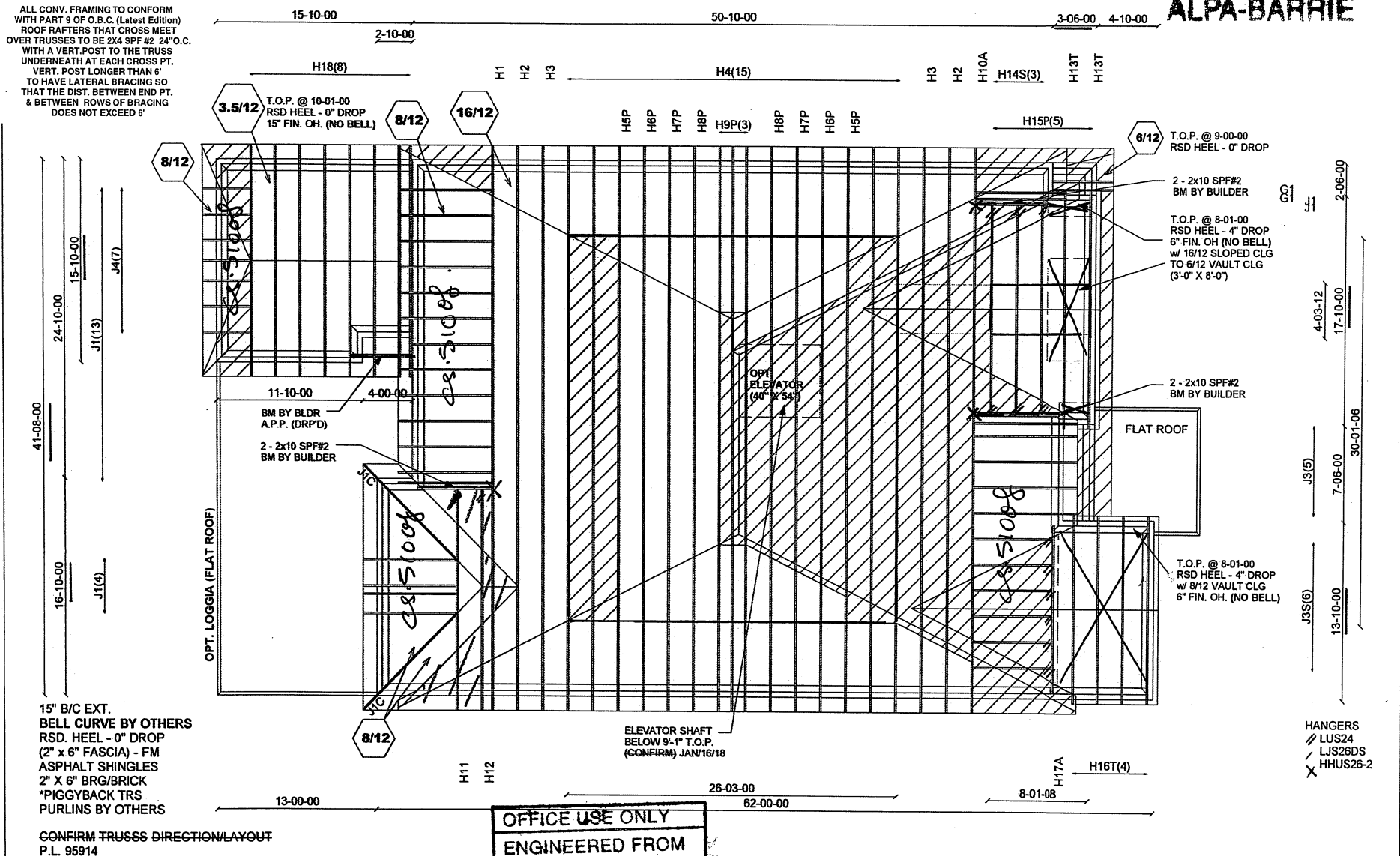


ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. (Latest Edition)
 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



P.L. 99352



Job Track: **45147**
 Layout ID: **292655**
 Plan Log: **95914**

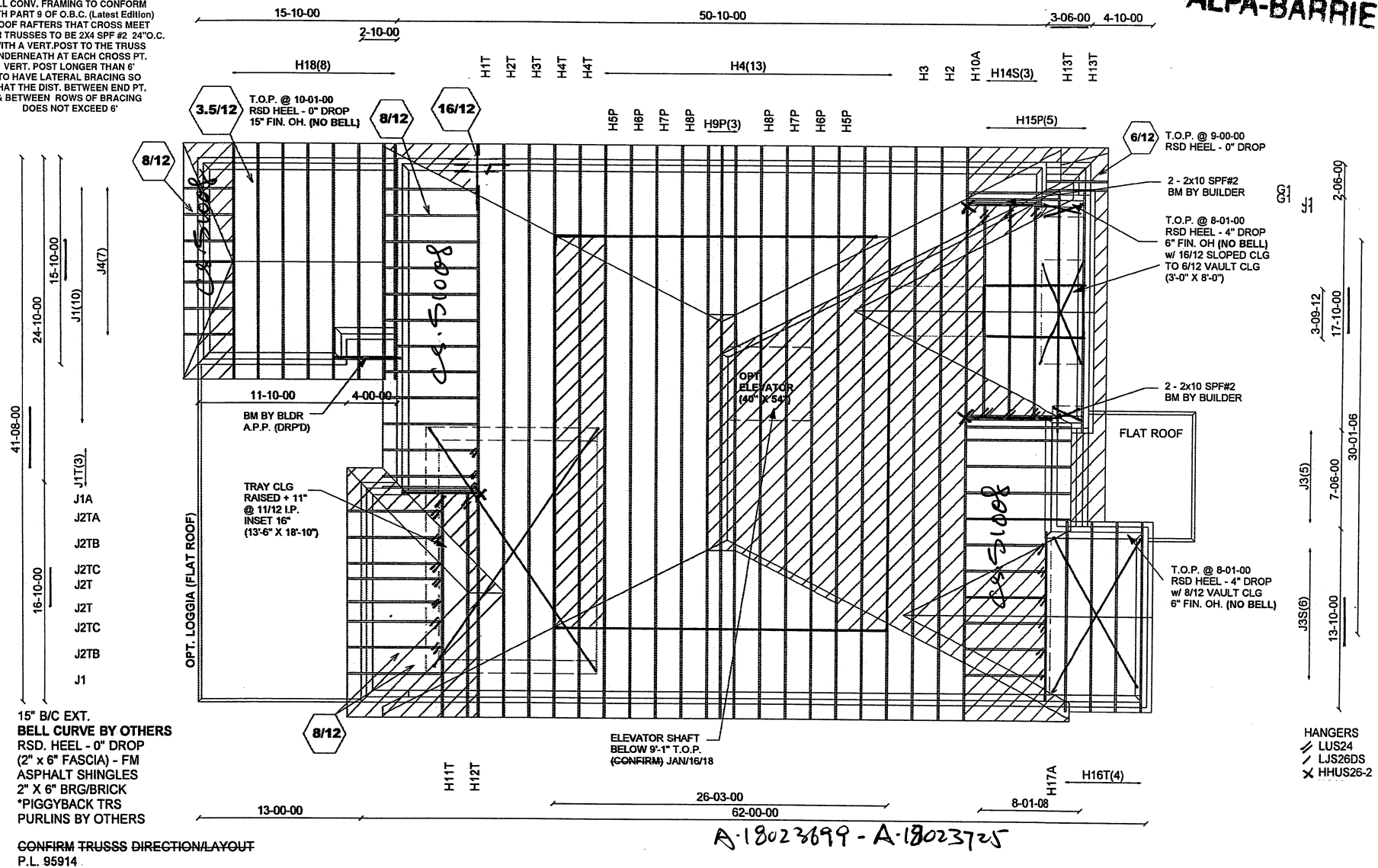
Builder / Location:
GOLD PARK HOMES / VAUGHAN
 Project: **PINE VALLEY**
 Date: 1/19/2018 Designer: AMANDA

Model / Elevation:
5005 REV1
 STD OR OPT 2ND FLR (5BED) AND/OR OPT ELEVATOR OR OPT LOGGIA (FLAT)

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

ALL CONV. FRAMING TO CONFORM WITH PART 8 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

ALPA-BARRIE



P.L. 99352



Job Track: 45147
Layout ID: 292656
Plan Log: 95914

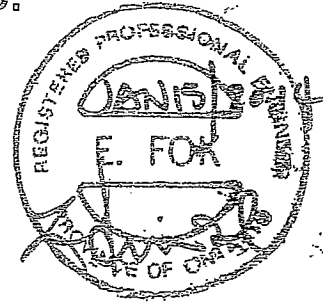
Builder / Location:
GOLD PARK HOMES / VAUGHAN
Project: **PINE VALLEY**
Date: 1/19/2018 Designer: AMANDA

Model / Elevation: 5005 REV1
OR OPT 2ND FLR (SEED) AND/OR / A OPT TRAY OR OPT ELEVATOR OR OPT LOGGIA

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Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

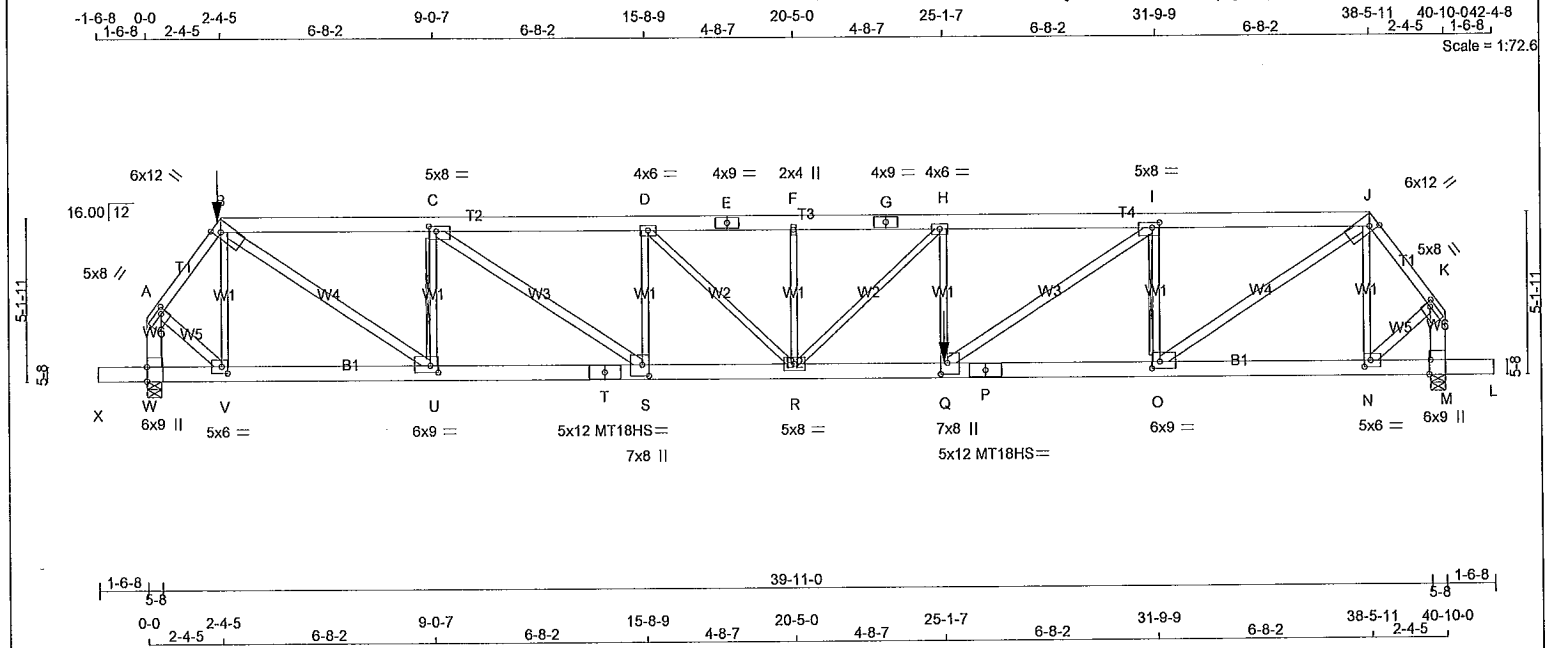
1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 14:12:28 2018 Page 1
ID:HSVaWIDnhr?B6oeH9vbyNds-nfJaliChin5qDyZQp8A0AfH_qB76l?y8Rosm5VzjubX



TOTAL WEIGHT = 234 lb

(M)

LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.		
A - B	2x4	DRY	No.2	SPF	
B - E	2x6	DRY	1650F 1.5E	SPF	
E - G	2x6	DRY	1650F 1.5E	SPF	
G - J	2x6	DRY	1650F 1.5E	SPF	
J - K	2x4	DRY	No.2	SPF	
W - A	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
X - T	2x6	DRY	2100F 1.8E	SPF	
T - P	2x6	DRY	2100F 1.8E	SPF	
P - L	2x6	DRY	2100F 1.8E	SPF	
ALL WEBS EXCEPT				SPF	
B - U	2x4	DRY	No.2	SPF	
C - S	2x4	DRY	No.2	SPF	
Q - I	2x4	DRY	No.2	SPF	
O - J	2x4	DRY	No.2	SPF	
A - V	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-I	MT20	5.0	8.0	1.75 2.00
B	TTVVW+h	MT20	6.0	12.0	3.25 2.00
C	TMVW-I	MT20	5.0	8.0	2.00 2.75
D	TMVW-I	MT20	4.0	6.0	
E	TS-I	MT20	4.0	9.0	
F	TMW+w	MT20	2.0	4.0	
G	TS-I	MT20	4.0	9.0	
H	TMVW-I	MT20	4.0	6.0	
I	TMVW-I	MT20	5.0	8.0	2.00 2.75
J	TTVVW+h	MT20	6.0	12.0	3.25 2.00
K	TMVW-I	MT20	5.0	8.0	1.75 2.00
M	BMV1+I	MT20	6.0	9.0	Edge 0.50
N	BMVW-I	MT20	5.0	6.0	2.50 2.25
O	BMVW-I	MT20	6.0	9.0	2.50 3.00
P	BS-I	MT18HS	5.0	12.0	
Q	BMVW+I	MT20	7.0	8.0	4.25 2.50
R	BMVW+I	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
FACTORED GROSS REACTION				MAXIMUM FACTORED GROSS REACTION		INPUT BRG	
JT	VERT	HORZ		DOWN	HORZ	IN-SX	IN-SX
W	4381	0		4381	0	5-8	5-8
M	3937	0		3937	0	5-8	4-11
UNFACTORED REACTIONS							
1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
W	3112	1964 / 0	0 / 0	0 / 0	0 / 0	1148 / 0	0 / 0
M	2787	1813 / 0	0 / 0	0 / 0	0 / 0	973 / 0	0 / 0
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M							
BEARING SIZE FACTOR = 1.15 AT JNT(S) W, M (BASED ON SUPPORT DEPTH = 1-8)							
BRACING							
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.							
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.							

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-U, I-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-3568 / 0	-78.0 -78.0	0.27 (1)	3.50	V-B	-1050 / 0	0.37 (1)
B-C	-6769 / 0	-153.5 -153.5	0.54 (1)	3.46	B-U	0 / 5683	1.00 (1)
C-D	-9437 / 0	-153.5 -153.5	0.71 (1)	2.84	U-C	-3039 / 0	0.64 (1)
D-E	-10229 / 0	-153.5 -153.5	0.50 (1)	2.92	C-S	0 / 3258	0.58 (1)
E-F	-10229 / 0	-153.5 -153.5	0.50 (1)	2.92	S-D	-1648 / 0	0.58 (1)
F-G	-10229 / 0	-153.5 -153.5	0.48 (1)	2.94	D-R	0 / 1119	0.28 (1)
G-H	-10229 / 0	-153.5 -153.5	0.48 (1)	2.94	R-F	-687 / 0	0.24 (1)
H-I	-10172 / 0	-78.0 -78.0	0.54 (1)	2.94	F-H	0 / 173	0.07 (4)
I-J	-6535 / 0	-78.0 -78.0	0.32 (1)	3.76	H-Q	-652 / 0	0.23 (1)
J-K	-3189 / 0	-78.0 -78.0	0.23 (1)	3.72	Q-I	0 / 4443	0.79 (1)
W-A	-4270 / 0	0.0 0.0	0.34 (1)	5.16	O-I	-3134 / 0	0.66 (1)
M-K	-3826 / 0	0.0 0.0	0.31 (1)	5.42	O-J	0 / 5673	1.00 (1)
					N-J	-1015 / 0	0.36 (1)
X-W	0 / 0	-96.5 -96.5	0.05 (1)	10.00	A-V	0 / 2513	0.44 (1)
W-V	0 / 0	-36.4 -36.4	0.09 (4)	10.00	N-K	0 / 2246	0.40 (1)
V-U	0 / 2114	-36.4 -36.4	0.19 (1)	10.00			
U-T	0 / 6769	-36.4 -36.4	0.43 (1)	10.00			
T-S	0 / 6769	-36.4 -36.4	0.43 (1)	10.00			
S-R	0 / 9437	-36.4 -36.4	0.55 (1)	10.00			
R-Q	0 / 10172	-36.4 -36.4	0.61 (1)	10.00			
Q-P	0 / 6535	-18.5 -18.5	0.41 (1)	10.00			
P-O	0 / 6535	-18.5 -18.5	0.41 (1)	10.00			
O-N	0 / 1888	-18.5 -18.5	0.16 (1)	10.00			
N-M	0 / 0	-18.5 -18.5	0.06 (1)	10.00			
M-L	0 / 0	-96.5 -96.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-4-5	-134	-134	---	FRONT	VERT	TOTAL
O	25-1-8	-1778	-1778	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:		
TOP CH.	LL	= 21.0 PSF
	DL	= 6.0 PSF
BOT CH.	LL	= 0.0 PSF
	DL	= 7.4 PSF
TOTAL LOAD = 34.4 PSF		

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 25-1-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.47")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL)= L/ 555 (0.88")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/ 999 (0.00")

CSI: TC=0.71/1.00 (C-D:1), BC=0.61/1.00 (Q-R:1), WB=1.00/1.00 (B-U:1), SS=0.41/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

A-18023696

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sun Feb 18 14:12:28 2018 Page 2
ID:HSVaWlDnhr?B6ceweH9vbyNdlS-nFJaliCHin5qDyZQp8A0Afh_qB76I?y8Rosm5WzjubX

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
S	BMWV+I	MT20	7.0	8.0	4.25	2.50
T	BS-I	MT18HS	5.0	12.0		
U	BMWV-I	MT20	6.0	9.0	2.50	3.00
V	BMWV-I	MT20	5.0	6.0	2.50	2.25
W	BMV1-I	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5
ON TOP CHORD, AND 1777.8 lbs FACTORED
DOWN AT 25-1-8 ON BOTTOM CHORD. DESIGN
FOR UNSPECIFIED CONNECTION(S) IS
DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT
RESPONSIBLE FOR QUALITY CONTROL IN
THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

NAME	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656
MT18HS	511	354	2455	1382	3004	2010

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)
JSI METAL= 0.94 (Q) (INPUT = 1.00)



A-1802368660

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 12:15:51 2018 Page 1

ID:HSVaWlIDnhr?B6oeweh9vbyNdIs-881QEBWJWYcHg9fQZ8B58OC6f5kOZGFTiQ07azjwls

1-6-8 0-0 4-4-5 8-1-0 12-5-5 20-5-0 28-4-12 36-5-11 40-10-042-4-8 1-6-8

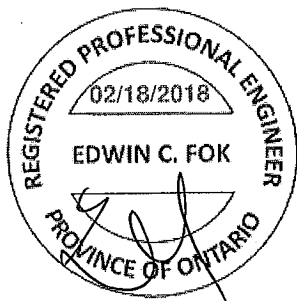
Scale = 1:72.8



ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

PLATES (table is in inches)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG IN-SX	REQ'D BRG IN-SX
JT	VERT	HORZ	DOWN	HORZ	UPLIFT		
S	2118	0	2118	0	0	5-8	3-3
K	2118	0	2118	0	0	5-8	3-3

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1510	922 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0
K	1510	922 / 0	0 / 0	0 / 0	0 / 0	588 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, G-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED				
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		UNBRAC		(LBS)	CSI (LC)	
FR-TO		FROM TO			LENGTH	FR-TO			
A-B	-1746 / 0	-78.0	-78.0	0.38 (1)	4.71	R-B	-245 / 0	0.28 (1)	
B-C	-2350 / 0	-78.0	-78.0	0.87 (1)	4.03	B-Q	0 / 1787	0.29 (1)	
C-D	-2732 / 0	-78.0	-78.0	0.93 (1)	3.76	Q-C	-1061 / 0	0.41 (1)	
D-E	-2732 / 0	-78.0	-78.0	0.93 (1)	3.76	C-O	0 / 524	0.08 (1)	
E-F	-2732 / 0	-78.0	-78.0	0.93 (1)	3.76	O-E	-574 / 0	0.65 (1)	
F-G	-2732 / 0	-78.0	-78.0	0.93 (1)	3.76	O-G	0 / 524	0.08 (1)	
G-H	-2350 / 0	-78.0	-78.0	0.87 (1)	4.03	M-G	-1061 / 0	0.41 (1)	
H-I	-1746 / 0	-78.0	-78.0	0.38 (1)	4.71	M-H	0 / 1787	0.29 (1)	
S-A	-1946 / 0	0.0	0.0	0.22 (1)	6.03	L-H	-245 / 0	0.28 (1)	
K-I	-1946 / 0	0.0	0.0	0.22 (1)	6.03	A-R	0 / 1107	0.25 (1)	
						L-I	0 / 1107	0.25 (1)	
T-S	0 / 0	-96.5	-96.5	0.16 (1)	10.00				
S-R	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
R-Q	0 / 1038	-18.5	-18.5	0.34 (4)	10.00				
Q-P	0 / 2351	-18.5	-18.5	0.51 (1)	10.00				
P-O	0 / 2351	-18.5	-18.5	0.51 (1)	10.00				
O-N	0 / 2351	-18.5	-18.5	0.51 (1)	10.00				
N-M	0 / 2351	-18.5	-18.5	0.51 (1)	10.00				
M-L	0 / 1038	-18.5	-18.5	0.34 (4)	10.00				
L-K	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
K-J	0 / 0	-96.5	-96.5	0.16 (1)	10.00				

SPECIFIED LOADS:

TOP	CH.	LL	"
		DL	"

BOT	CH.	DL =	6.0	PSF
		LL =	0.0	PSF
		DL =	7.4	PSF

TOTAL LOAD = 34.4 PSF

1000

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.29")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.93/1.00 (C-E:1) , BC=0.51/1.00 (O-Q:1)
WB=0.65/1.00 (E-O:1) , SS=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

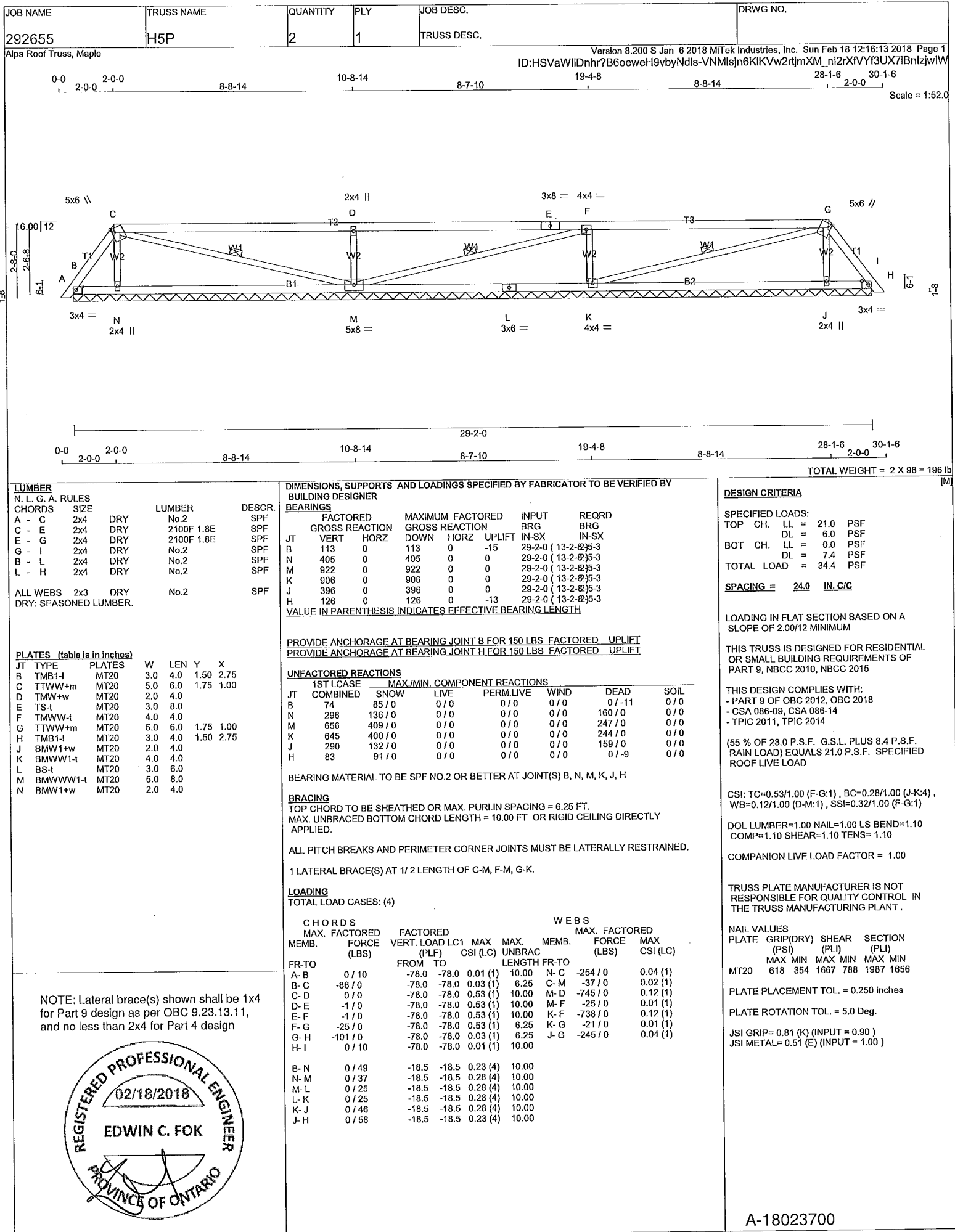
NAIL VALUES					
	GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.70 (N) (INPUT = 1.00)

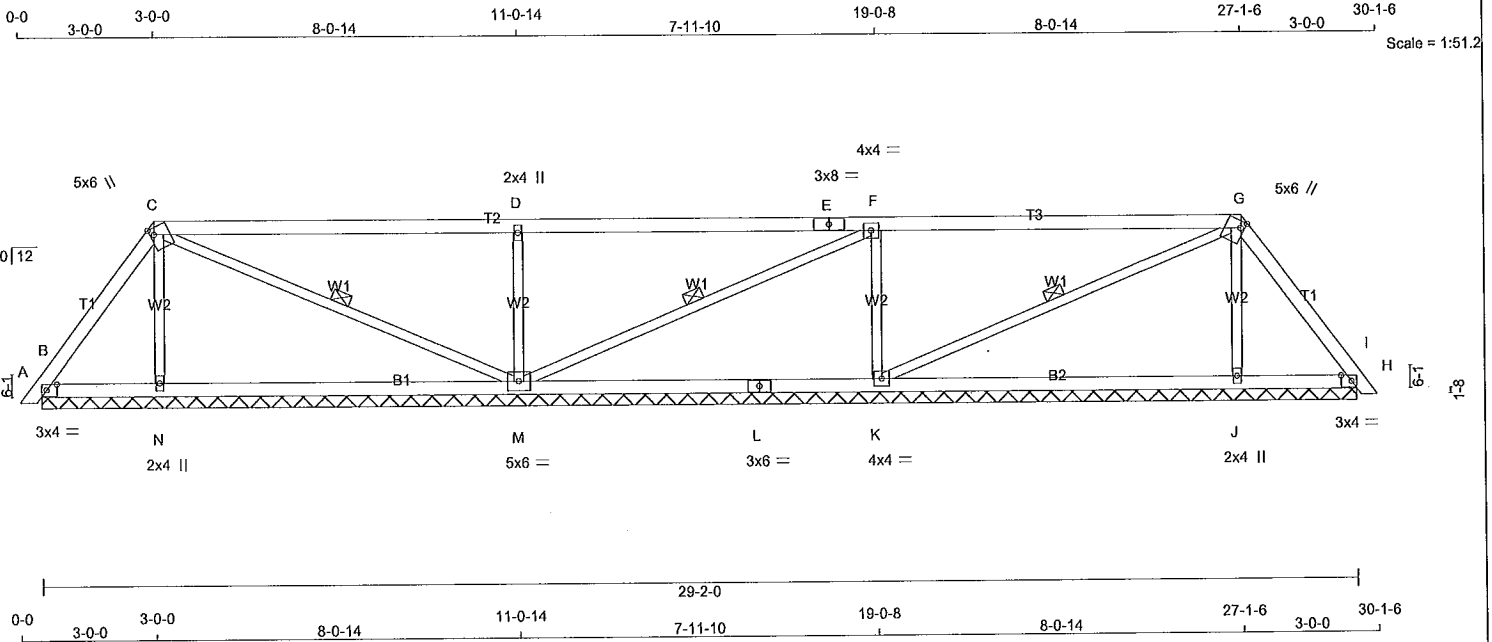
A-18023698 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H6P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER					DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER			
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
G - I	2x4	DRY	No.2	SPF		
B - L	2x4	DRY	No.2	SPF		
L - H	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	199	0	199	0	29-2-0 (13-2-8)2-10	
N	369	0	369	0	29-2-0 (13-2-8)2-10	
M	891	0	891	0	29-2-0 (13-2-8)2-10	
K	826	0	826	0	29-2-0 (13-2-8)2-10	
J	358	0	358	0	29-2-0 (13-2-8)2-10	
H	226	0	226	0	29-2-0 (13-2-8)2-10	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				WIND	DEAD	SOIL
		COMBINED	SNOW	LIVE	PERM.LIVE			
B	137	110 / 0	0 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
N	269	132 / 0	0 / 0	0 / 0	0 / 0	0 / 0	137 / 0	0 / 0
M	633	399 / 0	0 / 0	0 / 0	0 / 0	0 / 0	235 / 0	0 / 0
K	588	364 / 0	0 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
J	261	126 / 0	0 / 0	0 / 0	0 / 0	0 / 0	135 / 0	0 / 0
H	156	124 / 0	0 / 0	0 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

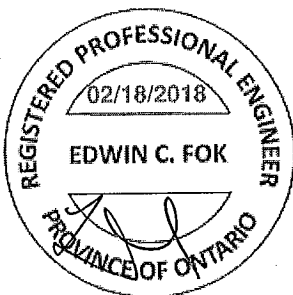
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		W E B S		FACTORED	
	MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	MAX. FORCE (LBS)	MAX. FORCE (LBS)	LC1 MAX (LC)
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	N-C	-245 / 0	0.06 (1)
B-C	-96 / 0	-78.0	-78.0	0.09 (1)	6.25	C-M	-70 / 0	0.04 (1)
C-D	0 / 16	-78.0	-78.0	0.09 (1)	10.00	M-D	-688 / 0	0.16 (1)
D-E	0 / 16	-78.0	-78.0	0.09 (1)	10.00	M-F	-60 / 0	0.03 (1)
E-F	0 / 16	-78.0	-78.0	0.09 (1)	10.00	K-F	-663 / 0	0.16 (1)
F-G	-39 / 0	-78.0	-78.0	0.09 (1)	6.25	K-G	-31 / 0	0.02 (1)
G-H	-128 / 0	-78.0	-78.0	0.09 (1)	6.25	J-G	-235 / 0	0.05 (1)
H-I	0 / 10	-78.0	-78.0	0.01 (1)	10.00			
B-N	0 / 56	-18.5	-18.5	0.18 (4)	10.00			
N-M	0 / 48	-18.5	-18.5	0.24 (4)	10.00			
M-L	0 / 39	-18.5	-18.5	0.24 (4)	10.00			
L-K	0 / 39	-18.5	-18.5	0.24 (4)	10.00			
K-J	0 / 68	-18.5	-18.5	0.24 (4)	10.00			
J-H	0 / 74	-18.5	-18.5	0.18 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.69/1.00 (F-G:1), BC=0.24/1.00 (J-K:4), WB=0.16/1.00 (D-M:1), SSI=0.29/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

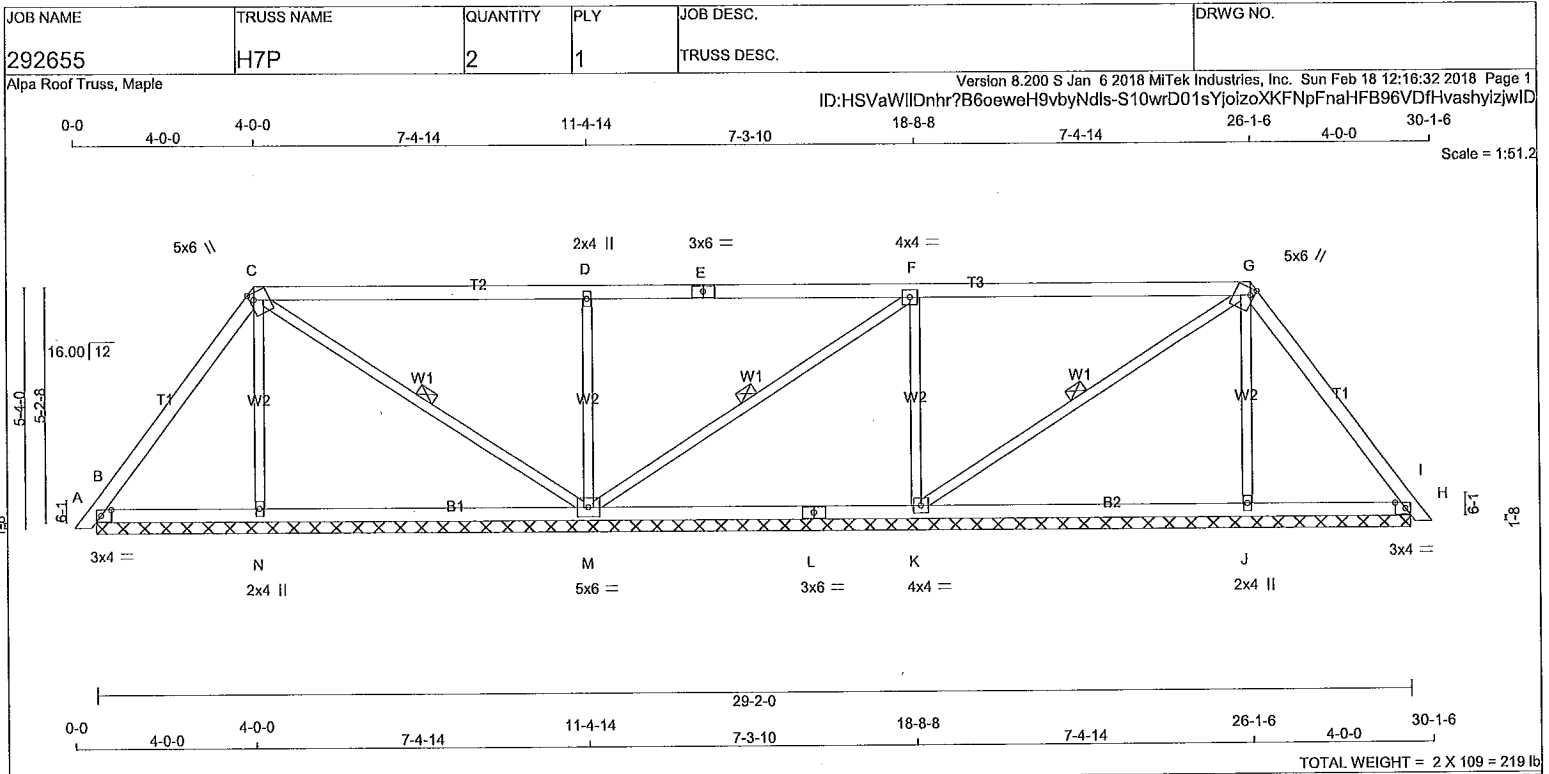
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (M) (INPUT = 0.90)
JSI METAL= 0.60 (E) (INPUT = 1.00)

A-18023701



LUMBER				N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF			
C - E	2x4	DRY	No.2	SPF			
E - G	2x4	DRY	No.2	SPF			
G - I	2x4	DRY	No.2	SPF			
B - L	2x4	DRY	No.2	SPF			
L - H	2x4	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	260	0	260	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1
N	347	0	347	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1
M	885	0	885	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1
K	733	0	733	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1
J	340	0	340	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1
H	304	0	304	0	0	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1	29-2-0 (13-2-0)1-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	182	132 / 0	0 / 0	0 / 0	0 / 0	50 / 0	0 / 0
N	253	125 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
M	628	402 / 0	0 / 0	0 / 0	0 / 0	225 / 0	0 / 0
K	522	320 / 0	0 / 0	0 / 0	0 / 0	202 / 0	0 / 0
J	248	121 / 0	0 / 0	0 / 0	0 / 0	127 / 0	0 / 0
H	212	155 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX CSI (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	N-C	-231 / 0	0.09 (1)
B-C	-98 / 0	-78.0	-78.0 0.17 (1)	6.25	C-M	-106 / 0	0.06 (1)
C-D	0 / 37	-78.0	-78.0 0.58 (1)	10.00	M-D	-631 / 0	0.25 (1)
D-E	0 / 36	-78.0	-78.0 0.58 (1)	10.00	M-F	-104 / 0	0.06 (1)
E-F	0 / 36	-78.0	-78.0 0.58 (1)	10.00	K-F	-572 / 0	0.23 (1)
F-G	-50 / 0	-78.0	-78.0 0.58 (1)	6.25	K-G	-41 / 0	0.02 (1)
G-H	-152 / 0	-78.0	-78.0 0.17 (1)	6.25	J-G	-224 / 0	0.09 (1)
H-I	0 / 10	-78.0	-78.0 0.01 (1)	10.00			
B-N	0 / 57	-18.5	-18.5 0.15 (4)	10.00			
N-M	0 / 53	-18.5	-18.5 0.20 (4)	10.00			
M-L	0 / 50	-18.5	-18.5 0.20 (4)	10.00			
L-K	0 / 50	-18.5	-18.5 0.20 (4)	10.00			
K-J	0 / 84	-18.5	-18.5 0.21 (4)	10.00			
J-H	0 / 89	-18.5	-18.5 0.15 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.58/1.00 (D-F:1), BC=0.21/1.00 (J-K:4), WB=0.25/1.00 (D-M:1), SSI=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

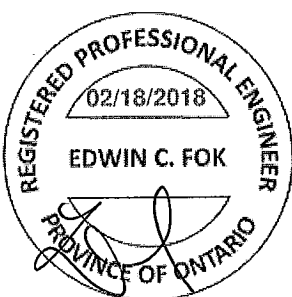
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

A-18023702

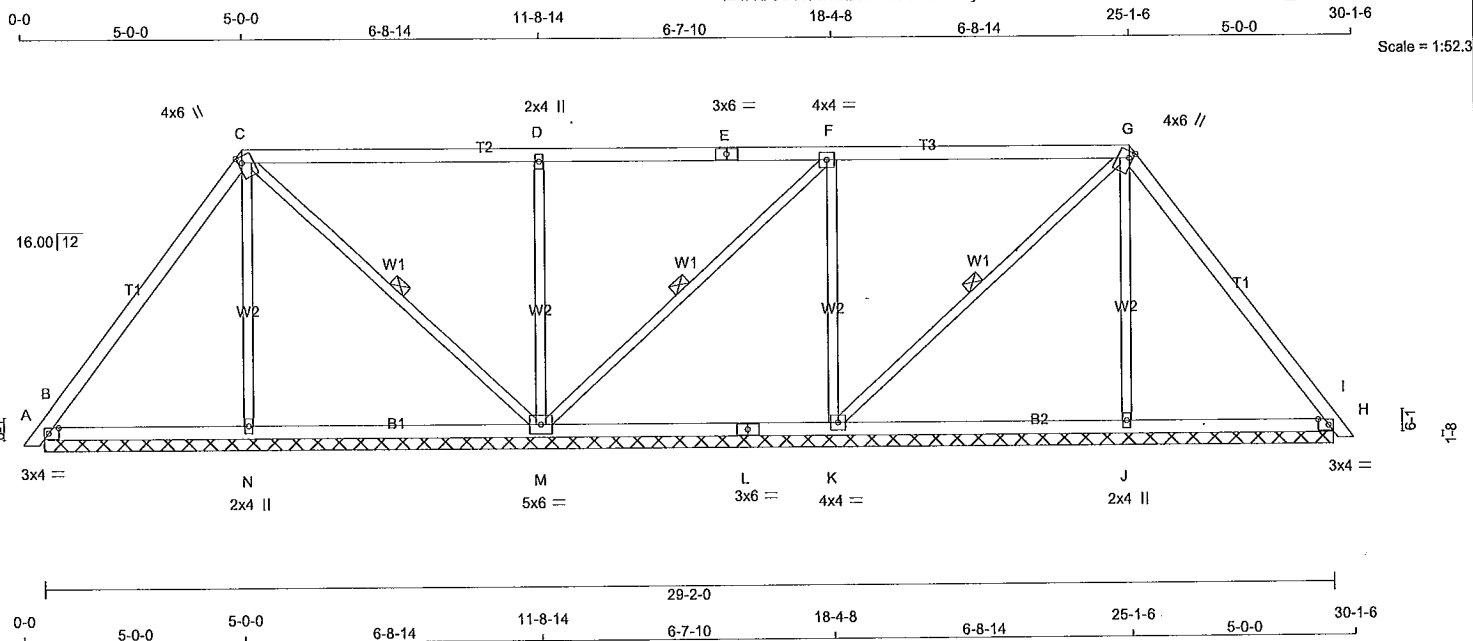


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H8P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
B - L	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
JT								
B	310	0	310	0	29-2-0 (13-2-8)2-0			
N	332	0	332	0	29-2-0 (13-2-8)2-0			
M	890	0	890	0	29-2-0 (13-2-8)2-0			
K	638	0	638	0	29-2-0 (13-2-8)2-0			
J	330	0	330	0	29-2-0 (13-2-8)2-0			
H	369	0	369	0	29-2-0 (13-2-8)2-0			

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LC CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
B	218	151 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
N	242	117 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
M	629	412 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0
K	455	277 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
J	241	115 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
H	259	183 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FACTORED	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED	FORCE (LBS)
FR-TO						FR-TO		
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	N-C	-216 / 0	0.15 (1)
B-C	-95 / 0	-78.0	-78.0	0.27 (1)	6.25	C-M	-139 / 0	0.08 (1)
C-D	0 / 51	-78.0	-78.0	0.48 (1)	10.00	M-D	-573 / 0	0.40 (1)
D-E	0 / 50	-78.0	-78.0	0.48 (1)	10.00	M-F	-143 / 0	0.08 (1)
E-F	0 / 50	-78.0	-78.0	0.48 (1)	10.00	K-F	-475 / 0	0.33 (1)
F-G	-53 / 0	-78.0	-78.0	0.48 (1)	6.25	K-G	-57 / 0	0.03 (1)
G-H	-168 / 0	-78.0	-78.0	0.28 (1)	6.25	J-G	-214 / 0	0.15 (1)
H-I	0 / 10	-78.0	-78.0	0.01 (1)	10.00			
B-N	0 / 56	-18.5	-18.5	0.14 (4)	10.00			
N-M	0 / 52	-18.5	-18.5	0.17 (4)	10.00			
M-L	0 / 53	-18.5	-18.5	0.17 (4)	10.00			
L-K	0 / 53	-18.5	-18.5	0.17 (4)	10.00			
K-J	0 / 85	-18.5	-18.5	0.17 (4)	10.00			
J-H	0 / 99	-18.5	-18.5	0.15 (4)	10.00			

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (D-F:1), BC=0.17/1.00 (J-K:4), WB=0.40/1.00 (D-M:1), SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

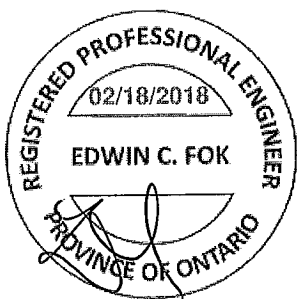
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

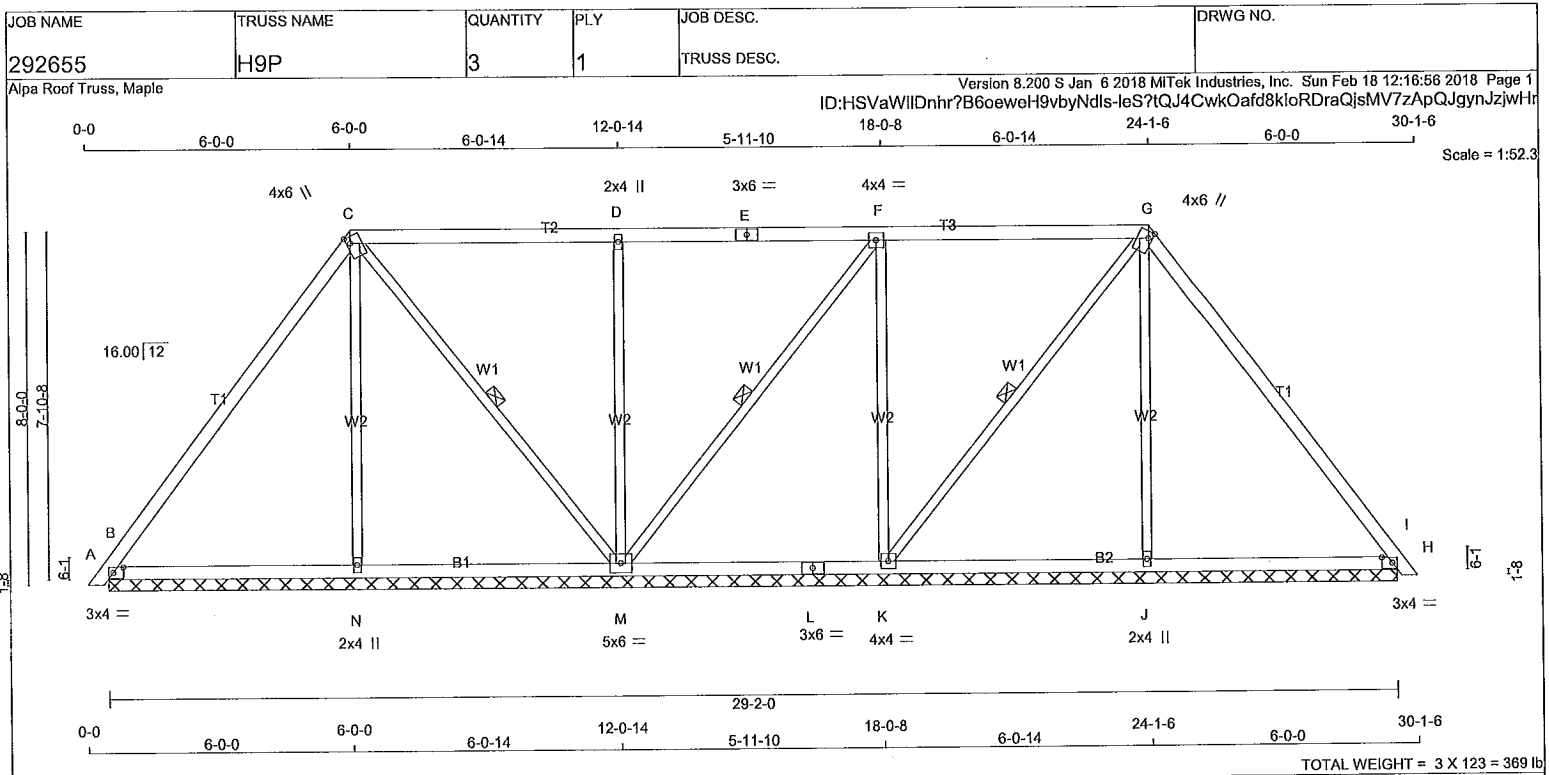
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.17 (H) (INPUT = 1.00)

A-18023703





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0	1.50 2.75
C	TTWW+m	MT20	4.0	6.0	1.75 1.00
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-I	MT20	4.0	4.0	
G	TTWW+m	MT20	4.0	6.0	1.75 1.00
H	TMB1-I	MT20	3.0	4.0	1.50 2.75
J	BMW1+w	MT20	2.0	4.0	
K	BMWW1-I	MT20	4.0	4.0	
L	BS-t	MT20	3.0	6.0	
M	BMWWW1-I	MT20	5.0	6.0	
N	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT		VERT	HORZ	UP/LIFT	IN-SX
B	356	0	356	0	29-2-0 (13-2-8)4-1
N	325	0	325	0	29-2-0 (13-2-8)4-1
M	883	0	883	0	29-2-0 (13-2-8)4-1
K	555	0	555	0	29-2-0 (13-2-8)4-1
J	325	0	325	0	29-2-0 (13-2-8)4-1
H	424	0	424	0	29-2-0 (13-2-8)4-1

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	251	170 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
N	238	110 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
M	623	417 / 0	0 / 0	0 / 0	0 / 0	206 / 0	0 / 0
K	396	241 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0
J	238	110 / 0	0 / 0	0 / 0	0 / 0	128 / 0	0 / 0
H	298	206 / 0	0 / 0	0 / 0	0 / 0	91 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB.	MAX. FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 10	-78.0	-78.0 0.01 (1)	10.00	N-C	-205 / 0	0.24 (1)
B-C	-90 / 0	-78.0	-78.0 0.41 (1)	6.25	C-M	-165 / 0	0.11 (1)
C-D	0 / 53	-78.0	-78.0 0.39 (1)	10.00	M-D	-516 / 0	0.60 (1)
D-E	0 / 53	-78.0	-78.0 0.39 (1)	10.00	M-F	-166 / 0	0.11 (1)
E-F	0 / 53	-78.0	-78.0 0.39 (1)	10.00	K-F	-385 / 0	0.45 (1)
F-G	-50 / 0	-78.0	-78.0 0.39 (1)	6.25	K-G	-79 / 0	0.05 (1)
G-H	-174 / 0	-78.0	-78.0 0.42 (1)	6.25	J-G	-204 / 0	0.24 (1)
H-I	0 / 10	-78.0	-78.0 0.01 (1)	10.00			
B-N	0 / 53	-18.5	-18.5 0.15 (4)	10.00			
N-M	0 / 50	-18.5	-18.5 0.15 (4)	10.00			
M-L	0 / 50	-18.5	-18.5 0.13 (4)	10.00			
L-K	0 / 50	-18.5	-18.5 0.13 (4)	10.00			
K-J	0 / 100	-18.5	-18.5 0.16 (4)	10.00			
J-H	0 / 103	-18.5	-18.5 0.16 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (G-H:1), BC=0.16/1.00 (H-J:4), WB=0.60/1.00 (D-M:1), SSI=0.22/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

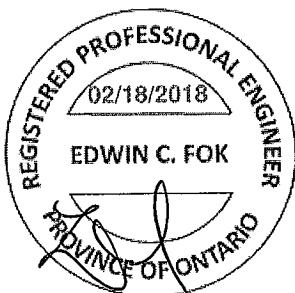
NAIL VALUES	GRIP(DRY)	SHEAR	SECTION
PLATE	(PSI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 Inches

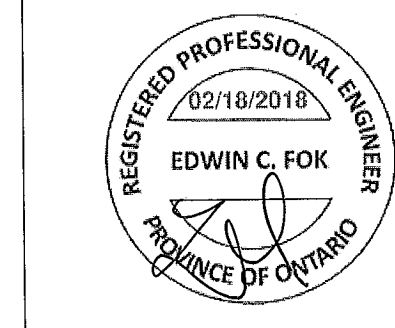
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.19 (H) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023704



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H10A	1	1	TRUSS DESC.	

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ID:HSVavWlIDnhr?B6oeweH9vbyNdis-?2fNm3GBUnWAvdMPIm5VLOIf4IRDZ1nWV1JELjzw7a

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMWV-1	MT20	5.0	6.0	2.50	2.25
P	BMWV-1	MT20	6.0	7.0	2.00	2.25
Q	BS-1	MT20	6.0	7.0		
R	BMWV-1	MT20	5.0	6.0	2.25	2.25
S	BMWV-1	MT20	8.0	9.0	4.00	3.00
T	BS-1	MT18HS	5.0	12.0		
U	BMWV-1	MT20	2.0	4.0		
V	BMWV-1	MT20	7.0	8.0	2.00	1.75
W	BMWV-1	MT20	4.0	9.0	3.00	1.50
X	BMWV-1	MT20	5.0	8.0	4.25	2.00
Y	BMV1-t	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5, AND 199.7 lbs FACTORED DOWN AT 37-11-11 ON TOP CHORD, AND 1176.1 lbs FACTORED DOWN AT 2-10-14, AND 1376.2 lbs FACTORED DOWN AT 19-2-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MAX UNBRAC	MEMB.	FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
N-M	0/0	-96.5	-96.5	0.06 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	2-4-5	-134	-134	---	FRONT	VERT	TOTAL
K	37-11-11	-200	-200	---	FRONT	VERT	TOTAL
S	19-2-10	-1376	-1376	---	FRONT	VERT	TOTAL
AB	2-10-14	-1176	-1176	---	FRONT	VERT	TOTAL

ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.60/1.00 (H-J:1), BC=0.80/1.00 (R-S:1),
WB=0.92/1.00 (K-P:1), SSI=0.78/1.00 (W-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656
MT18HS	511	354	2455 1382 3004 2010

PLATE PLACEMENT TOL. = 0.250 inches

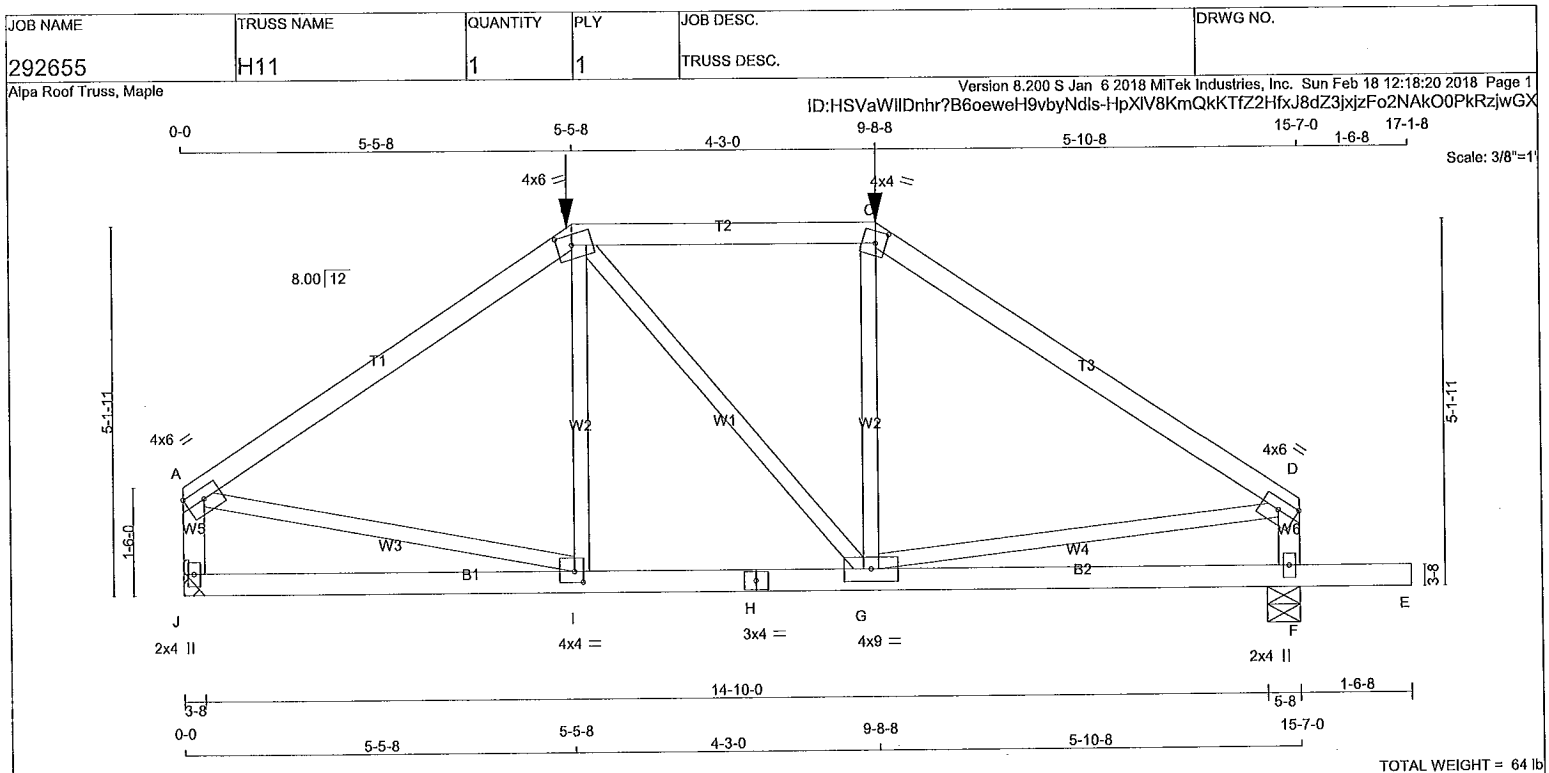
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (L) (INPUT = 0.90)
JSI METAL= 0.98 (Q) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023705(2)



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
C - D	2x4 DRY	No.2	SPF		
J - A	2x4 DRY	No.2	SPF		
J - D	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
H - E	2x4 DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	6.0	1.75	Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.75	1.50
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 334.0 lbs FACTORED DOWN AT 9-8-8, AND 310.3 lbs FACTORED DOWN AT 5-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	UP
JT				
J	1384	0	1384	0
F	1513	0	1513	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	988	592 / 0	0 / 0	0 / 0	0 / 0	0 / 0	396 / 0	0 / 0
F	1081	646 / 0	0 / 0	0 / 0	0 / 0	0 / 0	435 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC) (PLF)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					FR-TO		
A-B	-1479 / 0	-78.0	-78.0	0.54 (1)	I-B	-78 / 106	0.04 (4)
B-C	-1271 / 0	-153.5	-153.5	0.57 (1)	B-G	0 / 68	0.02 (1)
C-D	-1531 / 0	-78.0	-78.0	0.65 (1)	G-C	-38 / 134	0.05 (4)
J-A	-1297 / 0	0.0	0.0	0.15 (1)	A-I	0 / 1257	0.31 (1)
F-D	-1271 / 0	0.0	0.0	0.14 (1)	G-D	0 / 1287	0.32 (1)
J-I	0 / 0	-36.4	-36.4	0.26 (4)			
I-H	0 / 1228	-36.4	-36.4	0.41 (4)			
H-G	0 / 1228	-36.4	-36.4	0.41 (4)			
G-F	0 / 0	-36.4	-36.4	0.30 (4)			
F-E	0 / 0	-96.5	-96.5	0.17 (1)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-5-8	-310	-310	---	FRONT	VERT	TOTAL
C	9-8-8	-334	-334	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 5-5-8
RIGHT SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.65/1.00 (C-D:1), BC=0.41/1.00 (G-I:4), WB=0.32/1.00 (D-G:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

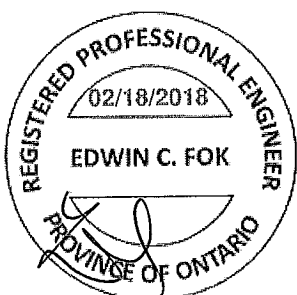
COMPANION LIVE LOAD FACTOR = 1.00

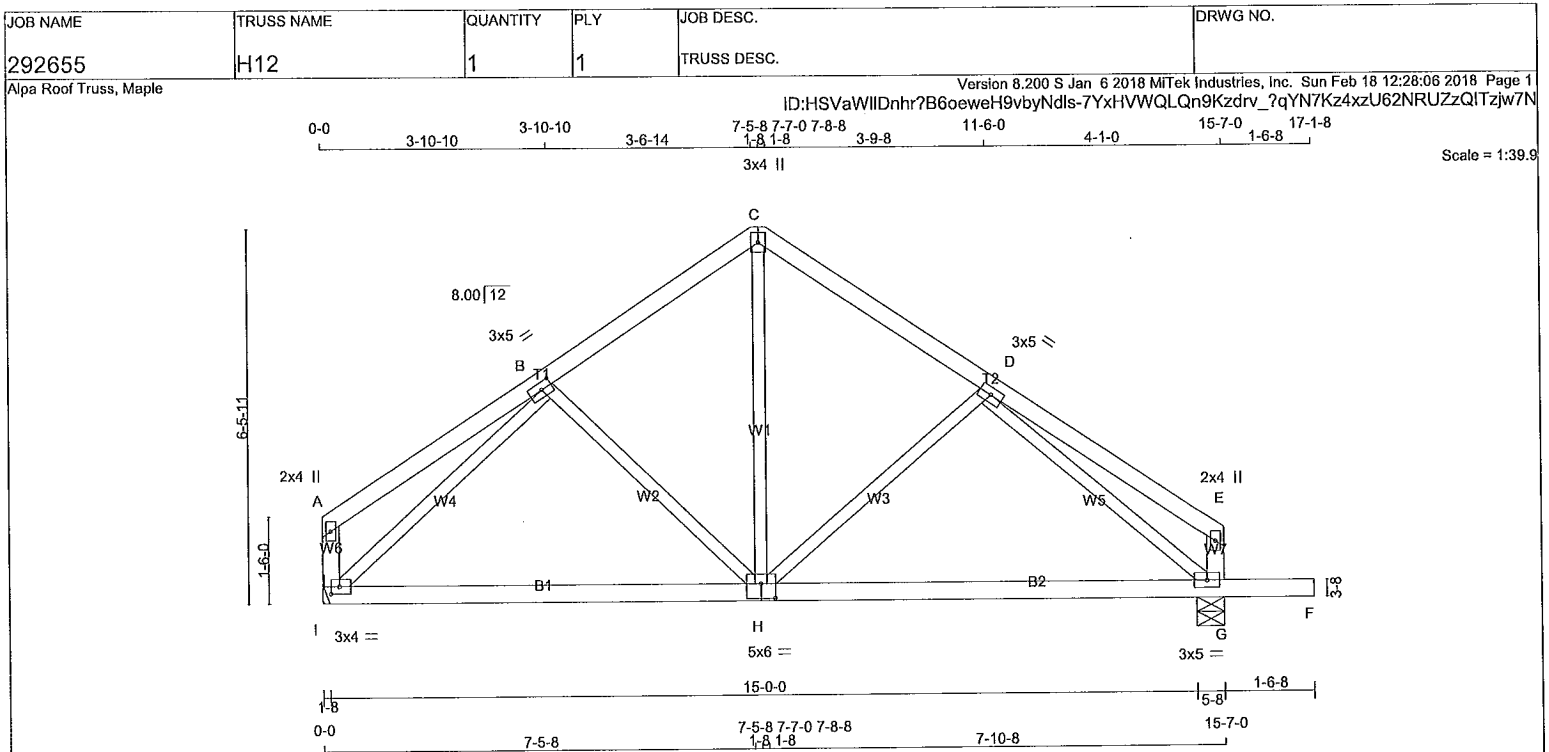
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

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CONTINUED ON PAGE 2





LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVW-t	MT20	3.0	5.0	1.50	2.25
C	TTW+p	MT20	3.0	4.0		
D	TMVW-t	MT20	3.0	5.0		
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	5.0		
H	BSWW1-t	MT20	5.0	6.0	3.00	3.00
I	BMVW1-t	MT20	3.0	4.0	1.50	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
I	752	0	752	0	5-8	1-8
G	900	0	900	0	5-8	1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. SNOW		LIVE		PERM.LIVE		WIND		DEAD		SOIL	
	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE	SNOW	LIVE
I	536	327 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0	0 / 0	0 / 0
G	642	392 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	250 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (PLF)	MAX (PLF)			FORCE (LBS)	MAX (PLF)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 19	-78.0	-78.0	0.18 (1)	10.00	H-C	0 / 389	0.09 (1)	
B-C	-583 / 0	-78.0	-78.0	0.14 (1)	6.25	I-B	-812 / 0	0.36 (1)	
C-D	-584 / 0	-78.0	-78.0	0.15 (1)	6.25	B-H	-155 / 0	0.07 (1)	
D-E	0 / 20	-78.0	-78.0	0.20 (1)	10.00	H-D	-208 / 0	0.10 (1)	
I-A	-113 / 0	0.0	0.0	0.01 (1)	7.81	D-G	-839 / 0	0.39 (1)	
G-E	-119 / 0	0.0	0.0	0.01 (1)	7.81				
I-H	0 / 580	-18.5	-18.5	0.36 (4)	10.00				
H-G	0 / 625	-18.5	-18.5	0.40 (4)	10.00				
G-F	0 / 0	-96.5	-96.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/360 (0.52")
 CALCULATED VERT. DEFL.(TL) = L/937 (0.20")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
 ALLOWABLE DEFL.(TL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (D-E:1), BC=0.40/1.00 (G-H:4), WB=0.39/1.00 (D-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

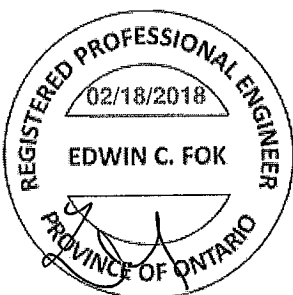
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
 JSI METAL= 0.23 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

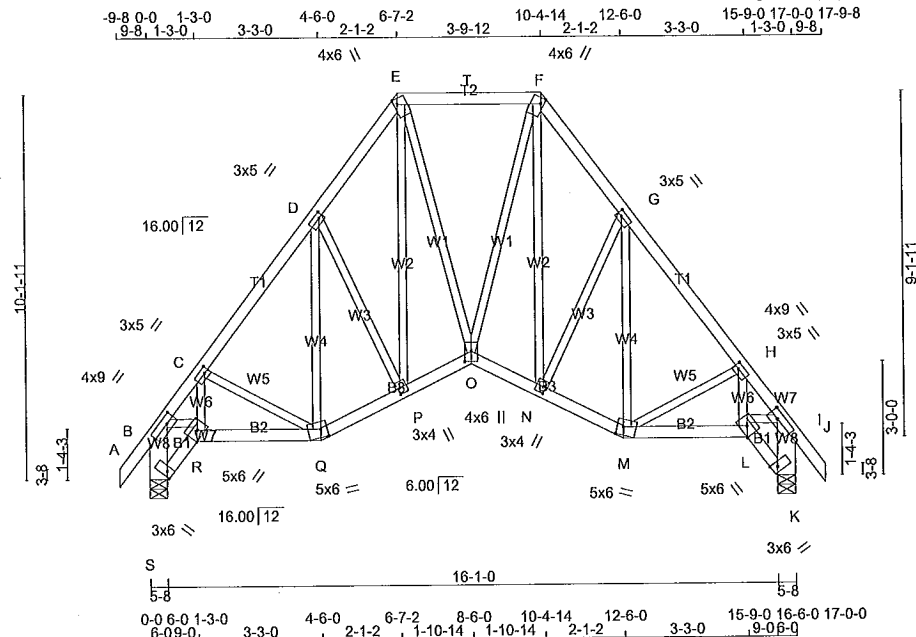


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Alpa Roof Truss, Maple

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Scale = 1:60.8



TOTAL WEIGHT = $2 \times 110 = 220 \text{ lb}$

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K
BEARING SIZE FACTOR = 1.15 AT JNT(S) S, K (BASED ON SUPPORT DEPTH = 1-8)

BRACING

FOR SECTION E-F, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.14
FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

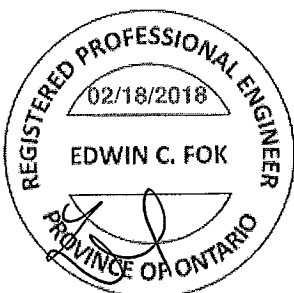
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED			MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX.	MEMB.	FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)	UNBRAC			(LBS)	CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-B	0 / 28	-78.0	-78.0	0.05 (1)	10.00	R-C	0 / 94	0.02 (4)	
B-C	-1031 / 0	-78.0	-78.0	0.05 (1)	6.14	C-Q	-169 / 0	0.04 (1)	
C-D	-792 / 0	-78.0	-78.0	0.07 (1)	6.25	Q-D	-118 / 0	0.07 (1)	
D-E	-760 / 0	-78.0	-78.0	0.07 (1)	6.25	D-P	-84 / 0	0.04 (1)	
E-T	-507 / 0	-85.5	-85.5	0.21 (1)	2.00	P-E	0 / 138	0.03 (1)	
T-F	-507 / 0	-85.5	-85.5	0.21 (1)	2.00	E-O	0 / 243	0.05 (1)	
F-G	-760 / 0	-78.0	-78.0	0.07 (1)	6.25	O-F	0 / 243	0.05 (1)	
G-H	-792 / 0	-78.0	-78.0	0.07 (1)	6.25	N-F	0 / 138	0.03 (1)	
H-I	-1031 / 0	-78.0	-78.0	0.05 (1)	6.14	N-G	-84 / 0	0.04 (1)	
I-J	0 / 28	-78.0	-78.0	0.05 (1)	10.00	M-G	-118 / 0	0.07 (1)	
S-B	-894 / 0	0.0	0.0	0.06 (1)	7.81	M-H	-169 / 0	0.04 (1)	
K-I	-894 / 0	0.0	0.0	0.06 (1)	7.81	L-H	0 / 94	0.02 (4)	
						B-R	0 / 633	0.14 (1)	
						L-I	0 / 633	0.14 (1)	
S-R	0 / 0	-18.5	-18.5	0.01 (4)	10.00				
R-Q	0 / 626	-18.5	-18.5	0.13 (1)	10.00				
Q-P	0 / 534	-18.5	-18.5	0.10 (1)	10.00				
P-O	0 / 492	-18.5	-18.5	0.09 (1)	10.00				
O-N	0 / 492	-18.5	-18.5	0.09 (1)	10.00				
N-M	0 / 534	-18.5	-18.5	0.10 (1)	10.00				
M-L	0 / 626	-18.5	-18.5	0.13 (1)	10.00				
L-K	0 / 0	-18.5	-18.5	0.01 (4)	10.00				

PLATES (table is in inches)

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.57")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.57")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.21/1.00 (E-F:1), BC=0.13/1.00 (L-M:1),
WB=0.14/1.00 (I-L:1), SSI=0.13/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.90)
JSI METAL= 0.33 (R) (INPUT = 1.00)

A-18023708

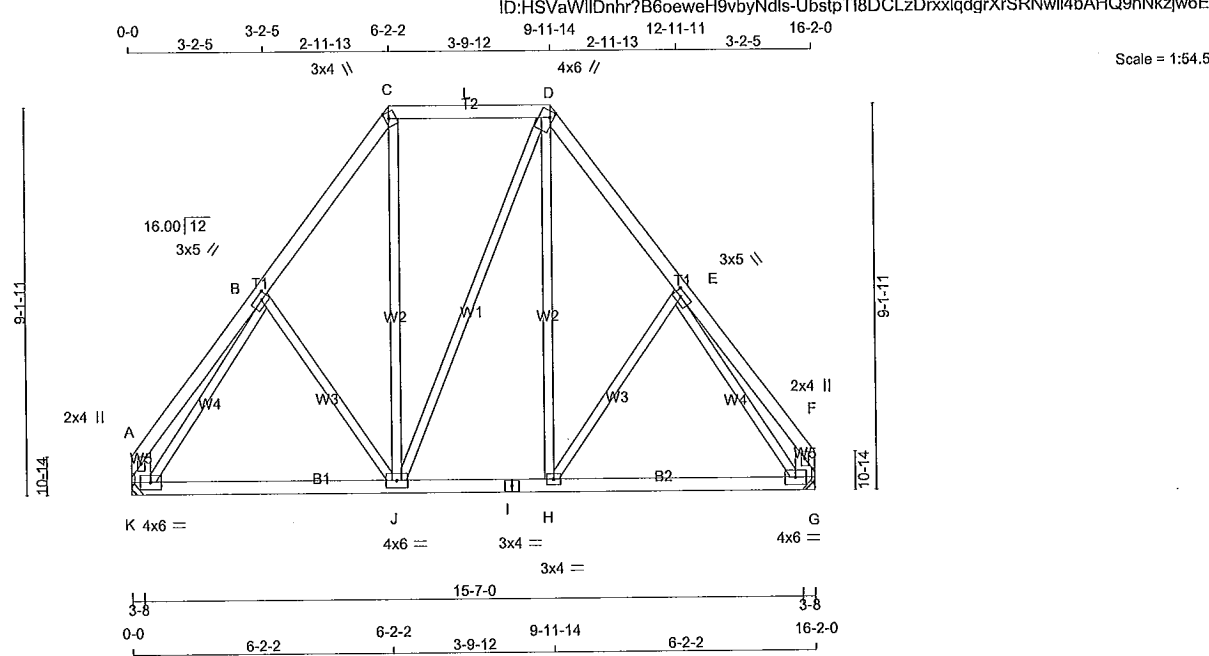
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H14S	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 3 X 88 = 264 lb

LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										DESIGN CRITERIA						
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS										SPECIFIED LOADS:						
A - C	2x4	DRY	No.2	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQRD							TOP CH.	LL	=	21.0 PSF			
C - D	2x4	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG							DL	=	6.0 PSF				
D - F	2x4	DRY	No.2	SPF	JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX							BOT CH.	LL	=	0.0 PSF
K - A	2x6	DRY	No.2	SPF	K 794	0	794	0	0	HANGER BY OTHERS						DL	=	7.4 PSF			
G - F	2x6	DRY	No.2	SPF	G 794	0	794	0	0	MIN. SEAT SIZE: 1-8						TOTAL LOAD = 34.4 PSF					
K - I	2x4	DRY	No.2	SPF	HANGER BY OTHERS										SPACING = 24.0 IN. C/C						
I - G	2x4	DRY	No.2	SPF	MIN. SEAT SIZE: 1-8										LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.						
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	UNFACTORED REACTIONS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
DRY: SEASONED LUMBER.					1ST LCASE MAX./MIN. COMPONENT REACTIONS										THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, OBC 2018 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014						
					JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.									
					K 567	339 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015									
					G 567	339 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0	THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015									
					BRACING										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FOR SECTION C-D, MAX. PURLIN SPACING = 2.00 FT.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					LOADING										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					TOTAL LOAD CASES: (4)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					CHORDS										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					MEMB. MAX. FACTORED FORCE (LBS)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FACTORED VERT. LOAD (PLF)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					MAX LC1 (C)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					MAX LC2 (C)										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					MAX UNBRACED LENGTH										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
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					FR-TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
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					TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
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					TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FR-TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FROM										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015						
					FR-TO										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC						

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

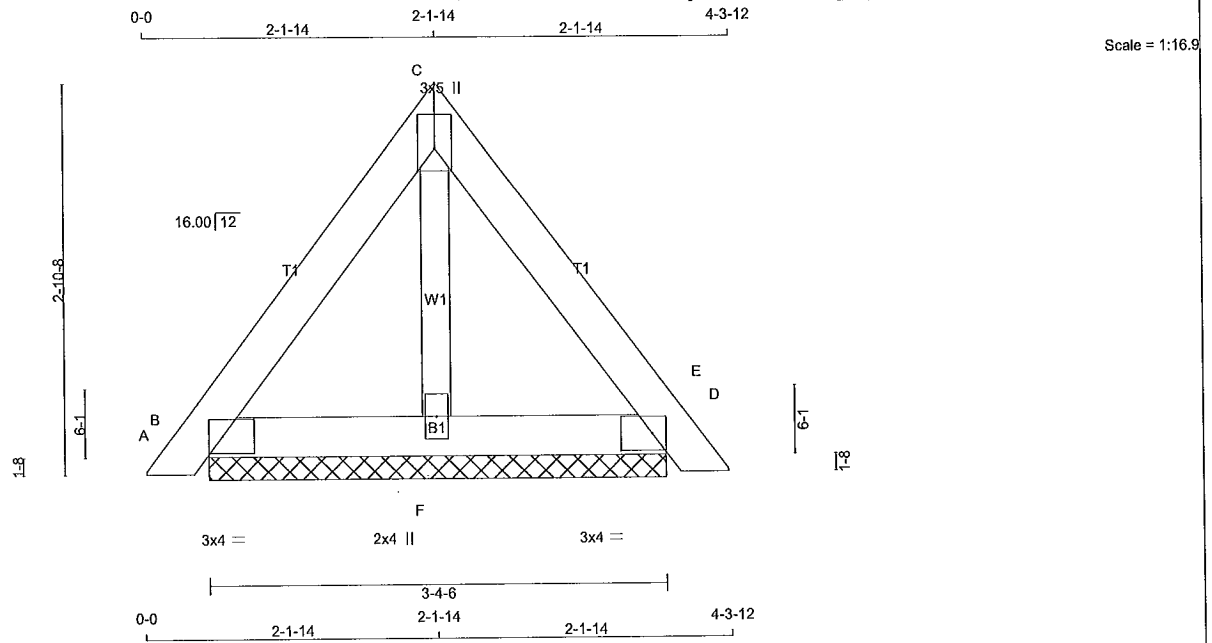
A-18023709

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H15P	5	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIIDnhr?B6oeuweH9vbyNdls-F7LvVDO9LgMqB3YUDWnY9DBs9biNamKM6f5CfHzjw66



TOTAL WEIGHT = 5 X 14 = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQRD
	VERT	HORZ	DOWN	HORZ
JT				
B	141	0	141	0
D	141	0	141	0
F	97	0	97	0

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT							
B	99	70 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	99	70 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
F	71	31 / 0	0 / 0	0 / 0	0 / 0	40 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

	CHORDS	WEBS
	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		
A-B	0 / 10	-78.0 -78.0 0.01 (1) 10.00
B-C	-45 / 0	-78.0 -78.0 0.04 (1) 6.25
C-D	-45 / 0	-78.0 -78.0 0.04 (1) 6.25
D-E	0 / 10	-78.0 -78.0 0.01 (1) 10.00
B-F	0 / 26	-18.5 -18.5 0.02 (4) 10.00
F-D	0 / 26	-18.5 -18.5 0.02 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.04/1.00 (B-C:1), BC=0.02/1.00 (B-F:4), WB=0.01/1.00 (C-F:1), SSI=0.03/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

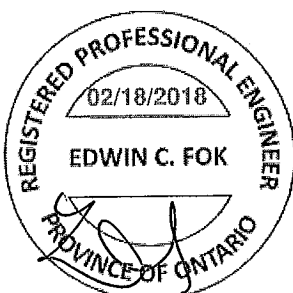
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

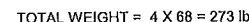
JSI GRIP= 0.17 (B) (INPUT = 0.80)
JSI METAL= 0.06 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023710

Scale: $3/16"=1'$

A-18023711

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H17A	1	1	TRUSS DESC.	

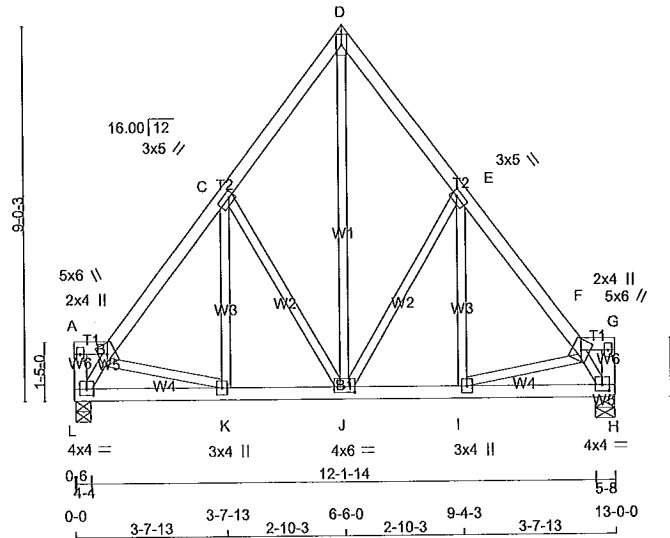
Alpa Roof Truss, Maple

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ID:HSVaWIDnhr7B6oeWeH9vbyNdis-1UPDtO6ASooTm8YpBPKO2f7xyNINVOJ1hLtgDMzjw5B

0-0 9-10 3-7-13 6-6-0 2-10-3 9-4-3 12-2-6 13-0-0
 9-10 2-10-3 2-10-3 2-10-3 9-10
 3x6 II

Scale = 1:55.4



TOTAL WEIGHT = 70 lb

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
L - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
L - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
JT	1391	0	1391	0	4-4	1-8
L	1391	0	1391	0	5-8	1-8
H	1391	0	1391	0		

UNFACTORED REACTIONS

	1ST LCASE COMBINED		MAX / MIN. SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	JT	L	JT	L	JT	L	JT	L	JT	L	JT	L	JT	L
JT	992	605 / 0	992	605 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	387 / 0	387 / 0	0 / 0	0 / 0
L	992	605 / 0	992	605 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	387 / 0	387 / 0	0 / 0	0 / 0
H	992	605 / 0	992	605 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	387 / 0	387 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.51 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LBS)	MAX. UNBRACED LENGTH (LBS)	MAX. FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LBS)	MAX. UNBRACED LENGTH (LBS)
FR-TO								
L-A	-31 / 0	0.0	0.0	0.00 (1)	7.81	L-B	-1436 / 0	0.22 (1)
A-B	0 / 0	-78.0	-78.0	0.01 (1)	10.00	B-K	-62 / 0	0.01 (1)
B-C	-1305 / 0	-78.0	-78.0	0.12 (1)	5.51	K-C	0 / 537	0.13 (1)
C-D	-901 / 0	-78.0	-78.0	0.11 (1)	6.25	C-J	-529 / 0	0.29 (1)
D-E	-901 / 0	-78.0	-78.0	0.11 (1)	6.25	J-D	0 / 1235	0.31 (1)
E-F	-1305 / 0	-78.0	-78.0	0.12 (1)	5.51	J-E	-529 / 0	0.29 (1)
F-G	0 / 0	-78.0	-78.0	0.01 (1)	10.00	E-I	0 / 537	0.13 (1)
H-G	-31 / 0	0.0	0.0	0.00 (1)	7.81	I-F	-62 / 0	0.01 (1)
L-K	0 / 857	-136.1	-136.1	0.39 (1)	10.00	F-H	-1436 / 0	0.22 (1)
K-J	0 / 800	-136.1	-136.1	0.36 (1)	10.00			
J-I	0 / 800	-136.1	-136.1	0.36 (1)	10.00			
I-H	0 / 857	-136.1	-136.1	0.39 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CSld Girder
 START DISTANCE = 0-0
 START SPAN CARRIED = 6-10-8
 END DISTANCE = 13-0-0
 END SPAN CARRIED = 6-10-8
 END WALL WIDTH = 0-0
 APPLIED TO FRONT SIDE OF BOTTOM CHORD.
 - ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL) = L/360 (0.43")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.12/1.00 (E-F:1), BC=0.39/1.00 (K-L:1), WB=0.31/1.00 (D-J:1), SSI=0.25/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (L) (INPUT = 0.90)
 JSI METAL= 0.48 (B) (INPUT = 1.00)

A-18023712

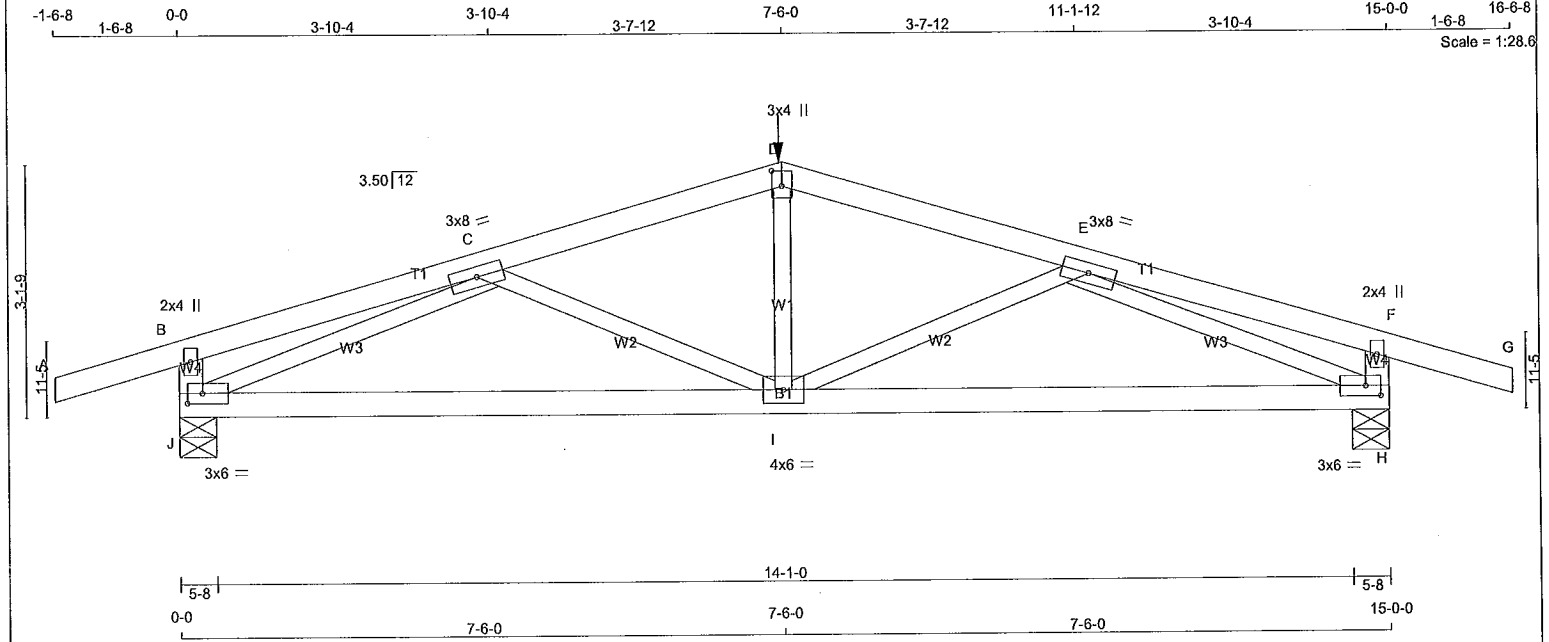
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H18-Cond1	8	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIDnhr?B6oeweH9vbyNdls-Rb8Ce1ZNk9SDiqtwrgV32S9EfIMf44rUBbLH0jzw4c



TOTAL WEIGHT = 8 X 55 = 437 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0			
C	TMWW-t	MT20	3.0	8.0			
D	TTW+p	MT20	3.0	4.0	2.25	1.50	
E	TMWW-t	MT20	3.0	8.0			
F	TMV+p	MT20	2.0	4.0			
H	BMVW1-t	MT20	3.0	6.0	1.50	2.25	
I	BMWWV-t	MT20	4.0	6.0			
J	BMVW1-t	MT20	3.0	6.0	1.50	2.25	

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 164.5 lbs FACTORED DOWN AT 7-6-0 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
J	941	0	941	0
H	941	0	941	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	426 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
J	668	426 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0
H	668	426 / 0	0 / 0	0 / 0	0 / 0	242 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.44 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX. MEMB.	MAX. FACTORED	MAX
MEMB.	FORCE (LBS)	VERT. (PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	MAX
FR-TO								
A-B	0 / 17	-78.0	-78.0	0.15 (1)	10.00	I-D	0 / 322	0.08 (1)
B-C	0 / 8	-78.0	-78.0	0.15 (1)	10.00	I-E	-163 / 13	0.05 (1)
C-D	-1293 / 0	-78.0	-78.0	0.18 (1)	5.44	C-I	-163 / 13	0.05 (1)
D-E	-1293 / 0	-78.0	-78.0	0.18 (1)	5.44	J-C	-1521 / 0	0.47 (1)
E-F	0 / 8	-78.0	-78.0	0.15 (1)	10.00	E-H	-1521 / 0	0.47 (1)
F-G	0 / 17	-78.0	-78.0	0.15 (1)	10.00			
J-B	-244 / 0	0.0	0.0	0.03 (1)	7.81			
H-F	-244 / 0	0.0	0.0	0.03 (1)	7.81			
J-I	0 / 1381	-20.1	-20.1	0.48 (4)	10.00			
I-H	0 / 1381	-20.1	-20.1	0.48 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
D	7-6-0	-164	-164	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	21.0	PSF
DL =	6.0	PSF	
BOT CH.	LL =	0.0	PSF
DL =	7.4	PSF	
TOTAL LOAD =	34.4	PSF	

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 7-6-0
END SETBACK = 2-4-2
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.18/1.00 (D-E:1), BC=0.48/1.00 (I-J:4), WB=0.47/1.00 (C-J:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

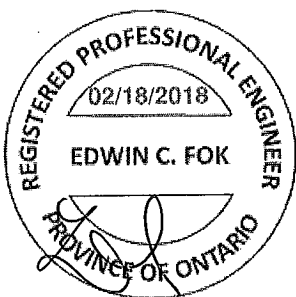
NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

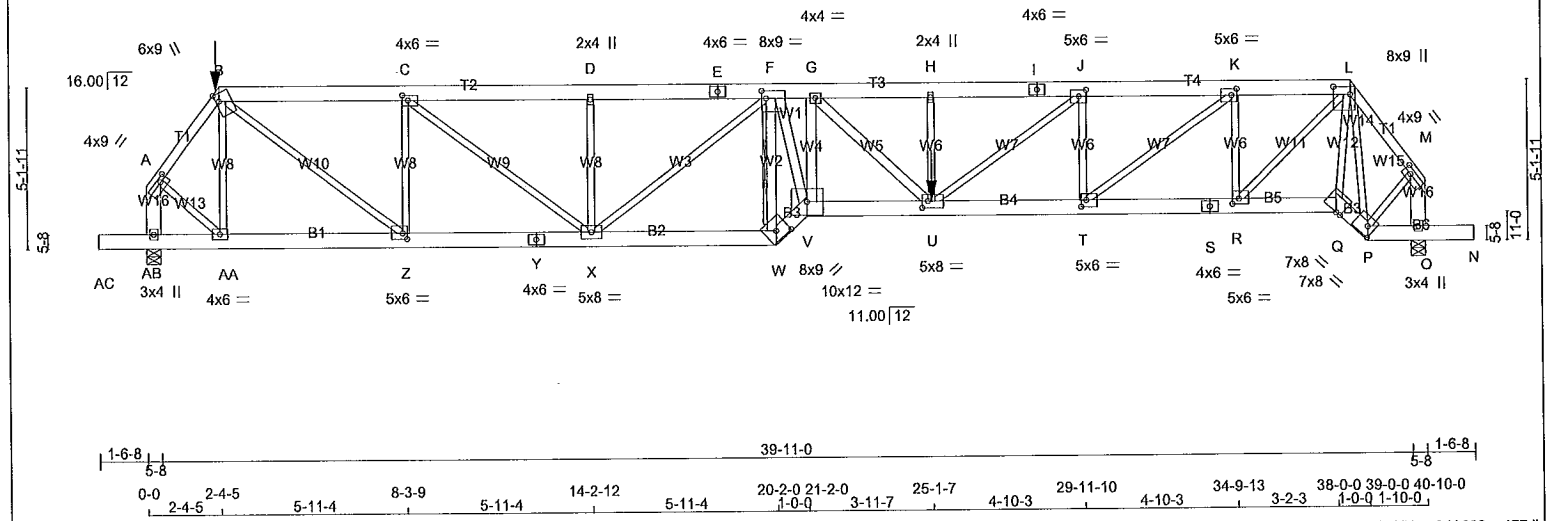
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (H) (INPUT = 0.90)
JSI METAL= 0.50 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023713



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - E	2x6	DRY	No.2	SPF	
E - I	2x6	DRY	No.2	SPF	
I - L	2x6	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
AB - A	2x6	DRY	No.2	SPF	
O - M	2x6	DRY	No.2	SPF	
AC - Y	2x6	DRY	No.2	SPF	
Y - W	2x6	DRY	No.2	SPF	
W - V	2x6	DRY	No.2	SPF	
V - S	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	No.2	SPF	
Q - P	2x6	DRY	No.2	SPF	
P - N	2x6	DRY	No.2	SPF	

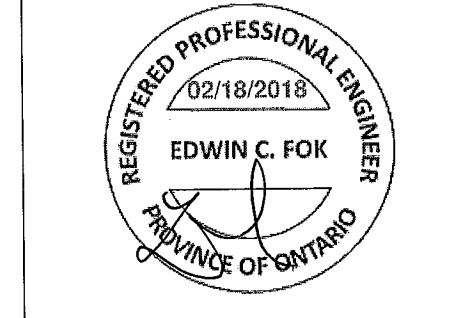
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
W - F	2x4	DRY	No.2	SPF
F - V	2x4	DRY	No.2	SPF
V - G	2x4	DRY	No.2	SPF
G - U	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B 1	12	SIDE(61.0)
L-M 1	12	TOP
B-E 2	12	SIDE(37.8)
E-I 2	12	SIDE(37.8)
I-L 2	12	TOP
AB-A 2	12	TOP
O-M 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
AC-Y 2	12	SIDE(10.0)
Y-W 2	12	SIDE(10.0)
W-V 2	12	SIDE(7.4)
V-S 2	12	SIDE(146.6)
S-Q 2	12	TOP

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
AB	4395	0	4395	0	0	5-8	2-5	5-8	2-2
O	3964	0	3964	0	0	5-8	2-2	5-8	2-2

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE
AB	3122	1972 / 0	0 / 0	0 / 0	0 / 0	1150 / 0	0 / 0	AB	3122	1972 / 0	0 / 0
O	2806	1827 / 0	0 / 0	0 / 0	0 / 0	978 / 0	0 / 0	O	2806	1827 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AB, O

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT F-W

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		W E B S		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MAX. UNBRAC	
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	-3575 / 0	-78.0 -78.0	0.09 (1)	4.91	AA-B	-1069 / 0	0.19 (1)		
B-C	-6336 / 0	-153.5 -153.5	0.26 (1)	4.52	B-Z	0 / 5369	0.66 (1)		
C-D	-9007 / 0	-153.5 -153.5	0.35 (1)	3.83	Z-C	-3122 / 0	0.55 (1)		
D-E	-9007 / 0	-153.5 -153.5	0.32 (1)	3.85	C-X	0 / 3402	0.42 (1)		
E-F	-9007 / 0	-153.5 -153.5	0.32 (1)	3.85	X-D	-849 / 0	0.15 (1)		
F-G	-12865 / 0	-153.5 -153.5	0.50 (1)	3.08	X-F	-1807 / 0	0.95 (1)		
G-H	-12865 / 0	-78.0 -78.0	0.51 (1)	3.11	W-F	-9081 / 0	0.82 (1)		
H-I	-12865 / 0	-78.0 -78.0	0.39 (1)	3.27	F-V	0 / 9313	0.82 (1)		
I-J	-12865 / 0	-78.0 -78.0	0.39 (1)	3.27	V-G	0 / 795	0.07 (1)		
J-K	-9456 / 0	-78.0 -78.0	0.23 (1)	3.86	G-U	-215 / 0	0.04 (4)		
K-L	-5637 / 0	-78.0 -78.0	0.09 (1)	4.92	U-H	-480 / 0	0.06 (1)		
L-M	-2836 / 0	-78.0 -78.0	0.07 (1)	5.40	U-J	0 / 4091	0.51 (1)		
AB-A	-4278 / 0	0.0 0.0	0.17 (1)	6.92	T-J	-2681 / 0	0.36 (1)		
O-M	-3736 / 0	0.0 0.0	0.15 (1)	7.29	T-K	0 / 4835	0.60 (1)		
					R-K	-3323 / 0	0.41 (1)		
AC-AB	0 / 0	-96.5 -96.5	0.04 (1)	10.00	Q-L	0 / 1974	0.24 (1)		
AB-AA	0 / 0	-36.4 -36.4	0.07 (4)	10.00	L-P	-3333 / 0	0.55 (1)		
AA-Z	0 / 2118	-36.4 -36.4	0.21 (1)	10.00	A-AA	0 / 2518	0.31 (1)		
Z-Y	0 / 6336	-36.4 -36.4	0.48 (1)	10.00	P-M	0 / 2185	0.27 (1)		
Y-X	0 / 6336	-36.4 -36.4	0.48 (1)	10.00	R-L	0 / 4749	0.59 (1)		
X-W	0 / 10413	-36.4 -36.4	0.75 (1)	10.00					
W-V	0 / 13738	-36.4 -36.4	0.89 (1)	10.00					
V-U	0 / 12835	-112.0 -112.0	0.95 (1)	10.00					
U-T	0 / 9456	-18.5 -18.5	0.68 (1)	10.00					
T-S	0 / 5637	-18.5 -18.5	0.40 (1)	10.00					
S-R	0 / 5637	-18.5 -18.5	0.40 (1)	10.00					
R-Q	0 / 2325	-18.5 -18.5	0.17 (1)	10.00					
Q-P	0 / 2815	-18.5 -18.5	0.18 (1)	10.00					
P-O	-1 / 0	-18.5 -18.5	0.04 (1)	10.00					
O-N	0 / 0	-96.5 -96.5	0.04 (1)	10.00					

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	
TOP CH. LL	= 21.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
SIDE SETBACK = 2-4-5
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL. LOADS BASED ON 55 % OF GSL.
LOADS APPLIED TO FIRST 21-2-0 OF SPAN MEASURED FROM THE LEFT.

GIRDER TYPE: CStdGirder
START DISTANCE = 21-2-0
START SPAN CARRIED = 5-10-8
END DISTANCE = 25-1-8
END SPAN CARRIED = 5-10-8
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL. LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.40")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/657 (0.75")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/494 (0.04")

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	H1T	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 13:35:44 2018 Page 2
ID:HSVaWIIIDnhr?B6oewehH9vbyNdis-JE8NB5XnibQYpF4qlFdArtOOAlqlvDCKdvz8jGzjvZz

Q-P 2 12
P-N 2 12
WEBS: (0.122"x3") SPIRAL NAILS
2x3 1 6
H-U 1 5
2x4 1 6

TOP
TOP

FACTORED CONCENTRATED LOADS (LBS)						FACE	DIR.	TYPE
JT	LOC.	LC1	MAX-	MAX+		FRONT	VERT	TOTAL
B	2-4-5	-134	-134	---		FRONT <td>VERT <td>TOTAL</td> </td>	VERT <td>TOTAL</td>	TOTAL
SIDE(338LB)	25-1-8	-1808	-1808	---		FRONT <td>VERT <td>TOTAL</td> </td>	VERT <td>TOTAL</td>	TOTAL

SIDE(338LB)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	9.0	1.50	3.00
B	TTWW+m	MT20	6.0	9.0	3.00	1.25
C	TMWW-t	MT20	4.0	6.0	2.00	2.25
D	TMW+w	MT20	2.0	4.0		
E	TS-t	MT20	4.0	6.0		
F	TMWWWW-t	MT20	8.0	9.0	3.00	1.75
G	TMWW-t	MT20	4.0	4.0		
H	TMW+w	MT20	2.0	4.0		
I	TS-t	MT20	4.0	6.0		
J	TMWW-t	MT20	5.0	6.0	2.50	2.75
K	TMWW-t	MT20	5.0	6.0	2.50	2.00
L	TTWWW+p	MT20	8.0	9.0	3.00	6.25
M	TMWW-t	MT20	4.0	9.0	1.75	3.00
O	BMV1+p	MT20	3.0	4.0		
P	BBWW-h	MT20	7.0	8.0	3.50	2.75
Q	BBW-h	MT20	7.0	8.0		2.00
R	BMWW-t	MT20	5.0	6.0	2.00	2.50
S	BS-t	MT20	4.0	6.0		
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMWWW-t	MT20	5.0	8.0	2.50	2.25
V	BBWW-t	MT20	10.0	12.0		
W	BBW-h	MT20	8.0	9.0	3.50	4.75
X	BMWWW-t	MT20	5.0	8.0		
Y	BS-t	MT20	4.0	6.0		
Z	BMWW-t	MT20	5.0	6.0	2.25	1.75
AA	BMWW-t	MT20	4.0	6.0		
AB	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

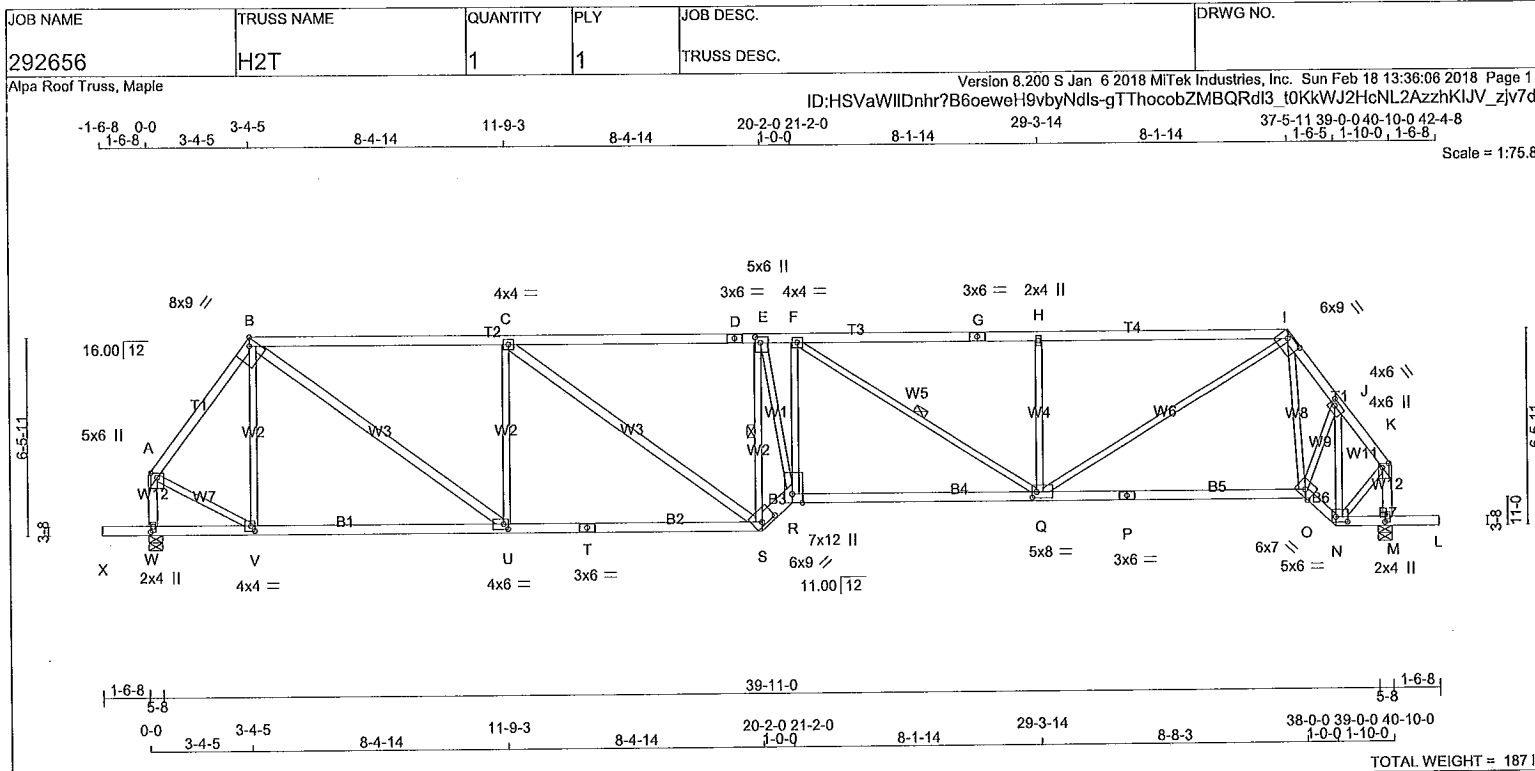
- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5 ON TOP CHORD, AND 1808.0 lbs FACTORED DOWN AT 25-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/266 (0.07")
CSI: TC=0.51/1.00 (G-H:1), BC=0.95/1.00 (U-V:1), WB=0.95/1.00 (F-X:1), SSI=0.28/1.00 (F-G:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00
COMPANION LIVE LOAD FACTOR = 1.00
AUTOSOLVE HEELS OFF
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.90 (X) (INPUT = 0.90)
JSI METAL= 0.90 (F) (INPUT = 1.00)

A-18023714(2)



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	2100F 1.8E	SPF	
D - G	2x4	DRY	2100F 1.8E	SPF	
G - I	2x4	DRY	2100F 1.8E	SPF	
I - K	2x4	DRY	No.2	SPF	
W - A	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
X - T	2x4	DRY	No.2	SPF	
T - S	2x4	DRY	No.2	SPF	
S - R	2x6	DRY	No.2	SPF	
R - P	2x4	DRY	No.2	SPF	
P - O	2x4	DRY	No.2	SPF	
O - N	2x4	DRY	No.2	SPF	
N - L	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
B - U	2x4	DRY	No.2	SPF	
C - S	2x4	DRY	No.2	SPF	

DRY; SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	5.0	6.0	1.75 2.50
B	TTWW-h	MT20	8.0	9.0	Edge 2.75
C	TMWW-t	MT20	4.0	4.0	
D	TS-t	MT20	3.0	6.0	
E	TMWW-t	MT20	5.0	6.0	2.25 2.00
F	TMWW-t	MT20	4.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMW+w	MT20	2.0	4.0	
I	TTWW-h	MT20	6.0	9.0	1.50 5.75
J	TMWW-t	MT20	4.0	6.0	1.75 2.00
K	TMVW+p	MT20	4.0	6.0	1.75 2.50
M	BMV1+p	MT20	2.0	4.0	2.25 1.00
N	BBWW-t	MT20	5.0	6.0	2.00 4.50
O	BBWW-h	MT20	6.0	7.0	Edge
P	BS-t	MT20	3.0	6.0	
Q	BMVWW-t	MT20	5.0	8.0	2.25 1.75
R	BBWW+p	MT20	7.0	12.0	Edge 4.00

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR FOR DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
	DOWN	HORZ	UPLIFT	IN-SX	IN-SX				
JT	2115	0	2115	0	0	5-8	3-3		
W	2121	0	2121	0	0	5-8	3-4		
M	2121	0	2121	0	0	5-8	3-4		
UNFACTORED REACTIONS									
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS							
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
JT	1508	921/0	0/0	0/0	0/0	588/0	0/0		
W	1512	923/0	0/0	0/0	0/0	589/0	0/0		
M	1512	923/0	0/0	0/0	0/0	589/0	0/0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.									
1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-S, F-Q.									
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (L.C.)	MAX. MEMB. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (L.C.)	MAX. MEMB. UNBRAC LENGTH	MEMB.
FR-TO		FROM TO			FR-TO				
A-B	-1694/0	-78.0 -78.0	0.21 (1)	4.92	V-B	-336/0	0.23 (1)		
B-C	-2774/0	-78.0 -78.0	0.81 (1)	4.21	B-U	0/2192	0.35 (1)		
C-D	-3341/0	-78.0 -78.0	0.91 (1)	3.87	U-C	-1152/0	0.78 (1)		
D-E	-3341/0	-78.0 -78.0	0.91 (1)	3.87	C-S	0/701	0.11 (1)		
E-F	-3904/0	-78.0 -78.0	0.39 (1)	3.97	S-E	-3414/0	0.93 (1)		
F-G	-3211/0	-78.0 -78.0	0.81 (1)	3.99	E-R	0/2965	0.67 (1)		
G-H	-3211/0	-78.0 -78.0	0.81 (1)	3.99	R-F	0/292	0.07 (1)		
H-I	-3211/0	-78.0 -78.0	0.82 (1)	3.99	F-Q	-821/0	0.54 (1)		
I-J	-1917/0	-78.0 -78.0	0.07 (1)	4.82	Q-H	-727/0	0.34 (1)		
J-K	-1386/0	-78.0 -78.0	0.06 (1)	5.48	Q-I	0/2455	0.55 (1)		
W-A	-1959/0	0.0 0.0	0.23 (1)	6.01	I-O	-108/4	0.05 (1)		
M-K	-1893/0	0.0 0.0	0.22 (1)	6.10	O-J	0/972	0.22 (1)		
					N-J	-1482/0	0.40 (1)		
X-W	0/0	-96.5 -96.5	0.16 (1)	10.00	A-V	0/1113	0.25 (1)		
W-V	0/0	-18.5 -18.5	0.18 (4)	10.00	N-K	0/1100	0.25 (1)		
V-U	0/1004	-18.5 -18.5	0.40 (4)	10.00					
U-T	0/2775	-18.5 -18.5	0.58 (1)	10.00					
T-S	0/2775	-18.5 -18.5	0.58 (1)	10.00					
S-R	0/4594	-18.5 -18.5	0.54 (1)	10.00					
R-Q	0/3898	-18.5 -18.5	0.82 (1)	10.00					
Q-P	0/1143	-18.5 -18.5	0.48 (4)	10.00					
P-O	0/1143	-18.5 -18.5	0.48 (4)	10.00					
O-N	0/1076	-18.5 -18.5	0.18 (1)	10.00					
N-M	0/0	-18.5 -18.5	0.16 (1)	10.00					
M-L	0/0	-96.5 -96.5	0.16 (1)	10.00					

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	21.0 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	0.0 PSF
	DL	=	7.4 PSF
TOTAL LOAD	=	34.4 PSF	
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF OBC 2012, OBC 2018			
- CSA 086-09, CSA 086-14			
- TPIC 2011, TPIC 2014			
DESIGN ASSUMPTIONS			
-OVERHANG NOT TO BE ALTERED OR CUT OFF.			
(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (1.36")			
CALCULATED VERT. DEFL.(LL)= L/999 (0.29")			
ALLOWABLE DEFL.(TL)= L/360 (1.36")			
CALCULATED VERT. DEFL.(TL)= L/820 (0.60")			
CANTILEVER DEFLECTION:			
ALLOWABLE DEFL.(LL)= L/120 (0.19")			
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")			
ALLOWABLE DEFL.(TL)= L/120 (0.19")			
CALCULATED VERT. DEFL.(TL)= L/906 (0.02")			
CSI: TC=0.91/1.00 (C-E:1), BC=0.82/1.00 (Q-R:1), WB=0.93/1.00 (E-S:1), SI=0.31/1.00 (B-C:1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1656
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.90 (A) (INPUT = 0.90)			
JSI METAL= 0.78 (T) (INPUT = 1.00)			

A-18023715 CONTINUED ON PAGE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	H2T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

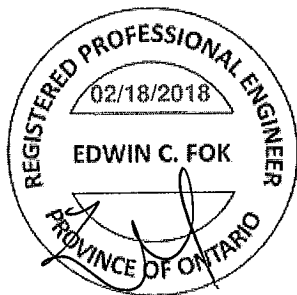
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PLATES (table is in inches)

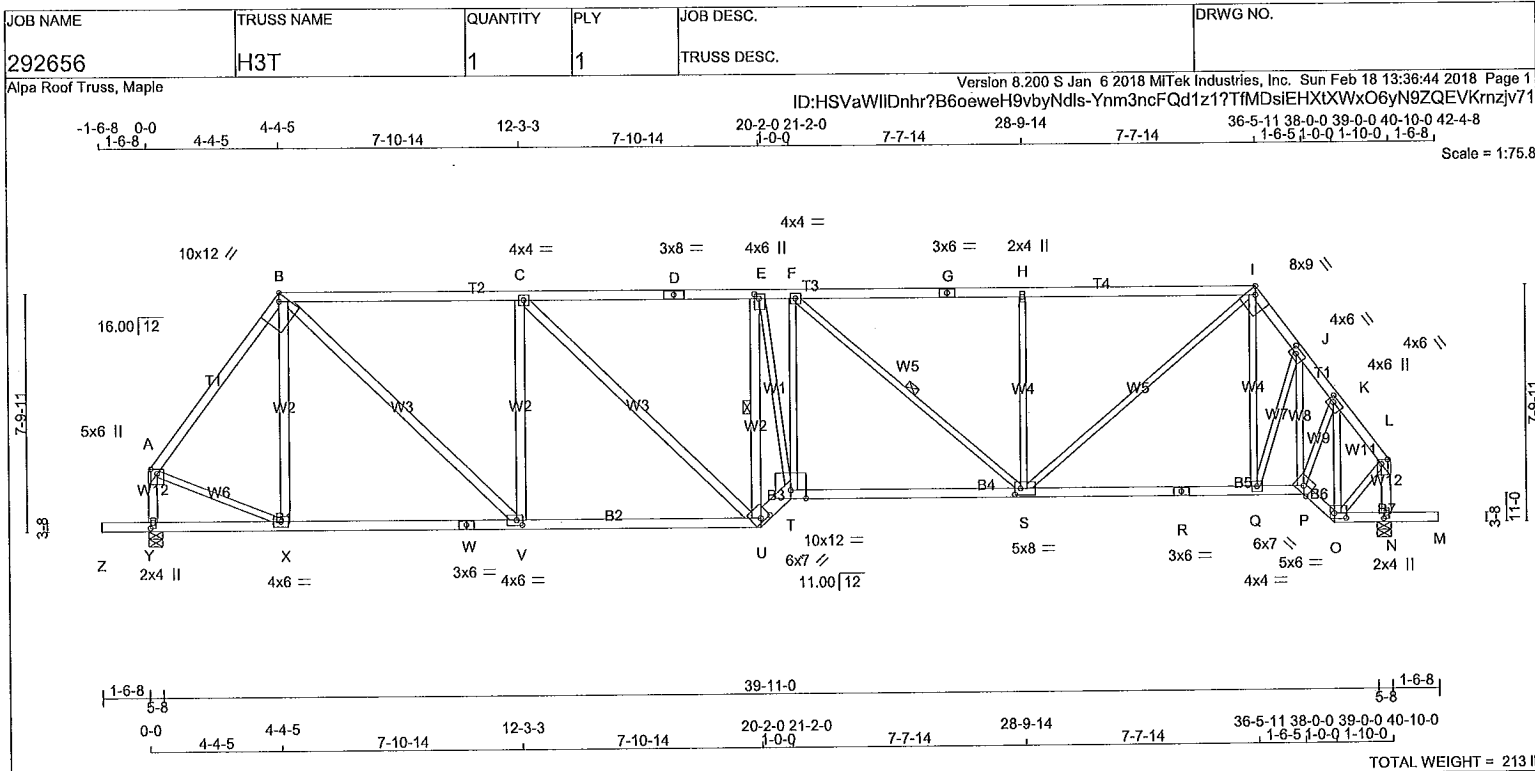
JT	TYPE	PLATES	W	LEN	Y	X
S	BBWW-h	MT20	6.0	9.0	1.50	5.50
T	BS-t	MT20	3.0	6.0		
U	BMWW-t	MT20	4.0	6.0	2.00	1.75
V	BMWW-t	MT20	4.0	4.0	2.00	1.50
W	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

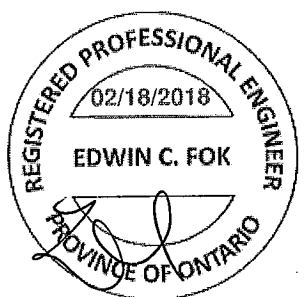
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023715(2)



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 21.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 34.4 PSF			
A - B 2x4 DRY No.2 SPF B - D 2x4 DRY 1650F 1.5E SPF D - G 2x4 DRY 1650F 1.5E SPF G - I 2x4 DRY No.2 SPF I - L 2x4 DRY No.2 SPF Y - A 2x4 DRY No.2 SPF N - L 2x4 DRY No.2 SPF Z - W 2x4 DRY No.2 SPF W - U 2x4 DRY No.2 SPF U - T 2x6 DRY No.2 SPF T - R 2x4 DRY No.2 SPF R - P 2x4 DRY No.2 SPF P - O 2x4 DRY No.2 SPF O - M 2x4 DRY No.2 SPF				SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, OBC 2018 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS -OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL)= L/380 (1.36") CALCULATED VERT. DEFL.(LL)= L/999 (0.21") ALLOWABLE DEFL.(TL)= L/360 (1.36") CALCULATED VERT. DEFL.(TL)= L/999 (0.46") CANTILEVER DEFLECTION: ALLOWABLE DEFL.(LL)= L/120 (0.19") CALCULATED VERT. DEFL.(LL)= L/999 (0.01") ALLOWABLE DEFL.(TL)= L/120 (0.19") CALCULATED VERT. DEFL.(TL)= L/999 (0.01") CSI: TC=0.92/1.00 (C-E:1), BC=0.64/1.00 (S-T:1), WB=0.88/1.00 (C-V:1), SSI=0.29/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (V) (INPUT = 0.90) JSI METAL= 0.67 (U) (INPUT = 1.00)			
ALL WEBS EXCEPT X - B 2x4 DRY No.2 SPF B - V 2x4 DRY No.2 SPF V - C 2x4 DRY No.2 SPF C - U 2x4 DRY No.2 SPF U - E 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD LC1 (PLF) MAX. FACTORED CS1 (LC) MAX. UNBRACED LENGTH (FR-TO) A-B -1743 / 0 -78.0 -78.0 0.38 (1) 4.72 X-B -249 / 0 0.20 (1) B-C -2329 / 0 -78.0 -78.0 0.84 (1) 4.06 B-V 0 / 1780 0.29 (1) C-D -2738 / 0 -78.0 -78.0 0.92 (1) 3.77 V-C -1080 / 0 0.88 (1) D-E -2738 / 0 -78.0 -78.0 0.92 (1) 3.77 C-U 0 / 560 0.09 (1) E-F -3102 / 0 -78.0 -78.0 0.42 (1) 4.01 U-E -2832 / 0 0.78 (1) F-G -2610 / 0 -78.0 -78.0 0.82 (1) 3.93 E-T 0 / 2388 0.54 (1) G-H -2610 / 0 -78.0 -78.0 0.82 (1) 3.93 T-F 0 / 1274 0.06 (1) H-I -2610 / 0 -78.0 -78.0 0.83 (1) 3.93 F-S -648 / 0 0.47 (1) I-J -1971 / 0 -78.0 -78.0 0.07 (1) 4.76 S-H -684 / 0 0.55 (1) J-K -1916 / 0 -78.0 -78.0 0.06 (1) 4.83 S-I 0 / 1899 0.43 (1) K-L -1386 / 0 -78.0 -78.0 0.06 (1) 5.49 Q-I 0 / 81 0.03 (4) Y-A -1940 / 0 0.0 0.0 0.22 (1) 6.04 Q-J 0 / 60 0.02 (4) N-L -1893 / 0 0.0 0.0 0.22 (1) 6.10 P-J -211 / 0 0.08 (1) Z-Y 0 / 0 -96.5 -96.5 0.16 (1) 10.00 P-K 0 / 1000 0.23 (1) Y-X 0 / 0 -18.5 -18.5 0.15 (4) 10.00 O-K -1483 / 0 0.40 (1) X-W 0 / 1036 -18.5 -18.5 0.39 (4) 10.00 A-X 0 / 1105 0.25 (1) W-V 0 / 1036 -18.5 -18.5 0.39 (4) 10.00 O-L 0 / 1100 0.25 (1) V-U 0 / 2329 -18.5 -18.5 0.53 (1) 10.00 U-T 0 / 3758 -18.5 -18.5 0.45 (1) 10.00 T-S 0 / 3098 -18.5 -18.5 0.64 (1) 10.00 S-R 0 / 1167 -18.5 -18.5 0.38 (4) 10.00 R-Q 0 / 1167 -18.5 -18.5 0.38 (4) 10.00 Q-P 0 / 1147 -18.5 -18.5 0.28 (1) 10.00 P-O 0 / 1077 -18.5 -18.5 0.18 (1) 10.00 O-N 0 / 0 -18.5 -18.5 0.16 (1) 10.00 N-M 0 / 0 -96.5 -96.5 0.16 (1) 10.00			
PLATES (table is in inches) JT TYPE PLATES W LEN Y X A TMVW+p MT20 5.0 6.0 1.75 2.50 B TTWW-h MT20 10.0 12.0 Edge 2.75 C TMVW-l MT20 4.0 4.0 D TS-l MT20 3.0 8.0 E TMVW-h MT20 4.0 6.0 1.75 1.75 F TMVW-l MT20 4.0 4.0 G TS-l MT20 3.0 6.0 H TMVW-w MT20 2.0 4.0 I TTWW-h MT20 8.0 9.0 Edge 2.75 J TMVW-l MT20 4.0 6.0 2.00 2.50 K TMVW-l MT20 4.0 6.0 1.75 2.00 L TMVW+p MT20 4.0 6.0 1.75 2.50 N BMV1+p MT20 2.0 4.0 2.25 1.00 O BBWW-l MT20 5.0 6.0 2.00 4.50				NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design			



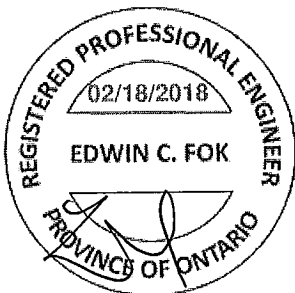
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	H3T	1	1	TRUSS DESC.	
Alpa Roof Truss, Maple					
Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sun Feb 18 13:36:45 2018 Page 2					
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PLATES (table is in inches)

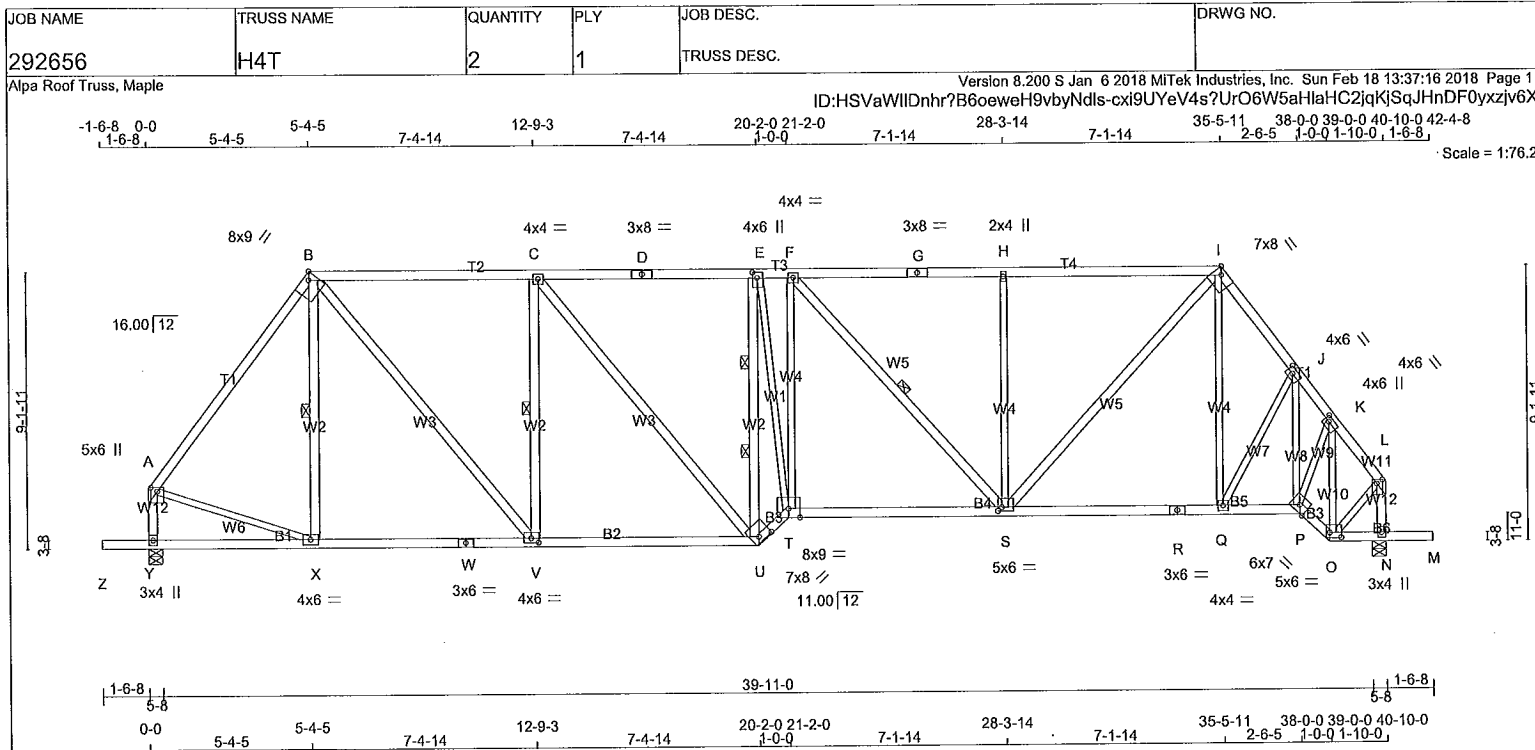
JT	TYPE	PLATES	W	LEN	Y	X
P	BBWW-h	MT20	6.0	7.0	Edge	
Q	BMWW-I	MT20	4.0	4.0		
R	BS-I	MT20	3.0	6.0		
S	BMWWW-I	MT20	5.0	8.0	2.50	2.25
T	BBWW-I	MT20	10.0	12.0	3.25	6.00
U	BBWW-h	MT20	6.0	7.0	1.50	3.50
V	BMWW-I	MT20	4.0	6.0	2.00	2.25
W	BS-I	MT20	3.0	6.0		
X	BMWW-I	MT20	4.0	6.0		
Y	BMV1+p	MT20	2.0	4.0	2.25	1.00

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design



A-18023716(2)

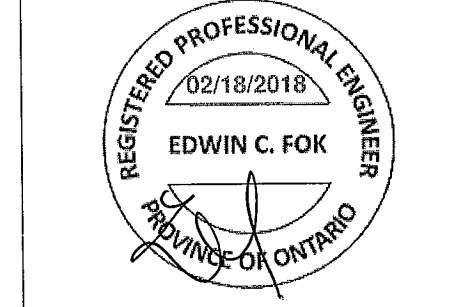


LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
A - B	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
Y - A	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Z - W	2x4	DRY	No.2	SPF
W - U	2x4	DRY	No.2	SPF
U - T	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT				SPF
X - B	2x4	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
V - C	2x4	DRY	No.2	SPF
C - U	2x4	DRY	No.2	SPF
U - E	2x4	DRY	No.2	SPF
F - S	2x4	DRY	No.2	SPF
S - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW+p	MT20	5.0	6.0 1.50 2.50
B	TTWW-h	MT20	8.0	9.0 Edge 2.75
C	CTWW-h	MT20	4.0	4.0
D	TS-t	MT20	3.0	8.0
E	CTWW-h	MT20	4.0	6.0 2.25 1.75
F	CTWW-h	MT20	4.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMW+w	MT20	2.0	4.0
I	TTWW-h	MT20	7.0	8.0 Edge 2.75
J	TMVW-t	MT20	4.0	6.0 2.00 2.50
K	CTWW-h	MT20	4.0	6.0 1.75 2.00
L	TMVW+p	MT20	4.0	6.0 1.50 2.25

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
Y	2228	0	2228	0	0	5-8	3-9	5-8	3-9
N	2234	0	2234	0	0	5-8	3-9	5-8	3-9

UNFACTORED REACTIONS		1ST LCASE		MAX./MIN. COMPONENT REACTIONS		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	DEAD	SOIL
Y	1599	921 / 0	0 / 0	0 / 0	0 / 0	678 / 0	0 / 0	678 / 0	0 / 0
N	1603	923 / 0	0 / 0	0 / 0	0 / 0	680 / 0	0 / 0	680 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y, N

BRACING
FOR SECTION B-I, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

- 1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-X, C-V, F-S.
- 2 LATERAL BRACE(S) AT 1/3 LENGTH OF E-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	LC1 (LC)	MAX. (LC)	UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. (LC)	UNBRAC LENGTH
FR-TO		FROM	TO			FR-TO			
A-B	-1877 / 0	-78.0	-78.0	0.44 (1)	4.57	X-B	-201 / 0	0.08 (1)	0.08 (1)
B-C	-2157 / 0	-85.5	-85.5	0.70 (1)	2.00	B-V	0 / 1614	0.26 (1)	0.26 (1)
C-D	-2470 / 0	-85.5	-85.5	0.71 (1)	2.00	V-C	-1089 / 0	0.42 (1)	0.42 (1)
D-E	-2470 / 0	-85.5	-85.5	0.71 (1)	2.00	C-U	0 / 487	0.08 (1)	0.08 (1)
E-F	-2739 / 0	-85.5	-85.5	0.43 (1)	2.00	U-E	-2590 / 0	0.51 (1)	0.51 (1)
F-G	-2372 / 0	-85.5	-85.5	0.65 (1)	2.00	E-T	0 / 2121	0.48 (1)	0.48 (1)
G-H	-2372 / 0	-85.5	-85.5	0.65 (1)	2.00	T-F	0 / 253	0.06 (1)	0.06 (1)
H-I	-2371 / 0	-85.5	-85.5	0.68 (1)	2.00	F-S	-548 / 0	0.33 (1)	0.33 (1)
I-J	-2083 / 0	-78.0	-78.0	0.11 (1)	4.63	S-H	-702 / 0	0.93 (1)	0.93 (1)
J-K	-2021 / 0	-78.0	-78.0	0.06 (1)	4.73	S-I	0 / 1692	0.27 (1)	0.27 (1)
K-L	-1472 / 0	-78.0	-78.0	0.06 (1)	5.36	Q-I	0 / 86	0.03 (4)	0.03 (4)
Y-A	-2041 / 0	0.0	0.0	0.23 (1)	5.91	Q-J	0 / 26	0.01 (4)	0.01 (4)
N-L	-2006 / 0	0.0	0.0	0.23 (1)	5.96	P-J	-224 / 0	0.08 (1)	0.08 (1)
						P-K	0 / 1089	0.25 (1)	0.25 (1)
Z-Y	0 / 0	-96.5	-96.5	0.16 (1)	10.00	O-K	-1569 / 0	0.42 (1)	0.42 (1)
Y-X	0 / 0	-18.5	-18.5	0.16 (4)	10.00	A-X	0 / 1168	0.26 (1)	0.26 (1)
X-W	0 / 1118	-18.5	-18.5	0.37 (4)	10.00	O-L	0 / 1166	0.26 (1)	0.26 (1)
W-V	0 / 1118	-18.5	-18.5	0.37 (4)	10.00				
V-U	0 / 2157	-18.5	-18.5	0.48 (1)	10.00				
U-T	0 / 3387	-18.5	-18.5	0.55 (1)	10.00				
T-S	0 / 2736	-18.5	-18.5	0.57 (1)	10.00				
S-R	0 / 1235	-18.5	-18.5	0.37 (4)	10.00				
R-Q	0 / 1235	-18.5	-18.5	0.37 (4)	10.00				
Q-P	0 / 1226	-18.5	-18.5	0.27 (1)	10.00				
P-O	0 / 1141	-18.5	-18.5	0.19 (1)	10.00				
O-N	0 / 0	-18.5	-18.5	0.12 (1)	10.00				
N-M	0 / 0	-96.5	-96.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")
ALLOWABLE VERT. DEFL.(LL)= L/999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.36")
CALCULATED VERT. DEFL.(TL)= L/999 (0.39")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.71/1.00 (C-E:1), BC=0.57/1.00 (S-T:1), WB=0.93/1.00 (H-S:1), SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	H4T	2	1	TRUSS DESC.	
Alpa Roof Truss, Maple					
Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 13:37:16 2018 Page 2					
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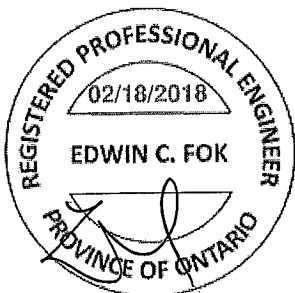
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
N	BMV1+p	MT20	3.0	4.0		
O	BBWW-l	MT20	5.0	6.0	2.00	4.50
P	BBWW-h	MT20	6.0	7.0	Edge	
Q	BMWW-l	MT20	4.0	4.0		
R	BS-l	MT20	3.0	6.0		
S	BMWWW-l	MT20	5.0	6.0	1.50	1.50
T	BBWW-l	MT20	8.0	9.0	Edge	
U	BBWW-h	MT20	7.0	8.0	2.00	5.00
V	BMWW-l	MT20	4.0	6.0	1.75	3.00
W	BS-l	MT20	3.0	6.0		
X	BMWW-l	MT20	4.0	6.0		
Y	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

JSI GRIP= 0.90 (P) (INPUT = 0.90)
JSI METAL= 0.62 (A) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4
for Part 9 design as per OBC 9.23.13.11,
and no less than 2x4 for Part 4 design

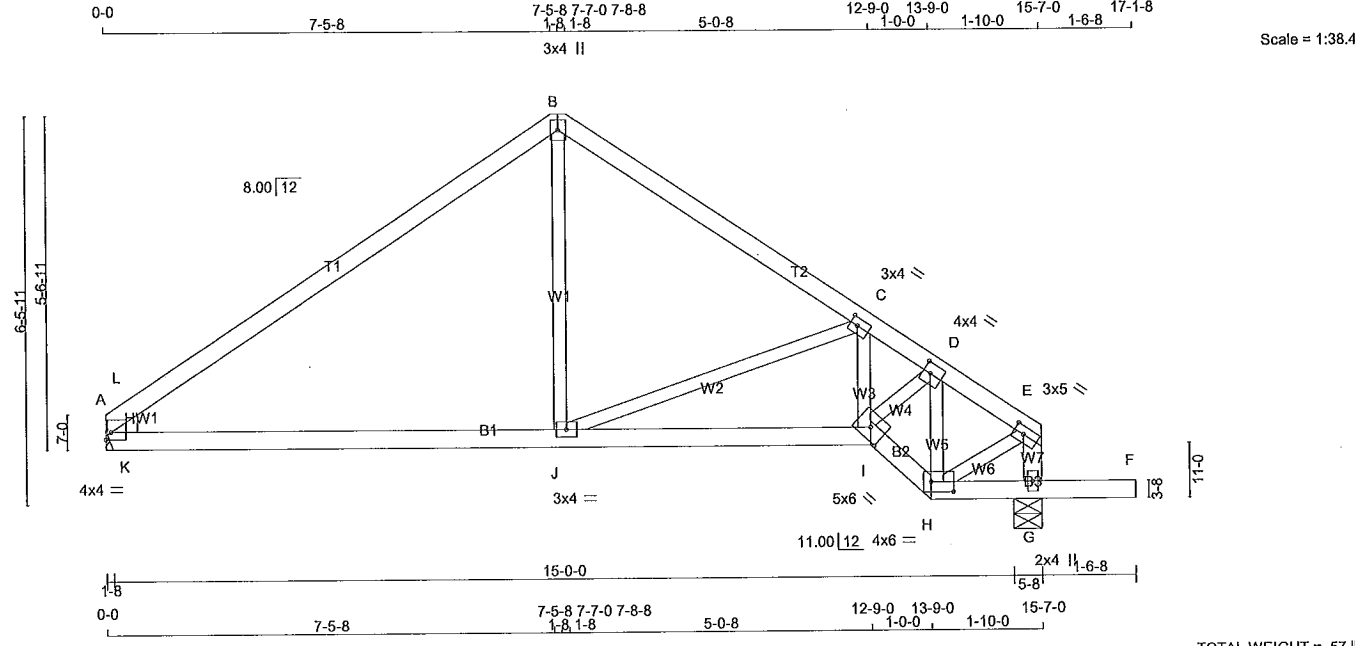


A-18023717(2)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	H12T	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIDnhr?B6oewehH9vbyNdls-80gCr0qXJn0DmrKa1xZVDftqsHpsCEhdSI7sX?zjv6H



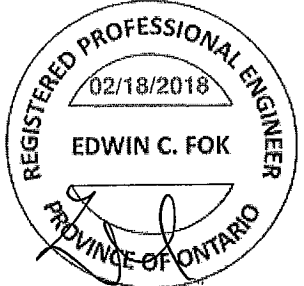
TOTAL WEIGHT = 57 lb
(M)(F)

LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT G - E	2x3	DRY	No.2	SPF
	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMBH1-I	MT20	4.0	4.0 1.50 Edge
B	TTW+p	MT20	3.0	4.0
C	TMWW-I	MT20	3.0	4.0 1.50 1.50
D	TMWW-I	MT20	4.0	4.0 2.00 1.50
E	TMWW-I	MT20	3.0	5.0 1.50 2.00
G	BMW1+w	MT20	2.0	4.0
H	BBWW-I	MT20	4.0	6.0 2.00 4.50
I	BBWW-h	MT20	5.0	6.0 2.25 3.00
J	BMWW-I	MT20	3.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG	HEEL WEDGE
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	
A	736	0	736	0	0	2x4 L
G	916	0	916	0	0	

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	525	320 / 0	0 / 0	0 / 0	0 / 0	204 / 0	0 / 0
G	653	399 / 0	0 / 0	0 / 0	0 / 0	255 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-L	-428 / 0	J-C	-393 / 0
L-B	-692 / 0	I-C	-88 / 19
B-C	-694 / 0	I-D	0 / 709
C-D	-1004 / 0	H-D	-710 / 0
D-E	-565 / 0	J-B	0 / 341
		G-E	-657 / 0
A-K	0 / 560	H-E	0 / 530
K-J	0 / 560	K-L	-587 / 15
J-I	0 / 927		
I-H	0 / 573		
H-G	-1 / 0		
G-F	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)=	L/360 (0.51")
CALCULATED VERT. DEFL.(LL)=	L/999 (0.10")
ALLOWABLE DEFL.(TL)=	L/360 (0.51")
CALCULATED VERT. DEFL.(TL)=	L/876 (0.21")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/651 (0.03")

CSI: TC=0.57/1.00 (B-L:1), BC=0.41/1.00 (J-K:1), WB=0.20/1.00 (C-J:1), SSI=0.47/1.00 (A-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	
PLATE GRIP(DRY)	SHEAR SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

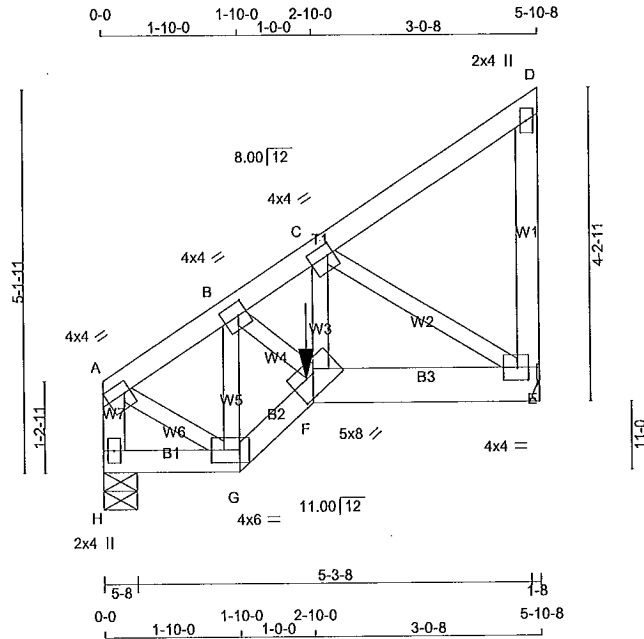
JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.23 (I) (INPUT = 1.00)

A-18023719

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J1A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

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Scale = 1:31.0

TOTAL WEIGHT = 2 X 30 = 60 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4 DRY No.2	SPF			
A - D	2x4 DRY No.2	SPF			
E - D	2x4 DRY No.2	SPF			
H - G	2x4 DRY No.2	SPF			
G - F	2x4 DRY No.2	SPF			
F - E	2x6 DRY No.2	SPF			

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
H-A 1	12	TOP
A-D 1	12	TOP
D-E 1	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
H-G 1	12	TOP
G-F 1	12	SIDE(61.0)
F-E 2	12	SIDE(163.8)
WEBS : (0.122"x3") SPIRAL NAILS		
F-C 1	2	SIDE(236.5)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	FACTORED	DOWN	FACTORED	DOWN	BRG	IN-SX	BRG	IN-SX
H	1336	0	1336	0	0	5-8	1-8		
E	1762	0	1762	0	0	HANGER BY OTHERS			

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN.	COMPONENT REACTIONS	WIND	DEAD	SOIL
H	952	581 / 0	0 / 0	0 / 0	371 / 0	0 / 0
E	1256	767 / 0	0 / 0	0 / 0	489 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	LC1	MAX	CS1 (LC)	UNBRAC	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO								FR-TO			
H-A	-1319 / 0	0.0	0.0	0.07 (1)	7.81	A-G	0 / 1181	0.15 (1)			
A-B	-1298 / 0	-78.0	-78.0	0.05 (1)	6.25	G-B	-1435 / 0	0.12 (1)			
B-C	-2233 / 0	-78.0	-78.0	0.06 (1)	5.92	B-F	0 / 1258	0.16 (1)			
C-D	-7 / 0	-78.0	-78.0	0.05 (1)	10.00	F-C	0 / 2139	0.26 (1)			
E-D	-101 / 0	0.0	0.0	0.01 (1)	7.81	C-E	-2102 / 0	0.23 (1)			
H-G	0 / 0	-18.5	-18.5	0.01 (4)	10.00						
G-F	0 / 1373	-18.5	-18.5	0.12 (1)	10.00						
F-E	0 / 1776	-346.1	-346.1	0.26 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
F	2-10-0	-1534	-1534		FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder
START DISTANCE = 3-1-4
START SPAN CARRIED = 15-7-0
END DISTANCE = 5-10-8
END SPAN CARRIED = 15-7-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.07/1.00 (A-H:1), BC=0.26/1.00 (E-F:1), WB=0.26/1.00 (C-F:1), SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

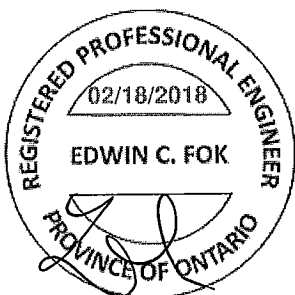
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

A-18023720

CONTINUED ON PAGE 2

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J1A	1	2	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 13:29:08 2018 Page 2
ID:HSVawIIdnhr?B6oeweH9vbyNdis-v2Dg3hkO0wXklOojV5WJoN?pt6qK6HsRZl2BrDzivE9

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	4.0	1.75	Edge
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMWW-t	MT20	4.0	4.0	1.75	1.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1-t	MT20	4.0	4.0	2.00	1.75
F	BBWW-h	MT20	5.0	8.0	3.00	4.00
G	BBWW-l	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

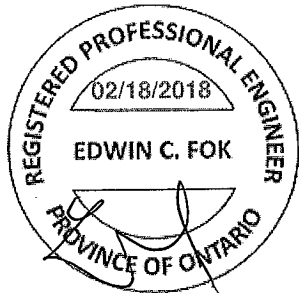
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

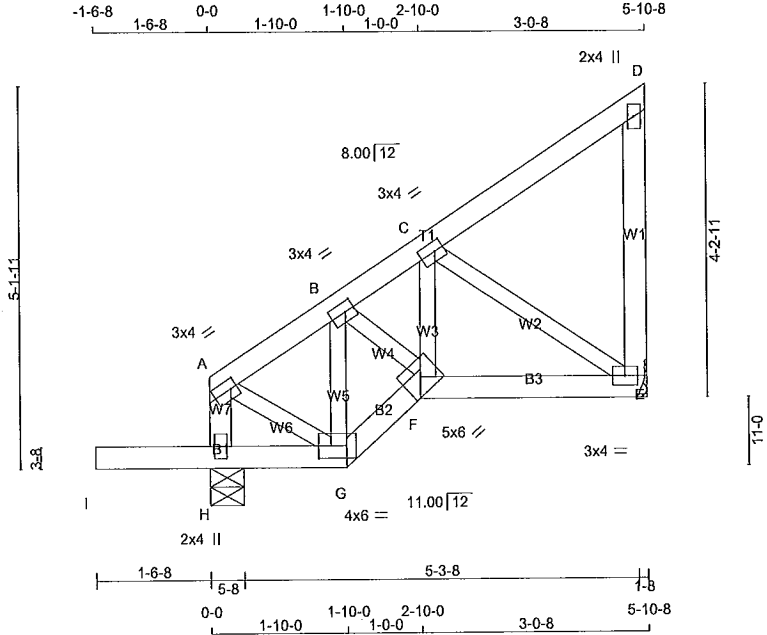
- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1634.4 lbs FACTORED DOWN AT 2-10-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023720(2)



TOTAL WEIGHT = 3 X 30 = 90 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BBVW-t	MT20	5.0	6.0		
G	BBVW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
H	452	0	452	0	5-8	1-8
E	264	0	264	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
		FROM	TO						
H-A	-223 / 0	0.0	0.0	0.02 (1)	7.81	A- G	0 / 154	0.03 (1)	
A- B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25	G- B	-220 / 0	0.03 (1)	
B- C	-239 / 0	-78.0	-78.0	0.08 (1)	6.25	B- F	0 / 130	0.03 (1)	
C- D	-11 / 0	-78.0	-78.0	0.08 (1)	6.25	F- C	0 / 63	0.02 (4)	
E- D	-95 / 0	0.0	0.0	0.03 (1)	7.81	C- E	-259 / 0	0.06 (1)	
I- H	0 / 0	-96.5	-96.5	0.16 (1)	10.00				
H- G	0 / 0	-18.5	-18.5	0.16 (1)	10.00				
G- F	0 / 173	-18.5	-18.5	0.03 (1)	10.00				
F- E	0 / 216	-18.5	-18.5	0.07 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 -OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/588 (0.03")

CSI: TC=0.08/1.00 (B-C:1), BC=0.16/1.00 (G-H:1)
 , WB=0.06/1.00 (C-E:1), SSI=0.12/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

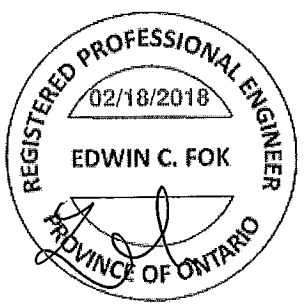
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (A) (INPUT = 0.90)
 JSI METAL= 0.08 (E) (INPUT = 1.00)

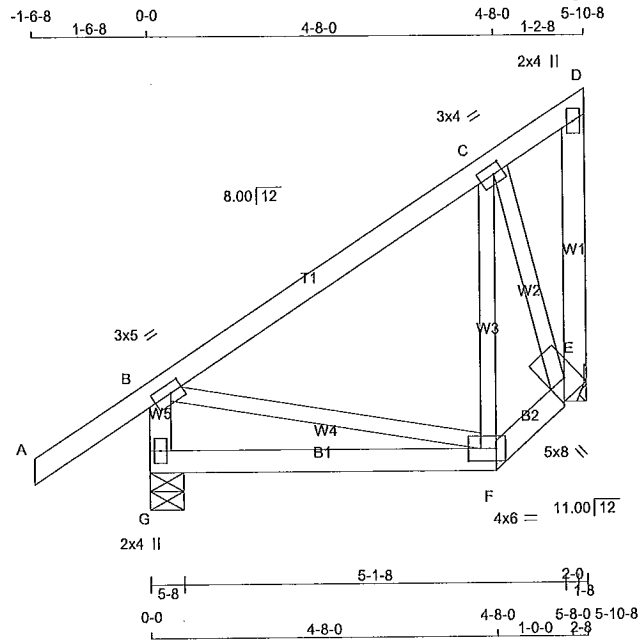
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J2T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWlIDnhr?B6oewelH9vbyNdls-GnUY8GCLF?ozMSdLnE5XR_62uN7VEsJ2MNOSZGzjv5p



TOTAL WEIGHT = 2 X 32 = 63 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
G - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	5.0	1.50	2.00
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW1+I	MT20	5.0	8.0	2.75	2.75
F	BMVW-I	MT20	4.0	6.0	2.00	4.50
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	410	0	410	0	5-8	1-8
E	283	0	283	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	290	191 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
E	202	123 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. FACTORED CS (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
FR-TO							FR-TO		
G-B	-367 / 0	0.0	0.0	0.04 (1)	7.81		B-F	0 / 115	0.03 (1)
A-B	0 / 35	-78.0	-78.0	0.15 (1)	10.00		F-C	-71 / 20	0.02 (1)
B-C	-117 / 0	-78.0	-78.0	0.22 (1)	6.25		C-E	-261 / 0	0.06 (1)
C-D	-86 / 0	-78.0	-78.0	0.21 (1)	6.25				
E-D	0 / 68	0.0	0.0	0.01 (1)	10.00				
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00				
F-E	0 / 153	-18.5	-18.5	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), BC=0.12/1.00 (F-G:4), WB=0.06/1.00 (C-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 L.S BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

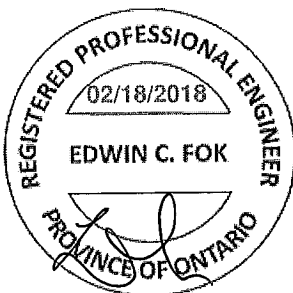
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

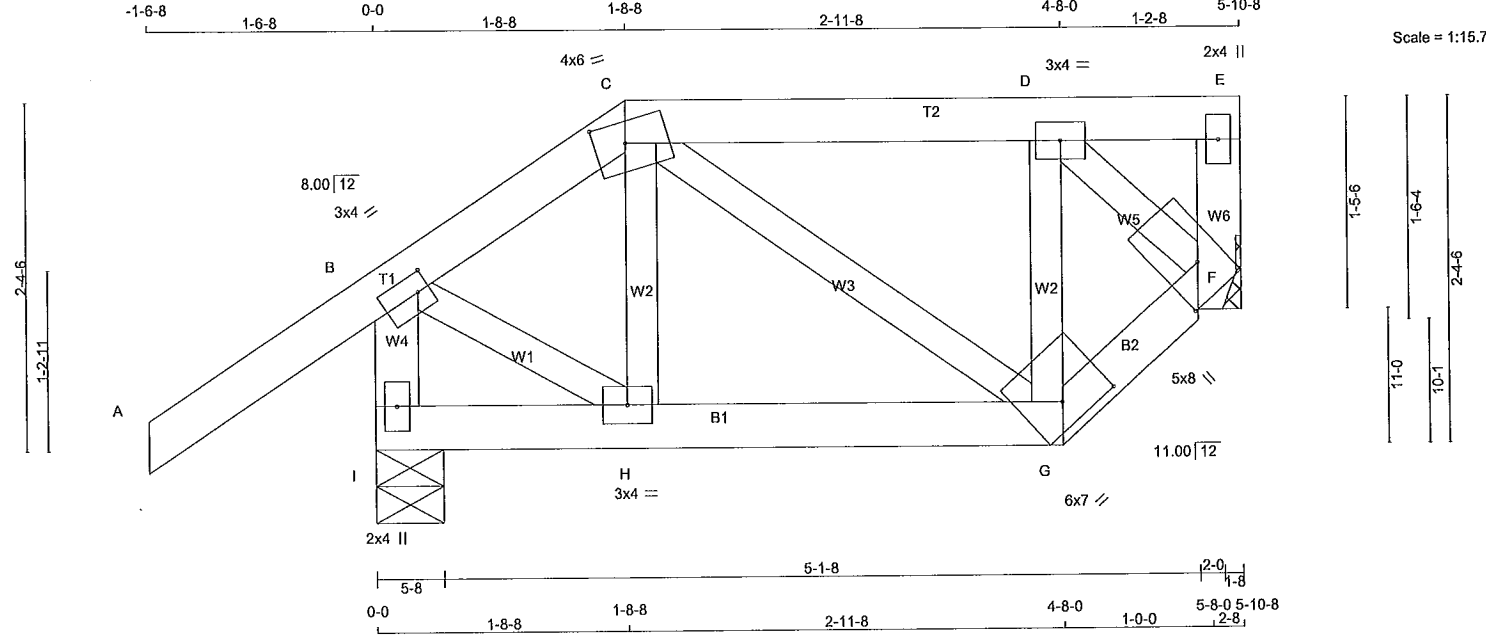
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023722



TOTAL WEIGHT = 26 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	3.0	4.0	1.50	1.00
C	TTVW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW-t	MT20	3.0	4.0		
E	TMV+p	MT20	2.0	4.0		
F	BMVW1-H	MT20	5.0	8.0	2.75	2.75
G	BMVW-h	MT20	6.0	7.0	2.00	4.00
H	BMVW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
I	427	0	427	0	5-8	1-8
F	266	0	266	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
I	302	201 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
F	190	114 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 35	-78.0 -78.0	0.15 (1)	10.00	H-C	-18 / 28	0.01 (4)
B-C	-202 / 0	-78.0 -78.0	0.14 (1)	6.25	C-G	-11 / 22	0.00 (1)
C-D	-156 / 0	-78.0 -78.0	0.07 (1)	6.25	G-D	-118 / 0	0.02 (1)
D-E	0 / 0	-78.0 -78.0	0.07 (1)	10.00	B-H	0 / 156	0.04 (1)
F-E	-10 / 0	0.0 0.0	0.00 (1)	7.81	D-F	-183 / 0	0.03 (1)
I-B	-417 / 0	0.0 0.0	0.04 (1)	7.81			
I-H	0 / 0	-18.5 -18.5	0.03 (4)	10.00			
H-G	0 / 138	-18.5 -18.5	0.05 (4)	10.00			
G-F	0 / 206	-18.5 -18.5	0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.05/1.00 (G-H:4), WB=0.04/1.00 (B-H:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

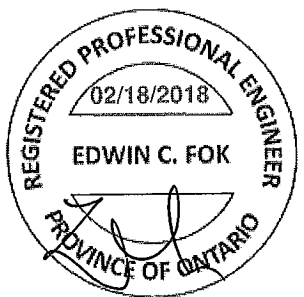
PLATE	GRIP(DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

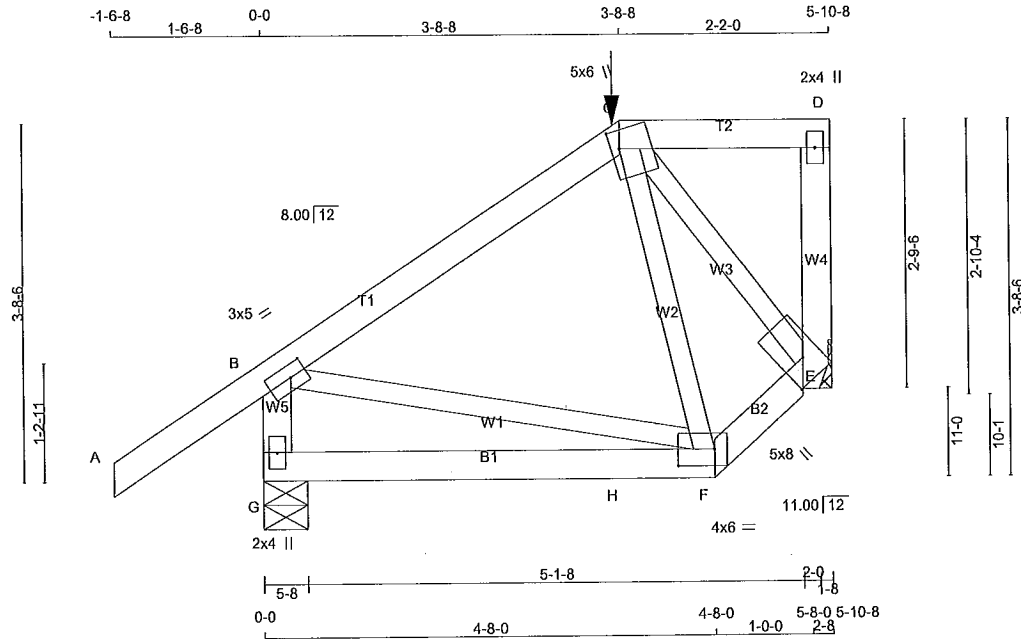
NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J2TB	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWlIDnhr?B6oeweh9vbyNds-bGYBUzo1hmMR6oZz7UfnJeNREdF5d_humPHjDUzjvGf



TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	5.0	1.50	2.00
C	TTWW+m	MT20	5.0	6.0	2.25	1.00
D	TMV+p	MT20	2.0	4.0		
E	BMVW1+H	MT20	5.0	8.0	2.75	2.75
F	BBWW-I	MT20	4.0	6.0	2.00	4.50
G	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.4 lbs FACTORED DOWN AT 3-8-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
G	486	0	0	5-8
E	433	0	0	1-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
G	345	220 / 0	0 / 0	125 / 0	0 / 0
E	308	191 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 35	-78.0 -78.0	0.16 (1)	10.00	C-F	-101 / 20	0.02 (1)
B-C	-215 / 0	-78.0 -78.0	0.20 (1)	6.25	B-F	0 / 181	0.04 (1)
C-D	0 / 0	-111.3 -111.3	0.10 (1)	10.00	C-E	-216 / 0	0.04 (1)
E-D	-121 / 0	0.0 0.0	0.02 (1)	7.81			
G-B	-424 / 0	0.0 0.0	0.05 (1)	7.81			
G-H	0 / 0	-26.4 -26.4	0.19 (4)	10.00			
H-F	0 / 0	-26.4 -26.4	0.19 (4)	10.00			
F-E	0 / 206	-26.4 -26.4	0.04 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	3-8-8	-106	-106	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 3-8-8

RIGHT SETBACK = 0-0

END SETBACK = 3-8-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")

CALCULATED VERT. DEFL.(LL) = L/999 (0.00")

ALLOWABLE DEFL.(TL) = L/360 (0.20")

CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.20/1.00 (B-C:1), BC=0.19/1.00 (F-G:4), WB=0.04/1.00 (C-E:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

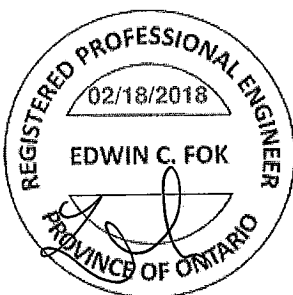
PLATE ROTATION TOL. = 5.0 Deg.

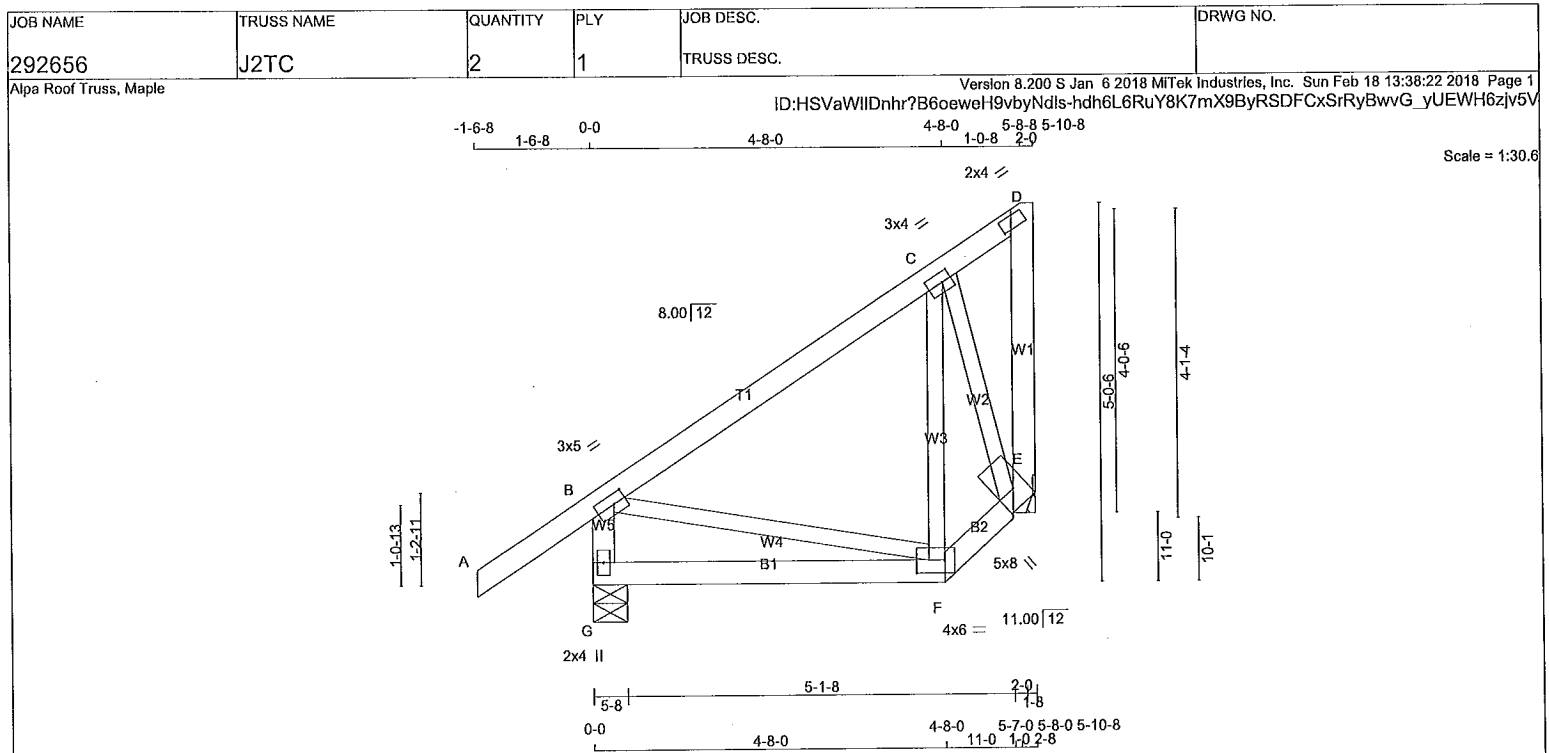
JSI GRIP= 0.39 (B) (INPUT = 0.90)

JSI METAL= 0.11 (B) (INPUT = 1.00)

A-18023724

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design





TOTAL WEIGHT = 2 X 32 = 63 lb

LUMBER					DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - D	2x4	DRY	No.2	SPF	
E - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	5.0	1.50	2.00
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TVM-I	MT20	2.0	4.0	1.00	1.75
E	BMVW1+I	MT20	5.0	8.0	2.75	2.75
F	BBVW-I	MT20	4.0	6.0	2.00	4.50
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	410	0	410	0	5-8	1-8
E	283	0	283	0	5-8	1-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	290	191 / 0	0 / 0	0 / 0	0 / 0	98 / 0
E	202	123 / 0	0 / 0	0 / 0	0 / 0	79 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-78.0	-78.0 0.15 (1)	10.00	F-C	-71 / 20	0.02 (1)
B-C	-117 / 0	-78.0	-78.0 0.22 (1)	6.25	C-E	-261 / 0	0.06 (1)
C-D	-66 / 0	-78.0	-78.0 0.21 (1)	6.25	B-F	0 / 115	0.03 (1)
E-D	0 / 68	0.0	0.0 0.01 (1)	10.00			
G-B	-367 / 0	0.0	0.0 0.04 (1)	7.81			
G-F	0 / 0	-18.5	-18.5 0.12 (4)	10.00			
F-E	0 / 153	-18.5	-18.5 0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL)= L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL)= L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), BC=0.12/1.00 (F-G:4), WB=0.06/1.00 (C-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

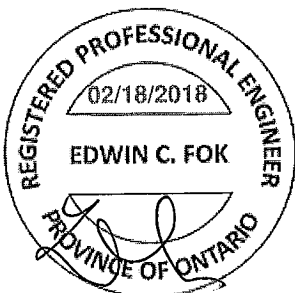
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

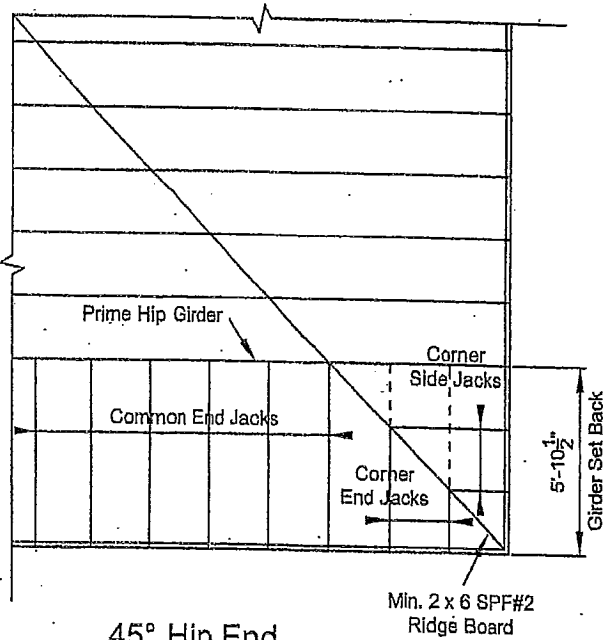
JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023725

STRACON ENGINEERING INC.

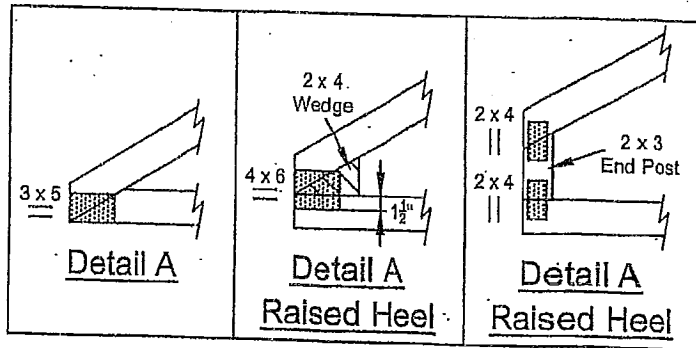
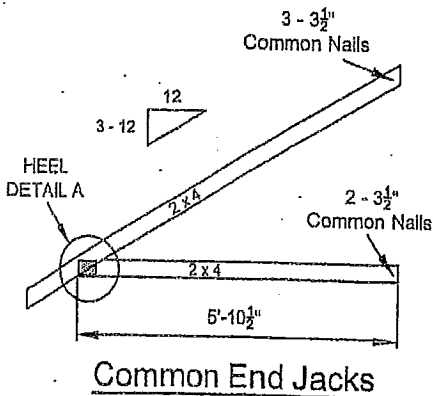
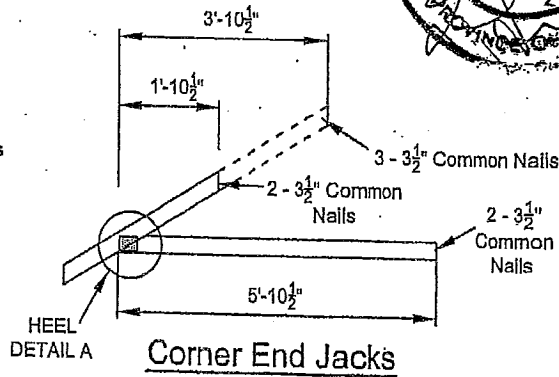
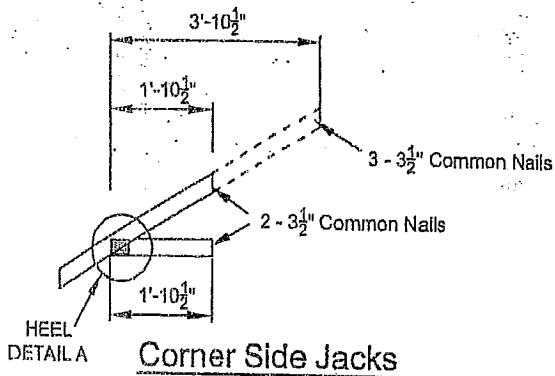


LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

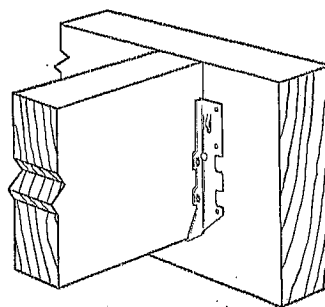
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

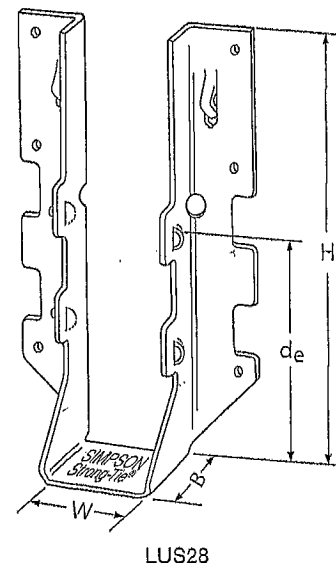
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



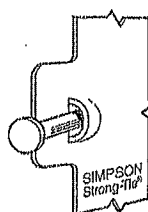
Typical LUS Installation



LUS28

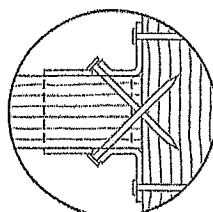
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d ₁	Face	Joist	Uplift (K _p =1.15)	Normal (K _p =1.00)	Uplift (K _p =1.15)	Normal (K _p =1.00)
LUS24	18	1½	3½	1¼	1 11/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1 11/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¼	3¾	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1¼	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7 11/16	1¼	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8 11/16	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and is not normal or available as of March 1, 2017.

This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and related warranty or see strongtie.com.

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TSPECLUS17/3/17 exp. 6/19

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strongtie.com

HHUS – Double Shear Joist Hangers

All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

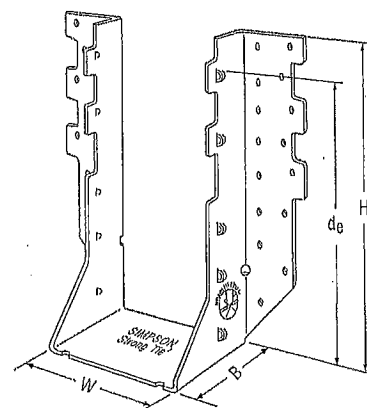
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

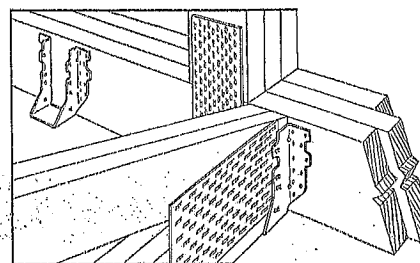
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

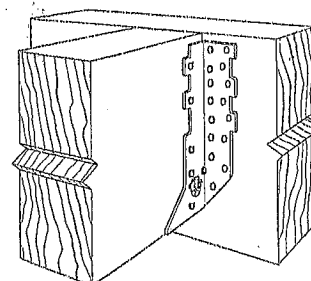
- See current catalogue for options



HHUS410



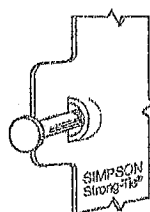
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



Typical HHUS Installation

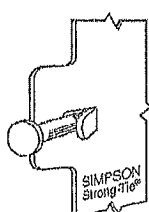
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HHUS26-2	14	3 1/16	5 13/16	3	3 15/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 1/2	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 3/2	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 11/16	9	3	7 15/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 3/8	5 13/32	3	3 15/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 3/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

1. d_e is the distance from the seat of the hanger to the highest joist nail.

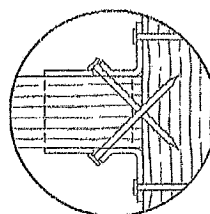


Double Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,680



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

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T-SPECHHUS17-3/17 exp. 6/19

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HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

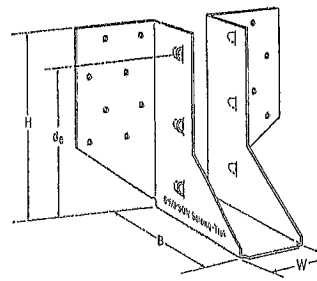
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

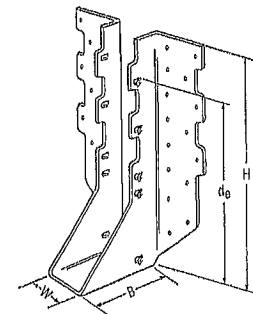
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

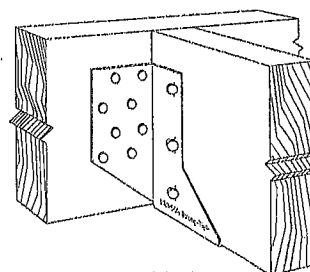
- See current catalogue for options



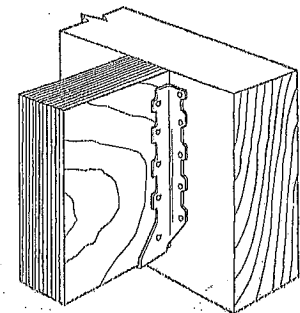
LJS26DS



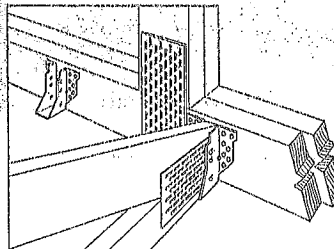
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



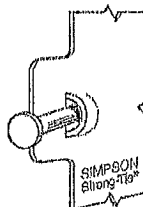
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

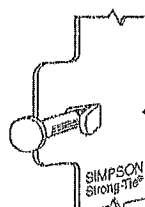
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₆ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₆ is the distance from the seat of the hanger to the highest joist nail.

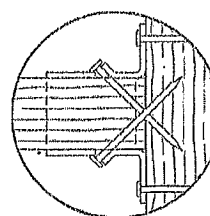


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

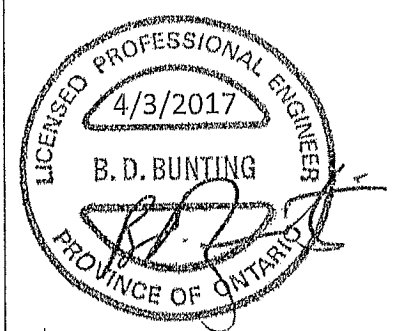
U.S. Patent
5,803,680



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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