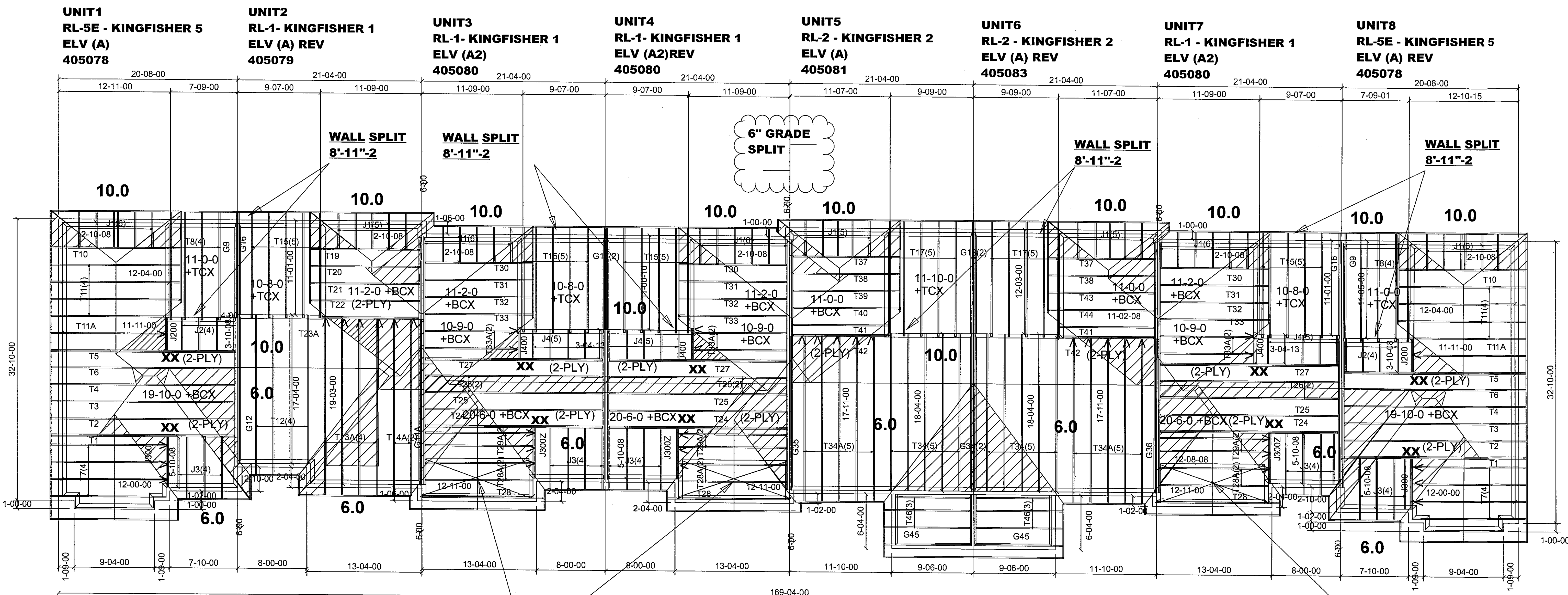


ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HGUS26-2 -(XX)
LUS26-2 -(X)
LJS26DS -(V)

8/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

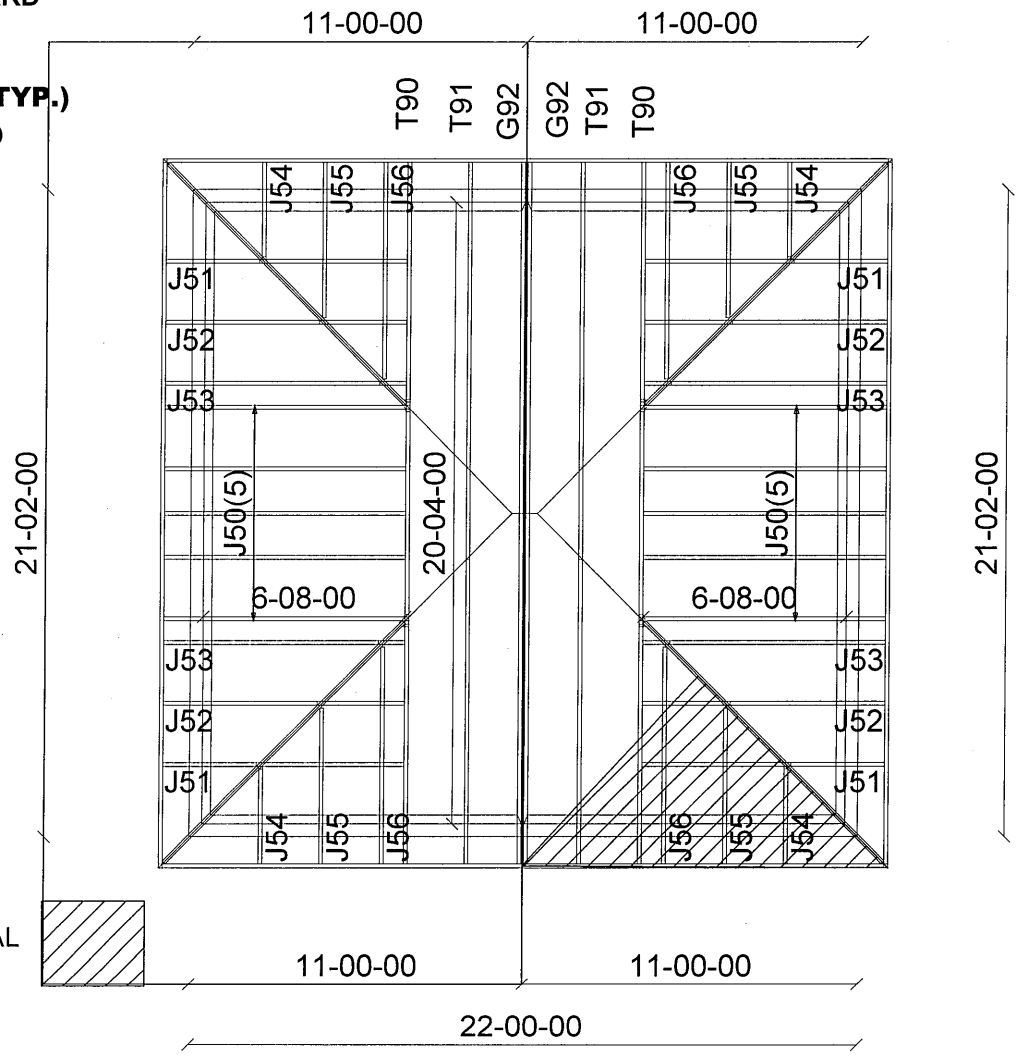
TAMARACK LUMBER INC. / ALPA LUMBER GROUP	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL — Model / Elevation: BLOCK 4 / UNIT1-8 — Mitek ver 8.5.3.233
	Plan Log: 200921	Project: ALCONA SHORES
	Layout ID: 402637	Date: 2023-03-28 — Sales: MiTek — Designer: wassim/s.v

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 2
DG-1
EL:A (REV)
404952

UNIT 1
DG-1
EL:A2
404952

6/12 PITCHES (TYP.)
UNLESS NOTED



DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES CONVENTIONAL FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 4 -DG-1 EL:A UNIT 2 PAIRED WITH DG-1 EL:A2 UNIT 1
	Plan Log: 200921	Project: ALCONA SHORES	
	Layout ID: 405101	Date: 2023-03-28 Sales: MiTek Designer: S.V	

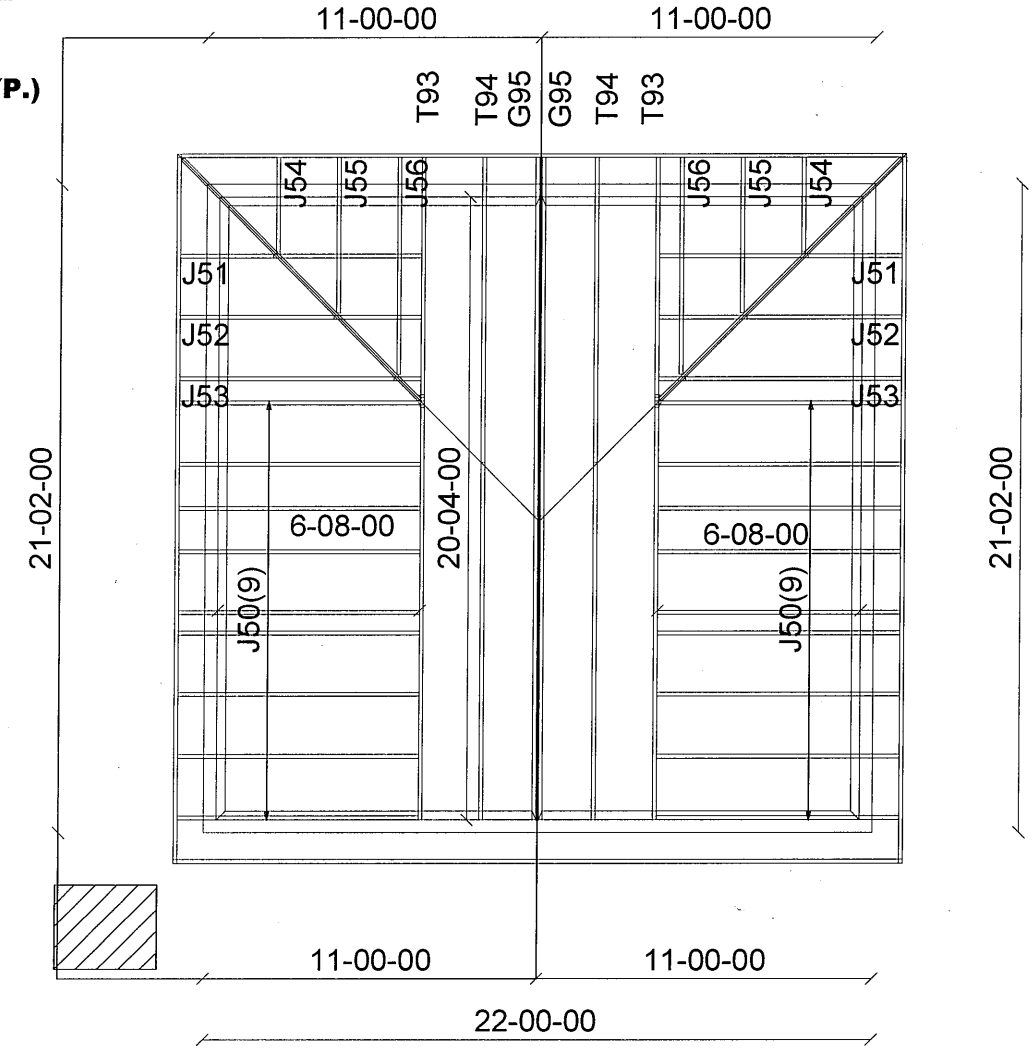
Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 4
DG-1
EL:A3 (REV)
404959

UNIT 3
DG-1
EL:A3
404959

6/12 PITCHES (TYP.)
UNLESS NOTED



DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

DENOTES
CONVENTIONAL
FRAMING

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 4 -DG-1 EL:A3 UNIT 4 PAIRED WITH DG-1 EL:A3 UNIT 3
	Plan Log: 200921	Project: ALCONA SHORES	
	Layout ID: 405102	Date: 2023-03-28 Sales: MiTek Designer: S.V	

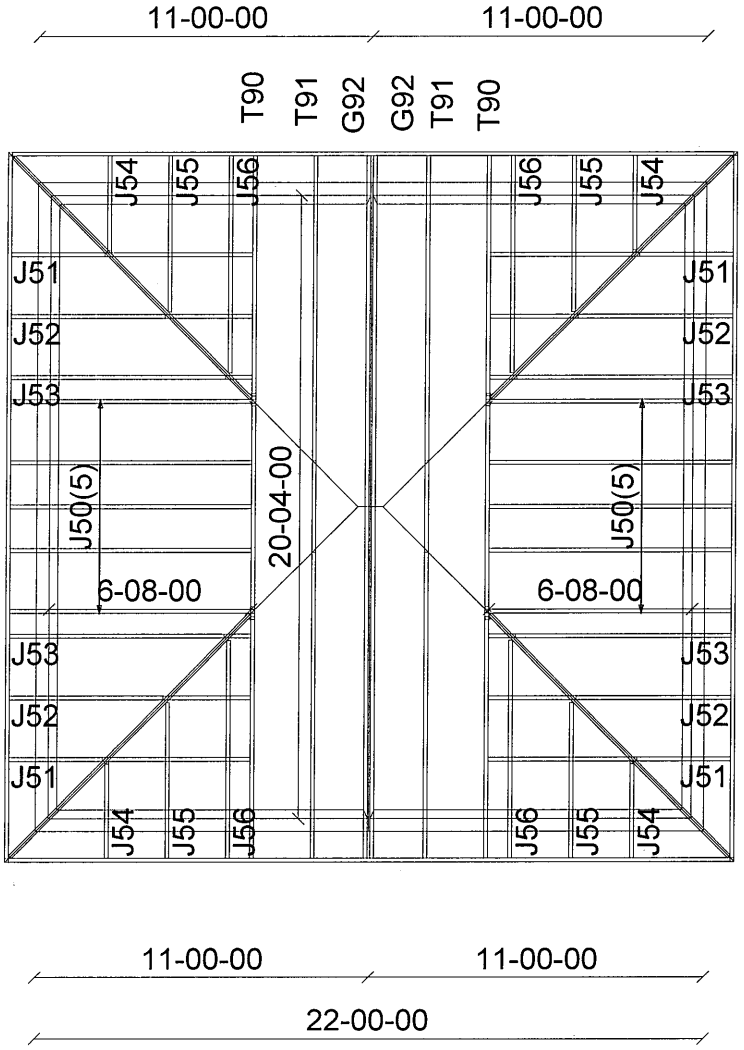
Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

UNIT 6
DG-1
EL:A (REV)
404952

UNIT 5
DG-1
EL:A
404952

6/12 PITCHES (TYP.)
UNLESS NOTED



DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf

All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

DENOTES CONVENTIONAL FRAMING

GARAGE

TAMARACK LUMBER INC.	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 4 -DG-1 EL:A UNIT 6 PAIRED WITH DG-1 EL:A UNIT 5
	Plan Log: 200921	Project: ALCONA SHORES	
	Layout ID: 405100	Date: 2023-03-28 Sales: MiTek Designer: S.V	

Mitek ver 8.5.3.233

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C
2X4 EXTERIOR WALLS
2X6 FASCIA BOARD

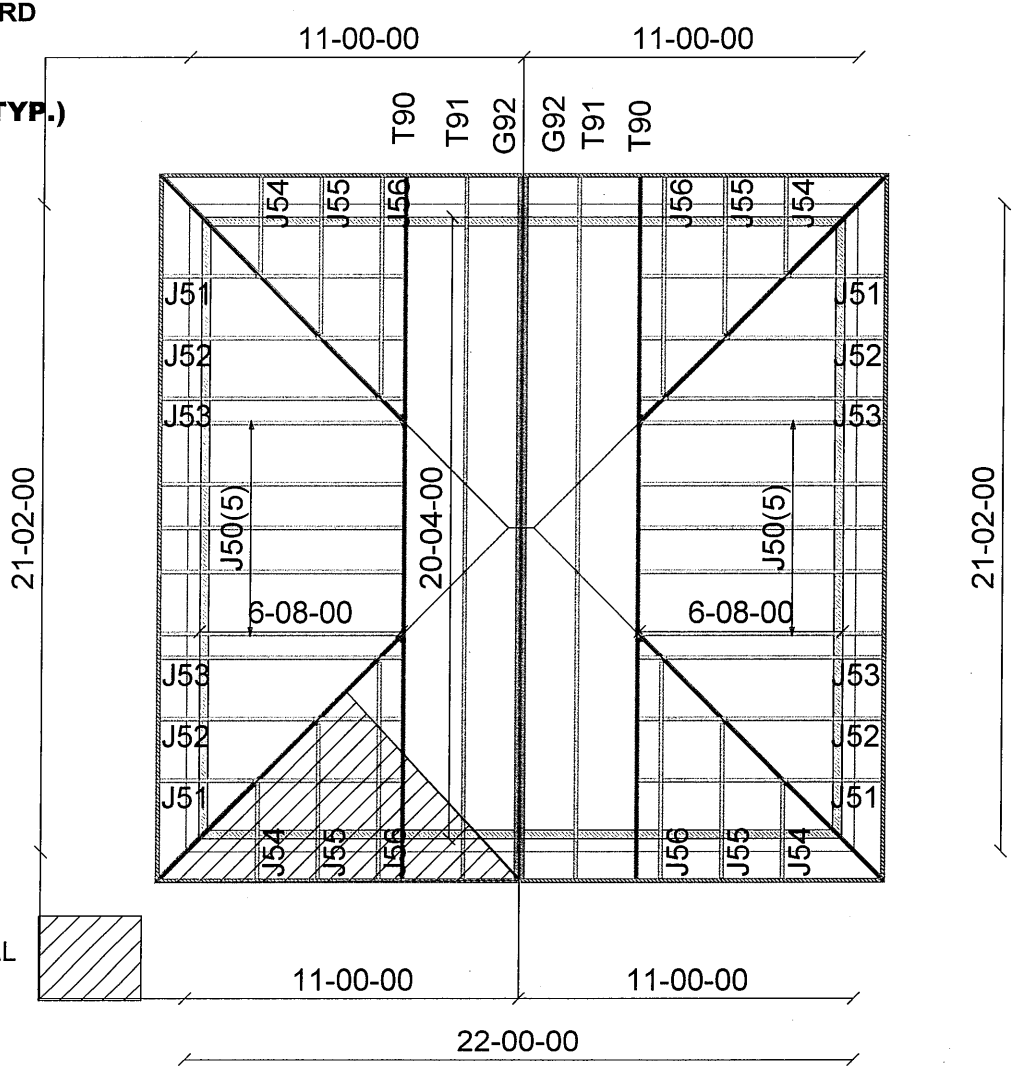
UNIT 8
DG-1
EL:A2 (REV)
404952

UNIT 7
DG-1
EL:A
404952

6/12 PITCHES (TYP.)
UNLESS NOTED

DESIGN CONFORMS WITH OBC 2012(2019 AMENDMENT)
OCCUPANCY: RESIDENTIAL | PART: 9
Ss = 54.3 psf | Sr = 8.4 psf

DESIGN LOADS:
TCSL = 38.2 psf
TCDL = 6.0 psf
BCLL = 0.0 psf
BCDL = 7.4 psf



All conventional framing to conform with Part 9 of O.B.C. 2012. Roof rafters that cross over or meet trusses to be min. 2x4 SPF #2 @ 24" o/c with a vertical post to the truss at each cross point. Vertical posts longer than 6' to have lateral bracing so that the distance between the post end points and lateral bracing does not exceed 6'.

GARAGE

TAMARACK LUMBER INC.	Job Track: 50469	Builder / Location: BAYVIEW WELLINGTON / INNISFIL	Model / Elevation: BLOCK 4 -DG-1 EL:A2 UNIT 8 PAIRED WITH DG-1 EL:A UNIT 7
	Plan Log: 200921	Project: ALCONA SHORES	
	Layout ID: 405098	Date: 2023-03-28 Sales: MiTek Designer: S.V	

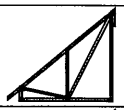
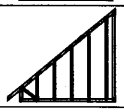
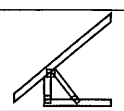
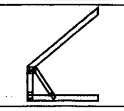
Mitek ver 8.5.3.233

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405078
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A - UNIT1 OR UNIT8	Designer:	
		Sales Rep:	MiTek

Roof Trusses

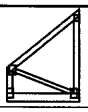
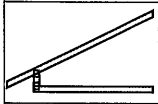
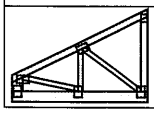
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	8 /12	**19-10-00**	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 1-08-02	192.47 127.33		
	1	T2 Hip	8 /12	**19-10-00**	5-01-04	2 x 4	1-03-08	1-04-13 1-08-02	86.29 56.50		
	1	T3 Hip	8 /12	**19-10-00**	6-01-04	2 x 4	1-03-08	1-04-13 1-08-02	87.84 57.67		
	1	T4 Hip	8 /12	**19-10-00**	7-01-04	2 x 4	1-03-08	1-04-13 1-08-02	92.54 60.17		
	1 2-ply	T5 Half Hip Girder	8 /12	**19-10-00**	5-01-05	2 x 4 2 x 6	1-03-08	1-04-13 5-01-05	200.79 130.00		
	1	T6 Hip	8 /12	**19-10-00**	6-09-05	2 x 4	1-03-08	1-04-13 1-08-02	90.78 59.67		
	4	T7 Common	8 /12	**12-00-00**	5-04-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	212 127.33		
	4	T8 Monopitch	10 /12	**11-03-08**	11-00-10	2 x 4	1-03-08	1-07-11 11-00-10	272.5 170.67		
	1	G9 GABLE	10 /12	**11-00-00**	10-09-11	2 x 4	1-03-08	1-07-11 10-09-11	70.31 45.33		
	1	T10 Hip Girder	8 /12	**12-04-00**	4-00-07	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	52.2 33.67		
	4	T11 Common	8 /12	**12-04-00**	5-06-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	217 139.33		
	1	T11A Common	8 /12	**11-11-00**	5-06-02	2 x 4	1-03-08	1-04-13 1-04-13	51.52 33.50		
	6	J1 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	75.07 50.00		
	4	J2 Jack-Open	10 /12	**3-10-08**	5-01-05	2 x 4		1-10-09 5-01-05	54.61 36.00		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405078
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A - UNIT1 OR UNIT8	Designer:	
		Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	J200 Jack-Closed Girder	10 /12	3-10-08	5-01-05	2 x 4 2 x 6		1-10-09 5-01-05	44.37 29.33		
	4	J3 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	1 2-ply	J300 Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		

TOTAL # TRUSS= **41** TOTAL BFT OF ALL TRUSSES= **1236.84** BFT. TOTAL WEIGHT OF ALL TRSSES **1925.87** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

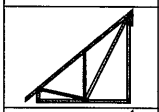
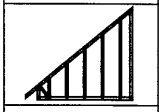
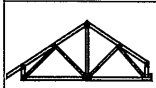
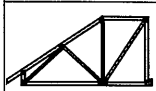
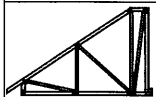
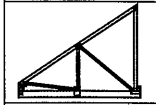
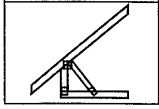
TOTAL NUMBER OF ITEMS= **6**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405079
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A -UNIT2	Designer:	
		Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	4	T12 Roof Special	6 /12	**17-04-00**	6-08-15	2 x 4	1-03-08	1-02-00 1-07-04	288.19 188.00		
	1	G12 GABLE	6 /12	**17-04-00**	6-08-15	2 x 4	1-03-08	1-02-00 1-07-04	72 48.00		
	4	T13A Roof Special	6 /12	**19-03-00**	6-08-15	2 x 4	1-03-08	1-02-00 1-11-06	325.39 208.67		
	2	T14A Half Hip	6 /12	**19-03-00**	6-11-02	2 x 4	1-03-08	1-02-00 6-11-02	180.33 115.00		
	1	G14A GABLE	6 /12	**19-03-00**	6-11-02	2 x 4	1-03-08	1-02-00 6-11-02	92.05 59.00		
	5	T15 Monopitch	10 /12	**10-11-08**	10-09-04	2 x 4	1-03-08	1-07-11 10-09-04	330.02 206.67		
	1	G16 GABLE	10 /12	**10-08-00**	10-06-06	2 x 4	1-03-08	1-07-11 10-06-06	66.47 42.00		
	1	T19 Hip Girder	8 /12	**11-02-00**	4-00-07	2 x 4	1-03-08	1-04-13 1-06-08	47.96 31.83		
	1	T20 Common	8 /12	**11-02-00**	5-03-02	2 x 4	1-03-08	1-04-13 1-06-08	49.38 31.83		
	1	T21 Half Hip	8 /12	**11-02-00**	6-01-07	2 x 4	1-03-08	1-04-13 6-01-07	54.57 35.50		
	1	T22 Half Hip	8 /12	**11-02-00**	7-09-07	2 x 4	1-03-08	1-04-13 7-09-07	63.51 41.83		
	1 2-ply	T23A Monopitch Girder	8 /12	**11-03-00**	8-10-02	2 x 4 2 x 8		1-04-13 8-10-13	133.09 83.33		
	5	J1 Jack-Open	10 /12	**2-10-08**	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TOTAL # TRUSS= **29** TOTAL BFT OF ALL TRUSSES= **1133.33** BFT. TOTAL WEIGHT OF ALL TRSSES **1765.5** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	ALCONA SHORES
Location:	INNISFIL
Model:	BLOCK 4
Lot #:	
Elevation:	EL:A -UNIT2

Job Track:	50469
PlanLog:	200921
Layout ID:	405079
Ref #	
Page:	2 of 2
Date:	03-28-2023
Designer:	
Sales Rep:	MiTek

HARDWARE

QTY	TYPE	MODEL	LENGTH
6	Hardware	LJS26DS	

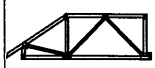
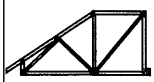
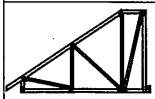
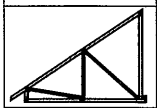
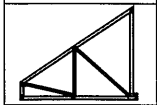
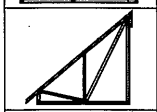
TOTAL NUMBER OF ITEMS= **6**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405080
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A2 -UNIT3, UNIT4 OR UNIT7	Designer:	
		Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T24 Flat Girder	0 /12	20-06-00	4-01-04	2 x 4 2 x 6		4-01-04 4-01-04	234.57 152.00		
	1	T25 Flat	0 /12	20-06-00	5-01-04	2 x 4		5-01-04 5-01-04	91.36 58.17		
	2	T26 Flat	0 /12	20-06-00	6-01-04	2 x 4		6-01-04 6-01-04	196.85 126.00		
	1 2-ply	T27 Flat Girder	0 /12	20-06-00	4-05-04	2 x 4 2 x 6		4-05-04 4-05-04	222.56 140.00		
	1	T28 Scissor	8 /12 4 /12	12-11-00	5-08-08	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	56.12 37.67		
	2	T28A Scissor	8 /12 4 /12	12-06-00	5-08-08	2 x 4	1-03-08	1-04-13 1-04-13	106.7 71.67		
	2	T29A Common	8 /12	12-06-00	5-08-08	2 x 4		1-04-13 1-08-02	105.57 69.00		
	1	T30 Half Hip Girder	8 /12	11-02-00	4-00-07	2 x 4	1-03-08	1-04-13 4-00-07	47.99 31.00		
	1	T31 Half Hip	8 /12	11-02-00	5-08-07	2 x 4	1-03-08	1-04-13 5-08-07	53.22 34.83		
	1	T32 Half Hip	8 /12	11-02-00	7-04-07	2 x 4	1-03-08	1-04-13 7-04-07	61.78 39.17		
	1	T33 Monopitch	8 /12	11-02-00	8-10-02	2 x 4	1-03-08	1-04-13 8-10-02	54.95 35.00		
	2	T33A Monopitch	8 /12	10-09-00	8-10-02	2 x 4		1-04-13 8-10-02	104.33 67.67		
	5	T15 Monopitch	10 /12	10-11-08	10-09-04	2 x 4	1-03-08	1-07-11 10-09-04	330.02 206.67		
	1	G16 GABLE	10 /12	10-08-00	10-06-06	2 x 4	1-03-08	1-07-11 10-06-06	66.47 42.00		

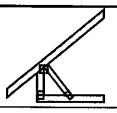
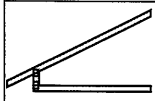
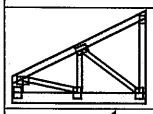
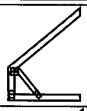
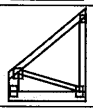
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	ALCONA SHORES
Location:	INNISFIL
Model:	BLOCK 4
Lot #:	
Elevation:	EL:A2 -UNIT3, UNIT4 OR UNIT7

Job Track:	50469
PlanLog:	200921
Layout ID:	405080
Ref #	
Page:	2 of 2
Date:	03-28-2023
Designer:	
Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	6	J1 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	75.07 50.00		
	4	J3 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	67.18 42.67		
	1 2-ply	J300Z Jack-Closed Girder	6 /12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.39 37.67		
	5	J4 Jack-Open	10 /12	3-04-13	4-05-04	2 x 4		1-07-04 4-05-04	59.54 41.67		
	1 2-ply	J400 Jack-Closed Girder	10 /12	3-07-13	4-05-04	2 x 4 2 x 6		1-04-12 4-05-04	41.01 26.00		

TOTAL # TRUSS= **43** TOTAL BFT OF ALL TRUSSES= **1308.86** BFT. TOTAL WEIGHT OF ALL TRSSES **2033.67** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
5	Hardware	LJS26DS	

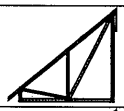
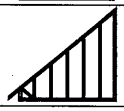
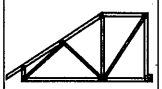
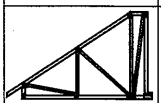
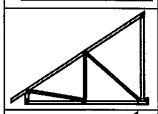
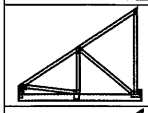
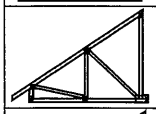
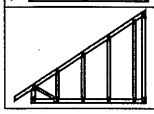
TOTAL NUMBER OF ITEMS= **7**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405081
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A- UNIT5	Designer:	
		Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	T34 Roof Special	6 /12	18-04-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	392.12 249.17		
	1	G34 GABLE	6 /12	18-04-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.97 52.83		
	5	T34A Roof Special	6 /12	17-11-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	387.8 243.33		
	1	G35 GABLE	6 /12	17-11-00	8-09-02	2 x 4		1-02-00 8-09-02	91.66 60.83		
	5	T17 Monopitch	10 /12	12-01-08	11-08-15	2 x 4	1-03-08	1-07-11 11-08-15	367.16 225.83		
	1	G18 GABLE	10 /12	11-10-00	11-06-00	2 x 4	1-03-08	1-07-11 11-06-00	72.61 45.67		
	1	T37 Hip Girder	8 /12	11-00-00	4-00-07	2 x 4	1-03-08	1-04-13 1-08-02	47.68 31.83		
	1	T38 Common	8 /12	11-00-00	5-02-08	2 x 4	1-03-08	1-04-13 1-08-02	48.94 31.83		
	1	T39 Half Hip	8 /12	11-00-00	6-00-07	2 x 4	1-03-08	1-04-13 6-00-07	53.99 35.50		
	1	T40 Half Hip	8 /12	11-00-00	7-08-07	2 x 4	1-03-08	1-04-13 7-08-07	62.91 41.83		
	1	T41 Monopitch	8 /12	11-00-00	8-08-13	2 x 4	1-03-08	1-04-13 8-08-13	54.21 34.33		
	1 2-ply	T42 Monopitch Girder	8 /12	11-00-00	8-08-13	2 x 4 2 x 8		1-04-13 8-08-13	142.02 88.00		
	3	T46 Monopitch	8 /12	8-08-00	7-02-02	2 x 4	1-03-08	1-04-13 7-02-02	131.65 86.50		
	1	G45 GABLE	8 /12	8-10-08	7-03-13	2 x 4	1-03-08	1-04-13 7-03-13	48.05 30.50		

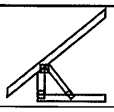
TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER
Builder:	BAYVIEW WELLINGTON
Project:	ALCONA SHORES
Location:	INNISFIL
Model:	BLOCK 4
Lot #:	
Elevation:	EL:A- UNIT5

Job Track:	50469
PlanLog:	200921
Layout ID:	405081
Ref #	
Page:	2 of 2
Date:	03-28-2023
Designer:	
Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J1 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TOTAL # TRUSS= **34** TOTAL BFT OF ALL TRUSSES= **1299.65** BFT. TOTAL WEIGHT OF ALL TRSSES **2046.33** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	

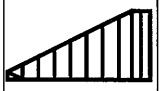
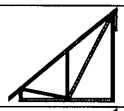
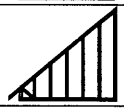
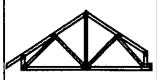
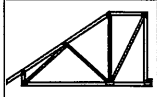
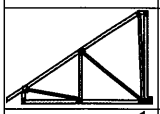
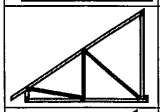
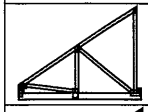
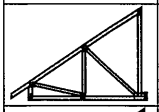
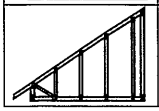
TOTAL NUMBER OF ITEMS= **5**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405083
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	1 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A -UNIT6	Designer:	
		Sales Rep:	MiTek

Roof Trusses

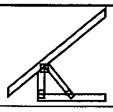
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	5	T34 Roof Special	6 /12	18-04-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	392.12 249.17		
	1	G34 GABLE	6 /12	18-04-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	82.97 52.83		
	5	T34A Roof Special	6 /12	17-11-00	7-05-01	2 x 4	1-03-08	1-02-00 2-06-14	387.8 243.33		
	1	G36 GABLE	6 /12	17-11-00	8-10-13	2 x 4		1-02-00 8-10-13	94.59 60.17		
	5	T17 Monopitch	10 /12	12-01-08	11-08-15	2 x 4	1-03-08	1-07-11 11-08-15	367.16 225.83		
	1	G18 GABLE	10 /12	11-10-00	11-06-00	2 x 4	1-03-08	1-07-11 11-06-00	72.61 45.67		
	1	T37 Hip Girder	8 /12	11-00-00	4-00-07	2 x 4	1-03-08	1-04-13 1-08-02	47.68 31.83		
	1	T38 Common	8 /12	11-00-00	5-02-08	2 x 4	1-03-08	1-04-13 1-08-02	48.94 31.83		
	1	T43 Half Hip	8 /12	11-00-00	6-06-07	2 x 4	1-03-08	1-04-13 6-06-07	55.93 36.00		
	1	T44 Half Hip	8 /12	11-00-00	8-02-07	2 x 4	1-03-08	1-04-13 8-02-07	59.26 37.83		
	1	T41 Monopitch	8 /12	11-00-00	8-08-13	2 x 4	1-03-08	1-04-13 8-08-13	54.21 34.33		
	1 2-ply	T42 Monopitch Girder	8 /12	11-00-00	8-08-13	2 x 4 2 x 8		1-04-13 8-08-13	142.02 88.00		
	3	T46 Monopitch	8 /12	8-08-00	7-02-02	2 x 4	1-03-08	1-04-13 7-02-02	131.65 86.50		
	1	G45 GABLE	8 /12	8-10-08	7-03-13	2 x 4	1-03-08	1-04-13 7-03-13	48.05 30.50		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200921
Project:	ALCONA SHORES	Layout ID:	405083
Location:	INNISFIL	Ref #	
Model:	BLOCK 4	Page:	2 of 2
Lot #:		Date:	03-28-2023
Elevation:	EL:A -UNIT6	Designer:	
		Sales Rep:	MiTek

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J1 Jack-Open	10 /12	2-10-08	4-00-07	2 x 4	1-03-08	1-07-11 4-00-07	62.56 41.67		

TOTAL # TRUSS= **34** TOTAL BFT OF ALL TRUSSES= **1295.49** BFT. TOTAL WEIGHT OF ALL TRSSES **2047.55** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LJS26DS	

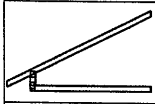
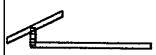
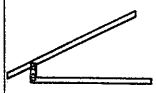
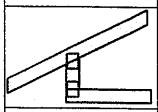
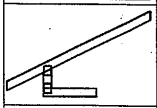
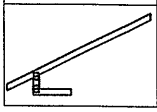
TOTAL NUMBER OF ITEMS= **5**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:A UNIT2 OR EL:A2 UNIT1	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	**20-04-00**	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	**20-04-00**	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	**20-04-00**	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	**6-08-00**	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	**3-09-07**	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	**5-09-07**	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	**1-09-07**	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	**1-10-08**	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	**1-10-08**	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

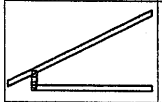
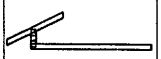
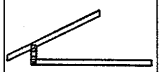
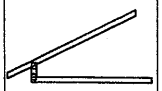
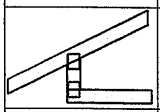
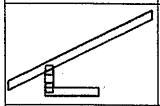
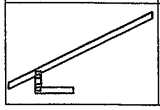
TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404959
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:A3 UNIT4 OR EL:A3 UNIT3	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T93 Half Hip Girder	6 /12	20-04-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	84.42 52.33		
	1	T94 Half Hip	6 /12	20-04-00	5-06-00	2 x 4	1-03-08	1-02-00 5-06-00	85.62 53.33		
	1	G95 GABLE	6 /12	20-04-00	6-04-08	2 x 4	1-03-08	1-02-00 6-04-08	95.24 59.67		
	9	J50 Jack-Open	6 /12	6-08-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	168.36 108.00		
	1	J51 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	12.48 8.00		
	1	J52 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	15.03 9.33		
	1	J53 Jack-Open	6 /12	5-09-07	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	17.58 11.33		
	1	J54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	7.02 4.67		
	1	J55 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	9.57 6.00		
	1	J56 Jack-Open	6 /12	1-10-08	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	12.12 8.00		

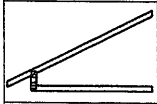
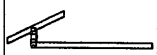
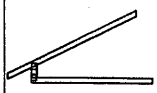
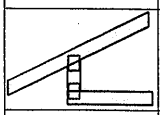
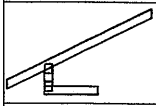
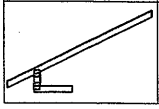
TOTAL # TRUSS= **18** TOTAL BFT OF ALL TRUSSES= **320.66** BFT. TOTAL WEIGHT OF ALL TRSSES **507.45** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:A UNIT6 OR EL:A UNIT5	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	20-04-00	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	20-04-00	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	20-04-00	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	6-08-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	5-09-07	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	1-10-08	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

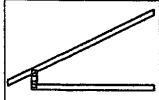
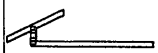
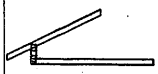
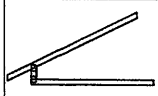
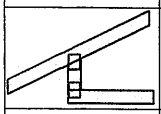
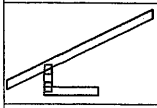
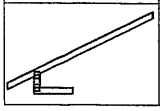
TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50469
Builder:	BAYVIEW WELLINGTON	PlanLog:	200893
Project:	ALCONA SHORES	Layout ID:	404952
Location:	INNISFIL	Ref #	
Model:	DG-1	Page:	1 of 1
Lot #:		Date:	08/20/2019
Elevation:	EL:A2 UNIT8 OR EL:A UNIT7	Designer:	
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T90 Hip Girder	6 /12	20-04-00	4-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	80.86 51.33		
	1	T91 Hip	6 /12	20-04-00	5-06-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	86.18 54.00		
	1	G92 GABLE	6 /12	20-04-00	6-03-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	79.66 49.67		
	5	J50 Jack-Open	6 /12	6-08-00	4-06-00	2 x 4	1-03-08	1-02-00 4-06-00	93.53 60.00		
	2	J51 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-10-09	1-02-00 2-00-12	24.97 16.00		
	2	J52 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-10-09	1-02-00 3-00-12	30.07 18.67		
	2	J53 Jack-Open	6 /12	5-09-07	4-00-12	2 x 4	1-03-08 10-09	1-02-00 4-00-12	35.17 22.67		
	2	J54 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J55 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J56 Jack-Open	6 /12	1-10-08	4-00-12	2 x 4	1-03-08 3-10-15	1-02-00 2-01-04	24.24 16.00		

TOTAL # TRUSS= **20** TOTAL BFT OF ALL TRUSSES= **309.67** BFT. TOTAL WEIGHT OF ALL TRSSES **487.86** LBS

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T1	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.530 S Feb 23 2022 MiTek Industries, Inc. Mon Apr 3 10:07:49 2023 Page 1

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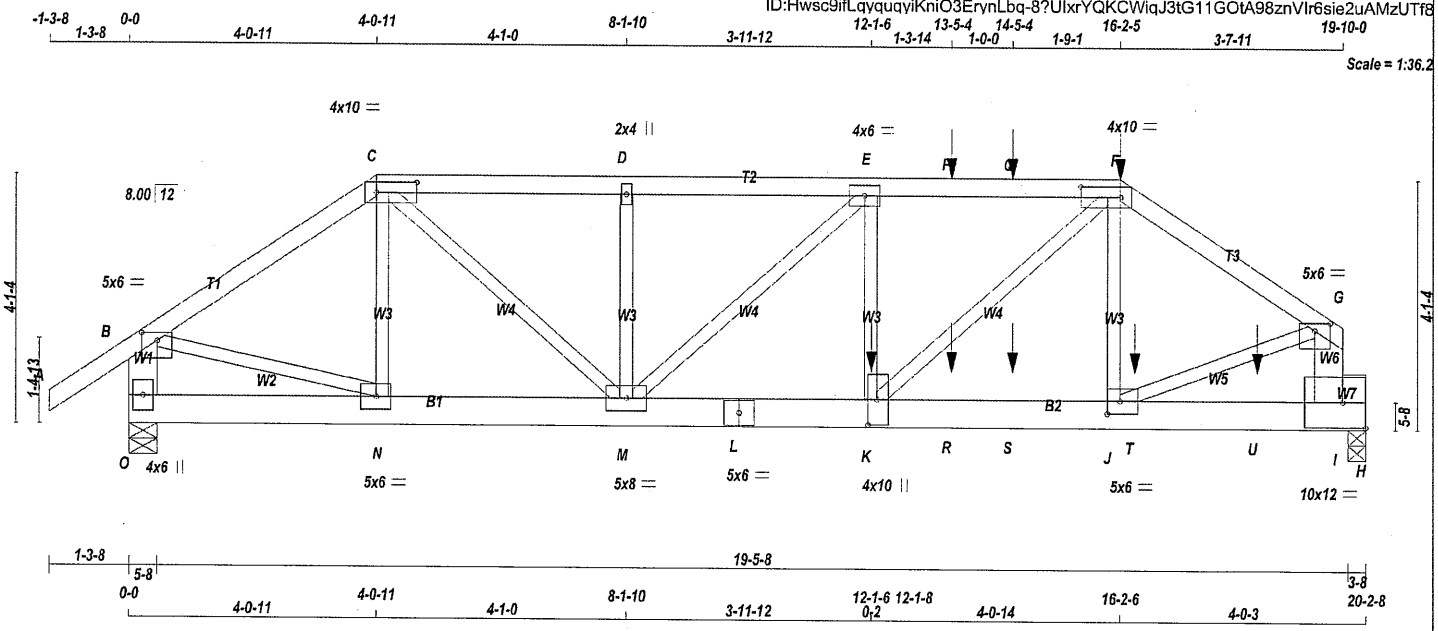

TOTAL WEIGHT = 4 X 96 = 385 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
O - B	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- F	1	12	SIDE(61.0)
F- G	1	12	SIDE(61.0)
O- B	2	12	TOP
I- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O- L	2	12	TOP
L- H	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
J- F	1	6	SIDE(26.1)
E- K	1	4	SIDE(433.8)
2x3	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040217

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2855	0	2855	0	0	5-8	5-8
H	3618	0	3618	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1999	1425 / 0	0 / 0	0 / 0	0 / 0	574 / 0	0 / 0
H	2540	1773 / 0	0 / 0	0 / 0	0 / 0	768 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00	N- C	-535 / 0	0.07 (1)
B- C	-3149 / 0	-129.6	-129.6	0.25 (1)	4.96	C- M	0 / 2648	0.33 (1)
C- D	-4559 / 0	-129.6	-129.6	0.23 (1)	4.29	M- D	-512 / 0	0.06 (1)
D- E	-4559 / 0	-129.6	-129.6	0.30 (1)	4.21	M- E	-1674 / 0	0.41 (1)
E- P	-5780 / 0	-129.6	-129.6	0.44 (1)	3.63	K- E	0 / 370	0.06 (4)
P- Q	-5780 / 0	-129.6	-129.6	0.44 (1)	3.63	K- F	0 / 2768	0.34 (1)
Q- F	-5780 / 0	-129.6	-129.6	0.44 (1)	3.63	J- F	-509 / 0	0.06 (1)
F- G	-4512 / 0	-129.6	-129.6	0.25 (1)	4.30	B- N	0 / 2690	0.33 (1)
O- B	-2821 / 0	0.0	0.0	0.10 (1)	7.81	J- G	0 / 3959	0.49 (1)
I- G	-4025 / 0	0.0	0.0	0.15 (1)	7.09			
O- N	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 2601	-18.5	-18.5	0.19 (1)	10.00			
M- L	0 / 5780	-18.5	-18.5	0.48 (1)	10.00			
L- K	0 / 5780	-18.5	-18.5	0.48 (1)	10.00			
K- R	0 / 3733	-18.5	-18.5	0.39 (1)	10.00			
R- S	0 / 3733	-18.5	-18.5	0.39 (1)	10.00			
S- J	0 / 3733	-18.5	-18.5	0.39 (1)	10.00			
J- T	0 / 0	-18.5	-18.5	0.50 (1)	10.00			
T- U	0 / 0	-18.5	-18.5	0.50 (1)	10.00			
U- I	0 / 0	-18.5	-18.5	0.50 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.21 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	16-2-5	-38	-38	---	FRONT	VERT	DEAD	---	C1
F	16-2-6	-111	-111	---	FRONT	VERT	TOTAL	---	C1
F	16-2-5	-239	-239	---	FRONT	VERT	SNOW	---	C1
K	12-1-8	-1679	-1679	---	FRONT	VERT	TOTAL	---	C1
P	13-5-4	-106	-106	---	FRONT	VERT	TOTAL	---	C1
Q	14-5-4	-106	-106	---	FRONT	VERT	TOTAL	---	C1
R	13-5-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
S	14-5-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
T	16-5-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1
U	18-5-4	-21	-21	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.50/1.00 (I-J:1) , WB=0.49/1.00 (G-J:1) , SSI=0.60/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	T1	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-I	MT20	4.0	10.0	2.00	7.75
D	TMW+w	MT20	2.0	4.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTWW-I	MT20	4.0	10.0	2.00	7.75
G	TMVW-p	MT20	5.0	6.0	1.50	3.00
I	BMVW-t	MT20	10.0	12.0		Edge
J	BMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMWW+t	MT20	4.0	10.0	5.00	1.75
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JSI GRIP= 0.85 (J) (INPUT = 0.90)
JSI METAL= 0.57 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040217

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T2	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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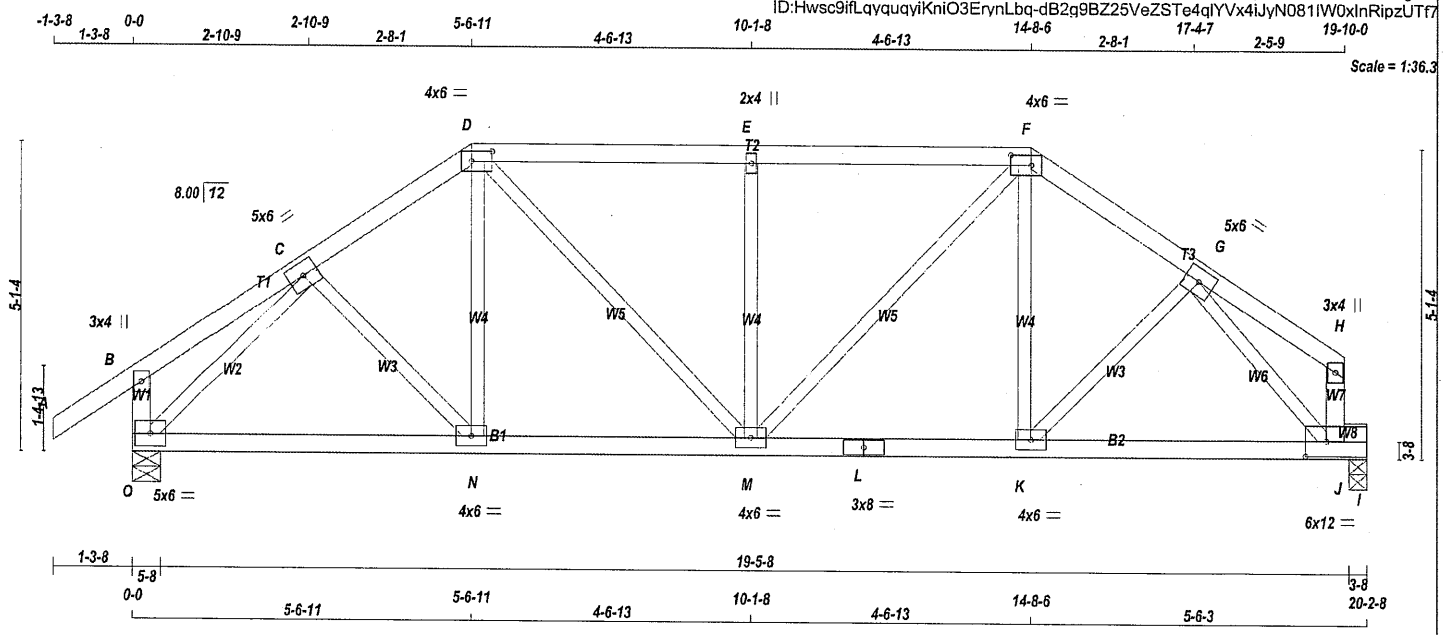


TOTAL WEIGHT = 2 X 86 = 173 lb [M]

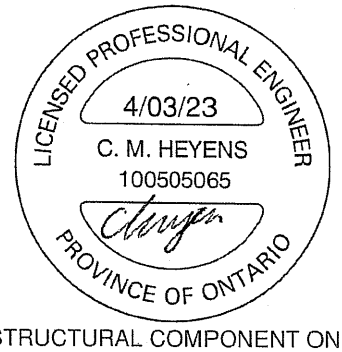
LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-I	MT20	4.0	6.0	2.00	4.00
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVWW-t	MT20	6.0	12.0	3.00	4.00
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0		



STRUCTURAL COMPONENT ONLY
DWG # TR23040218

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
O	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		C- N	0 / 51	0.02 (4)
B- C	0 / 20	-129.6	-129.6	0.14 (1)	10.00		N- D	0 / 96	0.03 (4)
C- D	-1547 / 0	-129.6	-129.6	0.16 (1)	5.13		D- M	0 / 523	0.12 (1)
D- E	-1630 / 0	-129.6	-129.6	0.50 (1)	4.53		M- E	-722 / 0	0.28 (1)
E- F	-1630 / 0	-129.6	-129.6	0.50 (1)	4.53		M- F	0 / 489	0.11 (1)
F- G	-1571 / 0	-129.6	-129.6	0.15 (1)	5.11		K- F	0 / 130	0.03 (4)
G- H	0 / 19	-129.6	-129.6	0.11 (1)	10.00		K- G	0 / 215	0.05 (1)
O- B	-322 / 0	0.0	0.0	0.03 (1)	7.81		O- C	-1804 / 0	0.48 (1)
J- H	-117 / 0	0.0	0.0	0.01 (1)	7.81		G- J	-1818 / 0	0.43 (1)
O- N	0 / 1240	-18.5	-18.5	0.28 (1)	10.00				
N- M	0 / 1270	-18.5	-18.5	0.28 (1)	10.00				
M- L	0 / 1293	-18.5	-18.5	0.48 (1)	10.00				
L- K	0 / 1293	-18.5	-18.5	0.48 (1)	10.00				
K- J	0 / 1139	-18.5	-18.5	0.92 (1)	10.00				
J- I	0 / 0	-18.5	-18.5	0.31 (1)	10.00				

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 961 (0.25")

CSI: TC=0.50/1.00 (D-E:1) , BC=0.92/1.00 (J-K:1) , WB=0.48/1.00 (C-O:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.41 (L) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T3	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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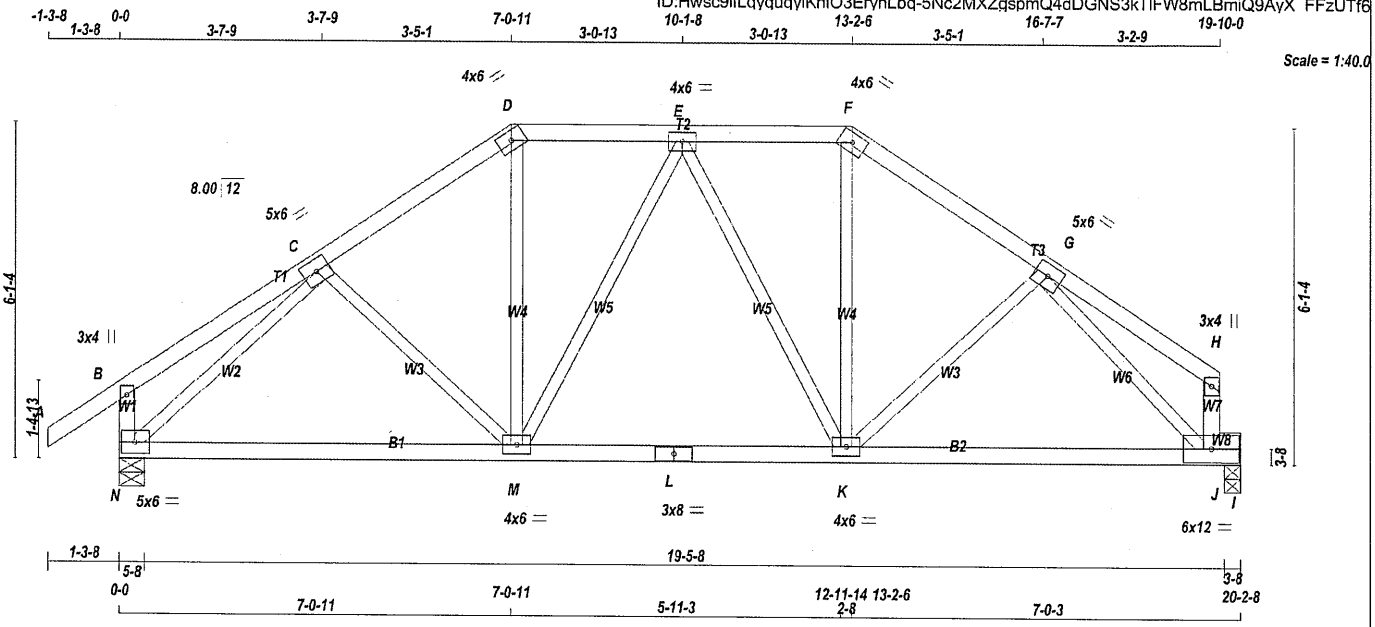

TOTAL WEIGHT = 2 X 88 = 176 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TTW-h	MT20	4.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVWW-t	MT20	6.0	12.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1674	0	1674	0	0	5-8	5-8
I	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
I	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- M	-141 / 17	0.06 (1)
B- C	0 / 28	-129.6	-129.6	0.24 (1)	10.00	M- D	0 / 457	0.10 (1)
C- D	-1460 / 0	-129.6	-129.6	0.28 (1)	5.10	K- F	0 / 469	0.11 (1)
D- E	-1197 / 0	-129.6	-129.6	0.20 (1)	5.60	K- G	0 / 48	0.02 (4)
E- F	-1224 / 0	-129.6	-129.6	0.20 (1)	5.55	N- C	-1804 / 0	0.69 (1)
F- G	-1488 / 0	-129.6	-129.6	0.25 (1)	5.10	G- J	-1809 / 0	0.61 (1)
G- H	0 / 28	-129.6	-129.6	0.22 (1)	10.00	M- E	-295 / 0	0.23 (1)
N- B	-356 / 0	0.0	0.0	0.04 (1)	7.81	E- K	-236 / 0	0.18 (1)
J- H	-151 / 0	0.0	0.0	0.02 (1)	7.81			
N- M	0 / 1292	-18.5	-18.5	0.34 (1)	10.00			
M- L	0 / 1331	-18.5	-18.5	0.51 (1)	10.00			
L- K	0 / 1331	-18.5	-18.5	0.51 (1)	10.00			
K- J	0 / 1214	-18.5	-18.5	0.93 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 595 (0.41")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.93/1.00 (J-K:1) , WB=0.69/1.00 (C-N:1) , SSI=0.57/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (G) (INPUT = 0.90)
JSI METAL= 0.49 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040219

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL Block 4 REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T4	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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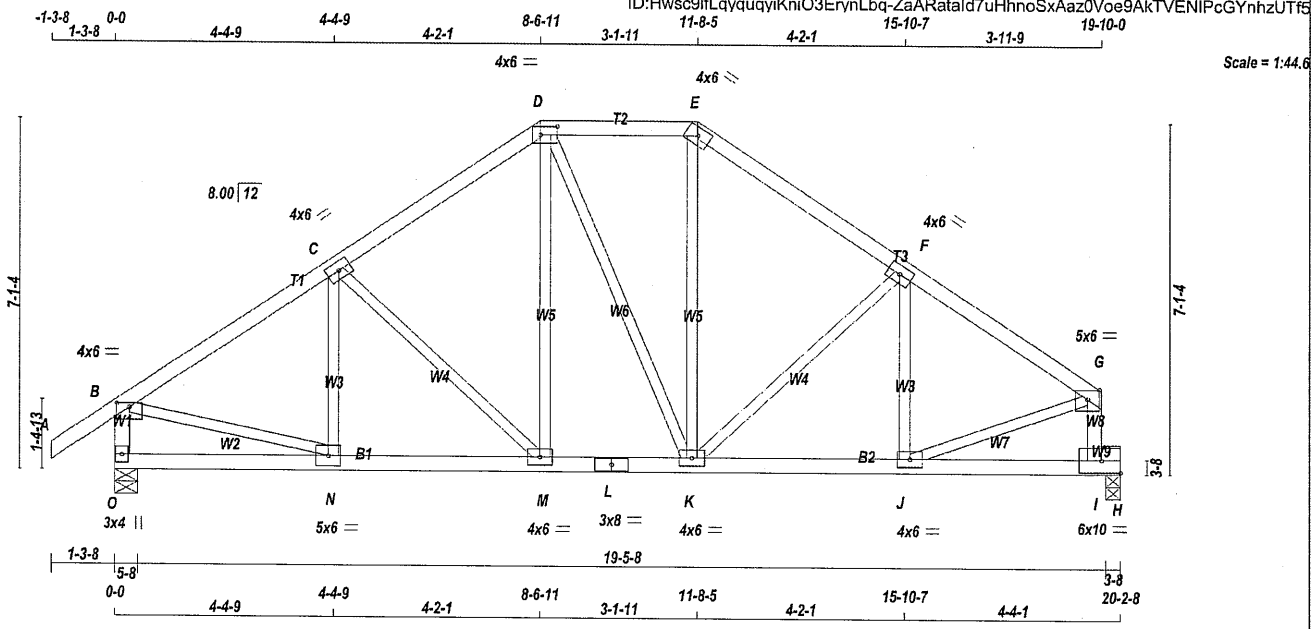


TOTAL WEIGHT = 2 X 93 = 185 lb [M]

LUMBER

N. L. G. A. RULES

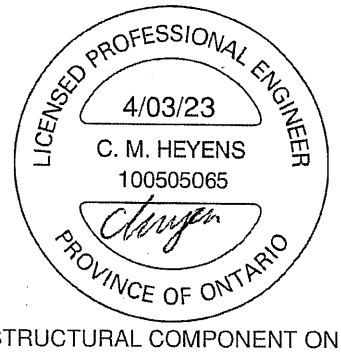
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TTW-h	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMVW-t	MT20	6.0	10.0		Edge
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040220

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	-1597 / 0	-129.6	-129.6	0.45 (1)	4.68	
C- D	-1343 / 0	-129.6	-129.6	0.43 (1)	5.03	
D- E	-1082 / 0	-129.6	-129.6	0.23 (1)	5.77	
E- F	-1330 / 0	-129.6	-129.6	0.39 (1)	5.11	
F- G	-1617 / 0	-129.6	-129.6	0.40 (1)	4.72	
O- B	-1638 / 0	0.0	0.0	0.17 (1)	6.47	
I- G	-1581 / 0	0.0	0.0	0.17 (1)	6.56	
O- N	0 / 0	-18.5	-18.5	0.08 (4)	10.00	
N- M	0 / 1359	-18.5	-18.5	0.25 (1)	10.00	
M- L	0 / 1090	-18.5	-18.5	0.21 (1)	10.00	
L- K	0 / 1090	-18.5	-18.5	0.21 (1)	10.00	
K- J	0 / 1375	-18.5	-18.5	0.43 (1)	10.00	
J- I	0 / 0	-18.5	-18.5	0.74 (1)	10.00	
I- H	0 / 0	-18.5	-18.5	0.31 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-251 / 18	0.07 (1)
C- M	-375 / 0	0.22 (1)
M- D	0 / 338	0.08 (1)
D- K	-19 / 0	0.02 (1)
K- E	0 / 308	0.07 (1)
K- F	-408 / 0	0.24 (1)
J- F	-178 / 32	0.05 (1)
B- N	0 / 1399	0.31 (1)
J- G	0 / 1453	0.33 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.74/1.00 (I-J:1) , WB=0.33/1.00 (G-J:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T5	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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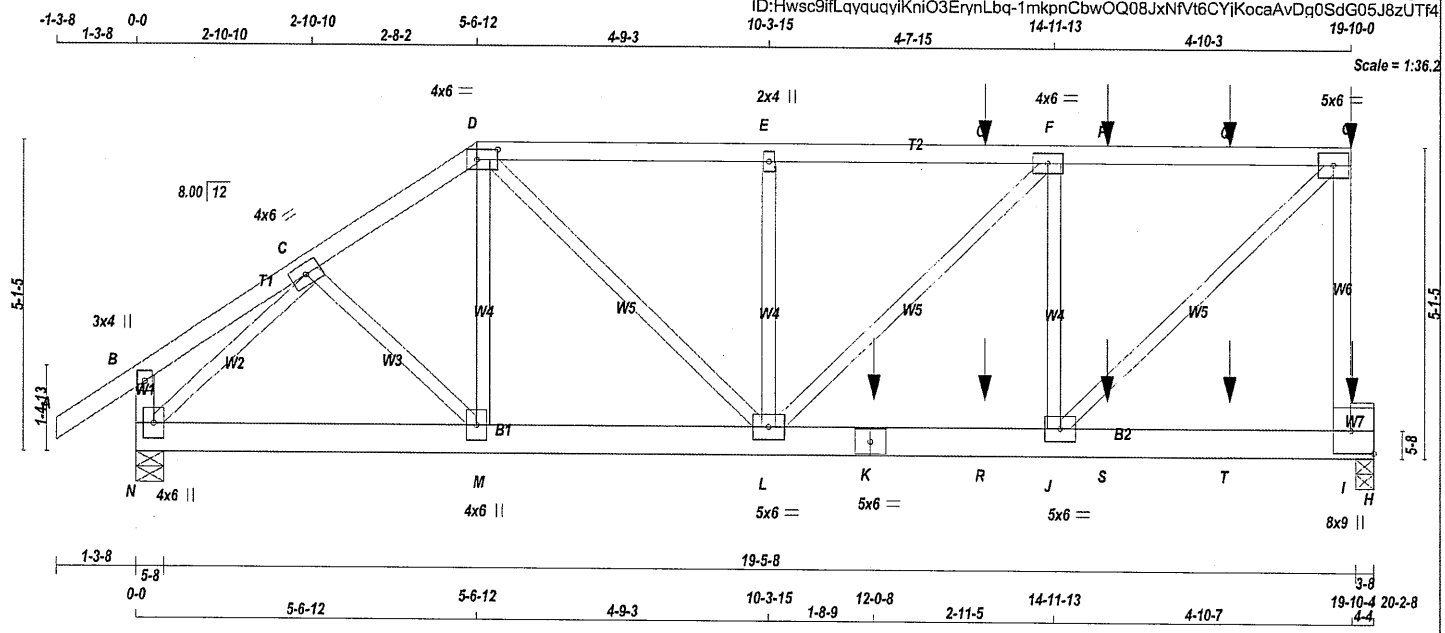

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
A- D 1	12	TOP
D- G 1	12	SIDE(61.0)
G- I 1	12	TOP
N- B 1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
N- K 2	12	SIDE(0.0)
K- H 2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS		
2x3 1	6	
2x6 2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040221

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
N	2053	0	2053	0	0	5-8	5-8
H	2398	0	2398	0	0	3-8	3-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1427	1078 / 0	0 / 0	0 / 0	0 / 0	349 / 0	0 / 0
H	1670	1244 / 0	0 / 0	0 / 0	0 / 0	426 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.10 (1)	10.00	C- M	0 / 127	0.02 (1)
B- C	0 / 19	-129.6	-129.6	0.07 (1)	10.00	M- D	-28 / 65	0.01 (4)
C- D	-2097 / 0	-129.6	-129.6	0.09 (1)	6.02	N- C	-2342 / 0	0.32 (1)
D- E	-2511 / 0	-129.6	-129.6	0.23 (1)	5.44	J- G	0 / 2963	0.37 (1)
E- O	-2511 / 0	-129.6	-129.6	0.35 (1)	5.26	D- L	0 / 1107	0.14 (1)
O- F	-2511 / 0	-129.6	-129.6	0.35 (1)	5.26	J- F	-1372 / 0	0.26 (1)
F- P	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63	L- E	-656 / 0	0.12 (1)
P- Q	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63	L- F	0 / 557	0.07 (1)
Q- G	-2120 / 0	-129.6	-129.6	0.34 (1)	5.63			
I- G	-2581 / 0	0.0	0.0	0.54 (1)	7.06			
N- B	-323 / 0	0.0	0.0	0.02 (1)	7.81			
N- M	0 / 1634	-18.5	-18.5	0.13 (1)	10.00			
M- L	0 / 1724	-18.5	-18.5	0.17 (1)	10.00			
L- K	0 / 2120	-18.5	-18.5	0.34 (1)	10.00			
K- R	0 / 2120	-18.5	-18.5	0.34 (1)	10.00			
R- J	0 / 2120	-18.5	-18.5	0.34 (1)	10.00			
J- S	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
S- T	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
T- I	0 / 0	-18.5	-18.5	0.33 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.14 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	19-10-0	-128	-128	---	BACK	VERT	TOTAL	---	C1
I	19-10-4	-19	-19	---	BACK	VERT	TOTAL	---	C1
K	12-0-8	-485	-485	---	BACK	VERT	TOTAL	---	C1
O	13-10-4	-83	-83	---	BACK	VERT	TOTAL	---	C1
P	15-10-4	-83	-83	---	BACK	VERT	TOTAL	---	C1
Q	17-10-4	-83	-83	---	BACK	VERT	TOTAL	---	C1
R	13-10-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
S	15-10-4	-14	-14	---	BACK	VERT	TOTAL	---	C1
T	17-10-4	-14	-14	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

TOTAL WEIGHT = 4 X 100 = 402 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.54/1.00 (G-I:1) , BC=0.34/1.00 (J-L:1) , WB=0.37/1.00 (G-J:1) , SSI=0.40/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.90)
JSI METAL= 0.44 (K) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T5	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
I	BMVW+p	MT20	8.0	9.0	4.50	Edge
J	BMWW-t	MT20	5.0	6.0		
K	BS-t	MT20	5.0	6.0		
L	BMWWW-t	MT20	5.0	6.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMVW1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040221

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T6	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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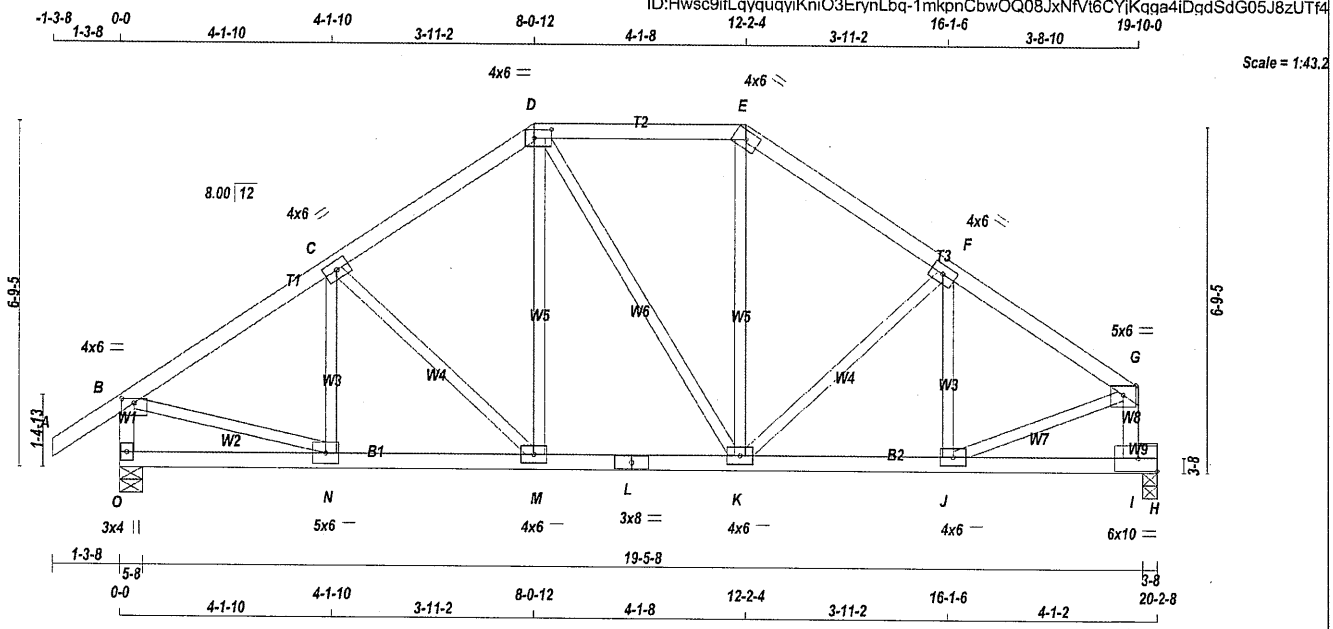

TOTAL WEIGHT = 2 X 91 = 182 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TTW-h	MT20	4.0	6.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-p	MT20	5.0	6.0	Edge	
I	BMVW-t	MT20	6.0	10.0	3.00	4.50
J	BMWW-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1674	0	1674	0	0	5-8	5-8
H	1449	0	1449	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1164	877 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
H	1010	744 / 0	0 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	LC1 TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B		0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C		-1590 / 0	-129.6	-129.6	0.40 (1)	4.77
C- D		-1384 / 0	-129.6	-129.6	0.38 (1)	5.05
D- E		-1117 / 0	-129.6	-129.6	0.40 (1)	5.42
E- F		-1371 / 0	-129.6	-129.6	0.34 (1)	5.13
F- G		-1611 / 0	-129.6	-129.6	0.35 (1)	4.80
O- B		-1639 / 0	0.0	0.0	0.17 (1)	6.47
I- G		-1593 / 0	0.0	0.0	0.17 (1)	6.55
O- N		0 / 0	-18.5	-18.5	0.07 (4)	10.00
N- M		0 / 1351	-18.5	-18.5	0.26 (1)	10.00
M- L		0 / 1126	-18.5	-18.5	0.22 (1)	10.00
L- K		0 / 1126	-18.5	-18.5	0.22 (1)	10.00
K- J		0 / 1368	-18.5	-18.5	0.42 (1)	10.00
J- I		0 / 0	-18.5	-18.5	0.74 (1)	10.00
I- H		0 / 0	-18.5	-18.5	0.31 (1)	10.00

WEBS MEMB.	FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C		-277 / 5	0.07 (1)
C- M		-317 / 0	0.17 (1)
M- D		0 / 306	0.07 (1)
D- K		-16 / 0	0.02 (1)
K- E		0 / 280	0.06 (1)
K- F		-352 / 0	0.18 (1)
J- F		-202 / 19	0.05 (1)
B- N		0 / 1396	0.31 (1)
J- G		0 / 1456	0.33 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.67")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.67")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.74/1.00 (I-J:1) , WB=0.33/1.00 (G-J:1) , SSI=0.57/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.38 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040222

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T7	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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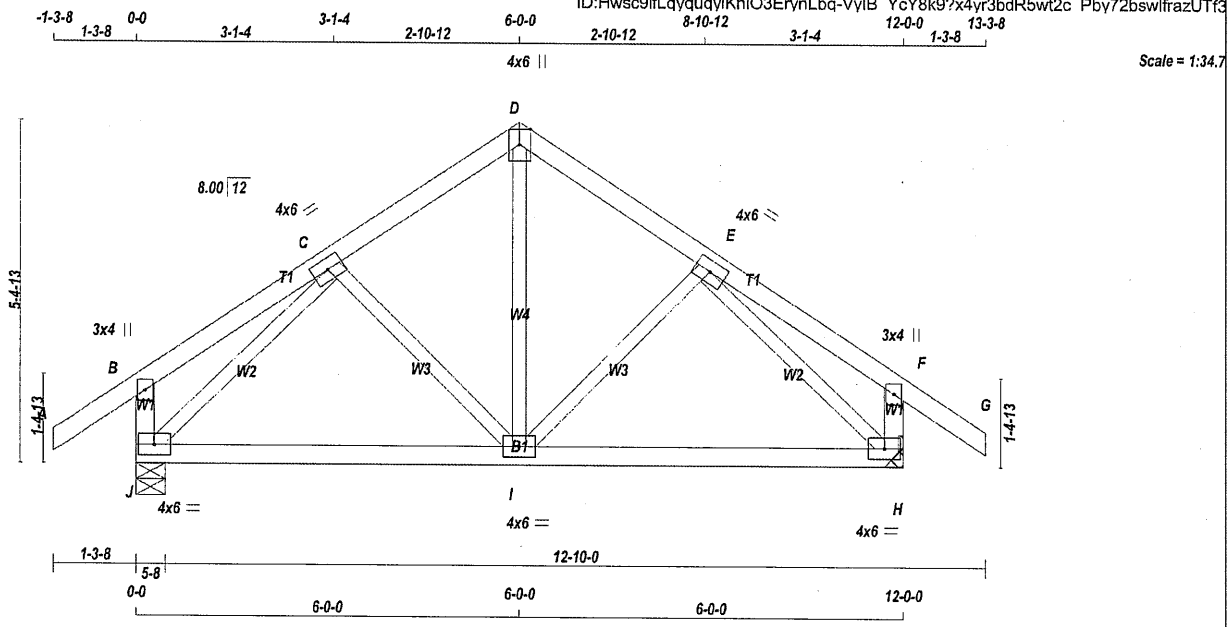

TOTAL WEIGHT = 8 X 53 = 424 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
J	1329	0	1329	0	0	IN-SX 5-8	IN-SX 5-8
H	1329	0	1329	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	951	563 / 0	0 / 0	0 / 0	0 / 0	387 / 0	0 / 0
H	951	563 / 0	0 / 0	0 / 0	0 / 0	387 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	I- D	0 / 709	0.19 (4)
B- C	0 / 23	-129.6	-129.6	0.19 (1)	10.00	I- E	-136 / 62	0.04 (1)
C- D	-940 / 0	-129.6	-129.6	0.20 (1)	6.11	C- I	-136 / 62	0.04 (1)
D- E	-940 / 0	-129.6	-129.6	0.20 (1)	6.11	J- C	-1231 / 0	0.38 (1)
E- F	0 / 23	-129.6	-129.6	0.19 (1)	10.00	E- H	-1231 / 0	0.38 (1)
F- G	0 / 49	-129.6	-129.6	0.19 (1)	10.00			
J- B	-331 / 0	0.0	0.0	0.04 (1)	7.81			
H- F	-331 / 0	0.0	0.0	0.04 (1)	7.81			
J- I	0 / 859	-37.3	-87.2	0.82 (4)	10.00			
I- H	0 / 859	-87.3	-37.3	0.82 (4)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.20/1.00 (D-E:1) , BC=0.82/1.00 (I-J:4) , WB=0.38/1.00 (E-H:1) , SSI=0.38/1.00 (H-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040223

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 4
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T8	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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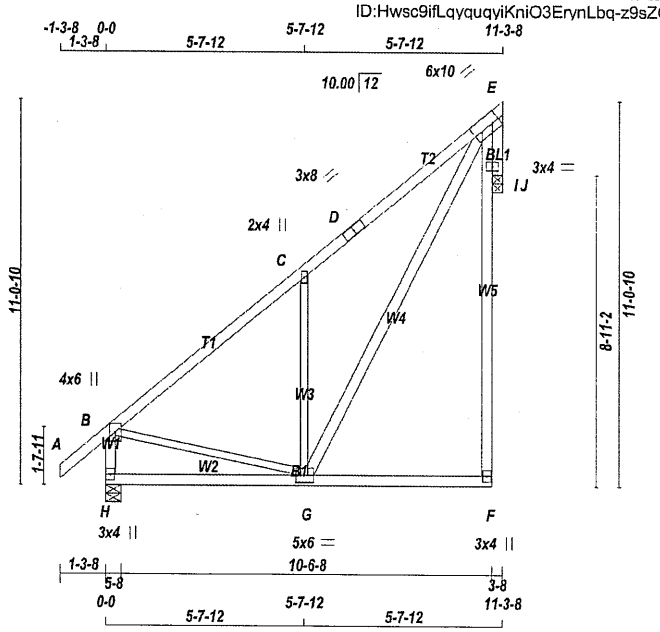

Scale = 1:63.2

TOTAL WEIGHT = 8 X 68 = 545 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	804	0	804	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	1005	0	1005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	560	415 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	698	532 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	G- C	-865 / 0	0.56 (1)
B- C	-569 / 0	-129.6	-129.6	0.51 (1)	6.25	G- E	0 / 984	0.16 (1)
C- D	-646 / 0	-129.6	-129.6	0.51 (1)	6.25	B- G	0 / 491	0.11 (1)
D- E	-646 / 0	-129.6	-129.6	0.51 (1)	6.25	E- J	-807 / 0	0.09 (1)
F- I	0 / 40	0.0	0.0	0.14 (1)	10.00	I- J	0 / 69	0.00 (1)
I- E	0 / 40	0.0	0.0	0.14 (1)	10.00			
H- B	-962 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
G- F	0 / 11	-18.5	-18.5	0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.16/1.00 (F-G:4) , WB=0.56/1.00 (C-G:1) , SSI=0.27/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (G) (INPUT = 0.90)
JSI METAL= 0.46 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040224

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G9	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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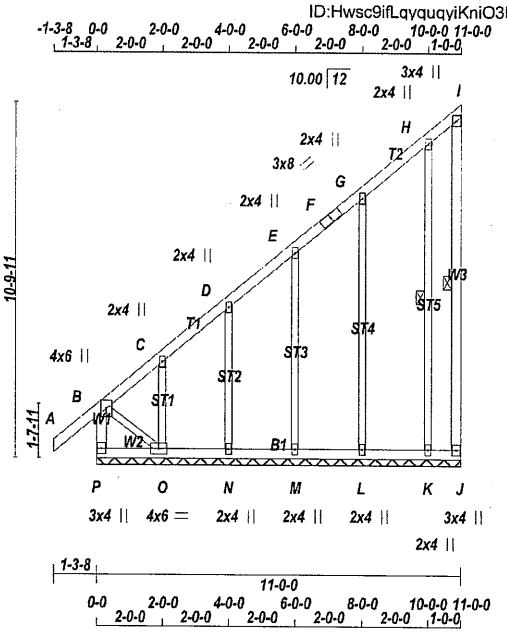

Scale = 1:67.0

TOTAL WEIGHT = 2 X 70 = 141 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-388 / 0	0.0	0.0	0.04 (1)	7.81	O- C	-181 / 0	0.03 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	N- D	-271 / 0	0.10 (1)
B- C	-61 / 0	-129.6	-129.6	0.17 (1)	6.25	M- E	-251 / 0	0.18 (1)
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25	L- G	-271 / 0	0.37 (1)
D- E	-14 / 0	-129.6	-129.6	0.07 (1)	6.25	K- H	-209 / 0	0.14 (1)
E- F	-5 / 0	-129.6	-129.6	0.06 (1)	10.00	B- O	0 / 24	0.01 (1)
F- G	-5 / 0	-129.6	-129.6	0.06 (1)	10.00			
G- H	-10 / 0	-129.6	-129.6	0.06 (1)	6.25			
H- I	-14 / 0	-129.6	-129.6	0.03 (1)	6.25			
J- I	-50 / 0	0.0	0.0	0.03 (1)	6.25			
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 15	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 9	-18.5	-18.5	0.01 (4)	10.00			
M- L	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 2	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (N-O:4) , WB=0.37/1.00 (G-L:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040200

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T10	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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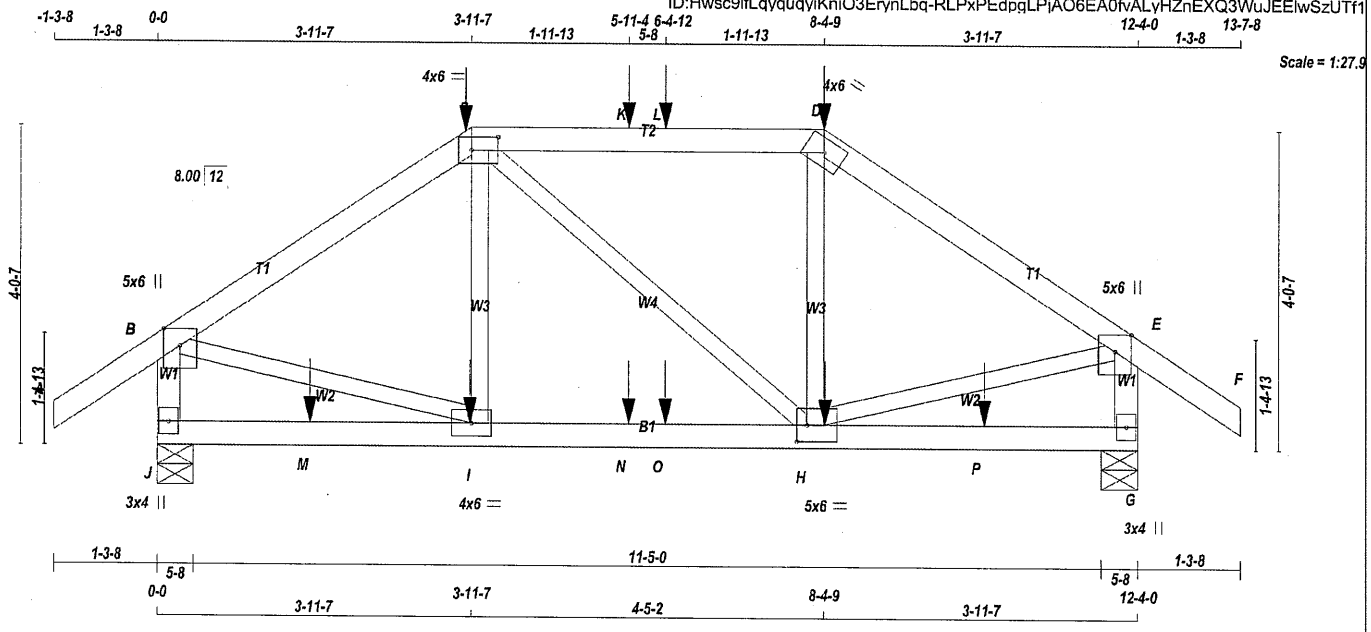


TOTAL WEIGHT = 2 X 52 = 104 lb

[M]

LUMBER

N. L. G. A. RULES

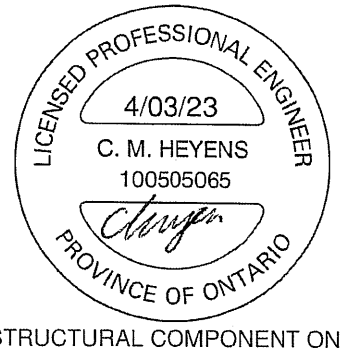
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040225

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1398	0	1398	0	0	5-8	5-8
G	1398	0	1398	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	971	738 / 0	0 / 0	0 / 0	0 / 0	233 / 0	0 / 0
G	971	738 / 0	0 / 0	0 / 0	0 / 0	233 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	I- C	-161 / 51	0.04 (1)
B- C	-1194 / 0	-129.6	-129.6	0.42 (1)	5.24	C- H	0 / 2	0.00 (4)
C- K	-989 / 0	-129.6	-129.6	0.68 (1)	4.98	H- D	-158 / 52	0.04 (1)
K- L	-989 / 0	-129.6	-129.6	0.68 (1)	4.98	B- I	0 / 1027	0.25 (1)
L- D	-989 / 0	-129.6	-129.6	0.68 (1)	4.98	H- E	0 / 1030	0.25 (1)
D- E	-1199 / 0	-129.6	-129.6	0.42 (1)	5.24			
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00			
J- B	-1363 / 0	0.0	0.0	0.15 (1)	6.91			
G- E	-1362 / 0	0.0	0.0	0.15 (1)	6.92			
J- M	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
M- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
I- N	0 / 988	-18.5	-18.5	0.22 (1)	10.00			
N- O	0 / 988	-18.5	-18.5	0.22 (1)	10.00			
O- H	0 / 988	-18.5	-18.5	0.22 (1)	10.00			
H- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
P- G	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-45	-45	---	BACK	VERT	TOTAL	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	8-4-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	8-4-9	-45	-45	---	BACK	VERT	TOTAL	---	C1
D	8-4-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
H	8-4-12	-6	-6	---	BACK	VERT	TOTAL	---	C1
I	3-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
K	5-11-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
L	6-4-12	-39	-39	---	BACK	VERT	TOTAL	---	C1
M	1-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
N	5-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
O	6-4-12	-6	-6	---	BACK	VERT	TOTAL	---	C1
P	10-4-12	-6	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.68/1.00 (C-D:1) , BC=0.22/1.00 (H-I:1) , WB=0.25/1.00 (E-H:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.53 (E) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	T10	2	1	TRUSS DESC.	

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040225

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T11	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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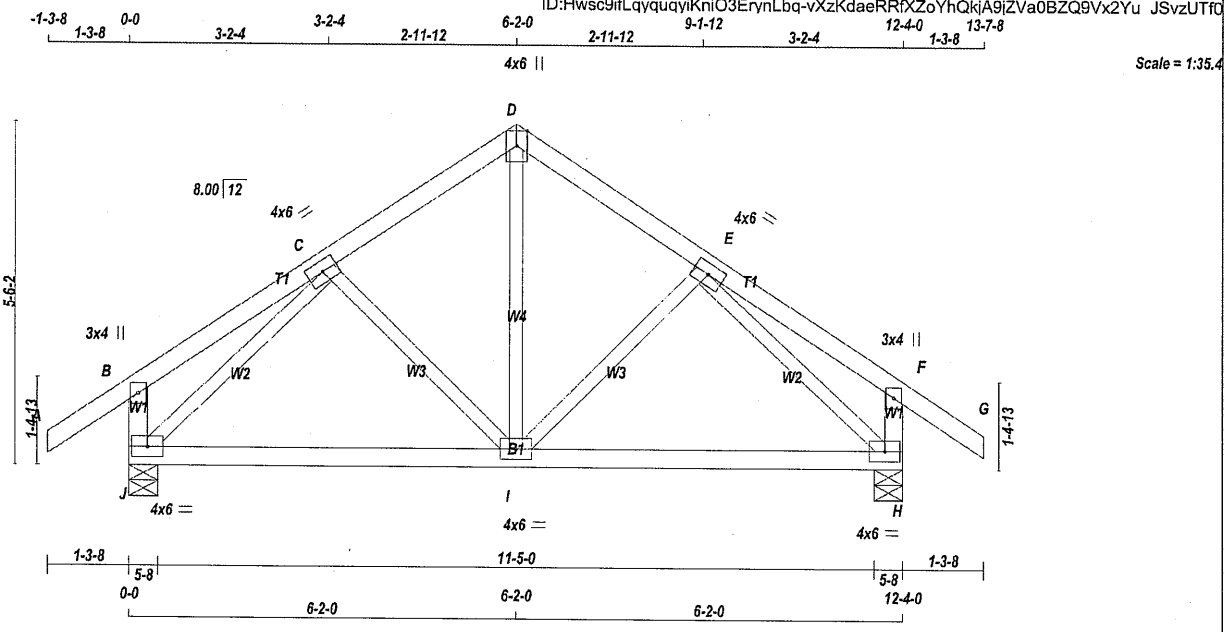

TOTAL WEIGHT = 8 X 54 = 434 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1092	0	1092	0	0	5-8	5-8
H	1092	0	1092	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	758	576 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
H	758	576 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD LC1 (PLF) TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	I- D	0 / 430	0.10 (1)
B- C	0 / 25	-129.6	-129.6	0.19 (1)	10.00	I- E	-205 / 0	0.07 (1)
C- D	-695 / 0	-129.6	-129.6	0.15 (1)	6.25	C- I	-205 / 0	0.07 (1)
D- E	-695 / 0	-129.6	-129.6	0.15 (1)	6.25	J- C	-1001 / 0	0.31 (1)
E- F	0 / 25	-129.6	-129.6	0.19 (1)	10.00	E- H	-1001 / 0	0.31 (1)
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00			
J- B	-333 / 0	0.0	0.0	0.03 (1)	7.81			
H- F	-333 / 0	0.0	0.0	0.03 (1)	7.81			
J- I	0 / 702	-18.5	-18.5	0.24 (4)	10.00			
I- H	0 / 702	-18.5	-18.5	0.24 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (E-F:1) , BC=0.24/1.00 (I-J:4) , WB=0.31/1.00 (E-H:1) , SSI=0.16/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.54 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040226

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T11A	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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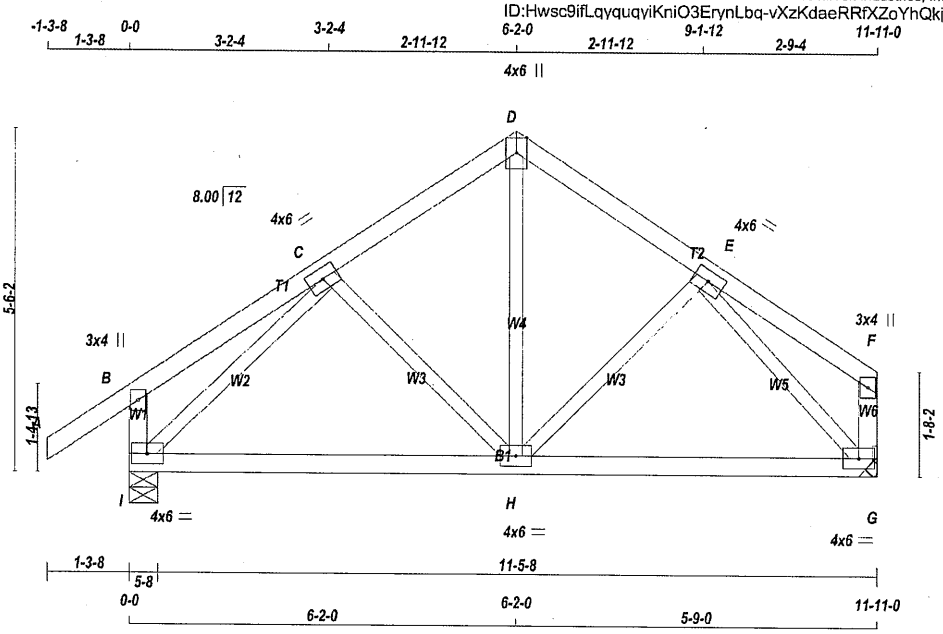

TOTAL WEIGHT = 2 X 52 = 103 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1061	0	1061	0	0	5-8	5-8
G	883	0	883	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	736	560 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
G	615	455 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	H- D	0 / 375	0.08 (1)
B- C	0 / 25	-129.6	-129.6	0.19 (1)	10.00	H- E	-124 / 11	0.04 (1)
C- D	-650 / 0	-129.6	-129.6	0.15 (1)	6.25	C- H	-215 / 0	0.07 (1)
D- E	-646 / 0	-129.6	-129.6	0.13 (1)	6.25	I- C	-957 / 0	0.29 (1)
E- F	0 / 25	-129.6	-129.6	0.17 (1)	10.00	E- G	-938 / 0	0.26 (1)
I- B	-332 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-127 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 671	-18.5	-18.5	0.23 (4)	10.00			
H- G	0 / 607	-18.5	-18.5	0.22 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.23/1.00 (H-I:4) , WB=0.29/1.00 (C-I:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040227

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T12	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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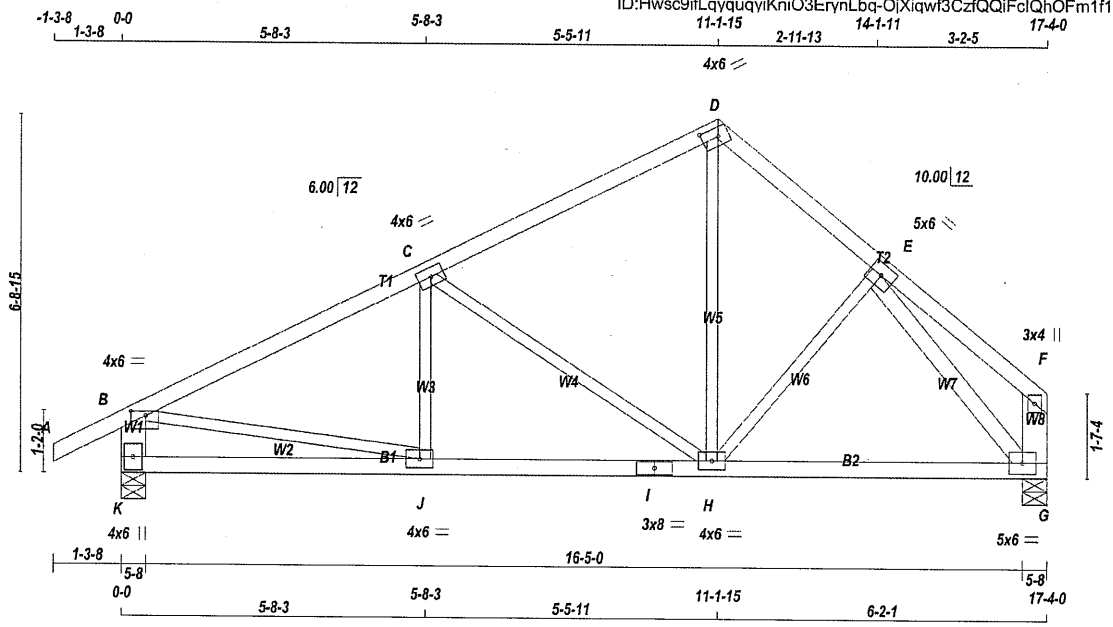

TOTAL WEIGHT = 4 X 75 = 300 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TMWW-t	MT20	4.0	6.0		
D	TTW-I	MT20	4.0	6.0	2.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1460	0	1460	0	0	5-8	5-8
G	1284	0	1284	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1015	766 / 0	0 / 0	0 / 0	0 / 0	249 / 0	0 / 0
G	895	662 / 0	0 / 0	0 / 0	0 / 0	232 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	J- C	-119 / 65	0.03 (1)
B- C	-1601 / 0	-129.6	-129.6	0.56 (1)	4.52	C- H	-793 / 0	0.61 (1)
C- D	-947 / 0	-129.6	-129.6	0.52 (1)	5.59	H- D	0 / 651	0.15 (1)
D- E	-1074 / 0	-129.6	-129.6	0.16 (1)	5.92	H- E	-101 / 16	0.04 (1)
E- F	0 / 28	-129.6	-129.6	0.19 (1)	10.00	B- J	0 / 1483	0.33 (1)
K- B	-1413 / 0	0.0	0.0	0.09 (1)	7.81	E- G	-1387 / 0	0.36 (1)
G- F	-153 / 0	0.0	0.0	0.01 (1)	7.81			
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
J- I	0 / 1466	-18.5	-18.5	0.32 (1)	10.00			
I- H	0 / 1466	-18.5	-18.5	0.32 (1)	10.00			
H- G	0 / 867	-18.5	-18.5	0.23 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.32/1.00 (H-J:1) , WB=0.61/1.00 (C-H:1) , SSI=0.32/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.45 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040228

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G12	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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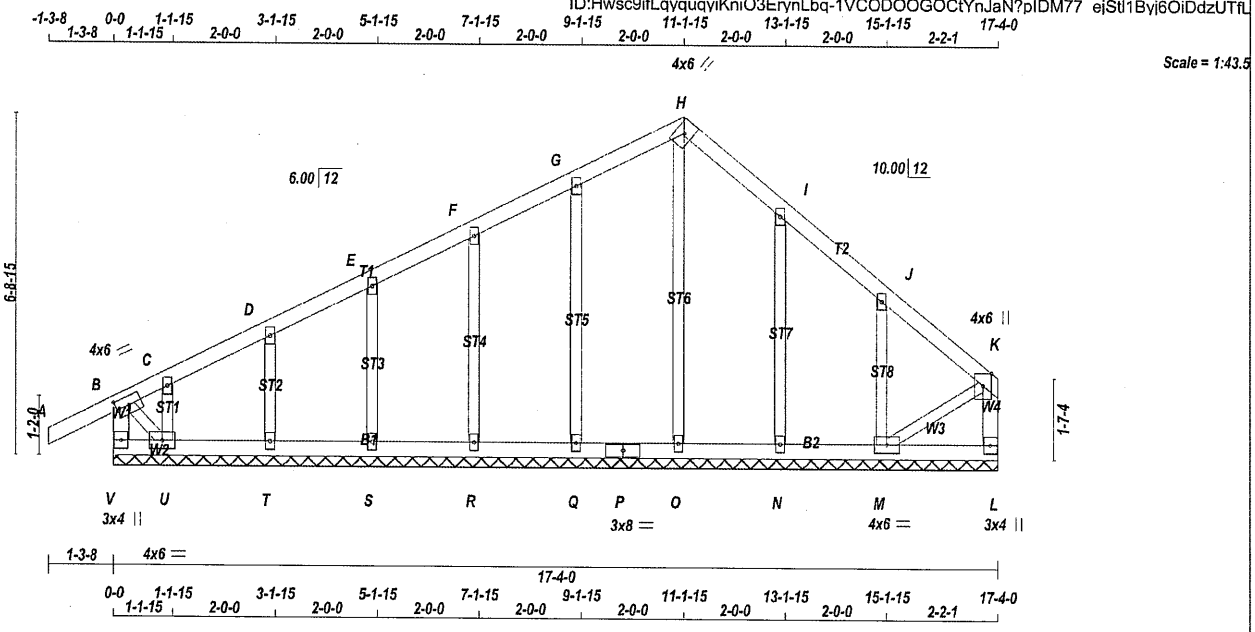


TOTAL WEIGHT = 74 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

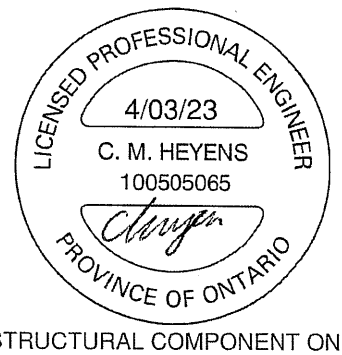
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW+h	MT20	4.0	6.0		
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, Q, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
P	BS-t	MT20	3.0	8.0		
U	BMWW1-t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040201

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-93 / 0	-129.6	-129.6	0.16 (1)	6.25
C- D	-35 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-36 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-29 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-21 / 0	-129.6	-129.6	0.08 (1)	6.25
G- H	-32 / 0	-129.6	-129.6	0.08 (1)	6.25
H- I	-43 / 0	-129.6	-129.6	0.07 (1)	6.25
I- J	-38 / 0	-129.6	-129.6	0.08 (1)	6.25
J- K	-29 / 0	-129.6	-129.6	0.08 (1)	6.25
V- B	-399 / 0	0.0	0.0	0.04 (1)	7.81
L- K	-164 / 0	0.0	0.0	0.02 (1)	7.81
V- U	0 / 0	-18.5	-18.5	0.01 (4)	10.00
U- T	0 / 38	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 31	-18.5	-18.5	0.02 (4)	10.00
S- R	0 / 26	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 22	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 18	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 18	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 21	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 27	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- H	-192 / 0	0.14 (1)
Q- G	-304 / 0	0.15 (1)
R- F	-248 / 0	0.08 (1)
S- E	-254 / 0	0.06 (1)
T- D	-275 / 0	0.05 (1)
U- C	-72 / 0	0.01 (1)
N- I	-273 / 0	0.11 (1)
M- J	-296 / 0	0.06 (1)
B- U	0 / 54	0.01 (1)
M- K	0 / 41	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (M-N:4) , WB=0.15/1.00 (G-Q:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (B) (INPUT = 0.90)
JSI METAL= 0.16 (J) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T13A	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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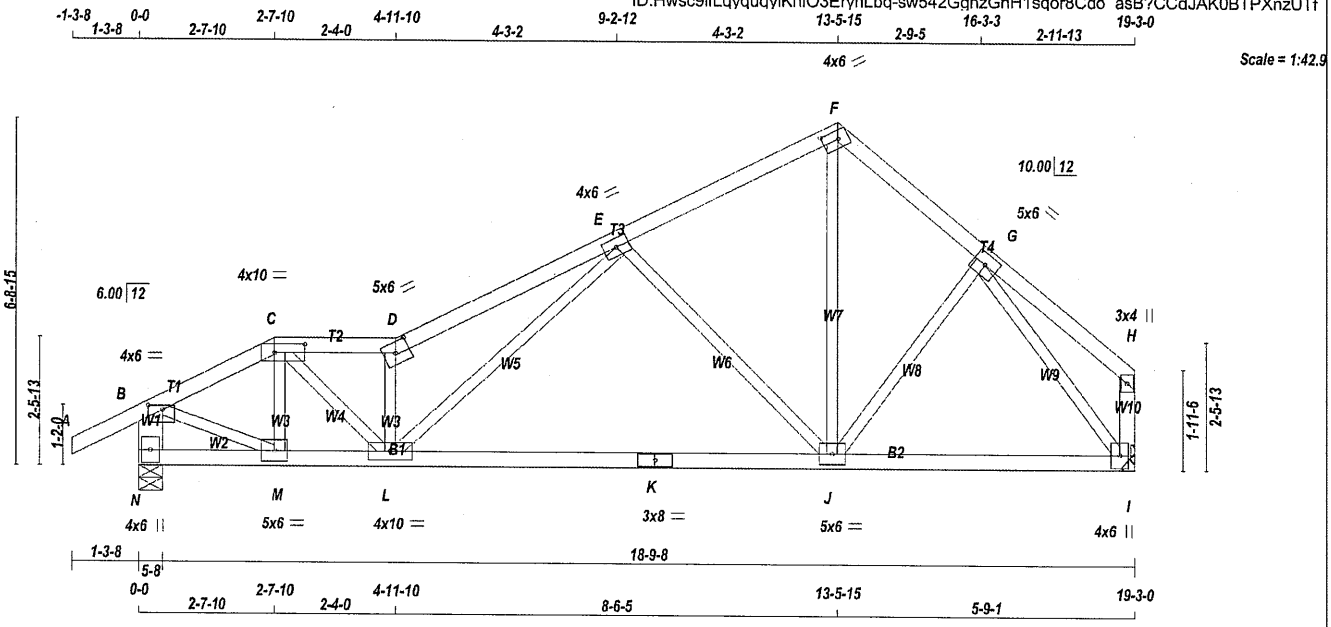

TOTAL WEIGHT = 4 X 82 = 327 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	1.00	3.25
C	TTWW-I	MT20	4.0	10.0	2.00	7.00
D	TTW-h	MT20	5.0	6.0	2.50	3.25
E	TMWW-t	MT20	4.0	6.0		
F	TTW-I	MT20	4.0	6.0	2.00	3.50
G	TMWW-t	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWWW-t	MT20	4.0	10.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1602	0	1602	0	0	5-8	5-8
I	1426	0	1426	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1114	839 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0
I	994	736 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	M- C	-455 / 0	0.07 (1)
B- C	-1683 / 0	-129.6	-129.6	0.15 (1)	5.00	C- L	0 / 1270	0.29 (1)
C- D	-2388 / 0	-129.6	-129.6	0.15 (1)	4.33	L- D	-1589 / 0	0.25 (1)
D- E	-2723 / 0	-129.6	-129.6	0.40 (1)	3.83	L- E	0 / 1162	0.26 (1)
E- F	-1064 / 0	-129.6	-129.6	0.32 (1)	5.71	E- J	-1024 / 0	0.64 (1)
F- G	-1214 / 0	-129.6	-129.6	0.13 (1)	5.68	J- F	0 / 864	0.19 (1)
G- H	0 / 24	-129.6	-129.6	0.15 (1)	10.00	J- G	0 / 50	0.02 (4)
N- B	-1574 / 0	0.0	0.0	0.10 (1)	7.81	B- M	0 / 1575	0.35 (1)
I- H	-147 / 0	0.0	0.0	0.02 (1)	7.81	G- I	-1528 / 0	0.59 (1)
N- M	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
M- L	0 / 1481	-18.5	-18.5	0.30 (1)	10.00			
L- K	0 / 1637	-18.5	-18.5	0.41 (1)	10.00			
K- J	0 / 1637	-18.5	-18.5	0.41 (1)	10.00			
J- I	0 / 888	-18.5	-18.5	0.30 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.40/1.00 (D-E:1) , BC=0.41/1.00 (J-L:1) , WB=0.64/1.00 (E-J:1) , SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.66 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040229

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T14A	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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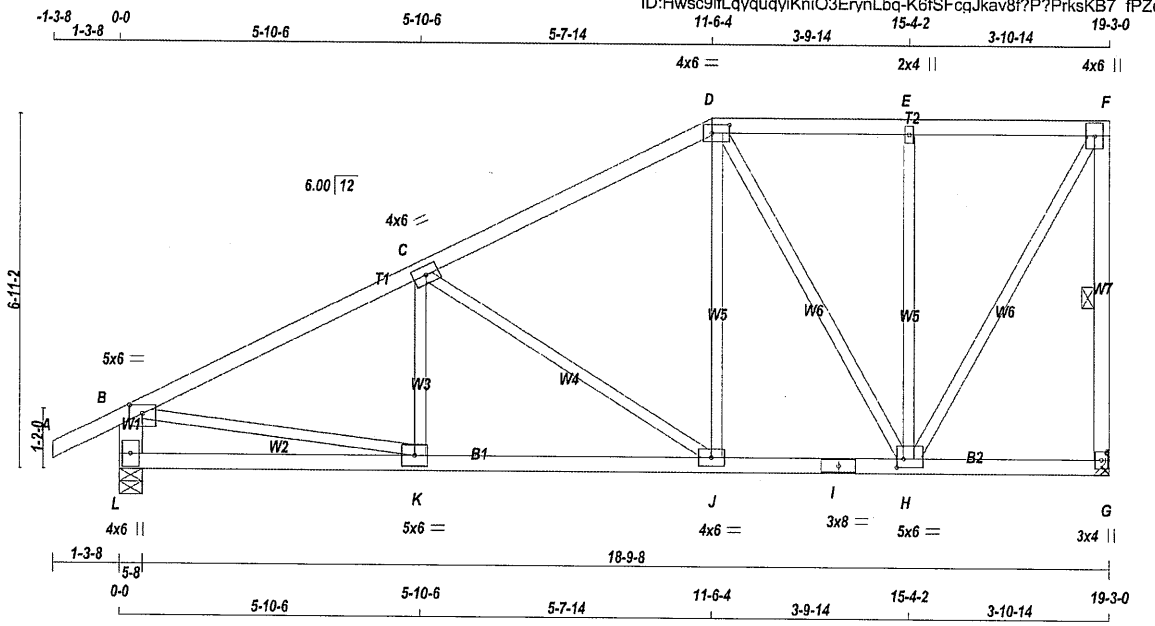

TOTAL WEIGHT = 2 X 91 = 181 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-l	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.00	1.50
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1426	0	1426	0	0	MECHANICAL	
L	1602	0	1602	0	0	5-8	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	994	736 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0
L	1114	839 / 0	0 / 0	0 / 0	0 / 0	274 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	K- C	-128 / 77	0.03 (1)
B- C	-1842 / 0	-129.6	-129.6	0.61 (1)	4.17	C- J	-827 / 0	0.74 (1)
C- D	-1151 / 0	-129.6	-129.6	0.57 (1)	5.11	J- D	0 / 545	0.12 (1)
D- E	-705 / 0	-129.6	-129.6	0.24 (1)	6.25	B- K	0 / 1701	0.38 (1)
E- F	-705 / 0	-129.6	-129.6	0.24 (1)	6.25	H- F	0 / 1390	0.31 (1)
G- F	-1393 / 0	0.0	0.0	0.30 (1)	5.52	D- H	-592 / 0	0.68 (1)
L- B	-1556 / 0	0.0	0.0	0.10 (1)	7.81	H- E	-616 / 0	0.50 (1)
L- K	0 / 0	-18.5	-18.5	0.15 (4)	10.00			
K- J	0 / 1683	-18.5	-18.5	0.34 (1)	10.00			
J- I	0 / 1001	-18.5	-18.5	0.21 (1)	10.00			
I- H	0 / 1001	-18.5	-18.5	0.21 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.34/1.00 (J-K:1) , WB=0.74/1.00 (C-J:1) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040230

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G14A	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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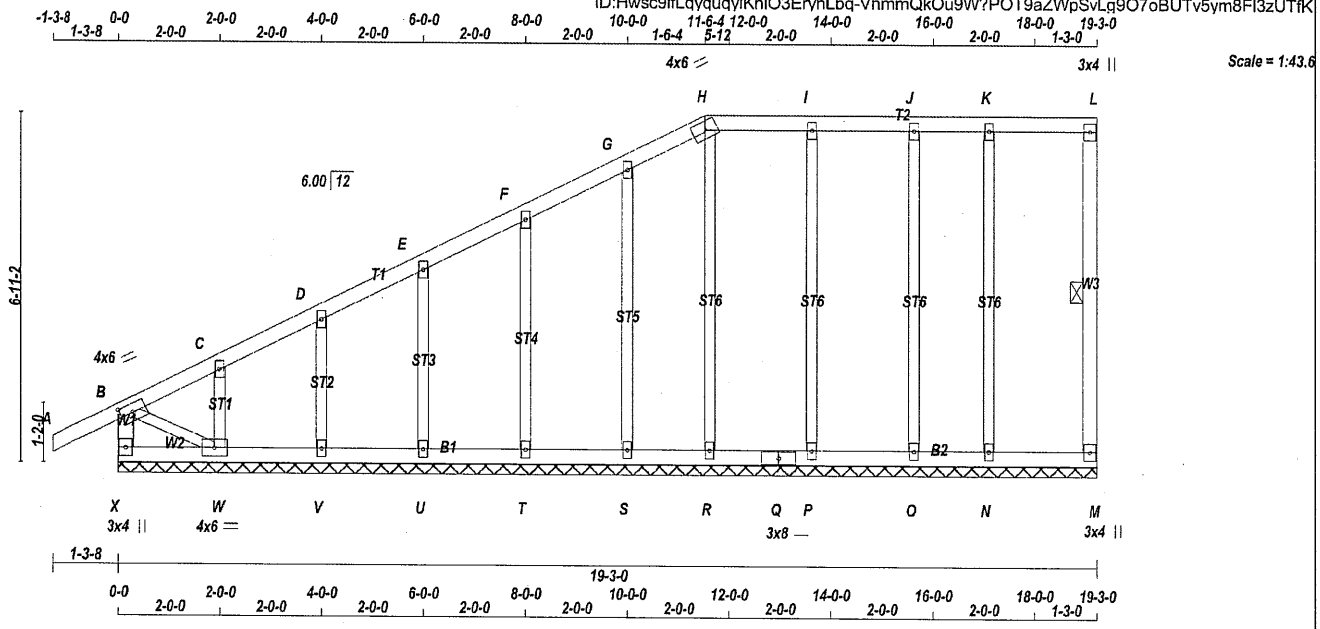

TOTAL WEIGHT = 92 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - B	2x4	DRY	No.2	SPF
X - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J, K						
C	TMW+w	MT20	2.0	4.0		
H	TTW-h	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, R, S, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
W	BMWW1-t	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	W- C	-282 / 0	0.04 (1)
B- C	-21 / 0	-129.6	-129.6	0.07 (1)	6.25	V- D	-250 / 0	0.05 (1)
C- D	-24 / 0	-129.6	-129.6	0.07 (1)	6.25	U- E	-258 / 0	0.07 (1)
D- E	-15 / 0	-129.6	-129.6	0.06 (1)	6.25	T- F	-260 / 0	0.10 (1)
E- F	-10 / 0	-129.6	-129.6	0.06 (1)	6.25	S- G	-254 / 0	0.15 (1)
F- G	-7 / 0	-129.6	-129.6	0.06 (1)	10.00	R- H	-186 / 0	0.15 (1)
G- H	-15 / 0	-129.6	-129.6	0.05 (1)	6.25	P- I	-304 / 0	0.25 (1)
H- I	0 / 0	-129.6	-129.6	0.08 (1)	10.00	O- J	-205 / 0	0.17 (1)
I- J	0 / 0	-129.6	-129.6	0.08 (1)	10.00	N- K	-265 / 0	0.22 (1)
J- K	0 / 0	-129.6	-129.6	0.06 (1)	10.00	B- W	0 / 32	0.01 (1)
K- L	0 / 0	-129.6	-129.6	0.07 (1)	10.00			
M- L	-118 / 0	0.0	0.0	0.03 (1)	6.25			
X- B	-302 / 0	0.0	0.0	0.03 (1)	7.81			
X- W	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
W- V	0 / 19	-18.5	-18.5	0.02 (4)	10.00			
V- U	0 / 14	-18.5	-18.5	0.01 (4)	10.00			
U- T	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
T- S	0 / 6	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 3	-18.5	-18.5	0.01 (4)	10.00			
R- Q	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (V-W:4) , WB=0.25/1.00 (I-P:1) , SSI=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.12 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040202

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T15	20	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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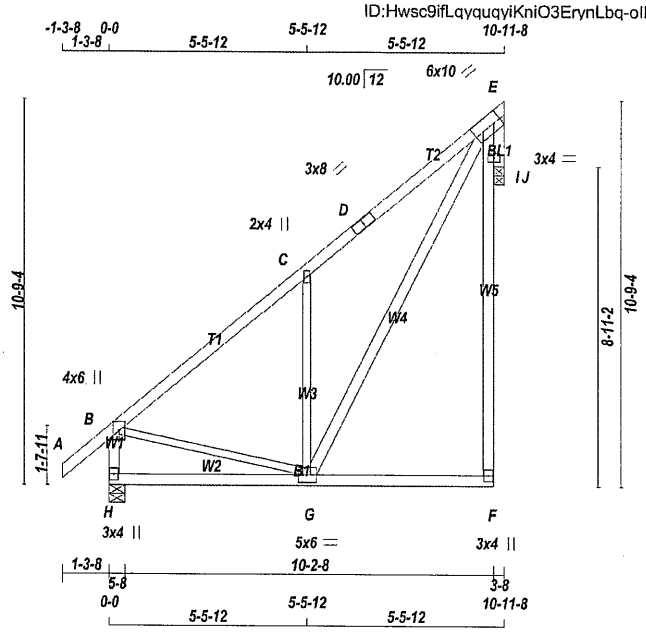

Scale = 1:61.5

TOTAL WEIGHT = 20 X 66 = 1320 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J(E)	779	0	779	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	980	0	980	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	543	402 / 0	0 / 0	0 / 0	0 / 0	141 / 0	0 / 0
H	680	519 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-547 / 0	-129.6	-129.6	0.48 (1)	6.25
C- D	-622 / 0	-129.6	-129.6	0.48 (1)	6.25
D- E	-622 / 0	-129.6	-129.6	0.48 (1)	6.25
F- I	0 / 39	0.0	0.0	0.14 (1)	10.00
I- E	0 / 39	0.0	0.0	0.14 (1)	10.00
H- B	-938 / 0	0.0	0.0	0.10 (1)	7.81
H- G	0 / 0	-18.5	-18.5	0.15 (4)	10.00
G- F	0 / 11	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-839 / 0	0.51 (1)
G- E	0 / 949	0.15 (1)
B- G	0 / 474	0.11 (1)
E- J	-784 / 0	0.09 (1)
I- J	0 / 80	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.48/1.00 (B-C:1) , BC=0.15/1.00 (F-G:4) , WB=0.51/1.00 (C-G:1) , SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (G) (INPUT = 0.90)
JSI METAL= 0.45 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040231

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G16	4	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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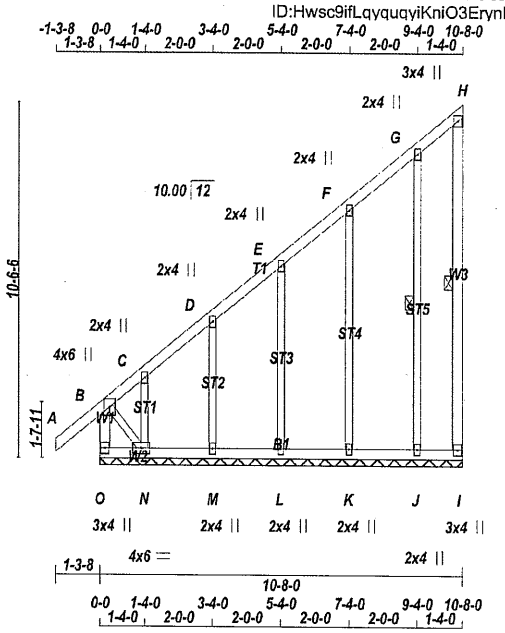

TOTAL WEIGHT = 4 X 66 = 266 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F, G						
C	TMW+w	MT20	2.0	4.0		
H	TMV+p	MT20	3.0	4.0		
I	BMV1+p	MT20	3.0	4.0		
J, K, L, M						
J	BMW1+w	MT20	2.0	4.0		
N	BMWW1-t	MT20	4.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-I, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O- B	-386 / 0	0.0	0.0	0.04 (1)	7.81	L- E	-250 / 0	0.15 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	M- D	-275 / 0	0.08 (1)
B- C	-83 / 0	-129.6	-129.6	0.17 (1)	6.25	N- C	-101 / 0	0.02 (1)
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25	K- F	-266 / 0	0.30 (1)
D- E	-15 / 0	-129.6	-129.6	0.07 (1)	6.25	J- G	-234 / 0	0.14 (1)
E- F	-6 / 0	-129.6	-129.6	0.06 (1)	10.00	B- N	0 / 28	0.01 (1)
F- G	-8 / 0	-129.6	-129.6	0.06 (1)	10.00			
G- H	-14 / 0	-129.6	-129.6	0.04 (1)	6.25			
I- H	-71 / 0	0.0	0.0	0.04 (1)	6.25			
O- N	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
N- M	0 / 16	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 10	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 6	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 2	-18.5	-18.5	0.02 (4)	10.00			
J- I	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (M-N:4) , WB=0.30/1.00 (F-K:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.15 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040203

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T17	10	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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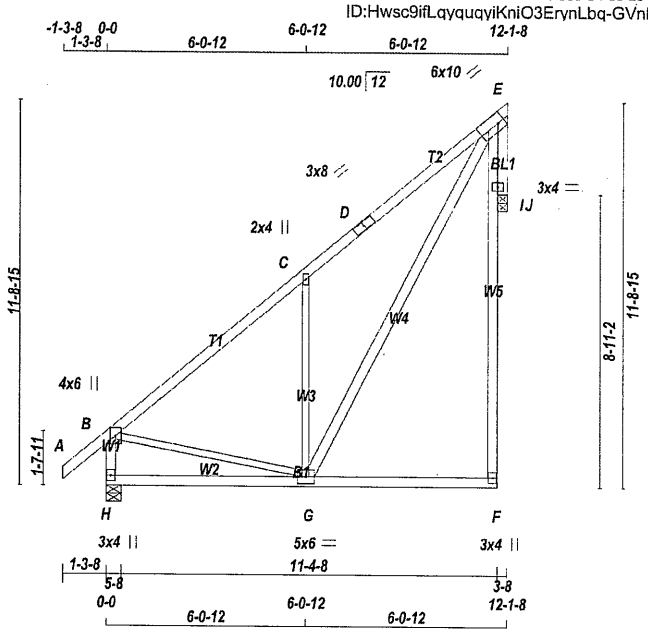

Scale = 1:67.2

TOTAL WEIGHT = 10 X 73 = 734 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMVWK1-t	MT20	6.0	10.0	Edge	2.50
F	BMV+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		
I	KP-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	BRG IN-SX
J(E)	866	0	866	0	0	3-8	3-8 (** SEE "BEARING NOTE" **)
H	1067	0	1067	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J(E)	603	447 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
H	741	564 / 0	0 / 0	0 / 0	0 / 0	177 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J(E), H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00	
B- C	-622 / 0	-129.6	-129.6	0.60 (1)	6.25	
C- D	-708 / 0	-129.6	-129.6	0.60 (1)	6.05	
D- E	-708 / 0	-129.6	-129.6	0.60 (1)	6.05	
F- I	0 / 44	0.0	0.0	0.14 (1)	10.00	
I- E	0 / 44	0.0	0.0	0.14 (1)	10.00	
H- B	-1021 / 0	0.0	0.0	0.11 (1)	7.75	
H- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00	
G- F	0 / 11	-18.5	-18.5	0.18 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-933 / 0	0.69 (1)
G- E	0 / 1070	0.17 (1)
B- G	0 / 535	0.12 (1)
E- J	-867 / 0	0.13 (1)
I- J	0 / 53	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.18/1.00 (F-G:4) , WB=0.69/1.00 (C-G:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (G) (INPUT = 0.90)
JSI METAL= 0.50 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040232

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G18	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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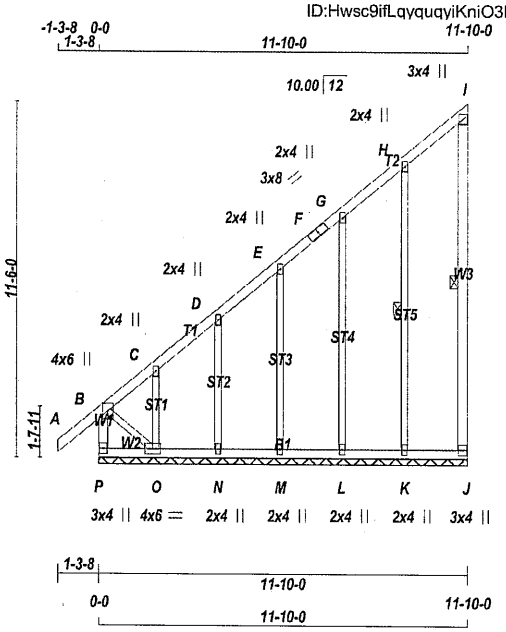

Scale = 1:71.2

TOTAL WEIGHT = 2 X 73 = 145 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, G, H						
C	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
I	TMV+p	MT20	3.0	4.0		
J	BMV1+p	MT20	3.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF I-J, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- B	-386 / 0	0.0	0.0	0.04 (1)	7.81
A- B	0 / 57	-129.6	-129.6	0.18 (1)	10.00
B- C	-67 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	-10 / 0	-129.6	-129.6	0.07 (1)	6.25
D- E	-15 / 0	-129.6	-129.6	0.07 (1)	6.25
E- F	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
F- G	-8 / 0	-129.6	-129.6	0.06 (1)	10.00
G- H	-1 / 0	-129.6	-129.6	0.07 (1)	10.00
H- I	-15 / 0	-129.6	-129.6	0.07 (1)	6.25
J- I	-112 / 0	0.0	0.0	0.08 (1)	6.25
P- O	0 / 0	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 16	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 10	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 6	-18.5	-18.5	0.01 (4)	10.00
L- K	0 / 3	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- H	-291 / 0	0.19 (1)
L- G	-253 / 0	0.33 (1)
M- E	-253 / 0	0.17 (1)
N- D	-273 / 0	0.09 (1)
O- C	-161 / 0	0.03 (1)
B- O	0 / 26	0.01 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (A-B:1) , BC=0.02/1.00 (K-L:4) , WB=0.33/1.00 (G-L:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.15 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040204

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T19	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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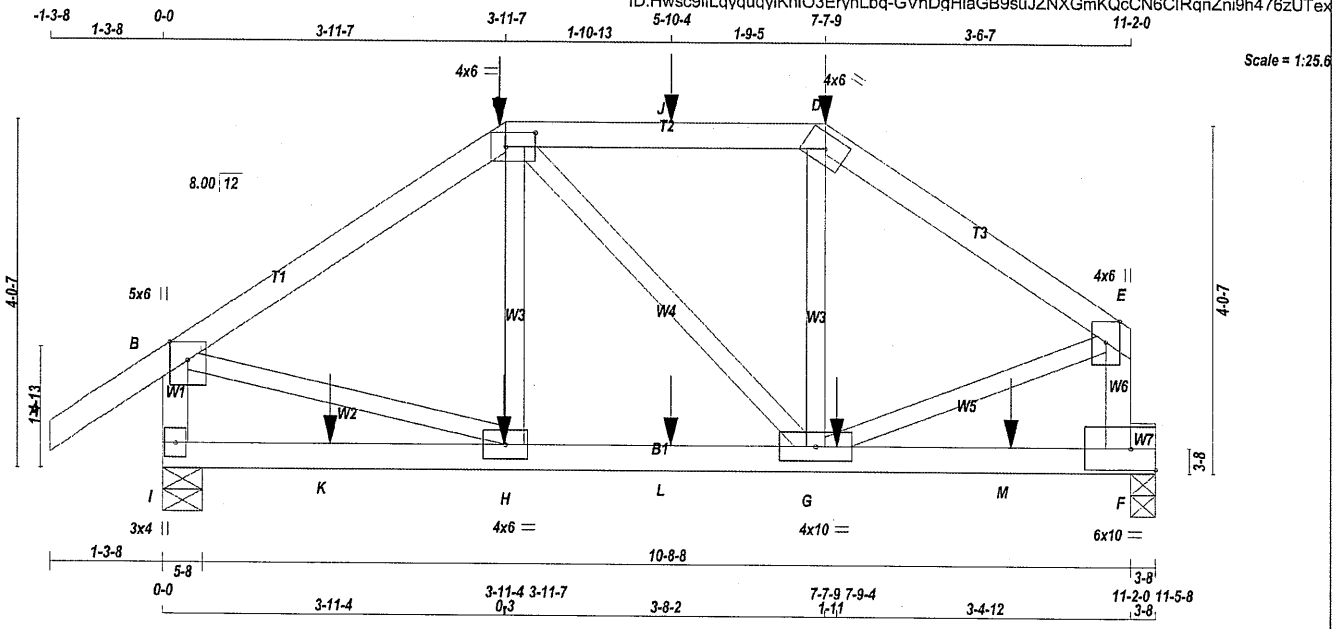


TOTAL WEIGHT = 48 lb

LUMBER
N. L. G. A. RULES

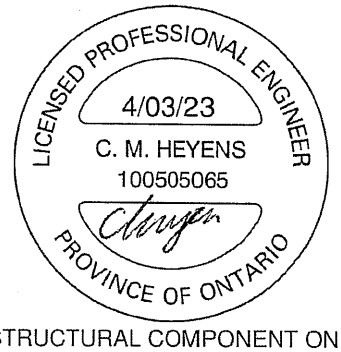
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
F	BMVW1-t	MT20	6.0	10.0	3.00	Edge
G	BMWWW-t	MT20	4.0	10.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040233

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1268	0	1268	0	0	5-8	5-8
F	1111	0	1111	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	880	671 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
F	773	577 / 0	0 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	H- C	-134 / 50	0.04 (1)
B- C	-1026 / 0	-129.6	-129.6	0.41 (1)	5.57	C- G	-59 / 0	0.03 (1)
C- J	-808 / 0	-129.6	-129.6	0.42 (1)	6.05	G- D	-176 / 34	0.05 (1)
J- D	-808 / 0	-129.6	-129.6	0.42 (1)	6.05	B- H	0 / 882	0.22 (1)
D- E	-978 / 0	-129.6	-129.6	0.33 (1)	5.82	G- E	0 / 867	0.21 (1)
I- B	-1232 / 0	0.0	0.0	0.14 (1)	7.19			
F- E	-1077 / 0	0.0	0.0	0.13 (1)	7.57			
I- K	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
K- H	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
H- L	0 / 848	-18.5	-18.5	0.19 (1)	10.00			
L- G	0 / 848	-18.5	-18.5	0.19 (1)	10.00			
G- M	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
M- F	0 / 0	-18.5	-18.5	0.07 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-45	-45	---	BACK	VERT	TOTAL	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	7-7-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	7-7-9	-44	-44	---	BACK	VERT	TOTAL	---	C1
D	7-7-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
G	7-9-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
H	3-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
J	5-10-4	-39	-39	---	BACK	VERT	TOTAL	---	C1
K	1-11-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
L	5-10-4	-6	-6	---	BACK	VERT	TOTAL	---	C1
M	9-9-4	-6	-6	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.42/1.00 (C-D:1) , BC=0.19/1.00 (G-H:1) , WB=0.22/1.00 (B-H:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (E) (INPUT = 0.90)
JSI METAL= 0.51 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T20	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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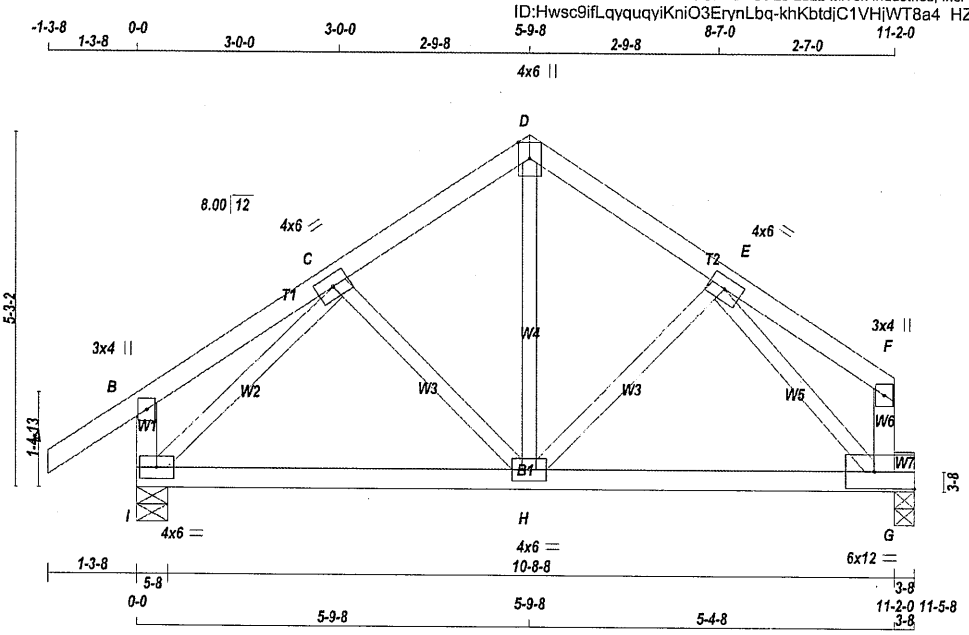

TOTAL WEIGHT = 49 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0		Edge
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVWW1-t	MT20	6.0	12.0	3.00	Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	1005	0	1005	0	0	5-8	5-8
G	827	0	827	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	698	532 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0
G	576	427 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-193 / 0	0.06 (1)
B- C	0 / 23	-129.6	-129.6	0.17 (1)	10.00	H- D	0 / 339	0.08 (1)
C- D	-599 / 0	-129.6	-129.6	0.13 (1)	6.25	H- E	-104 / 13	0.03 (1)
D- E	-596 / 0	-129.6	-129.6	0.12 (1)	6.25	I- C	-886 / 0	0.25 (1)
E- F	0 / 23	-129.6	-129.6	0.14 (1)	10.00	E- G	-869 / 0	0.22 (1)
I- B	-323 / 0	0.0	0.0	0.03 (1)	7.81			
G- F	-118 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 614	-18.5	-18.5	0.20 (4)	10.00			
H- G	0 / 551	-18.5	-18.5	0.20 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.20/1.00 (H-I:4) , WB=0.25/1.00 (C-I:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.55 (E) (INPUT = 0.90)
JSI METAL= 0.19 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040234

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T21	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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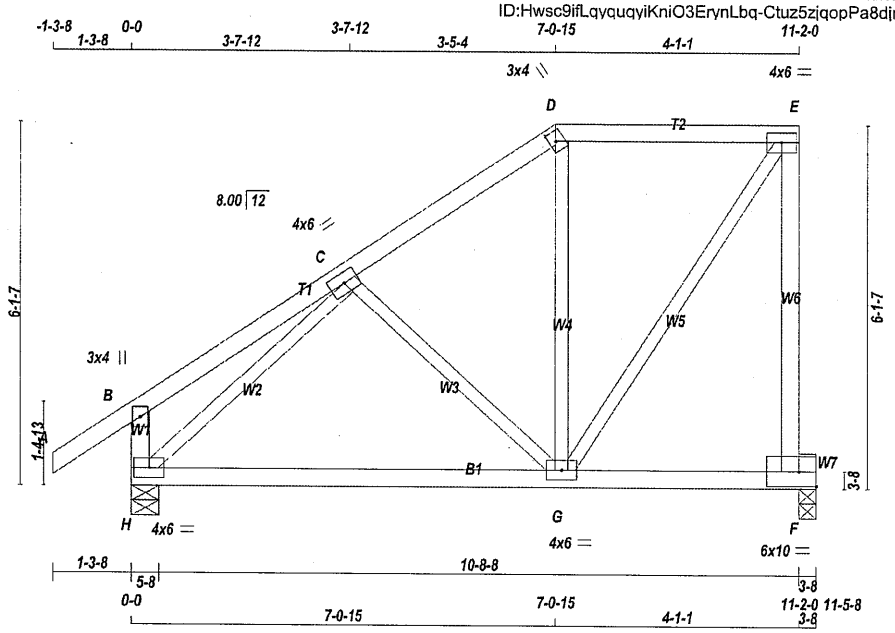

TOTAL WEIGHT = 55 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	6.0	10.0	3.00	Edge
G	BMWWW-t	MT20	4.0	6.0		
H	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	827	0	827	0	0	3-8	3-8
H	1005	0	1005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	576	427 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
H	698	532 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- G	-325 / 0	0.14 (1)	
B- C	0 / 30	-129.6	-129.6	0.27 (1)	10.00	G- D	-189 / 15	0.11 (1)	
C- D	-484 / 0	-129.6	-129.6	0.20 (1)	6.25	G- E	0 / 662	0.15 (1)	
D- E	-374 / 0	-129.6	-129.6	0.28 (1)	6.25	H- C	-857 / 0	0.33 (1)	
F- E	-810 / 0	0.0	0.0	0.59 (1)	7.81				
H- B	-352 / 0	0.0	0.0	0.04 (1)	7.81				
H- G	0 / 614	-18.5	-18.5	0.24 (4)	10.00				
G- F	0 / 0	-18.5	-18.5	0.20 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.59/1.00 (E-F:1) , BC=0.24/1.00 (G-H:4) , WB=0.33/1.00 (C-H:1) , SSI=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (G) (INPUT = 0.90)
JSI METAL= 0.20 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040235

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T22	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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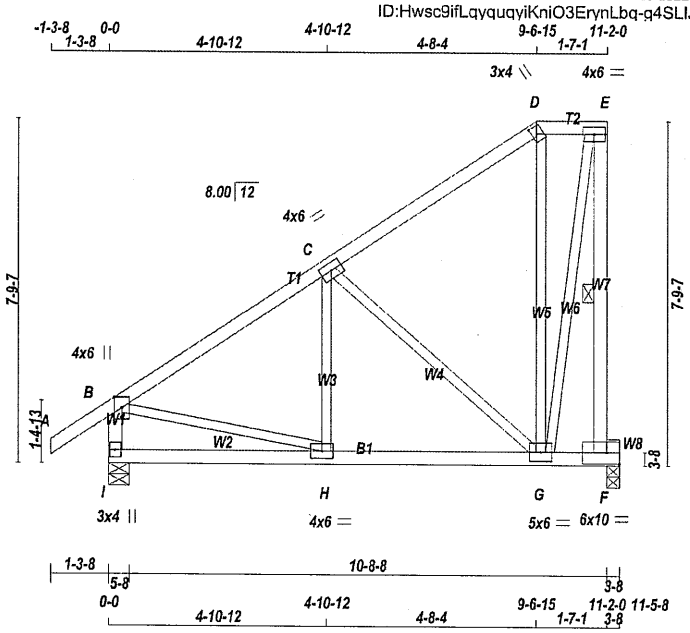

TOTAL WEIGHT = 64 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
F	BMVW1-t	MT20	6.0	10.0	3.00	Edge
G	BMWWW-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	827	0	827	0	0	3-8	3-8
I	1005	0	1005	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	576	427 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
I	698	532 / 0	0 / 0	0 / 0	0 / 0	166 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-701 / 0	-129.6	-129.6	0.39 (1)	6.25
C- D	-216 / 0	-129.6	-129.6	0.38 (1)	6.25
D- E	-140 / 0	-129.6	-129.6	0.04 (1)	6.25
F- E	-811 / 0	0.0	0.0	0.22 (1)	6.25
I- B	-969 / 0	0.0	0.0	0.10 (1)	7.81
I- H	0 / 0	-18.5	-18.5	0.11 (4)	10.00
H- G	0 / 618	-18.5	-18.5	0.16 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.04 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-28 / 84	0.03 (4)
C- G	-640 / 0	0.48 (1)
G- D	-234 / 0	0.26 (1)
G- E	0 / 723	0.16 (1)
B- H	0 / 633	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.16/1.00 (G-H:1) , WB=0.48/1.00 (C-G:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (G) (INPUT = 0.90)
JSI METAL= 0.45 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040236

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T23A	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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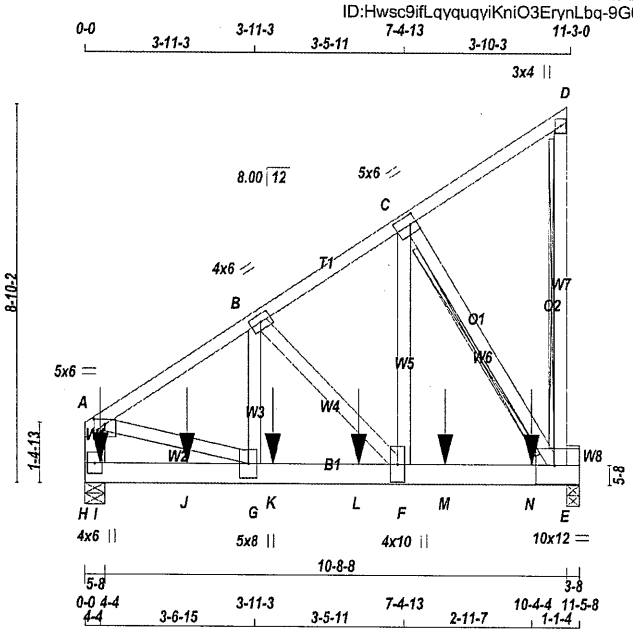

TOTAL WEIGHT = 2 X 74 = 148 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- E	1	12	TOP
H- A	2	2	SIDE(2.5)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	5	SIDE(366.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040237

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	4878	0	4878	0	0	3-8	3-8
H	5229	0	5229	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	3394	2543 / 0	0 / 0	0 / 0	0 / 0	851 / 0	0 / 0
H	3638	2724 / 0	0 / 0	0 / 0	0 / 0	914 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-E, C-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO			
A- B	-4327 / 0	-129.6	-129.6	0.23 (1)	4.40	A- G		0 / 3730	0.33 (1)
B- C	-2611 / 0	-129.6	-129.6	0.16 (1)	5.46	G- B		0 / 1955	0.17 (1)
C- D	-26 / 0	-129.6	-129.6	0.13 (1)	6.25	B- F		-2074 / 0	0.30 (1)
E- D	-197 / 0	0.0	0.0	0.06 (1)	7.81	F- C		0 / 4357	0.39 (1)
H- A	-3549 / 0	0.0	0.0	0.13 (1)	7.44	C- E		-4067 / 0	0.76 (1)
H- I	0 / 0	-18.5	-18.5	0.40 (1)	10.00				
I- J	0 / 0	-18.5	-18.5	0.40 (1)	10.00				
J- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00				
G- K	0 / 3615	-18.5	-18.5	0.58 (1)	10.00				
K- L	0 / 3615	-18.5	-18.5	0.58 (1)	10.00				
L- F	0 / 3615	-18.5	-18.5	0.58 (1)	10.00				
F- M	0 / 2179	-18.5	-18.5	0.51 (1)	10.00				
M- N	0 / 2179	-18.5	-18.5	0.51 (1)	10.00				
N- E	0 / 2179	-18.5	-18.5	0.51 (1)	10.00				

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	4-4	-984	-984	---	FRONT	VERT	TOTAL	---	C1
J	2-4-4	-979	-979	---	FRONT	VERT	TOTAL	---	C1
K	4-4-4	-979	-979	---	FRONT	VERT	TOTAL	---	C1
L	6-4-4	-979	-979	---	FRONT	VERT	TOTAL	---	C1
M	8-4-4	-979	-979	---	FRONT	VERT	TOTAL	---	C1
N	10-4-4	-980	-980	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.58/1.00 (F-G:1) , WB=0.76/1.00 (C-E:1) , SSI=0.56/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.65 (G) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T23A	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Hwsc9ifLqyquqyiKniO3ErynLbq-9G0jWfl4KQfINwt9m6qGaSN66pYCmT3MdnfHGtzUTet

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1-t	MT20	10.0	12.0	5.25	5.00
F	BMWW+t	MT20	4.0	10.0		
G	BMWW+t	MT20	5.0	8.0		
H	BMV1+p	MT20	4.0	6.0		

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040237

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T24	3	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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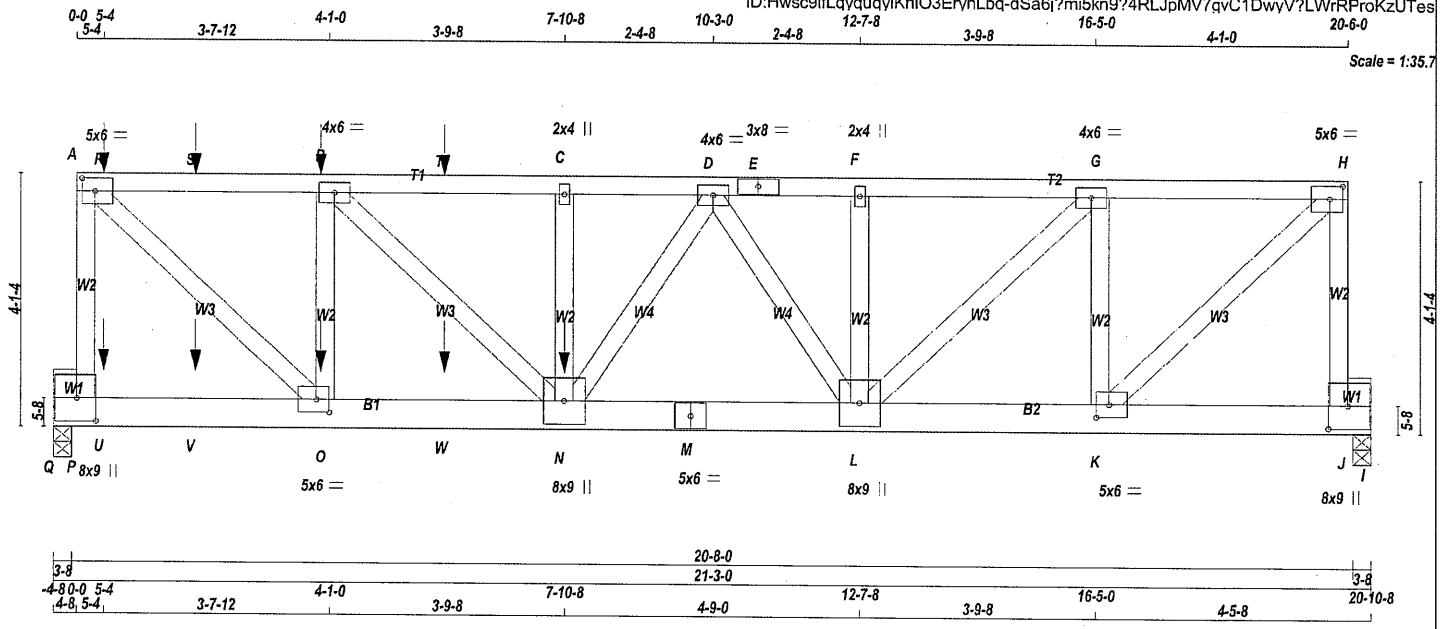


TOTAL WEIGHT = 6 X 117 = 704 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

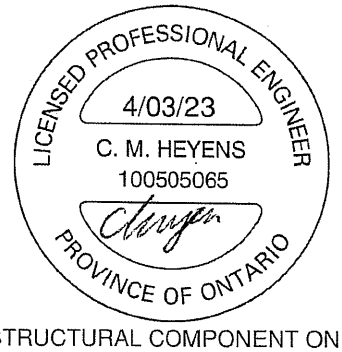
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)		LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
P- A	1	12	TOP
A- E	1	12	SIDE(61.0)
E- H	1	12	TOP
H- J	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	SIDE(183.1)
M- I	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
C- N	1	4	SIDE(381.4)
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.



STRUCTURAL COMPONENT ONLY
DWG # TR23040238

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3497	0	3497	0	0	3-8	3-8
I	2486	0	2486	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2435	1813 / 0	0 / 0	0 / 0	0 / 0	622 / 0	0 / 0
I	1731	1289 / 0	0 / 0	0 / 0	0 / 0	442 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
P- A	-3819 / 0	0.0	0.0	0.48 (1)	6.03	A- O	0 / 4977	0.44 (1)
A- R	-3645 / 0	-129.6	-129.6	0.31 (1)	4.60	O- B	-2723 / 0	0.25 (1)
R- S	-3645 / 0	-129.6	-129.6	0.31 (1)	4.60	B- N	0 / 2501	0.22 (1)
S- B	-3645 / 0	-129.6	-129.6	0.31 (1)	4.60	N- C	-442 / 0	0.04 (1)
B- T	-5445 / 0	-129.6	-129.6	0.38 (1)	3.81	N- D	0 / 950	0.08 (1)
T- C	-5445 / 0	-129.6	-129.6	0.38 (1)	3.81	D- L	-1265 / 0	0.15 (1)
C- D	-5445 / 0	-129.6	-129.6	0.17 (1)	4.05	L- F	-392 / 0	0.04 (1)
D- E	-4255 / 0	-129.6	-129.6	0.12 (1)	4.53	L- G	0 / 2153	0.19 (1)
E- F	-4255 / 0	-129.6	-129.6	0.12 (1)	4.53	K- G	-2089 / 0	0.19 (1)
F- G	-4255 / 0	-129.6	-129.6	0.23 (1)	4.42	K- H	0 / 3695	0.33 (1)
G- H	-2706 / 0	-129.6	-129.6	0.19 (1)	5.35			
J- H	-2724 / 0	0.0	0.0	0.34 (1)	6.92			
Q- P	0 / 0	-18.5	-18.5	0.20 (1)	10.00			
P- U	0 / 0	-18.5	-18.5	0.48 (1)	10.00			
U- V	0 / 0	-18.5	-18.5	0.48 (1)	10.00			
V- O	0 / 0	-18.5	-18.5	0.48 (1)	10.00			
O- W	0 / 3645	-18.5	-18.5	0.35 (1)	10.00			
W- N	0 / 3645	-18.5	-18.5	0.35 (1)	10.00			
N- M	0 / 4935	-18.5	-18.5	0.40 (1)	10.00			
M- L	0 / 4935	-18.5	-18.5	0.40 (1)	10.00			
L- K	0 / 2706	-18.5	-18.5	0.25 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.34 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.14 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-11-4	-106	-106	---	BACK	VERT	TOTAL	---	C1
N	7-10-8	-1502	-1502	---	BACK	VERT	TOTAL	---	C1
O	3-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
R	5-4	-131	-131	---	BACK	VERT	TOTAL	---	C1
S	1-11-4	-106	-106	---	BACK	VERT	TOTAL	---	C1
T	5-11-4	-106	-106	---	BACK	VERT	TOTAL	---	C1
U	5-4	-23	-23	---	BACK	VERT	TOTAL	---	C1
V	1-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1
W	5-11-4	-21	-21	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.48/1.00 (A-P:1) , BC=0.48/1.00 (O-P:1) , WB=0.44/1.00 (A-O:1) , SSI=0.58/1.00 (P-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.51 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T24	3	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.50
B, D, G						
B	TMWW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMW+w	MT20	2.0	4.0		
H	TMVW-t	MT20	5.0	6.0	2.50	2.50
J	BMVW+p	MT20	8.0	9.0	4.50	3.75
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWWW+t	MT20	8.0	9.0		
M	BS-t	MT20	5.0	6.0		
N	BMWWW+t	MT20	8.0	9.0		
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BMVW+p	MT20	8.0	9.0	4.50	3.75

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040238

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T25	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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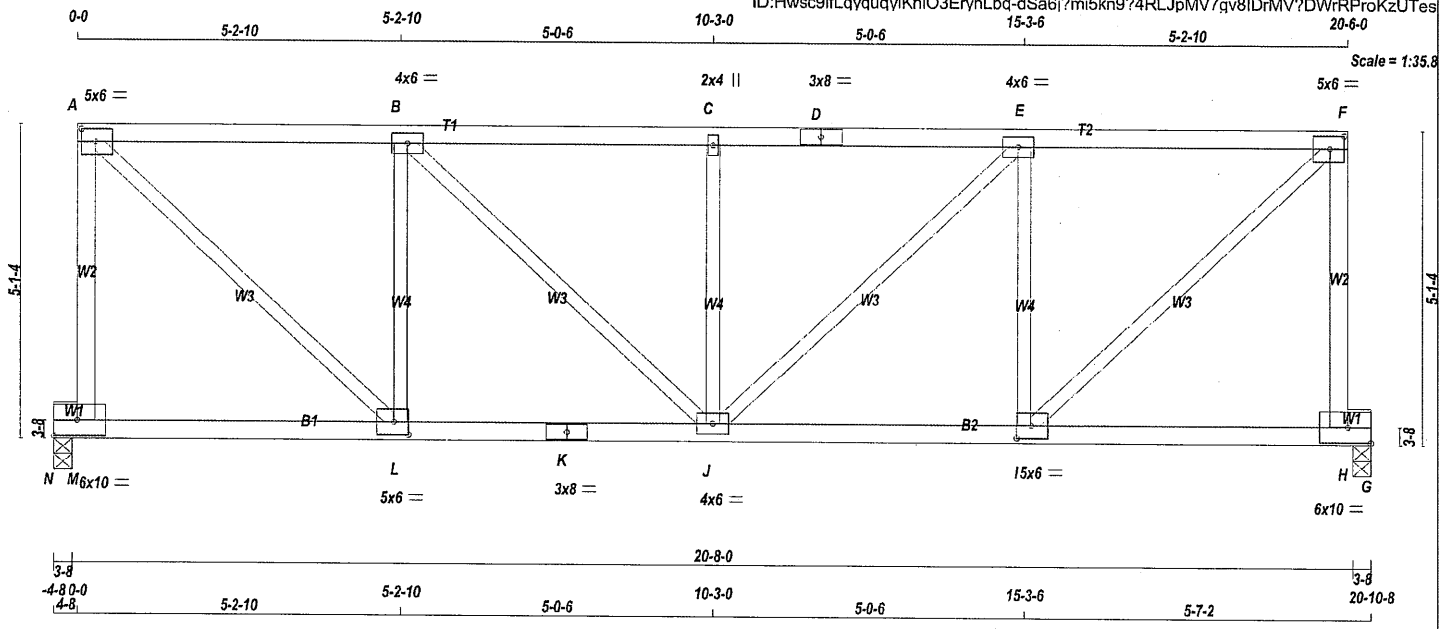

TOTAL WEIGHT = 3 X 91 = 274 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMWW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0	2.50	2.75
H	BMVW-t	MT20	6.0	10.0		Edge
I	BMWW-t	MT20	5.0	6.0	2.50	2.75
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMVW-t	MT20	6.0	10.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1525	0	1525	0	0	3-8	3-8
G	1525	0	1525	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1064	783 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	1064	783 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FROM	FACTORED VERT. LOAD (PLF) TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
M- A	-1619 / 0	0.0	0.0	0.71 (1)	6.50		I- F	0 / 1990	0.45 (1)
A- B	-1463 / 0	-129.6	-129.6	0.56 (1)	4.64		A- L	0 / 1990	0.45 (1)
B- C	-1767 / 0	-129.6	-129.6	0.58 (1)	4.28		I- E	-1052 / 0	0.40 (1)
C- D	-1767 / 0	-129.6	-129.6	0.58 (1)	4.28		L- B	-1052 / 0	0.40 (1)
D- E	-1767 / 0	-129.6	-129.6	0.58 (1)	4.28		J- E	0 / 421	0.09 (1)
E- F	-1463 / 0	-129.6	-129.6	0.56 (1)	4.64		B- J	0 / 421	0.09 (1)
H- F	-1619 / 0	0.0	0.0	0.71 (1)	6.50		J- C	-596 / 0	0.23 (1)
N- M	0 / 0	-18.5	-18.5	0.32 (1)	10.00				
M- L	0 / 0	-18.5	-18.5	0.78 (1)	10.00				
L- K	0 / 1463	-18.5	-18.5	0.52 (1)	10.00				
K- J	0 / 1463	-18.5	-18.5	0.52 (1)	10.00				
J- I	0 / 1463	-18.5	-18.5	0.52 (1)	10.00				
I- H	0 / 0	-18.5	-18.5	0.78 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.32 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 889 (0.29")

CSI: TC=0.71/1.00 (A-M:1) , BC=0.78/1.00 (L-M:1) , WB=0.45/1.00 (F-I:1) , SSI=0.60/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040239

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T26	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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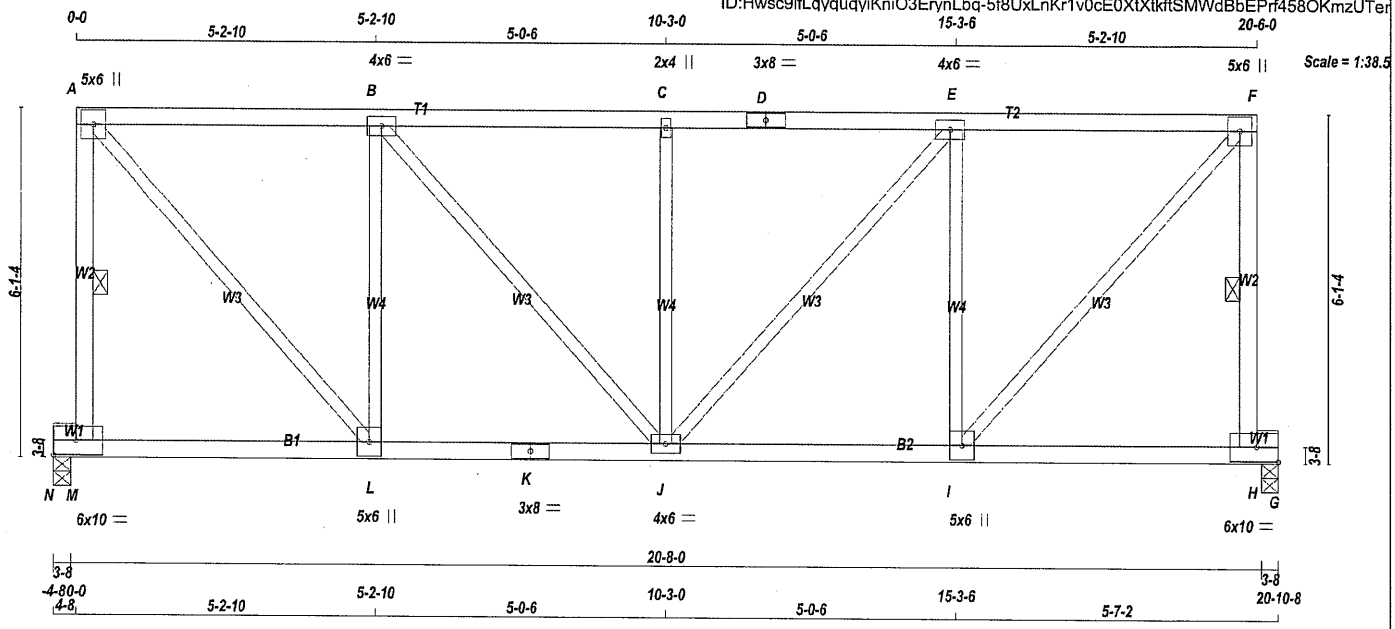

TOTAL WEIGHT = 6 X 98 = 591 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
K - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW+p	MT20	5.0	6.0		
H	BMVW-t	MT20	6.0	10.0	3.00	Edge
I	BMWW+t	MT20	5.0	6.0		
J	BMWWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	8.0		
L	BMWW+t	MT20	5.0	6.0		
M	BMVW-t	MT20	6.0	10.0	3.00	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1525	0	1525	0	0	3-8	3-8
G	1525	0	1525	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1064	783 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0
G	1064	783 / 0	0 / 0	0 / 0	0 / 0	280 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF A-M, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- A	-1620 / 0	0.0	0.0	0.29 (1)	5.20	I- F	0 / 1814	0.41 (1)
A- B	-1212 / 0	-129.6	-129.6	0.54 (1)	5.02	A- L	0 / 1814	0.41 (1)
B- C	-1464 / 0	-129.6	-129.6	0.56 (1)	4.64	I- E	-1052 / 0	0.62 (1)
C- D	-1464 / 0	-129.6	-129.6	0.56 (1)	4.64	L- B	-1052 / 0	0.62 (1)
D- E	-1464 / 0	-129.6	-129.6	0.56 (1)	4.64	J- E	0 / 384	0.09 (1)
E- F	-1212 / 0	-129.6	-129.6	0.54 (1)	5.02	B- J	0 / 384	0.09 (1)
H- F	-1620 / 0	0.0	0.0	0.29 (1)	5.20	J- C	-596 / 0	0.35 (1)
N- M	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
M- L	0 / 0	-18.5	-18.5	0.78 (1)	10.00			
L- K	0 / 1212	-18.5	-18.5	0.48 (1)	10.00			
K- J	0 / 1212	-18.5	-18.5	0.48 (1)	10.00			
J- I	0 / 1212	-18.5	-18.5	0.48 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.78 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 914 (0.28")

CSI: TC=0.56/1.00 (C-E:1) , BC=0.78/1.00 (L-M:1) , WB=0.62/1.00 (E-I:1) , SSI=0.60/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (I) (INPUT = 0.90)
JSI METAL= 0.44 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040240

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T27	3	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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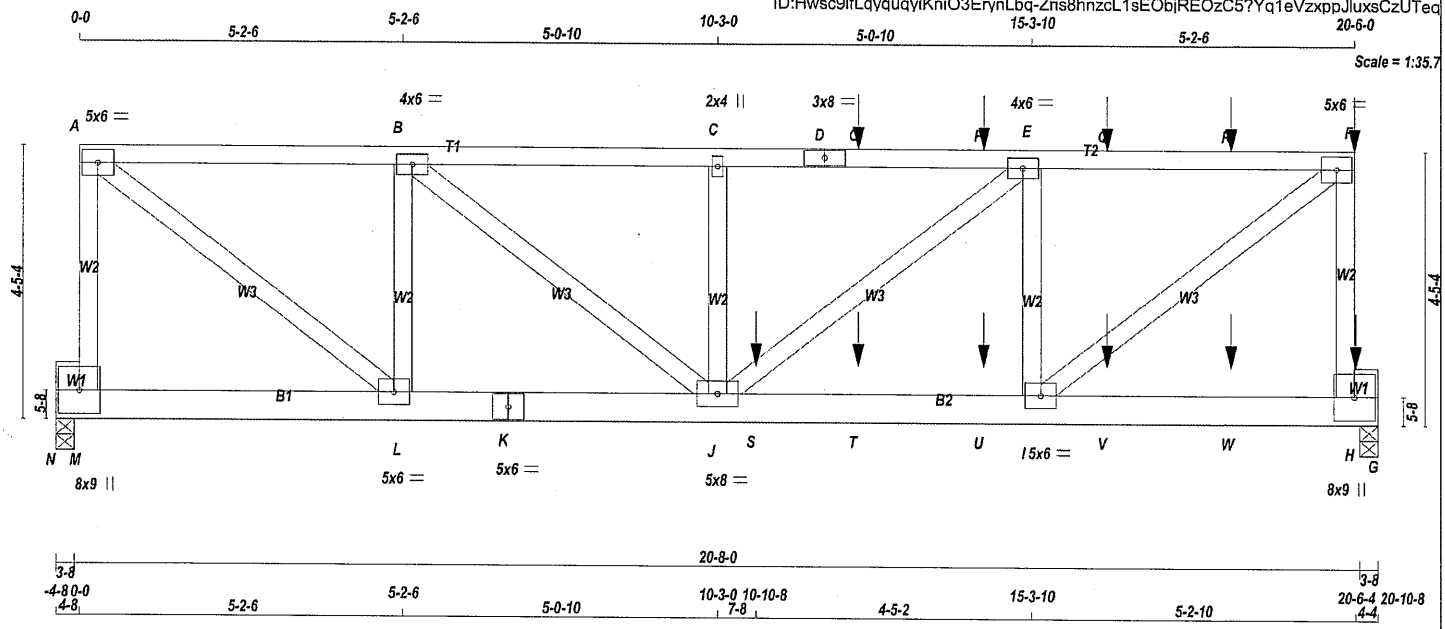


TOTAL WEIGHT = 6 X 111 = 668 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
M - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
K - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

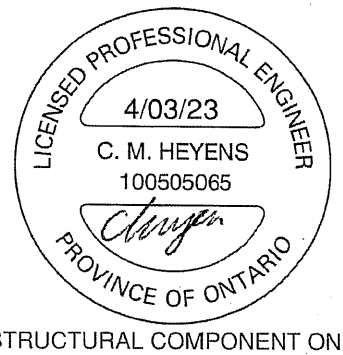
CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
M- A	1	12	TOP
A- D	1	12	TOP
D- F	1	12	SIDE(61.0)
F- H	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- K	2	12	TOP
K- G	2	12	SIDE(122.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.



STRUCTURAL COMPONENT ONLY
DWG # TR23040241

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	2068	0	2068	0	0	3-8	3-8
G	2497	0	2497	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1440	1071 / 0	0 / 0	0 / 0	0 / 0	369 / 0	0 / 0
G	1739	1296 / 0	0 / 0	0 / 0	0 / 0	443 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
M- A	-2195 / 0	0.0	0.0	0.32 (1)	7.52
A- B	-2463 / 0	-129.6	-129.6	0.31 (1)	5.38
B- C	-3542 / 0	-129.6	-129.6	0.34 (1)	4.63
C- D	-3542 / 0	-129.6	-129.6	0.46 (1)	4.46
D- O	-3542 / 0	-129.6	-129.6	0.46 (1)	4.46
O- P	-3542 / 0	-129.6	-129.6	0.46 (1)	4.46
P- E	-3542 / 0	-129.6	-129.6	0.46 (1)	4.46
E- Q	-2793 / 0	-129.6	-129.6	0.43 (1)	4.94
Q- R	-2793 / 0	-129.6	-129.6	0.43 (1)	4.94
R- F	-2793 / 0	-129.6	-129.6	0.43 (1)	4.94
H- F	-2660 / 0	0.0	0.0	0.39 (1)	6.99
N- M	0 / 0	-18.5	-18.5	0.12 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.28 (1)	10.00
L- K	0 / 2463	-18.5	-18.5	0.22 (1)	10.00
K- J	0 / 2463	-18.5	-18.5	0.22 (1)	10.00
J- S	0 / 2793	-18.5	-18.5	0.35 (1)	10.00
S- T	0 / 2793	-18.5	-18.5	0.35 (1)	10.00
T- U	0 / 2793	-18.5	-18.5	0.35 (1)	10.00
U- I	0 / 2793	-18.5	-18.5	0.35 (1)	10.00
I- V	0 / 0	-18.5	-18.5	0.34 (1)	10.00
V- W	0 / 0	-18.5	-18.5	0.34 (1)	10.00
W- H	0 / 0	-18.5	-18.5	0.34 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- F	0 / 3545	0.31 (1)
A- L	0 / 3126	0.28 (1)
I- E	-1632 / 0	0.17 (1)
L- B	-1621 / 0	0.17 (1)
J- E	0 / 961	0.08 (1)
B- J	0 / 1385	0.12 (1)
J- C	-646 / 0	0.07 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	20-6-0	-107	-107	---	BACK	VERT	TOTAL	---	C1
H	20-6-4	-15	-15	---	BACK	VERT	TOTAL	---	C1
O	12-6-4	-62	-62	---	BACK	VERT	TOTAL	---	C1
P	14-6-4	-62	-62	---	BACK	VERT	TOTAL	---	C1
Q	16-6-4	-62	-62	---	BACK	VERT	TOTAL	---	C1
R	18-6-4	-62	-62	---	BACK	VERT	TOTAL	---	C1
S	10-10-8	-640	-640	---	BACK	VERT	TOTAL	---	C1
T	12-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
U	14-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
V	16-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1
W	18-6-4	-10	-10	---	BACK	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.46/1.00 (C-E:1) , BC=0.35/1.00 (I-J:1) , WB=0.31/1.00 (F-I:1) , SSI=0.41/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (A) (INPUT = 0.90)
JSI METAL= 0.35 (A) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	T27	3	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:Hwsc9ifLqyquqyiKniO3ErynLbq-Zris8hnzcL1sEObjREOzC5?Yq1eVzxppJluxsCzUTeq

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		
B	TMWW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
H	BMVW+p	MT20	8.0	9.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMVW+p	MT20	8.0	9.0		

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040241

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T28	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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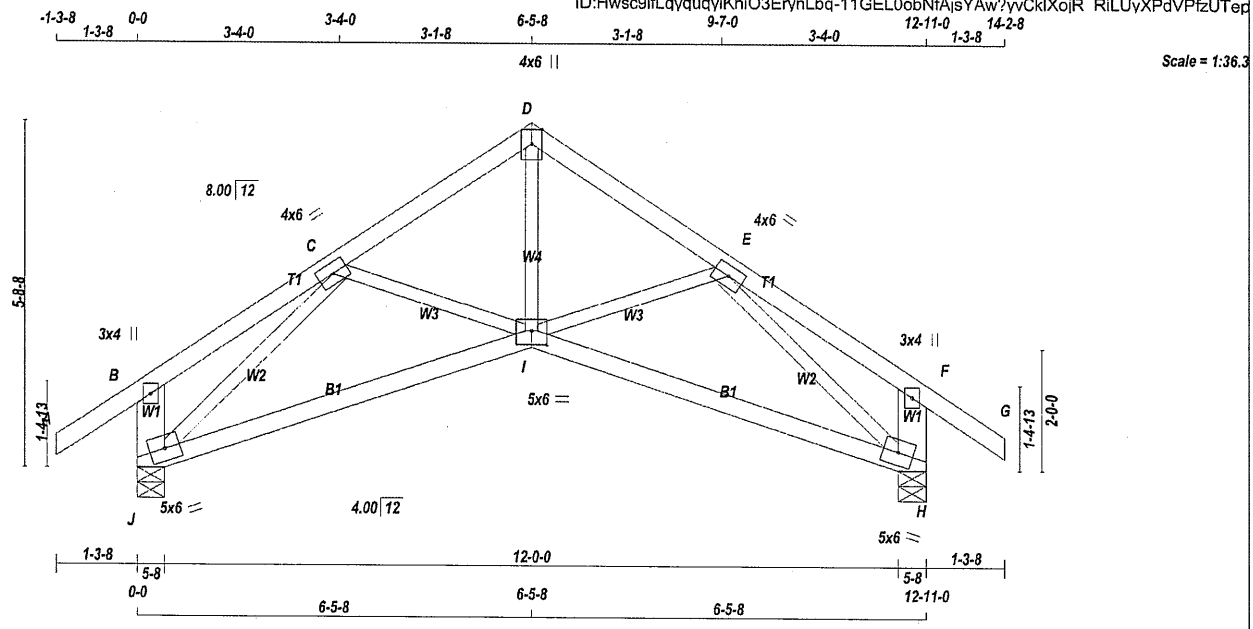

TOTAL WEIGHT = 3 X 56 = 168 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	5.0	6.0		
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1135	0	1135	0	0	5-8	5-8
H	1135	0	1135	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	788	599 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0
H	788	599 / 0	0 / 0	0 / 0	0 / 0	190 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.82 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	I- D	0 / 887	0.20 (1)
B- C	0 / 23	-129.6	-129.6	0.19 (1)	10.00	I- E	-118 / 18	0.03 (1)
C- D	-1123 / 0	-129.6	-129.6	0.16 (1)	5.82	C- I	-118 / 18	0.03 (1)
D- E	-1123 / 0	-129.6	-129.6	0.16 (1)	5.82	J- C	-1483 / 0	0.48 (1)
E- F	0 / 23	-129.6	-129.6	0.19 (1)	10.00	E- H	-1483 / 0	0.48 (1)
F- G	0 / 49	-129.6	-129.6	0.17 (1)	10.00			
J- B	-345 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-345 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 1080	-18.5	-18.5	0.30 (1)	10.00			
I- H	0 / 1080	-18.5	-18.5	0.30 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.19/1.00 (E-F:1) , BC=0.30/1.00 (H-I:1) , WB=0.48/1.00 (C-J:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (J) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040242

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T28A	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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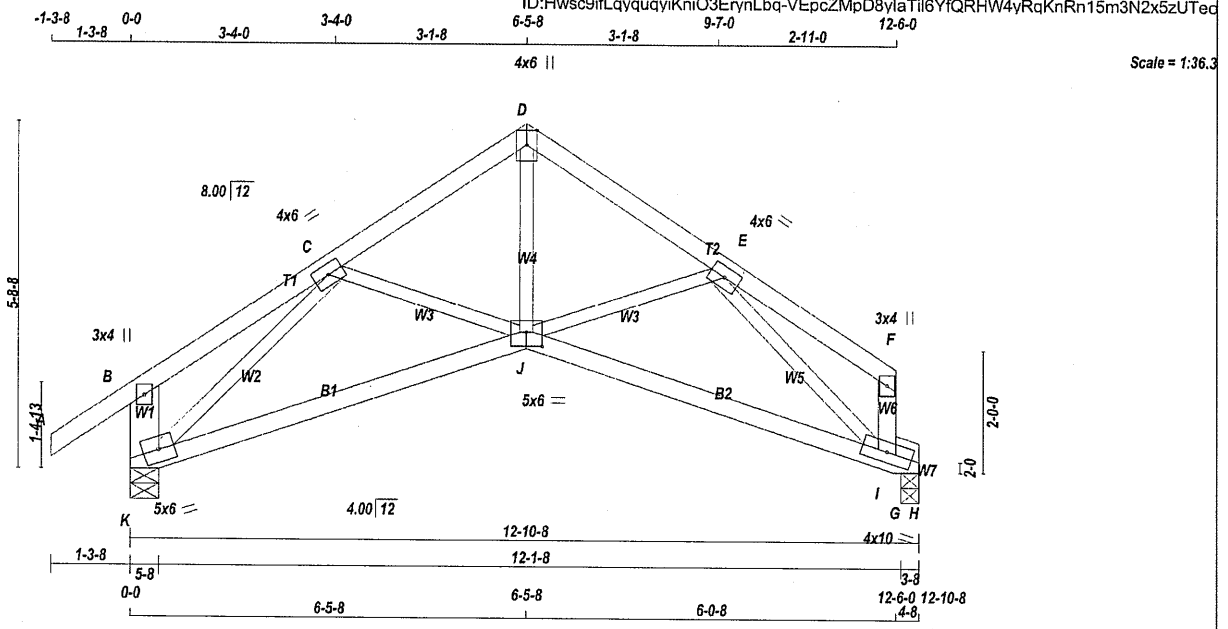

TOTAL WEIGHT = 6 X 53 = 320 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
I	BMVWW-t	MT20	4.0	10.0		
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1104	0	1104	0	0	5-8	5-8
H	983	0	983	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	767	583 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
H	685	506 / 0	0 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.96 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		J- D	0 / 803	0.18 (1)
B- C	0 / 24	-129.6	-129.6	0.19 (1)	10.00		J- E	-40 / 32	0.01 (4)
C- D	-1052 / 0	-129.6	-129.6	0.16 (1)	5.96		C- J	-135 / 14	0.03 (1)
D- E	-1049 / 0	-129.6	-129.6	0.15 (1)	5.97		K- C	-1421 / 0	0.46 (1)
E- F	0 / 24	-129.6	-129.6	0.17 (1)	10.00		E- I	-1362 / 0	0.40 (1)
K- B	-345 / 0	0.0	0.0	0.02 (1)	7.81				
I- F	-140 / 0	0.0	0.0	0.01 (1)	7.81				
K- J	0 / 1035	-18.5	-18.5	0.30 (1)	10.00				
J- I	0 / 942	-18.5	-18.5	0.27 (1)	10.00				
I- G	0 / 637	-153.3	-153.3	0.25 (1)	10.00				
G- H	-983 / 0	0.0	0.0	0.05 (1)	6.25				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 120 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 120 (0.00")

CSI: TC=0.19/1.00 (B-C:1) , BC=0.30/1.00 (J-K:1) , WB=0.46/1.00 (C-K:1) , SSI=0.29/1.00 (G-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (K) (INPUT = 0.90)
JSI METAL= 0.31 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040243

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	T29A	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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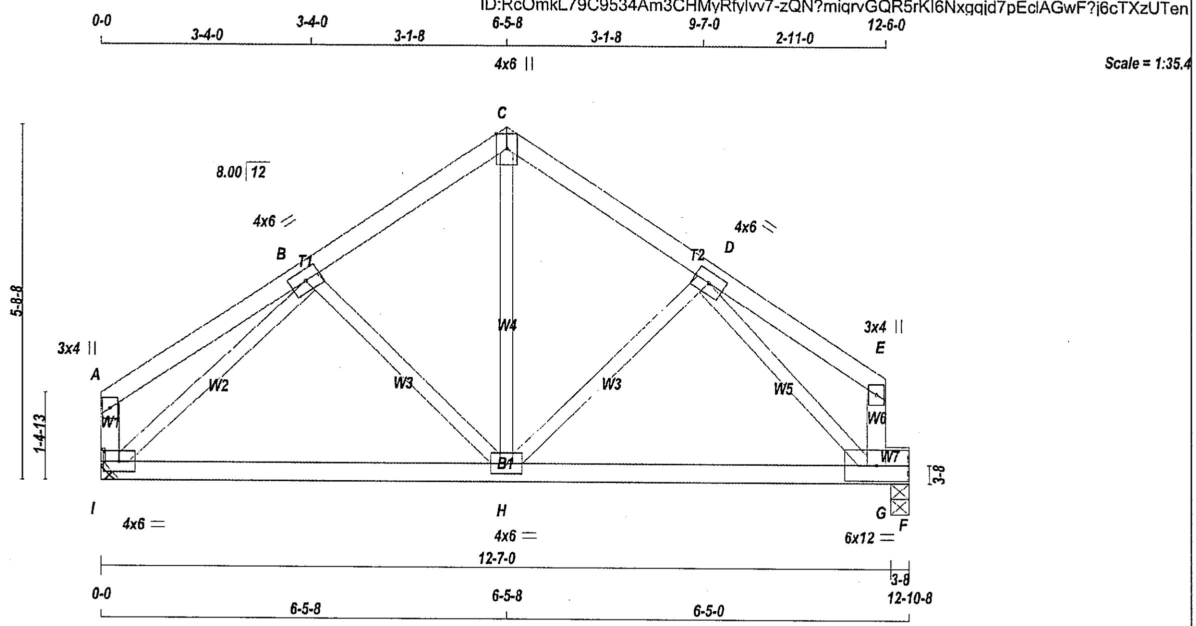

TOTAL WEIGHT = 6 X 53 = 317 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	4.0	6.0		
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	6.0	12.0		
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	906	0	906	0	0	3-8	3-8
I	953	0	953	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
F	632	464 / 0	0 / 0	0 / 0	0 / 0	168 / 0	0 / 0
I	664	492 / 0	0 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-129.6	-129.6	0.21 (1)	10.00
B- C	-746 / 0	-129.6	-129.6	0.22 (1)	6.25
C- D	-743 / 0	-129.6	-129.6	0.19 (1)	6.25
D- E	0 / 26	-129.6	-129.6	0.18 (1)	10.00
I- A	-162 / 0	0.0	0.0	0.02 (1)	7.81
G- E	-134 / 0	0.0	0.0	0.01 (1)	7.81
I- H	0 / 755	-18.5	-18.5	0.34 (1)	10.00
H- G	0 / 687	-18.5	-18.5	0.57 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.19 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- H	-220 / 0	0.08 (1)
H- C	0 / 466	0.10 (1)
H- D	-124 / 13	0.04 (1)
I- B	-1069 / 0	0.35 (1)
D- G	-1048 / 0	0.31 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	38.2	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	0.0	PSF
		DL	=	7.4	PSF
TOTAL	LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 744 (0.21")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.57/1.00 (G-H:1) , WB=0.35/1.00 (B-I:1) , SSI=0.35/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040244

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T30	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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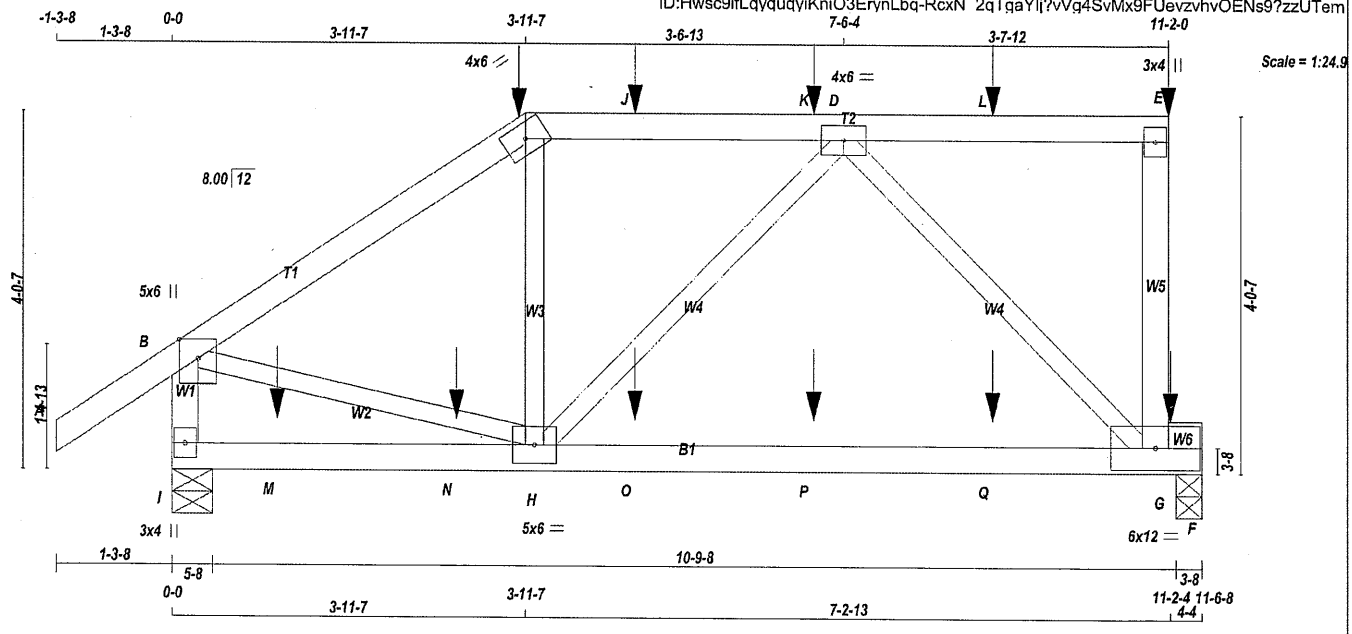

TOTAL WEIGHT = 3 X 48 = 144 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTW-h	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	6.0	12.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040245

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	1229	0	1229	0	0	5-8	5-8
F	1119	0	1119	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	854	648 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0
F	780	578 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	
B- C	-1038 / 0	-129.6	-129.6	0.41 (1)	5.54	
C- J	-860 / 0	-129.6	-129.6	0.38 (1)	6.00	
J- K	-860 / 0	-129.6	-129.6	0.38 (1)	6.00	
K- D	-860 / 0	-129.6	-129.6	0.38 (1)	6.00	
D- L	0 / 0	-129.6	-129.6	0.36 (1)	10.00	
L- E	0 / 0	-129.6	-129.6	0.36 (1)	10.00	
G- E	-319 / 0	0.0	0.0	0.08 (1)	7.81	
I- B	-1238 / 0	0.0	0.0	0.14 (1)	7.18	
I- M	0 / 0	-18.5	-18.5	0.34 (4)	10.00	
M- N	0 / 0	-18.5	-18.5	0.34 (4)	10.00	
N- H	0 / 0	-18.5	-18.5	0.34 (4)	10.00	
H- O	0 / 775	-18.5	-18.5	0.76 (1)	10.00	
O- P	0 / 775	-18.5	-18.5	0.76 (1)	10.00	
P- Q	0 / 775	-18.5	-18.5	0.76 (1)	10.00	
Q- G	0 / 775	-18.5	-18.5	0.76 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.26 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-48 / 73	0.03 (4)
H- D	0 / 127	0.04 (4)
D- G	-1110 / 0	0.50 (1)
B- H	0 / 891	0.22 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
E	11-2-0	-84	-84	---	FRONT	VERT	TOTAL	---	C1
G	11-2-4	-11	-11	---	FRONT	VERT	TOTAL	---	C1
J	5-2-4	-39	-39	---	FRONT	VERT	TOTAL	---	C1
K	7-2-4	-39	-39	---	FRONT	VERT	TOTAL	---	C1
L	9-2-4	-39	-39	---	FRONT	VERT	TOTAL	---	C1
M	1-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	3-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
O	5-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
P	7-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
Q	9-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 891 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 403 (0.34")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.76/1.00 (G-H:1) , WB=0.50/1.00 (D-G:1) , SSI=0.48/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	T30	3	1	TRUSS DESC.	

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040245

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T31	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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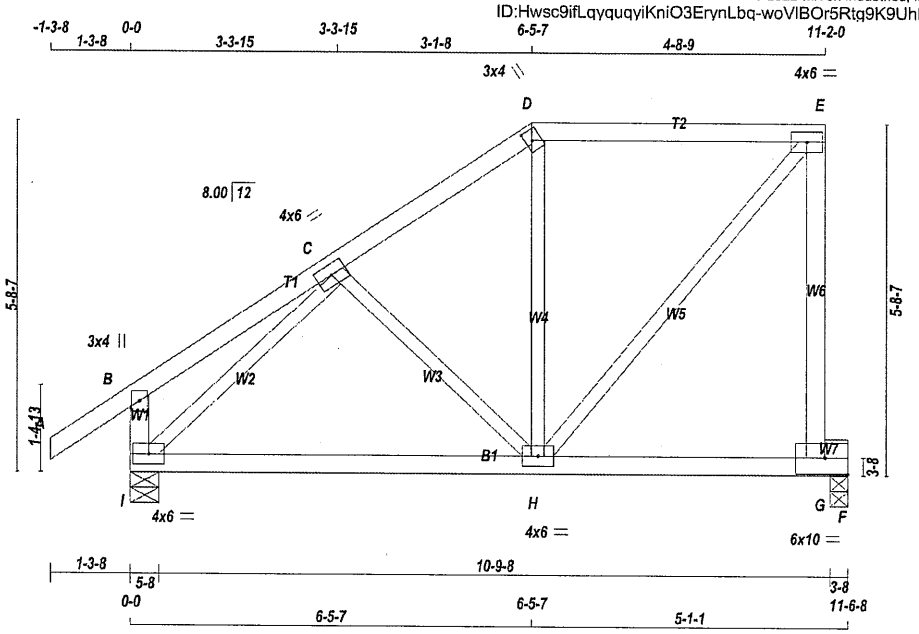

TOTAL WEIGHT = 3 X 53 = 160 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.25	Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	807	0	807	0	0	3-8	3-8
I	1032	0	1032	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0
I	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-254 / 0		0.09 (1)
B- C	0 / 27	-129.6	-129.6	0.22 (1)	10.00	H- D	-154 / 27		0.08 (1)
C- D	-592 / 0	-129.6	-129.6	0.22 (1)	6.25	H- E	0 / 719		0.16 (1)
D- E	-466 / 0	-129.6	-129.6	0.50 (1)	6.25	I- C	-923 / 0		0.31 (1)
G- E	-851 / 0	0.0	0.0	0.51 (1)	7.81				
I- B	-338 / 0	0.0	0.0	0.03 (1)	7.81				
I- H	0 / 652	-18.5	-18.5	0.28 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.41 (1)	10.00				
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.51/1.00 (E-G:1) , BC=0.41/1.00 (G-H:1) , WB=0.31/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (H) (INPUT = 0.90)
JSI METAL= 0.21 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040246

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T32	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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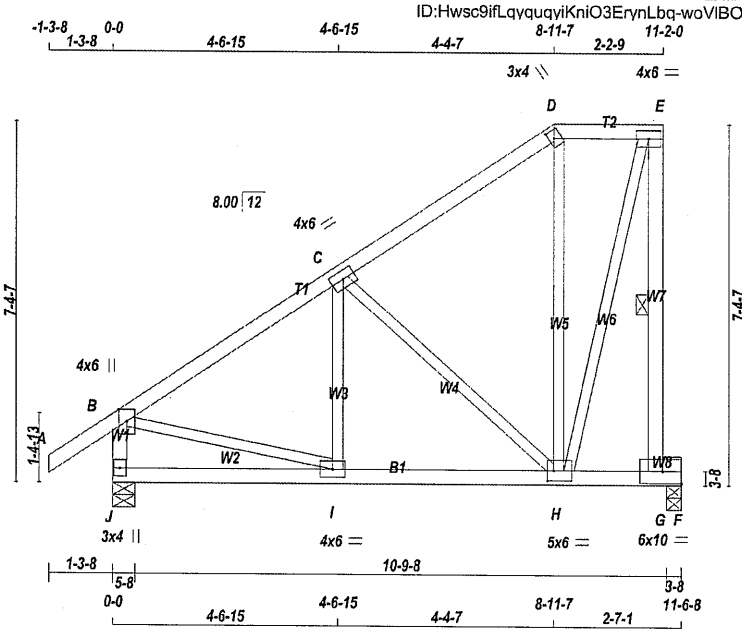

TOTAL WEIGHT = 3 X 62 = 185 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0	3.00	4.50
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1032	0	1032	0	0	5-8	5-8
F	807	0	807	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
F	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	-746 / 0	-129.6	-129.6	0.46 (1)	6.20	
C- D	-333 / 0	-129.6	-129.6	0.44 (1)	6.25	
D- E	-240 / 0	-129.6	-129.6	0.11 (1)	6.25	
G- E	-949 / 0	0.0	0.0	0.23 (1)	6.25	
J- B	-995 / 0	0.0	0.0	0.10 (1)	7.81	
J- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00	
I- H	0 / 653	-18.5	-18.5	0.18 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.41 (1)	10.00	
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI
I- C	-72 / 67	0.02 (4)
C- H	-558 / 0	0.36 (1)
H- D	-193 / 0	0.19 (1)
H- E	0 / 842	0.19 (1)
B- I	0 / 671	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.46/1.00 (B-C:1) , BC=0.41/1.00 (G-H:1) , WB=0.36/1.00 (C-H:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040247

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T33	3	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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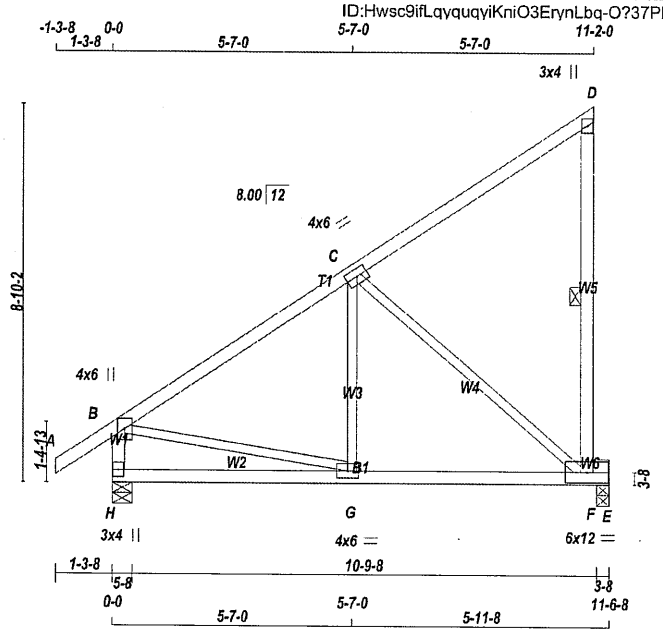

Scale = 1:51.6

TOTAL WEIGHT = 3 X 55 = 165 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1032	0	1032	0	0	5-8	5-8
E	807	0	807	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	717	546 / 0	0 / 0	0 / 0	0 / 0	171 / 0	0 / 0
E	563	413 / 0	0 / 0	0 / 0	0 / 0	150 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G- C	0 / 133	0.05 (4)
B- C	-725 / 0	-129.6	-129.6	0.73 (1)	5.55	C- F	-850 / 0	0.86 (1)
C- D	-49 / 0	-129.6	-129.6	0.68 (1)	6.25	B- G	0 / 656	0.15 (1)
F- D	-273 / 0	0.0	0.0	0.10 (1)	6.25			
H- B	-1005 / 0	0.0	0.0	0.10 (1)	7.80			
H- G	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
G- F	0 / 644	-18.5	-18.5	0.51 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 856 (0.16")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.51/1.00 (F-G:1) , WB=0.86/1.00 (C-F:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (B) (INPUT = 0.90)
JSI METAL= 0.47 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040248

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T33A	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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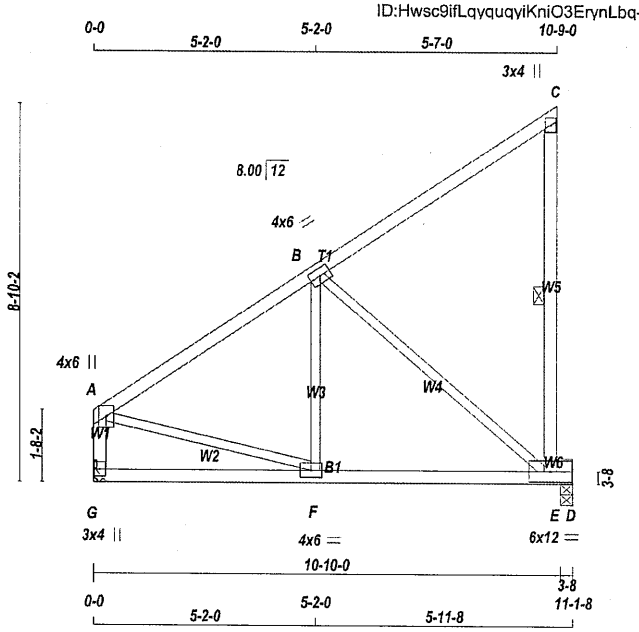

Scale = 1:51.6

TOTAL WEIGHT = 6 X 52 = 313 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	6.0	12.0		
F	BMWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	823	0	823	0	0	MECHANICAL	
D	776	0	776	0	0	3-8	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	574	425 / 0	0 / 0	0 / 0	0 / 0	149 / 0	0 / 0
D	542	397 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-669 / 0	-129.6	-129.6	0.67 (1)	5.91	F- B	0 / 119	0.04 (4)
B- C	-46 / 0	-129.6	-129.6	0.63 (1)	6.25	B- E	-789 / 0	0.80 (1)
E- C	-279 / 0	0.0	0.0	0.10 (1)	6.25	A- F	0 / 618	0.14 (1)
G- A	-801 / 0	0.0	0.0	0.09 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
F- E	0 / 598	-18.5	-18.5	0.49 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 858 (0.16")

CSI: TC=0.67/1.00 (A-B:1) , BC=0.49/1.00 (E-F:1) , WB=0.80/1.00 (B-E:1) , SSI=0.30/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.63 (A) (INPUT = 0.90)
JSI METAL= 0.39 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040249

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T34	10	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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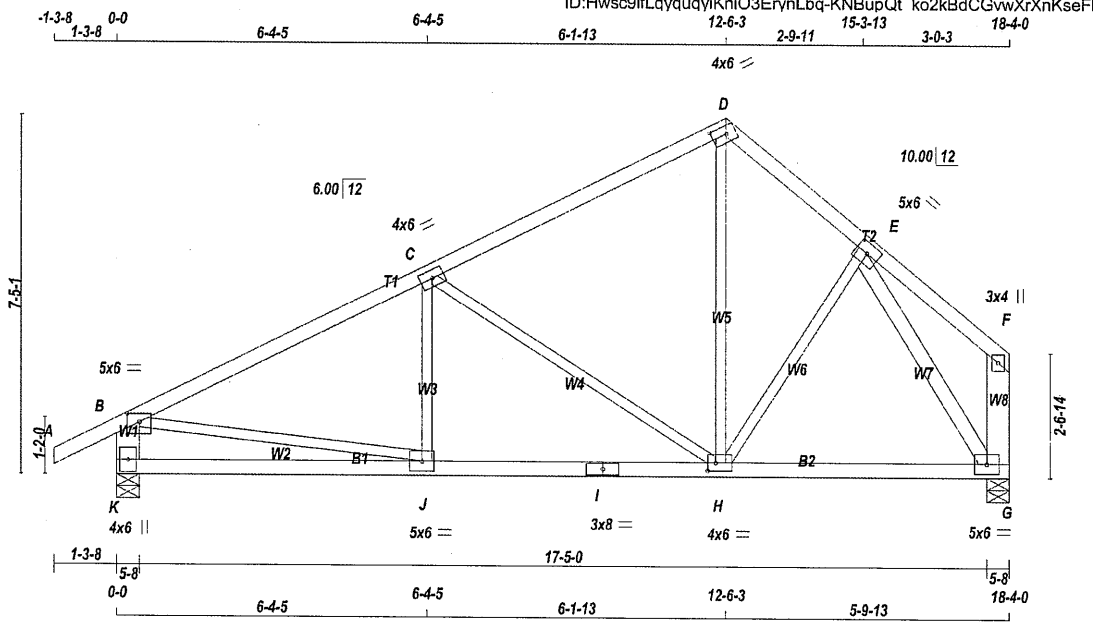

TOTAL WEIGHT = 10 X 82 = 821 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTW-I	MT20	4.0	6.0	2.00	3.50
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	5.0	6.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	1534	0	1534	0	0	5-8	5-8
G	1358	0	1358	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1066	804 / 0	0 / 0	0 / 0	0 / 0	262 / 0	0 / 0
G	946	701 / 0	0 / 0	0 / 0	0 / 0	246 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	J- C	-89 / 89	0.03 (4)
B- C	-1704 / 0	-129.6	-129.6	0.72 (1)	4.06	C- H	-944 / 0	0.97 (1)
C- D	-913 / 0	-129.6	-129.6	0.66 (1)	5.33	H- D	0 / 580	0.13 (1)
D- E	-1029 / 0	-129.6	-129.6	0.14 (1)	6.04	H- E	0 / 81	0.02 (4)
E- F	0 / 27	-129.6	-129.6	0.17 (1)	10.00	B- J	0 / 1577	0.35 (1)
K- B	-1483 / 0	0.0	0.0	0.10 (1)	7.81	E- G	-1376 / 0	0.46 (1)
G- F	-143 / 0	0.0	0.0	0.01 (1)	7.81			
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00			
J- I	0 / 1562	-18.5	-18.5	0.34 (1)	10.00			
I- H	0 / 1562	-18.5	-18.5	0.34 (1)	10.00			
H- G	0 / 724	-18.5	-18.5	0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.34/1.00 (H-J:1) , WB=0.97/1.00 (C-H:1) , SSI=0.36/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (E) (INPUT = 0.90)
JSI METAL= 0.50 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040250

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G34	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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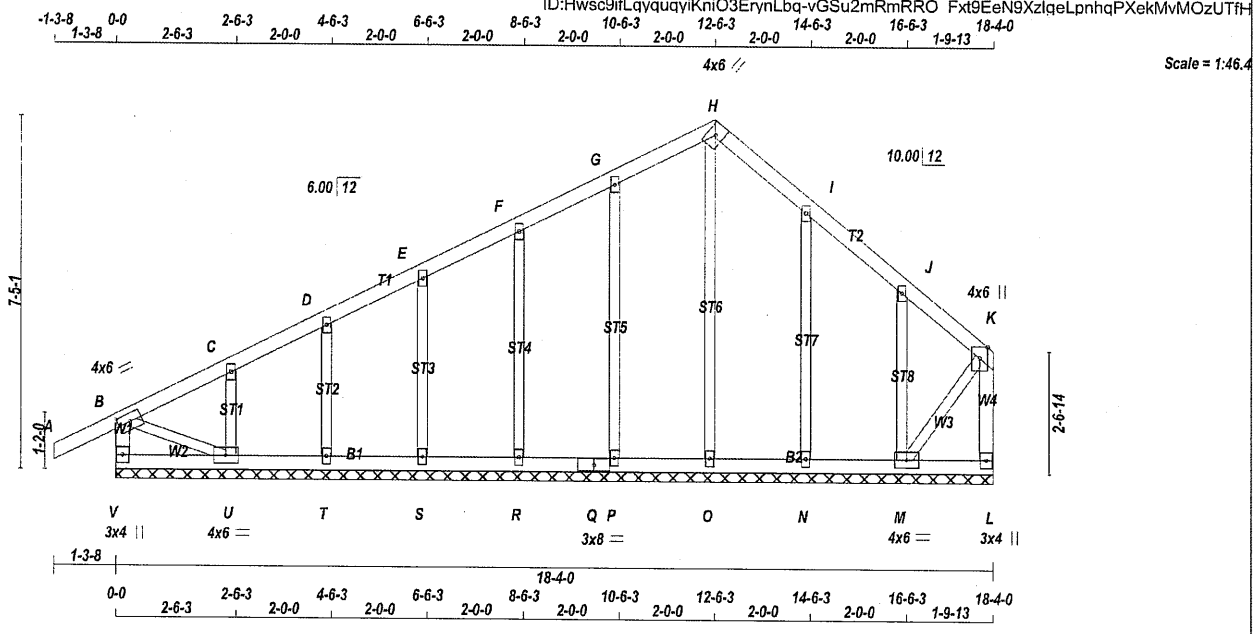

TOTAL WEIGHT = 2 X 83 = 166 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
V - B	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
V - Q	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C, D, E, F, G, I, J						
C	TMW+w	MT20	2.0	4.0		
H	TTW+h	MT20	4.0	6.0		
K	TMVW+p	MT20	4.0	6.0	Edge	
L	BMV1+p	MT20	3.0	4.0		
M	BMWW1-t	MT20	4.0	6.0		
N, O, P, R, S, T						
N	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
U	BMWW1-t	MT20	4.0	6.0		
V	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	O- H	-192 / 0	0.19 (1)
B- C	-35 / 0	-129.6	-129.6	0.09 (1)	6.25	P- G	-304 / 0	0.20 (1)
C- D	-43 / 0	-129.6	-129.6	0.09 (1)	6.25	R- F	-247 / 0	0.11 (1)
D- E	-29 / 0	-129.6	-129.6	0.06 (1)	6.25	S- E	-263 / 0	0.08 (1)
E- F	-27 / 0	-129.6	-129.6	0.06 (1)	6.25	T- D	-238 / 0	0.05 (1)
F- G	-19 / 0	-129.6	-129.6	0.08 (1)	6.25	U- C	-321 / 0	0.05 (1)
G- H	-31 / 0	-129.6	-129.6	0.08 (1)	6.25	N- I	-283 / 0	0.15 (1)
H- I	-42 / 0	-129.6	-129.6	0.07 (1)	6.25	M- J	-265 / 0	0.07 (1)
I- J	-30 / 0	-129.6	-129.6	0.07 (1)	6.25	B- U	0 / 46	0.01 (1)
J- K	-25 / 0	-129.6	-129.6	0.06 (1)	6.25	M- K	0 / 48	0.01 (1)
V- B	-336 / 0	0.0	0.0	0.03 (1)	7.81			
L- K	-156 / 0	0.0	0.0	0.02 (1)	7.81			
V- U	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
U- T	0 / 33	-18.5	-18.5	0.03 (4)	10.00			
T- S	0 / 28	-18.5	-18.5	0.02 (4)	10.00			
S- R	0 / 23	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 20	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 20	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 17	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 20	-18.5	-18.5	0.02 (4)	10.00			
N- M	0 / 25	-18.5	-18.5	0.02 (4)	10.00			
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.03/1.00 (T-U:4) , WB=0.20/1.00 (G-P:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (M) (INPUT = 0.90)
JSI METAL= 0.15 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040205

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T34A	10	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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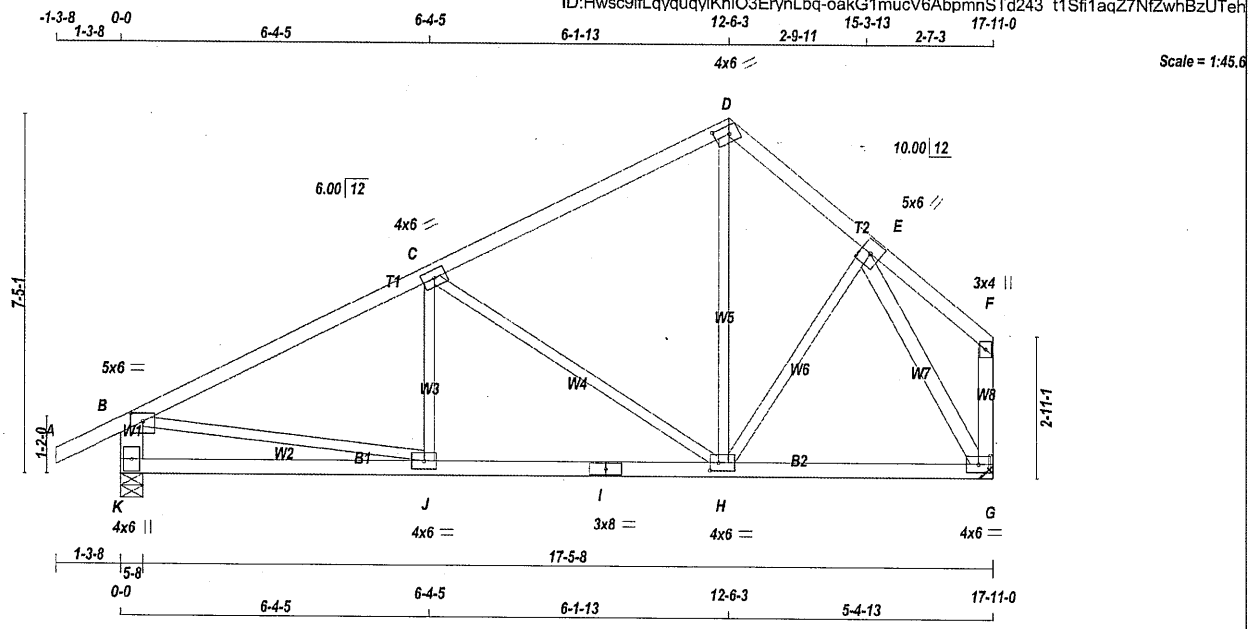

TOTAL WEIGHT = 10 X 80 = 796 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	2.00	3.00
C	TMWW-t	MT20	4.0	6.0		
D	TTW-t	MT20	4.0	6.0	2.00	3.50
E	TMWW+t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-t	MT20	4.0	6.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMV1+p	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1503	0	1503	0	0	5-8	5-8
G	1327	0	1327	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1045	788 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
G	925	685 / 0	0 / 0	0 / 0	0 / 0	240 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)		10.00
B- C	-1650 / 0	-129.6	-129.6	0.72 (1)		4.13
C- D	-851 / 0	-129.6	-129.6	0.66 (1)		5.48
D- E	-955 / 0	-129.6	-129.6	0.13 (1)		6.21
E- F	0 / 27	-129.6	-129.6	0.15 (1)		10.00
K- B	-1453 / 0	0.0	0.0	0.09 (1)		7.81
G- F	-115 / 0	0.0	0.0	0.02 (1)		7.81
K- J	0 / 0	-18.5	-18.5	0.16 (4)		10.00
J- I	0 / 1514	-18.5	-18.5	0.32 (1)		10.00
I- H	0 / 1514	-18.5	-18.5	0.32 (1)		10.00
H- G	0 / 619	-18.5	-18.5	0.18 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-81 / 92	0.03 (4)
C- H	-953 / 0	0.98 (1)
H- D	0 / 503	0.11 (1)
H- E	0 / 173	0.04 (1)
B- J	0 / 1528	0.34 (1)
E- G	-1325 / 0	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.72/1.00 (B-C:1) , BC=0.32/1.00 (H-J:1) , WB=0.98/1.00 (C-H:1) , SSI=0.36/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.49 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040251

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G35	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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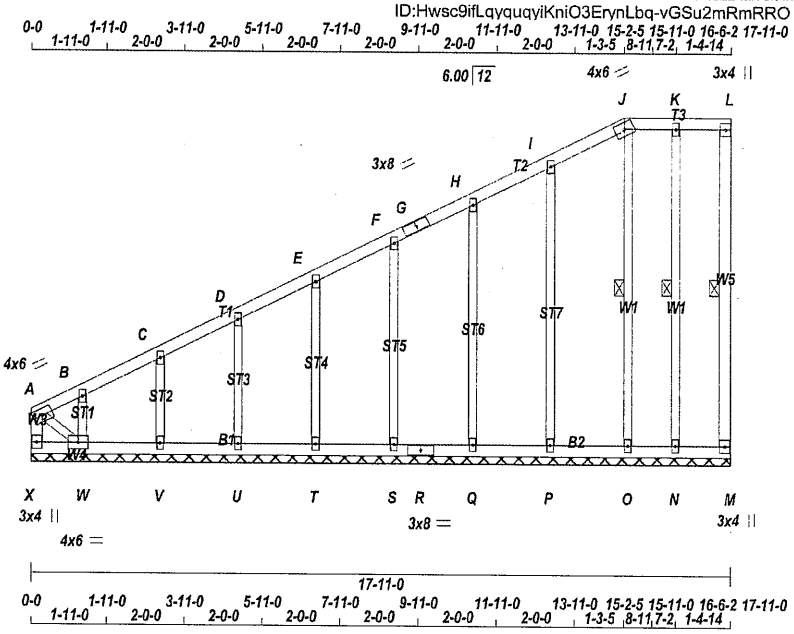

TOTAL WEIGHT = 92 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
X - A	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B, C, D, E, F, H, I, K						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
J	TTW-h	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, S, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	8.0		
W	BMWW1-t	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO									
X- A	-93 / 0	0.0	0.0	0.01 (1)		7.81	P- I	-284 / 0	0.33 (1)
A- B	-27 / 0	-129.6	-129.6	0.05 (1)		6.25	Q- H	-254 / 0	0.20 (1)
B- C	-26 / 0	-129.6	-129.6	0.06 (1)		6.25	S- F	-258 / 0	0.13 (1)
C- D	-22 / 0	-129.6	-129.6	0.06 (1)		6.25	T- E	-258 / 0	0.09 (1)
D- E	-16 / 0	-129.6	-129.6	0.06 (1)		6.25	U- D	-256 / 0	0.06 (1)
E- F	-11 / 0	-129.6	-129.6	0.06 (1)		6.25	V- C	-262 / 0	0.04 (1)
F- G	-8 / 0	-129.6	-129.6	0.06 (1)		10.00	W- B	-228 / 0	0.03 (1)
G- H	-8 / 0	-129.6	-129.6	0.06 (1)		10.00	N- K	-204 / 0	0.10 (1)
H- I	-4 / 0	-129.6	-129.6	0.07 (1)		10.00	O- J	-173 / 0	0.09 (1)
I- J	-14 / 0	-129.6	-129.6	0.07 (1)		6.25	A- W	0 / 43	0.01 (1)
J- K	0 / 0	-129.6	-129.6	0.03 (1)		10.00			
K- L	0 / 0	-129.6	-129.6	0.03 (1)		10.00			
M- L	-78 / 0	0.0	0.0	0.03 (1)		6.25			
X- W	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
W- V	0 / 26	-18.5	-18.5	0.02 (4)		10.00			
V- U	0 / 19	-18.5	-18.5	0.02 (4)		10.00			
U- T	0 / 14	-18.5	-18.5	0.01 (4)		10.00			
T- S	0 / 10	-18.5	-18.5	0.01 (4)		10.00			
S- R	0 / 7	-18.5	-18.5	0.01 (4)		10.00			
R- Q	0 / 7	-18.5	-18.5	0.01 (4)		10.00			
Q- P	0 / 5	-18.5	-18.5	0.02 (4)		10.00			
P- O	0 / 2	-18.5	-18.5	0.02 (4)		10.00			
O- N	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
N- M	0 / 0	-18.5	-18.5	0.01 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (I-J:1) , BC=0.02/1.00 (V-W:4) , WB=0.33/1.00 (I-P:1) , SSI=0.11/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (A) (INPUT = 0.90)
JSI METAL= 0.12 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040206

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G36	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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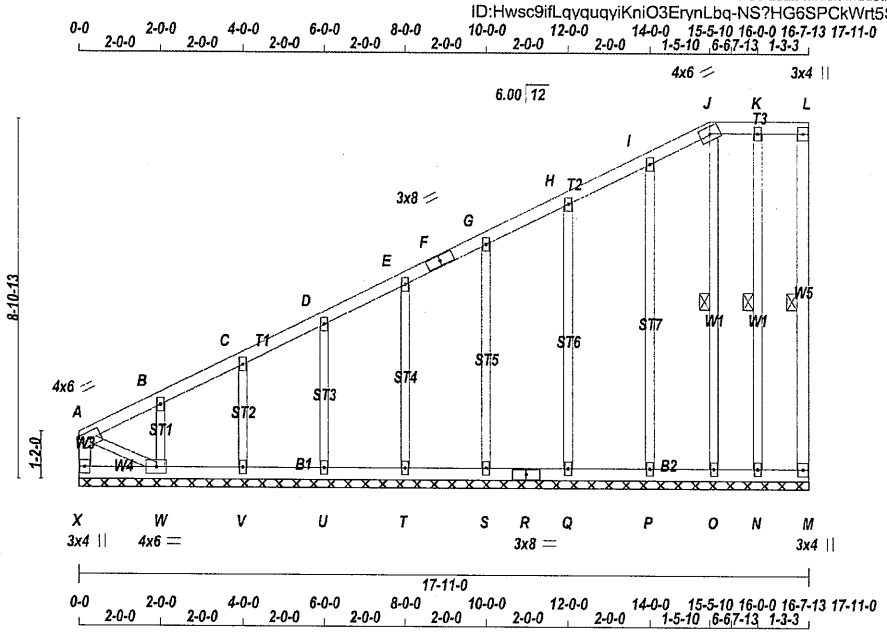

Scale = 1:54.6

TOTAL WEIGHT = 95 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
X - R	2x4	DRY	No.2	SPF
R - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		Edge
B, C, D, E, G, H, I, K						
B	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
J	TTW-h	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
M	BMV1+p	MT20	3.0	4.0		
N, O, P, Q, S, T, U, V						
N	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	8.0		
W	BMWW1-t	MT20	4.0	6.0		
X	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF L-M, J-O, K-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-25 / 0	-129.6	-129.6	0.07 (1)		6.25	W- B	-282 / 0	0.04 (1)
B- C	-28 / 0	-129.6	-129.6	0.07 (1)		6.25	V- C	-250 / 0	0.05 (1)
C- D	-19 / 0	-129.6	-129.6	0.06 (1)		6.25	U- D	-258 / 0	0.07 (1)
D- E	-14 / 0	-129.6	-129.6	0.06 (1)		6.25	T- E	-257 / 0	0.10 (1)
E- F	-10 / 0	-129.6	-129.6	0.06 (1)		6.25	S- G	-257 / 0	0.16 (1)
F- G	-10 / 0	-129.6	-129.6	0.06 (1)		6.25	Q- H	-263 / 0	0.24 (1)
G- H	-7 / 0	-129.6	-129.6	0.06 (1)		10.00	P- I	-248 / 0	0.32 (1)
H- I	-6 / 0	-129.6	-129.6	0.06 (1)		10.00	A- W	0 / 36	0.01 (1)
I- J	-13 / 0	-129.6	-129.6	0.05 (1)		6.25	O- J	-142 / 0	0.07 (1)
J- K	0 / 0	-129.6	-129.6	0.02 (1)		10.00	N- K	-177 / 0	0.09 (1)
K- L	0 / 0	-129.6	-129.6	0.03 (1)		10.00			
M- L	-73 / 0	0.0	0.0	0.03 (1)		6.25			
X- A	-129 / 0	0.0	0.0	0.01 (1)		7.81			
X- W	0 / 0	-18.5	-18.5	0.02 (4)		10.00			
W- V	0 / 23	-18.5	-18.5	0.02 (4)		10.00			
V- U	0 / 17	-18.5	-18.5	0.02 (4)		10.00			
U- T	0 / 13	-18.5	-18.5	0.02 (4)		10.00			
T- S	0 / 9	-18.5	-18.5	0.01 (4)		10.00			
S- R	0 / 6	-18.5	-18.5	0.01 (4)		10.00			
R- Q	0 / 6	-18.5	-18.5	0.01 (4)		10.00			
Q- P	0 / 4	-18.5	-18.5	0.01 (4)		10.00			
P- O	0 / 2	-18.5	-18.5	0.01 (4)		10.00			
O- N	0 / 0	-18.5	-18.5	0.01 (4)		10.00			
N- M	0 / 0	-18.5	-18.5	0.01 (4)		10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.07/1.00 (B-C:1) , BC=0.02/1.00 (V-W:4) , WB=0.32/1.00 (I-P:1) , SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (C) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040207

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T37	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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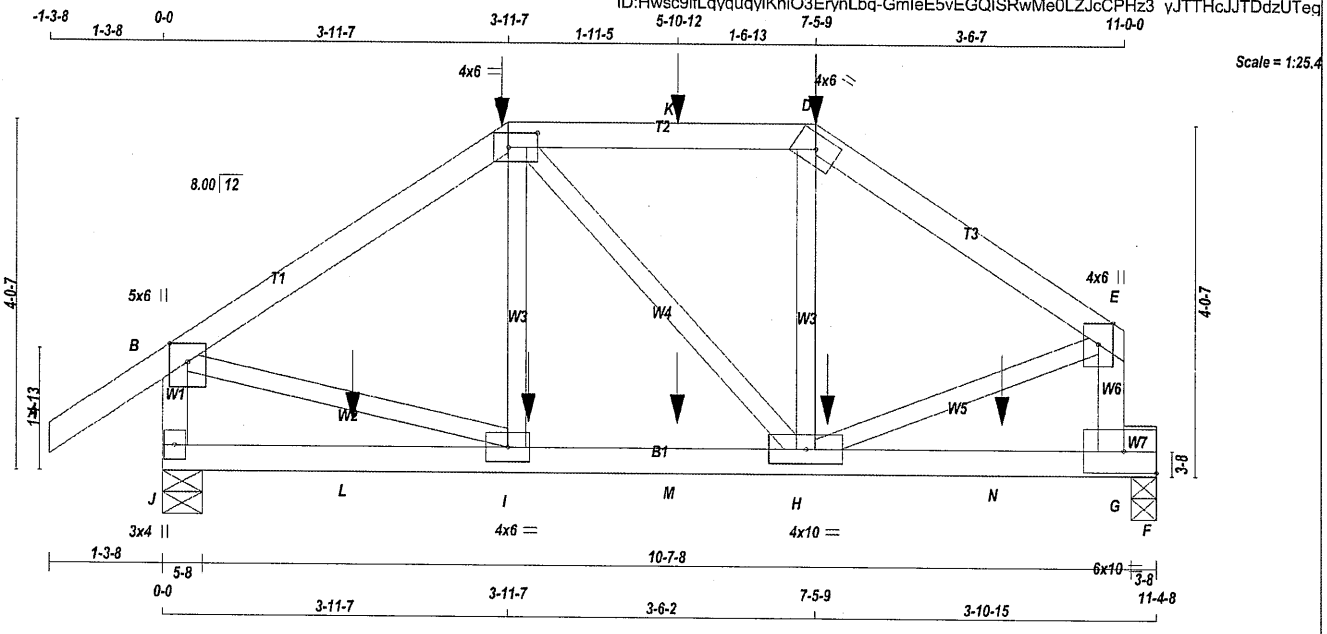


TOTAL WEIGHT = 2 X 48 = 95 lb

[M]

LUMBER

N. L. G. A. RULES

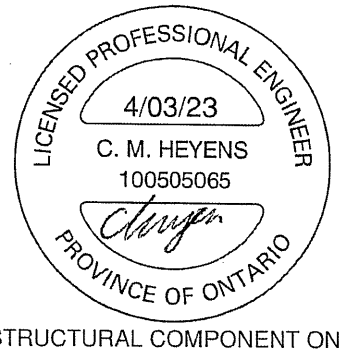
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	4.0	6.0	Edge	
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	4.0	10.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040252

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1290	0	1290	0	0	5-8	5-8
F	1070	0	1070	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	895	683 / 0	0 / 0	0 / 0	0 / 0	212 / 0	0 / 0
F	745	553 / 0	0 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	I- C	-185 / 36	0.05 (1)
B- C	-1047 / 0	-129.6	-129.6	0.41 (1)	5.52	C- H	0 / 48	0.01 (1)
C- K	-897 / 0	-129.6	-129.6	0.39 (1)	5.89	H- D	-109 / 50	0.03 (1)
K- D	-897 / 0	-129.6	-129.6	0.39 (1)	5.89	B- I	0 / 900	0.22 (1)
D- E	-1083 / 0	-129.6	-129.6	0.33 (1)	5.59	H- E	0 / 960	0.24 (1)
J- B	-1249 / 0	0.0	0.0	0.14 (1)	7.16			
G- E	-1168 / 0	0.0	0.0	0.14 (1)	7.34			
J- L	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
L- I	0 / 0	-18.5	-18.5	0.09 (4)	10.00			
I- M	0 / 864	-18.5	-18.5	0.34 (1)	10.00			
M- H	0 / 864	-18.5	-18.5	0.34 (1)	10.00			
H- N	0 / 0	-18.5	-18.5	0.60 (1)	10.00			
N- G	0 / 0	-18.5	-18.5	0.60 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.25 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-11-7	-15	-15	---	FRONT	VERT	DEAD	---	C1
C	3-11-7	-43	-43	---	FRONT	VERT	TOTAL	---	C1
C	3-11-7	-95	-95	---	FRONT	VERT	SNOW	---	C1
D	7-5-9	-15	-15	---	FRONT	VERT	DEAD	---	C1
D	7-5-9	-44	-44	---	FRONT	VERT	TOTAL	---	C1
D	7-5-9	-95	-95	---	FRONT	VERT	SNOW	---	C1
H	7-7-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
I	4-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
K	5-10-12	-39	-39	---	FRONT	VERT	TOTAL	---	C1
L	2-2-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1
M	5-10-12	-6	-6	---	FRONT	VERT	TOTAL	---	C1
N	9-7-4	-6	-6	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.60/1.00 (G-H:1) , WB=0.24/1.00 (E-H:1) , SSI=0.46/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.56 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T38	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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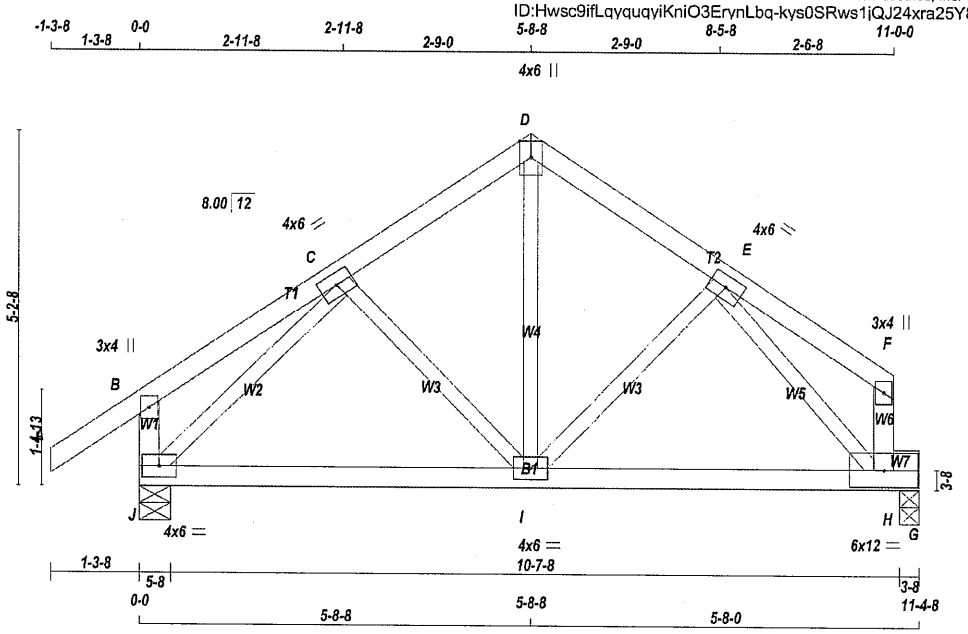

TOTAL WEIGHT = 2 X 49 = 98 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	6.0	12.0		
I	BMWWW-t	MT20	4.0	6.0		
J	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	795	0	795	0	0	3-8	3-8
J	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
J	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 22	-129.6	-129.6	0.16 (1)	10.00	
C- D	-643 / 0	-129.6	-129.6	0.16 (1)	6.25	
D- E	-640 / 0	-129.6	-129.6	0.14 (1)	6.25	
E- F	0 / 23	-129.6	-129.6	0.14 (1)	10.00	
J- B	-322 / 0	0.0	0.0	0.03 (1)	7.81	
H- F	-117 / 0	0.0	0.0	0.01 (1)	7.81	
J- I	0 / 638	-18.5	-18.5	0.28 (1)	10.00	
I- H	0 / 573	-18.5	-18.5	0.50 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.17 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- I	-176 / 0	0.05 (1)
I- D	0 / 391	0.09 (1)
I- E	-82 / 18	0.02 (1)
J- C	-924 / 0	0.25 (1)
E- H	-907 / 0	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 959 (0.14")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.50/1.00 (H-I:1) , WB=0.25/1.00 (C-J:1) , SSI=0.31/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (E) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040253

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T39	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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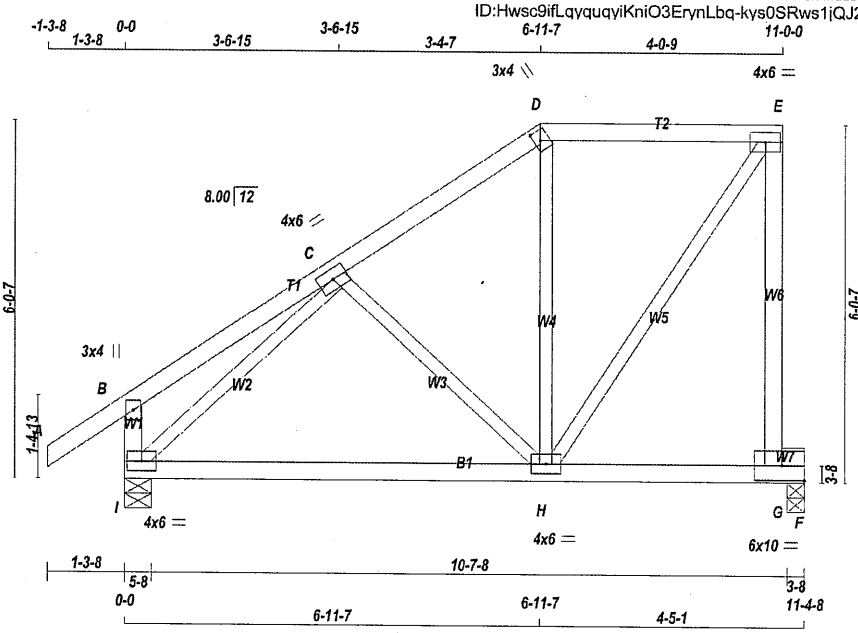

TOTAL WEIGHT = 54 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	4.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	795	0	795	0	0	3-8	3-8
I	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
I	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-307 / 0	0.12 (1)
B- C	0 / 29	-129.6	-129.6	0.26 (1)	10.00	H- D	-160 / 22	0.09 (1)
C- D	-527 / 0	-129.6	-129.6	0.26 (1)	6.25	H- E	0 / 726	0.16 (1)
D- E	-411 / 0	-129.6	-129.6	0.37 (1)	6.25	I- C	-891 / 0	0.33 (1)
G- E	-859 / 0	0.0	0.0	0.60 (1)	7.81			
I- B	-350 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	0 / 637	-18.5	-18.5	0.28 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.60/1.00 (E-G:1) , BC=0.40/1.00 (G-H:1) , WB=0.33/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (H) (INPUT = 0.90)
JSI METAL= 0.22 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040254

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOWN OF INNISFIL
Block 4
REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T40	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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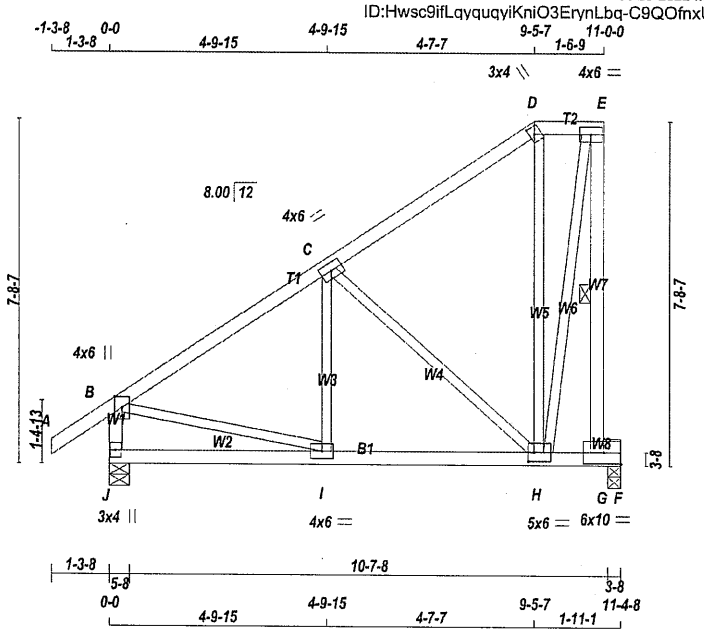

TOTAL WEIGHT = 63 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1020	0	1020	0	0	5-8	5-8
F	795	0	795	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.15 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-722 / 0	-129.6	-129.6	0.51 (1)	6.15
C- D	-257 / 0	-129.6	-129.6	0.49 (1)	6.25
D- E	-175 / 0	-129.6	-129.6	0.05 (1)	6.25
G- E	-996 / 0	0.0	0.0	0.27 (1)	6.25
J- B	-983 / 0	0.0	0.0	0.10 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- H	0 / 635	-18.5	-18.5	0.15 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- C	-43 / 79	0.03 (4)
C- H	-618 / 0	0.45 (1)
H- D	-206 / 0	0.23 (1)
H- E	0 / 914	0.21 (1)
B- I	0 / 650	0.15 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.40/1.00 (G-H:1) , WB=0.45/1.00 (C-H:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040255

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T41	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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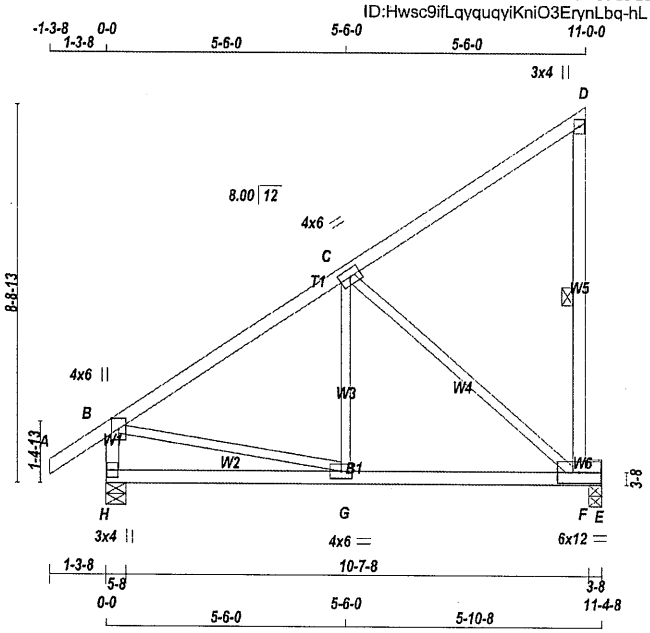

Scale = 1:50.9

TOTAL WEIGHT = 2 X 54 = 108 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1020	0	1020	0	0	5-8	5-8
E	795	0	795	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
E	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	G- C	0 / 131	0.05 (4)
B- C	-712 / 0	-129.6	-129.6	0.71 (1)	5.66	C- F	-836 / 0	0.81 (1)
C- D	-49 / 0	-129.6	-129.6	0.66 (1)	6.25	B- G	0 / 645	0.15 (1)
F- D	-269 / 0	0.0	0.0	0.09 (1)	6.25			
H- B	-993 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
G- F	0 / 633	-18.5	-18.5	0.51 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 881 (0.15")

CSI: TC=0.71/1.00 (B-C:1) , BC=0.51/1.00 (F-G:1) , WB=0.81/1.00 (C-F:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (B) (INPUT = 0.90)
JSI METAL= 0.46 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040256

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T42	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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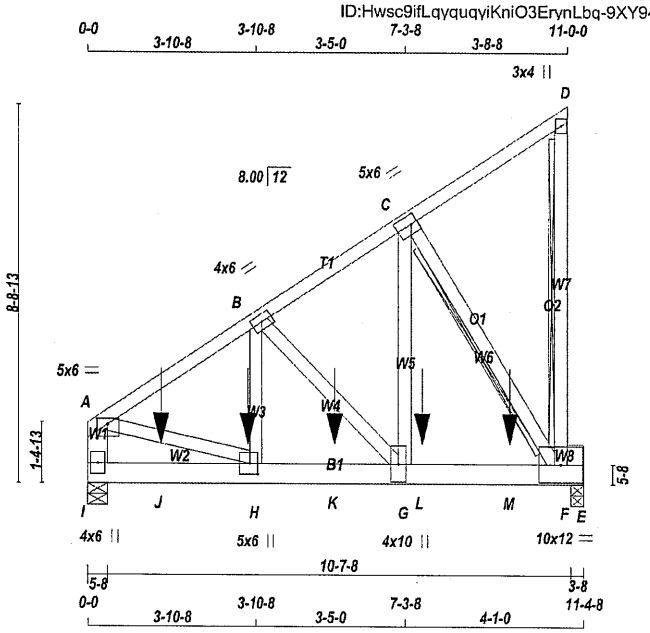

TOTAL WEIGHT = 4 X 73 = 294 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - A	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
I- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
H- B	1	6	SIDE(158.3)
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040257

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
I	4092	0	4092	0	0	5-8	5-8
E	4088	0	4088	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2846	2135 / 0	0 / 0	0 / 0	0 / 0	712 / 0	0 / 0
E	2845	2130 / 0	0 / 0	0 / 0	0 / 0	715 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-F, C-F

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-4148 / 0	-129.6	-129.6	1	0.22 (1)	4.49	A- H	0 / 3579	0.32 (1)
B- C	-2721 / 0	-129.6	-129.6	1	0.16 (1)	5.37	H- B	0 / 1558	0.14 (1)
C- D	-25 / 0	-129.6	-129.6	1	0.13 (1)	6.25	B- G	-1732 / 0	0.25 (1)
F- D	-195 / 0	0.0	0.0	1	0.06 (1)	7.81	G- C	0 / 4579	0.40 (1)
I- A	-3421 / 0	0.0	0.0	1	0.12 (1)	7.54	C- F	-4244 / 0	0.77 (1)
I- J	0 / 0	-18.5	-18.5	1	0.41 (1)	10.00			
J- H	0 / 0	-18.5	-18.5	1	0.41 (1)	10.00			
H- K	0 / 3466	-18.5	-18.5	1	0.64 (1)	10.00			
K- G	0 / 3466	-18.5	-18.5	1	0.64 (1)	10.00			
G- L	0 / 2270	-18.5	-18.5	1	0.85 (1)	10.00			
L- M	0 / 2270	-18.5	-18.5	1	0.85 (1)	10.00			
M- F	0 / 2270	-18.5	-18.5	1	0.85 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	1	0.23 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	3-8-4	-910	-910	---	FRONT	VERT	TOTAL	---	C1
J	1-8-4	-910	-910	---	FRONT	VERT	TOTAL	---	C1
K	5-8-4	-910	-910	---	FRONT	VERT	TOTAL	---	C1
L	7-8-4	-910	-910	---	FRONT	VERT	TOTAL	---	C1
M	9-8-4	-910	-910	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.22/1.00 (A-B:1) , BC=0.85/1.00 (F-G:1) , WB=0.77/1.00 (C-F:1) , SSI=0.79/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.62 (H) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T42	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.50	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	5.0	6.0	2.50	1.75
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	10.0	12.0		
G	BMWW+t	MT20	4.0	10.0		
H	BMWW+t	MT20	5.0	6.0	3.00	2.25
I	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040257

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T43	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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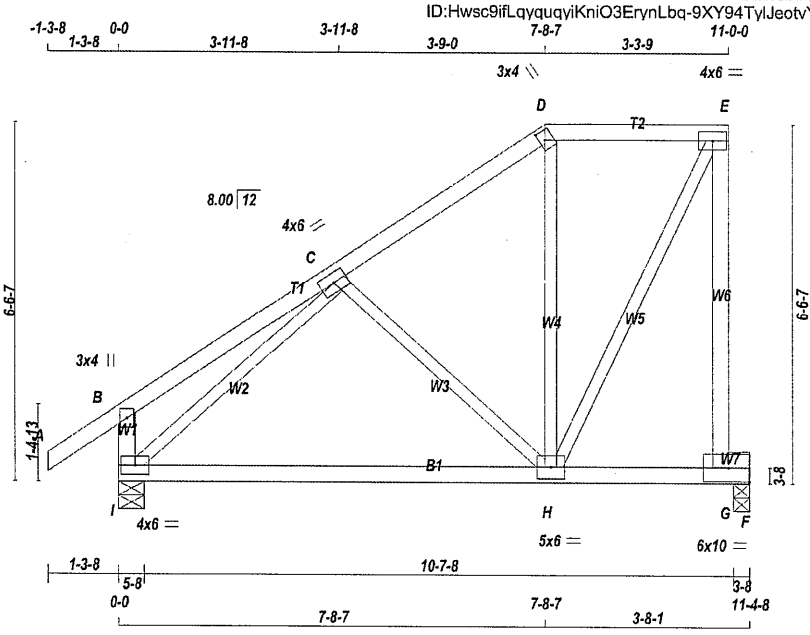

TOTAL WEIGHT = 56 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTW+h	MT20	3.0	4.0	2.00	1.25
E	TMVW-t	MT20	4.0	6.0		
G	BMVW-t	MT20	6.0	10.0		Edge
H	BMWWW-t	MT20	5.0	6.0		
I	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
						IN-SX	IN-SX
F	795	0	795	0	0	3-8	3-8
I	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
I	708	539 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	C- H	-379 / 0	0.18 (1)
B- C	0 / 33	-129.6	-129.6	0.32 (1)	10.00	H- D	-170 / 16	0.12 (1)
C- D	-456 / 0	-129.6	-129.6	0.32 (1)	6.25	H- E	0 / 766	0.17 (1)
D- E	-349 / 0	-129.6	-129.6	0.24 (1)	6.25	I- C	-869 / 0	0.39 (1)
G- E	-894 / 0	0.0	0.0	0.79 (1)	7.81			
I- B	-367 / 0	0.0	0.0	0.04 (1)	7.81			
I- H	0 / 631	-18.5	-18.5	0.30 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.40 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.79/1.00 (E-G:1) , BC=0.40/1.00 (G-H:1) , WB=0.39/1.00 (C-I:1) , SSI=0.31/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.21 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040258

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T44	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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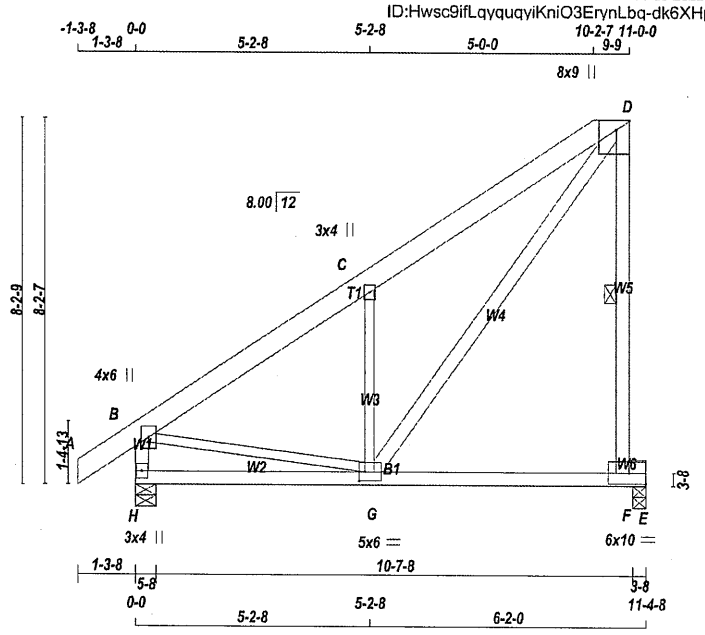

TOTAL WEIGHT = 68 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0		
C	TMW+w	MT20	3.0	4.0		
D	TMVW+p	MT20	8.0	9.0	Edge	3.50
F	BMVW-t	MT20	6.0	10.0		Edge
G	BMWWW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1026	0	1026	0	0	5-8	5-8
E	795	0	795	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	712	543 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
E	555	407 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 51	-129.6	-129.6	0.09 (1)	10.00	G- C	-848 / 0	0.27 (1)
B- C	-763 / 0	-129.6	-129.6	0.30 (1)	6.25	B- G	0 / 687	0.15 (1)
C- D	-803 / 0	-129.6	-129.6	0.31 (1)	6.25	G- D	0 / 1120	0.18 (1)
F- D	-810 / 0	0.0	0.0	0.26 (1)	6.25			
H- B	-1001 / 0	0.0	0.0	0.10 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.20 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.40 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 782 (0.17")

CSI: TC=0.31/1.00 (C-D:1) , BC=0.40/1.00 (F-G:1) , WB=0.27/1.00 (C-G:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (G) (INPUT = 0.90)
JSI METAL= 0.48 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040259

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	G45	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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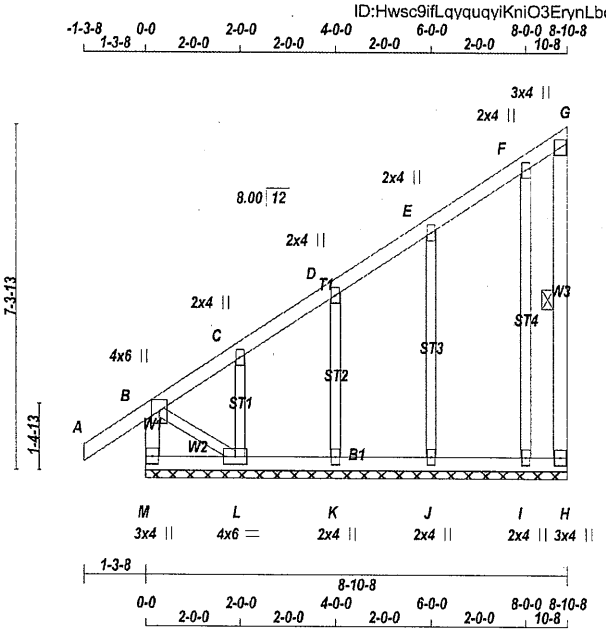

TOTAL WEIGHT = 2 X 48 = 96 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, E, F						
C	TMW+w	MT20	2.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMV1+p	MT20	3.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMWW1-t	MT20	4.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	L- C	-185 / 0	0.03 (1)
B- C	-52 / 0	-129.6	-129.6	0.17 (1)	6.25	K- D	-269 / 0	0.07 (1)
C- D	-8 / 0	-129.6	-129.6	0.06 (1)	10.00	J- E	-267 / 0	0.12 (1)
D- E	-9 / 0	-129.6	-129.6	0.06 (1)	10.00	I- F	-203 / 0	0.15 (1)
E- F	-9 / 0	-129.6	-129.6	0.06 (1)	10.00	B- L	0 / 22	0.00 (1)
F- G	-14 / 0	-129.6	-129.6	0.03 (1)	6.25			
H- G	-37 / 0	0.0	0.0	0.01 (1)	6.25			
M- B	-378 / 0	0.0	0.0	0.04 (1)	7.81			
M- L	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
L- K	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 7	-18.5	-18.5	0.02 (4)	10.00			
J- I	0 / 3	-18.5	-18.5	0.02 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (K-L:4) , WB=0.15/1.00 (F-I:1) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040208

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	T46	6	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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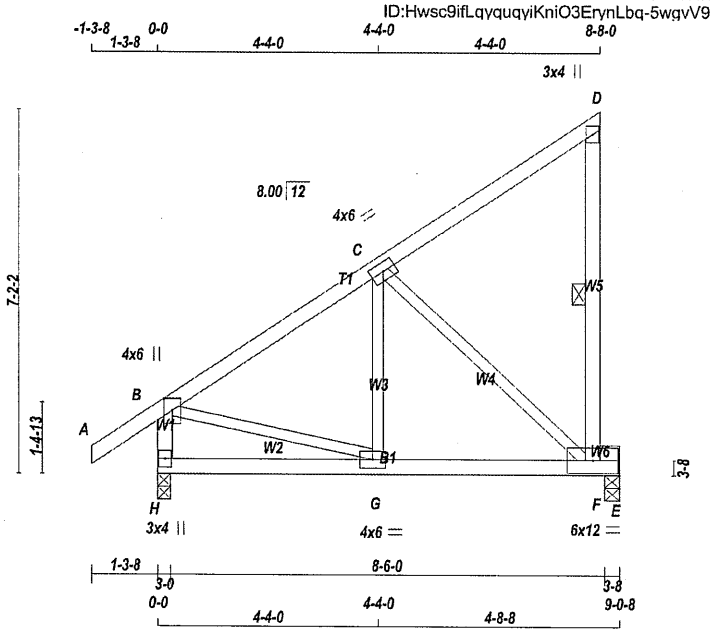

Scale = 1:43.5

TOTAL WEIGHT = 6 X 44 = 263 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	6.0		
D	TMV+p	MT20	3.0	4.0		
F	BMVWW-t	MT20	6.0	12.0		
G	BMWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	847	0	847	0	0	3-0	3-0
E	622	0	622	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	587	450 / 0	0 / 0	0 / 0	0 / 0	138 / 0	0 / 0
E	434	317 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00		G- C	0 / 101	0.04 (4)
B- C	-536 / 0	-129.6	-129.6	0.32 (1)	6.25		C- F	-648 / 0	0.36 (1)
C- D	-38 / 0	-129.6	-129.6	0.31 (1)	6.25		B- G	0 / 492	0.11 (1)
F- D	-212 / 0	0.0	0.0	0.05 (1)	6.25				
H- B	-828 / 0	0.0	0.0	0.09 (1)	7.81				
H- G	0 / 0	-18.5	-18.5	0.13 (4)	10.00				
G- F	0 / 478	-18.5	-18.5	0.39 (1)	10.00				
F- E	0 / 0	-18.5	-18.5	0.13 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.39/1.00 (F-G:1) , WB=0.36/1.00 (C-F:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.36 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040260

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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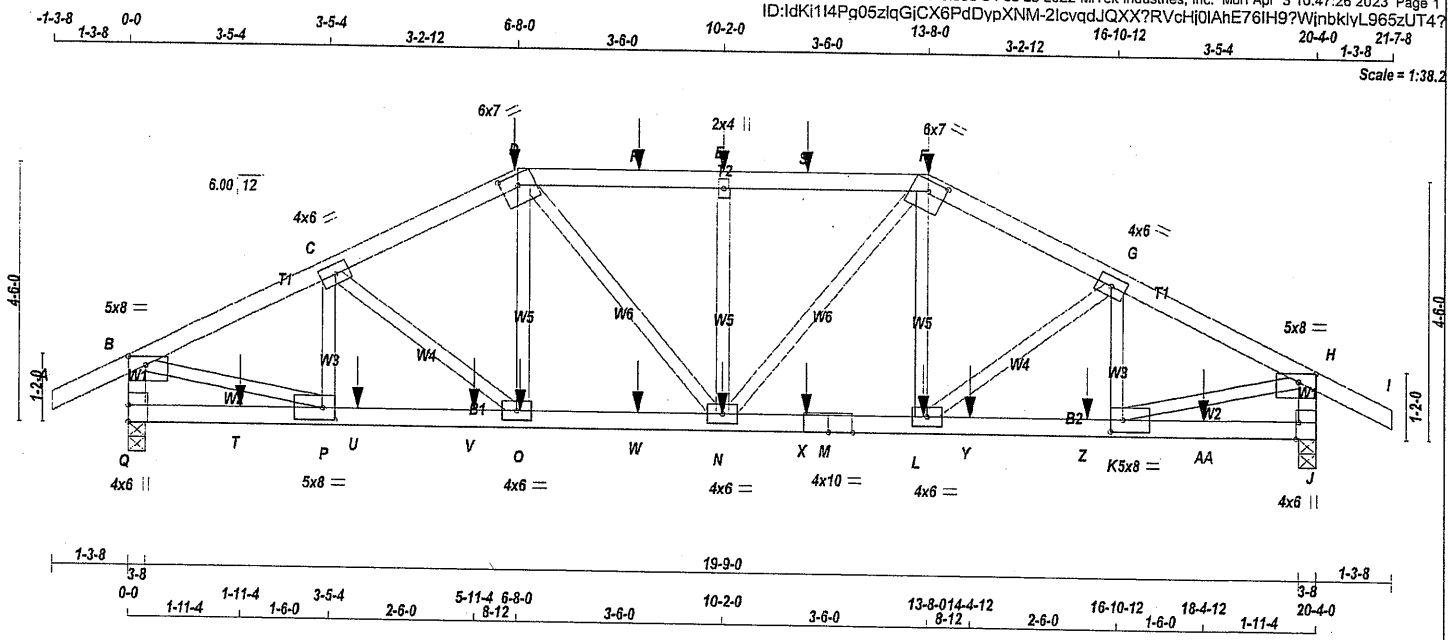

TOTAL WEIGHT = 2 X 86 = 173 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.75	3.50
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-h	MT20	6.0	7.0	2.25	3.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW-h	MT20	6.0	7.0	2.25	3.50
G	TMWW-t	MT20	4.0	6.0		
H	TMVW-p	MT20	5.0	8.0	1.75	3.50
J	BMV1+t	MT20	4.0	6.0	Edge	0.50
K	BMWW-t	MT20	5.0	8.0	2.50	2.50
L	BMWW-t	MT20	4.0	6.0		
M	BS-t	MT20	4.0	10.0		
N	BMWWW-t	MT20	4.0	6.0		
O	BMWW-t	MT20	4.0	6.0		
P	BMWW-t	MT20	5.0	8.0	2.50	2.50
Q	BMV1+t	MT20	4.0	6.0	3.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2969	0	2969	0	0	3-8	3-8
J	2969	0	2969	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0
J	2070	1527 / 0	0 / 0	0 / 0	0 / 0	543 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.67 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00	O- D	0 / 143	0.05 (4)
B- C	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22	L- F	0 / 143	0.05 (4)
C- D	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09	N- E	-1021 / 0	0.32 (1)
D- R	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67	N- F	0 / 737	0.18 (1)
R- E	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67	D- N	0 / 737	0.18 (1)
E- S	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67	C- O	0 / 231	0.06 (1)
S- F	-4070 / 0	-129.6	-129.6	0.69 (1)	2.67	P- C	-754 / 0	0.14 (1)
F- G	-4046 / 0	-129.6	-129.6	0.44 (1)	3.09	B- P	0 / 3520	0.87 (1)
G- H	-3802 / 0	-129.6	-129.6	0.42 (1)	3.22	L- G	0 / 231	0.06 (1)
H- I	0 / 39	-129.6	-129.6	0.19 (1)	10.00	K- G	-754 / 0	0.14 (1)
Q- B	-2921 / 0	0.0	0.0	0.32 (1)	4.98	K- H	0 / 3520	0.87 (1)
J- H	-2921 / 0	0.0	0.0	0.32 (1)	4.98			
Q- T	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
T- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
P- U	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
U- V	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
V- O	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
O- W	0 / 3599	-18.5	-18.5	0.70 (1)	10.00			
W- N	0 / 3599	-18.5	-18.5	0.70 (1)	10.00			
N- X	0 / 3599	-18.5	-18.5	0.70 (1)	10.00			
X- M	0 / 3599	-18.5	-18.5	0.70 (1)	10.00			
M- L	0 / 3599	-18.5	-18.5	0.70 (1)	10.00			
L- Y	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
Y- Z	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
Z- K	0 / 3416	-18.5	-18.5	0.66 (1)	10.00			
K-AA	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
AA- J	0 / 0	-18.5	-18.5	0.10 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	—	BACK	VERT	TOTAL	—	C1
E	10-2-0	-133	-133	—	BACK	VERT	TOTAL	—	C1
F	13-8-0	-561	-561	—	BACK	VERT	TOTAL	—	C1
L	13-7-4	-26	-26	—	BACK	VERT	TOTAL	—	C1
N	10-2-0	-26	-26	—	BACK	VERT	TOTAL	—	C1
O	6-8-12	-26	-26	—	BACK	VERT	TOTAL	—	C1
R	8-8-12	-133	-133	—	BACK	VERT	TOTAL	—	C1
S	11-7-4	-133	-133	—	BACK	VERT	TOTAL	—	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.69/1.00 (D-E:1) , BC=0.70/1.00 (L-N:1) , WB=0.87/1.00 (B-P:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.81 (M) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T90	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	1-11-4	-25	-25	--	BACK	VERT	TOTAL	--	C1
U	3-11-4	-26	-26	--	BACK	VERT	TOTAL	--	C1
V	5-11-4	-26	-26	--	BACK	VERT	TOTAL	--	C1
W	8-8-12	-26	-26	--	BACK	VERT	TOTAL	--	C1
X	11-7-4	-26	-26	--	BACK	VERT	TOTAL	--	C1
Y	14-4-12	-26	-26	--	BACK	VERT	TOTAL	--	C1
Z	16-4-12	-26	-26	--	BACK	VERT	TOTAL	--	C1
AA	18-4-12	-25	-25	--	BACK	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040330

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	T91	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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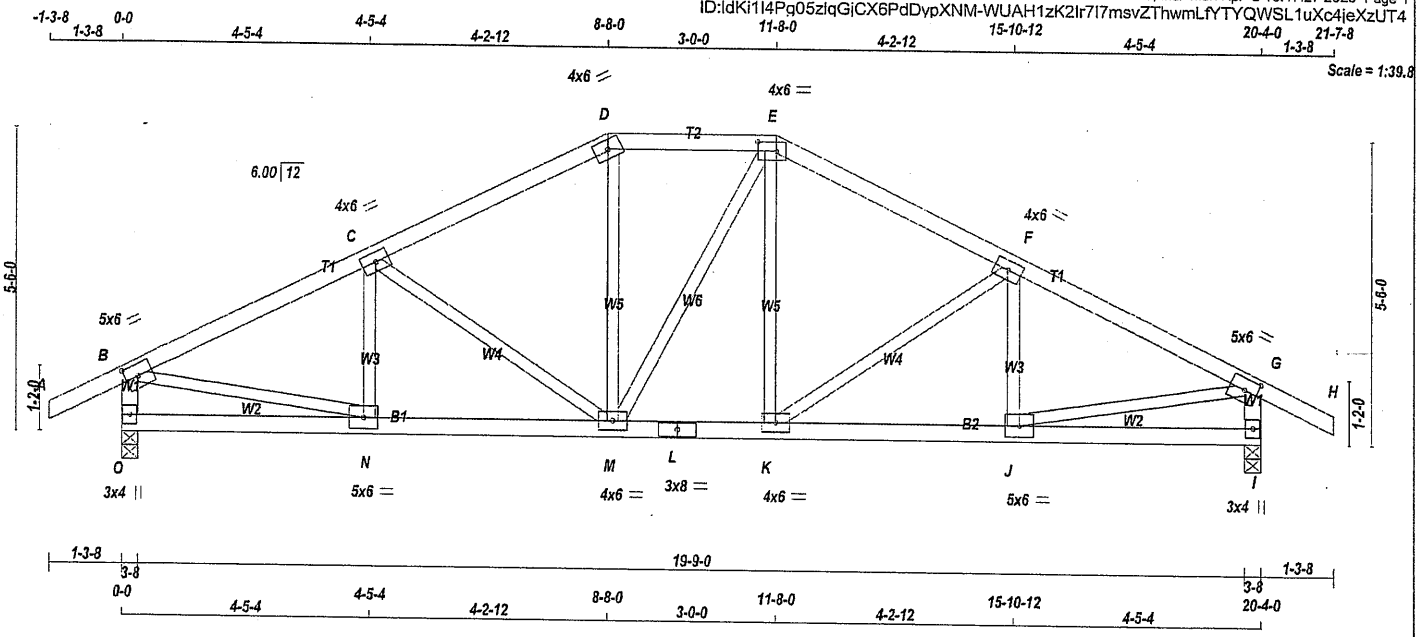

TOTAL WEIGHT = 2 X 86 = 172 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	6.0		
D	TTW-h	MT20	4.0	6.0		
E	TTWW-t	MT20	4.0	6.0	2.00	4.00
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0	2.50	2.75
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	6.0		
L	BS-t	MT20	3.0	8.0		
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040331

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1682	0	1682	0	0	3-8	3-8
I	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0
I	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	M- D	0 / 320	0.07 (1)
B- C	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46	M- E	0 / 3	0.00 (1)
C- D	-1640 / 0	-129.6	-129.6	0.33 (1)	4.83	K- E	0 / 317	0.07 (1)
D- E	-1445 / 0	-129.6	-129.6	0.17 (1)	5.27	C- M	-434 / 0	0.20 (1)
E- F	-1638 / 0	-129.6	-129.6	0.33 (1)	4.83	N- C	-265 / 15	0.05 (1)
F- G	-1977 / 0	-129.6	-129.6	0.35 (1)	4.46	B- N	0 / 1826	0.41 (1)
G- H	0 / 39	-129.6	-129.6	0.17 (1)	10.00	K- F	-437 / 0	0.21 (1)
O- B	-1644 / 0	0.0	0.0	0.17 (1)	6.46	J- F	-263 / 16	0.05 (1)
I- G	-1644 / 0	0.0	0.0	0.17 (1)	6.46	J- G	0 / 1827	0.41 (1)
O- N	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
N- M	0 / 1793	-18.5	-18.5	0.32 (1)	10.00			
M- L	0 / 1444	-18.5	-18.5	0.26 (1)	10.00			
L- K	0 / 1444	-18.5	-18.5	0.26 (1)	10.00			
K- J	0 / 1794	-18.5	-18.5	0.33 (1)	10.00			
J- I	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.33/1.00 (J-K:1) , WB=0.41/1.00 (G-J:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)
JSI METAL= 0.50 (G) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404951	G92	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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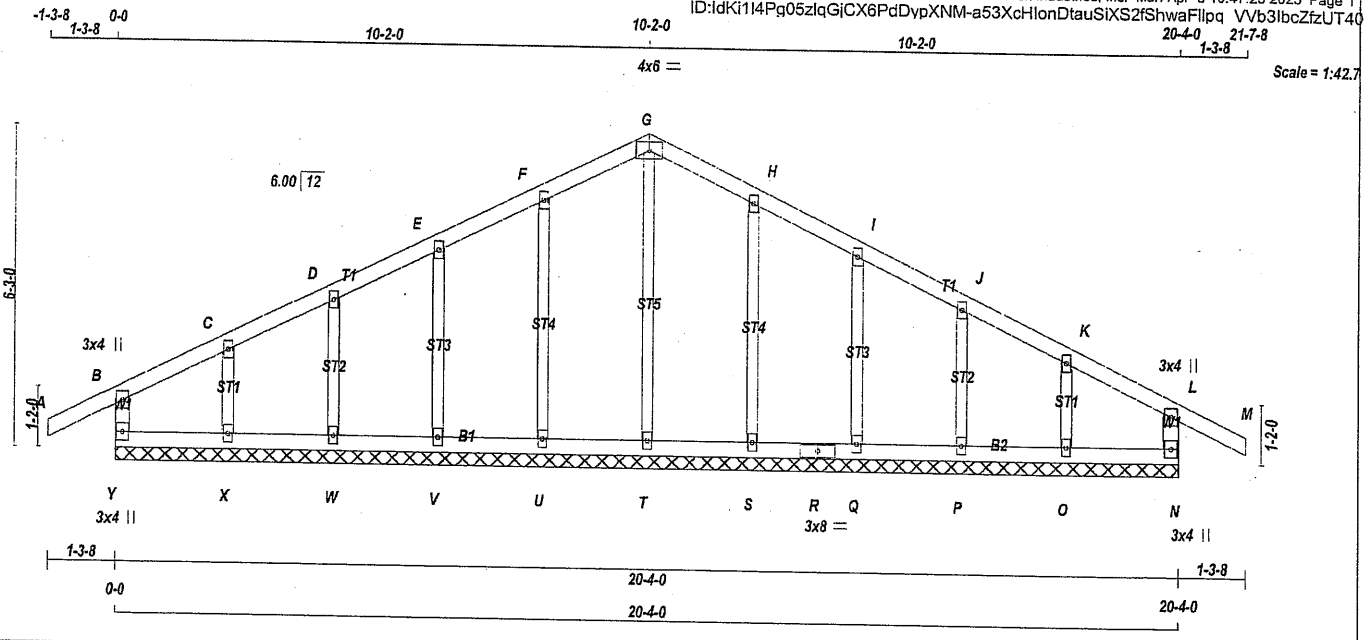

TOTAL WEIGHT = 2 X 80 = 159 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	6.0		
L	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, Q, S, T, U, V, W, X						
O	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	8.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y- B	-332 / 0	0.0	0.0	0.02 (1)	7.81
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00
B- C	-39 / 0	-129.6	-129.6	0.07 (1)	6.25
C- D	-28 / 0	-129.6	-129.6	0.06 (1)	6.25
D- E	-22 / 0	-129.6	-129.6	0.06 (1)	6.25
E- F	-17 / 0	-129.6	-129.6	0.06 (1)	6.25
F- G	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
G- H	-13 / 0	-129.6	-129.6	0.06 (1)	6.25
H- I	-17 / 0	-129.6	-129.6	0.06 (1)	6.25
I- J	-22 / 0	-129.6	-129.6	0.06 (1)	6.25
J- K	-28 / 0	-129.6	-129.6	0.06 (1)	6.25
K- L	-39 / 0	-129.6	-129.6	0.07 (1)	6.25
L- M	0 / 39	-129.6	-129.6	0.17 (1)	10.00
N- L	-332 / 0	0.0	0.0	0.02 (1)	7.81
Y- X	0 / 34	-18.5	-18.5	0.02 (1)	10.00
X- W	0 / 25	-18.5	-18.5	0.02 (4)	10.00
W- V	0 / 19	-18.5	-18.5	0.02 (4)	10.00
V- U	0 / 15	-18.5	-18.5	0.02 (4)	10.00
U- T	0 / 12	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 12	-18.5	-18.5	0.02 (4)	10.00
S- R	0 / 15	-18.5	-18.5	0.02 (4)	10.00
R- Q	0 / 15	-18.5	-18.5	0.02 (4)	10.00
Q- P	0 / 19	-18.5	-18.5	0.02 (4)	10.00
P- O	0 / 25	-18.5	-18.5	0.02 (4)	10.00
O- N	0 / 34	-18.5	-18.5	0.02 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- G	-257 / 0	0.16 (1)
U- F	-262 / 0	0.11 (1)
V- E	-256 / 0	0.07 (1)
W- D	-256 / 0	0.05 (1)
X- C	-259 / 0	0.04 (1)
S- H	-262 / 0	0.11 (1)
Q- I	-256 / 0	0.07 (1)
P- J	-256 / 0	0.05 (1)
O- K	-259 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (L-M:1) , BC=0.02/1.00 (N-O:1) , WB=0.16/1.00 (G-T:1) , SSI=0.12/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (I) (INPUT = 0.90)
JSI METAL= 0.11 (L) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040329

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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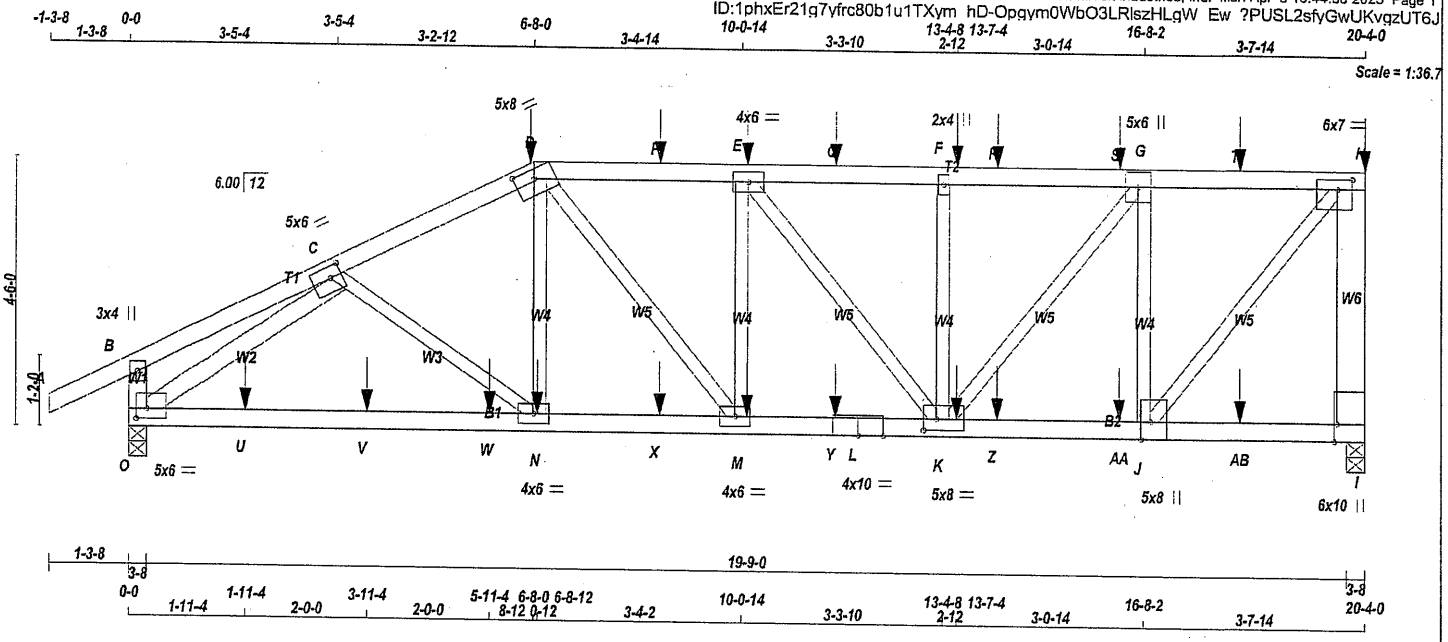


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW-h	MT20	5.0	8.0	2.00	4.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWW+t	MT20	5.0	6.0		
H	TMVW-t	MT20	6.0	7.0	2.00	3.00
I	BMV1+t	MT20	6.0	10.0	Edge	0.50
J	BMWW+t	MT20	5.0	8.0	Edge	1.75
K	BMWWW-t	MT20	5.0	8.0	2.25	2.50
L	BS-t	MT20	4.0	10.0		
M	BMWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMVW1-t	MT20	5.0	6.0	2.00	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040327

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	3091	0	3091	0	0	3-8	3-8
O	2885	0	2885	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2152	1606 / 0	0 / 0	0 / 0	0 / 0	546 / 0	0 / 0
O	2012	1480 / 0	0 / 0	0 / 0	0 / 0	533 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.19 (1)	10.00
B- C	0 / 17	-129.6	-129.6	0.20 (1)	10.00
C- D	-3919 / 0	-129.6	-129.6	0.43 (1)	3.16
D- P	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
P- E	-3855 / 0	-129.6	-129.6	0.60 (1)	2.92
E- Q	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
Q- F	-3403 / 0	-129.6	-129.6	0.54 (1)	3.17
F- R	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
R- S	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
S- G	-3403 / 0	-129.6	-129.6	0.59 (1)	3.09
G- T	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
T- H	-2216 / 0	-129.6	-129.6	0.50 (1)	3.94
I- H	-3053 / 0	0.0	0.0	0.64 (1)	5.99
O- B	-355 / 0	0.0	0.0	0.04 (1)	7.81
O- U	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
U- V	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
V- W	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
W- N	0 / 3237	-18.5	-18.5	0.78 (1)	10.00
N- X	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
X- M	0 / 3488	-18.5	-18.5	0.80 (1)	10.00
M- Y	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
Y- L	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
L- K	0 / 3855	-18.5	-18.5	0.77 (1)	10.00
K- Z	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
Z-AA	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
AA- J	0 / 2216	-18.5	-18.5	0.45 (1)	10.00
J-AB	0 / 0	-18.5	-18.5	0.10 (4)	10.00
AB- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- N	0 / 310	0.08 (1)
N- D	0 / 206	0.08 (4)
O- C	-4057 / 0	0.89 (1)
J- H	0 / 3379	0.84 (1)
D- M	0 / 583	0.14 (1)
J- G	-2411 / 0	0.75 (1)
M- E	-367 / 9	0.11 (1)
K- G	0 / 1922	0.48 (1)
E- K	-733 / 0	0.35 (1)
K- F	-808 / 0	0.25 (1)

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	6-8-0	-561	-561	—	FRONT	VERT	TOTAL	—	C1
E	10-2-0	-133	-133	—	FRONT	VERT	TOTAL	—	C1
F	13-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
H	20-4-0	-174	-174	—	FRONT	VERT	TOTAL	—	C1
K	13-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
M	10-2-0	-26	-26	—	FRONT	VERT	TOTAL	—	C1
N	6-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1

TOTAL WEIGHT = 2 X 94 = 187 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.64/1.00 (H-I:1) , BC=0.80/1.00 (M-N:1) , WB=0.89/1.00 (C-O:1) , SSI=0.41/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 0.94 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T93	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

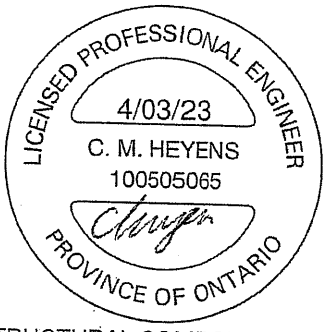
SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
P	8-8-12	-133	-133	—	FRONT	VERT	TOTAL	—	C1
Q	11-7-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
R	14-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
S	16-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
T	18-3-4	-133	-133	—	FRONT	VERT	TOTAL	—	C1
U	1-11-4	-25	-25	—	FRONT	VERT	TOTAL	—	C1
V	3-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
W	5-11-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
X	8-8-12	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Y	11-7-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
Z	14-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AA	16-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1
AB	18-3-4	-26	-26	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A
MINIMUM OF 2X4 SPF #2



STRUCTURAL COMPONENT ONLY
DWG # TR23040327

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	T94	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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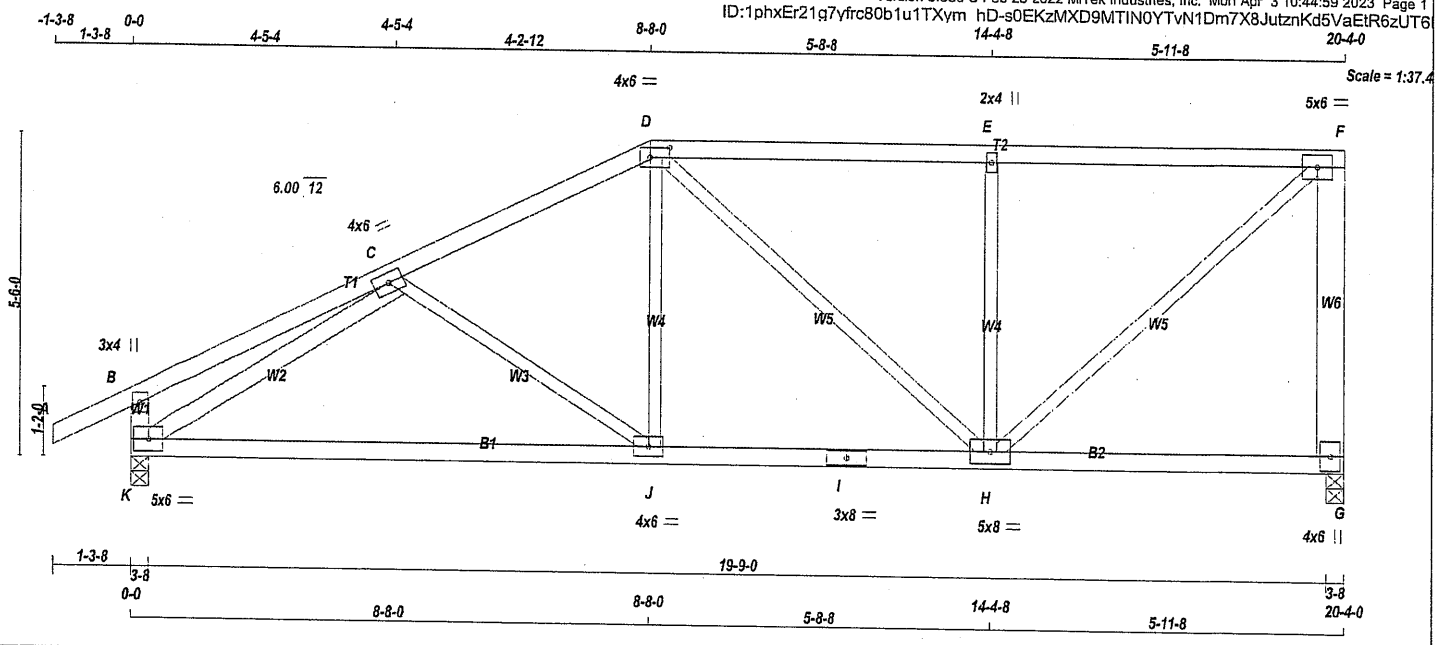

TOTAL WEIGHT = 2 X 89 = 177 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TTWW-I	MT20	4.0	6.0	2.00	4.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	5.0	6.0		
G	BMV1+p	MT20	4.0	6.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BS-t	MT20	3.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1506	0	1506	0	0	3-8	3-8
K	1682	0	1682	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	1050	777 / 0	0 / 0	0 / 0	0 / 0	272 / 0	0 / 0
K	1169	881 / 0	0 / 0	0 / 0	0 / 0	289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.13 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	
B- C	0 / 29	-129.6	-129.6	0.38 (1)	10.00	
C- D	-1659 / 0	-129.6	-129.6	0.44 (1)	4.61	
D- E	-1327 / 0	-129.6	-129.6	0.82 (1)	4.13	
E- F	-1327 / 0	-129.6	-129.6	0.83 (1)	4.13	
G- F	-1457 / 0	0.0	0.0	0.50 (1)	7.81	
K- B	-395 / 0	0.0	0.0	0.04 (1)	7.81	
K- J	0 / 1750	-18.5	-18.5	0.45 (1)	10.00	
J- I	0 / 1464	-18.5	-18.5	0.39 (1)	10.00	
I- H	0 / 1464	-18.5	-18.5	0.39 (1)	10.00	
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-356 / 0	0.18 (1)
J- D	0 / 363	0.08 (1)
K- C	-2133 / 0	0.72 (1)
H- F	0 / 1763	0.40 (1)
D- H	-185 / 0	0.22 (1)
H- E	-938 / 0	0.43 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.83/1.00 (E-F:1) , BC=0.45/1.00 (J-K:1) , WB=0.72/1.00 (C-K:1) , SSI=0.37/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.44 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040328

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	G95	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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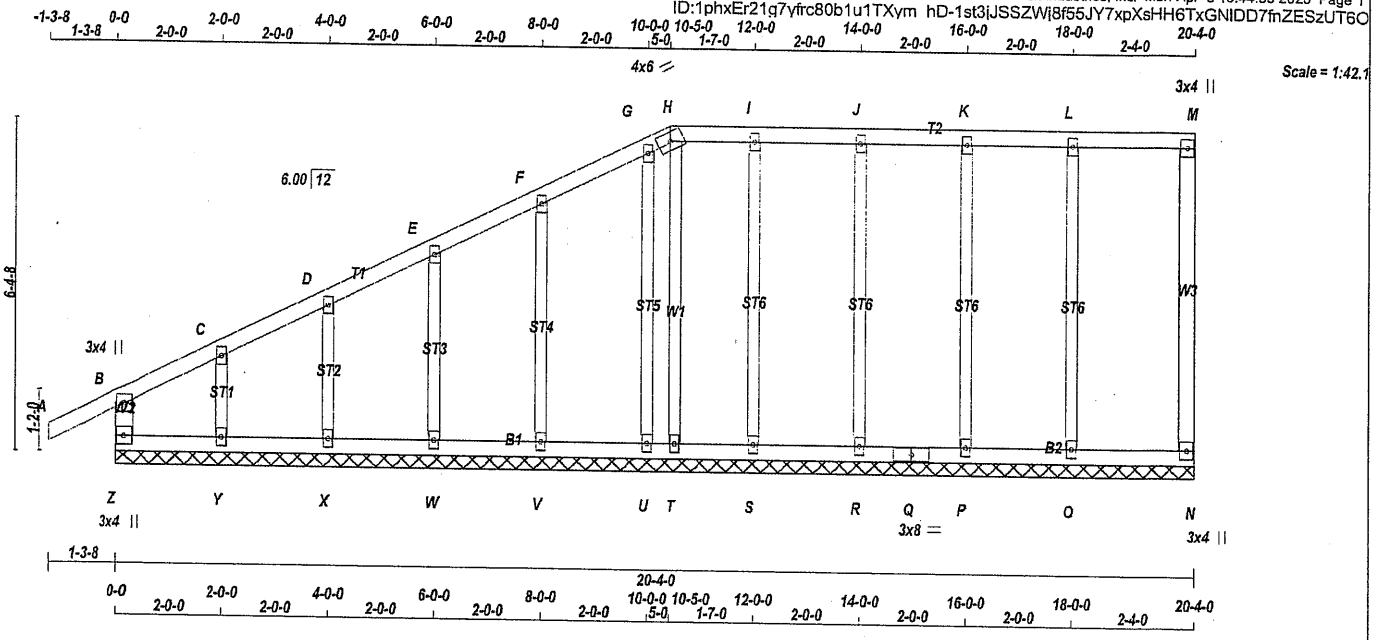

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Z - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - M	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
Z - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I, J, K, L						
C	TMW+w	MT20	2.0	4.0		
H	TTW-h	MT20	4.0	6.0		
M	TMV+p	MT20	3.0	4.0		
N	BMV1+p	MT20	3.0	4.0		
O, P, R, S, T, U, V, W, X, Y						
O	BMW1+w	MT20	2.0	4.0		
Q	BS-t	MT20	3.0	8.0		
Z	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- B	-317 / 0	0.0	0.0	0.02 (1)	7.81	Y- C	-249 / 0	0.04 (1)
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00	X- D	-258 / 0	0.05 (1)
B- C	-36 / 0	-129.6	-129.6	0.06 (1)	6.25	W- E	-256 / 0	0.07 (1)
C- D	-25 / 0	-129.6	-129.6	0.06 (1)	6.25	V- F	-266 / 0	0.11 (1)
D- E	-19 / 0	-129.6	-129.6	0.06 (1)	6.25	U- G	-159 / 0	0.10 (1)
E- F	-14 / 0	-129.6	-129.6	0.06 (1)	6.25	S- I	-230 / 0	0.15 (1)
F- G	-13 / 0	-129.6	-129.6	0.06 (1)	6.25	R- J	-267 / 0	0.17 (1)
G- H	-11 / 0	-129.6	-129.6	0.04 (1)	6.25	P- K	-249 / 0	0.16 (1)
H- I	-5 / 0	-129.6	-129.6	0.04 (1)	10.00	O- L	-304 / 0	0.20 (1)
I- J	-5 / 0	-129.6	-129.6	0.06 (1)	10.00	T- H	-120 / 0	0.08 (1)
J- K	-5 / 0	-129.6	-129.6	0.06 (1)	10.00			
K- L	-5 / 0	-129.6	-129.6	0.08 (1)	10.00			
L- M	-5 / 0	-129.6	-129.6	0.08 (1)	10.00			
N- M	-136 / 0	0.0	0.0	0.04 (1)	7.81			
Z- Y	0 / 32	-18.5	-18.5	0.02 (1)	10.00			
Y- X	0 / 23	-18.5	-18.5	0.02 (4)	10.00			
X- W	0 / 17	-18.5	-18.5	0.02 (4)	10.00			
W- V	0 / 13	-18.5	-18.5	0.02 (4)	10.00			
V- U	0 / 9	-18.5	-18.5	0.02 (4)	10.00			
U- T	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
T- S	0 / 5	-18.5	-18.5	0.01 (4)	10.00			
S- R	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
R- Q	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
Q- P	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
P- O	0 / 5	-18.5	-18.5	0.02 (4)	10.00			
O- N	0 / 5	-18.5	-18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 95 = 190 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1) , BC=0.02/1.00 (O-P:4) , WB=0.20/1.00 (L-O:1) , SSI=0.13/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.38 (E) (INPUT = 0.90)
JSI METAL= 0.11 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040319

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T96	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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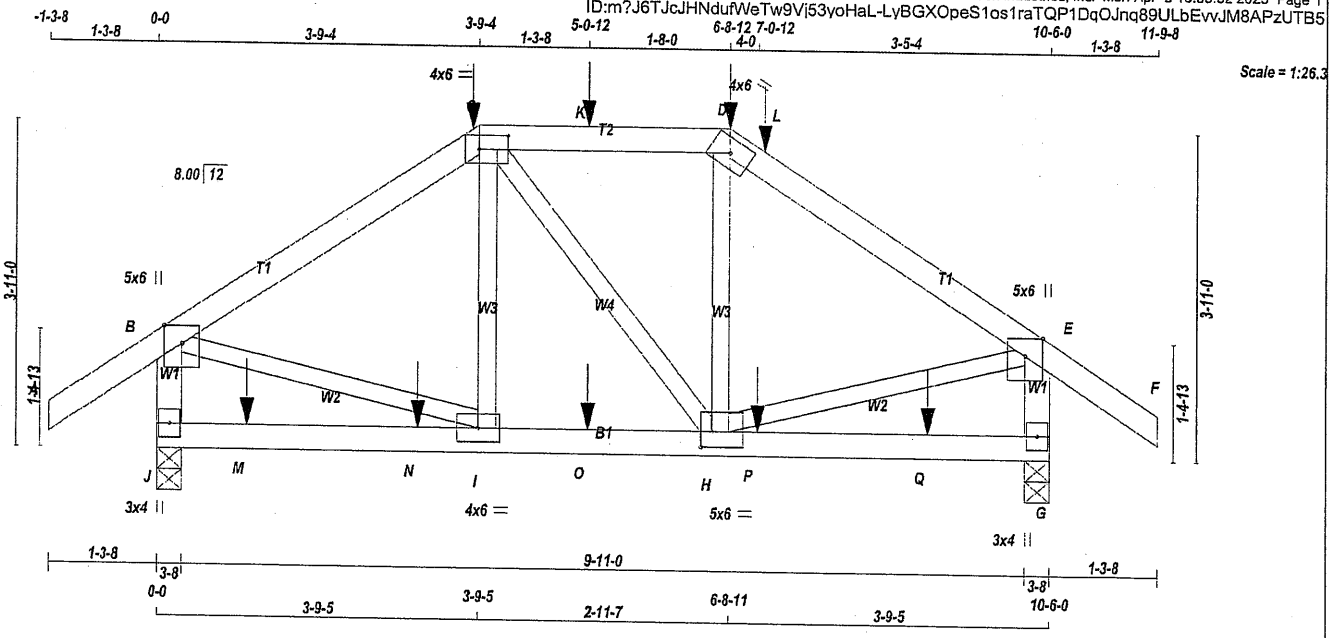

TOTAL WEIGHT = 47 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-I	MT20	4.0	6.0	2.00	4.00
D	TTW-h	MT20	4.0	6.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.25	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1516	0	1516	0	0	3-8	3-8
G	1569	0	1569	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1053	800 / 0	0 / 0	0 / 0	0 / 0	252 / 0	0 / 0
G	1088	834 / 0	0 / 0	0 / 0	0 / 0	254 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 49	-129.6	-129.6	0.19 (1)	10.00	I- C	-198 / 50	0.05 (1)
B- C	-1323 / 0	-129.6	-129.6	0.39 (1)	5.09	C- H	0 / 66	0.02 (1)
C- K	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40	H- D	-261 / 43	0.07 (1)
K- D	-1133 / 0	-129.6	-129.6	0.38 (1)	5.40	B- I	0 / 1140	0.28 (1)
D- L	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98	H- E	0 / 1188	0.29 (1)
L- E	-1338 / 0	-129.6	-129.6	0.44 (1)	4.98			
E- F	0 / 49	-129.6	-129.6	0.19 (1)	10.00			
J- B	-1465 / 0	0.0	0.0	0.17 (1)	6.72			
G- E	-1520 / 0	0.0	0.0	0.17 (1)	6.62			
J- M	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
M- N	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
N- I	0 / 0	-18.5	-18.5	0.11 (4)	10.00			
I- O	0 / 1093	-18.5	-18.5	0.24 (1)	10.00			
O- H	0 / 1093	-18.5	-18.5	0.24 (1)	10.00			
H- P	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
P- Q	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
Q- G	0 / 0	-18.5	-18.5	0.12 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-9-4	-34	-34	—	FRONT	VERT	DEAD	—	C1
C	3-9-4	-217	-217	—	FRONT	VERT	SNOW	—	C1
D	6-8-12	-34	-34	—	FRONT	VERT	DEAD	—	C1
D	6-8-12	-217	-217	—	FRONT	VERT	SNOW	—	C1
K	5-0-12	-94	-94	—	BACK	VERT	TOTAL	—	C1
L	7-0-12	-123	-123	—	BACK	VERT	TOTAL	—	C1
M	1-0-12	-19	-19	—	BACK	VERT	TOTAL	—	C1
N	3-0-12	-19	-19	—	BACK	VERT	TOTAL	—	C1
O	5-0-12	-19	-19	—	BACK	VERT	TOTAL	—	C1
P	7-0-12	-19	-19	—	BACK	VERT	TOTAL	—	C1
Q	9-0-12	-19	-19	—	BACK	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.44/1.00 (D-E:1) , BC=0.24/1.00 (H-I:1) , WB=0.29/1.00 (E-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
JSI METAL= 0.60 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040316

TOWN OF INNISFIL Block 4 REVIEWED

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T97	7	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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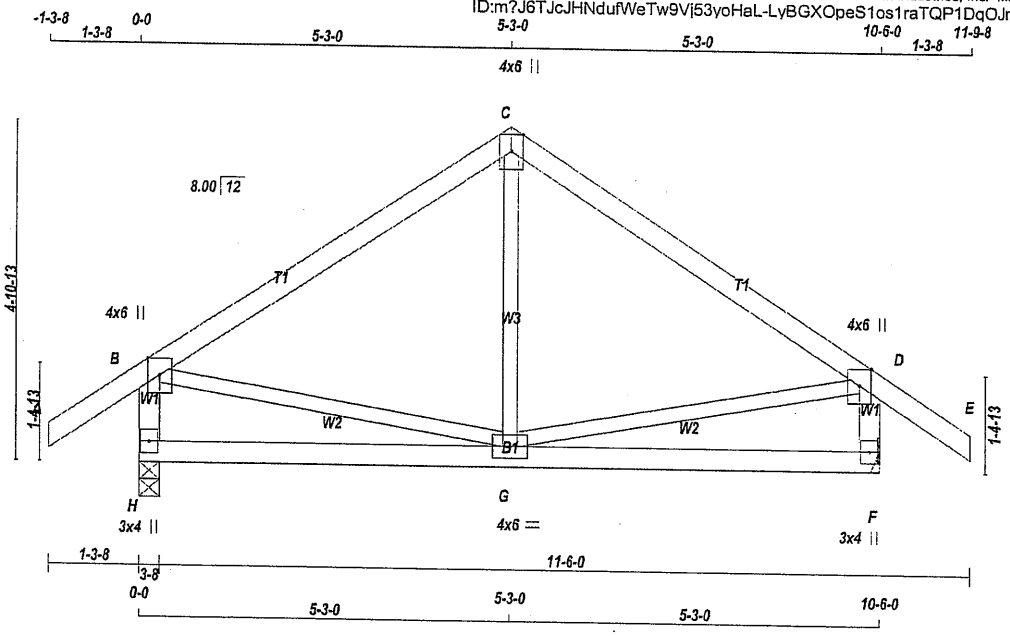

TOTAL WEIGHT = 7 X 44 = 308 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	956	0	956	0	0	IN-SX	IN-SX
F	956	0	956	0	0	3-8 MECHANICAL	3-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	663	506 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
B- C	-551 / 0	-129.6	-129.6	0.46 (1)	6.25	
C- D	-551 / 0	-129.6	-129.6	0.46 (1)	6.25	
D- E	0 / 49	-129.6	-129.6	0.17 (1)	10.00	
H- B	-918 / 0	0.0	0.0	0.09 (1)	7.81	
F- D	-918 / 0	0.0	0.0	0.09 (1)	7.81	
H- G	0 / 0	-18.5	-18.5	0.14 (4)	10.00	
G- F	0 / 0	-18.5	-18.5	0.14 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- C	-70 / 82	0.03 (4)
B- G	0 / 468	0.11 (1)
G- D	0 / 468	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.46/1.00 (C-D:1) , BC=0.14/1.00 (F-G:4) , WB=0.11/1.00 (D-G:1) , SSI=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (G) (INPUT = 0.90)
JSI METAL= 0.39 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040317

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G97	1	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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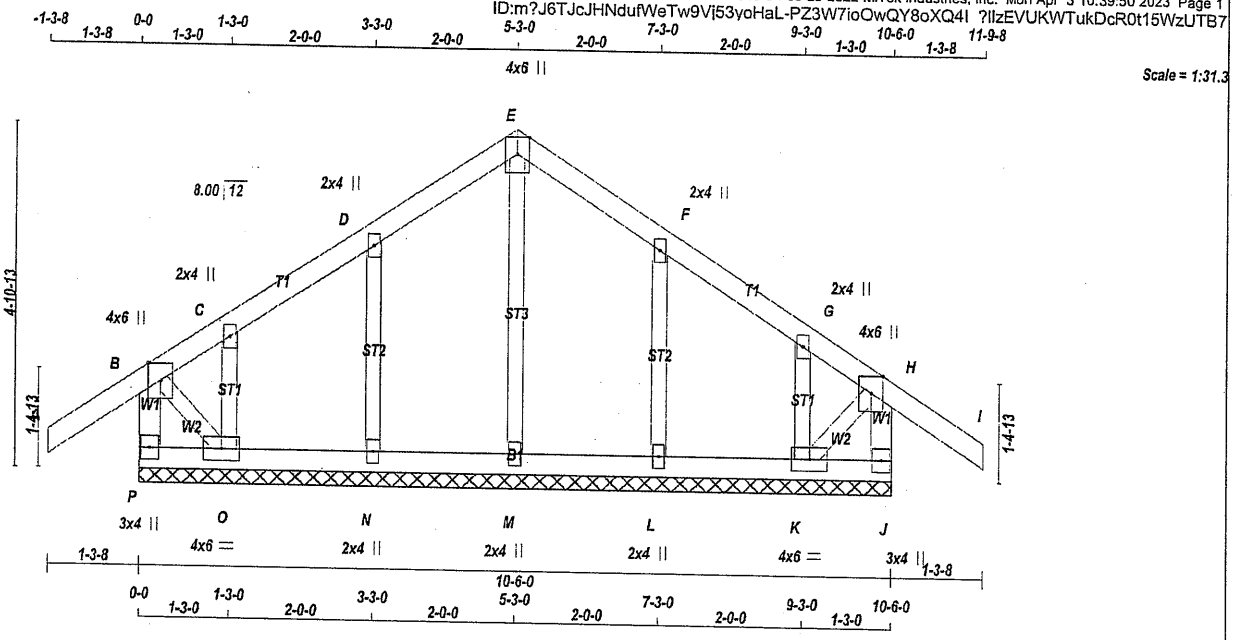

TOTAL WEIGHT = 46 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
P - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C, D, F, G						
C	TMW+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	6.0	Edge	
H	TMVW+p	MT20	4.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L, M, N						
L	BMW1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 49	-129.6	-129.6	0.17 (1)	10.00
B- C	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
C- D	0 / 0	-129.6	-129.6	0.08 (1)	10.00
D- E	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
E- F	-20 / 0	-129.6	-129.6	0.08 (1)	6.25
F- G	0 / 0	-129.6	-129.6	0.08 (1)	10.00
G- H	-70 / 0	-129.6	-129.6	0.17 (1)	6.25
H- I	0 / 49	-129.6	-129.6	0.17 (1)	10.00
P- B	-371 / 0	0.0	0.0	0.04 (1)	7.81
J- H	-371 / 0	0.0	0.0	0.04 (1)	7.81
P- O	0 / 0	-18.5	-18.5	0.01 (4)	10.00
O- N	0 / 11	-18.5	-18.5	0.02 (4)	10.00
N- M	0 / 3	-18.5	-18.5	0.02 (4)	10.00
M- L	0 / 3	-18.5	-18.5	0.02 (4)	10.00
L- K	0 / 11	-18.5	-18.5	0.02 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- E	-206 / 0	0.07 (1)
N- D	-315 / 0	0.07 (1)
O- C	-83 / 0	0.01 (1)
L- F	-315 / 0	0.07 (1)
K- G	-83 / 0	0.01 (1)
B- O	0 / 20	0.00 (1)
K- H	0 / 20	0.00 (1)

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (H-I:1) , BC=0.02/1.00 (N-O:4) , WB=0.07/1.00 (E-M:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040313

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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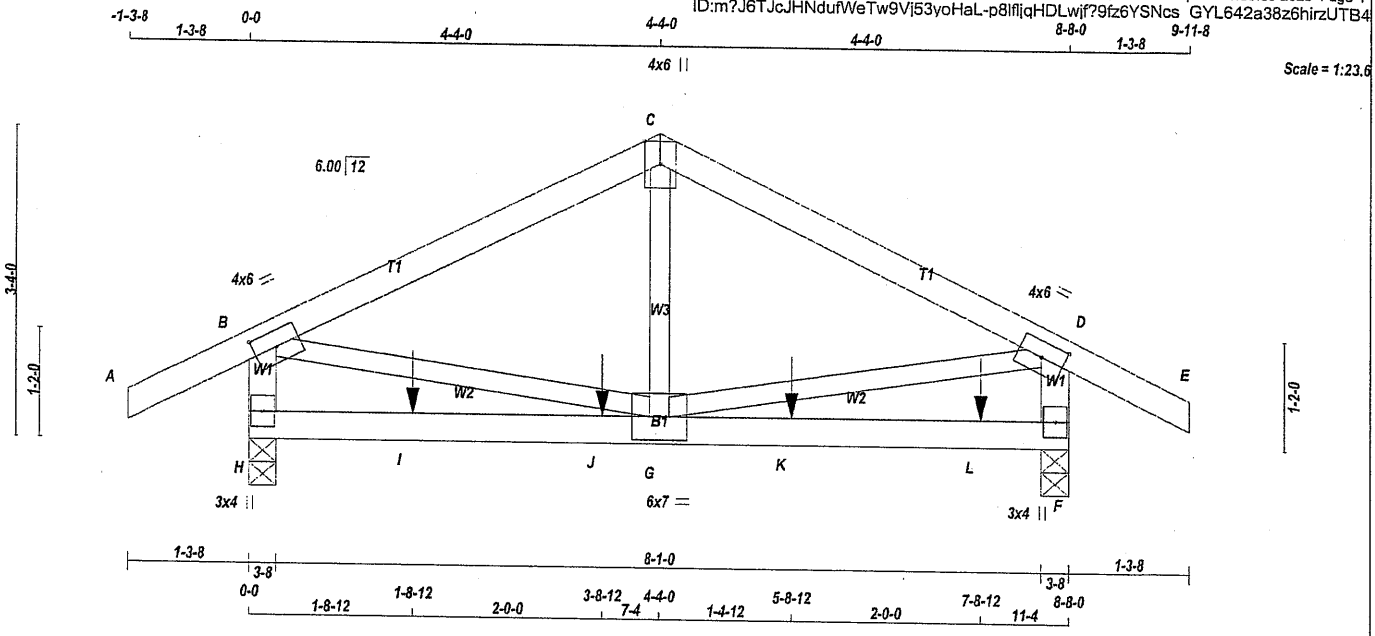


TOTAL WEIGHT = 2 X 35 = 70 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
H- B	1	12	TOP
F- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- F	1	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

STRUCTURAL COMPONENT ONLY
DWG # TR23040318

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2521	0	2521	0	0	3-8	3-8
F	2864	0	2864	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1746	1355 / 0	0 / 0	0 / 0	0 / 0	391 / 0	0 / 0
F	1983	1540 / 0	0 / 0	0 / 0	0 / 0	444 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.09 (1)	10.00	G- C	0 / 1721	0.21 (1)
B- C	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	B- G	0 / 2327	0.29 (1)
C- D	-2552 / 0	-129.6	-129.6	0.27 (1)	5.36	G- D	0 / 2327	0.29 (1)
D- E	0 / 39	-129.6	-129.6	0.09 (1)	10.00			
H- B	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
F- D	-2050 / 0	0.0	0.0	0.11 (1)	7.72			
H- I	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
I- J	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
J- G	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
G- K	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
K- L	0 / 0	-18.5	-18.5	0.85 (1)	10.00			
L- F	0 / 0	-18.5	-18.5	0.85 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
J	3-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
K	5-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1
L	7-8-12	-649	-649	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1**: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. 'G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.29")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.29")
CALCULATED VERT. DEFL.(TL) = L/ 837 (0.12")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.85/1.00 (G-H:1) , WB=0.29/1.00 (B-G:1) , SSI=0.64/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (G) (INPUT = 0.90)
JSI METAL= 0.31 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

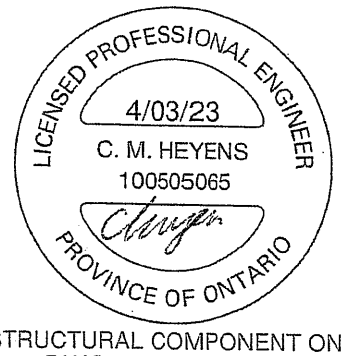
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	T98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	4.0	6.0	2.00	3.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	6.0	7.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



STRUCTURAL COMPONENT ONLY
DWG # TR23040318

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	G98	1	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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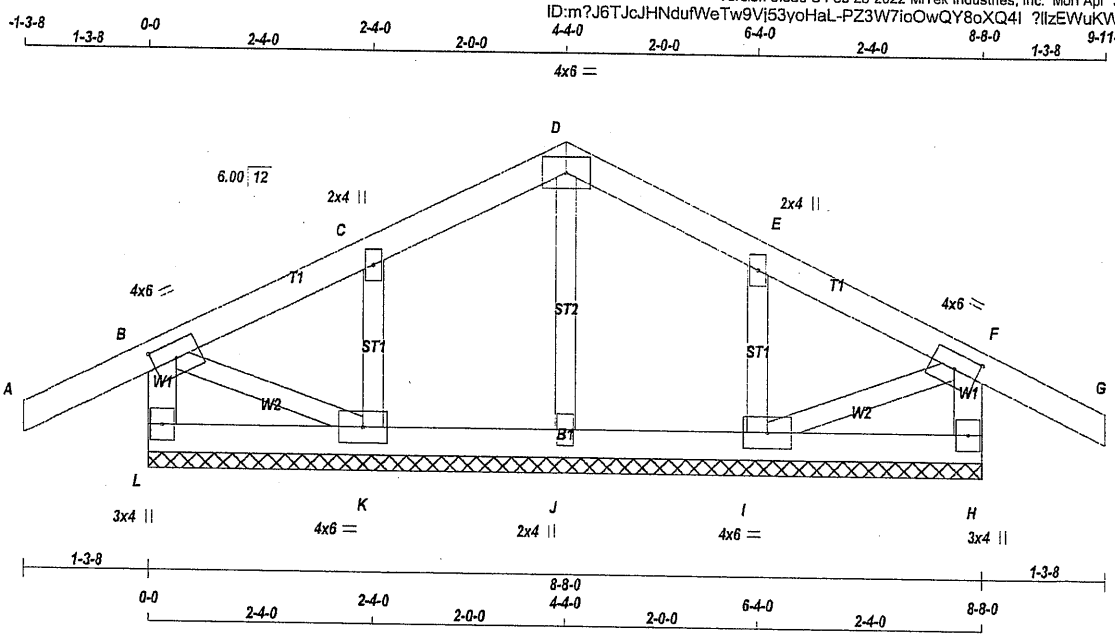

TOTAL WEIGHT = 2 X 35 = 69 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	TOP
L- B	1	12	TOP
H- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	2.00	3.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	6.0	2.00	3.00
H	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 39	-129.6	-129.6	0.08 (1)	10.00	J- D	-188 / 0	0.02 (1)
B- C	-6 / 0	-129.6	-129.6	0.05 (1)	10.00	K- C	-345 / 0	0.03 (1)
C- D	-25 / 0	-129.6	-129.6	0.05 (1)	6.25	I- E	-345 / 0	0.03 (1)
D- E	-25 / 0	-129.6	-129.6	0.05 (1)	6.25	B- K	0 / 20	0.00 (1)
E- F	-6 / 0	-129.6	-129.6	0.05 (1)	10.00	I- F	0 / 20	0.00 (1)
F- G	0 / 39	-129.6	-129.6	0.08 (1)	10.00			
L- B	-306 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-306 / 0	0.0	0.0	0.02 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.01 (4)	10.00			
K- J	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
J- I	0 / 7	-18.5	-18.5	0.01 (4)	10.00			
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00			

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (F-G:1) , BC=0.01/1.00 (H-I:4) , WB=0.03/1.00 (E-I:1) , SSI=0.06/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (C) (INPUT = 0.90)
JSI METAL= 0.07 (C) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
404995	G98	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
I	BMWW1-t	MT20	4.0	6.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040314

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J1	45	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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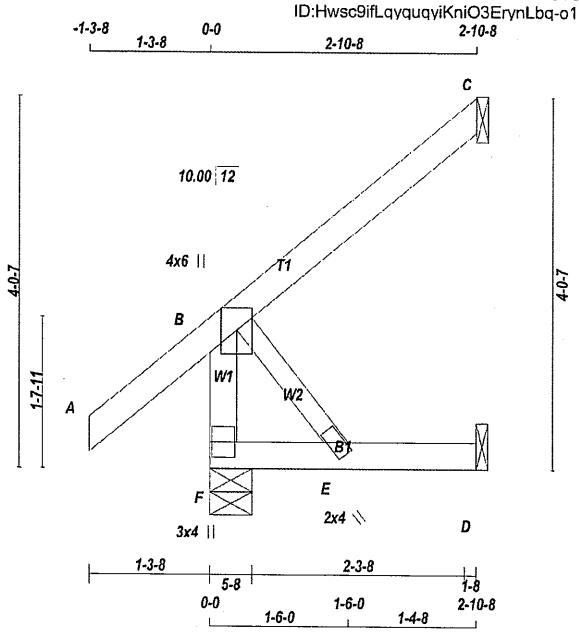

TOTAL WEIGHT = 45 X 13 = 563 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	393	0	393	0	0	5-8	5-8
C	186	0	186	0	0	1-8	1-8
D	27	0	30	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	271	216 / 0	0 / 0	0 / 0	0 / 0	55 / 0	0 / 0
C	127	110 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	21	0 / 0	0 / 0	0 / 0	0 / 0	21 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-366 / 0	0.0	0.0	0.04 (1)		7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 57	-129.6	-129.6	0.18 (1)		10.00			
B- C	0 / 0	-129.6	-129.6	0.18 (1)		10.00			
F- E	0 / 0	-18.5	-18.5	0.04 (4)		10.00			
E- D	0 / 0	-18.5	-18.5	0.04 (4)		10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.18/1.00 (B-C:1) , BC=0.04/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040209

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J2	8	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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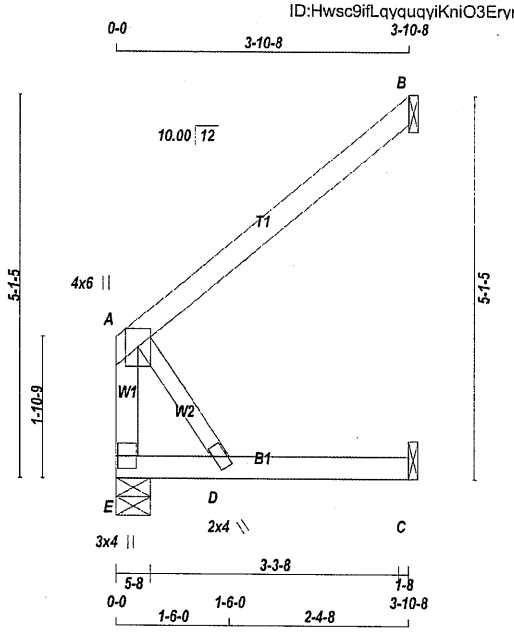

TOTAL WEIGHT = 8 X 14 = 109 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	287	0	287	0	0	5-8	5-8
B	251	0	251	0	0	1-8	1-8
C	36	0	40	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	200	148 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
B	171	148 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
C	29	0 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-251 / 0	0.0	0.0	0.03 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.33 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.07 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.33/1.00 (A-B:1) , BC=0.08/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (A) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040210

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J3	20	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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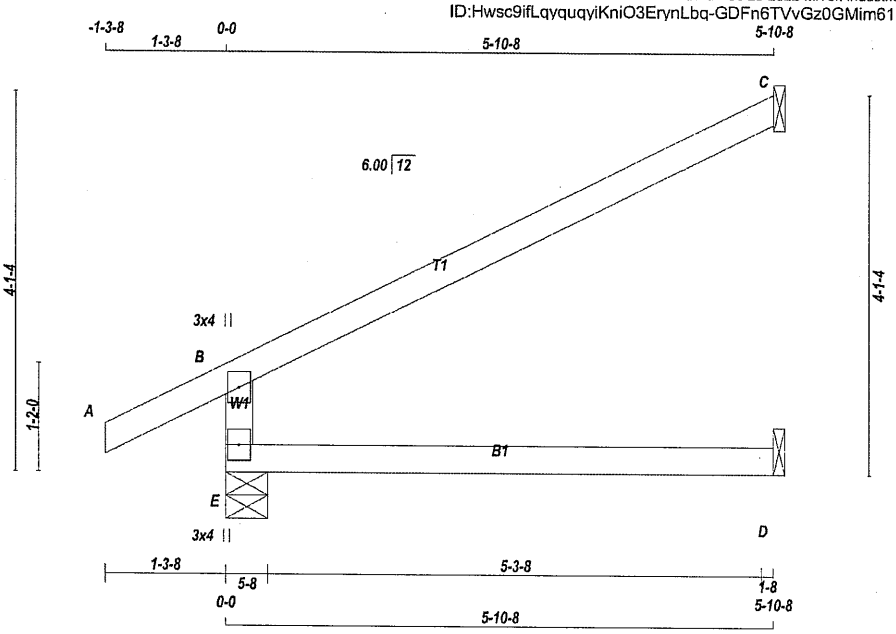

TOTAL WEIGHT = 20 X 17 = 336 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	716	0	716	0	0	5-8	5-8
C	286	0	286	0	0	1-8	1-8
D	45	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	496	384 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	195	168 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	36	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-652 / 0	0.0	0.0	0.13 (4)		7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)		10.00			
B- C	-43 / 0	-129.6	-129.6	0.76 (1)		6.25			
E- D	0 / 0	-18.5	-18.5	0.13 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040211

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

[M]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J4	15	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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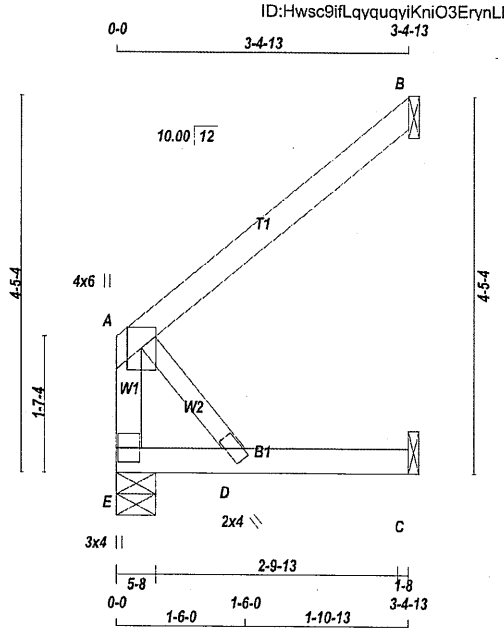

TOTAL WEIGHT = 15 X 12 = 179 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	Edge	
D	BMW+w	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	252	0	252	0	0	5-8	5-8
B	220	0	220	0	0	1-8	1-8
C	31	0	35	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	176	130 / 0	0 / 0	0 / 0	0 / 0	46 / 0	0 / 0
B	150	130 / 0	0 / 0	0 / 0	0 / 0	20 / 0	0 / 0
C	25	0 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- A	-220 / 0	0.0	0.0	0.02 (1)	7.81	A- D	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.26 (1)	10.00			
E- D	0 / 0	-18.5	-18.5	0.06 (4)	10.00			
D- C	0 / 0	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.06/1.00 (C-D:4) , WB=0.00/1.00 (A-D:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (A) (INPUT = 0.90)
JSI METAL= 0.06 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040212

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J50	18	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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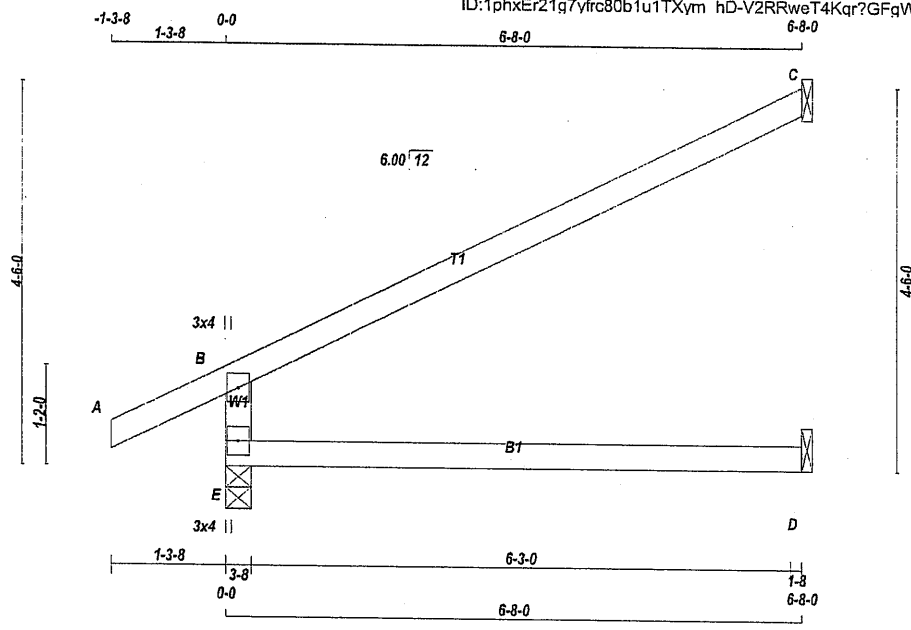

TOTAL WEIGHT = 18 X 19 = 337 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	789	0	789	0	0	3-8	3-8
C	324	0	324	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	547	422 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
C	221	191 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-716 / 0	0.0	0.0	0.14 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-48 / 0	-129.6	-129.6	0.76 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.76/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.38/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.29 (E) (INPUT = 0.90)
JSI METAL= 0.20 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040320

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J51	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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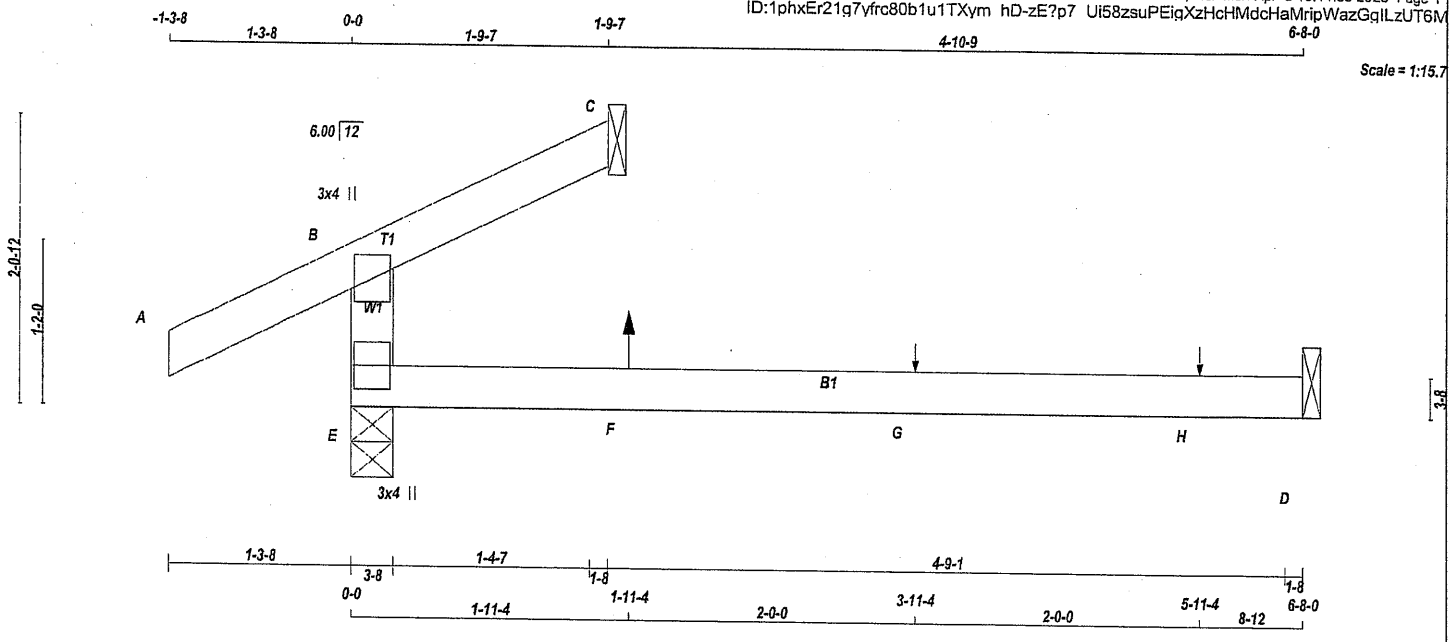

TOTAL WEIGHT = 2 X 12 = 25 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	386	0	386	0	0	3-8	3-8
C	86	0	86	0	0	1-8	1-8
D	49	0	59	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	268	206 / 0	0 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	63	30 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0
D	39	0 / -3	0 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-324 / 0	0.0	0.0	0.12 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-15 / 14	-129.6	-129.6	0.12 (4)	6.25			
E- F	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
F- G	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
G- H	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
H- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	7	1	12	FRONT	VERT	TOTAL	—	C1
G	3-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1
H	5-11-4	1	1	—	FRONT	VERT	TOTAL	—	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (E) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040321

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J52	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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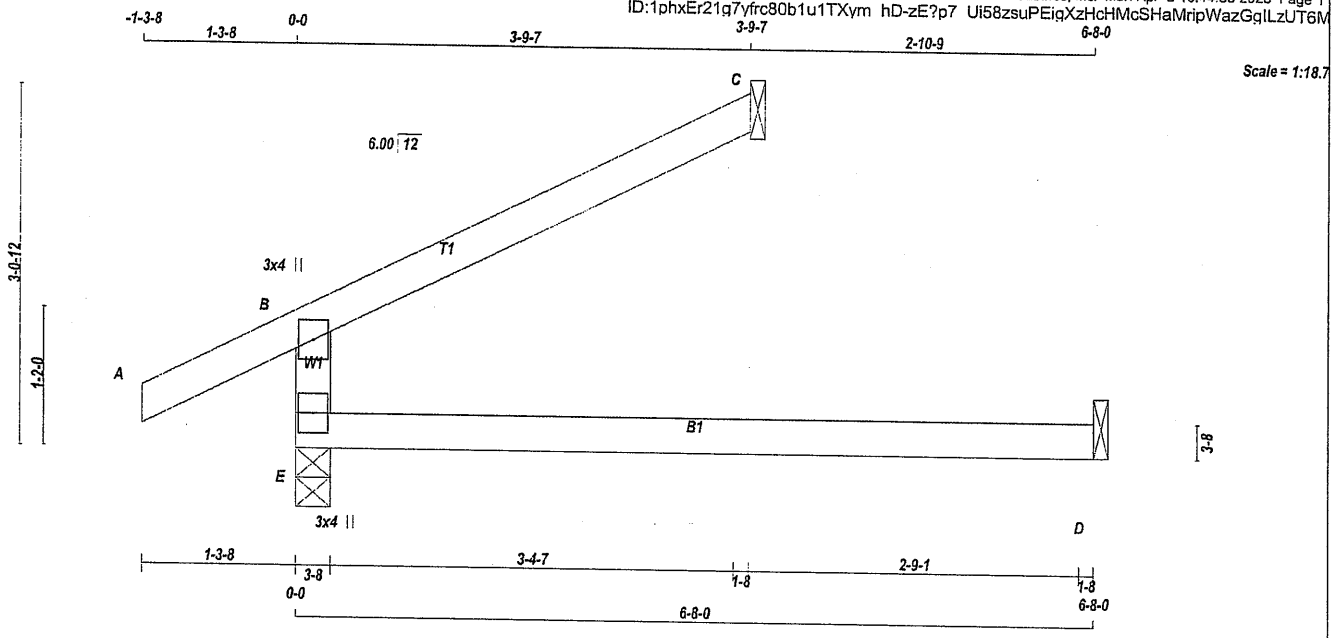

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	555	0	555	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	387	284 / 0	0 / 0	0 / 0	0 / 0	103 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-483 / 0	0.0	0.0	0.15 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-27 / 0	-129.6	-129.6	0.24 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 2 X 15 = 30 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54,3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.24/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040322

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J53	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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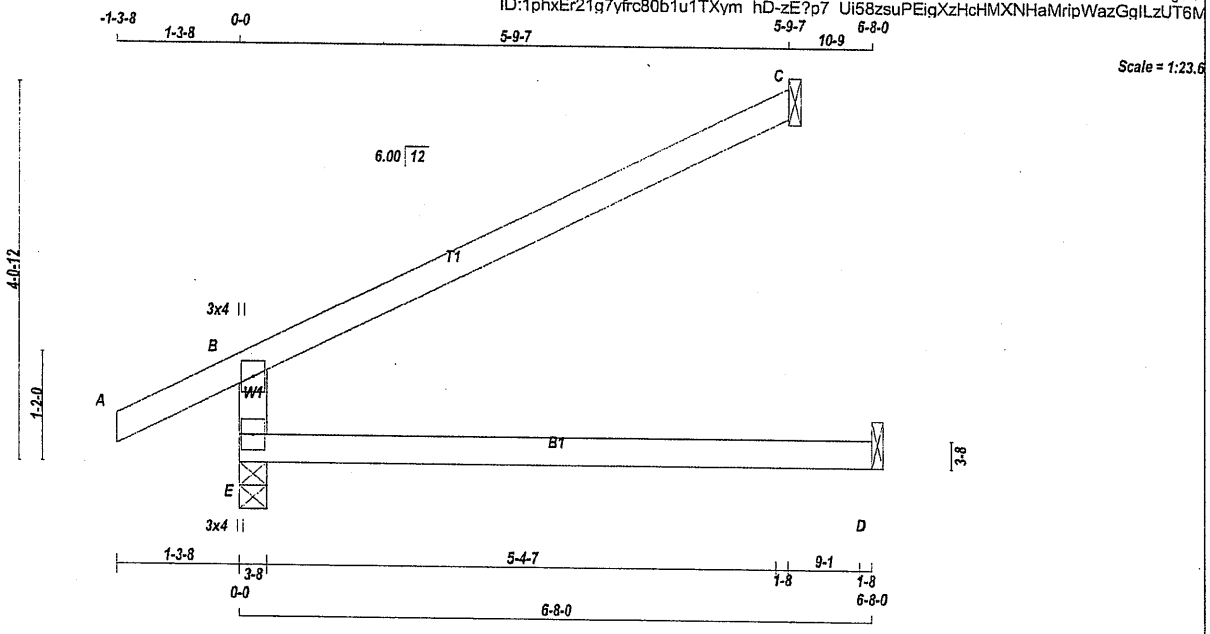

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	718	0	718	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	51	0	57	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	498	380 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	40	0 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-645 / 0	0.0	0.0	0.15 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-42 / 0	-129.6	-129.6	0.57 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

TOTAL WEIGHT = 2 X 18 = 35 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.22")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.22")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.17/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040323

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J54	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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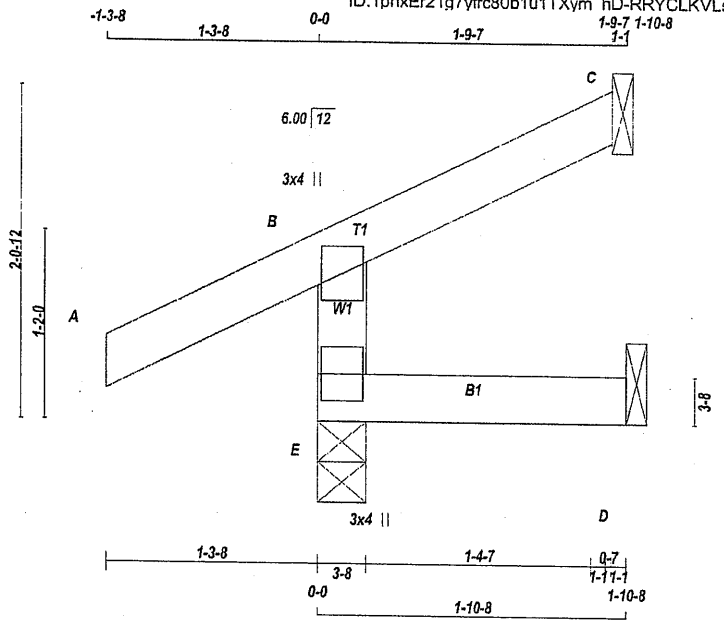

Scale = 1:13.6

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	375	0	375	0	0	3-8	3-8
C	62	0	62	0	-36	1-8	1-8
D	5	0	17	0	-8	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	258	211 / 0	0 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	43	36 / -27	0 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	5	0 / -12	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-345 / 0	0.0	0.0	0.05 (5)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-24 / 0	-129.6	-129.6	0.13 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.05 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040324

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

TOTAL WEIGHT = 2 X 7 = 14 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.05/1.00 (D-E:5) , WB=0.00/1.00 (n/a:0) , SSI=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J55	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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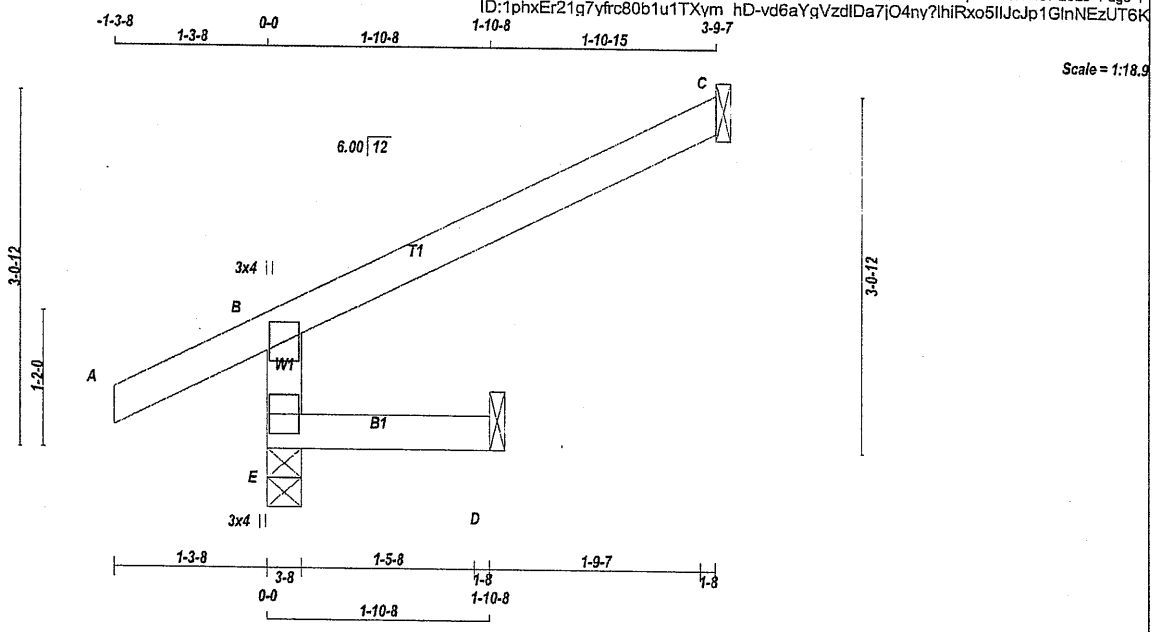

TOTAL WEIGHT = 2 X 10 = 19 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	502	0	502	0	0	3-8	3-8
C	184	0	184	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	344	284 / 0	0 / 0	0 / 0	0 / 0	60 / 0	0 / 0
C	126	109 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-483 / 0	0.0	0.0	0.01 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-27 / 0	-129.6	-129.6	0.32 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

PATTERN-LOADING CHECK APPLIED TO THIS TRUSS.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (E) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040325

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404958	J56	2	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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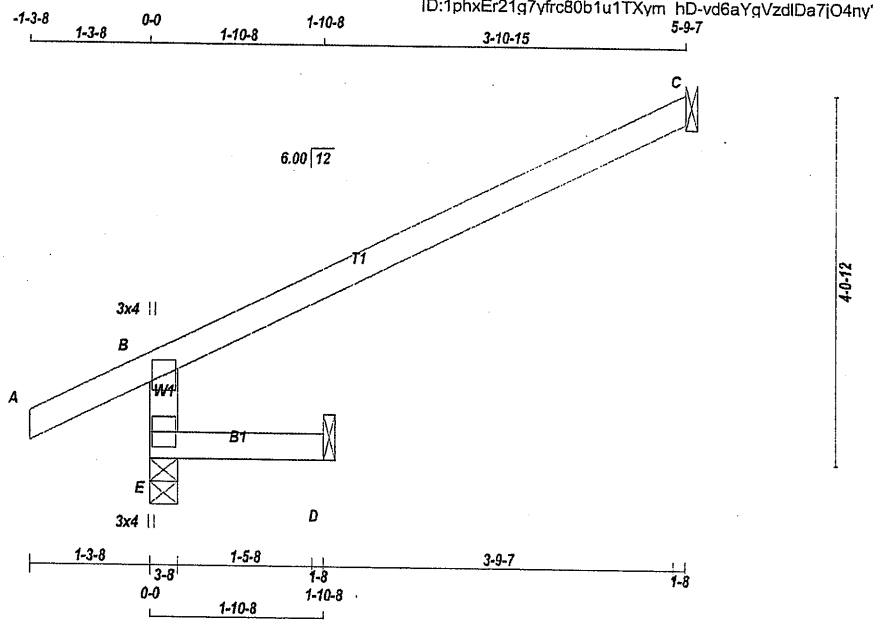

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	664	0	664	0	0	3-8	3-8
C	281	0	281	0	0	1-8	1-8
D	16	0	17	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	455	380 / 0	0 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	192	166 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	12	0 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-645 / 0	0.0	0.0	0.01 (1)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-42 / 0	-129.6	-129.6	0.74 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

TOTAL WEIGHT = 2 X 12 = 24 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.74/1.00 (B-C:1) , BC=0.02/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.33/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (E) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040326

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
404995	J57	5	1	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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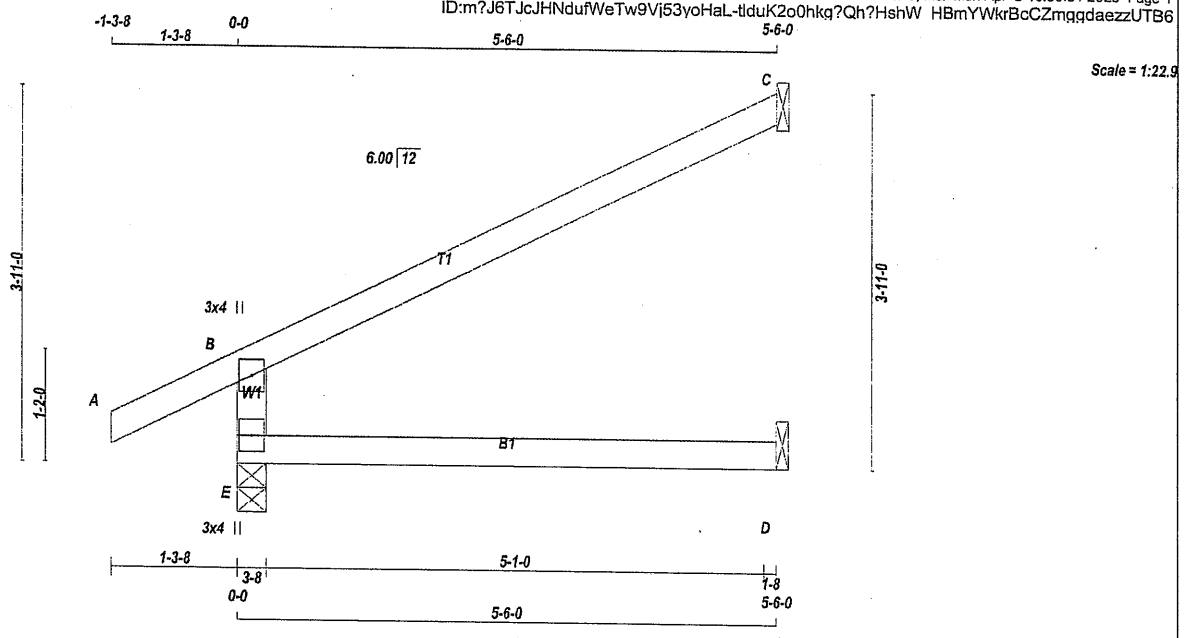

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	681	0	681	0	0	3-8	3-8
C	267	0	267	0	0	1-8	1-8
D	42	0	47	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL MSD2015-H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	472	366 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	182	158 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
D	34	0 / 0	0 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-621 / 0	0.0	0.0	0.11 (4)	7.81			
A- B	0 / 39	-129.6	-129.6	0.17 (1)	10.00			
B- C	-40 / 0	-129.6	-129.6	0.67 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

TOTAL WEIGHT = 5 X 16 = 79 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	38.2	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.11/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (E) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040315

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J200	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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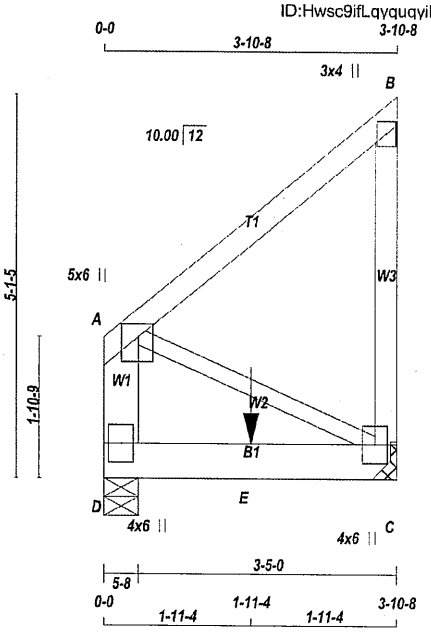

Scale = 1:29.4

TOTAL WEIGHT = 4 X 23 = 93 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040213

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
D	719	0	719	0	0	5-8	5-8
C	719	0	719	0	0	IN-SX MECHANICAL	IN-SX

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	500	376 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
C	500	376 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO			FR-TO		
D- A	-251 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.18 (1)	10.00			
C- B	-251 / 0	0.0	0.0	0.05 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.32 (1)	10.00			
E- C	0 / 0	-18.5	-18.5	0.32 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	-600	-600	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.18/1.00 (A-B:1) , BC=0.32/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.06 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	J200	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040213

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J300	2	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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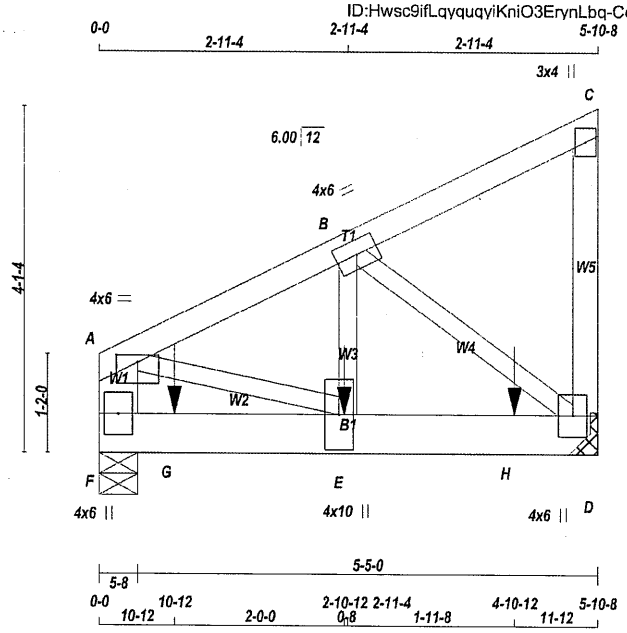

Scale = 1:26.1

TOTAL WEIGHT = 4 X 29 = 117 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- E	1	5	SIDE(158.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040214

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	2421	0	2421	0	0	5-8	5-8
D	2383	0	2383	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1721	1078 / 0	0 / 0	0 / 0	0 / 0	643 / 0	0 / 0
D	1694	1062 / 0	0 / 0	0 / 0	0 / 0	632 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1585 / 0	0.0	0.0	0.06 (1)	7.81	A- E	0 / 1932	0.24 (1)
A- B	-2074 / 0	-129.6	-129.6	0.09 (1)	6.05	E- B	0 / 1915	0.24 (1)
B- C	-15 / 0	-129.6	-129.6	0.07 (1)	6.25	B- D	-2351 / 0	0.28 (1)
D- C	-158 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.27 (1)	10.00			
E- H	0 / 1868	-18.5	-18.5	0.41 (1)	10.00			
H- D	0 / 1868	-18.5	-18.5	0.41 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	2-10-12	-936	-936	---	BACK	VERT	TOTAL	---	C1
G	10-12	-937	-937	---	BACK	VERT	TOTAL	---	C1
H	4-10-12	-936	-936	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.41/1.00 (D-E:1) , WB=0.28/1.00 (B-D:1) , SSI=0.28/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.42 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	J300	2	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	10.0		
F	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040214

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J300Z	3	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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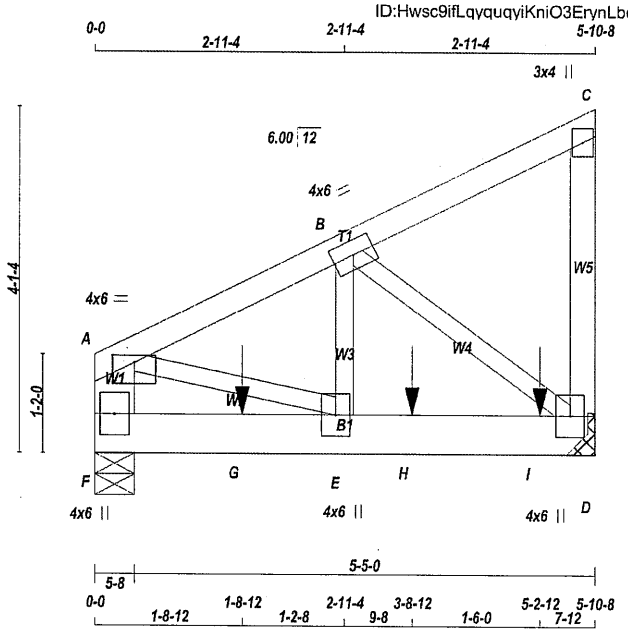

Scale = 1:26.1

TOTAL WEIGHT = 6 X 29 = 175 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	1	12	TOP
C- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	6.0	1.00	3.00
B	TMWW-t	MT20	4.0	6.0		
C	TMV+p	MT20	3.0	4.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040215

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1646	0	1646	0	0	5-8	5-8
D	2182	0	2182	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1143	869 / 0	0 / 0	0 / 0	0 / 0	273 / 0	0 / 0
D	1517	1146 / 0	0 / 0	0 / 0	0 / 0	371 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- A	-1336 / 0	0.0	0.0	0.05 (1)	7.81	A- E	0 / 1599	0.20 (1)
A- B	-1713 / 0	-129.6	-129.6	0.09 (1)	6.25	E- B	0 / 1503	0.19 (1)
B- C	-16 / 0	-129.6	-129.6	0.08 (1)	6.25	B- D	-1946 / 0	0.23 (1)
D- C	-155 / 0	0.0	0.0	0.02 (1)	7.81			
F- G	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
G- E	0 / 0	-18.5	-18.5	0.19 (1)	10.00			
E- H	0 / 1547	-18.5	-18.5	0.30 (1)	10.00			
H- I	0 / 1547	-18.5	-18.5	0.30 (1)	10.00			
I- D	0 / 1547	-18.5	-18.5	0.30 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-8-12	-752	-752	---	TOP	VERT	TOTAL	---	C1
H	3-8-12	-649	-649	---	FRONT	VERT	TOTAL	---	C1
I	5-2-12	-652	-652	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.09/1.00 (A-B:1) , BC=0.30/1.00 (D-E:1) , WB=0.23/1.00 (B-D:1) , SSI=0.36/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (D) (INPUT = 0.90)
JSI METAL= 0.35 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	J300Z	3	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMVW1+p	MT20	4.0	6.0		
E	BMWW+t	MT20	4.0	6.0		
F	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040215

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402637	J400	3	2	BAYVIEW WELLINGTON TRUSS DESC.	

Tamarack Roof Truss, Burlington

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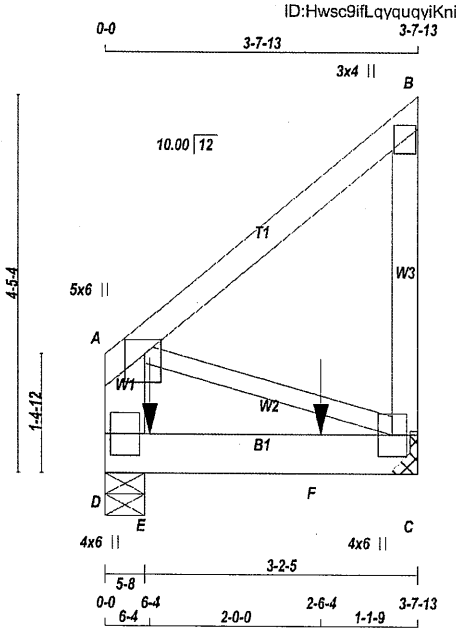

Scale = 1:25.9

TOTAL WEIGHT = 6 X 21 = 123 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - A	2x6	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D- A	2	12	TOP
A- B	1	12	TOP
B- C	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
D- C	2	8	SIDE(244.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMV+p	MT20	3.0	4.0		
C	BMVW1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040216

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	1213	0	1213	0	0	5-8	5-8
C	942	0	942	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	844	635 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
C	655	493 / 0	0 / 0	0 / 0	0 / 0	161 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- A	-237 / 0	0.0	0.0	0.01 (1)	7.81	A- C	0 / 0	0.00 (1)
A- B	0 / 0	-129.6	-129.6	0.16 (1)	10.00			
C- B	-237 / 0	0.0	0.0	0.03 (1)	7.81			
D- E	0 / 0	-18.5	-18.5	0.29 (1)	10.00			
E- F	0 / 0	-18.5	-18.5	0.29 (1)	10.00			
F- C	0 / 0	-18.5	-18.5	0.29 (1)	10.00			

SPECIFIED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	6-4	-562	-562	---	BACK	VERT	TOTAL	---	C1
F	2-6-4	-559	-559	---	BACK	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) **C1:** A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

LATERAL BRACES TO BE A MINIMUM OF 2X4 SPF #2

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	38.2	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	51.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , ABC 2019
- PART 9 OF OBC 2012 (2019 AMENDMENT)
- CSA 086-14
- TPIC 2014

(55 % OF 54.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 38.2 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.16/1.00 (A-B:1) , BC=0.29/1.00 (C-D:1) , WB=0.00/1.00 (A-C:1) , SSI=0.32/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (D) (INPUT = 0.90)
JSI METAL= 0.05 (D) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. BAYVIEW WELLINGTON	DRWG NO.
402637	J400	3	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
D	BMV1+p	MT20	4.0	6.0		

LICENSED PROFESSIONAL ENGINEER
4/03/23
C. M. HEYENS
100505065
PROVINCE OF ONTARIO

STRUCTURAL COMPONENT ONLY
DWG # TR23040216

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

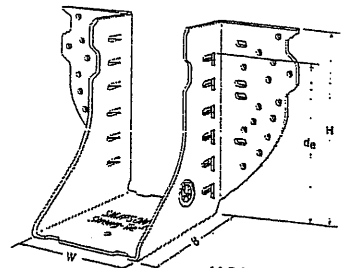
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

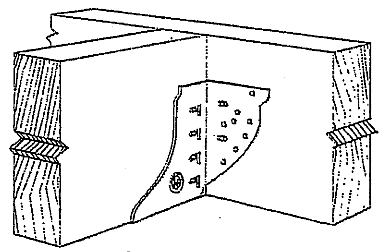
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

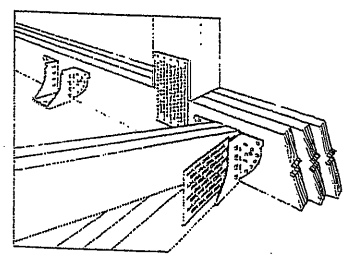
Options:

- See current catalogue for options



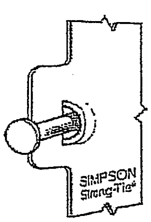
HGUS28-2


Typical HGUS Installation


Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

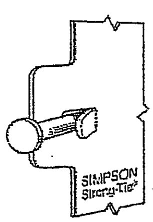
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1 5/8	5 3/8	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 5/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210	12	1 5/8	9 1/8	5	7 7/8	(46) 16d	(16) 16d	3535	11070	2510	8090
HGUS210-2	12	3 5/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS210-3	12	4 15/16	9 1/4	4	8 3/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 3/16	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10 5/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12 5/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

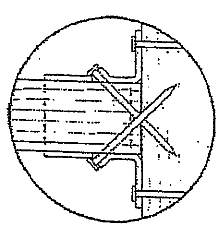


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECHGUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

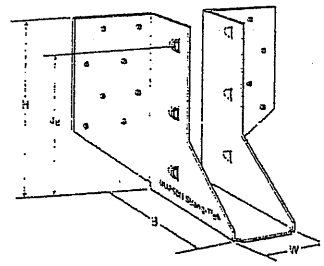
- Factored resistances are in accordance with CSA 086-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

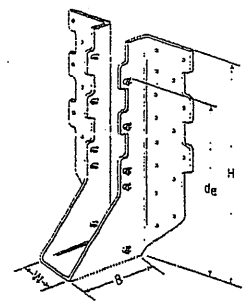
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

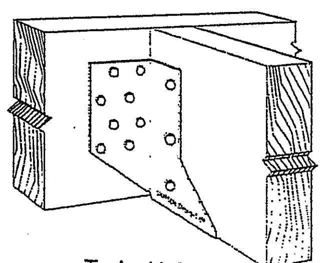
- See current catalogue for options



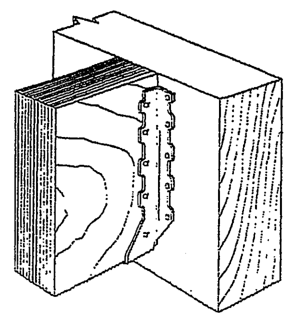
LJS26DS



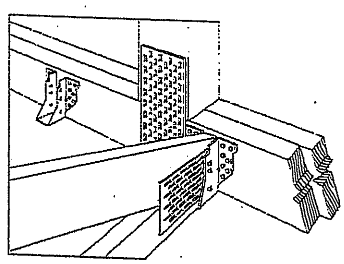
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



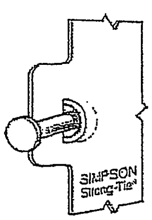
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

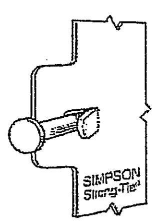
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1³⁄₁₆	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7³¹⁄₃₂	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

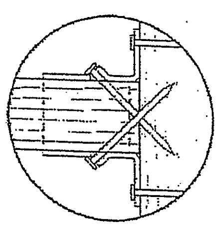


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
04/01/2020
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECHUS20 3/20 exp. 6/22

(800) 999-5099
strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

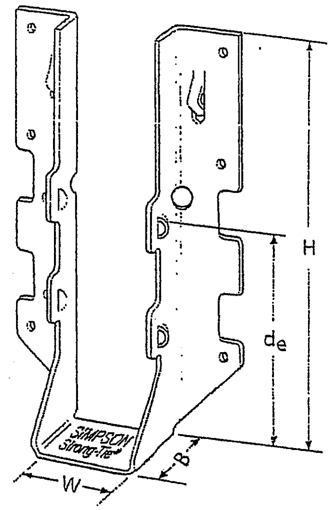
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

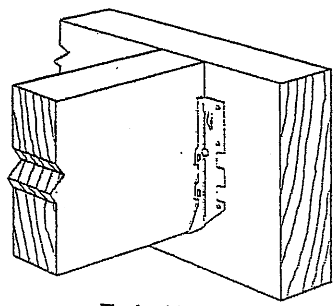
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

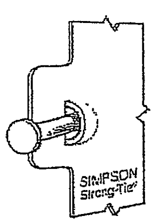


LUS28


Typical LUS Installation

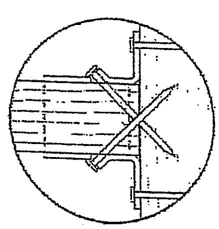
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3⅛	1¾	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4 3/16	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6⅜	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 3/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.

LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2022, and reflects information available as of April 1, 2020. This information is updated periodically and should not be relied upon after June 30, 2022. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B97791H2

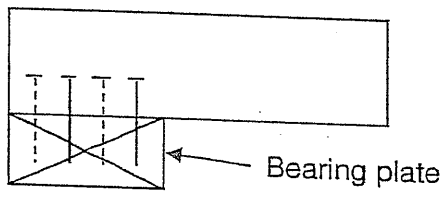
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	30	42
	3.25	0.144	32	45
	3.50	0.160	38	52
COMMON SPIRAL	3.00	0.122	26	36
	3.25	0.122	28	40
	3.50	0.152	36	50

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

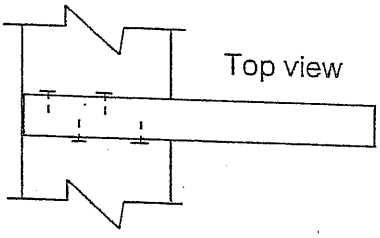
NOTES:

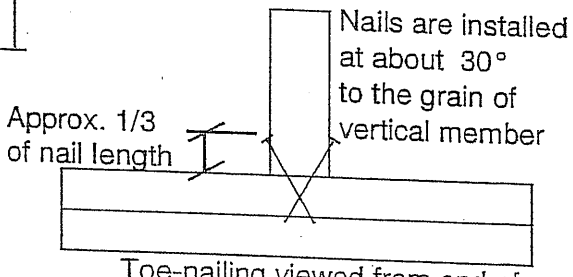
1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind** or **earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-14, section 12.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: G = 0.42(SPF), G = 0.49(D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-14, section 12.9.5
9. This design is not valid after March 31, 2021.

Toe-nailing on 2x6 Bearing Plate



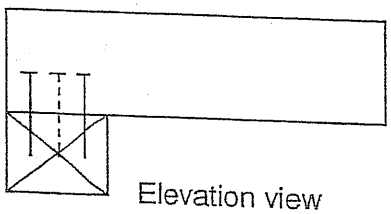
Elevation view



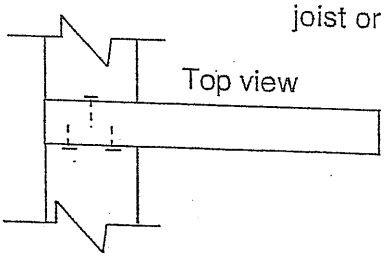


Toe-nailing viewed from end of joist or truss

Toe-nailing on 2x4 Bearing Plate



Elevation view



PEO
Certificate No. 10889485

MiTek®

MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

December 2, 2019

MiTek®

STANDARD DETAIL MSD2015-H

Issued: SEPTEMBER 22, 2020
Expiry: APRIL 30, 2022

TOE-NAIL CAPACITY DETAILS

LATERAL AND WITHDRAWAL RESISTANCE OF BEARING ANCHORAGE BY TOE-NAILS

			SPF	D. FIR	SPF	D. FIR
COMMON WIRE	3.00	0.144	122	139	30	42
	3.25	0.144	127	144	32	45
	3.50	0.160	152	173	38	52
COMMON SPIRAL	3.00	0.122	96	108	26	36
	3.25	0.122	97	108	28	40
	3.50	0.152	142	161	36	50
3.25" Gun nail	3.25	0.120	94	105	28	39

Note: If using truss with D. Fir lumber and SPF bearing plate, use tabulated SPF values in table.

Nail type:	Common wire	Common spiral	Common wire	Common spiral	Gun Nail
Diameter (in.)	0.160	0.152	0.144	0.122	0.120
Length (in.)	3.50	3.50	3.00	3.00	3.25
2x4 SPF	2	2	3	3	3
2x6 SPF	4	4	4	5	5
2x4 D. FIR	2	2	2	2	2
2x6 D. FIR	3	3	3	4	4

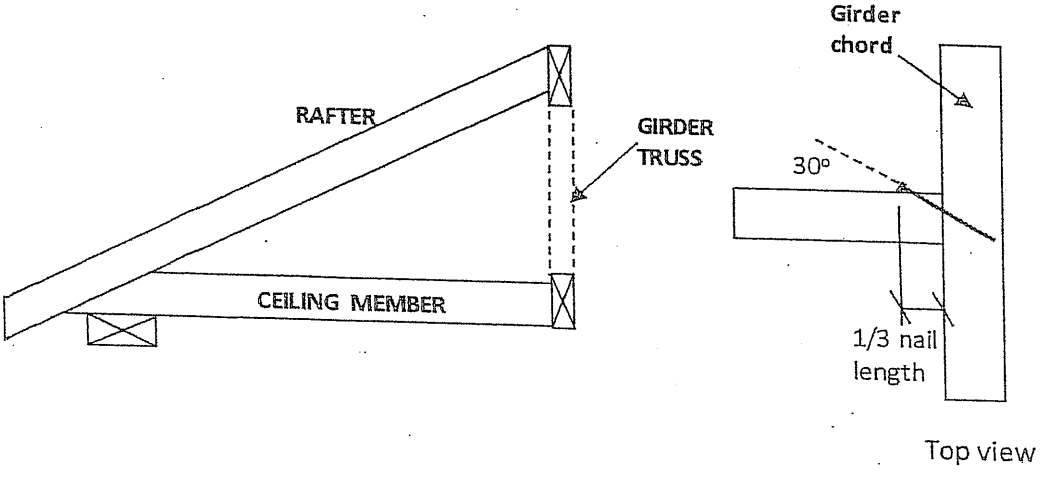
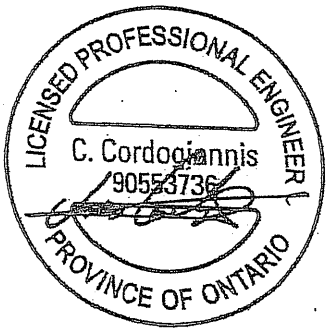


Figure 1: Toe-Nailing Rafter / Ceiling Member to Girder Truss

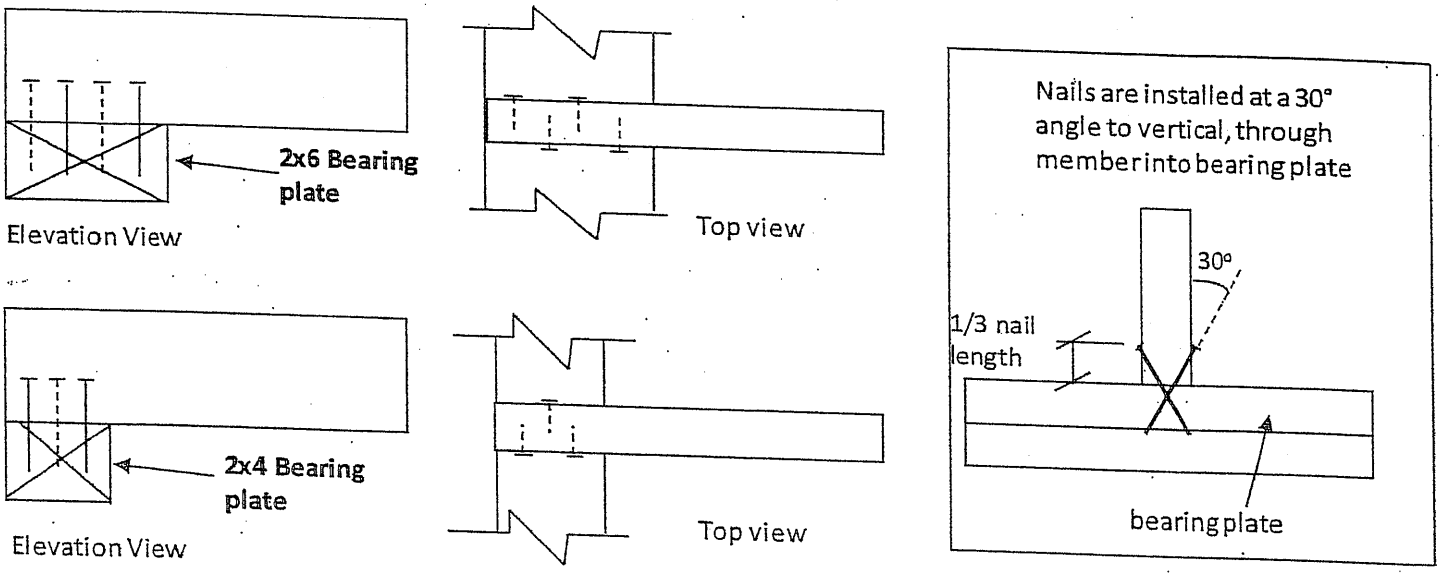
PEO
Certificate No. 10889485



December 21, 2020

TOE-NAIL CAPACITY DETAILS

Figure 2: Toe-Nail Anchorage to Bearing Plate for Uplift



NOTES:

1. Rafter and ceiling members may be connected to top and bottom chords of girder truss by toe-nailing the members into the girder chords (see fig. 1), provided the factored vertical reactions of the supported members do not exceed the lateral resistance of the toe-nails. Mechanical connectors (hangers) are required if factored vertical reactions exceed the toe-nail capacity, or if the connection must resist horizontal loads (loads perpendicular to the face of girder or rafter).
2. Trusses, rafters or ceiling members may be anchored to the bearing plate with toe-nails (see fig. 2), provided that the factored uplift reactions due to **wind or earthquake loads** do not exceed the **withdrawal resistance of the toe-nails.** Mechanical anchors (tie-downs) are required for reactions that exceed the toe-nail withdrawal capacity. Toe-nail anchorage to bearing plates is **NOT** permitted if uplift reactions are generated from gravity loads (snow, floor live, dead).
3. Tabulated toe-nail resistances on page 1 are for **one** toe-nail. Multiply unit values by the number of nails used in the connection. Maximum number of nails in a connection shall not exceed the tabulated limits shown on page 1 for a given lumber size /species.
4. Nail values are based on specific gravity of G = 0.42 (SPF) and G = 0.49 (D. Fir).
5. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member.
6. For wind / earthquake loads, tabulated lateral resistances may be multiplied by 1.15 (K_D factor). No increases are permitted for tabulated withdrawal resistances.
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-19, Clause 12.9.

PEO
Certificate No. 10889485

LICENSED PROFESSIONAL ENGINEER
C. Cordogiannis
90553736
PROVINCE OF ONTARIO

December 21, 2020

EWP DESIGN INC.

(905) 832-2250 FAX (905) 832-0286

RESPONSIBILITIES AND SPECIFICATIONS

RESPONSIBILITIES

1. EWP DESIGN INC. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on each drawing meet or exceed the actual dead load imposed by the structure, the live load imposed by the intended use and the snow load imposed by local building code or authorities with jurisdictions.
3. All dimensions are to be verified by the owner, contractor, architect or other authorities with jurisdictions before truss fabrication.
4. EWP DESIGN INC. bears no responsibility for the erection of trusses. Persons erecting trusses are cautioned to seek professional advice regarding the temporary and permanent bracing for the system. Bracing shown on EWP DESIGN INC. drawing is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacturer's responsibility to ensure that trusses are manufactured in conformance with specifications of EWP DESIGN INC. as outlined below.

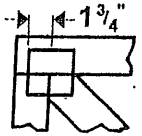
SPECIFICATIONS

1. Trusses designed by EWP DESIGN INC. conform to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type, as indicated on the EWP DESIGN INC. drawings, and conform to the design procedures established by the Truss Plate Institute of Canada. Unit stresses used for truss designs are as per the edition of CSA-O86 shown on EWP DESIGN INC. drawings.
2. Lumber is to be the size, species and grade as specified on EWP DESIGN INC. drawings.
3. Moisture content of lumber shall not exceed 19% in service unless specified otherwise.
4. Metal connector plates shall be applied to both faces of truss at each joint and shall be positioned as specified.
5. Top chords of trusses are assumed to be continuously braced laterally by roof sheathing or by purlins at intervals not exceeding 12.5 times the thickness of top chord member.
6. Bottom chords shall be laterally braced at intervals not exceeding 3M (10') o.c., where rigid ceiling is not applied directly to the underside of chords.

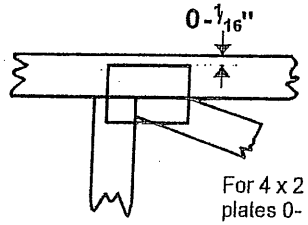
February 1, 2019

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

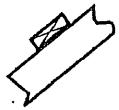
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

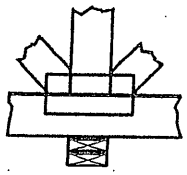
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

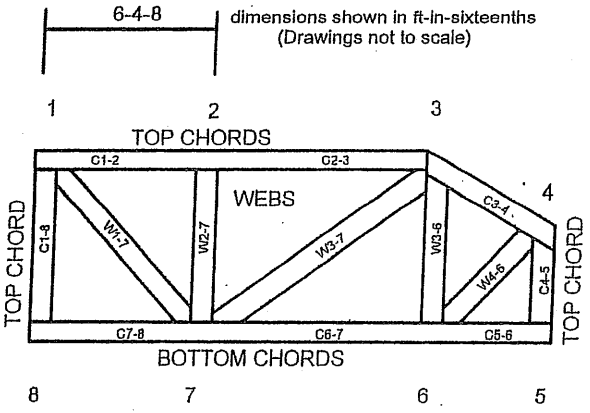


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek®

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.