



Job Track: 40297
Layout ID: 252538
Plan Log: 79711

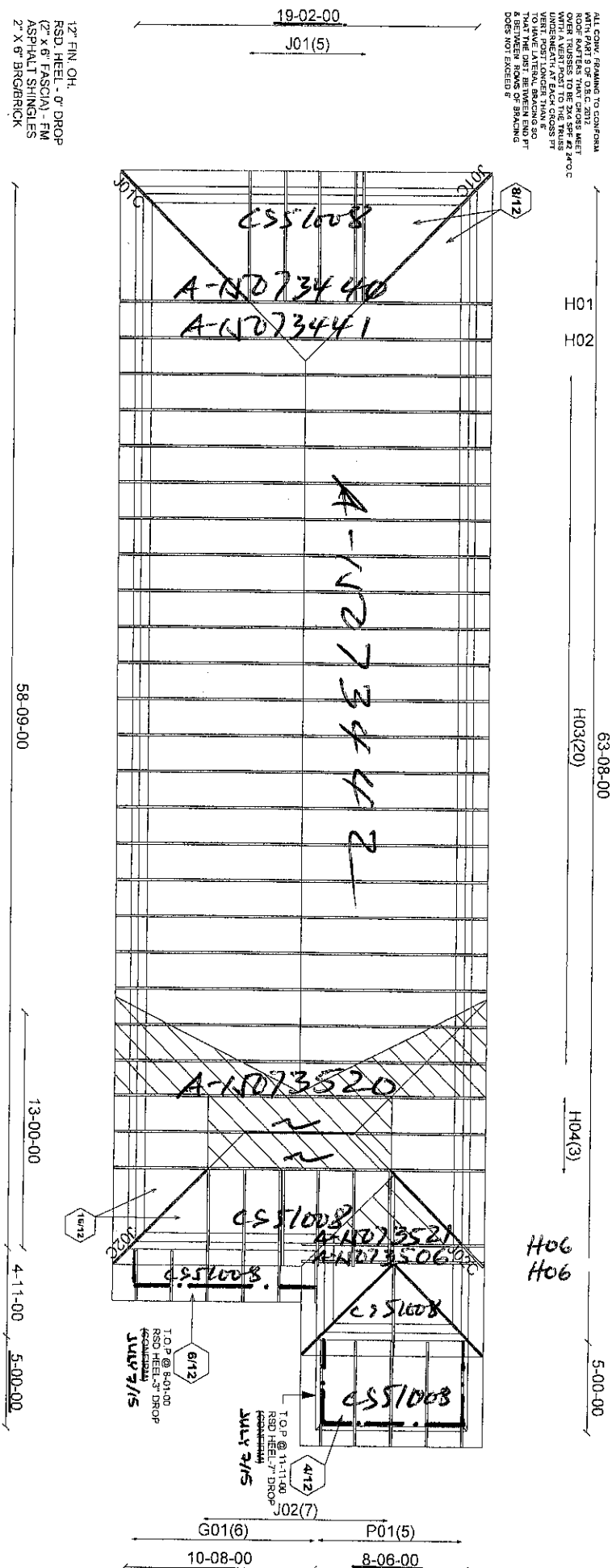
Builder / Location:	GOLD PARK / KLEINBURG		
Project:	HUNTINGTON & NASHVILLE		
Date:	4/29/2015	Designer:	cheryl/a

Model / Elevation:
25-6 / A

CONVENTIONAL
FRAMING BY
OTHERS

Mitek Ver. 7.5.0

ALL CHGV. FRAMING TO CONFORM WITH PART 9 OF D.B.C. 2012. ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SFP #2 24" O.C WITH A VERT POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT VERT POST LONGER THAN 8' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT & BETWEEN ROWS OF BRACING DOES NOT EXCEED 8'



Mitek Ver. 7.5.0

ALL GUY BEAMS TO CONFORM WITH AISC 360-10. ALL ROOF TRUSSES TO BE 2X4 SPF #2 F.O.C. OVER TRUSSES TO BE 2X4 SPF #2 F.O.C. UNDERNEATH AT EACH CROSS PT. VERT. POSTS LONGER THAN 6' TO BE 2X4 SPF #2 F.O.C. THAT THE DIST. BETWEEN END PT. & BETWEEN ROOFS OF BRACING DOES NOT EXCEED 8'

ALPHA-BARRIE

63-08-00

5-00-00

H01
H02

H03(21)

H10(3)

LD
HI

H06(3)

2 - 2x10 SP#2
BM BY BUILDER

VAULT CLG
@ 12/12

8-06-00

10-08-00

19-02-00

J01(5)

12" FIN. OH.
RSD. HEEL - 0" DROP
(2" x 6" FASCOA) - FM
ASPHALT SHINGLES
2" X 6" BRICK

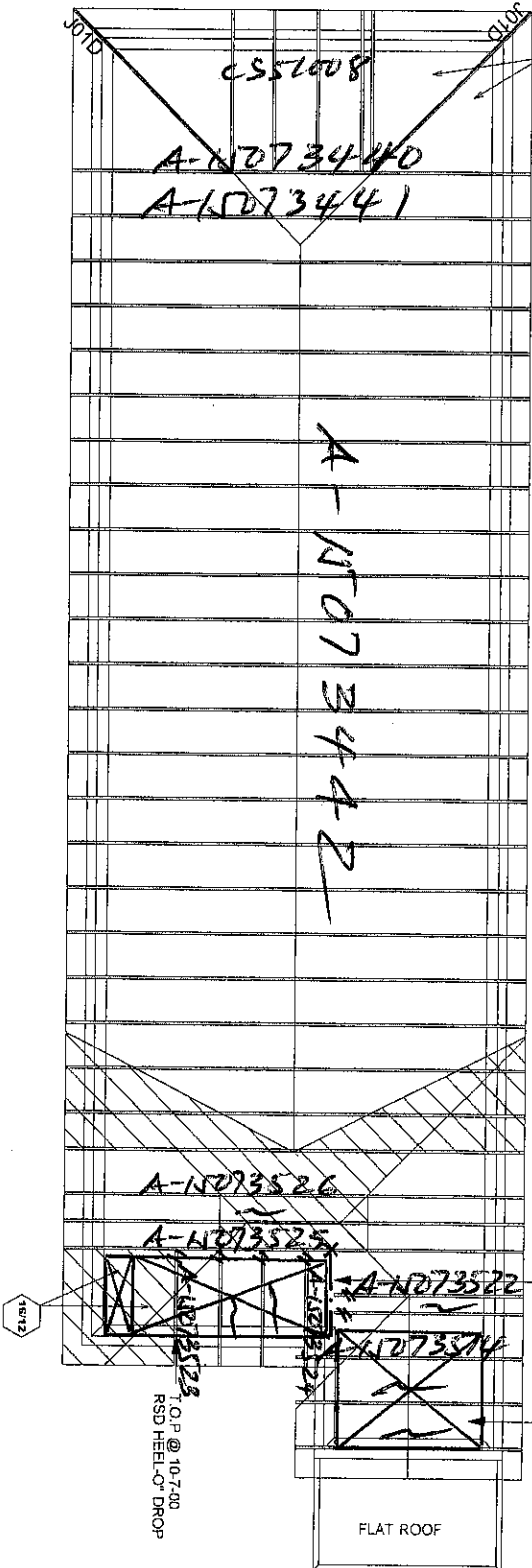
58-09-00

13-00-00

4-11-00

5-00-00

HANGERS
// LUS24



PL 80980



Job Track: 40297

Layout ID: 252541

Plan Log: 79711

Builder / Location:

GOLD PARK / KLEINBURG

Project:

HUNTINGTON & NASHVILLE

Date:

4/20/2015

Designer:

Model / Elevation:

25-6

/ B

CONVENTIONAL
FRAMING BY
OTHERS

Mike Ver. 7.5.0

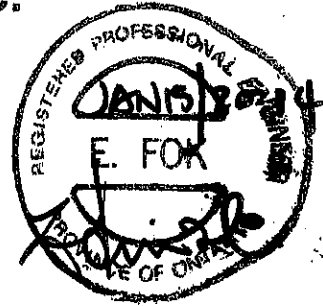
THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPHA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPHA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPHA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALFA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALFA ROOF TRUSSES INC. AND WILL BE RETRACTED BY ALFA ROOF TRUSSES INC. IF UTILIZED FOR ANY OTHER PURPOSE.

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



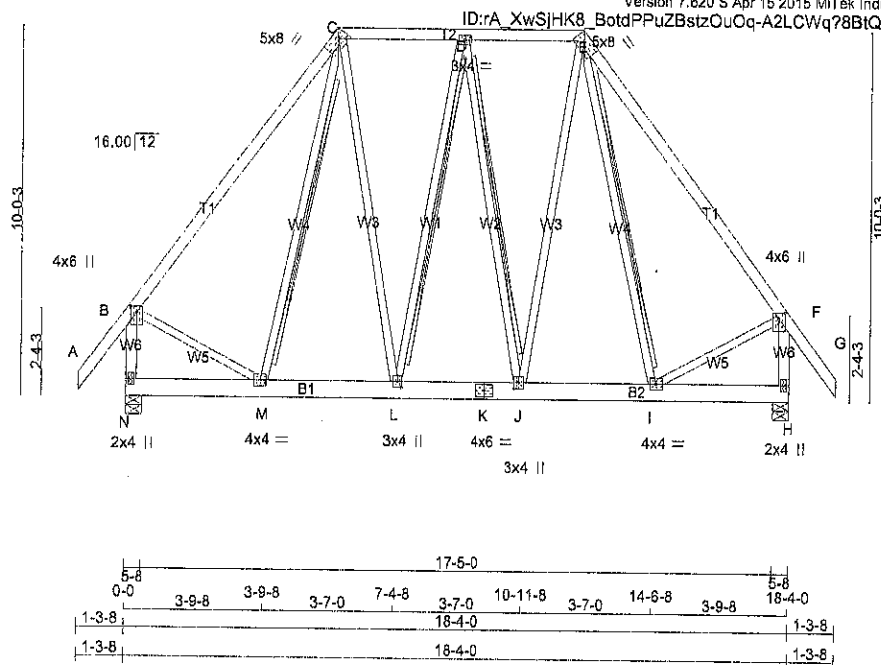
RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	2100F 1.8E	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	2100F 1.8E	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTWW-h	MT20	5.0	8.0	Edge	2.75
D	TMWW-t	MT20	3.0	4.0		
E	TTWW-h	MT20	5.0	8.0	Edge	2.75
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	3.0	4.0	2.00	1.25
K	BS-t	MT20	4.0	6.0		
L	BMWW-t	MT20	3.0	4.0	2.00	1.25
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	905	0	905	0	5-8	1-8
H	905	0	905	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERMLIVE	WIND			
N	836	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	836	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-M, D-L, D-J, E-I
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5 -70.5	0.07 (1)	10.00	M-C	-83 / 27	0.06 (1)
B-C	-524 / 0	-70.5 -70.5	0.31 (1)	6.25	C-L	0 / 202	0.05 (1)
C-D	-364 / 0	-70.5 -70.5	0.14 (1)	8.25	L-D	-144 / 0	0.11 (1)
D-E	-365 / 0	-70.5 -70.5	0.14 (1)	8.25	D-J	-146 / 0	0.11 (1)
E-F	-524 / 0	-70.5 -70.5	0.31 (1)	6.25	J-E	0 / 205	0.05 (1)
F-G	0 / 40	-70.5 -70.5	0.07 (1)	10.00	E-I	-83 / 27	0.06 (1)
N-B	-879 / 0	0.0 0.0	0.12 (1)	7.81	B-M	0 / 350	0.08 (1)
H-F	-879 / 0	0.0 0.0	0.12 (1)	7.81	I-F	0 / 350	0.09 (1)
N-M	0 / 0	-17.5 -17.5	0.03 (4)	10.00			
M-L	0 / 330	-17.5 -17.5	0.06 (1)	10.00			
L-K	0 / 391	-17.5 -17.5	0.07 (1)	10.00			
K-J	0 / 391	-17.5 -17.5	0.07 (1)	10.00			
J-I	0 / 330	-17.5 -17.5	0.06 (1)	10.00			
I-H	0 / 0	-17.5 -17.5	0.03 (4)	10.00			

TOTAL WEIGHT = 3 X 124 = 373 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.31 (E-F:1), BC=0.07 (J-L:1), WB=0.11 (D-J:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

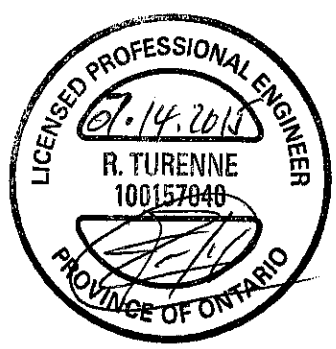
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1656	

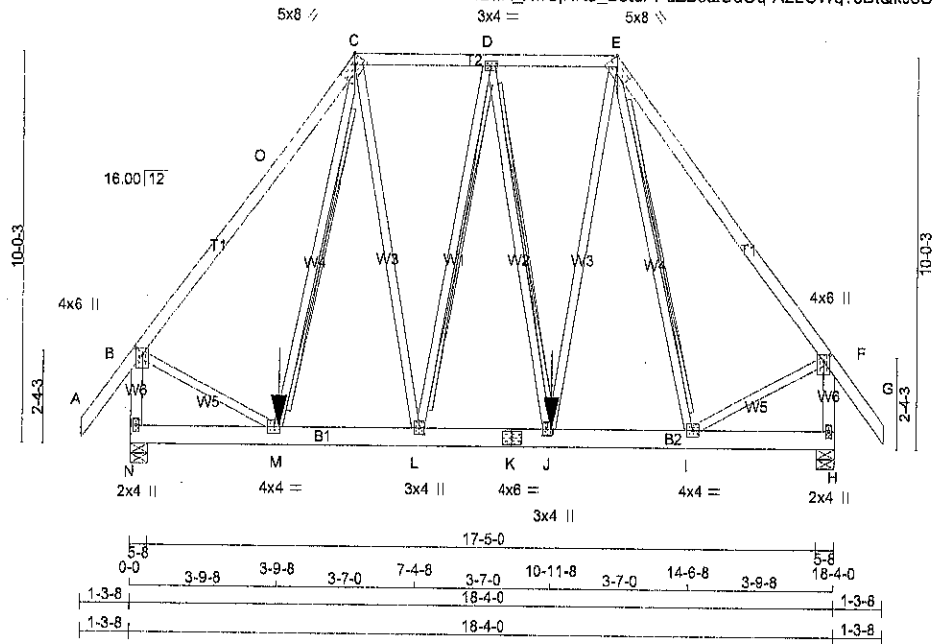
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (D) (INPUT = 0.90)
JSI METAL= 0.16 (H) (INPUT = 1.00)



A-1073526



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	2100F 1.8E	SPF
C - E	2x4 DRY	No.2	SPF
E - G	2x4 DRY	2100F 1.8E	SPF
N - B	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
N - K	2x6 DRY	No.2	SPF
K - H	2x6 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTVW-h	MT20	5.0	8.0	Edge 2.75
D	TMVW-h	MT20	3.0	4.0	
E	TTVW-h	MT20	5.0	8.0	Edge 2.75
F	TMVW+p	MT20	4.0	6.0	2.00 2.00
H	BMV1+p	MT20	2.0	4.0	
I	BMVW-h	MT20	4.0	4.0	
J	BMVW-h	MT20	3.0	4.0	2.00 1.25
K	BS-t	MT20	4.0	6.0	
L	BMVW-h	MT20	3.0	4.0	2.00 1.25
M	BMVW-h	MT20	4.0	4.0	
N	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 389.2 lbs FACTORED DOWN AT 10-11-8, AND 227.1 lbs FACTORED DOWN AT 3-9-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REORD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
N	1545	0	0	1545	0	0	0	5-8	1-11
H	1293	0	0	1293	0	0	0	5-8	1-8

UNFACTORED REACTIONS		1ST CASE		MAX./MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND
N	1099	682 / 0	0 / 0	0 / 0	0 / 0
H	929	530 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

2x4 DRY SPF No.2 T-BRACE REQUIRED AT C-M, D-L, D-J, E-I
FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.07 (1)	10.00	M-C	0 / 76	0.02 (4)
B-O	-1032 / 0	-103.5	-103.5 0.47 (1)	6.25	C-L	0 / 395	0.10 (4)
O-C	-1032 / 0	-70.5	-70.5 0.47 (1)	6.25	L-D	-147 / 0	0.12 (1)
C-D	-658 / 0	-70.5	-70.5 0.14 (1)	6.25	D-J	-129 / 98	0.10 (1)
D-E	-662 / 0	-70.5	-70.5 0.14 (1)	6.25	J-E	0 / 584	0.14 (1)
E-F	-880 / 0	-70.5	-70.5 0.34 (1)	6.25	E-I	-180 / 0	0.14 (1)
F-G	0 / 40	-70.5	-70.5 0.07 (1)	10.00	B-M	0 / 679	0.17 (1)
N-B	-1504 / 0	0.0	0.0 0.20 (1)	6.65	I-F	0 / 588	0.15 (1)
H-F	-1271 / 0	0.0	0.0 0.17 (1)	7.11			
N-M	0 / 0	-25.7	-25.7 0.04 (4)	10.00			
M-L	0 / 591	-58.7	-58.7 0.13 (1)	10.00			
L-K	0 / 685	-58.7	-58.7 0.14 (1)	10.00			
K-J	0 / 685	-58.7	-58.7 0.14 (1)	10.00			
J-I	0 / 563	-17.5	-17.5 0.10 (1)	10.00			
I-H	0 / 0	-17.5	-17.5 0.04 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)				TYPE	
JT	LOC.	LC1	MAX-	FACE	DIR.
J	10-11-8	-348	-348	FRONT	VERT
M	3-9-12	-227	-227	FRONT	VERT

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH.	LL = 21.0 PSF
	DL = 3.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 31.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-9-12
START SPAN CARRIED = 3-10-8
END DISTANCE = 10-11-8
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 3-9-12 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/999 (0.03")

CSI: TC=0.47 (B-C:1), BC=0.14 (J-L:1), WB=0.17 (B-M:1), SSI=0.15 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

A-1073525



NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

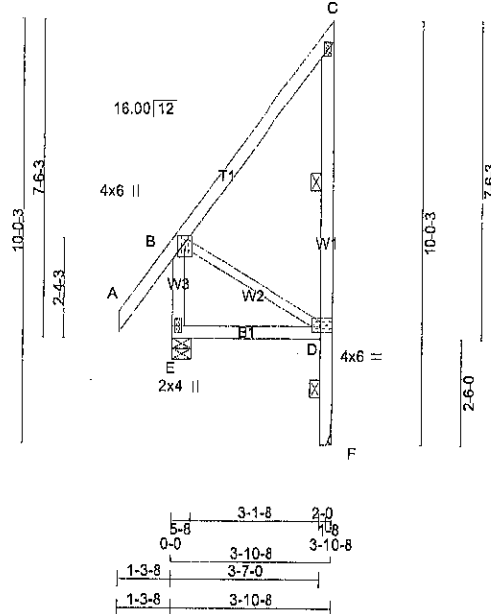
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.28 (N) (INPUT = 1.00)



A-1073525(2)



LUMBER				N. L. G. A. RULES		
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	DESCR.
E - B	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TMV+p	MT20	2.0	4.0	
D	BMVW-l	MT20	4.0	6.0	1.50 2.50
E	BMV1+p	MT20	2.0	4.0	

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	270	0	270	0	5-8	1-6
F	170	0	170	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-6

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND			
E	168	140 / 0	0 / 0	0 / 0	0 / 0		47 / 0	0 / 0
F	120	81 / 0	0 / 0	0 / 0	0 / 0		39 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED FOR D-F, C-D. BRACE LOCATED AT EVERY 1/2 LENGTH OF D-F, C-D.
 END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)
FR-TO		FROM TO		FROM TO		FROM TO
E-B	-238 / 0	0.0	0.0	0.03 (1)	7.81	0.00 (1)
A-B	0 / 40	-70.5	-70.5	0.10 (1)	10.00	
B-C	0 / 0	-70.5	-70.5	0.18 (1)	10.00	
F-D	-170 / 0	0.0	0.0	0.09 (1)	6.25	
D-C	-136 / 0	0.0	0.0	0.07 (1)	6.25	
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 3 X 31 = 92 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.18 (B-C:1), BC=0.08 (D-E:4),
 WB=0.00 (B-D:1), SS=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

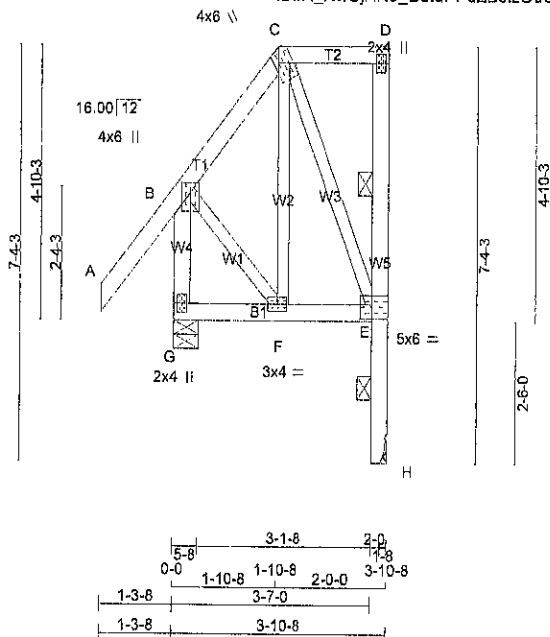
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.14 (B) (INPUT = 0.90)
 JSI METAL=0.04 (E) (INPUT = 1.00)



A-15073524



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	6.0	2.00 2.00
C	TTWW+m	MT20	4.0	6.0	1.75 1.00
D	TMV+p	MT20	2.0	4.0	
E	BMVW-l	MT20	5.0	6.0	3.00 2.50
F	BMVW-l	MT20	3.0	4.0	
G	BMV1+p	MT20	2.0	4.0	

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	UP	BRG	BRG	BRG
H	227	0	0	227	0	0	5-8	1-8	1-8
G	362	0	0	362	0	0	5-8	1-8	1-8

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	180	106 / -11	0 / 0	0 / 0	0 / 0	54 / 0	0 / 0
G	252	187 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

1 LATERAL BRACE(S) REQUIRED FOR E-H, D-E. BRACE LOCATED AT EVERY 1/2 LENGTH OF E-H, D-E.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING				TOTAL LOAD CASES: (5)			
CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	F-C	0 / 80	0.02 (4)
B-C	-112 / 0	-70.5	-70.5 0.10 (1)	6.25	C-E	-124 / 0	0.05 (1)
C-D	0 / 0	-70.5	-70.5 0.05 (1)	10.00	B-F	0 / 68	0.02 (1)
H-E	-227 / 0	0.0	0.0 0.06 (1)	6.25			
E-D	-70 / 0	0.0	0.0 0.02 (1)	6.25			
G-B	-316 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	0 / 0	-58.0	-58.0 0.03 (1)	10.00			
F-E	0 / 50	-58.0	-58.0 0.04 (1)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 32 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-9-0
END DISTANCE = 3-10-8
END SPAN CARRIED = 3-9-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 56 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 23.0 P.S.F. G.S.I.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.18")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.18")
CALCULATED VERT. DEFL.(TL)= L/999 (0.00")

CSI: TC=0.11 (A-B:1), BC=0.04 (E-F:1), WB=0.05 (C-E:1), SSI=0.06 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

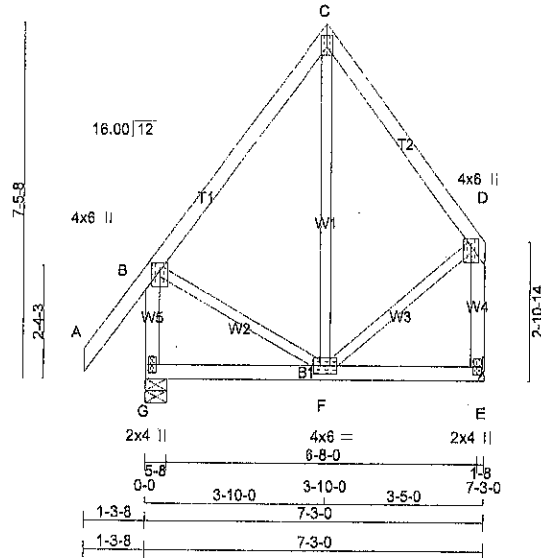
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-N073523





LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMVWW-I	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES - (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
G	418	0	418	0	5-8	1-8
E	319	0	319	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS				DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND				
G	292	211 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0	0 / 0
E	225	152 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 40	-70.5 -70.5	0.10 (1)	10.00	F-C	-28 / 50	0.03 (1)
B-C	-142 / 0	-70.5 -70.5	0.13 (1)	6.25	B-F	0 / 96	0.02 (1)
C-D	-142 / 0	-70.5 -70.5	0.11 (1)	6.25	F-D	0 / 106	0.02 (1)
G-B	-391 / 0	0.0 0.0	0.05 (1)	7.81			
E-D	-297 / 0	0.0 0.0	0.05 (1)	7.81			
G-F	0 / 0	-17.5 -17.5	0.06 (4)	10.00			
F-E	0 / 0	-17.5 -17.5	0.06 (4)	10.00			

TOTAL WEIGHT = 2 X 42 = 85 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.24")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.24")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.13 (B-C:1), BC=0.06 (F-G:4), WB=0.03 (C-F:1), SSI=0.06 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

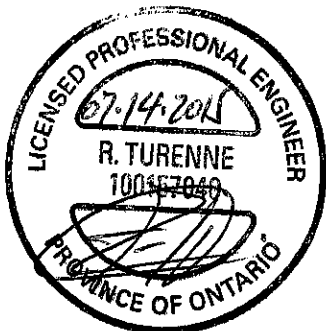
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2264 1656

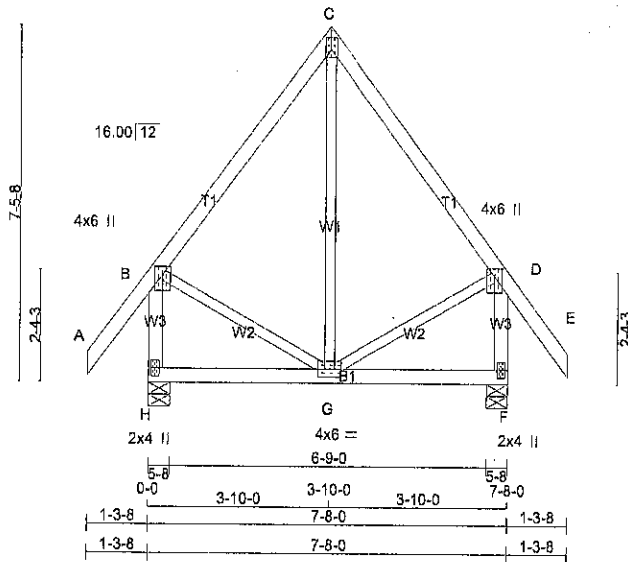
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
 JSI METAL= 0.07 (G) (INPUT = 1.00)



A-15073522



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
 1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.2.3.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
H	533	0	533	0	5-8	1-8
F	591	0	591	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	373	288 / 0	0 / 0	0 / 0	0 / 0	107 / 0	0 / 0
F	414	294 / 0	0 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 40	-70.5	-70.5 0.11 (1)	10.00	G-C	0 / 108	0.04 (4)		
B-C	-236 / 0	-70.5	-70.5 0.20 (1)	6.25	B-G	0 / 159	0.04 (1)		
C-D	-236 / 0	-70.5	-70.5 0.20 (1)	6.25	G-D	0 / 159	0.04 (1)		
D-E	0 / 40	-70.5	-70.5 0.11 (1)	10.00					
H-B	-496 / 0	0.0	0.0 0.07 (1)	7.81					
F-D	-496 / 0	0.0	0.0 0.07 (1)	7.81					
H-G	0 / 0	-17.5	-59.9 0.13 (1)	10.00					
G-F	0 / 0	-61.9	-61.9 0.13 (1)	10.00					

TOTAL WEIGHT = 45 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 3.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

GIRDER TYPE: CStdGirder
 START DISTANCE = 0-0
 START SPAN CARRIED = 2-0-0
 END DISTANCE = 4-0-4
 END SPAN CARRIED = 4-0-4
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 -ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
 START DISTANCE = 4-0-4
 START SPAN CARRIED = 4-0-4
 END DISTANCE = 7-8-0
 END SPAN CARRIED = 4-0-4
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 -ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BC9C 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
 8.4 P.S.F. RAIN LOAD EQUALS
 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.20 (C-D:1), BC=0.13 (G-H:1),
 WB=0.04 (B-G:1), SSI=0.12 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

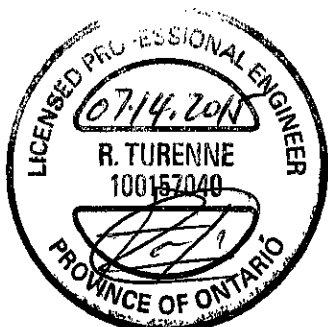
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

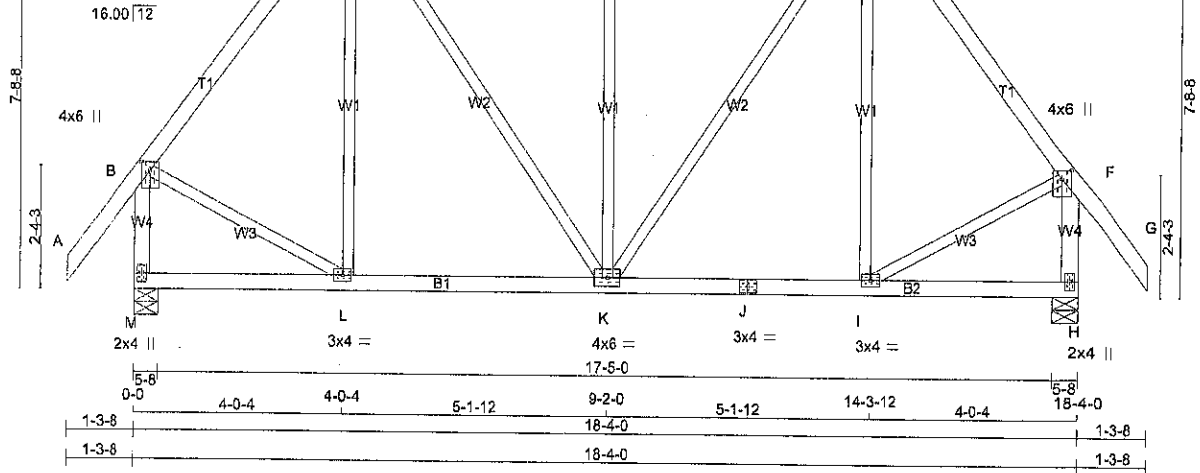
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-15073521





DRY: SEASONED LUMBER.

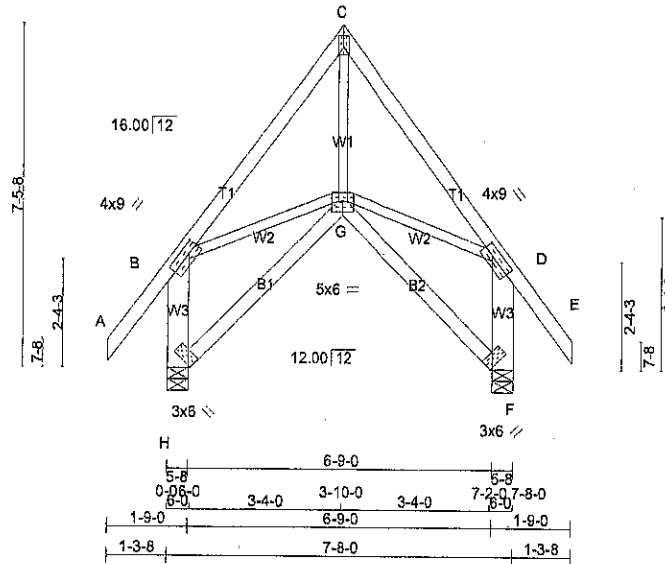
Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 119.0 lbs FACTORED DOWN AT 14-3-12, AND 119.0 lbs FACTORED DOWN AT 4-0-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-4	-119	-119	--	FRONT	VERT	TOTAL
E	14-3-12	-119	-119	--	FRONT	VERT	TOTAL

A-1073520



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW-t	MT20	4.0	9.0	2.00	3.00
F	BVM1-H	MT20	3.0	6.0	Edge	0.25
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BVM1-H	MT20	3.0	6.0	Edge	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	436	0	436	0	0
F	436	0	436	0	0

UNFACTORED REACTIONS

JT	1ST LC CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	305	220 / 0	0 / 0	0 / 0	85 / 0	0 / 0
F	COMBINED	305	220 / 0	0 / 0	0 / 0	85 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F
BEARING SIZE FACTOR = 1.16 AT JNT(S) H (BASED ON SUPPORT DEPTH = 1-8)
BEARING SIZE FACTOR = 1.16 AT JNT(S) F (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MEMB.	FORCE (LBS)	MAX	LC1 (LBS)
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 40	-70.5	-70.5 0.10 (1)	G-C	0 / 199	0.04 (1)	
B-C	-293 / 0	-70.5	-70.5 0.13 (1)	B-G	0 / 188	0.04 (1)	
C-D	-293 / 0	-70.5	-70.5 0.13 (1)	G-D	0 / 188	0.04 (1)	
D-E	0 / 40	-70.5	-70.5 0.10 (1)				
H-B	-403 / 0	0.0	0.0 0.04 (1)				
F-D	-403 / 0	0.0	0.0 0.04 (1)				
H-G	0 / 0	-17.5	-17.5 0.08 (4)				
G-F	0 / 0	-17.5	-17.5 0.08 (4)				

TOTAL WEIGHT = 5 X 49 = 246 lb

[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.13 (C-D:1), BC=0.08 (F-G:4),
WB=0.04 (C-G:1), SSI=0.06 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

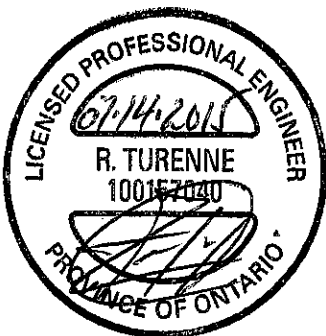
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

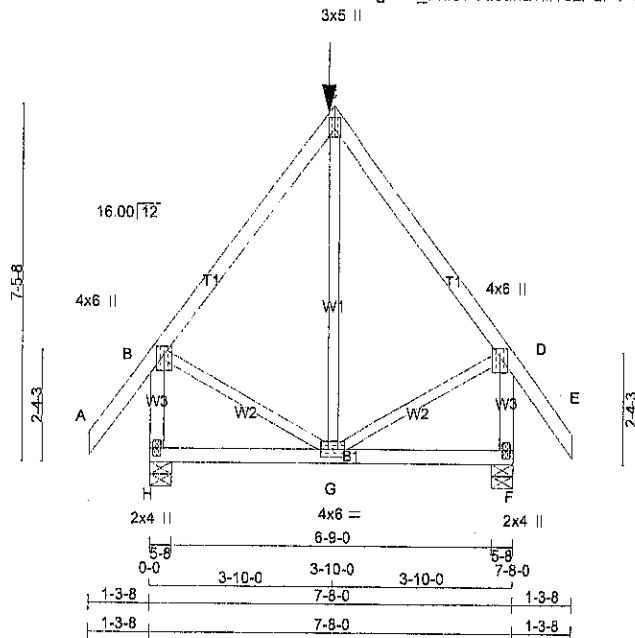
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP=0.30 (C) (INPUT = 0.90)
JSI METAL=0.08 (D) (INPUT = 1.00)



A-11073514



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-I	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 213.8 lbs FACTORED DOWN AT 3-10-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

- Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	575	0	575	0
F	575	0	575	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
H	403	284 / 0	0 / 0	0 / 0
F	403	284 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 40	-70.5	-70.5	0.11 (1)	10.00	G-C	-47 / 83
B-C	-273 / 0	-70.5	-70.5	0.20 (1)	6.25	B-G	0 / 184
C-D	-273 / 0	-70.5	-70.5	0.20 (1)	6.25	G-D	0 / 184
D-E	0 / 40	-70.5	-70.5	0.11 (1)	10.00		
H-B	-537 / 0	0.0	0.0	0.07 (1)	7.81		
F-D	-537 / 0	0.0	0.0	0.07 (1)	7.81		
H-G	0 / 0	-25.7	-25.7	0.12 (4)	10.00		
G-F	0 / 0	-25.7	-25.7	0.12 (4)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-10-0	-214	-214	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 45 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 3-10-0

END SETBACK = 3-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 56 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 066-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.26")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20 (C-D:1), BC=0.12 (G-H:4),
 WB=0.05 (B-G:1), SSI=0.09 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.80

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1967 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.90)
 JSI METAL= 0.10 (F) (INPUT = 1.00)

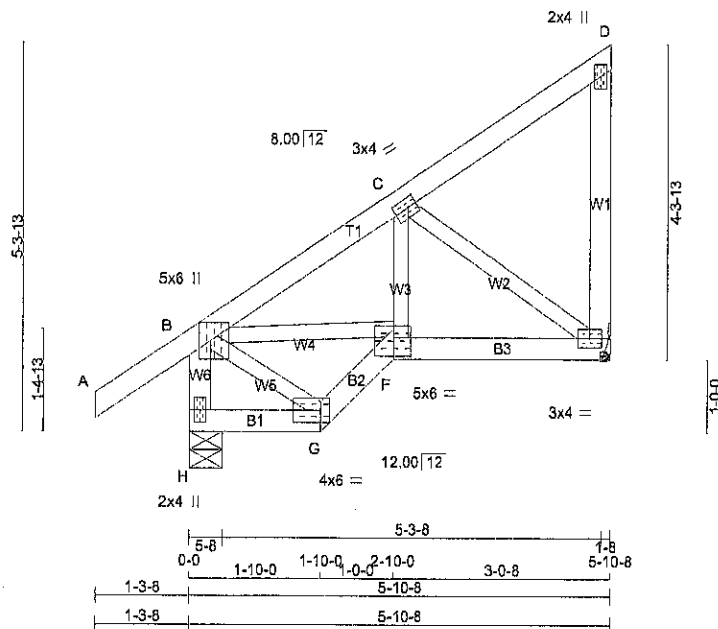


A-11073506

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01C	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.820 S Apr 15 2015 MITek Industries, Inc. Mon Jul 13 13:31:55 2015 Page 1
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Scale: 3/8"=1'



TOTAL WEIGHT = 30 lb
(M)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
H - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW+p	MT20	5.0	6.0	Edge	1.50
C	TMVWW-l	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-l	MT20	5.0	6.0	3.00	3.00
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	355	0	355	0
E	258	0	258	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	248	181 / 0	0 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	182	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX	WEBS
FR-TO		FROM	TO	LENGTH	FR-TO							FROM	TO	LENGTH	FR-TO				
H-B	-339 / 0	0.0	0.0	0.03 (1)	7.81	B-G	0 / 20	0.01 (4)											
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	B-F	0 / 209	0.05 (1)											
B-C	-257 / 0	-70.5	-70.5	0.10 (1)	6.25	F-C	0 / 57	0.02 (4)											
C-D	-13 / 0	-70.5	-70.5	0.10 (1)	6.25	C-E	-271 / 0	0.06 (1)											
E-D	-79 / 0	0.0	0.0	0.02 (1)	7.81														
H-G	0 / 0	-17.5	-17.5	0.02 (4)	10.00														
G-F	0 / 25	-17.5	-17.5	0.01 (4)	10.00														
F-E	0 / 223	-17.5	-17.5	0.06 (1)	10.00														

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.06 (E-F:1), WB=0.06 (C-E:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
B22	2284	1656	

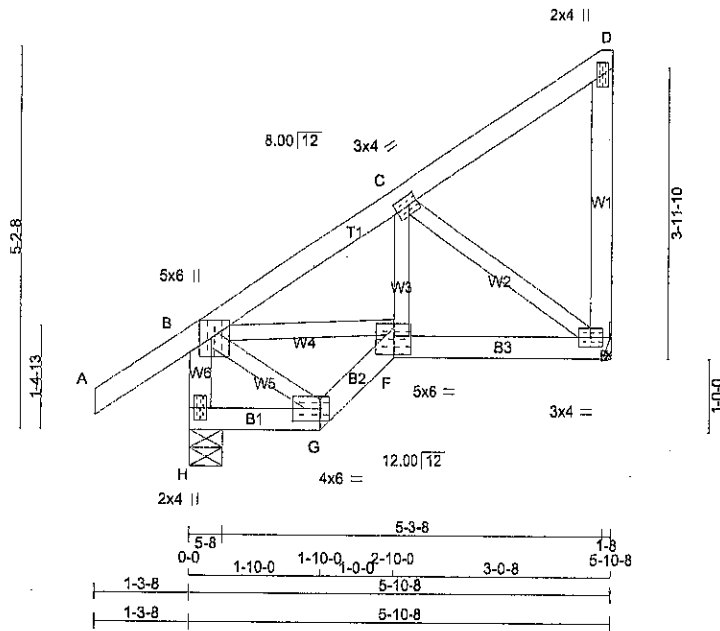
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)



A-15073480



TOTAL WEIGHT = 30 lb

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVWW+p	MT20	5.0	6.0	Edge	
C TMVW-1	MT20	3.0	4.0	1.50	1.50
D TMV+p	MT20	2.0	4.0		
E BMVW-1	MT20	3.0	4.0		
F BBVW-1	MT20	5.0	6.0	3.00	3.00
G BBW-1	MT20	4.0	6.0	2.00	4.50
H BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
E	258	0	258	0	0
H	355	0	355	0	0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	182	123 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0
H	248	181 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO		FROM TO	CSI (LC)	FR-TO		FROM TO	CSI (LC)
A-B	0 / 27	-70.5 -70.5	0.09 (1)	B-F	0 / 209	0.05 (1)	
B-C	-257 / 0	-70.5 -70.5	0.10 (1)	F-C	0 / 57	0.02 (4)	
C-D	-13 / 0	-70.5 -70.5	0.10 (1)	C-E	-271 / 0	0.06 (1)	
E-D	-79 / 0	0.0 0.0	0.02 (1)	B-G	0 / 20	0.01 (4)	
H-B	-339 / 0	0.0 0.0	0.03 (1)				
H-G	0 / 0	-17.5 -17.5	0.02 (4)				
G-F	0 / 25	-17.5 -17.5	0.01 (4)				
F-E	0 / 223	-17.5 -17.5	0.06 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.10 (B-C:1), BC=0.06 (E-F:1), WB=0.06 (C-E:1), SSI=0.08 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (B) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

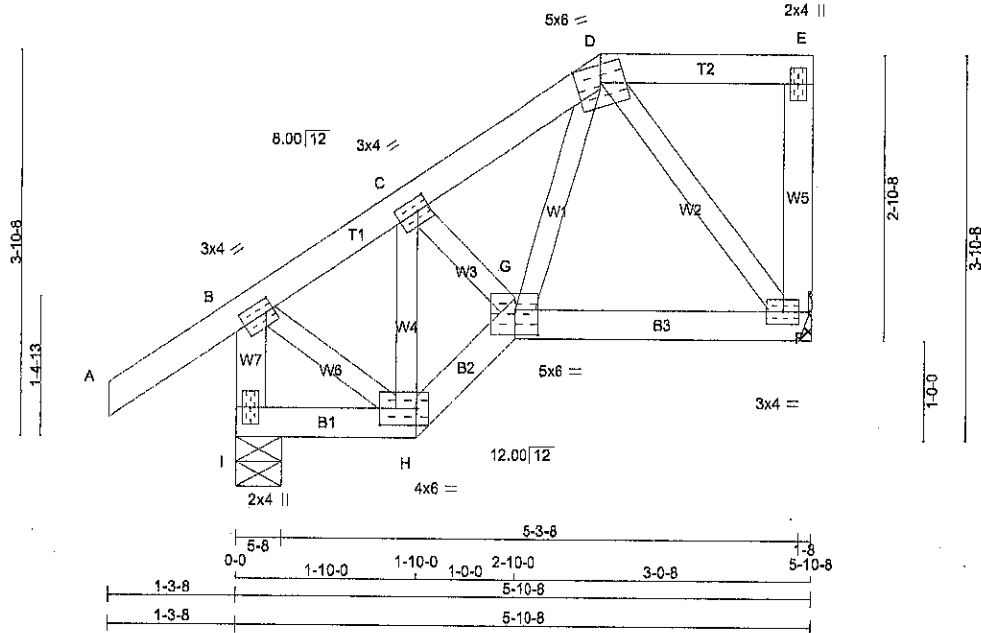


A-15073479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	J01A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MiTek Industries, Inc. Mon Jul 13 13:31:54 2015 Page 1
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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
F - E	2x4	DRY	No.2
I - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
G - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-1	MT20	3.0	4.0	1.50 1.00
C	TMWW-1	MT20	3.0	4.0	1.50 1.50
D	TTWW-m	MT20	5.0	6.0	Edge
E	TMV+p	MT20	2.0	4.0	
F	BMVW-1	MT20	3.0	4.0	
G	BBWW-1	MT20	5.0	6.0	3.00 3.00
H	BBWW-1	MT20	4.0	6.0	2.00 4.50
I	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	MIN. SEAT SIZE: 1-8	1-8
F	247	0	247	0	0	HANGER BY OTHERS			
I	366	0	366	0	0				

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	174	117 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
I	256	188 / 0	0 / 0	0 / 0	0 / 0	68 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 27	-70.5 -70.5	0.09 (1)	H-C	-170 / 0	0.03 (1)	
B-C	-173 / 0	-70.5 -70.5	0.09 (1)	C-G	0 / 81	0.01 (1)	
C-D	-197 / 0	-70.5 -70.5	0.04 (1)	G-D	0 / 119	0.03 (1)	
D-E	0 / 0	-70.5 -70.5	0.06 (1)	D-F	-190 / 0	0.04 (1)	
F-E	-76 / 0	0.0 0.0	0.01 (1)	B-H	0 / 148	0.03 (1)	
I-B	-350 / 0	0.0 0.0	0.04 (1)				
I-H	0 / 0	-17.5 -17.5	0.02 (4)				
H-G	0 / 171	-17.5 -17.5	0.03 (1)				
G-F	0 / 123	-17.5 -17.5	0.06 (4)				

TOTAL WEIGHT = 29 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.09 (A-B:1), BC=0.06 (F-G:4), WB=0.04 (D-F:1), SSI=0.06 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 822 2284 1656

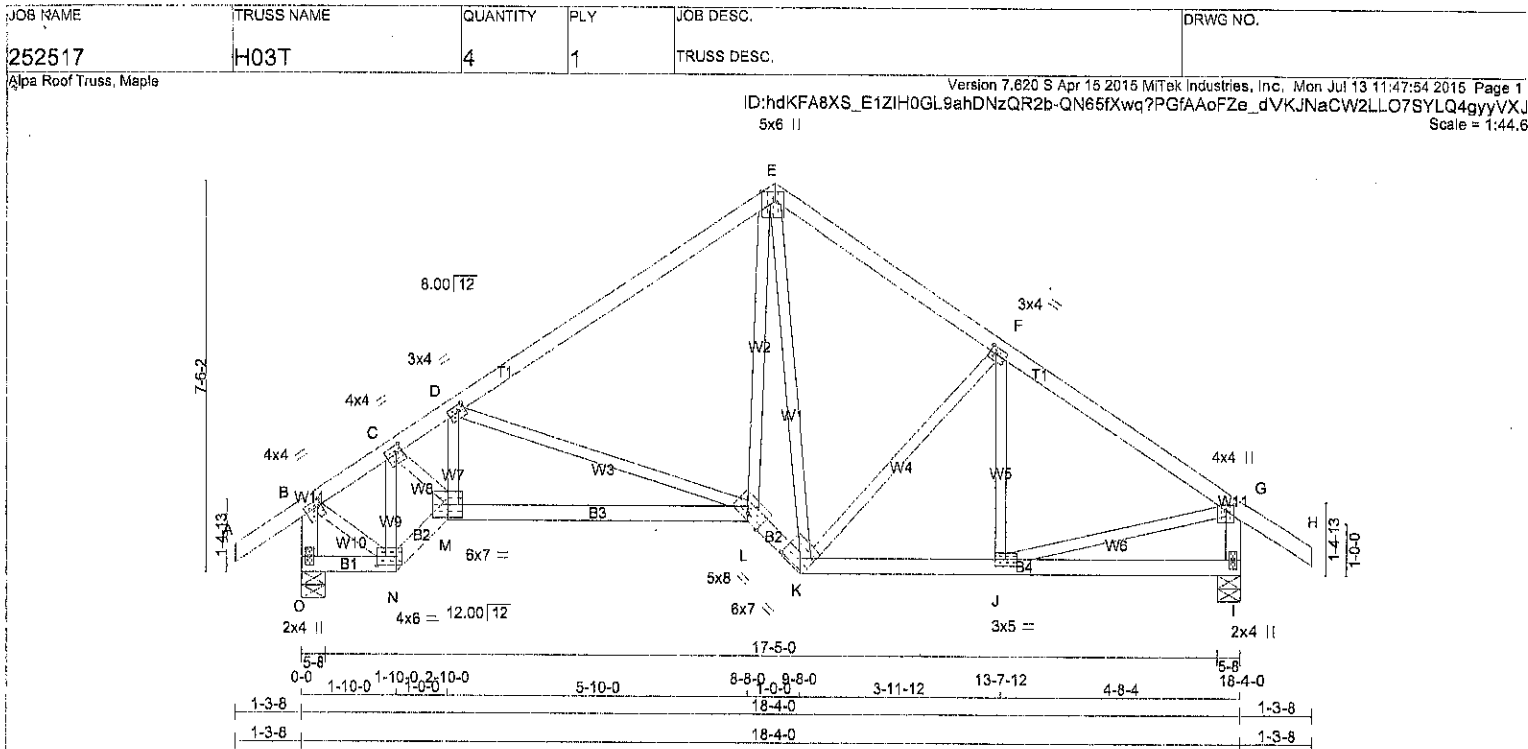
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.48 (B) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)



A-15073478



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	4.0	4.0	1.50	1.00
C	TMVW-I	MT20	4.0	4.0	1.75	1.50
D	TMVW-I	MT20	3.0	4.0	1.50	1.50
E	TMVW-I	MT20	5.0	6.0	Edge	3.00
F	TMVW-I	MT20	3.0	4.0	1.50	1.50
G	TMVW-I	MT20	4.0	4.0	1.25	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BMVW-I	MT20	3.0	5.0		
K	BBVW-h	MT20	6.0	7.0	3.75	6.00
L	BBVW-h	MT20	5.0	8.0	Edge	
M	BBVW-I	MT20	6.0	7.0		
N	BBVW-I	MT20	4.0	6.0	2.00	4.50
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
O	907	0	0	907	0	0	5-8	1-8	
I	899	0	0	899	0	0	5-8	1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN		COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
O	836	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0		
I	831	440 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO				FR-TO			
A-B	0 / 27	-70.5	-70.5 0.09 (1)	10.00	N-C	-768 / 0	0.12 (1)
B-C	-677 / 0	-70.5	-70.5 0.08 (1)	6.25	C-M	0 / 888	0.20 (1)
C-D	-1152 / 0	-70.5	-70.5 0.30 (1)	5.57	M-D	-104 / 67	0.02 (4)
D-E	-761 / 0	-70.5	-70.5 0.33 (1)	6.25	D-L	-512 / 0	0.35 (1)
E-F	-656 / 0	-70.5	-70.5 0.19 (1)	6.25	L-E	0 / 781	0.18 (1)
F-G	-838 / 0	-70.5	-70.5 0.20 (1)	6.25	E-K	-313 / 0	0.32 (1)
G-H	0 / 27	-70.5	-70.5 0.09 (1)	10.00	K-F	-272 / 0	0.15 (1)
O-B	-890 / 0	0.0	0.0 0.09 (1)	7.81	J-F	-79 / 51	0.02 (1)
I-G	-865 / 0	0.0	0.0 0.09 (1)	7.81	B-N	0 / 607	0.14 (1)
				J-G	0 / 734	0.17 (1)	
O-N	0 / 0	-17.5	-17.5 0.02 (4)	10.00			
N-M	0 / 893	-17.5	-17.5 0.11 (1)	10.00			
M-L	0 / 1096	-17.5	-17.5 0.28 (1)	10.00			
L-K	0 / 777	-17.5	-17.5 0.13 (1)	10.00			
K-J	0 / 715	-17.5	-17.5 0.16 (1)	10.00			
J-I	0 / 0	-17.5	-17.5 0.09 (4)	10.00			

TOTAL WEIGHT = 4 X 86 = 343 lb [M/F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
BOT CH.	LL = 3.0	PSF
	DL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.10")

CSI: TC=0.33 (D-E:1), BC=0.28 (L-M:1), WB=0.35 (D-L:1), SSI=0.21 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 822 2284 1686	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (C) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

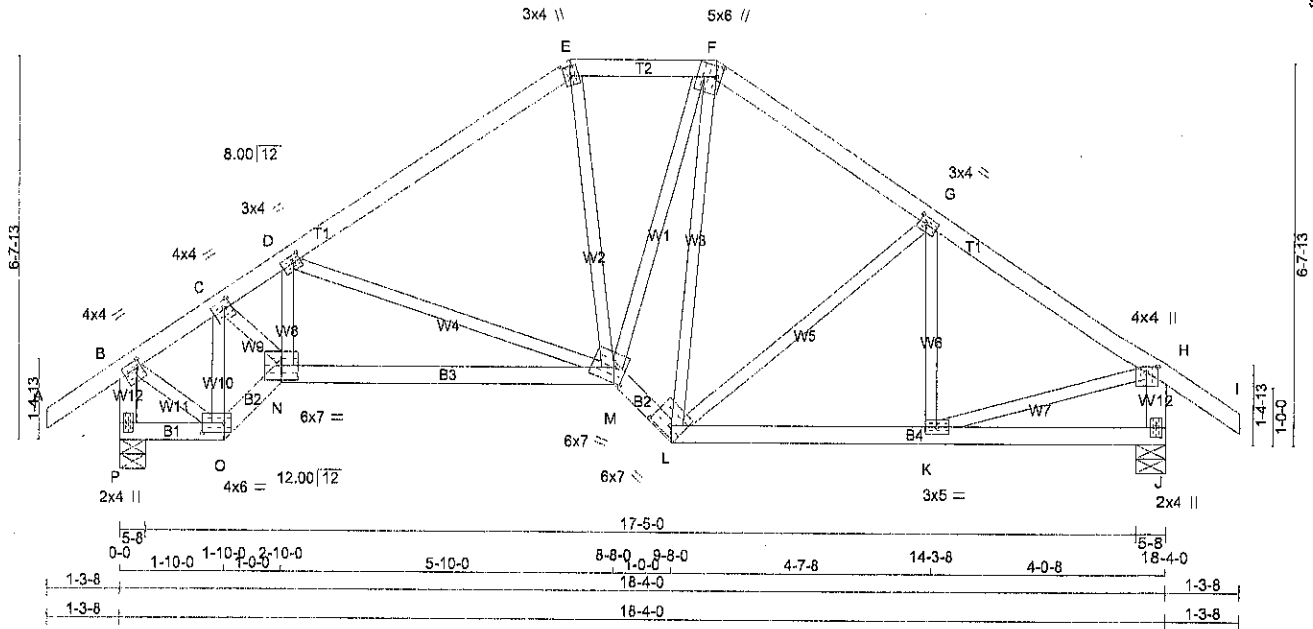


A-15073472

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252517	H02T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:hdKFA8XS_E1ZIH0GL9ahDNzQR2b-QN55fXwq?PGfAAoFZe_dVKJPcCWBLMI7SYLQ4gyyVX.
Scale = 1:40.2



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - N	2x4	DRY	No.2
N - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWW-t	MT20	4.0	4.0 1.50 1.00
C	TMWW-t	MT20	4.0	4.0 1.75 1.50
D	TMWW-t	MT20	3.0	4.0 1.50 1.50
E	TTW+m	MT20	3.0	4.0 2.00 1.25
F	TTW+m	MT20	6.0	6.0 2.25 1.00
G	TMWW-t	MT20	3.0	4.0 1.50 1.50
H	TMWW-p	MT20	4.0	4.0 1.25 2.00
J	BMV1+p	MT20	2.0	4.0
K	BMWW-h	MT20	3.0	5.0
L	BBWW-h	MT20	6.0	7.0 2.00 4.25
M	BBWW-m	MT20	8.0	7.0 3.00 2.50
N	BBWW-h	MT20	6.0	7.0
O	BBWW-h	MT20	4.0	6.0 2.00 4.50
P	BMV1+p	MT20	2.0	4.0

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD.
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
P	907	0	907	0	5-8
J	899	0	899	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	636	444 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0	0 / 0	0 / 0
J	631	440 / 0	0 / 0	0 / 0	0 / 0	191 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	MAX UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX	FACTORED	MAX
FR-TO		FROM TO	CSI (LC)		FR-TO		CSI (LC)		
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	O-C	-782 / 0		0.13 (1)
B-C	-682 / 0	-70.5	-70.5	0.07 (1)	6.25	C-N	0 / 797		0.18 (1)
C-D	-1151 / 0	-70.5	-70.5	0.18 (1)	5.73	N-D	-28 / 67		0.02 (4)
D-E	-797 / 0	-70.5	-70.5	0.20 (1)	8.25	D-M	-416 / 0		0.26 (1)
E-F	-677 / 0	-70.5	-70.5	0.06 (1)	8.25	E-M	0 / 203		0.06 (1)
F-G	-679 / 0	-70.5	-70.5	0.14 (1)	8.25	M-F	0 / 532		0.12 (1)
G-H	-838 / 0	-70.5	-70.5	0.15 (1)	6.25	L-F	-331 / 0		0.25 (1)
H-I	0 / 27	-70.5	-70.5	0.09 (1)	10.00	L-G	-210 / 0		0.13 (1)
P-B	-890 / 0	0.0	0.0	0.09 (1)	7.81	K-G	-104 / 42		0.03 (1)
J-H	-872 / 0	0.0	0.0	0.09 (1)	7.81	B-O	0 / 618		0.14 (1)
						K-H	0 / 737		0.17 (1)
P-O	0 / 0	-17.5	-17.5	0.02 (4)	10.00				
O-N	0 / 706	-17.5	-17.5	0.12 (1)	10.00				
N-M	0 / 1040	-17.5	-17.5	0.27 (1)	10.00				
M-L	0 / 718	-17.5	-17.5	0.12 (1)	10.00				
L-K	0 / 712	-17.5	-17.5	0.16 (1)	10.00				
K-J	0 / 0	-17.5	-17.5	0.09 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.09")

CSI: TO=0.20 (D-E:1), BC=0.27 (M-N:1), WB=0.26 (D-M:1), SSI=0.14 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1856

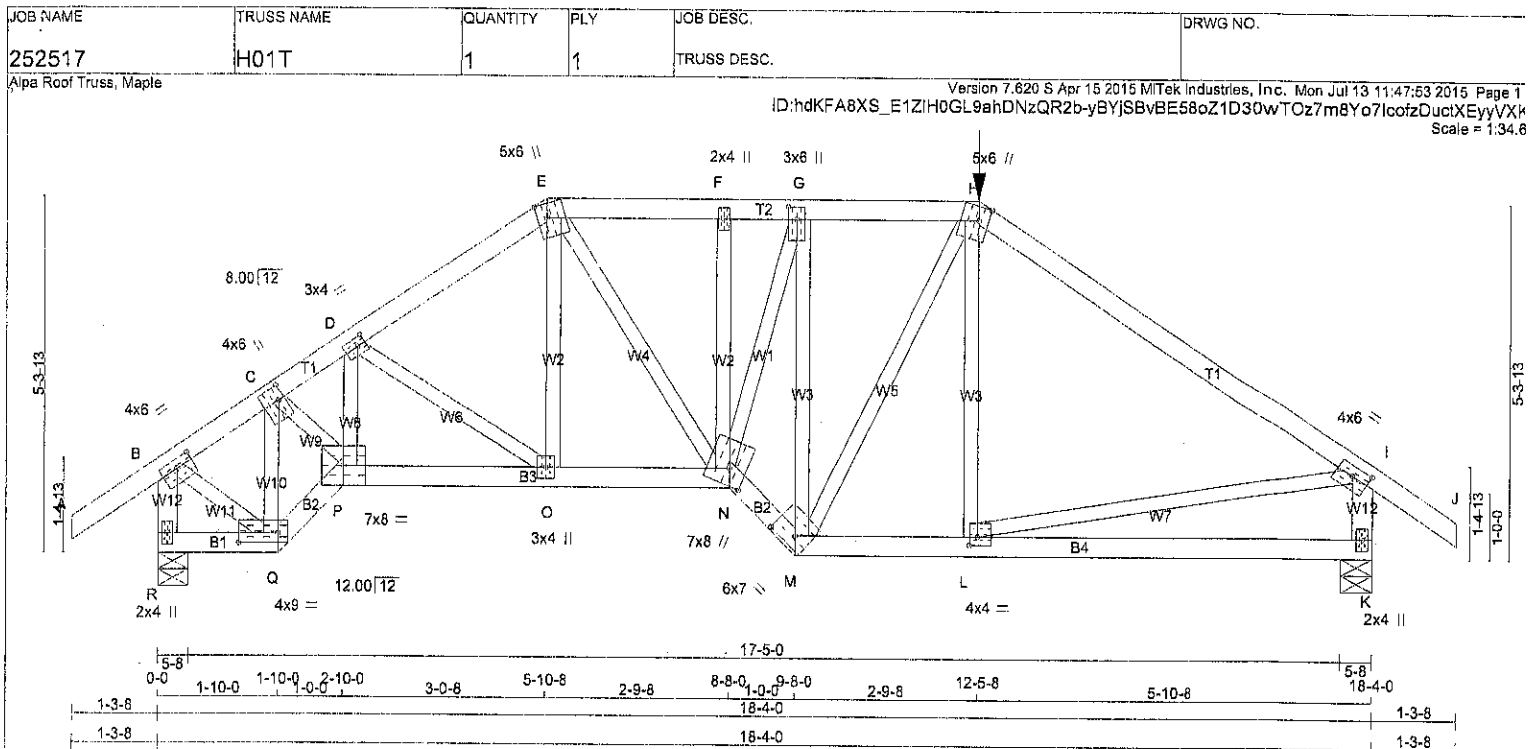
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (E) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)



A-15073471



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY No.2	SPF
E - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x4	DRY No.2	SPF
K - I	2x4	DRY No.2	SPF
R - Q	2x4	DRY No.2	SPF
Q - P	2x4	DRY No.2	SPF
P - N	2x4	DRY No.2	SPF
N - M	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	6.0	2.50	1.00
D	TMVW-t	MT20	3.0	4.0	1.50	1.50
E	TTWV+m	MT20	5.0	6.0	Edge	1.75
F	TMVW-t	MT20	2.0	4.0		
G	TMVW-t	MT20	3.0	6.0	2.25	1.50
H	TTWV+m	MT20	5.0	6.0	Edge	1.75
I	TMVW-t	MT20	4.0	6.0	1.75	3.00
K	BMV1+p	MT20	2.0	4.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BBWV-h	MT20	6.0	7.0	2.00	4.25
N	BBWVW+m	MT20	7.0	8.0	3.00	3.00
O	BMVW-t	MT20	3.0	4.0		
P	BBWV-h	MT20	7.0	8.0		
Q	BBWV-h	MT20	4.0	9.0	1.75	7.25
R	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UPLIFT
JT	1730	0	1730	0
R	1730	0	1730	0
K	1553	0	1553	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
JT	1215	842 / 0	0 / 0 0 / 0 0 / 0 373 / 0 0 / 0
R	1095	737 / 0	0 / 0 0 / 0 0 / 0 358 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 27	-70.5 -70.5	0.10 (1)	10.00	Q-C	-1680 / 0	0.29 (1)
B-C	-1413 / 0	-138.7 -138.7	0.13 (1)	5.33	C-P	0 / 1378	0.34 (1)
C-D	-2377 / 0	-138.7 -138.7	0.13 (1)	4.32	P-D	0 / 220	0.05 (1)
D-E	-1980 / 0	-70.5 -70.5	0.15 (1)	4.64	D-O	-422 / 0	0.11 (1)
E-F	-1885 / 0	-70.5 -70.5	0.16 (1)	4.72	O-E	0 / 568	0.14 (1)
F-G	-1983 / 0	-138.7 -138.7	0.16 (1)	4.72	E-N	0 / 402	0.10 (1)
G-H	-1536 / 0	-138.7 -138.7	0.17 (1)	5.11	N-F	-83 / 3	0.02 (1)
H-I	-1648 / 0	-70.5 -70.5	0.61 (1)	4.43	F-G	-83 / 3	0.02 (1)
I-J	0 / 27	-70.5 -70.5	0.10 (1)	10.00	G-H	-1758 / 0	0.76 (1)
R-B	-1698 / 0	0.0 0.0	0.18 (1)	6.33	M-H	0 / 342	0.08 (1)
K-I	-1468 / 0	0.0 0.0	0.17 (1)	8.71	L-H	-57 / 151	0.06 (4)
					B-Q	0 / 1324	0.33 (1)
					L-I	0 / 1392	0.34 (1)
R-Q	0 / 0	-34.5 -34.5	0.04 (4)	10.00			
Q-P	0 / 1512	-34.5 -34.5	0.27 (1)	10.00			
P-O	0 / 1990	-102.7 -102.7	0.50 (1)	10.00			
O-N	0 / 1654	-102.7 -102.7	0.44 (1)	10.00			
N-M	0 / 2120	-34.5 -34.5	0.38 (1)	10.00			
M-L	0 / 1368	-34.5 -34.5	0.41 (4)	10.00			
L-K	0 / 0	-34.5 -34.5	0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	12-5-8	-302	-302	---	FRONT	VERT	TOTAL

TOTAL WEIGHT = 90 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 0-0
END SETBACK = 5-10-6
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 2-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CSIdGirder
START DISTANCE = 2-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 8-8-0
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-8-0 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.61 (H-I:1), BC=0.50 (O-P:1), WB=0.76 (G-M:1), GS=0.13 (A-F:1)



A-NO13470
CONTINUED ON PAGE 2

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

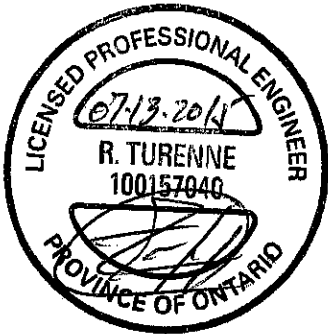
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.51 (L) (INPUT = 1.00)

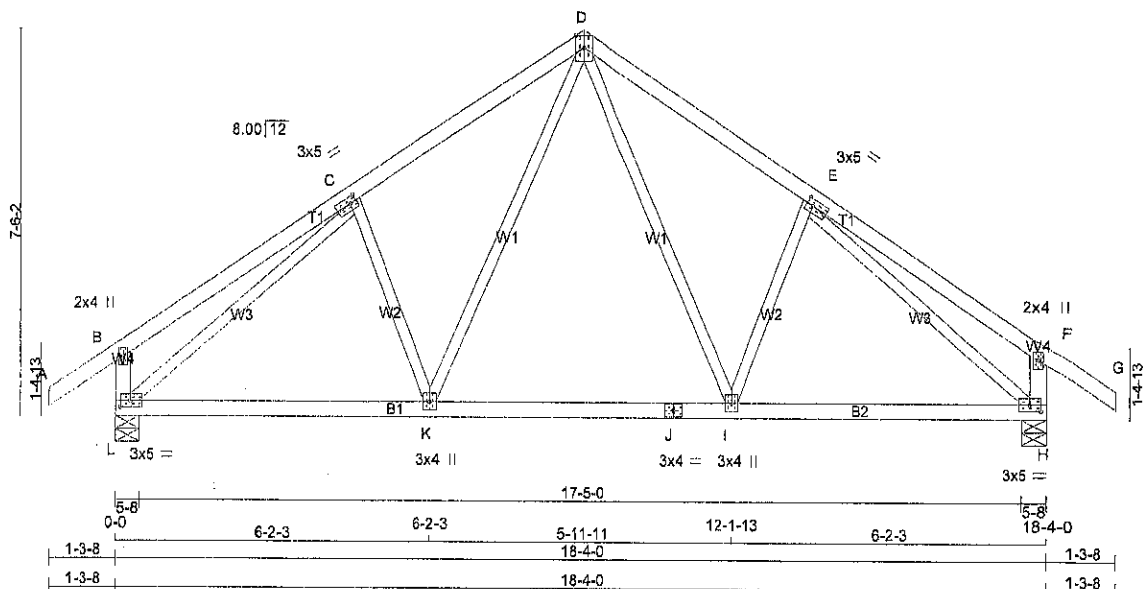


A-15073470(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H03	14	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 13 08:58:51 2015 Page 1
ID: gUzNWMpyWh_3Mf_ncsznAgzVRmF-v1kdHAThZLpQurtwV2wNj1pVkgJvD94_qBZ4yyY?o
4x6 II
Scale = 1:44.9



TOTAL WEIGHT = 14 X 81 = 1128 lb
(M/F)

LUMBER				
N. L. G. A. RULES	SIZE	LUMBER	DESCR.	
CHORDS				
A - D	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	3.0	5.0	1.50	2.25
I	BMWW-t	MT20	3.0	4.0		
J	BS-t	MT20	3.0	4.0		
K	BMWW-t	MT20	3.0	4.0		
L	BMVW1-t	MT20	3.0	5.0	1.50	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTES- (1)

1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
L	903	0	903	0	5-8	1-8
H	903	0	903	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
L	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0
H	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRAC LENGTH	MAX. FACTORED LC1 MAX (LC)
FR-TO				FR-TO			
A-B	0 / 27	-70.5	-70.5 0.09 (1)	D-I	0 / 323	10.00	0.07 (1)
B-C	0 / 21	-70.5	-70.5 0.24 (1)	I-E	-196 / 3	10.00	0.07 (1)
C-D	-764 / 0	-70.5	-70.5 0.19 (1)	K-D	0 / 323	6.25	0.07 (1)
D-E	-764 / 0	-70.5	-70.5 0.19 (1)	C-K	-196 / 3	6.25	0.07 (1)
E-F	0 / 21	-70.5	-70.5 0.24 (1)	L-C	-953 / 0	10.00	0.62 (1)
F-G	0 / 27	-70.5	-70.5 0.09 (1)	E-H	-953 / 0	10.00	0.62 (1)
L-B	-219 / 0	0.0	0.0 0.02 (1)				
H-F	-219 / 0	0.0	0.0 0.02 (1)				
L-K	0 / 707	-17.5	-17.5 0.20 (4)				
K-J	0 / 511	-17.5	-17.5 0.19 (4)				
J-I	0 / 511	-17.5	-17.5 0.19 (4)				
I-H	0 / 707	-17.5	-17.5 0.20 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	21.0	PSF
DL	=	3.0	PSF
BOT CH. LL	=	0.0	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.24 (E-F:1), BC=0.20 (H-I:4), WB=0.62 (E-H:1), SSI=0.13 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

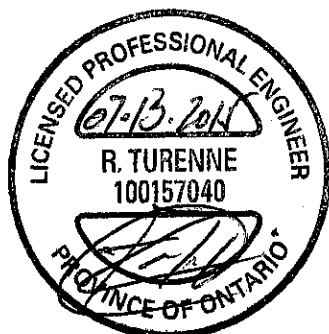
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.30 (C) (INPUT = 1.00)

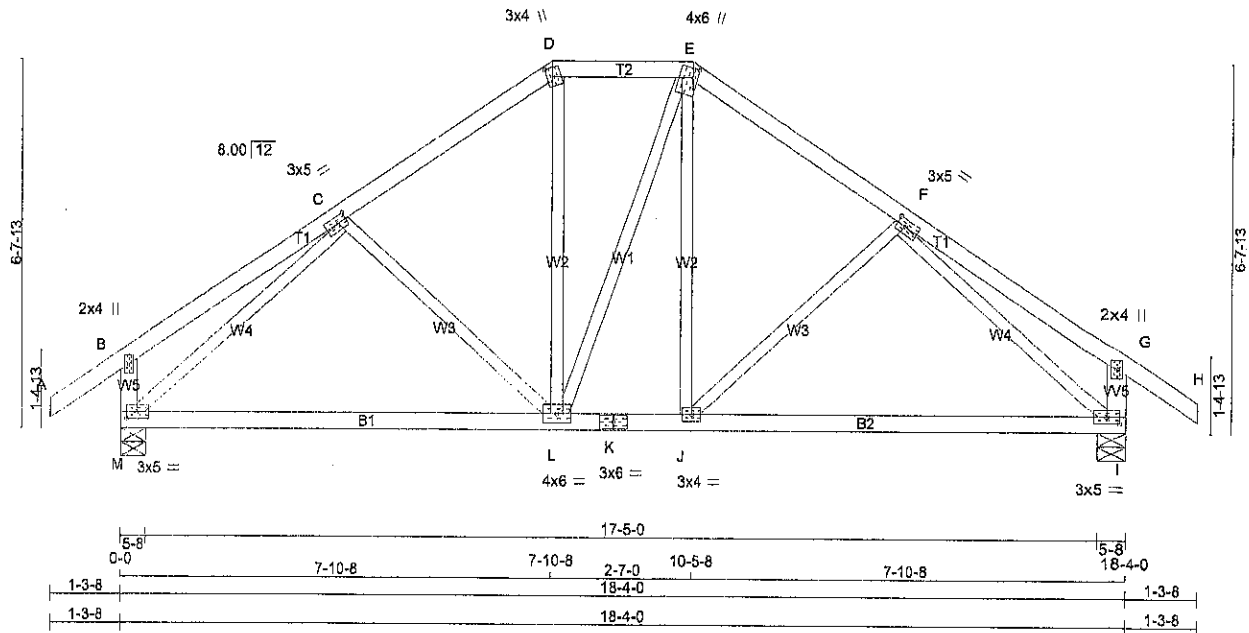


A-15073442

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
252512	H02	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 7.620 S Apr 15 2015 MITek Industries, Inc. Mon Jul 13 08:58:51 2015 Page 1
ID:gUzNWMpyWh_3Mf_ncsznAqzVRmF-v1kdtHATHzLqQurtwV2wNj2rVj1Jyv94_gBZ4yyY?o
Scale = 1:41.8



TOTAL WEIGHT = 63 lb
[M][F]

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
M - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
M - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+m	MT20	3.0	4.0	2.00	1.25
E	TTWW+m	MT20	4.0	6.0	2.00	1.00
F	TMWW-t	MT20	3.0	5.0	1.50	2.00
G	TMV+p	MT20	2.0	4.0		
I	BMVW1-t	MT20	3.0	5.0	1.50	2.25
J	BMWW-t	MT20	3.0	4.0		
K	9S-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	6.0		
M	BMVW1-t	MT20	3.0	5.0	1.50	2.25

NOTES- (1)
1) Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	SNOW	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
M	903	0	903	0	0	5-8	1-8		
I	903	0	903	0	0	5-8	1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	834	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0		
I	634	442 / 0	0 / 0	0 / 0	0 / 0	192 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRACED		(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO			
A-B	0 / 27	-70.5	-70.5	0.09 (1)	10.00	C-L	-133 / 10	0.07 (1)
B-C	0 / 18	-70.5	-70.5	0.17 (1)	10.00	L-D	0 / 192	0.04 (1)
C-D	-719 / 0	-70.5	-70.5	0.14 (1)	6.25	L-E	0 / 1	0.00 (1)
D-E	-566 / 0	-70.5	-70.5	0.06 (1)	6.25	J-E	0 / 191	0.04 (1)
E-F	-719 / 0	-70.5	-70.5	0.14 (1)	6.25	J-F	-133 / 10	0.07 (1)
F-G	0 / 18	-70.5	-70.5	0.17 (1)	10.00	M-C	-936 / 0	0.44 (1)
G-H	0 / 27	-70.5	-70.5	0.09 (1)	10.00	F-I	-936 / 0	0.44 (1)
M-B	-203 / 0	0.0	0.0	0.02 (1)	7.81			
I-G	-203 / 0	0.0	0.0	0.02 (1)	7.81			
M-L	0 / 681	-17.5	-17.5	0.28 (4)	10.00			
L-K	0 / 586	-17.5	-17.5	0.28 (4)	10.00			
K-J	0 / 586	-17.5	-17.5	0.28 (4)	10.00			
J-I	0 / 681	-17.5	-17.5	0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 21.0 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 31.0 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(65 % OF 23.0 P.S.F. G.S.L. PLUS
6.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.61")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.12")

CSI: TC=0.17 (F-G:1), BC=0.28 (J-L:4), WB=0.44 (C-M:1), SSI=0.11 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1666

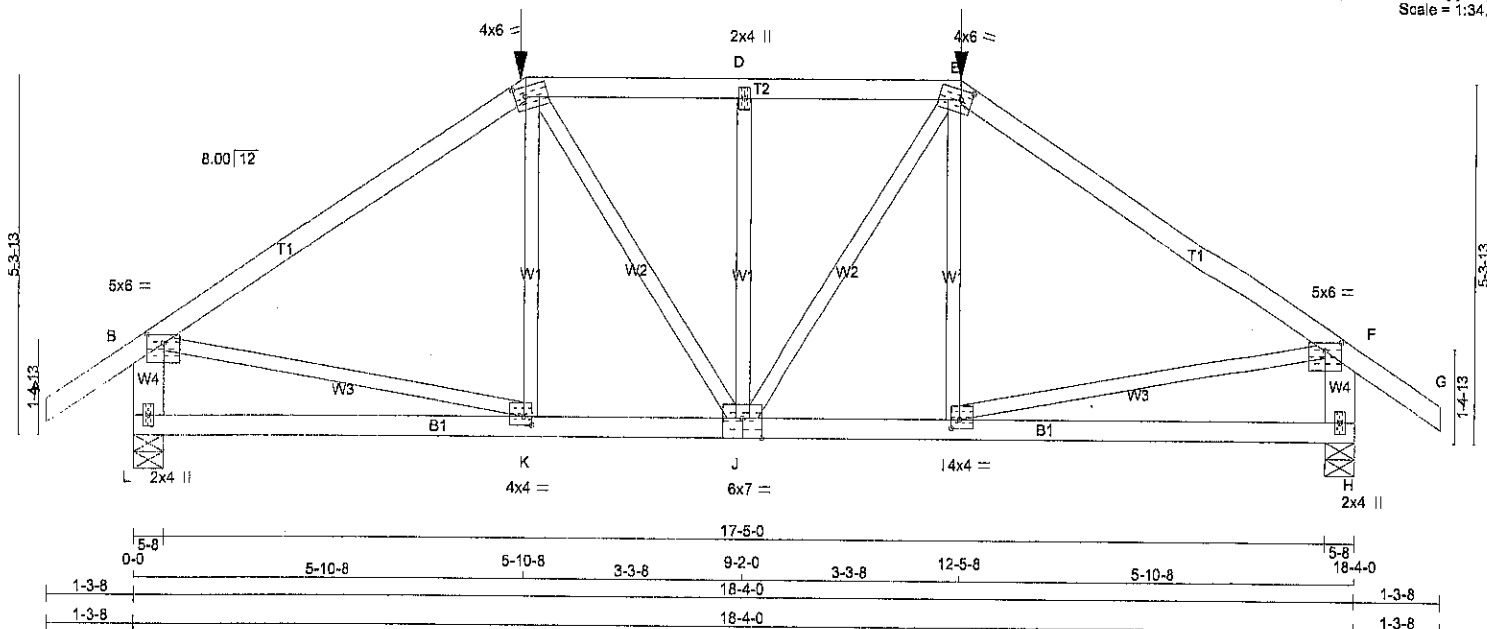
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.42 (K) (INPUT = 1.00)



A-15073441



TOTAL WEIGHT = 82 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
L - B	2x6	DRY No.2	SPF
H - F	2x6	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW-m	MT20	4.0	6.0	1.75	2.00
D	TMVW+w	MT20	2.0	4.0		
E	TTWW-m	MT20	4.0	6.0	1.75	2.00
F	TMVW-p	MT20	5.0	6.0	1.50	3.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	1.50	1.50
J	BSWWW-I	MT20	6.0	7.0	Edge	3.50
K	BMWW-t	MT20	4.0	4.0	1.50	1.50
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 301.8 lbs FACTORED DOWN AT 12-5-8, AND 301.8 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTES- (1)

1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	IN-SX	IN-SX
L	1585	0	1585	0
H	1585	0	1585	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
L	1117	766 / 0	0 / 0 0 / 0 0 / 0 361 / 0 0 / 0
H	1117	766 / 0	0 / 0 0 / 0 0 / 0 361 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00FT. OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MAX	MEMB.	FORCE	MAX
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 27	-70.5 -70.5	0.10 (1)	10.00	K-C	-37 / 163	0.06 (4)
B-C	-1694 / 0	-70.5 -70.5	0.62 (1)	4.38	C-J	0 / 341	0.08 (1)
C-D	-1594 / 0	-138.7 -138.7	0.29 (1)	4.66	J-D	-544 / 0	0.24 (1)
D-E	-1594 / 0	-138.7 -138.7	0.29 (1)	4.66	J-E	0 / 341	0.09 (1)
E-F	-1694 / 0	-70.5 -70.5	0.62 (1)	4.38	I-E	-37 / 163	0.05 (4)
F-G	0 / 27	-70.5 -70.5	0.10 (1)	10.00	B-K	0 / 1430	0.35 (1)
L-B	-1500 / 0	0.0 0.0	0.11 (1)	7.81	I-F	0 / 1430	0.35 (1)
H-F	-1500 / 0	0.0 0.0	0.11 (1)	7.81			
L-K	0 / 0	-34.5 -34.5	0.29 (4)	10.00			
K-J	0 / 1407	-34.5 -34.5	0.41 (4)	10.00			
J-I	0 / 1407	-34.5 -34.5	0.41 (4)	10.00			
I-H	0 / 0	-34.5 -34.5	0.29 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	5-10-8	-302	-302	--	FRONT	VERT	TOTAL
E	12-5-8	-302	-302	--	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	31.0	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 23.0 P.S.F. G.S.L. PLUS
8.4 P.S.F. RAIN LOAD EQUALS
21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.09")

CSI: TC=0.62 (B-C:1), BC=0.41 (J-K:4), WB=0.35 (F-I:1), SSI=0.24 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.53 (K) (INPUT = 1.00)



A-11073440

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

MATERIAL: 18 gauge

FINISH: G90 galvanized

DESIGN:

- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

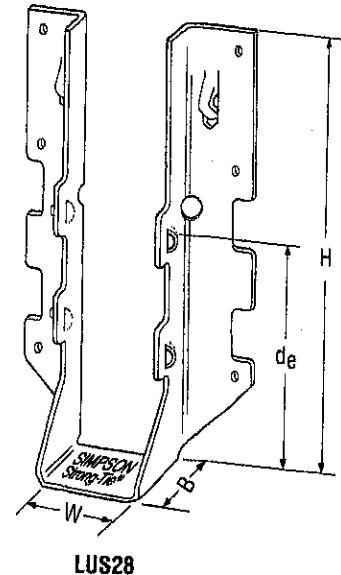
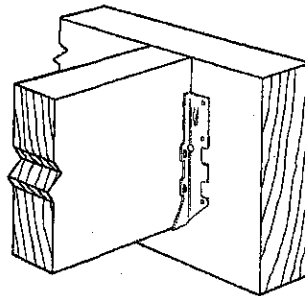
INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

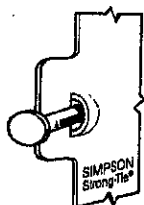
- These hangers cannot be modified.

Typical LUS
Installation



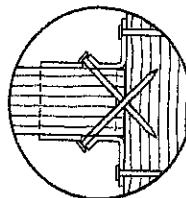
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	dg ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _D =1.15)	Normal (K _D =1.00)	Uplift (K _D =1.15)	Normal (K _D =1.00)
LUS24	18	1 1/16	3 1/8	1 3/4	1 15/16	4-10d	2-10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	4-16d	2-16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 3/8	4-10d	4-10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	4-16d	4-16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3 3/4	4-16d	4-16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 3/8	1 3/4	3 3/4	6-10d	4-10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	6-16d	4-16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	6-16d	4-16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 1/16	1 3/4	3 3/8	8-10d	4-10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	8-16d	6-16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 9/16	2	5 1/4	8-16d	6-16d	2580	3345	2320	2375

1. dg is the distance from the seat of the hanger to the highest joist nail.

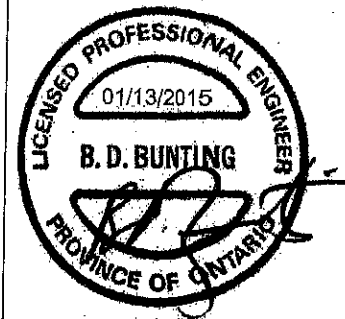


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580

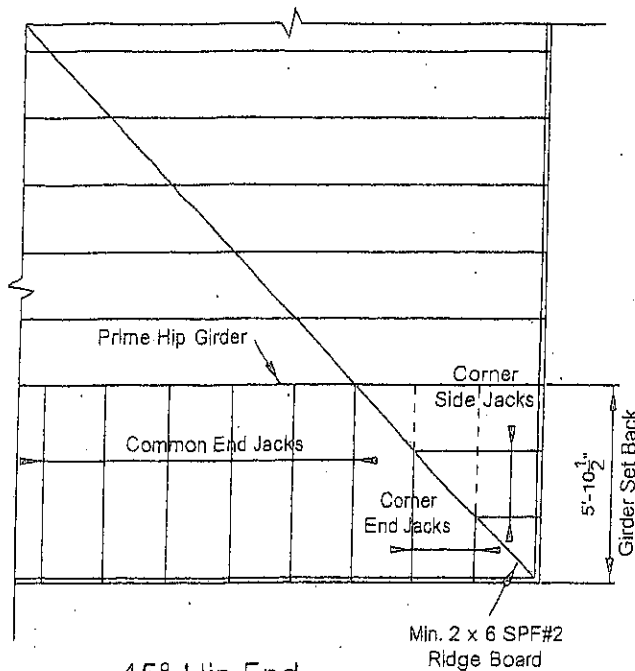


Double
Shear
Nailing
Top View.



800-999-5099
www.strongtie.com

STRACON ENGINEERING INC.

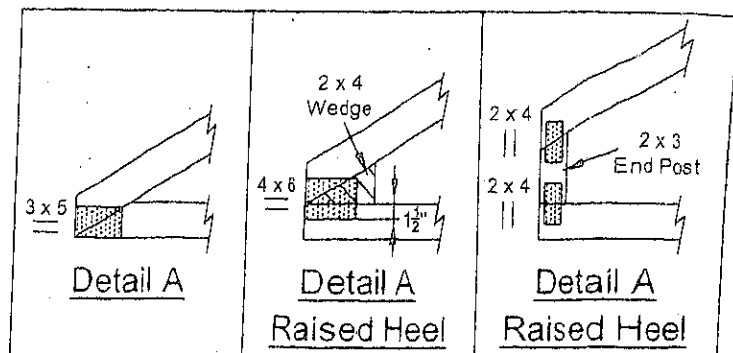
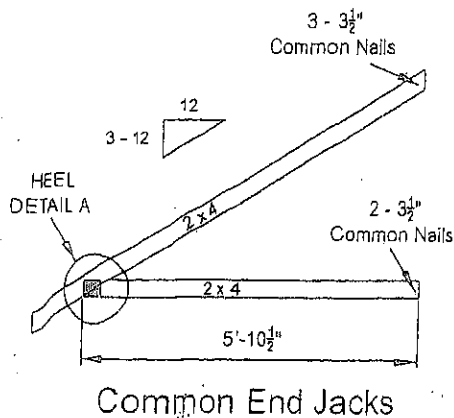
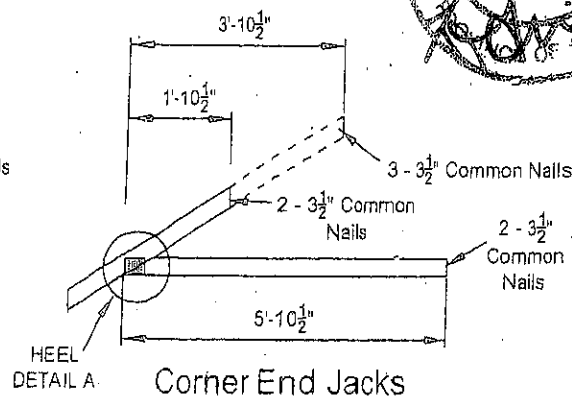
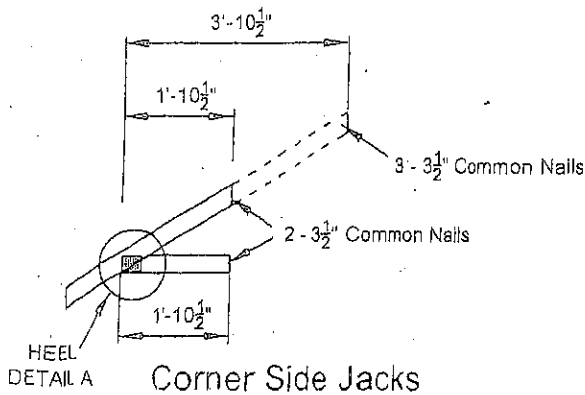


LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
 TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008