

Connector Summary			
Pt/ID	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	53		LT251188

**FLOOR LOADING**  
LIVE LOAD : 40 PSF  
DEAD LOAD : 15 PSF  
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
O.T.B - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RIMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GLUED

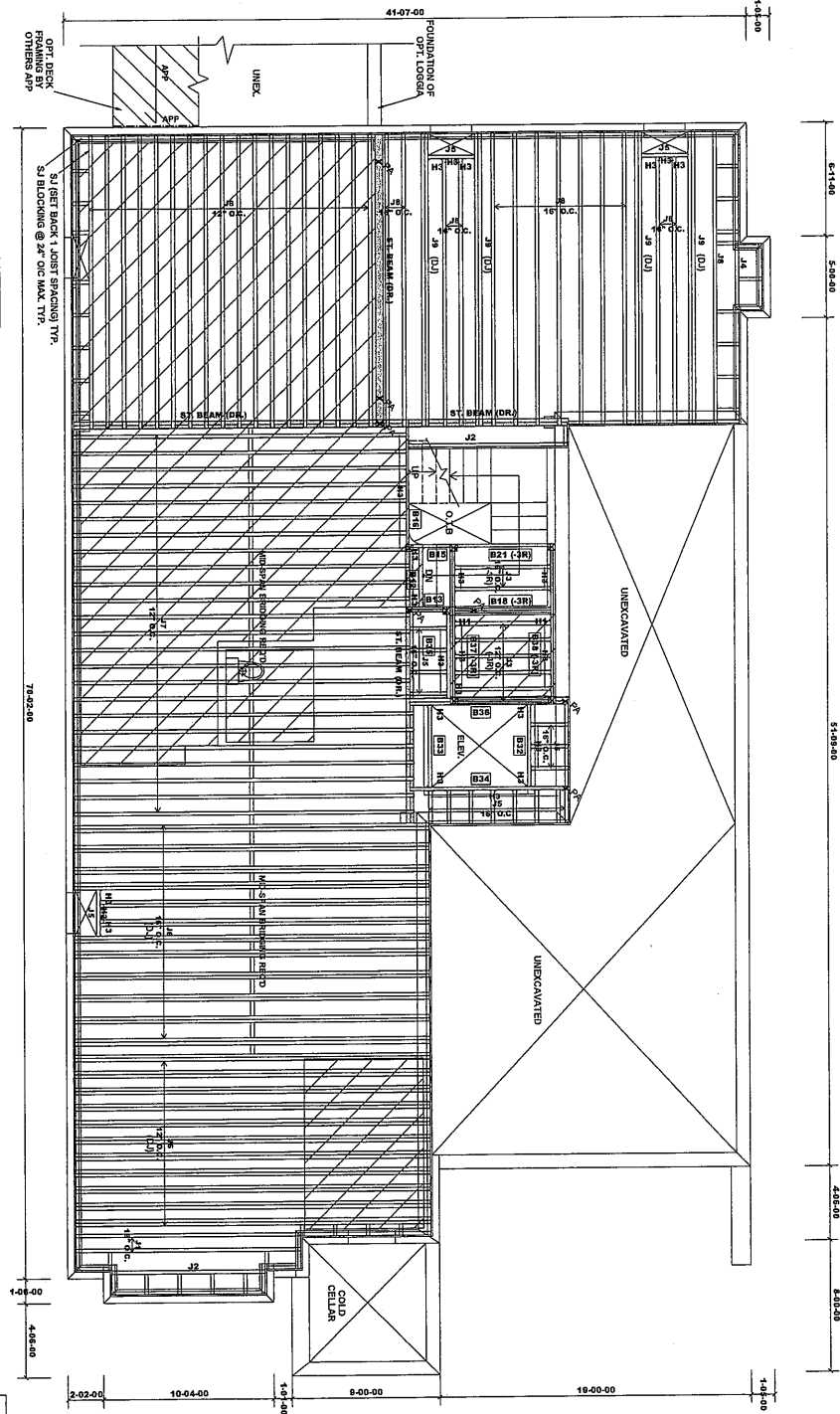
Blocking panels are required over all interior support columns.  
Squash blocks are required under concentrate loads.  
Ceramic Tile Application as per O.C. 9.30.5  
Provide I-Joist blocking between cantilevered joists (along bearing) and imboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

Salesperson: Derek F.  
Home Lumber Inc.





GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
+ W/ W.O.D. CONDITION
W/ ELEVATOR



FLOOR LOADING  
LIVE LOAD: 40 PSF  
DEAD LOAD: 15 PSF  
DEAD LOAD (TRUSSES): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
O.T.B. - OPEN TO BELOW  
GT - GIRDERS  
RT - ROOF TRUSS  
RIMBOARD  
1-1/2\"/>

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide Lateral bracing between cantilevered joists (along bearing) and inward closure at ends.  
Do not scale - refer to architectural plans for dimensions.

Connector Summary			
Field	Qty	Manuf	Product
H1	2		HGU5410
H2	2		HU312-2
H3	54		LT251188

Item	Length	Product	Qty	Unit	Qty
B12	4'-00-00	11/16" x 5/8" Timber Strand LSL	2	1	2
B13	3'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	2	2	2
B14	3'-6-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B15	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B16	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	2	2	2
B17	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B18	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B19	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B20	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B21	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B22	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B23	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B24	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B25	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B26	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B27	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B28	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B29	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B30	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B31	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B32	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B33	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B34	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B35	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B36	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B37	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B38	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B39	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B40	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B41	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B42	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B43	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B44	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B45	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B46	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B47	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B48	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B49	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B50	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B51	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B52	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B53	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B54	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B55	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B56	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B57	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B58	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B59	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B60	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B61	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B62	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B63	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B64	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B65	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B66	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B67	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B68	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B69	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B70	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B71	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B72	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B73	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B74	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B75	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B76	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B77	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B78	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B79	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B80	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B81	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B82	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B83	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B84	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B85	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B86	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B87	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B88	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B89	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B90	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B91	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B92	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B93	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B94	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B95	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B96	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B97	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B98	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B99	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2
B100	5'-00-00	1 3/4" x 11/8" x 5/8" Timber Strand LSL	1	2	2

JT/PL: 45147/116460  
LI: 34307\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 4 of 36  
Alpa Roof Trusses Inc.  
Stouffville, Ontario  
Salesperson: Derek F.  
Home Lumber Inc.



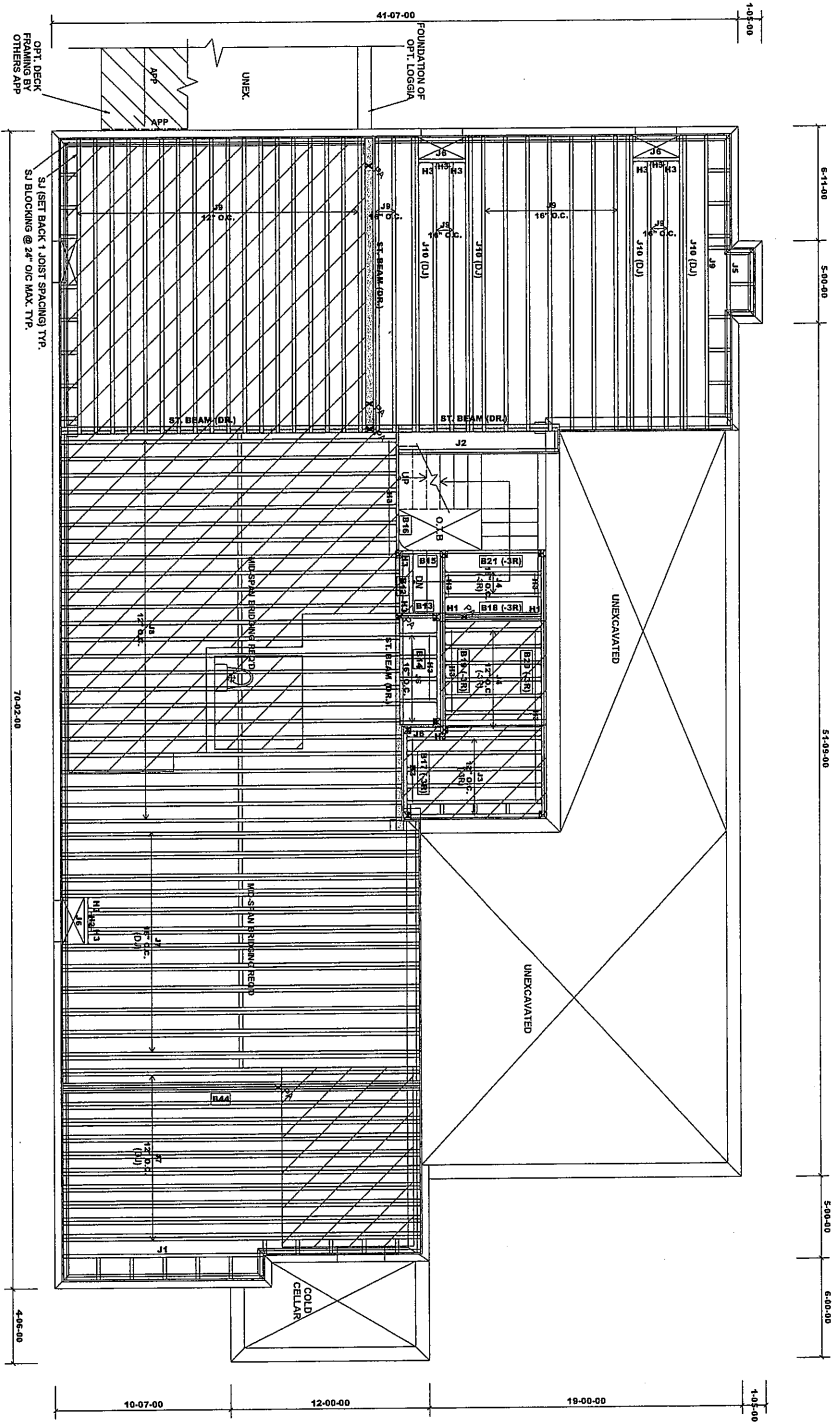
<b>GROUND FLOOR FRAMING</b>
UNIT 5013 - THE RIVERVIEW
ELEVATION C
+ W/W.O.D. CONDITION

FLOOR LOADING  
LIVE LOAD: 40 PSF  
DEAD LOAD: 15 PSF  
DEAD LOAD (TILE) 20 PSF

	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
P.O.B. - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RIMBOAR  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GLEUED

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.5  
Provide Liquid blocking between cantilevered joists (along bearing) and inboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.



ITEM	QTY	UNIT	PRODUCT	FIN	MT	QTY
B20	4.00-00	1	11/8" N-20	1	1	1
B13	3.00-00	1	13/4" x 11/8" 1.55E TimberStrand® LSL	2	2	2
B14	7.00-00	1	11/8" N-20	1	1	1
B15	1.00-00	1	11/8" N-20	1	1	1
B16	8.00-00	1	13/4" x 11/8" 1.55E TimberStrand® LSL	1	1	1
B17 (S)	6.00-00	1	11/8" N-20	1	1	1
B18 (S)	6.00-00	1	13/4" x 11/8" 1.55E TimberStrand® LSL	2	2	2
B19 (S)	6.00-00	1	11/8" N-20	1	1	1
B20 (S)	17.00-00	2	13/4" x 11/8" 1.55E TimberStrand® LSL	2	2	2
B21 (S)	7.00-00	1	11/8" N-20	1	1	1
B44	22.00-00	4	13/4" x 11/8" 1.55E TimberStrand® LSL	4	4	4
J2	10.00-00	1	11/8" N-20	1	1	1
J3	10.00-00	1	11/8" N-20	1	1	1
J4	6.00-00	1	11/8" N-20	1	1	1
J5	3.00-00	1	11/8" N-20	1	1	1
J6	3.00-00	1	11/8" N-20	1	1	1
J7	22.00-00	1	11/8" N-40x	2	44	44
J8	21.00-00	1	11/8" N-40x	2	24	24
J9	22.00-00	1	11/8" N-40x	2	24	24
J10	18.00-00	1	11/8" N-40x	2	8	8
C41	222.00-00	1	11/8" x 11/8" Rim Board	1	1	1
B41	105.00-00	1	11/8" N-20	1	1	1

FIELD	2	MANUF	HUS1310
11	2	PRODUCT	HUS1310
12	2	PRODUCT	HUS1310
13	2	PRODUCT	HUS1310

JT/PL: 45147/116460  
LI: 34307\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

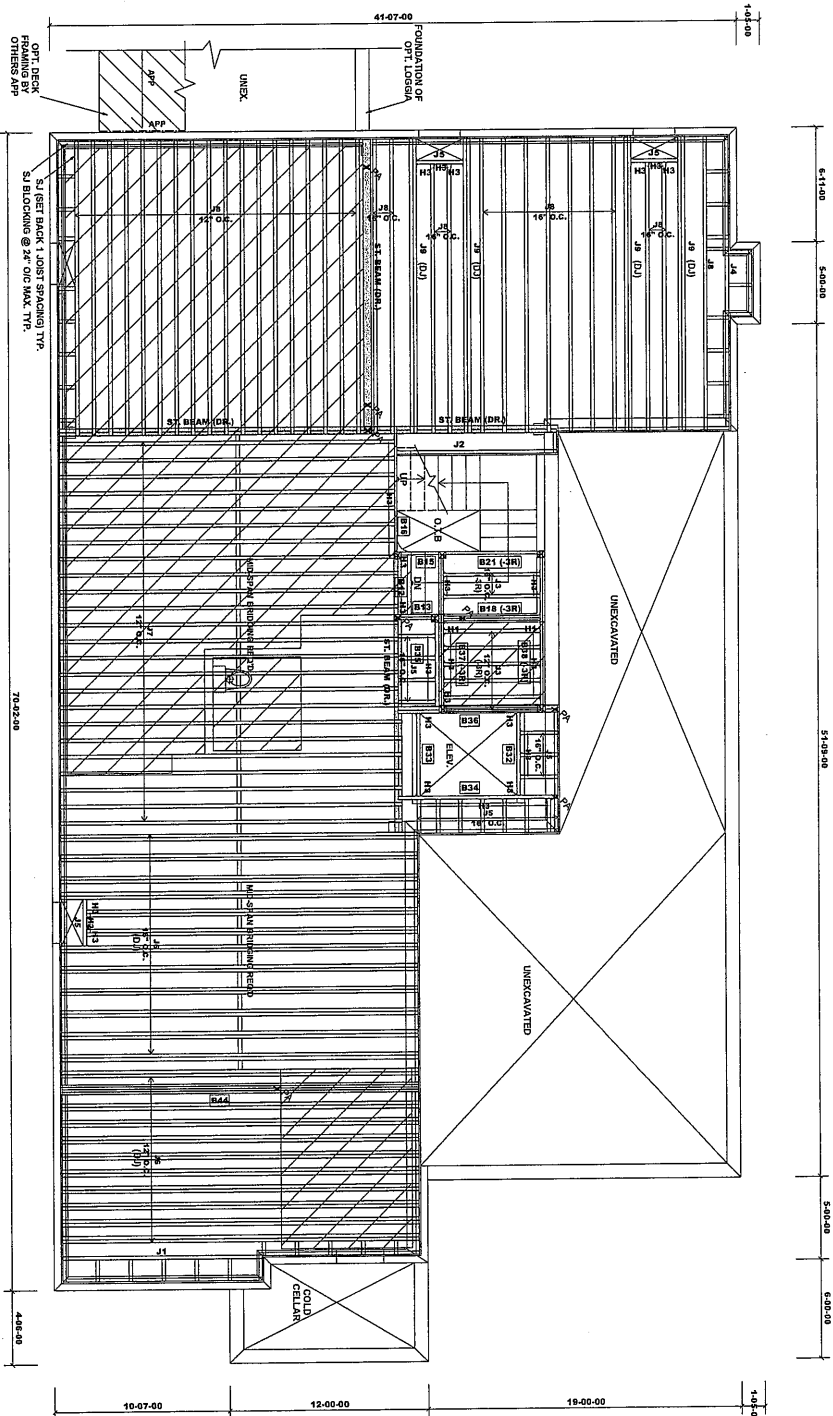
Designer: TL  
Sheet: 5 of 36

Alpa Roof Trusses Inc.  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.

ITEM	DESCRIPTION	QUANTITY	UNIT	PRICE	TOTAL
B1	11/16" N-20	1	1		
B2	11/16" N-20	1	1		
B3	11/16" N-20	1	1		
B4	11/16" N-20	1	1		
B5	11/16" N-20	1	1		
B6	11/16" N-20	1	1		
B7	11/16" N-20	1	1		
B8	11/16" N-20	1	1		
B9	11/16" N-20	1	1		
B10	11/16" N-20	1	1		
B11	11/16" N-20	1	1		
B12	11/16" N-20	1	1		
B13	11/16" N-20	1	1		
B14	11/16" N-20	1	1		
B15	11/16" N-20	1	1		
B16	11/16" N-20	1	1		
B17	11/16" N-20	1	1		
B18	11/16" N-20	1	1		
B19	11/16" N-20	1	1		
B20	11/16" N-20	1	1		
B21	11/16" N-20	1	1		
B22	11/16" N-20	1	1		
B23	11/16" N-20	1	1		
B24	11/16" N-20	1	1		
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B27	11/16" N-20	1	1		
B28	11/16" N-20	1	1		
B29	11/16" N-20	1	1		
B30	11/16" N-20	1	1		
B31	11/16" N-20	1	1		
B32	11/16" N-20	1	1		
B33	11/16" N-20	1	1		
B34	11/16" N-20	1	1		
B35	11/16" N-20	1	1		
B36	11/16" N-20	1	1		
B37	11/16" N-20	1	1		
B38	11/16" N-20	1	1		
B39	11/16" N-20	1	1		
B40	11/16" N-20	1	1		
B41	11/16" N-20	1	1		
B42	11/16" N-20	1	1		
B43	11/16" N-20	1	1		
B44	11/16" N-20	1	1		
B45	11/16" N-20	1	1		
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B58	11/16" N-20	1	1		
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B66	11/16" N-20	1	1		
B67	11/16" N-20	1	1		
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B70	11/16" N-20	1	1		
B71	11/16" N-20	1	1		
B72	11/16" N-20	1	1		
B73	11/16" N-20	1	1		
B74	11/16" N-20	1	1		
B75	11/16" N-20	1	1		
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B79	11/16" N-20	1	1		
B80	11/16" N-20	1	1		
B81	11/16" N-20	1	1		
B82	11/16" N-20	1	1		
B83	11/16" N-20	1	1		
B84	11/16" N-20	1	1		
B85	11/16" N-20	1	1		
B86	11/16" N-20	1	1		
B87	11/16" N-20	1	1		
B88	11/16" N-20	1	1		
B89	11/16" N-20	1	1		
B90	11/16" N-20	1	1		
B91	11/16" N-20	1	1		
B92	11/16" N-20	1	1		
B93	11/16" N-20	1	1		
B94	11/16" N-20	1	1		
B95	11/16" N-20	1	1		
B96	11/16" N-20	1	1		
B97	11/16" N-20	1	1		
B98	11/16" N-20	1	1		
B99	11/16" N-20	1	1		
B100	11/16" N-20	1	1		

ITEM	DESCRIPTION	QUANTITY	UNIT	PRICE	TOTAL
B1	11/16" N-20	1	1		
B2	11/16" N-20	1	1		
B3	11/16" N-20	1	1		
B4	11/16" N-20	1	1		
B5	11/16" N-20	1	1		
B6	11/16" N-20	1	1		
B7	11/16" N-20	1	1		
B8	11/16" N-20	1	1		
B9	11/16" N-20	1	1		
B10	11/16" N-20	1	1		
B11	11/16" N-20	1	1		
B12	11/16" N-20	1	1		
B13	11/16" N-20	1	1		
B14	11/16" N-20	1	1		
B15	11/16" N-20	1	1		
B16	11/16" N-20	1	1		
B17	11/16" N-20	1	1		
B18	11/16" N-20	1	1		
B19	11/16" N-20	1	1		
B20	11/16" N-20	1	1		
B21	11/16" N-20	1	1		
B22	11/16" N-20	1	1		
B23	11/16" N-20	1	1		
B24	11/16" N-20	1	1		
B25	11/16" N-20	1	1		
B26	11/16" N-20	1	1		
B27	11/16" N-20	1	1		
B28	11/16" N-20	1	1		
B29	11/16" N-20	1	1		
B30	11/16" N-20	1	1		
B31	11/16" N-20	1	1		
B32	11/16" N-20	1	1		
B33	11/16" N-20	1	1		
B34	11/16" N-20	1	1		
B35	11/16" N-20	1	1		
B36	11/16" N-20	1	1		
B37	11/16" N-20	1	1		
B38	11/16" N-20	1	1		
B39	11/16" N-20	1	1		
B40	11/16" N-20	1	1		
B41	11/16" N-20	1	1		
B42	11/16" N-20	1	1		
B43	11/16" N-20	1	1		
B44	11/16" N-20	1	1		
B45	11/16" N-20	1	1		
B46	11/16" N-20	1	1		
B47	11/16" N-20	1	1		
B48	11/16" N-20	1	1		
B49	11/16" N-20	1	1		
B50	11/16" N-20	1	1		
B51	11/16" N-20	1	1		
B52	11/16" N-20	1	1		
B53	11/16" N-20	1	1		
B54	11/16" N-20	1	1		
B55	11/16" N-20	1	1		
B56	11/16" N-20	1	1		
B57	11/16" N-20	1	1		
B58	11/16" N-20	1	1		
B59	11/16" N-20	1	1		
B60	11/16" N-20	1	1		
B61	11/16" N-20	1	1		
B62	11/16" N-20	1	1		
B63	11/16" N-20	1	1		
B64	11/16" N-20	1	1		
B65	11/16" N-20	1	1		
B66	11/16" N-20	1	1		
B67	11/16" N-20	1	1		
B68	11/16" N-20	1	1		
B69	11/16" N-20	1	1		
B70	11/16" N-20	1	1		
B71	11/16" N-20	1	1		
B72	11/16" N-20	1	1		
B73	11/16" N-20	1	1		
B74	11/16" N-20	1	1		
B75	11/16" N-20	1	1		
B76	11/16" N-20	1	1		
B77	11/16" N-20	1	1		
B78	11/16" N-20	1	1		
B79	11/16" N-20	1	1		
B80	11/16" N-20	1	1		
B81	11/16" N-20	1	1		
B82	11/16" N-20	1	1		
B83	11/16" N-20	1	1		
B84	11/16" N-20	1	1		
B85	11/16" N-20	1	1		
B86	11/16" N-20	1	1		
B87	11/16" N-20	1	1		
B88	11/16" N-20	1	1		
B89	11/16" N-20	1	1		
B90	11/16" N-20	1	1		
B91	11/16" N-20	1	1		
B92	11/16" N-20	1	1		
B93	11/16" N-20	1	1		
B94	11/16" N-20	1	1		
B95	11/16" N-20	1	1		
B96	11/16" N-20	1	1		
B97	11/16" N-20	1	1		
B98	11/16" N-20	1	1		
B99	11/16" N-20	1	1		
B100	11/16" N-20	1	1		



GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
+ W.W.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD: 40 PSF
DEAD LOAD: 15 PSF
DEAD LOAD (TYP): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BEO - BEAM BY OTHERS
P1 - FLOOR ABOVE
P2 - FLOOR BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS
RIMBOARD
1-1/8" X 11-7/8" O.S.B
SUBFLOOR: 3/4" VALUED & GULFED

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide 1/2" blocking between cantilevered joists (along bearing) and inboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460  
LI: 34307\*

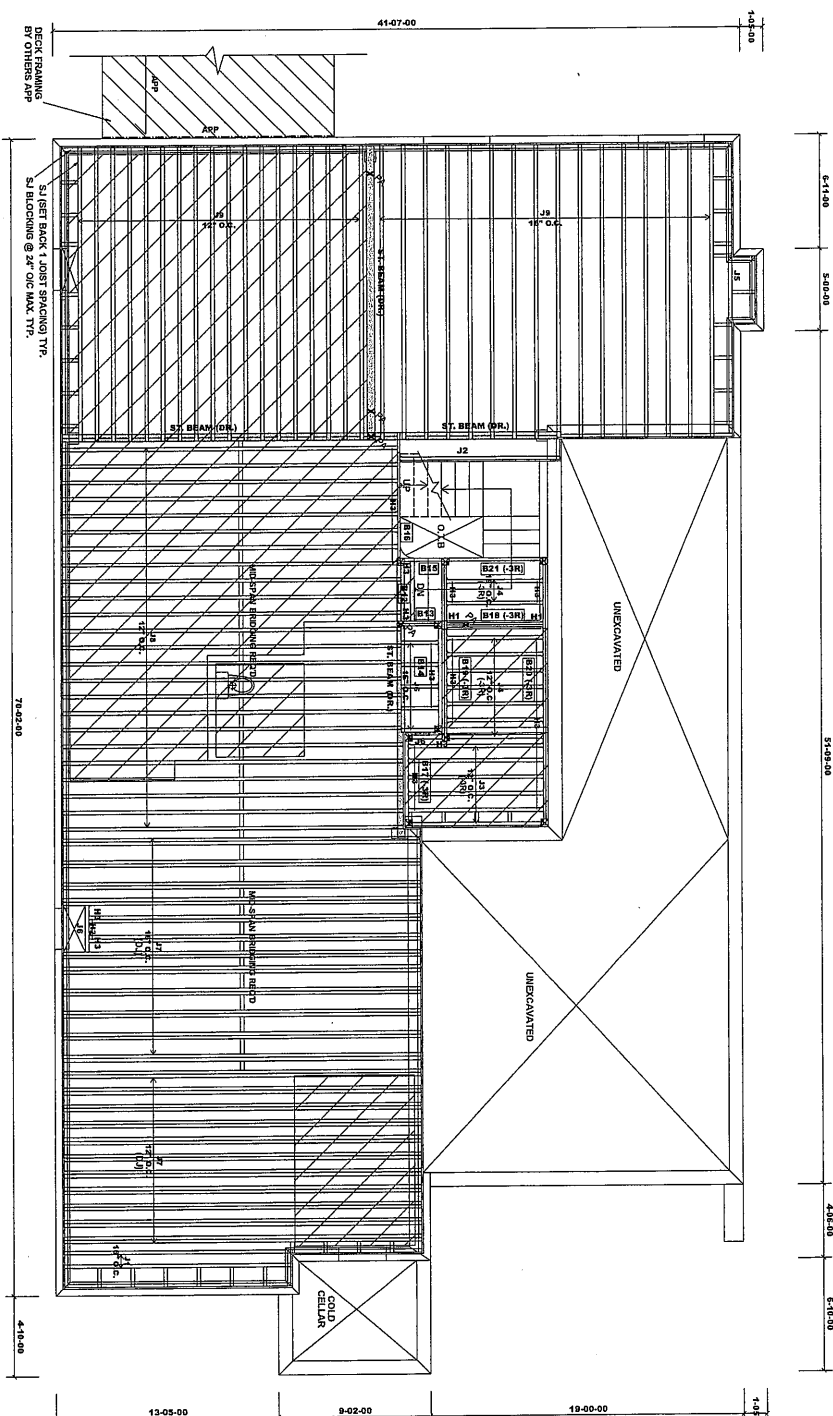
Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 6 of 36

Alpa Roof Trusses Inc.  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.



GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/L.O.D. CONDITION

FLOOR LOADING  
LIVE LOAD: 40 PSF  
DEAD LOAD: 15 PSF  
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

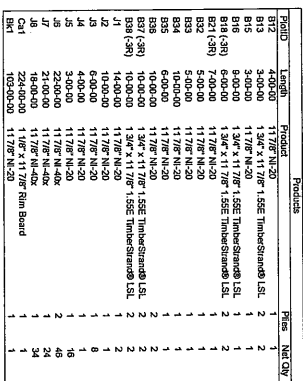
APP - AS PER PLAN  
BRO - BEAM BY OTHERS  
C.O.T.B. - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RUMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GULFED

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide 1:10 blocking between cantilevered joists (being bearing) and ribboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

ITEM	Length	Product	Plan	Net Qty
B12	4-00-00	11/7/8" N-20	1	1
B13	3-00-00	1 3/4" X 11 7/8" 155E TimberStrand LSL	2	2
B14	2-00-00	11 7/8" N-20	1	1
B15	2-00-00	11 7/8" N-20	1	1
B16	8-00-00	1 3/4" X 11 7/8" 155E TimberStrand LSL	1	1
B17 (-38)	6-00-00	11 7/8" N-20	1	1
B18 (-38)	6-00-00	1 3/4" X 11 7/8" 155E TimberStrand LSL	2	2
B19 (-38)	12-00-00	1 3/4" X 11 7/8" 155E TimberStrand LSL	2	2
B20 (-38)	17-00-00	1 3/4" X 11 7/8" 155E TimberStrand LSL	2	2
B21 (-38)	7-00-00	11 7/8" N-20	1	1
B1	14-00-00	11 7/8" N-20	1	1
B2	14-00-00	11 7/8" N-20	1	1
B3	8-00-00	11 7/8" N-20	1	1
B4	8-00-00	11 7/8" N-20	1	1
B5	4-00-00	11 7/8" N-20	1	1
B6	3-00-00	11 7/8" N-20	1	1
B7	22-00-00	11 7/8" N-40x	2	46
B8	21-00-00	11 7/8" N-40x	1	24
B9	14-00-00	11 7/8" N-20	1	1
B10	22-00-00	1 1/8" X 11 7/8" Rim Board	1	1
B11	10-00-00	11 7/8" N-20	1	1

Connected Summary			
ProdID	Qty	Manuf	Product
H1	2		H035710
H2	2		H03122
H3	45		L725188

JT/PL: 45147/116460  
LI: 343077\*  
Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May. 06, 2022  
Designer: TL  
Sheet: 7 of 36  
Alpa Roof Trusses Inc.  
Stouffville, Ontario  
Salesperson: Derek F.  
Home Lumber Inc.



Connector Summary			
FltId	Qty	Manuf	Product
H1	2		HGUS410
H2	2		HU312-2
H3	46		LT251168

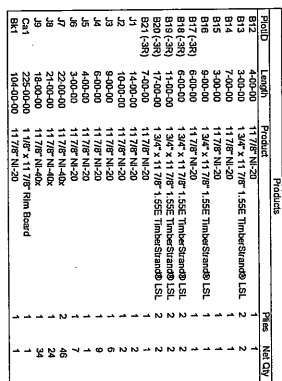
Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.6

Provide I-joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

Salesperson: Derek F.  
Home Lumber Inc.



Connector Summary			
PloID	Qty	Manuf	Product
H1	2		HGU5410
H2	2		HU312-2
H3	45		LT251168

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
BBC - BEAM BY OTHERS  
PA - POST ABOVE  
O.T.B - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RIMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GLUED\*

Blocking panels are required over all interior supports. Sequential blocks are required under concentrate loads.

Ceramic Tile Application as per O.B.C. 9.30.5

Provide 1-loist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

Salesperson: Derek F. Home Lumber Inc.

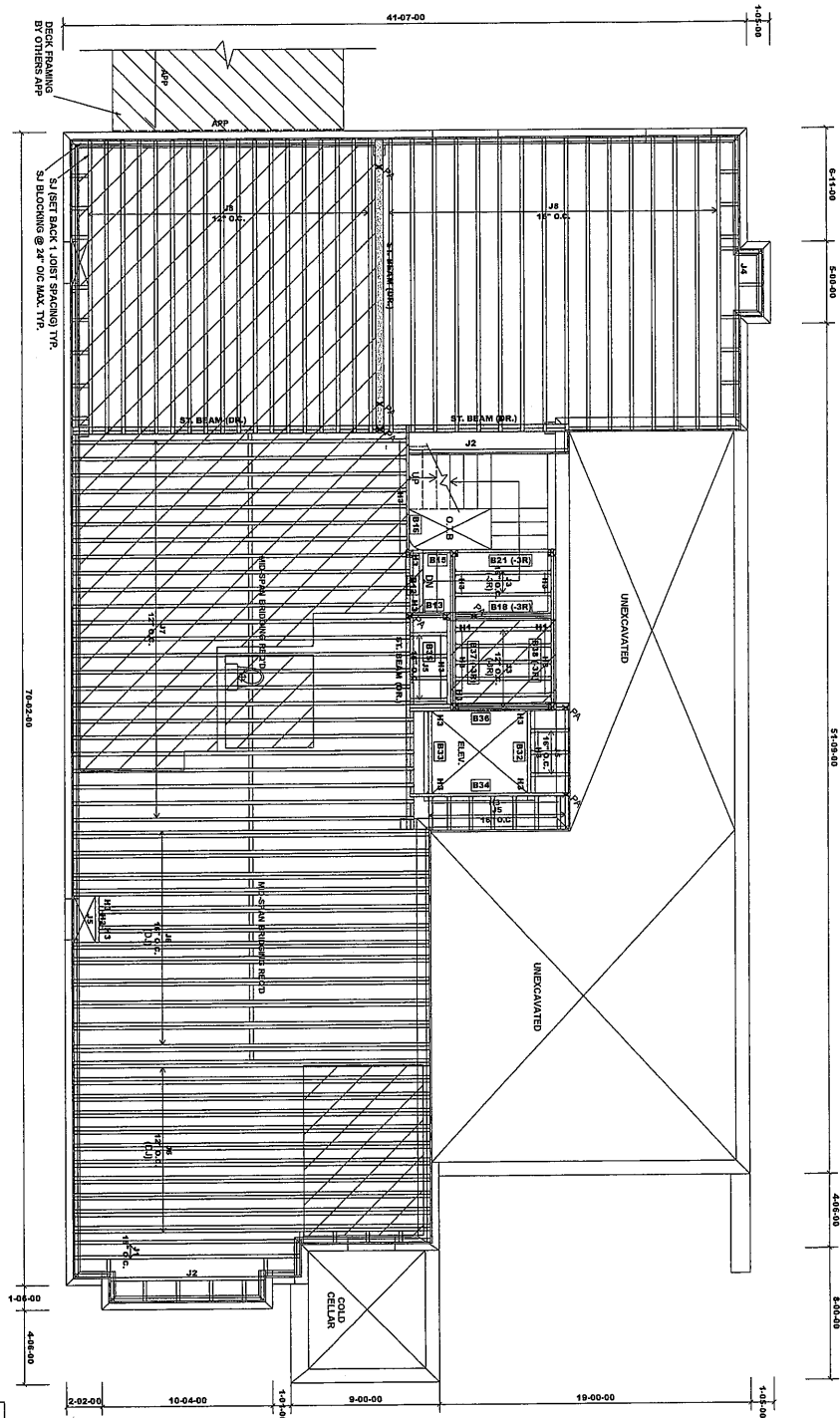
GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/ L.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND
Ceramic Tile
Conv. Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
PA - POST BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS
RAMBOARD
1-1/8" X 11-7/8" O.S.B
SUBFLOOR: 3/4" NAILLED & GULF*

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.
Ceramic Tile Application as per O.B.C. 9.30.6
Provide 1-inch blocking between cantilevered ends (along bearing) and inward closer at ends.
Do not scale - refer to architectural plans for dimensions.



Room	Length	Width	Product	Qty	Notes
B10	11'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B11	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B12	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B13	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B14	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B15	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B16	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B17	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B18	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B19	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B20	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B21	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B22	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B23	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B24	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B25	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B26	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B27	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B28	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B29	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B30	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B31	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B32	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B33	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B34	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B35	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B36	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B37	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B38	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B39	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B40	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B41	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B42	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B43	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B44	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B45	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B46	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B47	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B48	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B49	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B50	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B51	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B52	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B53	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B54	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B55	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B56	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B57	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B58	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B59	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B60	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B61	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B62	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B63	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B64	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B65	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B66	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B67	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B68	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B69	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B70	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B71	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B72	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B73	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B74	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B75	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B76	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B77	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B78	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B79	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B80	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B81	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B82	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B83	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B84	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B85	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B86	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B87	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B88	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B89	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B90	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B91	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B92	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B93	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B94	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B95	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B96	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B97	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B98	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B99	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	
B100	3'-00.00	11'-00.00	11/8" x 11/8" 1.55E TimberStrand® LSL	2	

FOOT	QTY	Material	Product
H1	2	H01S10	
H2	2	H01S12	
H3	46	L721188	

JT/PL: 45147/116460  
LI: 343077\*

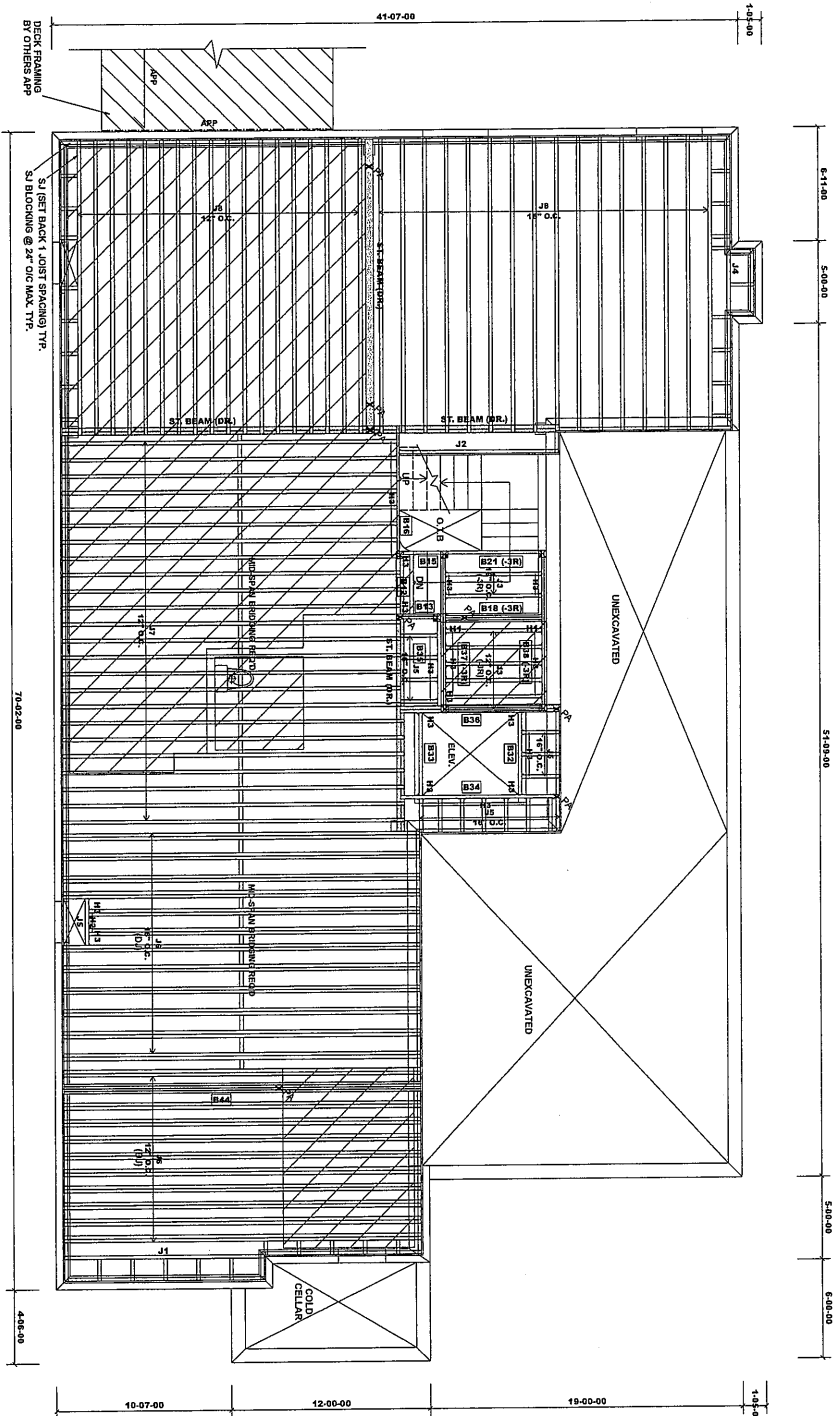
Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May, 06, 2022

Designer: TL  
Sheet: 10 of 36 Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.





GROUND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ L.O.D. CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD: 40 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv. Framed

APF - AS PER PLAN
BBO - BEAM BY OTHERS
OT B - OPEN TO BELOW
RT - ROOF TRUSS
RIMBOARD
1-1/8\"/>

Blocking panels are required over all interior supports. Squash blocks are required under concentrated loads. Ceramic Tile Application as per O.B.C. 9.30.6 Provide L-shaped blocking between cantilevered joists (along bearing) and imboard closure at ends. Do not scale - refer to architectural plans for dimensions.

ITEM	LENGTH	THICKNESS	QUANTITY	UNIT	QUANTITY
B12	4'-00-00	1 1/2" N-50	1	1	1
B13	3'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B14	3'-00-00	1 1/2" N-50	1	1	1
B15	3'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B16	7'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B17	7'-00-00	1 1/2" N-50	1	1	1
B18	5'-00-00	1 1/2" N-50	1	1	1
B19	5'-00-00	1 1/2" N-50	1	1	1
B20	5'-00-00	1 1/2" N-50	1	1	1
B21	5'-00-00	1 1/2" N-50	1	1	1
B22	5'-00-00	1 1/2" N-50	1	1	1
B23	5'-00-00	1 1/2" N-50	1	1	1
B24	5'-00-00	1 1/2" N-50	1	1	1
B25	10'-00-00	1 1/2" N-50	2	2	2
B26	10'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B27	10'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B28	10'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	2	2	2
B29	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B30	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B31	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B32	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B33	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B34	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B35	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B36	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B37	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B38	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B39	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B40	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B41	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B42	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B43	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B44	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B45	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B46	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B47	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B48	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B49	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B50	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B51	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B52	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B53	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B54	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B55	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B56	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
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B64	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B65	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B66	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B67	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B68	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
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B72	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B73	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B74	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B75	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B76	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B77	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B78	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B79	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B80	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B81	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B82	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B83	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B84	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B85	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B86	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B87	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B88	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B89	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B90	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B91	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
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B93	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B94	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B95	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B96	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B97	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B98	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B99	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B100	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B101	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B102	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B103	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B104	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B105	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B106	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B107	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B108	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B109	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B110	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B111	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B112	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B113	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B114	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B115	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B116	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B117	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B118	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B119	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B120	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B121	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B122	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B123	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B124	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B125	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B126	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B127	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B128	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B129	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B130	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B131	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B132	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B133	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B134	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B135	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B136	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B137	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B138	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B139	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B140	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B141	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B142	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B143	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B144	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B145	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B146	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B147	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B148	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B149	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B150	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B151	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
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B153	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B154	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B155	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B156	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
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B159	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B160	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B161	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B162	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B163	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B164	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B165	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B166	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B167	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B168	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B169	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B170	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B171	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B172	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B173	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B174	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B175	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B176	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B177	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B178	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B179	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B180	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B181	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B182	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B183	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B184	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B185	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	4
B186	22'-00-00	1 3/4" x 11 7/8" 15SE TimberStrand LSL	4	4	

PROJECT SUMMARY
PROJECT NO.
DATE
BY
CHECKED
APPROVED

JT/PL: 45147/116460  
 LI: 343077\*  
 Builder: Gold Park Homes  
 Project: Pine Valley Ph2  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 12 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.



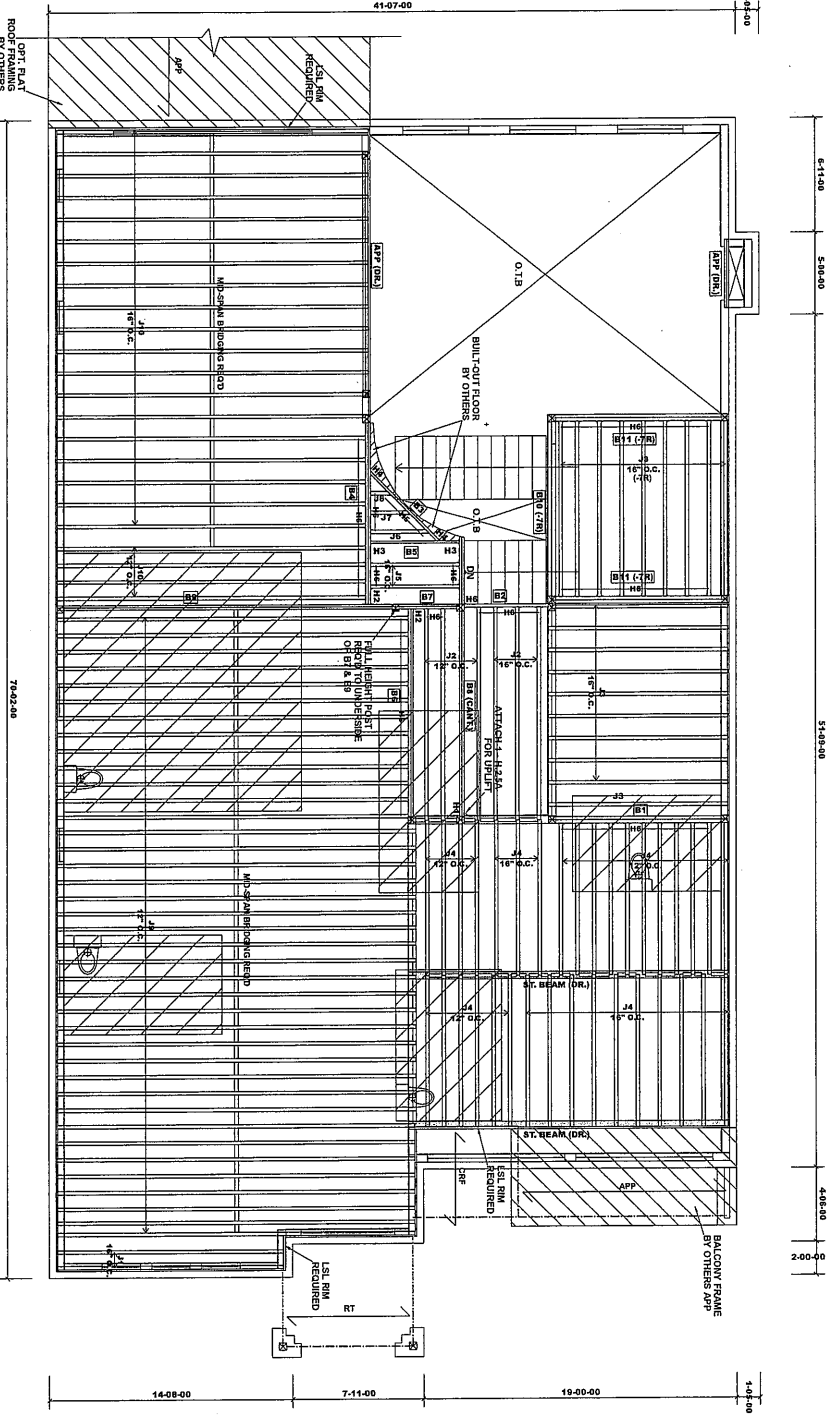
SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION

FLOOR LOADS
LIVE LOAD: 40 PSF
DEAD LOAD: 10 PSF
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
O.T.B. - OPEN TO BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS
RIMBOARD
1-1/8" X 11-7/8" O.S.B
SUBFLOOR: 3/4" NAILED & GULFED

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.
Ceramic Tile Application as per O.B.C. 9.30.6
Provide a 1/2" blocking between cantilevered joists (along bearing) and rimboard closure at ends.
Do not scale - refer to architectural plans for dimensions.



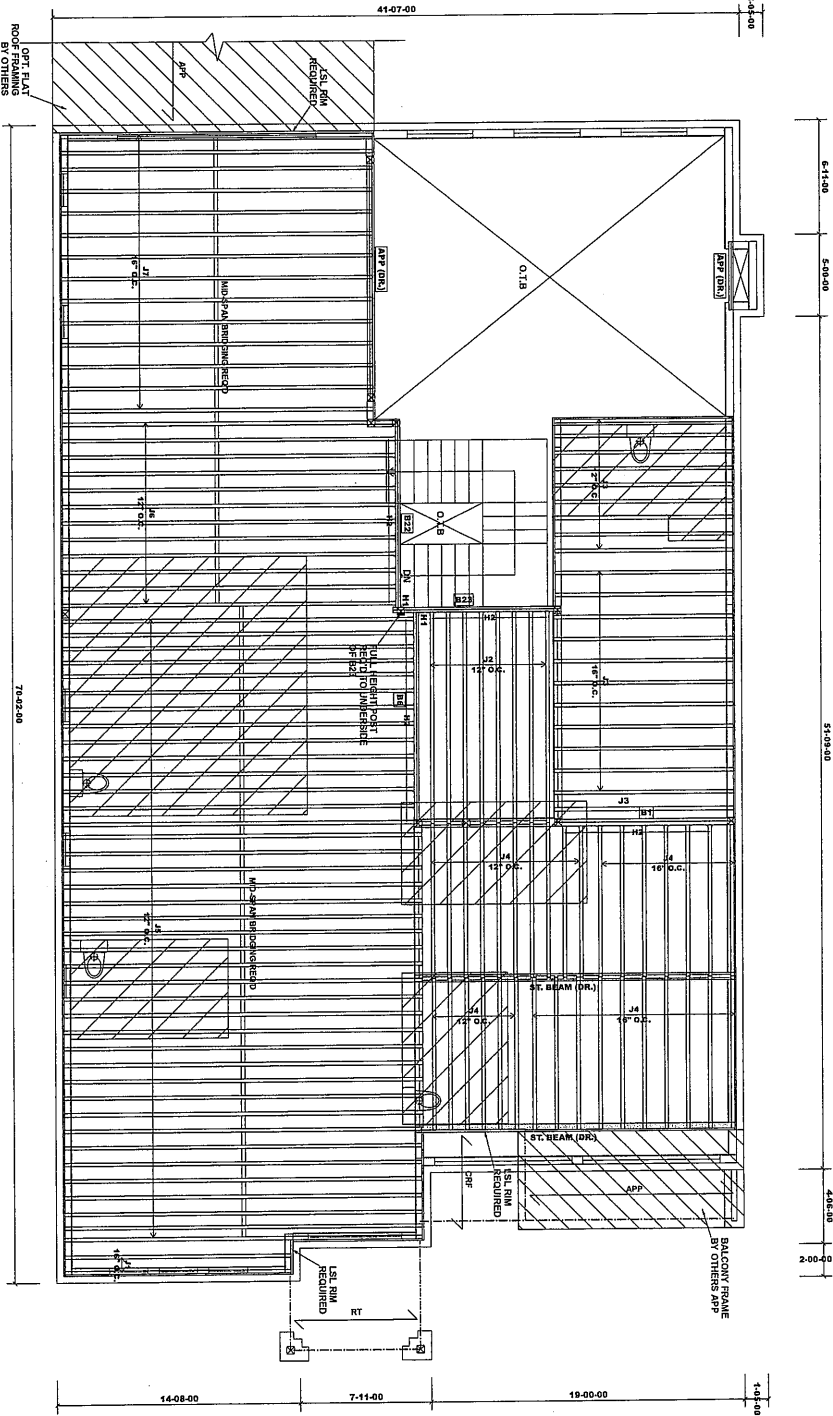
ITEM	QTY	UNIT	PRODUCT	PLAN	NET QTY
B1	11.00	00	11/7/8" N-20	2	2
B2	6.00	00	11/7/8" N-20	1	1
B3	7.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	1	1
B4	1.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	1	1
B5	6.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	1	1
B6	14.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	2	2
B7	4.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	2	2
B7 (CONT.)	1.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	2	2
B8	21.00	00	1 3/4" X 11/7/8" 1.55E TimberStrand® LSL	1	1
B9	11.00	00	11/7/8" N-20	2	2
B10 (7/8)	12.00	00	11/7/8" N-20	1	1
B11 (7/8)	11.00	00	11/7/8" N-20	2	2
J2	13.00	00	11/7/8" N-20	1	1
J3	11.00	00	11/7/8" N-20	1	1
J4	10.00	00	11/7/8" N-20	1	1
J5	11.00	00	11/7/8" N-20	1	1
J6	4.00	00	11/7/8" N-20	1	1
J7	3.00	00	11/7/8" N-20	1	1
J8	2.00	00	11/7/8" N-20	1	1
J9	2.00	00	11/7/8" N-20	1	1
J10	16.00	00	1 1/8" X 11/7/8" 1.55E TimberStrand® LSL	1	1
Cat	26.00	00	1 1/8" X 11/7/8" 1.55E TimberStrand® LSL	1	1
Cat	26.00	00	1 1/8" X 11/7/8" 1.55E TimberStrand® LSL	1	1
Cat	26.00	00	1 1/8" X 11/7/8" 1.55E TimberStrand® LSL	1	1

ITEM	QTY	UNIT	PRODUCT
H1	1	N/A	H2S110
H2	1	N/A	H2S110
H3	2	N/A	H2S110
H4	2	N/A	H2S110
H5	2	N/A	H2S110
H6	2	N/A	H2S110
H7	2	N/A	H2S110
H8	2	N/A	H2S110
H9	2	N/A	H2S110
H10	2	N/A	H2S110
H11	2	N/A	H2S110
H12	2	N/A	H2S110
H13	2	N/A	H2S110
H14	2	N/A	H2S110
H15	2	N/A	H2S110
H16	2	N/A	H2S110
H17	2	N/A	H2S110
H18	2	N/A	H2S110
H19	2	N/A	H2S110
H20	2	N/A	H2S110
H21	2	N/A	H2S110
H22	2	N/A	H2S110
H23	2	N/A	H2S110
H24	2	N/A	H2S110
H25	2	N/A	H2S110
H26	2	N/A	H2S110
H27	2	N/A	H2S110
H28	2	N/A	H2S110
H29	2	N/A	H2S110
H30	2	N/A	H2S110
H31	2	N/A	H2S110
H32	2	N/A	H2S110
H33	2	N/A	H2S110
H34	2	N/A	H2S110
H35	2	N/A	H2S110
H36	2	N/A	H2S110
H37	2	N/A	H2S110
H38	2	N/A	H2S110
H39	2	N/A	H2S110
H40	2	N/A	H2S110
H41	2	N/A	H2S110
H42	2	N/A	H2S110
H43	2	N/A	H2S110
H44	2	N/A	H2S110
H45	2	N/A	H2S110
H46	2	N/A	H2S110
H47	2	N/A	H2S110
H48	2	N/A	H2S110
H49	2	N/A	H2S110
H50	2	N/A	H2S110
H51	2	N/A	H2S110
H52	2	N/A	H2S110
H53	2	N/A	H2S110
H54	2	N/A	H2S110
H55	2	N/A	H2S110
H56	2	N/A	H2S110
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H59	2	N/A	H2S110
H60	2	N/A	H2S110
H61	2	N/A	H2S110
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H64	2	N/A	H2S110
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H66	2	N/A	H2S110
H67	2	N/A	H2S110
H68	2	N/A	H2S110
H69	2	N/A	H2S110
H70	2	N/A	H2S110
H71	2	N/A	H2S110
H72	2	N/A	H2S110
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H74	2	N/A	H2S110
H75	2	N/A	H2S110
H76	2	N/A	H2S110
H77	2	N/A	H2S110
H78	2	N/A	H2S110
H79	2	N/A	H2S110
H80	2	N/A	H2S110
H81	2	N/A	H2S110
H82	2	N/A	H2S110
H83	2	N/A	H2S110
H84	2	N/A	H2S110
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H86	2	N/A	H2S110
H87	2	N/A	H2S110
H88	2	N/A	H2S110
H89	2	N/A	H2S110
H90	2	N/A	H2S110
H91	2	N/A	H2S110
H92	2	N/A	H2S110
H93	2	N/A	H2S110
H94	2	N/A	H2S110
H95	2	N/A	H2S110
H96	2	N/A	H2S110
H97	2	N/A	H2S110
H98	2	N/A	H2S110
H99	2	N/A	H2S110
H100	2	N/A	H2S110

JT/PL: 45147/116460  
 LI: 343077\*  
 Builder: Gold Park Homes  
 Project: Pine Valley Ph2  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 13 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.

Field	Length	Product	Plan	Net Qty
B1	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B2	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B3	11-00-00	1 3/4" x 11 7/8" LSL	2	2
B4	11-00-00	1 3/4" x 11 7/8" LSL	2	2
B5	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B6	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B7	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B8	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B9	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B10	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B11	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B12	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B13	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B14	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B15	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B16	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B17	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B18	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B19	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B20	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B21	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B22	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B23	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B24	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B25	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B26	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B27	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B28	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B29	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B30	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B31	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B32	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B33	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B34	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B35	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B36	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B37	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B38	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B39	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B40	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B41	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B42	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B43	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B44	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B45	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B46	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B47	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B48	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B49	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B50	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B51	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B52	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B53	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B54	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B55	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B56	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B57	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B58	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B59	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B60	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B61	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B62	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B63	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B64	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B65	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B66	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B67	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B68	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B69	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B70	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B71	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B72	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B73	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B74	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B75	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B76	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B77	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B78	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B79	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B80	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B81	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B82	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B83	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B84	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B85	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B86	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B87	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B88	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B89	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B90	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B91	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B92	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B93	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B94	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B95	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B96	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B97	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B98	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B99	11-00-00	1 1/2" x 11 7/8" LSL	2	2
B100	11-00-00	1 1/2" x 11 7/8" LSL	2	2

Field	Qty	Material	Product
B1	41	1 1/2" x 11 7/8" LSL	L75/148



SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION A
WO/ LOFT CONDITION

FLOORLOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND
Ceramic Tile
Conv. Framed

APP. - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
OTB - OPEN TO BELOW
RT - ROOF TRUSS
RIM BOARD
1-1/2" x 11-7/8" O.S.B
SUBFLOOR: 3/4" NAILED & GLUED

Blocking panels are required over all interior spans. Squash blocks are required under concentrated loads. Ceramic Tile Application as per O.B.C. 9.30.6 Provide 1-1/2" blocking between cantilevered joists (during bearing) and inboard closure at ends. Do not scale - refer to architectural plans for dimensions.

JT/PL: 45147/116460  
 LI: 343077\*  
 Builder: Gold Park Homes  
 Project: Pine Valley Ph2  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 14 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.

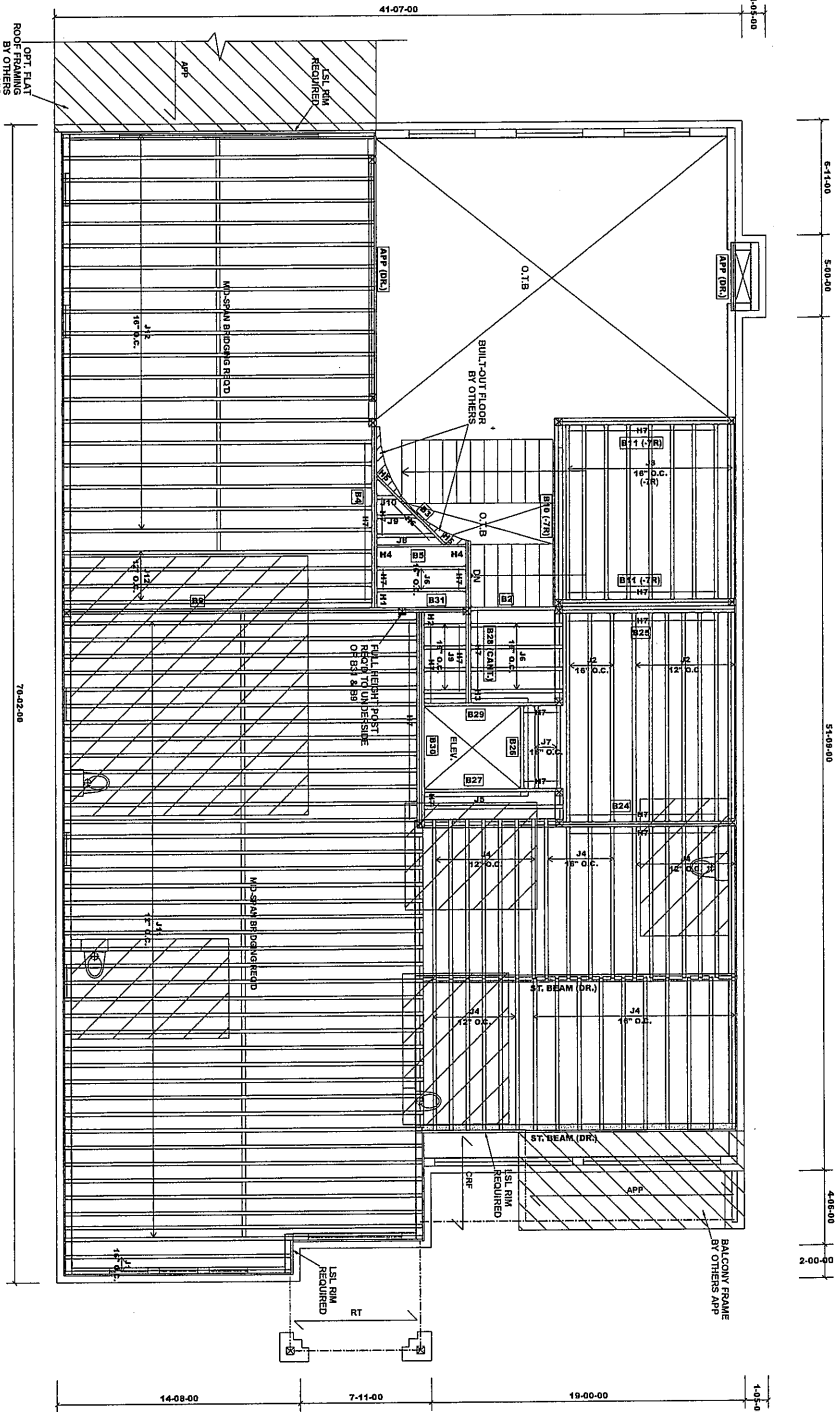
SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION A	
W/ LOFT CONDITION	
W/ ELEVATOR	

ELOOR LOADINGS  
DEAD LOAD (TILE) 20 PSF  
DEAD LOAD (TILE) 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP. AS PER PLAN  
PA - POST ABOVE  
O.T.B. - OPEN TO BELOW  
RT - ROOF TRUSS  
RIMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GLUED

Blocking panels are required over all interior supports.  
Joist blocking is required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide J-joist blocking between cantilevered joists (along bearing and inward closure at ends.  
Do not scale - refer to architectural plans for dimensions.



Field	Length	Product	Prodn	Qty	Unit
B2	6'-00-00	11/78" N-20	1	1	
B3	7'-00-00	13/4" x 11/78" 1.5SE TimberStrand LSL	1	1	
B4	7'-00-00	13/4" x 11/78" 1.5SE TimberStrand LSL	1	1	
B5	6'-00-00	13/4" x 11/78" 1.5SE TimberStrand LSL	1	1	
B6	21'-00-00	13/4" x 11/78" 1.5SE TimberStrand LSL	2	2	
B7	12'-00-00	11/78" N-20	1	1	
B8	12'-00-00	11/78" N-20	1	1	
B9	11'-00-00	13/4" x 11/78" 1.5SE TimberStrand LSL	2	2	
B10	11'-00-00	11/78" N-20	2	2	
B11	11'-00-00	11/78" N-20	2	2	
B12	11'-00-00	11/78" N-20	2	2	
B13	11'-00-00	11/78" N-20	2	2	
B14	11'-00-00	11/78" N-20	2	2	
B15	11'-00-00	11/78" N-20	2	2	
B16	11'-00-00	11/78" N-20	2	2	
B17	11'-00-00	11/78" N-20	2	2	
B18	11'-00-00	11/78" N-20	2	2	
B19	11'-00-00	11/78" N-20	2	2	
B20	11'-00-00	11/78" N-20	2	2	
B21	11'-00-00	11/78" N-20	2	2	
B22	11'-00-00	11/78" N-20	2	2	
B23	11'-00-00	11/78" N-20	2	2	
B24	11'-00-00	11/78" N-20	2	2	
B25	11'-00-00	11/78" N-20	2	2	
B26	11'-00-00	11/78" N-20	2	2	
B27	11'-00-00	11/78" N-20	2	2	
B28	11'-00-00	11/78" N-20	2	2	
B29	11'-00-00	11/78" N-20	2	2	
B30	11'-00-00	11/78" N-20	2	2	
B31	11'-00-00	11/78" N-20	2	2	
B32	11'-00-00	11/78" N-20	2	2	
B33	11'-00-00	11/78" N-20	2	2	
B34	11'-00-00	11/78" N-20	2	2	
B35	11'-00-00	11/78" N-20	2	2	
B36	11'-00-00	11/78" N-20	2	2	
B37	11'-00-00	11/78" N-20	2	2	
B38	11'-00-00	11/78" N-20	2	2	
B39	11'-00-00	11/78" N-20	2	2	
B40	11'-00-00	11/78" N-20	2	2	
B41	11'-00-00	11/78" N-20	2	2	
B42	11'-00-00	11/78" N-20	2	2	
B43	11'-00-00	11/78" N-20	2	2	
B44	11'-00-00	11/78" N-20	2	2	
B45	11'-00-00	11/78" N-20	2	2	
B46	11'-00-00	11/78" N-20	2	2	
B47	11'-00-00	11/78" N-20	2	2	
B48	11'-00-00	11/78" N-20	2	2	
B49	11'-00-00	11/78" N-20	2	2	
B50	11'-00-00	11/78" N-20	2	2	
B51	11'-00-00	11/78" N-20	2	2	
B52	11'-00-00	11/78" N-20	2	2	
B53	11'-00-00	11/78" N-20	2	2	
B54	11'-00-00	11/78" N-20	2	2	
B55	11'-00-00	11/78" N-20	2	2	
B56	11'-00-00	11/78" N-20	2	2	
B57	11'-00-00	11/78" N-20	2	2	
B58	11'-00-00	11/78" N-20	2	2	
B59	11'-00-00	11/78" N-20	2	2	
B60	11'-00-00	11/78" N-20	2	2	
B61	11'-00-00	11/78" N-20	2	2	
B62	11'-00-00	11/78" N-20	2	2	
B63	11'-00-00	11/78" N-20	2	2	
B64	11'-00-00	11/78" N-20	2	2	
B65	11'-00-00	11/78" N-20	2	2	
B66	11'-00-00	11/78" N-20	2	2	
B67	11'-00-00	11/78" N-20	2	2	
B68	11'-00-00	11/78" N-20	2	2	
B69	11'-00-00	11/78" N-20	2	2	
B70	11'-00-00	11/78" N-20	2	2	
B71	11'-00-00	11/78" N-20	2	2	
B72	11'-00-00	11/78" N-20	2	2	
B73	11'-00-00	11/78" N-20	2	2	
B74	11'-00-00	11/78" N-20	2	2	
B75	11'-00-00	11/78" N-20	2	2	
B76	11'-00-00	11/78" N-20	2	2	
B77	11'-00-00	11/78" N-20	2	2	
B78	11'-00-00	11/78" N-20	2	2	
B79	11'-00-00	11/78" N-20	2	2	
B80	11'-00-00	11/78" N-20	2	2	
B81	11'-00-00	11/78" N-20	2	2	
B82	11'-00-00	11/78" N-20	2	2	
B83	11'-00-00	11/78" N-20	2	2	
B84	11'-00-00	11/78" N-20	2	2	
B85	11'-00-00	11/78" N-20	2	2	
B86	11'-00-00	11/78" N-20	2	2	
B87	11'-00-00	11/78" N-20	2	2	
B88	11'-00-00	11/78" N-20	2	2	
B89	11'-00-00	11/78" N-20	2	2	
B90	11'-00-00	11/78" N-20	2	2	
B91	11'-00-00	11/78" N-20	2	2	
B92	11'-00-00	11/78" N-20	2	2	
B93	11'-00-00	11/78" N-20	2	2	
B94	11'-00-00	11/78" N-20	2	2	
B95	11'-00-00	11/78" N-20	2	2	
B96	11'-00-00	11/78" N-20	2	2	
B97	11'-00-00	11/78" N-20	2	2	
B98	11'-00-00	11/78" N-20	2	2	
B99	11'-00-00	11/78" N-20	2	2	
B100	11'-00-00	11/78" N-20	2	2	

Field	Qty	Material	Product
H1	1	ROOFING	
H2	1	ROOFING	
H3	1	ROOFING	
H4	2	ROOFING	
H5	2	ROOFING	
H6	3	ROOFING	
H7	3	ROOFING	
H8	3	ROOFING	
H9	3	ROOFING	
H10	3	ROOFING	
H11	3	ROOFING	
H12	3	ROOFING	
H13	3	ROOFING	
H14	3	ROOFING	
H15	3	ROOFING	
H16	3	ROOFING	
H17	3	ROOFING	
H18	3	ROOFING	
H19	3	ROOFING	
H20	3	ROOFING	
H21	3	ROOFING	
H22	3	ROOFING	
H23	3	ROOFING	
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H26	3	ROOFING	
H27	3	ROOFING	
H28	3	ROOFING	
H29	3	ROOFING	
H30	3	ROOFING	
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H32	3	ROOFING	
H33	3	ROOFING	
H34	3	ROOFING	
H35	3	ROOFING	
H36	3	ROOFING	
H37	3	ROOFING	
H38	3	ROOFING	
H39	3	ROOFING	
H40	3	ROOFING	
H41	3	ROOFING	
H42	3	ROOFING	
H43	3	ROOFING	
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H46	3	ROOFING	
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H49	3	ROOFING	
H50	3	ROOFING	
H51	3	ROOFING	
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H53	3	ROOFING	
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H90	3	ROOFING	
H91	3	ROOFING	
H92	3	ROOFING	
H93	3	ROOFING	
H94	3	ROOFING	
H95	3	ROOFING	
H96	3	ROOFING	
H97	3	ROOFING	
H98	3	ROOFING	
H99	3	ROOFING	
H100	3	ROOFING	

JT/PL: 45147/116460  
LI: 343077\*

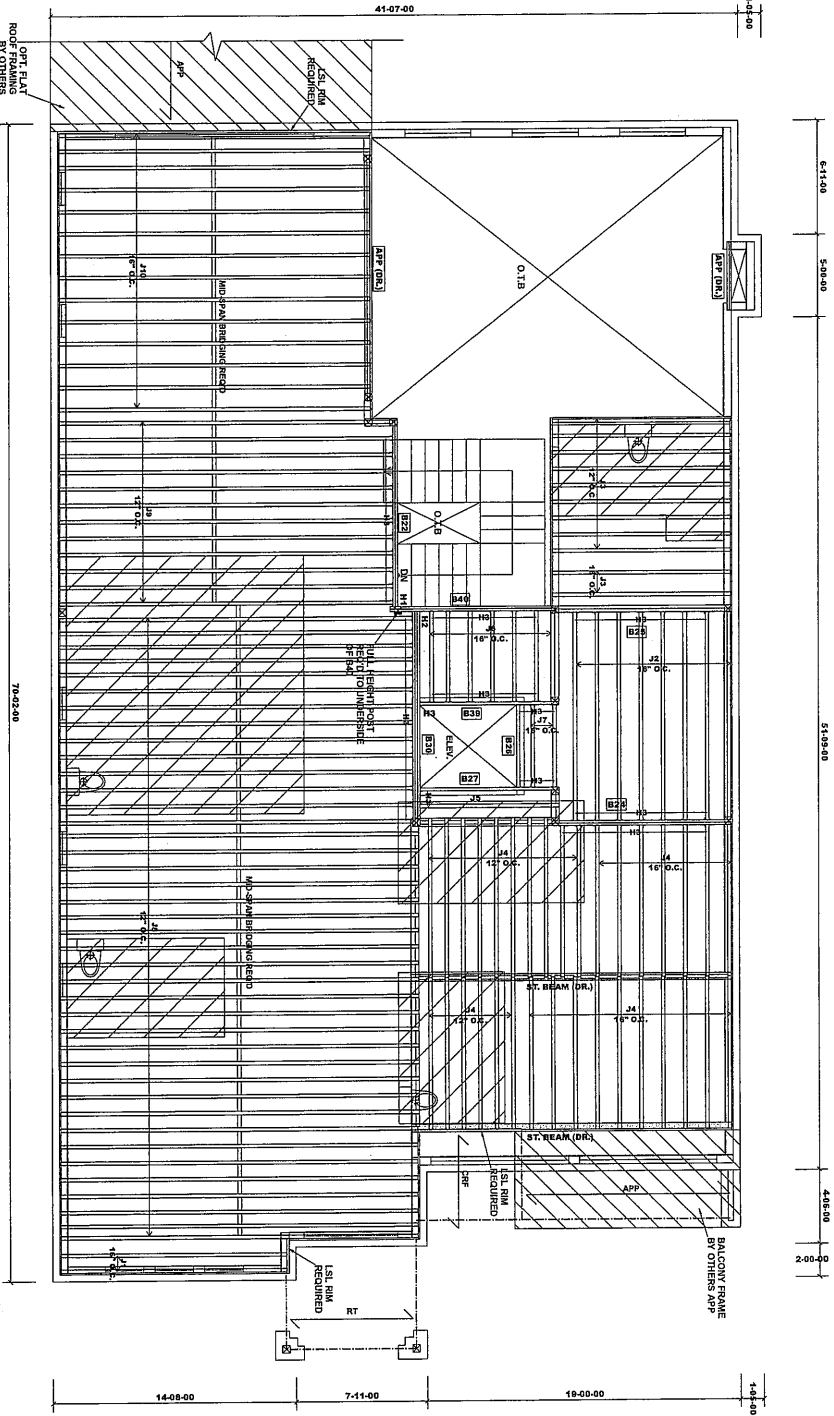
Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May, 06, 2022

Designer: TL  
Sheet: 15 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.

SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION A	
W/O LOFT CONDITION	
W/ ELEVATOR	



FLOOR LOADS  
DEAD LOAD: 15 PSF  
LIVE LOAD: 40 PSF  
DEAD LOAD TITLE: 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

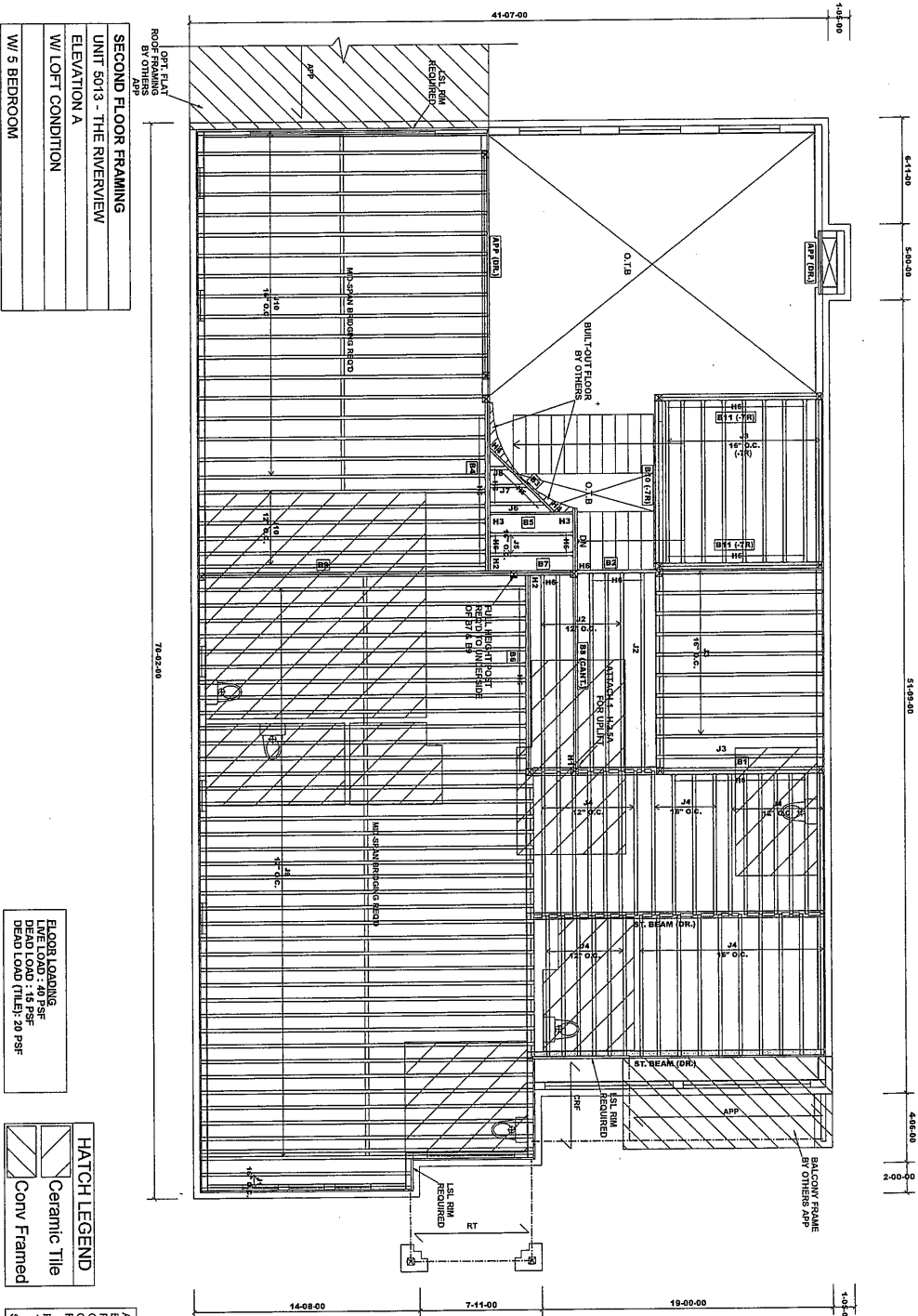
APP - AS PER PLAN  
BAY WINDOW  
DOOR - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RIMJOIST  
1-1/8" X 11-7/8" O.S.B.  
SUBFLOOR: 3/4" NAILER & GUEY\*

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide 1x6 blocking between cantilevered joists (along bearing) and rimboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

Concrete Summary	
Reinforced	1
Formwork	1
Household	1
Household	1
Household	1

Room	Length	Product	Area	Unit Qty
B22	13.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	2	2
B24	11.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	2	2
B26	11.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	2	2
B28	5.00-00	1 1/2" N-20	1	1
B30	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B32	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B34	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B36	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B38	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B40	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B42	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B44	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B46	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B48	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B50	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B52	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B54	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B56	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B58	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B60	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B62	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B64	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B66	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B68	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B70	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B72	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B74	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B76	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B78	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B80	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B82	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B84	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B86	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B88	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B90	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B92	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B94	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B96	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B98	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3
B100	14.00-00	1 3/4" X 11 7/8" 1.5SE Timber-Strand® LSL	3	3

JT/PL: 45147/116460  
LI: 343077\*  
Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May. 06, 2022  
Designer: TL  
Sheet: 16 of 36  
Salesperson: Derek F.  
Home Lumber Inc.



UNIT 5013 - THE RIVERVIEW
ELEVATION A
W/ LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING
LINE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APF - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE BELOW
GT - GIRDER TRUSS
RT - ROOF TRUSS
RIMBOARD
1-1/8" X 11-7/8" O.S.B
SUBFLOOR: 3/4" VALUED & GLUED

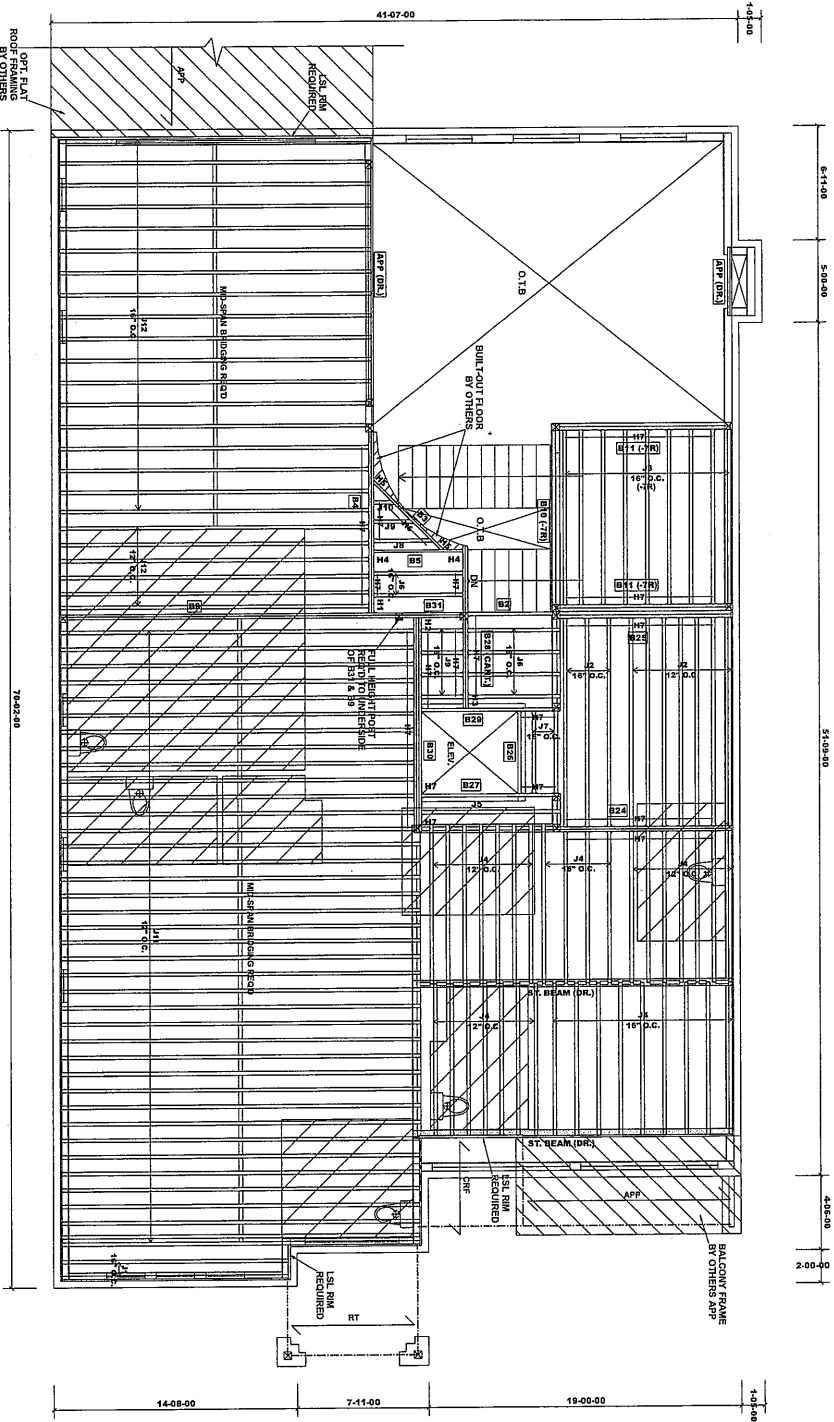
Blocking panels are required over all interior supports.
Square blocks are required under concentrated loads.
Ceramic Tile Application as per O.B.C. 9.30.6
Provide 1/4" blocking between cantilevered joists (along bearing) and inboard closure at ends.
Do not scale - refer to architectural plans for dimensions.

FIELD	Qty	Material	Product
H1	1	N/A	H25K
H2	2	11/8" x 11/8" O.S.B	HUS13110
H4	2	11/8" x 11/8" O.S.B	LSR11812
H5	2	11/8" x 11/8" O.S.B	LSR11812
H6	2	11/8" x 11/8" O.S.B	LSR11812

FIELD	Length	Product	Qty	Unit
B2	11.00-00	11/8" N-20	1	1
B3	7.00-00	11/8" N-20	1	1
B4	11.00-00	11/8" N-20	1	1
B5	11.00-00	11/8" N-20	1	1
B6	11.00-00	11/8" N-20	1	1
B7	11.00-00	11/8" N-20	1	1
B8	11.00-00	11/8" N-20	1	1
B9	11.00-00	11/8" N-20	1	1
B10	11.00-00	11/8" N-20	1	1
B11	11.00-00	11/8" N-20	1	1
B12	11.00-00	11/8" N-20	1	1
B13	11.00-00	11/8" N-20	1	1
B14	11.00-00	11/8" N-20	1	1
B15	11.00-00	11/8" N-20	1	1
B16	11.00-00	11/8" N-20	1	1
B17	11.00-00	11/8" N-20	1	1
B18	11.00-00	11/8" N-20	1	1
B19	11.00-00	11/8" N-20	1	1
B20	11.00-00	11/8" N-20	1	1
B21	11.00-00	11/8" N-20	1	1
B22	11.00-00	11/8" N-20	1	1
B23	11.00-00	11/8" N-20	1	1
B24	11.00-00	11/8" N-20	1	1
B25	11.00-00	11/8" N-20	1	1
B26	11.00-00	11/8" N-20	1	1
B27	11.00-00	11/8" N-20	1	1
B28	11.00-00	11/8" N-20	1	1
B29	11.00-00	11/8" N-20	1	1
B30	11.00-00	11/8" N-20	1	1
B31	11.00-00	11/8" N-20	1	1
B32	11.00-00	11/8" N-20	1	1
B33	11.00-00	11/8" N-20	1	1
B34	11.00-00	11/8" N-20	1	1
B35	11.00-00	11/8" N-20	1	1
B36	11.00-00	11/8" N-20	1	1
B37	11.00-00	11/8" N-20	1	1
B38	11.00-00	11/8" N-20	1	1
B39	11.00-00	11/8" N-20	1	1
B40	11.00-00	11/8" N-20	1	1
B41	11.00-00	11/8" N-20	1	1
B42	11.00-00	11/8" N-20	1	1
B43	11.00-00	11/8" N-20	1	1
B44	11.00-00	11/8" N-20	1	1
B45	11.00-00	11/8" N-20	1	1
B46	11.00-00	11/8" N-20	1	1
B47	11.00-00	11/8" N-20	1	1
B48	11.00-00	11/8" N-20	1	1
B49	11.00-00	11/8" N-20	1	1
B50	11.00-00	11/8" N-20	1	1
B51	11.00-00	11/8" N-20	1	1
B52	11.00-00	11/8" N-20	1	1
B53	11.00-00	11/8" N-20	1	1
B54	11.00-00	11/8" N-20	1	1
B55	11.00-00	11/8" N-20	1	1
B56	11.00-00	11/8" N-20	1	1
B57	11.00-00	11/8" N-20	1	1
B58	11.00-00	11/8" N-20	1	1
B59	11.00-00	11/8" N-20	1	1
B60	11.00-00	11/8" N-20	1	1
B61	11.00-00	11/8" N-20	1	1
B62	11.00-00	11/8" N-20	1	1
B63	11.00-00	11/8" N-20	1	1
B64	11.00-00	11/8" N-20	1	1
B65	11.00-00	11/8" N-20	1	1
B66	11.00-00	11/8" N-20	1	1
B67	11.00-00	11/8" N-20	1	1
B68	11.00-00	11/8" N-20	1	1
B69	11.00-00	11/8" N-20	1	1
B70	11.00-00	11/8" N-20	1	1
B71	11.00-00	11/8" N-20	1	1
B72	11.00-00	11/8" N-20	1	1
B73	11.00-00	11/8" N-20	1	1
B74	11.00-00	11/8" N-20	1	1
B75	11.00-00	11/8" N-20	1	1
B76	11.00-00	11/8" N-20	1	1
B77	11.00-00	11/8" N-20	1	1
B78	11.00-00	11/8" N-20	1	1
B79	11.00-00	11/8" N-20	1	1
B80	11.00-00	11/8" N-20	1	1
B81	11.00-00	11/8" N-20	1	1
B82	11.00-00	11/8" N-20	1	1
B83	11.00-00	11/8" N-20	1	1
B84	11.00-00	11/8" N-20	1	1
B85	11.00-00	11/8" N-20	1	1
B86	11.00-00	11/8" N-20	1	1
B87	11.00-00	11/8" N-20	1	1
B88	11.00-00	11/8" N-20	1	1
B89	11.00-00	11/8" N-20	1	1
B90	11.00-00	11/8" N-20	1	1
B91	11.00-00	11/8" N-20	1	1
B92	11.00-00	11/8" N-20	1	1
B93	11.00-00	11/8" N-20	1	1
B94	11.00-00	11/8" N-20	1	1
B95	11.00-00	11/8" N-20	1	1
B96	11.00-00	11/8" N-20	1	1
B97	11.00-00	11/8" N-20	1	1
B98	11.00-00	11/8" N-20	1	1
B99	11.00-00	11/8" N-20	1	1
B100	11.00-00	11/8" N-20	1	1

JT/PL: 45147/116460  
 LI: 34307\*  
 Builder: Gold Park Homes  
 Project: Pine Valley Ph2  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 17 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.

SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION A	
W/ LOFT CONDITION	
W/ ELEVATOR	
W/ 5 BEDROOM	



FLOOR LOADING  
LIVE LOAD : 40 PSF  
DEAD LOAD : 10 PSF  
DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND
Ceramic Tile
Conv. Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
GT - GIRDER TRUSS  
RT - ROOF TRUSS  
RIMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" VALED & GULFED

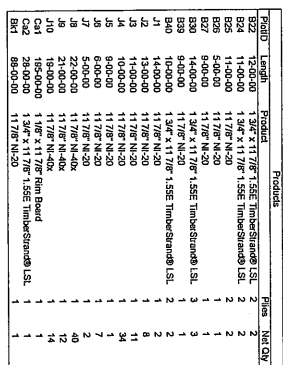
Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide 1x6 blocking between cantilevered ends (along ceiling) and interior columns at all dimensions.  
Do not scale - refer to architectural plans for dimensions.

ITEM	QTY	UNIT	DESCRIPTION
H1	1	sq. ft.	1000000
H2	1	sq. ft.	1000000
H3	1	sq. ft.	1000000
H4	1	sq. ft.	1000000
H5	1	sq. ft.	1000000
H6	1	sq. ft.	1000000
H7	1	sq. ft.	1000000
H8	1	sq. ft.	1000000
H9	1	sq. ft.	1000000
H10	1	sq. ft.	1000000
H11	1	sq. ft.	1000000
H12	1	sq. ft.	1000000
H13	1	sq. ft.	1000000
H14	1	sq. ft.	1000000
H15	1	sq. ft.	1000000
H16	1	sq. ft.	1000000
H17	1	sq. ft.	1000000
H18	1	sq. ft.	1000000
H19	1	sq. ft.	1000000
H20	1	sq. ft.	1000000
H21	1	sq. ft.	1000000
H22	1	sq. ft.	1000000
H23	1	sq. ft.	1000000
H24	1	sq. ft.	1000000
H25	1	sq. ft.	1000000
H26	1	sq. ft.	1000000
H27	1	sq. ft.	1000000
H28	1	sq. ft.	1000000
H29	1	sq. ft.	1000000
H30	1	sq. ft.	1000000
H31	1	sq. ft.	1000000
H32	1	sq. ft.	1000000
H33	1	sq. ft.	1000000
H34	1	sq. ft.	1000000
H35	1	sq. ft.	1000000
H36	1	sq. ft.	1000000
H37	1	sq. ft.	1000000
H38	1	sq. ft.	1000000
H39	1	sq. ft.	1000000
H40	1	sq. ft.	1000000
H41	1	sq. ft.	1000000
H42	1	sq. ft.	1000000
H43	1	sq. ft.	1000000
H44	1	sq. ft.	1000000
H45	1	sq. ft.	1000000
H46	1	sq. ft.	1000000
H47	1	sq. ft.	1000000
H48	1	sq. ft.	1000000
H49	1	sq. ft.	1000000
H50	1	sq. ft.	1000000
H51	1	sq. ft.	1000000
H52	1	sq. ft.	1000000
H53	1	sq. ft.	1000000
H54	1	sq. ft.	1000000
H55	1	sq. ft.	1000000
H56	1	sq. ft.	1000000
H57	1	sq. ft.	1000000
H58	1	sq. ft.	1000000
H59	1	sq. ft.	1000000
H60	1	sq. ft.	1000000
H61	1	sq. ft.	1000000
H62	1	sq. ft.	1000000
H63	1	sq. ft.	1000000
H64	1	sq. ft.	1000000
H65	1	sq. ft.	1000000
H66	1	sq. ft.	1000000
H67	1	sq. ft.	1000000
H68	1	sq. ft.	1000000
H69	1	sq. ft.	1000000
H70	1	sq. ft.	1000000
H71	1	sq. ft.	1000000
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H73	1	sq. ft.	1000000
H74	1	sq. ft.	1000000
H75	1	sq. ft.	1000000
H76	1	sq. ft.	1000000
H77	1	sq. ft.	1000000
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H81	1	sq. ft.	1000000
H82	1	sq. ft.	1000000
H83	1	sq. ft.	1000000
H84	1	sq. ft.	1000000
H85	1	sq. ft.	1000000
H86	1	sq. ft.	1000000
H87	1	sq. ft.	1000000
H88	1	sq. ft.	1000000
H89	1	sq. ft.	1000000
H90	1	sq. ft.	1000000
H91	1	sq. ft.	1000000
H92	1	sq. ft.	1000000
H93	1	sq. ft.	1000000
H94	1	sq. ft.	1000000
H95	1	sq. ft.	1000000
H96	1	sq. ft.	1000000
H97	1	sq. ft.	1000000
H98	1	sq. ft.	1000000
H99	1	sq. ft.	1000000
H100	1	sq. ft.	1000000

ITEM	QTY	UNIT	DESCRIPTION
B1	1	sq. ft.	1000000
B2	1	sq. ft.	1000000
B3	1	sq. ft.	1000000
B4	1	sq. ft.	1000000
B5	1	sq. ft.	1000000
B6	1	sq. ft.	1000000
B7	1	sq. ft.	1000000
B8	1	sq. ft.	1000000
B9	1	sq. ft.	1000000
B10	1	sq. ft.	1000000
B11	1	sq. ft.	1000000
B12	1	sq. ft.	1000000
B13	1	sq. ft.	1000000
B14	1	sq. ft.	1000000
B15	1	sq. ft.	1000000
B16	1	sq. ft.	1000000
B17	1	sq. ft.	1000000
B18	1	sq. ft.	1000000
B19	1	sq. ft.	1000000
B20	1	sq. ft.	1000000
B21	1	sq. ft.	1000000
B22	1	sq. ft.	1000000
B23	1	sq. ft.	1000000
B24	1	sq. ft.	1000000
B25	1	sq. ft.	1000000
B26	1	sq. ft.	1000000
B27	1	sq. ft.	1000000
B28	1	sq. ft.	1000000
B29	1	sq. ft.	1000000
B30	1	sq. ft.	1000000
B31	1	sq. ft.	1000000
B32	1	sq. ft.	1000000
B33	1	sq. ft.	1000000
B34	1	sq. ft.	1000000
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B36	1	sq. ft.	1000000
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B53	1	sq. ft.	1000000
B54	1	sq. ft.	1000000
B55	1	sq. ft.	1000000
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B57	1	sq. ft.	1000000
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B59	1	sq. ft.	1000000
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B61	1	sq. ft.	1000000
B62	1	sq. ft.	1000000
B63	1	sq. ft.	1000000
B64	1	sq. ft.	1000000
B65	1	sq. ft.	1000000
B66	1	sq. ft.	1000000
B67	1	sq. ft.	1000000
B68	1	sq. ft.	1000000
B69	1	sq. ft.	1000000
B70	1	sq. ft.	1000000
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B72	1	sq. ft.	1000000
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B80	1	sq. ft.	1000000
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B82	1	sq. ft.	1000000
B83	1	sq. ft.	1000000
B84	1	sq. ft.	1000000
B85	1	sq. ft.	1000000
B86	1	sq. ft.	1000000
B87	1	sq. ft.	1000000
B88	1	sq. ft.	1000000
B89	1	sq. ft.	1000000
B90	1	sq. ft.	1000000
B91	1	sq. ft.	1000000
B92	1	sq. ft.	1000000
B93	1	sq. ft.	1000000
B94	1	sq. ft.	1000000
B95	1	sq. ft.	1000000
B96	1	sq. ft.	1000000
B97	1	sq. ft.	1000000
B98	1	sq. ft.	1000000
B99	1	sq. ft.	1000000
B100	1	sq. ft.	1000000

JT/PL: 45147/116460  
 LT: 34307\*  
 Builder: Gold Park Homes  
 Project: Pine Valley Ph2  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 18 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.





Connector Summary			
ProdID	Qty	Manuf	Product
H1	1		HGUS410
H2	1		HGUS550710
H3	69		L7251188

**FLOOR LOADING**  
LIVE LOAD : 40 PSF  
DEAD LOAD : 15 PSF  
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
O.T.B - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS

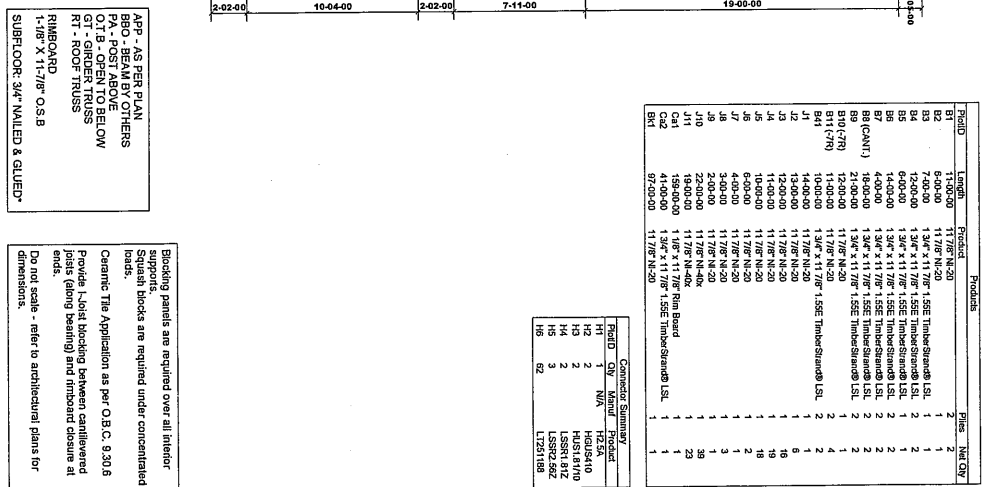
RIMBOARD  
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED\*

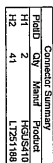
Bleeding panels are required over all interior supports.  
Suction blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide I-Joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

Salesperson: Derek F. Home Lumber Inc.

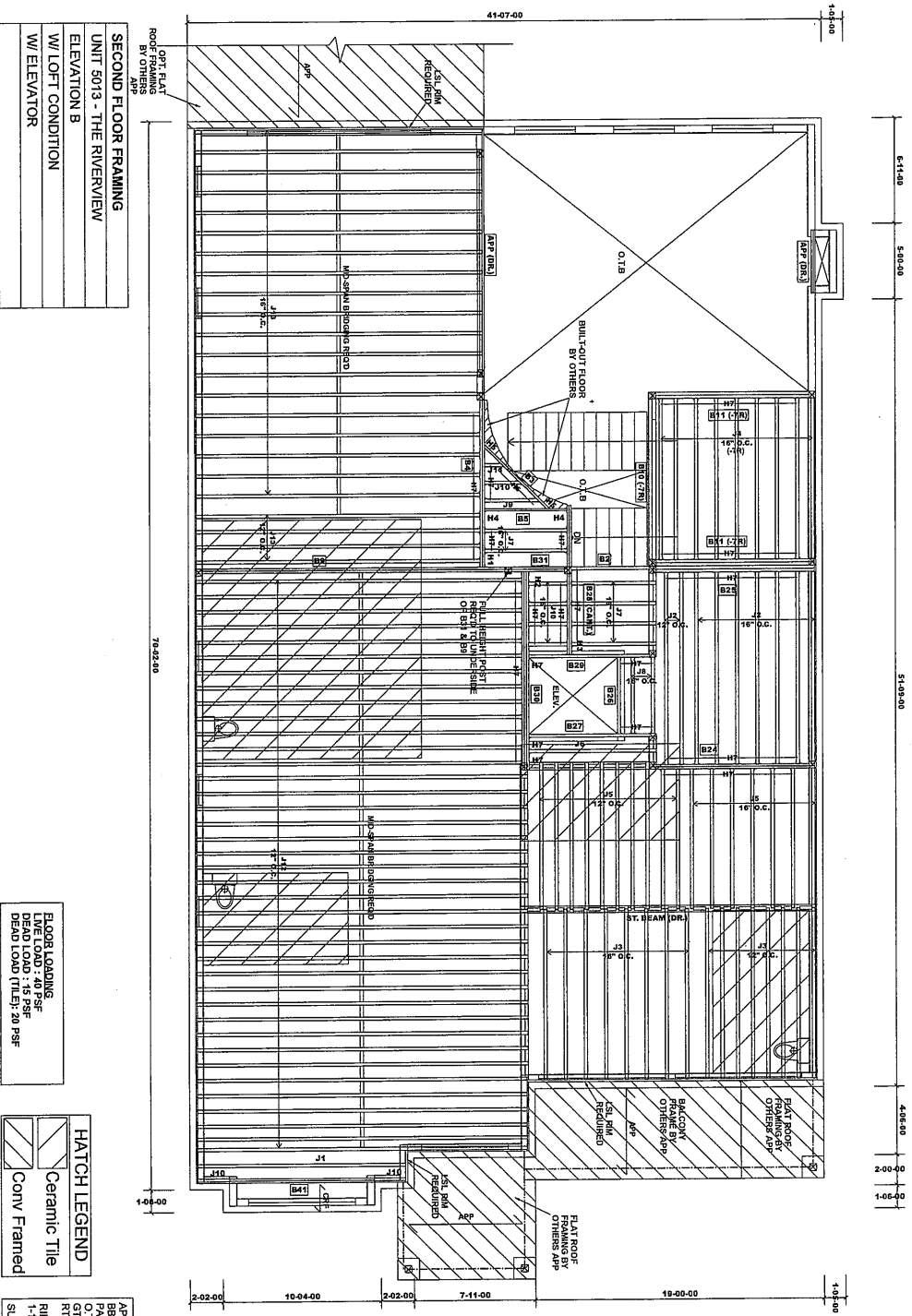




Designer: TL      Alpa Roof Trusses Inc.      Salesperson: Derek F.  
Sheet: 21 of 36      Stouffville, Ontario      Home Lumber Inc.



Salesperson: Derek F. Home Lumber Inc.



**FLOOR LOADING**  
LINE LOAD : 40 PSF  
DEAD LOAD (TILE): 20 PSF

**HATCH LEGEND**

	Ceramic Tile
	Conv Framed

**APP - AS PER PLAN**  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
OT B - OPEN TO BELOW  
RT - ROOF TRUSS  
RUMECARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" VAILED & GLUED

Blocking panels are required over all interior support.  
Squeak blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide 1-label blocking between cantilevered joists (girding blocking) and inboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.

Item	Length	Product	Units	Qty
B2	7'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B3	7'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	2	2
B4	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B5	6'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B6	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B7	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B8	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B9	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B10	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B11	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B12	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B13	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B14	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B15	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B16	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B17	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B18	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B19	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B20	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B21	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B22	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B23	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B24	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B25	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B26	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B27	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B28	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B29	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B30	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B31	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B32	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B33	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B34	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B35	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B36	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B37	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B38	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B39	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B40	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B41	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B42	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B43	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B44	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B45	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B46	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B47	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B48	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B49	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B50	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B51	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B52	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B53	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B54	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B55	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B56	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B57	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B58	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B59	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B60	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B61	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B62	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B63	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B64	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B65	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B66	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B67	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B68	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B69	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B70	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B71	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B72	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B73	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B74	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B75	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B76	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B77	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B78	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B79	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B80	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B81	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B82	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B83	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B84	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B85	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B86	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B87	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B88	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B89	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B90	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B91	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B92	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B93	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B94	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B95	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B96	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B97	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B98	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B99	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1
B100	12'-00-00	1 3/4" x 11 7/8" 1.55E TimberStrand LSL	1	1

**Connector Summary**

Item	Qty	Material	Product
H1	1	HELIX-10	
H2	1	HELIX-10	
H3	1	HELIX-10	
H4	2	HELIX-10	
H5	2	HELIX-10	
H6	3	HELIX-10	
H7	31	HELIX-10	

JT/PL: 45147/116460  
 LI: 343077\*  
 Builder: Gold Park Homes  
 Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Alpa Roof Trusses Inc.  
 Sheet: 23 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.

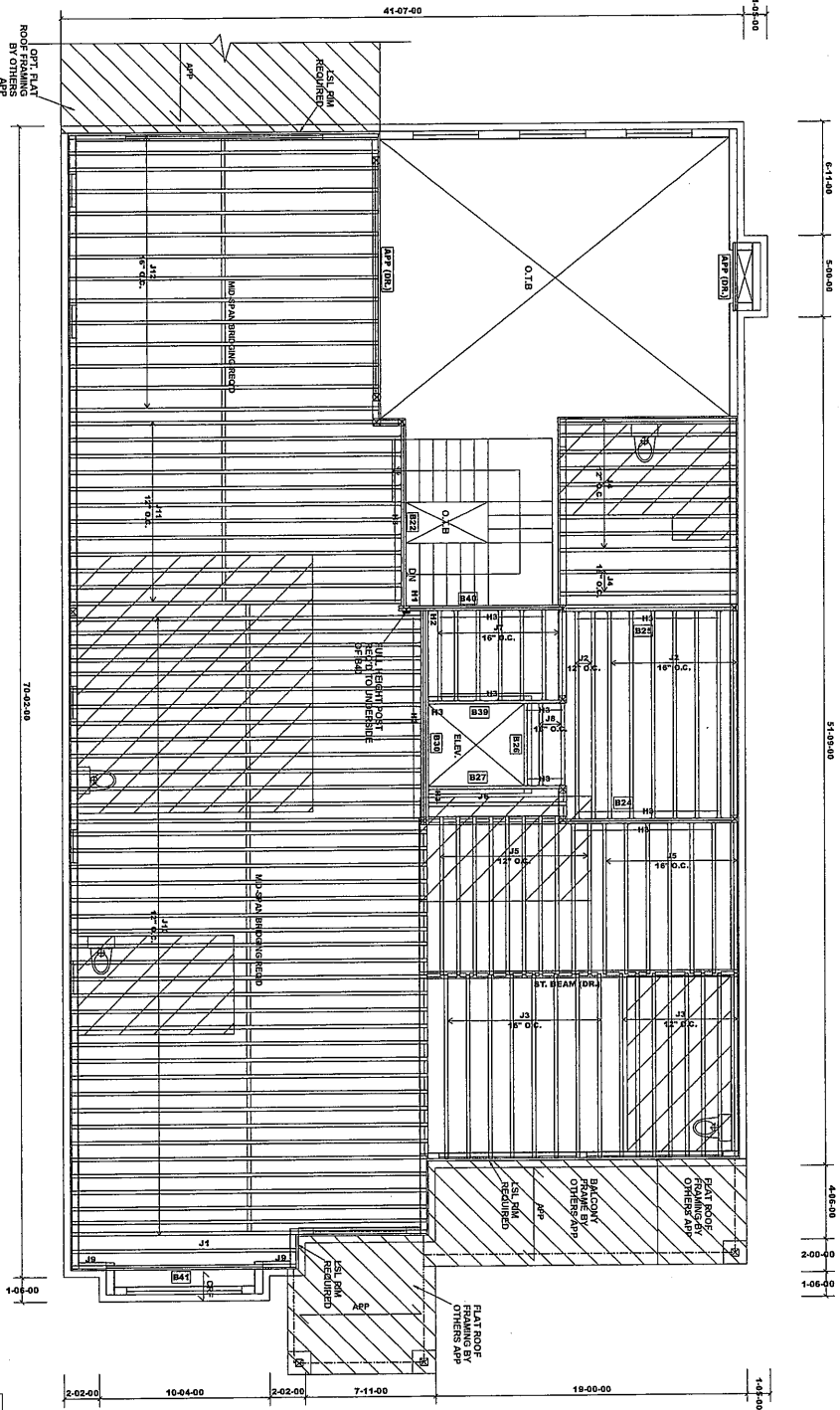
SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
WO/LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING  
 DEAD LOAD : 40 PSF  
 DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
 BBO - BEAM BY OTHERS  
 PA - POST ABOVE  
 OF B ROOF TRUSS  
 RT - ROOF TRUSS  
 RUMICARD  
 1-1/8" X 11-7/8" O.S.B  
 SUBFLOOR: 3/4" NAILED & GULDED

Bracing panels are required over all interior support beams.  
 Squash blocks are required under concentrated loads.  
 Ceramic Tile Application as per O.B.C. 9.30.6  
 Provide L-Joist blocking between cantilevered ends.  
 Provide blocking between and inboard closure at ends.  
 Do not scale - refer to architectural plans for dimensions.



FIELD	Length	Product	Qty	Unit	Notes
B22	11'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	2	2	
B25	11'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	2	2	
B28	5'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	1	1	
B30	14'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	3	3	
B39	5'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	1	1	
B40	10'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	2	2	
B41	14'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	1	1	
J2	13'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J3	12'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J4	10'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J5	10'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J6	8'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J7	6'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J8	3'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J9	22'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J10	21'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
J11	17'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
C41	17'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	
C42	41'-00-00	1 3/4" x 11 7/8" LSSB TimberStrand® LSL	1	1	
B41	8'-00-00	1 1/2" LSSB TimberStrand® LSL	1	1	

FIELD	Qty	Material	Product
H1	1	HIGH-10	HIGH-10
H2	1	HIGH-20	HIGH-20
H3	1	HIGH-30	HIGH-30

JT/PL: 45147/116460  
 LI: 34307\*

Builder: Gold Park Homes  
 Project: Pine Valley Ph2

Location: Vaughan, ON  
 Date: May. 06, 2022  
 Designer: TL  
 Sheet: 24 of 36  
 Stouffville, Ontario  
 Salesperson: Derek F.  
 Home Lumber Inc.

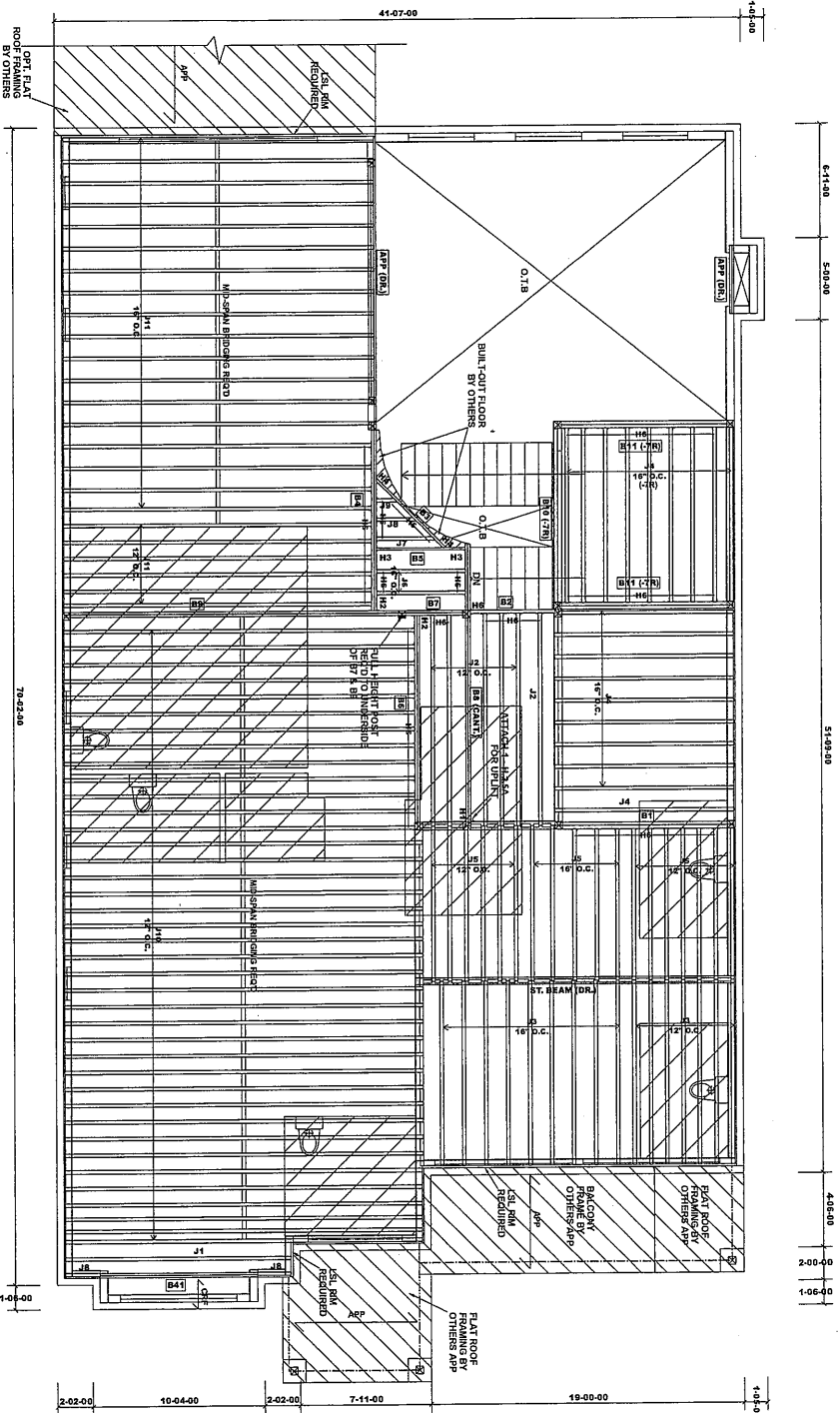
SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION B
W/LOFT CONDITION
W/ 5 BEDROOM

FLOOR LOADING  
LIVE LOAD: 40 PSF  
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
RT - OPEN TO BELOW  
RT - GIRDOR TRUSS  
RT - ROOF TRUSS  
RIMBOARD  
1-1/8" X 11-7/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GULED

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.20.6  
Provide L-shield blocking between cantilevered joists (along bearing and imboard closure at ends.  
Do not scale - refer to architectural plans for dimensions.



Block	Length	Product	Qty	Unit	Qty
B1	11'-00.00	11/78" N-20	2	1	1
B2	6'-00.00	11/78" N-20	2	1	1
B3	7'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	1	1	1
B4	13'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	1	1	1
B5	6'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	1	1	1
B6	14'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	2	2	2
B7	4'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	2	2	2
B8	13'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	2	2	2
B9	21'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	2	2	2
B10	12'-00.00	11/78" N-20	1	1	1
B11	11'-00.00	11/78" N-20	1	1	1
B12	14'-00.00	11/78" N-20	1	1	1
B13	13'-00.00	11/78" N-20	1	1	1
B14	10'-00.00	11/78" N-20	1	1	1
B15	10'-00.00	11/78" N-20	1	1	1
B16	6'-00.00	11/78" N-20	1	1	1
B17	3'-00.00	11/78" N-20	1	1	1
B18	3'-00.00	11/78" N-20	1	1	1
B19	2'-00.00	11/78" N-20	1	1	1
B20	22'-00.00	11/78" N-40x	1	1	39
B21	15'-00.00	11/78" N-40x	1	1	24
B22	41'-00.00	11/78" x 11/78" Rim Board	1	1	1
B23	14'-00.00	13/4" x 11/78" 1.55E TimberStrand® LSL	1	1	1
B24	39'-00.00	11/78" N-20	1	1	1

Block	Qty	Product
B1	1	12.5A
B2	2	10.5A/10
B3	2	10.5A/10
B4	2	10.5A/10
B5	3	10.5A/10
B6	2	10.5A/10
B7	2	10.5A/10
B8	2	10.5A/10
B9	2	10.5A/10
B10	2	10.5A/10
B11	2	10.5A/10
B12	2	10.5A/10
B13	2	10.5A/10
B14	2	10.5A/10
B15	2	10.5A/10
B16	2	10.5A/10
B17	2	10.5A/10
B18	2	10.5A/10
B19	2	10.5A/10
B20	2	10.5A/10
B21	2	10.5A/10
B22	2	10.5A/10
B23	2	10.5A/10
B24	2	10.5A/10
B25	2	10.5A/10
B26	2	10.5A/10
B27	2	10.5A/10
B28	2	10.5A/10
B29	2	10.5A/10
B30	2	10.5A/10
B31	2	10.5A/10
B32	2	10.5A/10
B33	2	10.5A/10
B34	2	10.5A/10
B35	2	10.5A/10
B36	2	10.5A/10
B37	2	10.5A/10
B38	2	10.5A/10
B39	2	10.5A/10
B40	2	10.5A/10
B41	2	10.5A/10
B42	2	10.5A/10
B43	2	10.5A/10
B44	2	10.5A/10
B45	2	10.5A/10
B46	2	10.5A/10
B47	2	10.5A/10
B48	2	10.5A/10
B49	2	10.5A/10
B50	2	10.5A/10
B51	2	10.5A/10
B52	2	10.5A/10
B53	2	10.5A/10
B54	2	10.5A/10
B55	2	10.5A/10
B56	2	10.5A/10
B57	2	10.5A/10
B58	2	10.5A/10
B59	2	10.5A/10
B60	2	10.5A/10
B61	2	10.5A/10
B62	2	10.5A/10
B63	2	10.5A/10
B64	2	10.5A/10
B65	2	10.5A/10
B66	2	10.5A/10
B67	2	10.5A/10
B68	2	10.5A/10
B69	2	10.5A/10
B70	2	10.5A/10
B71	2	10.5A/10
B72	2	10.5A/10
B73	2	10.5A/10
B74	2	10.5A/10
B75	2	10.5A/10
B76	2	10.5A/10
B77	2	10.5A/10
B78	2	10.5A/10
B79	2	10.5A/10
B80	2	10.5A/10
B81	2	10.5A/10
B82	2	10.5A/10
B83	2	10.5A/10
B84	2	10.5A/10
B85	2	10.5A/10
B86	2	10.5A/10
B87	2	10.5A/10
B88	2	10.5A/10
B89	2	10.5A/10
B90	2	10.5A/10
B91	2	10.5A/10
B92	2	10.5A/10
B93	2	10.5A/10
B94	2	10.5A/10
B95	2	10.5A/10
B96	2	10.5A/10
B97	2	10.5A/10
B98	2	10.5A/10
B99	2	10.5A/10
B100	2	10.5A/10

JT/PL: 45147/116460  
LI: 343077\*

Builder: Gold Park Homes  
Project: Pine Valley Pn2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 25 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.

SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION B	
W/ LOFT CONDITION	
W/ ELEVATOR	
W/ 5 BEDROOM	

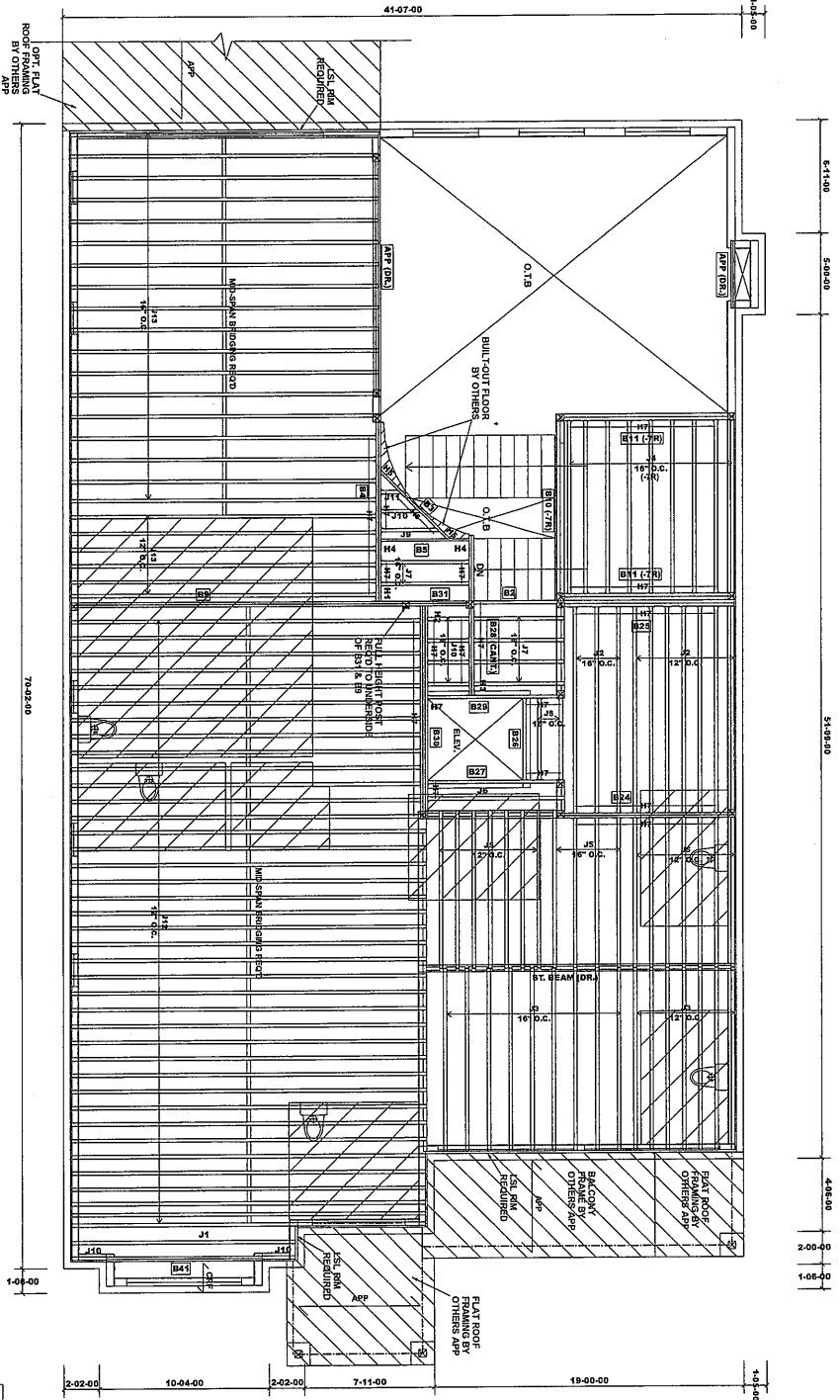
FLOOR LOADING	
LIVE LOAD: 40 PSF	
DEAD LOAD: 10 PSF	
DEAD LOAD (TILE): 20 PSF	

HATCH LEGEND	
Ceramic Tile	
Conv Framed	

APP - AS PER PLAN	
BBO - BEAM BY OTHERS	
P1 - POC ABOVE BELOW	
GT - GIRDER TRUSS	
RT - ROOF TRUSS	
RIMCOPED	
1-1/8" X 11-7/8" O.S.B	
SUBFLOOR: 3/4" NAILED & GULFED	

Blocking panels are required over all interior supports.	
Squash blocks are required under concentrated loads.	
Ceramic Tile Application as per O.B.C. 9.30.6	
Provide 1x4x12 blocking between concentrated loads (along bearing) and inboard cleave at ends.	
Do not scale - refer to architectural plans for dimensions.	

Posid	Qty	Material	Product
H1	1	H4410	H4410
H2	1	H4410	H4410
H3	1	H4410	H4410
H4	2	H4410	H4410
H5	2	H4410	H4410
H6	2	H4410	H4410
H7	95	H4410	H4410



Posid	Qty	Material	Product
H1	1	H4410	H4410
H2	1	H4410	H4410
H3	1	H4410	H4410
H4	2	H4410	H4410
H5	2	H4410	H4410
H6	2	H4410	H4410
H7	95	H4410	H4410

JT/PL: 45147/116460  
LI: 34307\*

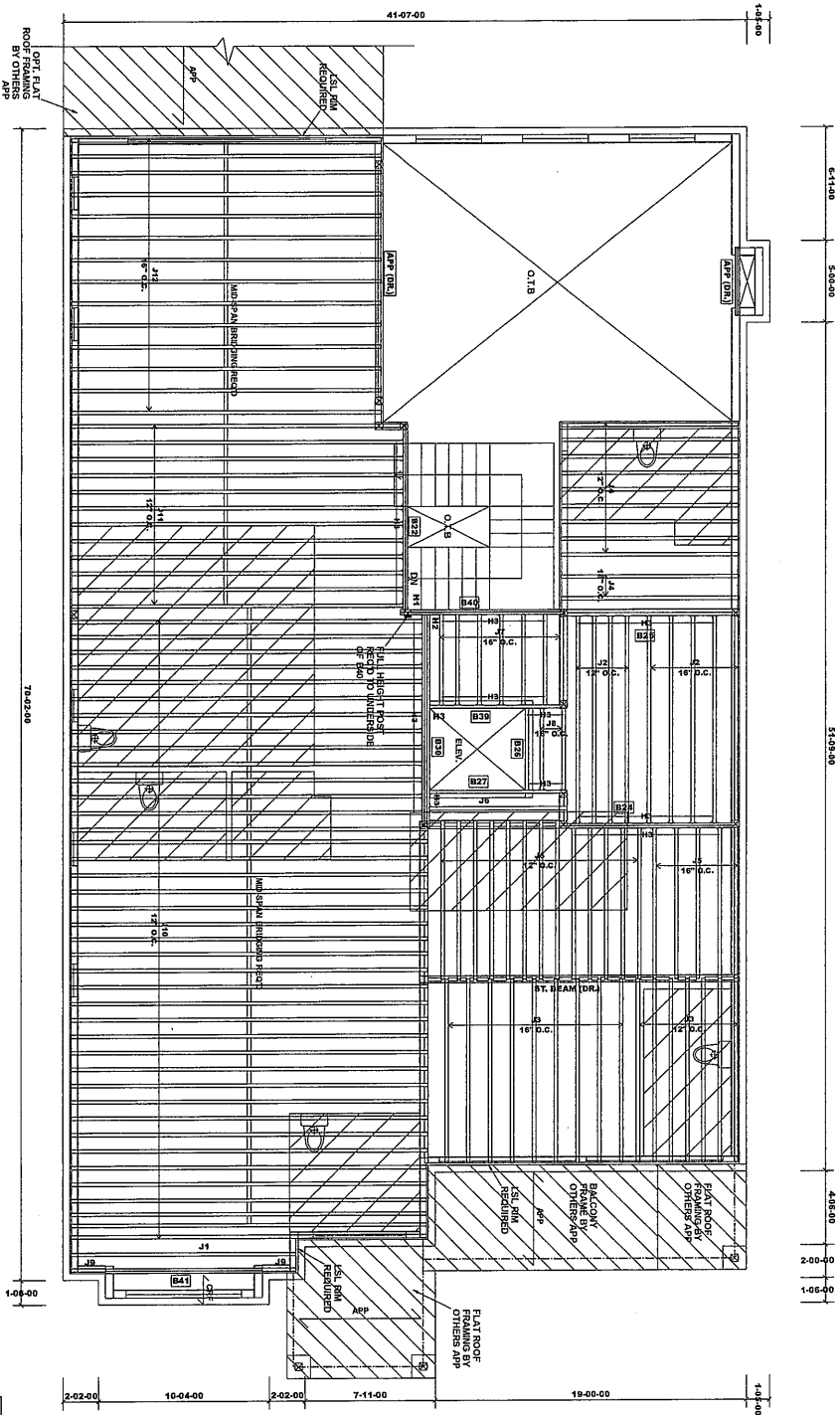
Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 26 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.





SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION B	
W/O LOFT CONDITION	
W/ ELEVATOR	
W/ 5 BEDROOM	

FLOOR LOADING	
LIVE LOAD : 40 PSF	
DEAD LOAD (TILE): 20 PSF	

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN	
BBO - BEAM BY OTHERS	
PA - POST ABOVE	
OTB - OPEN TO BELOW	
RT - ROOF TRUSS	
RIMBOARD	
1-1/8\" X 11-7/8\" OSB	
SUBFLOOR: 3/4\" NAILLED & GULFED	

Blocking panels are required over all interior Squash blocks are required under concentrated loads.

Ceramic Tile Application as per O.S.C. 9.30.6

Provide (a) blocking between cantilevered joists (along bearing) and inboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

FIELD	QTY	UNIT	HOUSE NO.
H1	1		H0510
H2	1		H0510
H3	70		L23118

FIELD	Length	Product	Plate	Qty
B22	12'-00-00	3/4\" X 11/8\" 1.55E Time-Strand LSL	2	2
B23	11'-00-00	11/16\" N-20	2	2
B24	11'-00-00	11/16\" N-20	2	2
B25	5'-00-00	11/16\" N-20	1	1
B26	5'-00-00	11/16\" N-20	1	1
B27	5'-00-00	11/16\" N-20	1	1
B28	5'-00-00	11/16\" N-20	1	1
B29	5'-00-00	11/16\" N-20	1	1
B30	5'-00-00	11/16\" N-20	1	1
B31	5'-00-00	11/16\" N-20	1	1
B32	5'-00-00	11/16\" N-20	1	1
B33	5'-00-00	11/16\" N-20	1	1
B34	5'-00-00	11/16\" N-20	1	1
B35	5'-00-00	11/16\" N-20	1	1
B36	5'-00-00	11/16\" N-20	1	1
B37	5'-00-00	11/16\" N-20	1	1
B38	5'-00-00	11/16\" N-20	1	1
B39	5'-00-00	11/16\" N-20	1	1
B40	5'-00-00	11/16\" N-20	1	1
B41	5'-00-00	11/16\" N-20	1	1
B42	5'-00-00	11/16\" N-20	1	1
B43	5'-00-00	11/16\" N-20	1	1
B44	5'-00-00	11/16\" N-20	1	1
B45	5'-00-00	11/16\" N-20	1	1
B46	5'-00-00	11/16\" N-20	1	1
B47	5'-00-00	11/16\" N-20	1	1
B48	5'-00-00	11/16\" N-20	1	1
B49	5'-00-00	11/16\" N-20	1	1
B50	5'-00-00	11/16\" N-20	1	1
B51	5'-00-00	11/16\" N-20	1	1
B52	5'-00-00	11/16\" N-20	1	1
B53	5'-00-00	11/16\" N-20	1	1
B54	5'-00-00	11/16\" N-20	1	1
B55	5'-00-00	11/16\" N-20	1	1
B56	5'-00-00	11/16\" N-20	1	1
B57	5'-00-00	11/16\" N-20	1	1
B58	5'-00-00	11/16\" N-20	1	1
B59	5'-00-00	11/16\" N-20	1	1
B60	5'-00-00	11/16\" N-20	1	1
B61	5'-00-00	11/16\" N-20	1	1
B62	5'-00-00	11/16\" N-20	1	1
B63	5'-00-00	11/16\" N-20	1	1
B64	5'-00-00	11/16\" N-20	1	1
B65	5'-00-00	11/16\" N-20	1	1
B66	5'-00-00	11/16\" N-20	1	1
B67	5'-00-00	11/16\" N-20	1	1
B68	5'-00-00	11/16\" N-20	1	1
B69	5'-00-00	11/16\" N-20	1	1
B70	5'-00-00	11/16\" N-20	1	1
B71	5'-00-00	11/16\" N-20	1	1
B72	5'-00-00	11/16\" N-20	1	1
B73	5'-00-00	11/16\" N-20	1	1
B74	5'-00-00	11/16\" N-20	1	1
B75	5'-00-00	11/16\" N-20	1	1
B76	5'-00-00	11/16\" N-20	1	1
B77	5'-00-00	11/16\" N-20	1	1
B78	5'-00-00	11/16\" N-20	1	1
B79	5'-00-00	11/16\" N-20	1	1
B80	5'-00-00	11/16\" N-20	1	1
B81	5'-00-00	11/16\" N-20	1	1
B82	5'-00-00	11/16\" N-20	1	1
B83	5'-00-00	11/16\" N-20	1	1
B84	5'-00-00	11/16\" N-20	1	1
B85	5'-00-00	11/16\" N-20	1	1
B86	5'-00-00	11/16\" N-20	1	1
B87	5'-00-00	11/16\" N-20	1	1
B88	5'-00-00	11/16\" N-20	1	1
B89	5'-00-00	11/16\" N-20	1	1
B90	5'-00-00	11/16\" N-20	1	1
B91	5'-00-00	11/16\" N-20	1	1
B92	5'-00-00	11/16\" N-20	1	1
B93	5'-00-00	11/16\" N-20	1	1
B94	5'-00-00	11/16\" N-20	1	1
B95	5'-00-00	11/16\" N-20	1	1
B96	5'-00-00	11/16\" N-20	1	1
B97	5'-00-00	11/16\" N-20	1	1
B98	5'-00-00	11/16\" N-20	1	1
B99	5'-00-00	11/16\" N-20	1	1
B100	5'-00-00	11/16\" N-20	1	1

JT/PL: 45147/116460  
LI: 343077\*

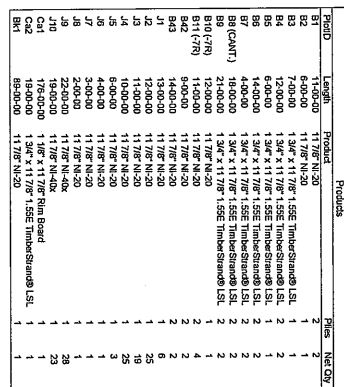
Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 28 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.





Connector Summary			
PartID	Qty	Manuf	Product
H1	3	N/A	H2.5A
H2	2		HGLIS410
H3	2		HUS1.817/0
H4	2		LSR1.817Z
H5	3		LSR2.56Z
H6	78		LT251188

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.

**Ceramic Tile Application as per O.B.C. 9.30.6**

Provide I-joist blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

**FLOOR LOADING:**  
LIVE LOAD : 40 PSF  
DEAD LOAD : 15 PSF  
DEAD LOAD (TILE): 20 PSF

<b>SECOND FLOOR FRAMING</b>
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION

Salesperson: Derek F. Home Lumber Inc.

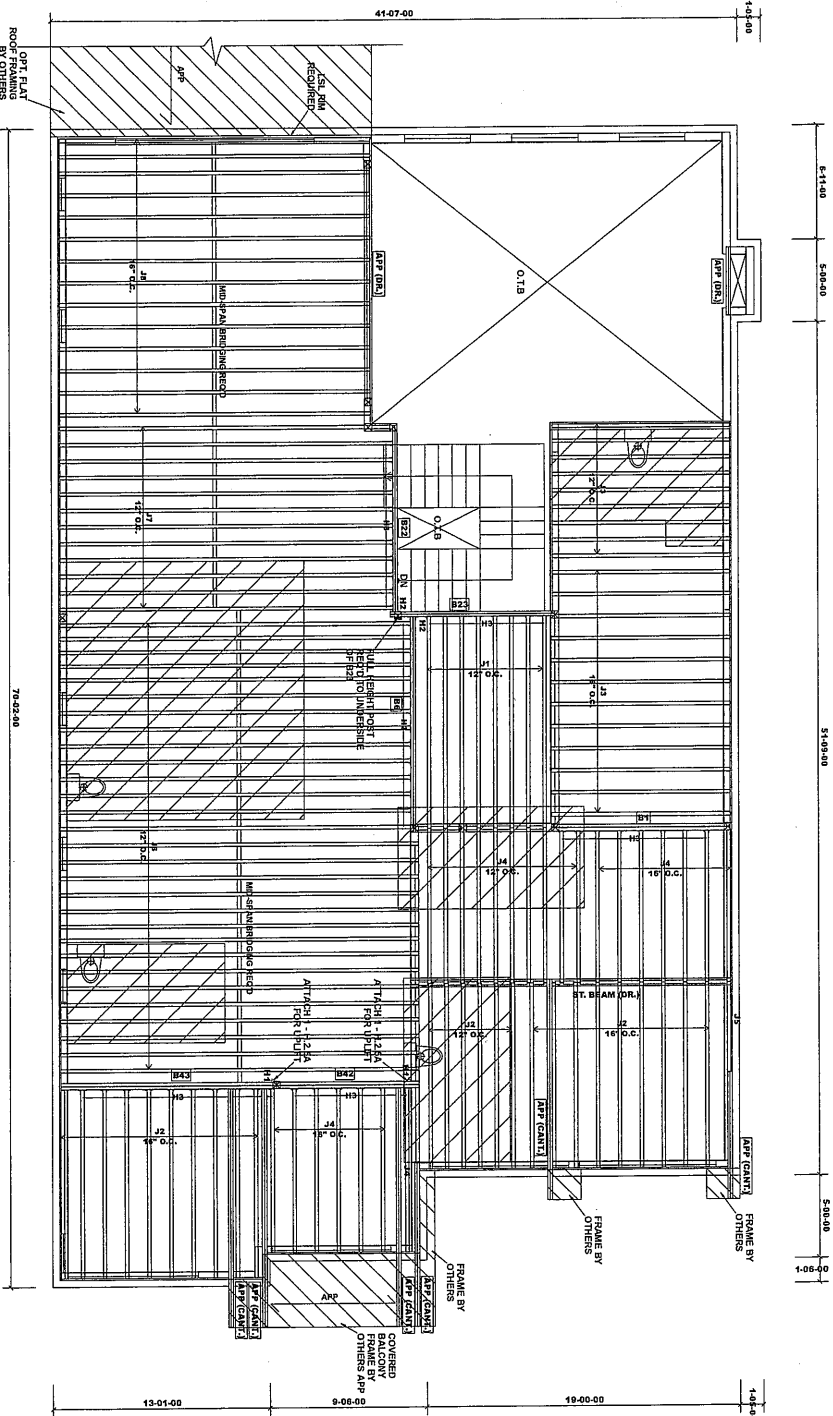
SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
WO/ LOFT CONDITION

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD : 15 PSF
DEAD LOAD (TILE) : 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
GT - GIRDER TRUSS
RT - ROOF TRUSS
RMGBOARD
1-1/8" X 11-7/8" O.S.B
SUBFLOOR: 3/4" NAILED & GLEUED

Blocking panels are required over all interior support.
Squash blocks are required under concentrated loads.
Ceramic Tile Application as per O.B.C. 3.30.6
Provide L-Joist blocking between cantilevered ends (along bearing) and masonry above at ends.
Do not scale - refer to architectural plans for dimensions.



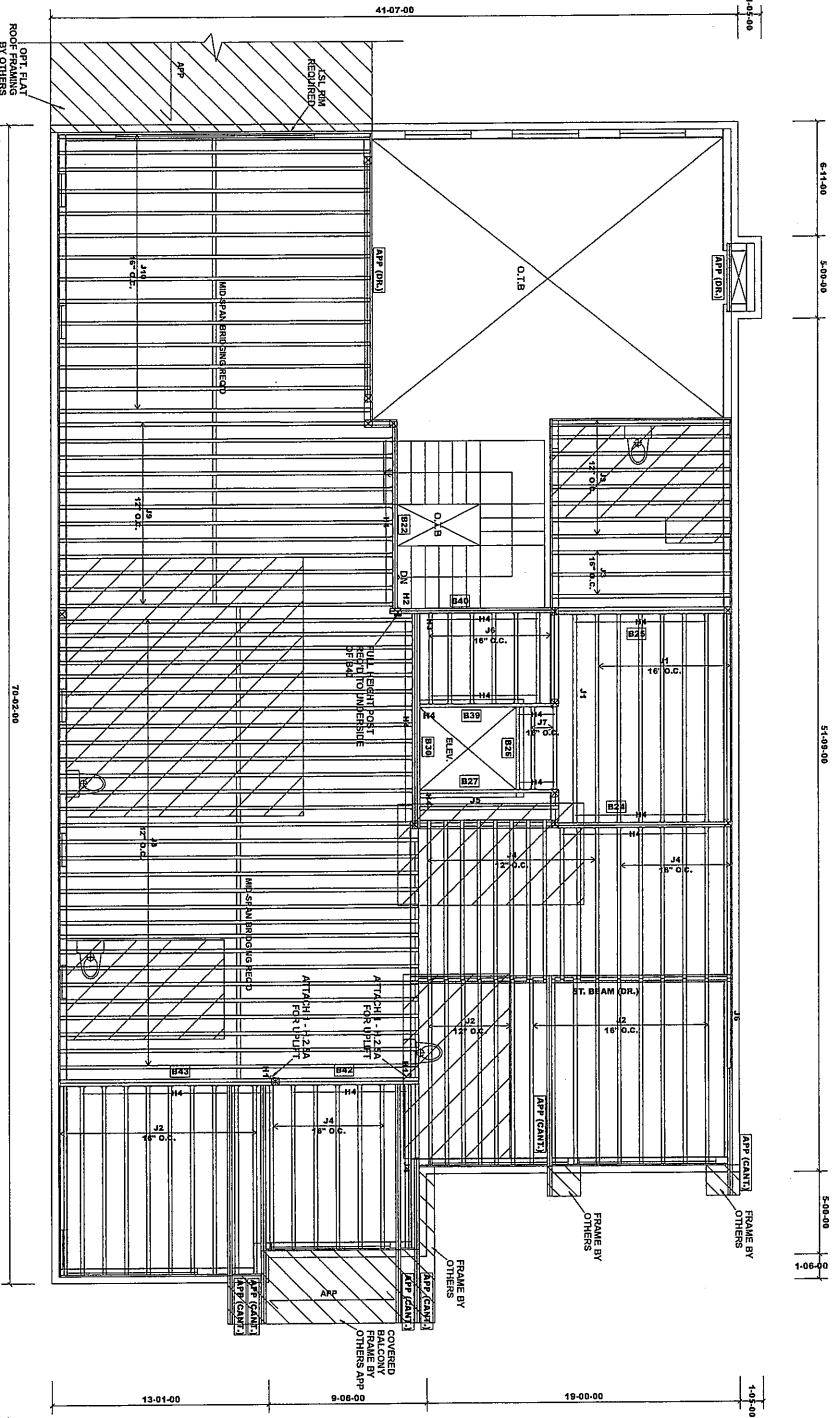
ITEM	Length	Product	Qty	Net Qty
B61	13'-00.00	1 3/4\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B62	12'-00.00	1 3/4\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B63	10'-00.00	1 3/4\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B64	14'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B65	13'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B66	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B67	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B68	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B69	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B70	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B71	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B72	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B73	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B74	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B75	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B76	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B77	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B78	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B79	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B80	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B81	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B82	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B83	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B84	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B85	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B86	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B87	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B88	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B89	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B90	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
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B92	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B93	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B94	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B95	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B96	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B97	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B98	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B99	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2
B100	12'-00.00	1 1/2\" x 11 7/8\" 1.55E Timberstrand LSL	2	2

JT/PL: 45147/116460  
LI: 343077\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May, 06, 2022

Designer: TL  
Sheet: 30 of 36  
Alpa Roof Trusses Inc.  
Stouffville, Ontario  
Salesperson: Derek F.  
Home Lumber Inc.





SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
WO/LOFT CONDITION
W/ ELEVATOR

FLOOR LOADING
LIVE LOAD: 40 PSF
DEAD LOAD: 15 PSF
DEAD LOAD: 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
OT - OPEN TRUSS
RT - ROOF TRUSS
RIMBOARD
1-1/8" X 1-1/8" O.S.B
SUBFLOOR: 3/4" NAILED & GULFED

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.
Ceramic Tile Application as per O.S.C. 3.30.6
Provide 1/2" solid blocking between cantilevered joist (along bearing) and rimboard closure at ends.
Do not scale - refer to architectural plans for dimensions.

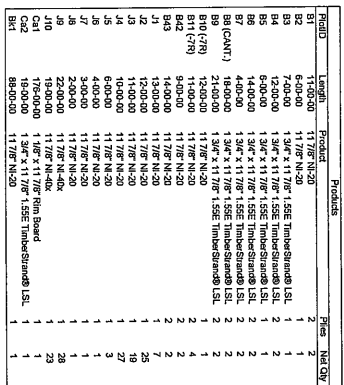
Beam Summary
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Beam 1
Beam 4
Beam 5
Beam 6
Beam 7
Beam 8
Beam 9
Beam 10
Beam 11
Beam 12
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Beam 91
Beam 92
Beam 93
Beam 94
Beam 95
Beam 96
Beam 97
Beam 98
Beam 99
Beam 100

Product	Length	Product	Price	Net Qty
B2	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B3	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B4	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B5	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B6	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B7	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B8	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B9	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
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B43	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
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B45	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B46	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B47	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B48	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B49	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B50	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
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B54	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B55	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B56	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B57	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B58	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B59	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B60	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B61	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B62	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B63	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B64	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B65	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B66	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B67	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B68	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B69	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B70	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B71	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B72	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B73	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B74	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B75	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B76	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B77	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B78	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B79	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B80	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B81	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B82	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B83	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B84	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B85	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B86	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B87	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B88	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B89	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B90	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B91	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B92	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B93	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B94	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B95	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B96	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B97	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B98	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B99	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2
B100	11-00-00	1 3/4" x 11 7/8" 1.5SE TimberStrand® LSL	2	2

JT/PL: 45147/116460  
LI: 343077\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May, 06, 2022

Designer: TL  
Sheet: 32 of 36  
Alpa Roof Trusses Inc.  
Stouffville, Ontario  
Salesperson: Derek F.  
Home Lumber Inc.



Connector Summary			
ProdID	Qty	Manuf	Product
H1	3	N/A	H25A
H2	2		H0US410
H3	2		HU51.8110
H4	2		LSSR1.81Z
H5	3		LSSR2.56Z
H6	80		L725188

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
O.T.B - OPEN TO BELOW  
GT - GIRDER TRUSS  
RT - ROOF TRUSS

RIMBOARD  
1-1/8" X 11-7/8" O.S.B

SUBFLOOR: 3/4" NAILED & GLUED\*

Blocking panels are required over all interior supports.  
Squash blocks are required under concentrated loads.

**Concrete Tie Application as per O.B.C. 9.30.6**

Provide L-joint blocking between cantilevered joists (along bearing) and rimboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

Salesperson: Derek F.  
Home Lumber Inc.

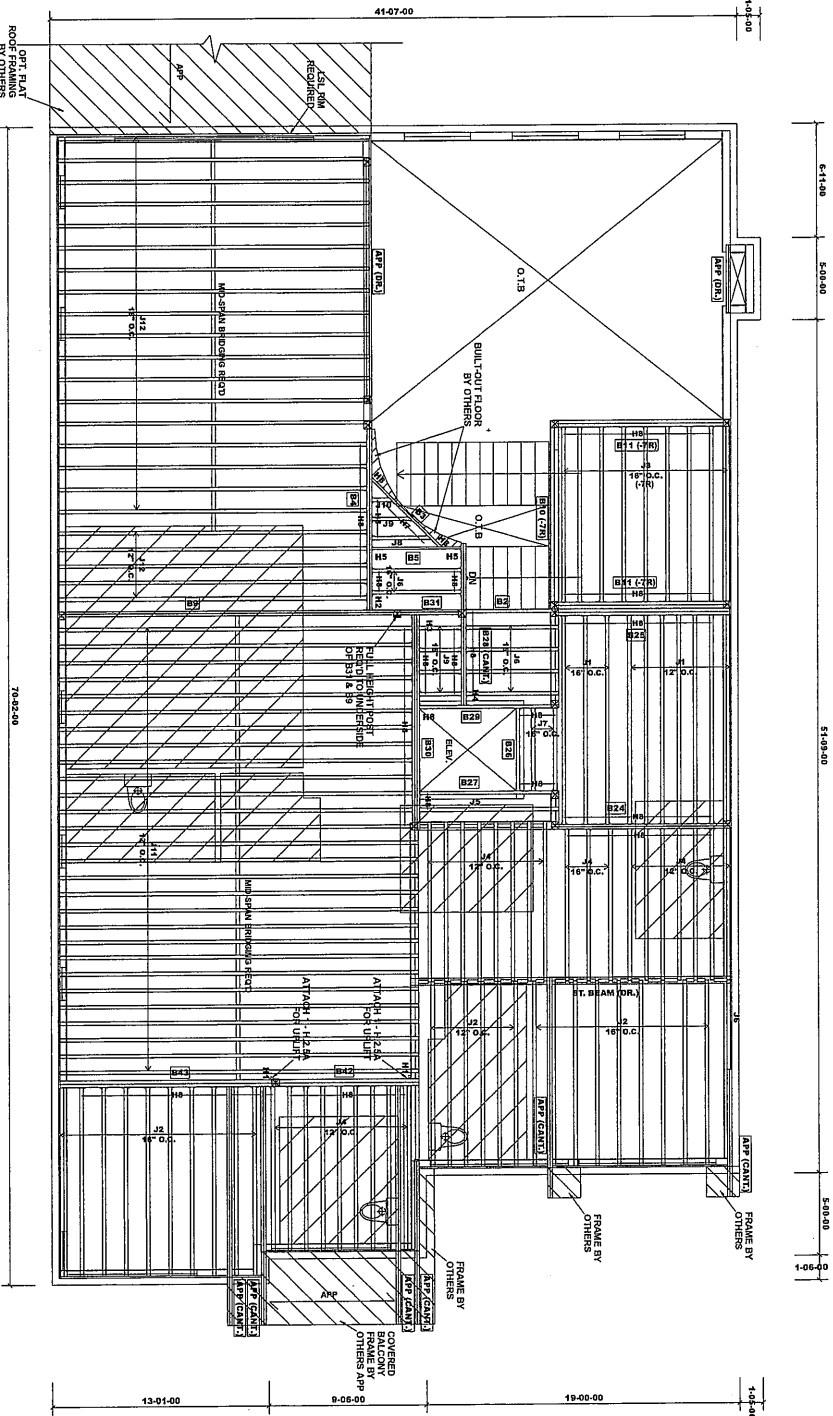
SECOND FLOOR FRAMING
UNIT 5013 - THE RIVERVIEW
ELEVATION C
W/ LOFT CONDITION
W/ ELEVATOR
W/ 5 BEDROOM

FLOOR LOADING  
LIVE LOAD : 40 PSF  
DEAD LOAD (TILE): 20 PSF

HATCH LEGEND
Ceramic Tile
Conv Framed

APP - AS PER PLAN  
BBO - BEAM BY OTHERS  
PA - POST ABOVE  
OT - OPEN TO BELOW  
RT - ROOF TRUSS  
RUMECARD  
1-1/8" X 11-1/8" O.S.B  
SUBFLOOR: 3/4" NAILED & GULFED

Blocking panels are required over all interior support blocks are required under concentrated loads.  
Ceramic Tile Application as per O.B.C. 9.30.6  
Provide L-shaped blocking between cantilevered joists (along bearing) and inboard crosser at ends.  
Do not scale - refer to architectural plans for dimensions.



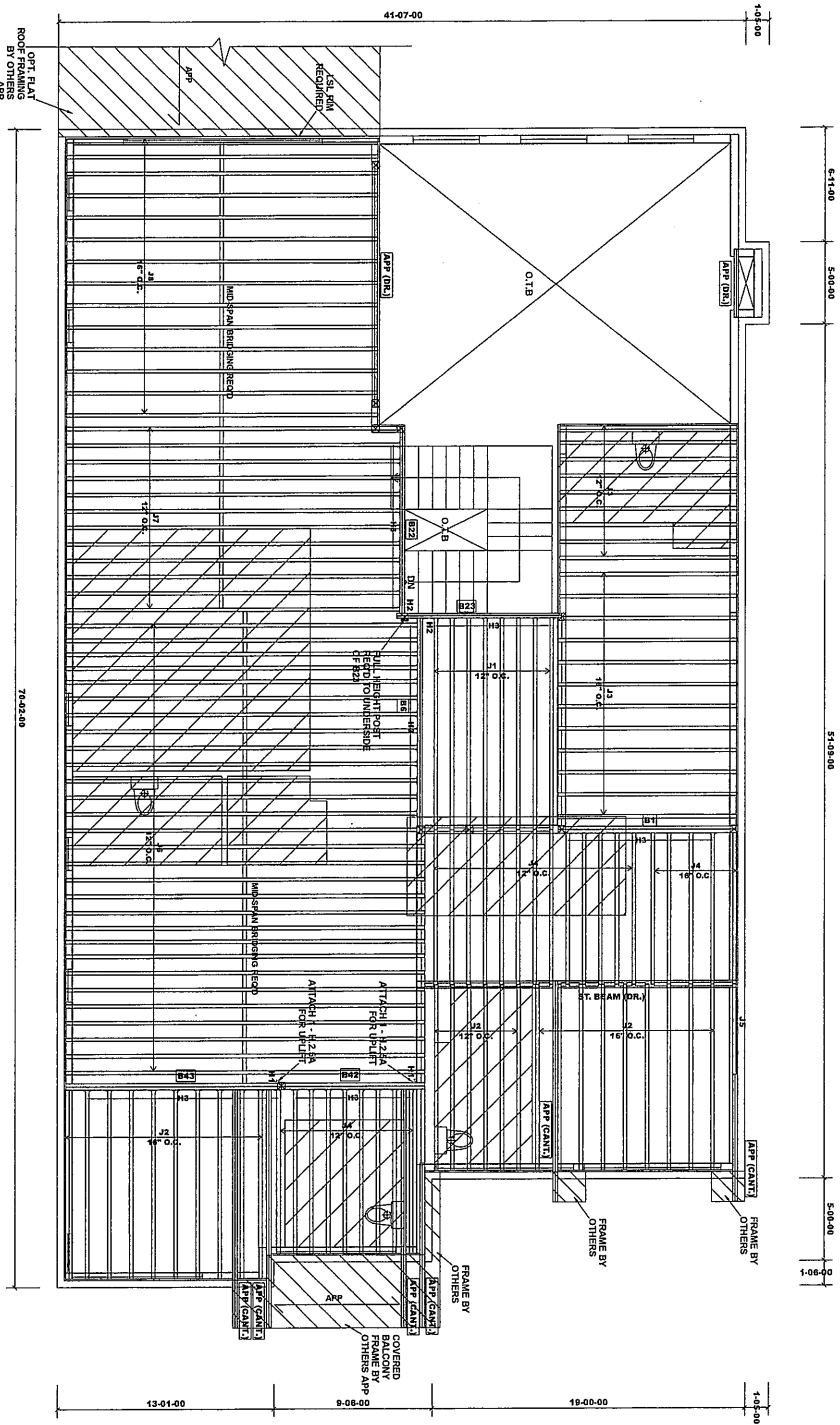
Item	Length	Product	Notes
B2	7'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
B4	12'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
B5	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
B6	12'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
B10 (7/8)	11'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
B24	11'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
B26	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
B27	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
B28 (CANT)	10'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
B30	14'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	3
B31	4'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
B42	9'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	2
J1	13'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J2	13'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J3	13'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J4	13'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J5	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J6	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J7	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J8	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J9	5'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J10	22'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J11	22'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
J12	16'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
C41	19'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1
C42	8'-00-00	1 3/4\" x 11 7/8\" 1.5SE TimberStrand LSL	1

Item	Material	Product
H1	2	H2.2A
H2	1	H2.2A
H3	1	H2.2A
H4	1	H2.2A
H5	2	H2.2A
H6	2	H2.2A
H7	2	H2.2A
H8	2	H2.2A
H9	2	H2.2A
H10	2	H2.2A
H11	2	H2.2A
H12	2	H2.2A

JT/PL: 45147/116460  
LI: 34307\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2  
Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 34 of 36  
Alpa Roof Trusses Inc.  
Stouffville, Ontario  
Salesperson: Derek F.  
Home Lumber Inc.



SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION C	
WO/LOFT CONDITION	
W/ 5 BEDROOM	

FLOORLOADING	
LINE LOAD : 40 PSF	
DEAD LOAD (TILE): 20 PSF	

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN	
BBO - BEAM BY OTHERS	
PA - POST ABOVE	
OT.B - OPEN TO BELOW	
RT - ROOF TRUSS	
RIMBOARD	
1-1/8\" X 11-7/8\" O.S.B	
SUBFLOOR 3/4\" NAILED & GLUED	

Blocking panels are required over all interior Squish blocks are required under concentrated loads.

Ceramic Tile Application as per O.B.C. 9.30.5 Provide 1-Joist blocking between cantilevered joists (along bearing) and imboard closure at ends.

Do not scale - refer to architectural plans for dimensions.

Item	Length	Product	Qty	Unit
B1	11'-00-00	11/7\" N-20	2	2
B2	12'-00-00	1 3/4\" x 11/7\" 1.55E TimberStrand LSL	2	2
B23	10'-00-00	1 3/4\" x 11/7\" 1.55E TimberStrand LSL	2	2
B24	9'-00-00	11/7\" N-20	2	2
J1	13'-00-00	11/7\" N-20	1	1
J2	12'-00-00	11/7\" N-20	1	1
J3	11'-00-00	11/7\" N-20	1	1
J4	10'-00-00	11/7\" N-20	1	1
J5	9'-00-00	11/7\" N-20	1	1
J6	8'-00-00	11/7\" N-20	1	1
J7	7'-00-00	11/7\" N-20	1	1
J8	6'-00-00	11/7\" N-20	1	1
J9	5'-00-00	11/7\" N-20	1	1
J10	4'-00-00	11/7\" N-20	1	1
J11	3'-00-00	11/7\" N-20	1	1
J12	2'-00-00	11/7\" N-20	1	1
J13	1'-00-00	11/7\" N-20	1	1
J14	0'-00-00	11/7\" N-20	1	1
J15	0'-00-00	11/7\" N-20	1	1
J16	0'-00-00	11/7\" N-20	1	1
J17	0'-00-00	11/7\" N-20	1	1
J18	0'-00-00	11/7\" N-20	1	1
J19	0'-00-00	11/7\" N-20	1	1
J20	0'-00-00	11/7\" N-20	1	1
J21	0'-00-00	11/7\" N-20	1	1
J22	0'-00-00	11/7\" N-20	1	1
J23	0'-00-00	11/7\" N-20	1	1
J24	0'-00-00	11/7\" N-20	1	1
J25	0'-00-00	11/7\" N-20	1	1
J26	0'-00-00	11/7\" N-20	1	1
J27	0'-00-00	11/7\" N-20	1	1
J28	0'-00-00	11/7\" N-20	1	1
J29	0'-00-00	11/7\" N-20	1	1
J30	0'-00-00	11/7\" N-20	1	1
J31	0'-00-00	11/7\" N-20	1	1
J32	0'-00-00	11/7\" N-20	1	1
J33	0'-00-00	11/7\" N-20	1	1
J34	0'-00-00	11/7\" N-20	1	1
J35	0'-00-00	11/7\" N-20	1	1
J36	0'-00-00	11/7\" N-20	1	1
J37	0'-00-00	11/7\" N-20	1	1
J38	0'-00-00	11/7\" N-20	1	1
J39	0'-00-00	11/7\" N-20	1	1
J40	0'-00-00	11/7\" N-20	1	1
J41	0'-00-00	11/7\" N-20	1	1
J42	0'-00-00	11/7\" N-20	1	1
J43	0'-00-00	11/7\" N-20	1	1
J44	0'-00-00	11/7\" N-20	1	1
J45	0'-00-00	11/7\" N-20	1	1
J46	0'-00-00	11/7\" N-20	1	1
J47	0'-00-00	11/7\" N-20	1	1
J48	0'-00-00	11/7\" N-20	1	1
J49	0'-00-00	11/7\" N-20	1	1
J50	0'-00-00	11/7\" N-20	1	1
J51	0'-00-00	11/7\" N-20	1	1
J52	0'-00-00	11/7\" N-20	1	1
J53	0'-00-00	11/7\" N-20	1	1
J54	0'-00-00	11/7\" N-20	1	1
J55	0'-00-00	11/7\" N-20	1	1
J56	0'-00-00	11/7\" N-20	1	1
J57	0'-00-00	11/7\" N-20	1	1
J58	0'-00-00	11/7\" N-20	1	1
J59	0'-00-00	11/7\" N-20	1	1
J60	0'-00-00	11/7\" N-20	1	1
J61	0'-00-00	11/7\" N-20	1	1
J62	0'-00-00	11/7\" N-20	1	1
J63	0'-00-00	11/7\" N-20	1	1
J64	0'-00-00	11/7\" N-20	1	1
J65	0'-00-00	11/7\" N-20	1	1
J66	0'-00-00	11/7\" N-20	1	1
J67	0'-00-00	11/7\" N-20	1	1
J68	0'-00-00	11/7\" N-20	1	1
J69	0'-00-00	11/7\" N-20	1	1
J70	0'-00-00	11/7\" N-20	1	1
J71	0'-00-00	11/7\" N-20	1	1
J72	0'-00-00	11/7\" N-20	1	1
J73	0'-00-00	11/7\" N-20	1	1
J74	0'-00-00	11/7\" N-20	1	1
J75	0'-00-00	11/7\" N-20	1	1
J76	0'-00-00	11/7\" N-20	1	1
J77	0'-00-00	11/7\" N-20	1	1
J78	0'-00-00	11/7\" N-20	1	1
J79	0'-00-00	11/7\" N-20	1	1
J80	0'-00-00	11/7\" N-20	1	1
J81	0'-00-00	11/7\" N-20	1	1
J82	0'-00-00	11/7\" N-20	1	1
J83	0'-00-00	11/7\" N-20	1	1
J84	0'-00-00	11/7\" N-20	1	1
J85	0'-00-00	11/7\" N-20	1	1
J86	0'-00-00	11/7\" N-20	1	1
J87	0'-00-00	11/7\" N-20	1	1
J88	0'-00-00	11/7\" N-20	1	1
J89	0'-00-00	11/7\" N-20	1	1
J90	0'-00-00	11/7\" N-20	1	1
J91	0'-00-00	11/7\" N-20	1	1
J92	0'-00-00	11/7\" N-20	1	1
J93	0'-00-00	11/7\" N-20	1	1
J94	0'-00-00	11/7\" N-20	1	1
J95	0'-00-00	11/7\" N-20	1	1
J96	0'-00-00	11/7\" N-20	1	1
J97	0'-00-00	11/7\" N-20	1	1
J98	0'-00-00	11/7\" N-20	1	1
J99	0'-00-00	11/7\" N-20	1	1
J100	0'-00-00	11/7\" N-20	1	1

Item	Qty	Unit	Product
H1	2	NA	12.5A
H2	2	NA	12.5A
H3	2	NA	12.5A
H4	2	NA	12.5A
H5	2	NA	12.5A
H6	2	NA	12.5A
H7	2	NA	12.5A
H8	2	NA	12.5A
H9	2	NA	12.5A
H10	2	NA	12.5A
H11	2	NA	12.5A
H12	2	NA	12.5A
H13	2	NA	12.5A
H14	2	NA	12.5A
H15	2	NA	12.5A
H16	2	NA	12.5A
H17	2	NA	12.5A
H18	2	NA	12.5A
H19	2	NA	12.5A
H20	2	NA	12.5A
H21	2	NA	12.5A
H22	2	NA	12.5A
H23	2	NA	12.5A
H24	2	NA	12.5A
H25	2	NA	12.5A
H26	2	NA	12.5A
H27	2	NA	12.5A
H28	2	NA	12.5A
H29	2	NA	12.5A
H30	2	NA	12.5A
H31	2	NA	12.5A
H32	2	NA	12.5A
H33	2	NA	12.5A
H34	2	NA	12.5A
H35	2	NA	12.5A
H36	2	NA	12.5A
H37	2	NA	12.5A
H38	2	NA	12.5A
H39	2	NA	12.5A
H40	2	NA	12.5A
H41	2	NA	12.5A
H42	2	NA	12.5A
H43	2	NA	12.5A
H44	2	NA	12.5A
H45	2	NA	12.5A
H46	2	NA	12.5A
H47	2	NA	12.5A
H48	2	NA	12.5A
H49	2	NA	12.5A
H50	2	NA	12.5A
H51	2	NA	12.5A
H52	2	NA	12.5A
H53	2	NA	12.5A
H54	2	NA	12.5A
H55	2	NA	12.5A
H56	2	NA	12.5A
H57	2	NA	12.5A
H58	2	NA	12.5A
H59	2	NA	12.5A
H60	2	NA	12.5A
H61	2	NA	12.5A
H62	2	NA	12.5A
H63	2	NA	12.5A
H64	2	NA	12.5A
H65	2	NA	12.5A
H66	2	NA	12.5A
H67	2	NA	12.5A
H68	2	NA	12.5A
H69	2	NA	12.5A
H70	2	NA	12.5A
H71	2	NA	12.5A
H72	2	NA	12.5A
H73	2	NA	12.5A
H74	2	NA	12.5A
H75	2	NA	12.5A
H76	2	NA	12.5A
H77	2	NA	12.5A
H78	2	NA	12.5A
H79	2	NA	12.5A
H80	2	NA	12.5A
H81	2	NA	12.5A
H82	2	NA	12.5A
H83	2	NA	12.5A
H84	2	NA	12.5A
H85	2	NA	12.5A
H86	2	NA	12.5A
H87	2	NA	12.5A
H88	2	NA	12.5A
H89	2	NA	12.5A
H90	2	NA	12.5A
H91	2	NA	12.5A
H92	2	NA	12.5A
H93	2	NA	12.5A
H94	2	NA	12.5A
H95	2	NA	12.5A
H96	2	NA	12.5A
H97	2	NA	12.5A
H98	2	NA	12.5A
H99	2	NA	12.5A
H100	2	NA	12.5A

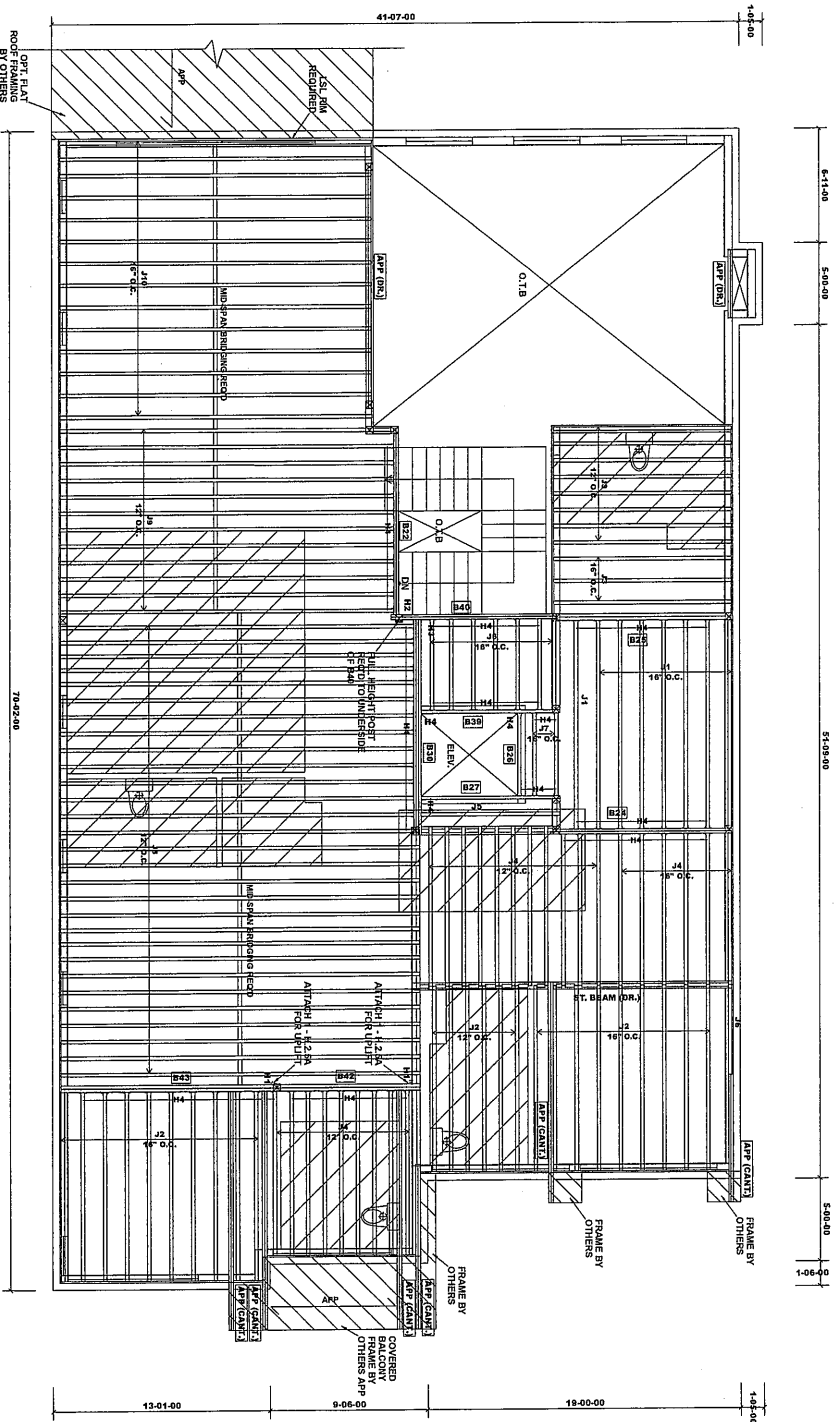
JT/PL: 45147/116460  
LI: 34307\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 35 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.



SECOND FLOOR FRAMING	
UNIT 5013 - THE RIVERVIEW	
ELEVATION C	
WO/LOFT CONDITION	
W/ ELEVATOR	
W/ 5 BEDROOM	

FLOOR LOADING
LIVE LOAD : 40 PSF
DEAD LOAD (TYP) : 20 PSF

HATCH LEGEND	
	Ceramic Tile
	Conv Framed

APP - AS PER PLAN
BBO - BEAM BY OTHERS
PA - POST ABOVE
RT - ROOF TRUSS
RT - ROOF TRUSS
RUMBOOD
117/8" X 117/8" O.S.B
SUBFLOOR, 3/4" VAILED & GULFED

Blocking panels are required over all interior supports.
Squash blocks are required under concentrated loads.
Ceramic Tile Application as per O.B.C. 9.20.6
Provide 1x6 blocking between cantilevered ends (along bearing) and inboard corner all ends.
Do not scale - refer to architectural plans for dimensions.

Summary	
HT	2
H2	1
H3	1
H4	1
H5	1

Item	Length	Product	Spec	Qty
B24	11'-00.00	117/8" x 117/8" 1.5SE TimberStrand LSL	2	2
B25	11'-00.00	117/8" N-20	2	2
B26	5'-00.00	117/8" N-20	2	2
B27	5'-00.00	117/8" N-20	2	2
B28	5'-00.00	117/8" N-20	2	2
B29	5'-00.00	117/8" N-20	2	2
B30	5'-00.00	117/8" N-20	2	2
B31	5'-00.00	117/8" N-20	2	2
B32	5'-00.00	117/8" N-20	2	2
B33	5'-00.00	117/8" N-20	2	2
B34	5'-00.00	117/8" N-20	2	2
B35	5'-00.00	117/8" N-20	2	2
B36	5'-00.00	117/8" N-20	2	2
B37	5'-00.00	117/8" N-20	2	2
B38	5'-00.00	117/8" N-20	2	2
B39	5'-00.00	117/8" N-20	2	2
B40	5'-00.00	117/8" N-20	2	2
B41	5'-00.00	117/8" N-20	2	2
B42	5'-00.00	117/8" N-20	2	2
B43	5'-00.00	117/8" N-20	2	2
B44	5'-00.00	117/8" N-20	2	2
B45	5'-00.00	117/8" N-20	2	2
B46	5'-00.00	117/8" N-20	2	2
B47	5'-00.00	117/8" N-20	2	2
B48	5'-00.00	117/8" N-20	2	2
B49	5'-00.00	117/8" N-20	2	2
B50	5'-00.00	117/8" N-20	2	2
B51	5'-00.00	117/8" N-20	2	2
B52	5'-00.00	117/8" N-20	2	2
B53	5'-00.00	117/8" N-20	2	2
B54	5'-00.00	117/8" N-20	2	2
B55	5'-00.00	117/8" N-20	2	2
B56	5'-00.00	117/8" N-20	2	2
B57	5'-00.00	117/8" N-20	2	2
B58	5'-00.00	117/8" N-20	2	2
B59	5'-00.00	117/8" N-20	2	2
B60	5'-00.00	117/8" N-20	2	2
B61	5'-00.00	117/8" N-20	2	2
B62	5'-00.00	117/8" N-20	2	2
B63	5'-00.00	117/8" N-20	2	2
B64	5'-00.00	117/8" N-20	2	2
B65	5'-00.00	117/8" N-20	2	2
B66	5'-00.00	117/8" N-20	2	2
B67	5'-00.00	117/8" N-20	2	2
B68	5'-00.00	117/8" N-20	2	2
B69	5'-00.00	117/8" N-20	2	2
B70	5'-00.00	117/8" N-20	2	2
B71	5'-00.00	117/8" N-20	2	2
B72	5'-00.00	117/8" N-20	2	2
B73	5'-00.00	117/8" N-20	2	2
B74	5'-00.00	117/8" N-20	2	2
B75	5'-00.00	117/8" N-20	2	2
B76	5'-00.00	117/8" N-20	2	2
B77	5'-00.00	117/8" N-20	2	2
B78	5'-00.00	117/8" N-20	2	2
B79	5'-00.00	117/8" N-20	2	2
B80	5'-00.00	117/8" N-20	2	2
B81	5'-00.00	117/8" N-20	2	2
B82	5'-00.00	117/8" N-20	2	2
B83	5'-00.00	117/8" N-20	2	2
B84	5'-00.00	117/8" N-20	2	2
B85	5'-00.00	117/8" N-20	2	2
B86	5'-00.00	117/8" N-20	2	2
B87	5'-00.00	117/8" N-20	2	2
B88	5'-00.00	117/8" N-20	2	2
B89	5'-00.00	117/8" N-20	2	2
B90	5'-00.00	117/8" N-20	2	2
B91	5'-00.00	117/8" N-20	2	2
B92	5'-00.00	117/8" N-20	2	2
B93	5'-00.00	117/8" N-20	2	2
B94	5'-00.00	117/8" N-20	2	2
B95	5'-00.00	117/8" N-20	2	2
B96	5'-00.00	117/8" N-20	2	2
B97	5'-00.00	117/8" N-20	2	2
B98	5'-00.00	117/8" N-20	2	2
B99	5'-00.00	117/8" N-20	2	2
B100	5'-00.00	117/8" N-20	2	2

JT/PL: 45147/116460  
LI: 343077\*

Builder: Gold Park Homes  
Project: Pine Valley Ph2

Location: Vaughan, ON  
Date: May. 06, 2022

Designer: TL  
Sheet: 36 of 36  
Stouffville, Ontario

Salesperson: Derek F.  
Home Lumber Inc.





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1**  
Level: **Second Floor**  
Label: **B1 - i50389**  
Type: **Beam**

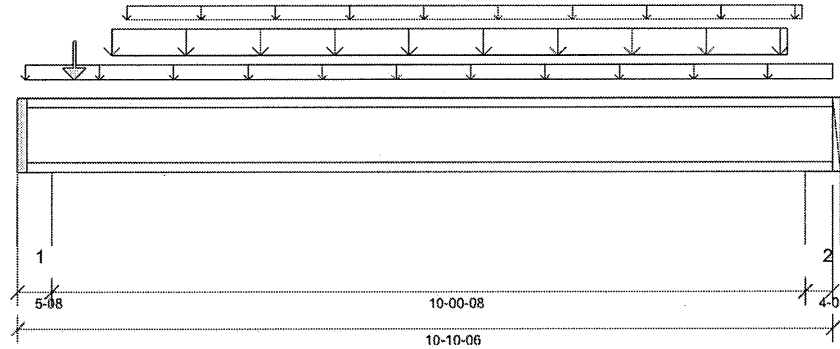
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:05



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 10'- 7"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 9 1/8"	1.25D + 1.5L	1.00	5751 lb ft	11160 lb ft	Passed - 52%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	1.00	2310 lb	4480 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	5'- 5 3/4"	L		0.122"	L/360	Passed - L/990
Total Load (TL) Pos. Defl.:	5'- 5 3/4"	D + L		0.182"	L/240	Passed - L/663

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	2326 lb		4480 lb	16918 lb	Passed - 52%
2	4-06	1.25D + 1.5L	1.00	2135 lb		4480 lb	13458 lb	Passed - 48%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 10 3/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 1 1/4"	10'- 10 3/8"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	1'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Front	87 lb/ft	189 lb/ft	-	-
Uniform	1'- 5 1/2"	10'- 5 1/2"	FC3 Floor Decking (Plan View Fill)	Top	2 lb/ft	-	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J4(i50378)	Front	82 lb	216 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	9(i41702)	528 lb	1111 lb	-	-
2	10'- 6"	10'- 10 3/8"	E25(i41701)	500 lb	1007 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



53046658



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B2 - i50409**  
Type: **Beam**

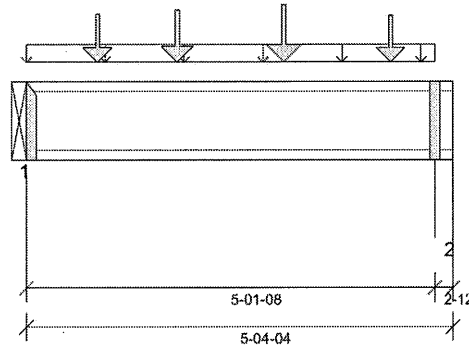
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:06



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 5'- 2 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 9 9/16"	1.25D + 1.5L	1.00	1932 lb ft	5580 lb ft	Passed - 35%
Factored Shear:	5'- 1 7/16"	1.25D + 1.5L	1.00	1392 lb	2240 lb	Passed - 62%
Live Load (LL) Pos. Defl.:	2'- 7 1/8"	L		0.030"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 7 3/16"	D + L		0.048"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	1359 lb		1970 lb	-	Passed - 69%
2	2-12	1.25D + 1.5L	1.00	1396 lb		2090 lb	4230 lb	Passed - 67%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.						

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 4 1/4"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'- 1 1/2"	User Load	Top	60 lb/ft	-	-	-
Point	0'- 10 3/4"	0'- 10 3/4"	J2(i50074)	Front	105 lb	269 lb	-	-
Point	1'- 10 3/4"	1'- 10 3/4"	J2(i50075)	Front	115 lb	307 lb	-	-
Point	3'- 2 3/4"	3'- 2 3/4"	J2(i50504)	Front	132 lb	351 lb	-	-
Point	4'- 6 3/4"	4'- 6 3/4"	J2(i50340)	Front	99 lb	263 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B8 (CANT.)(i50483)	386 lb	584 lb	-	-
2	5'- 1 1/2"	5'- 4 1/4"	9(i41702)	388 lb	607 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



CG046659



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B3 - i50326**  
Type: **Beam**

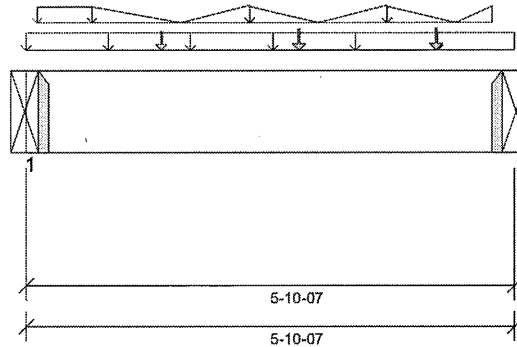
**1 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:06



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'- 1 3/4" Bottom: 1'- 5 1/8"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 10 7/16"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 3 3/8"	1.25D + 1.5L	1.00	585 lb ft	13266 lb ft	Passed - 4%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	256 lb	7207 lb	Passed - 4%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	332 lb		3440 lb	-	Passed - 10%
2	1-08	1.25D + 1.5L	1.00	397 lb		3440 lb	-	Passed - 12%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LSSR1.81Z		-	-	-	Connector manually specified by the user.
2	LSSR1.81Z		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	5'- 10 7/16"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	-0'	5'- 10 7/16"	User Load	Top	10 lb/ft	27 lb/ft	-	-
Tapered	0'- 1 5/8"	0'- 9 1/2"	FC3 Floor Decking (Plan View Fill)	Top	2 To 4 lb/ft	4 To 11 lb/ft	-	-
Tapered	0'- 9 1/2"	1'- 10 3/8"	FC3 Floor Decking (Plan View Fill)	Top	4 To 0 lb/ft	11 To 0 lb/ft	-	-
Tapered	1'- 10 3/8"	2'- 8 1/4"	FC3 Floor Decking (Plan View Fill)	Top	0 To 12 lb/ft	0 To 33 lb/ft	-	-
Tapered	2'- 8 1/4"	3'- 6 3/16"	FC3 Floor Decking (Plan View Fill)	Top	6 To 0 lb/ft	17 To 0 lb/ft	-	-
Tapered	3'- 6 3/16"	4'- 4 1/16"	FC3 Floor Decking (Plan View Fill)	Top	0 To 12 lb/ft	0 To 33 lb/ft	-	-
Tapered	4'- 4 1/16"	5'- 1 15/16"	FC3 Floor Decking (Plan View Fill)	Top	6 To 0 lb/ft	17 To 0 lb/ft	-	-
Tapered	5'- 1 15/16"	5'- 7 1/4"	FC3 Floor Decking (Plan View Fill)	Top	0 To 7 lb/ft	0 To 18 lb/ft	-	-
Point	1'- 7 9/16"	1'- 7 9/16"	J8(i50020)	Front	10 lb	28 lb	-	-
Point	3'- 3 3/8"	3'- 3 3/8"	J7(i50303)	Front	18 lb	49 lb	-	-
Point	4'- 11 1/8"	4'- 11 1/8"	J6(i50022)	Front	20 lb	53 lb	-	-

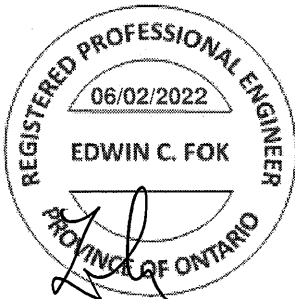
#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	-0'	0'- 1 3/4"	B4(i49924)	76 lb	153 lb	-	-
2	5'- 8 11/16"	5'- 10 7/16"	B5(i50292)	90 lb	192 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

53046660





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B4 - i49924**  
Type: **Beam**

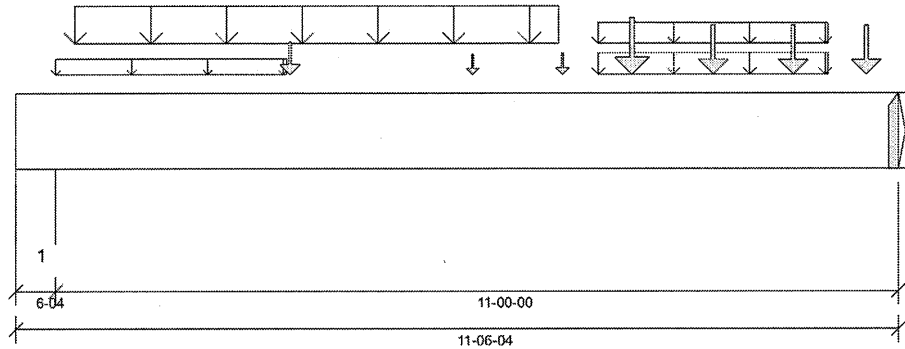
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:07



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 5 1/4"
- 769 psi Beam @ 11'- 6 1/4"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 9" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 11 5/8"	1.25D + 1.5L	1.00	14090 lb ft	26531 lb ft	Passed - 53%
Factored Shear:	10'- 6 3/8"	1.25D + 1.5L	1.00	4544 lb	14414 lb	Passed - 32%
Live Load (LL) Pos. Defl.:	6'	L		0.211"	L/360	Passed - L/627
Total Load (TL) Pos. Defl.:	6'- 1/16"	D + L		0.300"	L/240	Passed - L/439

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	6-04	1.25D + 1.5L	1.00	4683 lb		28665 lb	13458 lb	Passed - 35%
2	1-08	1.25D + 1.5L	1.00	5160 lb		6880 lb	-	Passed - 75%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	HGUS410		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'- 6 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 6 1/4"	3'- 6 7/16"	User Load	Top	10 lb/ft	27 lb/ft	-	-
Uniform	0'- 9 1/4"	7'- 1 1/16"	Smoothed Load	Front	148 lb/ft	395 lb/ft	-	-
Uniform	7'- 7 1/4"	10'- 7 1/4"	Smoothed Load	Front	169 lb/ft	-	-	-
Tapered	7'- 7 1/4"	10'- 7 1/4"	Smoothed Load	Back	-	163 To 146 lb/ft	-	-
Point	8'- 9/16"	8'- 9/16"	-	Front	84 lb	434 lb	-	-
Point	9'- 1 5/16"	9'- 1 5/16"	-	Front	58 lb	372 lb	-	-
Point	10'- 1 13/16"	10'- 1 13/16"	-	Front	54 lb	372 lb	-	-
Point	11'- 1 1/4"	11'- 1 1/4"	J10(i50070)	Front	131 lb	291 lb	-	-
Point	3'- 6 15/16"	3'- 6 15/16"	B3(i50326)	Back	76 lb	153 lb	-	-
Point	5'- 11 5/8"	5'- 11 5/8"	J7(i50303)	Back	21 lb	56 lb	-	-
Point	7'- 1 5/8"	7'- 1 5/8"	J6(i50022)	Back	24 lb	64 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 6 1/4"	7(i41696)	977 lb	2311 lb	-	-
2	11'- 6 1/4"	11'- 6 1/4"	B9(i49806)	1107 lb	2514 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

SG020666/



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B5 - I50292**  
Type: **Beam**

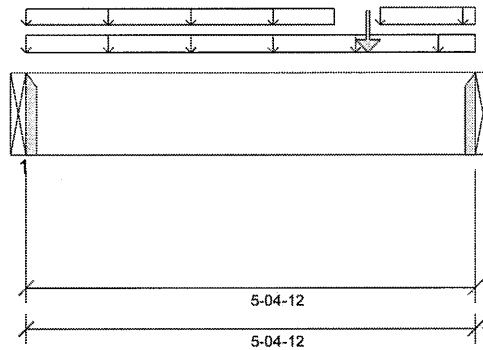
**1 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MITek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:07



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 4'- 9/16"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 4 3/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/16"	1.25D + 1.5L	1.00	630 lb ft	13266 lb ft	Passed - 5%
Factored Shear:	4'- 4 7/8"	1.25D + 1.5L	1.00	446 lb	7207 lb	Passed - 6%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	342 lb		3440 lb	-	Passed - 10%
2	1-08	1.25D + 1.5L	1.00	579 lb		3440 lb	-	Passed - 17%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	HUS1.81/10		-	-	-	Connector manually specified by the user.
2	HUS1.81/10		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 4 3/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	5'- 4 3/4"	-	Top	10 lb/ft	27 lb/ft	-	-
Uniform	0'	3'- 8 7/16"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	12 lb/ft	-	-
Uniform	4'- 3 1/16"	5'- 4 3/4"	FC3 Floor Decking (Plan View Fill)	Top	11 lb/ft	29 lb/ft	-	-
Point	4'- 1 5/16"	4'- 1 5/16"	B3(I50326)	Back	90 lb	192 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B4(I49924)	84 lb	165 lb	-	-
2	5'- 4 3/4"	5'- 4 3/4"	B8 (CANT.)(I50483)	132 lb	270 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



*Signature*





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1**  
Level: **Second Floor**  
Label: **B6 - i50354**  
Type: **Beam**

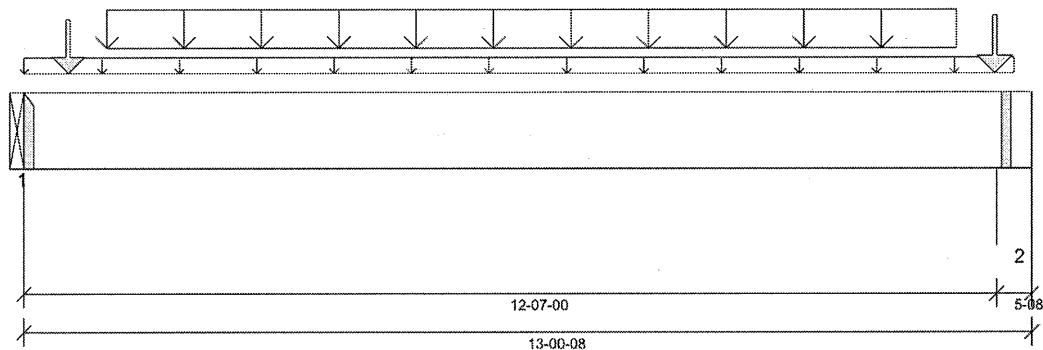
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MITek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:08



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

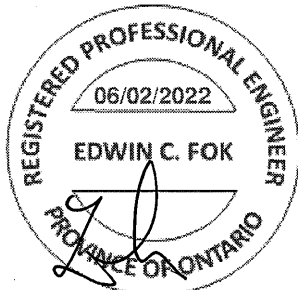
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 12'- 8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 5" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 6 3/4"	1.25D + 1.5L	1.00	18610 lb ft	26531 lb ft	Passed - 70%
Factored Shear:	11'- 7 1/8"	1.25D + 1.5L	1.00	6119 lb	14414 lb	Passed - 42%
Live Load (LL) Pos. Defl.:	6'- 4"	L		0.348"	L/360	Passed - L/433
Total Load (TL) Pos. Defl.:	6'- 4 1/16"	D + L		0.513"	L/240	Passed - L/294
Permanent Deflection:	6'- 4 3/16"			-	L/360	Passed - L/943

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	5674 lb		6880 lb	-	Passed - 82%
2	5-08	1.25D + 1.5L	1.00	6192 lb		25225 lb	11842 lb	Passed - 52%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	HGUS410		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/2"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	12'- 9 3/4"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	1'- 3/4"	12'- 3/4"	Smoothed Load	Front	189 lb/ft	424 lb/ft	-	-
Point	0'- 6 3/4"	0'- 6 3/4"	J9(i50295)	Front	157 lb	362 lb	-	-
Point	12'- 6 3/4"	12'- 6 3/4"	J9(i50177)	Front	166 lb	406 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B7(i50432)	1272 lb	2718 lb	-	-
2	12'- 7"	13'- 1/2"	4(i41664)	1404 lb	2962 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SG046663



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B7 - i50432**  
Type: **Beam**

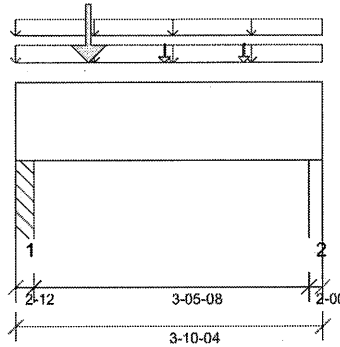
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:08



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 10 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 1 3/4"
- 615 psi Wall @ 3'- 9 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	0'- 11"	1.25D + 1.5L	1.00	3907 lb ft	26531 lb ft	Passed - 15%
Factored Shear:	1'- 2 5/8"	1.25D + 1.5L	1.00	4966 lb	14414 lb	Passed - 34%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	5160 lb		12613 lb	12844 lb	Passed - 41%
2	2-00	1.25D + 1.5L	1.00	2136 lb		9171 lb	4306 lb	Passed - 50%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 10 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Point	0'- 11"	0'- 11"	B6(i50354)	Front	1272 lb	2718 lb	-	-
Point	1'- 10 1/2"	1'- 10 1/2"	J2(i50072)	Front	103 lb	254 lb	-	-
Point	2'- 10 1/2"	2'- 10 1/2"	J2(i50531)	Front	112 lb	275 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	Pt1(i49783)	1243 lb	2389 lb	-	-
2	3'- 8 1/4"	3'- 10 1/4"	10(i41708)	567 lb	967 lb	-	-

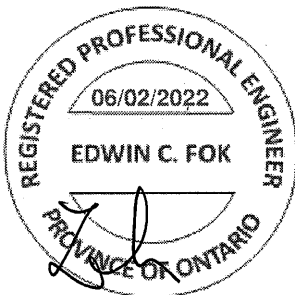
#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- User loads assume a bearing length of 3.5" in determining member capacity for loads near supports.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5667 lb, Qr=9555 lb, Result=59.31%.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 6" O/C  
STAGGERED IN 2 ROWS



SE046664



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1)**  
Level: **Second Floor**  
Label: **B8 (CANT.) - i50483**  
Type: **Beam**

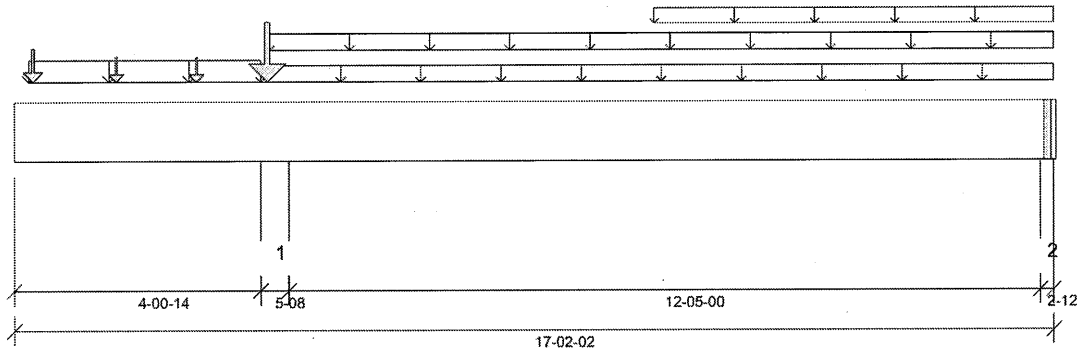
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MITek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:08



### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'-2 7/8" Bottom: 12'-5"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 4'-3 5/8"
- 615 psi Wall @ 17'-3/8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS

### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	11'-10 5/8"	1.25D + 1.5L	0.82	1431 lb ft	21868 lb ft	Passed - 7%
Factored Neg. Moment:	4'-3 5/8"	1.25D + 1.5L	1.00	5448 lb ft	5612 lb ft	Passed - 97%
Factored Shear:	3'-1"	1.25D + 1.5L	1.00	1795 lb	14414 lb	Passed - 12%
Live Load (LL) Neg. Defl.:	9'-8 3/16"	L		0.063"	L/360	Passed - L/999
Total Load (TL) Neg. Defl.:	9'-3 7/16"	D + L		0.069"	L/240	Passed - L/999

### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L	1.00	4479 lb		25225 lb	11843 lb	Passed - 38%
2	2'-12"	1.25D + 1.5L	0.82	573 lb		10395 lb	4880 lb	Passed - 12%
2	2'-12"	0.9D + 1.5L	1.00		-198 lb	-	-	

### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	-0'	17'-2 1/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'-2 7/8"	4'-7/8"	User Load	Top	38 lb/ft	100 lb/ft	-	-
Uniform	4'-7/8"	17'-2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	21 lb/ft	-	-
Uniform	4'-2 5/8"	17'-2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	23 lb/ft	-	-
Uniform	10'-6 7/8"	17'-2 1/8"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	-	-	-
Point	0'-3 3/4"	0'-3 3/4"	B5(i50292)	Front	132 lb	270 lb	-	-
Point	1'-8 1/8"	1'-8 1/8"	J5(i50491)	Front	61 lb	163 lb	-	-
Point	3'-1/8"	3'-1/8"	J5(i50024)	Front	56 lb	148 lb	-	-
Point	4'-2 1/8"	4'-2 1/8"	B2(i50409)	Back	386 lb	584 lb	-	-

### UNFACTORED REACTIONS

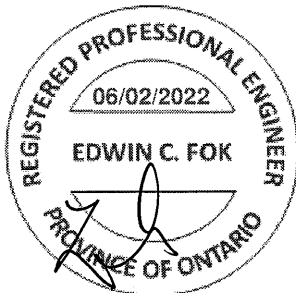
ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	4'-7/8"	4'-6 3/8"	10(i41708)	1129 lb	2032 lb	-	-
2	16'-11 3/8"	17'-2 1/8"	3(i41663)	125 lb	282/197 lb	-	-

### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



3204665





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B9 - i49806**  
Type: **Beam**

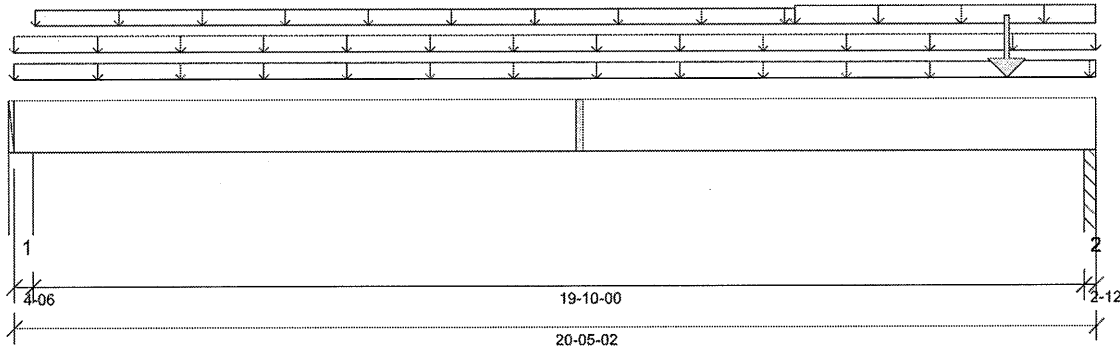
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:09



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

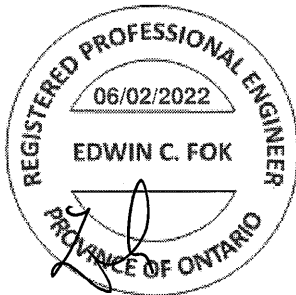
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 10'- 3 3/4"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 3 3/8"
- 1334 psi Column @ 20'- 3 3/8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	15'- 8 13/16"	1.25D + 1.5L	1.00	9437 lb ft	26531 lb ft	Passed - 36%
Factored Shear:	19'- 2 1/2"	1.25D + 1.5L	1.00	5658 lb	14414 lb	Passed - 39%
Live Load (LL) Pos. Defl.:	11'- 2 7/8"	L		0.355"	L/360	Passed - L/670
Total Load (TL) Pos. Defl.:	11'- 1 7/8"	D + L		0.631"	L/240	Passed - L/377
Permanent Deflection:	11'- 9/16"			-	L/360	Passed - L/888

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	1179 lb		20065 lb	9420 lb	Passed - 13%
2	2-12	1.25D + 1.5L	1.00	5861 lb		12613 lb	12844 lb	Passed - 46%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	20'- 5 1/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	20'- 5 1/8"	FC3 Floor Decking (Plan View Fill)	Top	5 lb/ft	14 lb/ft	-	-
Uniform	0'	18'- 8 7/8"	FC3 Floor Decking (Plan View Fill)	Top	4 lb/ft	11 lb/ft	-	-
Uniform	0'- 4 7/8"	14'- 8 7/8"	FC3 Floor Decking (Plan View Fill)	Top	3 lb/ft	-	-	-
Uniform	14'- 8 7/8"	20'- 5 1/8"	User Load	Top	60 lb/ft	-	-	-
Uniform	18'- 8 7/8"	20'- 5 1/8"	FC3 Floor Decking (Plan View Fill)	Top	-	24 lb/ft	-	-
Point	18'- 8 7/8"	18'- 8 7/8"	B4(i49924)	Back	1107 lb	2514 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 4 3/8"	E12(i41614)	393 lb	458 lb	-	-
2	20'- 2 3/8"	20'- 5 1/8"	Pt1(i49783)	1570 lb	2599 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SL046666



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Second Floor**  
Label: **B10 (-7R) - i49990**  
Type: **Beam**

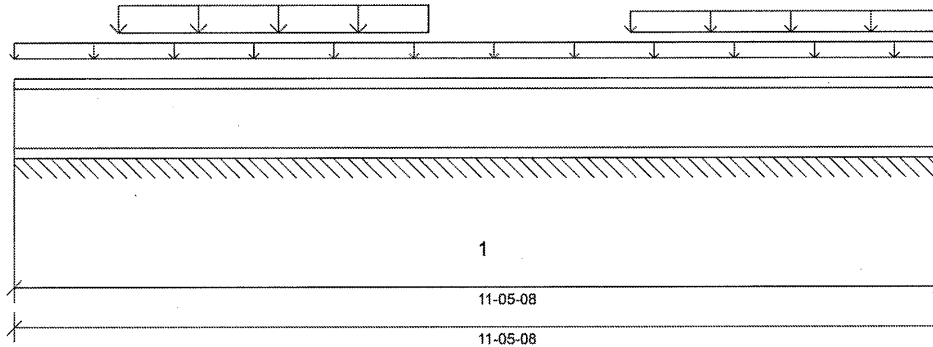
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:09



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit:

TL Deflection Limit:

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'

#### Factored Resistance of Support Material:

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
	N/A	1.25D + 1.5L	1.00	396 lb/ft		3300 lb/ft	-	Passed - 12%
	N/A	1.4D	0.65	59 lb/ft		2145 lb/ft	-	Passed - 3%

#### SPECIFIED LOADS

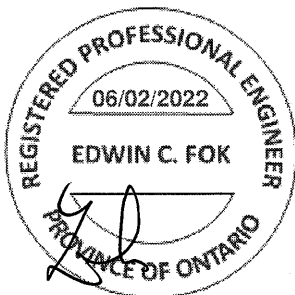
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'- 5 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	11'- 5 1/2"	FC4 Floor Decking (Plan View Fill)	Top	6 lb/ft	15 lb/ft	-	-
Uniform	1'- 3 1/2"	5'- 1 1/2"	User Load	Top	69 lb/ft	184 lb/ft	-	-
Uniform	7'- 7 1/2"	11'- 5 1/2"	FC4 Floor Decking (Plan View Fill)	Top	34 lb/ft	89 lb/ft	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	11'- 5 1/2"	-	189 lb	487 lb	-	-
++>	0'	1'- 9 1/2"	2(i41666)	8 lb/ft	15 lb/ft	-	-
++>	1'- 3 1/2"	1'- 9 1/2"	2(i41666)	69 lb/ft	184 lb/ft	-	-
++>	1'- 9 1/2"	5'- 1 1/2"	5(i41694)	69 lb/ft	184 lb/ft	-	-
++>	1'- 9 1/2"	11'- 5 1/2"	5(i41694)	8 lb/ft	15 lb/ft	-	-
++>	7'- 7 1/2"	11'- 5 1/2"	5(i41694)	34 lb/ft	89 lb/ft	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.



83046667



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1**  
Level: **Second Floor**  
Label: **B11 (-7R) - i49912**  
Type: **Beam**

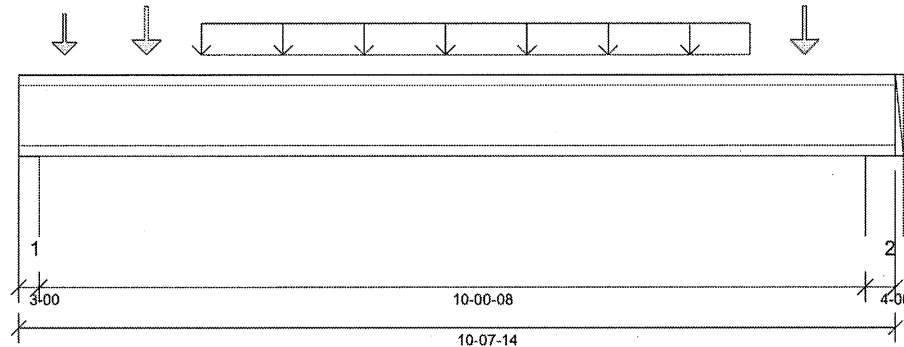
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:10



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 2"
- 615 psi Wall @ 10'- 4 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 6 5/8"	1.25D + 1.5L	1.00	5996 lb ft	11160 lb ft	Passed - 54%
Factored Shear:	0'- 3 1/16"	1.25D + 1.5L	1.00	2350 lb	4480 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	5'- 3 1/4"	L		0.133"	L/360	Passed - L/903
Total Load (TL) Pos. Defl.:	5'- 3 1/4"	D + L		0.187"	L/240	Passed - L/645

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	2352 lb		4240 lb	9228 lb	Passed - 55%
2	4-06	1.25D + 1.5L	1.00	2198 lb		4480 lb	13457 lb	Passed - 49%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 7 7/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	2'- 2 5/8"	8'- 10 5/8"	Smoothed Load	Front	86 lb/ft	230 lb/ft	-	-
Point	0'- 6 5/8"	0'- 6 5/8"	J3(i50278)	Front	78 lb	208 lb	-	-
Point	1'- 6 5/8"	1'- 6 5/8"	J3(i50478)	Front	100 lb	267 lb	-	-
Point	9'- 6 5/8"	9'- 6 5/8"	J3(i50507)	Front	100 lb	267 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	2(i41666)	471 lb	1175 lb	-	-
2	10'- 3 1/2"	10'- 7 7/8"	E24(i41700)	442 lb	1097 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



32046668



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B12 - i50356**  
Type: **Beam**

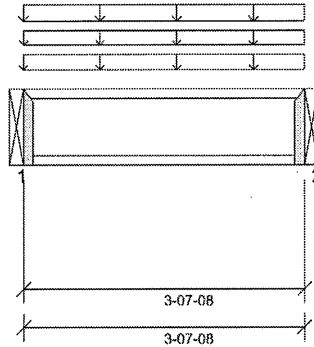
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:10



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 3'- 7 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 3'- 7 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 9 3/4"	1.25D + 1.5L	1.00	164 lb ft	5580 lb ft	Passed - 3%
Factored Shear:	3'- 7 7/16"	1.25D + 1.5L	1.00	181 lb	2240 lb	Passed - 8%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	187 lb		1970 lb	-	Passed - 10%
2	1-12	1.25D + 1.5L	1.00	181 lb		1970 lb	-	Passed - 9%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188	-	-	-	-	Connector manually specified by the user.
2	LT251188	-	-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 7 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	0'	3'- 7 1/2"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	14 lb/ft	-	-
Uniform	0'	3'- 7 1/2"	FC2 Floor Decking (Plan View Fill)	Top	2 lb/ft	-	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B15(i50756)	43 lb	89 lb	-	-
2	3'- 7 1/2"	3'- 7 1/2"	B13(i50410)	41 lb	87 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SE046669



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B13 - i50410**  
Type: **Beam**

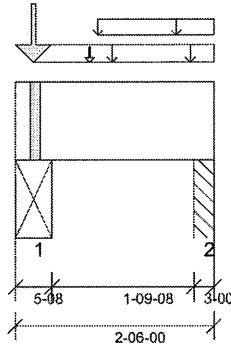
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:10



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

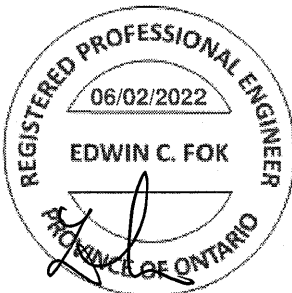
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 5 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 4 1/2"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 4" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Neg. Moment:	0'- 4 1/2"	1.25D + 1.5L	1.00	1146 lb ft	25307 lb ft	Passed - 5%
Factored Moment:	0'- 4 1/2"	1.25D + 1.5L	1.00	1146 lb ft	25307 lb ft	Passed - 5%
Factored Moment:				0 lb ft	0 lb ft	
Factored Moment:				0 lb ft	0 lb ft	
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	704 lb	14414 lb	Passed - 5%
Live Load (LL) Deflection:	1'- 2 5/16"	L		0.000"	L/360	Passed - L/999
Total Load (TL) Deflection:	1'- 2 1/8"	D + L		0.000"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	8689 lb		25225 lb	14803 lb	Passed - 59%
2	3-00	1.25D + 1.5L	1.00		-361 lb	-	-	

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	2'- 6"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 2 3/4"	2'- 6"	User Load	Top	60 lb/ft	-	-	-
Uniform	1'- 1/2"	2'- 6"	FC2 Floor Decking (Plan View Fill)	Top	-	26 lb/ft	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	B12(i50356)	Back	41 lb	87 lb	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	Pt1(i49783)	Top	2042 lb	3523 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	ST. BEAM (DR.)(i41671)	2550 lb	4272 lb	-	-
2	2'- 3"	2'- 6"	Pt1(i49863)	-280 lb	-612 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=5.500", W=3.499". LDF=1.00, Pf=7837 lb, Qr=16814 lb, Result=46.61%.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

32046670



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1**  
Level: **Ground Floor**  
Label: **B14 - i49988**  
Type: **Beam**

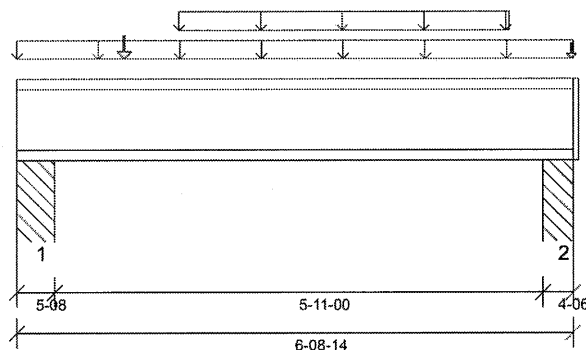
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:10



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 6'- 5 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 6 3/4"	1.25D + 1.5L	0.86	814 lb ft	4780 lb ft	Passed - 17%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	0.86	504 lb	1919 lb	Passed - 26%
Live Load (LL) Pos. Defl.:	3'- 5"	L		0.010"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 5"	D + L		0.025"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L	0.86	540 lb		1919 lb	15718 lb	Passed - 28%
2	4'-06	1.25D + 1.5L	0.86	527 lb		1919 lb	12503 lb	Passed - 27%

#### SPECIFIED LOADS

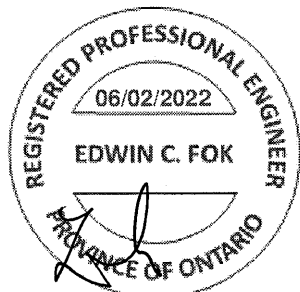
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	6'- 8 7/8"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 8 7/8"	User Load	Top	60 lb/ft	-	-	-
Uniform	1'- 11 5/8"	5'- 11 5/8"	Smoothed Load	Front	20 lb/ft	51 lb/ft	-	-
Point	1'- 3 5/8"	1'- 3 5/8"	J6(i50551)	Front	24 lb	64 lb	-	-
Point	6'- 8 5/8"	6'- 8 5/8"	FC2 Floor Decking (Plan View Fill)	Top	-	5 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	Pt1(i49863)	267 lb	138 lb	-	-
2	6'- 4 1/2"	6'- 8 7/8"	Pt1(i49863)	260 lb	135 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SG046671





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B15 - i50756**  
Type: **Beam**

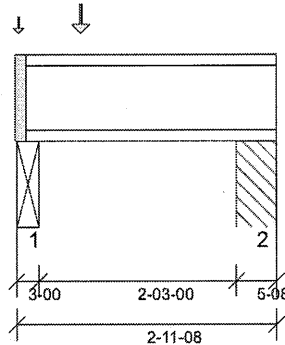
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:11



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part 9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 2'- 1 1/2" Bottom: 2'- 1 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 2"
- 1334 psi Column @ 2'- 7"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	0'- 8 3/4"	1.25D + 1.5L	1.00	82 lb ft	5580 lb ft	Passed - 1%
Factored Shear:	0'- 3 1/16"	1.25D + 1.5L	1.00	148 lb	2240 lb	Passed - 7%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3'-00"	1.25D + 1.5L	1.00	151 lb		2120 lb	5770 lb	Passed - 7%
2	5'-08"	1.25D + 1.5L	1.00	49 lb		2240 lb	18348 lb	Passed - 2%

#### SPECIFIED LOADS

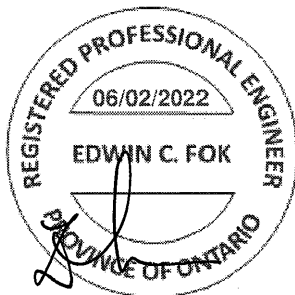
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	2'- 11 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Point	0'- 8 3/4"	0'- 8 3/4"	B12(i50356)	Front	43 lb	89 lb	-	-
Point	0'- 1/4"	0'- 1/4"	FC2 Floor Decking (Plan View Fill)	Top	1 lb	1 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	ST. BEAM (DR.)(i41671)	38 lb	70 lb	-	-
2	2'- 6"	2'- 11 1/2"	PT3(i49948)	15 lb	21 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SC046672



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1**  
Level: **Ground Floor**  
Label: **B16 - i49929**  
Type: **Beam**

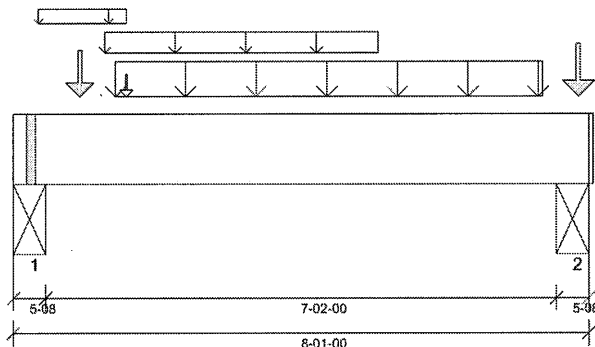
**1 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:11



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 4 1/2"
- 769 psi Beam @ 7'- 8 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 11 1/4"	1.25D + 1.5L	1.00	8725 lb ft	13266 lb ft	Passed - 66%
Factored Neg. Moment:	7'- 8 1/2"	1.25D + 1.5L	1.00	215 lb ft	13266 lb ft	Passed - 2%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	4425 lb	7207 lb	Passed - 61%
Live Load (LL) Pos. Defl.:	3'- 11 7/8"	L		0.096"	L/360	Passed - L/896
Total Load (TL) Pos. Defl.:	4'	D + L		0.161"	L/240	Passed - L/533

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L	1.00	4603 lb		12613 lb	7402 lb	Passed - 62%
2	5'-08"	1.25D + 1.5L	1.00	5001 lb		12613 lb	7402 lb	Passed - 68%

#### SPECIFIED LOADS

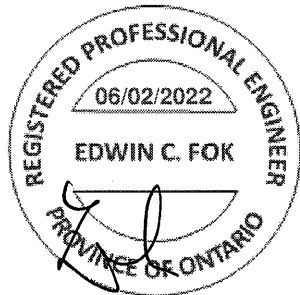
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 1"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 4 1/4"	1'- 7 1/8"	FC2 Floor Decking (Plan View Fill)	Top	-	45 lb/ft	-	-
Uniform	1'- 3 1/2"	5'- 1 1/2"	User Load	Top	69 lb/ft	184 lb/ft	-	-
Uniform	1'- 5 1/4"	7'- 5 1/4"	Smoothed Load	Front	315 lb/ft	408 lb/ft	-	-
Point	0'- 11 1/4"	0'- 11 1/4"	J8(i50720)	Front	245 lb	318 lb	-	-
Point	7'- 11 1/4"	7'- 11 1/4"	J8(i50727)	Front	261 lb	407 lb	-	-
Point	1'- 7 1/8"	1'- 7 1/8"	J2(i50533)	Back	42 lb	113 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	ST. BEAM (DR.)(i41668)	1322 lb	1990 lb	-	-
2	7'- 7 1/2"	8'- 1"	ST. BEAM (DR.)(i41671)	1466 lb	2088 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SEA6673





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B17 (-3R) - i49970**  
Type: **Beam**

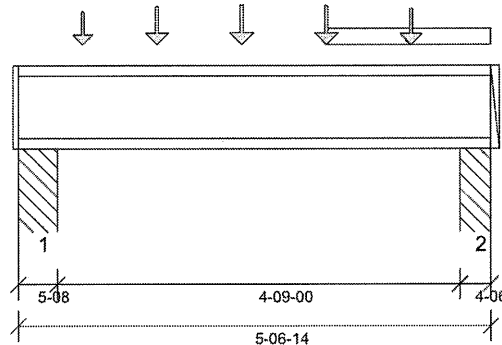
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:11



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 5'- 3 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 7 5/8"	1.25D + 1.5L	1.00	1118 lb ft	5580 lb ft	Passed - 20%
Factored Shear:	0'- 5 9/16"	1.25D + 1.5L	1.00	869 lb	2240 lb	Passed - 39%
Live Load (LL) Pos. Defl.:	2'- 10"	L		0.017"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 10"	D + L		0.026"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L	1.00	880 lb		2240 lb	18348 lb	Passed - 39%
2	4'-06	1.25D + 1.5L	1.00	826 lb		2240 lb	14595 lb	Passed - 37%

#### SPECIFIED LOADS

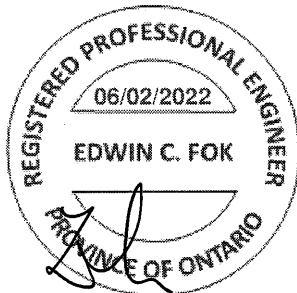
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 6 7/8"	Self Weight	Top	3 lb/ft	-	-	-
Tapered	3'- 7 5/8"	5'- 6 7/8"	FC1 Floor Decking (Plan View Fill)	Top	4 To 3 lb/ft	11 To 8 lb/ft	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J3(i49975)	Back	62 lb	123 lb	-	-
Point	1'- 7 5/8"	1'- 7 5/8"	J3(i50443)	Back	80 lb	161 lb	-	-
Point	2'- 7 5/8"	2'- 7 5/8"	J3(i49874)	Back	86 lb	172 lb	-	-
Point	3'- 7 5/8"	3'- 7 5/8"	J3(i50377)	Back	83 lb	166 lb	-	-
Point	4'- 7 5/8"	4'- 7 5/8"	J3(i50332)	Back	74 lb	148 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	P12(i49962)	213 lb	409 lb	-	-
2	5'- 2 1/2"	5'- 6 7/8"	P12(i49898)	200 lb	384 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SE04667P



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B18 (-3R) - i49844**  
Type: **Beam**

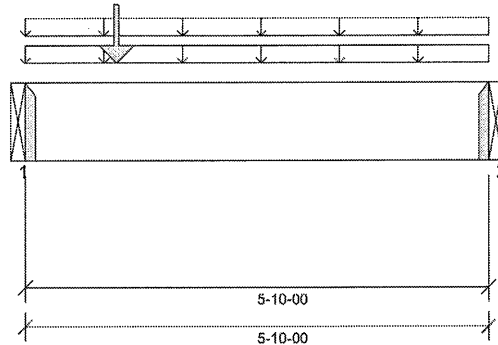
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:12



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'- 10"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 6" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 1 3/4"	1.25D + 1.5L	1.00	6609 lb ft	26531 lb ft	Passed - 25%
Factored Shear:	0'- 11 7/8"	1.25D + 1.5L	1.00	5694 lb	14414 lb	Passed - 40%
Live Load (LL) Pos. Defl.:	2'- 6 3/4"	L		0.018"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	2'- 7 1/16"	D + L		0.031"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	5891 lb		6880 lb	-	Passed - 86%
2	1-08	1.25D + 1.5L	1.00	1844 lb		6880 lb	-	Passed - 27%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Top	Face	Member	Other Information or Requirement for Reinforcement Accessories
1	HGUS410		-	-	-	Connector manually specified by the user.
2	HGUS410		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 10"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	5'- 10"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC1 Floor Decking (Plan View Fill)	Top	16 lb/ft	42 lb/ft	-	-
Point	1'- 1 3/4"	1'- 1 3/4"	10(i41708)	Top	1734 lb	2999 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B19 (-3R)(i49909)	1660 lb	2544 lb	-	-
2	5'- 10"	5'- 10"	B20 (-3R)(i50485)	607 lb	724 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046675



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B19 (-3R) - i49909**  
Type: **Beam**

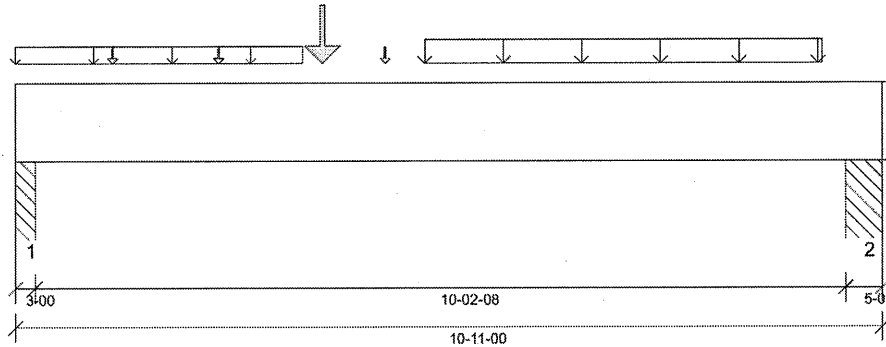
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:12



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 2"
- 1334 psi Column @ 10'- 6 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/2"	1.25D + 1.5L	1.00	17160 lb ft	26531 lb ft	Passed - 65%
Factored Shear:	1'- 2 7/8"	1.25D + 1.5L	1.00	5062 lb	14414 lb	Passed - 35%
Live Load (LL) Pos. Defl.:	5'- 9/16"	L		0.166"	L/360	Passed - L/735
Total Load (TL) Pos. Defl.:	5'- 9/16"	D + L		0.272"	L/240	Passed - L/450

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	5191 lb		13759 lb	14011 lb	Passed - 38%
2	5-08	1.25D + 1.5L	1.00	3484 lb		25225 lb	25687 lb	Passed - 14%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 11"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	5'- 1 15/16"	10'- 1 15/16"	Smoothed Load	Back	63 lb/ft	127 lb/ft	-	-
Point	1'- 2 3/4"	1'- 2 3/4"	J4(i50290)	Back	67 lb	178 lb	-	-
Point	2'- 6 3/4"	2'- 6 3/4"	J4(i50382)	Back	64 lb	170 lb	-	-
Point	3'- 10 1/2"	3'- 10 1/2"	B18 (-3R)(i49844)	Back	1660 lb	2544 lb	-	-
Point	4'- 7 15/16"	4'- 7 15/16"	J4(i50516)	Back	57 lb	115 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	PI2(i50280)	1416 lb	2281 lb	-	-
2	10'- 5 1/2"	10'- 11"	PI2(i49934)	961 lb	1522 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
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- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



83046676



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B20 (-3R) - i50485**  
Type: **Beam**

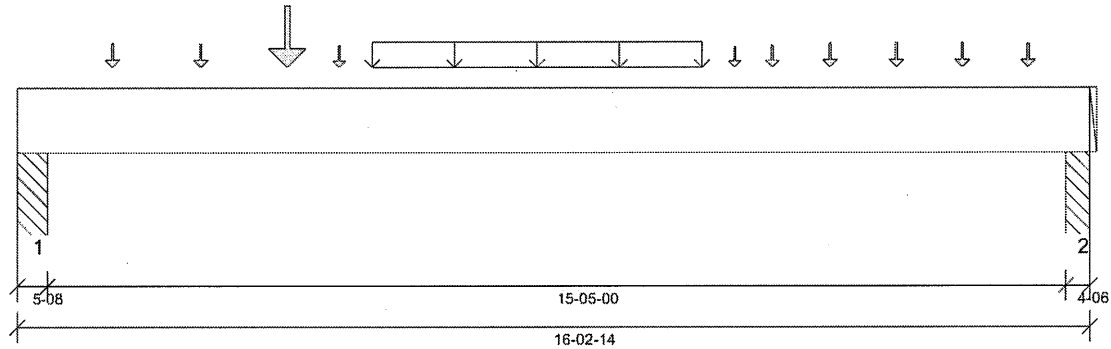
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:12



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 15'- 11 1/2"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 10 7/16"	1.25D + 1.5L	1.00	12333 lb ft	26531 lb ft	Passed - 46%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	3348 lb	14414 lb	Passed - 23%
Live Load (LL) Pos. Defl.:	8'- 1/8"	L		0.322"	L/360	Passed - L/574
Total Load (TL) Pos. Defl.:	7'- 11 11/16"	D + L		0.533"	L/240	Passed - L/347
Permanent Deflection:	7'- 11 1/8"			-	L/360	Passed - L/903

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	3371 lb		25225 lb	25687 lb	Passed - 13%
2	4-06	1.25D + 1.5L	1.00	2929 lb		20065 lb	20433 lb	Passed - 15%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	16'- 2 7/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	5'- 4 7/16"	10'- 4 7/16"	Smoothed Load	Front	64 lb/ft	128 lb/ft	-	-
Point	1'- 5 1/4"	1'- 5 1/4"	J4(i50290)	Front	67 lb	178 lb	-	-
Point	2'- 9 1/4"	2'- 9 1/4"	J4(i50382)	Front	64 lb	170 lb	-	-
Point	4'- 1"	4'- 1"	B18 (-3R)(i49844)	Front	607 lb	724 lb	-	-
Point	4'- 10 7/16"	4'- 10 7/16"	J4(i50516)	Front	57 lb	115 lb	-	-
Point	10'- 10 3/8"	10'- 10 3/8"	J4(i50519)	Front	46 lb	99 lb	-	-
Point	11'- 5 1/8"	11'- 5 1/8"	J3(i49975)	Front	63 lb	126 lb	-	-
Point	12'- 3 5/8"	12'- 3 5/8"	J3(i50443)	Front	82 lb	164 lb	-	-
Point	13'- 3 5/8"	13'- 3 5/8"	J3(i49874)	Front	88 lb	175 lb	-	-
Point	14'- 3 5/8"	14'- 3 5/8"	J3(i50377)	Front	87 lb	174 lb	-	-
Point	15'- 3 5/8"	15'- 3 5/8"	J3(i50332)	Front	80 lb	160 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	P12(i50363)	971 lb	1438 lb	-	-
2	15'- 10 1/2"	16'- 2 7/8"	P12(i49905)	799 lb	1287 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SG046677



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A (1,**  
Level: **Ground Floor**  
Label: **B21 (-3R) - i50402**  
Type: **Beam**

**1 Ply Member**

**11 7/8" NI-20**

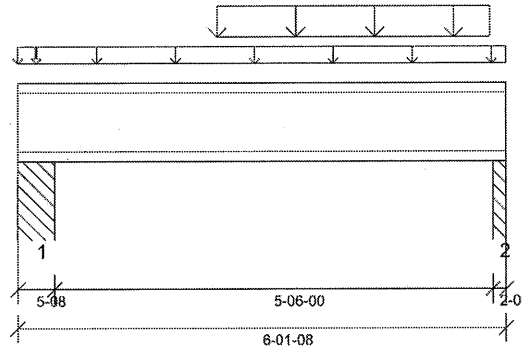
Status:

**Design  
Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/02/2022 17:13



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 6'- 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 6 3/4"	1.25D + 1.5L	1.00	1831 lb ft	5580 lb ft	Passed - 29%
Factored Shear:	5'- 11 7/16"	1.25D + 1.5L	1.00	1256 lb	2240 lb	Passed - 56%
Live Load (LL) Pos. Defl.:	3'- 3 7/8"	L		0.031"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 3 7/8"	D + L		0.043"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	751 lb		2240 lb	18349 lb	Passed - 34%
2	2-00	1.25D + 1.5L	1.00	1283 lb		2000 lb	6672 lb	Passed - 64%

#### SPECIFIED LOADS

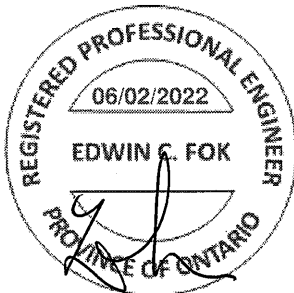
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	6'- 1 1/2"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1 1/2"	FC1 Floor Decking (Plan View Fill)	Top	11 lb/ft	29 lb/ft	-	-
Uniform	2'- 6"	5'- 11"	User Load	Top	90 lb/ft	240 lb/ft	-	-
Point	0'- 2 3/4"	0'- 2 3/4"	PT3(49948)	Top	13 lb	12 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	P12(i50280)	153 lb	360 lb	-	-
2	5'- 11 1/2"	6'- 1 1/2"	P12(i50363)	254 lb	656 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



52046628





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Second A WO Loft (14)**  
Level: **Second Floor**  
Label: **B22 - i54142**  
Type: **Beam**

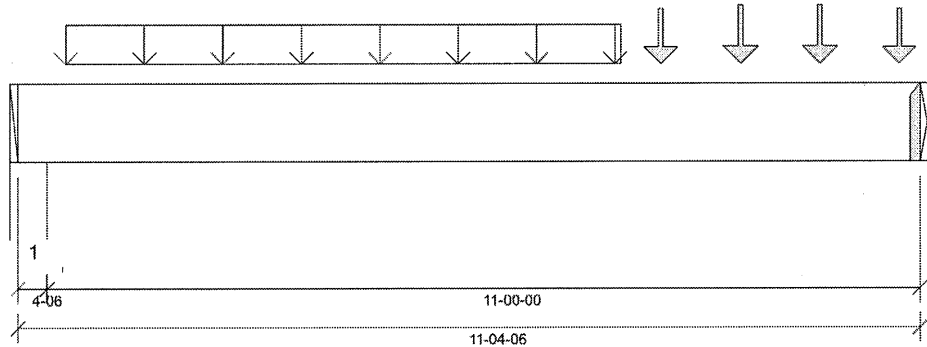
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 14:34



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

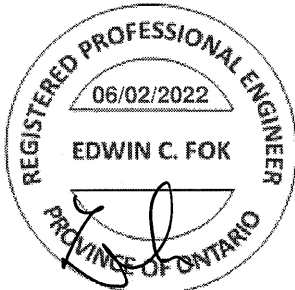
Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 0'-9 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'-3 3/8"
- 769 psi Beam @ 11'-4 3/8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 9" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'-1 1/4"	1.25D + 1.5L	1.00	13470 lb ft	26531 lb ft	Passed - 51%
Factored Shear:	10'-4 1/2"	1.25D + 1.5L	1.00	4454 lb	14414 lb	Passed - 31%
Live Load (LL) Pos. Defl.:	5'-10 3/8"	L		0.201"	L/360	Passed - L/658
Total Load (TL) Pos. Defl.:	5'-10 3/8"	D + L		0.284"	L/240	Passed - L/464

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	4443 lb		20065 lb	9420 lb	Passed - 47%
2	1-08	1.25D + 1.5L	1.00	5413 lb		6880 lb	-	Passed - 79%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	HGUS410		-	-	-	Connector manually specified by the user.
* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.						

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	11'-4 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'-7 1/4"	7'-7 1/4"	Smoothed Load	Front	151 lb/ft	403 lb/ft	-	-
Point	8'-1 1/4"	8'-1 1/4"	J6(i54036)	Front	190 lb	476 lb	-	-
Point	9'-1 1/4"	9'-1 1/4"	J6(i54037)	Front	217 lb	508 lb	-	-
Point	10'-1 1/4"	10'-1 1/4"	J6(i54084)	Front	217 lb	508 lb	-	-
Point	11'-1 1/4"	11'-1 1/4"	J6(i54039)	Front	201 lb	461 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'-4 3/8"	12(i51771)	909 lb	2189 lb	-	-
2	11'-4 3/8"	11'-4 3/8"	B23(i53621)	1152 lb	2663 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

36046679



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Second A WO Loft (14)**  
Level: **Second Floor**  
Label: **B23 - i53621**  
Type: **Beam**

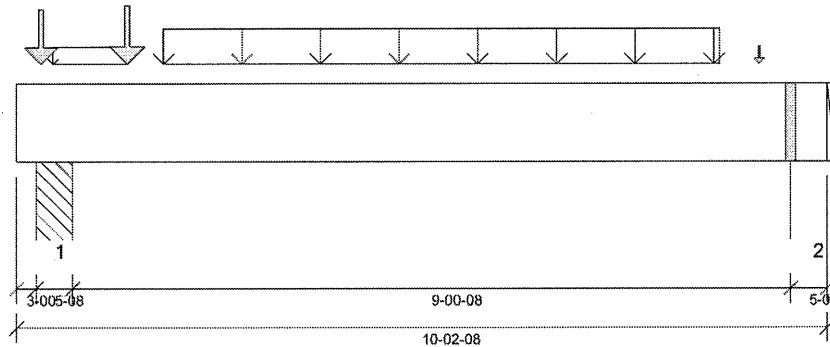
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 14:35



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

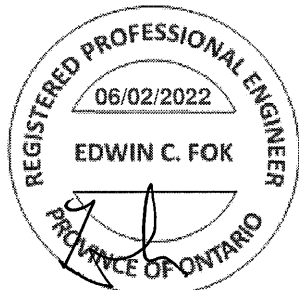
#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:  
Top: 0' Bottom: 0'-9 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'-5 3/4"
- 615 psi Wall @ 9'-10"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'-4 1/4"	1.25D + 1.5L	1.00	8884 lb ft	26531 lb ft	Passed - 33%
Factored Neg. Moment:	0'-5 3/4"	1.25D + 1.5L	1.00	923 lb ft	26531 lb ft	Passed - 3%
Factored Shear:	1'-8 3/8"	1.25D + 1.5L	1.00	7523 lb	14414 lb	Passed - 52%
Live Load (LL) Pos. Defl.:	4'-11 5/8"	L		0.086"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'-11 13/16"	D + L		0.138"	L/240	Passed - L/786

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L	1.00	13135 lb		25225 lb	25687 lb	Passed - 52%
2	5'-08"	1.25D + 1.5L	1.00	3183 lb		25226 lb	11843 lb	Passed - 27%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'-2 1/2"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'-5 1/2"	1'-4 3/4"	FC3 Floor Decking (Plan View Fill)	Top	51 lb/ft	-	-	-
Uniform	1'-10 1/4"	8'-10 1/4"	Smoothed Load	Front	159 lb/ft	264 lb/ft	-	-
Point	1'-4 3/4"	1'-4 3/4"	B6(i54076)	Front	1344 lb	2840 lb	-	-
Point	9'-4 1/4"	9'-4 1/4"	J2(i54010)	Front	104 lb	197 lb	-	-
Point	0'-3 3/4"	0'-3 3/4"	B22(i54142)	Back	1152 lb	2663 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'-3"	0'-8 1/2"	P11(i50787)	3054 lb	6148 lb	-	-
2	9'-9"	10'-2 1/2"	9(i41702)	903 lb	1433 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5940 lb, Qr=13759 lb, Result=43.17%.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

SE046680



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B24 - i54731**  
Type: **Beam**

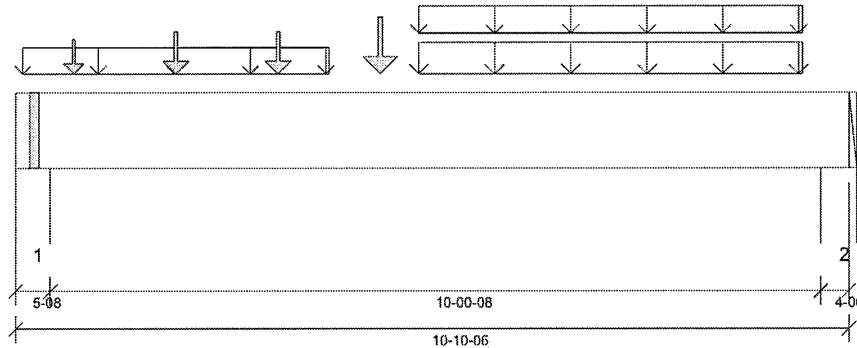
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MITek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:44



### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

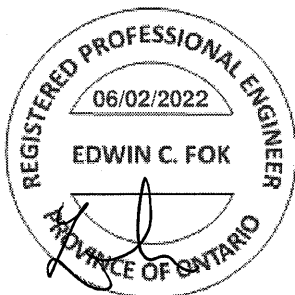
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 10'- 7"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 9 1/8"	1.25D + 1.5L	1.00	11938 lb ft	26531 lb ft	Passed - 45%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	4649 lb	14414 lb	Passed - 32%
Live Load (LL) Pos. Defl.:	5'- 5 13/16"	L		0.148"	L/360	Passed - L/816
Total Load (TL) Pos. Defl.:	5'- 5 15/16"	D + L		0.213"	L/240	Passed - L/566

### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4673 lb		25225 lb	11842 lb	Passed - 39%
2	4-06	1.25D + 1.5L	1.00	4410 lb		20065 lb	9420 lb	Passed - 47%

### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 10 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 1 1/8"	4'- 1 1/8"	Smoothed Load	Front	70 lb/ft	187 lb/ft	-	-
Uniform	5'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Back	104 lb/ft	258 lb/ft	-	-
Uniform	5'- 3 1/8"	10'- 3 1/8"	Smoothed Load	Front	91 lb/ft	187 lb/ft	-	-
Point	4'- 9 1/8"	4'- 9 1/8"	-	Front	200 lb	519 lb	-	-
Point	0'- 9 1/8"	0'- 9 1/8"	J2(i54765)	Back	94 lb	252 lb	-	-
Point	2'- 1 1/8"	2'- 1 1/8"	J2(i54839)	Back	129 lb	344 lb	-	-
Point	3'- 5 1/8"	3'- 5 1/8"	J2(i54852)	Back	129 lb	344 lb	-	-

### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	24(i51833)	975 lb	2302 lb	-	-
2	10'- 6"	10'- 10 3/8"	E25(i41701)	974 lb	2129 lb	-	-

### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

36046081





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B25 - i54388**  
Type: **Beam**

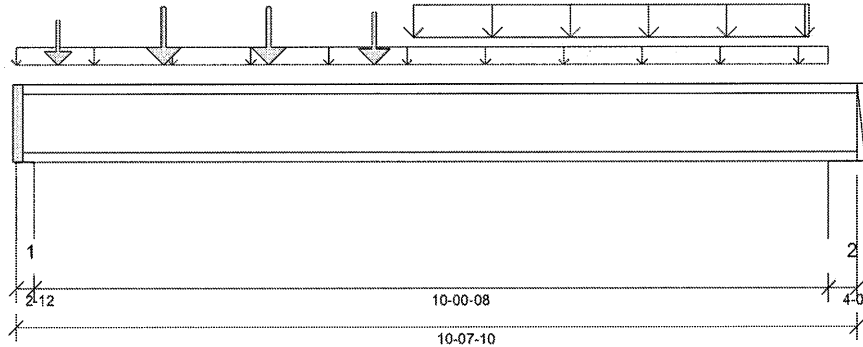
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:44



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/4"
- 615 psi Wall @ 10'- 4 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 6 3/8"	1.25D + 1.5L	1.00	7975 lb ft	11160 lb ft	Passed - 71%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	1.00	3054 lb	4480 lb	Passed - 68%
Live Load (LL) Pos. Defl.:	5'- 3 1/16"	L		0.156"	L/360	Passed - L/769
Total Load (TL) Pos. Defl.:	5'- 3 1/16"	D + L		0.254"	L/240	Passed - L/475

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	3073 lb		4180 lb	8459 lb	Passed - 74%
2	4-06	1.25D + 1.5L	1.00	2946 lb		4480 lb	13457 lb	Passed - 66%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	10'- 7 5/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	10'- 3 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	5'- 3/8"	10'- 3/8"	Smoothed Load	Front	101 lb/ft	269 lb/ft	-	-
Point	0'- 6 3/8"	0'- 6 3/8"	J2(i54765)	Front	94 lb	249 lb	-	-
Point	1'- 10 3/8"	1'- 10 3/8"	J2(i54839)	Front	135 lb	359 lb	-	-
Point	3'- 2 3/8"	3'- 2 3/8"	J2(i54852)	Front	135 lb	359 lb	-	-
Point	4'- 6 3/8"	4'- 6 3/8"	J2(i54657)	Front	118 lb	314 lb	-	-

#### UNFACTORED REACTIONS

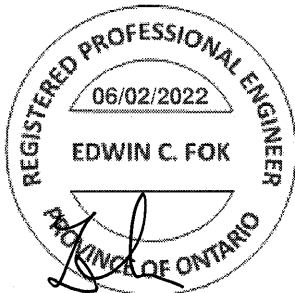
ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	9(i41702)	849 lb	1341 lb	-	-
2	10'- 3 1/4"	10'- 7 5/8"	E25(i41701)	815 lb	1285 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



*Signature*



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B26 - i54803**  
Type: **Beam**

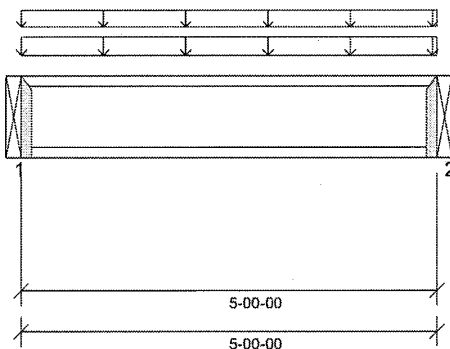
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MITek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:44



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6"	1.25D + 1.5L	0.72	369 lb ft	4021 lb ft	Passed - 9%
Factored Shear:	0'- 1/16"	1.25D + 1.5L	0.72	294 lb	1614 lb	Passed - 18%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.72	295 lb		1970 lb	-	Passed - 15%
2	1-12	1.25D + 1.5L	0.72	295 lb		1970 lb	-	Passed - 15%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

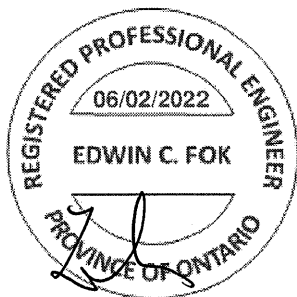
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	19(i51781)	Top	61 lb/ft	-	-	-
Uniform	0'	5'	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	20 lb/ft	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B29(i54286)	177 lb	49 lb	-	-
2	5'	5'	B27(i54263)	177 lb	49 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



3430776683



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B27 - i54263**  
Type: **Beam**

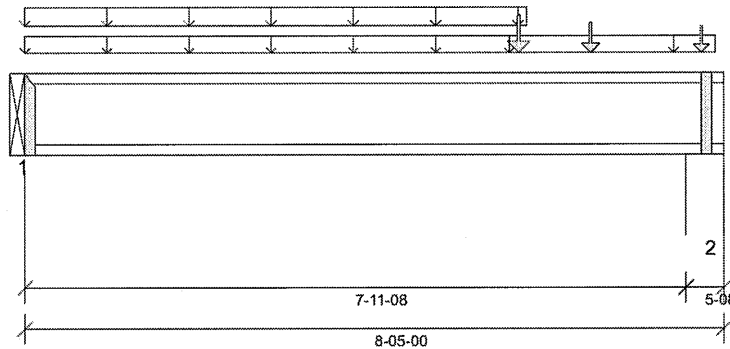
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:45



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'- 10"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 7 1/16"	1.4D	0.65	1062 lb ft	3627 lb ft	Passed - 29%
Factored Shear:	7'- 11 7/16"	1.25D + 1.5L	0.89	786 lb	1991 lb	Passed - 40%
Live Load (LL) Pos. Defl.:	4'- 2 7/8"	L		0.019"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 1 7/8"	D + L		0.066"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.4D	0.65	463 lb		1970 lb	-	Passed - 23%
2	5-08	1.25D + 1.5L	0.89	974 lb		1991 lb	7519 lb	Passed - 49%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	LT251188		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

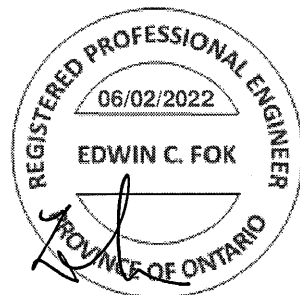
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	22(i51778)	Top	61 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	22 lb/ft	-	-
Uniform	5'- 10"	8'- 3 3/4"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	20 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Back	190 lb	49 lb	-	-
Point	6'- 9 3/4"	6'- 9 3/4"	J7(i54649)	Back	44 lb	118 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i54650)	Back	33 lb	87 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i54763)	338 lb	120 lb	-	-
2	7'- 11 1/2"	8'- 5"	24(i51833)	390 lb	316 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SEDA 1684



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B28 (CANT.) - i54890**  
Type: **Beam**

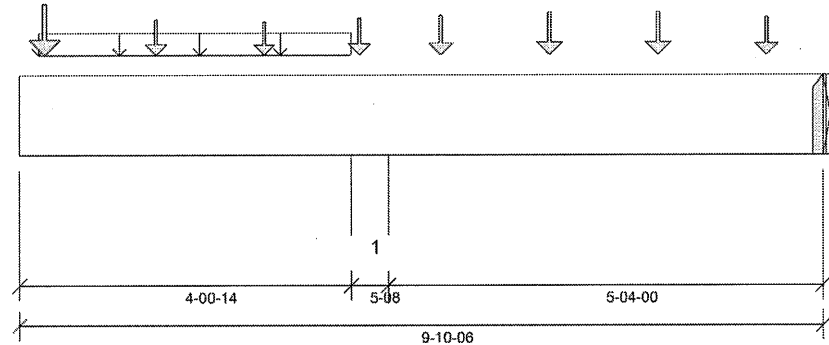
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:45



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0'- 2 7/8" Bottom: 1'- 2 1/4"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 4'- 3 5/8"
- 769 psi Beam @ 9'- 10 3/8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	7'- 10 1/8"	0.9D + 1.5L	0.96	760 lb ft	25348 lb ft	Passed - 3%
Factored Neg. Moment:	4'- 3 5/8"	1.25D + 1.5L	1.00	5317 lb ft	25719 lb ft	Passed - 21%
Factored Shear:	3'- 1"	1.25D + 1.5L	1.00	1795 lb	14414 lb	Passed - 12%
Live Load (LL) Neg. Defl.:	6'- 7 13/16"	L		0.012"	L/360	Passed - L/999
Total Load (TL) Neg. Defl.:	6'- 7 1/16"	D + L		0.015"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	4143 lb		25225 lb	11843 lb	Passed - 35%
2	1-08	1.25D + 1.5L	0.96	603 lb		603 lb	-	Passed - 100%
2	1-08	0.9D + 1.5L	1.00		-705 lb	-	-	

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
2	HU410		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'- 0"	9'- 10 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 2 7/8"	4'- 7/8"	User Load	Top	38 lb/ft	100 lb/ft	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B5(i54715)	Front	132 lb	270 lb	-	-
Point	1'- 8 1/8"	1'- 8 1/8"	J6(i54899)	Front	61 lb	163 lb	-	-
Point	3'- 1/8"	3'- 1/8"	J6(i54447)	Front	56 lb	148 lb	-	-
Point	5'- 2 1/8"	5'- 2 1/8"	-	Front	74 lb	197 lb	-	-
Point	6'- 6 1/8"	6'- 6 1/8"	-	Front	83 lb	222 lb	-	-
Point	7'- 10 1/8"	7'- 10 1/8"	-	Front	83 lb	222 lb	-	-
Point	9'- 2 1/8"	9'- 2 1/8"	-	Front	69 lb	184 lb	-	-
Point	4'- 2 1/8"	4'- 2 1/8"	B2(i54372)	Back	183 lb	57 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	4'- 7/8"	4'- 6 3/8"	10(i41708)	1039 lb	1887 lb	-	-
2	9'- 10 3/8"	9'- 10 3/8"	B29(i54286)	-19 lb	422/-450 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

SEAN BERT



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B29 - i54286**  
Type: **Beam**

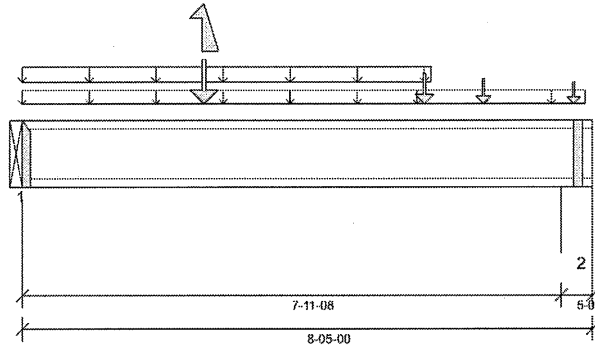
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:45



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 3'

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 1"	1.25D + 1.5L	1.00	2183 lb ft	5580 lb ft	Passed - 39%
Factored Neg. Moment:	2'- 8 1/4"	0.9D + 1.5L	0.91	684 lb ft	5053 lb ft	Passed - 14%
Factored Shear:	0'- 1/16"	1.25D + 1.5L	1.00	962 lb	2240 lb	Passed - 43%
Live Load (LL) Pos. Defl.:	3'- 10 3/16"	L		0.055"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	3'- 11 13/16"	D + L		0.097"	L/240	Passed - L/984

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	963 lb		1970 lb	-	Passed - 49%
1	1-12	0.9D + 1.5L	0.91		-170 lb	-	-	
2	5-08	1.25D + 1.5L	1.00	1142 lb		2240 lb	8459 lb	Passed - 51%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	LT251188		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

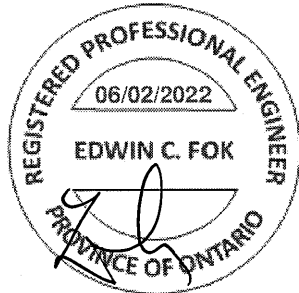
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	20(i51780)	Top	61 lb/ft	-	-	-
Uniform	0'	5'- 10"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	5'- 10"	8'- 3 3/4"	FC3 Floor Decking (Plan View Fill)	Top	6 lb/ft	16 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Front	191 lb	49 lb	-	-
Point	6'- 9 3/4"	6'- 9 3/4"	J7(i54649)	Front	44 lb	118 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i54650)	Front	33 lb	87 lb	-	-
Point	2'- 8 1/4"	2'- 8 1/4"	B28 (CANT.) (i54890)	Back	-19 lb	422/450 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i54763)	317 lb	385/300 lb	-	-
2	7'- 11 1/2"	8'- 5"	25(i51910)	379 lb	439/150 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



33046686





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B30 - i54763**  
Type: **Beam**

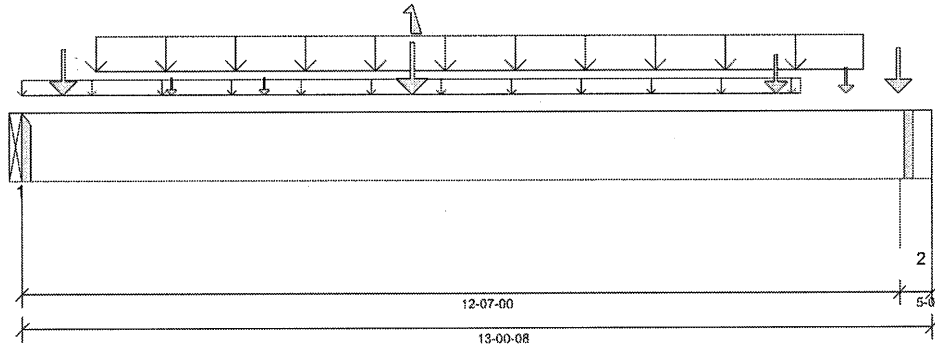
**3 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:46



### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

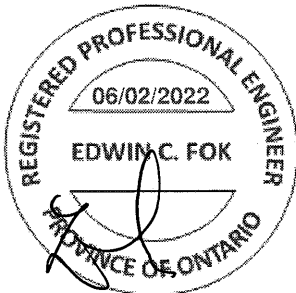
Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 0'- 9 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 12'- 8"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 6" O/C  
STAGGERED IN 2 ROWS



### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 7 1/4"	1.25D + 1.5L	1.00	24190 lb ft	39797 lb ft	Passed - 61%
Factored Shear:	11'- 7 1/8"	1.25D + 1.5L	1.00	7828 lb	21621 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	6'- 3 5/8"	L		0.270"	L/360	Passed - L/559
Total Load (TL) Pos. Defl.:	6'- 3 13/16"	D + L		0.447"	L/240	Passed - L/337
Permanent Deflection:	6'- 4"			-	L/360	Passed - L/877

### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-08	1.25D + 1.5L	1.00	7153 lb		10319 lb	-	Passed - 69%
2	5-08	1.25D + 1.5L	1.00	7937 lb		37838 lb	17764 lb	Passed - 45%

### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	HGUS5.50/10		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/2"	Self Weight	Top	19 lb/ft	-	-	-
Uniform	0'	11'- 2"	-	Top	60 lb/ft	-	-	-
Uniform	1'- 3/4"	12'- 3/4"	Smoothed Load	Front	187 lb/ft	432 lb/ft	-	-
Point	0'- 7 1/8"	0'- 7 1/8"	-	Front	159 lb	434 lb	-	-
Point	12'- 6 3/4"	12'- 6 3/4"	J11(i54564)	Front	164 lb	411 lb	-	-
Point	2'- 1 3/4"	2'- 1 3/4"	J9(i54678)	Back	-	79 lb	-	-
Point	3'- 5 3/4"	3'- 5 3/4"	J9(i54856)	Back	-	79 lb	-	-
Point	5'- 7 1/4"	5'- 7 1/4"	B29(i54286)	Back	317 lb	385/-300 lb	-	-
Point	10'- 9 3/4"	10'- 9 3/4"	B27(i54263)	Back	338 lb	120 lb	-	-
Point	11'- 9 3/4"	11'- 9 3/4"	J5(i54672)	Back	72 lb	156 lb	-	-

### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B31(i54841)	1967 lb	3077/-165 lb	-	-
2	12'- 7"	13'- 1/2"	-	2258 lb	3462/-135 lb	-	-
++>	12'- 10 1/4"	12'- 10 1/4"	4(i41664)	1828 lb	2803/-109 lb	-	-
++>	13'	13'	3(i41663)	430 lb	659/-26 lb	-	-

### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

8-04-2022



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Second Floor**  
Label: **B31 - i54841**  
Type: **Beam**

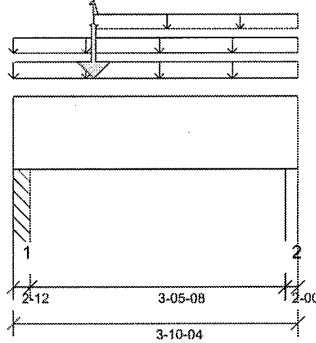
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:46



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 2'- 6 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 1 3/4"
- 615 psi Wall @ 3'- 9 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	1'- 1 1/8"	1.25D + 1.5L	1.00	5171 lb ft	26531 lb ft	Passed - 19%
Factored Shear:	1'- 2 5/8"	1.25D + 1.5L	1.00	4462 lb	14414 lb	Passed - 31%

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	5561 lb		12613 lb	12844 lb	Passed - 44%
2	2-00	1.25D + 1.5L	1.00	2196 lb		9171 lb	4306 lb	Passed - 51%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	3'- 10 1/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	User Load	Top	60 lb/ft	-	-	-
Uniform	0'	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Uniform	1'- 1 1/8"	3'- 10 1/4"	FC3 Floor Decking (Plan View Fill)	Top	-	19 lb/ft	-	-
Point	1'- 1 1/8"	1'- 1 1/8"	B30(i54763)	Front	1967 lb	3077/-165 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	P11(i54208)	1620 lb	2343/-121 lb	-	-
2	3'- 8 1/4"	3'- 10 1/4"	10(i41708)	692 lb	902/-44 lb	-	-

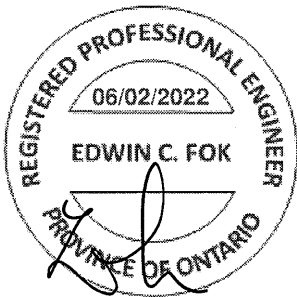
#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- User loads assume a bearing length of 3.5" in determining member capacity for loads near supports.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=6827 lb, Q'r=9555 lb, Result=71.45%.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 6" O/C  
STAGGERED IN 2 ROWS



Signature



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B32 - i54376**  
Type: **Beam**

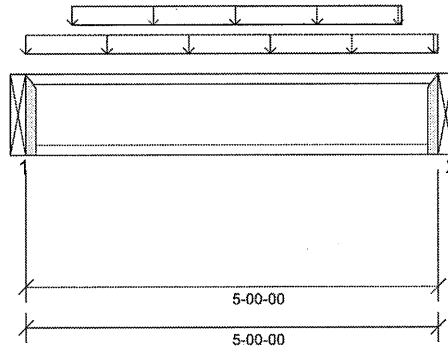
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:46



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6 3/4"	1.25D + 1.5L	0.84	605 lb ft	4679 lb ft	Passed - 13%
Factored Shear:	4'- 11 15/16"	1.25D + 1.5L	0.84	427 lb	1878 lb	Passed - 23%
Total Load (TL) Pos. Defl.:	2'- 6"	D + L		0.015"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.84	423 lb		1970 lb	-	Passed - 21%
2	1-12	1.25D + 1.5L	0.84	428 lb		1970 lb	-	Passed - 22%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	15(i51774)	Top	68 lb/ft	-	-	-
Tapered	0'- 6 3/4"	4'- 6 3/4"	Smoothed Load	Back	21 To 18 lb/ft	54 To 49 lb/ft	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B36(i54820)	216 lb	102 lb	-	-
2	5'	5'	B34(i54292)	217 lb	104 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SC046689





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B33 - i54203**  
Type: **Beam**

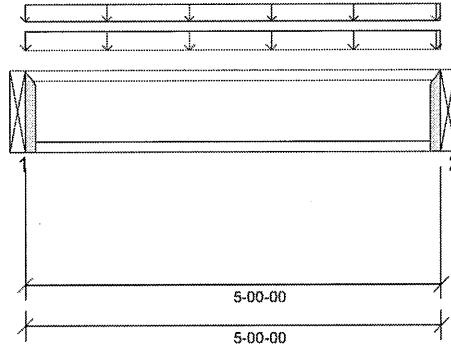
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:46



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 5'

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 769 psi Beam @ 5'

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6"	1.25D + 1.5L	0.74	424 lb ft	4109 lb ft	Passed - 10%
Factored Shear:	4'- 11 15/16"	1.25D + 1.5L	0.74	338 lb	1650 lb	Passed - 21%
Total Load (TL) Pos. Defl.:	2'- 6"	D + L		0.011"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	0.74	339 lb		1970 lb	-	Passed - 17%
2	1-12	1.25D + 1.5L	0.74	339 lb		1970 lb	-	Passed - 17%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
1	LT251188		-	-	-	Connector manually specified by the user.
2	LT251188		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

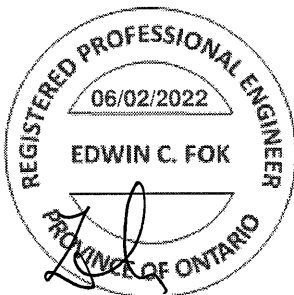
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'	Self Weight	Top	3 lb/ft	-	-	-
Uniform	-0'	5'	17(i51776)	Top	68 lb/ft	-	-	-
Uniform	0'	5'	FC2 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B36(i54820)	200 lb	60 lb	-	-
2	5'	5'	B34(i54292)	200 lb	59 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SE046690



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B34 - i54292**  
Type: **Beam**

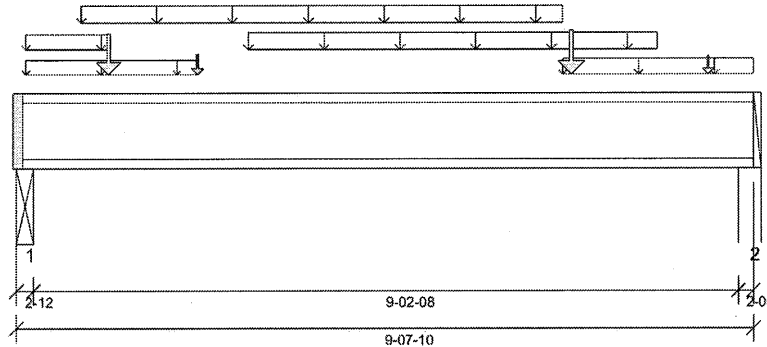
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 1 3/4"
- 615 psi Wall @ 9'- 6 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	5'- 1/2"	1.25D + 1.5L	0.88	2621 lb ft	4927 lb ft	Passed - 53%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	0.88	1156 lb	1978 lb	Passed - 58%
Live Load (LL) Pos. Defl.:	4'- 11 1/8"	L		0.055"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 10 5/8"	D + L		0.155"	L/240	Passed - L/711

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	0.88	1161 lb		1845 lb	4669 lb	Passed - 63%
2	2-06	1.25D + 1.5L	0.88	1130 lb		1806 lb	3225 lb	Passed - 63%

#### SPECIFIED LOADS

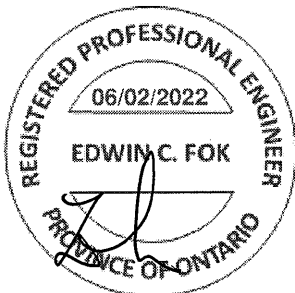
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 7 5/8"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'- 1 1/2"	2'- 4 1/2"	FC2 Floor Decking (Plan View Fill)	Top	-	4 lb/ft	-	-
Uniform	0'- 1 1/2"	1'- 2 7/8"	FC2 Floor Decking (Plan View Fill)	Top	-	15 lb/ft	-	-
Uniform	0'- 10 1/4"	7'- 1 3/4"	18(i51777)	Top	68 lb/ft	-	-	-
Uniform	7'- 1 3/4"	9'- 7 5/8"	FC2 Floor Decking (Plan View Fill)	Top	9 lb/ft	24 lb/ft	-	-
Tapered	3'- 1/2"	8'- 4 1/2"	Smoothed Load	Front	18 To 16 lb/ft	48 To 42 lb/ft	-	-
Point	1'- 2 9/16"	1'- 2 9/16"	-	Front	215 lb	100 lb	-	-
Point	2'- 4 1/2"	2'- 4 1/2"	J5(i55176)	Front	21 lb	55 lb	-	-
Point	9'- 1/2"	9'- 1/2"	J5(i55181)	Front	14 lb	39 lb	-	-
Point	7'- 3 1/8"	7'- 3 1/8"	-	Back	248 lb	104 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	ST. BEAM (DR.) (i41671)	583 lb	289 lb	-	-
2	9'- 5 1/4"	9'- 7 5/8"	W41(i51905)	495 lb	340 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
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- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



86046091



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B35 - i54409**  
Type: **Beam**

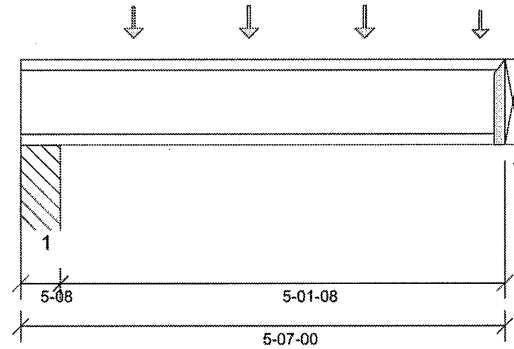
**1 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 769 psi Beam @ 5'- 7"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 7 5/8"	1.25D + 1.5L	0.90	614 lb ft	5002 lb ft	Passed - 12%
Factored Shear:	5'- 6 15/16"	1.25D + 1.5L	0.90	469 lb	2008 lb	Passed - 23%
Total Load (TL) Pos. Defl.:	2'- 11 5/8"	D + L		0.016"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	0.90	410 lb		2008 lb	16447 lb	Passed - 20%
2	1-12	1.25D + 1.5L	0.90	469 lb		1970 lb	-	Passed - 24%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories
			Top	Face	Member	
2	LT251188		-	-	-	Connector manually specified by the user.

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

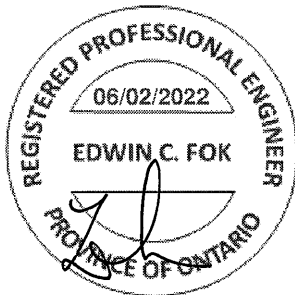
Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	5'- 7"	Self Weight	Top	3 lb/ft	-	-	-
Point	1'- 3 5/8"	1'- 3 5/8"	J5(i54973)	Front	103 lb	64 lb	-	-
Point	2'- 7 5/8"	2'- 7 5/8"	J5(i54974)	Front	106 lb	68 lb	-	-
Point	3'- 11 5/8"	3'- 11 5/8"	J5(i54975)	Front	106 lb	68 lb	-	-
Point	5'- 3 5/8"	5'- 3 5/8"	J5(i54976)	Front	72 lb	50 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	P1(i54293)	186 lb	112 lb	-	-
2	5'- 7"	5'- 7"	B36(i54820)	217 lb	138 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.



SE046092



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B36 - i54820**  
Type: **Beam**

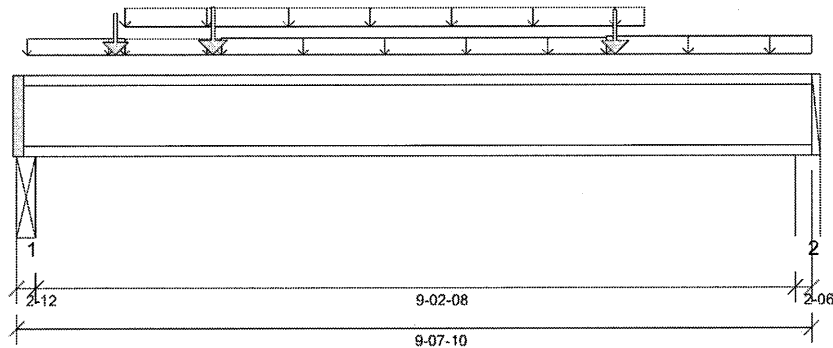
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:47



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 4'- 8"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'- 1 3/4"
- 615 psi Wall @ 9'- 6 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 5 1/4"	1.25D + 1.5L	0.81	2576 lb ft	9086 lb ft	Passed - 28%
Factored Shear:	0'- 2 13/16"	1.25D + 1.5L	0.81	1295 lb	3647 lb	Passed - 36%
Live Load (LL) Pos. Defl.:	4'- 9 7/16"	L		0.022"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 3/16"	D + L		0.079"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2'-12	1.25D + 1.5L	0.81	1301 lb		3403 lb	8610 lb	Passed - 38%
2	2'-06	1.25D + 1.5L	0.81	1025 lb		3330 lb	5948 lb	Passed - 31%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 7 5/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'- 1 1/2"	1'- 3 3/4"	FC2 Floor Decking (Plan View Fill)	Top	-	18 lb/ft	-	-
Uniform	1'- 3 3/4"	7'- 7 1/4"	16(i51775)	Top	68 lb/ft	-	-	-
Uniform	1'- 3 3/4"	2'- 5 3/4"	FC2 Floor Decking (Plan View Fill)	Top	-	14 lb/ft	-	-
Uniform	2'- 5 3/4"	7'- 1 3/4"	FC2 Floor Decking (Plan View Fill)	Top	7 lb/ft	18 lb/ft	-	-
Uniform	7'- 1 3/4"	9'- 7 5/8"	FC2 Floor Decking (Plan View Fill)	Top	15 lb/ft	39 lb/ft	-	-
Point	1'- 2 3/8"	1'- 2 3/8"	-	Front	228 lb	60 lb	-	-
Point	7'- 3"	7'- 3"	B32(i54376)	Front	216 lb	102 lb	-	-
Point	2'- 4 1/2"	2'- 4 1/2"	B35(i54409)	Back	217 lb	138 lb	-	-

#### UNFACTORED REACTIONS

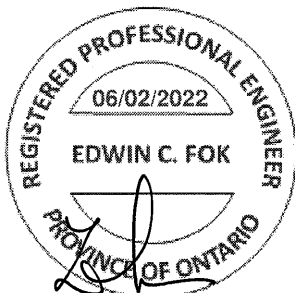
ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	ST. BEAM (DR.)(i41671)	712 lb	270 lb	-	-
2	9'- 5 1/4"	9'- 7 5/8"	W41(i51905)	520 lb	253 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



53046693



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B37 (-3R) - i54321**  
Type: **Beam**

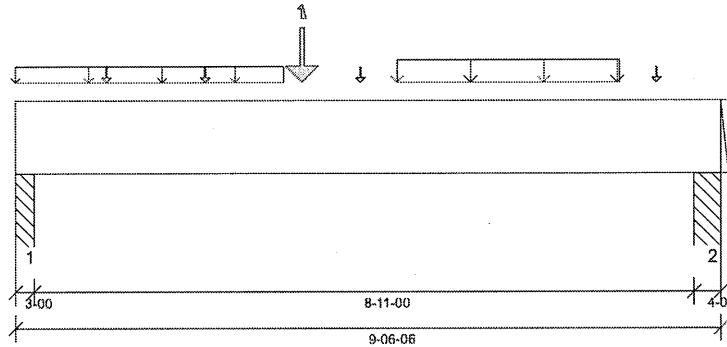
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:48



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 2"
- 1334 psi Column @ 9'- 3"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 10 1/2"	1.25D + 1.5L	1.00	14921 lb ft	26531 lb ft	Passed - 56%
Factored Shear:	1'- 2 7/8"	1.25D + 1.5L	1.00	4458 lb	14414 lb	Passed - 31%
Live Load (LL) Pos. Defl.:	4'- 6 3/16"	L		0.108"	L/360	Passed - L/993
Total Load (TL) Pos. Defl.:	4'- 6 3/16"	D + L		0.181"	L/240	Passed - L/592

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	3-00	1.25D + 1.5L	1.00	4587 lb		13759 lb	14011 lb	Passed - 33%
2	4-06	1.25D + 1.5L	1.00	3496 lb		20065 lb	20432 lb	Passed - 17%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 6 3/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	3'- 7 1/2"	User Load	Top	13 lb/ft	34 lb/ft	-	-
Uniform	5'- 1 15/16"	8'- 1 15/16"	Smoothed Load	Back	64 lb/ft	128 lb/ft	-	-
Point	1'- 2 3/4"	1'- 2 3/4"	J3(i54701)	Back	67 lb	178 lb	-	-
Point	2'- 6 3/4"	2'- 6 3/4"	J3(i54792)	Back	64 lb	170 lb	-	-
Point	3'- 10 1/2"	3'- 10 1/2"	B18 (-3R)(i54371)	Back	1688 lb	2376/-35 lb	-	-
Point	4'- 7 15/16"	4'- 7 15/16"	J3(i54929)	Back	57 lb	115 lb	-	-
Point	8'- 7 15/16"	8'- 7 15/16"	J3(i54829)	Back	56 lb	113 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 3"	P12(i54691)	1302 lb	1973/-21 lb	-	-
2	9'- 2"	9'- 6 3/8"	P12(i54221)	1003 lb	1495/-14 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
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- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

86046694





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground A + Second A...**  
Level: **Ground Floor**  
Label: **B38 (-3R) - i54892**  
Type: **Beam**

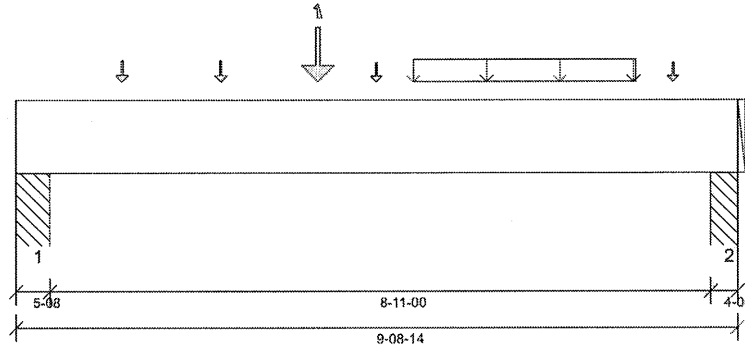
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/03/2022 19:48



### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 4 1/2"
- 1334 psi Column @ 9'- 5 1/2"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS



### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 1"	1.25D + 1.5L	1.00	6165 lb ft	26531 lb ft	Passed - 23%
Factored Shear:	1'- 5 3/8"	1.25D + 1.5L	1.00	2044 lb	14414 lb	Passed - 14%
Live Load (LL) Pos. Defl.:	4'- 9 11/16"	L		0.046"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 9/16"	D + L		0.080"	L/240	Passed - L/999

### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08"	1.25D + 1.5L	1.00	2068 lb		25225 lb	25687 lb	Passed - 8%
2	4'-06"	1.25D + 1.5L	1.00	1867 lb		20065 lb	20432 lb	Passed - 9%

### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 8 7/8"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	5'- 4 7/16"	8'- 4 7/16"	Smoothed Load	Front	64 lb/ft	128 lb/ft	-	-
Point	1'- 5 1/4"	1'- 5 1/4"	J3(i54701)	Front	67 lb	178 lb	-	-
Point	2'- 9 1/4"	2'- 9 1/4"	J3(i54792)	Front	64 lb	170 lb	-	-
Point	4'- 1"	4'- 1"	B18 (-3R)(i54371)	Front	614 lb	683/-9 lb	-	-
Point	4'- 10 7/16"	4'- 10 7/16"	J3(i54929)	Front	57 lb	115 lb	-	-
Point	8'- 10 7/16"	8'- 10 7/16"	J3(i54829)	Front	56 lb	113 lb	-	-

### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	P12(i54771)	621 lb	861/-5 lb	-	-
2	9'- 4 1/2"	9'- 8 7/8"	P12(i54318)	556 lb	782/-4 lb	-	-

### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

83046695



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Second A W Elevator WO**  
Level: **Second Floor**  
Label: **B39 - i55302**  
Type: **Beam**

**1 Ply Member**

**11 7/8" NI-20**

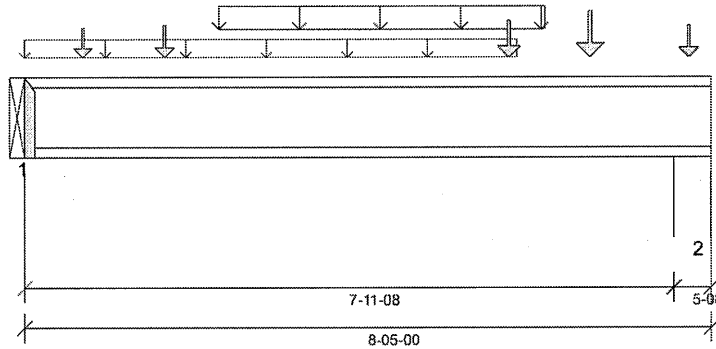
Status:

**Design  
Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/04/2022 11:16



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 769 psi Beam @ 0'
- 615 psi Wall @ 8'- 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 4 5/8"	1.25D + 1.5L	1.00	2925 lb ft	5580 lb ft	Passed - 52%
Factored Shear:	7'- 11 7/16"	1.25D + 1.5L	1.00	1436 lb	2240 lb	Passed - 64%
Live Load (LL) Pos. Defl.:	4'- 15/16"	L		0.066"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 1 1/16"	D + L		0.130"	L/240	Passed - L/732

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	1-12	1.25D + 1.5L	1.00	1290 lb		1970 lb	-	Passed - 65%
2	5-08	1.25D + 1.5L	1.00	1702 lb		2240 lb	8459 lb	Passed - 76%

#### CONNECTOR INFORMATION

ID	Part No.	Manufacturer	Nailing Requirements			Other Information or Requirement for Reinforcement Accessories		
			Top	Face	Member			
1	LT251188		-	-	-	Connector manually specified by the user.		

\* Connectors: Refer to manufacturer's specifications, fasteners requirements and installation instruction. Where header fasteners are longer than the width of the supporting member, install backer block or clinch header nails.

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 5"	Self Weight	Top	3 lb/ft	-	-	-
Uniform	0'	6'- 1/2"	20(i51780)	Top	61 lb/ft	-	-	-
Uniform	2'- 4 5/8"	6'- 4 5/8"	Smoothed Load	Back	44 lb/ft	118 lb/ft	-	-
Point	5'- 11 5/16"	5'- 11 5/16"	-	Front	191 lb	49 lb	-	-
Point	6'- 11 1/4"	6'- 11 1/4"	-	Front	94 lb	251 lb	-	-
Point	8'- 1 3/4"	8'- 1 3/4"	J7(i55666)	Front	50 lb	134 lb	-	-
Point	0'- 8 5/8"	0'- 8 5/8"	J6(i57067)	Back	43 lb	115 lb	-	-
Point	1'- 8 5/8"	1'- 8 5/8"	J6(i56330)	Back	52 lb	138 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'	B30(i55779)	465 lb	471 lb	-	-
2	7'- 11 1/2"	8'- 5"	25(i51910)	533 lb	692 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
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86046696



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Second A W Elevator WO**  
Level: **Second Floor**  
Label: **B40 - i56220**  
Type: **Beam**

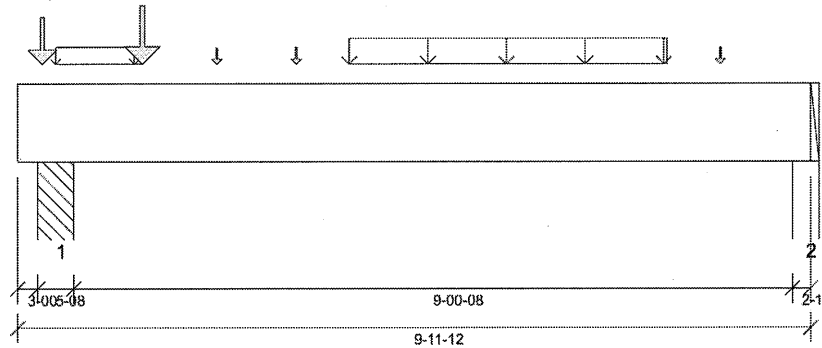
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/04/2022 11:17



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 1334 psi Column @ 0'- 5 3/4"
- 615 psi Wall @ 9'- 10"

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ 12" O/C  
STAGGERED IN 2 ROWS

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	2'- 6 1/8"	1.25D + 1.5L	1.00	7777 lb ft	26531 lb ft	Passed - 29%
Factored Neg. Moment:	0'- 5 3/4"	1.25D + 1.5L	1.00	914 lb ft	25803 lb ft	Passed - 4%
Factored Shear:	1'- 8 3/8"	1.25D + 1.5L	1.00	7628 lb	14414 lb	Passed - 53%
Live Load (LL) Pos. Defl.:	4'- 9 3/8"	L		0.064"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 9 11/16"	D + L		0.118"	L/240	Passed - L/923

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5-08	1.25D + 1.5L	1.00	13211 lb		25225 lb	25687 lb	Passed - 52%
2	2-12	1.25D + 1.5L	1.00	2144 lb		12613 lb	5921 lb	Passed - 36%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 11 3/4"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'- 5 3/4"	1'- 6 7/8"	FC3 Floor Decking (Plan View Fill)	Top	47 lb/ft	-	-	-
Uniform	4'- 2 1/8"	8'- 2 1/8"	Smoothed Load	Front	106 lb/ft	122 lb/ft	-	-
Point	1'- 6 7/8"	1'- 6 7/8"	B30(i55779)	Front	2098 lb	3131 lb	-	-
Point	2'- 6 1/8"	2'- 6 1/8"	J6(i57067)	Front	102 lb	118 lb	-	-
Point	3'- 6 1/8"	3'- 6 1/8"	J6(i56330)	Front	123 lb	142 lb	-	-
Point	8'- 10 1/8"	8'- 10 1/8"	J6(i56332)	Front	107 lb	139 lb	-	-
Point	0'- 3 3/4"	0'- 3 3/4"	B22(i56221)	Back	1152 lb	2669 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'- 3"	0'- 8 1/2"	Pt1(i55224)	3470 lb	5851 lb	-	-
2	9'- 9"	9'- 11 3/4"	9(i41702)	757 lb	864 lb	-	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- The deflection at the cantilever for either live and/or total loads is less than 3/8" and therefore has been excluded from the deflection ratio considerations.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.
- Bearing capacity of member at support 1, 2 was verified for the effect of concentrated load applied near the support. At support 1. Required Load Area: L=3.500", W=3.500". LDF=1.00, Pf=5444 lb, Qr=13759 lb, Result=39.56%.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



82046097





Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground B + Second B (3,**  
Level: **Second Floor**  
Label: **B41 - i51809**  
Type: **Beam**

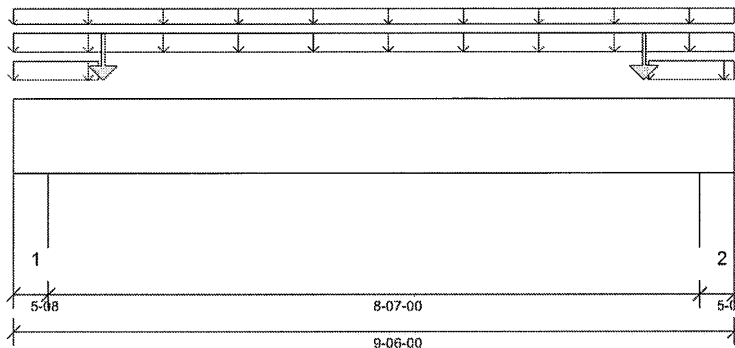
**2 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/05/2022 13:45



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:

Top: 0' Bottom: 8'- 7"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 4 1/2"
- 615 psi Wall @ 9'- 1 1/2"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	4'- 9"	1.4D	0.65	1828 lb ft	17245 lb ft	Passed - 11%
Factored Shear:	8'- 5/8"	1.25D + 1.5L	0.86	1020 lb	12332 lb	Passed - 8%
Total Load (TL) Pos. Defl.:	4'- 9"	D + L		0.033"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	5'-08	1.25D + 1.5L	0.86	1599 lb		21583 lb	10133 lb	Passed - 16%
2	5'-08	1.25D + 1.5L	0.86	1599 lb		21583 lb	10133 lb	Passed - 16%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	9'- 6"	Self Weight	Top	13 lb/ft	-	-	-
Uniform	0'	9'- 6"	E19(i41624)	Top	101 lb/ft	-	-	-
Uniform	0'	9'- 6"	FC3 Floor Decking (Plan View Fill)	Top	8 lb/ft	23 lb/ft	-	-
Uniform	0'	1'- 1 1/2"	E19(i41624)	Top	41 lb/ft	63 lb/ft	-	-
Uniform	8'- 4 1/2"	9'- 6"	E19(i41624)	Top	41 lb/ft	63 lb/ft	-	-
Point	1'- 2 1/4"	1'- 2 1/4"	E19(i41624)	Top	162 lb	228 lb	-	-
Point	8'- 3 3/4"	8'- 3 3/4"	E19(i41624)	Top	162 lb	228 lb	-	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 5 1/2"	E33(i51784)	790 lb	405 lb	-	-
2	9'- 1/2"	9'- 6"	E31(i51783)	792 lb	409 lb	-	-

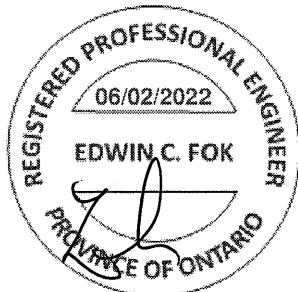
#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

NAIL ONE PLY TO ANOTHER WITH  
3-1/2" SPIRAL NAILS @ (2" O/C  
STAGGERED IN 2 ROWS



53046088



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground C + Second C (\$**  
Level: **Second Floor**  
Label: **B42 - i54253**  
Type: **Beam**

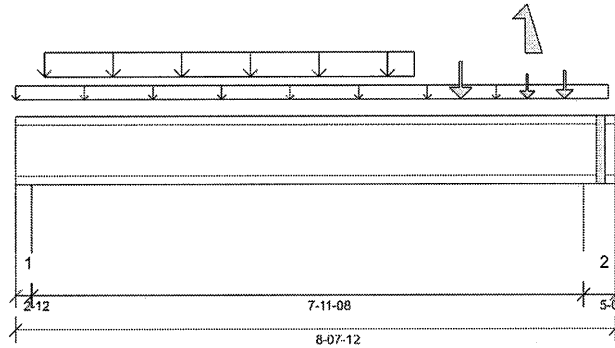
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version 8.5.3.233.Update5,15

Report Version: 2021.03.26 05/06/2022 16:53



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/4"
- 615 psi Wall @ 8'- 3 1/4"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	3'- 9"	1.25D + 1.5L	1.00	3568 lb ft	11160 lb ft	Passed - 32%
Factored Neg. Moment:	7'- 4 1/2"	0.9D + 1.5L + S	0.85	228 lb ft	9531 lb ft	Passed - 2%
Factored Shear:	8'- 2 3/16"	1.25D + 1.5L	1.00	1669 lb	4480 lb	Passed - 37%
Live Load (LL) Pos. Defl.:	4'- 2 7/16"	L		0.057"	L/360	Passed - L/999
Total Load (TL) Pos. Defl.:	4'- 2 1/4"	D + L		0.077"	L/240	Passed - L/999

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-12	1.25D + 1.5L	1.00	1661 lb		4180 lb	8459 lb	Passed - 40%
2	5-08	1.25D + 1.5L + S	1.00	1531 lb		4480 lb	16918 lb	Passed - 34%
2	5-08	0.9D + 1.5L + S	0.85		-199 lb	-	-	

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	8'- 7 3/4"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	8'- 6 1/2"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	0'- 5"	5'- 9"	Smoothed Load	Front	75 lb/ft	201 lb/ft	-	-
Point	6'- 5"	6'- 5"	J4(i54292)	Front	86 lb	230 lb	-	-
Point	7'- 4 1/2"	7'- 4 1/2"	APP (CANT.) (i54354)	Front	-80 lb	153/-219 lb	-175 lb	-
Point	7'- 11"	7'- 11"	J4(i54311)	Front	83 lb	116 lb	-	-

#### UNFACTORED REACTIONS

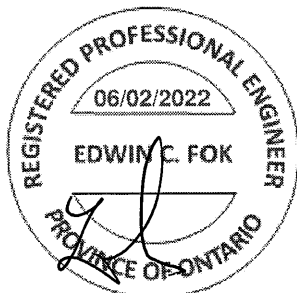
ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/4"	23(i51824)	325 lb	840/-24 lb	-19 lb	-
2	8'- 2 1/4"	8'- 7 3/4"	4(i41664)	275 lb	891/-195 lb	-156 lb	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



86046099



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground C + Second C (\$**  
Level: **Second Floor**  
Label: **B43 - i54260**  
Type: **Beam**

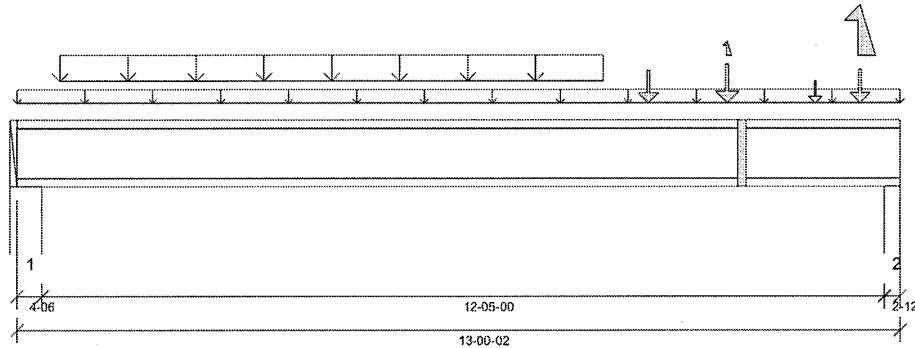
**2 Ply Member**  
**11 7/8" NI-20**

Status:  
**Design Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in Mitek® Structure Version 8.5.3.233.Update5.15

Report Version: 2021.03.26 05/06/2022 16:53



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, CBC 2018, ABC 2019, OBC 2012 (2019 Amendment)

Design Methodology: LSD

Service Condition: Dry

LL Deflection Limit: L/360,

TL Deflection Limit: L/240,

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 1 1/2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 3 3/8"
- 615 psi Wall @ 12'- 10 3/8"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	6'- 7 5/8"	1.25D + 1.5L	1.00	9712 lb ft	11160 lb ft	Passed - 87%
Factored Neg. Moment:	12'- 3 1/8"	0.9D + 1.5L + S	0.79	165 lb ft	8764 lb ft	Passed - 2%
Factored Shear:	0'- 4 7/16"	1.25D + 1.5L	1.00	2910 lb	4480 lb	Passed - 65%
Live Load (LL) Pos. Defl.:	6'- 6 13/16"	L		0.313"	L/360	Passed - L/476
Total Load (TL) Pos. Defl.:	6'- 6 5/8"	D + L		0.431"	L/240	Passed - L/345

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	4-06	1.25D + 1.5L	1.00	2929 lb		4480 lb	13457 lb	Passed - 65%
2	2-12	1.25D + 1.5L + S	1.00	2691 lb		4180 lb	8459 lb	Passed - 64%
2	2-12	0.9D + 1.5L + S	0.79		-212 lb	-	-	

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	13'- 1/8"	Self Weight	Top	6 lb/ft	-	-	-
Uniform	0'	13'- 1/8"	FC3 Floor Decking (Plan View Fill)	Top	7 lb/ft	19 lb/ft	-	-
Uniform	0'- 7 5/8"	8'- 7 5/8"	Smoothed Load	Front	86 lb/ft	229 lb/ft	-	-
Point	9'- 3 5/8"	9'- 3 5/8"	J2(i54041)	Front	97 lb	260 lb	-	-
Point	10'- 5 1/2"	10'- 5 1/2"	-	Front	130 lb	334/-20 lb	-14 lb	-
Point	11'- 9 1/8"	11'- 9 1/8"	J2(i54013)	Front	44 lb	119 lb	-	-
Point	12'- 5 1/16"	12'- 5 1/16"	-	Front	74/-257 lb	301/-228 lb	-188 lb	-

#### UNFACTORED REACTIONS

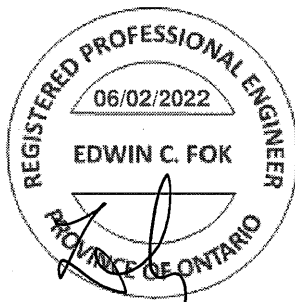
ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 4 3/8"	E12(i41614)	573 lb	1470/-15 lb	-12 lb	-
2	12'- 9 3/8"	13'- 1/8"	23(i51824)	365 lb	1622/-233 lb	-190 lb	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.



52046700



Customer: **Gold Park Homes**  
Job Address: **Pine Valley Ph2**  
City: **Vaughan**  
Job Track: **45147**

Job Name: **343077 Ground C + Second C (\$**  
Level: **Ground Floor**  
Label: **B44 - i54757**  
Type: **Beam**

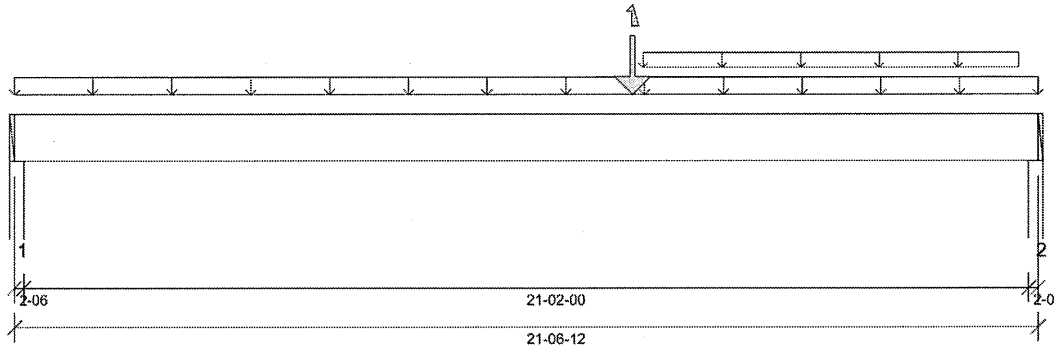
**4 Ply Member**  
**1 3/4" x 11 7/8" 1.55E**  
**TimberStrand® LSL**

Status:  
**Design**  
**Passed**

Illustration Not to Scale. Pitch: 0/12

Designed by Single Member Design Engine in MiTek® Structure Version  
8.5.3.233.Update5.15

Report Version: 2021.03.26 05/06/2022 16:54



#### DESIGN INFORMATION

Building Code: NBCC 2015, Part9, BCBC 2018,  
ABC 2019, OBC 2012 (2019  
Amendment)  
Design Methodology: LSD  
Service Condition: Dry  
LL Deflection Limit: L/360,  
TL Deflection Limit: L/240.

#### Lateral Restraint Requirements:

Both ends of the member and the outer supports  
must be laterally restrained. Top and bottom edges  
of the member must be fully restrained or have the  
following maximum unbraced length:  
Top: 0' Bottom: 21'- 2"

#### Factored Resistance of Support Material:

- 615 psi Wall @ 0'- 1 3/8"
- 615 psi Wall @ 21'- 5 3/8"

#### ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Factored Pos. Moment:	13'- 1/8"	1.25D + 1.5L	1.00	29608 lb ft	53063 lb ft	Passed - 56%
Factored Shear:	20'- 4 1/2"	1.25D + 1.5L	1.00	3879 lb	28828 lb	Passed - 13%
Live Load (LL) Pos. Defl.:	11'- 3 13/16"	L		0.679"	L/360	Passed - L/374
Total Load (TL) Pos. Defl.:	11'- 3 1/4"	D + L		0.975"	L/240	Passed - L/260
Permanent Deflection:	11'- 2"			-	L/360	Passed - L/883

#### SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Factored Downward Reaction	Factored Uplift Reaction	Factored Resistance of Member	Factored Resistance of Support	Result
1	2-06	1.25D + 1.5L	1.00	3033 lb		21785 lb	10228 lb	Passed - 30%
2	2-06	1.25D + 1.5L	1.00	4023 lb		21785 lb	10228 lb	Passed - 39%

#### SPECIFIED LOADS

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Wind (W)
Self Weight	0'	21'- 6 3/4"	Self Weight	Top	26 lb/ft	-	-	-
Uniform	0'	21'- 6 3/4"	FC2 Floor Decking (Plan View Fill)	Top	15 lb/ft	40 lb/ft	-	-
Uniform	13'- 2 7/8"	21'- 1 7/8"	FC2 Floor Decking (Plan View Fill)	Top	5 lb/ft	-	-	-
Point	13'- 1/8"	13'- 1/8"	23(151824)	Top	721 lb	2462/-257 lb	-209 lb	-

#### UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Wind (W)
1	0'	0'- 2 3/8"	W12(41590)	728 lb	1382/-99 lb	-80 lb	-
2	21'- 4 3/8"	21'- 6 3/4"	W17(41595)	919 lb	1950/-158 lb	-129 lb	-

#### DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Lateral stability factor (KL) was based on user preference to use the width of one ply.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
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- When the applied loads are coming from a member/post/wall above that does not sit directly on this beam, adequate load transfer elements, such as squash blocks, wall studs, or beveled plates are required to transfer the loads to this beam.

#### PLY TO PLY CONNECTION

- Member design assumed proper ply to ply connection by others. Fastener spacing along length of member must not exceed 4 times depth of member. Verify connection between plies according to code specification and follow the manufacturer's installation instruction. Loads assumed to be distributed equally to each ply.

CONNECT & PUT MEMBERS  
WITH SIMPSON SD-22624  
WOOD SCREWS @ 24" O.C.,  
STACKED IN 2 ROWS  
(TOP LOADED)



22046701

## Maximum Floor Spans – M4.1, L/360

### Design Criteria

Spans:	Simple span
Loads:	Live load = 40 psf and dead load = 20 psf
Deflection limits:	L/360 under live load and L/240 under total load
Sheathing:	3/4 in. nailed-glued oriented strand board (OSB) sheathing



### Maximum Floor Spans

Joist depth	Joist series	Bare On centre spacing				1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	15'-11"	15'-0"	14'-6"	13'-5"	16'-5"	15'-5"	14'-6"	13'-5"
	NI-40x	17'-0"	16'-0"	15'-5"	14'-10"	17'-5"	16'-5"	15'-10"	14'-11"
	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-7"	16'-7"	16'-0"	15'-4"
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	16'-1"
11-7/8"	NI-20	17'-11"	16'-11"	16'-3"	15'-8"	18'-7"	17'-5"	16'-10"	16'-1"
	NI-40x	19'-4"	17'-11"	17'-3"	16'-7"	19'-11"	18'-6"	17'-9"	17'-0"
	NI-60	19'-7"	18'-2"	17'-6"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"
	NI-80	21'-1"	19'-6"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"
	NI-90	21'-6"	19'-10"	18'-11"	17'-11"	22'-0"	20'-4"	19'-5"	18'-4"
14"	NI-40x	21'-5"	19'-11"	18'-11"	18'-0"	22'-1"	20'-7"	19'-7"	18'-7"
	NI-60	21'-10"	20'-2"	19'-3"	18'-3"	22'-6"	20'-10"	19'-11"	18'-10"
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"
	NI-90	23'-10"	22'-1"	21'-0"	19'-10"	24'-5"	22'-7"	21'-6"	20'-4"
16"	NI-60	23'-9"	22'-0"	21'-0"	19'-10"	24'-6"	22'-9"	21'-8"	20'-7"
	NI-80	25'-6"	23'-7"	22'-5"	21'-2"	26'-2"	24'-3"	23'-1"	21'-10"
	NI-90	26'-0"	24'-0"	22'-10"	21'-6"	26'-7"	24'-8"	23'-5"	22'-2"

Joist depth	Joist series	Mid-span blocking with 1x4 inch strap On centre spacing				Mid-span blocking and 1/2 in. gypsum ceiling On centre spacing			
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
9-1/2"	NI-20	17'-1"	15'-5"	14'-6"	13'-5"	17'-1"	15'-5"	14'-6"	13'-5"
	NI-40x	18'-8"	17'-6"	16'-7"	14'-11"	19'-2"	17'-8"	16'-7"	14'-11"
	NI-60	18'-11"	17'-8"	16'-10"	15'-7"	19'-5"	18'-0"	16'-10"	15'-7"
	NI-80	20'-3"	18'-10"	17'-11"	17'-2"	20'-8"	19'-3"	18'-4"	17'-5"
11-7/8"	NI-20	20'-3"	18'-8"	17'-6"	16'-1"	20'-7"	18'-8"	17'-6"	16'-1"
	NI-40x	21'-10"	20'-4"	19'-0"	17'-0"	22'-5"	20'-10"	19'-0"	17'-0"
	NI-60	22'-1"	20'-7"	19'-8"	18'-7"	22'-8"	21'-2"	20'-3"	18'-8"
	NI-80	23'-8"	22'-0"	20'-11"	19'-10"	24'-1"	22'-6"	21'-6"	20'-4"
	NI-90	24'-1"	22'-5"	21'-4"	20'-2"	24'-7"	22'-11"	21'-10"	20'-8"
14"	NI-40x	24'-5"	22'-9"	20'-11"	18'-8"	25'-1"	22'-11"	20'-11"	18'-8"
	NI-60	24'-10"	23'-2"	22'-1"	20'-10"	25'-6"	23'-10"	22'-9"	21'-4"
	NI-80	26'-6"	24'-8"	23'-6"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"
	NI-90	27'-0"	25'-1"	23'-11"	22'-7"	27'-6"	25'-8"	24'-6"	23'-2"
16"	NI-60	27'-3"	25'-5"	24'-3"	22'-11"	28'-0"	26'-2"	25'-0"	23'-1"
	NI-80	29'-1"	27'-1"	25'-9"	24'-4"	29'-8"	27'-9"	26'-5"	25'-0"
	NI-90	29'-7"	27'-6"	26'-2"	24'-9"	30'-2"	28'-2"	26'-10"	25'-5"

### Notes:

1. The tabulated clear spans are based on CSA O86-14 and NBC 2015, and are applicable to residential floor construction meeting the above design criteria.
2. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
3. Minimum bearing length shall be 1-3/4 inch for end bearings, and 3-1/2 inches for intermediate bearings.
4. Bearing stiffeners are not required when I-joists are used in accordance with this table, except as required for hangers.
5. Nordic I-joists are listed in CCMC Evaluation Report 13032-R and APA Product Report PR-L274C.

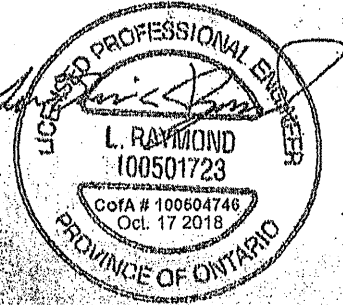
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

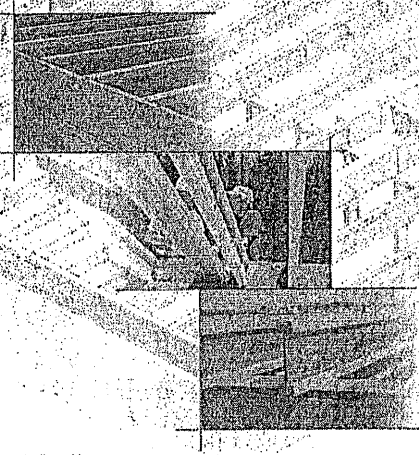
This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



# NORDIC ENGINEERED WOOD

## INSTALLATION GUIDE FOR RESIDENTIAL FLOORS



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### SAFETY AND CONSTRUCTION PRECAUTIONS

#### WARNING

I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

Avoid Accidents by Following these Important Guidelines:



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



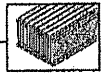
Never stack building materials over unbraced I-joists. Once sheathed, do not over-stress I-joist with concentrated loads from building materials.

1. Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross bracing at joist ends. When I-joists are supported continuously over interior supports and a load-bearing wall is placed at that location, blocking will be required at the interior support.
2. When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
3. Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on center, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing in a lateral restraint at the end of each bay. Top ends of outlying bracing over at least two I-joists.
4. On sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
5. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bracing.
6. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
7. Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole size and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

### STORAGE AND HANDLING GUIDELINES

1. Bundle wrap can be slippery when wet. Avoid walking on wrapped bundles.
2. Store, stack, and handle I-joists vertically and level only.
3. Always stack and handle I-joists in the upright position only.
4. Do not store I-joists in direct contact with the ground and/or debris.
5. Protect I-joists from weather, and use spacers to separate bundles.
6. Bundled units should be kept intact until time of installation.
7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury to your work crew.
8. Pick I-joists in bundles as shipped by the supplier.
9. Orient the bundles so that the webs of the I-joists are vertical.
10. Pick the bundles at the 5th points, using a spreader bar if necessary.
11. Do not handle I-joists in a horizontal orientation.
12. NEVER USE OR TRY TO REPAIR A DAMAGED I-JOIST.

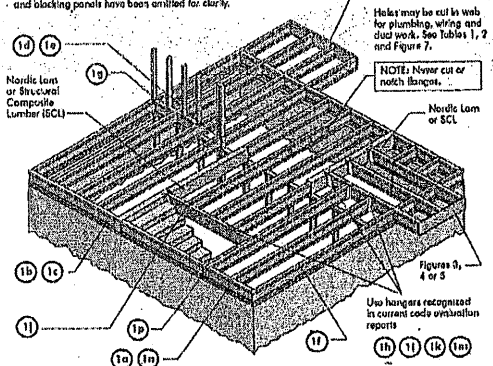


### INSTALLING NORDIC I-JOISTS

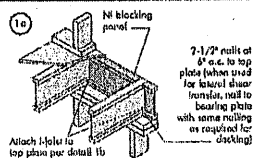
1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contact your supplier.
2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.
4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple-span joists must be level.
5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings.
6. When using hangers, seal I-joist flange to hanger bottoms to minimize rot/moisture.
7. Leave a 1/16-inch gap between the I-joist and a hanger.
8. Concentrated loads greater than those that can normally be supported by residential construction should only be applied to the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the I-joist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the I-joist. Or, attach the load to blocking that has been securely fastened to the I-joist webs.
9. Never install I-joists where they will be permanently exposed to weather, or where they will remain in direct contact with concrete or masonry.
10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joist or I-joist blocking panels.
11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. I-joist blocking panels or other engineered wood products - such as rim board - must be cut to fit between the I-joists, and on I-joist-compatible depth selected.
13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, support the bottom flange of all cantilevered I-joists at the end support and to the cantilever extension. In the completed structure, the apparent wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used.
14. If square-edge panels are used, edges must be supported between I-joists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed.
15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

FIGURE 1  
TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

Some framing requirements such as erection bracing and blocking panels have been omitted for clarity.

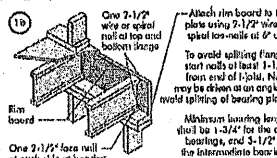


All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122") dia 1 common steel nails may be substituted for 2-1/2" (0.128") dia common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity.



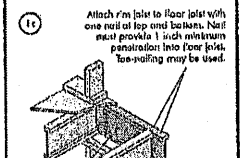
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load <sup>a</sup> (psf)
NJ Joists	3,300

<sup>a</sup>The uniform vertical load is limited to a joist depth of 1.6 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



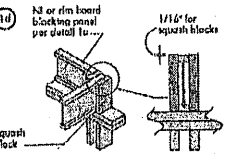
Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load <sup>a</sup> (psf)
1-1/2" Rim Board Plus	6,000

<sup>a</sup>The uniform vertical load is limited to a rim board depth of 1.6 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load <sup>a</sup> (psf)
NJ Joists	3,300

<sup>a</sup>The uniform vertical load is limited to a joist depth of 1.6 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



Blocking Panel or Rim Joist	Maximum Factored Uniform Vertical Load <sup>a</sup> (psf)
NJ Joists	3,300

<sup>a</sup>The uniform vertical load is limited to a joist depth of 1.6 inches or less and is based on standard term load duration. It shall not be used in the design of a bearing member, such as joist, header, or rafter. For concentrated vertical load transfer, see detail 1d.



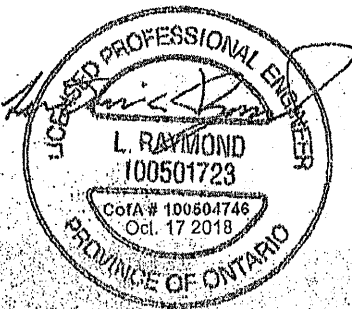
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Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



## MAXIMUM FLOOR SPANS

- Maximum clear spans applicable to single-span or multiple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate live loads are based on the factors listed in 1.504 + 1.2SD. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. For multiple-span applications, the end spans shall be 40% or more of the adjacent span.
- Spans are based on a composite floor with glued-nail or oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive shall meet the requirements given in CGOS-71.26 Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the use of gypsum or other low modulus blocking at mid-span.
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- This span chart is based on uniform loads. For applications with other than uniform loads, an engineering analysis may be required based on the use of the design properties.
- Tables are based on Limit States Design per CAN/CSA C06-09 Standard, and NBC 7010.
- SI units conversion: 1 inch = 25.4 mm, 1 foot = 0.305 m

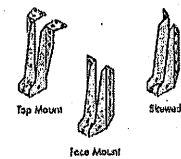
MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS  
SIMPLE AND MULTIPLE SPANS

I-Joist Size	2-1/2" x 7-1/2" (63.5 x 190.5 mm)				3-1/2" x 7-1/2" (88.9 x 190.5 mm)				4-1/2" x 7-1/2" (114.3 x 190.5 mm)				5-1/2" x 7-1/2" (140.0 x 190.5 mm)				6-1/2" x 7-1/2" (165.1 x 190.5 mm)			
	19.2" (488 mm)	24" (609.6 mm)	30" (762 mm)	36" (914 mm)	19.2" (488 mm)	24" (609.6 mm)	30" (762 mm)	36" (914 mm)	19.2" (488 mm)	24" (609.6 mm)	30" (762 mm)	36" (914 mm)	19.2" (488 mm)	24" (609.6 mm)	30" (762 mm)	36" (914 mm)	19.2" (488 mm)	24" (609.6 mm)	30" (762 mm)	36" (914 mm)
10-10	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
11-11	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
12-12	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
13-13	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
14-14	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
15-15	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
16-16	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
17-17	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
18-18	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
19-19	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
20-20	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
21-21	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
22-22	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
23-23	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
24-24	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
25-25	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
26-26	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
27-27	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
28-28	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
29-29	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'
30-30	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'	12.1'	14.2'	15.9'	17.4'

CCMC EVALUATION REPORT 13024R

## I-JOIST HANGERS

- Hangers shown illustrate the three most commonly used metal hangers to support I-joists.
- All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the clear of the hangers do not laterally brace the top flange of the I-joist.

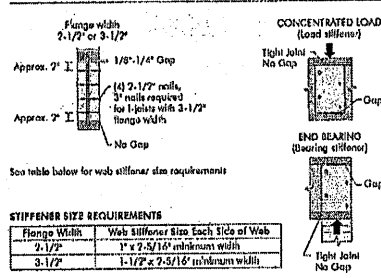


## WEB STIFFENERS

## RECOMMENDATIONS:

- A bearing stiffener is required in all engineered applications with hanger reactions greater than shown in the I-joist properties table found in the Joist Construction Guide (C101) the gap between the stiffener and the flange is at the top.
- A bearing stiffener is required when the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and support, the top flange. The gap between the stiffener and flange is at the top.
- A load stiffener is required in locations where a lateral concentrated load greater than 3,370 lbs is applied to the top flange between supports, or in the case of a cantilever, anywhere between the cantilever tip and the support. These values are for standard term load duration, and may be adjusted for other load durations as permitted by the code. The gap between the stiffener and the flange is at the bottom.

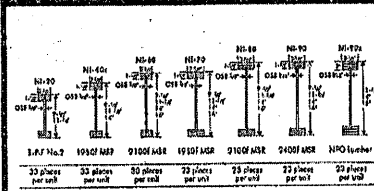
SI units conversion: 1 inch = 25.4 mm

FIGURE 2  
WEB STIFFENER INSTALLATION DETAILS

## STIFFENER SIZE REQUIREMENTS

Flange Width	Web Stiffener Size Each Side of Web
2-1/2"	1" x 7-1/2" minimum width
3-1/2"	1-1/2" x 7-1/2" minimum width

## NORDIC I-JOIST SERIES



Chenier's Chissegouard Ltd. harvests its own trees, which enables Nordic products to adhere to strict quality control procedures throughout the manufacturing process. Every phase of the operation, from forest to the finished product, reflects our commitment to quality.

Nordic Engineered Wood I-joists use only finger-jointed black spruce lumber in their flanges, ensuring consistent quality, superior strength, and longer span carrying capacity.

**10** Transfer load from above to bearing below. Install equal blocks per detail 1d. Match bearing area of blocks below to post above.

**11** Use double I-joist for loads up to 3,300 gft. Double I-joist for loads up to 6,500 gft (filler block not required). Attach I-joist to top plate using 2-1/2" nails at 6" o.c.

**12** Provide backup for lifting attachment unless reliable chocking is used.

**13** Rein board may be used in lieu of I-joist. Backup is not required when rim board is used. Blocking per code shall be carried to the foundation.

**14** Load bearing wall above shall align vertically with the bearing below. Other conditions, such as offset bearing walls, are not covered by this detail.

**15** Blocking required over all interior supports under load-bearing walls or when floor joists are not continuous over support.

**16** Backer block (use 2 hanger load exceeds 360 lbs) before installing a backer block to a double I-joist, drive three additional 3" nails through the walls and backer block where the backer block will fit. Clinch, install backer right to top flange. Use twelve 3" nails, clinched when possible. Maximum tapered resistance for hanger for this detail = 1,220 lbs.

**17** Double I-joist header.

**18** Top- or face-mount hanger.

**19** Notes: Unless hanger sides internally support the top flange, bearing stiffeners shall be used.

**20** Backer block required (both sides for face-mount hangers).

**21** Filler block per detail 1p.

**22** For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

**23** BACKER BLOCKS (blocks must be long enough to permit required nailing without splitting).

Flange Width	Material Thickness Required*	Minimum Depth**
2-1/2"	1"	5-1/2"
3-1/2"	1-1/2"	7-1/4"

\* Minimum grade for backer block material shall be S-P No. 2 or better for solid sawn lumber and wood structural panels conforming to CAN/CSA-C095 or CAN/CSA-C097 Standard.

\*\* For face-mount hangers use not less than 3-1/4" for joist with 1-1/2" thick flanges. For 2" thick flanges use not depth minus 4-1/4".

**24** Top- or face-mount hanger installed per manufacturer's recommendations.

**25** For nailing schedule for multiple beams, see the manufacturer's recommendations.

**26** Notes: Unless hanger sides internally support the top flange, bearing stiffeners shall be used.

**27** Top-mount hanger installed per manufacturer's recommendations.

**28** Notes: Unless hanger sides internally support the top flange, bearing stiffeners shall be used.

**29** 2x plate flush with inside face of wall or beam. 1/8" overhang allowed past inside face of wall or beam.

**30** Multiple I-joist header with full depth filler block shown. Nordic Lumber or SCL headers may also be used. Verify double I-joist capacity to support concentrated loads.

**31** Filler block per detail 1p.

**32** Install hanger per manufacturer's recommendations.

**33** Backer block attached per detail 1b. Nail with twelve 3" nails, clinch when possible.

**34** Maximum support capacity = 1,220 lbs.

**35** Do not bowl-cut joint beyond inside face of wall.

**36** Attach I-joist per detail 1b.

**37** Notes: Blocking required at bearing for lateral support, not shown for clarity.

**38** Lumber 2x4 min., install block to face of adjacent wall. Two 2-1/2" spud nails from each web to lumber place, alternate on opposite side.

**39** NI blocking panel.

**40** Options: Minimum 1x4 inch drop applied to underside of joist in blocking line or 1/2 inch minimum gypsum ceiling attached to underside of joist.

**41** Cuts 2-1/2" nails at top and bottom flange.

**42** Two 2-1/2" nails from each web to lumber place.

**43** Two 2-1/2" nails from each web to lumber place.

**44** I-joist blocking panel.

**45** One 2-1/2" nails are 8" o.c. only.

**46** One 2-1/2" nails at 6" o.c.

**47** Notes:

- In some local codes, blocking is prescriptively required in the first joist space (for first and second joist space) next to the center joist. Where required, see local code requirements for spacing of the blocking.
- All nailing is common nailing in this detail.

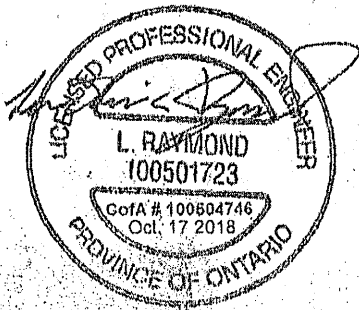
The construction details for residential designs are prone to changes.

Details released after April 2014 supersedes N-C301

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

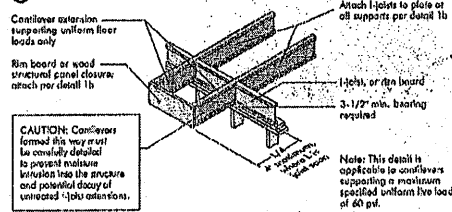
Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



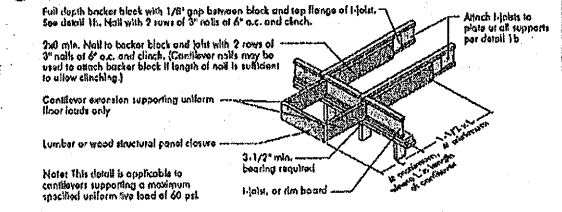
N-C301/April 2014

### CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)

#### 3a I-JOIST CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

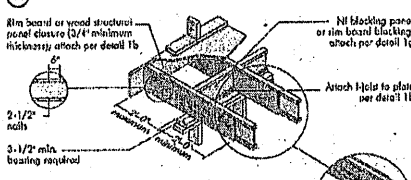


#### 3b LUMBER CANTILEVER DETAIL FOR BALCONIES (No Wall Load)

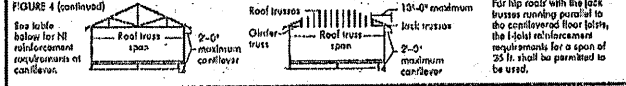
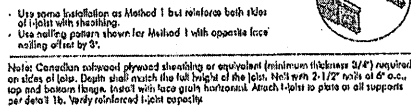


### CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

#### 4a Method 1 - SHEATHING REINFORCEMENT ONE SIDE



#### Method 2 - SHEATHING REINFORCEMENT TWO SIDES



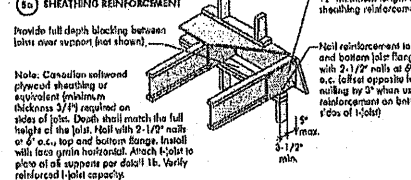
#### CANTILEVER REINFORCEMENT METHODS ALLOWED

COPING OF THE ROOF																					
COPING OF THE ROOF																					
7-1/2	26	N	N	N	1	2	N	N	1	2	X	X	N	2	X	X	X	X	X	X	X
	28	N	N	N	1	1	X	X	1	1	X	X	N	2	X	X	X	X	X	X	X
	29	N	N	N	1	1	X	X	1	1	X	X	N	2	X	X	X	X	X	X	X
	32	N	N	N	1	2	X	X	N	N	2	2	X	1	1	X	X	X	X	X	X
	35	N	N	1	2	2	X	X	N	1	2	X	X	N	1	1	X	X	X	X	X
11-3/4	26	1	N	N	1	1	1	N	N	1	2	2	N	2	N	1	1	2	X	X	X
	30	1	N	N	1	1	1	2	N	N	1	1	2	2	N	1	1	2	2	X	X
	32	1	N	N	1	1	1	2	2	N	1	1	2	2	1	1	2	2	2	X	X
	34	1	N	N	1	1	1	2	2	N	1	1	2	2	1	1	2	2	2	X	X
	36	2	N	N	1	2	2	X	X	1	2	2	X	X	2	1	2	2	2	X	X
14	26	N	N	N	N	N	X	X	N	N	N	1	N	N	N	N	1	1	2	2	X
	30	N	N	N	N	N	X	X	N	N	N	1	N	N	N	N	1	1	2	2	X
	32	N	N	N	N	N	X	X	N	N	N	1	N	N	N	N	1	1	2	2	X
	34	N	N	N	N	N	1	N	N	N	1	2	N	N	N	N	1	1	2	2	X
	36	N	N	N	N	1	1	N	N	N	1	2	N	N	N	N	1	1	2	2	X
16	26	N	N	N	N	N	N	N	N	1	2	2	N	N	1	1	2	X	X	X	X
	30	N	N	N	N	N	N	N	N	N	1	1	N	N	N	N	1	1	2	2	X
	32	N	N	N	N	N	N	N	N	N	1	1	N	N	N	N	1	1	2	2	X
	34	N	N	N	N	N	N	N	N	N	1	1	N	N	N	N	1	1	2	2	X
	36	N	N	N	N	1	1	N	N	1	2	2	N	N	N	N	1	1	2	2	X

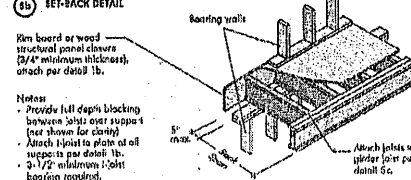
1. N = No reinforcement required.
2. 1 = 14 reinforcement spaced at 12\"/>

### BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

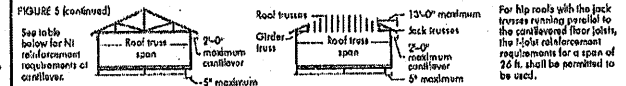
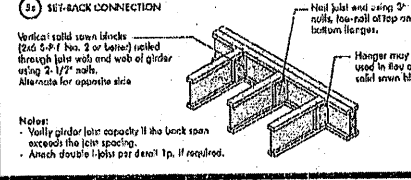
#### 5a SHEATHING REINFORCEMENT



#### 5b SET-BACK DETAIL



#### 5c SET-BACK CONNECTION



#### BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

Span (ft)	N	N				N				N				N				N			
		1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4	1	2	3	4
9-1/2	22	1	X	X	X	1	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	20	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	18	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	16	2	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	14	2	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
11-3/4	22	1	X	X	X	1	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	20	X	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	18	X	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	16	X	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	14	X	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
14	22	N	N	X	X	1	X	X	X	1	X	X	X	2	X	X	X	X	X	X	X
	20	1	X	X	X	1	X	X	X	1	X	X	X	2	X	X	X	X	X	X	X
	18	1	X	X	X	1	X	X	X	1	X	X	X	2	X	X	X	X	X	X	X
	16	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	14	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
16	22	1	X	X	X	1	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	20	1	X	X	X	1	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	18	1	X	X	X	1	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	16	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X
	14	1	X	X	X	2	X	X	X	2	X	X	X	2	X	X	X	X	X	X	X

1. N = No reinforcement required.
2. 1 = 14 reinforcement spaced at 12\"/>



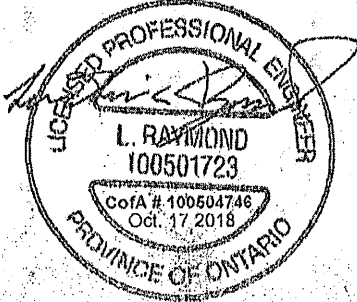
The construction details for residential designs are prone to changes.

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Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



## N-C301/April 2014

### WEB HOLES

#### RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

1. The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
2. Joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
3. Whenever possible, field-cut holes should be centred on the middle of the web.
4. The minimum size hole or its maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum 1/8 inch should always be maintained between the top or bottom of the hole or opening and its adjacent I-joist flange.
5. The sides of square holes or largest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
6. Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the largest side of the largest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
7. A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
8. Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a conditioned section of a joist. Holes of greater size may be permitted subject to verification.
9. A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
10. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the residential load above and as illustrated in Figure 7.
11. Limit three maximum 1/2 inch holes per span, of which one may be a duct chase opening.
12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1 LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 16 psf and Live Loads up to 40 psf

Span Length (ft)	16' Span		20' Span		24' Span		28' Span		32' Span		36' Span		40' Span	
	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
16	0	16	0	20	0	24	0	28	0	32	0	36	0	40
20	0	20	0	24	0	28	0	32	0	36	0	40	0	44
24	0	24	0	28	0	32	0	36	0	40	0	44	0	48
28	0	28	0	32	0	36	0	40	0	44	0	48	0	52
32	0	32	0	36	0	40	0	44	0	48	0	52	0	56
36	0	36	0	40	0	44	0	48	0	52	0	56	0	60
40	0	40	0	44	0	48	0	52	0	56	0	60	0	64

1. Above table may be used for joist spacing of 24 inches on centre or less.
2. Hole location distance is measured from inside face of support to centre of hole.
3. Distances in this table are based on uniformly loaded joists.

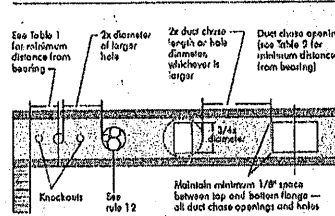
#### OPTIONALS

The above table is based on the joist used of four maximum spans. If the joists are placed at less than their full maximum span (see Maximum Floor Span), the minimum distance from the centreline of the hole to the face of any support (S) as given above may be reduced as follows:

$$D_{reduced} = \frac{S}{L} \times D$$

- Where:  $D_{reduced}$  = Distance from the inside face of any support to centre of hole, reduced for less than maximum span (see Maximum Floor Span). The reduced distance shall not be less than 3 inches from the face of the support to the hole.
- $S$  = The actual measured span distance between the inside face of supports (ft).
- $L$  = Span Adjustment Factor given in this table.
- $D$  = The minimum distance from the inside face of any support to centre of hole from this table.
- If  $L$  is greater than 1, use 1 in the above calculation for  $L$ .

FIGURE 7 FIELD-CUT HOLE LOCATOR



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances between holes.

Knockouts are precast holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never cut, cut or notch the flange, or overcut the web. Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid overcutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cuts between the holes is another good method to minimize damage to the joist.

TABLE 2 DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

Span Length (ft)	16' Span		20' Span		24' Span		28' Span		32' Span		36' Span		40' Span	
	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max	Min	Max
16	0	16	0	20	0	24	0	28	0	32	0	36	0	40
20	0	20	0	24	0	28	0	32	0	36	0	40	0	44
24	0	24	0	28	0	32	0	36	0	40	0	44	0	48
28	0	28	0	32	0	36	0	40	0	44	0	48	0	52
32	0	32	0	36	0	40	0	44	0	48	0	52	0	56
36	0	36	0	40	0	44	0	48	0	52	0	56	0	60
40	0	40	0	44	0	48	0	52	0	56	0	60	0	64

1. Above table may be used for joist spacing of 24 inches on centre or less.
2. Duct chase opening location distance is measured from inside face of support to centre of opening.
3. The above table is based on uniformly loaded joists only. For other applications, contact your local distributor.
4. Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf and a live load deflection limit of L/160. For other applications, contact your local distributor.

### INSTALLING THE GUEDED FLOOR SYSTEM

1. Wipe any mud, dirt, wax or ice from I-joist flanges before gluing.
2. Snap a chalk line across the I-joist floor line from the wall (or panel edge alignment) and on a boundary for spreading glue.
3. Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from the glue manufacturer.
4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
5. Apply a continuous line of glue (about 1/4 inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
6. Apply two lines of glue on I-joists where panel ends but to ensure proper gluing of each end.
7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time before laying the next row. Glue line may be continuous or spaced, but avoid gaps or voids by applying a thinner line (1/8 inch) than used on I-joist flanges.
8. Tap the second row of panels into place, using a block to protect groove edges.
9. Stagger and joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&O edges, is recommended. (Use a spacer tool or a 2-1/2" common nail to assure accurate and consistent spacing.)
10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerated cure times.) Use T-rings or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The finished deck can be walked on right away and will carry construction loads without damage to the glue bond.

#### FASTENERS FOR SHEATHING AND SUBFLOORING (1)

Span Length (ft)	Joist Spacing (in)	2"	2-1/4"	2"	2"	2"	2"
16	4/8	2"	2-1/4"	2"	2"	2"	2"
20	6/8	2"	2-1/4"	2"	2"	2"	2"
24	3/4	2"	2-1/4"	2"	2"	2"	2"

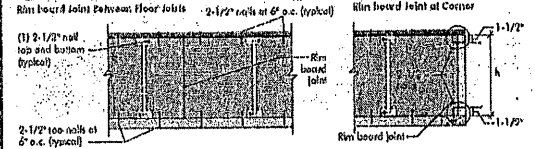
1. Fasteners of sheathing and subflooring shall conform to the above table.
2. Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
3. Flooring screws shall not be less than 1/8-inch in diameter.
4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess of the minimum shown.
5. Use only adhesives conforming to CAN/CSA-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Flooring for Floor Systems, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and vapour seal to be used, use only solvent-based glue; check with panel manufacturer.

Ref: NBC-CNBC, National Building Code of Canada 2010, Table 9.23.3.3.

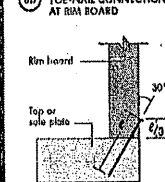
**IMPORTANT NOTE:** floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with your local distributor.

### RIM BOARD INSTALLATION DETAILS

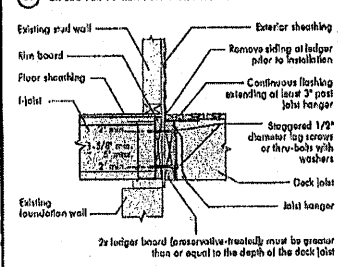
#### (a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT



#### (b) TOE-NAIL CONNECTION AT RIM BOARD



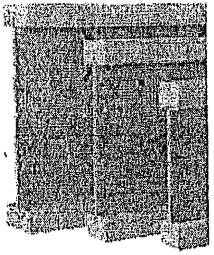
#### (c) 2X LEDGER TO RIM BOARD ATTACHMENT DETAIL



### PRODUCT WARRANTY

TimberTech warrants its products to be free from manufacturing defects in materials and workmanship for a period of 10 years.

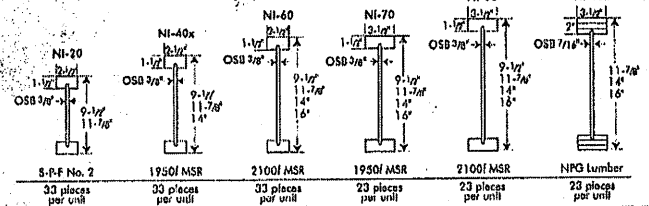
This warranty is void if the product is not installed in accordance with the manufacturer's instructions. The warranty is also void if the product is used in an application not intended by the manufacturer.



## CONSTRUCTION DETAILS FOR RESIDENTIAL FLOORS



Refer to the Installation Guide for Residential Floors for additional information.  
CCMC EVALUATION REPORT 13032-R



## WEB HOLE SPECIFICATIONS

RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS:

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- I-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified.
- Whenever possible, field cut holes should be centred on the middle of the web.
- The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained between the top or bottom of the hole or opening and the adjacent I-joist flange.
- The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- Where more than one hole is necessary, the distance between adjacent hole edges shall exceed twice the diameter of the largest round hole or twice the size of the largest square hole (or twice the length of the longest side of the largest rectangular hole or duct chase opening) and each hole and duct chase opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively.
- A knockout is not considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- Holes measuring 1-1/2 inches or smaller are permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to verification.
- A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- Limit three maximum size holes per span, of which one may be a duct chase opening.
- A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

TABLE 1  
LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

Joist Depth	Joist Series	Minimum Distance from Inside Face of Any Support to Centre of Hole (ft. - in.)											
		Round Hole Diameter (in.)											
		2	3	4	5	6	6-1/4	7	8	8-5/8	9	10	10-3/4
9-1/2"	NI-20	0-7"	1-4"	2-10"	4-3"	5-8"	6-0"	---	---	---	---	---	---
	NI-40x	0-7"	1-4"	3-0"	4-4"	6-0"	6-4"	---	---	---	---	---	---
	NI-60	1-3"	2-0"	4-0"	5-4"	7-0"	7-5"	---	---	---	---	---	---
	NI-70	2-3"	3-4"	4-9"	6-3"	8-0"	8-4"	---	---	---	---	---	---
	NI-80	2-3"	3-4"	5-0"	6-5"	8-2"	8-6"	---	---	---	---	---	---
11-7/8"	NI-20	0-7"	0-8"	1-0"	2-4"	3-8"	4-0"	5-0"	6-5"	7-9"	---	---	---
	NI-40x	0-7"	0-8"	1-3"	2-8"	4-0"	4-4"	5-5"	7-0"	8-4"	---	---	---
	NI-60	1-3"	1-8"	3-0"	4-3"	5-9"	6-0"	7-3"	8-10"	10-0"	---	---	---
	NI-70	1-3"	2-0"	4-0"	5-4"	6-9"	7-2"	8-4"	10-0"	11-2"	---	---	---
	NI-80	1-3"	2-10"	4-2"	5-4"	7-0"	7-5"	8-6"	10-3"	11-4"	---	---	---
14"	NI-40x	0-7"	0-8"	0-9"	2-5"	4-4"	4-9"	6-3"	---	---	---	---	---
	NI-60	0-7"	0-8"	0-8"	1-0"	2-4"	2-9"	3-9"	5-2"	6-0"	6-6"	8-3"	10-2"
	NI-70	0-9"	0-8"	1-8"	3-0"	4-3"	4-8"	5-8"	7-2"	8-0"	8-8"	10-4"	11-9"
	NI-80	0-9"	1-10"	3-0"	4-5"	5-10"	6-2"	7-3"	8-9"	9-7"	10-4"	12-0"	13-5"
	NI-90x	0-10"	2-0"	3-4"	4-9"	6-2"	6-5"	7-6"	9-0"	10-0"	10-8"	12-4"	13-9"
16"	NI-40x	0-7"	0-8"	0-8"	2-0"	3-9"	4-2"	5-5"	7-3"	8-5"	9-2"	---	---
	NI-60	0-7"	0-8"	0-8"	1-6"	2-10"	3-2"	4-2"	5-6"	6-4"	7-0"	8-5"	9-8"
	NI-70	0-7"	1-0"	2-3"	3-6"	4-10"	5-3"	6-3"	7-8"	8-8"	9-2"	10-8"	12-0"
	NI-80	0-7"	1-3"	2-6"	2-10"	3-2"	4-6"	5-6"	7-0"	8-0"	9-0"	11-0"	12-3"
	NI-90x	0-7"	0-8"	0-9"	2-0"	3-6"	4-0"	5-0"	6-9"	7-9"	8-4"	10-2"	11-6"

- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Hole location distance is measured from inside face of supports to centre of hole.
- Distances in this chart are based on uniformly loaded joists.
- The above table is based on the I-joist being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

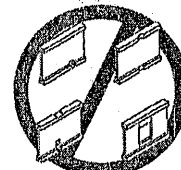
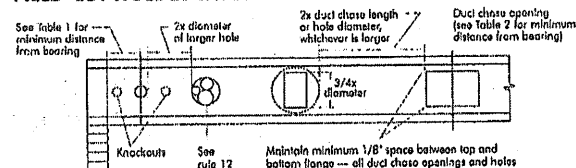
TABLE 2  
DUCT CHASE OPENING SIZES AND LOCATIONS

Simple Span Only

Joist Depth	Joist Series	Minimum Distance from Inside Face of Supports to Centre of Opening (ft. - in.)											
		Duct Chase Length (in.)											
		8	10	12	14	16	18	20	22	24			
9-1/2"	NI-20	4-1"	4-5"	4-10"	5-4"	5-8"	6-1"	6-6"	7-1"	7-5"			
	NI-40x	5-3"	5-8"	6-0"	6-5"	6-10"	7-3"	7-8"	8-2"	8-6"			
	NI-60	5-4"	6-2"	6-7"	7-1"	7-5"	8-0"	8-3"	8-9"	9-4"			
	NI-70	5-1"	5-8"	5-10"	6-3"	6-7"	7-1"	7-6"	8-1"	8-4"			
	NI-80	5-3"	5-8"	6-0"	6-5"	6-10"	7-3"	7-8"	8-2"	8-6"			
11-7/8"	NI-20	5-9"	6-2"	6-6"	7-1"	7-5"	7-9"	8-3"	8-9"	9-4"			
	NI-40x	6-8"	7-2"	7-6"	8-1"	8-5"	9-1"	9-5"	10-1"	10-9"			
	NI-60	7-3"	7-8"	8-0"	8-3"	8-9"	9-3"	9-9"	10-3"	11-0"			
	NI-70	7-1"	7-4"	7-9"	8-3"	8-7"	9-1"	9-6"	10-1"	10-4"			
	NI-80	7-2"	7-7"	8-0"	8-5"	8-10"	9-3"	9-8"	10-2"	10-8"			
14"	NI-40x	7-7"	8-1"	8-5"	8-10"	9-4"	9-8"	10-2"	10-8"	11-2"			
	NI-60	8-1"	8-7"	9-0"	9-4"	10-1"	10-7"	11-2"	11-8"	12-3"			
	NI-70	8-7"	9-1"	9-5"	9-10"	10-6"	10-8"	11-2"	11-7"	12-3"			
	NI-80	9-0"	9-3"	9-9"	10-1"	10-7"	11-1"	11-6"	12-1"	12-6"			
	NI-90x	9-4"	9-9"	10-3"	10-7"	11-1"	11-7"	12-1"	12-7"	13-2"			
16"	NI-60	10-3"	10-8"	11-2"	11-6"	12-1"	12-6"	13-2"	14-1"	14-10"			
	NI-70	10-1"	10-5"	11-0"	11-4"	11-9"	12-3"	12-8"	13-3"	14-0"			
	NI-80	10-4"	10-9"	11-3"	11-9"	12-3"	12-7"	13-1"	13-6"	14-4"			
	NI-90x	11-1"	11-5"	12-0"	12-4"	12-9"	13-3"	13-9"	14-4"	15-2"			

- Above table may be used for I-joist spacing of 24 inches on centre or less.
- Duct chase opening location distance is measured from inside face of supports to centre of opening.
- The above table is based on simple-span joists only. For other applications, contact your local distributor.
- Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/400.
- The above table is based on the I-joist being used at their maximum spans. The minimum distance as given above may be reduced for shorter spans; contact your local distributor.

FIGURE 7  
FIELD-CUT HOLE LOCATOR



Knockouts are predrilled holes provided for the contractor's convenience to install electrical or small plumbing lines. They are 1-1/2 inches in diameter, and are spaced 15 inches on centre along the length of the I-joist. Where possible, it is preferable to use knockouts instead of field-cut holes.

Never drill, cut, or notch the flange, or over-cut the web.

Holes in webs should be cut with a sharp saw.

For rectangular holes, avoid over-cutting the corners, as this can cause unnecessary stress concentrations. Slightly rounding the corners is recommended. Starting the rectangular hole by drilling a 1-inch diameter hole in each of the four corners and then making the cut between the holes is another good method to minimize damage to the I-joist.

## SAFETY AND CONSTRUCTION PRECAUTIONS



Do not walk on I-joists until fully fastened and braced, or serious injuries can result.



Never stick building materials over unbraced I-joists. Once sheathed, do not over-stress I-joists with concentrated loads from building materials.

WARNING: I-joists are not stable until completely installed, and will not carry any load until fully braced and sheathed.

## AVOID ACCIDENTS BY FOLLOWING THESE IMPORTANT GUIDELINES:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bracing at joist ends. When I-joists are applied continuously over interior supports and a load-bearing wall is planned at that location, blocking will be required at the interior support.
- When the building is completely sheathed, floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent I-joist rollover or buckling.
  - Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Top ends of existing bracing over at least two I-joists.
  - Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bracing.
- Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.
- Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.



## PRODUCT WARRANTY

Chambers Chibongwan guarantees that, in accordance with our specifications, Nordic products are free from manufacturing defects in material and workmanship.

Furthermore, Chambers Chibongwan warrants that our products, when utilized in accordance with our handling and installation instructions, will meet or exceed our specifications for the lifetime of the structure.

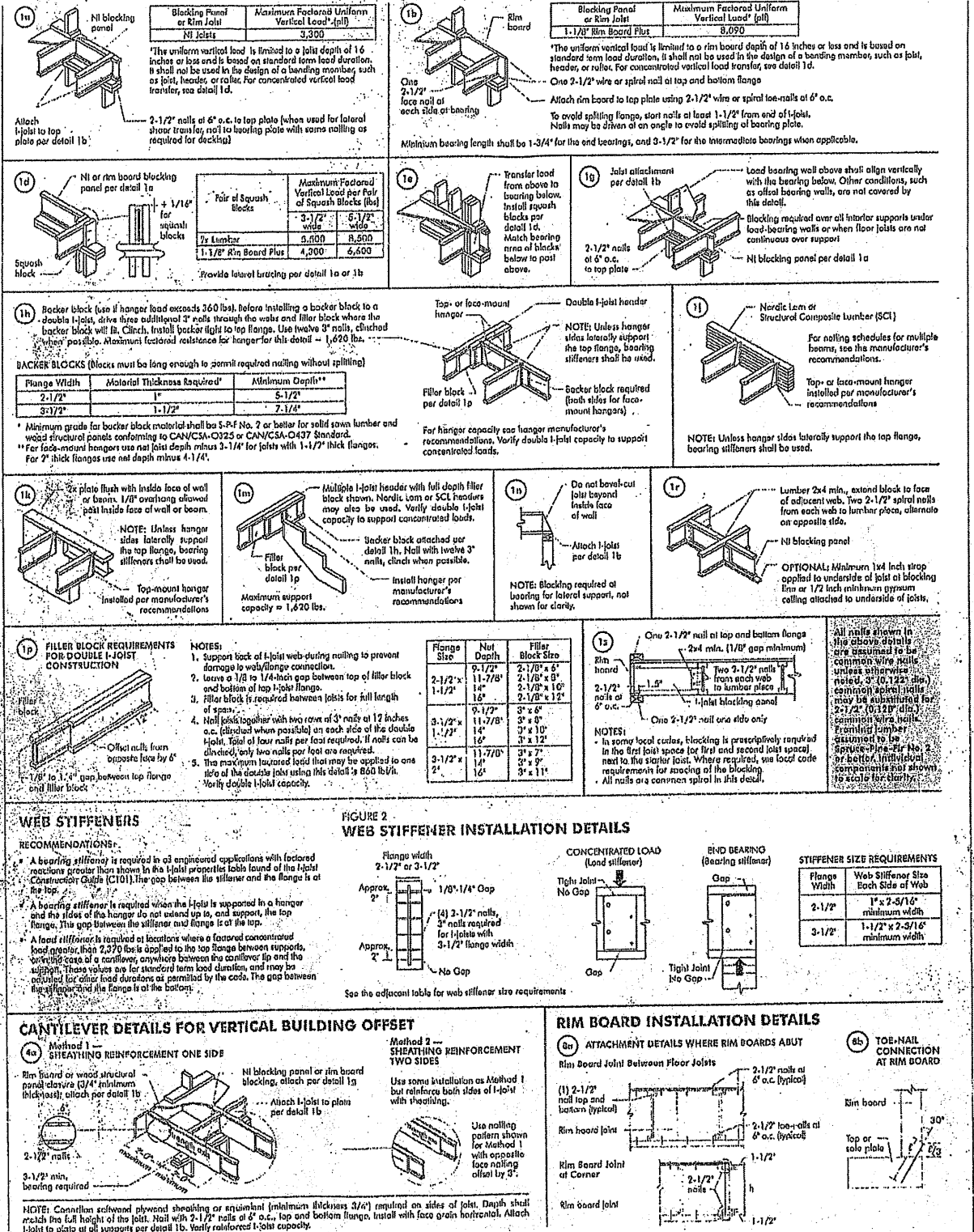
The construction details for residential designs are prone to changes.

Details released after September 2013 supersedes N-303

Installation must comply with latest documentation on I-Joist and other Nordic products from the <http://nordic.ca/>

This document does not constitute a record of the structural integrity of the building nor suitability of the design assumptions made. Nordic Structures is responsible only for the structural adequacy of its component based on the design criteria and loadings shown on the calculation sheets.

Document prepared for the use of Stephanie Gon from Alpa, Ontario. (Nordic Request 1810-095)



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