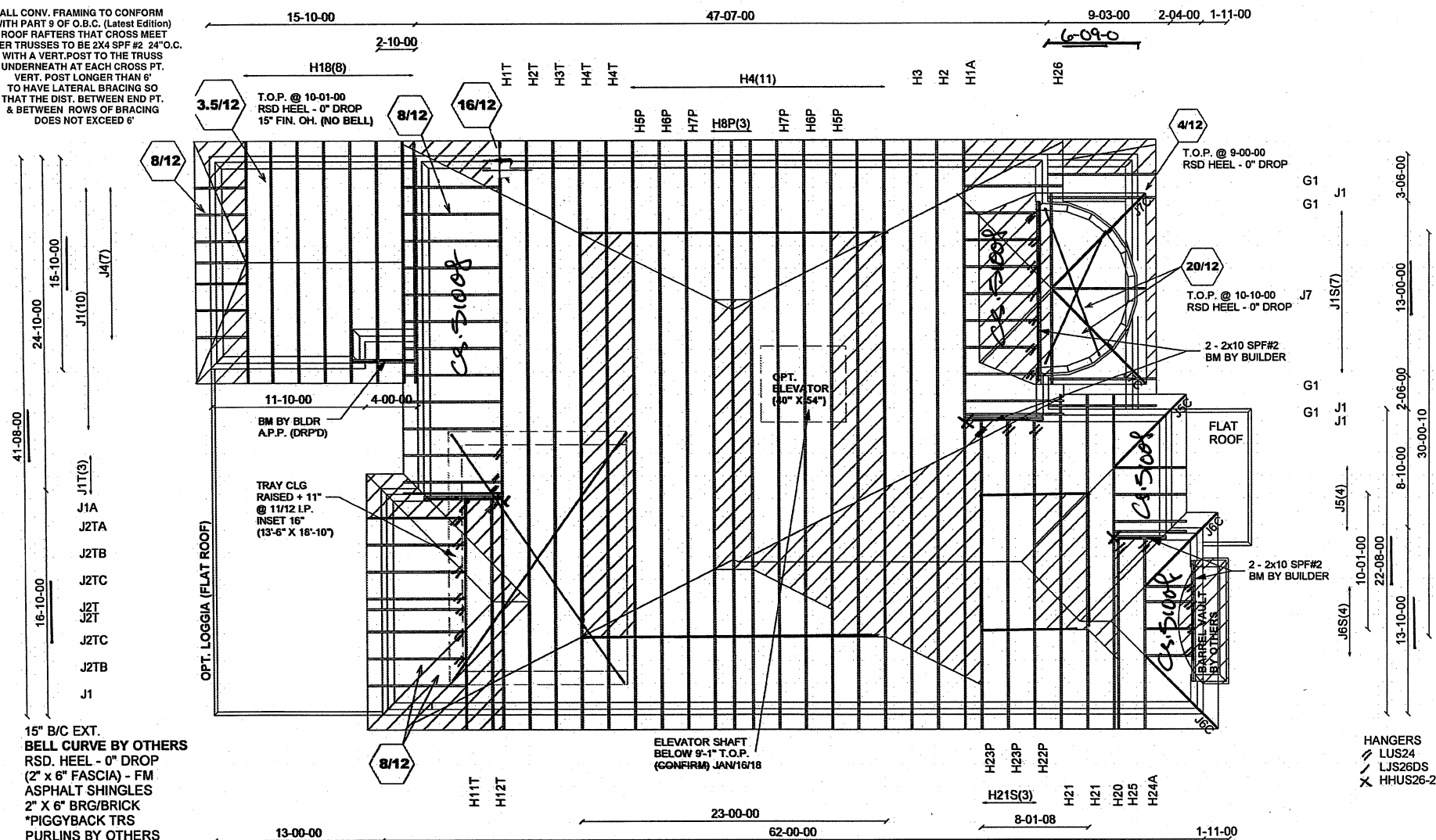


ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. (Latest Edition)
ROOF RAFTERS THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF #2 24"O.C.
WITH A VERT.POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'



CONFIRM TRUSSS DIRECTION/LAYOUT
P.L. 95914

A-18023199-A-18023703
A-18023706-A-18023707
A-18023713-A-18023721
A-18023723-A-18023735

HANGERS
 // LUS24
 / LJS26DS
 X HHUS26-2



CONVENTIONAL
FRAMING BY
OTHERS

P.L. 99352, 101745, 105728

~~HIGHBOURNE~~ KNIGHTSWOOD



Plan Log: 95914

Builder / Location:

GOLD PARK HOMES / VAUGHAN

Project: **PINE VALLEY**

Date: 1/19/2018

Designer: AMANDA

Model / Elevation:

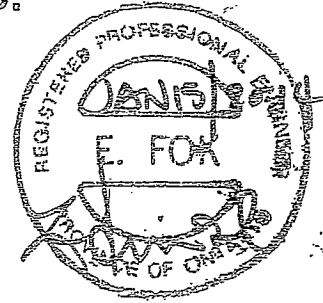
5005 REV1

STD W/ OR OPT 2ND FLR (SEED) AND/OR
/ B ~~OPT~~ TRAY OPT ELEVATOR OR OPT LOGGIA
OR REAR UPGRD

THESE DRAWINGS CONSTITUTE THE PROPERTY OF ALPA ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY ALPA ROOF TRUSSES INC AND WILL BE RETRACTED BY ALPA ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Stracon Engineering Inc.

69 Graydon Crescent
Richmond Hill, Ontario
L4B 3W7
(905) 832-2250 Fax (905) 832-0286



RESPONSIBILITIES

1. Stracon Engineering Inc. is responsible for the design of trusses as individual components.
2. It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities with jurisdictions.
3. All dimensions are to be verified by owner contractor, architect or other authority before manufacture.
4. Stracon Engineering Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Stracon Engineering Inc. drawings is specified for the truss as a component only and forms an integral part of the truss design.
5. It is the truss manufacture's responsibility to ensure that trusses are manufactured in conformance with Stracon Engineering Inc. specifications outlined below.

SPECIFICATIONS

1. Trusses designed by Stracon Engineering Inc. conforms to the relevant section of the Ontario Building Code of Canada (Part 9 or Part 4) or to the Canadian code for farm buildings, whichever applies to the building type indicated on the drawings as well as to the procedures established by the truss plate institute of Canada. Unit stresses used are as per CSA-086-09.
2. Lumber is to be the sizes and grade specified.
3. Moisture content of lumber is not to exceed 19% in service unless otherwise specified.
4. Lumber not to be treated with chemicals unless otherwise specified.
5. Plates shall be applied to both faces of the truss at each joint and shall be positioned as specified.
6. The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals not exceeding 12.5 times in thickness.
7. Where not rigid ceiling is attached directly to the bottom chord, laterally brace the chords at intervals not exceeding 3M (10') o.c.

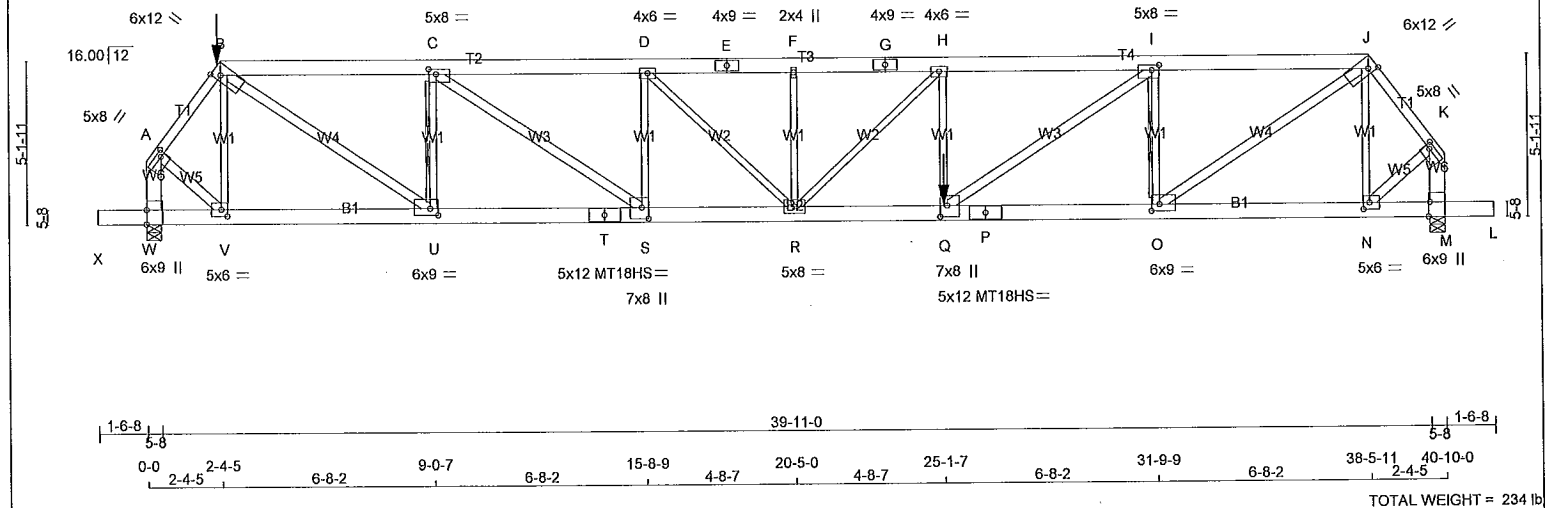
January 15, 2014

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sun Feb 18 14:12:28 2018 Page 1
ID:HSVaWIDnhr?B6oeweH9vbyNdls-nFJaliCHin5qDyZQp8A0Afh_qB76l?y8Rosm5VWzjubX

Scale = 1:72.6



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - B	2x4	DRY	No.2	SPF	
B - E	2x6	DRY	1650F 1.5E	SPF	
E - G	2x6	DRY	1650F 1.5E	SPF	
G - J	2x6	DRY	1650F 1.5E	SPF	
J - K	2x4	DRY	No.2	SPF	
W - A	2x6	DRY	No.2	SPF	
M - K	2x6	DRY	No.2	SPF	
X - T	2x6	DRY	2100F 1.8E	SPF	
T - P	2x6	DRY	2100F 1.8E	SPF	
P - L	2x6	DRY	2100F 1.8E	SPF	
ALL WEBS 2x3 DRY No.2				SPF	
EXCEPT					
B - U	2x4	DRY	No.2	SPF	
C - S	2x4	DRY	No.2	SPF	
O - I	2x4	DRY	No.2	SPF	
O - J	2x4	DRY	No.2	SPF	
A - V	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-I	MT20	5.0	8.0	1.75 2.00
B	TTVW+h	MT20	6.0	12.0	3.25 2.00
C	TMVW-t	MT20	5.0	8.0	2.00 2.75
D	TMVW-t	MT20	4.0	6.0	
E	TS-t	MT20	4.0	9.0	
F	TMW+w	MT20	2.0	4.0	
G	TS-t	MT20	4.0	9.0	
H	TMVW-t	MT20	4.0	6.0	
I	TMVW-t	MT20	5.0	8.0	2.00 2.75
J	TTVW+h	MT20	6.0	12.0	3.25 2.00
K	TMVW-I	MT20	5.0	8.0	1.75 2.00
M	BMV1+i	MT20	6.0	9.0	Edge 0.50
N	BMVW-t	MT20	5.0	6.0	2.50 2.25
O	BMVW-t	MT20	6.0	9.0	2.50 3.00
P	BS-t	MT18HS	5.0	12.0	
Q	BMVW+h	MT20	7.0	8.0	4.25 2.50
R	BMVW+h	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
FACTORED			MAXIMUM FACTORED		INPUT		REQRD		
GROSS REACTION			GROSS REACTION		BRG		BRG		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
W	4381	0	4381	0	0	5-8	5-8		
M	3937	0	3937	0	0	5-8	4-11		
UNFACTORED REACTIONS									
1ST LCASE			MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
W	3112	1964 / 0	0 / 0	0 / 0	0 / 0	1148 / 0	0 / 0		
M	2787	1813 / 0	0 / 0	0 / 0	0 / 0	973 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M									
BEARING SIZE FACTOR = 1.15 AT JNT(S) W, M (BASED ON SUPPORT DEPTH = 1-8)									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.84 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.									

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT C-U, I-O

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	LENGTH	FR-TO				
A - B	-3568 / 0	-78.0	-78.0 0.27 (1)	3.50	V - B	-1050 / 0	0.37 (1)		
B - C	-6769 / 0	-153.5	-153.5 0.54 (1)	3.46	B - U	0 / 5683	1.00 (1)		
C - D	-9437 / 0	-153.5	-153.5 0.71 (1)	2.84	U - C	-3039 / 0	0.64 (1)		
D - E	-10229 / 0	-153.5	-153.5 0.50 (1)	2.92	C - S	0 / 3258	0.58 (1)		
E - F	-10229 / 0	-153.5	-153.5 0.50 (1)	2.92	S - D	-1648 / 0	0.58 (1)		
F - G	-10229 / 0	-153.5	-153.5 0.48 (1)	2.94	D - R	0 / 1119	0.28 (1)		
G - H	-10229 / 0	-153.5	-153.5 0.48 (1)	2.94	R - F	-687 / 0	0.24 (1)		
H - I	-10172 / 0	-78.0	-78.0 0.54 (1)	2.94	R - H	0 / 173	0.07 (4)		
I - J	-6535 / 0	-78.0	-78.0 0.32 (1)	3.76	Q - H	-652 / 0	0.23 (1)		
J - K	-3189 / 0	-78.0	-78.0 0.23 (1)	3.72	Q - I	0 / 4443	0.79 (1)		
W - A	-4270 / 0	0.0	0.0 0.34 (1)	5.16	O - I	-3134 / 0	0.66 (1)		
M - K	-3826 / 0	0.0	0.0 0.31 (1)	5.42	O - J	0 / 5673	1.00 (1)		
					N - J	-1015 / 0	0.36 (1)		
X - W	0 / 0	-96.5	-96.5 0.05 (1)	10.00	A - V	0 / 2513	0.44 (1)		
W - V	0 / 0	-36.4	-36.4 0.09 (4)	10.00	N - K	0 / 2246	0.40 (1)		
V - U	0 / 2114	-36.4	-36.4 0.19 (1)	10.00					
U - T	0 / 6769	-36.4	-36.4 0.43 (1)	10.00					
T - S	0 / 6769	-36.4	-36.4 0.43 (1)	10.00					
R - R	0 / 9437	-36.4	-36.4 0.55 (1)	10.00					
S - Q	0 / 10172	-36.4	-36.4 0.61 (1)	10.00					
Q - P	0 / 6535	-18.5	-18.5 0.41 (1)	10.00					
P - O	0 / 6535	-18.5	-18.5 0.41 (1)	10.00					
O - N	0 / 1888	-18.5	-18.5 0.16 (1)	10.00					
N - M	0 / 0	-18.5	-18.5 0.06 (1)	10.00					
M - L	0 / 0	-96.5	-96.5 0.05 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	FACE DIR. TYPE
B	2-4-5	-134	-134	---	FRONT VERT TOTAL
Q	25-1-8	-1778	-1778	---	FRONT VERT TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:		
TOP CH.	LL =	21.0 PSF
	DL =	6.0 PSF
BOT CH.	LL =	0.0 PSF
	DL =	7.4 PSF
TOTAL LOAD =		34.4 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 2-4-5

END SETBACK = 5-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADD'TL LOADS BASED ON 55 % OF GSL.

LOADS APPLIED TO FIRST 25-1-8 OF SPAN MEASURED FROM THE LEFT.

*** NON STANDARD GIRDER ***

ADD'TL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.36")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.47")

ALLOWABLE DEFL.(TL)= L/360 (1.36")

CALCULATED VERT. DEFL.(TL) = L/ 555 (0.88")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.19")

CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")

ALLOWABLE DEFL.(TL)= L/120 (0.19")

CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.61/1.00 (Q-R:1) , WB=1.00/1.00 (B-U:1) , SSI=0.41/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

A-18023696 CONTINUED ON PAGE 5



A-18023696 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H1	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWlIDnhr?B6oeweH9vbyNdlis-nFJaliCHin5qDyZQp8A0Afh_qB76I?y8Rosm5WzjubX

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
S	BMWW+I	MT20	7.0	8.0	4.25	2.50
T	BS-I	MT18HS	5.0	12.0		
U	BMWW-I	MT20	6.0	9.0	2.50	3.00
V	BMWW-I	MT20	5.0	6.0	2.50	2.25
W	BMV1+I	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 2-4-5 ON TOP CHORD, AND 1777.8 lbs FACTORED DOWN AT 25-1-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

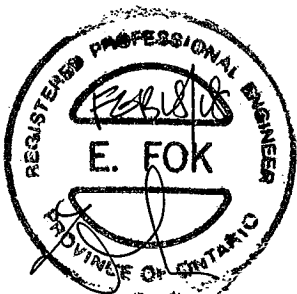
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	618	354	1667
MT18HS	511	354	2455

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (O) (INPUT = 0.90)

JSI METAL= 0.94 (Q) (INPUT = 1.00)

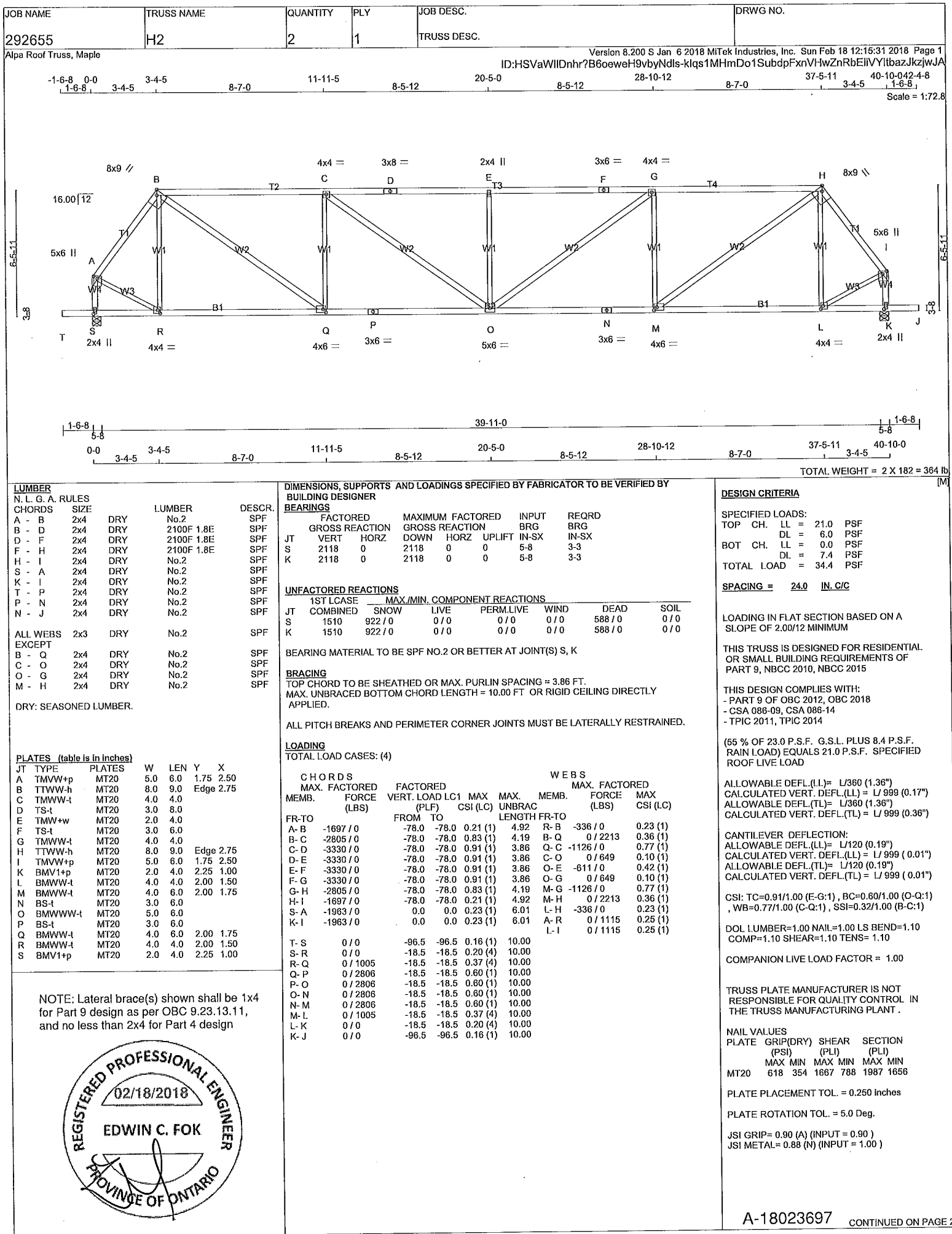


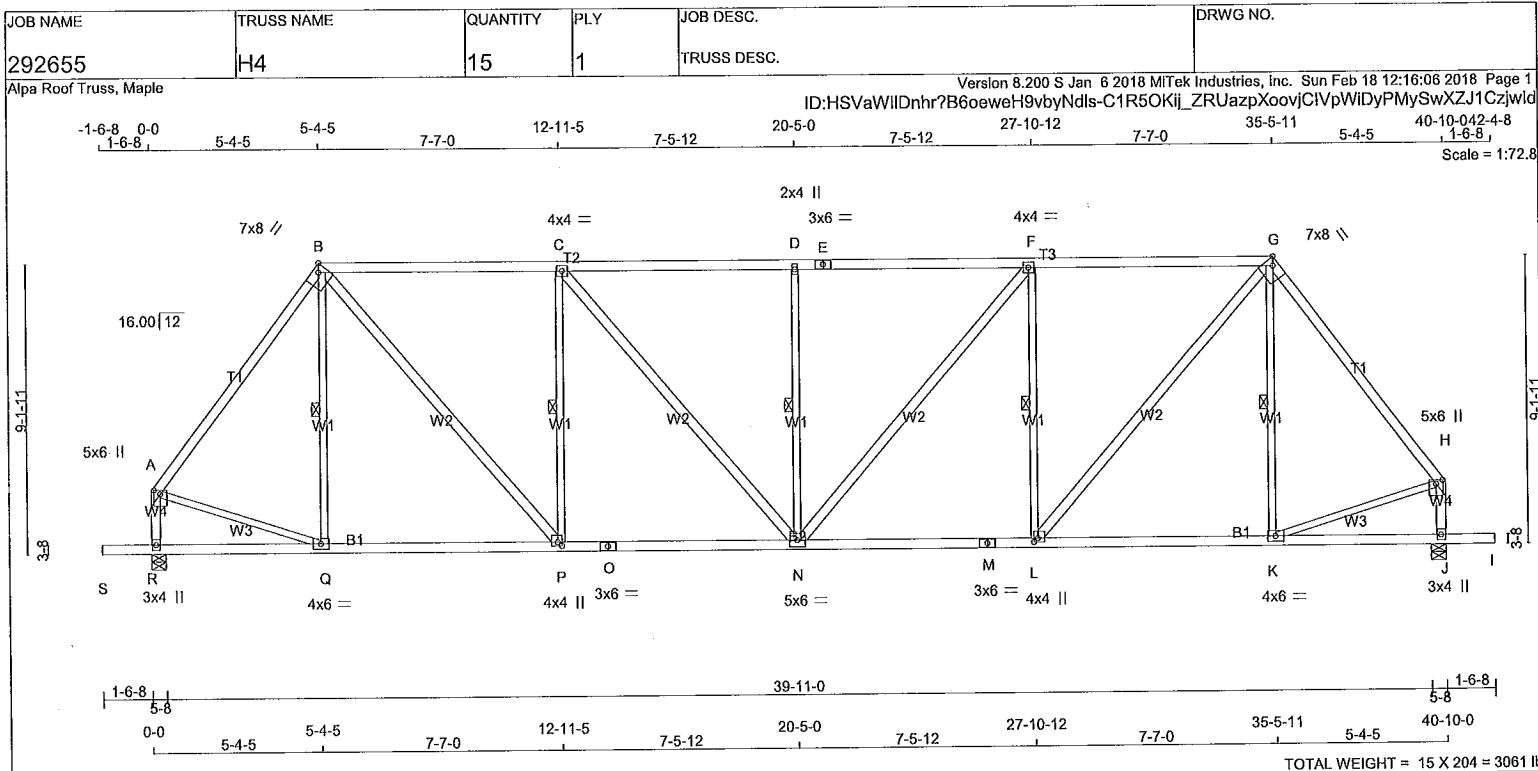
REGISTERED PROFESSIONAL ENGINEER
E. FOK
PROVINCE OF ONTARIO

A-18023696620



A-1802369660





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2
B - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
R - A	2x4	DRY	No.2
J - H	2x4	DRY	No.2
S - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
B - P	2x4	DRY	No.2
C - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2
L - G	2x4	DRY	No.2

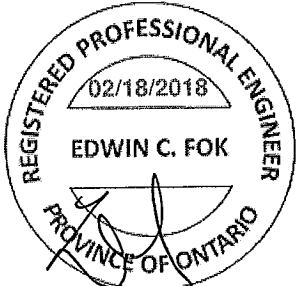
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	5.0	6.0	1.50	2.50
B	TTVW-h	MT20	7.0	8.0	Edge	2.75
C	TMVW-t	MT20	4.0	4.0		
D	TMVW-w	MT20	2.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTVW-h	MT20	7.0	8.0	Edge	2.75
H	TMVW+p	MT20	5.0	6.0	1.50	2.50
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	4.0	6.0		
L	BMVW-t	MT20	4.0	4.0	1.50	1.50
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0	1.50	1.50
Q	BMVW-t	MT20	4.0	6.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
R	2231	0	2231	0	5-8	3-9
J	2231	0	2231	0	5-8	3-9

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
R	1601	922 / 0	0 / 0	0 / 0	0 / 0	679 / 0	0 / 0
J	1601	922 / 0	0 / 0	0 / 0	0 / 0	679 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
FOR SECTION B-G, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-Q, C-P, D-N, F-L, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	-1879 / 0	-78.0	-78.0	0.44 (1)	4.57	Q-B	-198 / 3	0.11 (1)	
B-C	-2176 / 0	-85.5	-85.5	0.73 (1)	2.00	B-P	0 / 1618	0.26 (1)	
C-D	-2482 / 0	-85.5	-85.5	0.72 (1)	2.00	P-C	-1084 / 0	0.59 (1)	
D-E	-2482 / 0	-85.5	-85.5	0.72 (1)	2.00	C-N	0 / 474	0.08 (1)	
E-F	-2482 / 0	-85.5	-85.5	0.72 (1)	2.00	N-D	-589 / 0	0.32 (1)	
F-G	-2176 / 0	-85.5	-85.5	0.73 (1)	2.00	N-F	0 / 474	0.08 (1)	
G-H	-1879 / 0	-78.0	-78.0	0.44 (1)	4.57	L-F	-1084 / 0	0.59 (1)	
R-A	-2046 / 0	0.0	0.0	0.24 (1)	5.91	L-G	0 / 1618	0.26 (1)	
J-H	-2046 / 0	0.0	0.0	0.24 (1)	5.91	K-G	-198 / 3	0.11 (1)	
						A-Q	0 / 1170	0.26 (1)	
						K-H	0 / 1170	0.26 (1)	
S-R	0 / 0	-96.5	-96.5	0.16 (1)	10.00				
R-Q	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
Q-P	0 / 1120	-18.5	-18.5	0.33 (4)	10.00				
P-O	0 / 2176	-18.5	-18.5	0.47 (1)	10.00				
O-N	0 / 2176	-18.5	-18.5	0.47 (1)	10.00				
N-M	0 / 2176	-18.5	-18.5	0.47 (1)	10.00				
M-L	0 / 2176	-18.5	-18.5	0.47 (1)	10.00				
L-K	0 / 1120	-18.5	-18.5	0.33 (4)	10.00				
K-J	0 / 0	-18.5	-18.5	0.18 (4)	10.00				
J-I	0 / 0	-96.5	-96.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.36")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (1.36")
CALCULATED VERT. DEFL.(TL) = L/999 (0.25")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.73/1.00 (B-C:1), BC=0.47/1.00 (N-P:1), WB=0.59/1.00 (C-P:1), SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

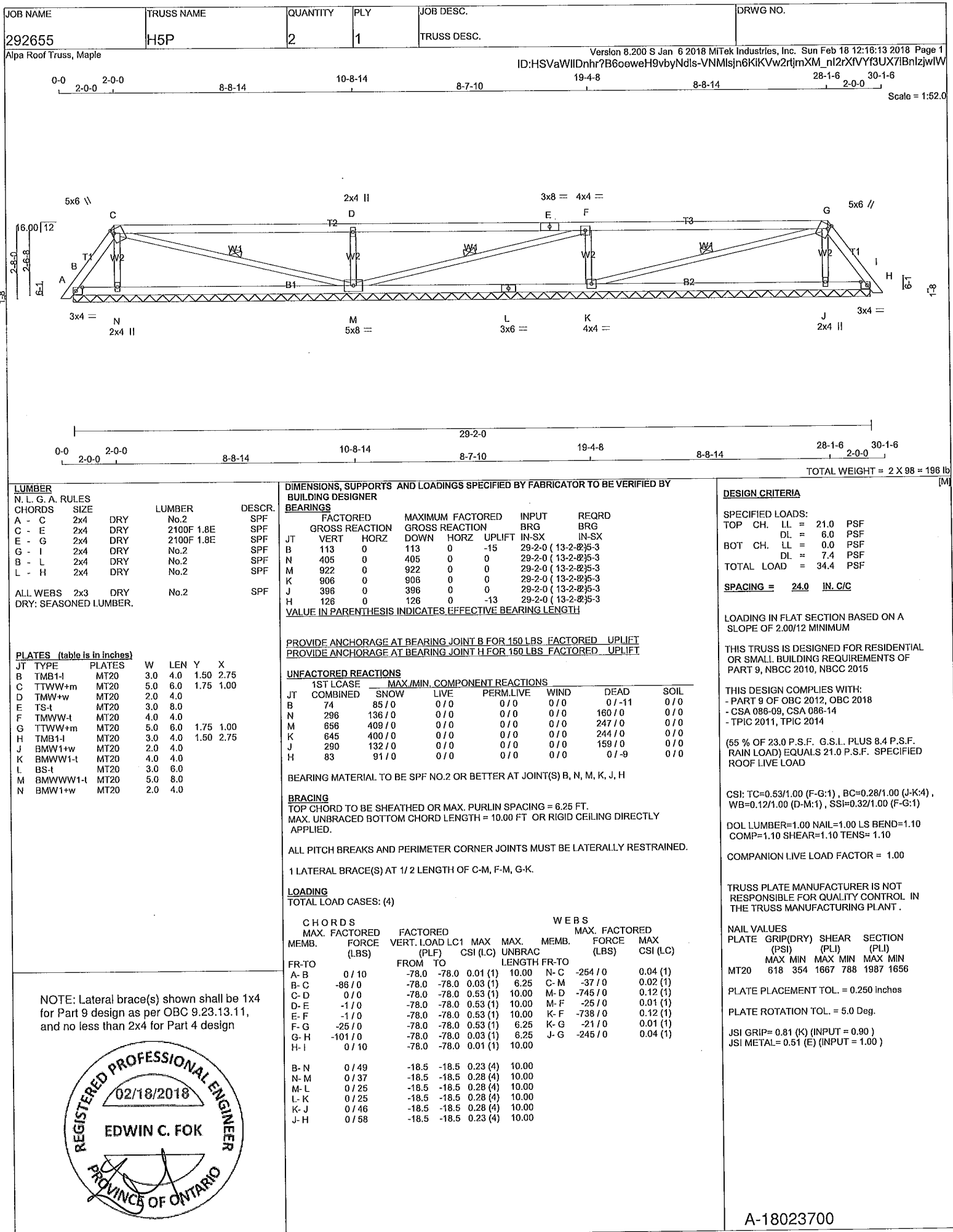
NAIL VALUES

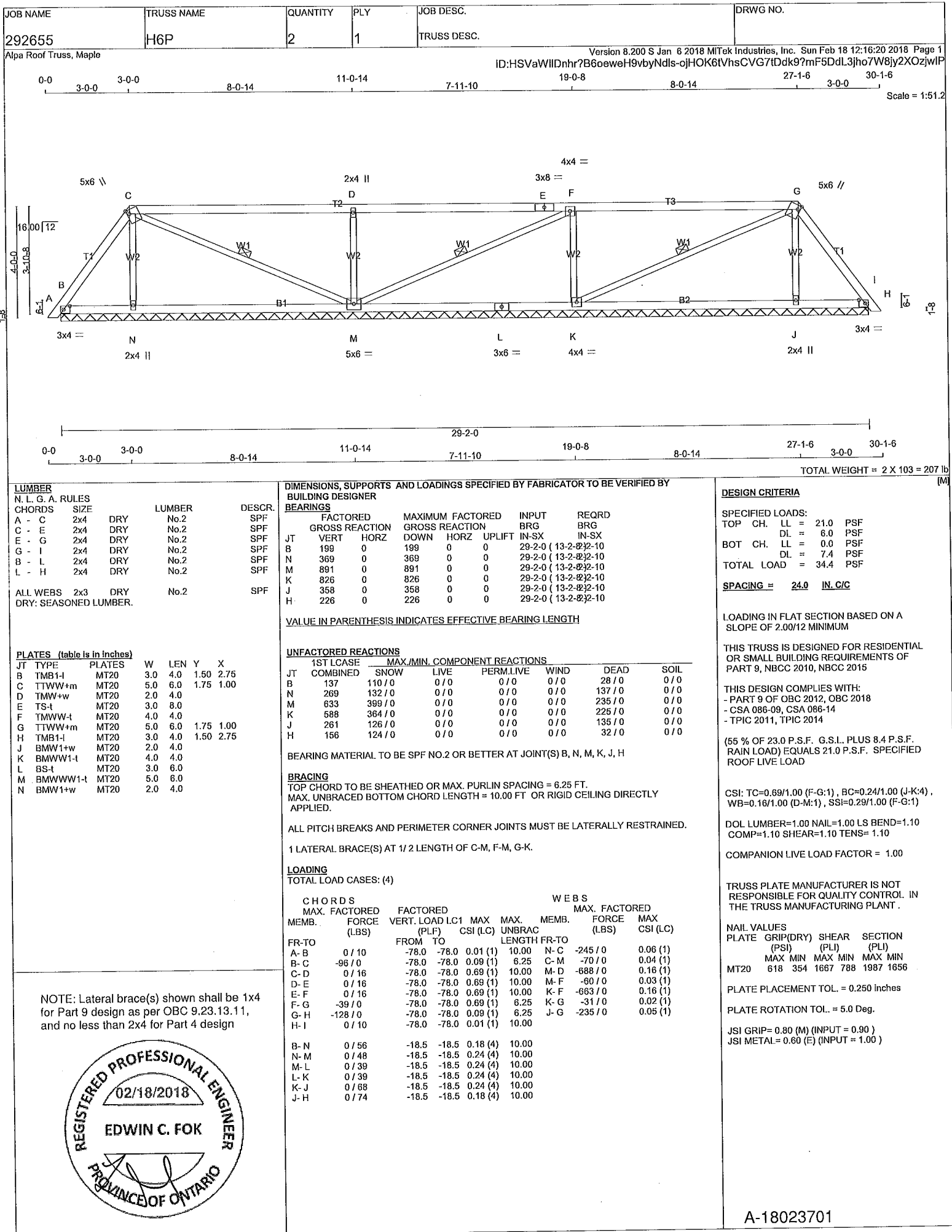
PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)
JSI METAL= 0.62 (H) (INPUT = 1.00)



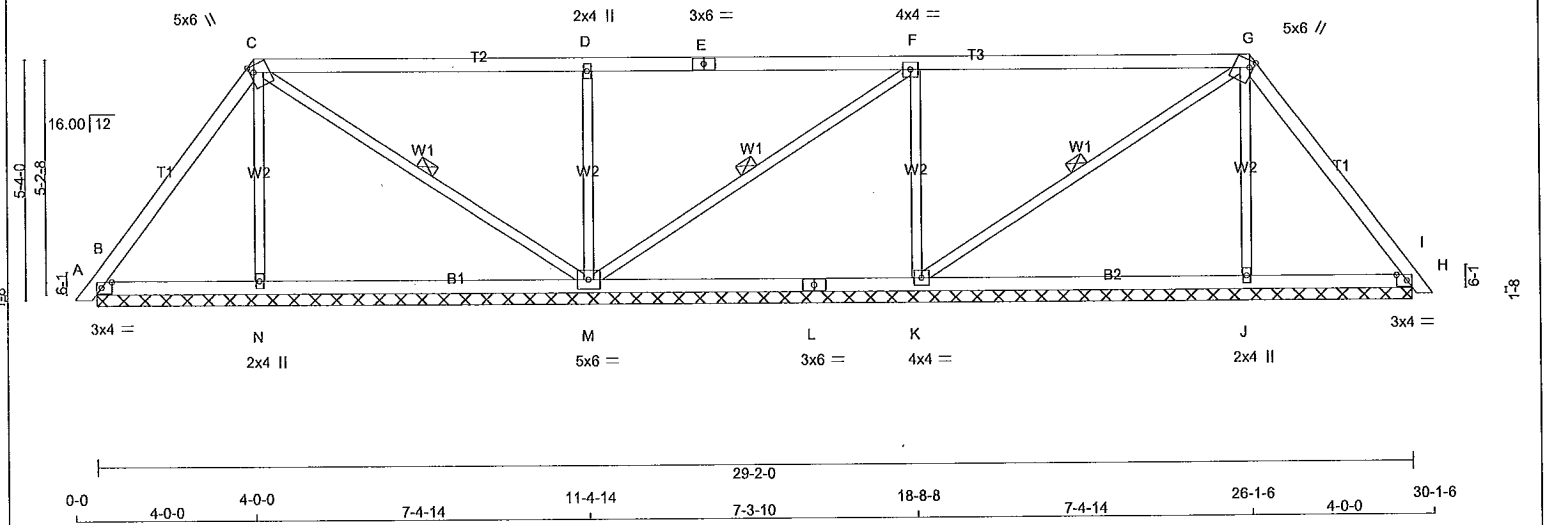


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H7P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIIDnhr?B6oeweH9vbyNdls-S10wrD01sYjoizoXKFNPfNaHFB96VDfHvashy/zjwID

0-0 4-0-0 4-0-0 7-4-14 11-4-14 7-3-10 18-8-8 7-4-14 26-1-6 4-0-0 30-1-6
Scale = 1:51.2



LUMBER					DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER					DESIGN CRITERIA				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	BEARINGS					SPECIFIED LOADS:				
A - C	2x4	DRY	No.2	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG		TOP CH. LL = 21.0 PSF				
E - G	2x4	DRY	No.2	SPF	DOWN	DOWN	UP	IN-SX		DL = 6.0 PSF				
E - G	2x4	DRY	No.2	SPF	UP	UP	DOWN	IN-SX		BOT CH. LL = 0.0 PSF				
G - I	2x4	DRY	No.2	SPF						DL = 7.4 PSF				
B - L	2x4	DRY	No.2	SPF						TOTAL LOAD = 34.4 PSF				
L - H	2x4	DRY	No.2	SPF										
ALL WEBS	2x3	DRY	No.2	SPF										
DRY: SEASONED LUMBER.														

PLATES (table is in inches)					UNFACTORED REACTIONS					LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM				
JT	TYPE	PLATES	W	LEN	Y	X	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
B	TMB1-I	MT20	3.0	4.0	1.50	2.75	JT COMBINED							
C	TTWW+m	MT20	5.0	6.0	1.75	1.00	B	182	132 / 0	0 / 0	0 / 0	50 / 0	0 / 0	
D	TMW+w	MT20	2.0	4.0			N	253	125 / 0	0 / 0	0 / 0	128 / 0	0 / 0	
E	TS-t	MT20	3.0	6.0			M	628	402 / 0	0 / 0	0 / 0	225 / 0	0 / 0	
F	TMWW-t	MT20	4.0	4.0			K	522	320 / 0	0 / 0	0 / 0	202 / 0	0 / 0	
G	TTWW+m	MT20	5.0	6.0	1.75	1.00	J	248	121 / 0	0 / 0	0 / 0	127 / 0	0 / 0	
H	TMB1-I	MT20	3.0	4.0	1.50	2.75	H	212	155 / 0	0 / 0	0 / 0	57 / 0	0 / 0	
J	BMW1+w	MT20	2.0	4.0										
K	BMWW1-t	MT20	4.0	4.0										
L	BS-t	MT20	3.0	6.0										
M	BMWW1-t	MT20	5.0	6.0										
N	BMW1+w	MT20	2.0	4.0										

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 10	-78.0	-78.0 0.01 (1)	N-C	-231 / 0	0.09 (1)	
B-C	-98 / 0	-78.0	-78.0 0.17 (1)	C-M	-108 / 0	0.06 (1)	
C-D	0 / 37	-78.0	-78.0 0.58 (1)	M-D	-631 / 0	0.25 (1)	
D-E	0 / 36	-78.0	-78.0 0.58 (1)	M-F	-104 / 0	0.06 (1)	
E-F	0 / 36	-78.0	-78.0 0.58 (1)	K-F	-572 / 0	0.23 (1)	
F-G	-50 / 0	-78.0	-78.0 0.58 (1)	K-G	-41 / 0	0.02 (1)	
G-H	-152 / 0	-78.0	-78.0 0.17 (1)	J-G	-224 / 0	0.09 (1)	
H-I	0 / 10	-78.0	-78.0 0.01 (1)				
B-N	0 / 57	-18.5	-18.5 0.15 (4)				
N-M	0 / 53	-18.5	-18.5 0.20 (4)				
M-L	0 / 50	-18.5	-18.5 0.20 (4)				
L-K	0 / 50	-18.5	-18.5 0.20 (4)				
K-J	0 / 84	-18.5	-18.5 0.21 (4)				
J-H	0 / 89	-18.5	-18.5 0.15 (4)				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.16 (E) (INPUT = 1.00)

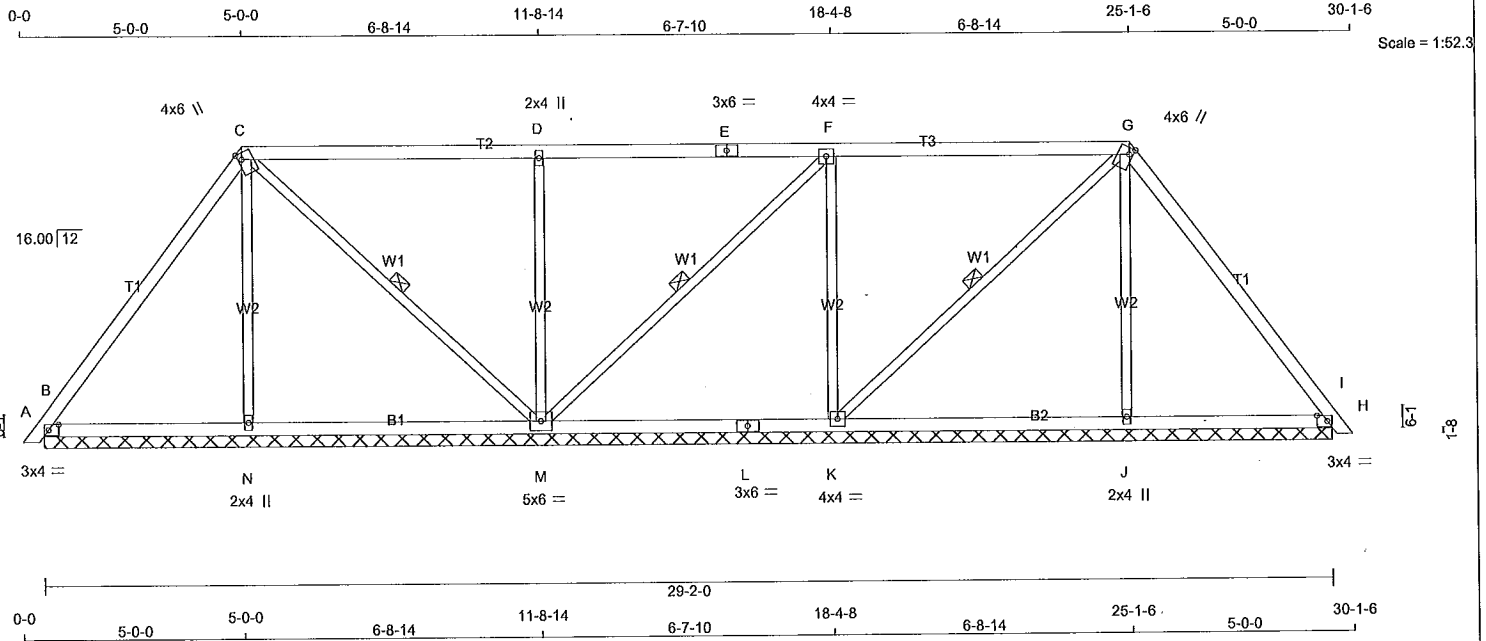
A-18023702

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292655	H8P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIIDnhr?B6oeweh9vbyNdls-d8A49z9wGw5EWf8eT33OC5YB_dwHa84vRo0mrZzjwI2



LUMBER	SIZE	DRY	NO.2	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	310	0	310	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0
N	332	0	332	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0
M	890	0	890	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0
K	638	0	638	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0
J	330	0	330	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0
H	369	0	369	0	29-2-0 (13-2-8)2-0	29-2-0 (13-2-8)2-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	
B	218	151 / 0	0 / 0	0 / 0	0 / 0	67 / 0	0 / 0
N	242	117 / 0	0 / 0	0 / 0	0 / 0	126 / 0	0 / 0
M	629	412 / 0	0 / 0	0 / 0	0 / 0	217 / 0	0 / 0
K	455	277 / 0	0 / 0	0 / 0	0 / 0	178 / 0	0 / 0
J	241	115 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
H	259	183 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, N, M, K, J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-M, F-M, G-K.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)		MAX. CS1 (LC)
		FROM	TO				FROM	TO	
FR-TO									
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	N-C	-216 / 0	0.15 (1)	
B-C	-95 / 0	-78.0	-78.0	0.27 (1)	6.25	C-M	-139 / 0	0.08 (1)	
C-D	0 / 51	-78.0	-78.0	0.48 (1)	10.00	M-D	-573 / 0	0.40 (1)	
D-E	0 / 50	-78.0	-78.0	0.48 (1)	10.00	M-F	-143 / 0	0.08 (1)	
E-F	0 / 50	-78.0	-78.0	0.48 (1)	10.00	K-F	-475 / 0	0.33 (1)	
F-G	-53 / 0	-78.0	-78.0	0.48 (1)	6.25	K-G	-57 / 0	0.03 (1)	
G-H	-168 / 0	-78.0	-78.0	0.28 (1)	6.25	J-G	-214 / 0	0.15 (1)	
H-I	0 / 10	-78.0	-78.0	0.01 (1)	10.00				
B-N	0 / 56	-18.5	-18.5	0.14 (4)	10.00				
N-M	0 / 52	-18.5	-18.5	0.17 (4)	10.00				
M-L	0 / 53	-18.5	-18.5	0.17 (4)	10.00				
L-K	0 / 53	-18.5	-18.5	0.17 (4)	10.00				
K-J	0 / 95	-18.5	-18.5	0.17 (4)	10.00				
J-H	0 / 99	-18.5	-18.5	0.15 (4)	10.00				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 0.0 PSF
	DL = 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (D-F:1), BC=0.17/1.00 (J-K:4), WB=0.40/1.00 (D-M:1), SSI=0.25/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

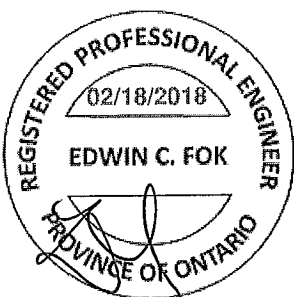
NAIL VALUES	GRIP(DRY)	SHEAR	SECTION
PLATE	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

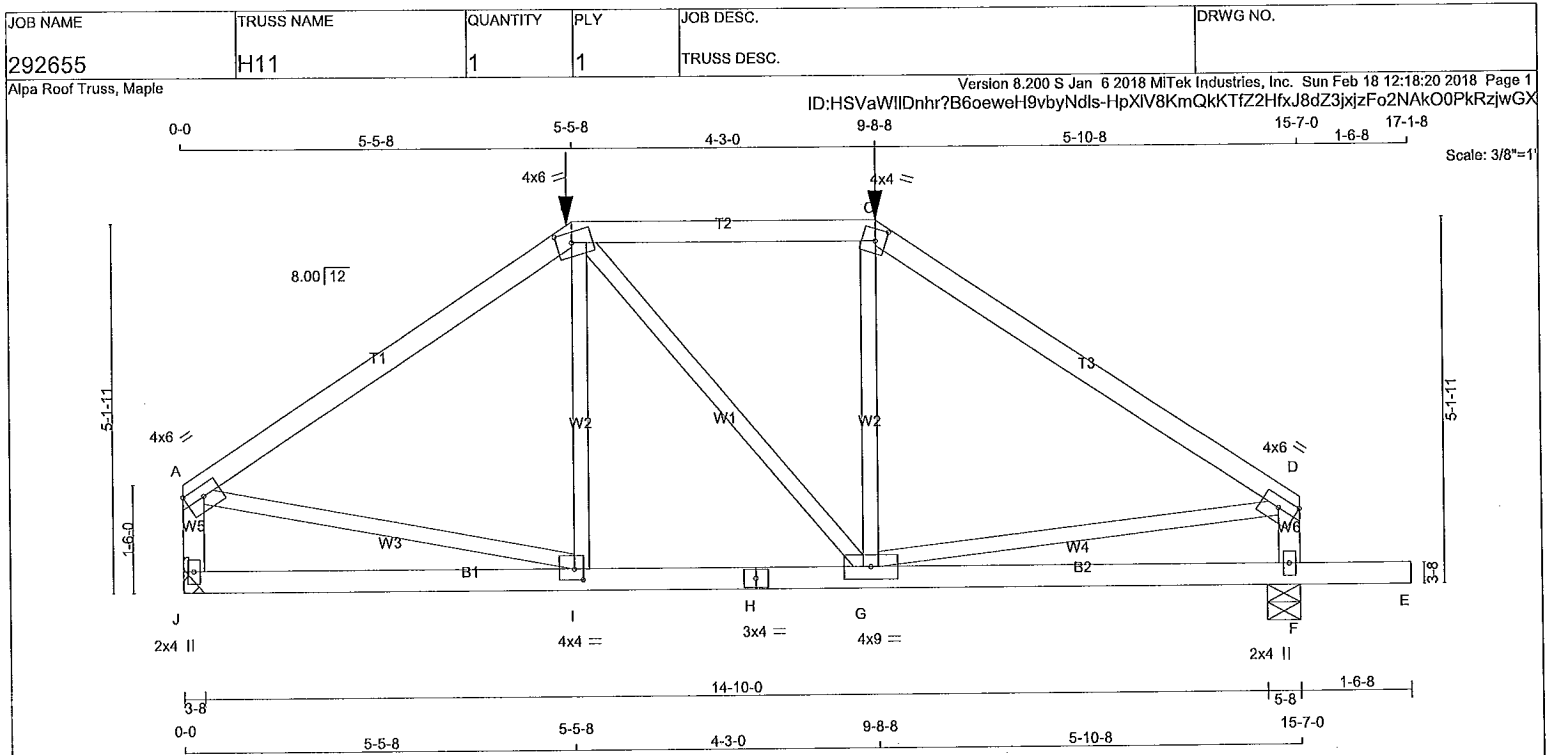
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.64 (G) (INPUT = 0.90)
JSI METAL= 0.17 (H) (INPUT = 1.00)

A-18023703





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
J - A	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
H - E	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	Edge
B	TTWW-m	MT20	4.0	6.0	1.75	2.50
C	TTW-m	MT20	4.0	4.0	2.00	1.75
D	TMVW-t	MT20	4.0	6.0	1.75	Edge
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BS-t	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0	1.75	1.50
J	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 334.0 lbs FACTORED DOWN AT 9-8-8, AND 310.3 lbs FACTORED DOWN AT 5-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
J	1384	0	1384	0
F	1513	0	1513	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	WIND	DEAD
J	988	592 / 0	0 / 0	0 / 0	386 / 0
F	1081	646 / 0	0 / 0	0 / 0	435 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)	MAX
FR-TO		FROM	TO					
A-B	-1479 / 0	-78.0	-78.0	0.54 (1)	4.69	I-B	-78 / 106	0.04 (4)
B-C	-1271 / 0	-153.5	-153.5	0.57 (1)	4.81	B-G	0 / 68	0.02 (1)
C-D	-1531 / 0	-78.0	-78.0	0.65 (1)	4.47	G-C	-38 / 134	0.05 (4)
J-A	-1297 / 0	0.0	0.0	0.15 (1)	7.05	A-I	0 / 1257	0.31 (1)
F-D	-1271 / 0	0.0	0.0	0.14 (1)	7.11	G-D	0 / 1287	0.32 (1)
J-I	0 / 0	-36.4	-36.4	0.26 (4)	10.00			
I-H	0 / 1228	-36.4	-36.4	0.41 (4)	10.00			
H-G	0 / 1228	-36.4	-36.4	0.41 (4)	10.00			
G-F	0 / 0	-36.4	-36.4	0.30 (4)	10.00			
F-E	0 / 0	-96.5	-96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	5-5-8	-310	-310	---	FRONT	VERT	TOTAL
C	9-8-8	-334	-334	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	0.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	34.4	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHlp
LEFT SETBACK = 5-5-8
RIGHT SETBACK = 5-10-8
END SETBACK = 5-10-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL)= L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL)= L/999 (0.10")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

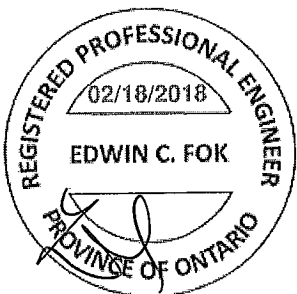
CSI: TC=0.65/1.00 (C-D:1), BC=0.41/1.00 (G-I:4), WB=0.32/1.00 (D-G:1), SSI=0.28/1.00 (B-C:1)

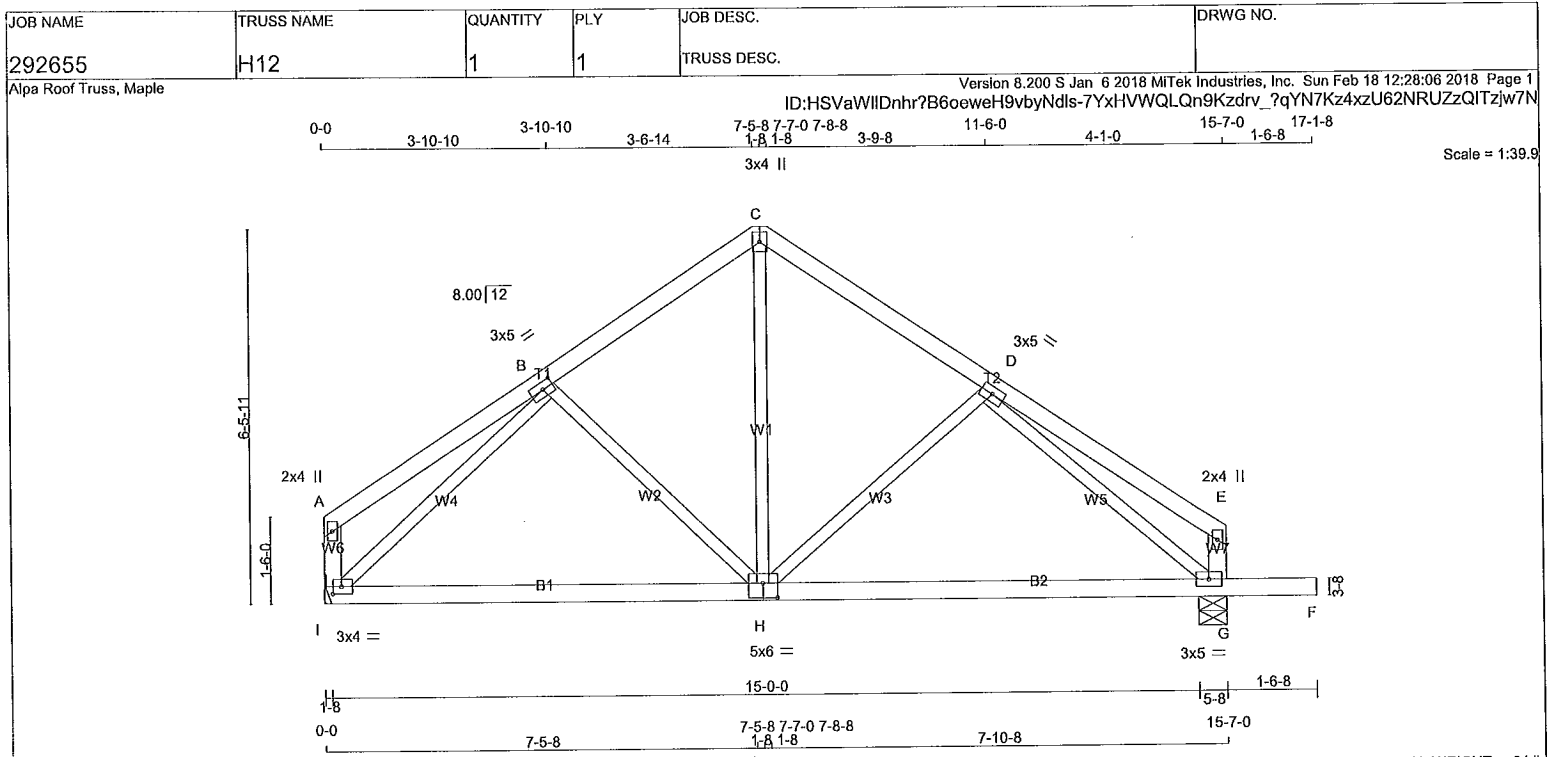
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656





LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER		
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
I - A	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY, SEASONED LUMBER. No.2 SPF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	2.0	4.0	
B	TMVW-t	MT20	3.0	5.0	1.50 2.25
C	TTW+p	MT20	3.0	4.0	
D	TMVW-t	MT20	3.0	5.0	
E	TMV+p	MT20	2.0	4.0	
G	BMVW1-t	MT20	3.0	5.0	
H	BSVW1-t	MT20	5.0	6.0	3.00 3.00
I	BMVW1-t	MT20	3.0	4.0	1.50 1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQRD	
JT		VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
I		752	0	752	0	0	HANGER BY OTHERS		
G		900	0	900	0	0	MIN. SEAT SIZE: 1-8	1-8	

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
I	536	327 / 0	0 / 0	0 / 0	0 / 0	209 / 0	0 / 0		
G	642	392 / 0	0 / 0	0 / 0	0 / 0	250 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LBS)	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 19	-78.0	-78.0 0.18 (1)	H-C	0 / 389	0.09 (1)	
B-C	-583 / 0	-78.0	-78.0 0.14 (1)	I-B	-812 / 0	0.36 (1)	
C-D	-584 / 0	-78.0	-78.0 0.15 (1)	B-H	-155 / 0	0.07 (1)	
D-E	0 / 20	-78.0	-78.0 0.20 (1)	H-D	-208 / 0	0.10 (1)	
I-A	-113 / 0	0.0	0.0 0.01 (1)	D-G	-839 / 0	0.39 (1)	
G-E	-119 / 0	0.0	0.0 0.01 (1)				
I-H	0 / 580	-18.5	-18.5 0.36 (4)				
H-G	0 / 625	-18.5	-18.5 0.40 (4)				
G-F	0 / 0	-96.5	-96.5 0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL) = L/937 (0.20")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (D-E:1), BC=0.40/1.00 (G-H:4), WB=0.39/1.00 (D-G:1), CSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

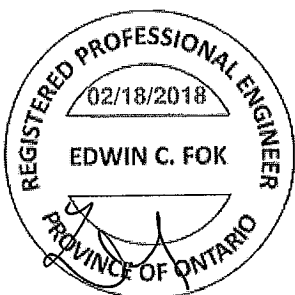
NAIL VALUES
PLATE GRIP(DRY) 'SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

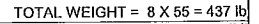
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.23 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



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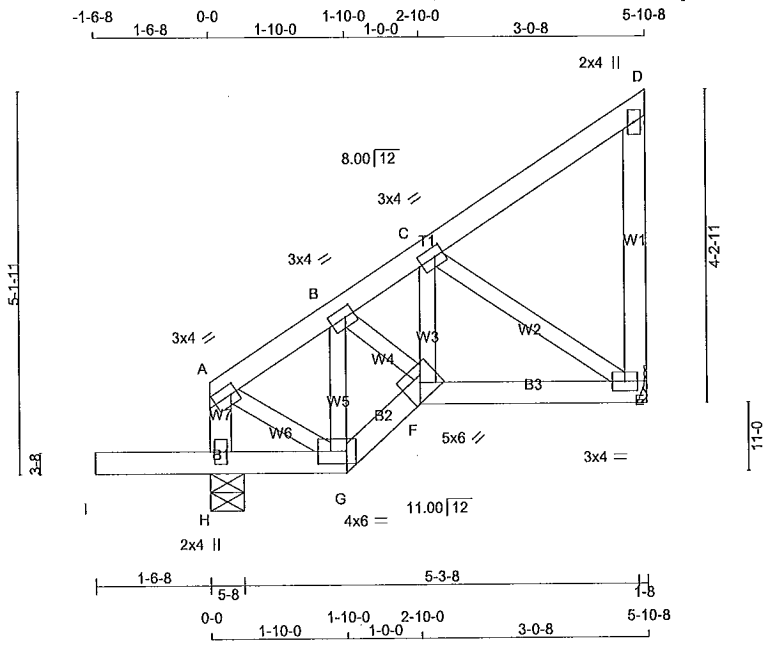


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J1T	3	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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LUMBER

N. L. G. A. RULES

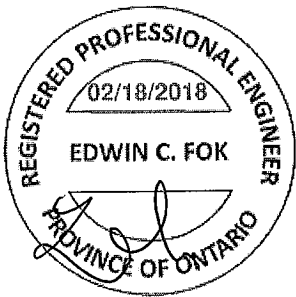
CHORDS	SIZE	LUMBER	DESCR.
H - A	2x4	DRY	No.2 SPF
A - D	2x4	DRY	No.2 SPF
E - D	2x4	DRY	No.2 SPF
I - G	2x4	DRY	No.2 SPF
G - F	2x4	DRY	No.2 SPF
F - E	2x4	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	4.0	1.50	1.00
B	TMVW-t	MT20	3.0	4.0	1.50	1.50
C	TMVW-t	MT20	3.0	4.0	1.50	1.50
D	TMVW-t	MT20	2.0	4.0		
E	BMVW-t	MT20	3.0	4.0		
F	BMVW-t	MT20	5.0	6.0		
G	BMVW-t	MT20	4.0	6.0	2.00	4.50
H	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	452	0	5-8	1-8
E	264	0		HANGER BY OTHERS

MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	322	197 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	188	115 / 0	0 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX	MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX
FR-TO						FR-TO		
H-A	-223 / 0	0.0	0.0	0.02 (1)	7.81	A-G	0 / 154	0.03 (1)
A-B	-165 / 0	-78.0	-78.0	0.04 (1)	6.25	G-B	-220 / 0	0.03 (1)
B-C	-239 / 0	-78.0	-78.0	0.08 (1)	6.25	B-F	0 / 130	0.03 (1)
C-D	-11 / 0	-78.0	-78.0	0.08 (1)	6.25	F-C	0 / 63	0.02 (4)
E-D	-95 / 0	0.0	0.0	0.03 (1)	7.81	C-E	-259 / 0	0.06 (1)
I-H	0 / 0	-96.5	-96.5	0.16 (1)	10.00			
H-G	0 / 0	-18.5	-18.5	0.16 (1)	10.00			
G-F	0 / 173	-18.5	-18.5	0.03 (1)	10.00			
F-E	0 / 216	-18.5	-18.5	0.07 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 21.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 0.0 PSF
 DL = 7.4 PSF
 TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, OBC 2018
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
 ALLOWABLE DEFL.(TL)= L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CANTILEVER DEFLECTION:
 ALLOWABLE DEFL.(LL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL)= L/120 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/588 (0.03")

CSI: TC=0.08/1.00 (B-C:1) , BC=0.16/1.00 (G-H:1) , WB=0.06/1.00 (C-E:1) , SSI=0.12/1.00 (H-I:1)

DOI. LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667
			788
			1987
			1656

PLATE PLACEMENT TOL. = 0.250 inches

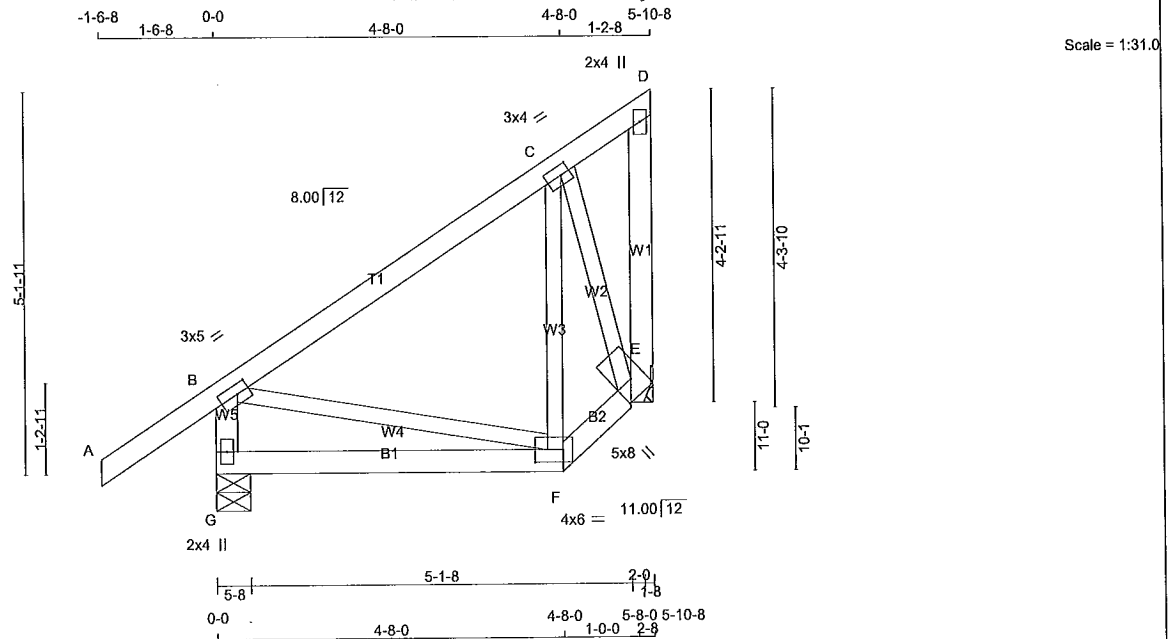
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.50 (A) (INPUT = 0.90)
 JSI METAL= 0.08 (E) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J2T	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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ID:HSVaWIIIDnhr?B6oeWeH9vbyNdlS-GnUY8GCLF?ozMSdLnE5XR_62uN7VEsJ2MNOSZGzjv5p



TOTAL WEIGHT = 2 X 32 = 63 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
G - B	2x4	DRY	No.2	SPF	
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	5.0	1.50	2.00
C	TMVW-I	MT20	3.0	4.0	1.50	1.50
D	TMV+p	MT20	2.0	4.0		
E	BMVW-I	MT20	5.0	8.0	2.75	2.75
F	BBWV-I	MT20	4.0	6.0	2.00	4.50
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
G	410	0	410	0	0	5-8	1-8	
E	283	0	283	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	290	191 / 0	0 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
E	202	123 / 0	0 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

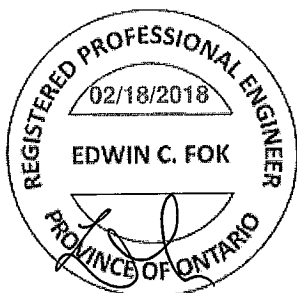
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1	MAX LC2	MAX UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1
FR-TO							FR-TO			
G-B	-367 / 0	0.0	0.0	0.04 (1)	7.81		B-F	0 / 115	0.03 (1)	
A-B	0 / 35	-78.0	-78.0	0.15 (1)	10.00		F-C	-71 / 20	0.02 (1)	
B-C	-117 / 0	-78.0	-78.0	0.22 (1)	6.25		C-E	-261 / 0	0.06 (1)	
C-D	-66 / 0	-78.0	-78.0	0.21 (1)	6.25					
E-D	0 / 68	0.0	0.0	0.01 (1)	10.00					
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00					
F-E	0 / 153	-18.5	-18.5	0.03 (1)	10.00					

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1) , BC=0.12/1.00 (F-G:4) , WB=0.06/1.00 (C-E:1) , SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

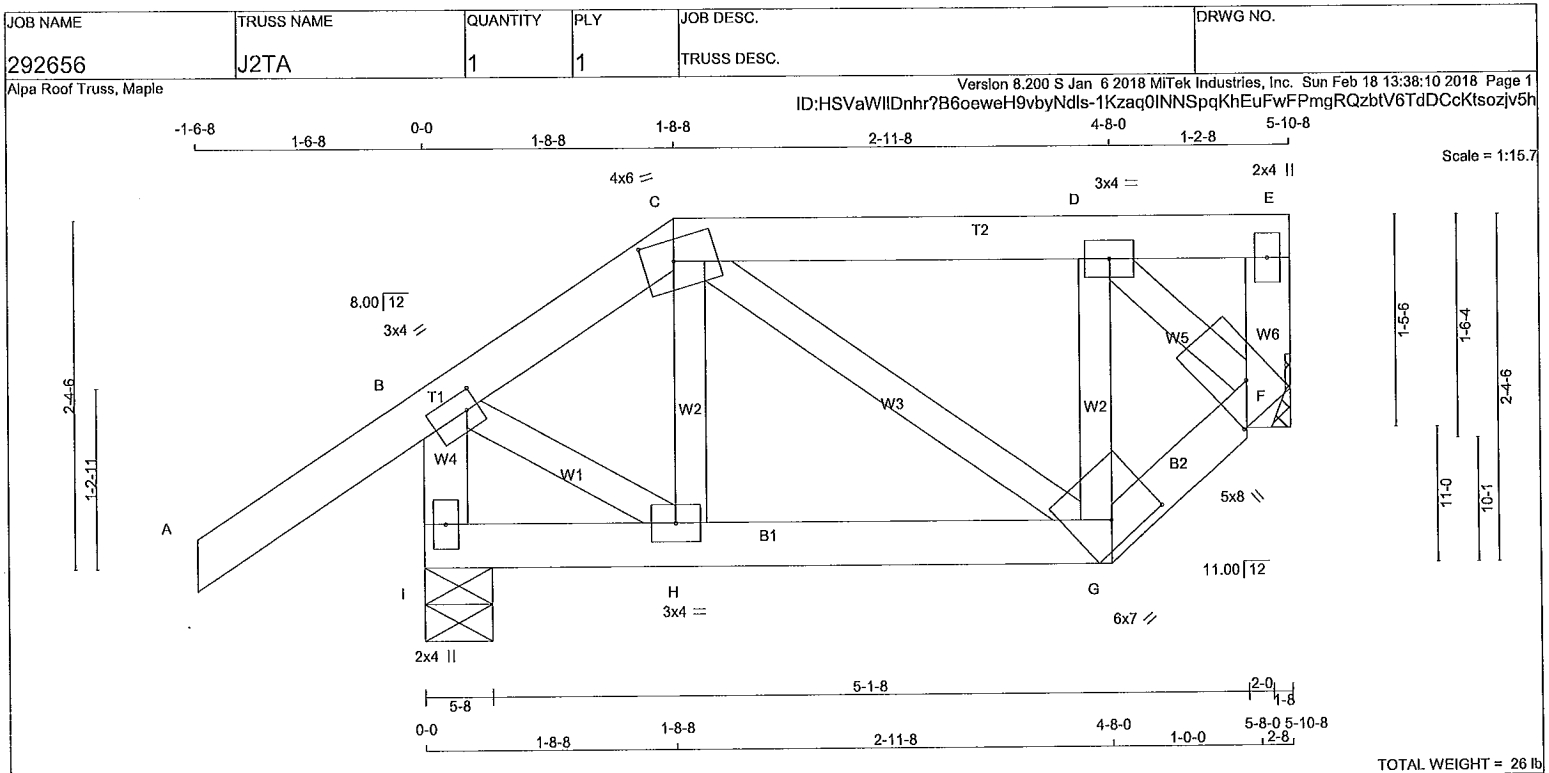
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

A-18023722



LUMBER				
N. L. G. A. RULES				
CHORDS		SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-I	MT20	3.0	4.0	1.50 1.00
C	TTWW-m	MT20	4.0	6.0	1.75 2.50
D	TMVW-I	MT20	3.0	4.0	
E	TMV+p	MT20	2.0	4.0	
F	BMVW1-H	MT20	5.0	8.0	2.75 2.75
G	BBVW-h	MT20	6.0	7.0	2.00 4.00
H	BMVW-I	MT20	3.0	4.0	
I	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
I	427	0	427	0	0	5-8	1-8		
F	266	0	266	0	0	HANGER BY OTHERS			
						MIN. SEAT SIZE: 1-8			

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	302	201 / 0	0 / 0	0 / 0	0 / 0	0 / 0	101 / 0	0 / 0
F	190	114 / 0	0 / 0	0 / 0	0 / 0	0 / 0	76 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (CSI (L))	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH (L)	MAX. FACTORED (CSI (L))
FR-TO		FROM	TO	FR-TO			
A-B	0 / 35	-78.0	-78.0 0.15 (1)	10.00	H-C	-18 / 28	0.01 (4)
B-C	-202 / 0	-78.0	-78.0 0.14 (1)	6.25	C-G	-11 / 22	0.00 (1)
C-D	-156 / 0	-78.0	-78.0 0.07 (1)	6.25	G-D	-118 / 0	0.02 (1)
D-E	0 / 0	-78.0	-78.0 0.07 (1)	10.00	B-H	0 / 156	0.04 (1)
F-E	-10 / 0	0.0	0.0 0.00 (1)	7.81	D-F	-183 / 0	0.03 (1)
I-B	-417 / 0	0.0	0.0 0.04 (1)	7.81			
I-H	0 / 0	-18.5	-18.5 0.03 (4)	10.00			
H-G	0 / 138	-18.5	-18.5 0.05 (4)	10.00			
G-F	0 / 206	-18.5	-18.5 0.04 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.05/1.00 (G-H:4), WB=0.04/1.00 (B-H:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

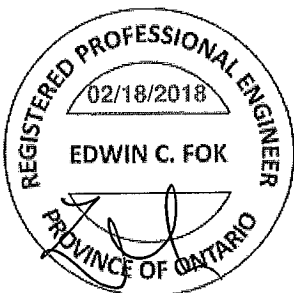
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.51 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

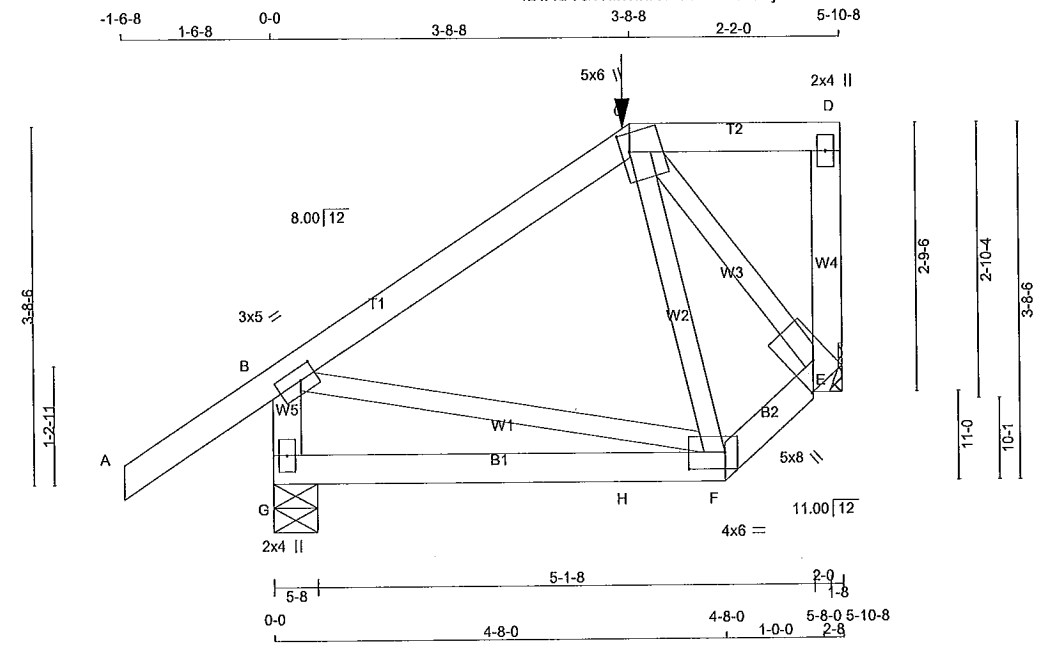


A-18023723

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J2TB	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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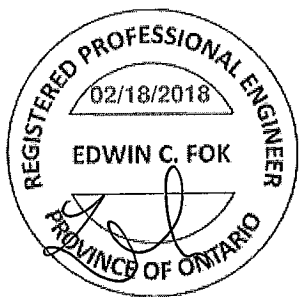
TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

<u>PLATES (table is in inches)</u>							
JT	TYPE	PLATES	W	LEN	Y	X	
B	TMVW-t	MT20	3.0	5.0	1.50	2.00	
C	TTWW+m	MT20	5.0	6.0	2.25	1.00	
D	TMV+p	MT20	2.0	4.0			
E	BVMW+1	MT20	5.0	8.0	2.75	2.75	
F	BBWW-1	MT20	4.0	6.0	2.00	4.50	
G	BMV1+p	MT20	2.0	4.0			

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 106.4 lbs FACTORED DOWN AT 3-8-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQRD				
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	
G	486	0	486	0	0	5-8	1-8		
E	433	0	433	0	0	HANGER BY OTHERS			
						MIN. SEAT SIZE: 1-8			

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	345	220 / 0	0 / 0	0 / 0	0 / 0	125 / 0	0 / 0
E	308	191 / 0	0 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LC1 (LC)		
FR-TO		FROM	TO		FR-TO				
A-B	0 / 35	-78.0	-78.0	0.16 (1)	10.00	C-F	-101 / 20	0.02 (1)	
B-C	-215 / 0	-78.0	-78.0	0.20 (1)	6.25	B-F	0 / 181	0.04 (1)	
C-D	0 / 0	-111.3	-111.3	0.10 (1)	10.00	C-E	-216 / 0	0.04 (1)	
E-D	-121 / 0	0.0	0.0	0.02 (1)	7.81				
G-B	-424 / 0	0.0	0.0	0.05 (1)	7.81				
G-H	0 / 0	-26.4	-26.4	0.19 (4)	10.00				
H-F	0 / 0	-26.4	-26.4	0.19 (4)	10.00				
F-E	0 / 206	-26.4	-26.4	0.04 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	3-8-8	-106	-106	--	FRONT	VERT

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 21.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 34.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 3-8-8
RIGHT SETBACK = 0-0
END SETBACK = 3-8-8
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADD'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.20/1.00 (B-C:1), BC=0.19/1.00 (F-G:4), WB=0.04/1.00 (C-E:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	
MT20	618	354	1667	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.39 (B) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

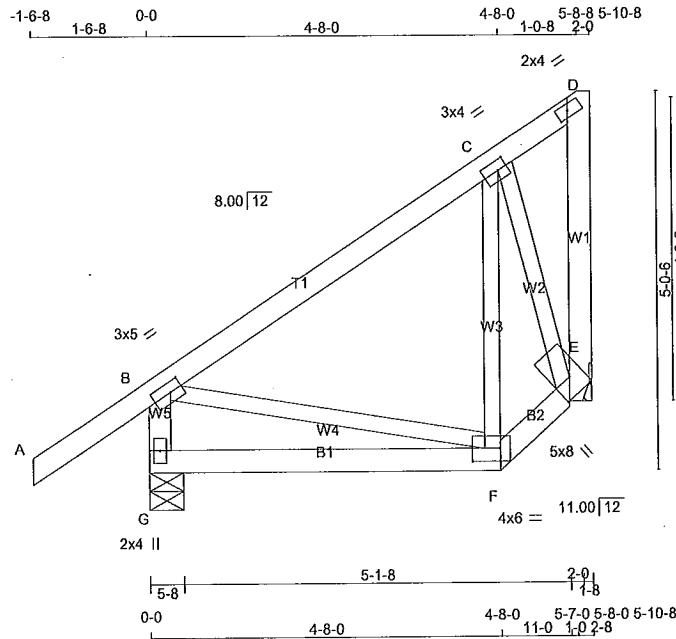
A-18023724

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292656	J2TC	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

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TOTAL WEIGHT = 2 X 32 = 63 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	
G - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	3.0	5.0	1.50	2.00
C	TMWW-I	MT20	3.0	4.0	1.50	1.50
D	TVM-I	MT20	2.0	4.0	1.00	1.75
E	BMVW1+I	MT20	5.0	8.0	2.75	2.75
F	BBWW-I	MT20	4.0	6.0	2.00	4.50
G	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	410	0	410	0	5-8	1-8
E	283	0	283	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	290	191 / 0	0 / 0	0 / 0	0 / 0	98 / 0	0 / 0
E	202	123 / 0	0 / 0	0 / 0	0 / 0	79 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0 / 35	-78.0	-78.0	0.15 (1)	10.00	F-C	-71 / 20	0.02 (1)
B-C	-117 / 0	-78.0	-78.0	0.22 (1)	6.25	C-E	-261 / 0	0.06 (1)
C-D	-66 / 0	-78.0	-78.0	0.21 (1)	6.25	B-F	0 / 115	0.03 (1)
E-D	0 / 68	0.0	0.0	0.01 (1)	10.00			
G-B	-367 / 0	0.0	0.0	0.04 (1)	7.81			
G-F	0 / 0	-18.5	-18.5	0.12 (4)	10.00			
F-E	0 / 153	-18.5	-18.5	0.03 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.22/1.00 (B-C:1), BC=0.12/1.00 (F-G:4), WB=0.06/1.00 (C-E:1), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

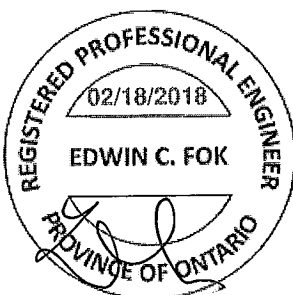
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

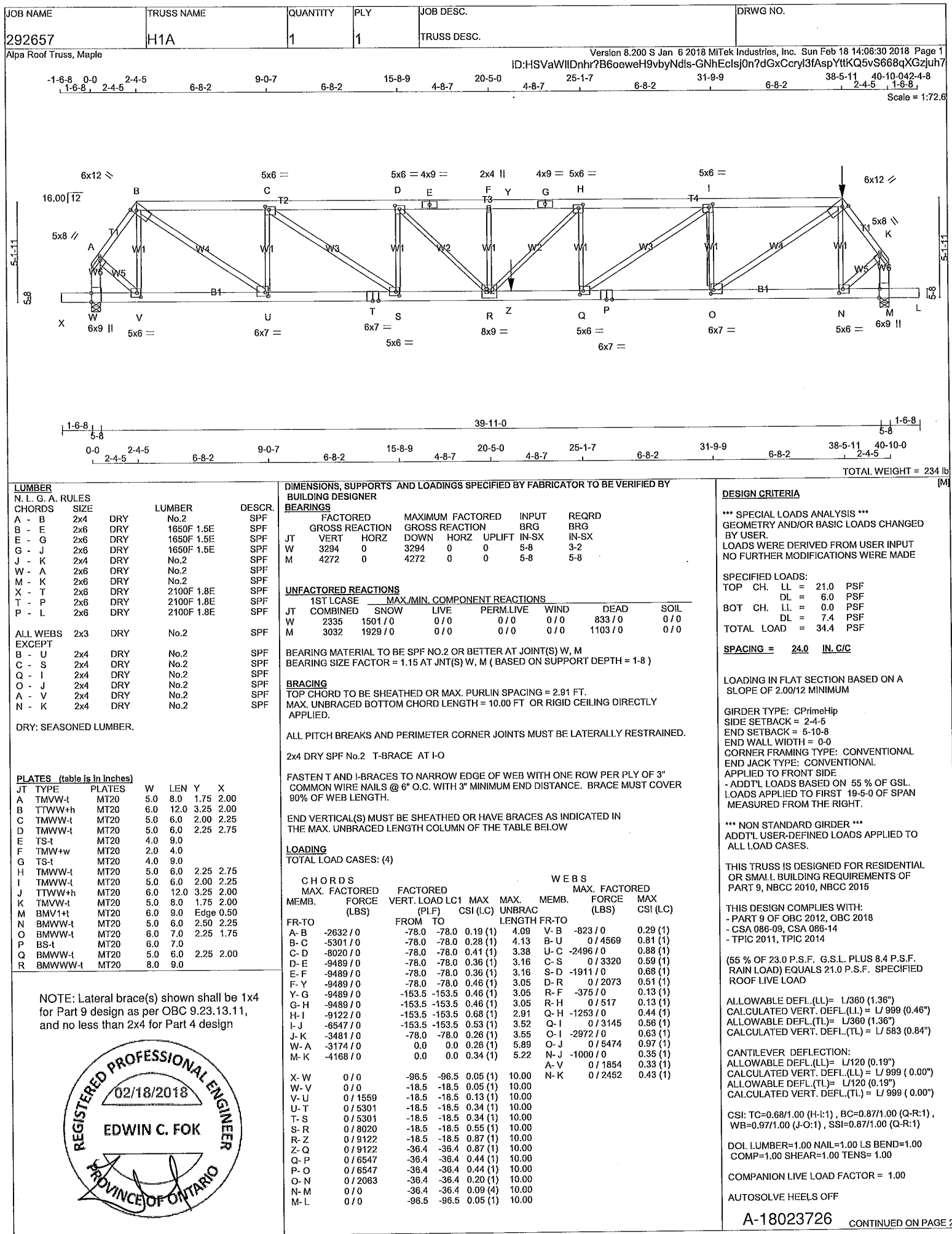
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023725



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292657	H1A	1	1	TRUSS DESC.	

PLATES (table is in inches)							FACTORED CONCENTRATED LOADS (LBS)							TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .				
JT	TYPE	PLATES	W	LEN	Y	X	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	NAIL VALUES			
S	BMWV-t	MT20	5.0	6.0	2.25	2.00	J	38-5-11	-134	-134	---	FRONT	VERT	TOTAL	PLATE	GRIP(DRY)	SHEAR	SECTION
T	BS-t	MT20	6.0	7.0			Z	21-6-8	-1559	-1559	---	FRONT	VERT	TOTAL	(PSI)	(PLI)	(PLI)	
U	BMWV-t	MT20	6.0	7.0	2.25	1.75									MAX	MIN	MAX	MIN
V	BMWV-t	MT20	5.0	6.0	2.50	2.25									MT20	618	354	1667
W	BMV1-t	MT20	6.0	9.0	Edge												788	1987
																		1656

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 134.1 lbs FACTORED DOWN AT 38-5-11 ON TOP CHORD, AND 1559.0 lbs FACTORED DOWN AT 21-6-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)

JSI METAL= 0.96 (U) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

REGISTERED PROFESSIONAL ENGINEER

02/18/2018

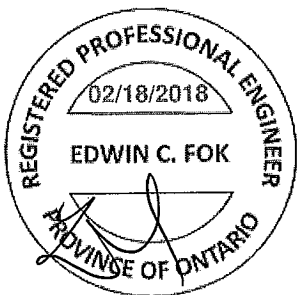
EDWIN C. FOK

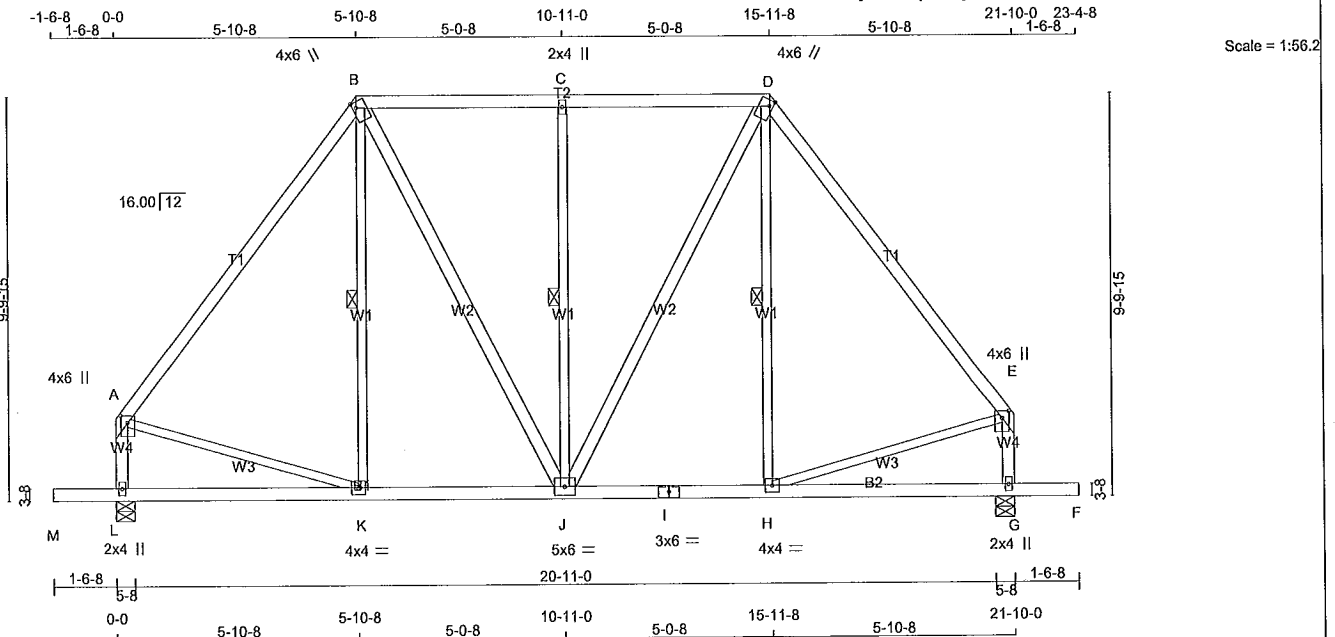
PROVINCE OF ONTARIO

A-18023726(v)

DRWG NO.

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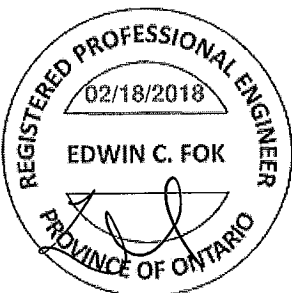
TOTAL WEIGHT = 2 X 123 = 245 lb

LUMBER		N. L. G. A. RULES		LUMBER		DESCR	
CHORDS	SIZE						
A - B	2x4	DRY	1650F	1.5E		SPF	
B - D	2x4	DRY	No.2			SPF	
D - E	2x4	DRY	1650F	1.5E		SPF	
L - A	2x4	DRY	No.2			SPF	
G - E	2x4	DRY	No.2			SPF	
M - I	2x4	DRY	No.2			SPF	
I - F	2x4	DRY	No.2			SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2			SPF	
B - J	2x4	DRY	No.2			SPF	
J - D	2x4	DRY	No.2			SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	6.0	2.00	2.00
B	TTWV+m	MT20	4.0	6.0	1.75	1.00
C	TMW+w	MT20	2.0	4.0		
D	TTWV+m	MT20	4.0	6.0	1.75	1.00
E	TMWV+p	MT20	4.0	6.0	2.00	2.00
G	BMV1+p	MT20	2.0	4.0		
H	BMWVW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWVWV-t	MT20	5.0	6.0		
K	BMWVW-t	MT20	4.0	4.0		
L	BMV1+p	MT20	2.0	4.0		

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
L	1240	0	1240	0	0	5-8	1-13
G	1240	0	1240	0	0	5-8	1-13

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	887	523 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0
G	887	523 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

FOR SECTION B-D, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, C-J, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)		LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM	TO			FR-TO			
A - B	-849 / 0	-78.0	-78.0	0.31 (1)	6.25	K - B	-16 / 75	0.03 (4)	
B - C	-665 / 0	-85.5	-85.5	0.28 (1)	2.00	B - J	0 / 337	0.05 (1)	
C - D	-665 / 0	-85.5	-85.5	0.28 (1)	2.00	J - C	-522 / 0	0.34 (1)	
D - E	-849 / 0	-78.0	-78.0	0.31 (1)	6.25	J - D	0 / 337	0.05 (1)	
L - A	-1046 / 0	0.0	0.0	0.12 (1)	7.69	H - D	-16 / 75	0.03 (4)	
G - E	-1046 / 0	0.0	0.0	0.12 (1)	7.69	A - K	0 / 526	0.12 (1)	
						H - E	0 / 526	0.12 (1)	
M - L	0 / 0	-96.5	-96.5	0.16 (1)	10.00				
L - K	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
K - J	0 / 507	-18.5	-18.5	0.20 (4)	10.00				
J - I	0 / 507	-18.5	-18.5	0.20 (4)	10.00				
I - H	0 / 507	-18.5	-18.5	0.20 (4)	10.00				
H - G	0 / 0	-18.5	-18.5	0.15 (4)	10.00				
G - F	0 / 0	-96.5	-96.5	0.16 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL =	21.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	0.0	PSF
		DL =	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON
PIGGYBACK TRUSS WITH SLOPES OF 6.00/12
AND -6.00/12 AND RESPECTIVE WALL
HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL
DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/120$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.31/1.00 (A-B:1), BC=0.20/1.00 (J-K:4),
WB=0.34/1.00 (C-J:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

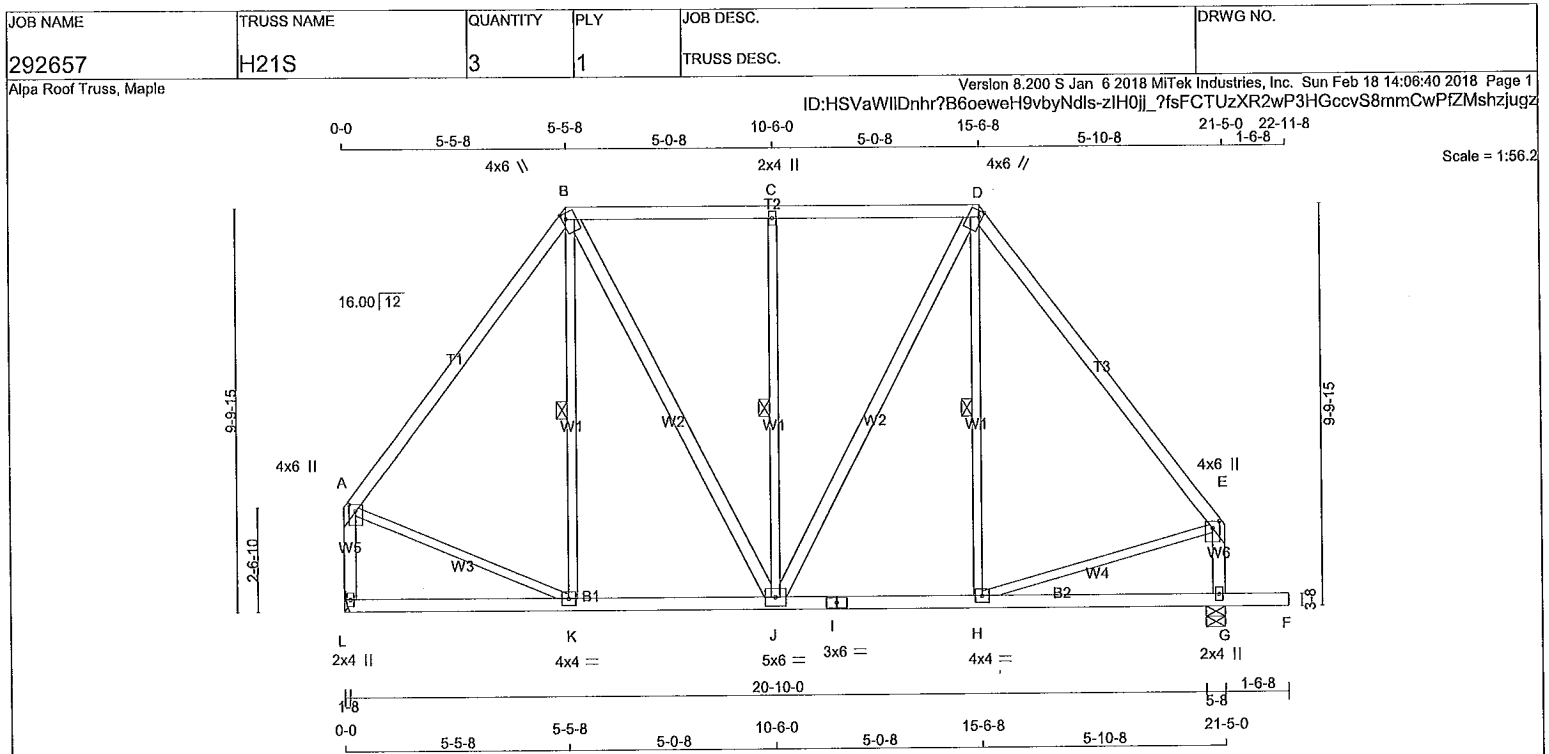
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (B) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

A-18023728



LUMBER				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
A - B	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	1650F 1.5E	SPF	
L - A	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
I - F	2x4	DRY	No.2	SPF	

ALL WEBS EXCEPT				N. L. G. A. RULES	
CHORDS	SIZE	LUMBER	DESCR.	CHORDS	SIZE
B - J	2x4	DRY	No.2	SPF	
J - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	6.0	2.00 2.00
B	TTWW+m	MT20	4.0	6.0	1.75 1.00
C	TMW+w	MT20	2.0	4.0	
D	TTWW+m	MT20	4.0	6.0	1.75 1.00
E	TMVW+p	MT20	4.0	6.0	2.00 2.00
G	BMV1+p	MT20	2.0	4.0	
H	BMWW-t	MT20	4.0	4.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	5.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	MIN. SEAT SIZE	1-8
JT	1072	0	1072	0	0	0	5-8	1-13
L	1219	0	1219	0	0	0	5-8	1-13

UNFACTORED REACTIONS

	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
	COMBINED															
JT	767	450 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0	0 / 0	0 / 0
G	872	514 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	358 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

FOR SECTION B-D, MAX. PURLIN SPACING = 2.00 FT.
FOR OTHER SECTIONS, TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-K, C-J, D-H.

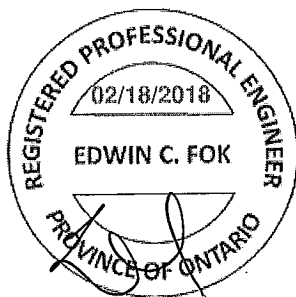
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MEMB.	FORCE (LBS)	MAX. UNBRACED LENGTH	MAX. CS1 (LC)
FR-TO				FR-TO			
A-B	-791 / 0	-78.0	-78.0 0.33 (1)	K-B	-89 / 48	0.04 (1)	0.04 (1)
B-C	-641 / 0	-85.5	-85.5 0.28 (1)	B-J	0 / 363	0.06 (1)	0.06 (1)
C-D	-641 / 0	-85.5	-85.5 0.28 (1)	J-C	-522 / 0	0.34 (1)	0.34 (1)
D-E	-828 / 0	-78.0	-78.0 0.31 (1)	J-D	0 / 314	0.05 (1)	0.05 (1)
L-A	-1030 / 0	0.0	0.0 0.14 (1)	H-D	-13 / 76	0.03 (4)	0.03 (4)
L-E	-1025 / 0	0.0	0.0 0.12 (1)	A-K	0 / 507	0.11 (1)	0.11 (1)
				H-E	0 / 512	0.12 (1)	0.12 (1)
L-K	0 / 0	-18.5	-18.5 0.13 (4)				
K-J	0 / 472	-18.5	-18.5 0.18 (4)				
J-I	0 / 494	-18.5	-18.5 0.20 (4)				
I-H	0 / 494	-18.5	-18.5 0.20 (4)				
H-G	0 / 0	-18.5	-18.5 0.15 (4)				
G-F	0 / 0	-96.5	-96.5 0.16 (1)				

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.4 PSF	
TOTAL LOAD = 34.4 PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON PIGGYBACK TRUSS WITH SLOPES OF 6.00/12 AND -6.00/12 AND RESPECTIVE WALL HEIGHTS OF 0-0 AND 0-0 AND AN ADDITIONAL DEAD LOAD OF 3.0 P.S.F.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.71")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.71")
CALCULATED VERT. DEFL.(TL)= L/999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL)= L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL)= L/999 (0.01")

CSI: TC=0.33/1.00 (A-B:1), BC=0.20/1.00 (H-J:4), WB=0.34/1.00 (C-J:1), SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR		SECTION	
(PSI)	(PLI)	(PSI)	(PLI)	(PSI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

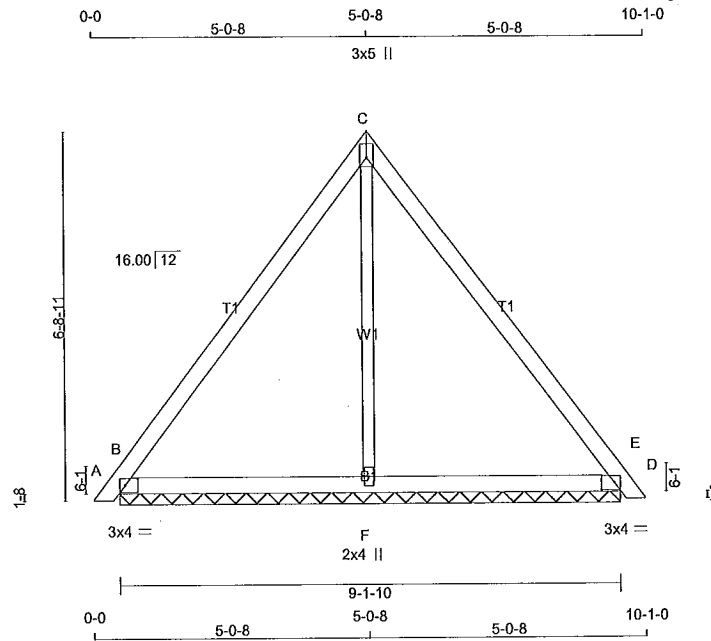
JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.32 (E) (INPUT = 1.00)

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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292657	H23P	2	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MITek Industries, Inc. Sun Feb 18 14:08:46 2018 Page 1
ID:HSVavWIIIDnhr?B6oeweH9vbyNdls-oSflzm3iFi0LBPQhnJ1pJYWeoKXtAYKoob0h4Lzjugt



Scale = 1:42.0

TOTAL WEIGHT = 2 X 34 = 69 lb

LUMBER					
N. L. G. A. RULES					
CHORDS	SIZE		LUMBER		DESCR.
A - C	2x4	DRY	No.2		SPF
C - E	2x4	DRY	No.2		SPF
B - D	2x4	DRY	No.2		SPF
ALL WEBS	2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.					

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	1.50	2.75
C	TTW+p	MT20	3.0	5.0	2.00	Edge
D	TMB1-I	MT20	3.0	4.0	1.50	2.75
F	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
B	335	0	335	0	9-1-10	11-0
D	335	0	335	0	9-1-10	11-0
F	267	0	267	0	9-1-10	11-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM.LIVE	WIND			
B	235	163 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	235	163 / 0	0 / 0	0 / 0	0 / 0	72 / 0	0 / 0
F	196	87 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF, NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD LC1 (PLF)		MAX. UNBRACED LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)	
	FROM	TO	CS1 (LC)	UNBRACED		FROM	TO
A-B	0 / 10	-78.0	-78.0	0.01 (1)	10.00	F-C	-161 / 0
B-C	-121 / 0	-78.0	-78.0	0.28 (1)	6.25		
C-D	-121 / 0	-78.0	-78.0	0.28 (1)	6.25		
D-E	0 / 10	-78.0	-78.0	0.01 (1)	10.00		
B-F	0 / 72	-18.5	-18.5	0.12 (4)	10.00		
F-D	0 / 72	-18.5	-18.5	0.12 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1), BC=0.12/1.00 (D-F:4), WB=0.11/1.00 (C-F:1), SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

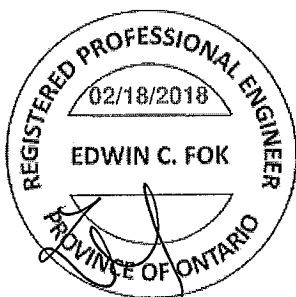
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

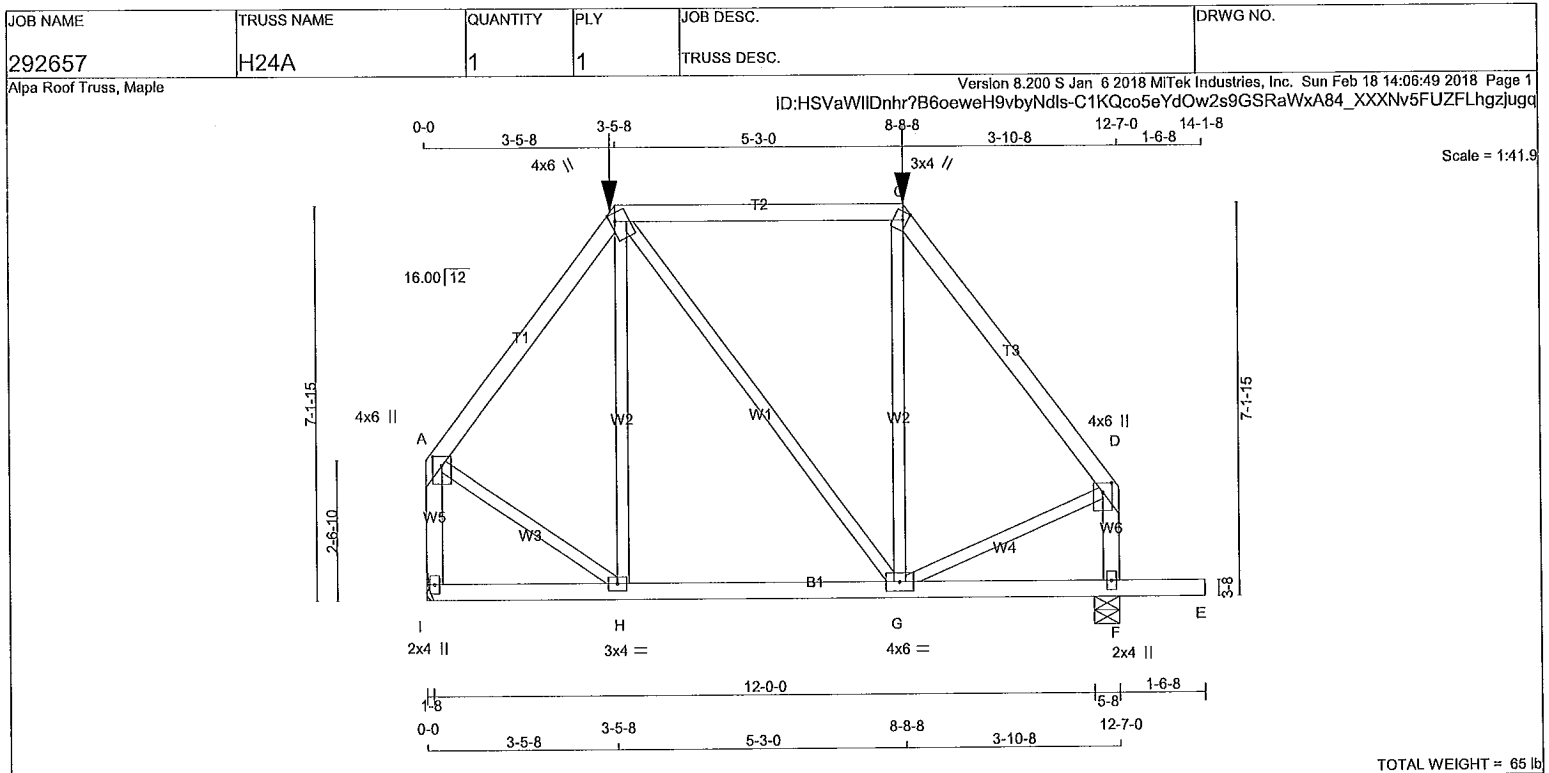
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (D) (INPUT = 0.80)
JSI METAL= 0.15 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



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LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	4.0	6.0	1.75	1.00
C	TTW+m	MT20	3.0	4.0	1.75	1.00
D	TMVW+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 119.6 lbs FACTORED DOWN AT 8-8-8, AND 106.7 lbs FACTORED DOWN AT 3-5-8 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX
I	875	0	875	0	0	HANGER BY OTHERS
F	1015	0	1015	0	0	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST I.CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
I	624	379 / 0	0 / 0	0 / 0	0 / 0	245 / 0	0 / 0
F	724	439 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	-606 / 0	-78.0 -78.0	0.19 (1)	6.25	H-B	-89 / 60	0.08 (1)
B-C	-385 / 0	-114.5 -114.5	0.60 (1)	6.25	B-G	0 / 42	0.01 (1)
C-D	-647 / 0	-78.0 -78.0	0.24 (1)	6.25	G-C	-52 / 80	0.05 (1)
I-A	-840 / 0	0.0 0.0	0.12 (1)	7.81	A-H	0 / 422	0.10 (1)
F-D	-826 / 0	0.0 0.0	0.10 (1)	7.81	G-D	0 / 416	0.10 (1)
I-H	0 / 0	-27.2 -27.2	0.13 (4)	10.00			
H-G	0 / 360	-27.2 -27.2	0.18 (4)	10.00			
G-F	0 / 0	-27.2 -27.2	0.14 (4)	10.00			
F-E	0 / 0	-96.5 -96.5	0.17 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
B	3-5-8	-107	-107	---	FRONT	VERT	TOTAL
C	8-8-8	-120	-120	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip

LEFT SETBACK = 3-5-8

RIGHT SETBACK = 3-10-8

END SETBACK = 3-10-8

END WALL WIDTH = 0-0

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, OBC 2018

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.60/1.00 (B-C:1), BC=0.18/1.00 (G-H:4),
WB=0.10/1.00 (A-H:1), SSI=0.26/1.00 (B-C:1)

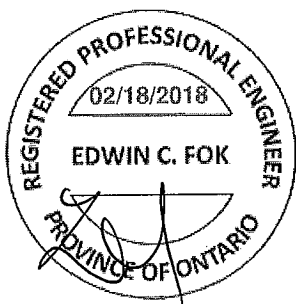
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

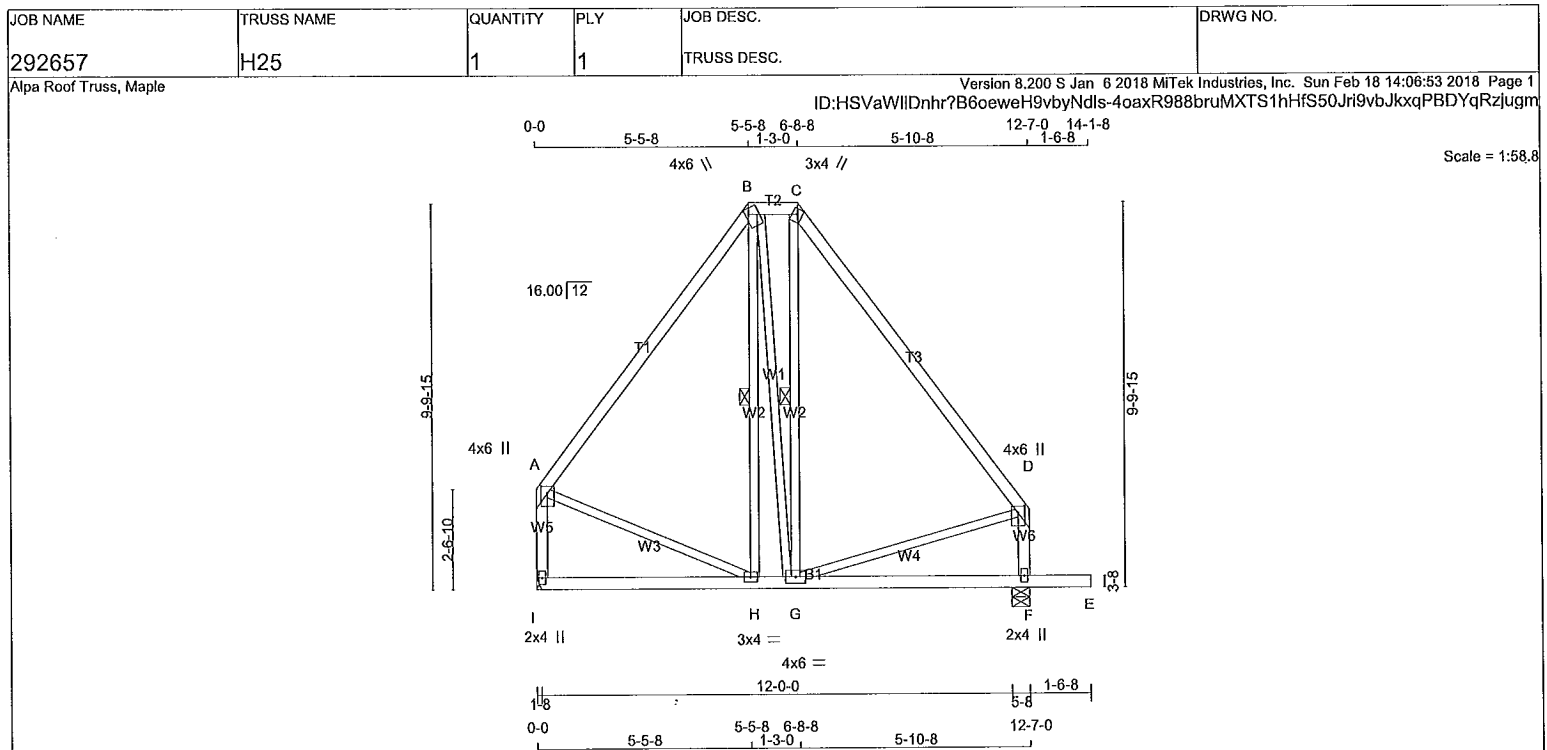
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design





TOTAL WEIGHT = 76 lb

LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	1650F 1.5E	SPF
I - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV+p	MT20	4.0	6.0	2.00	2.00
B	TTWW+m	MT20	4.0	6.0	1.75	1.00
C	TTW+m	MT20	3.0	4.0	Edge	
D	TMVV+p	MT20	4.0	6.0	2.00	2.00
F	BMV1+p	MT20	2.0	4.0		
G	BMVWW-t	MT20	4.0	6.0		
H	BMVWW-t	MT20	3.0	4.0		
I	BMV1+p	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ
JT				
I	607	0	607	0
F	756	0	756	0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. SNOW	MAX./MIN. LIVE	MAX./MIN. WIND	MAX./MIN. DEAD	MAX./MIN. SOIL
JT	433	264 / 0	0 / 0	0 / 0	169 / 0	0 / 0
I	539	329 / 0	0 / 0	0 / 0	210 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-H, C-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM TO	LENGTH FR-TO				
A-B	-341 / 0	-78.0	-78.0 0.30 (1)	H-B	-16 / 34	0.01 (4)	
B-C	-207 / 0	-78.0	-78.0 0.02 (1)	B-G	0 / 28	0.01 (4)	
C-D	-346 / 0	-78.0	-78.0 0.29 (1)	G-C	0 / 54	0.02 (4)	
I-A	-565 / 0	0.0	0.0 0.08 (1)	A-H	0 / 218	0.05 (1)	
F-D	-562 / 0	0.0	0.0 0.06 (1)	G-D	0 / 214	0.05 (1)	
I-H	0 / 0	-18.5	-18.5 0.13 (4)				
H-G	0 / 203	-18.5	-18.5 0.18 (4)				
G-F	0 / 0	-18.5	-18.5 0.15 (4)				
F-E	0 / 0	-96.5	-96.5 0.16 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 21.0	PSF
	DL = 6.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.4	PSF
TOTAL LOAD	= 34.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 23.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.42")
CALCULATED VERT. DEFL.(LL) =	L/999 (0.00")
ALLOWABLE DEFL.(TL) =	L/360 (0.42")
CALCULATED VERT. DEFL.(TL) =	L/999 (0.04")

CANTILEVER DEFLECTION:

ALLOWABLE DEFL.(LL) =	L/120 (0.19")
CALCULATED VERT. DEFL.(LL) =	L/999 (0.01")
ALLOWABLE DEFL.(TL) =	L/120 (0.19")
CALCULATED VERT. DEFL.(TL) =	L/999 (0.01")

CSI: TC=0.30/1.00 (A-B:1), BC=0.18/1.00 (G-H:4), WB=0.05/1.00 (A-H:1), SSI=0.12/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

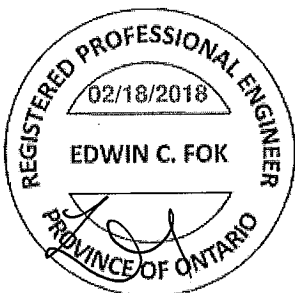
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.45 (C) (INPUT = 0.90)
JSI METAL= 0.17 (D) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design

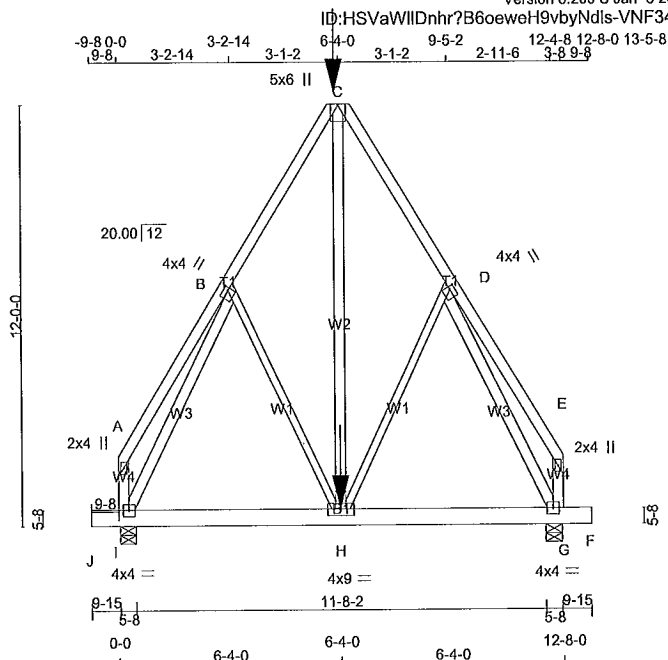


A-18023733

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292657	H26	1	1	TRUSS DESC.	

Alpa Roof Truss, Maple

Version 8.200 S Jan 6 2018 MiTek Industries, Inc. Sun Feb 18 14:08:56 2018 Page 1



Scale = 1:65.6

TOTAL WEIGHT = 97 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
I - A	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - F	2x6	DRY	No.2	SPF	

ALL WEBS	EXCEPT	2x4	DRY	No.2	SPF
B - H	2x3	DRY	No.2	SPF	
H - D	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
B	TMVW-t	MT20	4.0	4.0	1.50	1.00
C	TMTMW+p	MT20	5.0	6.0	Edge	
D	TMVW-t	MT20	4.0	4.0	1.50	1.00
E	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	4.0	4.0		
H	BMVW1-t	MT20	4.0	9.0		
I	BMVW1-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 793.3 lbs FACTORED DOWN AT 6-4-0 ON TOP CHORD, AND 306.6 lbs FACTORED DOWN AT 6-4-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
I	1364	0	1364	0
G	1364	0	1364	0

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	COMBINED						
I	973	593 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0
G	973	593 / 0	0 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	CS1 (LC)	WEBS	MAX. FACTORED	MAX	CS1 (LC)
MEMB.	FORCE (LBS)	VERT. (PLF)	FROM	TO	LENGTH	FR-TO	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO										
A-B	0 / 24	-78.0	-78.0	0.13 (1)	10.00	H-C	0 / 655	0.12 (1)		
B-C	-986 / 0	-78.0	-78.0	0.16 (1)	6.09	I-B	-1211 / 0	0.88 (1)		
C-D	-986 / 0	-78.0	-78.0	0.16 (1)	6.09	B-H	-33 / 13	0.04 (1)		
D-E	0 / 24	-78.0	-78.0	0.13 (1)	10.00	H-D	-33 / 13	0.04 (1)		
I-A	-91 / 0	0.0	0.0	0.01 (1)	7.81	D-G	-1211 / 0	0.88 (1)		
G-E	-91 / 0	0.0	0.0	0.01 (1)	7.81					
J-I	0 / 0	-96.5	-96.5	0.02 (1)	10.00					
I-H	0 / 511	-38.5	-38.5	0.26 (4)	10.00					
H-G	0 / 511	-38.5	-38.5	0.26 (4)	10.00					
G-F	0 / 0	-96.5	-96.5	0.02 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	6-4-0	-793	-793	---	FRONT	VERT	TOTAL
H	6-4-0	-307	-307	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	21.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	34.4	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CPrimeHip
LEFT SETBACK = 6-3-9
RIGHT SETBACK = 6-4-0
END SETBACK = 6-4-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL)= L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.16/1.00 (B-C:1), BC=0.26/1.00 (G-H:4), WB=0.88/1.00 (B-I:1), SS=0.17/1.00 (H-I:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

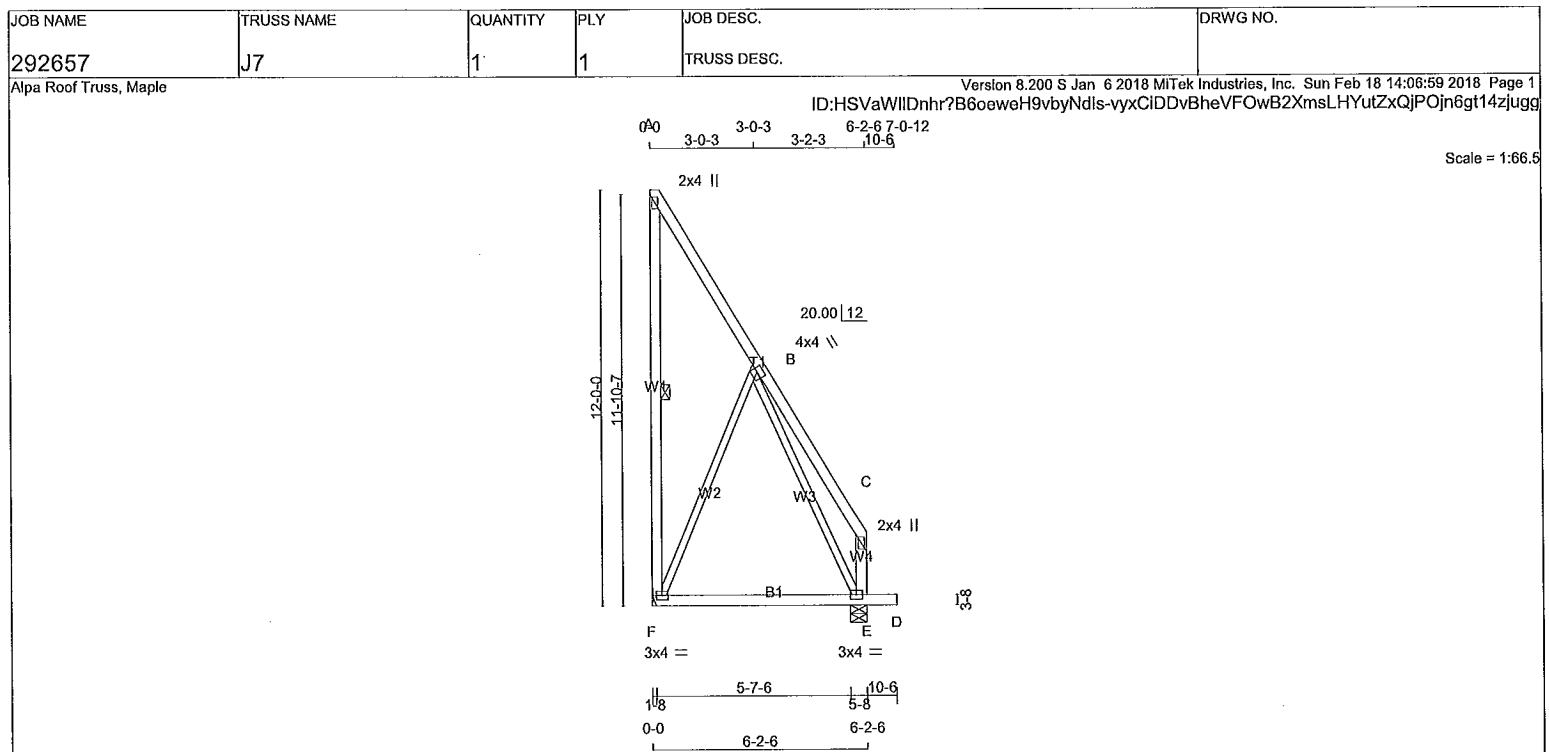
AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

A-18023734 CONTINUED ON PAGE 2

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design





LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER	DESCR.	
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMV+p	MT20	2.0	4.0
B	TMVW-t	MT20	4.0	4.0 2.00 1.50
C	TMV+p	MT20	2.0	4.0
E	BMVW1-t	MT20	3.0	4.0
F	BMVW1-t	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
F	299	0	299	0	0
E	382	0	382	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	213	130 / 0	0 / 0	0 / 0	0 / 0	0 / 0	83 / 0	0 / 0
E	273	166 / 0	0 / 0	0 / 0	0 / 0	0 / 0	106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. MEMB. FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
F-A	-96 / 0	0.0 0.0 0.08 (1)	6.25
A-B	-25 / 0	-78.0 -78.0 0.10 (1)	6.25
B-C	0 / 26	-78.0 -78.0 0.13 (1)	10.00
E-C	-86 / 0	0.0 0.0 0.01 (1)	7.81
F-E	0 / 70	-18.5 -18.5 0.21 (4)	10.00
E-D	0 / 0	-96.5 -96.5 0.05 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 21.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 34.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, OBC 2018
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 23.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 21.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CANTILEVER DEFLECTION:
ALLOWABLE DEFL.(LL) = L/120 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/120 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (B-C:1), BC=0.21/1.00 (E-F:4), WB=0.18/1.00 (B-E:1), SSI=0.08/1.00 (E-F:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

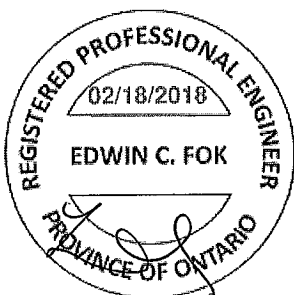
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

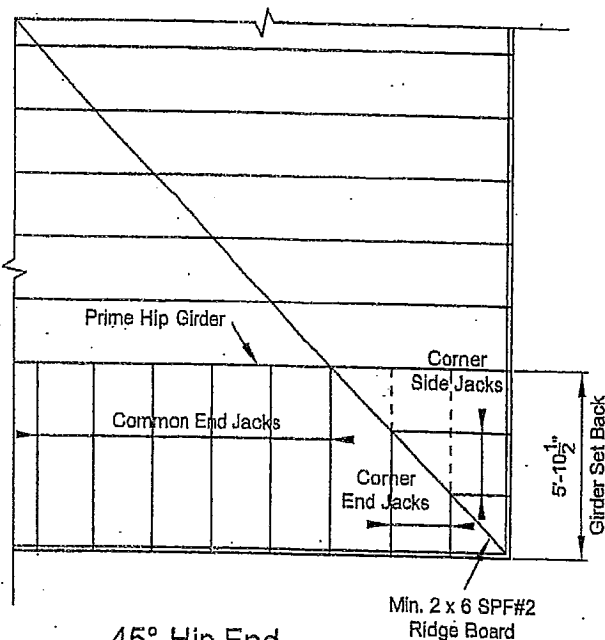
JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

NOTE: Lateral brace(s) shown shall be 1x4 for Part 9 design as per OBC 9.23.13.11, and no less than 2x4 for Part 4 design



A-18023735

STRACON ENGINEERING INC.



45° Hip End

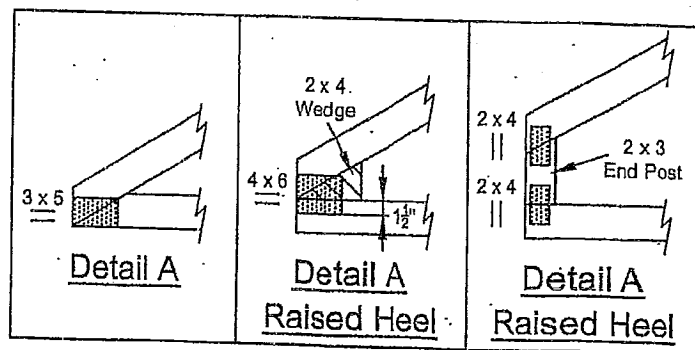
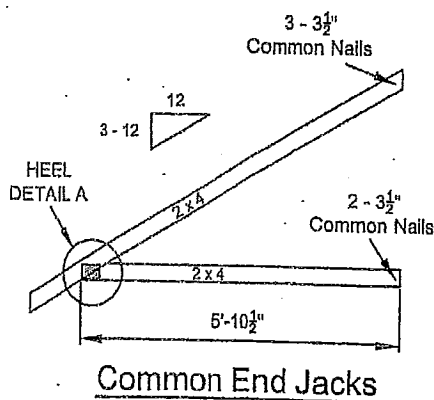
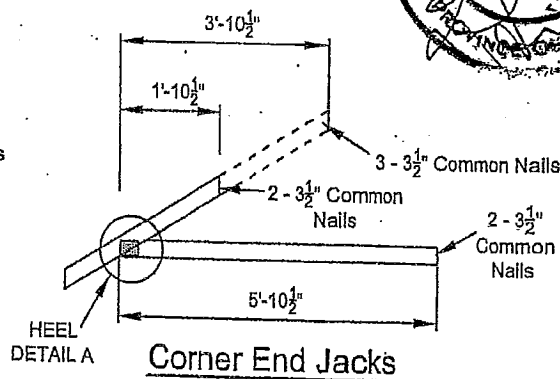
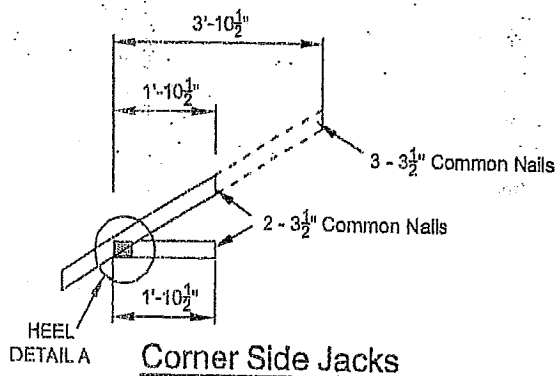
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

CS-51008

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

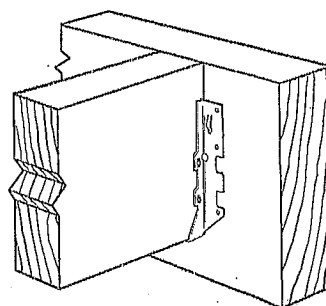
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

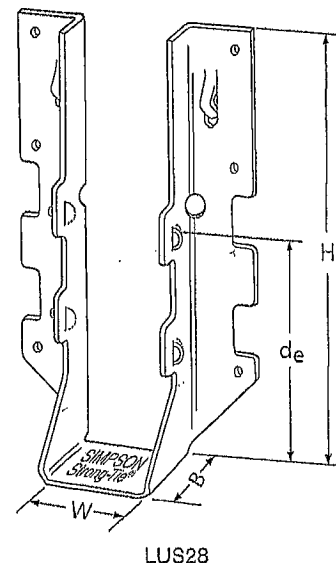
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or naller applications.

Options:

- These hangers cannot be modified



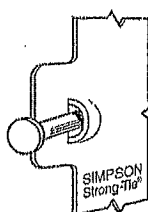
Typical LUS
Installation



LUS28

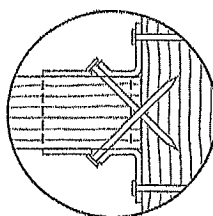
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d _o ¹	Face	Joist	Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 1/4	3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 3/8	2	3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 1/4	3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_o is the distance from the seat of the hanger to the highest joist nail.

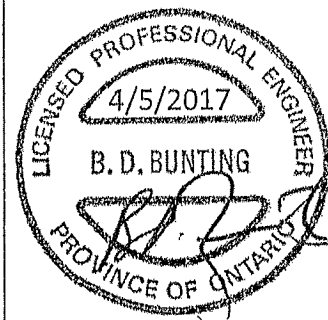


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 14, 2017.

This information is updated periodically and should not be relied upon after June 30, 2019.

Contact Simpson Strong-Tie for current information and limited warranty at strongtie.com.

© 2017 Simpson Strong-Tie Company Inc.

T-SPEC/LUS17/3/17 exp. 6/19

(800) 999-5099
strongtie.com

HHUS – Double Shear Joist Hangers



All HHUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 14 gauge

Finish: G90 galvanized

Design:

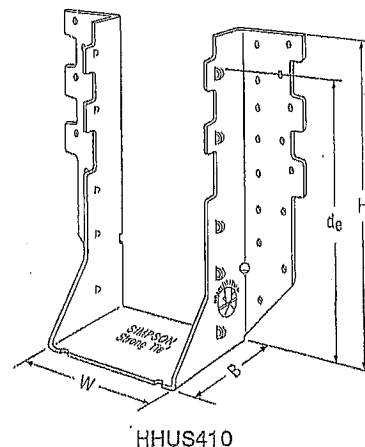
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

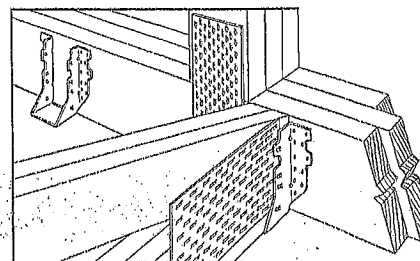
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

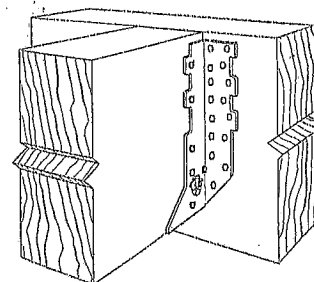
- See current catalogue for options



HHUS410



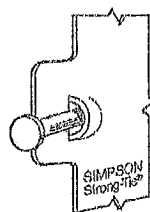
Typical HHUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)



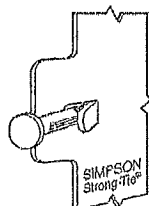
Typical HHUS Installation

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Flr-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
HHUS26-2	14	3 5/16	5 1/16	3	3 1/16	(14) 16d	(6) 16d	2850	7335	2065	5205
HHUS28-2	14	3 5/16	7 7/32	3	6 5/32	(22) 16d	(8) 16d	3765	8940	2675	6345
HHUS210-2	14	3 5/16	9 9/32	3	8	(30) 16d	(10) 16d	4745	9660	4310	7000
HHUS210-3	14	4 1/16	9	3	7 1/16	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS210-4	14	6 1/8	8 29/32	3	7 27/32	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS46	14	3 3/8	5 1/32	3	3 1/16	(14) 16d	(6) 16d	2540	7335	2065	5205
HHUS48	14	3 3/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	3765	8945	2267	6345
HHUS410	14	3 3/8	9	3	8	(30) 16d	(10) 16d	4745	9855	4310	7000
HHUS5.50/10	14	5 1/2	9	3	8	(30) 16d	(10) 16d	4745	10545	4310	7485
HHUS7.25/10	14	7 1/4	9	3 5/16	7 29/32	(30) 16d	(10) 16d	4745	10770	4310	7650

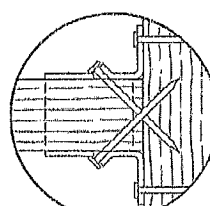
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHHUS17 3/17 exp. 6/19

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strongtie.com

HUS/LJS – Double Shear Joist Hangers



All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

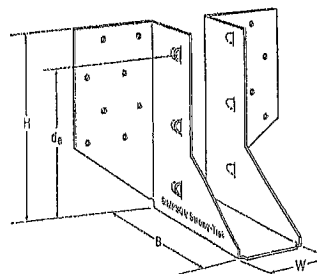
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

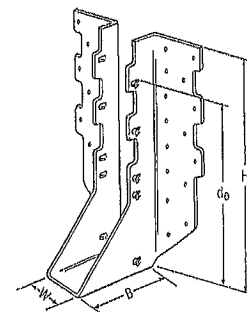
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

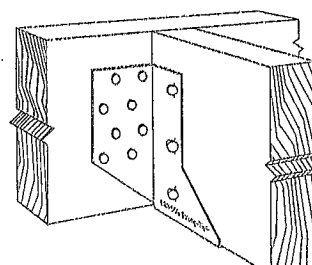
- See current catalogue for options



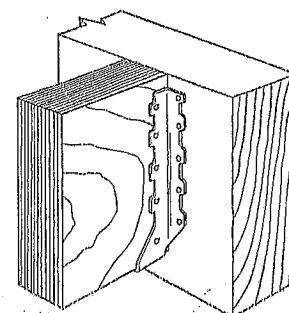
LJS26DS



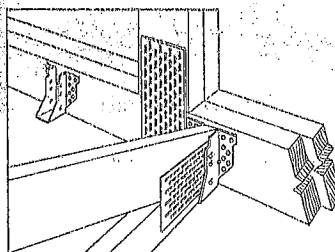
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



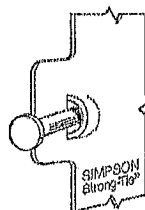
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

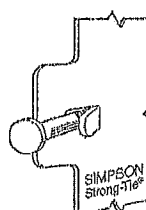
Model No.	Ga.	Dimensions (In.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d		2055	4265	1480	4115
HUS26	16	1 1/8	5 3/8	3	3 15/16	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/32	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 1/8	9 3/8	3	7 21/32	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

1. d₀ is the distance from the seat of the hanger to the highest joist nail.

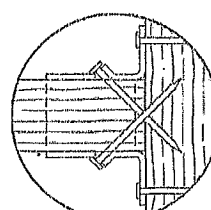


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

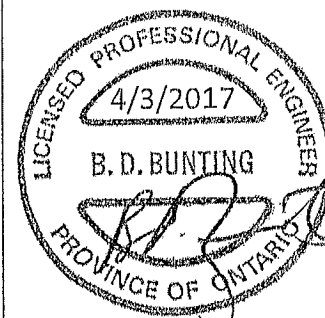
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com

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