1)	10.00			
N-M	0 / 2187	-18.5	-18.5 0.44 (1)	10.00			
M-L	0 / 0	-18.5	-18.5 0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/845 (0.47")

CSI: TC=0.68/1.00 (I-J:1), BC=0.57/1.00 (N-O:1), WB=0.63/1.00 (I-N:1), SSI=0.27/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.50 (P) (INPUT = 1.00)

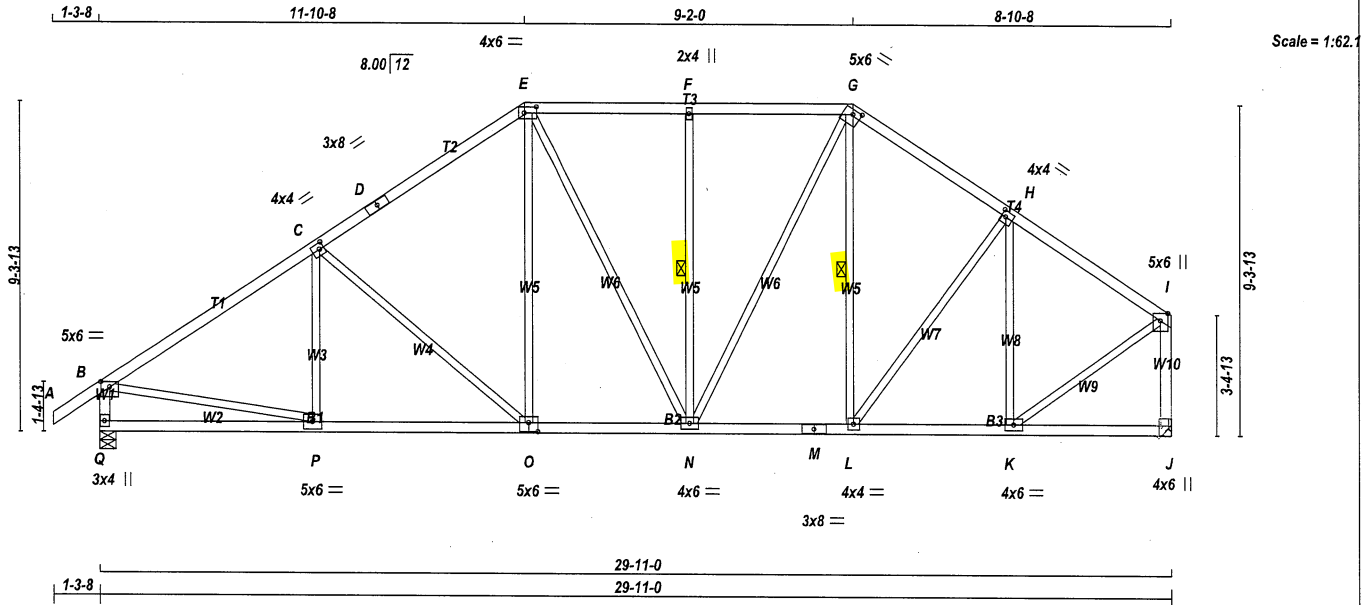


Structural component only
DWG# T-2213210

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T33A	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: msZCRjkkNthSDqhrbA0REzSQw2-Yt0u?rNwtreuS_m4Uhl02nqj0bE6hBOVeUEk8ezANRX



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
J - I	2x4	DRY	No.2	SPF	
Q - O	2x4	DRY	No.2	SPF	
O - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTWW-i	MT20	4.0	6.0	2.00	4.00
F	TMVW-w	MT20	2.0	4.0		
G	TTWW-h	MT20	5.0	6.0	1.50	2.75
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	Edge	
J	BMV1-t	MT20	4.0	6.0	Edge	0.50
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	6.0		
O	BSW-t	MT20	5.0	6.0	3.00	3.00
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
Q	2113	0	2113	0	0	5-8	5-8		
J	1958	0	1958	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1478	1061 / 0	0 / 0	0 / 0	0 / 0	417 / 0	0 / 0
J	1372	972 / 0	0 / 0	0 / 0	0 / 0	401 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-N, G-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	P-C	-218 / 53	0.10 (1)
B-C	-2306 / 0	-112.4 -112.4	0.64 (1)	3.81	C-O	-532 / 0	0.67 (1)
C-D	-1913 / 0	-112.4 -112.4	0.59 (1)	4.19	O-E	0 / 426	0.10 (1)
D-E	-1913 / 0	-112.4 -112.4	0.59 (1)	4.19	E-N	0 / 185	0.04 (1)
E-F	-1641 / 0	-112.4 -112.4	0.32 (1)	4.83	N-F	-626 / 0	0.36 (1)
F-G	-1641 / 0	-112.4 -112.4	0.32 (1)	4.83	N-G	0 / 619	0.14 (1)
G-H	-1672 / 0	-112.4 -112.4	0.32 (1)	4.81	L-G	-25 / 52	0.02 (4)
H-I	-1511 / 0	-112.4 -112.4	0.31 (1)	5.01	L-H	0 / 130	0.03 (1)
Q-B	-2067 / 0	0.0 0.0	0.21 (1)	5.88	K-H	-780 / 0	0.52 (1)
J-I	-1923 / 0	0.0 0.0	0.38 (1)	6.07	B-P	0 / 1986	0.45 (1)
					K-I	0 / 1551	0.35 (1)
Q-P	0 / 0	-18.5 -18.5	0.18 (4)	10.00			
P-O	0 / 1955	-18.5 -18.5	0.40 (1)	10.00			
O-N	0 / 1557	-18.5 -18.5	0.29 (1)	10.00			
N-M	0 / 1360	-18.5 -18.5	0.26 (1)	10.00			
M-L	0 / 1360	-18.5 -18.5	0.26 (1)	10.00			
L-K	0 / 1284	-18.5 -18.5	0.25 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.09 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.00")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (1.00")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.64/1.00 (B-C-1), BC=0.40/1.00 (O-P-1), WB=0.67/1.00 (C-O-1), SSI=0.27/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.61 (I) (INPUT = 1.00)

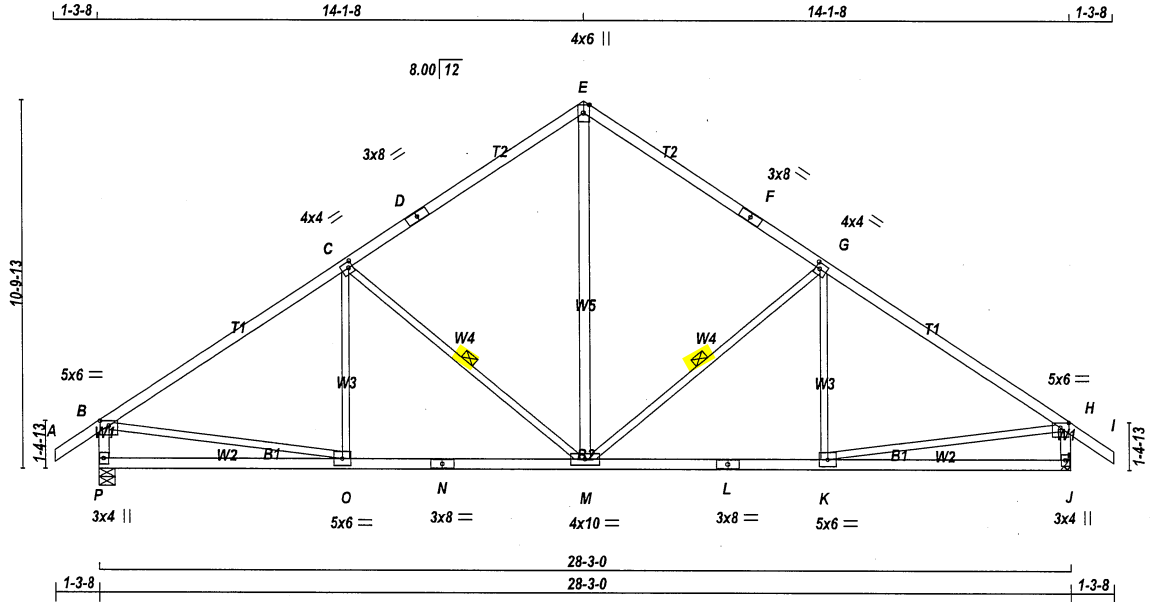


Structural component only
DWG# T-2213211

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T34	3	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-03aHCBNye9ml38LG1OGFa?NpL_alQiUetYzHg4zANRW



TOTAL WEIGHT = 3 X 123 = 370 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
M - E	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	6.0	1.75 3.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	8.0	
E	TTW+p	MT20	4.0	6.0	Edge
F	TS-t	MT20	3.0	8.0	
G	TMVW-t	MT20	4.0	4.0	2.00 1.50
H	TMVW-p	MT20	5.0	6.0	1.75 3.00
J	BMV1+p	MT20	3.0	4.0	
K	BMVW-t	MT20	5.0	6.0	
L	BS-t	MT20	3.0	8.0	
M	BMVW-t	MT20	4.0	10.0	
N	BS-t	MT20	3.0	8.0	
O	BMVW-t	MT20	5.0	6.0	
P	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED			MAXIMUM FACTORED			INPUT	REQD
	GROSS REACTION			GROSS REACTION			BRG	BRG
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
P	2004	0		2004	0	0	5-8	5-8
J	2004	0		2004	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1402	1007 / 0	0 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0
J	1402	1007 / 0	0 / 0	0 / 0	0 / 0	0 / 0	395 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)	10.00	M-E	0 / 1082	0.17 (1)
B-C	-2134 / 0	-112.4 -112.4	0.93 (1)	3.34	M-G	-744 / 0	0.43 (1)
C-D	-1558 / 0	-112.4 -112.4	0.82 (1)	4.00	K-G	-130 / 88	0.08 (1)
D-E	-1558 / 0	-112.4 -112.4	0.82 (1)	4.00	C-M	-744 / 0	0.43 (1)
E-F	-1558 / 0	-112.4 -112.4	0.82 (1)	4.00	O-C	-130 / 88	0.08 (1)
F-G	-1558 / 0	-112.4 -112.4	0.82 (1)	4.00	B-O	0 / 1840	0.41 (1)
G-H	-2134 / 0	-112.4 -112.4	0.93 (1)	3.34	K-H	0 / 1840	0.41 (1)
H-I	0 / 43	-112.4 -112.4	0.15 (1)	10.00			
P-B	-1949 / 0	0.0 0.0	0.20 (1)	6.03			
J-H	-1949 / 0	0.0 0.0	0.20 (1)	6.03			
P-O	0 / 0	-18.5 -18.5	0.22 (4)	10.00			
O-N	0 / 1820	-18.5 -18.5	0.40 (1)	10.00			
N-M	0 / 1820	-18.5 -18.5	0.40 (1)	10.00			
M-L	0 / 1820	-18.5 -18.5	0.40 (1)	10.00			
L-K	0 / 1820	-18.5 -18.5	0.40 (1)	10.00			
K-J	0 / 0	-18.5 -18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.94")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.94")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.93/1.00 (G-H:1), BC=0.40/1.00 (M-O:1), WB=0.43/1.00 (C-M:1), SSI=0.32/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

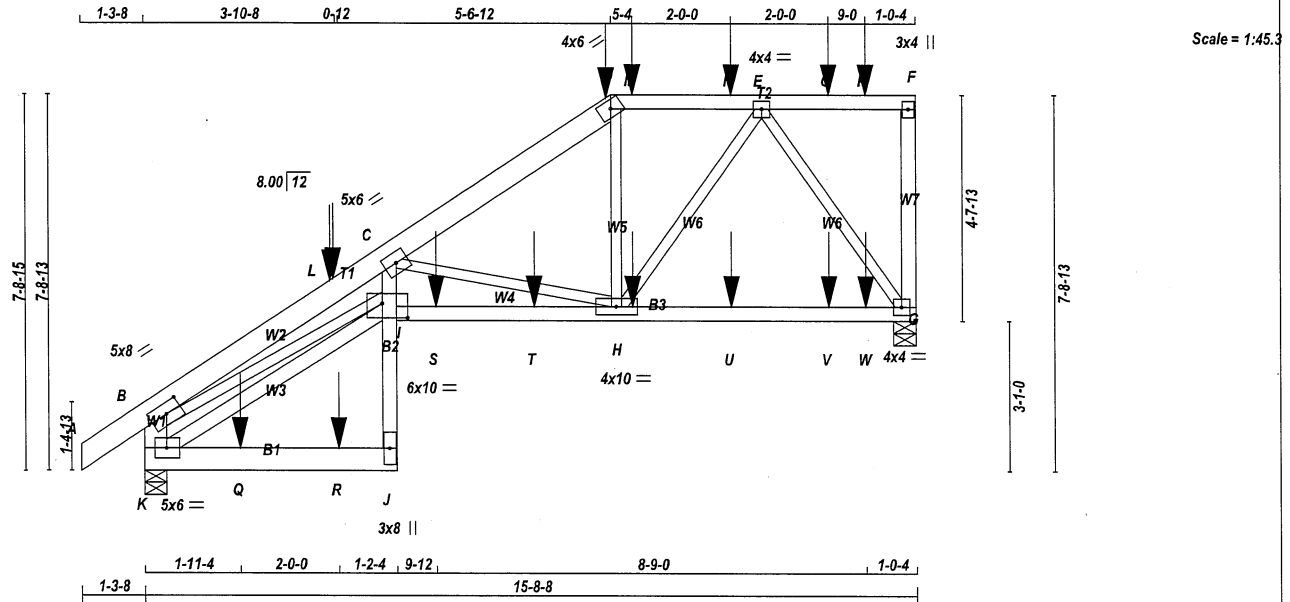
JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.59 (N) (INPUT = 1.00)



Structural component only
DWG# T-2213212

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T35S	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-03aHCBNy9ml38LG1OGFa?Nzu_YbQfJetYzHg4zANRW



TOTAL WEIGHT = 2 X 88 = 175 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x6	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - D 2	12	SIDE(122.0)
K - B 2	12	TOP
D - F 1	12	SIDE(61.0)
F - G 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
K - J 2	12	SIDE(0.0)
J - C 1	12	TOP
I - G 1	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQORD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
G	1958	0	1958	0	0	5-8	5-8		
K	1793	0	1793	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1371	981 / 0	0 / 0	0 / 0	0 / 0	390 / 0	0 / 0
K	1253	906 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 44	-112.4	-112.4 0.04 (1)	10.00	C-H	-3232 / 0	0.63 (1)
B-L	-5477 / 0	-112.4	-112.4 0.32 (1)	4.69	H-D	0 / 217	0.03 (4)
L-C	-5477 / 0	-112.4	-112.4 0.32 (1)	4.69	K-I	-109 / 0	0.02 (1)
C-D	-2015 / 0	-112.4	-112.4 0.15 (1)	6.25	B-I	0 / 4976	0.62 (1)
D-M	-1694 / 0	-112.4	-112.4 0.21 (1)	6.25	H-E	0 / 979	0.12 (1)
M-N	-1694 / 0	-112.4	-112.4 0.21 (1)	6.25	E-G	-1970 / 0	0.47 (1)
N-E	-1694 / 0	-112.4	-112.4 0.21 (1)	6.25			
E-O	0 / 0	-112.4	-112.4 0.20 (1)	10.00			
O-P	0 / 0	-112.4	-112.4 0.20 (1)	10.00			
P-F	0 / 0	-112.4	-112.4 0.20 (1)	10.00			
G-F	-273 / 0	0.0	0.0 0.05 (1)	7.81			
K-B	-1676 / 0	0.0	0.0 0.06 (1)	7.81			
K-Q	0 / 94	-18.5	-18.5 0.06 (4)	10.00			
Q-R	0 / 94	-18.5	-18.5 0.06 (4)	10.00			
R-J	0 / 94	-18.5	-18.5 0.06 (4)	10.00			
J-I	0 / 75	0.0	0.0 0.22 (1)	10.00			
I-C	0 / 1996	0.0	0.0 0.39 (1)	10.00			
I-S	0 / 4814	-18.5	-18.5 0.51 (1)	10.00			
S-T	0 / 4814	-18.5	-18.5 0.51 (1)	10.00			
T-H	0 / 4814	-18.5	-18.5 0.51 (1)	10.00			
H-U	0 / 1153	-18.5	-18.5 0.20 (4)	10.00			
U-V	0 / 1153	-18.5	-18.5 0.20 (4)	10.00			
V-W	0 / 1153	-18.5	-18.5 0.20 (4)	10.00			
W-G	0 / 1153	-18.5	-18.5 0.20 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	9-6-0	-34	-34	---	FRONT	VERT	DEAD	---	C1
H	9-6-0	-180	-180	---	FRONT	VERT	SNOW	---	C1
D	9-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
L	3-10-8	-168	-168	---	FRONT	VERT	TOTAL	---	C1
L	3-11-4	-72	-72	---	FRONT	VERT	TOTAL	---	C1
M	9-11-4	-132	-132	---	FRONT	VERT	TOTAL	---	C1
N	11-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
O	13-11-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
P	14-8-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
Q	1-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
R	3-11-4	-14	-14	---	FRONT	VERT	TOTAL	---	C1
S	5-11-4	-22	-22	---	FRONT	VERT	TOTAL	---	C1
T	7-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	11-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 096-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.52")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (0.52")
CALCULATED VERT. DEFL. (TL) = L/808 (0.23")

CSI: TC=0.32/1.00 (B-C:1), BC=0.51/1.00 (H-I:1), WB=0.63/1.00 (C-H:1), SSI=0.18/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.52 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213213

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T35S	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	3.75
C	TMVW-t	MT20	5.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVWW-t	MT20	4.0	10.0		
I	BVMWW-t	MT20	6.0	10.0	3.50	6.25
J	BMV+p	MT20	3.0	8.0		
K	BMVW1-t	MT20	5.0	6.0		

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	13-11-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
W	14-8-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LICENSED PROFESSIONAL ENGINEER

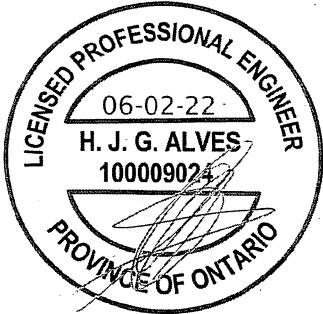
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PROVINCE OF ONTARIO

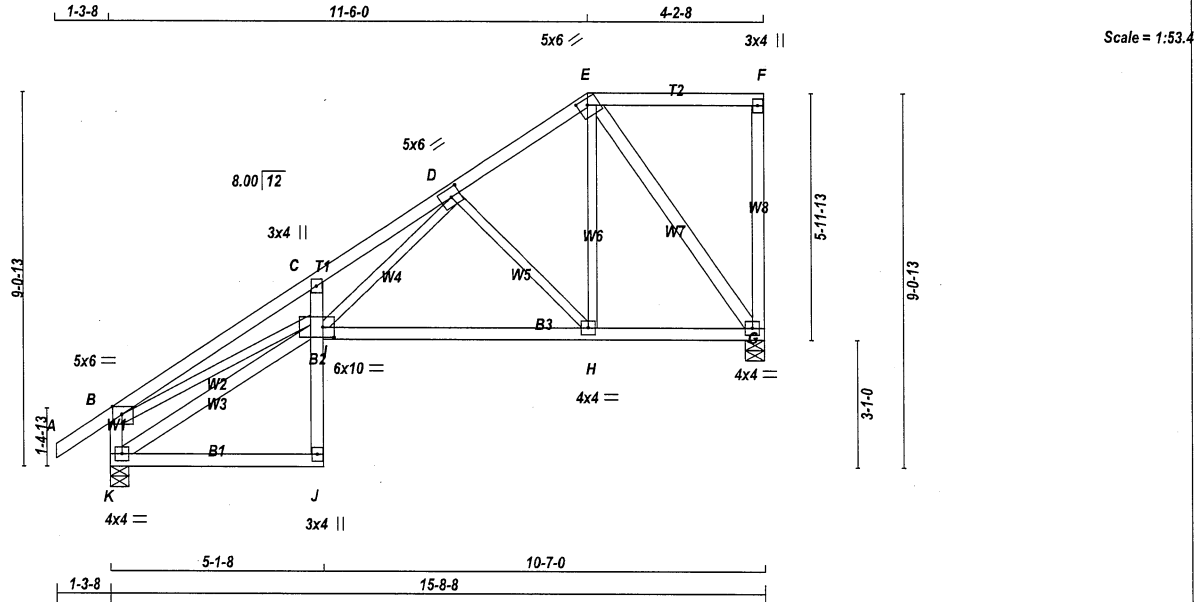
Structural component only
DWG# T-2213213



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T36S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 82 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD	SPECIFIED LOADS:	
A - E	2x4	DRY	No.2	SPF	GT	GROSS REACTION	GROSS REACTION	BRG	BRG	TOP CH. LL =	32.5 PSF
E - F	2x4	DRY	No.2	SPF	G	VERT	HORZ	DOWN	HORZ	DL =	6.0 PSF
G - F	2x4	DRY	No.2	SPF	K	1028	0	1028	0	BOT CH. LL =	0.0 PSF
K - B	2x4	DRY	No.2	SPF	K	1183	0	1183	0	DL =	7.4 PSF
K - J	2x4	DRY	No.2	SPF						TOTAL LOAD =	45.9 PSF
J - C	2x4	DRY	No.2	SPF							
I - G	2x4	DRY	No.2	SPF							

ALL WEBS EXCEPT				UNFACTORED REACTIONS				SPACING = 240 IN. C/C			
E - G	2x4	DRY	No.2	1ST LCASE	MAX. / MIN. COMPONENT REACTIONS					LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM	
K - I	2x4	DRY	No.2	GT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
				G	721	510 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0
				K	826	599 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0

DRY: SEASONED LUMBER.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

C H O R D S				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)	H-E	0 / 763	0.17 (1)	
B-C	-3144 / 0	-112.4 -112.4	0.56 (1)	E-G	-960 / 0	0.61 (1)	
C-D	-3268 / 0	-112.4 -112.4	0.26 (1)	K-I	-63 / 0	0.03 (1)	
D-E	-694 / 0	-112.4 -112.4	0.14 (1)	B-I	0 / 2830	0.64 (1)	
E-F	0 / 0	-112.4 -112.4	0.34 (1)	I-D	0 / 2262	0.51 (1)	
G-F	-237 / 0	0.0 0.0	0.16 (1)	D-H	-891 / 0	0.35 (1)	
K-B	-1103 / 0	0.0 0.0	0.11 (1)				
K-J	0 / 54	-18.5 -18.5	0.14 (4)				
J-I	0 / 52	0.0 0.0	0.23 (1)				
I-C	-530 / 0	0.0 0.0	0.18 (1)				
I-H	0 / 1183	-18.5 -18.5	0.29 (1)				
H-G	0 / 572	-18.5 -18.5	0.22 (4)				

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/608 (0.31")

CSI: TC=0.56/1.00 (B-C:1), BC=0.29/1.00 (H-I:1), WB=0.64/1.00 (B-I:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.62 (I) (INPUT = 1.00)

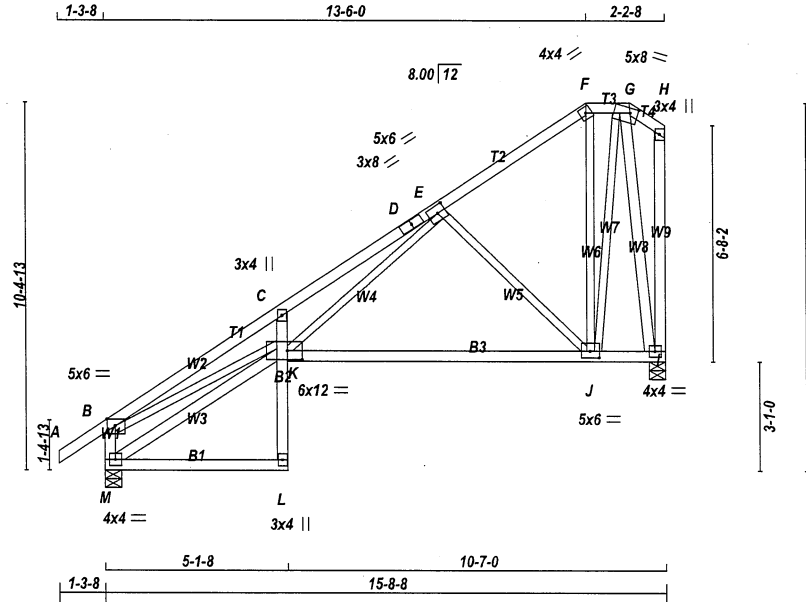


Structural component only
DWG# T-2213214

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T37S	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:62.3

TOTAL WEIGHT = 93 lb

LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA				
N. L. G. A. RULES					BEARINGS										SPECIFIED LOADS:				
CHORDS		SIZE	LUMBER		DESCR.	FACTORED		MAXIMUM FACTORED		INPUT	REQRD	TOP CH.		LL = 32.5 PSF					
A - D	2x4	DRY	No.2	SPF		GROSS REACTION		GROSS REACTION		BRG	BRG	DL = 6.0 PSF							
D - F	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BOT CH.		LL = 0.0 PSF				
F - G	2x4	DRY	No.2	SPF	I	1028	0	1028	0	0	5-8	5-8	DL = 7.4 PSF						
G - H	2x4	DRY	No.2	SPF	M	1183	0	1183	0	0	5-8	5-8	TOTAL LOAD = 45.9 PSF						
M - B	2x4	DRY	No.2	SPF															
I - H	2x4	DRY	No.2	SPF															
M - L	2x4	DRY	No.2	SPF															
L - C	2x4	DRY	No.2	SPF															
K - I	2x4	DRY	No.2	SPF															
ALL WEBS 2x3 DRY No.2 SPF																			
EXCEPT																			
M - K	2x4	DRY	No.2	SPF															
G - I	2x4	DRY	No.2	SPF															
DRY: SEASONED LUMBER.																			
PLATES (table is in inches)																			
JT	TYPE	PLATES	W	LEN	Y	X													
B	TMWV-p	MT20	5.0	6.0	2.25	2.75													
C	TMV+p	MT20	3.0	4.0															
D	TS-I	MT20	3.0	8.0															
E	TMWV-t	MT20	5.0	6.0	2.50	2.75													
F	TTW-h	MT20	4.0	4.0															
G	TTWV-m	MT20	5.0	8.0	1.75	2.50													
H	TMV+p	MT20	3.0	4.0															
I	BMVW1-t	MT20	4.0	4.0															
J	BMVWW1-t	MT20	5.0	6.0	2.25	3.00													
K	BMVWWVW-I	MT20	6.0	12.0	3.00	5.00													
L	BMV+p	MT20	3.0	4.0															
M	BMVW1-t	MT20	4.0	4.0															
NOTES: (1)																			
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2																			
CHORDS																			
MEMB.		MAX. FACTORED	FACTORED		VERT. LOAD		LC1	MAX	MAX.	WEBS		MAX. FACTORED		MAX					
		(LBS)			(PLF)		CSI (LC)	UNBRAC	MEMB.	FORCE		(LBS)		MAX					
FR-TO				FROM TO				LENGTH	FR-TO										
A-B	0 / 43	-112.4		-112.4	0.15	(1)	10.00	E-J	-943 / 0	0.60		(1)							
B-C	-3153 / 0	-112.4		-112.4	0.56	(1)	3.44	J-F	-58 / 30	0.05		(1)							
C-D	-3264 / 0	-112.4		-112.4	0.41	(1)	3.56	M-K	-64 / 0	0.03		(1)							
D-E	-3264 / 0	-112.4		-112.4	0.41	(1)	3.56	B-K	0 / 2840	0.64		(1)							
E-F	-376 / 0	-112.4		-112.4	0.25	(1)	6.25	G-I	-1025 / 0	0.71		(1)							
F-G	-285 / 0	-112.4		-112.4	0.02	(1)	6.25	J-G	0 / 904	0.20		(1)							
G-H	0 / 0	-112.4		-112.4	0.02	(1)	10.00	K-E	0 / 2465	0.55		(1)							
M-B	-1103 / 0	0.0		0.0	0.11	(1)	7.54												
I-H	-55 / 0	0.0		0.0	0.05	(1)	7.81												
M-L	0 / 54	-18.5		-18.5	0.14	(4)	10.00												
L-K	0 / 52	0.0		0.0	0.23	(1)	10.00												
K-C	-573 / 0	0.0		0.0	0.18	(1)	7.81												
K-J	0 / 961	-18.5		-18.5	0.38	(4)	10.00												
J-I	0 / 141	-18.5		-18.5	0.33	(4)	10.00												
UNFACTORED REACTIONS																			
1ST LCASE		MAX./MIN. COMPONENT REACTIONS																	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL												
I	721	510 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0												
M	826	599 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0												
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, M																			
BRACING																			
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.44 FT.																			
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT. OR RIGID CEILING DIRECTLY APPLIED.																			
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.																			
LOADING																			
TOTAL LOAD CASES: (4)																			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.25	2.75
C	TMV+p	MT20	3.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVW-t	MT20	5.0	6.0	2.50	2.75
F	TTW-h	MT20	4.0	4.0		
G	TTW-m	MT20	5.0	8.0	1.75	2.50
H	TMV+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BMVW-t	MT20	5.0	6.0	2.25	3.00
K	BMVW-t	MT20	6.0	12.0	3.00	5.00
L	BMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	4.0	4.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

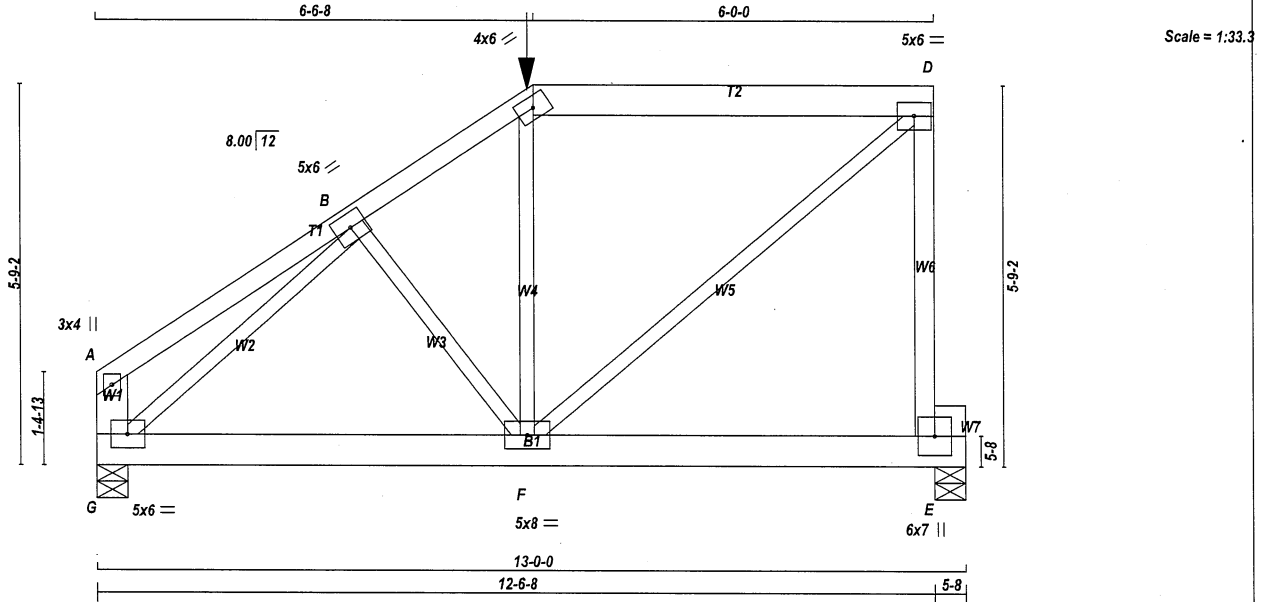


Structural component only
DWG# T-2213215

JOB NAME 426687	TRUSS NAME T38	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Thu Jun 2 09:33:19 2022 Page 1
ID: _msZCRjkkNthSDqhrbA0REzSQw2-ySh1dtPpAm0SJUSf9pljfQS9FoEquXpxKsSOlzzANRU



TOTAL WEIGHT = 68 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x6	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - A	2x6	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-h	MT20	4.0	6.0		
D	TMVW-t	MT20	5.0	6.0		
E	BMVW1+p	MT20	6.0	7.0		
F	BMVWW-t	MT20	5.0	8.0		
G	BMVW1-t	MT20	5.0	6.0		

NOTES-

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
E	1966	0	1966	0
G	1724	0	1724	0

UNFACTORED REACTIONS

	1ST LOASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	1376	984 / 0	0 / 0	0 / 0	0 / 0	393 / 0	0 / 0
G	1211	844 / 0	0 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. VERT. LOAD (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 21	-112.4	-112.4	0.21 (1)	10.00	F-C	-219 / 91
B-C	-1594 / 0	-112.4	-112.4	0.25 (1)	4.92	F-D	0 / 1739
C-D	-1313 / 0	-183.9	-183.9	0.63 (1)	5.54	B-F	-72 / 29
E-D	-1698 / 0	0.0	0.0	0.90 (1)	6.33	G-B	-1847 / 0
G-A	-175 / 0	0.0	0.0	0.01 (1)	7.81		
G-F	0 / 1352	-113.4	-113.4	0.53 (1)	10.00		
F-E	0 / 0	-113.4	-113.4	0.35 (1)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	6-6-8	-294	-294	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 5-0-0
END DISTANCE = 13-0-0
END SPAN CARRIED = 5-0-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 6-6-8
RIGHT SETBACK = 0-0
END SETBACK = 5-0-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.90/1.00 (D-E:1), BC=0.53/1.00 (F-G:1), WB=0.76/1.00 (B-G:1), SSI=0.37/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
		788	1987
			1873

CONTINUED ON PAGE 2



Structural component only
DWG# T-2213216

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T38	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:19 2022 Page 2

ID: msZCRjkkNthSDqhrbA0REzSQw2-ySh1dtPoAm0SJSu9plfQS9FoEquXpxKsSOlzzANRU

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)

JSI METAL= 0.41 (B) (INPUT = 1.00)



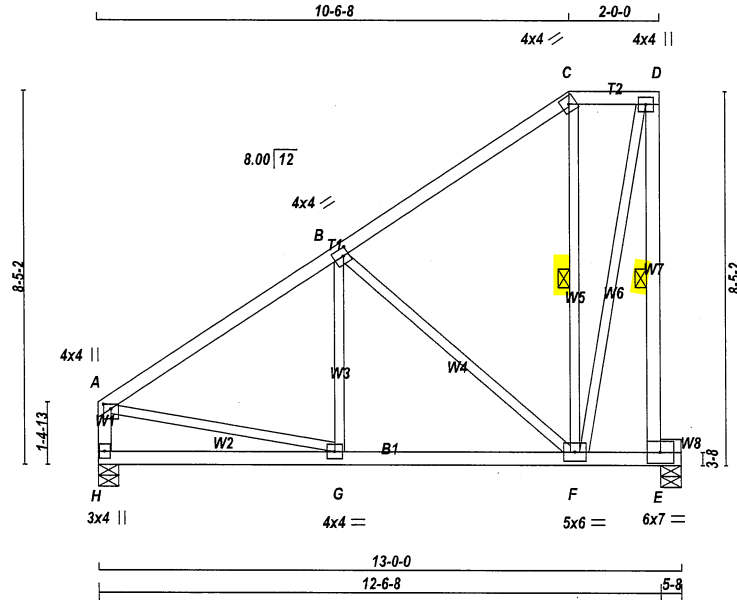
Structural component only

DWG# T-2213216

JOB NAME 426687	TRUSS NAME T40	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:21 2022 Page 1
ID: _msZCRjkkNthSDqhrbA0REzSQw2-vqpn2YR3IOGAYle1GEKBlrXdNc?IMSuEoAxVprzANRS



Scale = 1:49.7

TOTAL WEIGHT = 68 lb

LUMBER DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TTW-h	MT20	4.0	4.0		
D	TMVW+p	MT20	4.0	4.0		
E	BMVW1-t	MT20	6.0	7.0		
F	BMVW1-t	MT20	5.0	6.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMV1-p	MT20	3.0	4.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

BEARINGS

FACTORED		MAXIMUM FACTORED			INPUT	REQD	
GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
E	821	0	821	0	0	5-8	5-8
H	821	0	821	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	575	407 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	575	407 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CSF (LC)	UNBRAC	LENGTH	FR-TO	MEMB.	FORCE (LBS)	CSF (LC)	MEMB.
FR-TO										
A-B	-723 / 0	-112.4	-112.4	0.41 (1)	6.25	G-B	-11 / 93	0.03 (4)		
B-C	-240 / 0	-112.4	-112.4	0.40 (1)	6.25	B-F	-627 / 0	0.59 (1)		
C-D	-162 / 0	-112.4	-112.4	0.06 (1)	6.25	F-C	-219 / 0	0.10 (1)		
E-D	-808 / 0	0.0	0.0	0.26 (1)	6.25	F-D	0 / 716	0.16 (1)		
H-A	-781 / 0	0.0	0.0	0.08 (1)	7.81	A-G	0 / 647	0.15 (1)		
H-G	0 / 0	-18.5	-18.5	0.13 (4)	10.00					
G-F	0 / 635	-18.5	-18.5	0.18 (4)	10.00					
F-E	0 / 0	-18.5	-18.5	0.05 (4)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.42")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.42")
CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

CSF: TC=0.41/1.00 (A-B:1), BC=0.18/1.00 (F-G:4), WB=0.59/1.00 (B-F:1), SS=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (F) (INPUT = 0.90)
JSI METAL = 0.22 (G) (INPUT = 1.00)

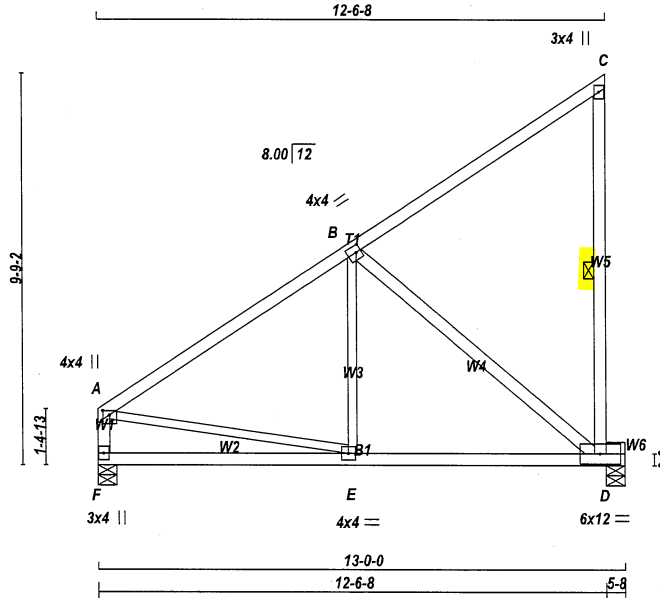


Structural component only
DWG# T-2213218

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T41	1	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-vqpn2YR3iOGAYle1GEKBlrXabc_dmp3EoAxVprzANRS



TOTAL WEIGHT = 62 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
D - C	2x4	DRY	No.2	SPF	
F - A	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TMVW-t	MT20	4.0	4.0	2.00 1.50
C	TMV+p	MT20	3.0	4.0	
D	BMVW1-t	MT20	6.0	12.0	
E	BMVW-t	MT20	4.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
FACTORED			MAXIMUM FACTORED			INPUT	REQD	
GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
D	821	0	821	0	0	5-8	5-8	
F	821	0	821	0	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	575	407 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
F	575	407 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	-684 / 0	-112.4	-112.4 0.59 (1)	E-B	0 / 131	0.05 (4)	
B-C	-48 / 0	-112.4	-112.4 0.58 (1)	B-D	-795 / 0	0.77 (1)	
D-C	-265 / 0	0.0	0.0 0.12 (1)	A-E	0 / 618	0.14 (1)	
F-A	-776 / 0	0.0	0.0 0.08 (1)				
F-E	0 / 0	-18.5	-18.5 0.21 (4)				
E-D	0 / 609	-18.5	-18.5 0.25 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.59/1.00 (A-B:1), BC=0.25/1.00 (D-E:4), WB=0.77/1.00 (B-D:1), SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

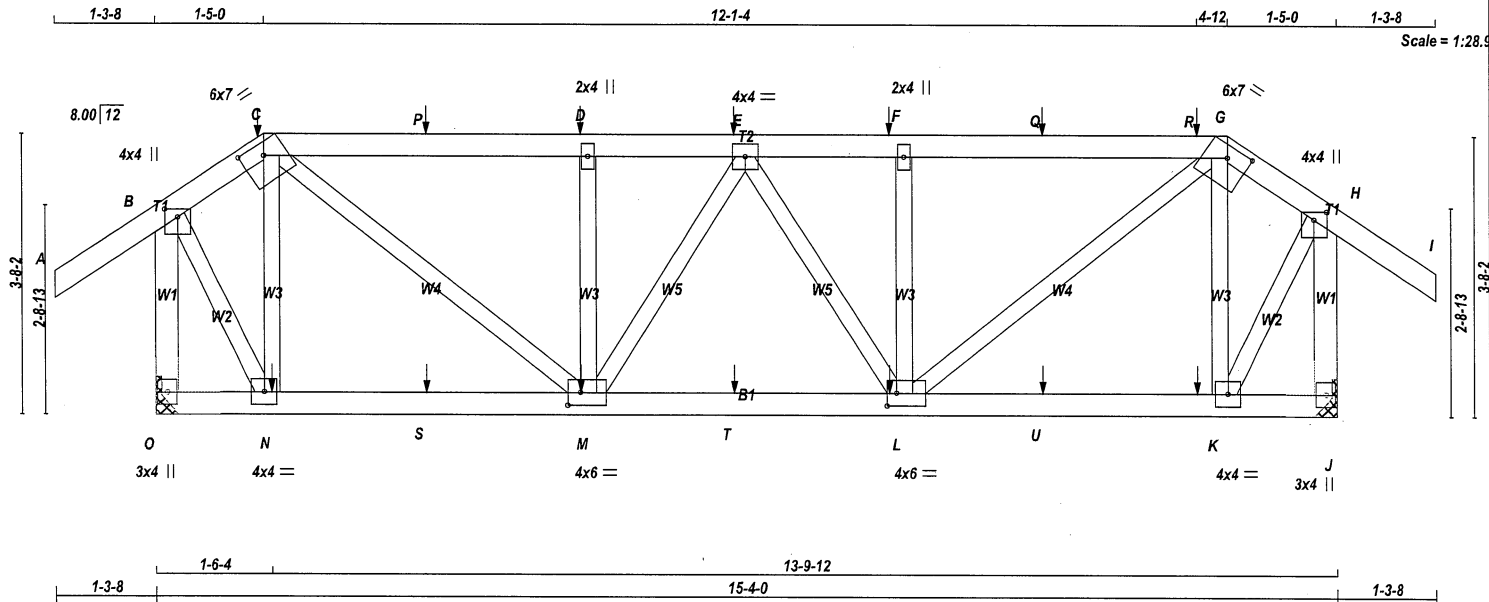
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.68 (A) (INPUT = 0.90)
JSI METAL = 0.21 (E) (INPUT = 1.00)



Structural component only
DWG# T-2213219



TOTAL WEIGHT = 73 lb
[M]

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No. 2	SPF
C - G	2x4	DRY	No. 2	SPF
G - I	2x4	DRY	No. 2	SPF
O - B	2x4	DRY	No. 2	SPF
J - H	2x4	DRY	No. 2	SPF
O - J	2x4	DRY	No. 2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV+p	MT20	4.0	4.0	1.25	2.00
C	TTWV-h	MT20	6.0	7.0	1.75	3.50
D	TMW+u	MT20	2.0	4.0		
E	TMWV-t	MT20	4.0	4.0		
F	TMW+u	MT20	2.0	4.0		
G	TTWV-h	MT20	6.0	7.0	1.75	3.50
H	TMWV+p	MT20	4.0	4.0	1.25	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWV-t	MT20	4.0	4.0		
L	BMWVWV-t	MT20	4.0	6.0	2.00	1.50
M	BMWVWV-t	MT20	4.0	6.0	2.00	2.00
N	BMWV-t	MT20	4.0	4.0		
O	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED		MAXIMUM FACTORED			INPUT	REQRD
	GROSS REACTION		GROSS REACTION			BRG	BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	1158	0	1158	0	0	MECHANICAL	
J	1158	0	1158	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT O, J. MINIMUM BEARING LENGTH AT JOINT O = 3-8, JOINT J = 3-8.

UNFACTORED REACTIONS

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	809	587 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0
J	809	587 / 0	0 / 0	0 / 0	0 / 0	222 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 43	-112.4 -112.4	0.17 (1)	10.00	N-C	-527 / 0	0.12 (1)
B-C	-496 / 0	-112.4 -112.4	0.16 (1)	6.25	C-M	0 / 921	0.23 (1)
C-P	-1077 / 0	-112.4 -112.4	0.28 (1)	5.69	M-D	-480 / 0	0.11 (1)
P-D	-1077 / 0	-112.4 -112.4	0.28 (1)	5.69	L-F	-480 / 0	0.11 (1)
D-E	-1078 / 0	-112.4 -112.4	0.27 (1)	5.69	L-G	0 / 921	0.23 (1)
E-F	-1078 / 0	-112.4 -112.4	0.27 (1)	5.69	K-G	-527 / 0	0.12 (1)
F-Q	-1077 / 0	-112.4 -112.4	0.28 (1)	5.69	B-N	0 / 705	0.17 (1)
Q-R	-1077 / 0	-112.4 -112.4	0.28 (1)	5.69	K-H	0 / 705	0.17 (1)
R-G	-1077 / 0	-112.4 -112.4	0.28 (1)	5.69	M-E	-26 / 4	0.01 (1)
G-H	-496 / 0	-112.4 -112.4	0.16 (1)	6.25	E-L	-26 / 0	0.01 (1)
H-I	0 / 43	-112.4 -112.4	0.17 (1)	10.00			
O-B	-1159 / 0	0.0	0.0	18.17 (1)	7.36		
J-H	-1159 / 0	0.0	0.0	0.18 (1)	7.36		
O-N	0 / 0	-18.5	-18.5	0.05 (4)	10.00		
N-S	0 / 360	-18.5	-18.5	0.10 (1)	10.00		
S-M	0 / 360	-18.5	-18.5	0.10 (1)	10.00		
M-T	0 / 1091	-18.5	-18.5	0.23 (1)	10.00		
T-L	0 / 1091	-18.5	-18.5	0.23 (1)	10.00		
L-U	0 / 360	-18.5	-18.5	0.10 (1)	10.00		
U-K	0 / 360	-18.5	-18.5	0.10 (1)	10.00		
K-J	0 / 0	-18.5	-18.5	0.05 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-5-0	1	1	108	FRONT	VERT	TOTAL	---	C1
D	5-6-4	1	1	108	FRONT	VERT	TOTAL	---	C1
E	7-6-4	1	1	108	FRONT	VERT	TOTAL	---	C1
F	9-6-4	1	1	108	FRONT	VERT	TOTAL	---	C1
K	13-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
L	9-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
M	5-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
N	1-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
P	3-6-4	1	1	108	FRONT	VERT	TOTAL	---	C1
Q	11-6-4	1	1	108	FRONT	VERT	TOTAL	---	C1
R	13-6-4	1	1	89	FRONT	VERT	TOTAL	---	C1
S	3-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
T	7-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1
U	11-6-4	1	1	---	FRONT	VERT	TOTAL	---	C1

<u>DESIGN CRITERIA</u>

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.06")

CSI: TC=0.28/1.00 (F-G:1) , BC=0.23/1.00 (L-M:1) ,
WB=0.23/1.00 (C-M:1) , SSI=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
JSI METAL= 0.25 (M) (INPUT = 1.00)

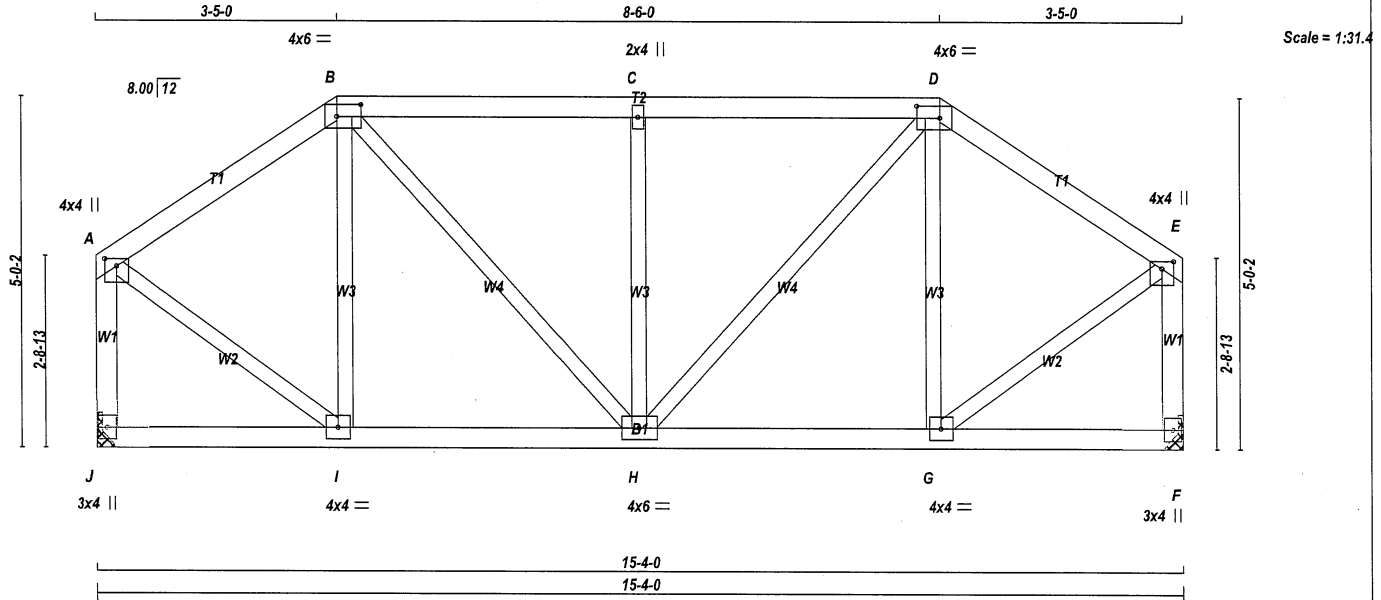


Structural component only
DWG# T-2213220

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T43	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 69 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
J - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-l	MT20	4.0	6.0	2.00	4.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVW-l	MT20	4.0	4.0		
H	BMVW-l	MT20	4.0	6.0		
I	BMVW-l	MT20	4.0	4.0		
J	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX	IN-SX		
J	1004	0	1004	0	0	MECHANICAL			
F	1004	0	1004	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
J	703 498 / 0 0 / 0 0 / 0 205 / 0 0 / 0
F	703 498 / 0 0 / 0 0 / 0 205 / 0 0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-699 / 0	-112.4 -112.4	0.23 (1)	6.25	I-B	-315 / 0	0.12 (1)
B-C	-868 / 0	-112.4 -112.4	0.34 (1)	6.07	B-H	0 / 440	0.10 (1)
C-D	-868 / 0	-112.4 -112.4	0.34 (1)	6.07	H-C	-584 / 0	0.22 (1)
D-E	-699 / 0	-112.4 -112.4	0.23 (1)	6.25	H-D	0 / 440	0.10 (1)
J-A	-979 / 0	0.0 0.0	0.14 (1)	7.81	G-D	-315 / 0	0.12 (1)
F-E	-979 / 0	0.0 0.0	0.14 (1)	7.81	A-I	0 / 704	0.16 (1)
					G-E	0 / 704	0.16 (1)
J-I	0 / 0	-18.5 -18.5	0.06 (4)	10.00			
I-H	0 / 573	-18.5 -18.5	0.13 (1)	10.00			
H-G	0 / 573	-18.5 -18.5	0.13 (1)	10.00			
G-F	0 / 0	-18.5 -18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.51")
CALCULATED VERT. DEFL. (LL) = L/999 (0.02")
ALLOWABLE DEFL. (TL) = L/360 (0.51")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.34/1.00 (B-C:1), BC=0.13/1.00 (G-H:1), WB=0.22/1.00 (C-H:1), SSI=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (G) (INPUT = 0.90)
JSI METAL= 0.20 (I) (INPUT = 1.00)

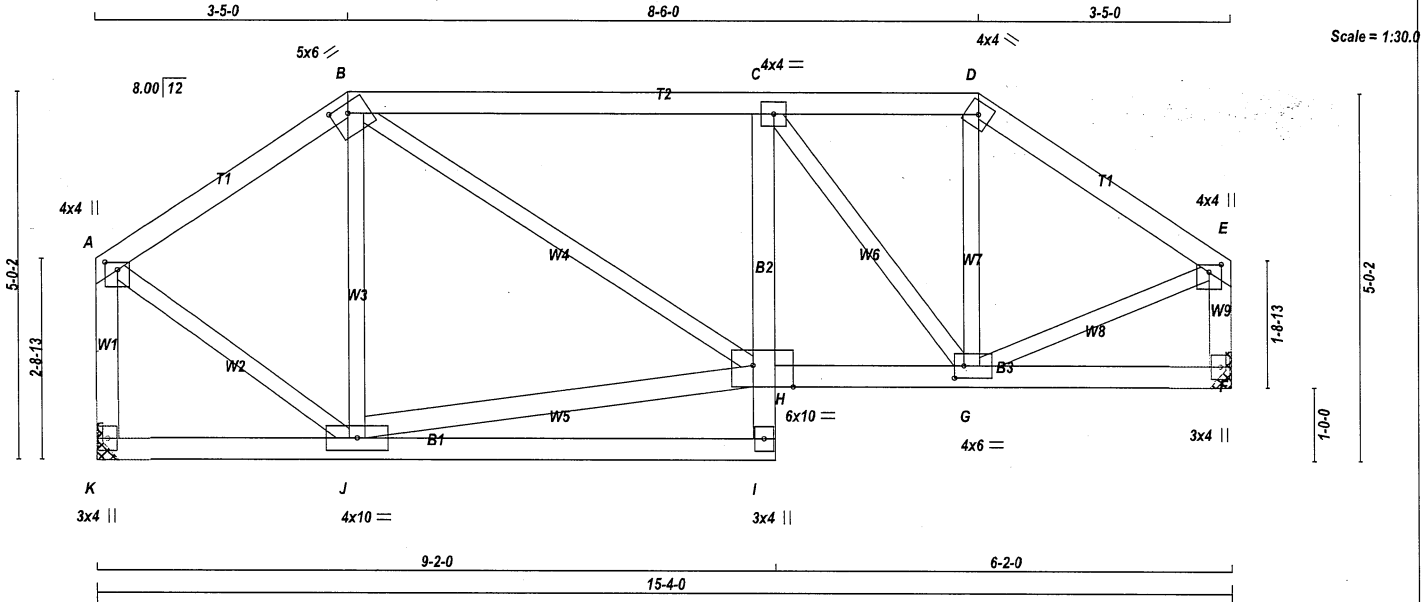


Structural component only
DWG# T-2213221

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426688	T43S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 73 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - C	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.25	2.00
B	TTWW-h	MT20	5.0	6.0	1.50	2.75
C	TMVW-t	MT20	4.0	4.0		
D	TTWW-h	MT20	4.0	4.0		
E	TMVW+p	MT20	4.0	4.0	1.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	6.0	2.00	1.50
H	BMVWW-t	MT20	6.0	10.0	Edge	6.50
I	BMV+p	MT20	3.0	4.0		
J	BMVWW-t	MT20	4.0	10.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES:

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	BRG	IN-SX	IN-SX
K	1004	0	1004	0	0	MECHANICAL			
F	1004	0	1004	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, F. MINIMUM BEARING LENGTH AT JOINT K = 3-8, JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	703	498 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
F	703	498 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	-697 / 0	-112.4	-112.4	0.23 (1)	J-B	-398 / 0	0.15 (1)
B-C	-1083 / 0	-112.4	-112.4	0.48 (1)	J-H	0 / 556	0.09 (1)
C-D	-737 / 0	-112.4	-112.4	0.44 (1)	B-H	0 / 606	0.14 (1)
D-E	-880 / 0	-112.4	-112.4	0.24 (1)	C-G	-590 / 0	0.20 (1)
K-A	-987 / 0	0.0	0.0	0.14 (1)	G-D	0 / 241	0.05 (1)
F-E	-976 / 0	0.0	0.0	0.11 (1)	G-J	0 / 708	0.16 (1)
					G-E	0 / 788	0.18 (1)
K-J	0 / 0	-18.5	-18.5	0.13 (4)			
J-I	0 / 31	-18.5	-18.5	0.13 (4)			
I-H	0 / 47	0.0	0.0	0.05 (1)			
H-C	-164 / 18	0.0	0.0	0.04 (1)			
H-G	0 / 1092	-18.5	-18.5	0.20 (1)			
G-F	0 / 0	-18.5	-18.5	0.05 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL)= L/999 (0.06")

CSI: TC=0.48/1.00 (B-C:1), BC=0.20/1.00 (G-H:1), WB=0.20/1.00 (C-G:1), SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

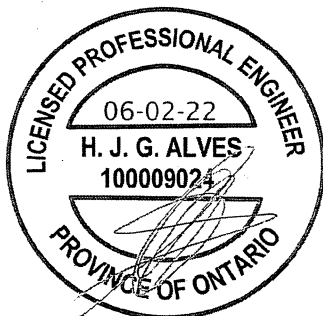
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747

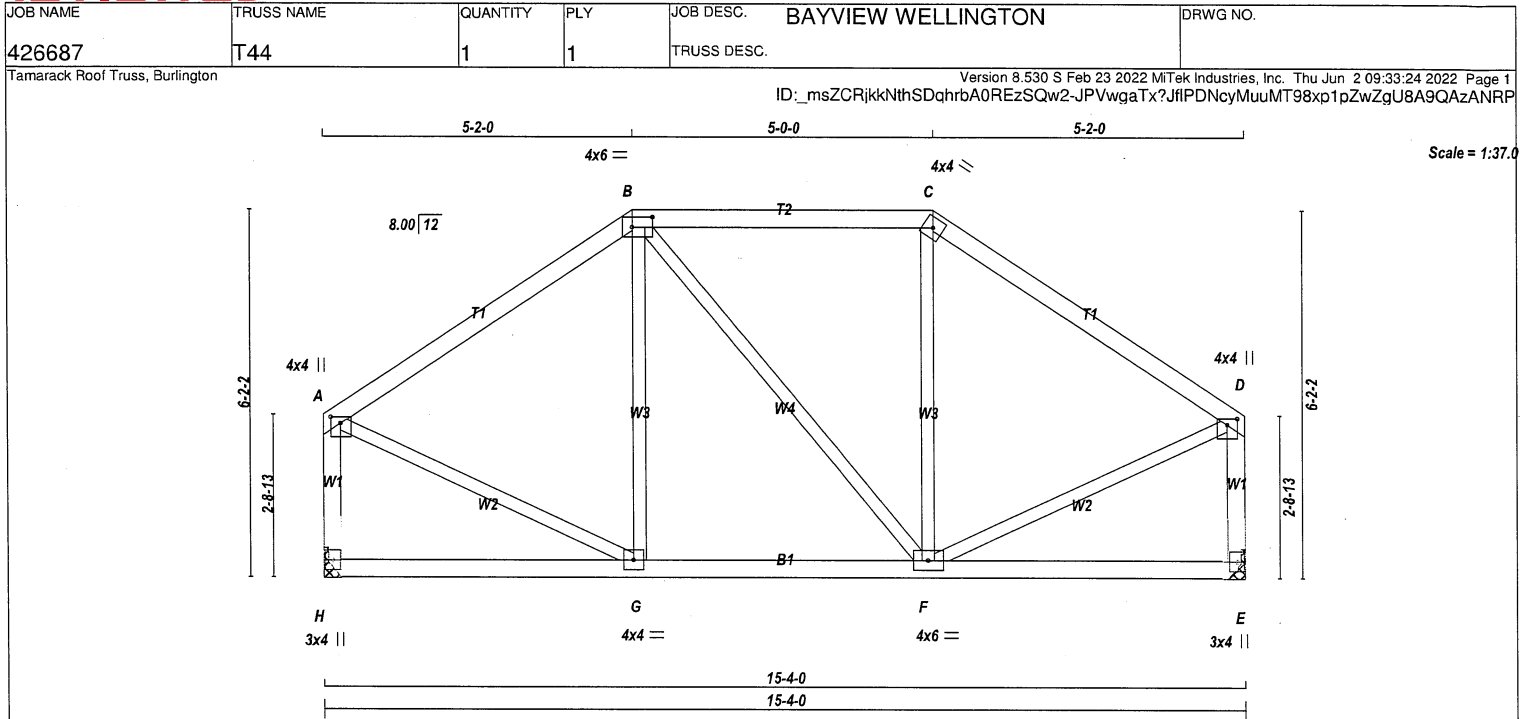
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.25 (G) (INPUT = 1.00)



Structural component only
DWG# T-2213234



TOTAL WEIGHT = 67 lb [M/F]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
H - A	2x4	DRY	No.2
E - D	2x4	DRY	No.2
H - E	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TTWW-l	MT20	4.0	6.0	2.00 4.00
C	TTWW-h	MT20	4.0	4.0	
D	TMVW+p	MT20	4.0	4.0	1.25 2.00
E	BMV1+p	MT20	3.0	4.0	
F	BMVWW-l	MT20	4.0	6.0	
G	BMVWW-l	MT20	4.0	4.0	
H	BMV1+p	MT20	3.0	4.0	

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	JT VERT HORZ	IN-SX	IN-SX
H 1004 0	1004 0	0	MECHANICAL
E 1004 0	1004 0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	703	498 / 0	0 / 0	0 / 0	0 / 0	205 / 0
E	703	498 / 0	0 / 0	0 / 0	0 / 0	205 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-718 / 0	-112.4 -112.4	0.39 (1)	G-B	-172 / 33	0.10 (1)	
B-C	-594 / 0	-112.4 -112.4	0.36 (1)	B-F	0 / 0	0.00 (1)	
C-D	-718 / 0	-112.4 -112.4	0.39 (1)	F-C	-172 / 33	0.10 (1)	
H-A	-963 / 0	0.0 0.0	0.14 (1)	A-G	0 / 656	0.15 (1)	
E-D	-963 / 0	0.0 0.0	0.14 (1)	F-D	0 / 656	0.15 (1)	
H-G	0 / 0	-18.5 -18.5	0.11 (4)				10.00
G-F	0 / 594	-18.5 -18.5	0.16 (1)				10.00
F-E	0 / 0	-18.5 -18.5	0.11 (4)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.51")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.51")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.39/1.00 (A-B:1) , BC=0.16/1.00 (F-G:1) ,
WB=0.15/1.00 (A-G:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (F) (INPUT = 0.90)
JSI METAL= 0.20 (G) (INPUT = 1.00)

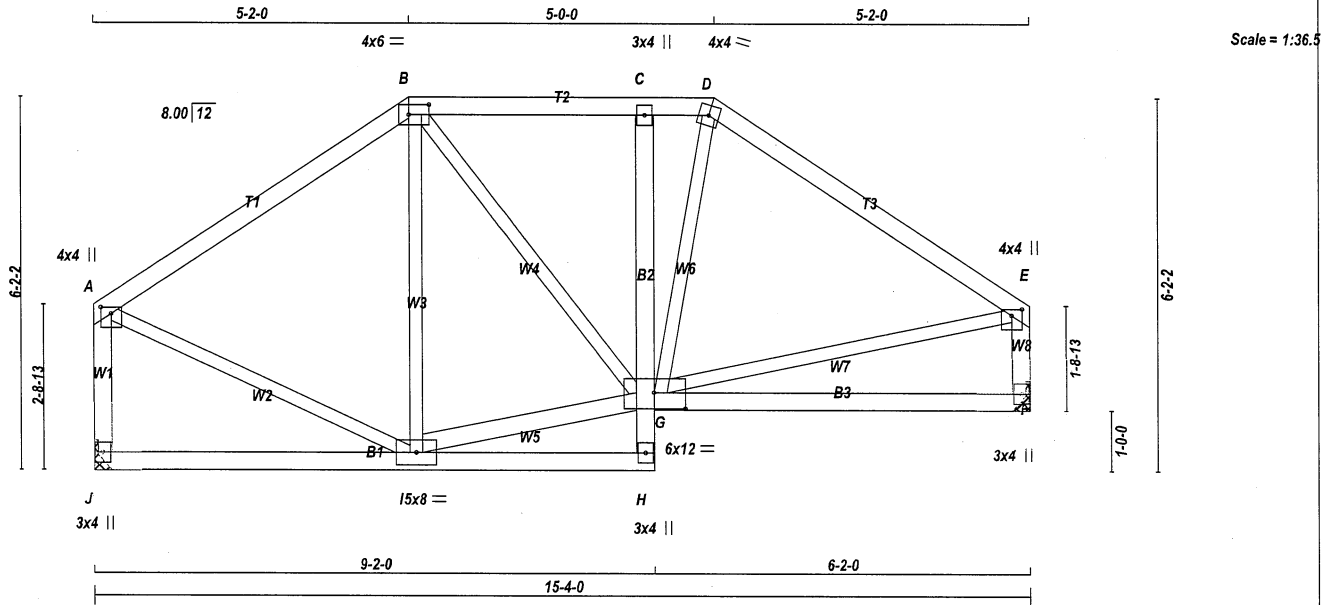


Structural component only
DWG# T-2213222

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426688	T44S	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-RCM8_BwvcVl4lkeeA_KYnURY2qSD_lX?jOfp9zANU



TOTAL WEIGHT = 75 lb
[M][F]

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - B 2x4 DRY No.2 SPF B - D 2x4 DRY No.2 SPF D - E 2x4 DRY No.2 SPF J - A 2x4 DRY No.2 SPF F - E 2x4 DRY No.2 SPF J - H 2x4 DRY No.2 SPF H - C 2x4 DRY No.2 SPF G - F 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT I - G 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX J 1004 0 1004 0 0 MECHANICAL F 1004 0 1004 0 0 MECHANICAL A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3-8, JOINT F = 3-8. UNFACTORED REACTIONS 1ST LOASE MAX/MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL J 703 498 / 0 0 / 0 0 / 0 0 / 0 205 / 0 0 / 0 F 703 498 / 0 0 / 0 0 / 0 0 / 0 205 / 0 0 / 0 BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS WEBS MAX. FACTORED MAX. FACTORED MEMB. FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX (LBS) (PLF) CSI (LC) UNBRAC LENGTH FR-TO (LBS) CSI (LC) FR-TO A-B -714 / 0 -112.4 -112.4 0.39 (1) 6.25 I-B -322 / 0 0.19 (1) B-C -788 / 0 -112.4 -112.4 0.19 (1) 6.25 I-G 0 / 596 0.10 (1) C-D -792 / 0 -112.4 -112.4 0.15 (1) 6.25 B-G 0 / 313 0.07 (1) D-E -881 / 0 -112.4 -112.4 0.40 (1) 5.99 G-D 0 / 268 0.06 (1) J-A -965 / 0 0.0 0.0 0.14 (1) 7.81 A-I 0 / 655 0.15 (1) F-E -945 / 0 0.0 0.0 0.10 (1) 7.81 G-E 0 / 751 0.17 (1) J-I 0 / 0 -18.5 -18.5 0.11 (4) 10.00 I-H 0 / 17 -18.5 -18.5 0.11 (4) 10.00 H-G 0 / 26 0.0 0.0 0.03 (1) 10.00 G-C -440 / 0 0.0 0.0 0.07 (1) 7.81 G-F 0 / 0 -18.5 -18.5 0.22 (4) 10.00				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 32.5 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 45.9 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, ABC 2019 - PART 9 OF OBC 2012 (2019 AMENDMENT) - CSA 086-14 - TPIC 2014 (55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/360 (0.51") CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02") ALLOWABLE DEFL.(TL)= L/360 (0.51") CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09") CSI: TC=0.40/1.00 (D-E:1), BC=0.22/1.00 (F-G:4), WB=0.19/1.00 (B-I:1), SSI=0.20/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 650 371 1747 788 1987 1873 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.78 (E) (INPUT = 0.90) JSI METAL= 0.23 (E) (INPUT = 1.00)			
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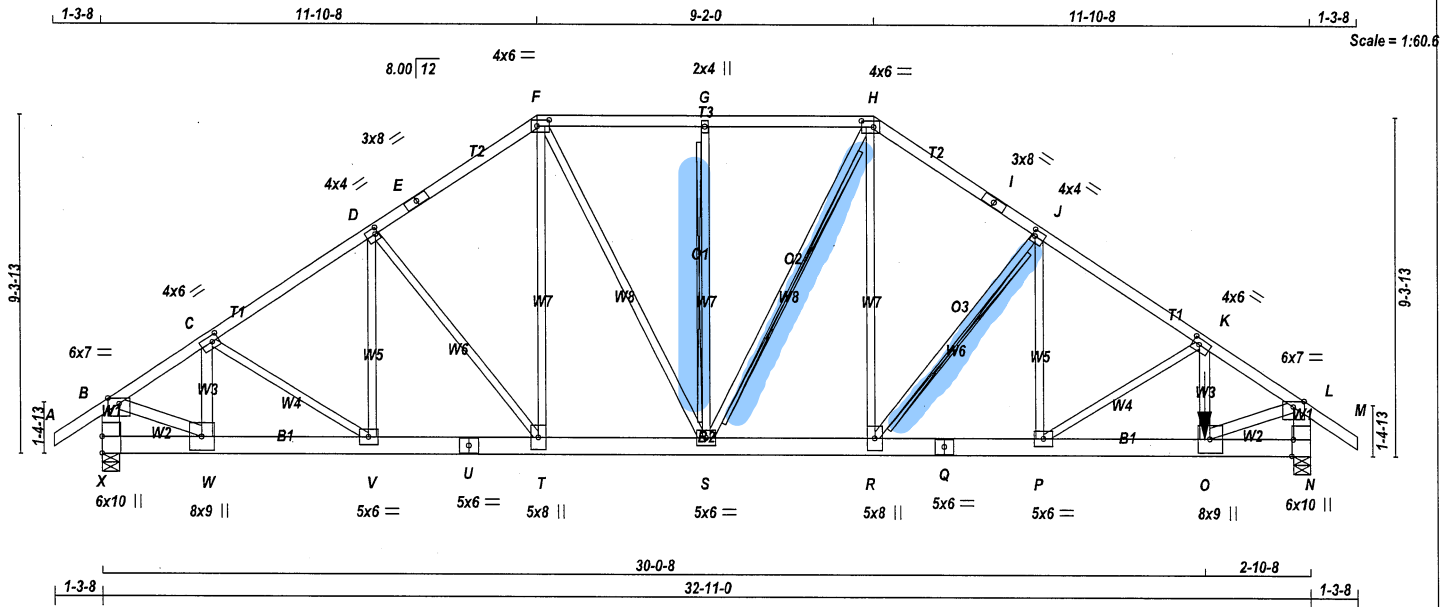
NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2213235

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T50	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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ID: msZCRjkkNthSDqhrbA0REzSQw2-nb3luwUZIcnc1NypV4P7vhiKZDgyIEPqjowydzANRO



TOTAL WEIGHT = 2 X 186 = 372 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
I - M	2x4	DRY	No.2	SPF
X - B	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
X - U	2x6	DRY	No.2	SPF
U - Q	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
W - C	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-E	12	TOP
E-F	12	TOP
F-H	12	TOP
H-I	12	TOP
I-M	12	TOP
X-B	12	TOP
N-L	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X-U	12	TOP
U-Q	12	TOP
Q-N	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(1283.3)
K-O	2	
2x4	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
X	2800	0	5-8	5-8
N	7439	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
X	1959	1405 / 0	0 / 0	0 / 0	554 / 0	0 / 0
N	5205	3735 / 0	0 / 0	0 / 0	1470 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-S, H-S, J-R

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
MEMB.				MEMB.		
FR-TO				FR-TO		
A-B	0 / 43	-112.4	-112.4 0.08 (1)	W-C	-807 / 0	0.06 (1)
B-C	-2922 / 0	-112.4	-112.4 0.12 (1)	C-V	0 / 341	0.04 (1)
C-D	-3275 / 0	-112.4	-112.4 0.25 (1)	V-D	-102 / 45	0.03 (1)
D-E	-3020 / 0	-112.4	-112.4 0.24 (1)	D-T	-410 / 0	0.23 (1)
E-F	-3020 / 0	-112.4	-112.4 0.24 (1)	T-F	0 / 417	0.05 (1)
F-G	-2922 / 0	-112.4	-112.4 0.25 (1)	F-S	0 / 946	0.12 (1)
G-H	-2922 / 0	-112.4	-112.4 0.25 (1)	S-G	-616 / 0	0.26 (1)
H-I	-3605 / 0	-112.4	-112.4 0.27 (1)	S-H	-148 / 0	0.08 (1)
I-J	-3605 / 0	-112.4	-112.4 0.27 (1)	R-H	0 / 1718	0.21 (1)
J-K	-5092 / 0	-112.4	-112.4 0.34 (1)	R-J	-2096 / 0	0.57 (1)
K-L	-8246 / 0	-112.4	-112.4 0.31 (1)	P-J	0 / 1896	0.23 (1)
L-M	0 / 43	-112.4	-112.4 0.08 (1)	P-K	-3112 / 0	0.75 (1)
X-B	-2756 / 0	0.0	0.0 0.10 (1)	O-K	0 / 3044	0.27 (1)
N-L	-7258 / 0	0.0	0.0 0.26 (1)	B-W	0 / 2594	0.23 (1)
				O-L	0 / 7270	0.64 (1)
X-W	0 / 0	-18.5	-18.5 0.02 (1)			
W-V	0 / 2451	-18.5	-18.5 0.19 (1)			
V-U	0 / 2737	-18.5	-18.5 0.19 (1)			
U-T	0 / 2737	-18.5	-18.5 0.19 (1)			
T-S	0 / 2490	-18.5	-18.5 0.18 (1)			
S-R	0 / 2989	-18.5	-18.5 0.21 (1)			
R-O	0 / 4257	-18.5	-18.5 0.29 (1)			
O-P	0 / 4257	-18.5	-18.5 0.29 (1)			
P-O	0 / 6869	-18.5	-18.5 0.61 (1)			
O-N	0 / 0	-18.5	-18.5 0.16 (1)			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	30-0-8	-3932	-3932	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.34/1.00 (J-K:1), BC=0.61/1.00 (O-P:1), WB=0.75/1.00 (K-P:1), SSI=0.14/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	PLATE GRIP(WET)	SECTION (PL)	SECTION (PL)	SECTION (PL)
(PSI)	(PL)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650	371	1747	788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

JSI METAL= 0.77 (W) (INPUT = 1.00)



Structural component only
DWG# T-2213223

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T50	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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ID: msZCRjkkNthSDqhrbA0REzSQw2-nb3luwUZlnc1NypV4P7vhikZDGYIEPgjowiydzANRO

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	7.0	Edge	
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.00
E	TS-t	MT20	3.0	8.0		
F	TTWW-l	MT20	4.0	6.0	2.00	4.00
G	TMW+w	MT20	2.0	4.0		
H	TTWW-l	MT20	4.0	6.0	2.00	4.00
I	TS-t	MT20	3.0	8.0		
J	TMWW-t	MT20	4.0	4.0	2.00	1.00
K	TMWW-t	MT20	4.0	6.0	2.00	2.25
L	TMVW-p	MT20	6.0	7.0	Edge	
N	BMV1-t	MT20	6.0	10.0	Edge	0.50
O	BMWW+t	MT20	8.0	9.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BS-t	MT20	5.0	6.0		
R	BMWW+t	MT20	5.0	8.0		
S	BMWWW-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	8.0		
U	BS-t	MT20	5.0	6.0		
V	BMWW-t	MT20	5.0	6.0		
W	BMWW+t	MT20	8.0	9.0		
X	BMV1-t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

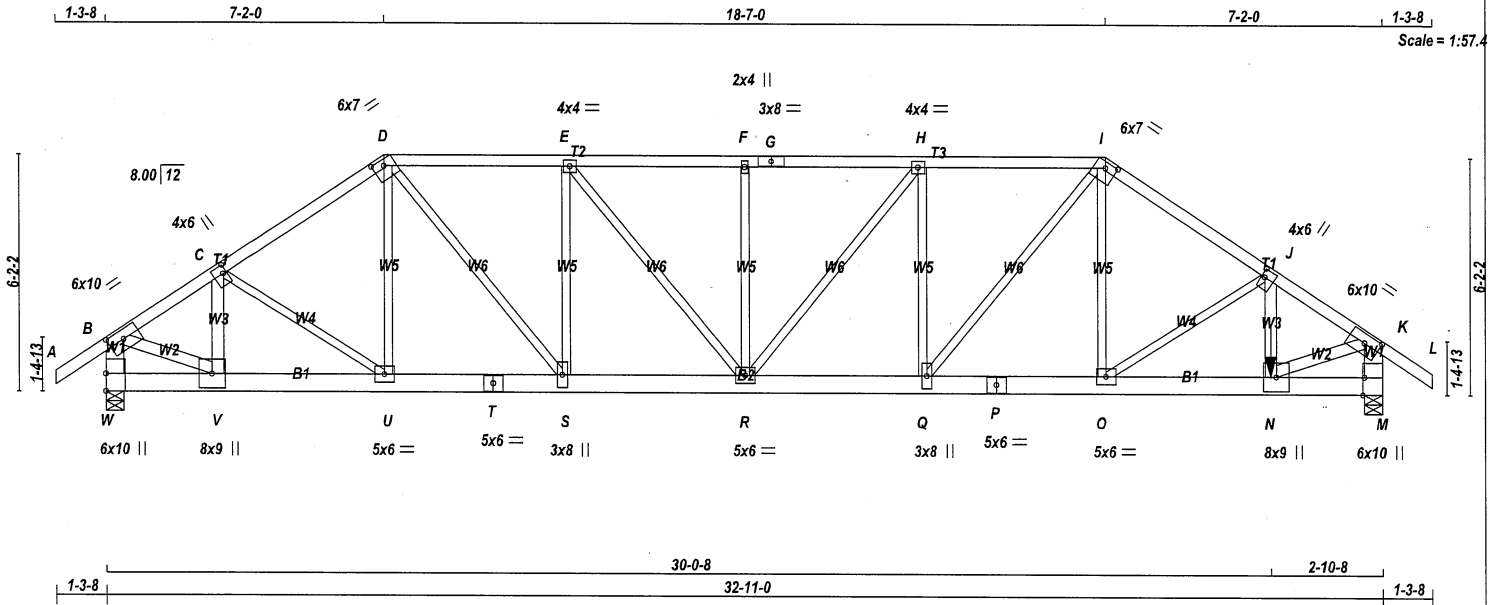


Structural component only
DWG# T-2213223

JOB NAME 426687	TRUSS NAME T51	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:25 2022 Page 1
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TOTAL WEIGHT = 2 X 171 = 343 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - L	2x4	DRY	No.2
W - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
W - T	2x6	DRY	No.2
T - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
V - C	2x4	DRY	No.2
N - J	2x4	DRY	No.2
B - V	2x4	DRY	No.2
N - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
G-I 1	12	TOP
I-L 1	12	TOP
W-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W-T 2	12	TOP
T-P 2	12	TOP
P-M 2	9	SIDE(305.2)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	SIDE(1311.6)
J-N 2	2	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	W	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
W	2851	0	2851	0	0	5-8	5-8	5-8	5-8
M	7965	0	7965	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX /MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
W	1994 1430 / 0 0 / 0 0 / 0 565 / 0 0 / 0
M	5573 3996 / 0 0 / 0 0 / 0 1577 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.07 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-112.4 -112.4	0.08 (1)	10.00	V-C	-861 / 0	0.06 (1)
B-C	-2982 / 0	-112.4 -112.4	0.17 (1)	5.17	C-U	0 / 311	0.04 (1)
C-D	-3357 / 0	-112.4 -112.4	0.19 (1)	4.93	U-D	-43 / 54	0.01 (1)
D-E	-3966 / 0	-112.4 -112.4	0.27 (1)	4.52	D-S	0 / 1900	0.24 (1)
E-F	-4606 / 0	-112.4 -112.4	0.29 (1)	4.22	S-E	-1391 / 0	0.40 (1)
F-G	-4606 / 0	-112.4 -112.4	0.29 (1)	4.22	E-R	0 / 1030	0.13 (1)
G-H	-4606 / 0	-112.4 -112.4	0.29 (1)	4.22	R-F	-473 / 0	0.14 (1)
H-I	-4822 / 0	-112.4 -112.4	0.30 (1)	4.13	R-H	-349 / 0	0.18 (1)
I-J	-5405 / 0	-112.4 -112.4	0.30 (1)	3.94	Q-I	-308 / 0	0.09 (1)
J-K	-8851 / 0	-112.4 -112.4	0.35 (1)	3.07	Q-I	0 / 483	0.06 (1)
K-L	0 / 43	-112.4 -112.4	0.08 (1)	10.00	O-I	0 / 2183	0.27 (1)
W-B	-2797 / 0	0.0 0.0	0.10 (1)	7.81	O-J	-3482 / 0	0.81 (1)
M-K	-7759 / 0	0.0 0.0	0.28 (1)	5.39	N-J	0 / 3380	0.30 (1)
					B-V	0 / 2657	0.23 (1)
					N-K	0 / 7812	0.69 (1)
W-V	0 / 0	-18.5 -18.5	0.03 (1)	10.00			
V-U	0 / 2510	-18.5 -18.5	0.19 (1)	10.00			
U-T	0 / 2769	-18.5 -18.5	0.20 (1)	10.00			
T-S	0 / 2769	-18.5 -18.5	0.20 (1)	10.00			
S-R	0 / 3966	-18.5 -18.5	0.29 (1)	10.00			
R-Q	0 / 4822	-18.5 -18.5	0.34 (1)	10.00			
Q-P	0 / 4518	-18.5 -18.5	0.34 (1)	10.00			
P-O	0 / 4518	-18.5 -18.5	0.34 (1)	10.00			
O-N	0 / 7381	-18.5 -18.5	0.67 (1)	10.00			
N-M	0 / 0	-18.5 -18.5	0.19 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	30-0-8	-4336	-4336	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	32.5 PSF
DL	=	6.0 PSF
BOT CH. LL	=	0.0 PSF
DL	=	7.4 PSF
TOTAL LOAD	=	45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL)= L/999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL)= L/999 (0.18")

CSI: TC=0.35/1.00 (J-K:1), BC=0.67/1.00 (N-O:1), WB=0.81/1.00 (J-O:1), SSI=0.14/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.83 (V) (INPUT = 1.00)



Structural component only
DWG# T-2213224

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T51	1	2	BAYVIEW WELLINGTON	
				TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	6.0	10.0	2.75	4.75
C	TMWW+t	MT20	4.0	6.0	2.50	1.00
D	TTWW-h	MT20	6.0	7.0	1.75	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW-h	MT20	6.0	7.0	1.75	3.50
J	TMWW+t	MT20	4.0	6.0	2.50	1.00
K	TMVW-t	MT20	6.0	10.0	2.75	4.75
M	BMV1+t	MT20	6.0	10.0	Edge	0.50
N	BMWW+t	MT20	8.0	9.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	5.0	6.0		
Q	BMWW+t	MT20	3.0	8.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMWW+t	MT20	3.0	8.0		
T	BS-t	MT20	5.0	6.0		
U	BMWW-t	MT20	5.0	6.0		
V	BMWW+t	MT20	8.0	9.0		
W	BMV1+t	MT20	6.0	10.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



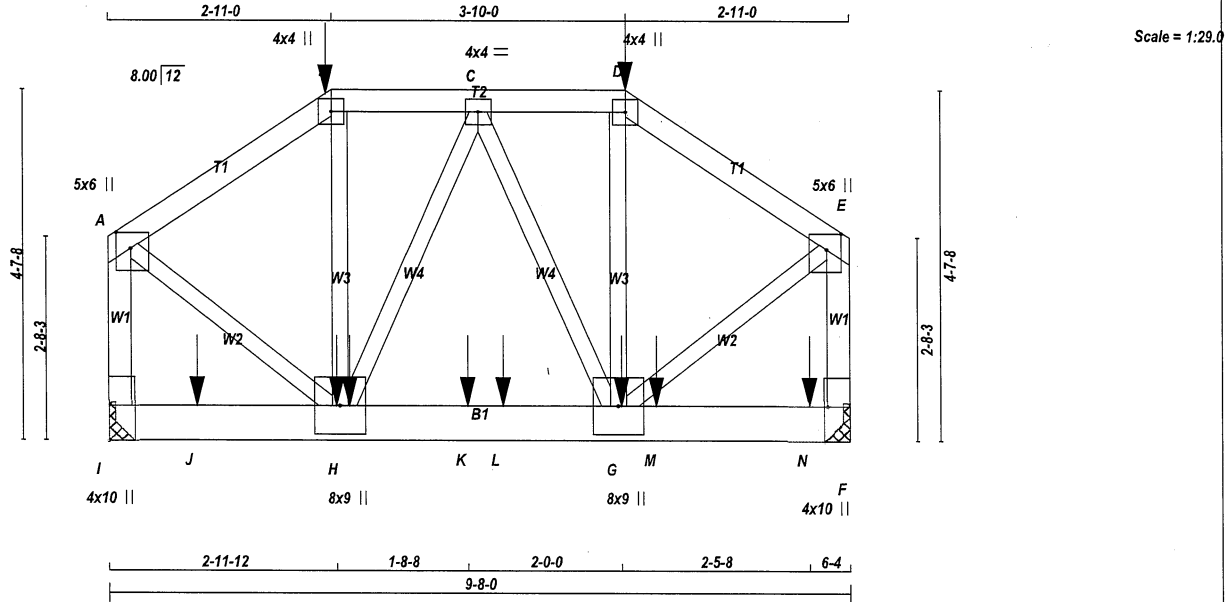
Structural component only
DWG# T-2213224

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T52	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 53 = 107 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - B	2x4	DRY	No.2
B - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
I - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - F	2x6	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - B	1	12
B - D	1	12
D - E	1	12
I - A	1	12
F - E	1	2
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
I - F	2	12
WEBS : (0.122"x3") SPIRAL NAILS		
H - B	1	6
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
I	5640	0	5640	0	0
F	6216	0	6216	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I, F. MINIMUM BEARING LENGTH AT JOINT I = 4-0, JOINT F = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3947	2823 / 0	0 / 0	0 / 0	0 / 0	1124 / 0	0 / 0
F	4351	3110 / 0	0 / 0	0 / 0	0 / 0	1241 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A - B	-3764 / 0	-112.4 -112.4	0.14 (1)	4.75	H - B	0 / 1639	0.20 (1)
B - C	-3165 / 0	-112.4 -112.4	0.08 (1)	5.16	H - C	-32 / 9	0.01 (1)
C - D	-3127 / 0	-112.4 -112.4	0.08 (1)	5.19	C - G	-129 / 0	0.02 (1)
D - E	-3719 / 0	-112.4 -112.4	0.14 (1)	4.77	G - D	0 / 1614	0.20 (1)
I - A	-4617 / 0	0.0 0.0	0.33 (1)	5.55	A - H	0 / 3912	0.48 (1)
F - E	-4564 / 0	0.0 0.0	0.32 (1)	5.58	G - E	0 / 3865	0.48 (1)
I - J	0 / 0	-33.5 -33.5	0.42 (1)	10.00			
J - H	0 / 0	-33.5 -33.5	0.42 (1)	10.00			
H - K	0 / 3178	-18.5 -18.5	0.57 (1)	10.00			
K - L	0 / 3178	-18.5 -18.5	0.57 (1)	10.00			
L - G	0 / 3178	-18.5 -18.5	0.57 (1)	10.00			
G - M	0 / 0	-33.5 -33.5	0.37 (1)	10.00			
M - N	0 / 0	-33.5 -33.5	0.37 (1)	10.00			
N - F	0 / 0	-33.5 -33.5	0.37 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-11-0	-106	-106	---	FRONT	VERT	TOTAL	---	C1
D	6-9-0	-106	-106	---	FRONT	VERT	TOTAL	---	C1
G	6-8-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
H	2-11-12	-111	-111	---	FRONT	VERT	TOTAL	---	C1
J	3-1-12	-1358	-1358	---	BACK	VERT	TOTAL	---	C1
K	1-1-12	-1358	-1358	---	BACK	VERT	TOTAL	---	C1
L	4-8-4	-111	-111	---	FRONT	VERT	TOTAL	---	C1
M	5-1-12	-1358	-1358	---	BACK	VERT	TOTAL	---	C1
N	7-1-12	-1358	-1358	---	BACK	VERT	TOTAL	---	C1
	9-1-12	-1361	-1361	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.32")
CALCULATED VERT. DEFL. (LL) = L/999 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.32")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.33/1.00 (A-I:1), BC=0.57/1.00 (G-H:1), WB=0.48/1.00 (A-H:1), SSI=0.77/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.71 (A) (INPUT = 1.00)



Structural component only
DWG# T-2213225

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T52	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	5.0	6.0	2.50 2.25
B	TTW+p	MT20	4.0	4.0	
C	TMWW-t	MT20	4.0	4.0	
D	TTW+p	MT20	4.0	4.0	
E	TMVW+p	MT20	5.0	6.0	2.50 2.25
F	BMV1+t	MT20	4.0	10.0	Edge 0.50
G	BMWWW+t	MT20	8.0	9.0	
H	BMWWW+t	MT20	8.0	9.0	
I	BMV1+t	MT20	4.0	10.0	5.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

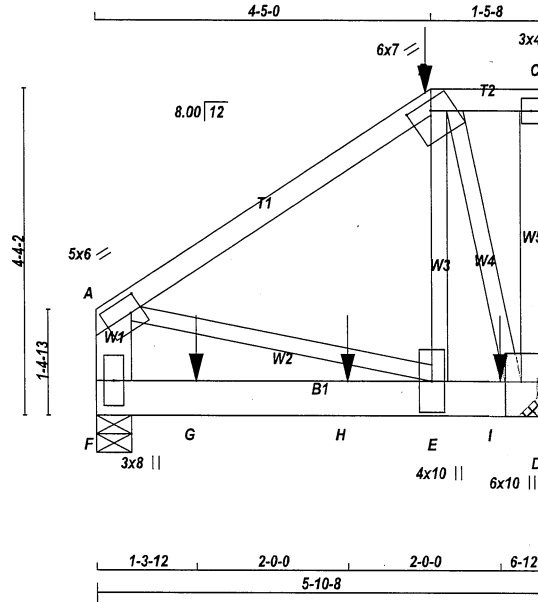
NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2



Structural component only
DWG# T-2213225

JOB NAME 426687	TRUSS NAME T54	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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Scale = 1:29.3

TOTAL WEIGHT = 2 X 33 = 67 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
F - A	2x6 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	SIDE(61.0)
B-C 1	12	SIDE(61.0)
C-D 1	12	TOP
F-A 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
E-B 1	6	SIDE(26.6)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	UPLIFT
D	4175	0	4175	0
F	3123	0	3123	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	2912	2140 / 0	0 / 0	0 / 0	0 / 0	771 / 0	0 / 0
F	2181	1589 / 0	0 / 0	0 / 0	0 / 0	592 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-1682 / 0	-112.4 -112.4	0.23 (1)	E-B	0 / 4122	0.51 (1)	
B-C	0 / 0	-112.4 -112.4	0.02 (1)	B-D	-4353 / 0	0.63 (1)	
D-C	-82 / 0	0.0 0.0	0.01 (1)	A-E	0 / 1431	0.18 (1)	
F-A	-1492 / 0	0.0 0.0	0.05 (1)				
F-G	0 / 0	-18.5 -18.5	0.78 (1)				
G-H	0 / 0	-18.5 -18.5	0.78 (1)				
H-E	0 / 0	-18.5 -18.5	0.78 (1)				
E-I	0 / 1506	-18.5 -18.5	0.68 (1)				
I-D	0 / 1506	-18.5 -18.5	0.68 (1)				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-5-0	-62	-62	---	FRONT	VERT	DEAD	---	C1
G	1-3-12	-1387	-1387	---	BACK	VERT	TOTAL	---	C1
H	3-3-12	-1387	-1387	---	BACK	VERT	TOTAL	---	C1
I	5-3-12	-1390	-1390	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/603 (0.12")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.78/1.00 (E-F:1) , WB=0.63/1.00 (B-D:1) , SSI=0.80/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 798	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (B) (INPUT = 0.90)
JSI METAL= 0.58 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2



Structural component only
DWG# T-2213226

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T54	1	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:27 2022 Page 2
ID: msZCRikkNthSDqhrbA0REzSQw2-j A3JcVgHE1KGg6BdVRb 6nim0ujmAl7A6Pp1VzANRM

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	1.75
B	TTWW-h	MT20	6.0	7.0	1.50	3.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+t	MT20	6.0	10.0	Edge	2.50
E	BMWW+t	MT20	4.0	10.0		
F	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LICENSED PROFESSIONAL ENGINEER

06-02-22

H. J. G. ALVES

100009024

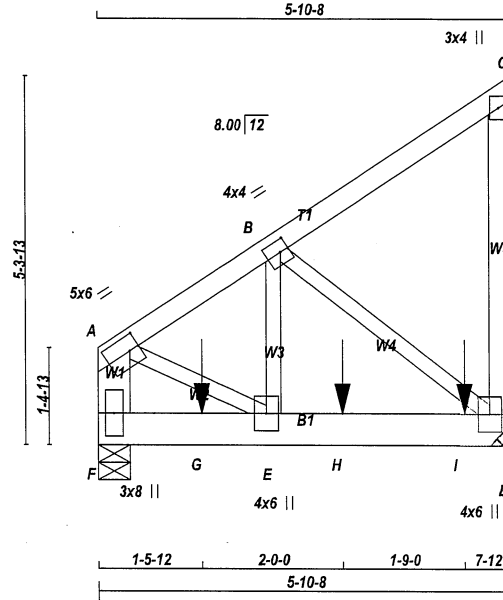
PROVINCE OF ONTARIO

Structural component only
DWG# T-2213226

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T55	2	2	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-CakRWyWS2X9BuqgOACyqXJKvHQMVMV_GPI8MZxANRL



Scale: 3/8"=1'

TOTAL WEIGHT = 4 X 32 = 129 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY No.2	SPF			
D - C	2x4 DRY No.2	SPF			
F - A	2x6 DRY No.2	SPF			
F - D	2x6 DRY No.2	SPF			

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1 12	TOP	
D - C 1 12	TOP	
F - A 2 12	TOP	
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2 12	SIDE(0.0)	
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0	2.50	1.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT HORZ	DOWN HORZ UPLIFT	IN-SX	IN-SX
D 2135 0	2135 0 0	MECHANICAL	
F 1748 0	1748 0 0	5-8	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
D 1493 1077 / 0	0 / 0	0 / 0	0 / 0	0 / 0	416 / 0	0 / 0	
F 1221 888 / 0	0 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				W E B S			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A - B	-1549 / 0	-112.4 -112.4	0.09 (1)	6.25	E - B	0 / 1497	0.19 (1)
B - C	-17 / 0	-112.4 -112.4	0.08 (1)	6.25	B - D	-1665 / 0	0.25 (1)
D - C	-159 / 0	0.0 0.0	0.04 (1)	7.81	A - E	0 / 1409	0.17 (1)
F - A	-1493 / 0	0.0 0.0	0.05 (1)	7.81			
F - G	0 / 0	-18.5 -18.5	0.21 (1)	10.00			
G - E	0 / 0	-18.5 -18.5	0.21 (1)	10.00			
E - H	0 / 1309	-18.5 -18.5	0.31 (1)	10.00			
H - I	0 / 1309	-18.5 -18.5	0.31 (1)	10.00			
I - D	0 / 1309	-18.5 -18.5	0.31 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-5-12	-794	---	---	BACK	VERT	TOTAL	---	C1
H	3-5-12	-689	---	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-691	---	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD	=	45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.09/1.00 (A-B:1), BC=0.31/1.00 (D-E:1), WB=0.25/1.00 (B-D:1), SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(FSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (B) (INPUT = 0.90)
JSI METAL = 0.29 (D) (INPUT = 1.00)



Structural component only
DWG# T-2213227

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	T55	2	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	8.0		

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LICENSED PROFESSIONAL ENGINEER

06-02-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2213227

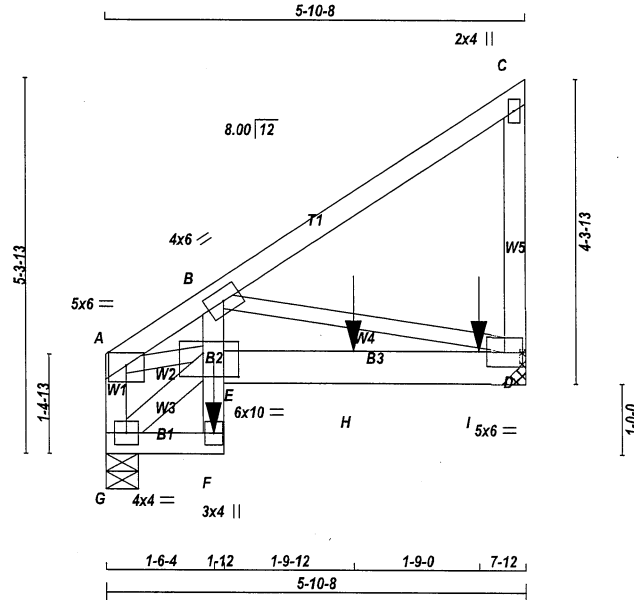


Structural component only
DWG# T-2213227

JOB NAME 426688	TRUSS NAME T55S	QUANTITY 1	PLY 2	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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Scale = 1:31.2

TOTAL WEIGHT = 2 X 31 = 62 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
G - A	2x4	DRY	No.2
A - C	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - B	2x4	DRY	No.2
E - D	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
B - D	2x3	DRY	No.2
A - E	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
G - A 1	12	TOP
A - C 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
G - F 1	12	SIDE(61.0)
F - B 1	3	SIDE(238.6)
E - D 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS								
	FACTORED		MAXIMUM FACTORED			INPUT	REQD	
	GROSS REACTION		GROSS REACTION			BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	1692	0	1692	0	0	5-8	5-8	
D	2173	0	2173	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1182	856 / 0	0 / 0	0 / 0	0 / 0	326 / 0	0 / 0
D	1519	1094 / 0	0 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
G-A	-1571 / 0	0.0	0.0	0.09 (1)	7.81
A-B	-3125 / 0	-112.4	-112.4	0.07 (1)	5.19
B-C	-13 / 0	-112.4	-112.4	0.16 (1)	6.25
G-F	0 / 135	-18.5	-18.5	0.02 (1)	10.00
F-E	0 / 1163	0.0	0.0	0.23 (1)	10.00
E-B	0 / 2046	0.0	0.0	0.30 (1)	10.00
E-H	0 / 2792	-18.5	-18.5	0.66 (1)	10.00
H-I	0 / 2792	-18.5	-18.5	0.66 (1)	10.00
I-D	0 / 2792	-18.5	-18.5	0.66 (1)	10.00

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-6-4	-801	-801	---	BACK	VERT	TOTAL	---	C1
H	3-5-12	-689	-689	---	BACK	VERT	TOTAL	---	C1
I	5-2-12	-692	-692	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/917 (0.07")

CSI: TC=0.16/1.00 (B-C:1), BC=0.66/1.00 (D-E:1), WB=0.46/1.00 (B-D:1), SSI=0.45/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)		
MAX MIN	MAX MIN	MAX MIN			
MT20	650 371	1747 788	1987 1873		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.33 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213237

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426688	T55S	1	2	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	Edge	
B	TMVW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	BMWW1-t	MT20	5.0	6.0		
E	BVMWW-l	MT20	6.0	10.0	4.00	6.00
F	BMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

LICENSED PROFESSIONAL ENGINEER

06-02-22

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only
DWG# T-2213237

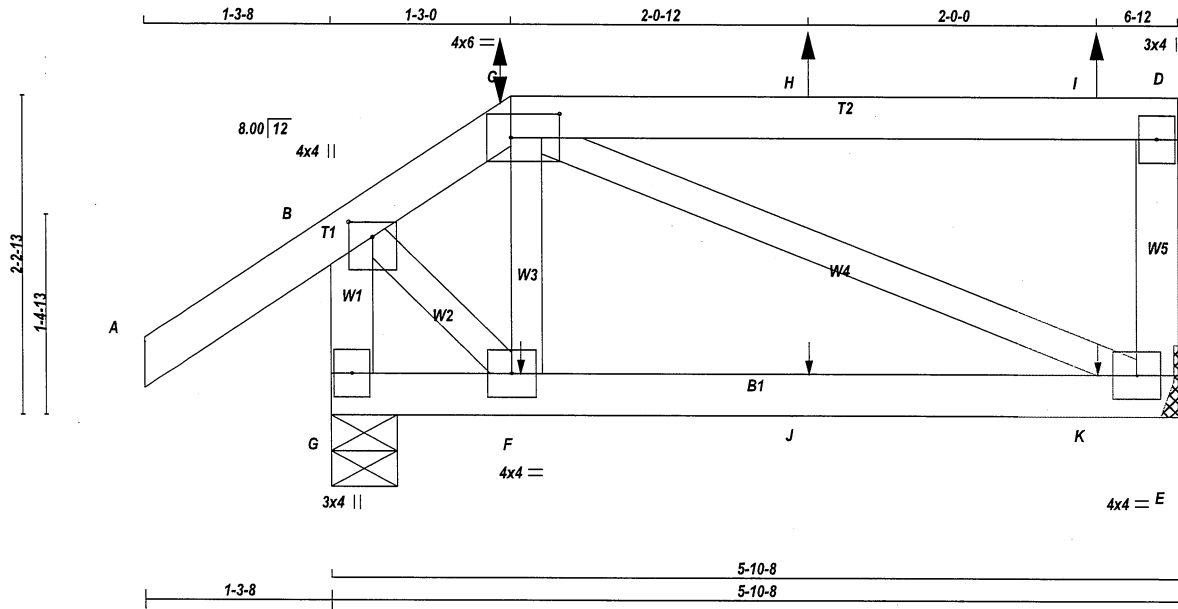


Structural component only
DWG# T-2213237

JOB NAME 426688	TRUSS NAME T56	QUANTITY 1	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: _msZCRjkkNthSDqhrbA0REzSQw2-53bx0YXIDT_vKuZwJ3Eic82ZZ058sb9BphXtHTZ90Lc



TOTAL WEIGHT = 25 lb [M]

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TTWW-l	MT20	4.0	6.0	2.00 4.00
D	TMV+p	MT20	3.0	4.0	
E	BMVW1-t	MT20	4.0	4.0	
F	BMVW-t	MT20	4.0	4.0	
G	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED	MAXIMUM FACTORED	INPUT	REQD		
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
E	352	0	352	0	MECHANICAL
G	575	0	575	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	244	186 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
G	398	311 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-112.4 -112.4	0.17 (1)	10.00	F-C	-66 / 75	0.02 (4)
B-C	-273 / 0	-112.4 -112.4	0.16 (1)	6.25	C-E	-198 / 0	0.08 (1)
C-H	0 / 0	-112.4 -112.4	0.42 (1)	10.00	B-F	0 / 238	0.06 (1)
H-I	0 / 0	-112.4 -112.4	0.42 (1)	10.00			
I-D	0 / 0	-112.4 -112.4	0.42 (1)	10.00			
E-D	-240 / 0	0.0 0.0	0.03 (1)	7.81			
G-B	-584 / 0	0.0 0.0	0.07 (1)	7.81			
G-F	0 / 0	-18.5 -18.5	0.09 (4)	10.00			
F-J	0 / 183	-18.5 -18.5	0.11 (4)	10.00			
J-K	0 / 183	-18.5 -18.5	0.11 (4)	10.00			
K-E	0 / 183	-18.5 -18.5	0.11 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-3-0	-22	-30	122	FRONT	VERT	TOTAL	---	C1
F	1-3-12	1	1	---	FRONT	VERT	TOTAL	---	C1
H	3-3-12	14	1	127	FRONT	VERT	TOTAL	---	C1
I	5-3-12	11	1	111	FRONT	VERT	TOTAL	---	C1
J	3-3-12	1	1	---	FRONT	VERT	TOTAL	---	C1
K	5-3-12	1	1	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	32.5	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	45.9	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.42/1.00 (C-D:1), BC=0.11/1.00 (E-F:4), WB=0.08/1.00 (C-E:1), SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.39 (B) (INPUT = 0.90)

JSI METAL = 0.12 (B) (INPUT = 1.00)

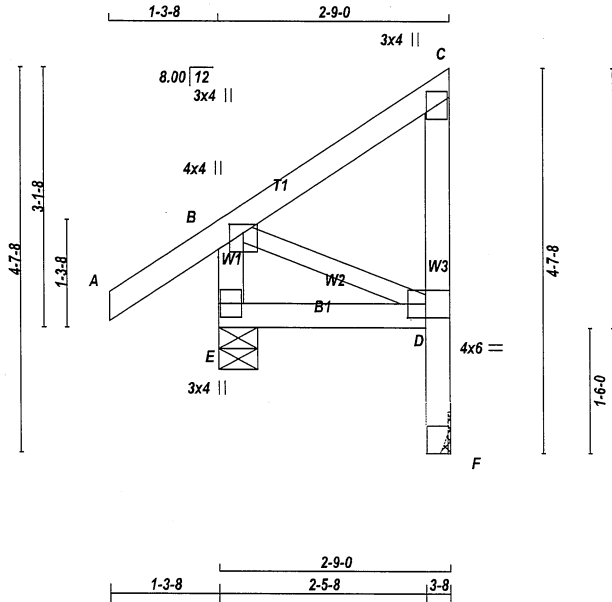


Structural component only
DWG# T-2213748

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426687	T57	3	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MTek Industries, Inc. Thu Jun 2 09:33:28 2022 Page 1
ID: msZCRjkkNthSDqhrbA0REzSQw2-CAkRWyWS2X9BuqgOACyqXJKuiQQYVmrGPI8MZxANRL



Scale = 1:26.4

TOTAL WEIGHT = 3 X 16 = 48 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
E - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
B	TP+p	MT20	3.0	4.0	3.50 3.50
C	TMV+p	MT20	3.0	4.0	
D	BVMW-l	MT20	4.0	6.0	2.00 2.50
E	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX IN-SX
F	180	0	180	0	0
E	334	0	334	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT COMBINED						
F	126	89 / 0	0 / 0	0 / 0	37 / 0	0 / 0
E	232	178 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 43	-112.4 -112.4	0.15 (1)	B-D	0 / 0	0.00 (1)	
B-C	0 / 0	-112.4 -112.4	0.14 (1)				
F-D	-180 / 0	0.0 0.0	0.02 (1)				7.81
D-C	-155 / 0	0.0 0.0	0.03 (1)				7.81
E-B	-309 / 0	0.0 0.0	0.03 (1)				7.81
E-D	0 / 0	-18.5 -18.5	0.04 (4)				10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.04/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

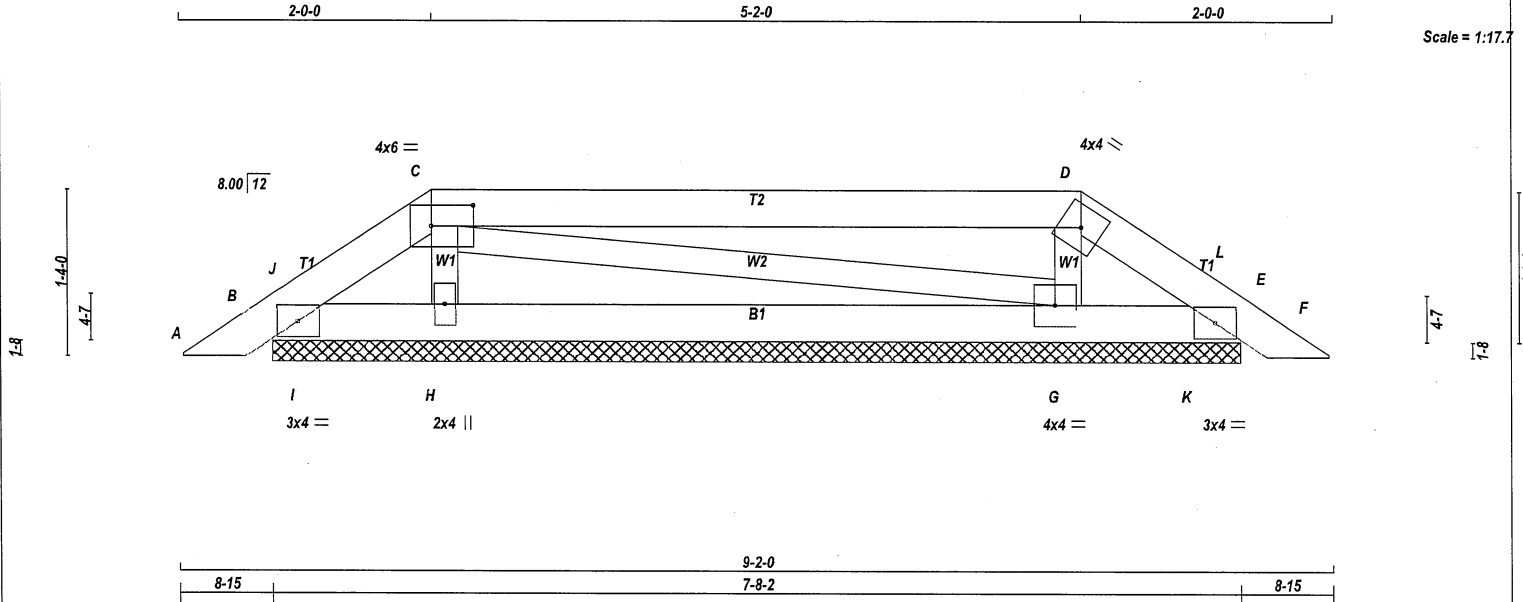
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213228

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	P3	2	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 25 = 50 lb [M]

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-I	MT20	4.0	6.0	2.00 4.00
D	TTWW-h	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	
H	BMW1-w	MT20	2.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
B	170	0	0	170	0	0	0	7-8-2	7-8-2
E	167	0	0	167	0	0	0	7-8-2	7-8-2
H	403	0	0	403	0	0	0	7-8-2	7-8-2
G	409	0	0	409	0	0	0	7-8-2	7-8-2

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	115	106 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	
E	113	104 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0	
H	286	184 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0	
G	290	188 / 0	0 / 0	0 / 0	0 / 0	102 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		W E B S		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)
FR-TO		FROM TO	CSI (LC)	FR-TO			
A-B	0 / 18	-112.4 -112.4	0.03 (1)	10.00	H-C	-300 / 0	0.04 (1)
B-J	-54 / 9	-112.4 -112.4	0.02 (4)	6.25	C-G	-6 / 0	0.00 (1)
J-C	-70 / 0	-112.4 -112.4	0.02 (4)	6.25	G-D	-304 / 0	0.04 (1)
C-D	-14 / 0	-112.4 -112.4	0.51 (1)	6.25	I-J	-97 / 0	0.00 (1)
D-L	-64 / 0	-112.4 -112.4	0.02 (4)	6.25	K-L	-97 / 0	0.00 (1)
L-E	-47 / 10	-112.4 -112.4	0.02 (4)	6.25			
E-F	0 / 18	-112.4 -112.4	0.03 (1)	10.00			
B-I	0 / 54	-18.5 -18.5	0.03 (1)	10.00			
I-H	0 / 54	-18.5 -18.5	0.08 (4)	10.00			
H-G	0 / 20	-18.5 -18.5	0.08 (4)	10.00			
G-K	0 / 49	-18.5 -18.5	0.08 (4)	10.00			
K-E	0 / 49	-18.5 -18.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.51/1.00 (C-D:1), BC=0.08/1.00 (H-I:4), WB=0.04/1.00 (D-G:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (D) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)

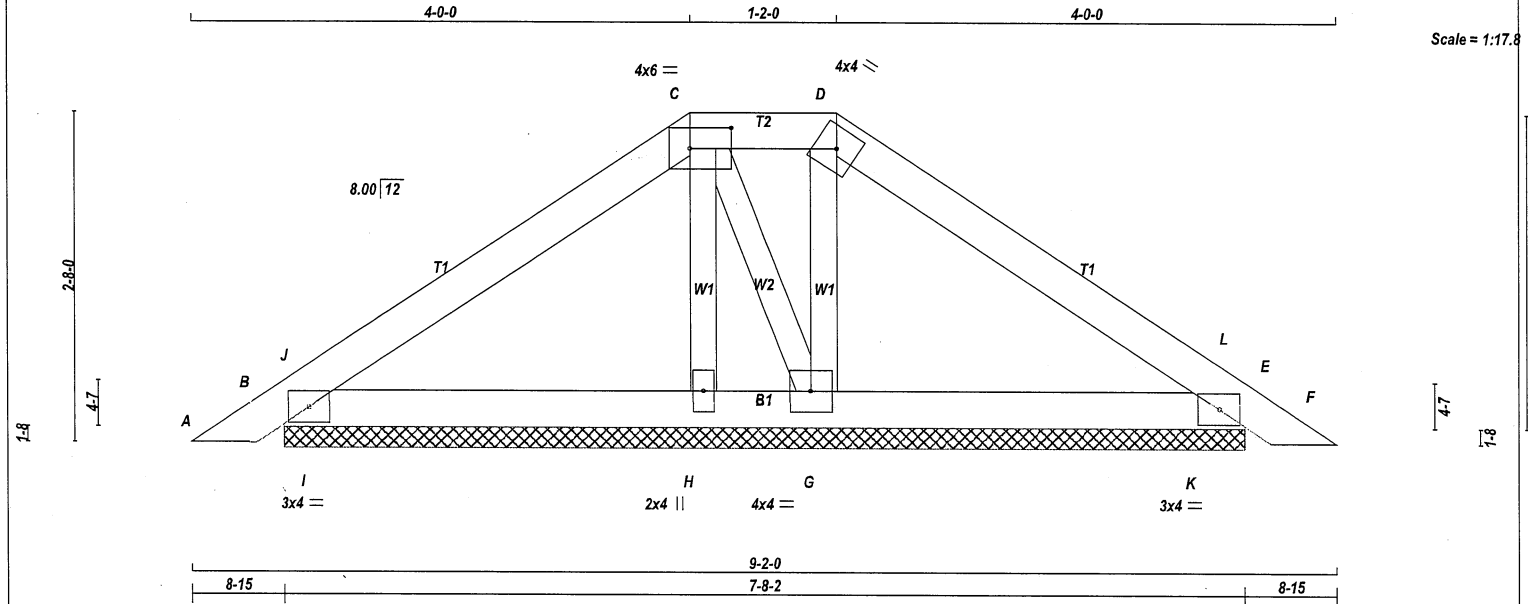


Structural component only
DWG# T-2213200

JOB NAME 426687	TRUSS NAME P4	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 26 = 52 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW-I	MT20	4.0	6.0	2.00 4.00
D	TTWW-h	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BMWV1-t	MT20	4.0	4.0	
H	BMWV1+w	MT20	2.0	4.0	

NOTES: (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UP	IN-SX	IN-SX	BRG
B	343	0	343	0	0	7-8-2	7-8-2	7-8-2	
E	327	0	327	0	0	7-8-2	7-8-2	7-8-2	
H	180	0	180	0	0	7-8-2	7-8-2	7-8-2	
G	304	0	304	0	0	7-8-2	7-8-2	7-8-2	

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	238	180 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
E	227	171 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0
H	128	81 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
G	213	153 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 19	-112.4 -112.4	0.04 (1)	10.00	H-C	-99 / 0	0.02 (1)
B-J	-51 / 0	-112.4 -112.4	0.05 (1)	6.25	C-G	-60 / 0	0.01 (1)
J-C	-126 / 0	-112.4 -112.4	0.14 (1)	6.25	G-D	-168 / 0	0.03 (1)
C-D	-69 / 0	-112.4 -112.4	0.03 (1)	6.25	I-J	-302 / 0	0.00 (1)
D-L	-99 / 0	-112.4 -112.4	0.14 (1)	6.25	K-L	-305 / 0	0.00 (1)
L-E	-30 / 0	-112.4 -112.4	0.05 (1)	6.25			
E-F	0 / 19	-112.4 -112.4	0.04 (1)	10.00			
B-I	0 / 99	-18.5 -18.5	0.14 (1)	10.00			
I-H	0 / 99	-18.5 -18.5	0.14 (1)	10.00			
H-G	0 / 94	-18.5 -18.5	0.08 (1)	10.00			
G-K	0 / 77	-18.5 -18.5	0.13 (1)	10.00			
K-E	0 / 77	-18.5 -18.5	0.13 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 45.9	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (C-J:1), BC=0.14/1.00 (B-I:1), WB=0.03/1.00 (D-G:1), SSI=0.23/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

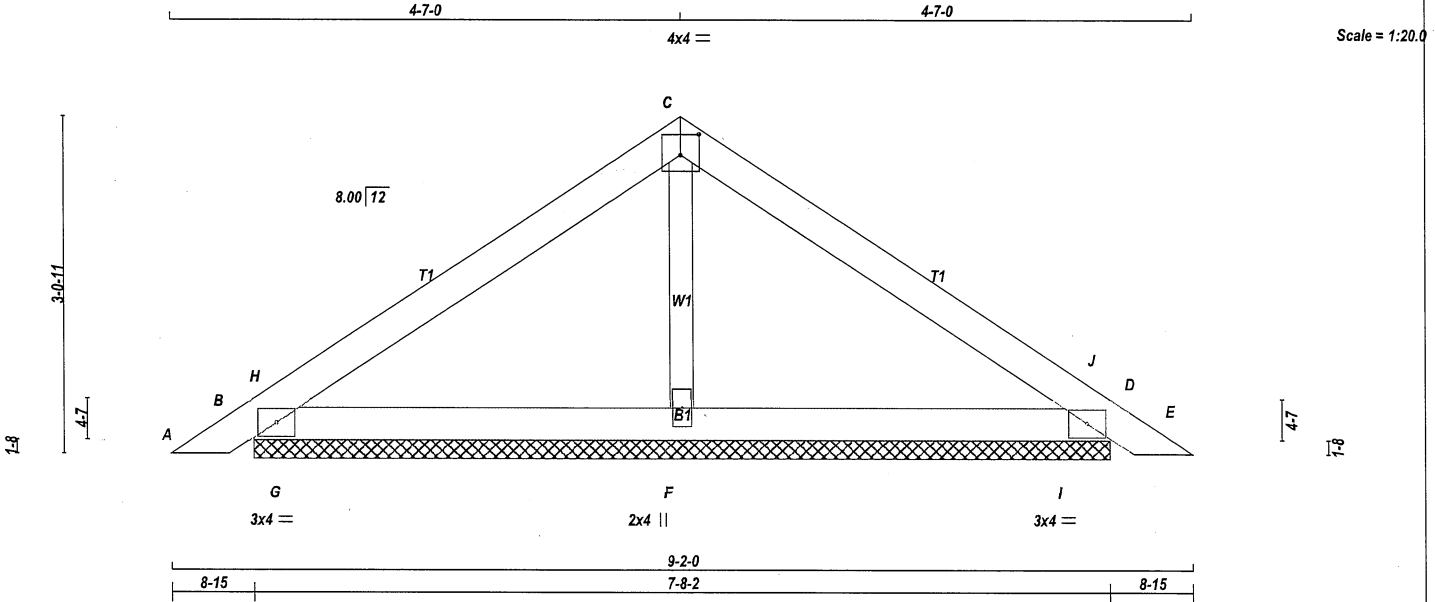
JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213201

JOB NAME 426687	TRUSS NAME P5	QUANTITY 4	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 4 X 23 = 93 lb

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	388	0	388	0	0
D	388	0	388	0	0
F	379	0	379	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX /MIN.	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	269	205 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
D	269	205 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
F	268	175 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 19	-112.4	-112.4	0.04 (1)	10.00	F-C	-184 / 0
B-H	-31 / 0	-112.4	-112.4	0.10 (1)	6.25	G-H	-447 / 0
H-C	-167 / 0	-112.4	-112.4	0.19 (1)	6.25	I-J	-447 / 0
C-J	-167 / 0	-112.4	-112.4	0.19 (1)	6.25		
J-D	-31 / 0	-112.4	-112.4	0.10 (1)	6.25		
D-E	0 / 19	-112.4	-112.4	0.04 (1)	10.00		
B-G	0 / 130	-18.5	-18.5	0.19 (1)	10.00		
G-F	0 / 130	-18.5	-18.5	0.19 (1)	10.00		
F-I	0 / 130	-18.5	-18.5	0.19 (1)	10.00		
I-D	0 / 130	-18.5	-18.5	0.19 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.19/1.00 (C-H:1), BC=0.19/1.00 (D-I:1), WB=0.03/1.00 (C-F:1), SSI=0.33/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

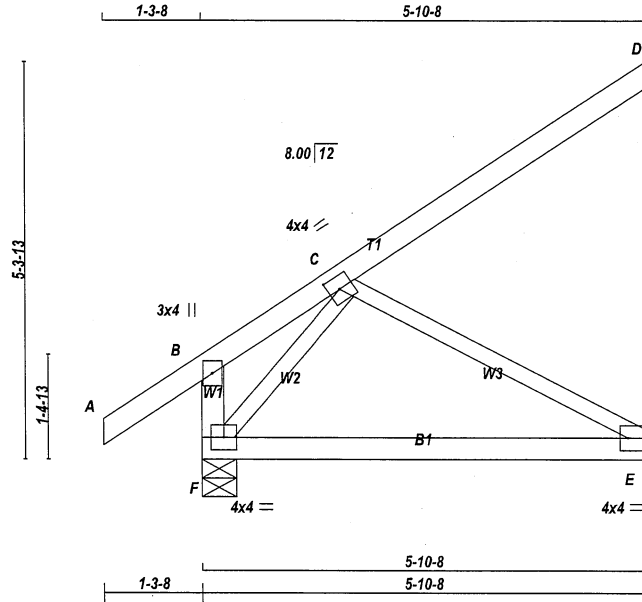


Structural component only
DWG# T-2213202

JOB NAME 426687	TRUSS NAME J7	QUANTITY 6	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:02 2022 Page 1
ID: _msZCRjkkNthSDqhrbA0REzSQw2-yAAc33C8AYttmqhOfiVktqFQBxW9P_yRQjr_eSzANRI



Scale = 1:29.5

TOTAL WEIGHT = 6 X 24 = 141 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
E	BMVW1-t	MT20	4.0	4.0	2.00	Edge
F	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES-

- 1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	BRG	BRG
F	554	0	554	0	0	5-8	5-8		
D	190	0	190	0	0	1-8	1-8		
E	178	0	178	0	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) D, E

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	385	289 / 0	0 / 0	0 / 0	0 / 0	96 / 0	0 / 0
D	130	110 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
E	128	72 / 0	0 / 0	0 / 0	0 / 0	56 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		FACTORED		
MEMB.	FORCE (LBS)	VERT. (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
F-B	-234 / 0	0.0	0.0	0.02 (1)	7.81	F-C	-341 / 0	0.06 (1)	
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-E	-246 / 0	0.09 (1)	
B-C	0 / 13	-112.4	-112.4	0.21 (1)	10.00				
C-D	-21 / 0	-112.4	-112.4	0.22 (1)	6.25				
F-E	0 / 212	-18.5	-18.5	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.05")

CSI: TC=0.22/1.00 (C-D:1), BC=0.19/1.00 (E-F:4),
WB=0.09/1.00 (C-E:1), SSI=0.17/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.26 (F) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



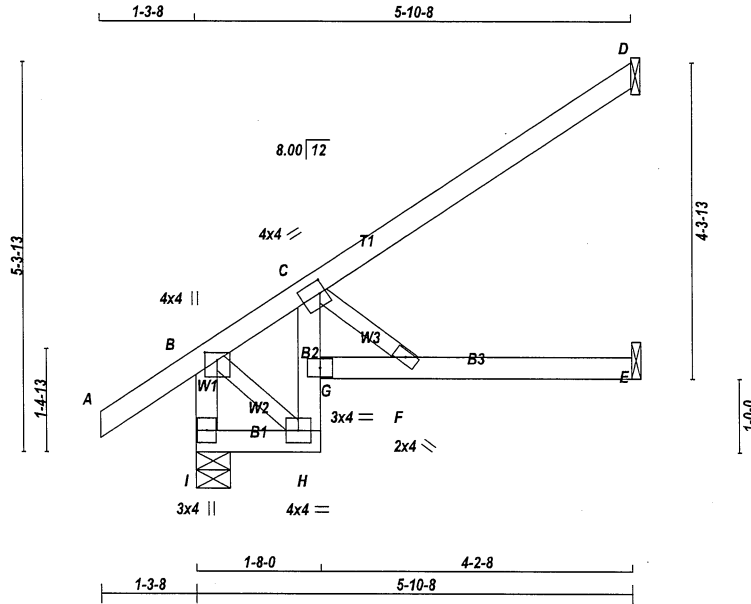
Structural component only
DWG# T-2213191

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	BAYVIEW WELLINGTON	DRWG NO.
426688	J7S	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 4 X 22 = 90 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
I - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - C	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.00
F	BMVW+w	MT20	2.0	4.0	
G	BMVH	MT20	3.0	4.0	
H	BMVW-t	MT20	4.0	4.0	
I	BMV1+p	MT20	3.0	4.0	

NOTES- (1)

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	COMBINED	GROSS REACTION	SNOW	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
I	557	0	557	0	0	5-8	5-8		
D	240	0	240	0	0	1-8	1-8		
E	127	0	127	0	0	1-8	1-8		

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) I, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	387	290 / 0	0 / 0	0 / 0	0 / 0	97 / 0	0 / 0
D	165	134 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
E	92	46 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX		WEBS		MAX	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO		LENGTH	FR-TO			
I-B	-543 / 0	0.0	0.0	0.06 (1)	7.81	B-H	0 / 221	0.05 (1)	
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00	C-F	-451 / 0	0.07 (1)	
B-C	-247 / 0	-112.4	-112.4	0.14 (1)	6.25				
C-D	-3 / 4	-112.4	-112.4	0.35 (1)	10.00				
I-H	0 / 0	-18.5	-18.5	0.01 (4)	10.00				
H-G	-114 / 0	0.0	0.0	0.25 (1)	7.81				
G-C	0 / 129	0.0	0.0	0.27 (1)	10.00				
G-F	0 / 345	-18.5	-18.5	0.47 (1)	10.00				
F-E	0 / 0	-18.5	-18.5	0.41 (1)	10.00				

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 32.5 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, ABC 2019

- PART 9 OF OBC 2012 (2019 AMENDMENT)

- CSA 086-14

- TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/ 887 (0.08")

ALLOWABLE DEFL.(TL)= L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/ 466 (0.15")

CS1: TC=0.35/1.00 (C-D:1), BC=0.47/1.00 (F-G:1),

WB=0.07/1.00 (C-F:1), SSI=0.19/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

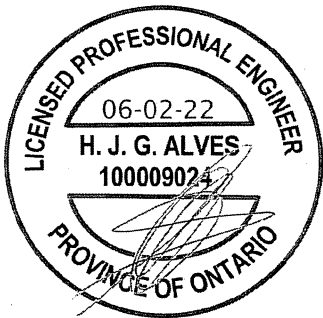
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (F) (INPUT = 0.90)

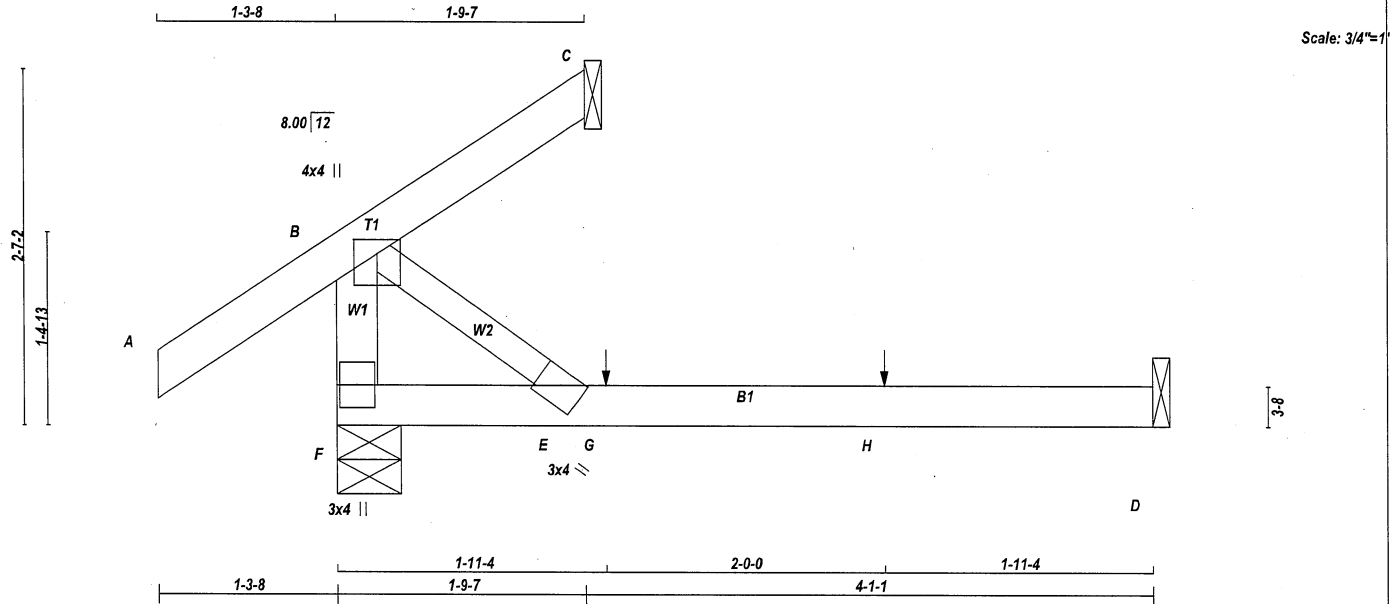
JSI METAL= 0.23 (F) (INPUT = 1.00)



Structural component only

DWG# T-2213229

JOB NAME 426687	TRUSS NAME J10	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Thu Jun 2 09:33:04 2022 Page 1 ID: msZCRjkkNthSDqhrbA0REzSQw2-vZHMUkDOi97b?8rmn9XCZFKnWIBLtwku0K5jKzANRj



TOTAL WEIGHT = 2 X 14 = 27 lb

LUMBER				DESCR.	
N. L. G. A. RULES	SIZE	LUMBER		SPF	
CHORDS					
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
E	BMW+w	MT20	3.0	4.0	2.00 1.25
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT		GROSS REACTION		GROSS REACTION		BRG		BRG	
F	368	0	368	0	0	5-8		5-8	
C	41	0	41	0	0	1-8		1-8	
D	54	0	61	0	0	1-8		1-8	

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS				
F	258	181 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
C	28	24 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
F-B	-314 / 0	0.0	0.0	0.04 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 43	-112.4	-112.4	0.17 (1)	10.00		
B-C	-33 / 0	-112.4	-112.4	0.16 (1)	6.25		
F-E	0 / 0	-18.5	-18.5	0.16 (4)	10.00		
E-G	0 / 0	-18.5	-18.5	0.21 (4)	10.00		
G-H	0 / 0	-18.5	-18.5	0.21 (4)	10.00		
H-D	0 / 0	-18.5	-18.5	0.21 (4)	10.00		

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.17/1.00 (A-B:1), BC=0.21/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

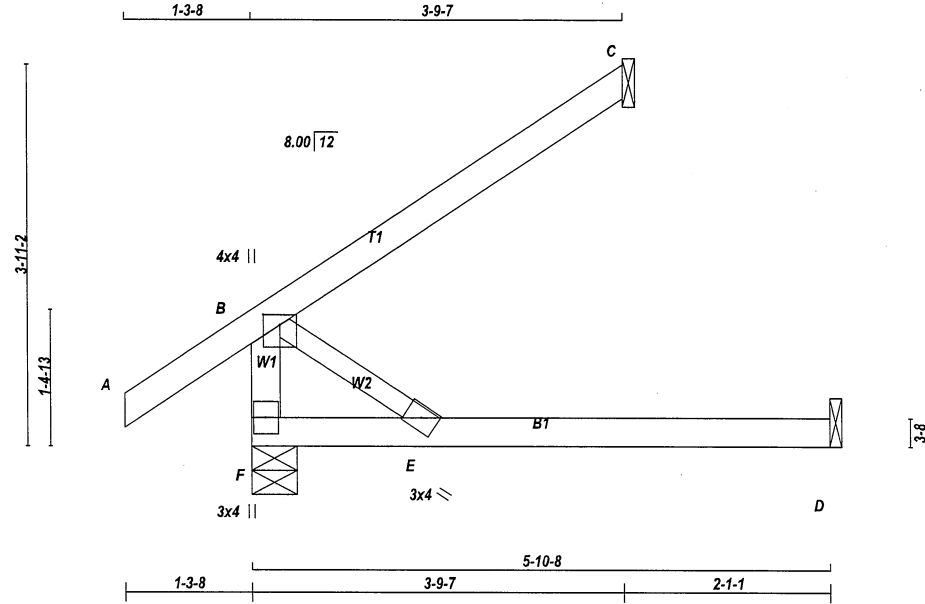


Structural component only
DWG# T-2213194

JOB NAME 426687	TRUSS NAME J11	QUANTITY 2	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 16 = 33 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMVW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	422	0	422	0	0	5-8	5-8
C	213	0	213	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	295	212 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
C	146	123 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
F- B	-367 / 0	0.0 0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)	
A- B	0 / 43	-112.4 -112.4	0.15 (1)	10.00				
B- C	0 / 0	-112.4 -112.4	0.27 (1)	10.00				
F- E	0 / 0	-18.5 -18.5	0.16 (4)	10.00				
E- D	0 / 0	-18.5 -18.5	0.19 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5 PSF
DL =	6.0 PSF	
BOT CH.	LL =	0.0 PSF
DL =	7.4 PSF	
TOTAL LOAD =	45.9 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55% OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CSI: TC=0.27/1.00 (B-C:1), BC=0.19/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

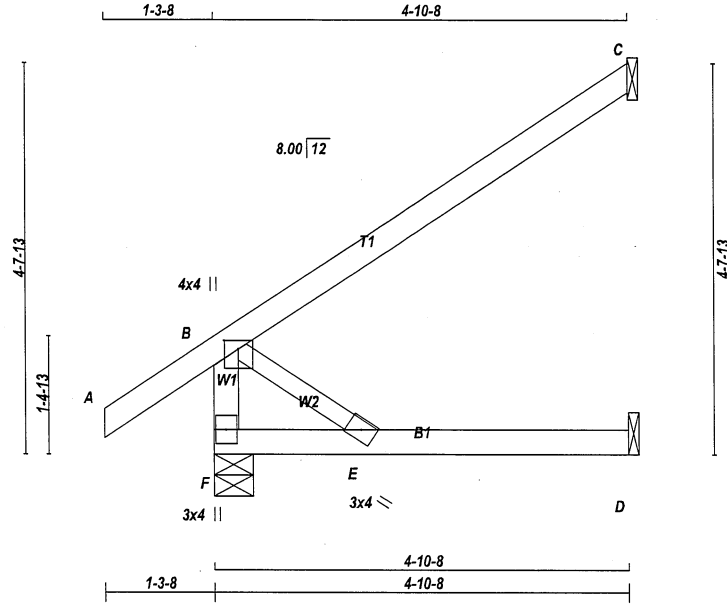
JSI GRIP = 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213195

JOB NAME 426687	TRUSS NAME J12	QUANTITY 6	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:26.2

TOTAL WEIGHT = 6 X 17 = 101 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.25 2.00
E	BMVW+w	MT20	3.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	DOWN	DOWN	BRG	BRG
F	473	0	473	0	5-8
C	274	0	274	0	1-8
D	45	0	51	0	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	329	247 / 0	0 / 0	0 / 0	0 / 0	82 / 0	0 / 0
C	188	158 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
F-B	-428 / 0	F-E	0 / 0
A-B	0 / 43		
B-C	0 / 0		
F-E	0 / 0		
E-D	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.03")

CSI: TC=0.45/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (B-E:1), SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

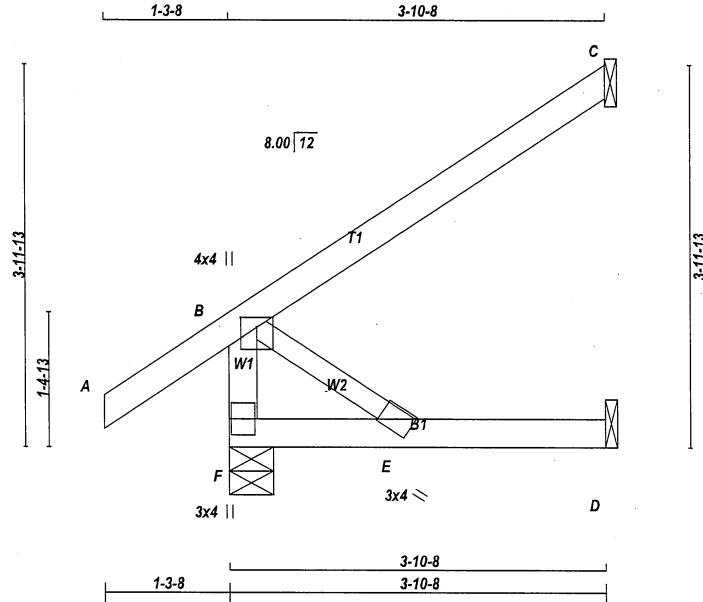
JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213196

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	J13	1	1	BAYVIEW WELLINGTON	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 14 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMV+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

NOTES-

(1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT
JT	VERT	HORIZ	DOWN	HORIZ
F	408	0	408	0
C	218	0	218	0
D	36	0	40	0

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
F	283	215 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	149	126 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
FR-TO							FR-TO						
F-B		-372 / 0		0.0	0.0	0.04 (1)	F-B		0 / 0		0.00 (1)		
A-B		0 / 43		-112.4	-112.4	0.16 (5)	A-B						
B-C		0 / 0		-112.4	-112.4	0.29 (1)	B-C						
F-E		0 / 0		-18.5	-18.5	0.08 (4)	F-E						
E-D		0 / 0		-18.5	-18.5	0.08 (4)	E-D						

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.29/1.00 (B-C:1) , BC=0.09/1.00 (D-E:4) ,
WB=0.00/1.00 (B-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	650	371	1747

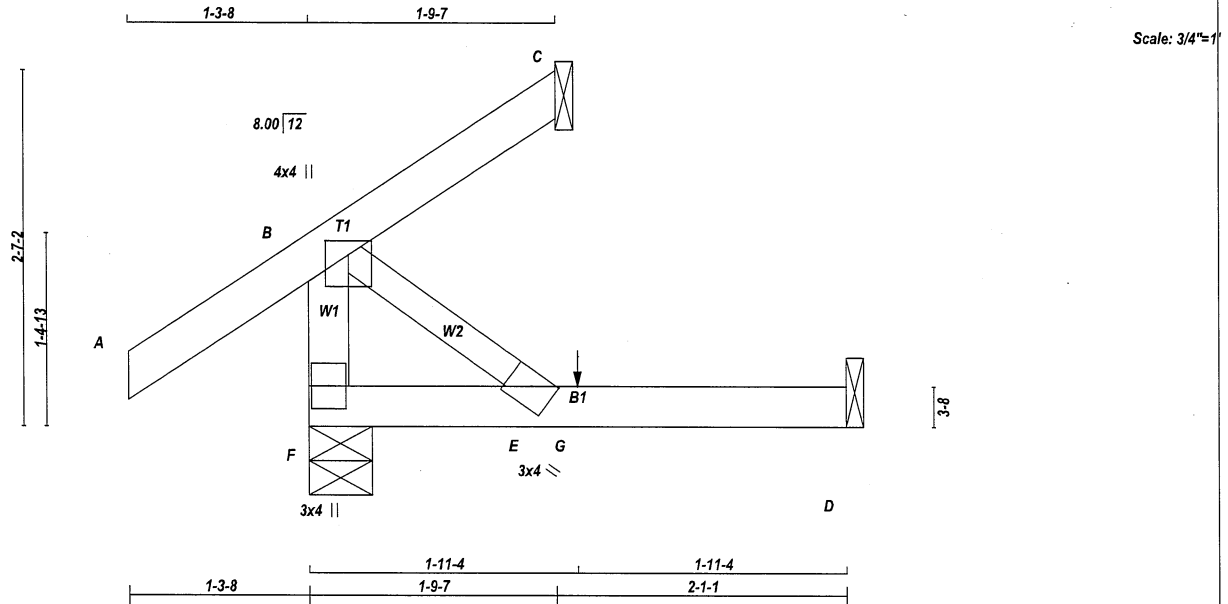
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213197



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
E	BMW+w	MT20	3.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER						
<u>BEARINGS</u>						
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	REQRD
	VERT	HORZ	DOWN	HORZ	BRG IN/IN X	BRG IN/IN X
F	350	0	350	0	5-8	5-8
C	41	0	93	0	-50	1-8
D	36	0	40	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	244	181 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	28	58 / -34	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (11)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM	TO		FR-TO				
F- B	-314 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0	0.00 (1)	
A- B	0 / 43	-112.4	-112.4	0.18 (5)	10.00				
B- C	-33 / 0	-112.4	-112.4	0.17 (5)	6.25				
F- E	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
E- G	0 / 0	-18.5	-18.5	0.09 (4)	10.00				
G- D	0 / 0	-18.5	-18.5	0.09 (4)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONC
G	1-11-4	1	1	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	32.5	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	45.9	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF PART
9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.18/1.00 (A-B:5), BC=0.09/1.00 (D-E:4),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (A-B:5)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

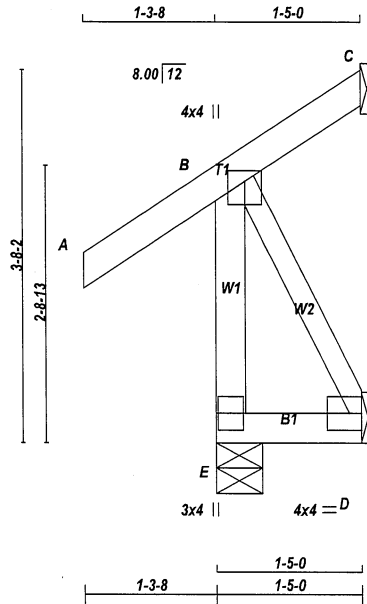


Structural component only
DWG# T-2213198

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
426687	J15	7	1	BAYVIEW WELLINGTON	

Tamarack Roof Truss, Burlington

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Scale = 1:21.7

TOTAL WEIGHT = 7 X 10 = 72 lb

LUMBER					
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	SPF	
CHORDS					
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1+t	MT20	4.0	4.0		Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG	IN-SX
E	321	0	321	0	0	5-8	5-8		
C	5	0	5	0	-67	1-8	1-8		
D	13	0	14	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	221	178 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0
C	3	3 / -43	0 / 0	0 / 0	0 / 0	1 / 0	0 / 0
D	10	0 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-309 / 0	0.0	0.0	0.04 (1)	7.81	B-D	0 / 0
A-B	0 / 43	-112.4	-112.4	0.15 (1)	10.00		0.00 (1)
B-C	-41 / 0	-112.4	-112.4	0.14 (1)	6.25		
E-D	0 / 0	-18.5	-18.5	0.01 (4)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 32.5 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.01/1.00 (D-E:4), WB=0.00/1.00 (B-D:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

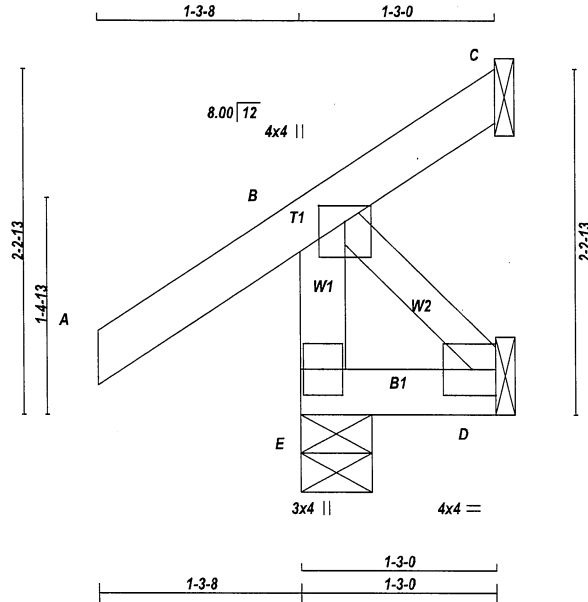
JSI GRIP = 0.21 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213199

JOB NAME 426688	TRUSS NAME J16	QUANTITY 3	PLY 1	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
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Scale = 1:14.2

TOTAL WEIGHT = 3 X 7 = 22 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
D	BMV1-t	MT20	4.0	4.0		Edge
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) NOTE: Lateral braces to be a minimum of 2x4 SPF #2

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
E	320	0	0	320	0	0	0	5-8	5-8
C	-14	0	0	0	0	-78	1-8	1-8	1-8
D	11	0	12	0	0	0	1-8	1-8	1-8

SEE MITTEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED					
E	220	179 / 0	0 / 0	0 / 0	42 / 0	0 / 0
C	-10	0 / -49	0 / 0	0 / 0	0 / -2	0 / 0
D	9	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC) (LC)	MEMB.	FORCE (LBS)	MAX (LC)	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
E-B	-309 / 0	0.0	0.0 0.03 (1)	7.81	B-D	0 / 0	0.00 (1)
A-B	0 / 43	-112.4	-112.4 0.15 (1)	10.00			
B-C	-47 / 0	-112.4	-112.4 0.14 (1)	6.25			
E-D	0 / 0	-18.5	-18.5 0.01 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 32.5 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD	= 45.9 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 096-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 43.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 32.5 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.01/1.00 (D-E:4),
WB=0.00/1.00 (B-D:1), SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-2213230



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering *em and General Safety notes.

T-1300213

Feb 09, 2018

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

NAIL TYPE	LUMBER DIAMETER (in.)	LUMBER THICKNESS (in.)	SPF		D. FIR	
			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
MAXIMUM NUMBER OF TOE NAILS					
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

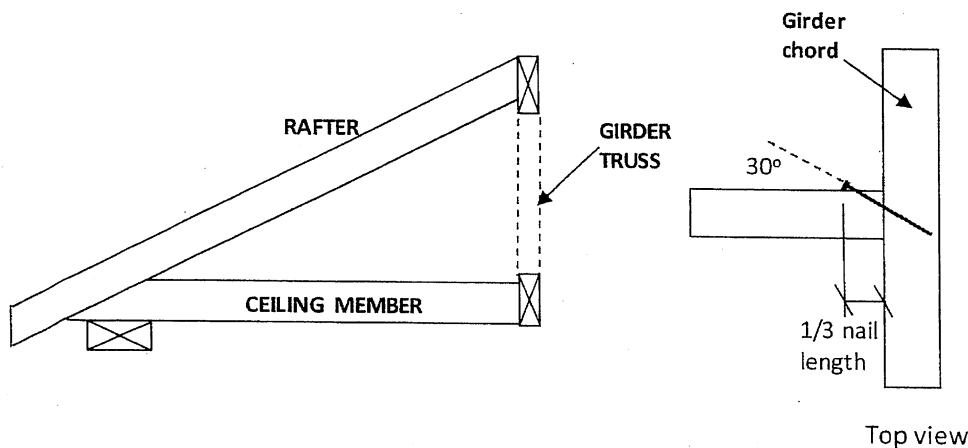


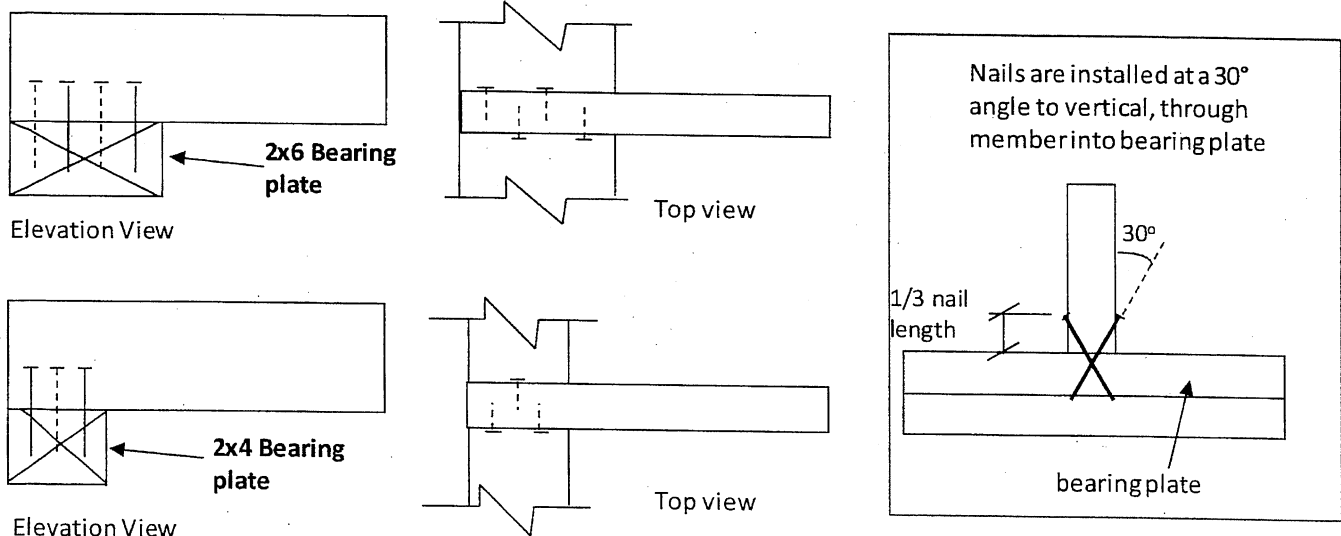
Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails**. Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of $G = 0.42$ (SPF) and $G = 0.49$ (D. Fir).
5. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485



LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 217–218.

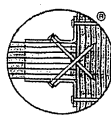
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 18–20.

Installation:

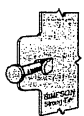
- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

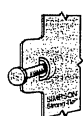
- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on pp. 105–107.



Double-Shear Nailing Top View

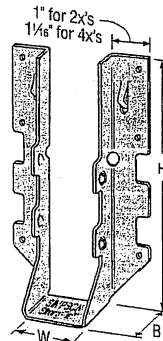
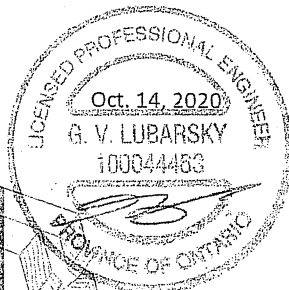
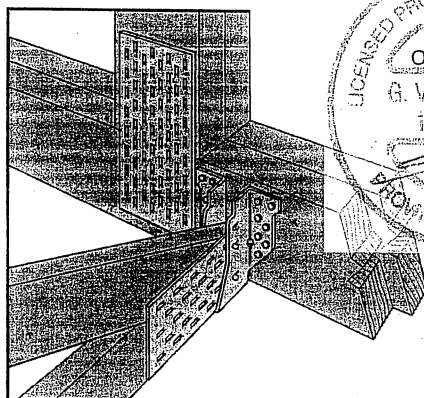


Double-Shear Nailing Side View; Do not bend tab

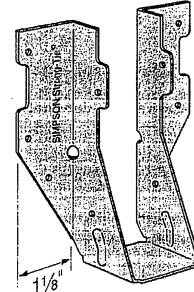


Dome Double-Shear Nailing Side View (available on some models)

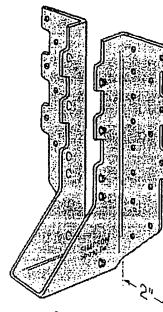
Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



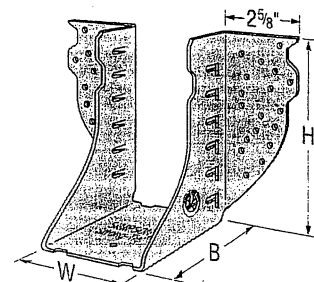
LUS28



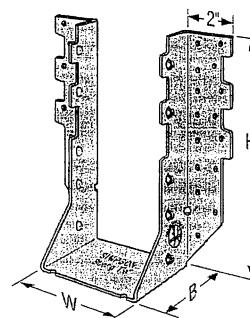
LU26L



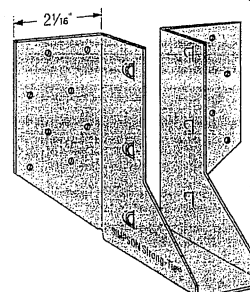
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2



LJS26DS

TECHNICAL BULLETIN

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

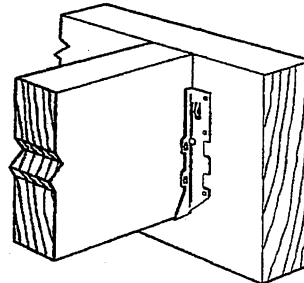
Finish: G90 galvanized

Design:

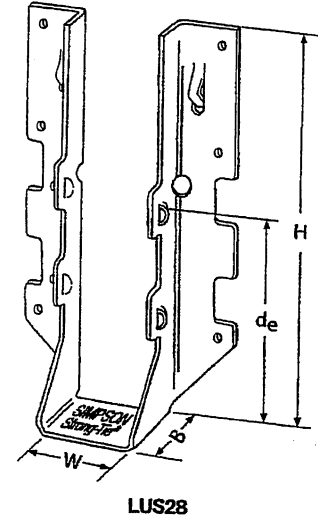
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



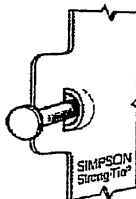
Typical LUS
Installation



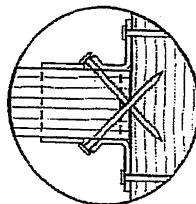
LUS28

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
									(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LUS24	18	1½	3½	1¾	1 15/16	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3½	3½	2	1 15/16	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1½	4¾	1¾	3½	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¾	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1½	6¾	1¾	3¾	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¾	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1½	7 13/16	1¾	3¾	(8) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4¾	8 3/16	2	5¼	(8) 16d	(6) 16d		2580	3345	2320	2375

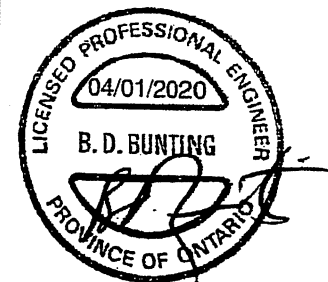
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

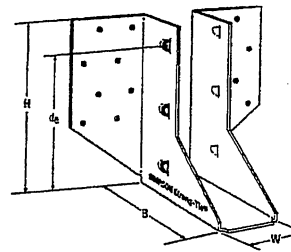
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

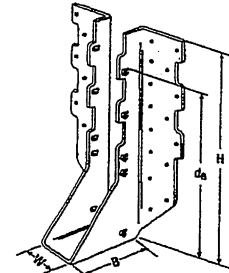
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

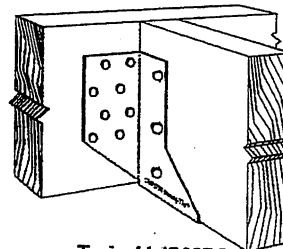
- See current catalogue for options



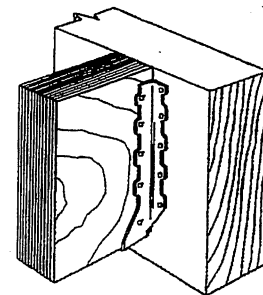
LJS26DS



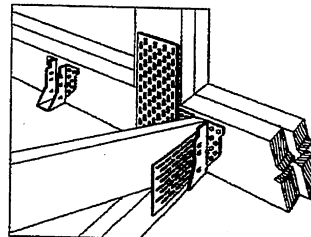
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



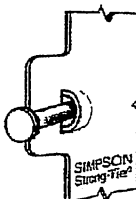
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

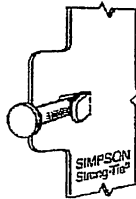
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15) lb.	Normal (K _n =1.00) lb.	Uplift (K _u =1.15) lb.	Normal (K _n =1.00) lb.
LJS26DS	18	1 1/8	5	3 1/2	4 3/8	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 1/8	5 3/8	3	3 1/8	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

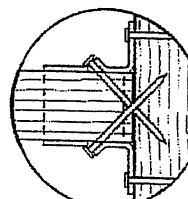


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC HUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

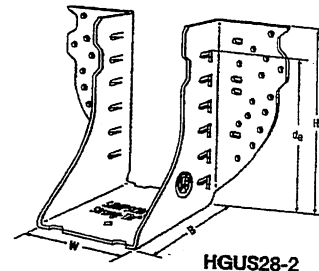
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

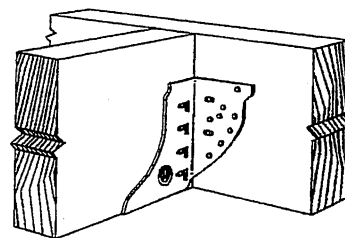
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

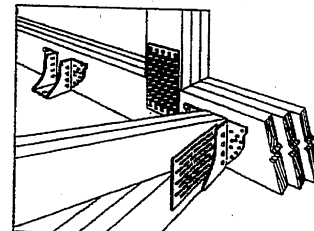
- See current catalogue for options



HGUS28-2



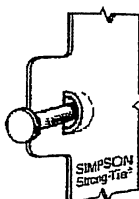
Typical HGUS Installation



Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

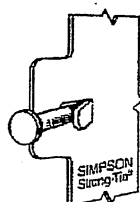
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1½	9½	5	7½	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

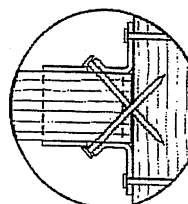


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

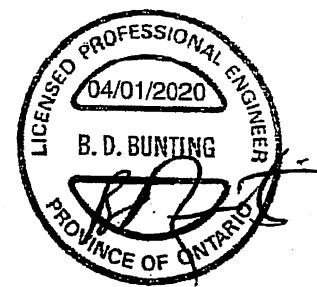
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHGUS20 3/20 exp. 6/22

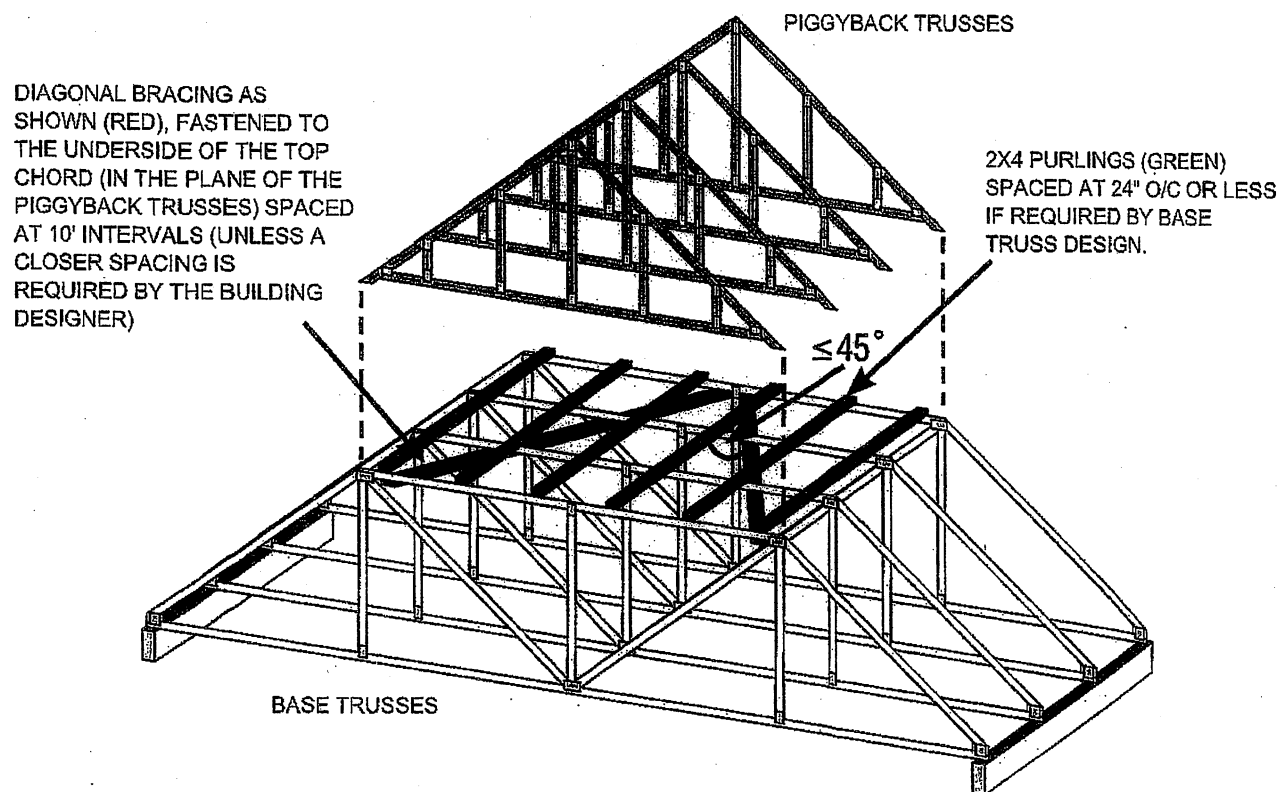
(800) 999-5099
strongtie.com

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

HRS/HST/ST/PS/LSTA/LSTI/MST/MSTA/MSTC/MSTI

Strap Ties

Straps are designed to transfer tension loads in a wide variety of applications.

HRS — Heavy strap designed for installation on the edge of 2x members. The HRS416Z installs with Strong-Drive® SDS Heavy-Duty Connector screws.

LSTA and MSTA — Designed for use on the edge of 2x members, with a nailing pattern that reduces the potential for splitting.

LSTI and MSTI — Light and medium straps that are suitable where pneumatic-nailing is necessary through diaphragm decking and wood chord open-web trusses.

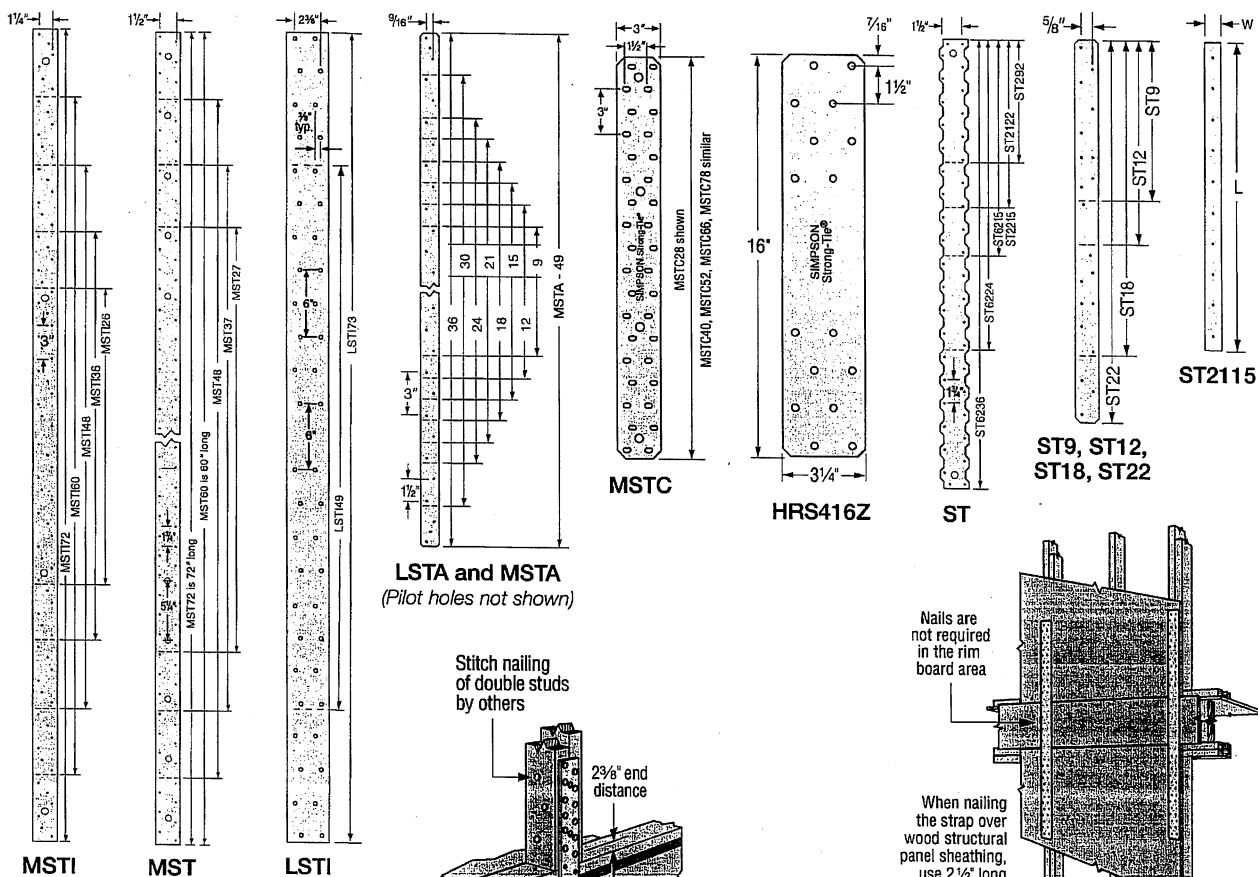
MST — High-capacity strap that can be installed with either nails or bolts. Suitable for double 2x member connections or greater.

MSTC — High-capacity strap that utilizes a staggered nail pattern to help minimize wood splitting. Nail slots have been countersunk to provide a lower nail head profile.

Finish: Galvanized. Some products are available in stainless steel, ZMAX® coating or black powder coat (add PC to sku); contact Simpson Strong-Tie. See Corrosion Information, pp. 18–20.

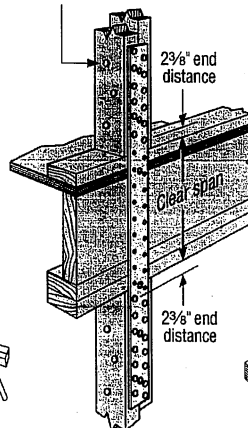
Installation: Use all specified fasteners; see General Notes

Options: Special sizes can be made to order; contact Simpson Strong-Tie for longer lengths

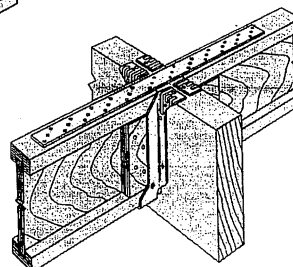


LSTA and MSTA
(Pilot holes not shown)

Stitch nailing
of double studs
by others



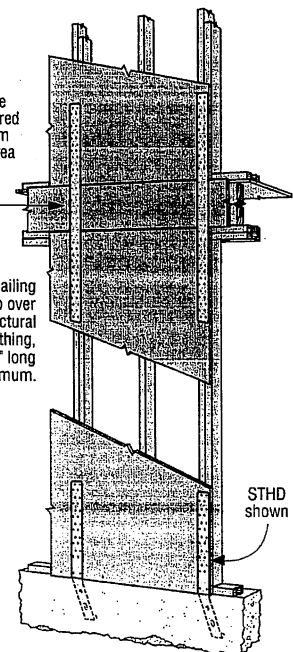
Floor-to-Floor Tie
Installation
Showing a
Clear Span



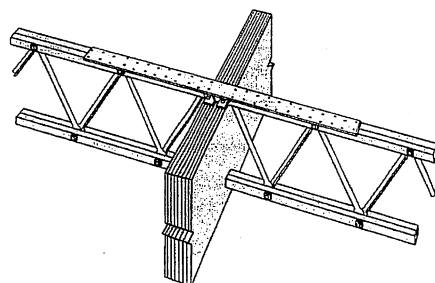
Typical MSTI Installation
(MIT hanger shown)
LSTI similar

Nails are
not required
in the rim
board area

When nailing
the strap over
wood structural
panel sheathing,
use 2 1/2\"/>



Typical Detail with
Strap Installed over
Wood Structural Panel
Sheathing

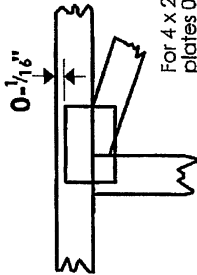


Typical LSTI Installation

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x-y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.



*Plate location details available in Mitek software or upon request.

PLATE SIZE

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

4 x 4

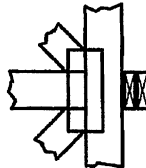
LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.



BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

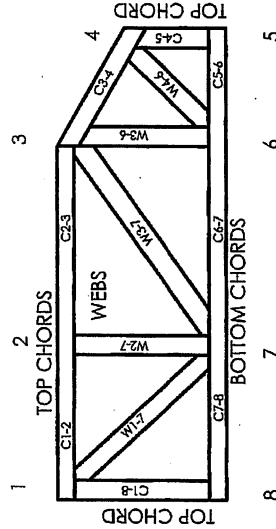


Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.