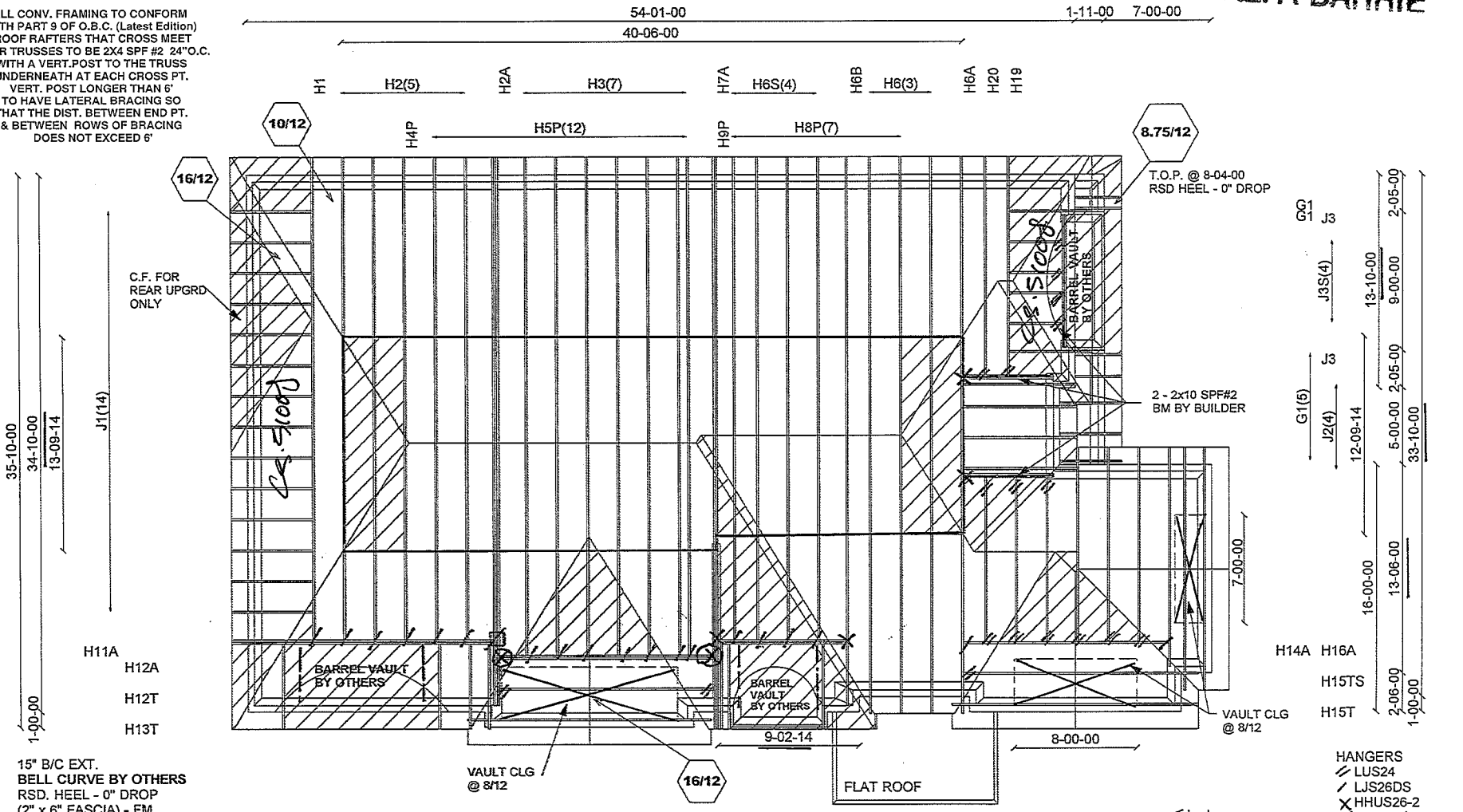
ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
☒ HGUS28-2
⊗ HGUS26-2

OFFICE USE ~~ONLY~~
ENGINEERED FROM
A-1802362 TO
A-1802368

CONVENTIONAL FRAMING BY OTHERS

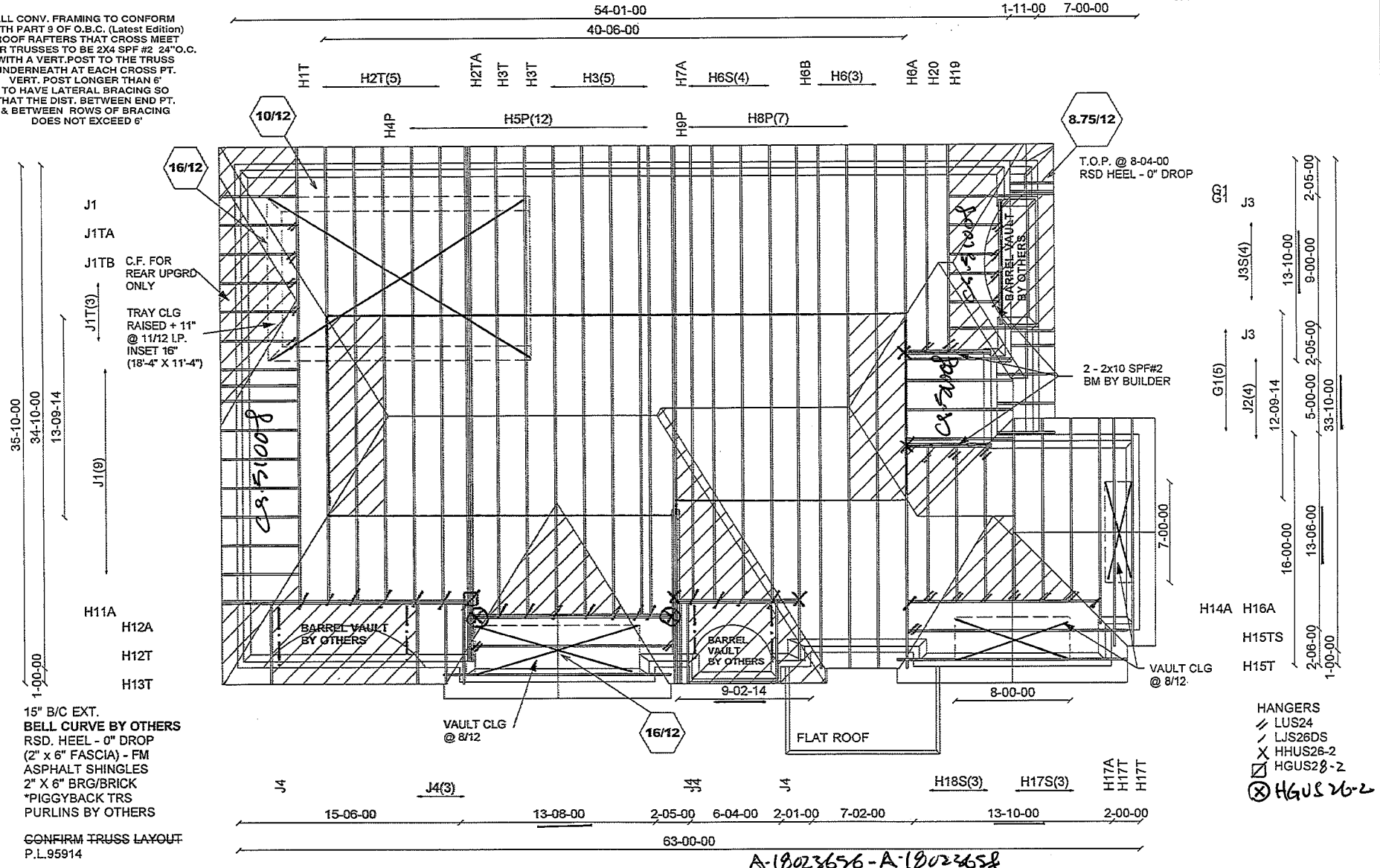
BROOKVALLEY

STD OR OPT 2ND FLR (5 BED) OR REAR UPG.

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1	/ B
Layout ID: 292616			
Plan Log: 95914	Project: PINE VALLEY		
	Date: 1/18/2018	Designer: AMANDA	

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
☒ HGUS28-2
⊗ HGUS26-2

CONVENTIONAL FRAMING BY OTHERS

A-18023656-A-18023658
A-18023665 + A-18023692

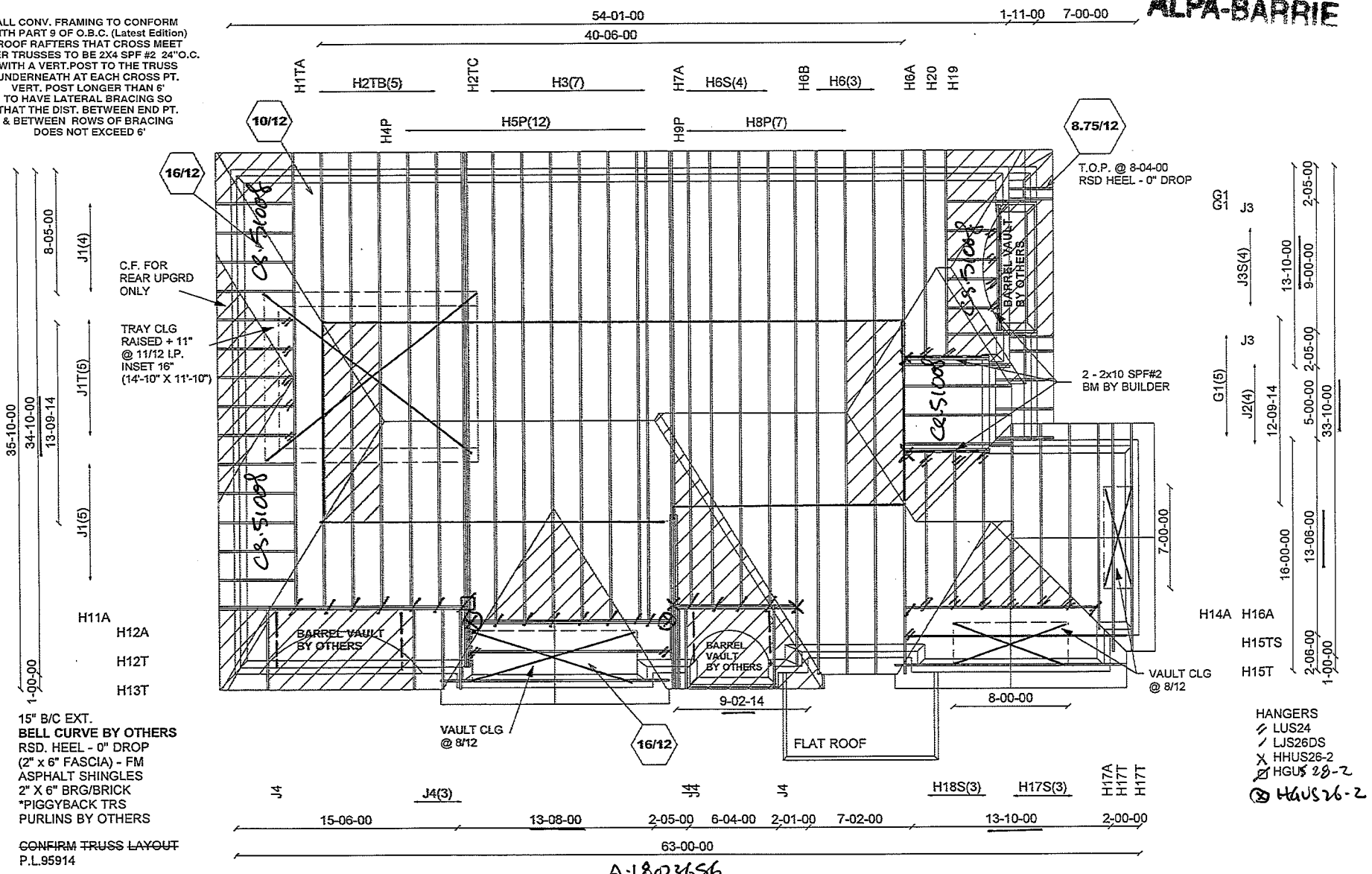
BROOKVALLEY

OR REAR UPG.

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1 / B OPT TRAY	
Layout ID: 292617	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/18/2018	Designer: AMANDA	

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT~~
P.L.95914

HANGERS
- LUS24
- LJS26DS
- HHUS26-2
- HGUS 28-2
- HGUS26-2

A-18023656
A-18023665 - A-18023695

CONVENTIONAL FRAMING BY OTHERS

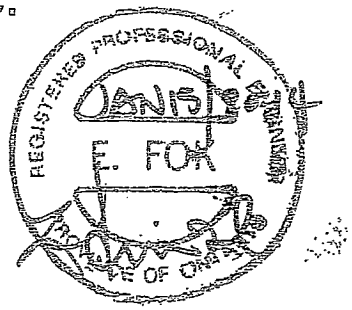
BROOKVALLEY

OR REAR UPG.

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 4204 REV1	/ B OPT TRAY 5 BED
Layout ID: 292618	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/18/2018 Designer: AMANDA		

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

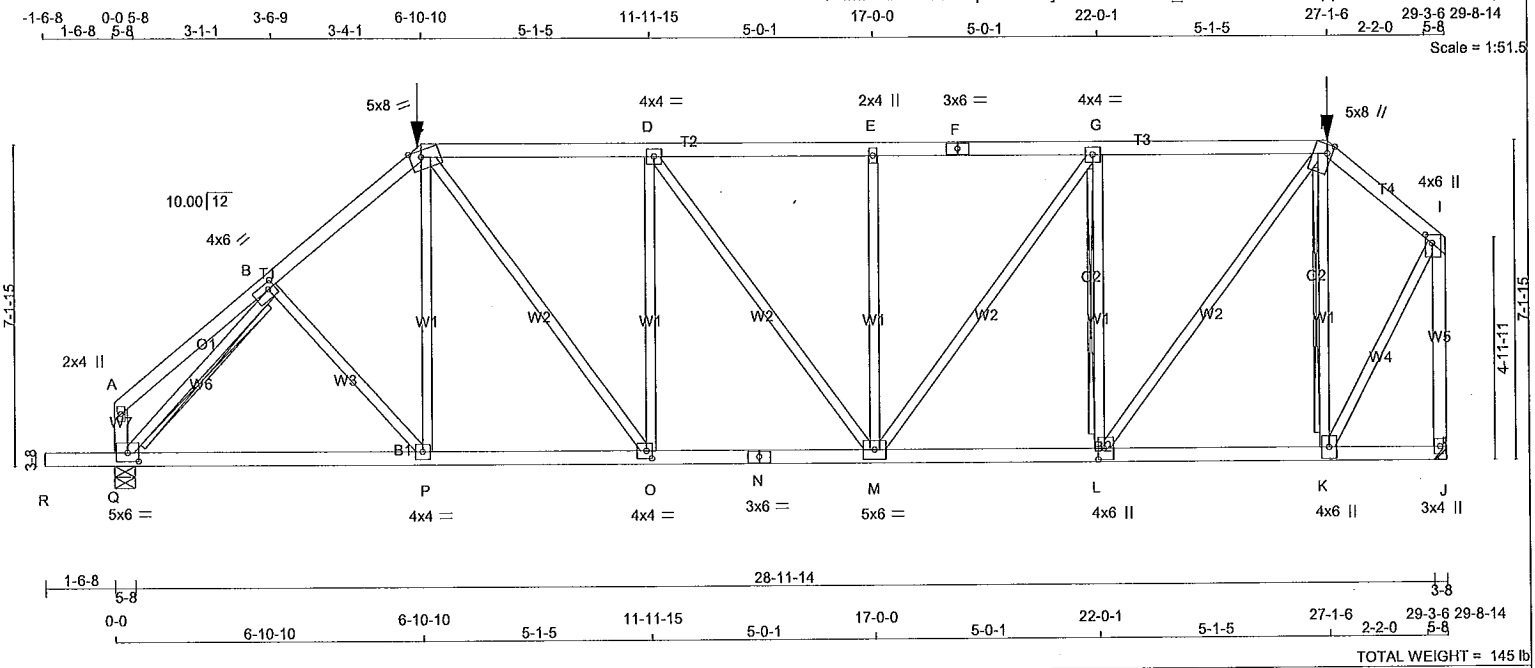
SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple



TOTAL WEIGHT = 145 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

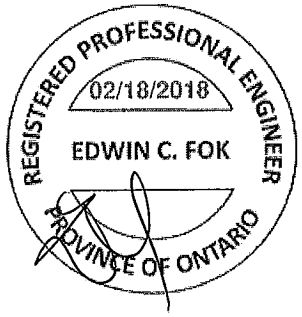
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	1.75	1.75
C	TTWW-m	MT20	5.0	8.0	Edge	3.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	8.0	Edge	1.25
I	TMVW+p	MT20	4.0	6.0	2.25	1.75
J	BMV1+p	MT20	3.0	4.0		
K	BMWW+t	MT20	4.0	6.0		
L	BMWW+t	MT20	4.0	6.0	3.00	1.50
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0	2.00	1.50
P	BMWW-t	MT20	4.0	4.0		
Q	BMVW1-t	MT20	5.0	6.0	2.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	2199	0	2199	0	0	5-8	2-5
J	2109	0	2109	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-6	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1569	951 / 0	0 / 0	0 / 0	0 / 0	618 / 0	0 / 0
J	1504	918 / 0	0 / 0	0 / 0	0 / 0	586 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT G-L, H-K, B-Q

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 17	-78.0	-78.0	0.14 (1)	10.00	B- P	0 / 142	0.04 (4)
B- C	-2256 / 0	-78.0	-78.0	0.22 (1)	4.33	P- C	0 / 151	0.06 (4)
C- D	-2301 / 0	-114.5	-114.5	0.61 (1)	3.77	C- O	0 / 979	0.24 (1)
D- E	-2312 / 0	-114.5	-114.5	0.61 (1)	3.76	O- D	-666 / 0	0.60 (1)
E- F	-2312 / 0	-114.5	-114.5	0.61 (1)	3.77	D- M	0 / 20	0.01 (4)
F- G	-2312 / 0	-114.5	-114.5	0.61 (1)	3.77	M- E	-527 / 0	0.48 (1)
G- H	-1839 / 0	-114.5	-114.5	0.56 (1)	4.21	M- G	0 / 803	0.20 (1)
H- I	-1023 / 0	-78.0	-78.0	0.12 (1)	6.04	L- G	-1303 / 0	0.48 (1)
Q- A	-107 / 0	0.0	0.0	0.01 (1)	7.81	L- H	0 / 1797	0.44 (1)
J- I	-2090 / 0	0.0	0.0	0.95 (1)	5.81	K- H	-1205 / 0	0.45 (1)
						Q- B	-2470 / 0	0.59 (1)
R- Q	0 / 0	-96.5	-96.5	0.17 (1)	10.00	K- I	0 / 1542	0.38 (1)
Q- P	0 / 1620	-27.2	-27.2	0.47 (4)	10.00			
P- O	0 / 1716	-27.2	-27.2	0.49 (4)	10.00			
O- N	0 / 2302	-27.2	-27.2	0.48 (1)	10.00			
N- M	0 / 2302	-27.2	-27.2	0.48 (1)	10.00			
M- L	0 / 1839	-27.2	-27.2	0.40 (1)	10.00			
L- K	0 / 765	-27.2	-27.2	0.22 (4)	10.00			
K- J	0 / 0	-27.2	-27.2	0.12 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-10-10	-213	-213	—	FRONT	VERT	TOTAL
H	27-1-6	-81	-81	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-10-10
RIGHT SETBACK = 2-7-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.95/1.00 (I-J:1) , BC=0.49/1.00 (O-P:4) , WB=0.60/1.00 (D-O:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

A-18023662

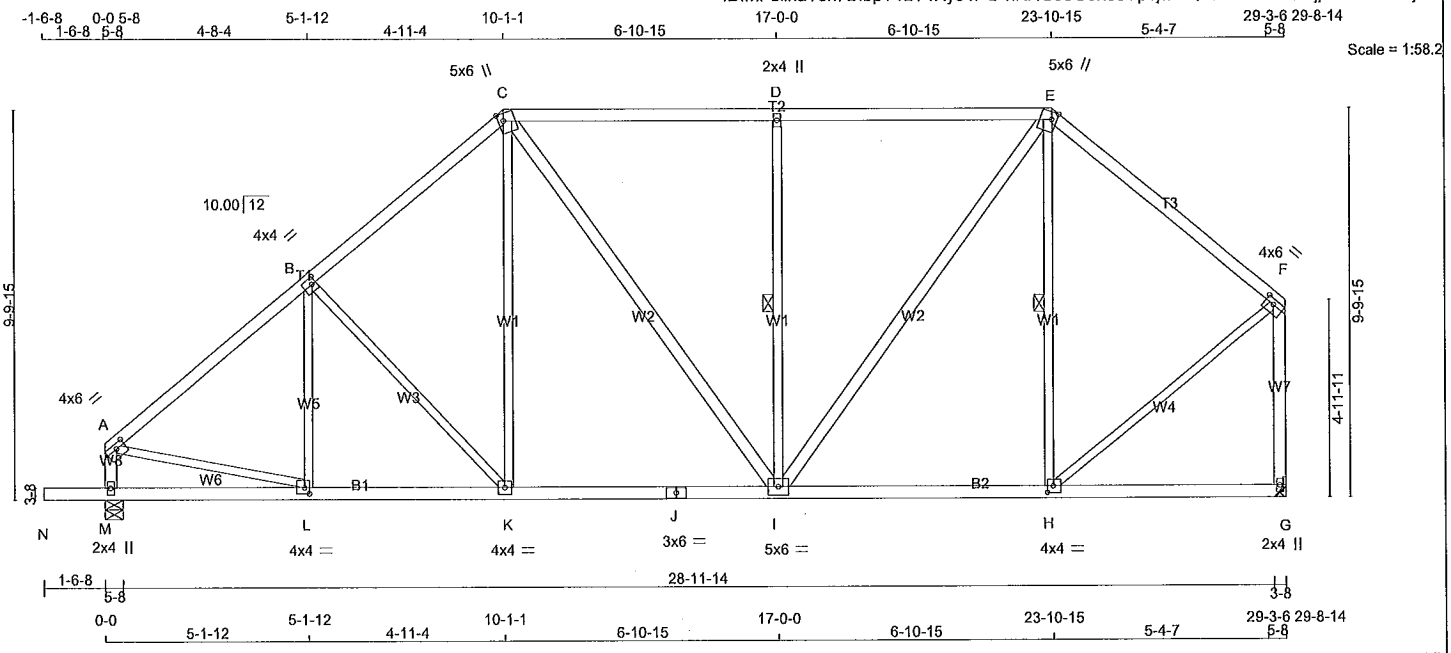
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H2	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:40:29 2018 Page 1

ID:MF8IiriaTok7DlbpT4B71Xys4PG-hKHGJeG52seVpJjwDQUMAUOPZ4EfjjX0ITKFAEzjxiG



TOTAL WEIGHT = 5 X 150 = 751 lb

[M][F]

LUMBER

N. L. G. A. RULES

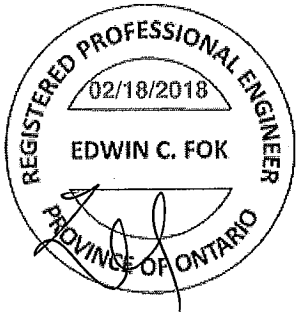
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - I	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW-t	MT20	4.0	6.0	1.50	2.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	1628	0	1628	0	0	5-8	2-7
G	1494	0	1494	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-10	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1164	689 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0
G	1070	624 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-I, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1570 / 0	-78.0	-78.0	0.30 (1)	4.97	L- B	-163 / 5	0.08 (1)
B- C	-1408 / 0	-78.0	-78.0	0.29 (1)	5.20	B- K	-253 / 0	0.27 (1)
C- D	-1215 / 0	-85.5	-85.5	0.56 (1)	2.00	K- C	0 / 297	0.07 (1)
D- E	-1215 / 0	-85.5	-85.5	0.56 (1)	2.00	C- I	0 / 267	0.04 (1)
E- F	-986 / 0	-78.0	-78.0	0.36 (1)	5.82	I- D	-728 / 0	0.47 (1)
M- A	-1438 / 0	0.0	0.0	0.15 (1)	6.81	I- E	0 / 789	0.13 (1)
G- F	-1452 / 0	0.0	0.0	0.65 (1)	6.79	H- E	-460 / 0	0.30 (1)
						A- L	0 / 1256	0.28 (1)
N- M	0 / 0	-96.5	-96.5	0.16 (1)	10.00	H- F	0 / 960	0.22 (1)
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00			
L- K	0 / 1229	-18.5	-18.5	0.27 (1)	10.00			
K- J	0 / 1059	-18.5	-18.5	0.28 (4)	10.00			
J- I	0 / 1059	-18.5	-18.5	0.28 (4)	10.00			
I- H	0 / 751	-18.5	-18.5	0.26 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.18 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.28/1.00 (I-K:4) , WB=0.47/1.00 (D-I:1) , SSI=0.29/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

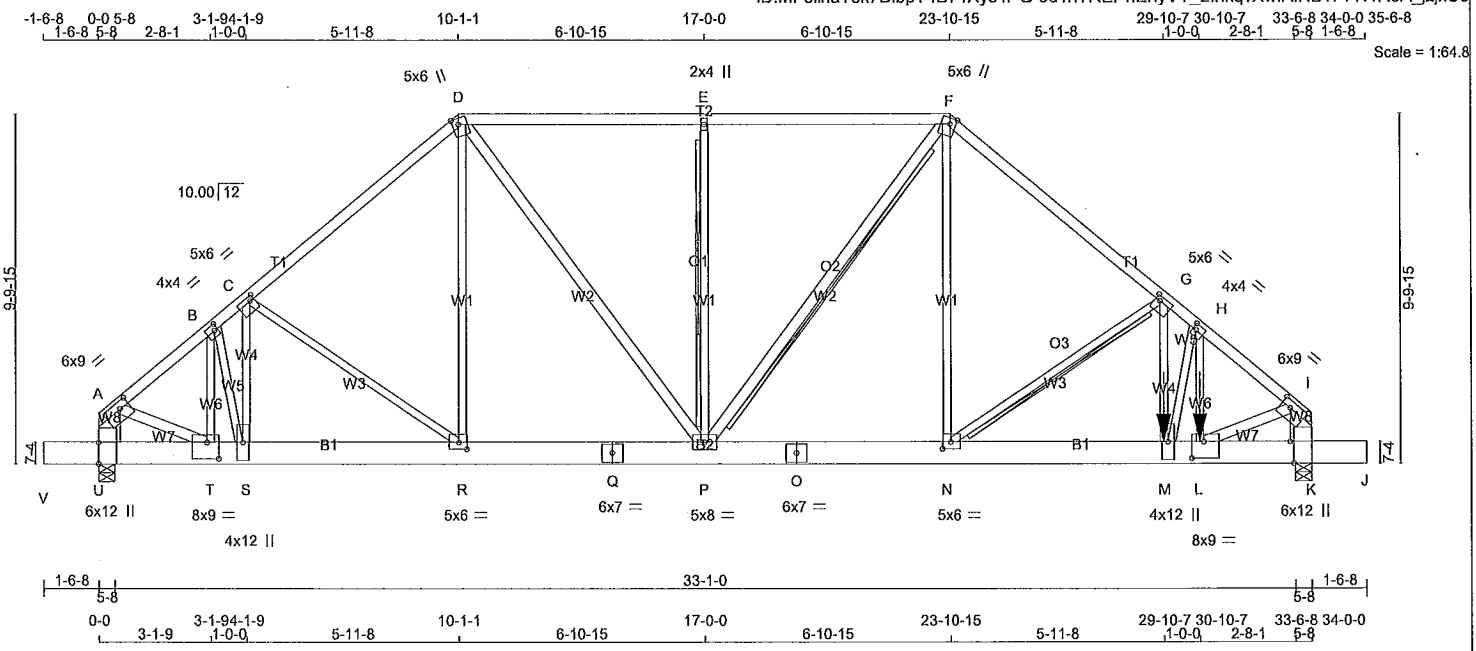
JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

A-18023663

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H2A	1	3	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8iiriaTok7DlbpT4B71Xys4PG-9d1nTREPhEnyVY_2IhkqvXwIAlNB?PFN1RoA_jzjxU6



TOTAL WEIGHT = 3 X 220 = 660 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
V - Q	2x8	DRY	No.2	SPF
Q - O	2x8	DRY	No.2	SPF
O - J	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

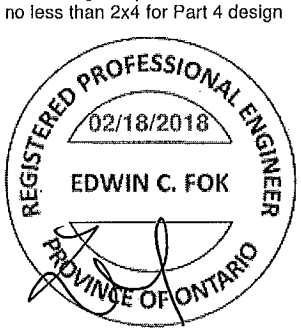
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
U- A	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- Q	2	12	TOP
Q- O	2	12	TOP
O- J	3	4	SIDE(1922.8)
WEBS : (0.122"X3") SPIRAL NAILS			
G- M	1	6	SIDE(269.9)
H- L	1	6	SIDE(193.7)
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	3225	0	3225	0	0	5-8	1-8
K	13797	0	13797	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2279	1504 / 0	0 / 0	0 / 0	0 / 0	776 / 0	0 / 0
K	9638	6995 / 0	0 / 0	0 / 0	0 / 0	2643 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-P, F-P, G-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3362 / 0	-78.0	-78.0	0.05 (1)	5.94	T- B	-619 / 0	0.05 (1)
B- C	-3421 / 0	-78.0	-78.0	0.14 (1)	5.79	B- S	0 / 574	0.04 (1)
C- D	-3684 / 0	-78.0	-78.0	0.19 (1)	5.63	S- C	-654 / 0	0.07 (1)
D- E	-4127 / 0	-85.5	-85.5	0.30 (1)	2.00	C- R	0 / 71	0.01 (1)
E- F	-4127 / 0	-85.5	-85.5	0.30 (1)	2.00	R- D	0 / 148	0.02 (4)
F- G	-6308 / 0	-78.0	-78.0	0.33 (1)	4.49	D- P	0 / 2228	0.12 (1)
G- H	-13544 / 0	-78.0	-78.0	0.36 (1)	3.03	P- E	-707 / 0	0.13 (1)
H- I	-14293 / 0	-78.0	-78.0	0.44 (1)	2.93	P- F	-1235 / 0	0.22 (1)
U- A	-3036 / 0	0.0	0.0	0.05 (1)	7.81	N- F	0 / 4606	0.35 (1)
K- I	-12578 / 0	0.0	0.0	0.21 (1)	5.94	N- G	-7289 / 0	0.83 (1)
						M- G	0 / 8201	0.62 (1)
V- U	0 / 0	-96.5	-96.5	0.02 (1)	10.00	M- H	-771 / 0	0.06 (1)
U- T	0 / 0	-18.5	-18.5	0.01 (1)	10.00	L- H	0 / 1499	0.11 (1)
T- S	0 / 2589	-18.5	-18.5	0.11 (1)	10.00	A- T	0 / 2700	0.14 (1)
S- R	0 / 2745	-18.5	-18.5	0.11 (1)	10.00	L- I	0 / 11403	0.61 (1)
R- Q	0 / 2804	-18.5	-18.5	0.11 (1)	10.00			
Q- P	0 / 2804	-18.5	-18.5	0.11 (1)	10.00			
P- O	0 / 4861	-18.5	-18.5	0.19 (1)	10.00			
O- N	0 / 4861	-18.5	-18.5	0.19 (1)	10.00			
N- M	0 / 10721	-18.5	-18.5	0.49 (1)	10.00			
M- L	0 / 10931	-18.5	-18.5	0.63 (1)	10.00			
L- K	0 / 0	-308.9	-308.9	0.27 (1)	10.00			
K- J	0 / 0	-96.5	-96.5	0.02 (1)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	21.0 PSF
		DL	=	6.0 PSF
BOT	CH.	LL	=	0.0 PSF
		DL	=	7.4 PSF
TOTAL	LOAD		=	34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-10-2
START SPAN CARRIED = 14-0-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 14-0-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.44/1.00 (H-I:1) , BC=0.63/1.00 (L-M:1) , WB=0.83/1.00 (G-N:1) , SSI=0.19/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

A-18023664

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H2A	1	3	TRUSS DESC.	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

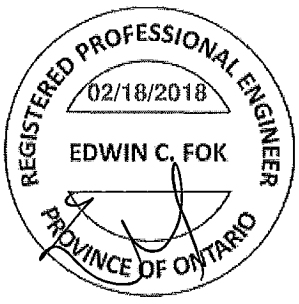
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.25	3.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TMWW-t	MT20	5.0	6.0	1.50	1.50
D	TTWW+m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	5.0	6.0	1.50	1.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.25
I	TMVW-t	MT20	6.0	9.0	2.25	3.25
K	BMV1+t	MT20	6.0	12.0	Edge	1.25
L	BMWW-t	MT20	8.0	9.0	5.50	4.00
M	BMWW+t	MT20	4.0	12.0		
N	BMWW-t	MT20	5.0	6.0	2.25	2.75
O	BS-t	MT20	6.0	7.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	6.0	7.0		
R	BMWW-t	MT20	5.0	6.0	2.25	2.75
S	BMWW+t	MT20	4.0	12.0		
T	BMWW-t	MT20	8.0	9.0	5.50	4.00
U	BMV1+t	MT20	6.0	12.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6742.7 lbs FACTORED DOWN AT 29-10-6, AND 5688.9 lbs FACTORED DOWN AT 30-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	30-10-6	-5689	-5689	---	FRONT	VERT	TOTAL
M	29-10-6	-6743	-6743	---	FRONT	VERT	TOTAL

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.80 (I) (INPUT = 1.00)

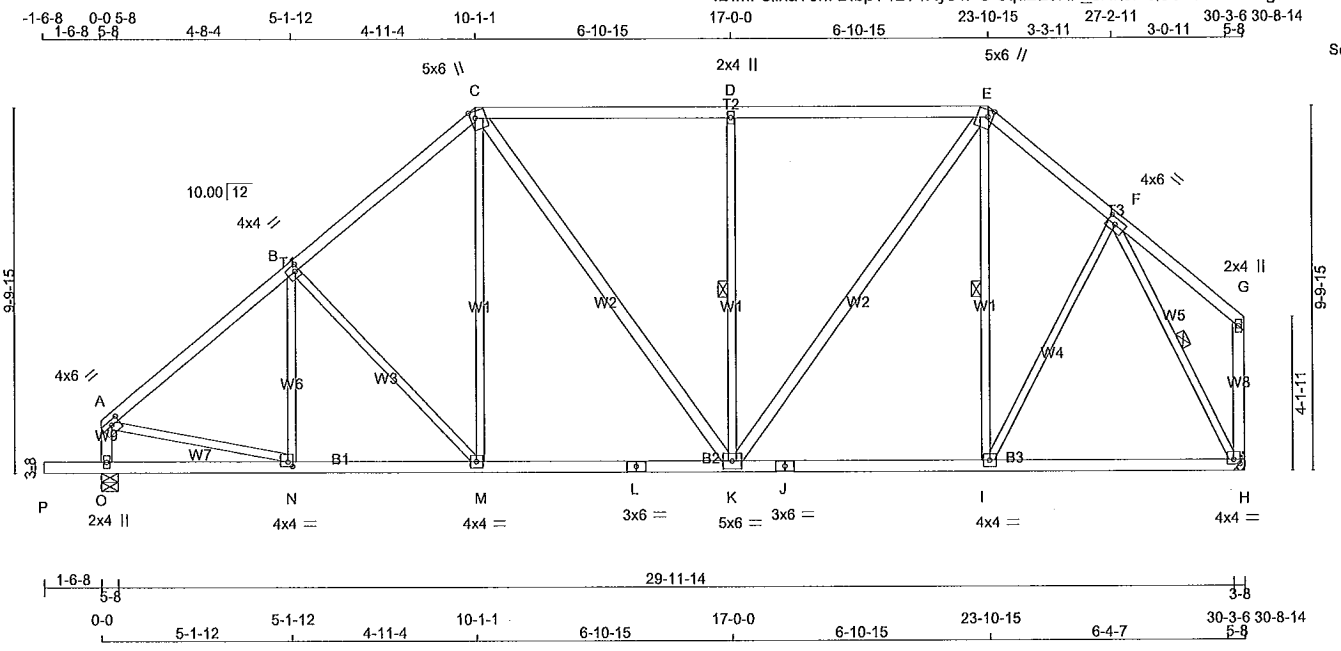
A-180236664(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H3	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:40:42 2018 Page 1

ID:MF8iriaTok7DlbpT4B71Xys4PG-oqaB25RF_sHftJDQUeDPCDRffKgJGbBxh?_R8zzjxi3



TOTAL WEIGHT = 7 X 158 = 1103 lb

[M][F]

LUMBER

N. L. G. A. RULES

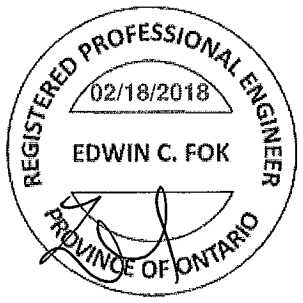
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - K	2x4	DRY	No.2	SPF
K - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table Is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	2.75
G	TMV+p	MT20	2.0	4.0		
H	BMVW1-t	MT20	4.0	4.0	1.50	2.00
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1678	0	1678	0	0	5-8	2-8
H	1540	0	1540	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1200	710 / 0	0 / 0	0 / 0	0 / 0	490 / 0	0 / 0
H	1103	645 / 0	0 / 0	0 / 0	0 / 0	458 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.90 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-K, E-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1632 / 0	-78.0	-78.0	0.31 (1)	4.90
B- C	-1477 / 0	-78.0	-78.0	0.29 (1)	5.10
C- D	-1304 / 0	-85.5	-85.5	0.56 (1)	2.00
D- E	-1304 / 0	-85.5	-85.5	0.56 (1)	2.00
E- F	-1170 / 0	-78.0	-78.0	0.12 (1)	5.77
F- G	0 / 19	-78.0	-78.0	0.14 (1)	10.00
O- A	-1488 / 0	0.0	0.0	0.15 (1)	6.72
H- G	-105 / 0	0.0	0.0	0.03 (1)	7.81
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00
O- N	0 / 0	-18.5	-18.5	0.10 (4)	10.00
N- M	0 / 1277	-18.5	-18.5	0.28 (1)	10.00
M- L	0 / 1112	-18.5	-18.5	0.28 (4)	10.00
L- K	0 / 1112	-18.5	-18.5	0.28 (4)	10.00
K- J	0 / 879	-18.5	-18.5	0.31 (4)	10.00
J- I	0 / 879	-18.5	-18.5	0.31 (4)	10.00
I- H	0 / 715	-18.5	-18.5	0.28 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-173 / 0	0.09 (1)
B- M	-245 / 0	0.26 (1)
M- C	0 / 292	0.07 (1)
C- K	0 / 327	0.05 (1)
K- D	-727 / 0	0.47 (1)
K- E	0 / 724	0.12 (1)
I- E	-175 / 0	0.11 (1)
I- F	0 / 359	0.08 (1)
A- N	0 / 1305	0.29 (1)
F- H	-1558 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.56/1.00 (D-E:1) , BC=0.31/1.00 (I-K:4) , WB=0.57/1.00 (F-H:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (H) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)

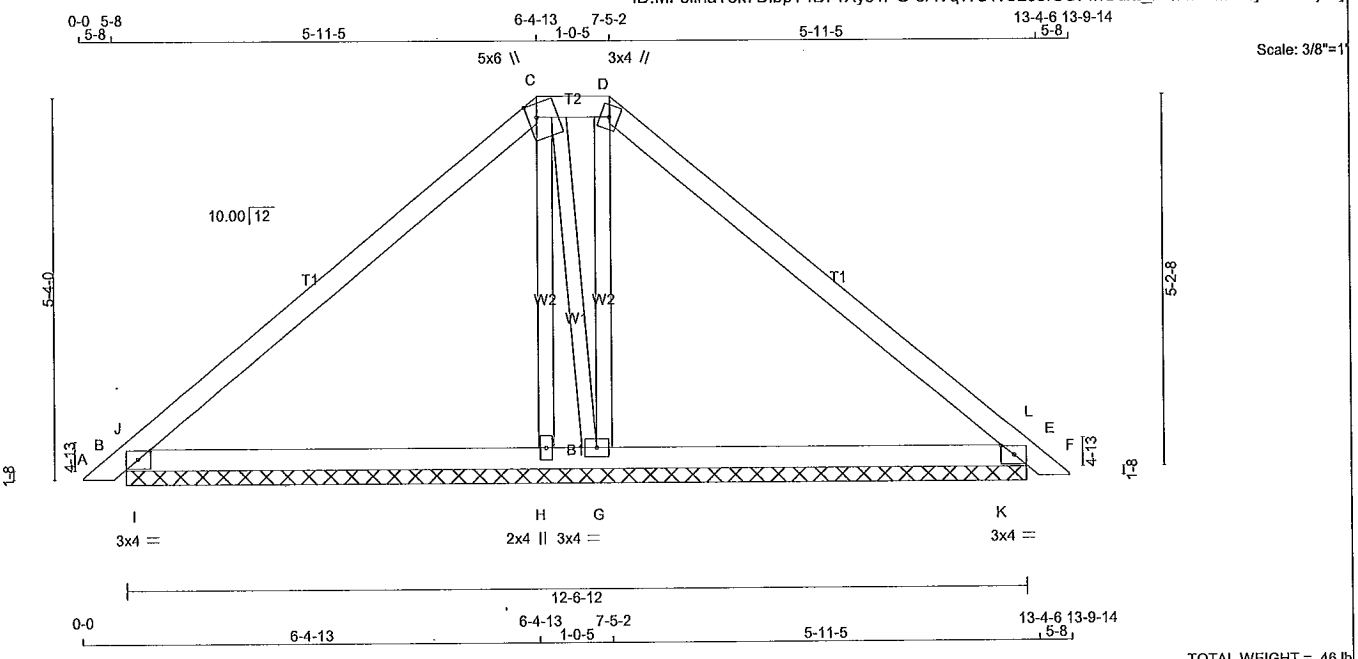
A-18023665

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H4P	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:40:49 2018 Page 1

ID:MF8iiriaTok7DlbpT4B71Xys4PG-5AVqWUWeL09fCOFmOdr2_iDvX83MPtNylbBlt3zjxhy



TOTAL WEIGHT = 46 lb

[M]

LUMBER

N. L. G. A. RULES

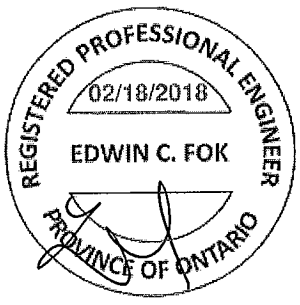
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is In Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+m	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	453	0	453	0	0	12-6-12	9-0
E	448	0	448	0	0	12-6-12	9-0
H	158	0	158	0	0	12-6-12	9-0
G	233	0	233	0	0	12-6-12	9-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	321	210 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
E	317	207 / 0	0 / 0	0 / 0	0 / 0	110 / 0	0 / 0
H	116	53 / 0	0 / 0	0 / 0	0 / 0	63 / 0	0 / 0
G	167	101 / 0	0 / 0	0 / 0	0 / 0	66 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	H- C	-41 / 0	0.02 (1)
B- J	-61 / 137	-78.0	-78.0	0.21 (1)	6.25	C- G	-36 / 0	0.01 (1)
J- C	-257 / 0	-78.0	-78.0	0.33 (1)	6.25	G- D	-81 / 0	0.03 (1)
C- D	-178 / 0	-78.0	-78.0	0.01 (1)	6.25	I- J	-758 / 0	0.00 (1)
D- L	-250 / 0	-78.0	-78.0	0.33 (1)	6.25	K- L	-759 / 0	0.00 (1)
L- E	-60 / 145	-78.0	-78.0	0.21 (1)	6.25			
E- F	0 / 12	-78.0	-78.0	0.02 (1)	10.00			
B- I	0 / 185	-18.5	-18.5	0.28 (1)	10.00			
I- H	0 / 185	-18.5	-18.5	0.28 (1)	10.00			
H- G	0 / 184	-18.5	-18.5	0.22 (1)	10.00			
G- K	0 / 179	-18.5	-18.5	0.28 (1)	10.00			
K- E	0 / 179	-18.5	-18.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1) , BC=0.28/1.00 (B-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.59/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

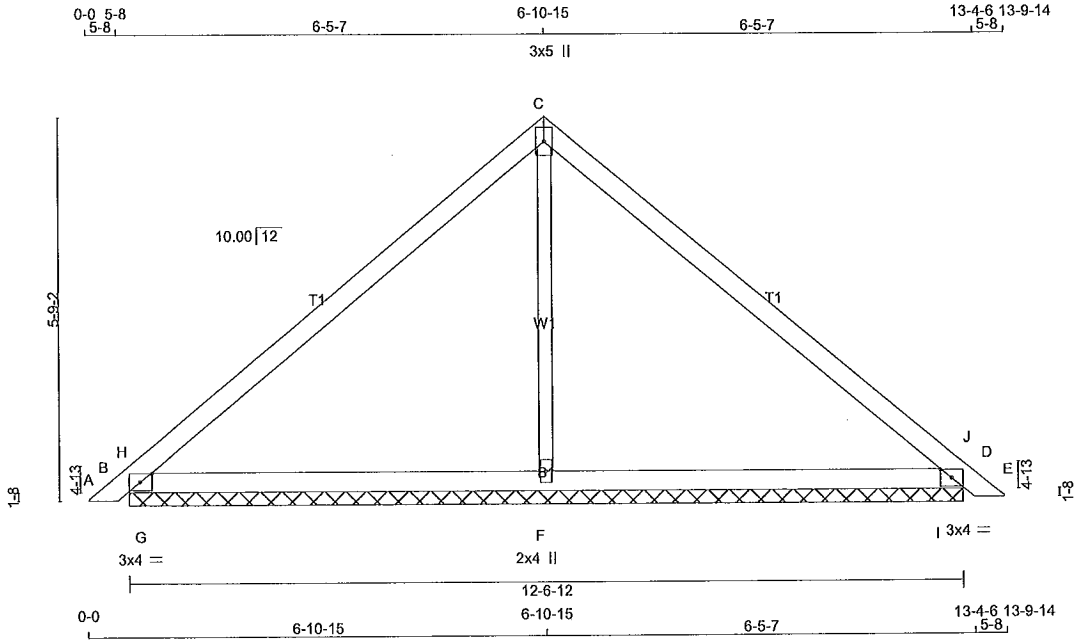
A-18023666

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H5P	12	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8IiriaTok7DlbpT4B71Xys4PG-wKs6mXbPwsvpxJjwltxSEzTw5Z5npbyrhWed4jzjxhs

Scale = 1:34.7



TOTAL WEIGHT = 12 X 39 = 466 lb [M]

LUMBER

N. L. G. A. RULES

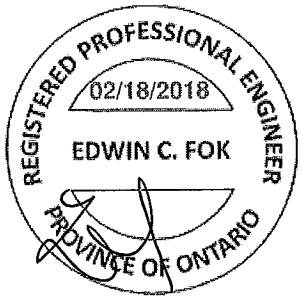
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	3.0	5.0		
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	491	0	491	0	0	12-6-12	1-5-11
D	491	0	491	0	0	12-6-12	1-5-11
F	312	0	312	0	0	12-6-12	1-5-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	346	231 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
D	346	231 / 0	0 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	228	109 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.02 (1)	10.00	F- C	-57 / 9	0.03 (1)
B- H	-40 / 219	-78.0	-78.0	0.28 (1)	6.25	G- H	-942 / 0	0.00 (1)
H- C	-296 / 0	-78.0	-78.0	0.39 (1)	6.25	I- J	-941 / 0	0.00 (1)
C- J	-296 / 0	-78.0	-78.0	0.39 (1)	6.25			
J- D	-40 / 219	-78.0	-78.0	0.28 (1)	6.25			
D- E	0 / 12	-78.0	-78.0	0.02 (1)	10.00			
B- G	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
G- F	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
F- I	0 / 213	-18.5	-18.5	0.34 (1)	10.00			
I- D	0 / 213	-18.5	-18.5	0.34 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.39/1.00 (C-H:1) , BC=0.34/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.73/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

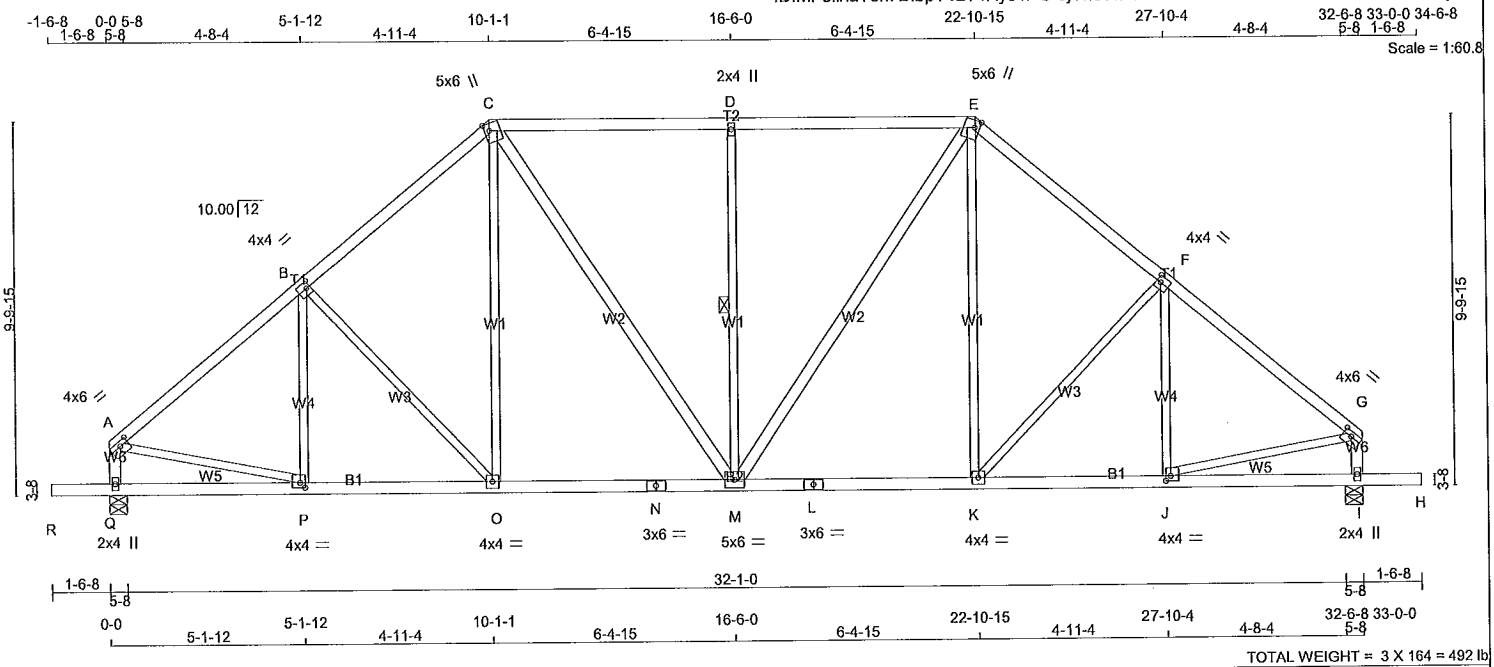
JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.12 (D) (INPUT = 1.00)

A-18023667

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H6	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:41:23 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-6jYhe6wzKSRrldsJOmS1NtehOsfAN9Uz7tQ635zjxhQ



TOTAL WEIGHT = 3 X 164 = 492 lb

{M}{F}

LUMBER

N. L. G. A. RULES

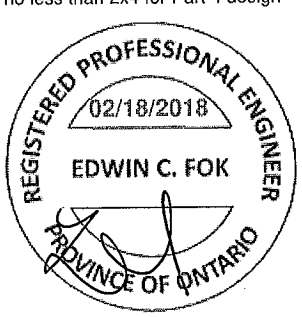
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	4.0	2.00	1.25
G	TMVW-t	MT20	4.0	6.0	1.50	2.75
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-t	MT20	4.0	4.0	1.50	1.50
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
Q	1789	0	1789	0	0	5-8	2-10
I	1789	0	1789	0	0	5-8	2-10

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1279	757 / 0	0 / 0	0 / 0	0 / 0	522 / 0	0 / 0
I	1279	757 / 0	0 / 0	0 / 0	0 / 0	522 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1771 / 0	-78.0	-78.0	0.32 (1)	4.73
B- C	-1629 / 0	-78.0	-78.0	0.30 (1)	4.90
C- D	-1495 / 0	-85.5	-85.5	0.49 (1)	2.00
D- E	-1495 / 0	-85.5	-85.5	0.49 (1)	2.00
E- F	-1629 / 0	-78.0	-78.0	0.30 (1)	4.90
F- G	-1771 / 0	-78.0	-78.0	0.32 (1)	4.73
Q- A	-1599 / 0	0.0	0.0	0.17 (1)	6.53
I- G	-1599 / 0	0.0	0.0	0.17 (1)	6.53
R- Q	0 / 0	-96.5	-96.5	0.16 (1)	10.00
Q- P	0 / 0	-18.5	-18.5	0.10 (4)	10.00
P- O	0 / 1383	-18.5	-18.5	0.29 (1)	10.00
O- N	0 / 1228	-18.5	-18.5	0.28 (4)	10.00
N- M	0 / 1228	-18.5	-18.5	0.28 (4)	10.00
M- L	0 / 1228	-18.5	-18.5	0.28 (4)	10.00
L- K	0 / 1228	-18.5	-18.5	0.28 (4)	10.00
K- J	0 / 1383	-18.5	-18.5	0.29 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.10 (4)	10.00
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-194 / 0	0.10 (1)
B- O	-230 / 0	0.25 (1)
O- C	0 / 273	0.06 (1)
C- M	0 / 478	0.08 (1)
M- D	-671 / 0	0.44 (1)
M- E	0 / 478	0.08 (1)
K- E	0 / 273	0.06 (1)
K- F	-230 / 0	0.25 (1)
J- F	-194 / 0	0.10 (1)
A- P	0 / 1413	0.32 (1)
J- G	0 / 1413	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.29/1.00 (J-K:1) , WB=0.44/1.00 (D-M:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

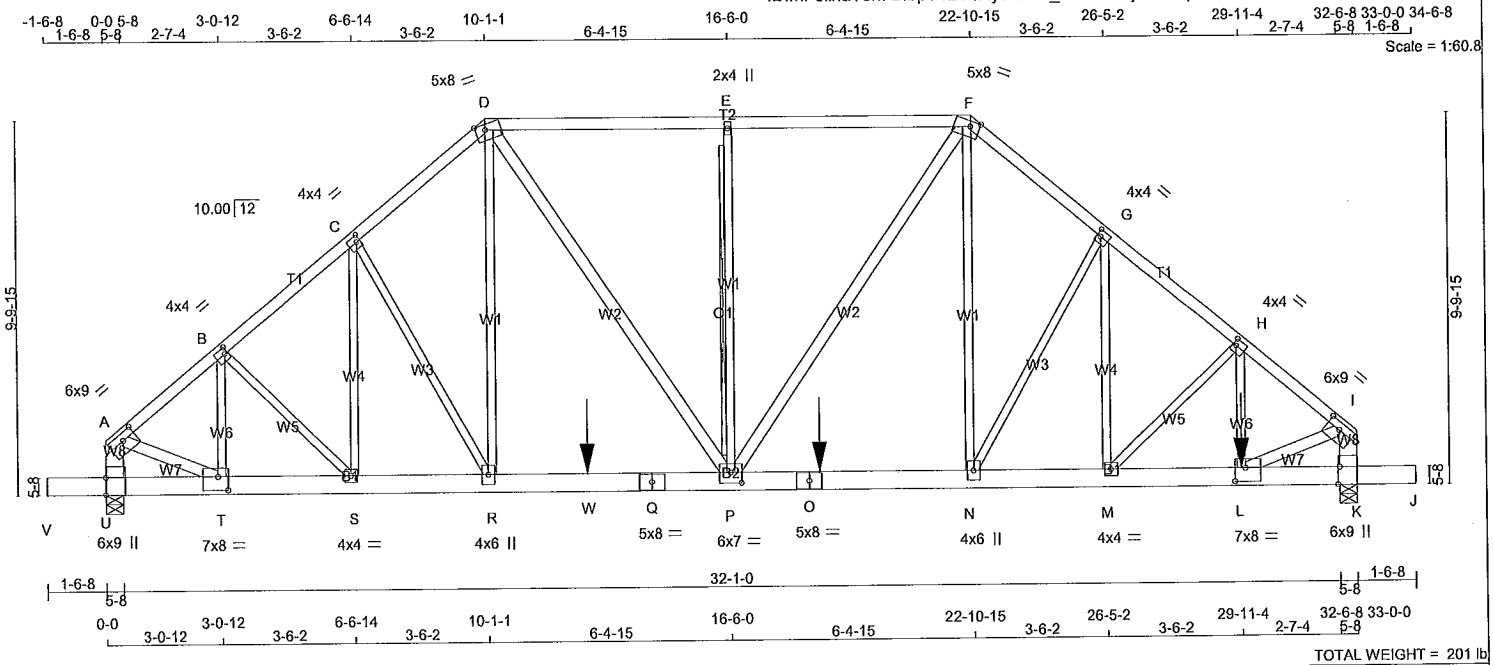
JSI GRIP= 0.85 (P) (INPUT = 0.90)
JSI METAL= 0.52 (P) (INPUT = 1.00)

A-18023668

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H6A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:03:22 2018 Page 1
ID:MF8liriaTok7DlbpT4B71Xys4PG-_TlKT7uKijdh1JBp9GcLK0m2DYhGvbl6QbQOs8zjxMp



LUMBER

N. L. G. A. RULES

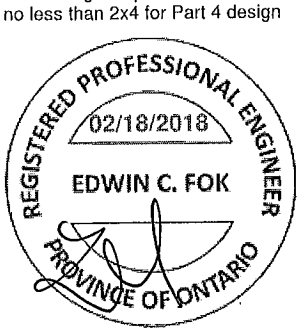
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	1650F 1.5E	SPF
Q - O	2x6	DRY	1650F 1.5E	SPF
O - J	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.25	4.25
B	TMWW-t	MT20	4.0	4.0	2.00	1.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.25
H	TMWW-t	MT20	4.0	4.0	2.00	1.00
I	TMVW-t	MT20	6.0	9.0	2.25	4.25
K	BMV1+t	MT20	6.0	9.0	Edge	0.50
L	BMWW-t	MT20	7.0	8.0	4.25	3.25
M	BMWW-t	MT20	4.0	4.0		
N	BMWW+t	MT20	4.0	6.0		
O	BS-t	MT20	5.0	8.0		
P	BMWWW-t	MT20	6.0	7.0	3.25	3.50
Q	BS-t	MT20	5.0	8.0		
R	BMWW+t	MT20	4.0	6.0		
S	BMWW-t	MT20	4.0	4.0		
T	BMWW-t	MT20	7.0	8.0	4.25	3.25
U	BMV1+t	MT20	6.0	9.0	Edge	

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	3062	0	3062	0	0	5-8	4-3
K	4622	0	4622	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2159	1453 / 0	0 / 0	0 / 0	0 / 0	706 / 0	0 / 0
K	3243	2275 / 0	0 / 0	0 / 0	0 / 0	968 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3109 / 0	-78.0	-78.0	0.25 (1)	3.74
B- C	-3402 / 0	-78.0	-78.0	0.32 (1)	3.53
C- D	-3505 / 0	-78.0	-78.0	0.33 (1)	3.48
D- E	-3321 / 0	-78.0	-78.0	0.70 (1)	3.65
E- F	-3321 / 0	-78.0	-78.0	0.70 (1)	3.65
F- G	-3505 / 0	-78.0	-78.0	0.35 (1)	3.47
G- H	-3961 / 0	-78.0	-78.0	0.40 (1)	3.22
H- I	-4781 / 0	-78.0	-78.0	0.45 (1)	2.87
U- A	-2883 / 0	0.0	0.0	0.21 (1)	6.13
K- I	-4376 / 0	0.0	0.0	0.32 (1)	5.10
V- U	0 / 0	-96.5	-96.5	0.06 (1)	10.00
U- T	0 / 0	-18.5	-18.5	0.02 (4)	10.00
T- S	0 / 2399	-18.5	-18.5	0.30 (1)	10.00
S- R	0 / 2616	-18.5	-18.5	0.59 (1)	10.00
R- W	0 / 2684	-18.5	-18.5	0.83 (1)	10.00
W- Q	0 / 2684	-18.5	-18.5	0.83 (1)	10.00
Q- P	0 / 2684	-18.5	-18.5	0.83 (1)	10.00
P- O	0 / 2682	-18.5	-18.5	0.69 (1)	10.00
O- N	0 / 2682	-18.5	-18.5	0.69 (1)	10.00
N- M	0 / 3050	-18.5	-18.5	0.36 (1)	10.00
M- L	0 / 3679	-18.5	-18.5	0.41 (1)	10.00
L- K	0 / 0	-18.5	-18.5	0.11 (1)	10.00
K- J	0 / 0	-96.5	-96.5	0.06 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-673 / 0	0.16 (1)
B- S	0 / 308	0.08 (1)
S- C	-469 / 0	0.36 (1)
C- R	-1 / 117	0.03 (1)
R- D	0 / 992	0.25 (1)
N- F	0 / 990	0.25 (1)
N- G	-777 / 0	0.84 (1)
M- G	0 / 664	0.16 (1)
M- H	-896 / 0	0.37 (1)
L- H	0 / 905	0.22 (1)
A- T	0 / 2525	0.45 (1)
L- I	0 / 3872	0.68 (1)
P- E	-596 / 0	0.43 (1)
P- F	0 / 1136	0.20 (1)
D- P	0 / 1133	0.20 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	29-11-4	-2159	-2159	---	FRONT	VERT	TOTAL
O	18-10-6	-761	-761	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.10")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.10")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.70/1.00 (E-F:1) , BC=0.83/1.00 (P-R:1) , WB=0.84/1.00 (G-N:1) , SSI=0.53/1.00 (P-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

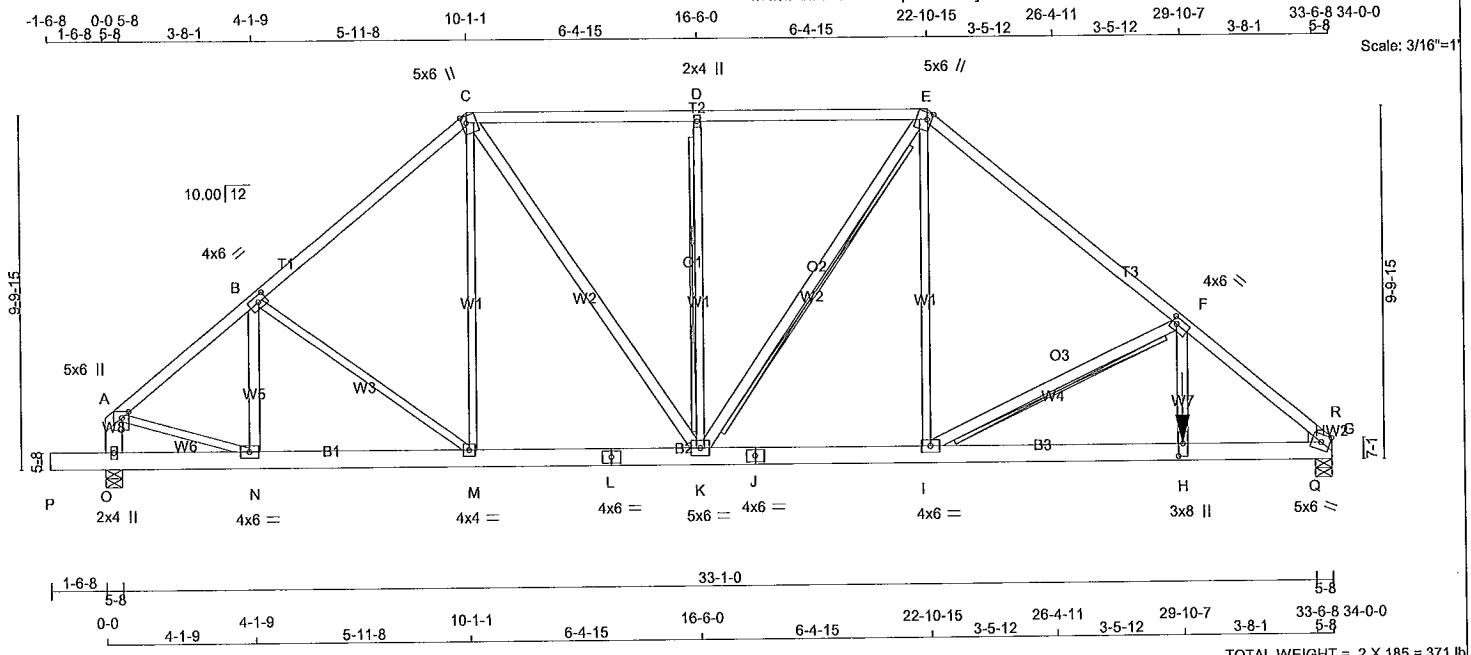
A-18023669

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H6B	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:05:45 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-IzuOU0d11feOhIdI1ogtdABXsBaswG39JKZSNMzjxKa



TOTAL WEIGHT = 2 X 185 = 371 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
O - A	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
B - M	2x3	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
K - D	2x3	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
A - N	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

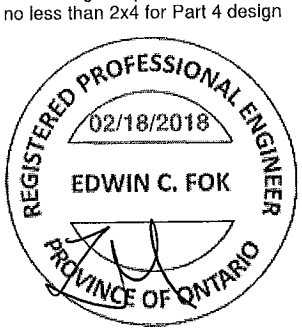
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- E	1	12	TOP
E- G	1	12	TOP
O- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P- L	2	12	TOP
L- J	2	12	TOP
J- G	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
F- H	1	2	SIDE(758.7)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
O	2277	0	2277	0	0	5-8	1-8	
G	4858	0	4858	0	0	5-8	2-10	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1620	1009 / 0	0 / 0	0 / 0	0 / 0	610 / 0	0 / 0
G	3410	2384 / 0	0 / 0	0 / 0	0 / 0	1026 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, G

BRACING
FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-K, F-I
2x6 DRY SPF No.2 T-BRACE AT E-K

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2363 / 0	-78.0	-78.0	0.23 (1)	5.58	
B- C	-2326 / 0	-78.0	-78.0	0.27 (1)	5.62	
C- D	-2356 / 0	-85.5	-85.5	0.38 (1)	2.00	
D- E	-2356 / 0	-85.5	-85.5	0.38 (1)	2.00	
E- F	-3180 / 0	-78.0	-78.0	0.46 (1)	4.86	
F- R	-6865 / 0	-78.0	-78.0	0.44 (1)	3.40	
R- G	-8067 / 0	-78.0	-78.0	0.26 (1)	3.27	
O- A	-2085 / 0	0.0	0.0	0.08 (1)	7.81	
P- O	0 / 0	-96.5	-96.5	0.04 (1)	10.00	
O- N	0 / 0	-18.5	-18.5	0.02 (1)	10.00	
N- M	0 / 1846	-18.5	-18.5	0.15 (1)	10.00	
M- L	0 / 1765	-18.5	-18.5	0.14 (1)	10.00	
L- K	0 / 1765	-18.5	-18.5	0.14 (1)	10.00	
K- J	0 / 2438	-18.5	-18.5	0.22 (1)	10.00	
J- I	0 / 2438	-18.5	-18.5	0.22 (1)	10.00	
I- H	0 / 5300	-18.5	-18.5	0.49 (1)	10.00	
H- Q	0 / 5300	-18.5	-18.5	0.57 (1)	10.00	
Q- G	0 / 5300	-18.5	-18.5	0.57 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- B	-375 / 0	0.05 (1)
B- M	-104 / 0	0.06 (1)
M- C	0 / 200	0.03 (4)
C- K	0 / 1053	0.09 (1)
K- D	-665 / 0	0.32 (1)
K- E	-144 / 38	0.03 (1)
I- E	0 / 1697	0.21 (1)
H- F	0 / 3359	0.30 (1)
A- N	0 / 1899	0.24 (1)
I- F	-3236 / 0	0.84 (1)
Q- R	0 / 1559	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	29-10-6	-3610	-3610	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.46/1.00 (E-F:1) , BC=0.57/1.00 (H-Q:1) , WB=0.84/1.00 (F-I:1) , SSI=0.42/1.00 (G-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023670

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H6B	1	2	TRUSS DESC.	

PLATES (table is in inches)

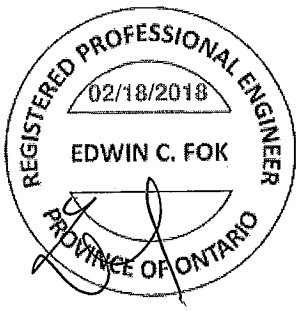
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	6.0	2.00	2.75
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMWW-t	MT20	4.0	6.0	2.00	1.75
G	TMBH1-m	MT20	5.0	6.0		Edge
H	BMW+w	MT20	3.0	8.0	4.25	1.50
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BS-t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	4.0	6.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3610.3 lbs FACTORED DOWN AT 29-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

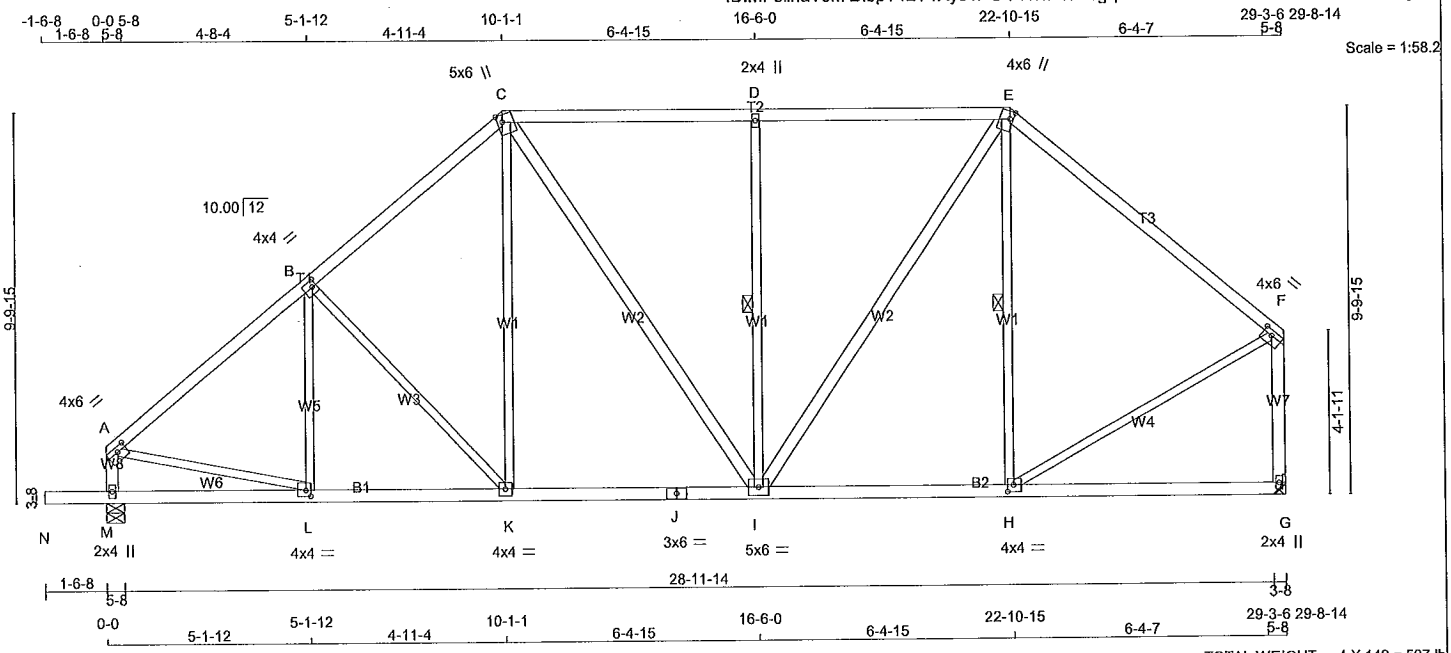
JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.86 (G) (INPUT = 1.00)

A-180236701

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H6S	4	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8iiriaTok7DlbpT4B71Xys4PG-PK?mhJD1gqTRAJhwootfLxdnYXsh?0OVecENwizjxh2



TOTAL WEIGHT = 4 X 149 = 597 lb

[M][F]

LUMBER

N. L. G. A. RULES

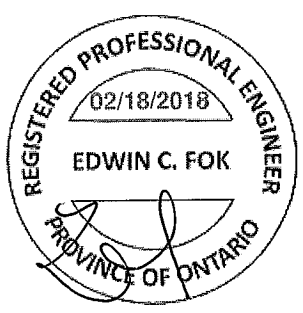
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	2100F 1.8E	SPF
M - A	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - I	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	4.0	2.00	1.25
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	4.0	6.0	2.25	1.00
F	TMVW-t	MT20	4.0	6.0	1.50	2.75
G	BMV1+p	MT20	2.0	4.0		
H	BMWW-t	MT20	4.0	4.0	2.00	1.75
I	BMWWW-t	MT20	5.0	6.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMWW-t	MT20	4.0	4.0	1.75	1.50
M	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	1626	0	1626	0	0	5-8	2-7
G	1488	0	1488	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-10	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1163	689 / 0	0 / 0	0 / 0	0 / 0	474 / 0	0 / 0
G	1065	624 / 0	0 / 0	0 / 0	0 / 0	441 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

FOR SECTION C-E, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-I, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1568 / 0	-78.0	-78.0	0.30 (1)	4.97
B- C	-1405 / 0	-78.0	-78.0	0.29 (1)	5.20
C- D	-1213 / 0	-85.5	-85.5	0.48 (1)	2.00
D- E	-1213 / 0	-85.5	-85.5	0.48 (1)	2.00
E- F	-1101 / 0	-78.0	-78.0	0.33 (1)	6.25
M- A	-1436 / 0	0.0	0.0	0.15 (1)	6.81
G- F	-1436 / 0	0.0	0.0	0.41 (1)	6.81
N- M	0 / 0	-96.5	-96.5	0.16 (1)	10.00
M- L	0 / 0	-18.5	-18.5	0.10 (4)	10.00
L- K	0 / 1227	-18.5	-18.5	0.27 (1)	10.00
K- J	0 / 1056	-18.5	-18.5	0.24 (4)	10.00
J- I	0 / 1056	-18.5	-18.5	0.24 (4)	10.00
I- H	0 / 841	-18.5	-18.5	0.30 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.21 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-161 / 6	0.08 (1)
B- K	-255 / 0	0.27 (1)
K- C	0 / 294	0.07 (1)
C- I	0 / 280	0.05 (1)
I- D	-673 / 0	0.44 (1)
I- E	0 / 665	0.11 (1)
H- E	-322 / 0	0.21 (1)
A- L	0 / 1254	0.28 (1)
H- F	0 / 964	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.48/1.00 (D-E:1) , BC=0.30/1.00 (H-I:4) , WB=0.44/1.00 (D-I:1) , SSI=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

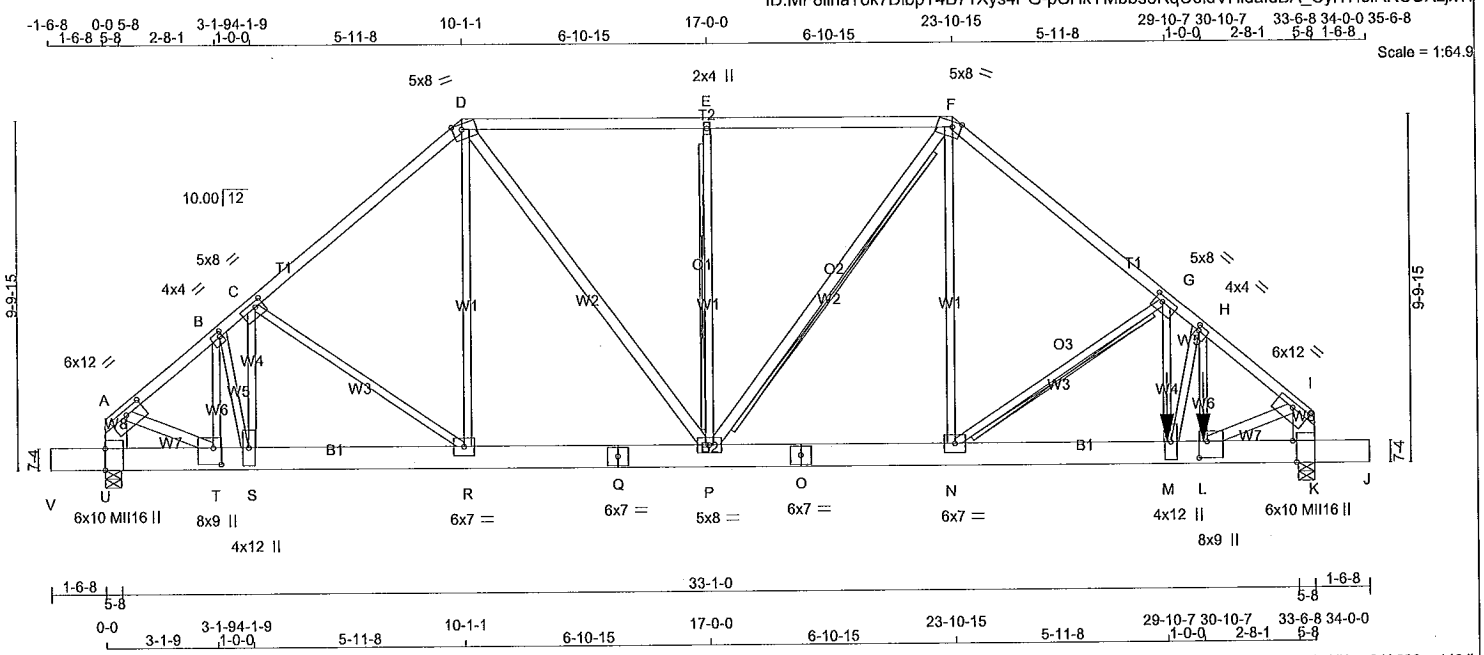
JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.46 (L) (INPUT = 1.00)

A-18023671

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H7A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:56:04 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-pCHkYMbbs8RqU0idVHldafdBA_CyITHeiRKCUXzjxTf



TOTAL WEIGHT = 2 X 220 = 440 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
U - A	2x8	DRY	No.2	SPF
K - I	2x8	DRY	No.2	SPF
V - Q	2x8	DRY	No.2	SPF
Q - O	2x8	DRY	No.2	SPF
O - J	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - P	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
A - T	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

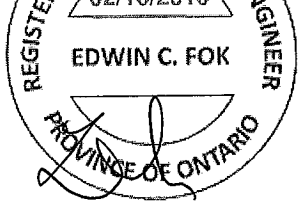
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- F	1	12	TOP
F- I	1	12	TOP
U- A	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- Q	2	12	TOP
Q- O	2	12	TOP
O- J	2	3	SIDE(915.6)
WEBS : (0.122"X3") SPIRAL NAILS			
G- M	1	3	SIDE(35.6)
H- L	1	2	SIDE(713.6)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2849	0	2849	0	0	5-8	1-8
K	11076	0	11076	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	2018	1306 / 0	0 / 0	0 / 0	0 / 0	713 / 0	0 / 0
K	7749	5563 / 0	0 / 0	0 / 0	0 / 0	2186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) K (BASED ON SUPPORT DEPTH = 1-8)

BRACING
FOR SECTION D-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-P
2x6 DRY SPF No.2 T-BRACE AT F-P, G-N

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2931 / 0	-78.0	-78.0	0.09 (1)	5.30
B- C	-2965 / 0	-78.0	-78.0	0.23 (1)	5.08
C- D	-3153 / 0	-78.0	-78.0	0.33 (1)	4.95
D- E	-3447 / 0	-85.5	-85.5	0.51 (1)	2.00
E- F	-3447 / 0	-85.5	-85.5	0.51 (1)	2.00
F- G	-5059 / 0	-78.0	-78.0	0.55 (1)	3.95
G- H	-10392 / 0	-78.0	-78.0	0.58 (1)	2.47
H- I	-11210 / 0	-78.0	-78.0	0.68 (1)	2.39
U- A	-2662 / 0	0.0	0.0	0.07 (1)	7.81
K- I	-9890 / 0	0.0	0.0	0.27 (1)	5.48
V- U	0 / 0	-96.5	-96.5	0.03 (1)	10.00
U- T	0 / 0	-18.5	-18.5	0.01 (4)	10.00
T- S	0 / 2258	-18.5	-18.5	0.15 (1)	10.00
S- R	0 / 2395	-18.5	-18.5	0.16 (1)	10.00
R- Q	0 / 2397	-18.5	-18.5	0.15 (1)	10.00
Q- P	0 / 2397	-18.5	-18.5	0.15 (1)	10.00
P- O	0 / 3890	-18.5	-18.5	0.24 (1)	10.00
O- N	0 / 3890	-18.5	-18.5	0.24 (1)	10.00
N- M	0 / 8260	-18.5	-18.5	0.57 (1)	10.00
M- L	0 / 8575	-18.5	-18.5	0.89 (1)	10.00
L- K	0 / 0	-308.9	-308.9	0.43 (1)	10.00
K- J	0 / 0	-96.5	-96.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- B	-535 / 0	0.06 (1)
B- S	0 / 506	0.06 (1)
S- C	-565 / 0	0.09 (1)
C- R	-10 / 0	0.01 (4)
R- D	0 / 163	0.03 (4)
D- P	0 / 1767	0.16 (1)
P- E	-712 / 0	0.33 (1)
P- F	-746 / 0	0.16 (1)
N- F	0 / 3413	0.42 (1)
N- G	-5434 / 0	0.97 (1)
M- G	0 / 5950	0.74 (1)
M- H	-1159 / 0	0.15 (1)
L- H	0 / 1853	0.23 (1)
A- T	0 / 2355	0.21 (1)
L- I	0 / 8945	0.79 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-10-6
START SPAN CARRIED = 14-0-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 14-0-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.68/1.00 (H-I:1) , BC=0.89/1.00 (L-M:1) , WB=0.97/1.00 (G-N:1) , SSI=0.36/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023672

CONTINUED ON PAGE 2

PLATES (table is in Inches)

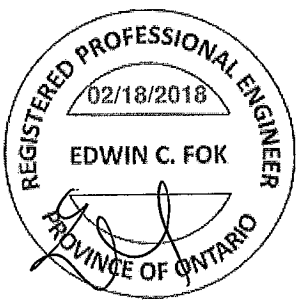
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	12.0	1.75	6.00
B	TMWW-t	MT20	4.0	4.0	1.75	1.00
C	TMWW-t	MT20	5.0	8.0	1.75	2.75
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	3.00
G	TMWW-t	MT20	5.0	8.0	1.75	2.75
H	TMWW-t	MT20	4.0	4.0	1.75	1.00
I	TMVW-t	MT20	6.0	12.0	1.75	6.00
K	BMV1+p	MII16	6.0	10.0	Edge	1.25
L	BMWW+t	MT20	8.0	9.0	5.50	2.75
M	BMWW+t	MT20	4.0	12.0		
N	BMWW-t	MT20	6.0	7.0		
O	BS-t	MT20	6.0	7.0		
P	BMWWW-t	MT20	5.0	8.0		
Q	BS-t	MT20	6.0	7.0		
R	BMWW-t	MT20	6.0	7.0		
S	BMWW+t	MT20	4.0	12.0		
T	BMWW+t	MT20	8.0	9.0	5.50	2.75
U	BMV1+p	MII16	6.0	10.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 5688.9 lbs FACTORED DOWN AT 30-10-6, AND 3646.3 lbs FACTORED DOWN AT 29-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	30-10-6	-5689	-5689	---	FRONT	VERT	TOTAL
M	29-10-6	-3646	-3646	---	FRONT	VERT	TOTAL

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

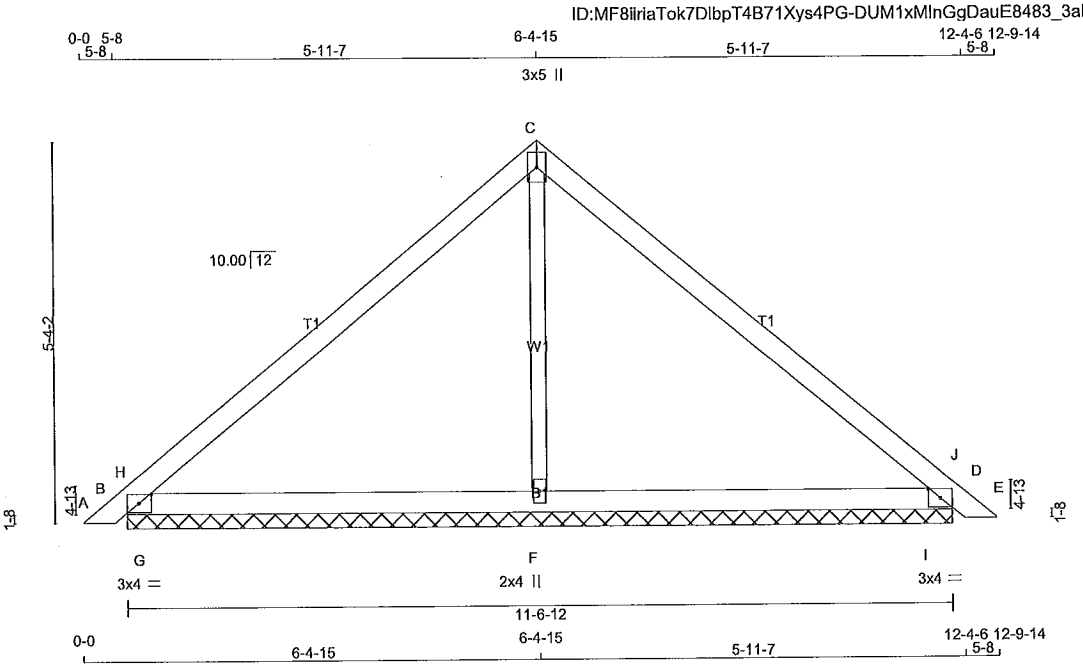
JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.97 (L) (INPUT = 1.00)

A-1802367(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H8P	7	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:41:53 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-DUM1xMlnGgDauE8483_3aBtrSyv9PqJN0Yhh6Mzjxgy



TOTAL WEIGHT = 7 X 36 = 251 lb [M]

LUMBER
N. L. G. A. RULES

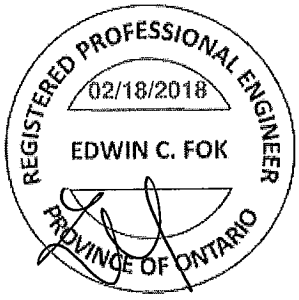
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTW+p	MT20	3.0	5.0		
D	TMB1-I	MT20	3.0	4.0	1.50	2.00
F	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	447	0	447	0	0	11-6-12	1-3-14
D	447	0	447	0	0	11-6-12	1-3-14
F	303	0	303	0	0	11-6-12	1-3-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	315	211 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
D	315	211 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
F	221	108 / 0	0 / 0	0 / 0	0 / 0	113 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 12	-78.0	-78.0	0.02 (1)		10.00
B- H	-38 / 167	-78.0	-78.0	0.23 (1)		6.25
H- C	-260 / 0	-78.0	-78.0	0.32 (1)		6.25
C- J	-260 / 0	-78.0	-78.0	0.32 (1)		6.25
J- D	-38 / 167	-78.0	-78.0	0.23 (1)		6.25
D- E	0 / 12	-78.0	-78.0	0.02 (1)		10.00
B- G	0 / 187	-18.5	-18.5	0.29 (1)		10.00
G- F	0 / 187	-18.5	-18.5	0.29 (1)		10.00
F- I	0 / 187	-18.5	-18.5	0.29 (1)		10.00
I- D	0 / 187	-18.5	-18.5	0.29 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-68 / 3	0.03 (1)
G- H	-802 / 0	0.00 (1)
I- J	-802 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.32/1.00 (C-H:1) , BC=0.29/1.00 (B-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.62/1.00 (B-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

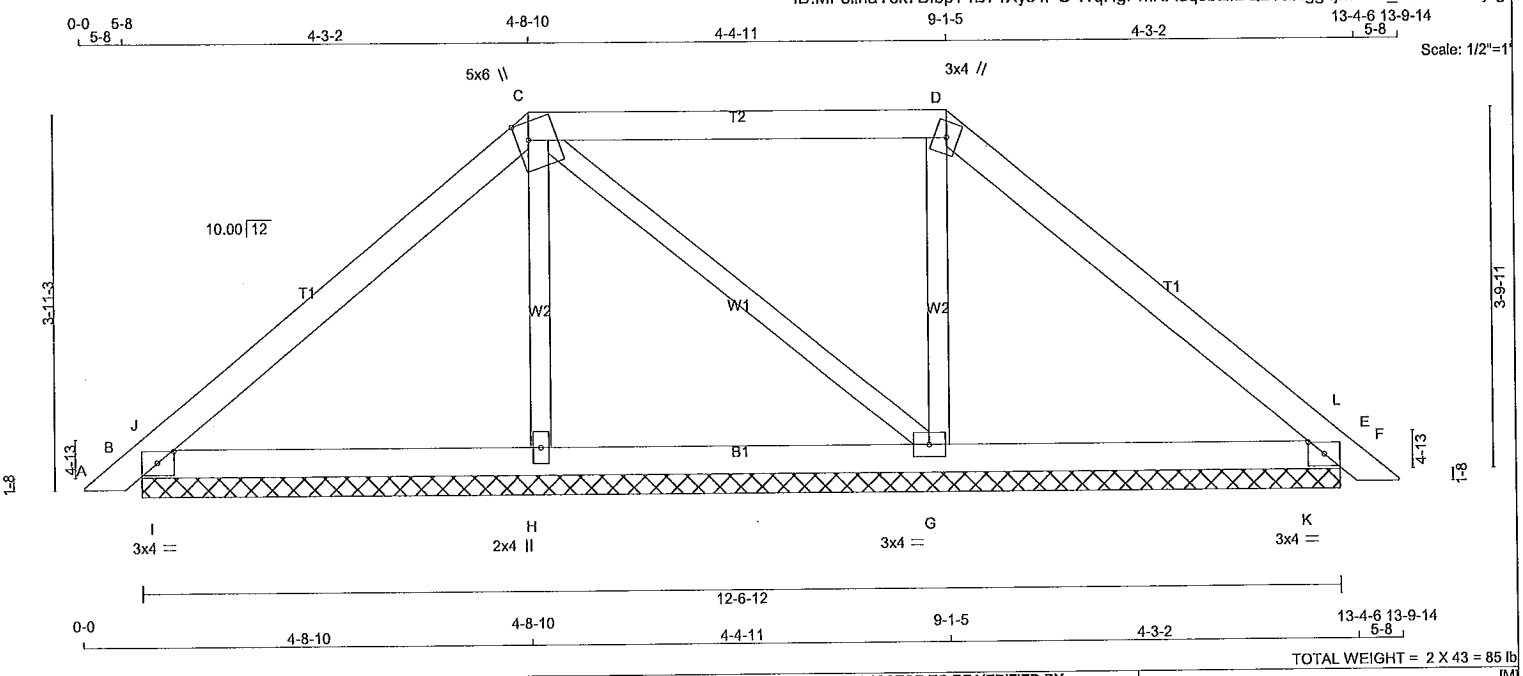
JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.11 (D) (INPUT = 1.00)

A-18023673

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H9P	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8IiriaTok7DlbpT4B71Xys4PG-WqHgPmNAdq6bEIBQ21ciNgg4jmLEY_3Pd7tZsSzjxgr



TOTAL WEIGHT = 2 X 43 = 85 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
C- D	1	12	TOP
D- F	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B- E	1	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

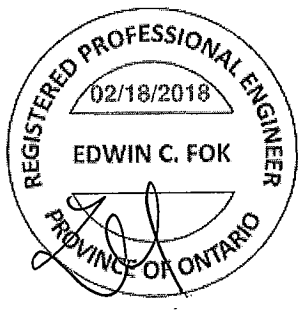
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW+m	MT20	3.0	4.0		
E	TMB1-I	MT20	3.0	4.0	1.50	2.00
G	BMWW1-t	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	339	0	339	0	0	12-6-12	8-2
E	315	0	315	0	0	12-6-12	8-2
H	285	0	285	0	0	12-6-12	8-2
G	355	0	355	0	0	12-6-12	8-2

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	239	162 / 0	0 / 0	0 / 0	0 / 0	77 / 0	0 / 0
E	223	149 / 0	0 / 0	0 / 0	0 / 0	74 / 0	0 / 0
H	206	111 / 0	0 / 0	0 / 0	0 / 0	94 / 0	0 / 0
G	254	150 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 12	-78.0	-78.0	0.01 (1)	10.00	H- C	-175 / 0	0.02 (1)
B- J	-48 / 0	-78.0	-78.0	0.04 (1)	6.25	C- G	-37 / 0	0.01 (1)
J- C	-179 / 0	-78.0	-78.0	0.08 (1)	6.25	G- D	-221 / 0	0.03 (1)
C- D	-97 / 0	-78.0	-78.0	0.13 (1)	6.25	I- J	-361 / 0	0.00 (1)
D- L	-144 / 0	-78.0	-78.0	0.08 (1)	6.25	K- L	-365 / 0	0.00 (1)
L- E	-41 / 9	-78.0	-78.0	0.04 (1)	6.25			
E- F	0 / 12	-78.0	-78.0	0.01 (1)	10.00			
B- I	0 / 130	-18.5	-18.5	0.07 (1)	10.00			
I- H	0 / 130	-18.5	-18.5	0.07 (1)	10.00			
H- G	0 / 125	-18.5	-18.5	0.04 (1)	10.00			
G- K	0 / 103	-18.5	-18.5	0.07 (1)	10.00			
K- E	0 / 103	-18.5	-18.5	0.07 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (C-D:1) , BC=0.07/1.00 (H-I:1) , WB=0.03/1.00 (D-G:1) , SSI=0.14/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

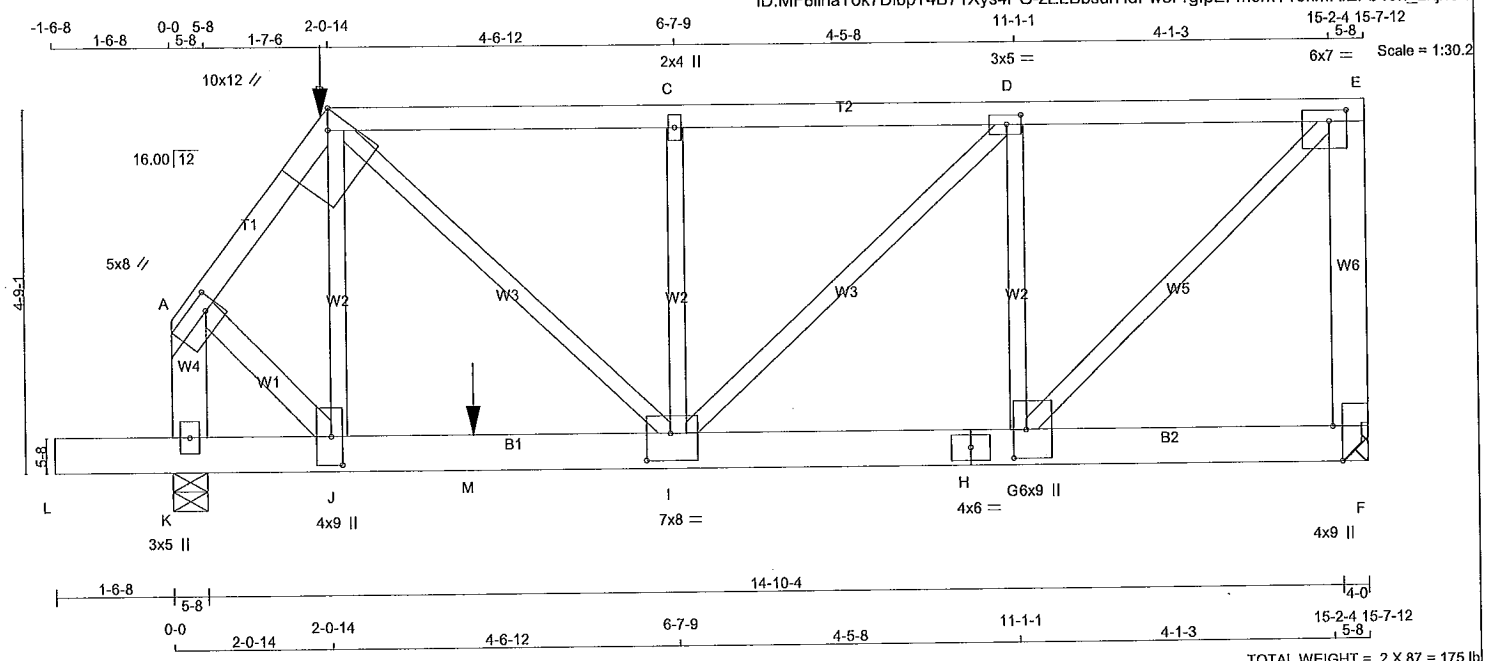
JSI GRIP= 0.18 (D) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

A-18023674

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H11A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:55:07 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-zELBbsurHdFw8FrgfpEFmemT10kMRiEFb40h_LzjxUY



TOTAL WEIGHT = 2 X 87 = 175 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - A	2x6	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

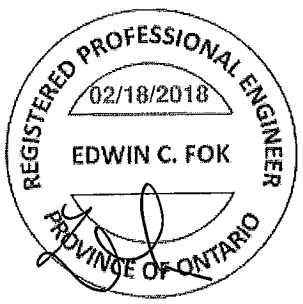
CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	SIDE(61.0)
B- E	1	12	SIDE(46.8)
E- F	2	12	TOP
K- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- H	2	8	SIDE(339.1)
H- F	2	8	SIDE(339.1)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	6584	0	6584	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
K	5821	0	5821	0	0	5-8	3-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	4684	2915 / 0	0 / 0	0 / 0	0 / 0	1769 / 0	0 / 0
K	4121	2679 / 0	0 / 0	0 / 0	0 / 0	1441 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-5114 / 0	-78.0	-78.0	0.12 (1)	4.21	J- B	0 / 540	0.07 (1)
B- C	-6345 / 0	-117.0	-117.0	0.34 (1)	3.62	B- I	0 / 4549	0.56 (1)
C- D	-6345 / 0	-117.0	-117.0	0.31 (1)	3.65	I- C	-495 / 0	0.08 (1)
D- E	-5242 / 0	-117.0	-117.0	0.29 (1)	4.00	I- D	0 / 1548	0.19 (1)
F- E	-5260 / 0	0.0	0.0	0.58 (1)	6.37	G- D	-1658 / 0	0.27 (1)
K- A	-6322 / 0	0.0	0.0	0.25 (1)	5.91	G- E	0 / 7268	0.90 (1)
						A- J	0 / 3704	0.33 (1)
L- K	0 / 0	-96.5	-96.5	0.04 (1)	10.00			
K- J	0 / 0	-27.7	-30.0	0.54 (1)	10.00			
J- M	0 / 3047	-30.0	-31.8	0.90 (1)	10.00			
M- I	0 / 3047	-696.8	-696.8	0.90 (1)	10.00			
I- H	0 / 5242	-696.8	-696.8	0.83 (1)	10.00			
H- G	0 / 5242	-696.8	-696.8	0.83 (1)	10.00			
G- F	0 / 0	-696.8	-696.8	0.46 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-0-14	-67	-67	—	FRONT	VERT	TOTAL
M	3-11-4	-2165	-2165	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 2-0-14
RIGHT SETBACK = 0-0
END SETBACK = 4-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 2-0-0
END DISTANCE = 3-11-4
END SPAN CARRIED = 2-2-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 3-11-4
START SPAN CARRIED = 29-8-14
END DISTANCE = 15-7-12
END SPAN CARRIED = 29-8-14
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

A-18023675

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H11A	1	2	TRUSS DESC.	

PLATES (table is in inches)

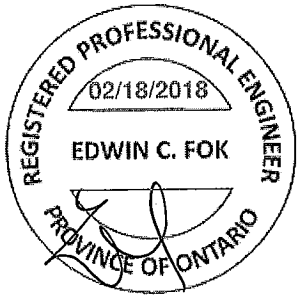
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TTWW-h	MT20	10.0	12.0	Edge	2.75
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	3.0	5.0	1.50	2.25
E	TMVW-t	MT20	6.0	7.0	1.75	2.75
F	BMV1+t	MT20	4.0	9.0	Edge	1.50
G	BMWW+t	MT20	6.0	9.0	4.50	2.00
H	BS-t	MT20	4.0	6.0		
I	BMWWW-t	MT20	7.0	8.0	4.25	3.75
J	BMWW+t	MT20	4.0	9.0	4.50	1.75
K	BMV1+p	MT20	3.0	5.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 67.3 lbs FACTORED DOWN AT 2-0-14 ON TOP CHORD, AND 2165.0 lbs FACTORED DOWN AT 3-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.58/1.00 (E-F:1) , BC=0.90/1.00 (I-J:1) , WB=0.90/1.00 (E-G:1) , SSI=0.71/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

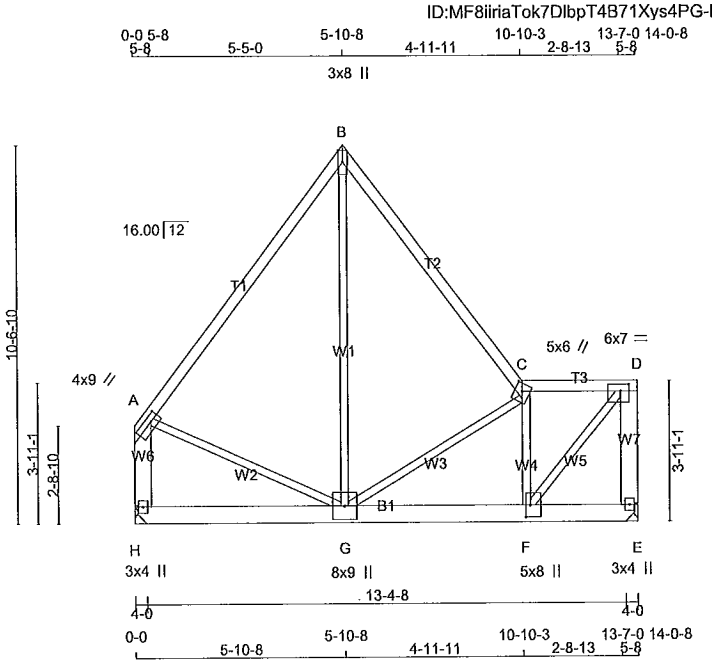
JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.85 (H) (INPUT = 1.00)

A-18023675(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H12A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8iiriaTok7DlbpT4B71Xys4PG-R2B0Hxl7AfDc9ixfCuCHxP75llSI8CFhpHQT9DzjxIx



TOTAL WEIGHT = 2 X 83 = 167 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - A	2x6	DRY	No.2	SPF
H - E	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

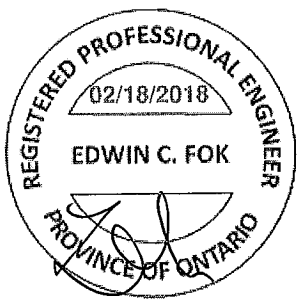
CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- B	1	12	TOP
B- C	1	12	TOP
C- D	1	12	TOP
D- E	2	12	TOP
H- A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	8	SIDE(346.6)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	5544	0	5544	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
H	5544	0	5544	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	3952	2412 / 0	0 / 0	0 / 0	0 / 0	1540 / 0	0 / 0
H	3952	2412 / 0	0 / 0	0 / 0	0 / 0	1540 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO					FR-TO			
A- B	-3511 / 0	-78.0	-78.0	0.57 (1)	4.66	G- B	0 / 5175	0.64 (1)
B- C	-3495 / 0	-78.0	-78.0	0.35 (1)	4.78	G- C	-2388 / 0	0.77 (1)
C- D	-4007 / 0	-78.0	-78.0	0.13 (1)	4.64	F- C	-1794 / 0	0.21 (1)
E- D	-4661 / 0	0.0	0.0	0.34 (1)	6.69	F- D	0 / 6098	0.75 (1)
H- A	-3843 / 0	0.0	0.0	0.20 (1)	7.22	A- G	0 / 2255	0.28 (1)
H- G	0 / 0	-711.6	-711.6	0.58 (1)	10.00			
G- F	0 / 4060	-711.6	-711.6	0.75 (1)	10.00			
F- E	0 / 0	-711.6	-711.6	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 30-8-14
END DISTANCE = 14-0-8
END SPAN CARRIED = 30-8-14
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 843 (0.20")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.75/1.00 (F-G:1) , WB=0.77/1.00 (C-G:1) , SSI=0.82/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.73 (F) (INPUT = 1.00)

A-18023676

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. / TRUSS DESC.	DRWG NO.
292616	H12A	1	2		

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:39:46 2018 Page 2

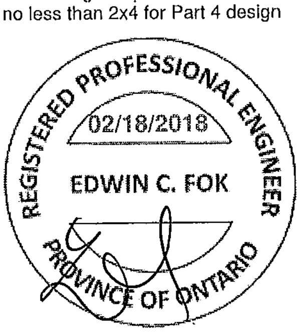
ID:MF8iirlaTok7DlbpT4B71Xys4PG-R2B0Hxl7AfDc9ixfCuCHxP75ilSI8CFhpHQT9Dzjxix

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	9.0	1.75	3.00
B	TTW+p	MT20	3.0	8.0	3.75	Edge
C	TTWW+m	MT20	5.0	6.0	2.50	2.50
D	TMVW-t	MT20	6.0	7.0	2.25	2.75
E	BMV1+p	MT20	3.0	4.0		
F	BMWW+t	MT20	5.0	8.0	4.00	1.50
G	BMWWW+t	MT20	8.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

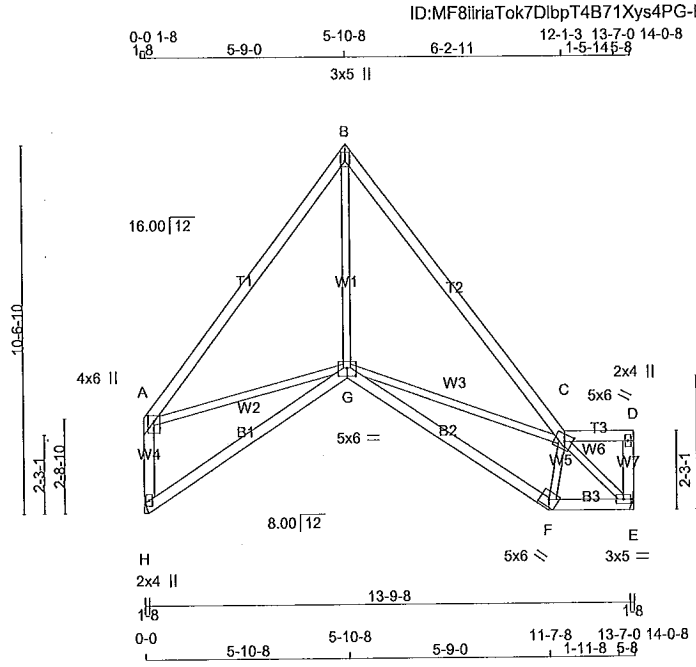


A·18023676(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H12T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:42:10 2018 Page 1
ID:MF8IiriaTok7DlbpT4B71Xys4PG-EluSWAVSGuMAQryLe7o2mn4mXojutUguwhI5DtzjxgH



TOTAL WEIGHT = 68 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	1650F 1.5E	SPF
B - C	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

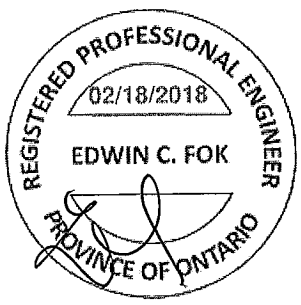
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TTWWW-m	MT20	5.0	6.0	2.25	2.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	5.0		
F	BBW-h	MT20	5.0	6.0	2.25	3.00
G	BBWWW-p	MT20	5.0	6.0	2.75	3.00
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	677	0	677	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	677	0	677	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	483	295 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0
H	483	295 / 0	0 / 0	0 / 0	0 / 0	188 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-612 / 0	-78.0	-78.0	0.30 (1)	6.25	G- B	0 / 505	0.11 (1)
B- C	-610 / 0	-78.0	-78.0	0.26 (1)	6.25	G- C	-165 / 0	0.14 (1)
C- D	0 / 0	-78.0	-78.0	0.05 (1)	10.00	F- C	-280 / 0	0.04 (1)
E- D	-76 / 0	0.0	0.0	0.01 (1)	7.81	C- E	-831 / 0	0.14 (1)
H- A	-623 / 0	0.0	0.0	0.09 (1)	7.81	A- G	0 / 380	0.09 (1)
H- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00			
G- F	0 / 631	-18.5	-18.5	0.24 (4)	10.00			
F- E	0 / 592	-18.5	-18.5	0.11 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.30/1.00 (A-B:1) , BC=0.24/1.00 (F-G:4) , WB=0.14/1.00 (C-E:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

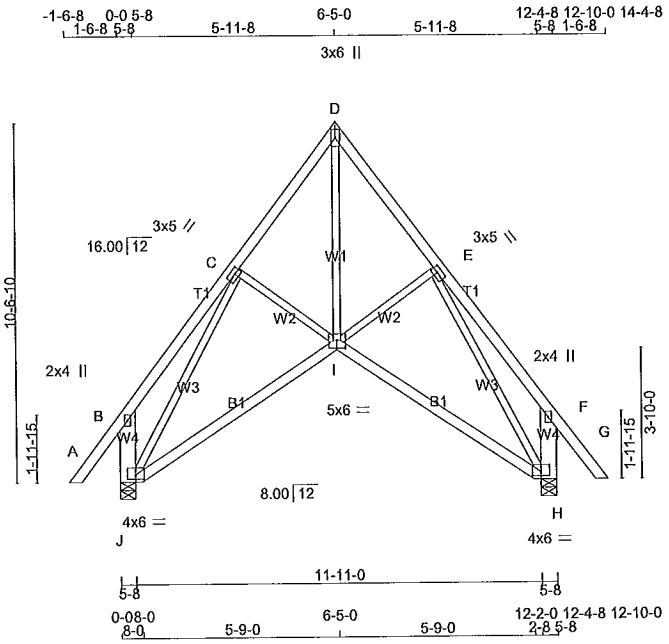
JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.32 (H) (INPUT = 1.00)

A-18023677

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H13T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:43:13 2018 Page 1
ID:MF8IiriaTok7DlbpT4B71Xys4PG-ttBGkjFzQFIEUWGSqsQqq3BaRBCN8re9Q_3wwjzjxfi



TOTAL WEIGHT = 75 lb

[M][F]

LUMBER
N. L. G. A. RULES

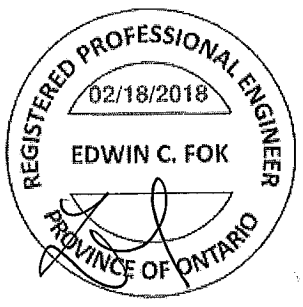
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMWW-t	MT20	3.0	5.0	1.50	2.00
D	TTW+p	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	5.0	1.50	2.00
F	TMV+p	MT20	2.0	4.0		
H	BVMW1-p	MT20	4.0	6.0	1.50	3.00
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BVMW1-p	MT20	4.0	6.0	1.50	3.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	727	0	727	0	0	5-8	1-8
H	727	0	727	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	516	327 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0
H	516	327 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) J, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS		
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 43	-78.0	-78.0	0.11 (1)	10.00	I- D	0 / 658	0.15 (1)
B- C	0 / 20	-78.0	-78.0	0.12 (1)	10.00	C- I	-53 / 5	0.01 (1)
C- D	-548 / 0	-78.0	-78.0	0.09 (1)	6.25	I- E	-53 / 5	0.01 (1)
D- E	-548 / 0	-78.0	-78.0	0.09 (1)	6.25	J- C	-780 / 0	0.70 (1)
E- F	0 / 20	-78.0	-78.0	0.12 (1)	10.00	E- H	-780 / 0	0.70 (1)
F- G	0 / 43	-78.0	-78.0	0.11 (1)	10.00			
J- B	-213 / 0	0.0	0.0	0.02 (1)	7.81			
H- F	-213 / 0	0.0	0.0	0.02 (1)	7.81			
J- I	0 / 430	-18.5	-18.5	0.26 (4)	10.00			
I- H	0 / 430	-18.5	-18.5	0.26 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.11")

CSI: TC=0.12/1.00 (E-F:1) , BC=0.26/1.00 (I-J:4) , WB=0.70/1.00 (E-H:1) , SSI=0.07/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

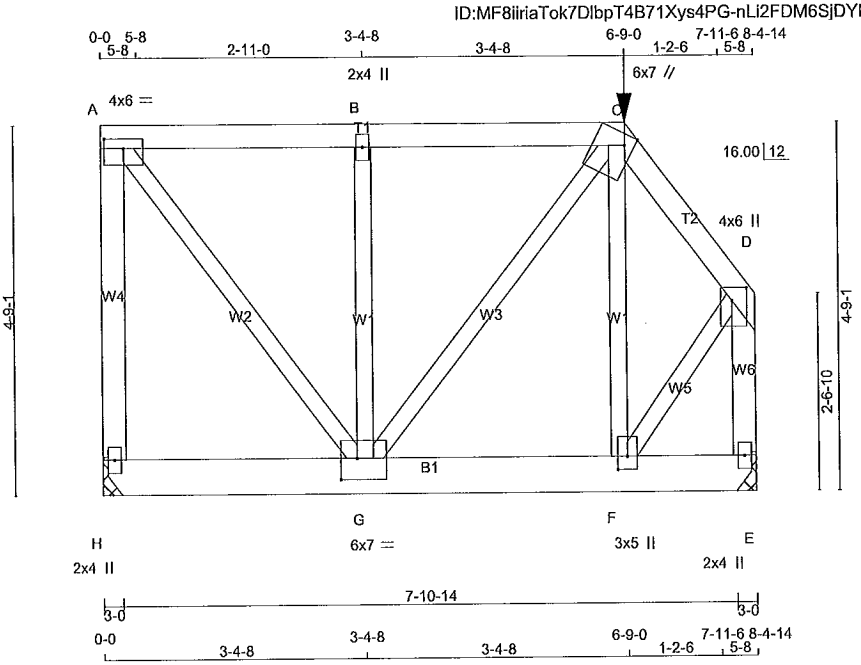
JSI GRIP= 0.85 (C) (INPUT = 0.90)
JSI METAL= 0.23 (C) (INPUT = 1.00)

A-18023678

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H14A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:46:07 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-nLi2FDM6SjDYMGZu0PYdtmOERKa2vdpmQ3D72ezjxd_



Scale = 1:29.6

TOTAL WEIGHT = 2 X 50 = 100 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	1	12	TOP
A- C	1	12	SIDE(37.9)
C- D	1	12	SIDE(57.3)
E- D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	8	SIDE(342.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

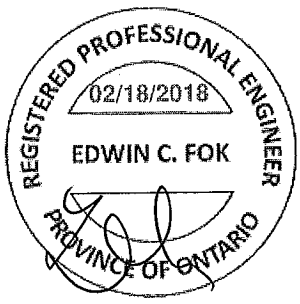
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	3552	0	3552	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-4	
E	3517	0	3517	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-3	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	2533	1546 / 0	0 / 0	0 / 0	0 / 0	986 / 0	0 / 0
E	2508	1527 / 0	0 / 0	0 / 0	0 / 0	981 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
H- A	-2565 / 0	0.0	0.0	0.45 (1)	7.08	A- G	0 / 2994	0.37 (1)
A- B	-1828 / 0	-141.3	-141.3	0.13 (1)	6.25	G- B	-540 / 0	0.09 (1)
B- C	-1827 / 0	-141.3	-141.3	0.13 (1)	6.25	G- C	0 / 1010	0.13 (1)
C- D	-2021 / 0	-78.0	-78.0	0.03 (1)	6.18	F- C	0 / 467	0.06 (1)
E- D	-3107 / 0	0.0	0.0	0.22 (1)	6.57	F- D	0 / 1851	0.23 (1)
H- G	0 / 0	-702.5	-702.5	0.26 (1)	10.00			
G- F	0 / 1207	-702.5	-702.5	0.33 (1)	10.00			
F- E	0 / 0	-702.5	-702.5	0.13 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-9-0	-81	-81	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 0-0
RIGHT SETBACK = 1-7-14
END SETBACK = 5-3-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 29-8-14
END DISTANCE = 8-4-14
END SPAN CARRIED = 29-8-14
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.45/1.00 (A-H:1) , BC=0.33/1.00 (F-G:1) , WB=0.37/1.00 (A-G:1) , SSI=0.46/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023679

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H14A	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	3.00
B	TMW+w	MT20	2.0	4.0		
C	TTWW+m	MT20	6.0	7.0	Edge	1.50
D	TMVW+p	MT20	4.0	6.0	2.00	2.25
E	BMV1+p	MT20	2.0	4.0		
F	BMWW+t	MT20	3.0	5.0	2.00	1.50
G	BMWWW-t	MT20	6.0	7.0	3.25	2.50
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 80.7 lbs FACTORED DOWN AT 6-9-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NAIL VALUES

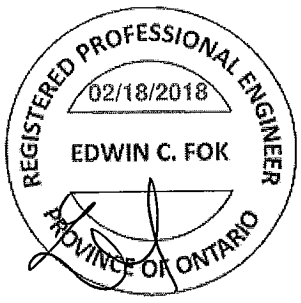
PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

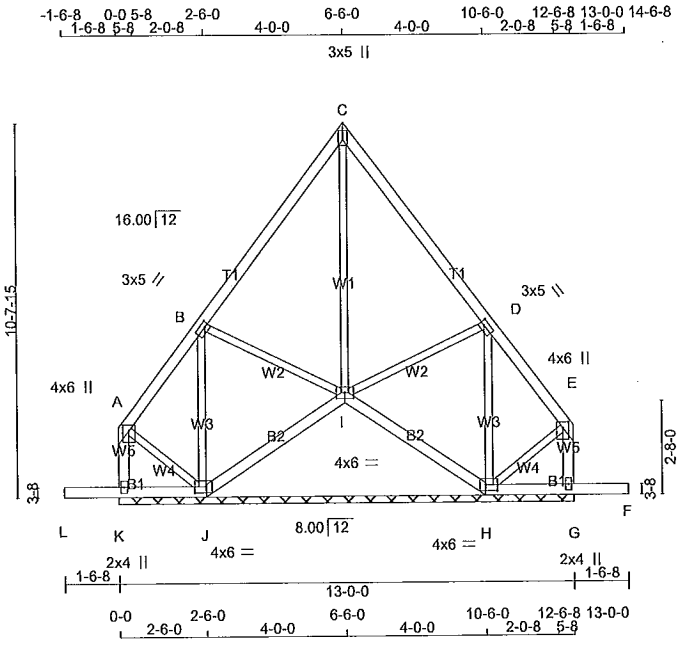


A1802367(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H15T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:56:17 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-wiZfHollo84_Y?C7lWUgbOfYoEpPqXwYIz_OSHzjxTS



TOTAL WEIGHT = 74 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

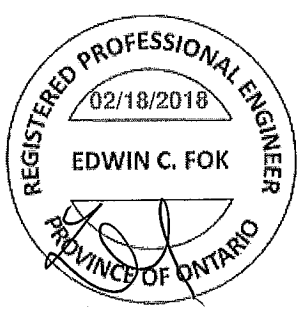
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMWW-t	MT20	3.0	5.0	1.50	1.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW1-I	MT20	4.0	6.0	2.00	4.00
I	BBWWW1-p	MT20	4.0	6.0		
J	BBWW1-I	MT20	4.0	6.0	2.00	4.00
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	180	0	180	0	0	13-0-0 (6-6-0)	1-1-13
I	443	0	443	0	0	13-0-0 (6-6-0)	1-1-13
H	180	0	180	0	0	13-0-0 (6-6-0)	1-1-13
K	375	0	375	0	0	13-0-0 (6-6-0)	1-1-13
G	375	0	375	0	0	13-0-0 (6-6-0)	1-1-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): I

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	131	64 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
I	315	199 / 0	0 / 0	0 / 0	0 / 0	116 / 0	0 / 0
H	131	64 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
K	265	174 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0
G	265	174 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, I, H, K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-95 / 0		-78.0	-78.0	0.17 (1)	6.25
B- C	-57 / 0		-78.0	-78.0	0.17 (1)	6.25
C- D	-57 / 0		-78.0	-78.0	0.17 (1)	6.25
D- E	-95 / 0		-78.0	-78.0	0.17 (1)	6.25
K- A	-203 / 0		0.0	0.0	0.02 (1)	7.81
G- E	-203 / 0		0.0	0.0	0.02 (1)	7.81
L- K	0 / 0		-96.5	-96.5	0.16 (1)	10.00
K- J	0 / 0		-18.5	-18.5	0.03 (4)	10.00
J- I	0 / 93		-18.5	-18.5	0.09 (4)	10.00
I- H	0 / 93		-18.5	-18.5	0.09 (4)	10.00
H- G	0 / 0		-18.5	-18.5	0.03 (4)	10.00
G- F	0 / 0		-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
I- C	-207 / 0		0.24 (1)
I- D	-64 / 0		0.02 (1)
H- D	-223 / 0		0.09 (1)
B- I	-64 / 0		0.02 (1)
J- B	-223 / 0		0.09 (1)
A- J	0 / 97		0.02 (1)
H- E	0 / 97		0.02 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (B-C:1) , BC=0.16/1.00 (F-G:1) , WB=0.24/1.00 (C-I:1) , SSI=0.12/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

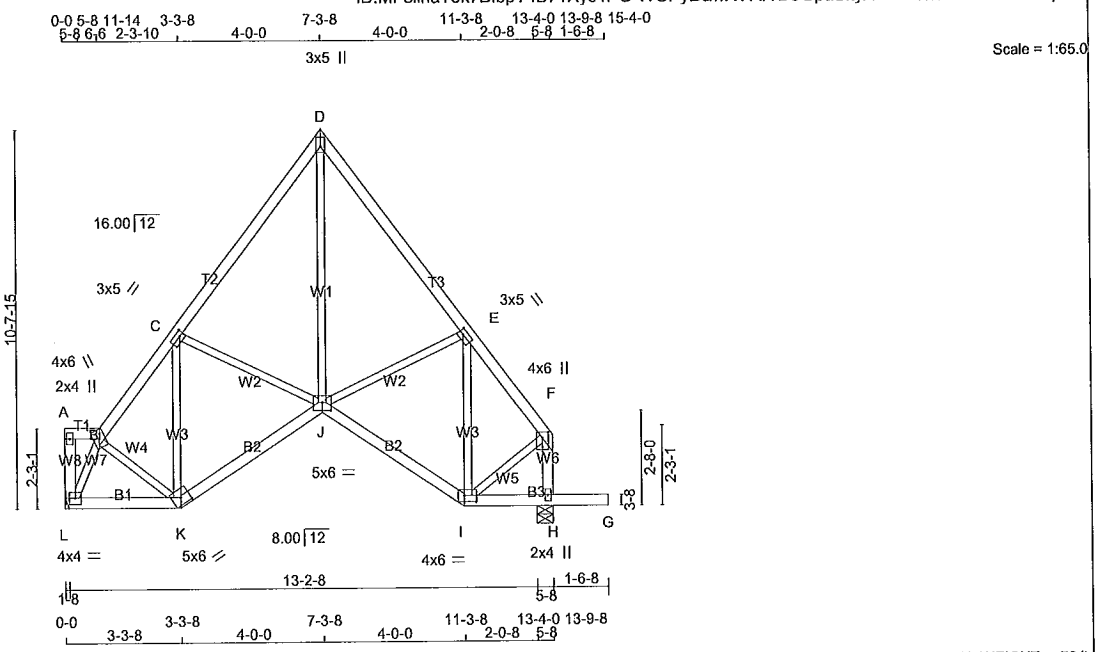
JSI GRIP= 0.32 (I) (INPUT = 0.90)
JSI METAL= 0.15 (I) (INPUT = 1.00)

A-18023680

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H15TS	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:56:31 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-WOPyDawXVRr?D9GpaSkyALExStbh6u3cw8N8xTzjxTE



TOTAL WEIGHT = 76 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TTWW+m	MT20	4.0	6.0		
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BBWW-l	MT20	4.0	6.0	2.00	4.00
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BBWW-h	MT20	5.0	6.0	2.25	3.00
L	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER
02/18/2018
EDWIN C. FOK
PROVINCE OF ONTARIO

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	665	0	665	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	814	0	814	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL.
L	474	289 / 0	0 / 0	0 / 0	0 / 0	185 / 0	0 / 0
H	580	354 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
L- A	-38 / 0	0.0	0.0	0.00 (1)	7.81
A- B	0 / 0	-78.0	-78.0	0.01 (1)	10.00
B- C	-560 / 0	-78.0	-78.0	0.13 (1)	6.25
C- D	-526 / 0	-78.0	-78.0	0.16 (1)	6.25
D- E	-526 / 0	-78.0	-78.0	0.16 (1)	6.25
E- F	-467 / 0	-78.0	-78.0	0.13 (1)	6.25
H- F	-642 / 0	0.0	0.0	0.07 (1)	7.81
L- K	0 / 308	-18.5	-18.5	0.09 (4)	10.00
K- J	0 / 434	-18.5	-18.5	0.13 (4)	10.00
J- I	0 / 357	-18.5	-18.5	0.12 (4)	10.00
I- H	0 / 0	-18.5	-18.5	0.03 (4)	10.00
H- G	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- B	-672 / 0	0.10 (1)
B- K	0 / 74	0.02 (1)
K- C	-220 / 0	0.09 (1)
C- J	-66 / 0	0.02 (1)
J- D	0 / 542	0.12 (1)
J- E	0 / 7	0.00 (4)
I- E	-332 / 0	0.13 (1)
I- F	0 / 360	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.16/1.00 (G-H:1) , WB=0.13/1.00 (E-I:1) , SSI=0.12/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

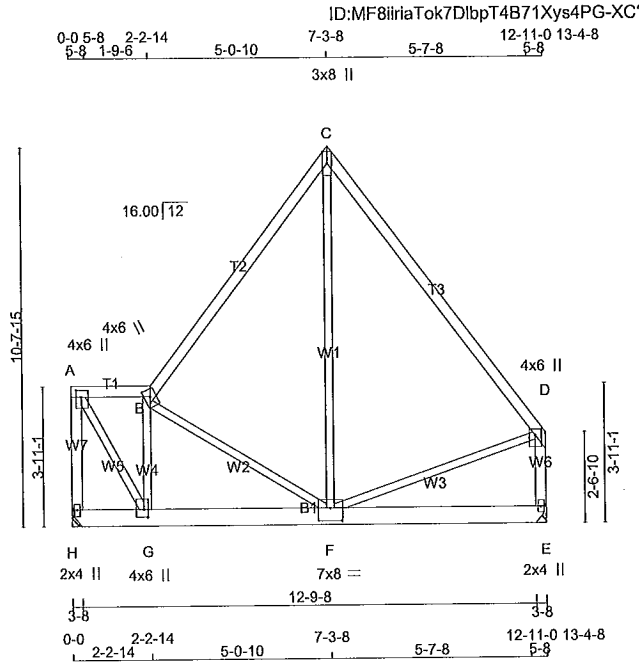
JSI GRIP= 0.83 (D) (INPUT = 0.90)
JSI METAL= 0.17 (F) (INPUT = 1.00)

A-18023681

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H16A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:57:22 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-XC?Ew0XWU6Gmr?gd3g9wk6sa3Rs6Zsl7eaDLE0zjxSR



TOTAL WEIGHT = 78 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	2100F 1.8E	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

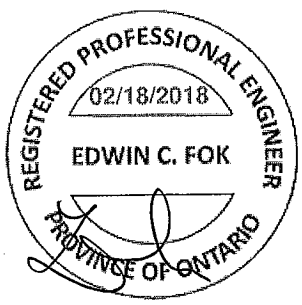
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.25	1.75
B	TTWW+m	MT20	4.0	6.0	2.50	2.00
C	TTW+p	MT20	3.0	8.0	3.75	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	7.0	8.0	4.25	4.00
G	BMWW+t	MT20	4.0	6.0	2.50	1.50
H	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	2103	0	2103	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-5	IN-SX
E	2103	0	2103	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-5	IN-SX

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1500	915 / 0	0 / 0	0 / 0	0 / 0	584 / 0	0 / 0
E	1500	915 / 0	0 / 0	0 / 0	0 / 0	584 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.12 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-1927 / 0	0.0	0.0	0.44 (1)	6.01	A- G	0 / 2175	0.54 (1)	
A- B	-1121 / 0	-78.0	-78.0	0.09 (1)	5.87	G- B	-1057 / 0	0.25 (1)	
B- C	-1272 / 0	-78.0	-78.0	0.51 (1)	5.12	B- F	-469 / 0	0.31 (1)	
C- D	-1278 / 0	-78.0	-78.0	0.47 (1)	6.15	F- C	0 / 1602	0.40 (1)	
E- D	-1521 / 0	0.0	0.0	0.21 (1)	6.62	F- D	0 / 810	0.20 (1)	
H- G	0 / 0	-236.6	-236.6	0.12 (1)	10.00				
G- F	0 / 1152	-236.6	-236.6	0.76 (1)	10.00				
F- E	0 / 0	-236.6	-236.6	0.61 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-0-8
END DISTANCE = 13-4-8
END SPAN CARRIED = 11-0-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.51/1.00 (B-C:1) , BC=0.76/1.00 (F-G:1) , WB=0.54/1.00 (A-G:1) , SSI=0.57/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.52 (G) (INPUT = 1.00)

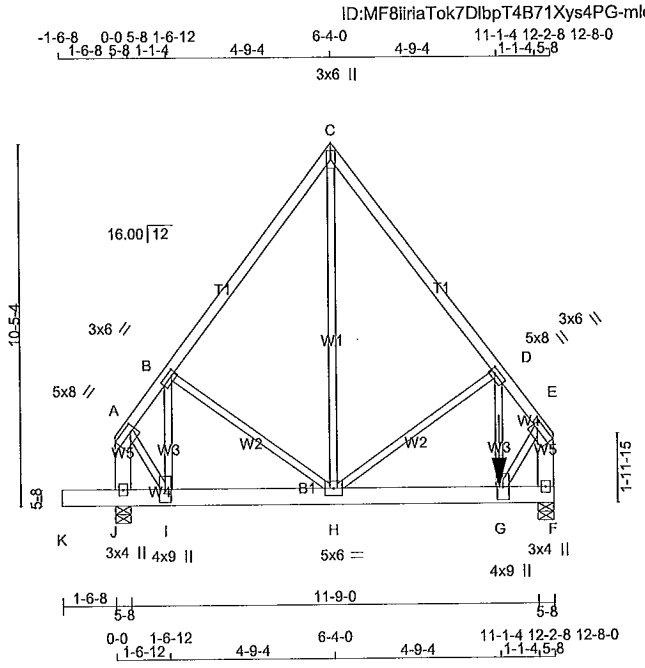
A-18023682

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H17A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:58:22 2018 Page 1

ID:MF8iiriaTok7DlbpT4B71Xys4PG-mldvWXG9MYqF2DF9aGE_AlLtXdSMd?9yRvlWKXzjxRV



TOTAL WEIGHT = 82 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
J - A	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
K - F	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

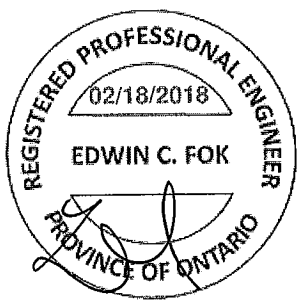
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.25	2.00
B	TMWW-t	MT20	3.0	6.0	1.50	1.75
C	TTW+p	MT20	3.0	6.0		
D	TMWW-t	MT20	3.0	6.0	1.50	1.75
E	TMVW-t	MT20	5.0	8.0	2.25	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWW+t	MT20	4.0	9.0		
H	BMWWW-t	MT20	5.0	6.0		
I	BMWW+t	MT20	4.0	9.0		
J	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2159.3 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	2495	0	2495	0	0	5-8	2-11
J	1035	0	1035	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1744	1259 / 0	0 / 0	0 / 0	0 / 0	485 / 0	0 / 0
J	733	475 / 0	0 / 0	0 / 0	0 / 0	258 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-520 / 0	-78.0	-78.0	0.25 (1)		6.25	H- C	0 / 668	0.17 (1)
B- C	-635 / 0	-78.0	-78.0	0.28 (1)		6.25	H- D	-865 / 0	0.54 (1)
C- D	-634 / 0	-78.0	-78.0	0.27 (1)		6.25	G- D	0 / 1059	0.26 (1)
D- E	-1676 / 0	-78.0	-78.0	0.28 (1)		4.77	B- H	-3 / 2	0.00 (4)
J- A	-792 / 0	0.0	0.0	0.06 (1)		7.81	I- B	-391 / 0	0.10 (1)
F- E	-2392 / 0	0.0	0.0	0.19 (1)		6.62	A- I	0 / 506	0.13 (1)
							G- E	0 / 1473	0.36 (1)
K- J	0 / 0	-96.5	-96.5	0.08 (1)		10.00			
J- I	0 / 0	-18.5	-18.5	0.08 (1)		10.00			
I- H	0 / 363	-18.5	-18.5	0.10 (1)		10.00			
H- G	0 / 1055	-18.5	-18.5	0.24 (1)		10.00			
G- F	0 / 0	-18.5	-18.5	0.10 (1)		10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	11-1-4	-2159	-2159	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.28/1.00 (D-E:1) , BC=0.24/1.00 (G-H:1) , WB=0.54/1.00 (D-H:1) , SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

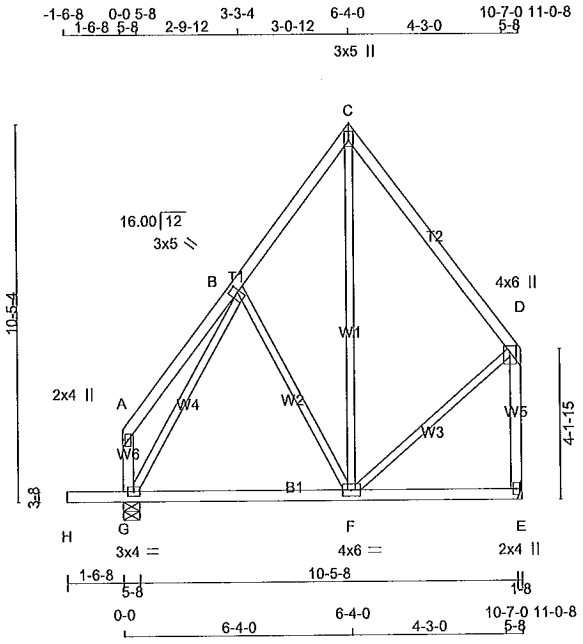
A-18023683

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H17S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:58:36 2018 Page 1
ID:MF8ilrIaTok7DlbpT4B71Xys4PG-MRTCSKQx3rbGjNKrPCUGkiwHuGFOvNc0f58FqjzjxRH



TOTAL WEIGHT = 3 X 65 = 195 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - A	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW+t	MT20	3.0	5.0	2.25	0.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	533	0	533	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	681	0	681	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	380	232 / 0	0 / 0	0 / 0	0 / 0	148 / 0	0 / 0
G	486	296 / 0	0 / 0	0 / 0	0 / 0	189 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-78.0	-78.0	0.13 (1)	10.00	B- F	-128 / 0	0.10 (1)
B- C	-273 / 0	-78.0	-78.0	0.10 (1)	6.25	F- C	0 / 119	0.04 (4)
C- D	-250 / 0	-78.0	-78.0	0.22 (1)	6.25	G- B	-448 / 0	0.35 (1)
G- A	-91 / 0	0.0	0.0	0.01 (1)	7.81	F- D	0 / 192	0.04 (1)
E- D	-504 / 0	0.0	0.0	0.15 (1)	7.81			
H- G	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
G- F	0 / 213	-18.5	-18.5	0.19 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.17 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.19/1.00 (F-G:4) , WB=0.35/1.00 (B-G:1) , SSI=0.12/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

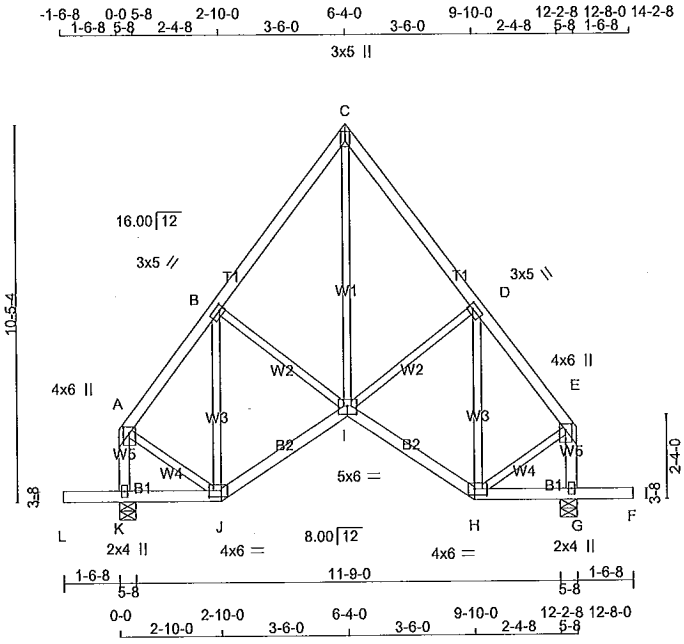
JSI GRIP= 0.56 (B) (INPUT = 0.90)
JSI METAL= 0.29 (B) (INPUT = 1.00)

A-18023684

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H17T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:58:43 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-foOrwjWKP?TH3RMBJA6vWBiVn4eZ2bm2GhL7apzjxRA



TOTAL WEIGHT = 2 X 74 = 149 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

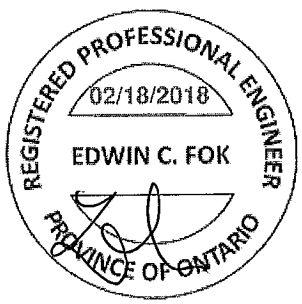
DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMWW-t	MT20	3.0	5.0	1.50	1.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW-I	MT20	4.0	6.0	2.00	4.00
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BBWW-I	MT20	4.0	6.0	2.00	4.00
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	760	0	760	0	0	5-8	1-8
G	760	0	760	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	542	331 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0
G	542	331 / 0	0 / 0	0 / 0	0 / 0	211 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-424 / 0	-78.0	-78.0	0.10 (1)	6.25	I- C	0 / 444	0.10 (1)
B- C	-441 / 0	-78.0	-78.0	0.12 (1)	6.25	I- D	-18 / 0	0.01 (1)
C- D	-441 / 0	-78.0	-78.0	0.12 (1)	6.25	H- D	-272 / 0	0.13 (1)
D- E	-424 / 0	-78.0	-78.0	0.10 (1)	6.25	B- I	-18 / 0	0.01 (1)
K- A	-585 / 0	0.0	0.0	0.07 (1)	7.81	J- B	-272 / 0	0.13 (1)
G- E	-585 / 0	0.0	0.0	0.07 (1)	7.81	A- J	0 / 311	0.07 (1)
						H- E	0 / 311	0.07 (1)
L- K	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
J- I	0 / 320	-18.5	-18.5	0.10 (4)	10.00			
I- H	0 / 320	-18.5	-18.5	0.10 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.04 (4)	10.00			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.12/1.00 (B-C:1) , BC=0.16/1.00 (F-G:1) , WB=0.13/1.00 (D-H:1) , SSI=0.12/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

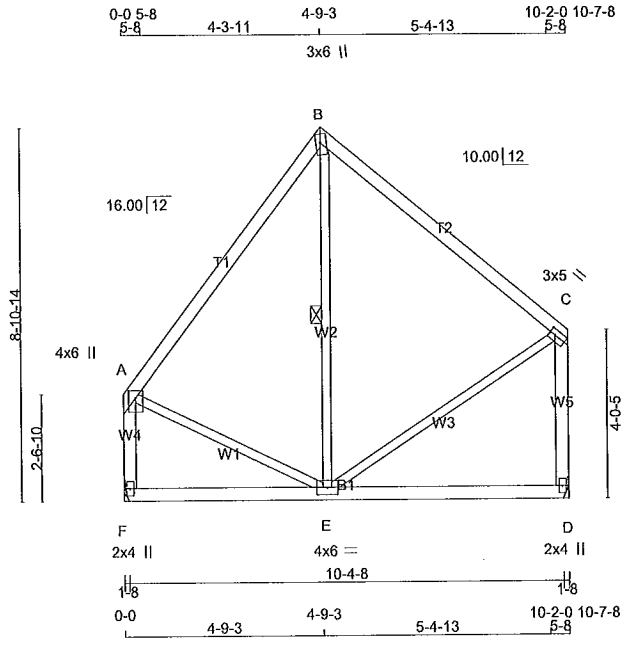
JSI GRIP= 0.68 (C) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

A-18023685

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H18S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:58:49 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-Txl7Ama5?rDQnMpLfRDJmSyTkVh0SKYwfcoRnTzjxR4



TOTAL WEIGHT = 3 X 53 = 160 lb

[M][F]

LUMBER
N. L. G. A. RULES

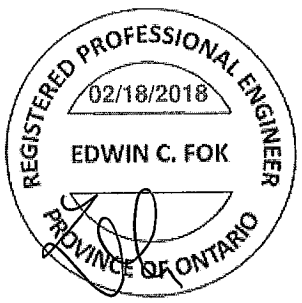
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+m	MT20	3.0	6.0	2.50	1.25
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	512	0	512	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
D	512	0	512	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	365	223 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0
D	365	223 / 0	0 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-276 / 0	-78.0	-78.0	0.23 (1)		6.25
B- C	-214 / 0	-78.0	-78.0	0.34 (1)		6.25
F- A	-481 / 0	0.0	0.0	0.07 (1)		7.81
D- C	-470 / 0	0.0	0.0	0.13 (1)		7.81
F- E	0 / 0	-18.5	-18.5	0.15 (4)		10.00
E- D	0 / 0	-18.5	-18.5	0.15 (4)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-56 / 62	0.03 (1)
A- E	0 / 181	0.04 (1)
E- C	0 / 196	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.35")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.35")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.34/1.00 (B-C:1) , BC=0.15/1.00 (E-F:4) , WB=0.04/1.00 (C-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

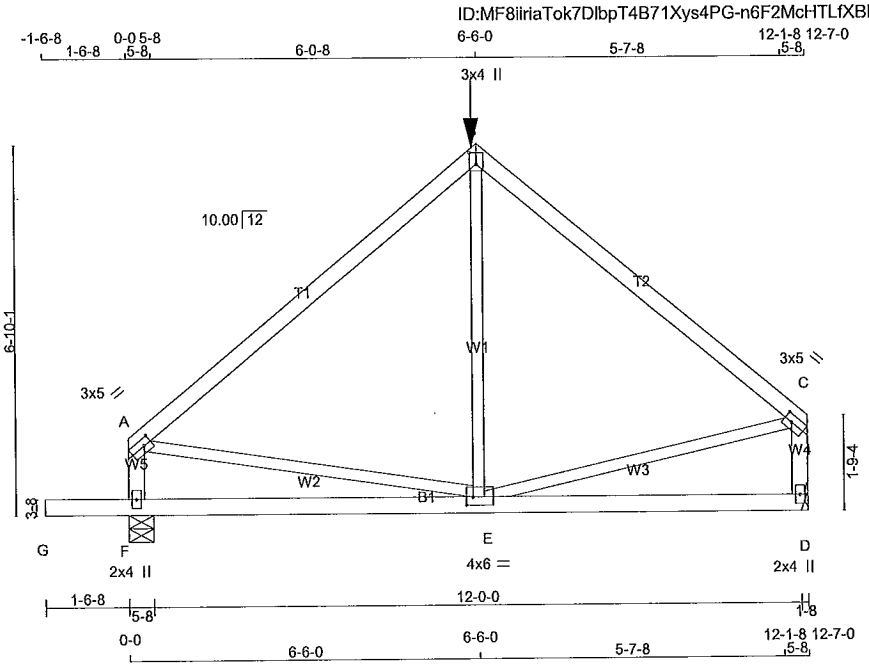
JSI GRIP= 0.42 (C) (INPUT = 0.90)
JSI METAL= 0.13 (A) (INPUT = 1.00)

A-18023686

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H19	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:59:47 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-n6F2McHTLfXBkHFU3dFw6hMJ2tRb1XVT_erWq6zjxQA



TOTAL WEIGHT = 53 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

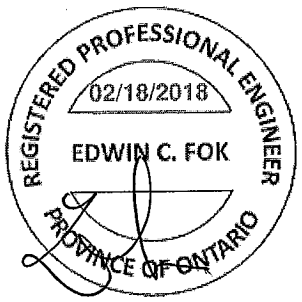
PLATES (table is In inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0	1.75	1.50
F	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 388.4 lbs FACTORED DOWN AT 6-6-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	862	0	862	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	998	0	998	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	615	372 / 0	0 / 0	0 / 0	0 / 0	243 / 0	0 / 0
F	712	430 / 0	0 / 0	0 / 0	0 / 0	282 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.10 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-685 / 0	-78.0	-78.0	0.44 (1)	6.25	E- B	-1 / 155	0.06 (4)
B- C	-685 / 0	-78.0	-78.0	0.60 (1)	6.10	A- E	0 / 533	0.13 (1)
F- A	-777 / 0	0.0	0.0	0.09 (1)	7.81	E- C	0 / 542	0.13 (1)
D- C	-805 / 0	0.0	0.0	0.10 (1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
F- E	0 / 0	-27.2	-27.2	0.34 (4)	10.00			
E- D	0 / 0	-27.2	-27.2	0.34 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	6-6-0	-388	-388	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-6-0
RIGHT SETBACK = 6-1-0
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.34/1.00 (E-F:4) , WB=0.13/1.00 (C-E:1) , SSI=0.17/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

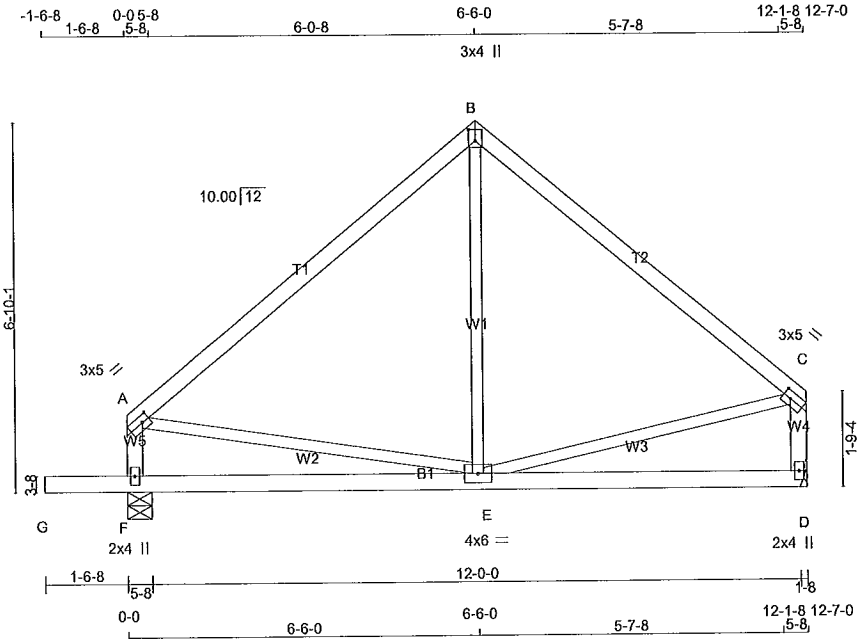
A-18023687

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292616	H20	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 10:59:52 2018 Page 1
ID:MF8liriaTok7DlbpT4B71Xys4PG-733xPJLc9B9Ur28SsAr5pk3ENuAoipjC8wZHVJzjxQ5



TOTAL WEIGHT = 53 lb

[M][F]

LUMBER

N. L. G. A. RULES

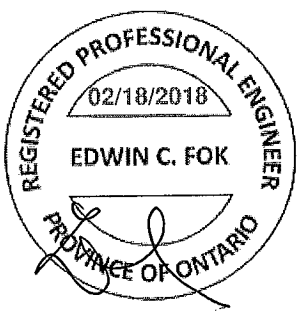
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	1.75
B	TTW+p	MT20	3.0	4.0	2.50	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	1.75
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	607	0	607	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	756	0	756	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	433	264 / 0	0 / 0	0 / 0	0 / 0	169 / 0	0 / 0
F	539	329 / 0	0 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-399 / 0	-78.0	-78.0	0.28 (1)	6.25	E- B	0 / 110	0.04 (4)
B- C	-399 / 0	-78.0	-78.0	0.37 (1)	6.25	A- E	0 / 310	0.07 (1)
F- A	-558 / 0	0.0	0.0	0.06 (1)	7.81	E- C	0 / 316	0.07 (1)
D- C	-568 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.21 (4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.07/1.00 (C-E:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.57 (E) (INPUT = 0.90)
JSI METAL= 0.17 (A) (INPUT = 1.00)

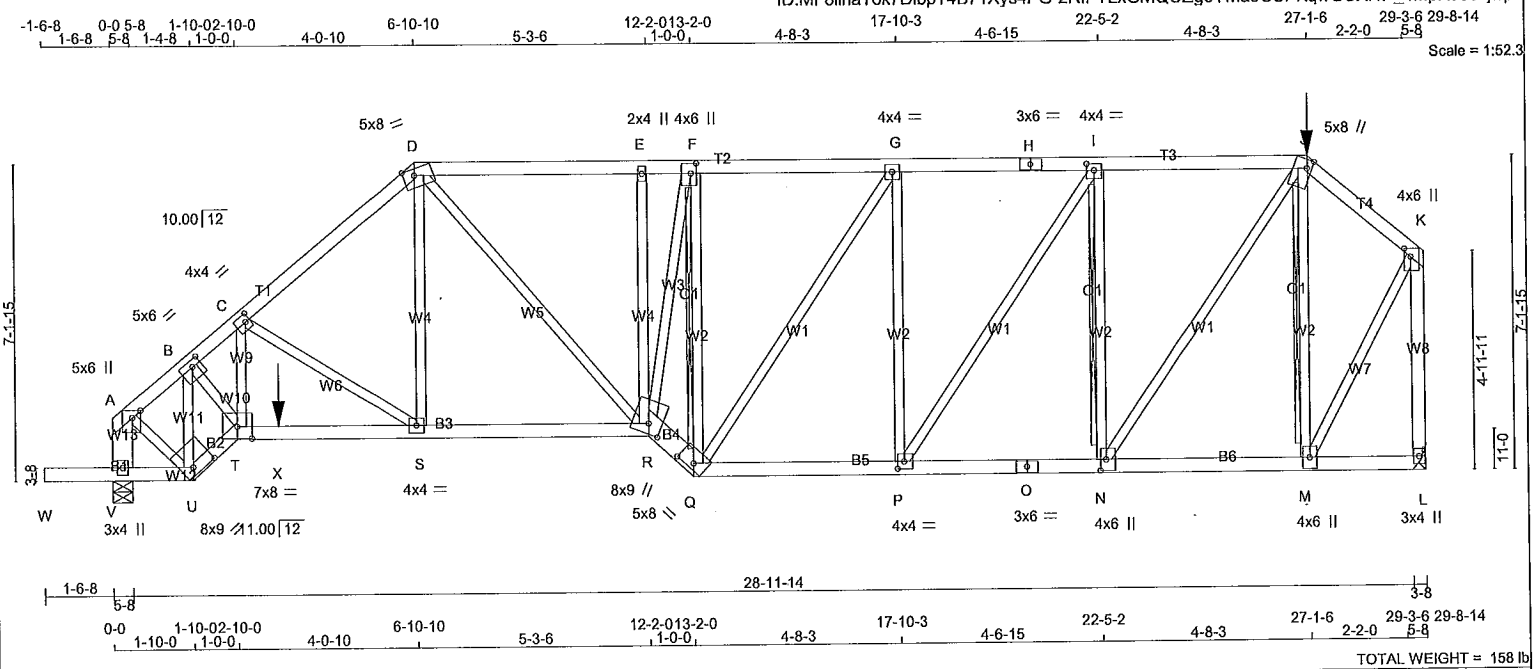
A-18023688

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292617	H1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:40:37 2018 Page 1

ID:MF8IlriaTok7DlbpT4B71Xys4PG-2RIFYLxGMQCZgoYMa9CSPKqwGUXl4P_4MpK9o6zjwpu



TOTAL WEIGHT = 158 lb [M]

LUMBER

N. L. G. A. RULES

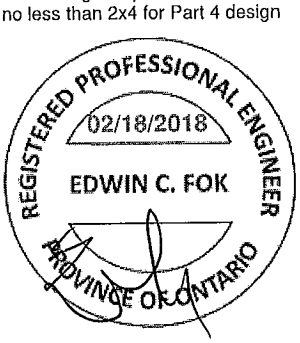
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
V - A	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x6	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	5.0	6.0	1.75	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW-m	MT20	5.0	8.0	Edge	3.00
E	TMW+w	MT20	2.0	4.0		
F	TMWW+t	MT20	4.0	6.0	2.75	1.50
G	TMWW-t	MT20	4.0	4.0		
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	1.75	2.00
J	TTWW+m	MT20	5.0	8.0	Edge	1.25
K	TMVW+p	MT20	4.0	6.0	2.25	1.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW+t	MT20	4.0	6.0		
N	BMWW+t	MT20	4.0	6.0	3.00	1.50
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	2.00	1.75
Q	BBWW-h	MT20	5.0	8.0	1.50	4.50
R	BBWWW+m	MT20	8.0	9.0	2.75	3.50
S	BMWW-t	MT20	4.0	4.0		
T	BBWW-l	MT20	7.0	8.0	3.25	4.00
U	BBWW-h	MT20	8.0	9.0	2.00	6.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	2327	0	2327	0	0	IN-SX 5-8	IN-SX 2-7
L	2108	0	2108	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-6	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1655	1034 / 0	0 / 0	0 / 0	0 / 0	622 / 0	0 / 0
L	1503	919 / 0	0 / 0	0 / 0	0 / 0	584 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-Q, I-N, J-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	-1866 / 0	-78.0	-78.0	0.13 (1)	4.77		U-B	-2083 / 0	0.39 (1)
B-C	-2914 / 0	-78.0	-78.0	0.24 (1)	3.82		B-T	0 / 1679	0.42 (1)
C-D	-2604 / 0	-78.0	-78.0	0.30 (1)	4.01		T-C	0 / 109	0.03 (1)
D-E	-2669 / 0	-78.0	-78.0	0.48 (1)	3.76		C-S	-365 / 0	0.15 (1)
E-F	-2664 / 0	-114.5	-114.5	0.34 (1)	3.86		S-D	0 / 597	0.15 (1)
F-G	-2330 / 0	-114.5	-114.5	0.41 (1)	4.04		D-R	0 / 1007	0.25 (1)
G-H	-2265 / 0	-114.5	-114.5	0.49 (1)	3.97		R-E	-253 / 0	0.16 (1)
H-I	-2265 / 0	-114.5	-114.5	0.49 (1)	3.97		R-F	0 / 2129	0.53 (1)
I-J	-1761 / 0	-114.5	-114.5	0.45 (1)	4.47		Q-F	-2448 / 0	0.91 (1)
J-K	-1022 / 0	-78.0	-78.0	0.12 (1)	6.04		Q-G	0 / 115	0.03 (1)
V-A	-2099 / 0	0.0	0.0	0.15 (1)	6.97		P-G	-615 / 0	0.56 (1)
L-K	-2088 / 0	0.0	0.0	0.95 (1)	5.81		P-I	0 / 907	0.22 (1)
							N-I	-1340 / 0	0.50 (1)
W-V	0 / 0	-96.5	-96.5	0.17 (1)	10.00		N-J	0 / 1768	0.44 (1)
V-U	0 / 0	-18.5	-18.5	0.17 (1)	10.00		M-J	-1209 / 0	0.45 (1)
U-T	0 / 1810	-18.5	-18.5	0.32 (1)	10.00		A-U	0 / 1642	0.41 (1)
T-X	0 / 2290	-18.5	-18.5	0.73 (1)	10.00		M-K	0 / 1541	0.38 (1)
X-S	0 / 2290	-63.7	-63.7	0.73 (1)	10.00				
S-R	0 / 1994	-63.7	-63.7	0.68 (1)	10.00				
R-Q	0 / 3318	-27.2	-27.2	0.43 (1)	10.00				
Q-P	0 / 2265	-27.2	-27.2	0.48 (1)	10.00				
P-O	0 / 1762	-27.2	-27.2	0.38 (1)	10.00				
O-N	0 / 1762	-27.2	-27.2	0.38 (1)	10.00				
N-M	0 / 764	-27.2	-27.2	0.20 (1)	10.00				
M-L	0 / 0	-27.2	-27.2	0.11 (4)	10.00				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-9-4
START SPAN CARRIED = 3-10-8
END DISTANCE = 12-2-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-10-10
RIGHT SETBACK = 2-7-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 17-6-14 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.15")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")

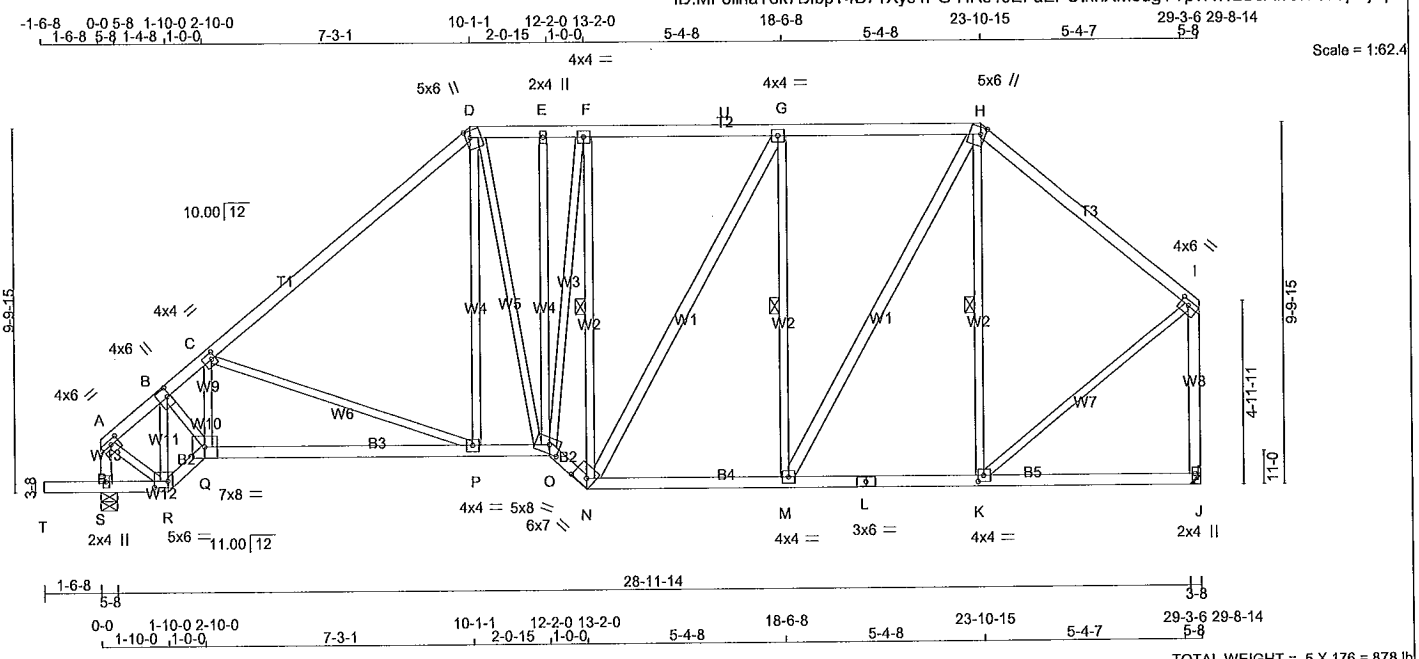
A-18023689

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292617	H2T	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:41:03 2018 Page 1
ID:MF8iiriaTok7DlbpT4B71Xys4PG-HRs40EFaEPUtnnXM6dgYTpvRWzU5ADcvLsdYjbzjwpU



[M][F]

LUMBER
N. L. G. A. RULES

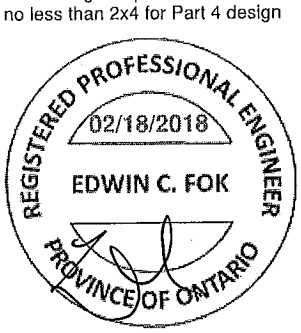
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - H	2x4	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW+t	MT20	4.0	6.0	3.00	1.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	5.0	6.0	2.25	1.50
I	TMVW-t	MT20	4.0	6.0	1.50	2.75
J	BMV1+p	MT20	2.0	4.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.75
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BBWW-h	MT20	6.0	7.0	2.25	4.50
O	BBWWW-m	MT20	5.0	8.0	2.75	3.50
P	BMWW-t	MT20	4.0	4.0		
Q	BBWW-t	MT20	7.0	8.0		
R	BBWW-t	MT20	5.0	6.0	2.00	4.50
S	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
S	1631	0	1631	0	0	5-8	2-7
J	1490	0	1490	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-10	

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1167	691 / 0	0 / 0	0 / 0	0 / 0	476 / 0	0 / 0
J	1067	623 / 0	0 / 0	0 / 0	0 / 0	445 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING
FOR SECTION D-H, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-N, G-M, H-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO						FR-TO		
A- B	-1218 / 0	-78.0	-78.0	0.13 (1)	5.68	R- B	-1349 / 0	0.23 (1)
B- C	-1849 / 0	-78.0	-78.0	0.47 (1)	4.38	B- Q	0 / 1342	0.30 (1)
C- D	-1554 / 0	-78.0	-78.0	0.61 (1)	4.62	Q- C	-248 / 0	0.04 (1)
D- E	-1248 / 0	-85.5	-85.5	0.11 (1)	2.00	C- P	-499 / 0	0.61 (1)
E- F	-1249 / 0	-85.5	-85.5	0.16 (1)	2.00	P- D	0 / 296	0.07 (4)
F- U	-1175 / 0	-85.5	-85.5	0.30 (1)	2.00	D- O	0 / 341	0.08 (1)
U- G	-1175 / 0	-85.5	-85.5	0.30 (1)	2.00	O- E	0 / 64	0.01 (1)
G- H	-1136 / 0	-85.5	-85.5	0.30 (1)	2.00	O- F	0 / 670	0.15 (1)
H- I	-982 / 0	-78.0	-78.0	0.36 (1)	5.83	N- F	-1078 / 0	0.70 (1)
S- A	-1403 / 0	0.0	0.0	0.15 (1)	6.87	N- G	0 / 78	0.01 (1)
J- I	-1446 / 0	0.0	0.0	0.65 (1)	6.80	M- G	-591 / 0	0.38 (1)
						M- H	0 / 791	0.13 (1)
T- S	0 / 0	-96.5	-96.5	0.16 (1)	10.00	K- H	-471 / 0	0.31 (1)
S- R	0 / 0	-18.5	-18.5	0.12 (1)	10.00	A- R	0 / 1053	0.24 (1)
R- Q	0 / 1160	-18.5	-18.5	0.19 (1)	10.00	K- I	0 / 955	0.21 (1)
Q- P	0 / 1633	-18.5	-18.5	0.39 (1)	10.00			
P- O	0 / 1167	-18.5	-18.5	0.35 (4)	10.00			
O- N	0 / 1578	-18.5	-18.5	0.26 (1)	10.00			
N- M	0 / 1136	-18.5	-18.5	0.25 (1)	10.00			
M- L	0 / 747	-18.5	-18.5	0.22 (4)	10.00			
L- K	0 / 747	-18.5	-18.5	0.22 (4)	10.00			
K- J	0 / 0	-18.5	-18.5	0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.65/1.00 (I-J:1) , BC=0.39/1.00 (P-Q:1) , WB=0.70/1.00 (F-N:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023690

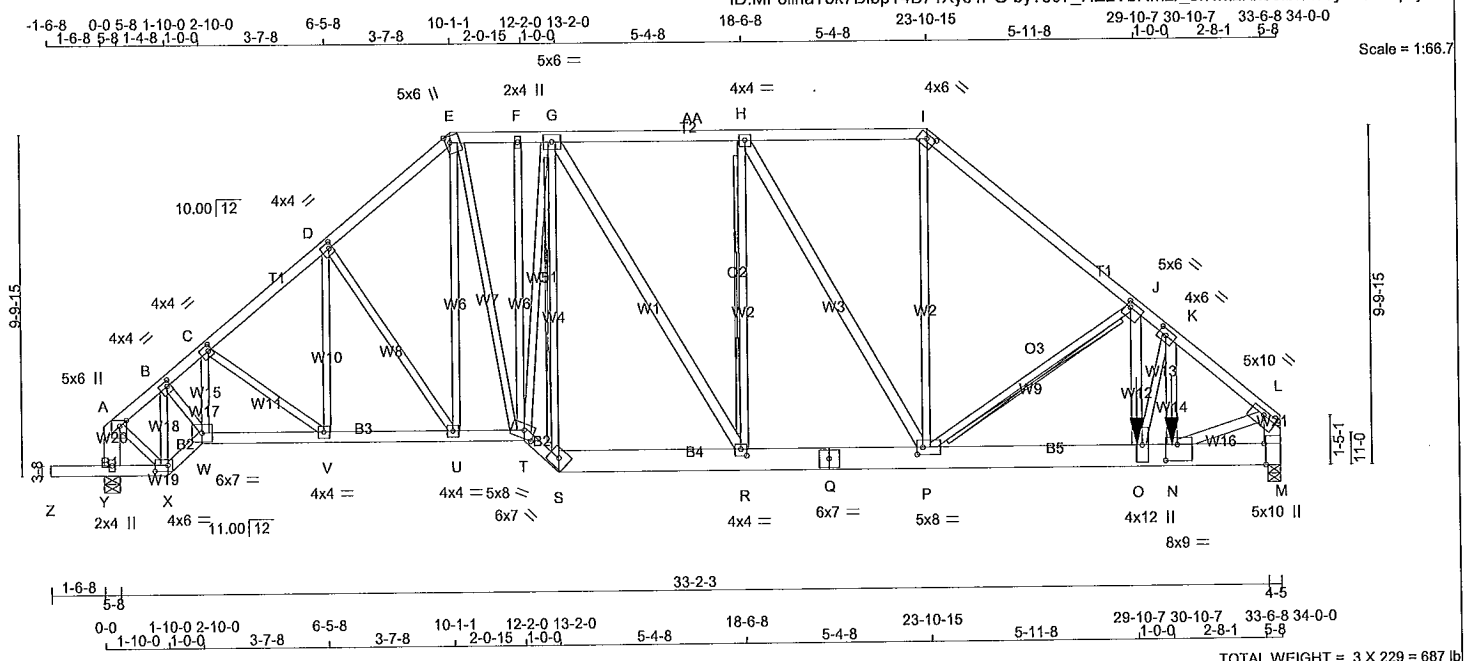
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292617	H2TA	1	3	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:43:26 2018 Page 1

ID:MF8iriaTok7DlbpT4B71Xys4PG-by?807_HZLVbRmzr_9k4mzKx0cE?As1yDcmcFpzjwnF



TOTAL WEIGHT = 3 X 229 = 687 lb

[M]

LUMBER

N. L. G. A. RULES

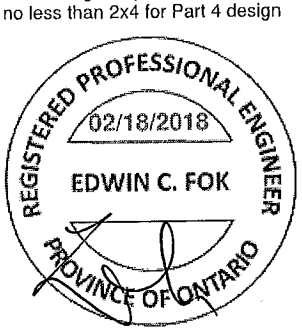
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
Y - A	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
Z - X	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - Q	2x8	DRY	No.2	SPF
Q - M	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - R	2x4	DRY	No.2	SPF
H - P	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- I	1	12	TOP
I- L	1	12	TOP
Y- A	2	12	TOP
M- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Z- X	1	12	TOP
X- W	1	12	TOP
W- T	1	12	TOP
T- S	1	12	TOP
S- Q	2	12	TOP
Q- M	3	5	SIDE(1379.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
J- O	2	4	SIDE(758.2)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	3229	0	3229	0	0	5-8	1-8
M	13644	0	13644	0	0	4-5	4-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2282	1505 / 0	0 / 0	0 / 0	0 / 0	777 / 0	0 / 0
M	9530	6929 / 0	0 / 0	0 / 0	0 / 0	2601 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, M

BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.

FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.96 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT G-S, H-R, J-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2697 / 0	-78.0	-78.0	0.03 (1)	6.25
B- C	-4244 / 0	-78.0	-78.0	0.04 (1)	5.46
C- D	-4173 / 0	-78.0	-78.0	0.07 (1)	5.46
D- E	-3976 / 0	-78.0	-78.0	0.07 (1)	5.57
E- F	-3501 / 0	-85.5	-85.5	0.07 (1)	2.00
F- G	-3503 / 0	-85.5	-85.5	0.11 (1)	2.00
G-AA	-4287 / 0	-85.5	-85.5	0.14 (1)	2.00
AA- H	-4287 / 0	-85.5	-85.5	0.14 (1)	2.00
H- I	-4855 / 0	-85.5	-85.5	0.15 (1)	2.00
I- J	-6312 / 0	-78.0	-78.0	0.31 (1)	4.50
J- K	-13599 / 0	-78.0	-78.0	0.35 (1)	3.03
K- L	-14319 / 0	-78.0	-78.0	0.42 (1)	2.96
Y- A	-3000 / 0	0.0	0.0	0.07 (1)	7.81
M- L	-12593 / 0	0.0	0.0	0.27 (1)	5.26
Z- Y	0 / 0	-96.5	-96.5	0.05 (1)	10.00
Y- X	0 / 0	-18.5	-18.5	0.05 (1)	10.00
X- W	0 / 2623	-18.5	-18.5	0.14 (1)	10.00
W- V	0 / 3291	-18.5	-18.5	0.19 (1)	10.00
V- U	0 / 3217	-18.5	-18.5	0.20 (1)	10.00
U- T	0 / 3036	-18.5	-18.5	0.19 (1)	10.00
T- S	0 / 4477	-18.5	-18.5	0.24 (1)	10.00
S- R	0 / 3399	-18.5	-18.5	0.13 (1)	10.00
R- Q	0 / 4287	-18.5	-18.5	0.18 (1)	10.00
Q- P	0 / 4287	-18.5	-18.5	0.18 (1)	10.00
P- O	0 / 10725	-18.5	-18.5	0.49 (1)	10.00
O- N	0 / 10960	-18.5	-18.5	0.62 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
X- B	-3001 / 0	0.17 (1)
B- W	0 / 2368	0.18 (1)
W- C	-103 / 0	0.01 (1)
C- V	-93 / 0	0.01 (1)
V- D	0 / 94	0.01 (4)
D- U	-332 / 0	0.09 (1)
U- E	0 / 332	0.02 (1)
E- T	0 / 1969	0.15 (1)
T- F	0 / 306	0.02 (1)
T- G	0 / 702	0.05 (1)
S- G	-2822 / 0	0.54 (1)
G- R	0 / 1760	0.09 (1)
R- H	-1501 / 0	0.27 (1)
H- P	0 / 1160	0.06 (1)
P- I	0 / 3642	0.27 (1)
P- J	-7287 / 0	0.83 (1)
O- J	0 / 8283	0.44 (1)
O- K	-866 / 0	0.07 (1)
N- K	0 / 1514	0.08 (1)
A- X	0 / 2378	0.18 (1)
N- L	0 / 11434	0.61 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-10-6
START SPAN CARRIED = 14-0-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 14-0-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.42/1.00 (K-L:1) , BC=0.62/1.00 (N-O:1) , WB=0.83/1.00 (J-P:1) , SSI=0.19/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

A-18023691

CONTINUED ON PAGE 2

K- N 2 4 SIDE(682.0)
2x4 1 6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

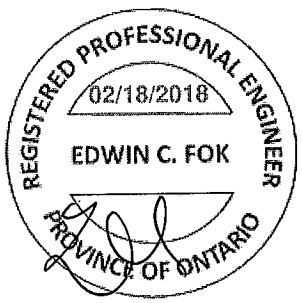
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	2.00	2.25
B	TMWW-t	MT20	4.0	4.0	1.75	1.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTW-h	MT20	4.0	6.0	1.50	3.00
J	TMWW-t	MT20	5.0	6.0	1.75	1.50
K	TMWW-t	MT20	4.0	6.0	2.00	2.75
L	TMVW-t	MT20	5.0	10.0	1.50	5.00
M	BMV1+t	MT20	5.0	10.0	Edge	0.50
N	BMWW-t	MT20	8.0	9.0	5.50	4.00
O	BMWW+t	MT20	4.0	12.0		
P	BMWWW-t	MT20	5.0	8.0	2.50	2.00
Q	BS-t	MT20	6.0	7.0		
R	BMWW-t	MT20	4.0	4.0	2.25	2.00
S	BBW-h	MT20	6.0	7.0		
T	BBWWW-m	MT20	5.0	8.0	2.75	3.50
U	BMWW-t	MT20	4.0	4.0		
V	BMWW-t	MT20	4.0	4.0		
W	BBWW-l	MT20	6.0	7.0		
X	BBWW-l	MT20	4.0	6.0	2.00	4.50
Y	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6742.7 lbs FACTORED DOWN AT 29-10-6, AND 5688.9 lbs FACTORED DOWN AT 30-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)		MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO						
N- M	0 / 0	-308.9	-308.9	0.27	(1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	30-10-6	-5689	-5689	---	FRONT	VERT	TOTAL
O	29-10-6	-6743	-6743	---	FRONT	VERT	TOTAL

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

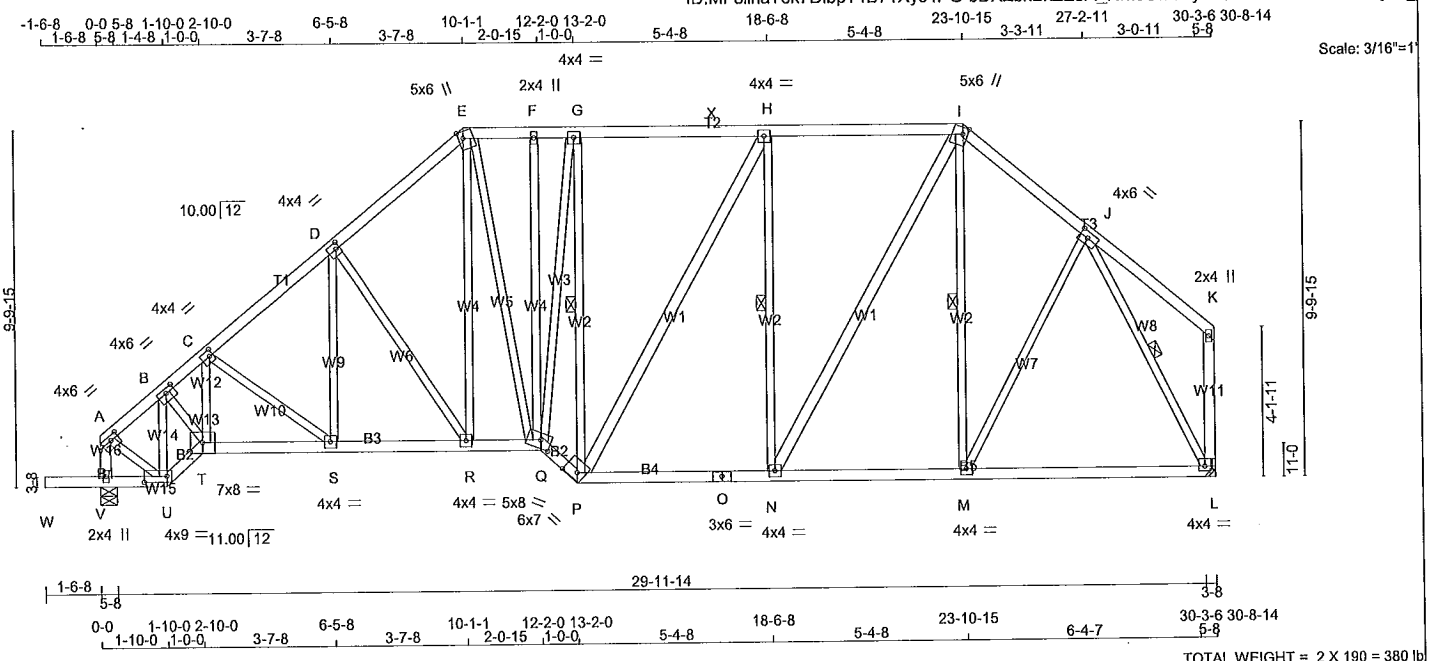
JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.76 (L) (INPUT = 1.00)

A-1802369(1)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292617	H3T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8iiriaTok7DlbpT4B71Xys4PG-bDXZbxBxZZeA_Nm6UDY4yZXtWT7Hfa4S8lO?LKzjwn_



TOTAL WEIGHT = 2 X 190 = 380 lb {M}{F}

LUMBER

N. L. G. A. RULES

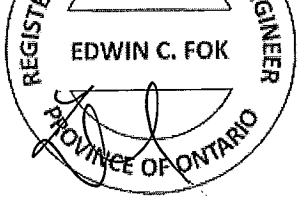
CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.50	2.75
B	TMWW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMW+w	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMWW-t	MT20	4.0	6.0	2.00	2.75
K	TMV+p	MT20	2.0	4.0		
L	BMVW1-t	MT20	4.0	4.0	1.50	2.00
M, N, R, S						
M	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BBWW-h	MT20	6.0	7.0	2.25	4.50
Q	BBWWW-m	MT20	5.0	8.0	2.75	3.50
T	BBWW-l	MT20	7.0	8.0		
U	BBWW-l	MT20	4.0	9.0	2.00	7.50
V	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
V	1682	0	1682	0	0	IN-SX	IN-SX
L	1536	0	1536	0	0	5-8	2-8
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-11	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1203	712 / 0	0 / 0	0 / 0	0 / 0	491 / 0	0 / 0
L	1100	644 / 0	0 / 0	0 / 0	0 / 0	457 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-P, H-N, I-M, J-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1275 / 0	-78.0	-78.0	0.06 (1)	5.66
B- C	-1982 / 0	-78.0	-78.0	0.08 (1)	4.75
C- D	-1838 / 0	-78.0	-78.0	0.15 (1)	4.83
D- E	-1601 / 0	-78.0	-78.0	0.15 (1)	5.10
E- F	-1318 / 0	-85.5	-85.5	0.11 (1)	2.00
F- G	-1319 / 0	-85.5	-85.5	0.17 (1)	2.00
G- X	-1244 / 0	-85.5	-85.5	0.30 (1)	2.00
X- H	-1244 / 0	-85.5	-85.5	0.30 (1)	2.00
H- I	-1233 / 0	-85.5	-85.5	0.30 (1)	2.00
I- J	-1165 / 0	-78.0	-78.0	0.12 (1)	5.79
J- K	0 / 19	-78.0	-78.0	0.14 (1)	10.00
V- A	-1453 / 0	0.0	0.0	0.15 (1)	6.78
L- K	-105 / 0	0.0	0.0	0.03 (1)	7.81
W- V	0 / 0	-96.5	-96.5	0.16 (1)	10.00
V- U	0 / 0	-18.5	-18.5	0.12 (1)	10.00
U- T	0 / 1241	-18.5	-18.5	0.20 (1)	10.00
T- S	0 / 1545	-18.5	-18.5	0.28 (1)	10.00
S- R	0 / 1421	-18.5	-18.5	0.25 (1)	10.00
R- Q	0 / 1216	-18.5	-18.5	0.22 (1)	10.00
Q- P	0 / 1672	-18.5	-18.5	0.27 (1)	10.00
P- O	0 / 1233	-18.5	-18.5	0.26 (1)	10.00
O- N	0 / 1233	-18.5	-18.5	0.26 (1)	10.00
N- M	0 / 875	-18.5	-18.5	0.28 (4)	10.00
M- L	0 / 712	-18.5	-18.5	0.26 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- B	-1440 / 0	0.25 (1)
B- T	0 / 1100	0.25 (1)
T- C	-14 / 21	0.01 (4)
C- S	-153 / 0	0.05 (1)
S- D	0 / 151	0.04 (4)
D- R	-375 / 0	0.31 (1)
R- E	0 / 359	0.08 (1)
E- Q	0 / 431	0.10 (1)
Q- F	0 / 70	0.02 (1)
Q- G	0 / 680	0.15 (1)
P- G	-1092 / 0	0.71 (1)
P- H	0 / 23	0.00 (1)
N- H	-542 / 0	0.35 (1)
N- I	0 / 728	0.12 (1)
M- I	-184 / 0	0.12 (1)
M- J	0 / 357	0.08 (1)
A- U	0 / 1126	0.25 (1)
J- L	-1552 / 0	0.57 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.02")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (1.02")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.30/1.00 (H-I:1) , BC=0.28/1.00 (S-T:1) , WB=0.71/1.00 (G-P:1) , SSI=0.22/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

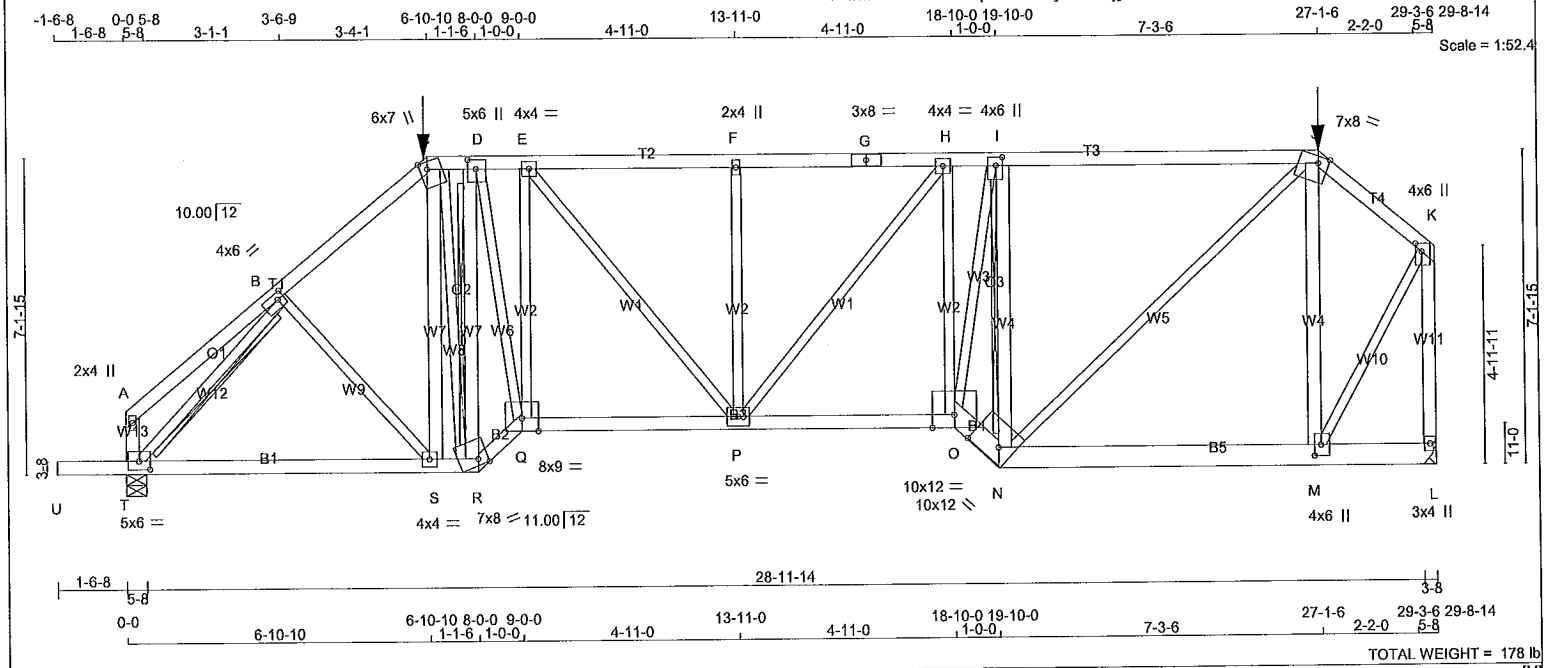
A-18023692

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292618	H1TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:52:11 2018 Page 1
ID:MF8IiriaTok7DlbpT4B71Xys4PG-jyEcCeKbwfe5wRckEvfCXhaY6FOPhlVEynRuAtzjwf2



TOTAL WEIGHT = 178 lb [M]

LUMBER
N. L. G. A. RULES

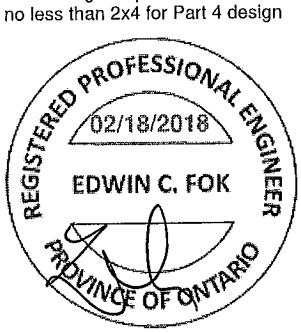
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	2100F 1.8E	SPF
G - J	2x4	DRY	2100F 1.8E	SPF
J - K	2x4	DRY	No.2	SPF
T - A	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
R - D	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	1.75	1.75
C	TTWW+m	MT20	6.0	7.0	2.00	2.00
D	TMWW+t	MT20	5.0	6.0	2.50	2.25
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	8.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	4.0	6.0	2.25	1.75
J	TTWW-m	MT20	7.0	8.0	Edge	2.75
K	TMVW+p	MT20	4.0	6.0	2.25	1.75
L	BMV1+p	MT20	3.0	4.0		
M	BMWW+t	MT20	4.0	6.0	3.00	1.75
N	BBWW-h	MT20	10.0	12.0	3.75	8.00
O	BBWW-l	MT20	10.0	12.0	Edge	
P	BMWWW-t	MT20	5.0	6.0		
Q	BBWW-l	MT20	8.0	9.0	Edge	
R	BBWW-m	MT20	7.0	8.0	Edge	2.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	2113	0	2113	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-6	
T	2203	0	2203	0	0	5-8	2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1506	920 / 0	0 / 0	0 / 0	0 / 0	586 / 0	0 / 0
T	1572	953 / 0	0 / 0	0 / 0	0 / 0	619 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-R, I-N, B-T

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-78.0	-78.0	0.13 (1)	10.00	
B- C	-2268 / 0	-78.0	-78.0	0.22 (1)	4.32	
C- D	-1872 / 0	-114.5	-114.5	0.03 (1)	5.82	
D- E	-2306 / 0	-114.5	-114.5	0.06 (1)	5.36	
E- F	-2753 / 0	-78.0	-78.0	0.28 (1)	4.75	
F- G	-2753 / 0	-78.0	-78.0	0.28 (1)	4.75	
G- H	-2753 / 0	-78.0	-78.0	0.28 (1)	4.75	
H- I	-2535 / 0	-114.5	-114.5	0.64 (1)	4.14	
I- J	-2209 / 0	-114.5	-114.5	0.75 (1)	4.41	
J- K	-1062 / 0	-78.0	-78.0	0.12 (1)	5.96	
T- A	-107 / 0	0.0	0.0	0.01 (1)	7.81	
L- K	-2121 / 0	0.0	0.0	0.89 (1)	5.77	
U- T	0 / 0	-96.5	-96.5	0.17 (1)	10.00	
T- S	0 / 1627	-27.2	-27.2	0.54 (4)	10.00	
S- R	0 / 1723	-27.2	-27.2	0.55 (4)	10.00	
R- Q	0 / 2585	-27.2	-27.2	0.46 (1)	10.00	
Q- P	0 / 2319	-63.7	-63.7	0.67 (1)	10.00	
P- O	0 / 2534	-63.7	-63.7	0.71 (1)	10.00	
O- N	0 / 2847	-27.2	-27.2	0.37 (1)	10.00	
N- M	0 / 788	-27.2	-27.2	0.22 (4)	10.00	
M- L	0 / 0	-27.2	-27.2	0.16 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- S	0 / 144	0.04 (4)
S- C	0 / 223	0.06 (4)
C- R	0 / 902	0.22 (1)
R- D	-2720 / 0	0.79 (1)
D- Q	0 / 2625	0.65 (1)
Q- E	-701 / 0	0.44 (1)
E- P	0 / 691	0.17 (1)
P- F	-479 / 0	0.30 (1)
P- H	0 / 349	0.09 (1)
O- H	-19 / 42	0.01 (4)
O- I	0 / 2047	0.51 (1)
N- I	-3091 / 0	0.86 (1)
N- J	0 / 1934	0.48 (1)
M- J	-1141 / 0	0.69 (1)
T- B	-2481 / 0	0.59 (1)
M- K	0 / 1563	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-10-10	-213	-213	---	FRONT	VERT	TOTAL
J	27-1-6	-81	-81	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-10-10
RIGHT SETBACK = 2-7-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 9-0-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 9-0-0
START SPAN CARRIED = 3-10-8
END DISTANCE = 18-10-0
END SPAN CARRIED = 3-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-10-10
RIGHT SETBACK = 2-7-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 10-10-14 OF SPAN MEASURED FROM THE RIGHT.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")

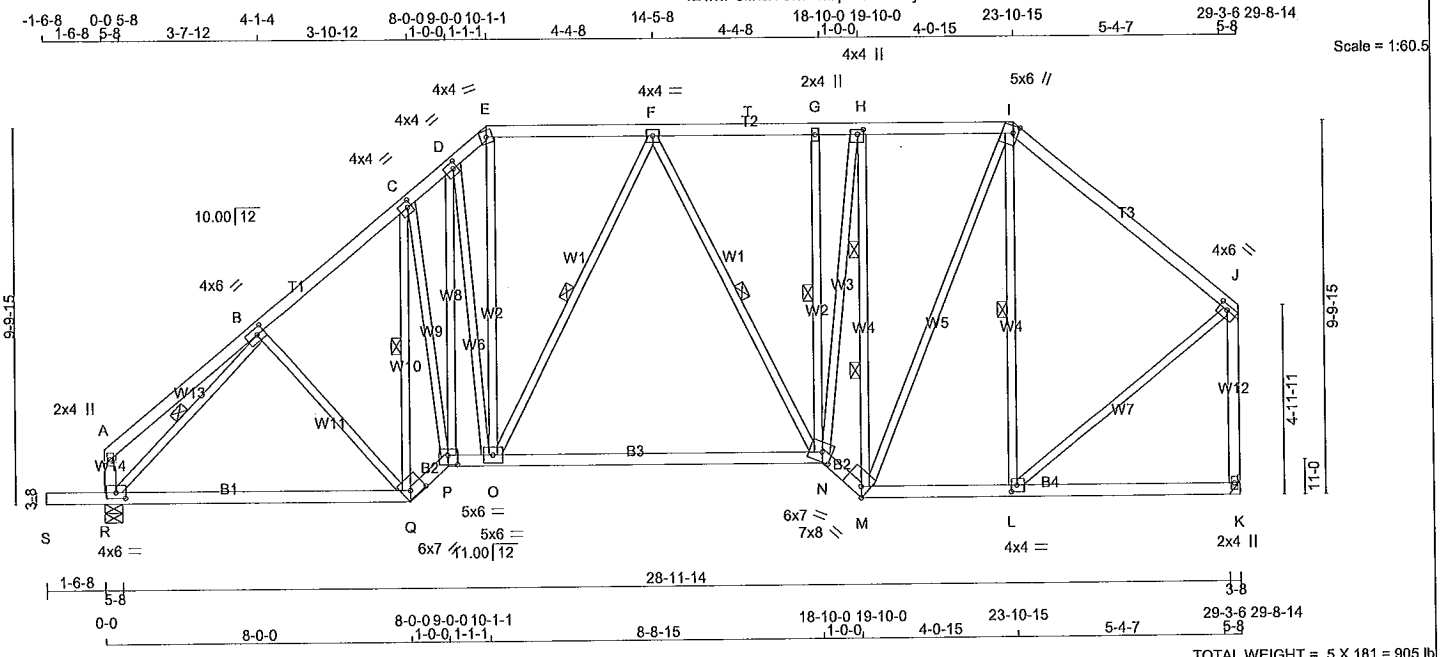
A-18023693

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292618	H2TB	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:MF8iiriaTok7DlbpT4B71Xys4PG-8oRAPUa8Do9GKW8aP60uLvP0sJHWNJTAYuHysjzjwek



LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

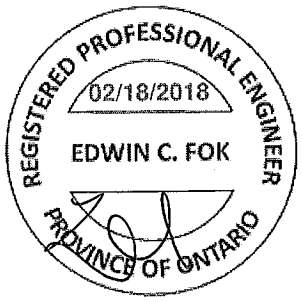
DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	6.0	2.00	2.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
D	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	4.0	1.50	1.75
I	TTWW+m	MT20	5.0	6.0	2.25	1.50
J	TMVW-t	MT20	4.0	6.0	1.50	2.75
K	BMV1+p	MT20	2.0	4.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BBWW-h	MT20	7.0	8.0	2.50	Edge
N	BBWWW-m	MT20	6.0	7.0	3.00	3.00
O	BMWWW-t	MT20	5.0	6.0		
P	BBWW-l	MT20	5.0	6.0	3.00	3.00
Q	BBWW-h	MT20	6.0	7.0	2.25	4.50
R	BMVW1-t	MT20	4.0	6.0	1.75	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1494	0	1494	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-10	
R	1628	0	1628	0	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1070	624 / 0	0 / 0	0 / 0	0 / 0	446 / 0	0 / 0
R	1164	689 / 0	0 / 0	0 / 0	0 / 0	475 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-Q, F-O, F-N, G-N, I-L, B-R.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-78.0	-78.0	0.18 (1)	10.00
B- C	-1492 / 0	-78.0	-78.0	0.17 (1)	5.22
C- D	-1693 / 0	-78.0	-78.0	0.16 (1)	4.97
D- E	-1456 / 0	-78.0	-78.0	0.15 (1)	5.28
E- F	-1187 / 0	-85.5	-85.5	0.21 (1)	2.00
F- T	-1220 / 0	-85.5	-85.5	0.21 (1)	2.00
T- G	-1220 / 0	-85.5	-85.5	0.21 (1)	2.00
G- H	-1219 / 0	-85.5	-85.5	0.16 (1)	2.00
H- I	-1068 / 0	-85.5	-85.5	0.16 (1)	2.00
I- J	-985 / 0	-78.0	-78.0	0.36 (1)	5.82
R- A	-123 / 0	0.0	0.0	0.01 (1)	7.81
K- J	-1450 / 0	0.0	0.0	0.65 (1)	6.79
S- R	0 / 0	-96.5	-96.5	0.16 (1)	10.00
R- Q	0 / 1162	-18.5	-18.5	0.46 (4)	10.00
Q- P	0 / 1559	-18.5	-18.5	0.25 (1)	10.00
P- O	0 / 1200	-18.5	-18.5	0.48 (4)	10.00
O- N	0 / 1315	-18.5	-18.5	0.49 (4)	10.00
N- M	0 / 1424	-18.5	-18.5	0.23 (1)	10.00
M- L	0 / 750	-18.5	-18.5	0.23 (4)	10.00
L- K	0 / 0	-18.5	-18.5	0.15 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- Q	-31 / 37	0.02 (1)
Q- C	-944 / 0	0.38 (1)
C- P	0 / 522	0.12 (1)
P- D	0 / 449	0.10 (1)
D- O	-209 / 4	0.28 (1)
O- E	0 / 680	0.15 (1)
O- F	-289 / 0	0.19 (1)
F- N	-211 / 0	0.14 (1)
N- G	-90 / 0	0.05 (1)
N- H	0 / 1325	0.30 (1)
M- H	-1668 / 0	0.99 (1)
M- I	0 / 805	0.18 (1)
L- I	-476 / 0	0.31 (1)
R- B	-1733 / 0	0.43 (1)
L- J	0 / 959	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.99")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.99")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.65/1.00 (J-K:1) , BC=0.49/1.00 (N-O:4) , WB=0.99/1.00 (H-M:1) , SSI=0.19/1.00 (O-P:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.43 (B) (INPUT = 1.00)

A-18023694

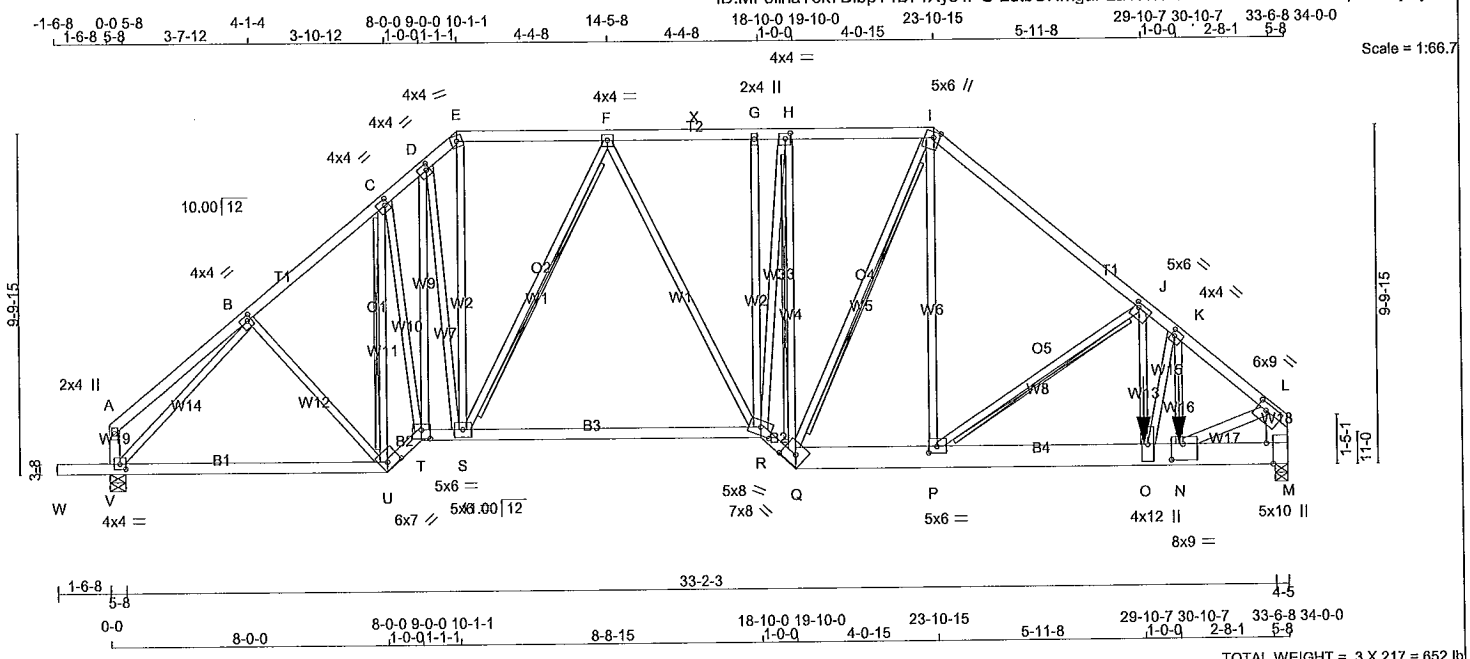
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292618	H2TC	1	3	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 11:55:32 2018 Page 1

ID:MF8liriaTok7DlbpT4B71Xys4PG-LdtbONmgaPzaYKTNWdmKB4P?QG0kHCfpAYe0jkzjwbv



TOTAL WEIGHT = 3 X 217 = 652 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
M - L	2x8	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - M	2x8	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

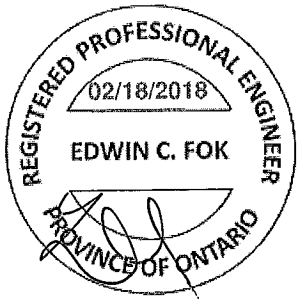
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- E	1	12	TOP
E- I	1	12	TOP
I- L	1	12	TOP
V- A	1	12	TOP
M- L	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
W- U	1	12	TOP
U- T	1	12	TOP
T- R	1	12	TOP
R- Q	1	12	TOP
Q- M	3	4	SIDE(1922.8)
WEBS : (0.122"X3") SPIRAL NAILS			
J- O	1	6	SIDE(269.9)
K- N	1	6	SIDE(193.7)
2x3	1	6	
2x4	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
M	13648	0	13648	0	0	4-5	4-5
V	3225	0	3225	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	9532	6930 / 0	0 / 0	0 / 0	0 / 0	2602 / 0	0 / 0
V	2279	1504 / 0	0 / 0	0 / 0	0 / 0	776 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, V
BEARING SIZE FACTOR = 1.15 AT JNT(S) M (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT C-U, F-S, H-Q, I-Q, J-P

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 18	-78.0	-78.0	0.05 (1)		10.00
B- C	-3643 / 0	-78.0	-78.0	0.08 (1)		5.75
C- D	-4246 / 0	-78.0	-78.0	0.18 (1)		5.29
D- E	-3755 / 0	-78.0	-78.0	0.17 (1)		5.54
E- F	-3081 / 0	-85.5	-85.5	0.10 (1)		2.00
F- X	-4710 / 0	-85.5	-85.5	0.12 (1)		2.00
X- G	-4710 / 0	-85.5	-85.5	0.12 (1)		2.00
G- H	-4712 / 0	-85.5	-85.5	0.12 (1)		2.00
H- I	-4440 / 0	-85.5	-85.5	0.13 (1)		2.00
I- J	-6290 / 0	-78.0	-78.0	0.33 (1)		4.50
J- K	-13557 / 0	-78.0	-78.0	0.36 (1)		3.03
K- L	-14296 / 0	-78.0	-78.0	0.44 (1)		2.93
V- A	-127 / 0	0.0	0.0	0.00 (1)		7.81
M- L	-12581 / 0	0.0	0.0	0.21 (1)		5.94
W- V	0 / 0	-96.5	-96.5	0.05 (1)		10.00
V- U	0 / 2605	-18.5	-18.5	0.21 (1)		10.00
U- T	0 / 3831	-18.5	-18.5	0.21 (1)		10.00
T- S	0 / 3053	-18.5	-18.5	0.24 (1)		10.00
S- R	0 / 4000	-18.5	-18.5	0.29 (1)		10.00
R- Q	0 / 5835	-18.5	-18.5	0.31 (1)		10.00
Q- P	0 / 4848	-18.5	-18.5	0.17 (1)		10.00
P- O	0 / 10731	-18.5	-18.5	0.49 (1)		10.00
O- N	0 / 10934	-18.5	-18.5	0.63 (1)		10.00
N- M	0 / 0	-308.9	-308.9	0.27 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- U	0 / 285	0.02 (1)
U- C	-2707 / 0	0.34 (1)
C- T	0 / 1911	0.14 (1)
T- D	0 / 644	0.05 (1)
D- S	0 / 62	0.01 (4)
S- E	0 / 2015	0.15 (1)
S- F	-2071 / 0	0.42 (1)
F- R	0 / 1566	0.12 (1)
R- G	0 / 128	0.01 (1)
R- H	0 / 2321	0.17 (1)
Q- H	-2847 / 0	0.55 (1)
Q- I	-1025 / 0	0.23 (1)
P- I	0 / 4627	0.35 (1)
P- J	-7318 / 0	0.83 (1)
O- J	0 / 8238	0.62 (1)
O- K	-744 / 0	0.06 (1)
N- K	0 / 1467	0.11 (1)
V- B	-3886 / 0	0.77 (1)
N- L	0 / 11406	0.61 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

GIRDER TYPE: CStdGirder
START DISTANCE = 30-10-6
START SPAN CARRIED = 14-0-8
END DISTANCE = 34-0-0
END SPAN CARRIED = 14-0-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.13")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (1.13")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.44/1.00 (K-L:1) , BC=0.63/1.00 (N-O:1) , WB=0.83/1.00 (J-P:1) , SSI=0.19/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

A-18023695

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292618	H2TC	1	3	TRUSS DESC.	

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

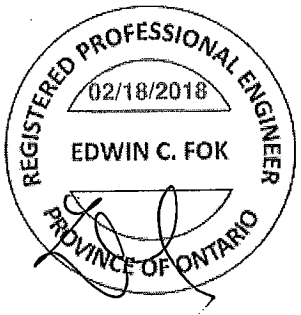
JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW-t	MT20	4.0	4.0	1.75	1.50
C, D, K						
C	TMWW-t	MT20	4.0	4.0	2.00	1.25
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.75
I	TTWW+m	MT20	5.0	6.0	2.00	2.00
J	TMWW-t	MT20	5.0	6.0	1.50	1.50
L	TMVW-t	MT20	6.0	9.0	2.25	3.25
M	BMV1+t	MT20	5.0	10.0	Edge	2.25
N	BMWW-t	MT20	8.0	9.0	5.50	4.00
O	BMWW+t	MT20	4.0	12.0		
P	BMWW-t	MT20	5.0	6.0	2.25	2.75
Q	BBWW-h	MT20	7.0	8.0	3.25	4.75
R	BBWWW-m	MT20	5.0	8.0	2.75	4.00
S	BMWWW-t	MT20	5.0	6.0		
T	BBWW-t	MT20	5.0	6.0	3.00	3.00
U	BBWW-h	MT20	6.0	7.0	2.25	4.50
V	BMVW1-t	MT20	4.0	4.0	1.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 6742.7 lbs FACTORED DOWN AT 29-10-6, AND 5688.9 lbs FACTORED DOWN AT 30-10-6 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
N	30-10-6	-5689	-5689	---	FRONT	VERT	TOTAL
O	29-10-6	-6743	-6743	---	FRONT	VERT	TOTAL

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

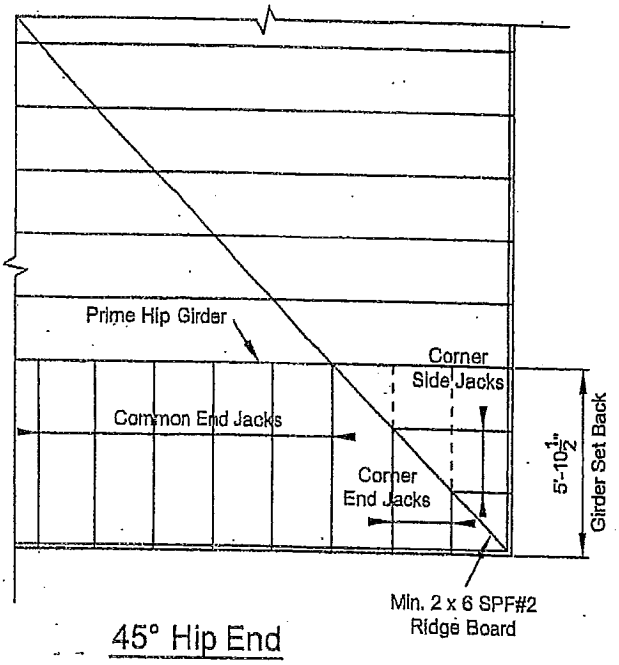
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (V) (INPUT = 0.90)
JSI METAL= 0.80 (L) (INPUT = 1.00)

A-18023695(2)

STRACON ENGINEERING INC.



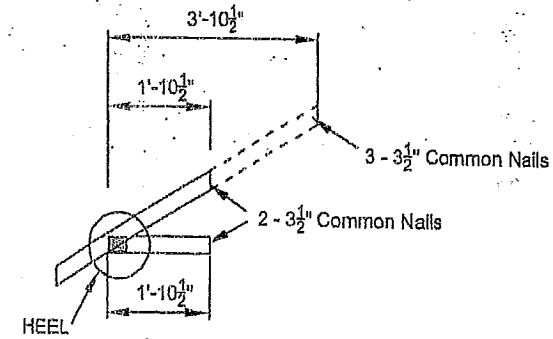
45° Hip End

LUMBER SPECIFICATION

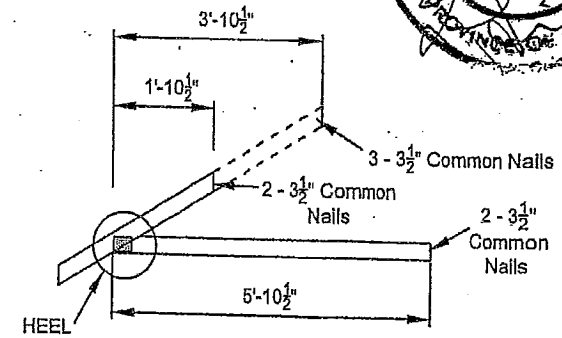
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

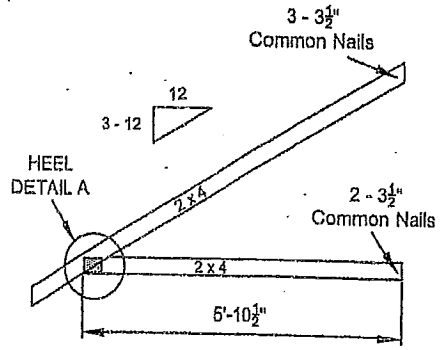
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



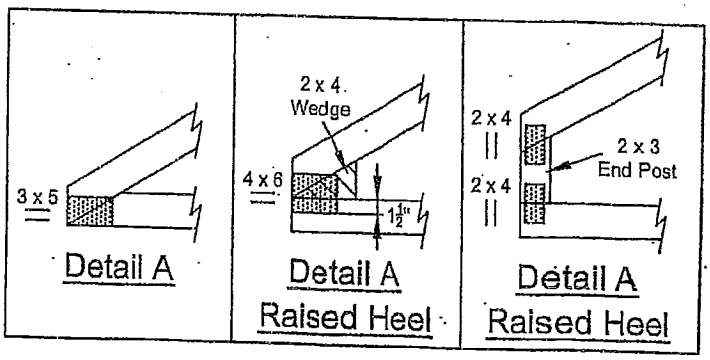
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

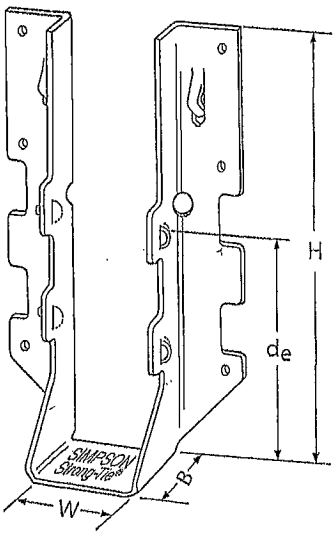
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

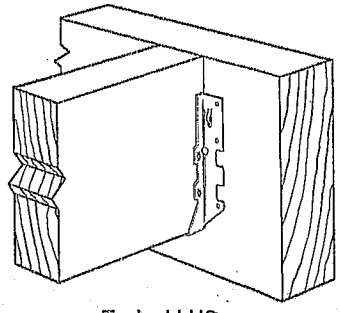
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

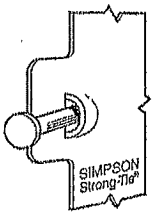


LUS28


Typical LUS Installation

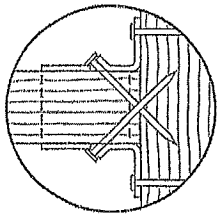
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

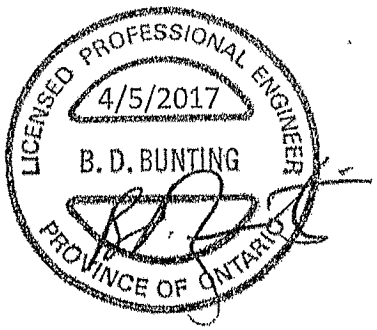
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective Until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty, or see strongtie.com.

T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

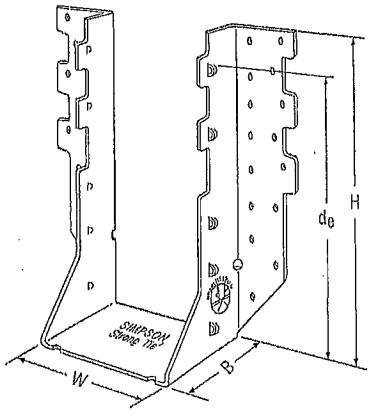
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



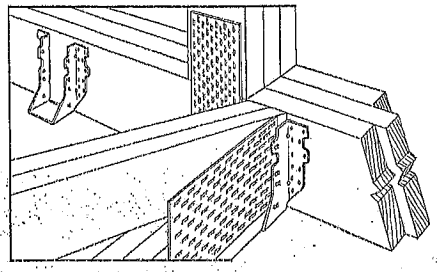
HHUS410

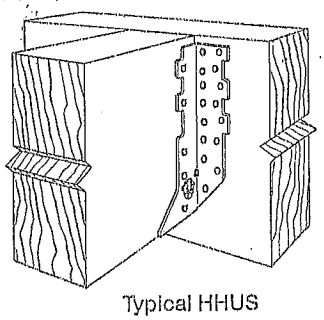
Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

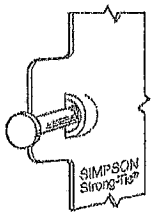
- See current catalogue for options


Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)


Typical HHUS Installation

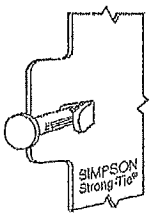
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

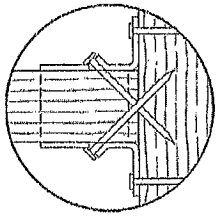


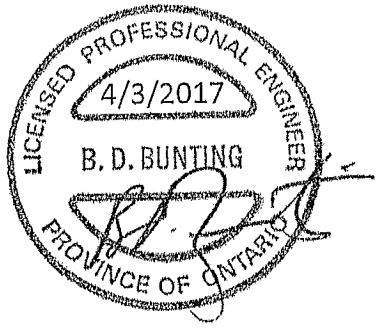
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

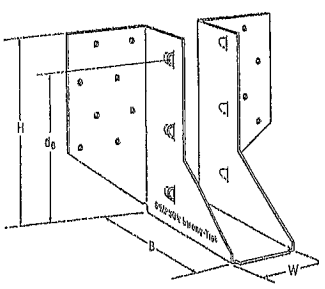
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

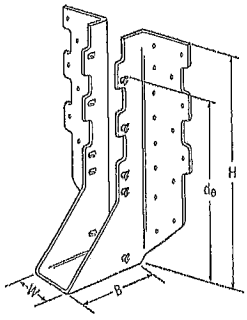
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

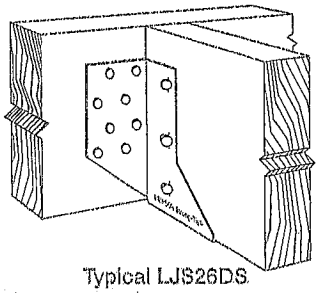
- See current catalogue for options

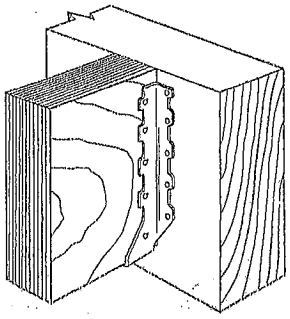


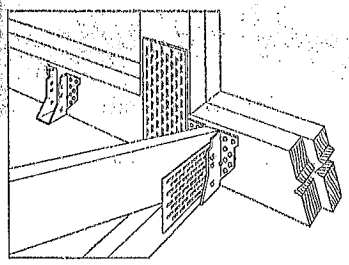
LJS26DS



HUS210
(HUS26, HUS28, similar)

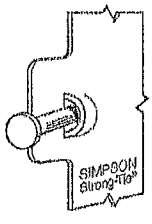

Typical LJS26DS Installation


Typical HUS Installation


Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

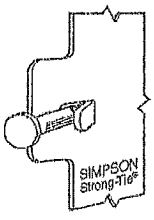
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 9/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

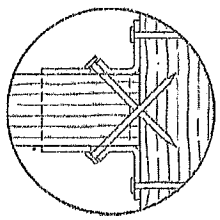


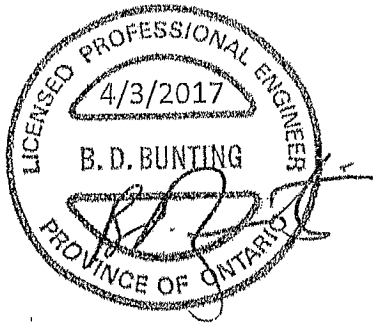
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

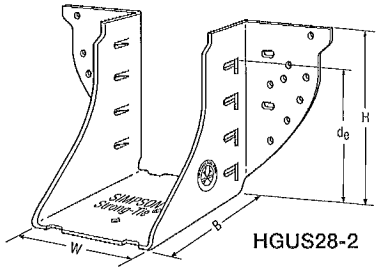
(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



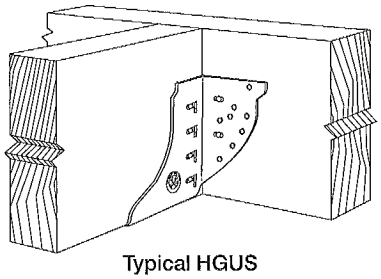
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

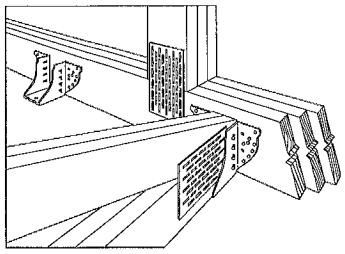
Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.


Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

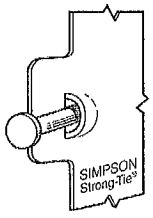

Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

- See current catalogue for options

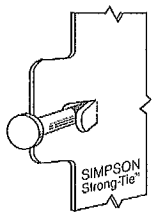
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	5½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 5/16	7 5/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 5/16	9 5/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 5/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 5/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_e is the distance from the seat of the hanger to the highest joist nail.

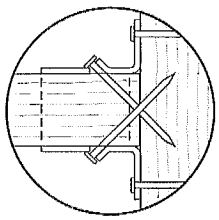


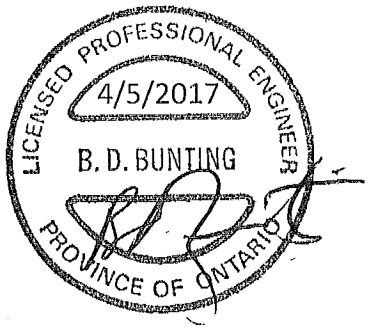
Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.


Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

(800) 999-5099
strongtie.com