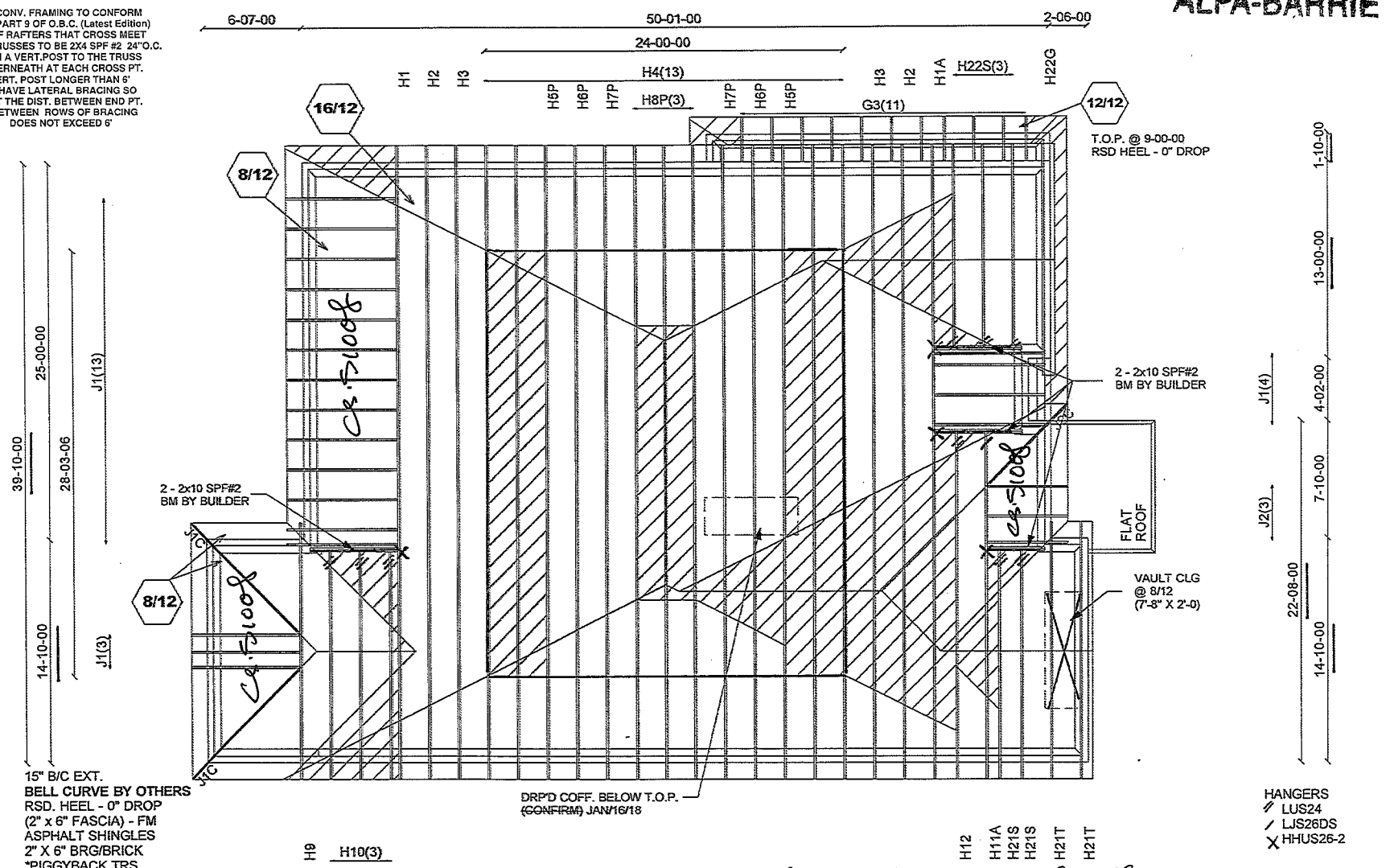
ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

A-18023873, A-18023875 - A-18023883
A-18023906 - A-18023912

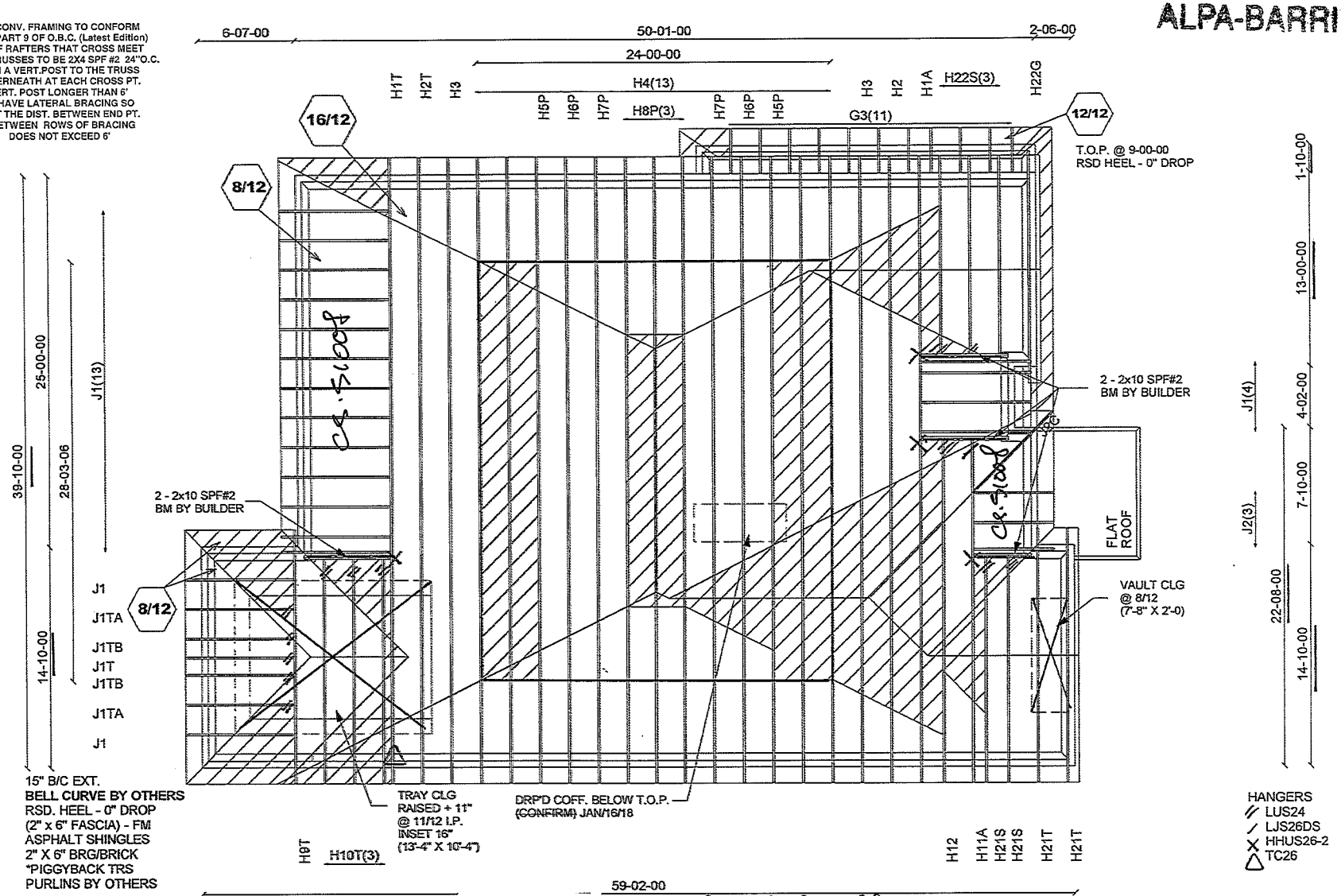
CONVENTIONAL FRAMING BY OTHERS

OAKGROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	STD OR OPT 2ND FLR (SBEC) OR OPT / B LOGGIA (FLAT)
Layout ID: 292639	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/19/2018 Designer: AMANDA		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
"PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
△ TC26

A-18023875 - A-18023881
A-18023894 - A-18023900
A-18023906 - A-18023912

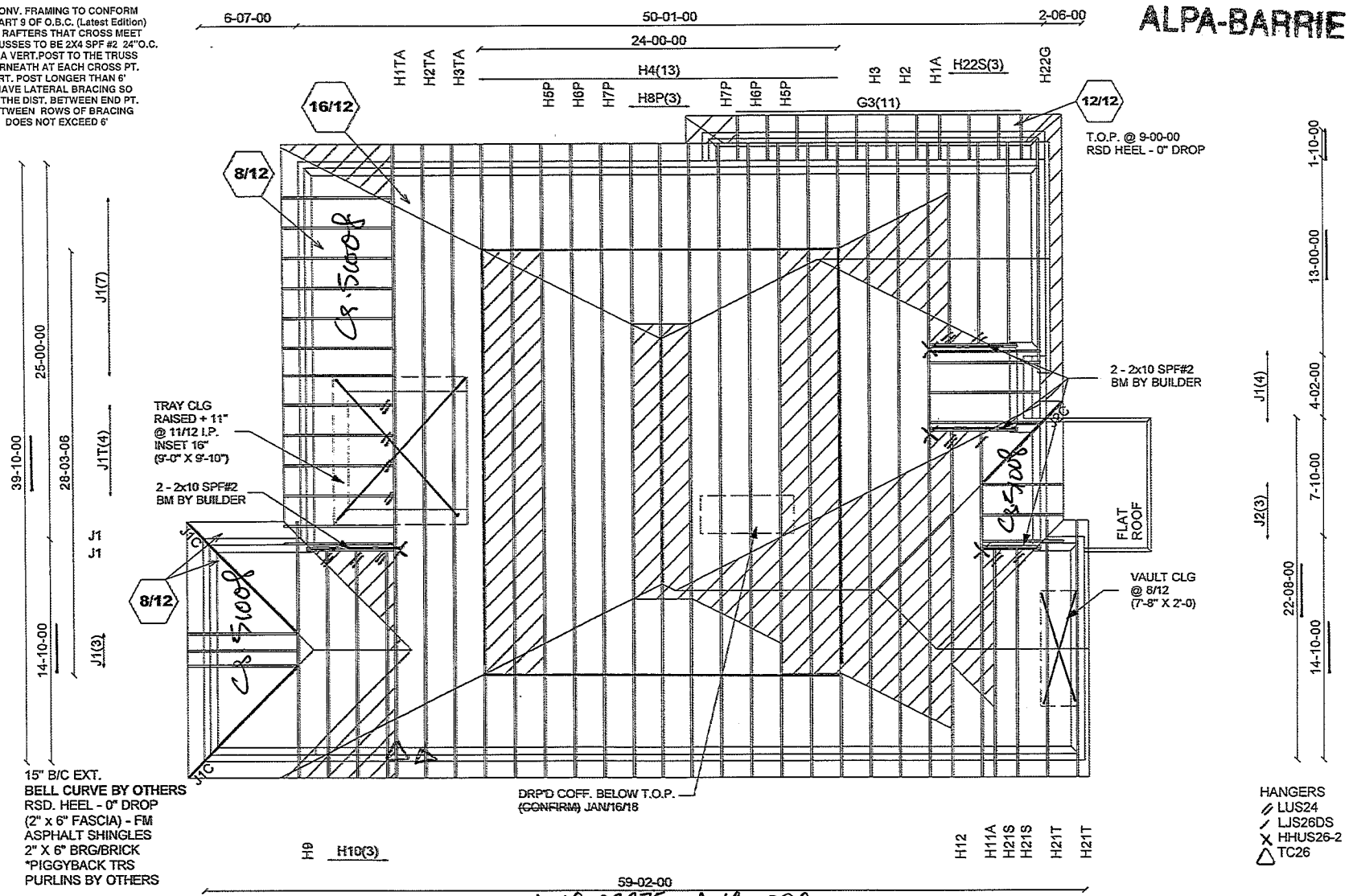
CONVENTIONAL FRAMING BY OTHERS

OAK GROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	OR OPT LOGGIA (FLAT) / B OPT TRAY MST
Layout ID: 292640			
Plan Log: 95914	Project: PINE VALLEY		
	Date: 1/19/2018 Designer: AMANDA		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



15" B/C EXT.
BELL CURVE BY OTHERS
RSD. HEEL - 0" DROP
(2" x 6" FASCIA) - FM
ASPHALT SHINGLES
2" X 6" BRG/BRICK
*PIGGYBACK TRS
PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
△ TC26

A-18023875 - A-18023883
A-18023898, A-18023901] - A-18023903

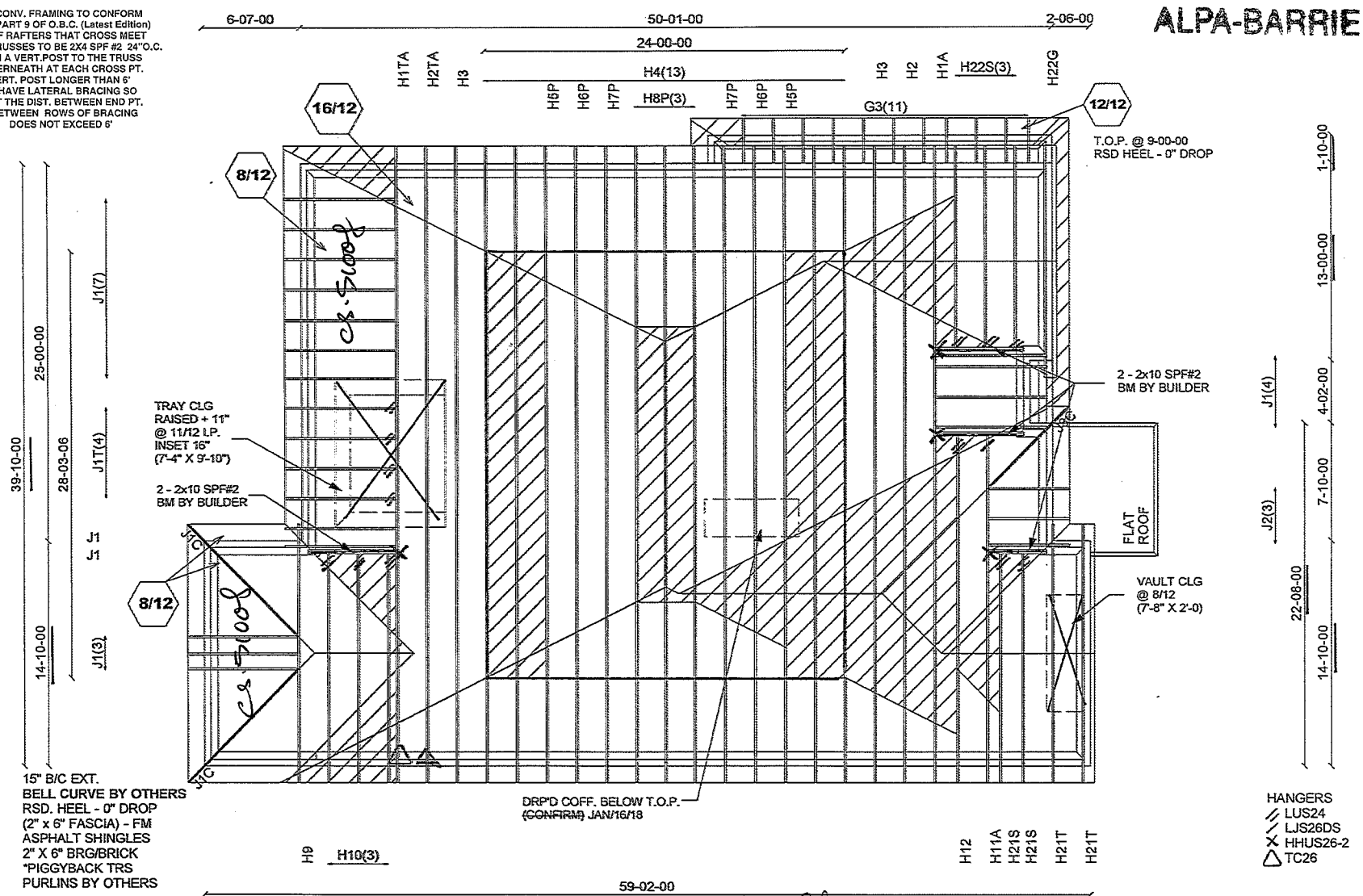
CONVENTIONAL FRAMING BY OTHERS

OAKGROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	OR OPT LOGGIA (FLAT) / B OPT TRAY SIT
Layout ID: 292641	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/19/2018 Designer: AMANDA		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



HANGERS
// LUS24
\ LJS26DS
X HHUS26-2
△ TC26

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

A-18023875 - A-18023883
A-18023898,
A-18023901 - A-18023902

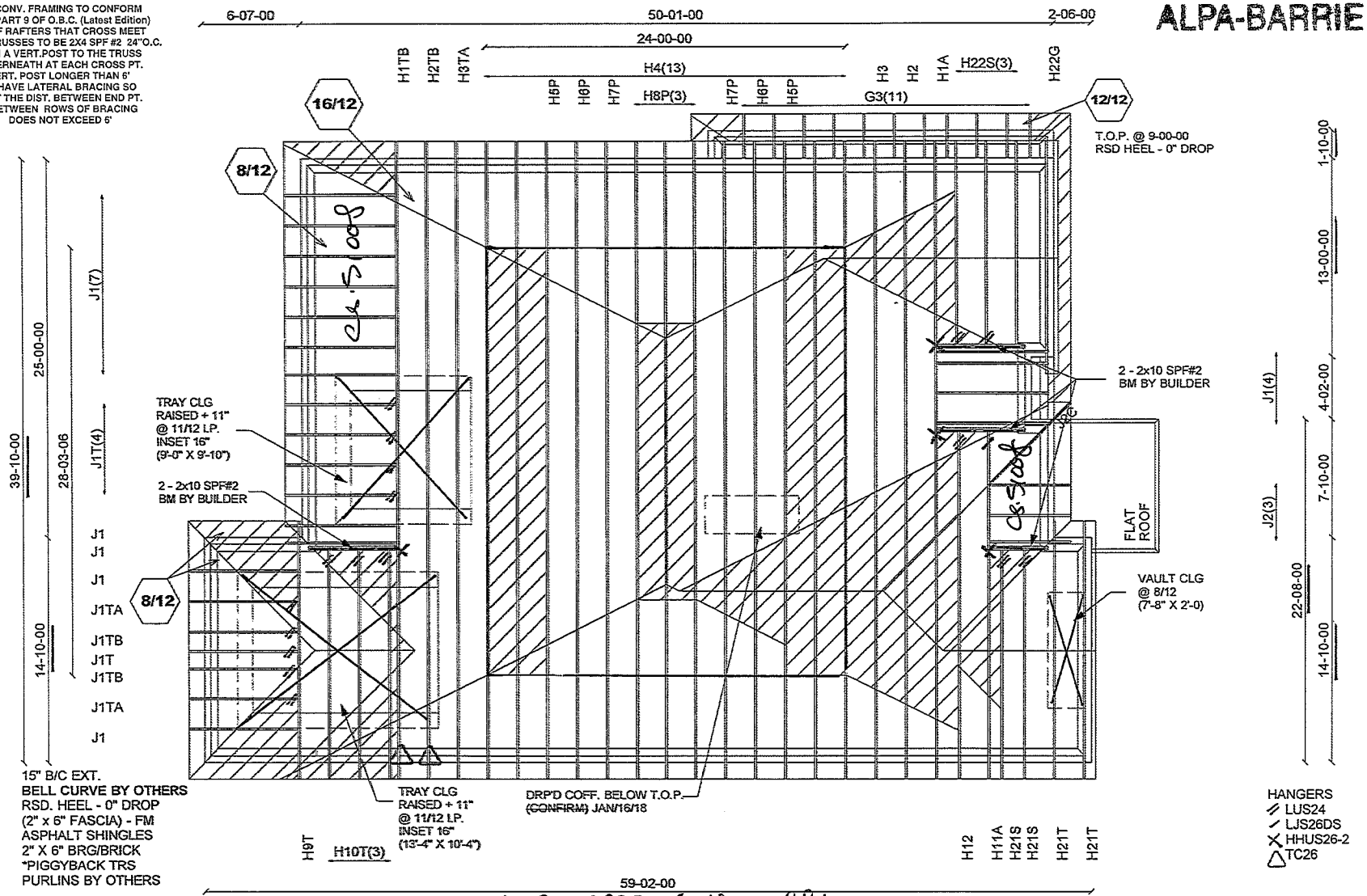
CONVENTIONAL FRAMING BY OTHERS

OAKGROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	OR OPT LOGGIA (FLAT) / B OPT TRAY SIT 5BD
Layout ID: 292644	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/19/2018 Designer: AMANDA		

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition) ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

A-18023875 - A-18023881
A-18023896 - A-18023900
A-18023903 - A-18023912

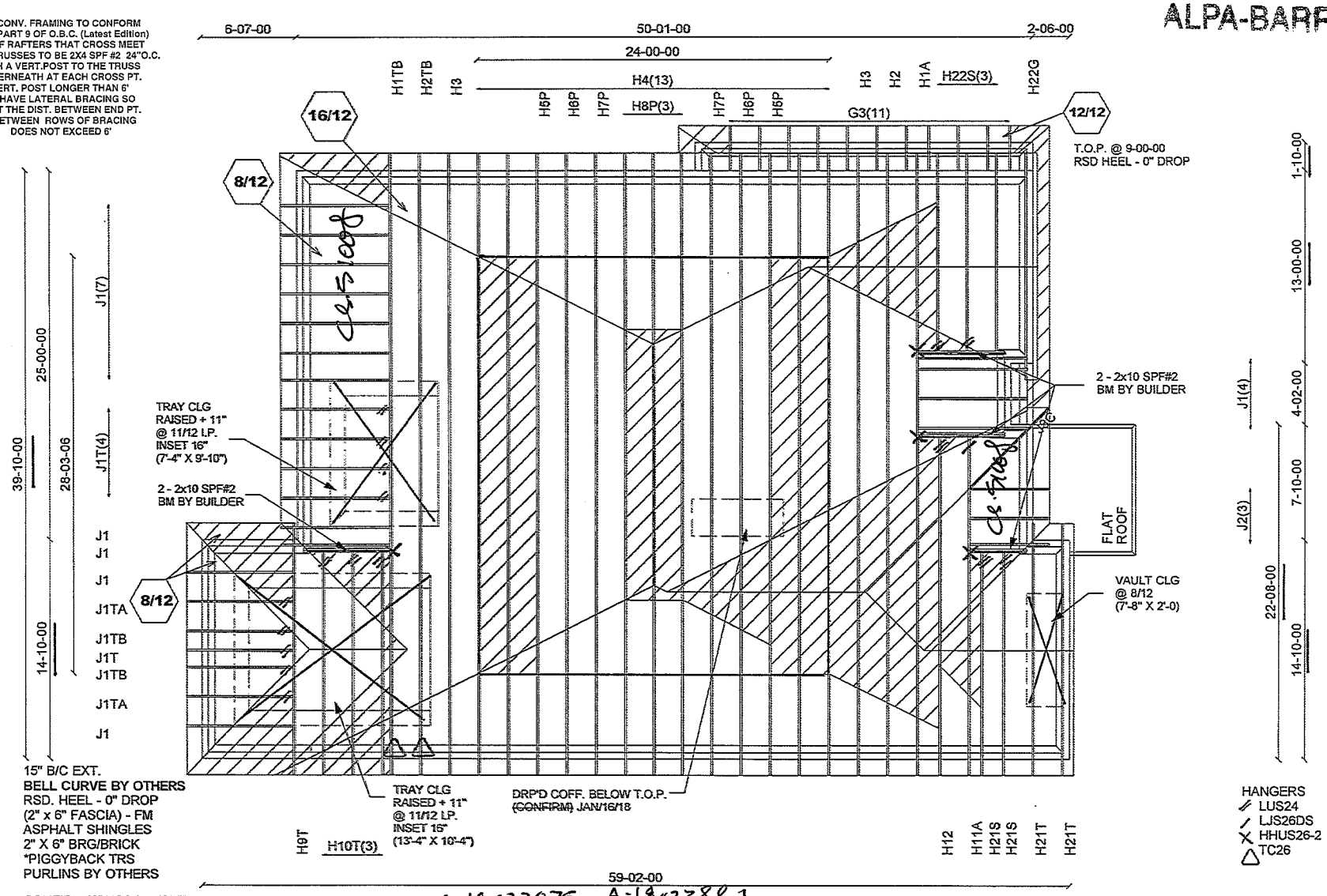
CONVENTIONAL FRAMING BY OTHERS

OAKGROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	CR OPT LOGGIA (FLAT) / B OPT TRAYS
Layout ID: 292642	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/19/2018 Designer: AMANDA		

ALPA-BARRIE

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24"O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'



15" B/C EXT.
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2" X 6" BRG/BRICK
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PURLINS BY OTHERS

~~CONFIRM TRUSS LAYOUT/DIRECTION~~
P.L. 95914

HANGERS
// LUS24
/ LJS26DS
X HHUS26-2
△ TC26

A-18023875 - A-18023881
A-18023896 - A-18023900
A-18023904 - A-18023912

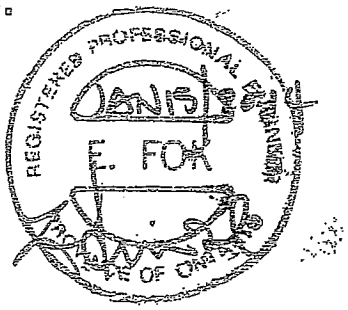
CONVENTIONAL FRAMING BY OTHERS

OAKGROVE

Job Track: 45147	Builder / Location: GOLD PARK HOMES / VAUGHAN	Model / Elevation: 5003 REV1	OPT LOGGIA (FLAT) / B OPT TRAYS 5BD
Layout ID: 292643	Project: PINE VALLEY		
Plan Log: 95914	Date: 1/19/2018 Designer: AMANDA		

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 09:50:02 2018 Page 1

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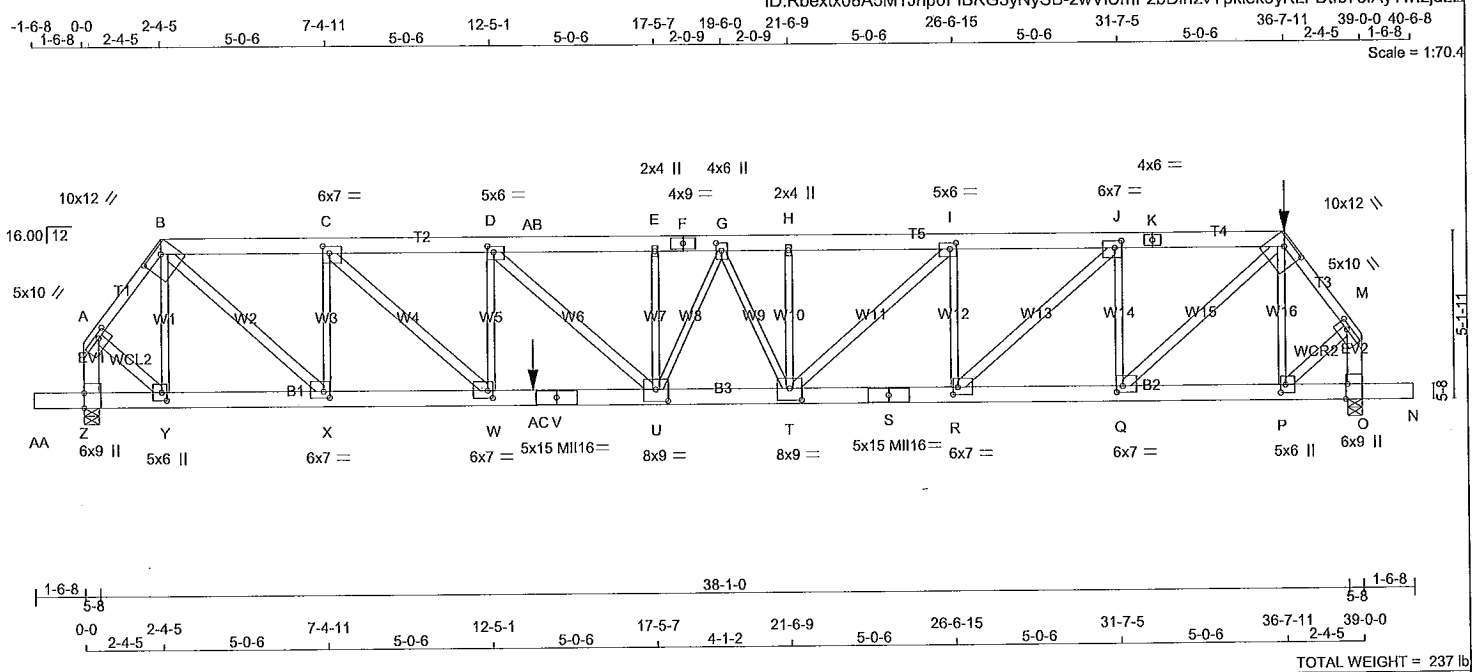


TOTAL WEIGHT = 237 lb [M]

LUMBER

N. L. G. A. RULES

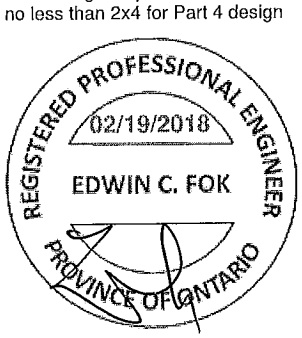
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	1650F 1.5E	SPF
F - K	2x6	DRY	1650F 1.5E	SPF
K - L	2x6	DRY	1650F 1.5E	SPF
L - M	2x4	DRY	No.2	SPF
Z - A	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA- V	2x6	DRY	2100F 1.8E	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
D - U	2x4	DRY	No.2	SPF
T - I	2x4	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
A - Y	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	10.0	1.50	3.75
B	TTWW-h	MT20	10.0	12.0	2.50	7.00
C	TMWW-t	MT20	6.0	7.0	2.50	2.50
D	TMWW-t	MT20	5.0	6.0	2.25	2.25
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	9.0		
G	TMWW+t	MT20	4.0	6.0	2.75	2.00
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	5.0	6.0	2.25	2.25
J	TMWW-t	MT20	6.0	7.0	2.50	2.50
K	TS-t	MT20	4.0	6.0		
L	TTWW-h	MT20	10.0	12.0	2.50	7.00
M	TMVW-t	MT20	5.0	10.0	1.50	3.75
O	BMV1+t	MT20	6.0	9.0	Edge	0.50
P	BMWW+t	MT20	5.0	6.0	3.00	1.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	3446	0	3446	0	0	5-8	3-7
O	3931	0	3931	0	0	5-8	4-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2445	1559 / 0	0 / 0	0 / 0	0 / 0	886 / 0	0 / 0
O	2797	1738 / 0	0 / 0	0 / 0	0 / 0	1059 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O

BEARING SIZE FACTOR = 1.15 AT JNT(S) Z, O (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.31 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2764 / 0	-78.0	-78.0	0.20 (1)		4.00
B- C	-4650 / 0	-78.0	-78.0	0.16 (1)		4.46
C- D	-7261 / 0	-78.0	-78.0	0.25 (1)		3.65
D-AB	-8282 / 0	-78.0	-78.0	0.39 (1)		3.31
AB- E	-8282 / 0	-153.5	-153.5	0.39 (1)		3.31
E- F	-8282 / 0	-153.5	-153.5	0.26 (1)		3.42
F- G	-8282 / 0	-153.5	-153.5	0.26 (1)		3.42
G- H	-8115 / 0	-153.5	-153.5	0.25 (1)		3.45
H- I	-8115 / 0	-153.5	-153.5	0.36 (1)		3.37
I- J	-7101 / 0	-153.5	-153.5	0.33 (1)		3.59
J- K	-5027 / 0	-153.5	-153.5	0.25 (1)		4.23
K- L	-5027 / 0	-153.5	-153.5	0.25 (1)		4.23
L- M	-3159 / 0	-78.0	-78.0	0.23 (1)		3.74
Z- A	-3328 / 0	0.0	0.0	0.27 (1)		5.77
O- M	-3791 / 0	0.0	0.0	0.31 (1)		5.45
AA- Z	0 / 0	-96.5	-96.5	0.05 (1)		10.00
Z- Y	0 / 0	-18.5	-18.5	0.05 (1)		10.00
Y- X	0 / 1637	-18.5	-18.5	0.14 (1)		10.00
X- W	0 / 4650	-18.5	-18.5	0.37 (1)		10.00
W-AC	0 / 7261	-18.5	-18.5	0.68 (1)		10.00
AC- V	0 / 7261	-36.4	-36.4	0.68 (1)		10.00
V- U	0 / 7261	-36.4	-36.4	0.68 (1)		10.00
U- T	0 / 8240	-36.4	-36.4	0.51 (1)		10.00
T- S	0 / 7101	-36.4	-36.4	0.44 (1)		10.00
S- R	0 / 7101	-36.4	-36.4	0.44 (1)		10.00
R- Q	0 / 5027	-36.4	-36.4	0.31 (1)		10.00
Q- P	0 / 1871	-36.4	-36.4	0.15 (1)		10.00
P- O	0 / 0	-36.4	-36.4	0.05 (1)		10.00
O- N	0 / 0	-96.5	-96.5	0.05 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- B	-856 / 0	0.30 (1)
B- X	0 / 4114	0.73 (1)
X- C	-2837 / 0	1.00 (1)
C- W	0 / 3566	0.63 (1)
W- D	-1466 / 0	0.52 (1)
D- U	0 / 1396	0.25 (1)
U- E	-588 / 0	0.21 (1)
U- G	-15 / 105	0.03 (1)
G- T	-313 / 0	0.13 (1)
T- H	-581 / 0	0.21 (1)
T- I	0 / 1385	0.24 (1)
R- I	-1727 / 0	0.61 (1)
R- J	0 / 2834	0.50 (1)
Q- J	-2775 / 0	0.98 (1)
Q- L	0 / 4308	0.76 (1)
P- L	-957 / 0	0.34 (1)
A- Y	0 / 1947	0.34 (1)
P- M	0 / 2225	0.39 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	36-7-11	-134	-134	---	FRONT	VERT	TOTAL
AC	13-8-8	-989	-989	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 25-5-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.34")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 732 (0.64")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.39/1.00 (D-E:1) , BC=0.68/1.00 (U-W:1) , WB=1.00/1.00 (C-X:1) , SSI=0.55/1.00 (U-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

A-18023873

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H1	1	1	TRUSS DESC.	

PLATES (table is in inches)

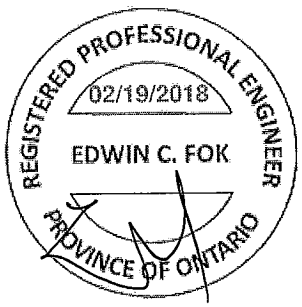
JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	6.0	7.0	2.25	2.25
R	BMWW-t	MT20	6.0	7.0	2.75	1.75
S	BS-t	MII16	5.0	15.0		
T	BMWWW-t	MT20	8.0	9.0	4.25	4.50
U	BMWWW-t	MT20	8.0	9.0	4.25	4.50
V	BS-t	MII16	5.0	15.0		
W	BMWW-t	MT20	6.0	7.0	2.75	1.75
X	BMWW-t	MT20	6.0	7.0	2.25	2.25
Y	BMWW+t	MT20	5.0	6.0	3.00	1.75
Z	BMV1+t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 36-7-11 ON TOP CHORD, AND 988.9 lbs FACTORED DOWN AT 13-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656
MII16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (Q) (INPUT = 0.90)
JSI METAL= 0.85 (V) (INPUT = 1.00)

A-1802873(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H2	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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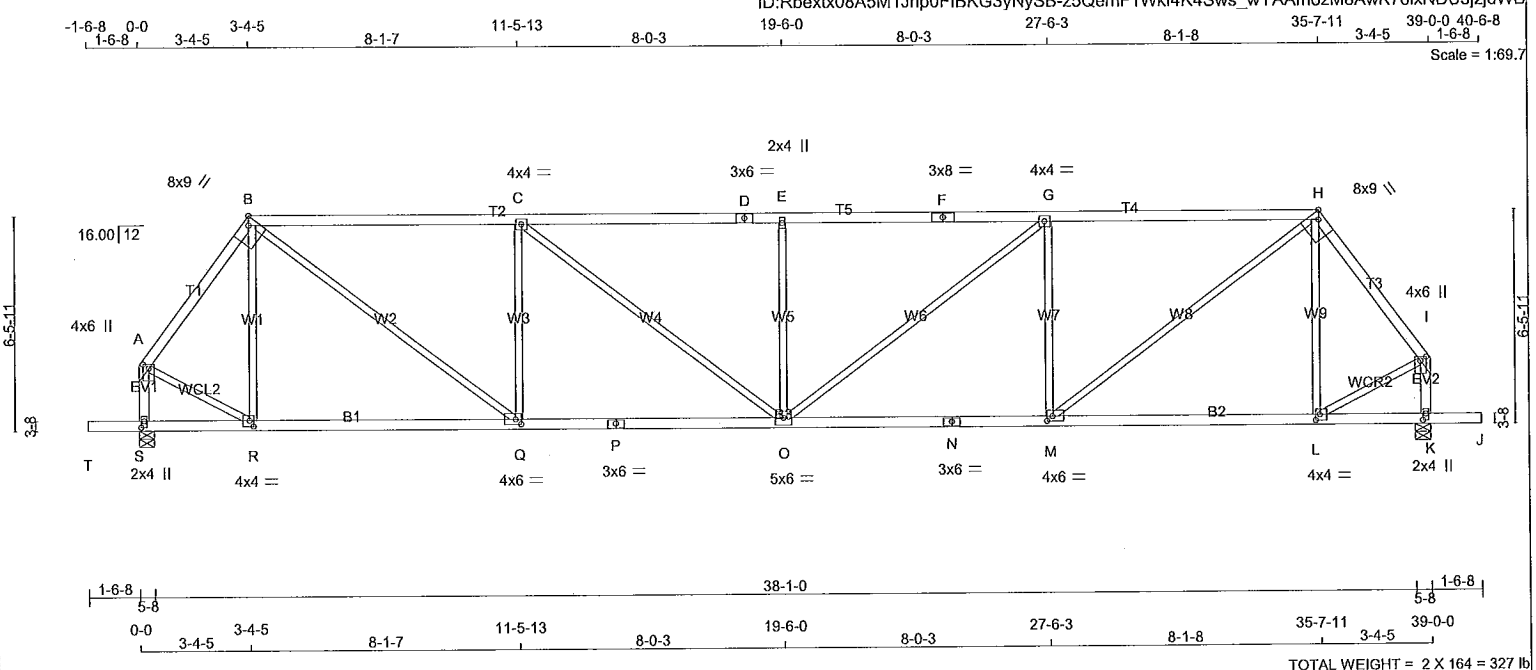


TOTAL WEIGHT = 2 X 164 = 327 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - H	2x4	DRY	1650F 1.5E	SPF
H - I	2x4	DRY	No.2	SPF
S - A	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
T - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

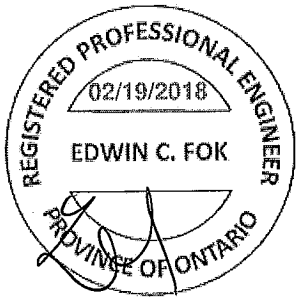
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	1.50	2.00
B	TTWW-h	MT20	8.0	9.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW-h	MT20	8.0	9.0	Edge	2.75
I	TMVW+p	MT20	4.0	6.0	1.50	2.00
K	BMV1+p	MT20	2.0	4.0	2.25	1.00
L	BMWW-t	MT20	4.0	4.0	2.00	1.50
M	BMWW-t	MT20	4.0	6.0	1.75	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	6.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	1.75	2.00
R	BMWW-t	MT20	4.0	4.0	2.00	1.50
S	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2030	0	2030	0	0	5-8	3-0
K	2030	0	2030	0	0	5-8	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0
K	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1614 / 0	-78.0	-78.0	0.21 (1)	5.02
B- C	-2566 / 0	-78.0	-78.0	0.92 (1)	3.86
C- D	-3036 / 0	-78.0	-78.0	1.00 (1)	3.56
D- E	-3036 / 0	-78.0	-78.0	1.00 (1)	3.56
E- F	-3036 / 0	-78.0	-78.0	1.00 (1)	3.56
F- G	-3036 / 0	-78.0	-78.0	1.00 (1)	3.56
G- H	-2566 / 0	-78.0	-78.0	0.92 (1)	3.86
H- I	-1614 / 0	-78.0	-78.0	0.21 (1)	5.02
S- A	-1873 / 0	0.0	0.0	0.22 (1)	6.13
K- I	-1873 / 0	0.0	0.0	0.22 (1)	6.13
T- S	0 / 0	-96.5	-96.5	0.16 (1)	10.00
S- R	0 / 0	-18.5	-18.5	0.18 (4)	10.00
R- Q	0 / 956	-18.5	-18.5	0.33 (4)	10.00
Q- P	0 / 2567	-18.5	-18.5	0.54 (1)	10.00
P- O	0 / 2567	-18.5	-18.5	0.54 (1)	10.00
O- N	0 / 2567	-18.5	-18.5	0.54 (1)	10.00
N- M	0 / 2567	-18.5	-18.5	0.54 (1)	10.00
M- L	0 / 956	-18.5	-18.5	0.33 (4)	10.00
L- K	0 / 0	-18.5	-18.5	0.18 (4)	10.00
K- J	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- B	-318 / 0	0.22 (1)
B- Q	0 / 2018	0.45 (1)
Q- C	-1065 / 0	0.73 (1)
C- O	0 / 592	0.13 (1)
O- E	-577 / 0	0.39 (1)
O- G	0 / 592	0.13 (1)
M- G	-1065 / 0	0.73 (1)
M- H	0 / 2018	0.45 (1)
L- H	-318 / 0	0.22 (1)
A- R	0 / 1060	0.24 (1)
L- I	0 / 1060	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=1.00/1.00 (C-E:1) , BC=0.54/1.00 (M-O:1) , WB=0.73/1.00 (G-M:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.85 (P) (INPUT = 1.00)

A-18023875

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H3	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 09:38:48 2018 Page 1
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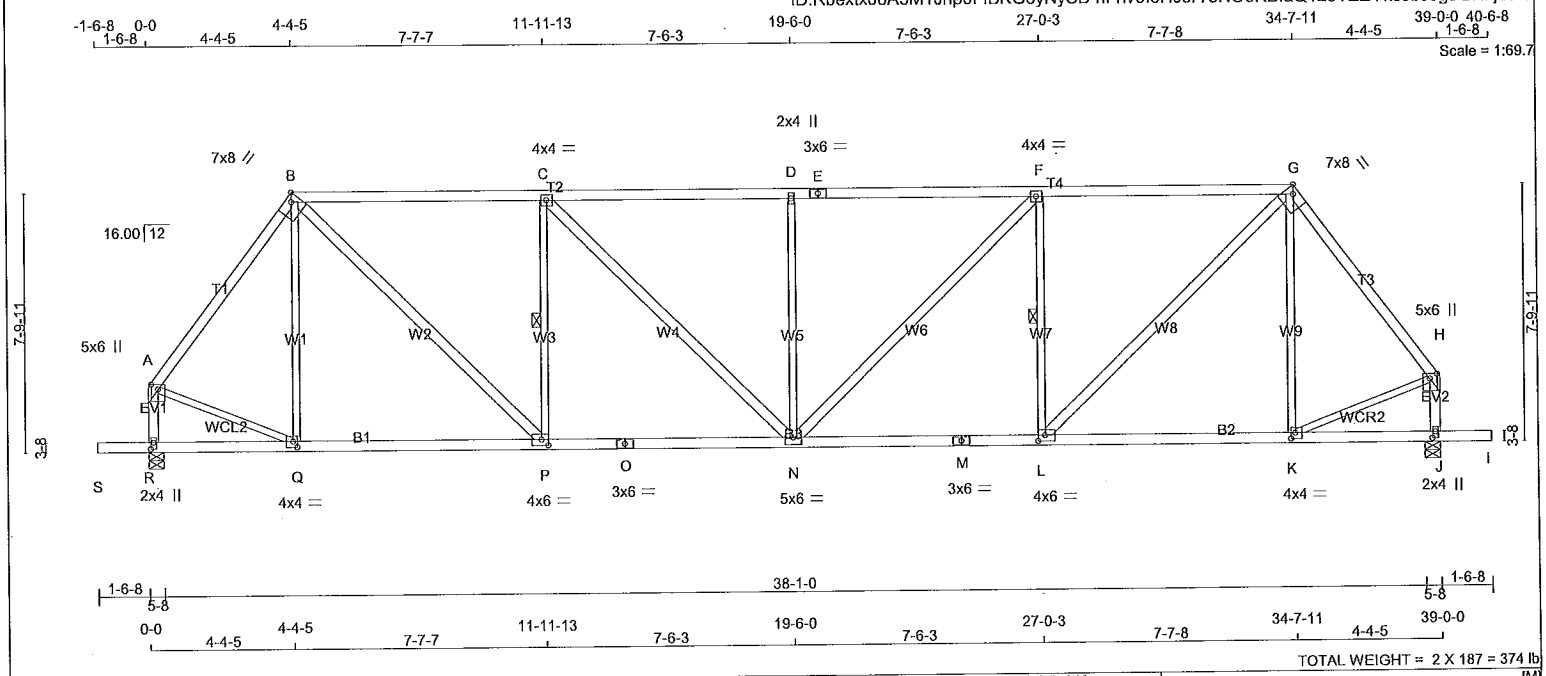


TOTAL WEIGHT = 2 X 187 = 374 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	1650F 1.5E	SPF
E - G	2x4	DRY	1650F 1.5E	SPF
G - H	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

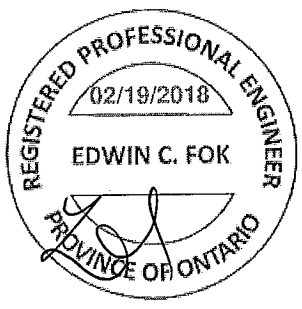
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.50
B	TTWW-h	MT20	7.0	8.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-h	MT20	7.0	8.0	Edge	2.75
H	TMVW+p	MT20	5.0	6.0	1.75	2.50
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMWW-t	MT20	4.0	6.0	2.00	2.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	6.0	2.00	2.50
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
R	2030	0	2030	0	0	5-8	3-0
J	2030	0	2030	0	0	5-8	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0
J	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-P, F-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1657 / 0	-78.0	-78.0	0.37 (1)	4.82
B- C	-2152 / 0	-78.0	-78.0	0.72 (1)	4.36
C- D	-2490 / 0	-78.0	-78.0	0.76 (1)	4.09
D- E	-2490 / 0	-78.0	-78.0	0.76 (1)	4.09
E- F	-2490 / 0	-78.0	-78.0	0.76 (1)	4.09
F- G	-2152 / 0	-78.0	-78.0	0.72 (1)	4.36
G- H	-1657 / 0	-78.0	-78.0	0.37 (1)	4.82
R- A	-1855 / 0	0.0	0.0	0.21 (1)	6.15
J- H	-1855 / 0	0.0	0.0	0.21 (1)	6.15
S- R	0 / 0	-96.5	-96.5	0.16 (1)	10.00
R- Q	0 / 0	-18.5	-18.5	0.16 (4)	10.00
Q- P	0 / 986	-18.5	-18.5	0.31 (4)	10.00
P- O	0 / 2152	-18.5	-18.5	0.46 (1)	10.00
O- N	0 / 2152	-18.5	-18.5	0.46 (1)	10.00
N- M	0 / 2152	-18.5	-18.5	0.46 (1)	10.00
M- L	0 / 2152	-18.5	-18.5	0.46 (1)	10.00
L- K	0 / 986	-18.5	-18.5	0.31 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.16 (4)	10.00
J- I	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- B	-232 / 0	0.26 (1)
B- P	0 / 1633	0.26 (1)
P- C	-999 / 0	0.38 (1)
C- N	0 / 478	0.08 (1)
N- D	-541 / 0	0.62 (1)
N- F	0 / 478	0.08 (1)
L- F	-999 / 0	0.38 (1)
L- G	0 / 1633	0.26 (1)
K- G	-232 / 0	0.26 (1)
A- Q	0 / 1051	0.24 (1)
K- H	0 / 1051	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.76/1.00 (D-F:1) , BC=0.46/1.00 (L-N:1) , WB=0.62/1.00 (D-N:1) , SSI=0.28/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (L) (INPUT = 0.90)
JSI METAL= 0.69 (O) (INPUT = 1.00)

A-18023876

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H4	13	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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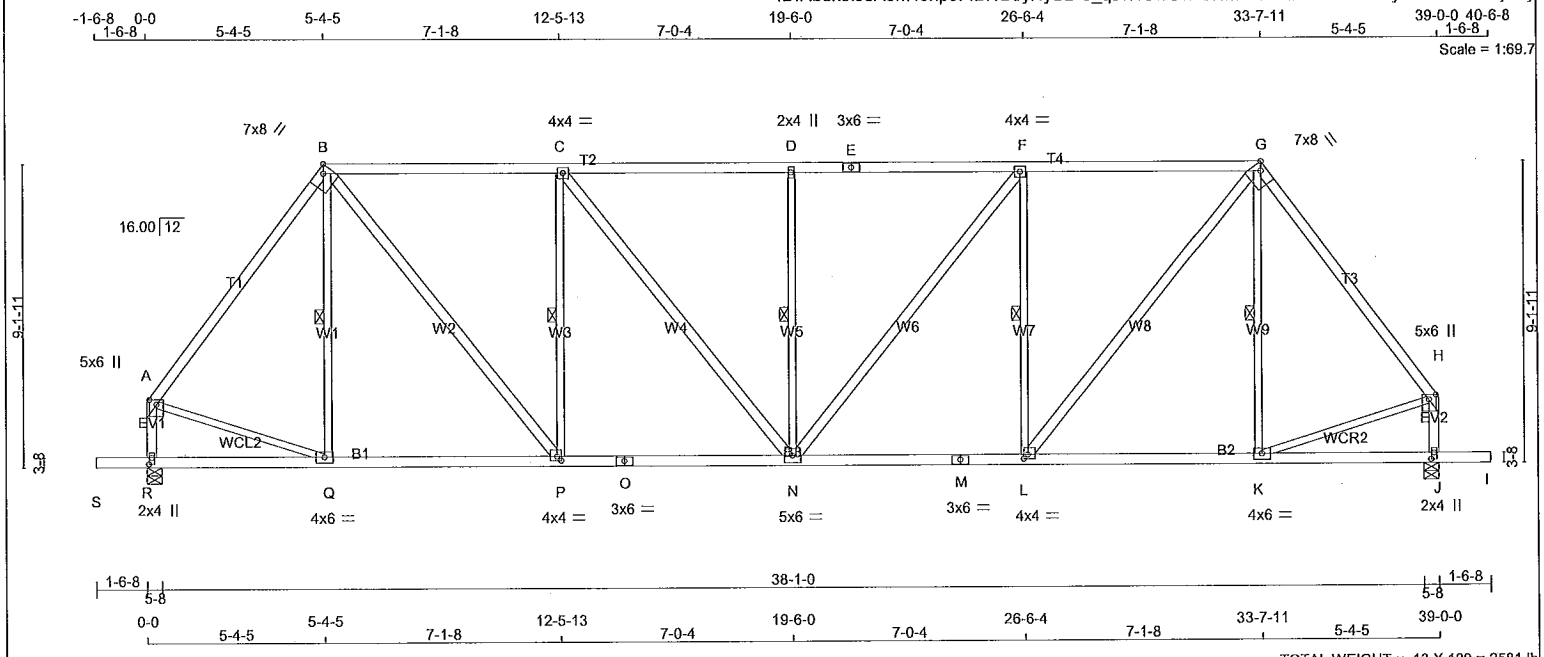

TOTAL WEIGHT = 13 X 199 = 2581 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
R - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - P	2x4	DRY	No.2	SPF
C - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
L - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.50
B	TTWW-h	MT20	7.0	8.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW-h	MT20	7.0	8.0	Edge	2.75
H	TMVW+p	MT20	5.0	6.0	1.75	2.50
J	BMV1+p	MT20	2.0	4.0	2.25	1.00
K	BMWW-t	MT20	4.0	6.0		
L	BMWW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0	1.50	1.50
Q	BMWW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2136	0	2136	0	0	5-8	3-4
J	2136	0	2136	0	0	5-8	3-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1532	883 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0
J	1532	883 / 0	0 / 0	0 / 0	0 / 0	649 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
FOR SECTION B-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-Q, C-P, D-N, F-L, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1781 / 0	-78.0	-78.0	0.43 (1)	4.68
B- C	-1992 / 0	-85.5	-85.5	0.61 (1)	2.00
C- D	-2261 / 0	-85.5	-85.5	0.61 (1)	2.00
D- E	-2261 / 0	-85.5	-85.5	0.61 (1)	2.00
E- F	-2261 / 0	-85.5	-85.5	0.61 (1)	2.00
F- G	-1992 / 0	-85.5	-85.5	0.61 (1)	2.00
G- H	-1781 / 0	-78.0	-78.0	0.43 (1)	4.68
R- A	-1950 / 0	0.0	0.0	0.22 (1)	6.03
J- H	-1950 / 0	0.0	0.0	0.22 (1)	6.03
S- R	0 / 0	-96.5	-96.5	0.16 (1)	10.00
R- Q	0 / 0	-18.5	-18.5	0.17 (4)	10.00
Q- P	0 / 1062	-18.5	-18.5	0.29 (4)	10.00
P- O	0 / 1992	-18.5	-18.5	0.42 (1)	10.00
O- N	0 / 1992	-18.5	-18.5	0.42 (1)	10.00
N- M	0 / 1992	-18.5	-18.5	0.42 (1)	10.00
M- L	0 / 1992	-18.5	-18.5	0.42 (1)	10.00
L- K	0 / 1062	-18.5	-18.5	0.29 (4)	10.00
K- J	0 / 0	-18.5	-18.5	0.17 (4)	10.00
J- I	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- B	-185 / 6	0.10 (1)
B- P	0 / 1479	0.24 (1)
P- C	-1018 / 0	0.55 (1)
C- N	0 / 433	0.07 (1)
N- D	-553 / 0	0.30 (1)
N- F	0 / 433	0.07 (1)
L- F	-1018 / 0	0.55 (1)
L- G	0 / 1479	0.24 (1)
K- G	-185 / 6	0.10 (1)
A- Q	0 / 1109	0.25 (1)
K- H	0 / 1109	0.25 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.42/1.00 (N-P:1) , WB=0.55/1.00 (F-L:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (H) (INPUT = 0.90)
JSI METAL= 0.61 (O) (INPUT = 1.00)

A-18023877

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H5P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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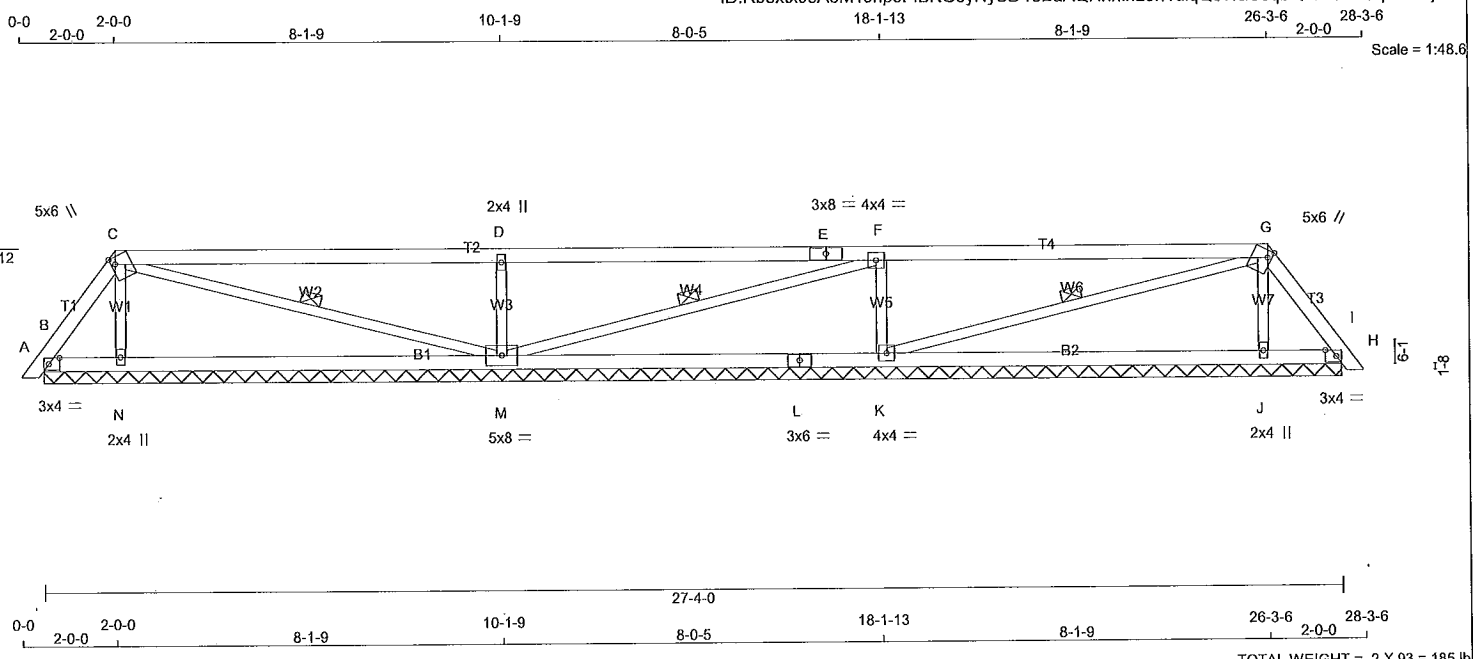


TOTAL WEIGHT = 2 X 93 = 185 lb

(M)

LUMBER

N. L. G. A. RULES

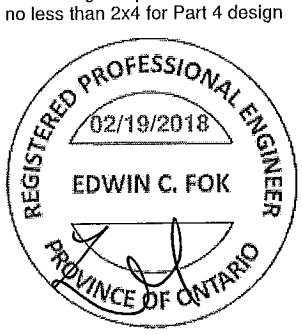
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	8.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.75
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW1-t	MT20	5.0	8.0		
N	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	119	0	119	0	-6	27-4-0 (11-4-8)2-15	
N	374	0	374	0	0	27-4-0 (11-4-8)2-15	
M	860	0	860	0	0	27-4-0 (11-4-8)2-15	
K	842	0	842	0	0	27-4-0 (11-4-8)2-15	
J	365	0	365	0	0	27-4-0 (11-4-8)2-15	
H	132	0	132	0	-4	27-4-0 (11-4-8)2-15	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

PROVIDE ANCHORAGE AT BEARING JOINT B FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT H FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	79	83 / 0	0 / 0	0 / 0	0 / 0	0 / -4	0 / 0
N	273	127 / 0	0 / 0	0 / 0	0 / 0	146 / 0	0 / 0
M	612	382 / 0	0 / 0	0 / 0	0 / 0	230 / 0	0 / 0
K	599	372 / 0	0 / 0	0 / 0	0 / 0	227 / 0	0 / 0
J	268	123 / 0	0 / 0	0 / 0	0 / 0	145 / 0	0 / 0
H	87	90 / 0	0 / 0	0 / 0	0 / 0	0 / -3	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 10	-78.0	-78.0	0.01 (1)		10.00
B- C	-82 / 0	-78.0	-78.0	0.03 (1)		6.25
C- D	0 / 1	-78.0	-78.0	0.69 (1)		10.00
D- E	0 / 0	-78.0	-78.0	0.69 (1)		10.00
E- F	0 / 0	-78.0	-78.0	0.69 (1)		10.00
F- G	-23 / 0	-78.0	-78.0	0.69 (1)		6.25
G- H	-98 / 0	-78.0	-78.0	0.03 (1)		6.25
H- I	0 / 10	-78.0	-78.0	0.01 (1)		10.00
B- N	0 / 47	-18.5	-18.5	0.20 (4)		10.00
N- M	0 / 36	-18.5	-18.5	0.24 (4)		10.00
M- L	0 / 24	-18.5	-18.5	0.24 (4)		10.00
L- K	0 / 24	-18.5	-18.5	0.24 (4)		10.00
K- J	0 / 45	-18.5	-18.5	0.24 (4)		10.00
J- H	0 / 56	-18.5	-18.5	0.20 (4)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-237 / 0	0.04 (1)
C- M	-38 / 0	0.02 (1)
M- D	-692 / 0	0.11 (1)
M- F	-26 / 0	0.01 (1)
K- F	-685 / 0	0.11 (1)
K- G	-22 / 0	0.01 (1)
J- G	-228 / 0	0.04 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.69/1.00 (F-G:1) , BC=0.24/1.00 (J-K:4) , WB=0.11/1.00 (D-M:1) , SSI=0.30/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (K) (INPUT = 0.90)
JSI METAL= 0.54 (E) (INPUT = 1.00)

A-18023878

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H6P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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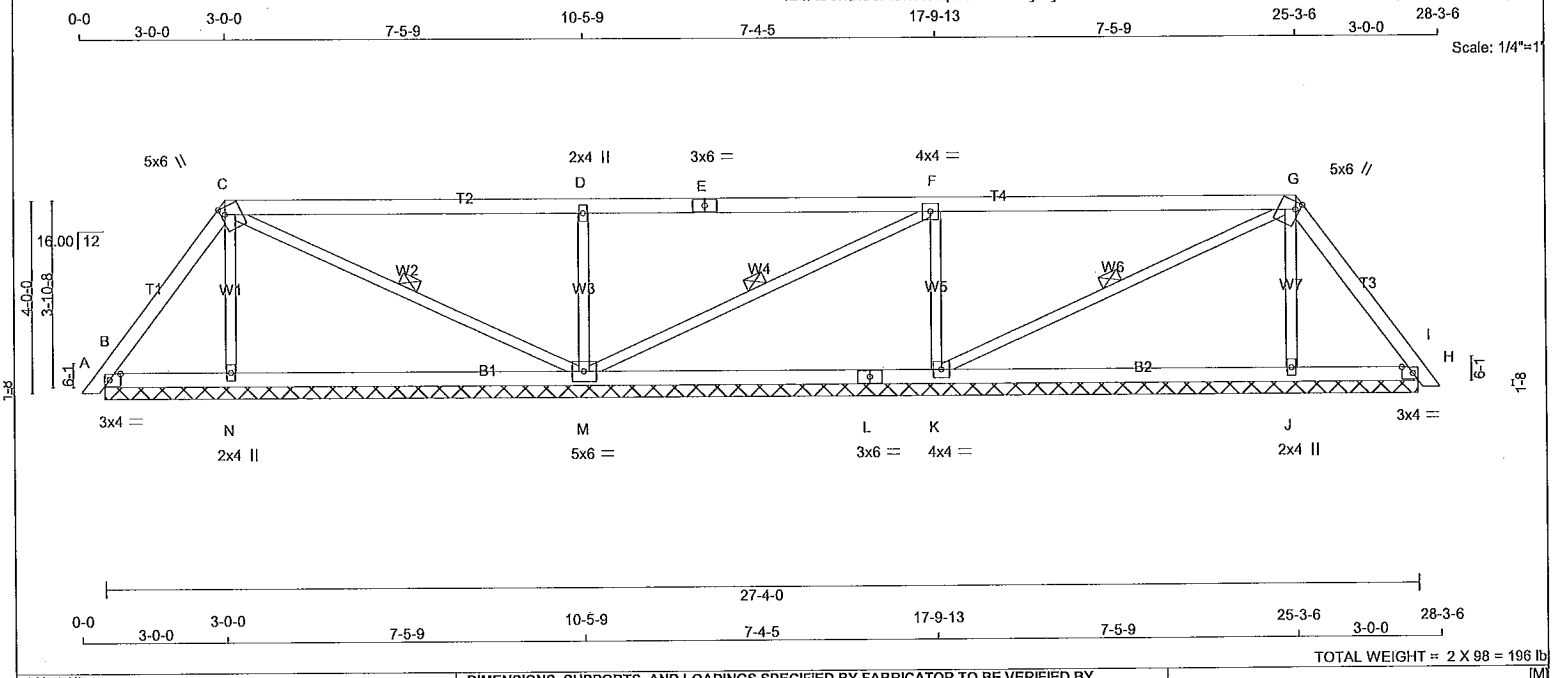


TOTAL WEIGHT = 2 X 98 = 196 lb

[M]

LUMBER
N. L. G. A. RULES

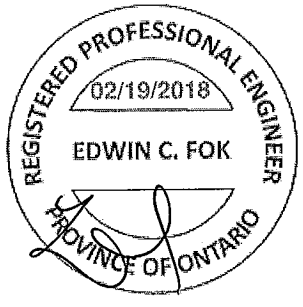
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.75
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW1-t	MT20	5.0	6.0		
N	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	200	0	200	0	0	27-4-0 (11-4-8)	0-10
N	341	0	341	0	0	27-4-0 (11-4-8)	0-10
M	833	0	833	0	0	27-4-0 (11-4-8)	0-10
K	759	0	759	0	0	27-4-0 (11-4-8)	0-10
J	331	0	331	0	0	27-4-0 (11-4-8)	0-10
H	227	0	227	0	0	27-4-0 (11-4-8)	0-10

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	138	108 / 0	0 / 0	0 / 0	0 / 0	31 / 0	0 / 0
N	248	122 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
M	592	373 / 0	0 / 0	0 / 0	0 / 0	218 / 0	0 / 0
K	541	334 / 0	0 / 0	0 / 0	0 / 0	207 / 0	0 / 0
J	242	117 / 0	0 / 0	0 / 0	0 / 0	124 / 0	0 / 0
H	157	122 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-M, G-K.

LOADING
TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	N- C	-227 / 0	0.05 (1)
B- C	-91 / 0	-78.0	-78.0	0.08 (1)	6.25	C- M	-71 / 0	0.03 (1)
C- D	0 / 19	-78.0	-78.0	0.59 (1)	10.00	M- D	-635 / 0	0.15 (1)
D- E	0 / 18	-78.0	-78.0	0.59 (1)	10.00	M- F	-63 / 0	0.03 (1)
E- F	0 / 18	-78.0	-78.0	0.59 (1)	10.00	K- F	-607 / 0	0.14 (1)
F- G	-38 / 0	-78.0	-78.0	0.59 (1)	6.25	K- G	-30 / 0	0.01 (1)
G- H	-125 / 0	-78.0	-78.0	0.09 (1)	6.25	J- G	-218 / 0	0.05 (1)
H- I	0 / 10	-78.0	-78.0	0.01 (1)	10.00			
B- N	0 / 53	-18.5	-18.5	0.15 (4)	10.00			
N- M	0 / 46	-18.5	-18.5	0.21 (4)	10.00			
M- L	0 / 39	-18.5	-18.5	0.21 (4)	10.00			
L- K	0 / 39	-18.5	-18.5	0.21 (4)	10.00			
K- J	0 / 66	-18.5	-18.5	0.21 (4)	10.00			
J- H	0 / 73	-18.5	-18.5	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.59/1.00 (D-F:1) , BC=0.21/1.00 (J-K:4) , WB=0.15/1.00 (D-M:1) , SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (M) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)

A-18023879

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H7P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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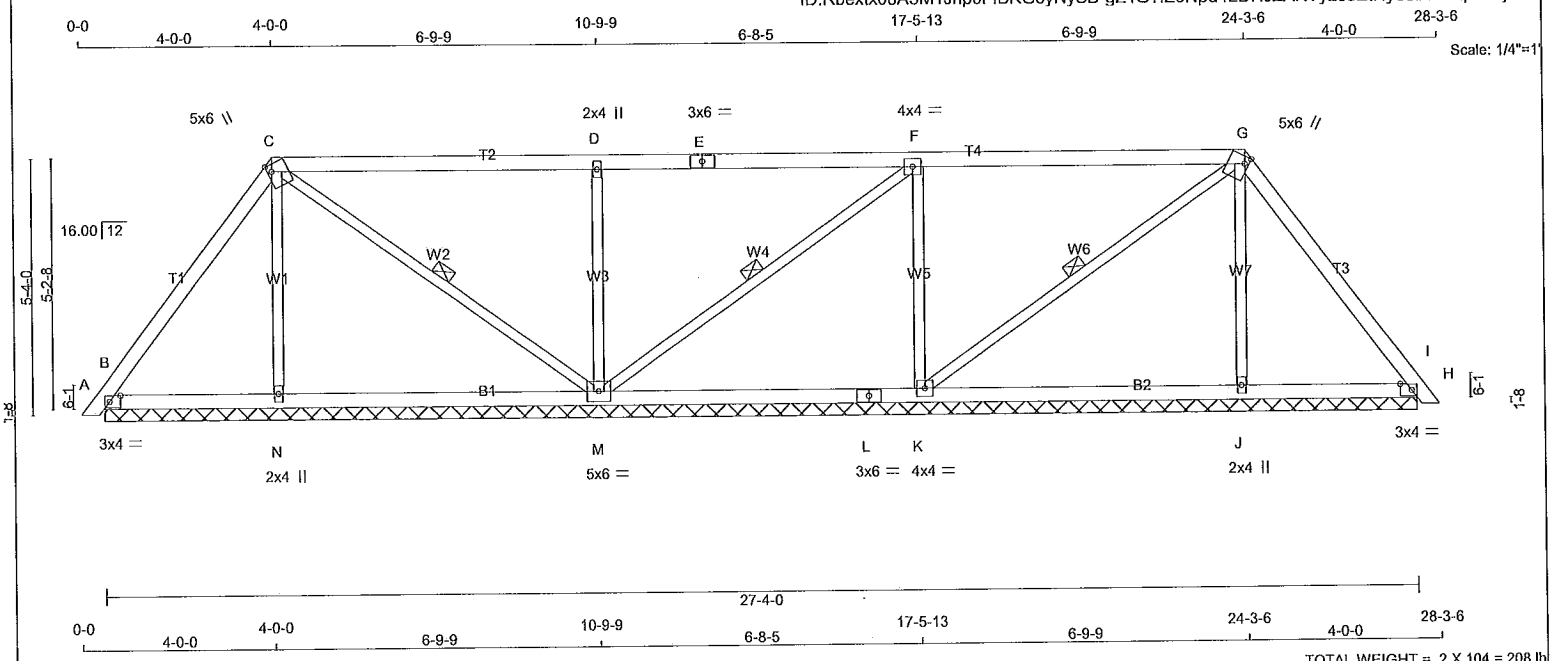


TOTAL WEIGHT = 2 X 104 = 208 lb [M]

LUMBER

N. L. G. A. RULES

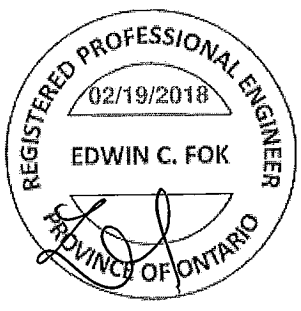
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	5.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	1.75	1.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.75
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW1-t	MT20	5.0	6.0		
N	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	258	0	258	0	0	27-4-0 (11-4-8)	11-3
N	321	0	321	0	0	27-4-0 (11-4-8)	11-3
M	831	0	831	0	0	27-4-0 (11-4-8)	11-3
K	665	0	665	0	0	27-4-0 (11-4-8)	11-3
J	315	0	315	0	0	27-4-0 (11-4-8)	11-3
H	303	0	303	0	0	27-4-0 (11-4-8)	11-3

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	180	129 / 0	0 / 0	0 / 0	0 / 0	52 / 0	0 / 0
N	234	115 / 0	0 / 0	0 / 0	0 / 0	119 / 0	0 / 0
M	589	379 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0
K	474	290 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
J	230	112 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	211	153 / 0	0 / 0	0 / 0	0 / 0	59 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- C	-92 / 0	-78.0	-78.0	0.17 (1)	6.25
C- D	0 / 38	-78.0	-78.0	0.49 (1)	10.00
D- E	0 / 38	-78.0	-78.0	0.49 (1)	10.00
E- F	0 / 38	-78.0	-78.0	0.49 (1)	10.00
F- G	-48 / 0	-78.0	-78.0	0.48 (1)	6.25
G- H	-147 / 0	-78.0	-78.0	0.17 (1)	6.25
H- I	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- N	0 / 54	-18.5	-18.5	0.13 (4)	10.00
N- M	0 / 49	-18.5	-18.5	0.17 (4)	10.00
M- L	0 / 48	-18.5	-18.5	0.17 (4)	10.00
L- K	0 / 48	-18.5	-18.5	0.17 (4)	10.00
K- J	0 / 82	-18.5	-18.5	0.17 (4)	10.00
J- H	0 / 86	-18.5	-18.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-213 / 0	0.09 (1)
C- M	-107 / 0	0.05 (1)
M- D	-578 / 0	0.23 (1)
M- F	-107 / 0	0.05 (1)
K- F	-514 / 0	0.21 (1)
K- G	-41 / 0	0.02 (1)
J- G	-207 / 0	0.08 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.49/1.00 (D-F:1) , BC=0.17/1.00 (J-K:4) , WB=0.23/1.00 (D-M:1) , SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (C) (INPUT = 0.90)
JSI METAL= 0.15 (H) (INPUT = 1.00)

A-18023880

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H8P	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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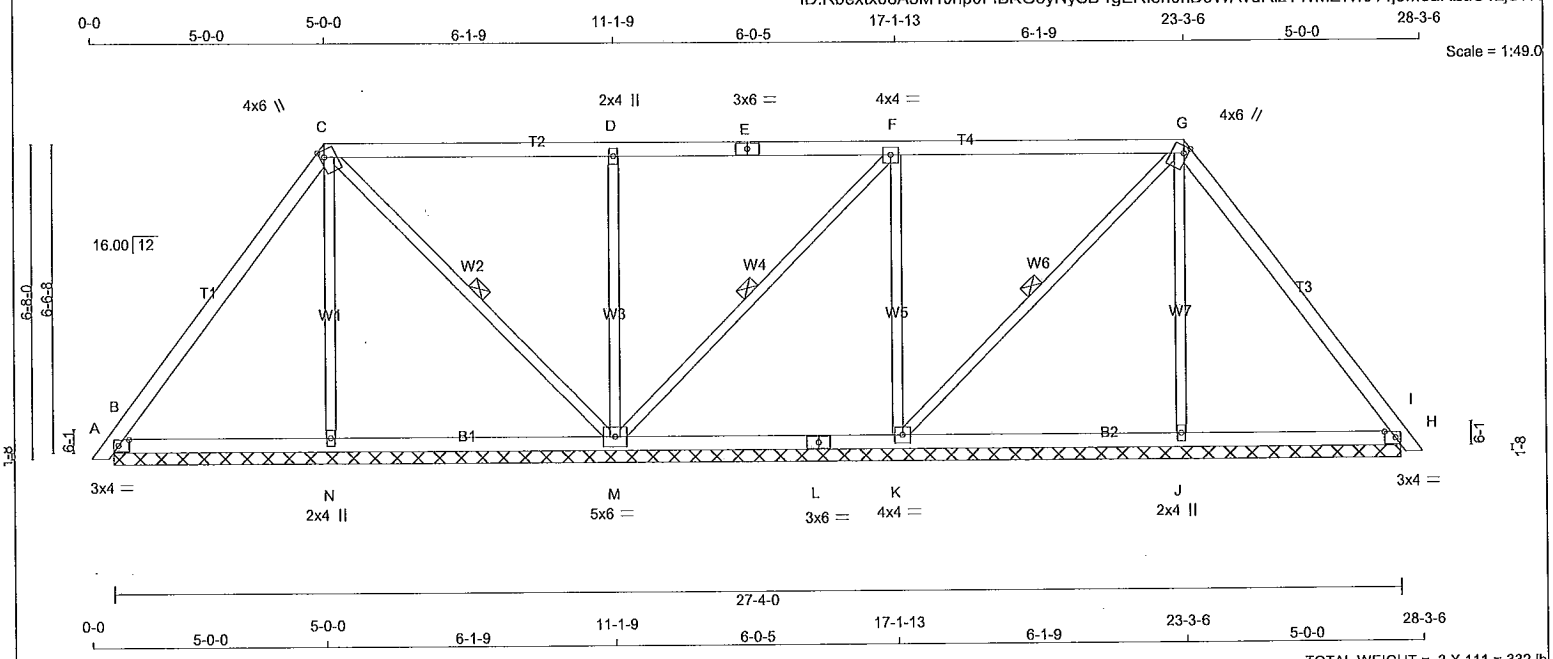


TOTAL WEIGHT = 3 X 111 = 332 lb

[M]

LUMBER

N. L. G. A. RULES

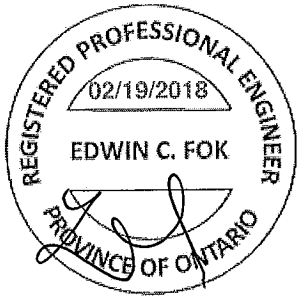
CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2.	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTWW+m	MT20	4.0	6.0	1.75	1.00
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	4.0	6.0	1.75	1.00
H	TMB1-I	MT20	3.0	4.0	1.50	2.75
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW1-t	MT20	5.0	6.0		
N	BMW1+w	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	306	0	306	0	0	27-4-0 (12-4-0)	11-10
N	308	0	308	0	0	27-4-0 (12-4-0)	11-10
M	834	0	834	0	0	27-4-0 (12-4-0)	11-10
K	572	0	572	0	0	27-4-0 (12-4-0)	11-10
J	307	0	307	0	0	27-4-0 (12-4-0)	11-10
H	365	0	365	0	0	27-4-0 (12-4-0)	11-10

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	215	148 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
N	225	107 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
M	590	389 / 0	0 / 0	0 / 0	0 / 0	201 / 0	0 / 0
K	408	248 / 0	0 / 0	0 / 0	0 / 0	160 / 0	0 / 0
J	224	106 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
H	256	179 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- C	-87 / 0	-78.0	-78.0	0.27 (1)	6.25
C- D	0 / 50	-78.0	-78.0	0.40 (1)	10.00
D- E	0 / 49	-78.0	-78.0	0.40 (1)	10.00
E- F	0 / 49	-78.0	-78.0	0.40 (1)	10.00
F- G	-49 / 0	-78.0	-78.0	0.39 (1)	6.25
G- H	-160 / 0	-78.0	-78.0	0.28 (1)	6.25
H- I	0 / 10	-78.0	-78.0	0.01 (1)	10.00
B- N	0 / 52	-18.5	-18.5	0.13 (4)	10.00
N- M	0 / 48	-18.5	-18.5	0.14 (4)	10.00
M- L	0 / 50	-18.5	-18.5	0.14 (4)	10.00
L- K	0 / 50	-18.5	-18.5	0.14 (4)	10.00
K- J	0 / 91	-18.5	-18.5	0.14 (4)	10.00
J- H	0 / 94	-18.5	-18.5	0.13 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- C	-199 / 0	0.14 (1)
C- M	-139 / 0	0.07 (1)
M- D	-521 / 0	0.36 (1)
M- F	-142 / 0	0.07 (1)
K- F	-418 / 0	0.29 (1)
K- G	-59 / 0	0.03 (1)
J- G	-198 / 0	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (D-F:1) , BC=0.14/1.00 (J-K:4) , WB=0.36/1.00 (D-M:1) , SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.19 (E) (INPUT = 1.00)

A-18023881

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H9	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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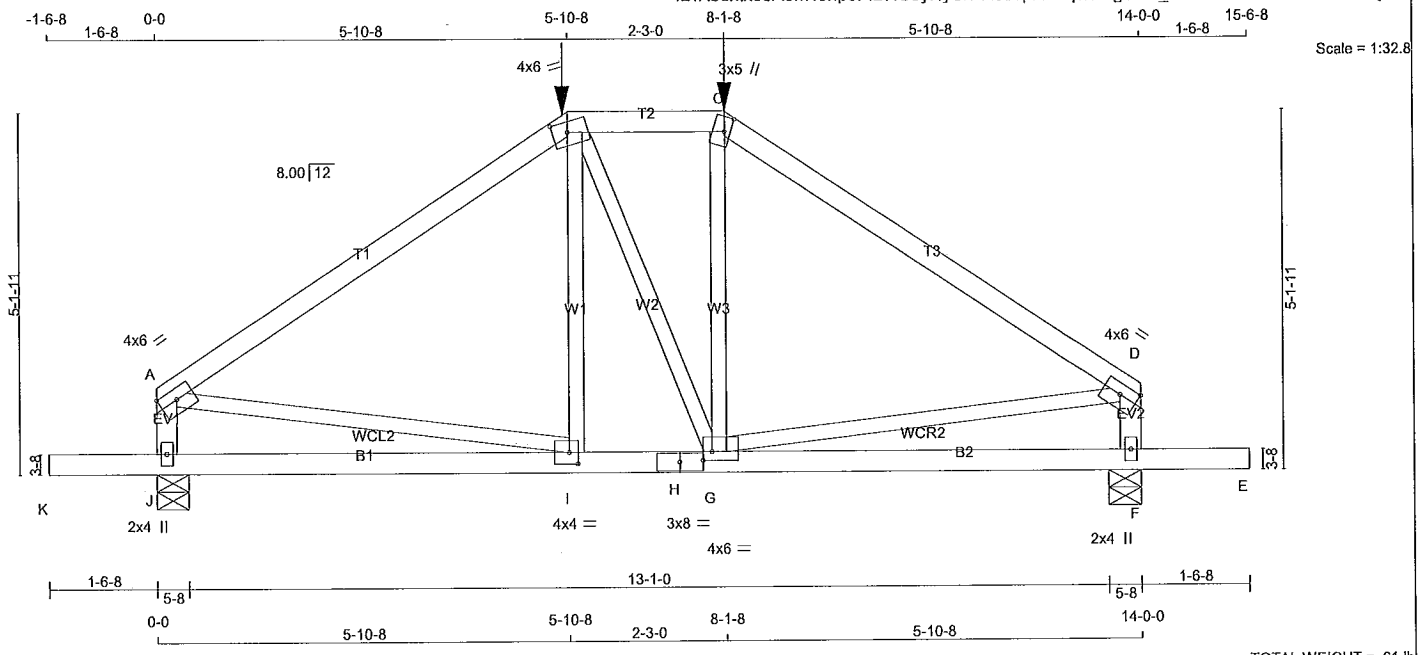


TOTAL WEIGHT = 61 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

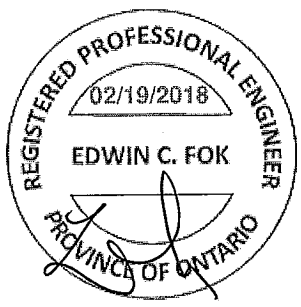
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW+m	MT20	3.0	5.0	2.50	1.00
D	TMVW-t	MT20	4.0	6.0	1.75	Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0	1.50	1.50
H	BS-t	MT20	3.0	8.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 334.0 lbs FACTORED DOWN AT 8-1-8, AND 334.0 lbs FACTORED DOWN AT 5-10-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1368	0	1368	0	0	5-8	2-0
F	1368	0	1368	0	0	5-8	2-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	978	584 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0
F	978	584 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1318 / 0	-78.0	-78.0	0.62 (1)	4.77	I- B	-5 / 117	0.04 (4)
B- C	-1097 / 0	-153.5	-153.5	0.16 (1)	5.82	B- G	0 / 4	0.00 (4)
C- D	-1320 / 0	-78.0	-78.0	0.62 (1)	4.77	G- C	-2 / 121	0.05 (4)
J- A	-1128 / 0	0.0	0.0	0.13 (1)	7.45	A- I	0 / 1108	0.27 (1)
F- D	-1127 / 0	0.0	0.0	0.13 (1)	7.45	G- D	0 / 1109	0.27 (1)
K- J	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
J- I	0 / 0	-36.4	-36.4	0.29 (4)	10.00			
I- H	0 / 1096	-36.4	-36.4	0.40 (4)	10.00			
H- G	0 / 1096	-36.4	-36.4	0.40 (4)	10.00			
G- F	0 / 0	-36.4	-36.4	0.30 (4)	10.00			
F- E	0 / 0	-96.5	-96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-10-8	-334	-334	---	FRONT	VERT	TOTAL
C	8-1-8	-334	-334	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.62/1.00 (C-D:1) , BC=0.40/1.00 (G-I:4) , WB=0.27/1.00 (D-G:1) , SSI=0.19/1.00 (F-G:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023882

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H10	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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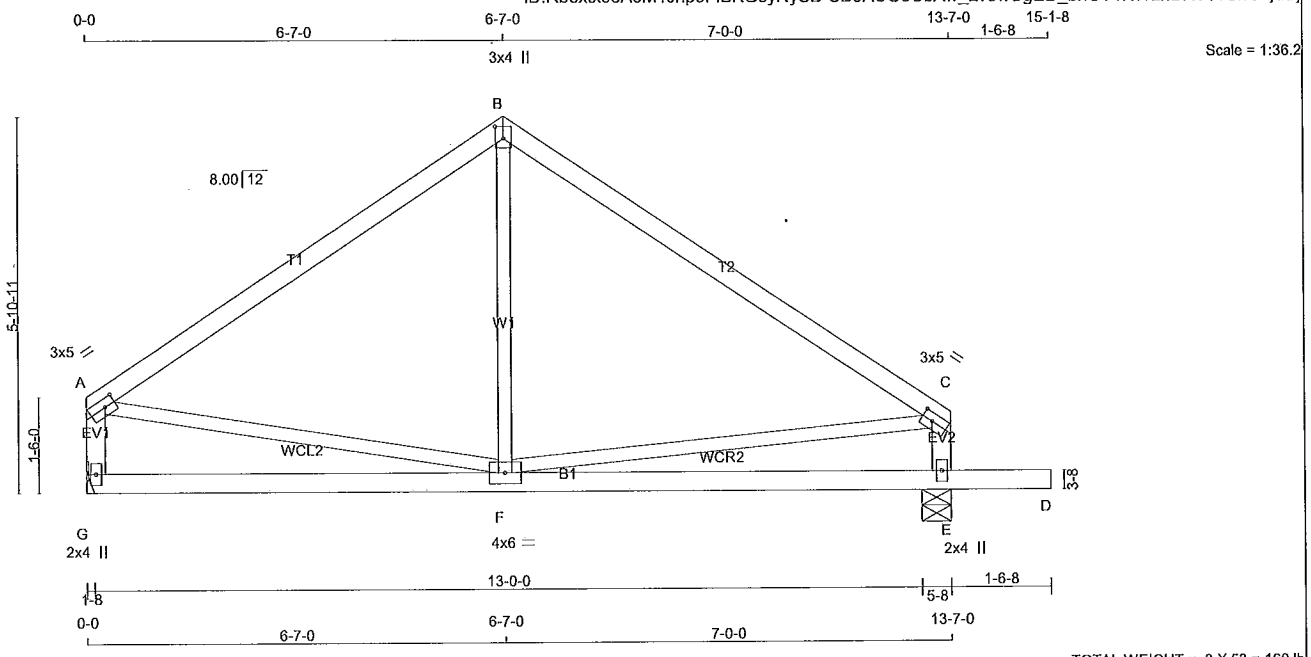


TOTAL WEIGHT = 3 X 53 = 160 lb

[M][F]

LUMBER
N. L. G. A. RULES

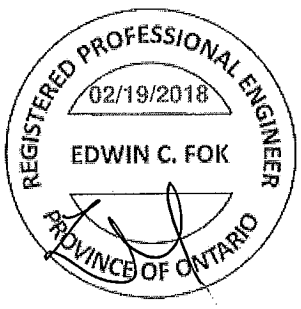
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	1650F 1.5E	SPF
B - C	2x4	DRY	2100F 1.8E	SPF
G - A	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TTW+p	MT20	3.0	4.0	2.25	1.50
C	TMVW-t	MT20	3.0	5.0	1.50	2.00
E	BMV1+p	MT20	2.0	4.0		
F	BMWWW-t	MT20	4.0	6.0		
G	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	655	0	655	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
E	804	0	804	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	467	285 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
E	573	350 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-502 / 0	-78.0	-78.0	0.37 (1)	6.25	F- B	0 / 121	0.04 (4)
B- C	-502 / 0	-78.0	-78.0	0.32 (1)	6.25	A- F	0 / 424	0.10 (1)
G- A	-610 / 0	0.0	0.0	0.06 (1)	7.81	F- C	0 / 421	0.09 (1)
E- C	-605 / 0	0.0	0.0	0.06 (1)	7.81			
G- F	0 / 0	-18.5	-18.5	0.25 (4)	10.00			
F- E	0 / 0	-18.5	-18.5	0.25 (4)	10.00			
E- D	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.37/1.00 (A-B:1) , BC=0.25/1.00 (F-G:4) , WB=0.10/1.00 (A-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (F) (INPUT = 0.90)
JSI METAL= 0.20 (C) (INPUT = 1.00)

A-18023883

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H11A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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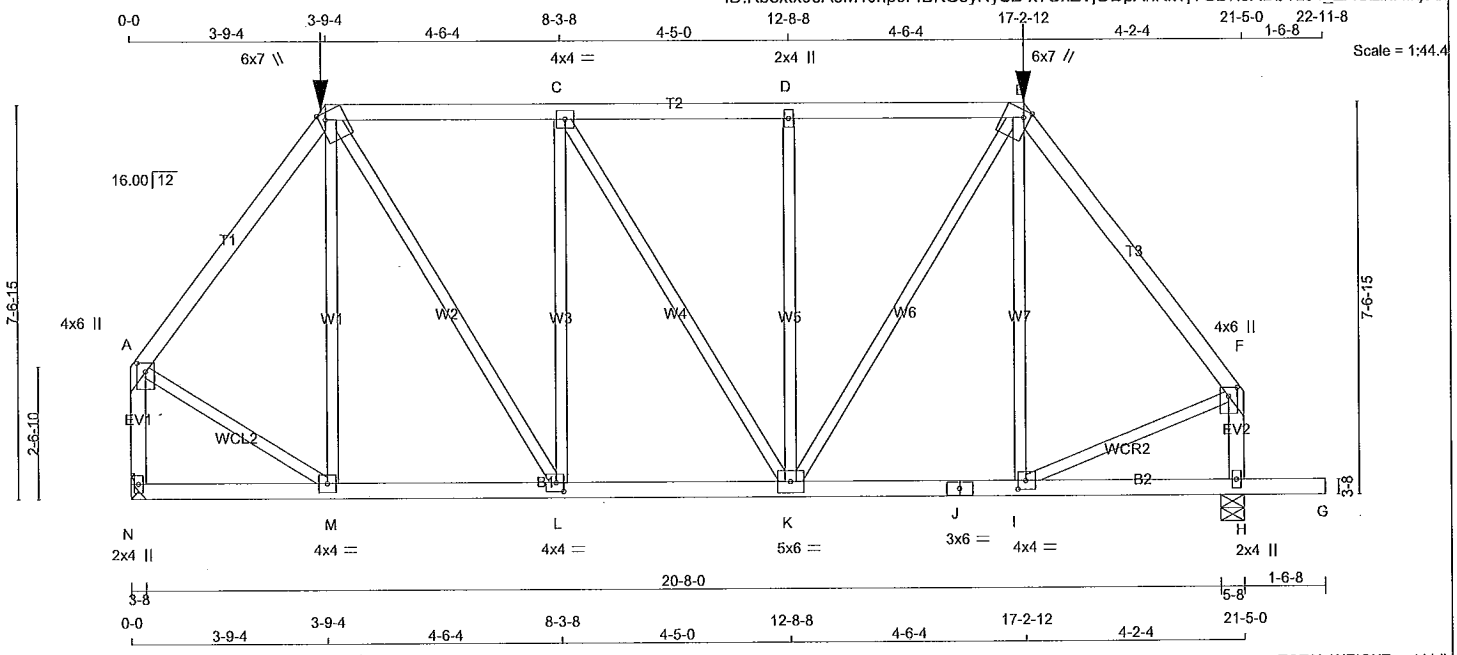


TOTAL WEIGHT = 111 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

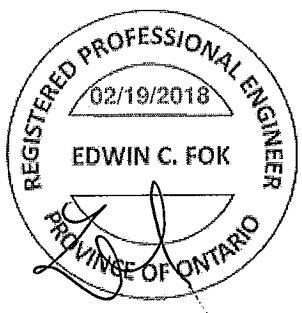
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.75
J	BS-t	MT20	3.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0	2.00	1.75
M	BMWW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 146.2 lbs FACTORED DOWN AT 17-2-12, AND 131.7 lbs FACTORED DOWN AT 3-9-4 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1571	0	1571	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-11	
H	1712	0	1712	0	0	5-8	2-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1121	681 / 0	0 / 0	0 / 0	0 / 0	440 / 0	0 / 0
H	1222	742 / 0	0 / 0	0 / 0	0 / 0	480 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1217 / 0	-78.0	-78.0	0.26 (1)	5.49	M- B	-271 / 0	0.29 (1)
B- C	-1181 / 0	-120.6	-120.6	0.39 (1)	5.32	B- L	0 / 867	0.21 (1)
C- D	-1199 / 0	-120.6	-120.6	0.39 (1)	5.29	L- C	-609 / 0	0.65 (1)
D- E	-1198 / 0	-120.6	-120.6	0.38 (1)	5.31	C- K	0 / 34	0.01 (1)
E- F	-1318 / 0	-78.0	-78.0	0.34 (1)	5.25	K- D	-575 / 0	0.61 (1)
N- A	-1529 / 0	0.0	0.0	0.22 (1)	6.61	K- E	0 / 782	0.19 (1)
H- F	-1516 / 0	0.0	0.0	0.19 (1)	6.64	I- E	-150 / 44	0.16 (1)
						A- M	0 / 831	0.21 (1)
N- M	0 / 0	-28.6	-28.6	0.13 (4)	10.00	I- F	0 / 840	0.21 (1)
M- L	0 / 722	-28.6	-28.6	0.20 (1)	10.00			
L- K	0 / 1181	-28.6	-28.6	0.27 (1)	10.00			
K- J	0 / 785	-28.6	-28.6	0.23 (4)	10.00			
J- I	0 / 785	-28.6	-28.6	0.23 (4)	10.00			
I- H	0 / 0	-28.6	-28.6	0.14 (4)	10.00			
H- G	0 / 0	-96.5	-96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-9-4	-132	-132	---	FRONT	VERT	TOTAL
E	17-2-12	-146	-146	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF	
	DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF	
	DL =	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-9-4
RIGHT SETBACK = 4-2-4
END SETBACK = 4-2-4
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.39/1.00 (B-C:1) , BC=0.27/1.00 (K-L:1) , WB=0.65/1.00 (C-L:1) , SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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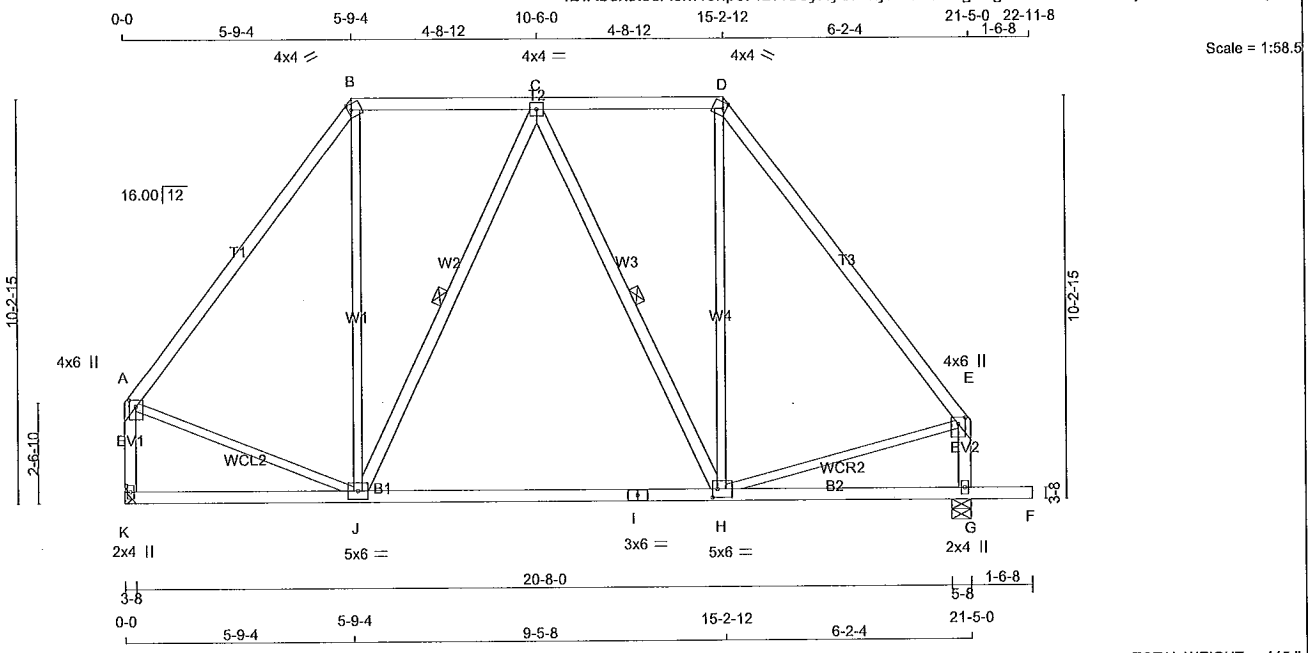

TOTAL WEIGHT = 115 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	2100F 1.8E	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
J - C	2x4	DRY	No.2	SPF
C - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW-m	MT20	4.0	4.0	1.75	1.00
C	TMWW-t	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0	1.75	1.00
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWWW-t	MT20	5.0	6.0	2.50	1.50
I	BS-t	MT20	3.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1033	0	1033	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
G	1182	0	1182	0	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	737	450 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	843	514 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-J, C-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-756 / 0	-78.0	-78.0	0.36 (1)	6.25	J- B	0 / 237	0.06 (4)
B- C	-454 / 0	-78.0	-78.0	0.22 (1)	6.25	J- C	-270 / 0	0.18 (1)
C- D	-473 / 0	-78.0	-78.0	0.22 (1)	6.25	C- H	-224 / 0	0.15 (1)
D- E	-788 / 0	-78.0	-78.0	0.27 (1)	6.25	H- D	0 / 246	0.06 (4)
K- A	-998 / 0	0.0	0.0	0.14 (1)	7.81	A- J	0 / 482	0.11 (1)
G- E	-994 / 0	0.0	0.0	0.11 (1)	7.81	H- E	0 / 486	0.11 (1)
K- J	0 / 0	-18.5	-18.5	0.27 (4)	10.00			
J- I	0 / 568	-18.5	-18.5	0.33 (4)	10.00			
I- H	0 / 568	-18.5	-18.5	0.33 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.28 (4)	10.00			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.36/1.00 (A-B:1) , BC=0.33/1.00 (H-J:4) , WB=0.18/1.00 (C-J:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (J) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

A-18023885

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H13T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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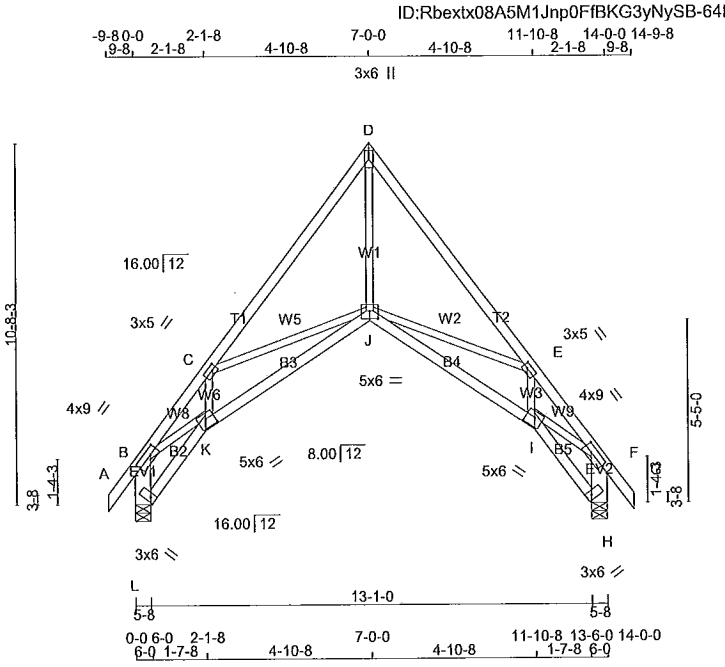


TOTAL WEIGHT = 2 X 74 = 149 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

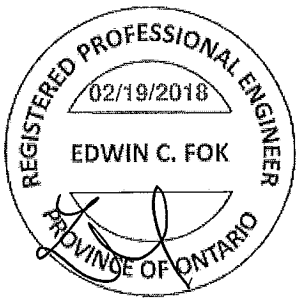
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+p	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMVW-t	MT20	4.0	9.0	2.00	3.00
H	BVM1+I	MT20	3.0	6.0	Edge	
I	BBWW-I	MT20	5.0	6.0	3.00	3.00
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BBWW-I	MT20	5.0	6.0	3.00	3.00
L	BVM1+I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	746	0	746	0	0	5-8	1-8
H	746	0	746	0	0	5-8	1-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	530	332 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0
H	530	332 / 0	0 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) L, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	J- D	0 / 987	0.22 (1)
B- C	-1380 / 0	-78.0	-78.0	0.17 (1)	5.37	J- E	-388 / 0	0.18 (1)
C- D	-836 / 0	-78.0	-78.0	0.24 (1)	6.25	I- E	0 / 55	0.02 (4)
D- E	-836 / 0	-78.0	-78.0	0.24 (1)	6.25	C- J	-388 / 0	0.18 (1)
E- F	-1381 / 0	-78.0	-78.0	0.17 (1)	5.37	K- C	0 / 55	0.02 (4)
F- G	0 / 28	-78.0	-78.0	0.05 (1)	10.00	B- K	0 / 1000	0.23 (1)
L- B	-726 / 0	0.0	0.0	0.05 (1)	7.81	I- F	0 / 1000	0.23 (1)
H- F	-726 / 0	0.0	0.0	0.05 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 1028	-18.5	-18.5	0.24 (1)	10.00			
J- I	0 / 1028	-18.5	-18.5	0.24 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.24/1.00 (D-E:1) , BC=0.24/1.00 (I-J:1) , WB=0.23/1.00 (F-I:1) , SSI=0.10/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (J) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

A-18023886

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H14A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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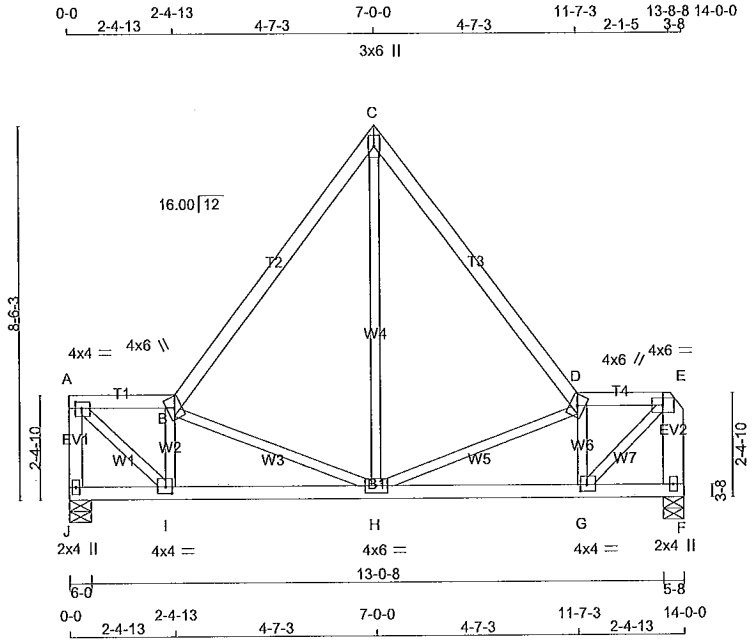


TOTAL WEIGHT = 66 lb

[M]

LUMBER

N. L. G. A. RULES

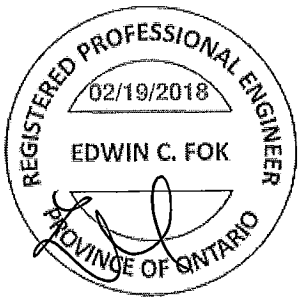
CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.75	1.75
B	TTWW+m	MT20	4.0	6.0		
C	TTW+p	MT20	3.0	6.0		
D	TTWW+m	MT20	4.0	6.0		
E	TVMW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	4.0	4.0	1.75	1.75
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0	1.75	1.75
J	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	921	0	921	0	0	6-0	1-8
F	921	0	921	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	657	401 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0
F	657	401 / 0	0 / 0	0 / 0	0 / 0	256 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-880 / 0	0.0	0.0	0.12 (1)	7.81
A- B	-883 / 0	-78.0	-78.0	0.09 (1)	6.25
B- C	-682 / 0	-78.0	-78.0	0.35 (1)	6.25
C- D	-682 / 0	-78.0	-78.0	0.35 (1)	6.25
D- E	-883 / 0	-78.0	-78.0	0.09 (1)	6.25
F- E	-880 / 0	0.0	0.0	0.08 (1)	7.81
J- I	0 / 0	-53.7	-53.7	0.08 (4)	10.00
I- H	0 / 914	-53.7	-53.7	0.33 (1)	10.00
H- G	0 / 914	-53.7	-53.7	0.33 (1)	10.00
G- F	0 / 0	-53.7	-53.7	0.08 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 1194	0.30 (1)
I- B	-612 / 0	0.10 (1)
B- H	-550 / 0	0.24 (1)
H- C	0 / 729	0.18 (1)
H- D	-551 / 0	0.24 (1)
G- D	-612 / 0	0.10 (1)
G- E	0 / 1194	0.30 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 3-5-8
END DISTANCE = 14-0-0
END SPAN CARRIED = 3-5-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.35/1.00 (B-C:1) , BC=0.33/1.00 (G-H:1) , WB=0.30/1.00 (E-G:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

A-18023887

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H15T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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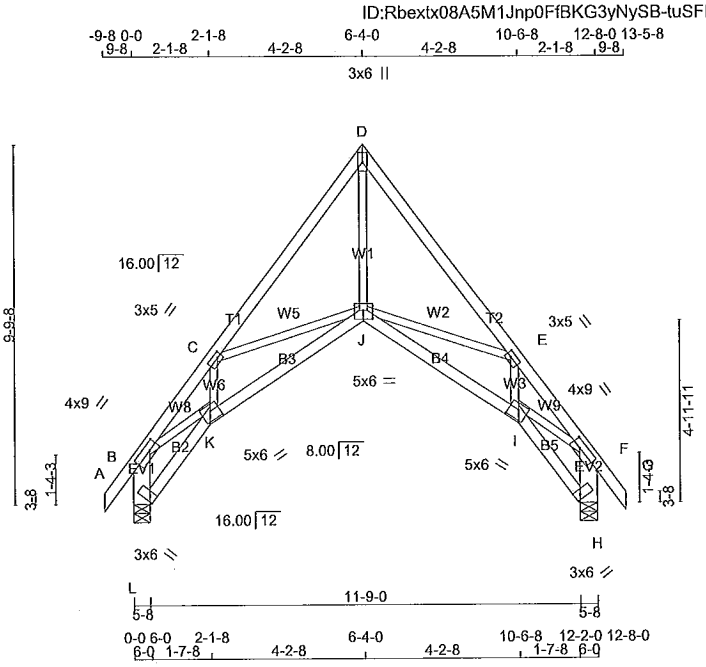

TOTAL WEIGHT = 2 X 68 = 137 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	9.0	2.00	3.00
C	TMWW-t	MT20	3.0	5.0	1.50	1.50
D	TTW+p	MT20	3.0	6.0		
E	TMWW-t	MT20	3.0	5.0	1.50	1.50
F	TMVW-t	MT20	4.0	9.0	2.00	3.00
H	BVM1+I	MT20	3.0	6.0	Edge	
I	BBWW-I	MT20	5.0	6.0	3.00	3.00
J	BBWWW-p	MT20	5.0	6.0	2.75	3.00
K	BBWW-I	MT20	5.0	6.0	3.00	3.00
L	BVM1+I	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	682	0	682	0	0	5-8	1-8
H	682	0	682	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	485	304 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0
H	485	304 / 0	0 / 0	0 / 0	0 / 0	181 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H
BEARING SIZE FACTOR = 1.15 AT JNT(S) L, H (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	J- D	0 / 900	0.20 (1)
B- C	-1208 / 0	-78.0	-78.0	0.12 (1)	5.71	J- E	-316 / 0	0.10 (1)
C- D	-752 / 0	-78.0	-78.0	0.18 (1)	6.25	I- E	0 / 51	0.02 (4)
D- E	-752 / 0	-78.0	-78.0	0.18 (1)	6.25	C- J	-316 / 0	0.10 (1)
E- F	-1208 / 0	-78.0	-78.0	0.12 (1)	5.71	K- C	0 / 51	0.02 (4)
F- G	0 / 28	-78.0	-78.0	0.05 (1)	10.00	B- K	0 / 869	0.20 (1)
L- B	-662 / 0	0.0	0.0	0.05 (1)	7.81	I- F	0 / 869	0.20 (1)
H- F	-662 / 0	0.0	0.0	0.05 (1)	7.81			
L- K	0 / 0	-18.5	-18.5	0.02 (4)	10.00			
K- J	0 / 893	-18.5	-18.5	0.20 (1)	10.00			
J- I	0 / 893	-18.5	-18.5	0.20 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.02 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.18/1.00 (D-E:1) , BC=0.20/1.00 (I-J:1) , WB=0.20/1.00 (D-J:1) , SSI=0.09/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL.. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.67 (J) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

A-18023888

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H16T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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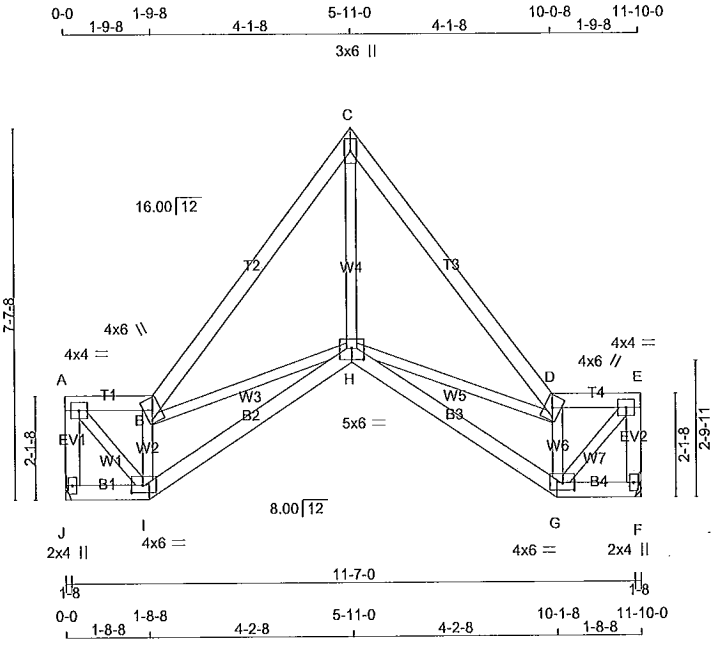


Scale = 1:47.4

TOTAL WEIGHT = 56 lb

[M][F]

LUMBER
N. L. G. A. RULES

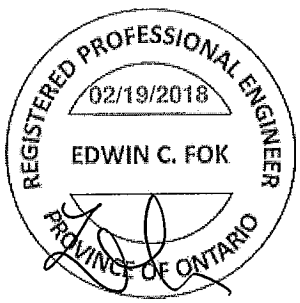
CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0		
B	TTWW+m	MT20	4.0	6.0		
C	TTW+p	MT20	3.0	6.0		
D	TTWW+m	MT20	4.0	6.0		
E	TMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	2.0	4.0		
G	BBWW-I	MT20	4.0	6.0	1.75	2.75
H	BBWWW-p	MT20	5.0	6.0	2.75	3.00
I	BBWW-I	MT20	4.0	6.0	1.75	2.75
J	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	571	0	571	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	571	0	571	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
F	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-555 / 0	0.0	0.0	0.06 (1)	7.81
A- B	-468 / 0	-78.0	-78.0	0.03 (1)	6.25
B- C	-636 / 0	-78.0	-78.0	0.18 (1)	6.25
C- D	-636 / 0	-78.0	-78.0	0.18 (1)	6.25
D- E	-468 / 0	-78.0	-78.0	0.03 (1)	6.25
F- E	-555 / 0	0.0	0.0	0.06 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.02 (4)	10.00
I- H	0 / 607	-18.5	-18.5	0.15 (4)	10.00
H- G	0 / 607	-18.5	-18.5	0.15 (4)	10.00
G- F	0 / 0	-18.5	-18.5	0.02 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 683	0.15 (1)
I- B	-782 / 0	0.12 (1)
B- H	-124 / 0	0.04 (1)
H- C	0 / 692	0.16 (1)
H- D	-124 / 0	0.04 (1)
G- D	-782 / 0	0.12 (1)
G- E	0 / 683	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.18/1.00 (C-D:1) , BC=0.15/1.00 (G-H:4) , WB=0.16/1.00 (C-H:1) , SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.71 (A) (INPUT = 0.90)
JSI METAL= 0.20 (A) (INPUT = 1.00)

A-18023889

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H17	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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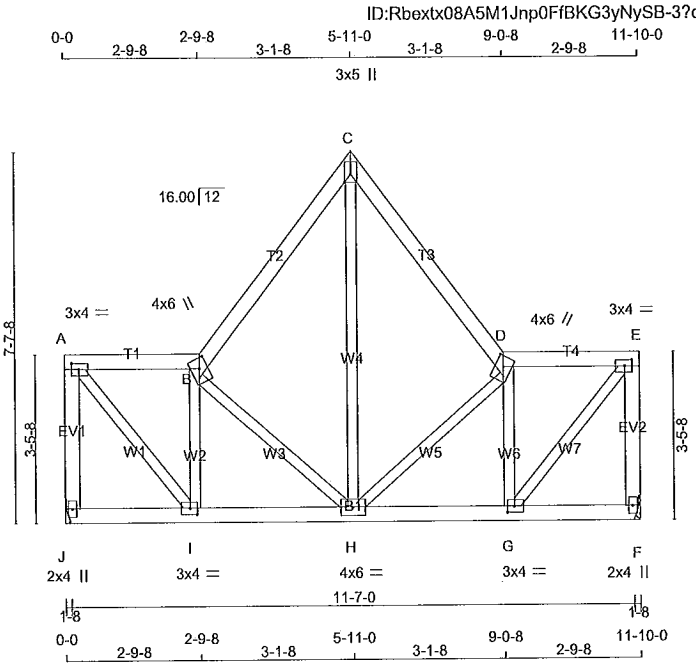


TOTAL WEIGHT = 2 X 62 = 124 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

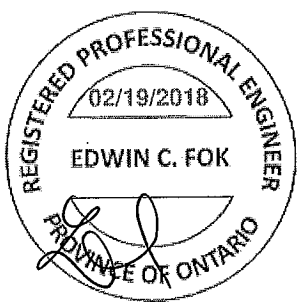
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.75
B	TTWW+m	MT20	4.0	6.0	2.50	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TTWW+m	MT20	4.0	6.0	2.50	2.00
E	TMVW-t	MT20	3.0	4.0	1.50	1.75
F	BMV1+p	MT20	2.0	4.0		
G	BMWW-t	MT20	3.0	4.0	1.50	1.75
H	BMWWW-t	MT20	4.0	6.0		
I	BMWW-t	MT20	3.0	4.0	1.50	1.75
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	571	0	571	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
F	571	0	571	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0
F	407	248 / 0	0 / 0	0 / 0	0 / 0	159 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-548 / 0	0.0	0.0	0.10 (1)	7.81
A- B	-378 / 0	-78.0	-78.0	0.08 (1)	6.25
B- C	-389 / 0	-78.0	-78.0	0.10 (1)	6.25
C- D	-389 / 0	-78.0	-78.0	0.10 (1)	6.25
D- E	-378 / 0	-78.0	-78.0	0.08 (1)	6.25
F- E	-548 / 0	0.0	0.0	0.10 (1)	7.81
J- I	0 / 0	-18.5	-18.5	0.03 (4)	10.00
I- H	0 / 391	-18.5	-18.5	0.08 (1)	10.00
H- G	0 / 391	-18.5	-18.5	0.08 (1)	10.00
G- F	0 / 0	-18.5	-18.5	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 585	0.13 (1)
I- B	-391 / 0	0.08 (1)
B- H	-221 / 0	0.07 (1)
H- C	0 / 375	0.08 (1)
H- D	-221 / 0	0.07 (1)
G- D	-391 / 0	0.08 (1)
G- E	0 / 585	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (A-J:1) , BC=0.08/1.00 (G-H:1) , WB=0.13/1.00 (E-G:1) , SSI=0.08/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.18 (G) (INPUT = 1.00)

A-18023890

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H18	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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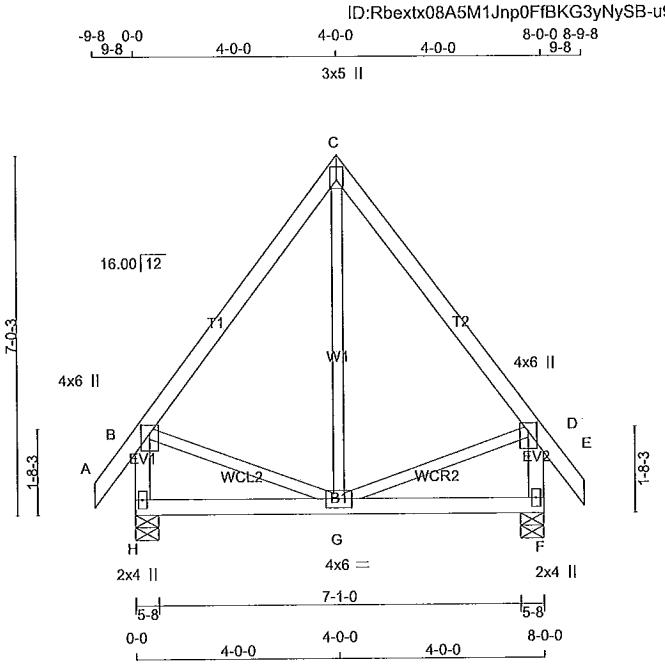


TOTAL WEIGHT = 2 X 42 = 85 lb

(M)[F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

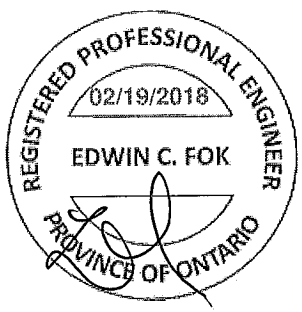
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	457	0	457	0	0	5-8	1-8
F	457	0	457	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	324	206 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0
F	324	206 / 0	0 / 0	0 / 0	0 / 0	118 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO								
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	G- C	0 / 69	0.02 (4)
B- C	-203 / 0	-78.0	-78.0	0.16 (1)	6.25	B- G	0 / 128	0.03 (1)
C- D	-203 / 0	-78.0	-78.0	0.16 (1)	6.25	G- D	0 / 128	0.03 (1)
D- E	0 / 28	-78.0	-78.0	0.05 (1)	10.00			
H- B	-428 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-428 / 0	0.0	0.0	0.05 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.08 (4)	10.00			
G- F	0 / 0	-18.5	-18.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (C-D:1) , BC=0.08/1.00 (F-G:4) , WB=0.03/1.00 (B-G:1) , SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (D) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

A-18023891

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H18G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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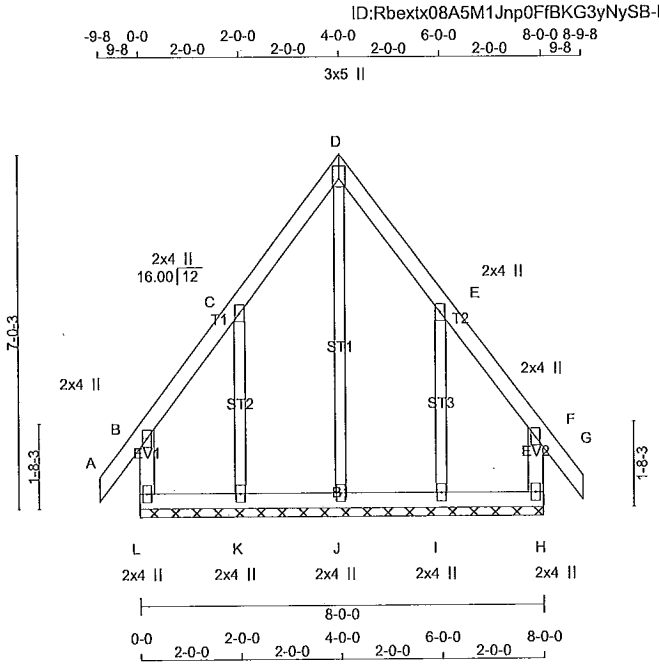


Scale = 1:45.7

TOTAL WEIGHT = 42 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

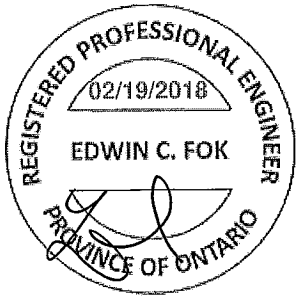
GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
E	TMW+w	MT20	2.0	4.0		
F	TMV+p	MT20	2.0	4.0		
H	BMV1+p	MT20	2.0	4.0		
I, J, K						
I	BMW1+w	MT20	2.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00		J- D	-124 / 0	0.10 (1)
B- C	-35 / 0	-78.0	-78.0	0.03 (1)	6.25		K- C	-149 / 0	0.04 (1)
C- D	-26 / 0	-78.0	-78.0	0.04 (1)	6.25		I- E	-149 / 0	0.04 (1)
D- E	-26 / 0	-78.0	-78.0	0.04 (1)	6.25				
E- F	-35 / 0	-78.0	-78.0	0.03 (1)	6.25				
F- G	0 / 28	-78.0	-78.0	0.05 (1)	10.00				
L- B	-172 / 0	0.0	0.0	0.02 (1)	7.81				
H- F	-172 / 0	0.0	0.0	0.02 (1)	7.81				
L- K	0 / 21	-18.5	-18.5	0.02 (1)	10.00				
K- J	0 / 17	-18.5	-18.5	0.02 (4)	10.00				
J- I	0 / 17	-18.5	-18.5	0.02 (4)	10.00				
I- H	0 / 21	-18.5	-18.5	0.02 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (A-B:1) , BC=0.02/1.00 (H-I:1) , WB=0.10/1.00 (D-J:1) , SSI=0.04/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

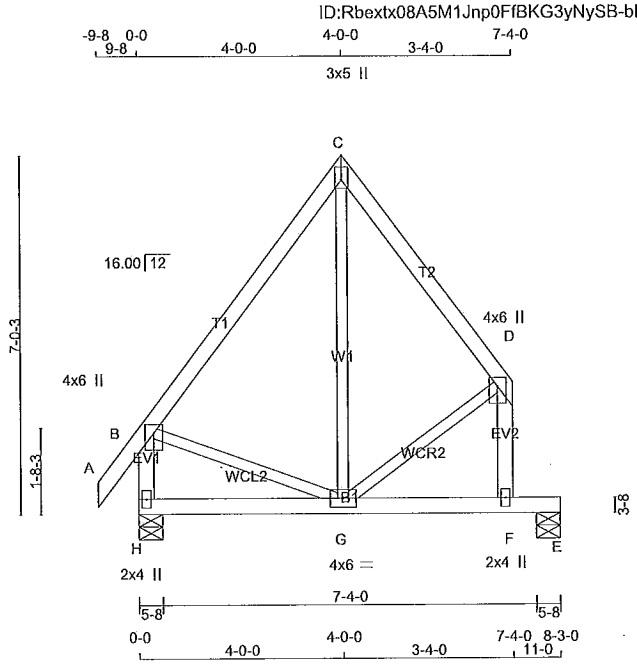
JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.09 (F) (INPUT = 1.00)

A-18023892

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292633	H18S	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

ID:Rbextx08A5M1Jnp0FfBKG3yNySB-bL6ugITPoFUWGU7zES3KKSedUzlBpWCfWNvJzyzjdK



TOTAL WEIGHT = 2 X 41 = 82 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

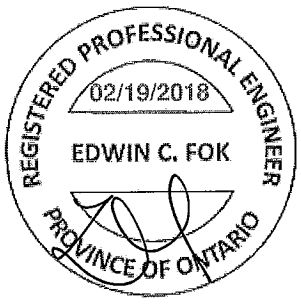
DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.00	2.00
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV+p	MT20	2.0	4.0		
G	BMWWW-t	MT20	4.0	6.0		
H	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	469	0	469	0	0	5-8	1-8
E	398	0	398	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	333	211 / 0	0 / 0	0 / 0	0 / 0	121 / 0	0 / 0
E	284	173 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	G- C	0 / 92	0.03 (4)
B- C	-232 / 0	-78.0	-78.0	0.22 (1)	6.25	B- G	0 / 146	0.03 (1)
C- D	-232 / 0	-78.0	-78.0	0.15 (1)	6.25	G- D	0 / 166	0.04 (1)
H- B	-457 / 0	0.0	0.0	0.05 (1)	7.81			
F- D	-406 / 0	0.0	0.0	0.06 (1)	7.81			
H- G	0 / 0	-18.5	-18.5	0.14 (1)	10.00			
G- F	0 / 0	-18.5	-18.5	0.44 (1)	10.00			
F- E	0 / 0	-96.5	-96.5	0.44 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 977 (0.10")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.44/1.00 (E-F:1) , WB=0.04/1.00 (D-G:1) , SSI=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.13 (B) (INPUT = 1.00)

A-18023893

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	H1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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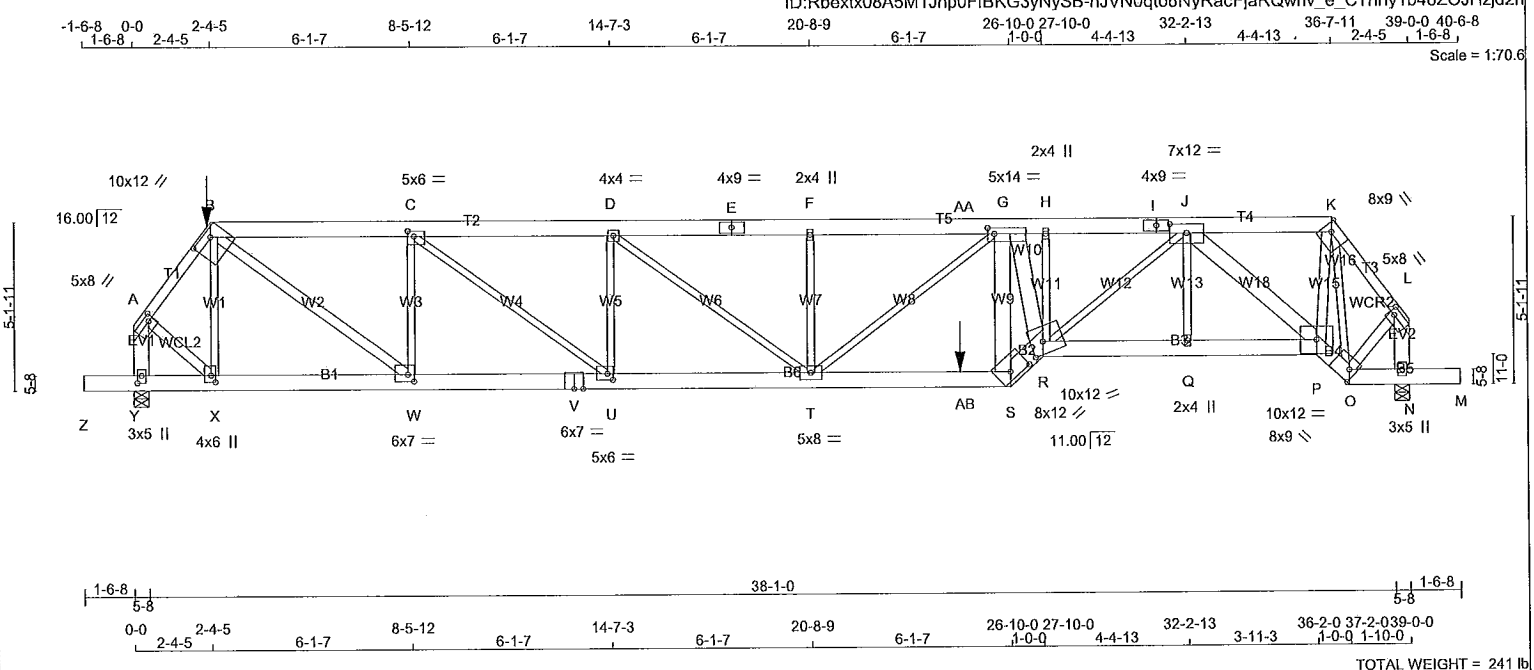


TOTAL WEIGHT = 241 lb

(M)

LUMBER

N. L. G. A. RULES

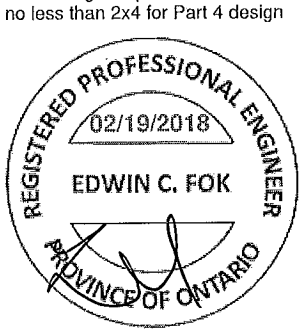
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x6	DRY	No.2	SPF
E - I	2x6	DRY	No.2	SPF
I - K	2x6	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
Y - A	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
Z - V	2x6	DRY	2100F 1.8E	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - R	2x8	DRY	1950F 1.7E	SPF
R - P	2x6	DRY	No.2	SPF
P - O	2x6	DRY	1650F 1.5E	SPF
O - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - W	2x4	DRY	No.2	SPF
S - G	2x6	DRY	No.2	SPF
G - R	2x6	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
A - X	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
J - P	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.00	2.00
B	TTWW-h	MT20	10.0	12.0	2.50	7.00
C	TMWW-t	MT20	5.0	6.0	2.00	2.25
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	4.0	9.0		
F	TMW+w	MT20	2.0	4.0		
G	TMWWW-t	MT20	5.0	14.0	2.25	2.50
H	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	4.0	9.0		
J	TMWWW-t	MT20	7.0	12.0	3.25	6.00
K	TTWW-h	MT20	8.0	9.0	3.25	3.00
L	TMVW-t	MT20	5.0	8.0	2.25	2.00
N	BMV1+p	MT20	3.0	5.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	3925	0	3925	0	0	5-8	4-10
N	3443	0	3443	0	0	5-8	5-7

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	2793	1735 / 0	0 / 0	0 / 0	0 / 0	1058 / 0	0 / 0
N	2443	1557 / 0	0 / 0	0 / 0	0 / 0	886 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N
BEARING SIZE FACTOR = 1.15 AT JNT(S) Y (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.35 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3160 / 0	-78.0	-78.0	0.23 (1)	3.74	X- B	-942 / 0	0.33 (1)
B- C	-5614 / 0	-153.5	-153.5	0.69 (1)	3.06	B- W	0 / 4708	0.83 (1)
C- D	-7675 / 0	-153.5	-153.5	0.93 (1)	2.35	W- C	-2625 / 0	0.93 (1)
D- E	-8362 / 0	-153.5	-153.5	0.89 (1)	2.36	C- U	0 / 2596	0.64 (1)
E- F	-8362 / 0	-153.5	-153.5	0.89 (1)	2.36	U- D	-1448 / 0	0.51 (1)
F-AA	-8362 / 0	-153.5	-153.5	0.88 (1)	2.35	D- T	0 / 865	0.21 (1)
AA- G	-8362 / 0	-78.0	-78.0	0.88 (1)	2.35	T- F	-890 / 0	0.31 (1)
G- H	-8263 / 0	-78.0	-78.0	0.77 (1)	2.38	T- G	0 / 1361	0.34 (1)
H- I	-8250 / 0	-78.0	-78.0	0.77 (1)	2.44	S- G	-5552 / 0	0.89 (1)
I- J	-8250 / 0	-78.0	-78.0	0.77 (1)	2.44	G- R	0 / 3920	0.51 (1)
J- K	-2362 / 0	-78.0	-78.0	0.20 (1)	5.14	R- H	0 / 463	0.11 (1)
K- L	-2431 / 0	-78.0	-78.0	0.17 (1)	4.25	R- J	0 / 3761	0.93 (1)
Y- A	-3793 / 0	0.0	0.0	0.31 (1)	5.45	Q- J	-79 / 45	0.02 (1)
N- L	-3215 / 0	0.0	0.0	0.26 (1)	5.86	P- K	0 / 4608	0.81 (1)
						K- O	-2863 / 0	0.95 (1)
Z- Y	0 / 0	-96.5	-96.5	0.05 (1)	10.00	A- X	0 / 2226	0.39 (1)
Y- X	0 / 0	-36.4	-36.4	0.07 (4)	10.00	O- L	0 / 1873	0.33 (1)
X- W	0 / 1873	-36.4	-36.4	0.16 (1)	10.00	J- P	-4134 / 0	0.93 (1)
W- V	0 / 5614	-36.4	-36.4	0.37 (1)	10.00			
V- U	0 / 5614	-36.4	-36.4	0.37 (1)	10.00			
U- T	0 / 7675	-36.4	-36.4	0.53 (1)	10.00			
T-AB	0 / 7300	-36.4	-36.4	0.85 (1)	10.00			
AB- S	0 / 7300	-18.5	-18.5	0.85 (1)	10.00			
S- R	0 / 9523	-18.5	-18.5	0.44 (1)	10.00			
R- Q	0 / 5363	-18.5	-18.5	0.72 (1)	10.00			
Q- P	0 / 5363	-18.5	-18.5	0.72 (1)	10.00			
P- O	0 / 2414	-18.5	-18.5	0.20 (1)	10.00			
O- N	-1 / 0	-18.5	-18.5	0.08 (1)	10.00			
N- M	0 / 0	-96.5	-96.5	0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-4-5	-134	-134	---	FRONT	VERT	TOTAL
AB	25-3-8	-989	-989	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL	LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 25-3-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.45")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 552 (0.85")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 389 (0.05")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 206 (0.09")

CSI: TC=0.93/1.00 (C-D:1) , BC=0.85/1.00 (S-T:1) , WB=0.95/1.00 (K-O:1) , SSI=0.51/1.00 (S-T:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

A-18023894

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	H1T	1	1	TRUSS DESC.	

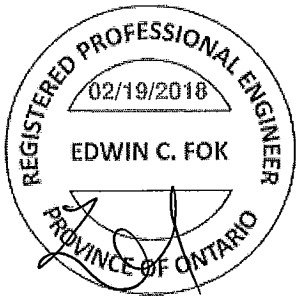
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BBWW-h	MT20	8.0	9.0	4.00	2.50
P	BBWW-l	MT20	10.0	12.0		
Q	BMW+w	MT20	2.0	4.0		
R	BBWWW-m	MT20	10.0	12.0	4.50	4.50
S	BBW-h	MT20	8.0	12.0	2.75	7.00
T	BMWWW-t	MT20	5.0	8.0		
U	BMWW-t	MT20	5.0	6.0	2.25	2.00
V	BS-t	MT20	6.0	7.0		
W	BMWW-t	MT20	6.0	7.0	2.50	2.25
X	BMWW+t	MT20	4.0	6.0	2.50	1.50
Y	BMV1+p	MT20	3.0	5.0	2.75	1.50

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5 ON TOP CHORD, AND 988.9 lbs FACTORED DOWN AT 25-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (X) (INPUT = 0.90)
JSI METAL= 0.99 (S) (INPUT = 1.00)

A-18023894(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	H2T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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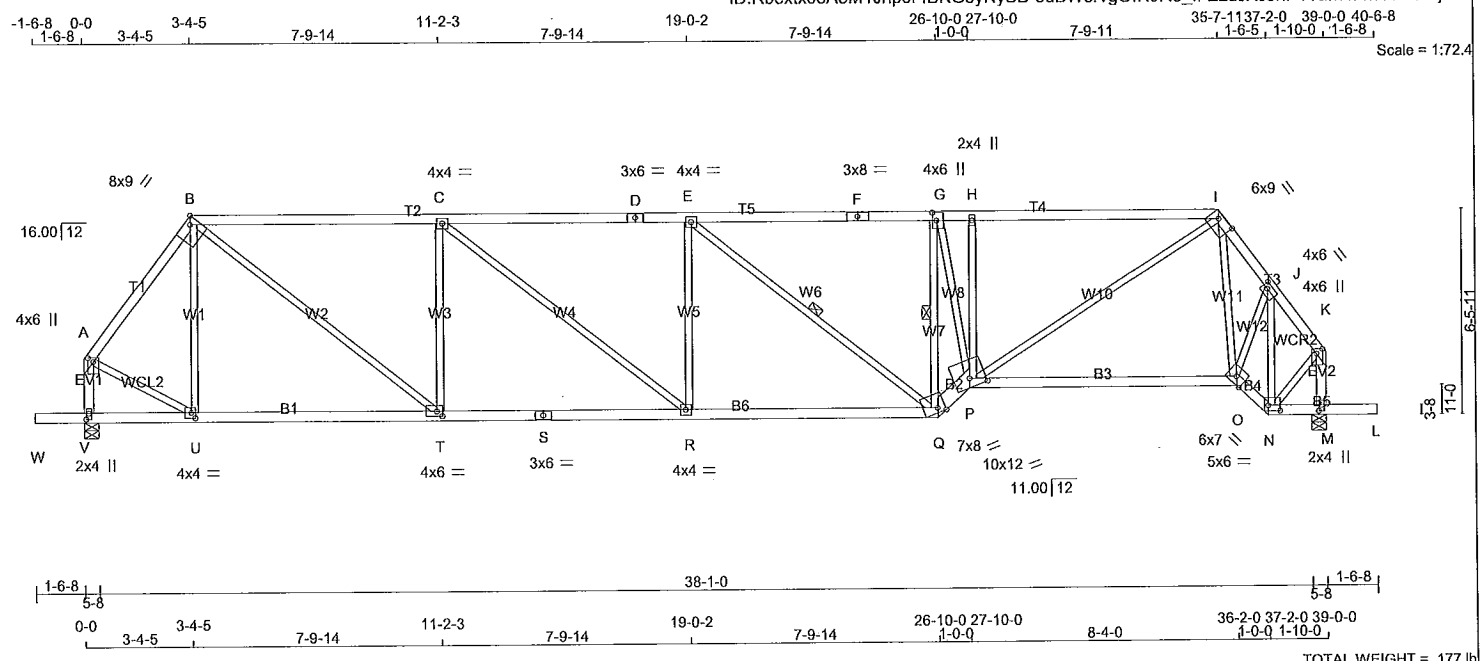


LUMBER

N. L. G. A. RULES

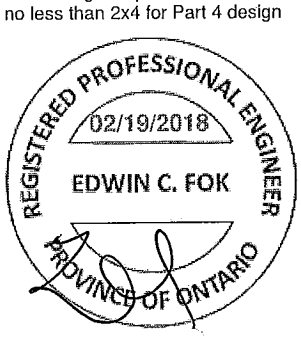
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	1650F 1.5E	SPF
D - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	1650F 1.5E	SPF
I - K	2x4	DRY	No.2	SPF
V - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
W - S	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - P	2x6	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	1.50	2.00
B	TTWW-h	MT20	8.0	9.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMWW+t	MT20	4.0	6.0	3.00	1.50
H	TMW+w	MT20	2.0	4.0		
I	TTWW-h	MT20	6.0	9.0	1.75	6.00
J	TMWW-t	MT20	4.0	6.0	1.75	2.00
K	TMVW+p	MT20	4.0	6.0	1.75	2.25
M	BMV1+p	MT20	2.0	4.0	2.25	1.00
N	BBWW-l	MT20	5.0	6.0	2.00	4.50
O	BBWW-h	MT20	6.0	7.0	Edge	
P	BBWWW-m	MT20	10.0	12.0	3.25	6.00
Q	BBWW-m	MT20	7.0	8.0	Edge	2.75
R	BMWW-t	MT20	4.0	4.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	6.0	1.75	2.00
U	BMWW-t	MT20	4.0	4.0	2.00	1.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
V	2027	0	2027	0	0	5-8	3-0
M	2033	0	2033	0	0	5-8	3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1445	882 / 0	0 / 0	0 / 0	0 / 0	563 / 0	0 / 0
M	1449	885 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-Q, G-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1610 / 0	-78.0	-78.0	0.21 (1)	5.02	U- B	-319 / 0	0.22 (1)
B- C	-2513 / 0	-78.0	-78.0	0.81 (1)	4.03	B- T	0 / 1982	0.45 (1)
C- D	-3034 / 0	-78.0	-78.0	0.90 (1)	3.70	T- C	-1082 / 0	0.74 (1)
D- E	-3034 / 0	-78.0	-78.0	0.90 (1)	3.70	C- R	0 / 663	0.15 (1)
E- F	-2625 / 0	-78.0	-78.0	0.67 (1)	4.22	R- E	-282 / 5	0.19 (1)
F- G	-2625 / 0	-78.0	-78.0	0.67 (1)	4.22	E- Q	-519 / 0	0.36 (1)
G- H	-2955 / 0	-78.0	-78.0	0.52 (1)	3.91	Q- G	-2012 / 0	0.55 (1)
H- I	-2964 / 0	-78.0	-78.0	0.77 (1)	3.91	G- P	0 / 1741	0.39 (1)
I- J	-1823 / 0	-78.0	-78.0	0.07 (1)	4.92	P- H	-425 / 0	0.20 (1)
J- K	-1319 / 0	-78.0	-78.0	0.05 (1)	5.59	P- I	0 / 2248	0.51 (1)
V- A	-1869 / 0	0.0	0.0	0.22 (1)	6.13	I- O	-93 / 22	0.04 (1)
M- K	-1805 / 0	0.0	0.0	0.21 (1)	6.23	O- J	0 / 921	0.21 (1)
						N- J	-1412 / 0	0.38 (1)
W- V	0 / 0	-96.5	-96.5	0.16 (1)	10.00	A- U	0 / 1057	0.24 (1)
V- U	0 / 0	-18.5	-18.5	0.17 (4)	10.00	N- K	0 / 1047	0.24 (1)
U- T	0 / 954	-18.5	-18.5	0.30 (4)	10.00			
T- S	0 / 2514	-18.5	-18.5	0.55 (1)	10.00			
S- R	0 / 2514	-18.5	-18.5	0.55 (1)	10.00			
R- Q	0 / 3034	-18.5	-18.5	0.63 (1)	10.00			
Q- P	0 / 3597	-18.5	-18.5	0.43 (1)	10.00			
P- O	0 / 1086	-18.5	-18.5	0.48 (4)	10.00			
O- N	0 / 1024	-18.5	-18.5	0.17 (1)	10.00			
N- M	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
M- L	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.46")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.90/1.00 (C-E:1) , BC=0.63/1.00 (Q-R:1) , WB=0.74/1.00 (C-T:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.83 (S) (INPUT = 1.00)

A-18023895

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	H9T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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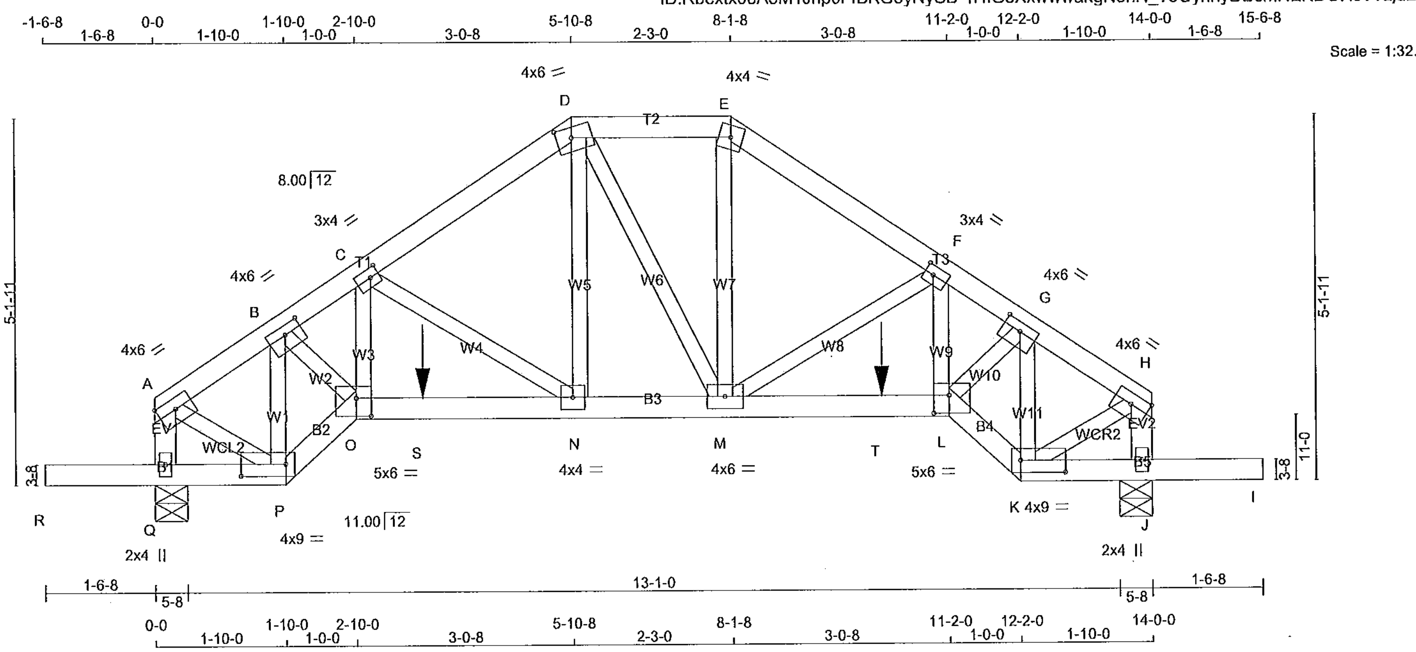


TOTAL WEIGHT = 67 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
Q - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

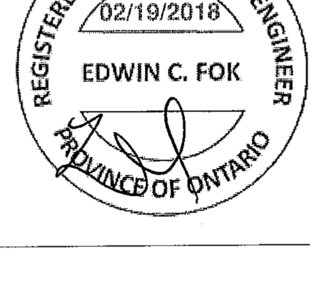
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TMWW-t	MT20	4.0	6.0	1.50	3.00
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TTWW-m	MT20	4.0	6.0	1.75	2.50
E	TTW-m	MT20	4.0	4.0		
F	TMWW-t	MT20	3.0	4.0	1.50	1.50
G	TMWW-t	MT20	4.0	6.0	1.50	3.00
H	TMVW-t	MT20	4.0	6.0	1.75	Edge
J	BMV1+p	MT20	2.0	4.0		
K	BBWW-I	MT20	4.0	9.0	2.00	7.50
L	BBWW-I	MT20	5.0	6.0	3.00	2.50
M	BMWWW-t	MT20	4.0	6.0		
N	BMWW-t	MT20	4.0	4.0		
O	BBWW-I	MT20	5.0	6.0	3.00	2.50
P	BBWW-I	MT20	4.0	9.0	2.00	7.50
Q	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	1549	0	1549	0	0	5-8	2-5
J	1549	0	1549	0	0	5-8	2-5

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
Q	1097	713 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0
J	1097	713 / 0	0 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1307 / 0	-78.0	-78.0	0.09 (1)	5.54
B- C	-2239 / 0	-78.0	-78.0	0.11 (1)	4.46
C- D	-1678 / 0	-78.0	-78.0	0.15 (1)	4.96
D- E	-1413 / 0	-78.0	-78.0	0.10 (1)	5.36
E- F	-1684 / 0	-78.0	-78.0	0.15 (1)	4.96
F- G	-2239 / 0	-78.0	-78.0	0.11 (1)	4.46
G- H	-1307 / 0	-78.0	-78.0	0.09 (1)	5.54
Q- A	-1321 / 0	0.0	0.0	0.15 (1)	7.00
J- H	-1321 / 0	0.0	0.0	0.15 (1)	7.00
R- Q	0 / 0	-96.5	-96.5	0.17 (1)	10.00
Q- P	0 / 0	-18.5	-18.5	0.17 (1)	10.00
P- O	0 / 1362	-18.5	-18.5	0.24 (1)	10.00
O- S	0 / 1863	-18.5	-18.5	0.81 (1)	10.00
S- N	0 / 1863	-112.0	-112.0	0.81 (1)	10.00
N- M	0 / 1408	-112.0	-112.0	0.51 (1)	10.00
M- T	0 / 1863	-112.0	-112.0	0.80 (1)	10.00
T- L	0 / 1863	-18.5	-18.5	0.80 (1)	10.00
L- K	0 / 1363	-18.5	-18.5	0.24 (1)	10.00
K- J	0 / 0	-18.5	-18.5	0.17 (1)	10.00
J- I	0 / 0	-96.5	-96.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- B	-1475 / 0	0.25 (1)
B- O	0 / 1250	0.31 (1)
O- C	0 / 408	0.10 (1)
C- N	-563 / 0	0.14 (1)
N- D	0 / 731	0.18 (1)
D- M	0 / 11	0.00 (1)
M- E	0 / 744	0.18 (1)
M- F	-558 / 0	0.14 (1)
L- F	0 / 402	0.10 (1)
L- G	0 / 1250	0.31 (1)
K- G	-1475 / 0	0.25 (1)
A- P	0 / 1197	0.30 (1)
K- H	0 / 1197	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
S	3-9-4	-423	-423	—	FRONT	VERT	TOTAL
T	10-2-12	-423	-423	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CStdGirder
START DISTANCE = 3-9-4
START SPAN CARRIED = 5-10-8
END DISTANCE = 10-2-12
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.15/1.00 (E-F:1) , BC=0.81/1.00 (N-O:1) , WB=0.31/1.00 (B-O:1) , SSI=0.30/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023896

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	H10T	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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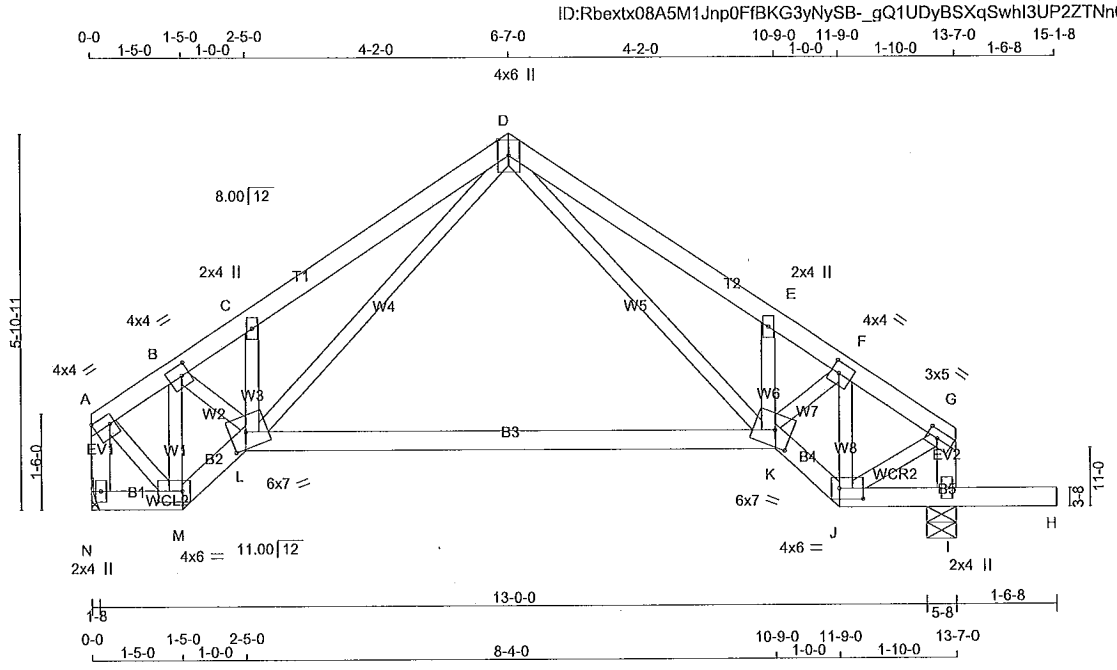


TOTAL WEIGHT = 3 X 60 = 179 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

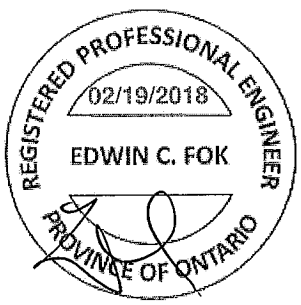
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.75	Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0	2.00	1.50
G	TMVW-t	MT20	3.0	5.0	1.50	2.00
I	BMV1+p	MT20	2.0	4.0		
J	BBWW-I	MT20	4.0	6.0	2.00	4.50
K	BBWWW-m	MT20	6.0	7.0	3.00	3.00
L	BBWWW-m	MT20	6.0	7.0	3.00	3.00
M	BBWW-I	MT20	4.0	6.0	2.00	4.50
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	647	0	647	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
I	812	0	812	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	461	281 / 0	0 / 0	0 / 0	0 / 0	180 / 0	0 / 0
I	579	354 / 0	0 / 0	0 / 0	0 / 0	226 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-468 / 0	-78.0	-78.0	0.04 (1)	6.25
B- C	-836 / 0	-78.0	-78.0	0.12 (1)	6.25
C- D	-919 / 0	-78.0	-78.0	0.17 (1)	6.25
D- E	-979 / 0	-78.0	-78.0	0.17 (1)	6.11
E- F	-905 / 0	-78.0	-78.0	0.12 (1)	6.25
F- G	-540 / 0	-78.0	-78.0	0.04 (1)	6.25
N- A	-634 / 0	0.0	0.0	0.07 (1)	7.81
I- G	-584 / 0	0.0	0.0	0.06 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00
M- L	0 / 475	-18.5	-18.5	0.08 (1)	10.00
L- K	0 / 434	-18.5	-18.5	0.41 (4)	10.00
K- J	0 / 562	-18.5	-18.5	0.09 (1)	10.00
J- I	0 / 0	-18.5	-18.5	0.12 (1)	10.00
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M- B	-610 / 0	0.09 (1)
B- L	0 / 565	0.13 (1)
L- C	-348 / 0	0.05 (1)
L- D	0 / 477	0.11 (1)
D- K	0 / 552	0.12 (1)
K- E	-332 / 0	0.05 (1)
K- F	0 / 546	0.12 (1)
J- F	-631 / 0	0.10 (1)
A- M	0 / 492	0.11 (1)
J- G	0 / 495	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 650 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.17/1.00 (D-E:1) , BC=0.41/1.00 (K-L:4) , WB=0.13/1.00 (B-L:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (K) (INPUT = 0.90)
JSI METAL= 0.22 (M) (INPUT = 1.00)

A-18023897

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	J1T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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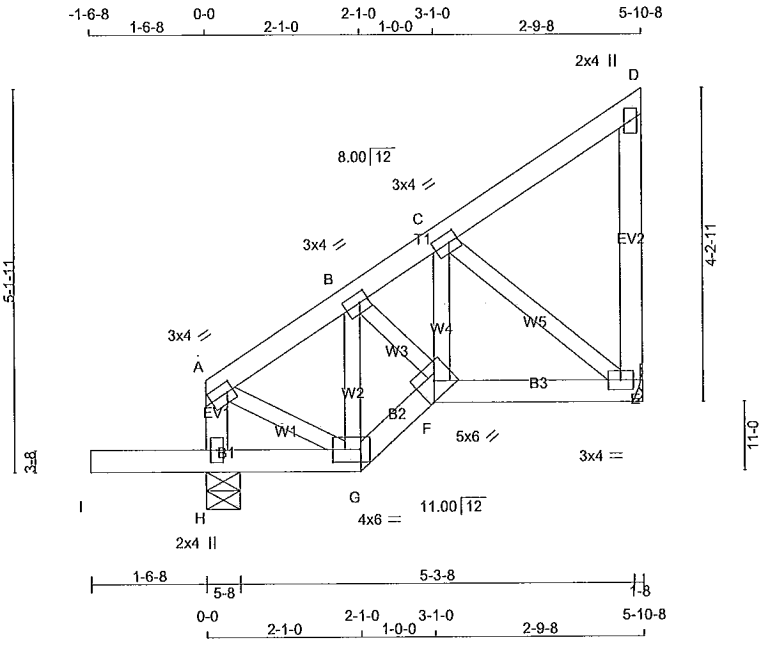


TOTAL WEIGHT = 30 lb

[M]

LUMBER

N. L. G. A. RULES

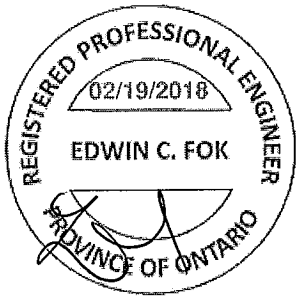
CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	452	0	452	0	0	5-8	1-8
E	264	0	264	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- A	-229 / 0	0.0	0.0	0.02 (1)		7.81	A- G	0 / 158	0.04 (1)
A- B	-171 / 0	-78.0	-78.0	0.04 (1)		6.25	G- B	-212 / 0	0.03 (1)
B- C	-223 / 0	-78.0	-78.0	0.06 (1)		6.25	B- F	0 / 96	0.02 (1)
C- D	-10 / 0	-78.0	-78.0	0.07 (1)		10.00	F- C	0 / 88	0.02 (4)
E- D	-87 / 0	0.0	0.0	0.02 (1)		7.81	C- E	-247 / 0	0.05 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)		10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (1)		10.00			
G- F	0 / 184	-18.5	-18.5	0.03 (1)		10.00			
F- E	0 / 194	-18.5	-18.5	0.06 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 542 (0.03")

CSI: TC=0.07/1.00 (C-D:1) , BC=0.16/1.00 (G-H:1) , WB=0.05/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

A-18023898

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	J1TA	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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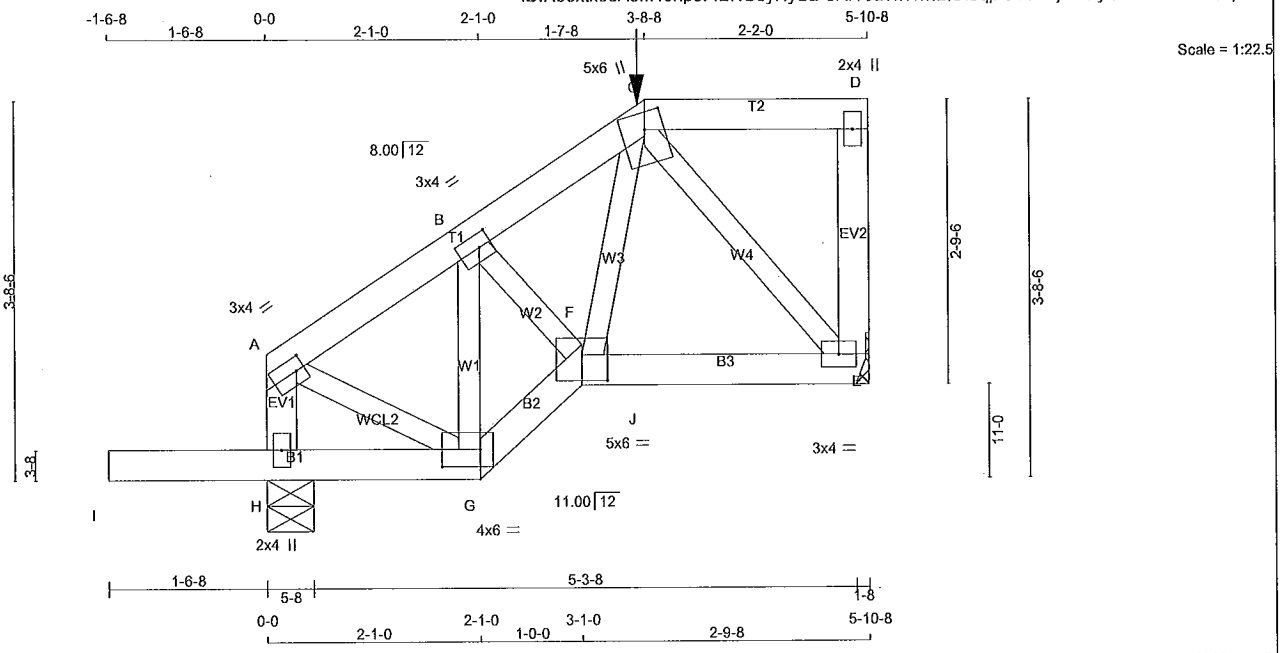


TOTAL WEIGHT = 2 X 29 = 57 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

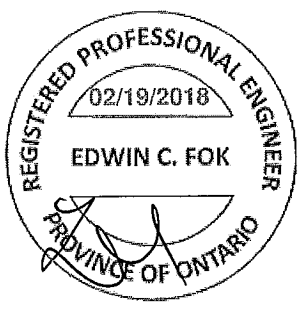
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TTWW+m	MT20	5.0	6.0	2.00	2.25
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-I	MT20	5.0	6.0	3.00	3.00
G	BBWW-I	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.4 lbs FACTORED DOWN AT 3-8-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	413	0	413	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	527	0	527	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	294	183 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
H	377	225 / 0	0 / 0	0 / 0	0 / 0	152 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-249 / 0	-78.0	-78.0	0.04 (1)	6.25	G- B	-287 / 0	0.05 (1)
B- C	-329 / 0	-78.0	-78.0	0.04 (1)	6.25	B- F	0 / 102	0.03 (1)
C- D	0 / 0	-111.3	-111.3	0.10 (1)	10.00	F- C	0 / 162	0.04 (1)
E- D	-121 / 0	0.0	0.0	0.02 (1)	7.81	C- E	-343 / 0	0.07 (1)
H- A	-296 / 0	0.0	0.0	0.03 (1)	7.81	A- G	0 / 231	0.06 (1)
I- H	0 / 0	-96.5	-96.5	0.17 (1)	10.00			
H- G	0 / 0	-26.4	-26.4	0.17 (1)	10.00			
G- F	0 / 270	-26.4	-26.4	0.05 (1)	10.00			
F- J	0 / 226	-26.4	-26.4	0.09 (4)	10.00			
J- E	0 / 226	-26.4	-26.4	0.09 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8-8	-106	-106	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-8-8
RIGHT SETBACK = 0-0
END SETBACK = 3-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 571 (0.03")

CSI: TC=0.10/1.00 (C-D:1) , BC=0.17/1.00 (G-H:1) , WB=0.07/1.00 (C-E:1) , SSI=0.13/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

A-18023899

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292634	J1TB	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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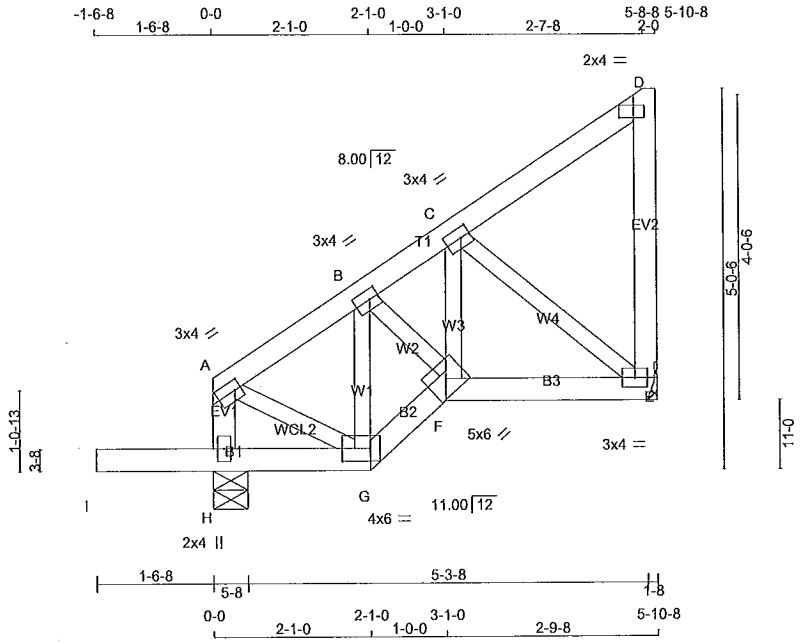


TOTAL WEIGHT = 2 X 30 = 61 lb

[M]

LUMBER
N. L. G. A. RULES

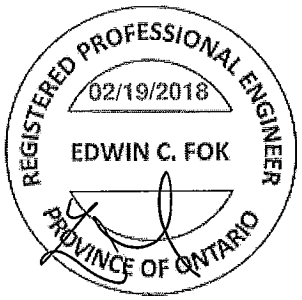
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - A	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMWW-t	MT20	3.0	4.0	1.50	1.50
C	TMWW-t	MT20	3.0	4.0	1.50	1.50
D	TVM-p	MT20	2.0	4.0	0.50	2.25
E	BMVW1-t	MT20	3.0	4.0		
F	BBWW-h	MT20	5.0	6.0		
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
E	264	0	264	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
H	452	0	452	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-171 / 0	-78.0	-78.0	0.04 (1)	6.25	G- B	-212 / 0	0.03 (1)
B- C	-223 / 0	-78.0	-78.0	0.06 (1)	6.25	B- F	0 / 96	0.02 (1)
C- D	-10 / 0	-78.0	-78.0	0.07 (1)	10.00	F- C	0 / 88	0.02 (4)
E- D	-87 / 0	0.0	0.0	0.03 (1)	7.81	C- E	-247 / 0	0.05 (1)
H- A	-229 / 0	0.0	0.0	0.02 (1)	7.81	A- G	0 / 158	0.04 (1)
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G- F	0 / 184	-18.5	-18.5	0.03 (1)	10.00			
F- E	0 / 194	-18.5	-18.5	0.06 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 542 (0.03")

CSI: TC=0.07/1.00 (C-D:1) , BC=0.16/1.00 (G-H:1) , WB=0.05/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.52 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

A-18023900

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292635	H1TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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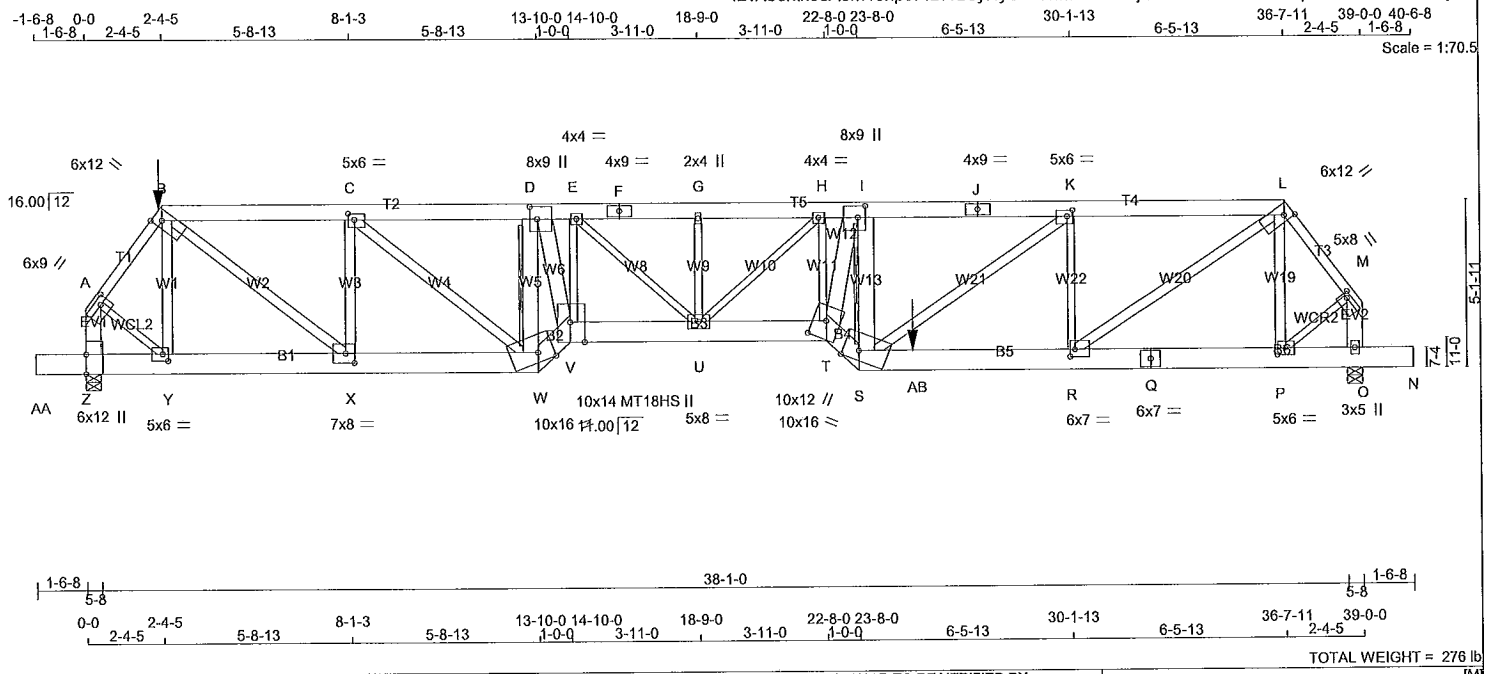


TOTAL WEIGHT = 276 lb

[M]

LUMBER

N. L. G. A. RULES

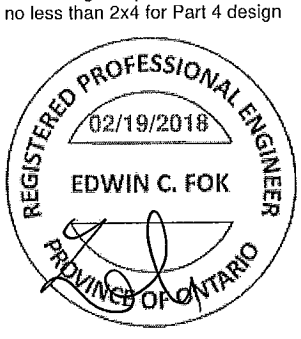
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	1650F 1.5E	SPF
F - J	2x6	DRY	1650F 1.5E	SPF
J - L	2x6	DRY	1650F 1.5E	SPF
L - M	2x4	DRY	No.2	SPF
Z - A	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA- W	2x8	DRY	No.2	SPF
W - V	2x8	DRY	1950F 1.7E	SPF
V - T	2x8	DRY	1950F 1.7E	SPF
T - S	2x8	DRY	1950F 1.7E	SPF
S - Q	2x8	DRY	1950F 1.7E	SPF
Q - N	2x8	DRY	1950F 1.7E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Y - B	2x4	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
X - C	2x4	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
W - D	2x6	DRY	No.2	SPF
D - V	2x6	DRY	1650F 1.5E	SPF
T - I	2x6	DRY	1650F 1.5E	SPF
S - I	2x6	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	6.0	9.0	2.25	3.00
B	TTWW+h	MT20	6.0	12.0	3.25	2.50
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TMWW+t	MT20	8.0	9.0	4.50	2.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	4.0	9.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWW+t	MT20	8.0	9.0	4.25	2.75

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	3933	0	3933	0	0	5-8	5-8
O	3443	0	3443	0	0	5-8	5-7

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2798	1739 / 0	0 / 0	0 / 0	0 / 0	1060 / 0	0 / 0
O	2443	1558 / 0	0 / 0	0 / 0	0 / 0	886 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O
BEARING SIZE FACTOR = 1.15 AT JNT(S) Z (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-W, I-S

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3241 / 0	-78.0	-78.0	0.24 (1)	3.69
B- C	-5492 / 0	-153.5	-153.5	0.37 (1)	3.96
C- D	-7713 / 0	-153.5	-153.5	0.47 (1)	3.33
D- E	-9825 / 0	-153.5	-153.5	0.41 (1)	3.03
E- F	-10378 / 0	-78.0	-78.0	0.47 (1)	2.91
F- G	-10378 / 0	-78.0	-78.0	0.47 (1)	2.91
G- H	-10378 / 0	-78.0	-78.0	0.47 (1)	2.91
H- I	-10382 / 0	-78.0	-78.0	0.44 (1)	2.92
I- J	-8241 / 0	-78.0	-78.0	0.41 (1)	3.33
J- K	-8241 / 0	-78.0	-78.0	0.41 (1)	3.33
K- L	-5639 / 0	-78.0	-78.0	0.28 (1)	4.02
L- M	-2799 / 0	-78.0	-78.0	0.20 (1)	3.97
Z- A	-3811 / 0	0.0	0.0	0.30 (1)	5.43
O- M	-3304 / 0	0.0	0.0	0.26 (1)	5.79
AA- Z	0 / 0	-96.5	-96.5	0.06 (1)	10.00
Z- Y	0 / 0	-36.4	-36.4	0.09 (1)	10.00
Y- X	0 / 1915	-36.4	-36.4	0.29 (1)	10.00
X- W	0 / 5493	-36.4	-36.4	0.68 (1)	10.00
W- V	0 / 9899	-36.4	-36.4	0.45 (1)	10.00
V- U	0 / 9838	-112.0	-112.0	0.54 (1)	10.00
U- T	0 / 10380	-112.0	-112.0	0.56 (1)	10.00
T- S	0 / 10632	-112.0	-112.0	0.49 (1)	10.00
S-AB	0 / 5642	-112.0	-112.0	0.58 (1)	10.00
AB- R	0 / 5642	-18.5	-18.5	0.58 (1)	10.00
R- Q	0 / 1649	-18.5	-18.5	0.15 (1)	10.00
Q- P	0 / 1649	-18.5	-18.5	0.15 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- B	-862 / 0	0.21 (1)
B- X	0 / 4587	0.81 (1)
X- C	-2769 / 0	0.66 (1)
C- W	0 / 2836	0.50 (1)
W- D	-8278 / 0	0.87 (1)
D- V	0 / 7900	0.64 (1)
V- E	-363 / 0	0.09 (1)
E- U	0 / 755	0.19 (1)
U- G	-483 / 0	0.11 (1)
U- H	0 / 88	0.03 (4)
T- H	0 / 129	0.03 (1)
T- I	0 / 7951	0.65 (1)
S- I	-8032 / 0	0.85 (1)
P- L	-893 / 0	0.21 (1)
A- Y	0 / 2244	0.56 (1)
P- M	0 / 1938	0.48 (1)
R- L	0 / 4893	0.87 (1)
S- K	0 / 3179	0.56 (1)
R- K	-2410 / 0	0.80 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 14-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 25-3-8
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 891 (0.53")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 469 (1.00")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

A-18023901

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292635	H1TA	1	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TS-t	MT20	4.0	9.0		
K	TMWW-t	MT20	5.0	6.0	2.25	2.00
L	TTWW+h	MT20	6.0	12.0	3.25	2.25
M	TMVW-t	MT20	5.0	8.0	1.50	2.00
O	BMV1+p	MT20	3.0	5.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BS-t	MT20	6.0	7.0		
R	BMWW-t	MT20	6.0	7.0	2.75	1.75
S	BBWW-m	MT20	10.0	16.0	Edge	5.75
T	BBWW+m	MT20	10.0	12.0	6.50	4.75
U	BMWWW-t	MT20	5.0	8.0		
V	BBWW+p	MT18HS	10.0	14.0	Edge	5.25
W	BBWW-m	MT20	10.0	16.0	Edge	5.75
X	BMWW-t	MT20	7.0	8.0	3.50	3.25
Y	BMWW-t	MT20	5.0	6.0	2.50	2.00
Z	BMV1+t	MT20	6.0	12.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5 ON TOP CHORD, AND 988.9 lbs FACTORED DOWN AT 25-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P- O	0 / 0	-18.5	-18.5	0.03 (4)	10.00			
O- N	0 / 0	-96.5	-96.5	0.03 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-4-5	-134	-134	---	FRONT	VERT	TOTAL
AB	25-3-8	-989	-989	---	FRONT	VERT	TOTAL

CSI: TC=0.47/1.00 (G-H:1) , BC=0.68/1.00 (W-X:1) , WB=0.87/1.00 (D-W:1) , SSI=0.52/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

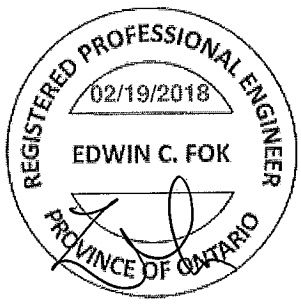
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MT18HS	511	354	2455	1382	3004	2010

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 1.00 (T) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292635	H2TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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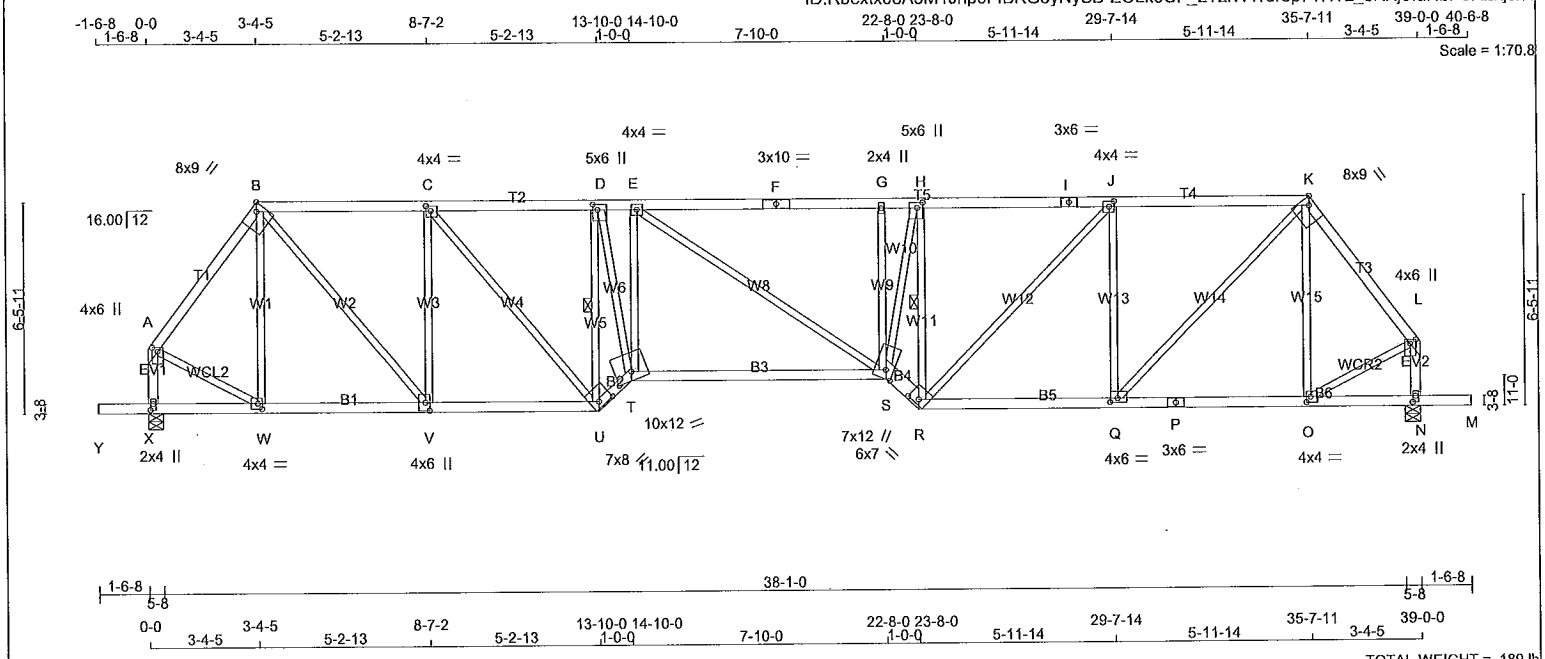


TOTAL WEIGHT = 189 lb

LUMBER
N. L. G. A. RULES

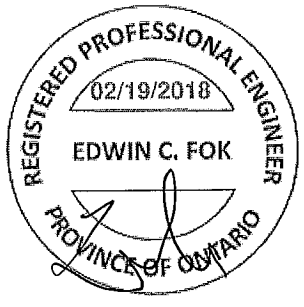
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	1650F 1.5E	SPF
I - K	2x4	DRY	1650F 1.5E	SPF
K - L	2x4	DRY	No.2	SPF
X - A	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - R	2x6	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	1.50	2.00
B	TTWW-h	MT20	8.0	9.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	1.75	1.75
D	TMWW+t	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	10.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	5.0	6.0	2.00	2.00
I	TS-t	MT20	3.0	6.0		
J	TMWW-t	MT20	4.0	4.0	2.00	1.75
K	TTWW-h	MT20	8.0	9.0	Edge	2.75
L	TMVW+p	MT20	4.0	6.0	1.50	2.00
N	BMV1+p	MT20	2.0	4.0	2.25	1.00
O	BMWW-t	MT20	4.0	4.0	2.00	1.50
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	6.0	1.50	2.75
R	BBWW-h	MT20	6.0	7.0	1.75	3.75
S	BBWWW+m	MT20	7.0	12.0	3.25	2.75
T	BBWW-m	MT20	10.0	12.0	3.25	6.00
U	BBWW-h	MT20	7.0	8.0	2.00	5.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2030	0	2030	0	0	5-8	3-0
N	2030	0	2030	0	0	5-8	3-0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0
N	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-U, H-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	MAX. UNBRAC LENGTH
A- B	-1602 / 0	-78.0	-78.0	0.21 (1)		5.04
B- C	-2083 / 0	-78.0	-78.0	0.32 (1)		4.93
C- D	-2749 / 0	-78.0	-78.0	0.35 (1)		4.40
D- E	-3350 / 0	-78.0	-78.0	0.38 (1)		3.96
E- F	-3506 / 0	-78.0	-78.0	0.64 (1)		3.87
F- G	-3506 / 0	-78.0	-78.0	0.64 (1)		3.87
G- H	-3497 / 0	-78.0	-78.0	0.39 (1)		3.87
H- I	-2897 / 0	-78.0	-78.0	0.47 (1)		4.19
I- J	-2897 / 0	-78.0	-78.0	0.47 (1)		4.19
J- K	-2220 / 0	-78.0	-78.0	0.43 (1)		4.68
K- L	-1604 / 0	-78.0	-78.0	0.21 (1)		5.03
X- A	-1860 / 0	0.0	0.0	0.21 (1)		6.15
N- L	-1862 / 0	0.0	0.0	0.21 (1)		6.15
Y- X	0 / 0	-96.5	-96.5	0.16 (1)		10.00
X- W	0 / 0	-18.5	-18.5	0.08 (4)		10.00
W- V	0 / 948	-18.5	-18.5	0.21 (1)		10.00
V- U	0 / 2084	-18.5	-18.5	0.40 (1)		10.00
U- T	0 / 3788	-18.5	-18.5	0.61 (1)		10.00
T- S	0 / 3361	-18.5	-18.5	0.73 (1)		10.00
S- R	0 / 3856	-18.5	-18.5	0.46 (1)		10.00
R- Q	0 / 2221	-18.5	-18.5	0.44 (1)		10.00
Q- P	0 / 950	-18.5	-18.5	0.25 (4)		10.00
P- O	0 / 950	-18.5	-18.5	0.25 (4)		10.00
O- N	0 / 0	-18.5	-18.5	0.09 (4)		10.00
N- M	0 / 0	-96.5	-96.5	0.16 (1)		10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- B	-354 / 0	0.24 (1)
W- V	0 / 1748	0.39 (1)
V- C	-1234 / 0	0.84 (1)
V- U	0 / 1026	0.23 (1)
C- U	-3282 / 0	0.89 (1)
U- D	0 / 3232	0.73 (1)
D- T	-567 / 0	0.26 (1)
T- E	0 / 174	0.04 (1)
E- S	-459 / 0	0.21 (1)
S- G	0 / 3142	0.71 (1)
S- H	-3210 / 0	0.87 (1)
R- H	0 / 966	0.22 (1)
R- J	-1210 / 0	0.82 (1)
Q- J	0 / 1820	0.41 (1)
Q- K	-347 / 0	0.24 (1)
O- K	0 / 1052	0.24 (1)
A- W	0 / 1053	0.24 (1)
O- L		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 625 (0.75")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.64/1.00 (E-G:1) , BC=0.73/1.00 (S-T:1) , WB=0.89/1.00 (D-U:1) , SSI=0.23/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.68 (R) (INPUT = 1.00)

A-18023902

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292635	H3TA	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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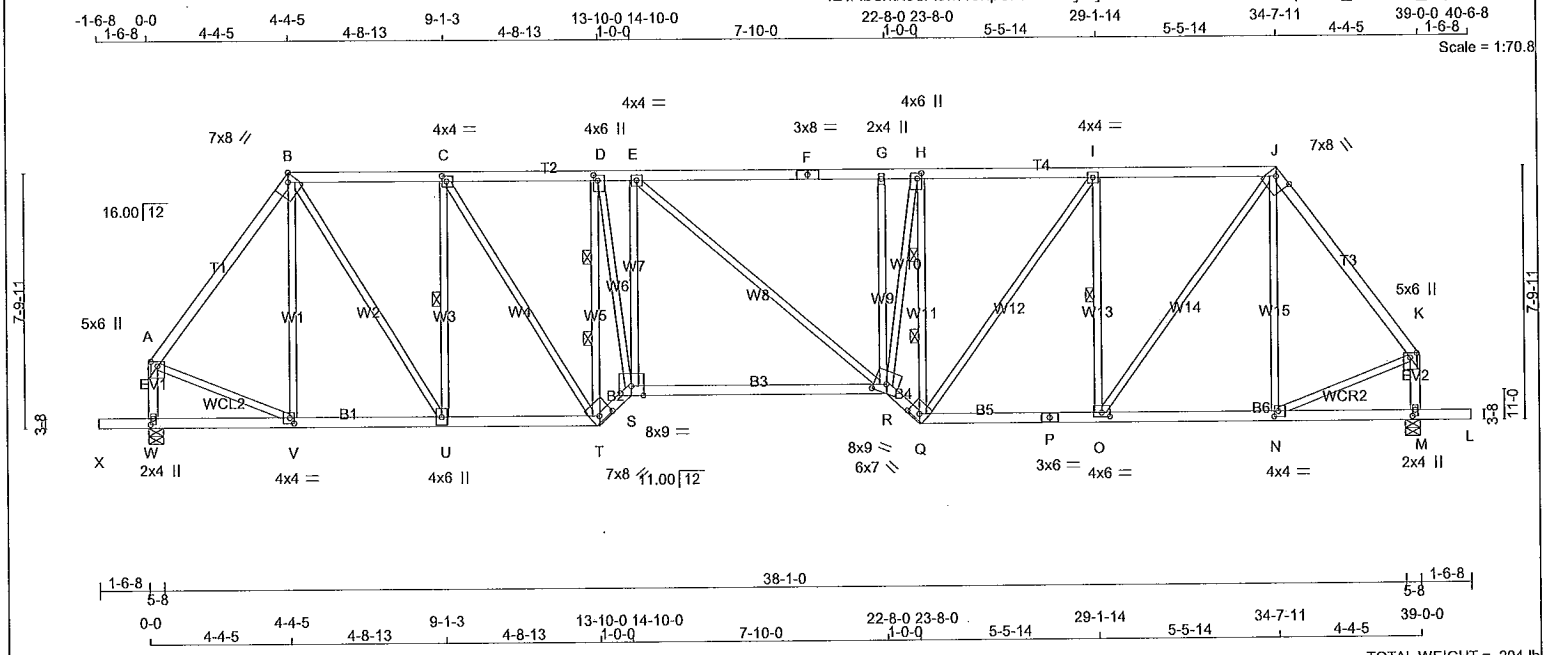


TOTAL WEIGHT = 204 lb

[M]

LUMBER

N. L. G. A. RULES

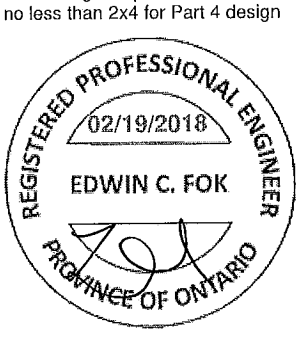
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
F - J	2x4	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
W - A	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
X - T	2x4	DRY	No.2	SPF
T - S	2x4	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - P	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.75	2.50
B	TTWW-h	MT20	7.0	8.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW+t	MT20	4.0	6.0	2.00	1.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	4.0	6.0	2.00	1.50
I	TMWW-t	MT20	4.0	4.0		
J	TTWW-h	MT20	7.0	8.0	2.00	5.25
K	TMVW+p	MT20	5.0	6.0	1.75	2.50
M	BMV1+p	MT20	2.0	4.0	2.25	1.00
N	BMWW-t	MT20	4.0	4.0	2.00	1.50
O	BMWW-t	MT20	4.0	6.0	1.50	3.00
P	BS-t	MT20	3.0	6.0		
Q	BBWW-h	MT20	6.0	7.0	2.00	4.00
R	BBWWW-m	MT20	8.0	9.0	3.25	4.50
S	BBWW-t	MT20	8.0	9.0	Edge	
T	BBWW-h	MT20	7.0	8.0	2.00	5.00
U	BMWW+t	MT20	4.0	6.0		
V	BMWW-t	MT20	4.0	4.0	2.00	1.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
W	2030	0	2030	0	0	5-8	3-0
M	2030	0	2030	0	0	5-8	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0
M	1447	883 / 0	0 / 0	0 / 0	0 / 0	564 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.60 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-U, I-O.
2 LATERAL BRACE(S) AT 1/ 3 LENGTH OF D-T, H-Q.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1651 / 0	-78.0	-78.0	0.37 (1)	4.83
B- C	-1775 / 0	-78.0	-78.0	0.33 (1)	4.68
C- D	-2258 / 0	-78.0	-78.0	0.37 (1)	4.21
D- E	-2671 / 0	-78.0	-78.0	0.48 (1)	3.68
E- F	-2766 / 0	-78.0	-78.0	0.80 (1)	3.60
F- G	-2766 / 0	-78.0	-78.0	0.80 (1)	3.60
G- H	-2758 / 0	-78.0	-78.0	0.49 (1)	3.61
H- I	-2357 / 0	-78.0	-78.0	0.49 (1)	3.99
I- J	-1882 / 0	-78.0	-78.0	0.45 (1)	4.42
J- K	-1652 / 0	-78.0	-78.0	0.37 (1)	4.82
W- A	-1849 / 0	0.0	0.0	0.21 (1)	6.16
M- K	-1850 / 0	0.0	0.0	0.21 (1)	6.16
X- W	0 / 0	-96.5	-96.5	0.16 (1)	10.00
W- V	0 / 0	-18.5	-18.5	0.09 (4)	10.00
V- U	0 / 982	-18.5	-18.5	0.21 (1)	10.00
U- T	0 / 1776	-18.5	-18.5	0.33 (1)	10.00
T- S	0 / 3107	-18.5	-18.5	0.50 (1)	10.00
S- R	0 / 2680	-18.5	-18.5	0.62 (1)	10.00
R- Q	0 / 3146	-18.5	-18.5	0.51 (1)	10.00
Q- P	0 / 1882	-18.5	-18.5	0.36 (1)	10.00
P- O	0 / 1882	-18.5	-18.5	0.36 (1)	10.00
O- N	0 / 982	-18.5	-18.5	0.22 (4)	10.00
N- M	0 / 0	-18.5	-18.5	0.10 (4)	10.00
M- L	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- B	-260 / 0	0.30 (1)
B- U	0 / 1482	0.33 (1)
U- C	-1168 / 0	0.45 (1)
C- T	0 / 902	0.20 (1)
T- D	-2807 / 0	1.00 (1)
D- S	0 / 2765	0.62 (1)
S- E	-558 / 0	0.45 (1)
E- R	0 / 113	0.03 (1)
R- G	-479 / 0	0.39 (1)
R- H	0 / 2650	0.60 (1)
Q- H	-2714 / 0	0.96 (1)
Q- I	0 / 804	0.18 (1)
O- I	-1125 / 0	0.43 (1)
O- J	0 / 1519	0.34 (1)
N- J	-253 / 0	0.29 (1)
A- V	0 / 1047	0.24 (1)
N- K	0 / 1048	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.25")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 721 (0.65")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.80/1.00 (E-G:1) , BC=0.62/1.00 (R-S:1) , WB=1.00/1.00 (D-T:1) , SSI=0.23/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.61 (D) (INPUT = 1.00)

A-18023903

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292636	H1TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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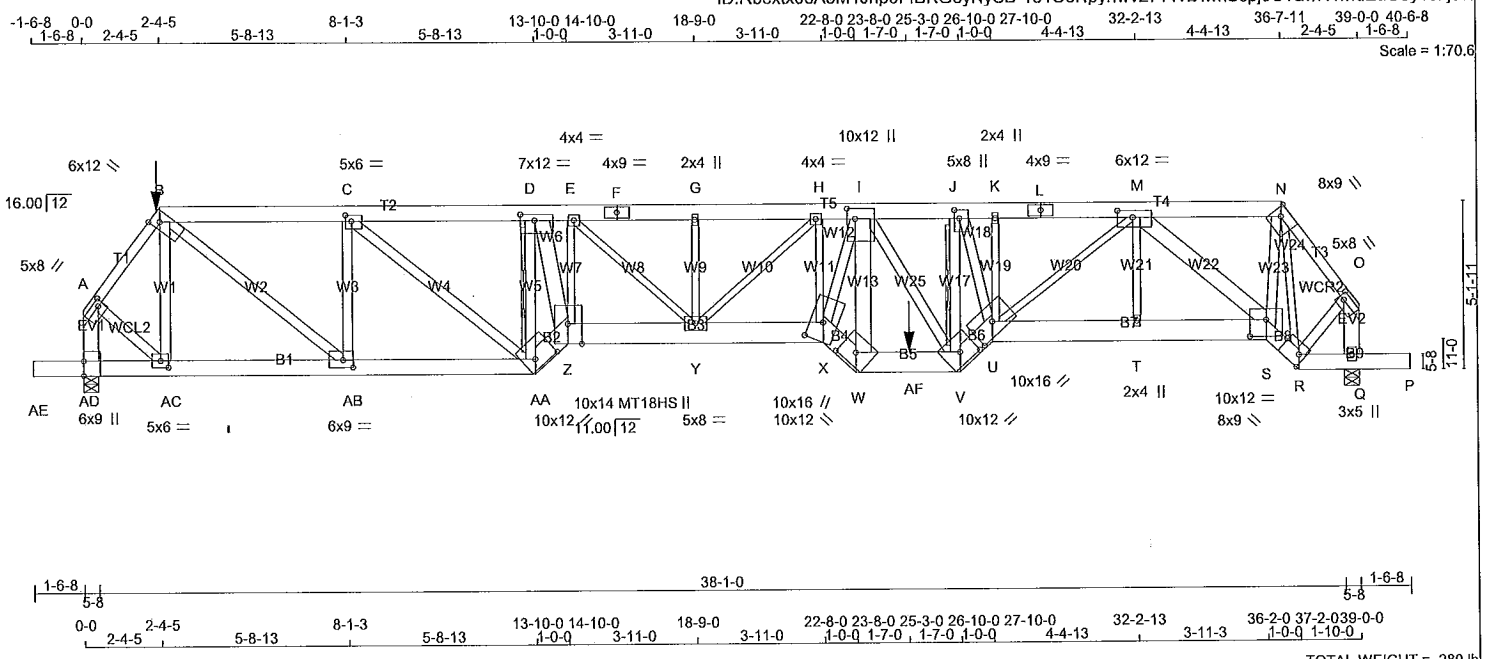


LUMBER

N. L. G. A. RULES

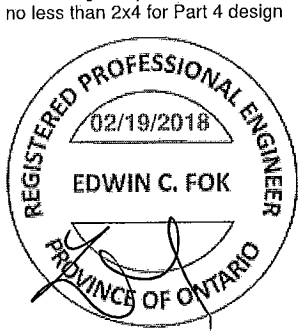
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	1650F 1.5E	SPF
F - L	2x6	DRY	1650F 1.5E	SPF
L - N	2x6	DRY	1650F 1.5E	SPF
N - O	2x4	DRY	No.2	SPF
AD- A	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
AE- AA	2x6	DRY	No.2	SPF
AA- Z	2x8	DRY	1950F 1.7E	SPF
Z - X	2x8	DRY	1950F 1.7E	SPF
X - W	2x8	DRY	1950F 1.7E	SPF
W - V	2x8	DRY	1950F 1.7E	SPF
V - U	2x8	DRY	1950F 1.7E	SPF
U - S	2x8	DRY	1950F 1.7E	SPF
S - R	2x6	DRY	No.2	SPF
R - P	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
AC- B	2x4	DRY	1650F 1.5E	SPF
B - AB	2x4	DRY	No.2	SPF
AB- C	2x4	DRY	1650F 1.5E	SPF
C - AA	2x4	DRY	No.2	SPF
AA- D	2x4	DRY	1650F 1.5E	SPF
D - Z	2x6	DRY	1650F 1.5E	SPF
X - I	2x4	DRY	2100F 1.8E	SPF
W - I	2x6	DRY	No.2	SPF
V - J	2x4	DRY	No.2	SPF
J - U	2x4	DRY	2100F 1.8E	SPF
M - S	2x6	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
A - AC	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
I - V	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.00	2.00
B	TTWW+h	MT20	6.0	12.0	3.25	2.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AD	3930	0	3930	0	0	5-8	5-8
Q	3446	0	3446	0	0	5-8	5-7

ALLOW FOR 0.4" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

1ST LCASE MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AD	2796	1738 / 0	0 / 0	0 / 0	0 / 0	1059 / 0	0 / 0
Q	2445	1559 / 0	0 / 0	0 / 0	0 / 0	886 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AD, Q
BEARING SIZE FACTOR = 1.15 AT JNT(S) AD (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-AA, J-V

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-3170 / 0	-78.0	-78.0	0.23 (1)	3.73	AC- B	-944 / 0	0.17 (1)
B- C	-5429 / 0	-153.5	-153.5	0.38 (1)	3.96	B-AB	0 / 4591	0.81 (1)
C- D	-7627 / 0	-153.5	-153.5	0.48 (1)	3.33	AB- C	-2788 / 0	0.50 (1)
D- E	-9807 / 0	-153.5	-153.5	0.42 (1)	3.02	C-AA	0 / 2815	0.50 (1)
E- F	-10371 / 0	-78.0	-78.0	0.48 (1)	2.90	AA- D	-8572 / 0	0.93 (1)
F- G	-10371 / 0	-78.0	-78.0	0.48 (1)	2.90	D- Z	0 / 8163	0.67 (1)
G- H	-10371 / 0	-78.0	-78.0	0.51 (1)	2.87	Z- E	-346 / 0	0.08 (1)
H- I	-10347 / 0	-78.0	-78.0	0.48 (1)	2.87	E- Y	0 / 772	0.19 (1)
I- J	-7199 / 0	-78.0	-78.0	0.30 (1)	3.59	Y- G	-526 / 0	0.12 (1)
J- K	-8334 / 0	-78.0	-78.0	0.39 (1)	3.26	Y- H	0 / 106	0.04 (4)
K- L	-8328 / 0	-78.0	-78.0	0.42 (1)	3.26	X- H	0 / 375	0.09 (1)
L- M	-8328 / 0	-78.0	-78.0	0.42 (1)	3.26	X- I	0 / 7725	0.64 (1)
M- N	-2389 / 0	-78.0	-78.0	0.14 (1)	5.77	W- I	-6547 / 0	0.99 (1)
N- O	-2433 / 0	-78.0	-78.0	0.17 (1)	4.25	V- J	-4330 / 0	0.66 (1)
AD- A	-3784 / 0	0.0	0.0	0.31 (1)	5.46	J- U	0 / 4241	0.35 (1)
Q- O	-3218 / 0	0.0	0.0	0.26 (1)	5.86	U- K	0 / 188	0.05 (1)
						U- M	0 / 3683	0.91 (1)
AE-AD	0 / 0	-96.5	-96.5	0.08 (1)	10.00	T- M	0 / 79	0.03 (4)
AD-AC	0 / 0	-36.4	-36.4	0.10 (4)	10.00	M- S	-4252 / 0	0.92 (1)
AC-AB	0 / 1870	-36.4	-36.4	0.33 (1)	10.00	S- N	0 / 4695	0.83 (1)
AB-AA	0 / 5429	-36.4	-36.4	0.80 (1)	10.00	N- R	-2938 / 0	0.97 (1)
AA- Z	0 / 10160	-36.4	-36.4	0.47 (1)	10.00	A-AC	0 / 2222	0.39 (1)
Z- Y	0 / 9818	-112.0	-112.0	0.53 (1)	10.00	R- O	0 / 1874	0.33 (1)
Y- X	0 / 10338	-112.0	-112.0	0.56 (1)	10.00	I- V	-1854 / 0	0.58 (1)
X- W	0 / 10682	-112.0	-112.0	0.49 (1)	10.00			
W-AF	0 / 8203	-112.0	-112.0	0.59 (1)	10.00			
AF- V	0 / 8203	-18.5	-18.5	0.59 (1)	10.00			

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 14-10-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-10-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 25-3-8
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 824 (0.57")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 434 (1.08")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 299 (0.06")

A-18023904

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292636	H1TB	1	1	TRUSS DESC.	

PLATES (table is in inches)

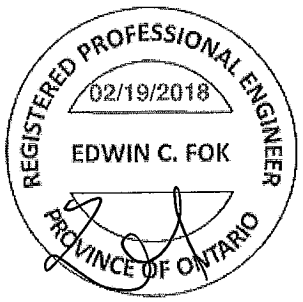
JT	TYPE	PLATES	W	LEN	Y	X
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TMWW-t	MT20	7.0	12.0	2.25	5.25
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	4.0	9.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	4.0		
I	TMWWW+t	MT20	10.0	12.0	3.75	3.00
J	TMWW+t	MT20	5.0	8.0	3.00	1.75
K	TMW+w	MT20	2.0	4.0		
L	TS-t	MT20	4.0	9.0		
M	TMWWW-t	MT20	6.0	12.0	2.50	5.50
N	TTWW-h	MT20	8.0	9.0	3.25	3.00
O	TMVW-t	MT20	5.0	8.0	2.25	2.00
Q	BMV1+p	MT20	3.0	5.0		
R	BBWW-h	MT20	8.0	9.0	4.00	2.25
S	BBWW-I	MT20	10.0	12.0	6.00	6.00
T	BMW+w	MT20	2.0	4.0		
U	BBWWW-h	MT20	10.0	16.0	4.50	8.00
V	BBWW-h	MT20	10.0	12.0	4.25	6.00
W	BBW-h	MT20	10.0	12.0	4.25	6.00
X	BBWW+m	MT20	10.0	16.0	6.75	4.75
Y	BMWWW-t	MT20	5.0	8.0		
Z	BBWW+p	MT18HS	10.0	14.0	Edge	5.25
AA	BBWW-h	MT20	10.0	12.0	3.50	7.75
AB	BMWW-t	MT20	6.0	9.0	2.75	3.75
AC	BMWW-t	MT20	5.0	6.0	2.50	2.75
AD	BMV1+t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5 ON TOP CHORD, AND 988.9 lbs FACTORED DOWN AT 25-3-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-U	0 / 9577	-18.5	-18.5	0.44 (1)	10.00			
U-T	0 / 5489	-18.5	-18.5	0.26 (1)	10.00			
T-S	0 / 5489	-18.5	-18.5	0.26 (1)	10.00			
S-R	0 / 2472	-18.5	-18.5	0.32 (1)	10.00			
R-Q	-1 / 0	-18.5	-18.5	0.08 (1)	10.00			
Q-P	0 / 0	-96.5	-96.5	0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-4-5	-134	-134	---	FRONT	VERT	TOTAL
AF	25-3-8	-989	-989	---	FRONT	VERT	TOTAL

ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 158 (0.12")

CSI: TC=0.51/1.00 (G-H:1) , BC=0.80/1.00 (AA-AB:1) , WB=0.99/1.00 (I-W:1) , SSI=0.37/1.00 (V-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MT18HS	511	354	2455	1382	3004	2010

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (AA) (INPUT = 0.90)
JSI METAL= 0.99 (D) (INPUT = 1.00)

A-18023904(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292636	H2TB	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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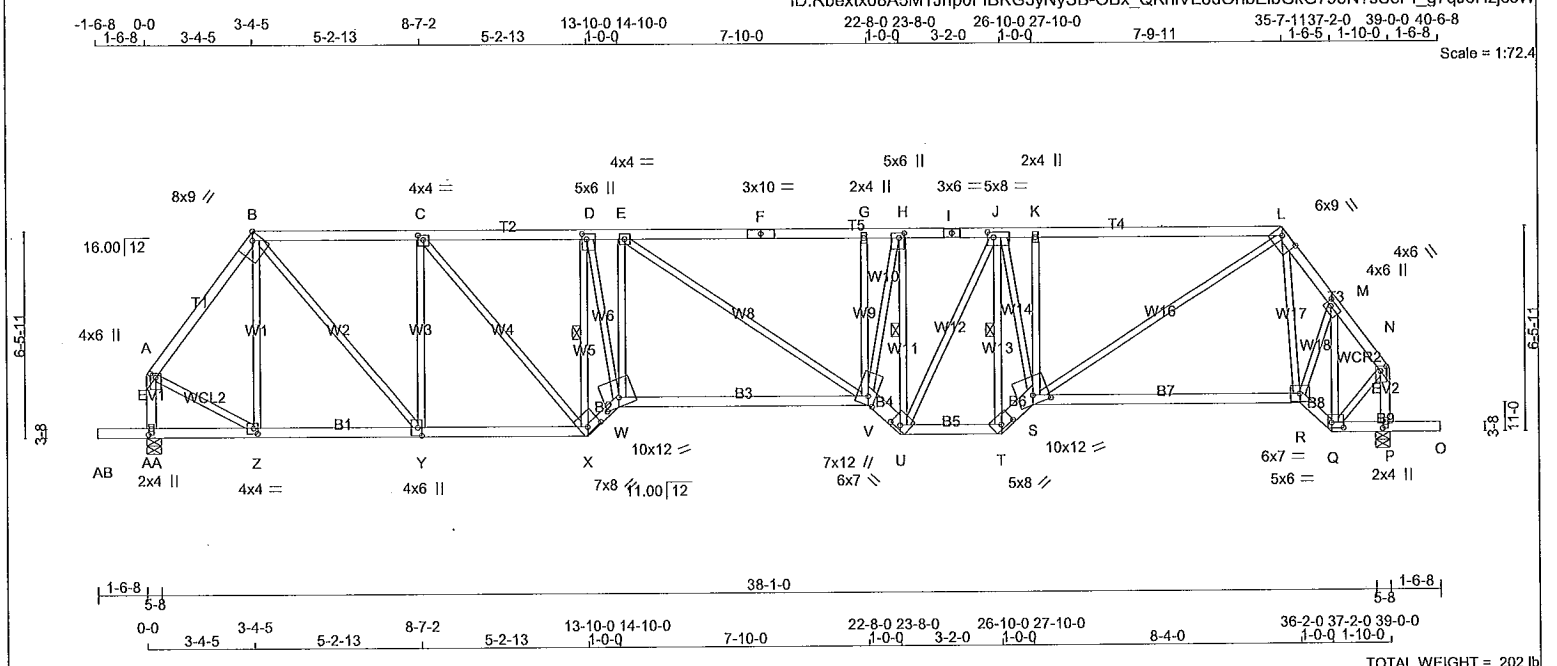


TOTAL WEIGHT = 202 lb

[M]

LUMBER

N. L. G. A. RULES

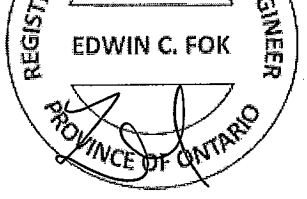
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x4	DRY	1650F 1.5E	SPF
F - I	2x4	DRY	1650F 1.5E	SPF
I - L	2x4	DRY	1650F 1.5E	SPF
L - N	2x4	DRY	No.2	SPF
AA- A	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AB- X	2x4	DRY	No.2	SPF
X - W	2x4	DRY	No.2	SPF
W - V	2x4	DRY	No.2	SPF
V - U	2x6	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - S	2x6	DRY	No.2	SPF
S - R	2x4	DRY	No.2	SPF
R - Q	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	1.50	2.00
B	TTWW-h	MT20	8.0	9.0	Edge	2.75
C	TMWW-t	MT20	4.0	4.0	1.75	2.00
D	TMWW+t	MT20	5.0	6.0	2.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	10.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW+t	MT20	5.0	6.0	1.75	2.00
I	TS-t	MT20	3.0	6.0		
J	TMWWW-t	MT20	5.0	8.0	2.00	2.25
K	TMW+w	MT20	2.0	4.0		
L	TTWW-h	MT20	6.0	9.0	1.75	6.00
M	TMWW-t	MT20	4.0	6.0	1.75	2.00
N	TMVW+p	MT20	4.0	6.0	1.75	2.25
P	BMV1+p	MT20	2.0	4.0	2.25	1.00
Q	BBWW-l	MT20	5.0	6.0	2.00	4.50
R	BBWW-l	MT20	6.0	7.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
AA	2027	0	2027	0	0	5-8	3-0
P	2033	0	2033	0	0	5-8	3-0

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	1445	882 / 0	0 / 0	0 / 0	0 / 0	563 / 0	0 / 0
P	1449	885 / 0	0 / 0	0 / 0	0 / 0	565 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, P

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-X, H-U, J-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1599 / 0	-78.0	-78.0	0.21 (1)	5.04
B- C	-2079 / 0	-78.0	-78.0	0.32 (1)	4.94
C- D	-2742 / 0	-78.0	-78.0	0.35 (1)	4.41
D- E	-3341 / 0	-78.0	-78.0	0.37 (1)	3.97
E- F	-3495 / 0	-78.0	-78.0	0.65 (1)	3.86
F- G	-3495 / 0	-78.0	-78.0	0.65 (1)	3.86
G- H	-3483 / 0	-78.0	-78.0	0.40 (1)	3.87
H- I	-2860 / 0	-78.0	-78.0	0.18 (1)	4.51
I- J	-2860 / 0	-78.0	-78.0	0.18 (1)	4.51
J- K	-2952 / 0	-78.0	-78.0	0.53 (1)	3.91
K- L	-2965 / 0	-78.0	-78.0	0.78 (1)	3.89
L- M	-1823 / 0	-78.0	-78.0	0.07 (1)	4.92
M- N	-1319 / 0	-78.0	-78.0	0.05 (1)	5.59
AA- A	-1857 / 0	0.0	0.0	0.21 (1)	6.15
P- N	-1805 / 0	0.0	0.0	0.21 (1)	6.23
AB-AA	0 / 0	-96.5	-96.5	0.16 (1)	10.00
AA- Z	0 / 0	-18.5	-18.5	0.08 (4)	10.00
Z- Y	0 / 947	-18.5	-18.5	0.21 (1)	10.00
Y- X	0 / 2080	-18.5	-18.5	0.40 (1)	10.00
X- W	0 / 3779	-18.5	-18.5	0.61 (1)	10.00
W- V	0 / 3353	-18.5	-18.5	0.73 (1)	10.00
V- U	0 / 3809	-18.5	-18.5	0.45 (1)	10.00
U- T	0 / 2598	-18.5	-18.5	0.45 (1)	10.00
T- S	0 / 3472	-18.5	-18.5	0.41 (1)	10.00
S- R	0 / 1086	-18.5	-18.5	0.48 (4)	10.00
R- Q	0 / 1024	-18.5	-18.5	0.17 (1)	10.00
Q- P	0 / 0	-18.5	-18.5	0.16 (1)	10.00
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Z- B	-353 / 0	0.24 (1)
B- Y	0 / 1745	0.39 (1)
Y- C	-1231 / 0	0.84 (1)
C- X	0 / 1022	0.23 (1)
X- D	-3273 / 0	0.89 (1)
D- W	0 / 3220	0.72 (1)
W- E	-561 / 0	0.26 (1)
E- V	0 / 171	0.04 (1)
V- G	-587 / 0	0.27 (1)
V- H	0 / 3242	0.73 (1)
U- H	-3056 / 0	0.83 (1)
U- J	0 / 582	0.13 (1)
T- J	-2308 / 0	0.63 (1)
J- S	0 / 1886	0.42 (1)
S- K	-641 / 0	0.30 (1)
S- L	0 / 2249	0.51 (1)
L- R	-93 / 22	0.04 (1)
R- M	0 / 921	0.21 (1)
Q- M	-1412 / 0	0.38 (1)
A- Z	0 / 1050	0.24 (1)
Q- N	0 / 1047	0.24 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 585 (0.80")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 531 (0.03")

CSI: TC=0.78/1.00 (K-L:1) , BC=0.73/1.00 (V-W:1) , WB=0.89/1.00 (D-X:1) , SSI=0.28/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.68 (U) (INPUT = 1.00)

A-18023905

CONTINUED ON PAGE 2

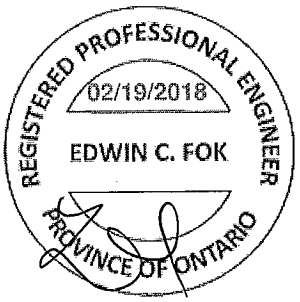
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292636	H2TB	1	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BBWWW-m	MT20	10.0	12.0	3.25	6.00
T	BBW-h	MT20	5.0	8.0	1.50	4.50
U	BBWW-h	MT20	6.0	7.0	1.50	3.50
V	BBWWW+m	MT20	7.0	12.0	3.25	2.75
W	BBWW-m	MT20	10.0	12.0	3.25	6.00
X	BBWW-h	MT20	7.0	8.0	2.00	5.00
Y	BMWW+t	MT20	4.0	6.0	3.00	1.50
Z	BMWW-t	MT20	4.0	4.0	2.00	1.50
AA	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-1802390S(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H1A	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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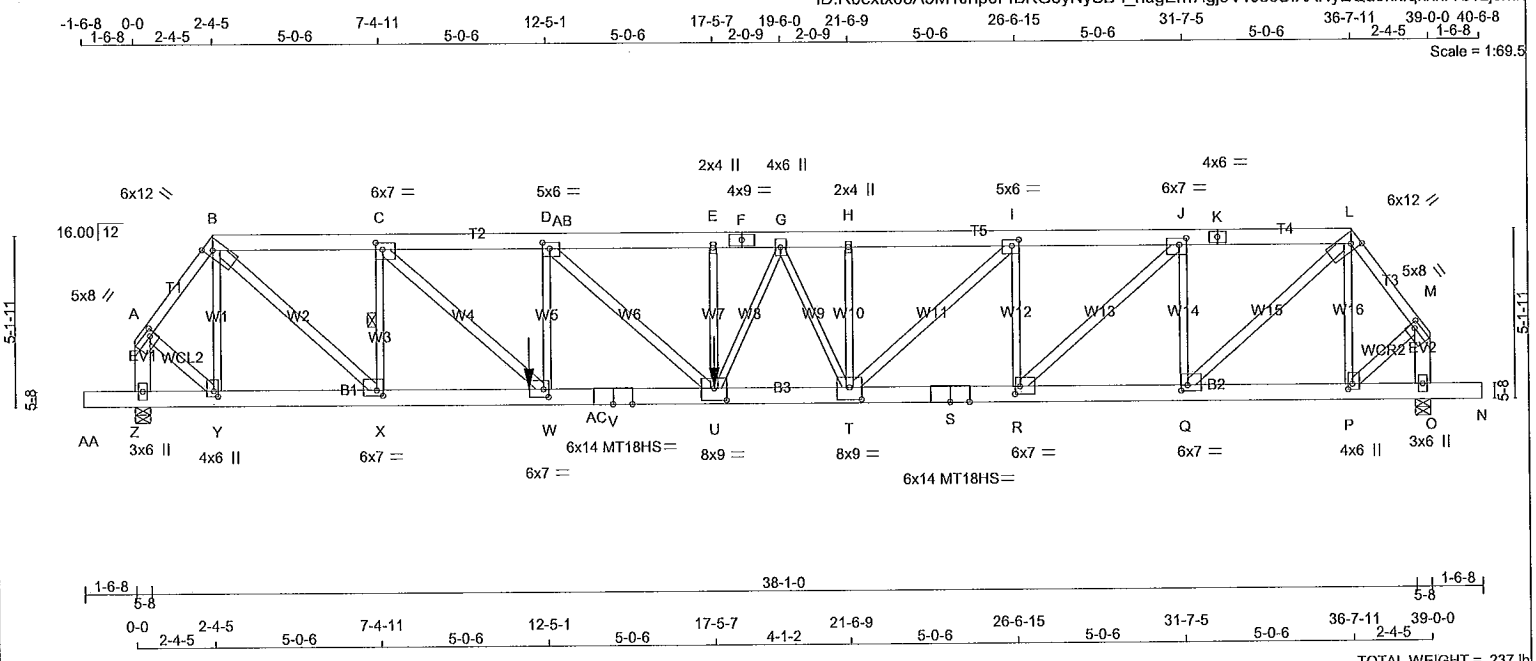

TOTAL WEIGHT = 237 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - F	2x6	DRY	1650F 1.5E	SPF
F - K	2x6	DRY	1650F 1.5E	SPF
K - L	2x6	DRY	1650F 1.5E	SPF
L - M	2x4	DRY	No.2	SPF
Z - A	2x6	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
AA- V	2x6	DRY	2100F 1.8E	SPF
V - S	2x6	DRY	2100F 1.8E	SPF
S - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - X	2x4	DRY	No.2	SPF
C - W	2x4	DRY	No.2	SPF
D - U	2x4	DRY	No.2	SPF
T - I	2x4	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF
Q - L	2x4	DRY	No.2	SPF
A - Y	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	2.00	2.00
B	TTWW+h	MT20	6.0	12.0	3.25	2.25
C	TMWW-t	MT20	6.0	7.0	2.50	2.50
D	TMWW-t	MT20	5.0	6.0	2.25	2.50
E	TMW+w	MT20	2.0	4.0		
F	TS-t	MT20	4.0	9.0		
G	TMWW+t	MT20	4.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	5.0	6.0	2.25	2.50
J	TMWW-t	MT20	6.0	7.0	2.50	2.50
K	TS-t	MT20	4.0	6.0		
L	TTWW+h	MT20	6.0	12.0	3.25	2.25
M	TMVW-t	MT20	5.0	8.0	2.00	2.00
O	BMV1+p	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0	2.00	1.50

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Z	4134	0	4134	0	0	5-8	5-3
O	3444	0	3444	0	0	5-8	3-7

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Z	2915	1959 / 0	0 / 0	0 / 0	0 / 0	956 / 0	0 / 0
O	2433	1609 / 0	0 / 0	0 / 0	0 / 0	825 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Z, O

BEARING SIZE FACTOR = 1.15 AT JNT(S) Z, O (BASED ON SUPPORT DEPTH = 1-8)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.86 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-X.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-3341 / 0	-78.0	-78.0	0.25 (1)	3.63
B- C	-5763 / 0	-78.0	-78.0	0.21 (1)	4.05
C-AB	-8950 / 0	-78.0	-78.0	0.35 (1)	3.25
AB- D	-8950 / 0	-154.5	-154.5	0.35 (1)	3.25
D- E	-10342 / 0	-154.5	-154.5	0.54 (1)	2.86
E- F	-10342 / 0	-78.0	-78.0	0.32 (1)	3.05
F- G	-10342 / 0	-78.0	-78.0	0.32 (1)	3.05
G- H	-9146 / 0	-78.0	-78.0	0.26 (1)	3.28
H- I	-9146 / 0	-78.0	-78.0	0.36 (1)	3.21
I- J	-7163 / 0	-78.0	-78.0	0.24 (1)	3.67
J- K	-4670 / 0	-78.0	-78.0	0.16 (1)	4.46
K- L	-4670 / 0	-78.0	-78.0	0.16 (1)	4.46
L- M	-2752 / 0	-78.0	-78.0	0.20 (1)	4.00
Z- A	-4004 / 0	0.0	0.0	0.32 (1)	5.31
O- M	-3314 / 0	0.0	0.0	0.27 (1)	5.78
AA- Z	0 / 0	-96.5	-96.5	0.05 (1)	10.00
Z- Y	0 / 0	-18.5	-18.5	0.04 (1)	10.00
Y- X	0 / 1977	-18.5	-18.5	0.14 (1)	10.00
X-AC	0 / 5763	-18.5	-18.5	0.42 (1)	10.00
AC- W	0 / 5763	-37.3	-37.3	0.42 (1)	10.00
W- V	0 / 8950	-37.3	-37.3	0.56 (1)	10.00
V- U	0 / 8950	-37.3	-37.3	0.56 (1)	10.00
U- T	0 / 9765	-18.5	-18.5	0.60 (1)	10.00
T- S	0 / 7163	-18.5	-18.5	0.42 (1)	10.00
S- R	0 / 7163	-18.5	-18.5	0.42 (1)	10.00
R- Q	0 / 4670	-18.5	-18.5	0.28 (1)	10.00
Q- P	0 / 1629	-18.5	-18.5	0.13 (1)	10.00
P- O	0 / 0	-18.5	-18.5	0.04 (1)	10.00
O- N	0 / 0	-96.5	-96.5	0.05 (1)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- B	-1118 / 0	0.39 (1)
B- X	0 / 5169	0.91 (1)
X- C	-3362 / 0	0.68 (1)
C- W	0 / 4354	0.77 (1)
W- D	-1961 / 0	0.69 (1)
D- U	0 / 1901	0.34 (1)
U- E	-471 / 0	0.17 (1)
U- G	0 / 1441	0.36 (1)
G- T	-1544 / 0	0.65 (1)
T- H	-302 / 0	0.11 (1)
T- I	0 / 2709	0.48 (1)
R- I	-2233 / 0	0.79 (1)
R- J	0 / 3406	0.60 (1)
Q- J	-2750 / 0	0.97 (1)
Q- L	0 / 4151	0.73 (1)
P- L	-886 / 0	0.31 (1)
A- Y	0 / 2353	0.42 (1)
P- M	0 / 1938	0.34 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.42")
ALLOWABLE DEFL.(TL)= L/360 (1.30")
CALCULATED VERT. DEFL.(TL) = L/ 619 (0.76")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.60/1.00 (T-U:1) , WB=0.97/1.00 (J-Q:1) , SSI=0.56/1.00 (W-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MT18HS	511	354	2455	1382	3004	2010

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CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H1A	1	1	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BMWW-t	MT20	6.0	7.0	2.00	2.25
R	BMWW-t	MT20	6.0	7.0	2.75	1.75
S	BS-t	MT18HS	6.0	14.0		
T	BMWWW-t	MT20	8.0	9.0	4.25	4.50
U	BMWWW-t	MT20	8.0	9.0	4.25	4.50
V	BS-t	MT18HS	6.0	14.0		
W	BMWW-t	MT20	6.0	7.0	2.75	1.75
X	BMWW-t	MT20	6.0	7.0	2.00	2.25
Y	BMWW+t	MT20	4.0	6.0	2.00	1.50
Z	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2131.9 lbs FACTORED DOWN AT 17-5-8, AND 855.1 lbs FACTORED DOWN AT 11-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

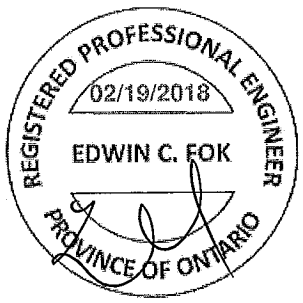
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
U	17-5-8	-2132	-2132	---	FRONT	VERT	TOTAL
AC	11-10-8	-855	-855	---	FRONT	VERT	TOTAL

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.88 (V) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H11A-Cond1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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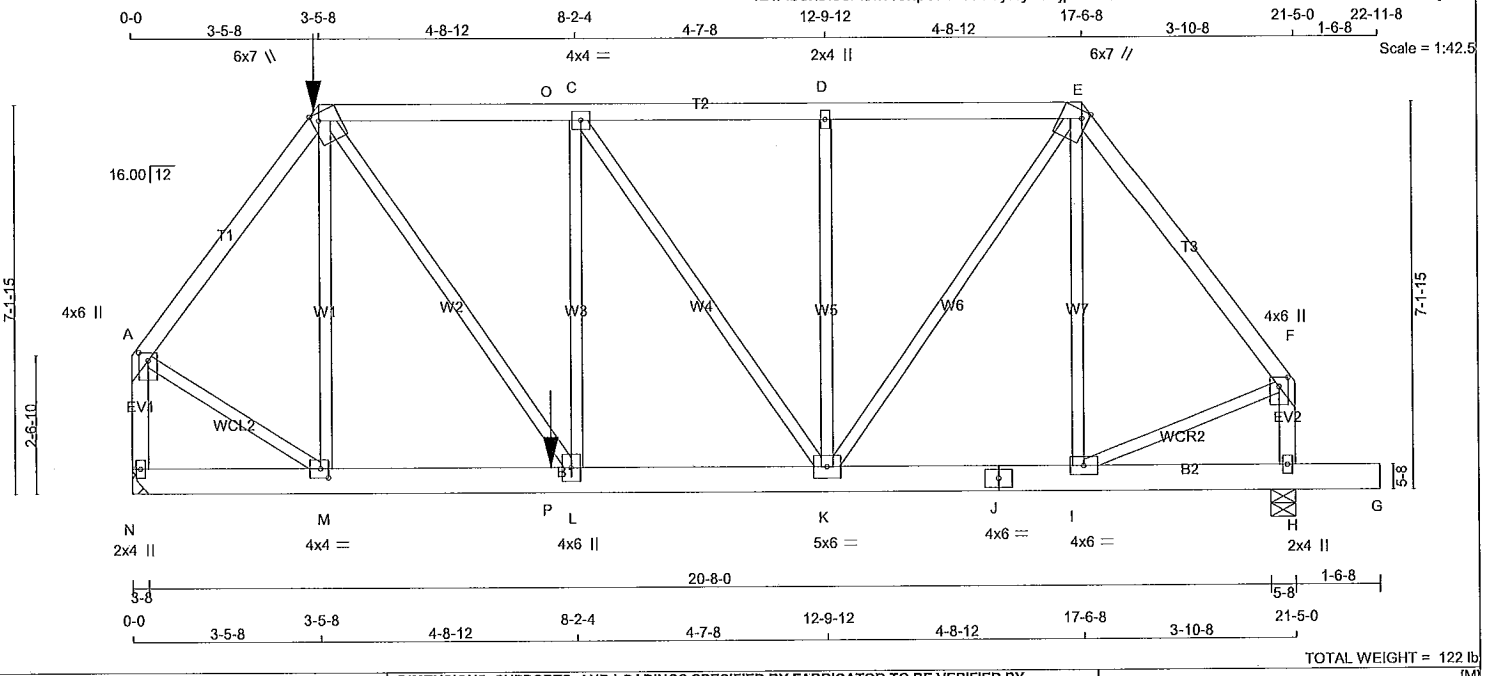


TOTAL WEIGHT = 122 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
N - A	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

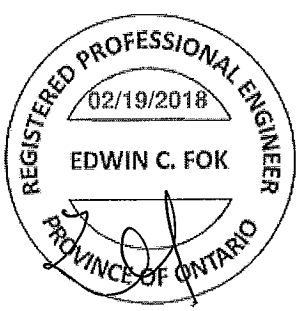
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	1.75	2.00
B	TTWW+m	MT20	6.0	7.0	Edge	1.50
C	TMWW-t	MT20	4.0	4.0		
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	6.0	7.0	Edge	1.50
F	TMVW+p	MT20	4.0	6.0	2.00	2.00
H	BMV1+p	MT20	2.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BS-t	MT20	4.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
L	BMWW+t	MT20	4.0	6.0		
M	BMWW-t	MT20	4.0	4.0	2.00	1.75
N	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.7 lbs FACTORED DOWN AT 3-5-8 ON TOP CHORD, AND 652.1 lbs FACTORED DOWN AT 7-8-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	1710	0	1710	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 2-1	
H	1486	0	1486	0	0	5-8	2-3

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1212	779 / 0	0 / 0	0 / 0	0 / 0	432 / 0	0 / 0
H	1055	669 / 0	0 / 0	0 / 0	0 / 0	386 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-1355 / 0	-78.0	-78.0	0.23 (1)	5.32	M- B	-260 / 0	0.22 (1)
B- O	-1412 / 0	-114.5	-114.5	0.38 (1)	4.98	B- L	0 / 1059	0.26 (1)
O- C	-1412 / 0	-78.0	-78.0	0.38 (1)	4.98	L- C	-150 / 0	0.13 (1)
C- D	-1188 / 0	-78.0	-78.0	0.36 (1)	5.36	C- K	-398 / 0	0.56 (1)
D- E	-1188 / 0	-78.0	-78.0	0.26 (1)	5.52	K- D	-370 / 0	0.32 (1)
E- F	-1128 / 0	-78.0	-78.0	0.27 (1)	5.65	K- E	0 / 901	0.22 (1)
N- A	-1693 / 0	0.0	0.0	0.24 (1)	6.34	I- E	-148 / 11	0.13 (1)
H- F	-1314 / 0	0.0	0.0	0.16 (1)	7.02	A- M	0 / 935	0.23 (1)
						I- F	0 / 720	0.18 (1)
N- M	0 / 0	-27.2	-27.2	0.08 (1)	10.00			
M- P	0 / 803	-27.2	-27.2	0.24 (1)	10.00			
P- L	0 / 803	-18.5	-18.5	0.24 (1)	10.00			
L- K	0 / 1412	-18.5	-18.5	0.27 (1)	10.00			
K- J	0 / 670	-18.5	-18.5	0.12 (1)	10.00			
J- I	0 / 670	-18.5	-18.5	0.12 (1)	10.00			
I- H	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
H- G	0 / 0	-96.5	-96.5	0.08 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-5-8	-107	-107	---	FRONT	VERT	TOTAL
P	7-8-8	-652	-652	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-5-8
RIGHT SETBACK = 3-10-8
END SETBACK = 3-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 7-8-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.27/1.00 (K-L:1) , WB=0.56/1.00 (C-K:1) , SSI=0.43/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

A-18023907

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H12	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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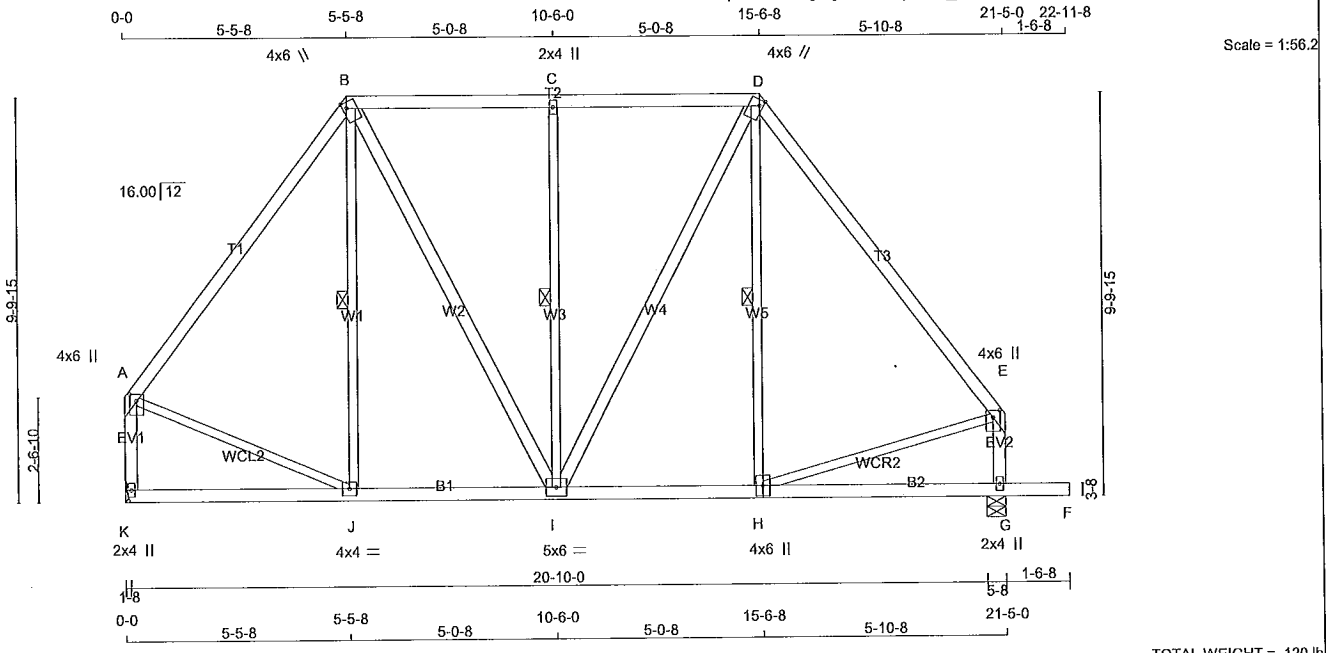


TOTAL WEIGHT = 120 lb

[M][F]

LUMBER

N. L. G. A. RULES

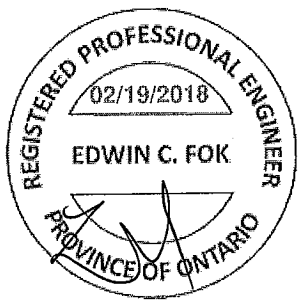
CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	1650F 1.5E	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - I	2x4	DRY	No.2	SPF
I - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	4.0	6.0	1.75	1.00
C	TMW+w	MT20	2.0	4.0		
D	TTWW+m	MT20	4.0	6.0	1.75	1.00
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BSWW+l	MT20	4.0	6.0		
I	BMWWW-t	MT20	5.0	6.0		
J	BMWW-t	MT20	4.0	4.0		
K	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	1033	0	1033	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	1182	0	1182	0	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	737	450 / 0	0 / 0	0 / 0	0 / 0	287 / 0	0 / 0
G	843	514 / 0	0 / 0	0 / 0	0 / 0	328 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF B-J, C-I, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-753 / 0	-78.0	-78.0	0.32 (1)	6.25
B- C	-608 / 0	-78.0	-78.0	0.25 (1)	6.25
C- D	-608 / 0	-78.0	-78.0	0.25 (1)	6.25
D- E	-778 / 0	-78.0	-78.0	0.31 (1)	6.25
K- A	-991 / 0	0.0	0.0	0.14 (1)	7.81
G- E	-978 / 0	0.0	0.0	0.11 (1)	7.81
K- J	0 / 0	-18.5	-18.5	0.12 (4)	10.00
J- I	0 / 449	-18.5	-18.5	0.17 (4)	10.00
I- H	0 / 465	-18.5	-18.5	0.14 (4)	10.00
H- G	0 / 0	-18.5	-18.5	0.19 (4)	10.00
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- B	-65 / 53	0.04 (1)
B- I	0 / 340	0.05 (1)
I- C	-476 / 0	0.31 (1)
I- D	0 / 307	0.05 (1)
H- D	-32 / 55	0.02 (1)
A- J	0 / 482	0.11 (1)
H- E	0 / 481	0.11 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.32/1.00 (A-B:1) , BC=0.19/1.00 (G-H:4) , WB=0.31/1.00 (C-I:1) , SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (H) (INPUT = 0.90)
JSI METAL= 0.30 (E) (INPUT = 1.00)

A-18023908

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H21S	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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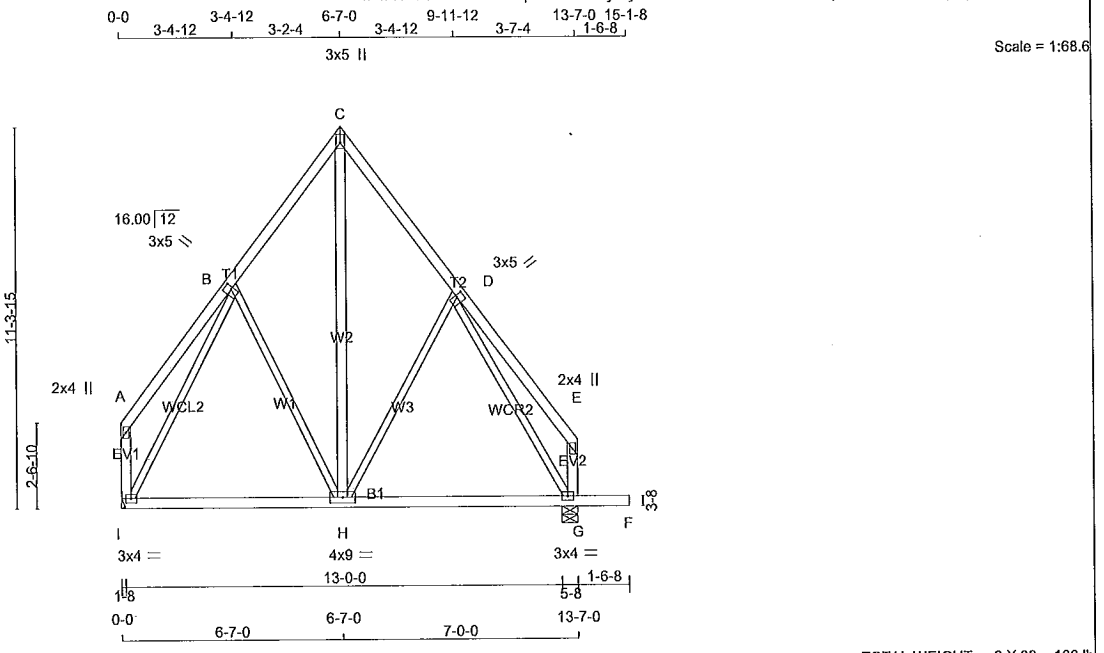


TOTAL WEIGHT = 2 X 83 = 166 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
H - C	2x4	DRY	No.2	SPF

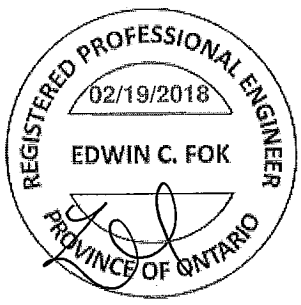
DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMWW+t	MT20	3.0	5.0	2.25	0.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMWW+t	MT20	3.0	5.0	2.25	0.75
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	655	0	655	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
G	804	0	804	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	467	285 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0
G	573	350 / 0	0 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 24	-78.0	-78.0	0.14 (1)	10.00	B- H	-96 / 0	0.10 (1)
B- C	-377 / 0	-78.0	-78.0	0.10 (1)	6.25	H- C	0 / 357	0.06 (1)
C- D	-379 / 0	-78.0	-78.0	0.12 (1)	6.25	H- D	-135 / 0	0.14 (1)
D- E	0 / 26	-78.0	-78.0	0.16 (1)	10.00	I- B	-575 / 0	0.58 (1)
I- A	-95 / 0	0.0	0.0	0.01 (1)	7.81	D- G	-576 / 0	0.55 (1)
G- E	-101 / 0	0.0	0.0	0.01 (1)	7.81			
I- H	0 / 257	-18.5	-18.5	0.27 (4)	10.00			
H- G	0 / 280	-18.5	-18.5	0.27 (4)	10.00			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.16/1.00 (D-E:1) , BC=0.27/1.00 (G-H:4) , WB=0.58/1.00 (B-I:1) , SSI=0.12/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (B) (INPUT = 0.90)
JSI METAL= 0.37 (D) (INPUT = 1.00)

A-18023909

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H21T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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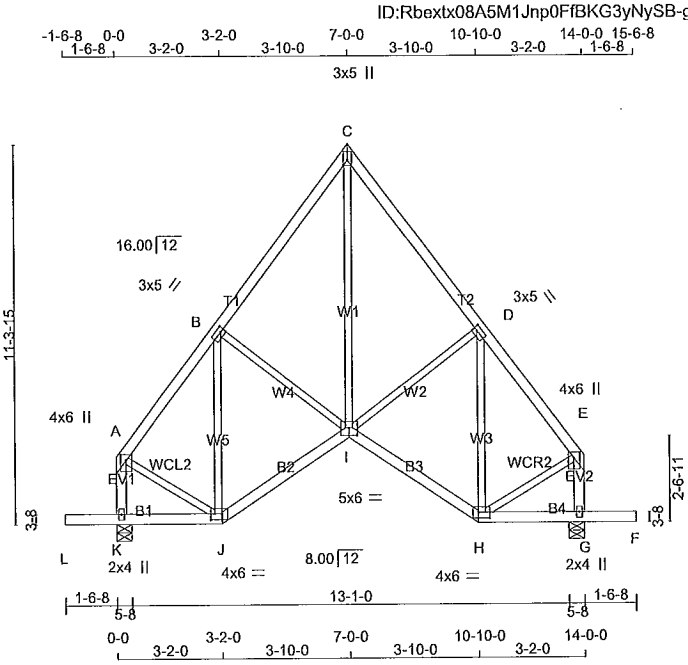

TOTAL WEIGHT = 2 X 81 = 162 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TMWW-t	MT20	3.0	5.0	1.50	1.50
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMWW-t	MT20	3.0	5.0	1.50	1.50
E	TMVW+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BBWW-I	MT20	4.0	6.0	2.00	4.00
I	BBWWW-p	MT20	5.0	6.0	2.75	3.00
J	BBWW-I	MT20	4.0	6.0	2.00	4.00
K	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	824	0	824	0	0	5-8	1-8
G	824	0	824	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	587	359 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0
G	587	359 / 0	0 / 0	0 / 0	0 / 0	229 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-482 / 0	-78.0	-78.0	0.13 (1)	6.25	I- C	0 / 508	0.11 (1)
B- C	-496 / 0	-78.0	-78.0	0.14 (1)	6.25	I- D	-25 / 0	0.01 (1)
C- D	-496 / 0	-78.0	-78.0	0.14 (1)	6.25	H- D	-293 / 0	0.17 (1)
D- E	-482 / 0	-78.0	-78.0	0.13 (1)	6.25	B- I	-25 / 0	0.01 (1)
K- A	-646 / 0	0.0	0.0	0.07 (1)	7.81	J- B	-293 / 0	0.17 (1)
G- E	-646 / 0	0.0	0.0	0.07 (1)	7.81	A- J	0 / 345	0.08 (1)
						H- E	0 / 345	0.08 (1)
L- K	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
K- J	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
J- I	0 / 364	-18.5	-18.5	0.11 (4)	10.00			
I- H	0 / 364	-18.5	-18.5	0.11 (4)	10.00			
H- G	0 / 0	-18.5	-18.5	0.05 (4)	10.00			
G- F	0 / 0	-96.5	-96.5	0.16 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.47")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.47")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (B-C:1) , BC=0.16/1.00 (F-G:1) , WB=0.17/1.00 (D-H:1) , SSI=0.12/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (C) (INPUT = 0.90)
JSI METAL= 0.18 (E) (INPUT = 1.00)

A-18023910

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H22G	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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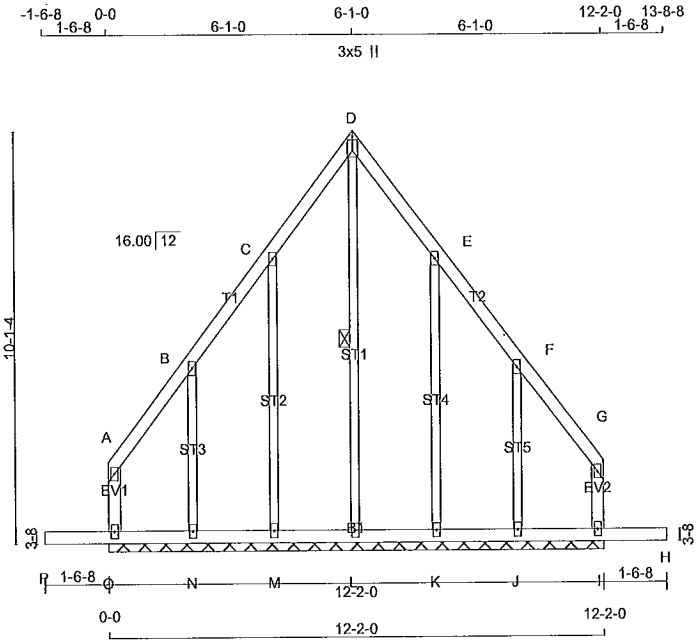


TOTAL WEIGHT = 70 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

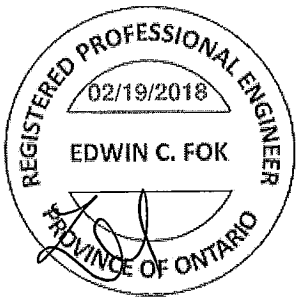
GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW+p	MT20	3.0	5.0	2.00	Edge
G	TMV+p	MT20	2.0	4.0		
I	BMV1+p	MT20	2.0	4.0		
J, K, L, M, N						
J	BMW1+w	MT20	2.0	4.0		
O	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
O- A	-66 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 14	-78.0	-78.0	0.04 (1)	10.00
B- C	0 / 22	-78.0	-78.0	0.04 (1)	10.00
C- D	0 / 23	-78.0	-78.0	0.04 (1)	10.00
D- E	0 / 23	-78.0	-78.0	0.04 (1)	10.00
E- F	0 / 22	-78.0	-78.0	0.04 (1)	10.00
F- G	0 / 14	-78.0	-78.0	0.04 (1)	10.00
I- G	-66 / 0	0.0	0.0	0.05 (1)	7.81
P- O	0 / 0	-96.5	-96.5	0.16 (1)	10.00
O- N	-8 / 0	-18.5	-18.5	0.11 (1)	10.00
N- M	-12 / 0	-18.5	-18.5	0.02 (1)	6.25
M- L	-15 / 0	-18.5	-18.5	0.02 (4)	6.25
L- K	-15 / 0	-18.5	-18.5	0.02 (4)	6.25
K- J	-12 / 0	-18.5	-18.5	0.02 (1)	6.25
J- I	-8 / 0	-18.5	-18.5	0.11 (1)	10.00
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L- D	-200 / 0	0.13 (1)
M- C	-161 / 0	0.16 (1)
N- B	-147 / 0	0.05 (1)
K- E	-161 / 0	0.16 (1)
J- F	-147 / 0	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (G-I:1) , BC=0.16/1.00 (H-I:1) , WB=0.16/1.00 (E-K:1) , SSI=0.12/1.00 (O-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.20 (D) (INPUT = 0.90)
JSI METAL= 0.08 (E) (INPUT = 1.00)

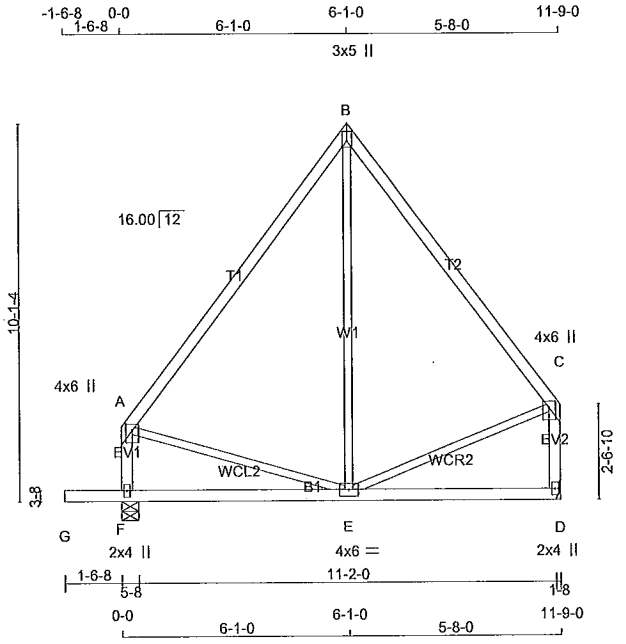
A-18023911

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292639	H22S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Mon Feb 19 10:53:22 2018 Page 1

ID:Rbextx08A5M1Jnp0FfBKG3yNySB-nz7G06EfhmWgYxPtQowlVLkJ2Hc?Sq76NSYI2zjcQB



TOTAL WEIGHT = 3 X 60 = 179 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	2100F 1.8E	SPF
B - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
G - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

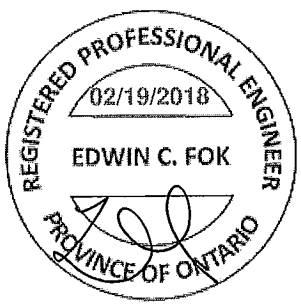
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTW+p	MT20	3.0	5.0	2.00	Edge
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	BMV1+p	MT20	2.0	4.0		
E	BMWWW-t	MT20	4.0	6.0		
F	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
D	567	0	567	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 1-8	
F	715	0	715	0	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	404	247 / 0	0 / 0	0 / 0	0 / 0	157 / 0	0 / 0
F	510	311 / 0	0 / 0	0 / 0	0 / 0	199 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	-298 / 0	-78.0	-78.0	0.25	(1)	6.25	E- B	0 / 103	0.04 (4)
B- C	-298 / 0	-78.0	-78.0	0.32	(1)	6.25	A- E	0 / 185	0.04 (1)
F- A	-523 / 0	0.0	0.0	0.06	(1)	7.81	E- C	0 / 192	0.04 (1)
D- C	-528 / 0	0.0	0.0	0.07	(1)	7.81			
G- F	0 / 0	-96.5	-96.5	0.16	(1)	10.00			
F- E	0 / 0	-18.5	-18.5	0.18	(4)	10.00			
E- D	0 / 0	-18.5	-18.5	0.18	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.32/1.00 (B-C:1) , BC=0.18/1.00 (E-F:4) , WB=0.04/1.00 (C-E:1) , SSI=0.12/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

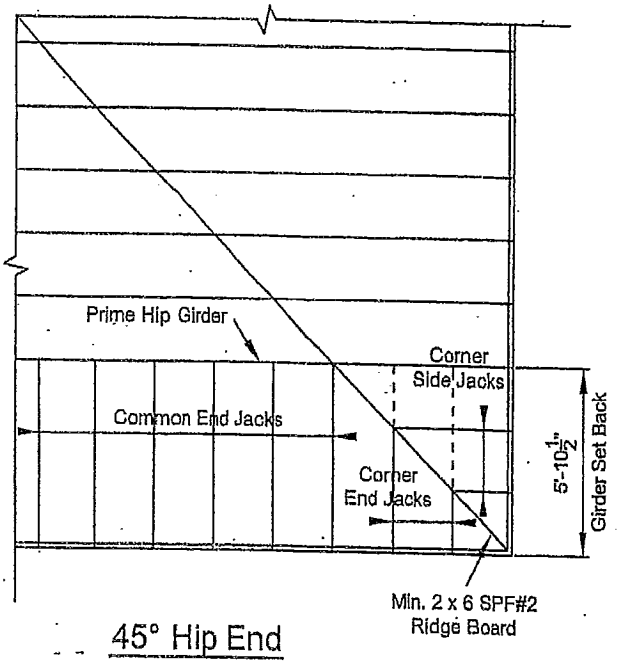
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (E) (INPUT = 0.90)
JSI METAL= 0.15 (A) (INPUT = 1.00)

A-18023912

STRACON ENGINEERING INC.



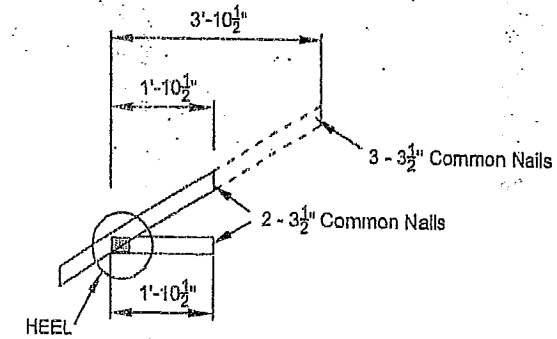
45° Hip End

LUMBER SPECIFICATION

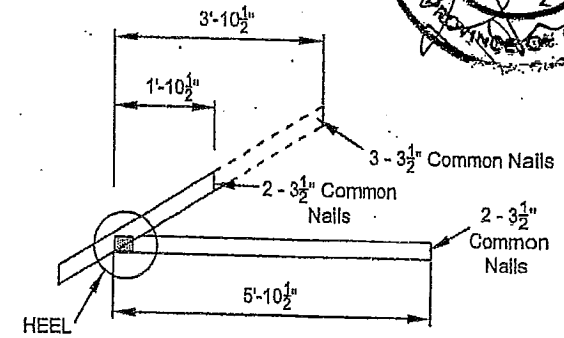
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

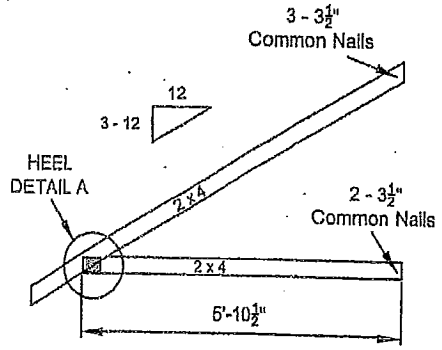
TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



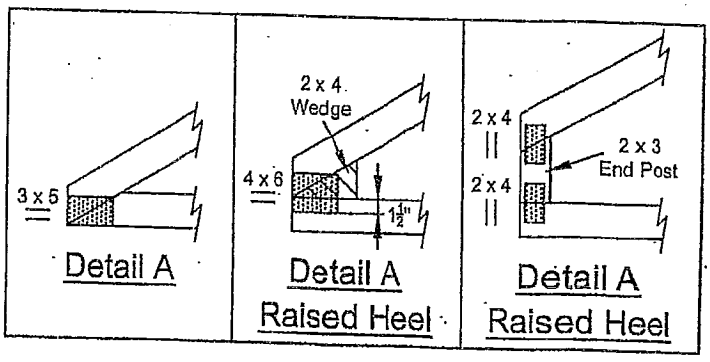
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

TECHNICAL BULLETIN

HHUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

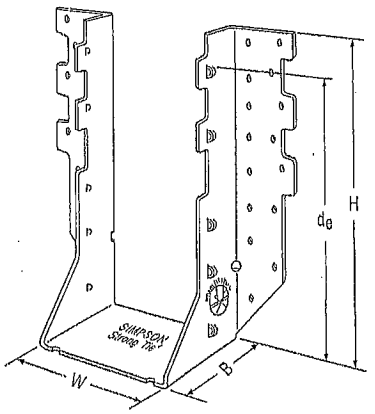
All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

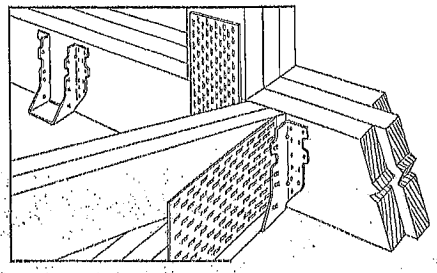
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



HHUS410

Installation:

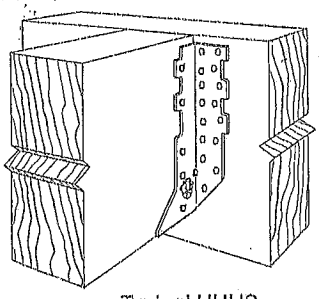
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HHUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Options:

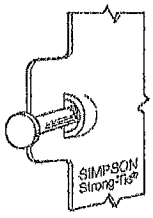
- See current catalogue for options



Typical HHUS Installation

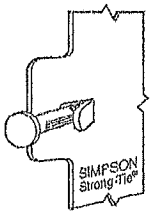
Model No.	Ga.	Dimensions (In.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HHUS26-2	14	3 5/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 5/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 5/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 5/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

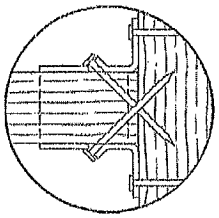


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

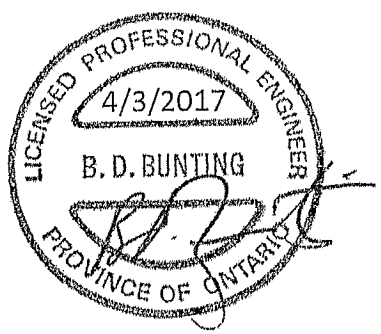
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

T-SPECHHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

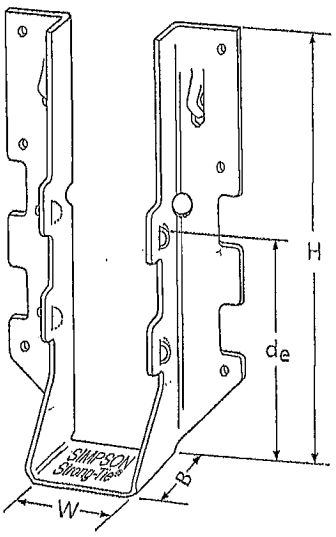
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

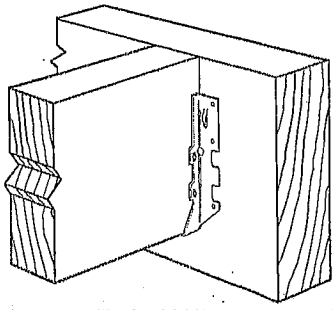
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

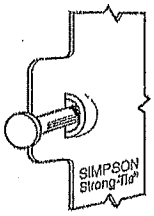


LUS28


Typical LUS Installation

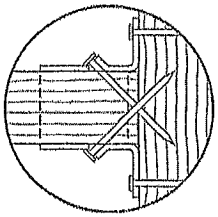
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 9/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

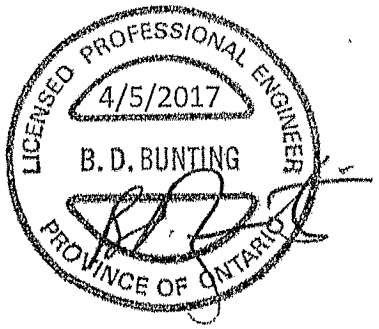
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



LIMIT STATES DESIGN

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T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

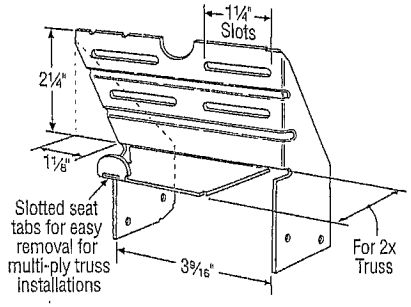
TC – Truss Connectors

The TC truss connector is an ideal connector for scissor trusses and can allow horizontal movement up to 1¼". The TC also attaches plated trusses to top plates or sill plates to resist uplift forces. Typically used on one or both ends of truss as determined by the building designer.

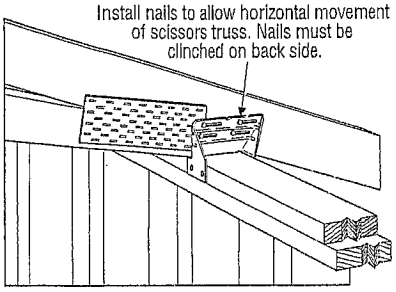
Material: 16 gauge

Finish: G90 galvanized

Design: Factored resistances are in accordance with CSA 086-14



TC24
U.S. Patent 4,932,173



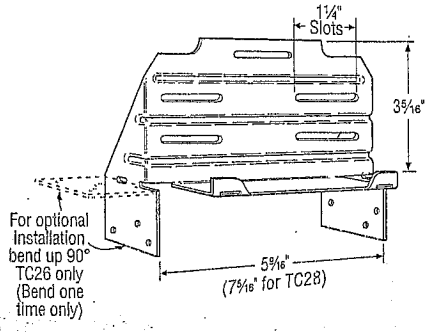
Typical TC24 Installation

Installation:

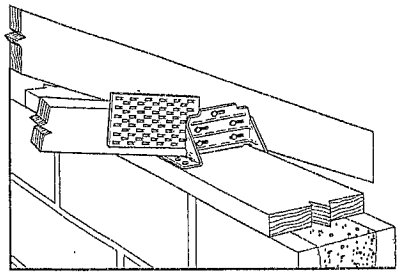
- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½ = 0.148" dia. x 1½" long.
- Drive 10d nails into the truss at the inside end of the slotted holes (inside end is towards the centre of the truss) and clinch on the back side. Do not seat these nails into the truss–allow room under the nail head for movement of the truss with respect to the wall.

Optional TC Installation:

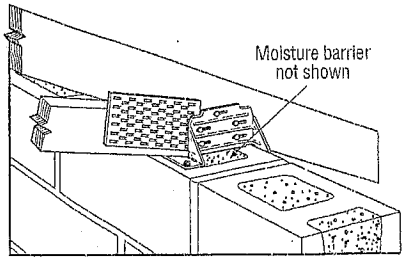
- Bend one flange up 90°. Drive specified nails into the top and face of the top plates or install Titen® screws into the top and face of masonry wall. See optional load tables and installation details.



TC26
(TC28 Similar)



Optional TC26 Installation for Grouted Concrete Block using a Wood Nailer
(8", 10", 12" Wall Installation Similar)



Optional TC26 Installation for Grouted Concrete Block using Titen Screws

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC24	(4) 10d	(4) 10d	605	430
TC26	(5) 10d	(6) 10d	1015	720
TC28	(5) 10d	(6) 10d	1015	720

Optional TC Installation Table

Model No.	Fasteners		Factored Resistance	
	Truss	Wall Plates	D.Fir-L	S-P-F
			Uplift (K_D=1.15)	Uplift (K_D=1.15)
			lb.	lb.
TC26	(5) 10d	(6) 10d x 1½"	810	660
	(5) 10d	(6) 10d	930	660

1. Factored resistances have been increased 15% for earthquake or wind loading; no further increase allowed; reduce where other loads govern.
2. Grout strength is 15 MPa minimum.
3. Optional TC26 installation with 10d nails requires minimum 3" top plate thickness.
4. TC26 fastened to grouted concrete block with (6) – ⅜" x 2¼" Titen screws has a factored uplift resistance of 275 lb.

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HUS/LJS – Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

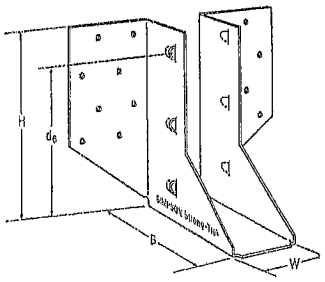
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

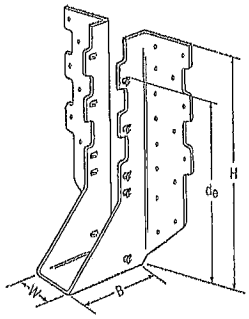
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

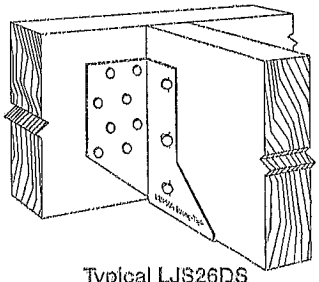
- See current catalogue for options



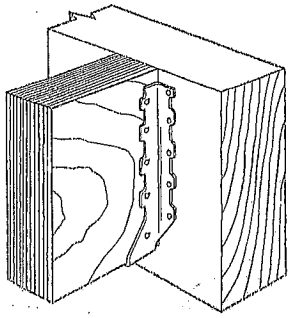
LJS26DS



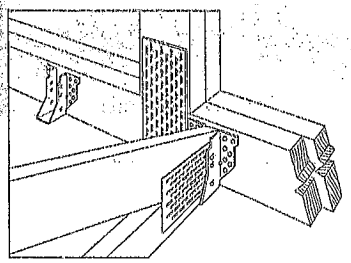
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



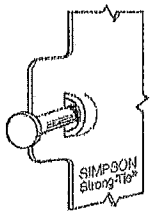
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

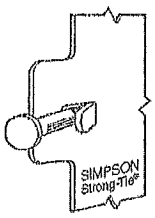
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
								lb.	lb.	lb.	lb.
LJS26DS	18	1 9/16	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

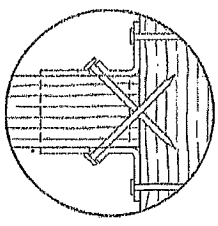


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

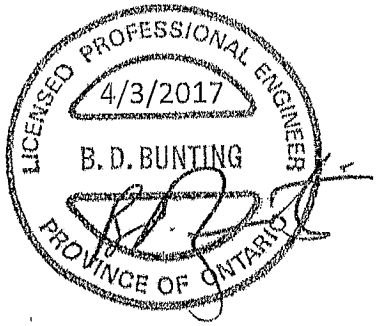
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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