Ground Floor

Qty Plies

Qty

3

2

Qty Plies

Beam/Girder

fasteners

4 10dx1 1/2

4 10dx1 1/2

Plies

2

2

2

Pcs Length

3 6-0-0

Member

12

14-0-0

2

2

1

Width Depth

1.75 11.875

1.75 11.875

Width Depth

1.5 11.875

1.5 11.875

1.5 11.875

1.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

2.5 11.875

Width Depth

Skew Slope

2.5 11.875 LinFt

Framer to verify dimensions on the architectural drawings.

3. Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls

2. Double joist only require filler/backer ply when supporting

. Install single-ply flush window header along inside face of

. Load transfer blocks to be installed under all point loads.

fastened as per the hanger manufacturer's standards.

6. Squash blocks recommended to be installed at end bearing on

all first level joists which support loading from above exceeding

8. It shall be the frame's responsibility that floor joists and beams are

Refer to Multiple Member Connection Detail to ply to ply nailing or

rim depth @ 16"o/c). All other components and structural elements

Hatch are represents ceramic tiled floor with an additional dead load

supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and

bracing for lateral stability are the responsibility of Others.

another member using a face-mounted hanger.

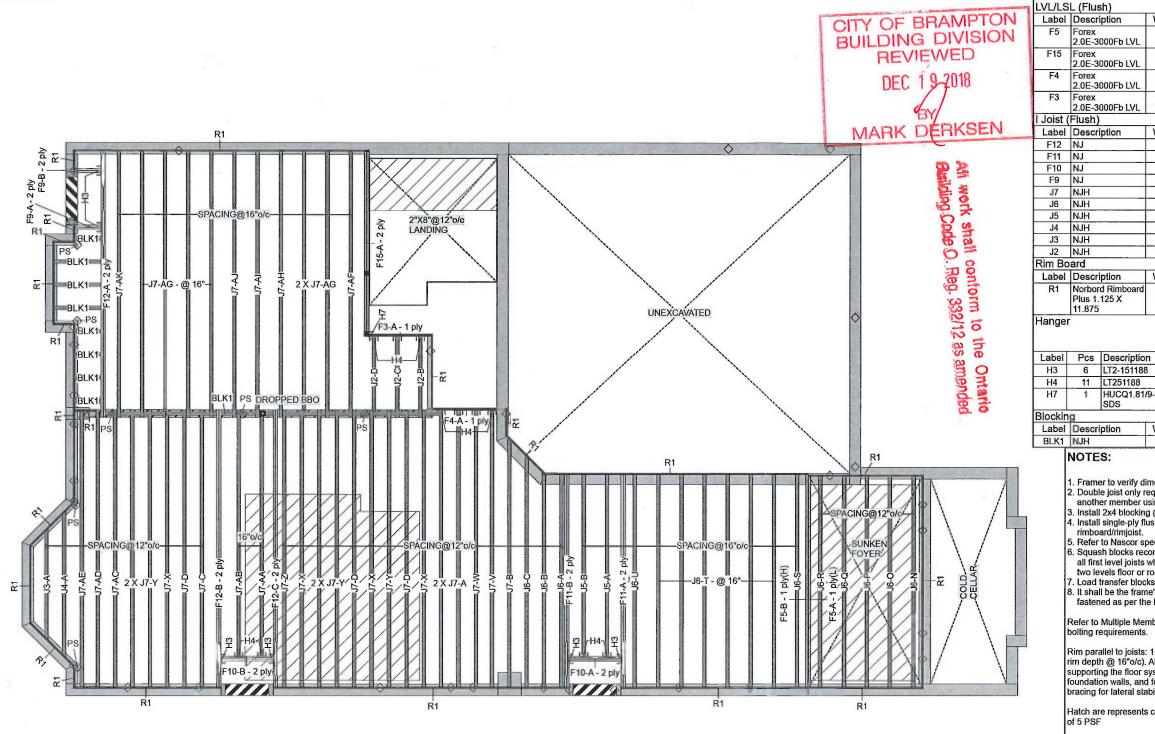
5. Refer to Nascor specifier guide for installation works.

1.125 11.875

1.75

11.875

11.875



This certification is to confirm that:

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout, 2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout,

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



September 13, 2018

1. OBC 2012 O.Reg 332/12 as amended

2. Nascor CCMC - 13535-R

3. LVL CCMC -14056-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

Engineered floor joists shall be installed The framing shown on this layout may deviate from the architectural in accordance with the supplier's layout an drior to construction.

and structural drawings. Project Engineer to review and apporve the deviation specifications forming part of the permit drawing ARCHITECTURAL DRAWINGS:

Ground Floor

Forex

Forex

Forex

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

2.0E-3000Fb LVL

Norbord Rimboa

11 LT251188

rimboard/rimjoist.

bolting requirements.

two levels floor or roof.

HUCQ1.81/9-SDS

Plus 1.125 X

11.875

NOTES:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

Legend

of 5 PSF

Point Load Support Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 11.875 N.I 11 875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

18-3332940000000 Floor

Version 18.40.162 Powered by iStruct™

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

Layout Name 12-0-0 MILLWOOD 2-ELEV 2 6-0-0 Design Method 4-0-0 Description Created June 25, 2018

Pcs Length 6 16-0-0 Builder 4 14-0-0 **GREENPARK** 4 4-0-0 Sales Rep 4 2-0-0 35 16-0-0 RM16 14-0-0 Designer 2 12-0-0 RO 1 10-0-0 1 8-0-0 Shipping

Project

Builder's Project Pcs Length Kott Lumber Company 14 Anderson Blvd

Stouffville, Ontario Canada K2H7V1 Supported 905-642-4400

fasteners Job Path 2 10dx1 1/2 S:\CUSTOMERS\GREENPARK 2 10dx1 1/2 \MINNISALE HOMES\MODELS \MILLWOOD 2\FLOORS\ELEV 2 \MILLWOOD 2-ELEV 2.isl

Ground Floor Width Depth Qty Plies Pcs Length Design Method

LSD Varies 29-0-0 Building Code NBCC 2010 / OBC 2012 Floor Loads 40 Live 15 Dead **Deflection Joist**

LL Span L/ 480 TL Span L/ 360 480 LL Cant 2L/ 360 TL Cant 2L/ **Deflection Girder** 360

240

480

240

3/4"

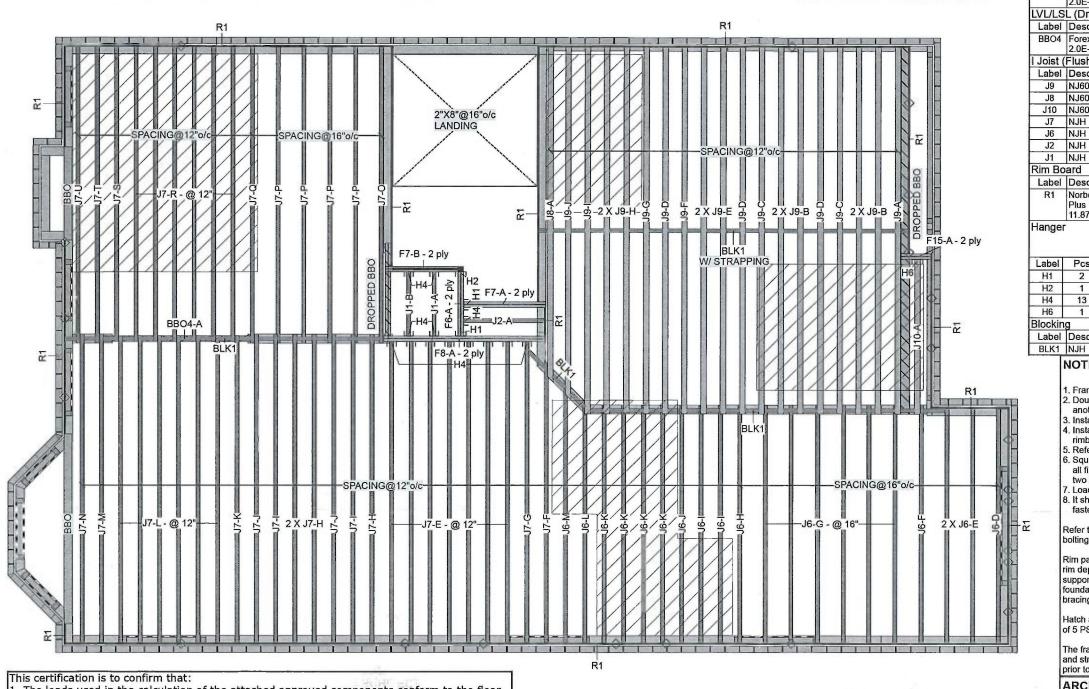
TL Span L/ LL Cant 2L/ TL Cant 2L/

LL Span L/

Decking Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than SPF Plywood Deck Thickness Fastener Nailed & Glued

Vibration

Second Floor



. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

\$

111111

PS

Legend

Point Load Support Load from Above Wall Opening Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875 NJ60U 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped) Forex 2.0E-3000Fb LVL 1.75 X 11.875

Label Description Width Depth Pcs Length Qty Plies F8 Forex 2.0E-3000Fb LVL 11.875 2 F7 1.75 11.875 6-0-0 2.0E-3000Fb LVL F6 Forex 4-0-0 11.875 2.0E-3000Fb LVL F15 11.875 2-0-0 Forex 1.75 2 2 2.0E-3000Fb LVL LVL/LSL (Dropped) Label Description Width Depth Qty Plies Pcs Length BBO4 Forex 1.75 9.5 8-0-0 2.0E-3000Fb LVL Joist (Flush) Label Description Width Depth | Qty | Plies Pcs Length J9 NJ60U 3.5 11.875 18 20-0-0 J8 NJ60U 3.5 11.875 18-0-0 1 J10 NJ60U 3.5 11.875 8-0-0 J7 NJH 2.5 11.875 40 16-0-0 J6 NJH 25 11 875 20 14-0-0 J2 NJH 2.5 11.875 6-0-0 J1 NJH 2.5 11.875 2 4-0-0 Rim Board Label Description Width Depth Qty Plies Pcs Length R1 Norbord Rimboar 1.125 11.875 14 Plus 1.125 X 11.875 Hanger Beam/Girder Supported

Skew Slope

Width Depth Qty Plies

2.5 11.875 LinFt

fasteners

46 16d

14 16d

4 10dx1 1/2

	MO	TES:
1	NO	IES.

Label Description

Label Pcs Description

H6 1 LT351188

H2

H4

2 HGUS410

13 LT251188

1 HUC410 (Min)

Second Floor LVL/LSL (Flush)

- 1 Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24*o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of
- rimboard/rimjoist. 5. Refer to Nascor specifier guide for installation works.
- 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- . Load transfer blocks to be installed under all point loads. It shall be the frame's responsibility that floor joists and beams are
- fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

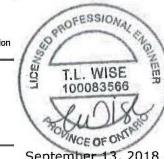
The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

Layout Name MILLWOOD 2-ELEV 2 Design Method Description Created June 25, 2018 Builder **GREENPARK** Sales Rep RM Designer RO Shipping Project Builder's Project **Kott Lumber Company** 14 Anderson Blvd Stouffville, Ontario Canada K2H7V1 905-642-4400 Job Path fasteners S:\CUSTOMERS\GREENPARK MINNISALE HOMESIMODELS 16 16d \MILLWOOD 2\FLOORS\ELEV 2 6 10d \MILLWOOD 2-ELEV 2.isl 2 10dx1 1/2 Second Floor 4 10dx1 1/2 2 10dx1 1/2 Design Method LSD Pcs Length Building Code NBCC 2010 / OBC 2012 Varies 40-0-0 Floor Loads 40 Live Dead 15 Deflection Joist LL Span L/ 480 360 TL Span L/ 480 LL Cant 2L/ TL Cant 2L/ 360 **Deflection Girder** LL Span L/ 360 TL Span L/ 240



LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Vibration

Deck

480

240

SPF Plywood

Nailed & Glued

Gypsum 1/2"

September 13, 2018



Version 18.40.162 Powered by iStruct™

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

MULTIPLE MEMBER CONNECTIONS

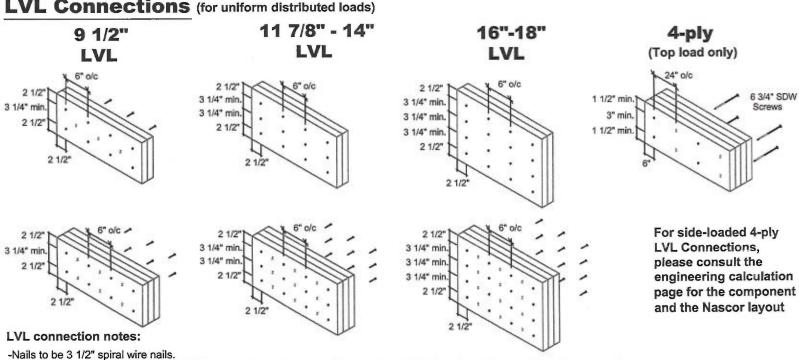
Conventional Connections (for uniform distributed loads)

2x10 2x12 2x6 2x8 2-ply 3-ply

Conventional connection notes:

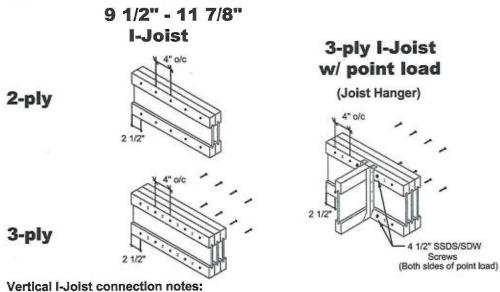
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

KOTT 3228 Moodie Drive Ottawa, ON **K2H7V1** Ph: 613-838-2775

DETAILS Date: November 30, 2016

MULTI-PLY CONNECTION

Scale: NTS

TW0918-058

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



MULTIPLE MEMBER CONNECTIONS

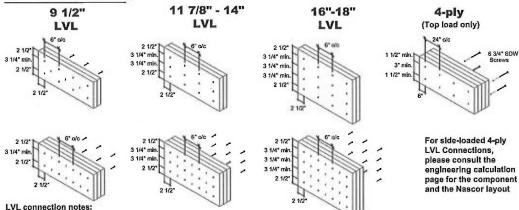
Conventional Connections (for uniform distributed loads)

2x12 2x6 2x8 2-ply 3-ply

Conventional connection notes:

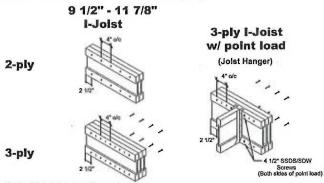
- -Nails to be 3* 10d spiral wire nails.
 -Nails to be located a minimum of 2* from the top and bottom of the member. Start all nails a minimum of 2 1/2* in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- LVL connection notes:
- -Nails to be 3 1/2" spiral wire nails.
 -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Naminum 3 1/4* spacing between rows.
 -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nall or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)

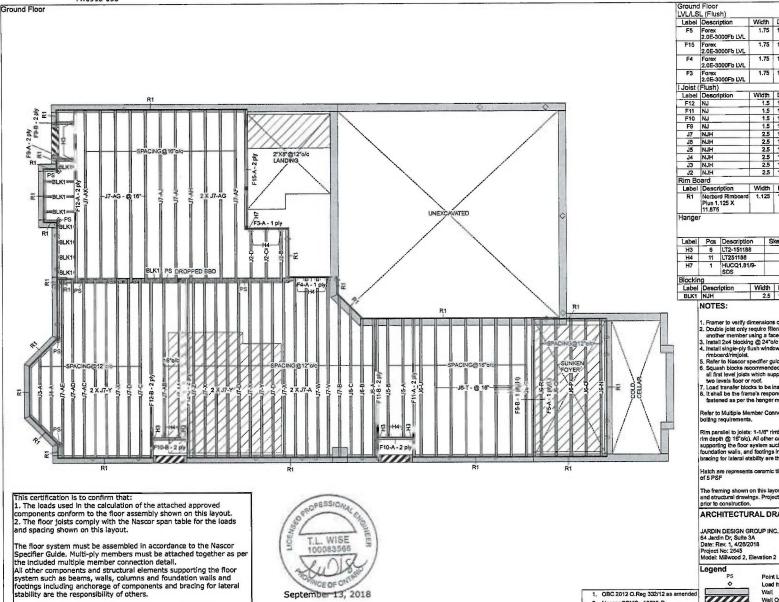


Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails
- -Nails to be to applia with hairs.

 -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nall driven from the opposite side.

MULTI -PLY CONNECTION



Qty Plies Pcs Length Width Depth F5 Forex 2.0E-3000Fb LVL 1.75 11.875 2 Layout Name 1.75 11.875 12-0-0 MILLWOOD 2-ELEV 2 2.0E-3000Fb LVL Forex 2.0E-3000Fb LVL 5-0-0 Design Method 1.75 11.875 LSD Forex 2.0E-3000Fb LVL 1.75 11,875 4-0-0 Description Created
 Width
 Depth
 Qty
 Piles

 1.5
 11.875
 3
 2
 Pcs Length Label Description June 25, 2018 6 16-0-0 Builder 1.5 11,875 4 14-0-0 GREENPARK 4 4-0-0 1.5 11.875 4 2-0-0 Sales Rep 1.5 11.875 2.5 11.875 2.5 11.875 35 18-0-0 RM 16 14-0-0 Designer 2 12-0-0 RO 2.5 11.875 2.5 11.875 1 10-0-0 1 8-0-0 Shipping 3 6-0-0 Project 2.5 11.875 Builder's Project Width Depth Qty Plies Pcs Length Label Description Kott Lumber Company 1.125 11.875 Norbord Rimbos 14 Anderson Blvd Plus 1,125 X 11,875 Stouffville, Ontario Canada Beam/Glider Supported K2H7V1 Member 905-642-4400 fasteners Label Pcs Description Skew Slope fasteners Job Path 6 LT2-151188 4 10dx1 1/2 2 10dx1 1/2 S:\CUSTOMERS\GREENPARK H4 11 LT251188 4 10dx1 1/2 2 10dx1 1/2 MINNISALE HOMES MODELS MILLWOOD 2/FLOORS/ELEV 2 1 HUCQ1.81/9-SDS WILLWOOD 2-ELEV 2.1st Ground Floor Width Depth Qty Plies Pcs Length Design Method 2.5 11.875 LinFt Building Code NBCC 2010 / OBC 2012 NOTES: Floor Loads Framer to verify dimensions on the architectural drawings.
 Double joist only require filter/backer ply when supporting another member using a face-mounted hanger. Live 40 Dead 15 Install 2x4 blocking @ 24*o/c under parallel non-load bearing walls.
 Install single-ply flush window header along inside face of Deflection Joist LL Span L/ 480 rimboard/rimjoist.

5. Refer to Nascor spedfler guide for installation works.

6. Squash blocks recommended to be installed at end bearing on TL Span L/ 360 LL Cant 2L/ 480 all first level joists which support loading from above exceeding two levels floor or roof. Load transfer blocks to be installed under all point loads. TL Cant 2L/ 360 Deflection Girder It shall be the frame's responsibility that floor joiets and beams are fastened as per the hanger manufacturer's standards. LL Span L/ 360 TL Span L/ 240 480 LL Cant 2L/ Refer to Multiple Member Connection Detail to ply to ply nailing or TL Cant 2L/ Decking Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than SPF Plywood Deck rim depth @ 18"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of component Thickness Fastener Nailed & Glued racing for lateral stability are the responsibility of Others. Vibration Hatch are represents ceramic tiled floor with an additional dead load The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction. ARCHITECTURAL DRAWINGS:

- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CANCSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

0 111111

Point Load Support Load from Above

Wall Opening Norbord Rimboard Plus 1 125 X 11.875 NJ 11.875 Forex 2.0E-3000Fb LVL 1.75 X 11.875

Version 18.40 162 Powered by iStruct*

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them



GREENPARK Client:

Project:

Address:

9/7/2018 Date: Designer:

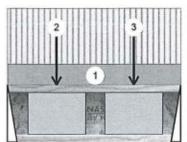
RO

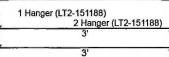
Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor





15 PSF



Wind

0

0

Page 1 of 1

Member Into	rmation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		

Bearings and Factored Reactions

Live

282

287

Brg

1 2

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	20%	132 / 423	555	L	1.25D+1.5L			
2 - Hanger	2.000"	21%	135 / 431	566	L	1.25D+1.5L			

Snow

0

0

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

106

108

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	401 ft-lb	1'4 1/2"	9020 ft-lb	0.044 (4%)	1.25D+1.5L	L
Unbraced	401 ft-lb	1'4 1/2"	5749 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	558 lb	2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/38142)	1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/14284)	1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.003 (L/10392)	1'5 9/16"	0.140 (L/240)	0.020 (2%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings

o bolloili liange	Diaced at Dearings.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF
2	Point	0-10-4		Far Face	87 lb	233 lb	0 lb
3	Point	2-2-4		Far Face	86 lb	229 lb	0 lb

Wind Comments 0 PSF Pass-Thru Framing Squash Block is required aball point loads over bearings

REPROFESSIONAL CAROLES

100083566

OUNCE OF ONT

September 13, 2018

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Subcursed Designs as responsable on the design criteria and loadings shown. It is the responsibility of the customer and/or the confractor to ensure the component suitability of the intended sections and loading and the property of the confractor to ensure the component suitability of the intended sections and loads.

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire relardant or corrosive

chemicals

Handling & Installation

landling & Installation.

Loist flanges must not be cut or drilled.
Refer to latest copy of the Loist product information details for framing details, stiffener tables, with hole chart, bridging details, must hole chart, bridging details, must holy framening details end handling/erection details.

Demaged Loist must not be used.

Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roots ponding

This design

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario





Client:

GREENPARK

Project:

Address:

9/7/2018 Date: Designer:

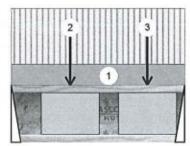
RO

Job Name: MILLWOOD 2-ELEV 1

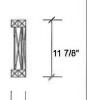
Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor



1 Hanger (LT2-151188) 2 Hanger (LT2-151188) 3'



Wind

0

0

Page 1 of 1

Member Intorn	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF	1	
Dead:	15 PSF	1	

Bearings	and	Factored	Reactions

Live

343

404

Brg

1 2

Unfactored Reactions UNPATTERNED lb (Uplift)

Dead

129

152

Snow

0

0

Cap. React D/L lb Ld. Comb. Bearing Length Total Ld. Case 2.000" 25% 161/514 675 1 1.25D+1.5L Hanger 2.000" 29% 189 / 606 795 L 1.25D+1.5L Hanger

Wind

0 PSF

0 lb J7

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	583 ft-lb	1' 1/4"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L
	Unbraced	583 ft-lb	1' 1/4"	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L
	Shear	788 lb	2'10 3/4"	3400 lb	0.232 (23%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/27610)	1'1 5/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.003 (L/10370)	1'1 5/16"	0.093 (L/360)	0.030 (3%)	L	L
	TL Defl inch	0.004 (L/7538)	1'1 5/16"	0.140 (L/240)	0.030 (3%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6

Bottom	Bottom flange braced at bearings.									
)	Load Type	Location	Trib Width	Side	Dead	Live	Snow			
	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF			
	Point	1-0-4		Far Face	130 lb	346 lb	0 lb			
	Point	2-4-4		Far Face	110 lb	293 lb	0 lb	P		
								re		



Pass Thru Framing Squash Block is required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

ID

1 2

3

arructured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loade

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

JOINING & Installiation

Joist flanges must not be cut or drilled

Refer to latest copy of the Libist product information
detals for framing detals, siftners tables, web hole
chart, bridging details, muti-ply fastening details and
handling/erection details

Damaged Libits must not be used

Deston assumes then finger to be laberaby restrained.

This design

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs
 PRAD ALL MODES ON T

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario 905-642-4400





Client:

Project: Address: **GREENPARK**

Date: 9/7/2018

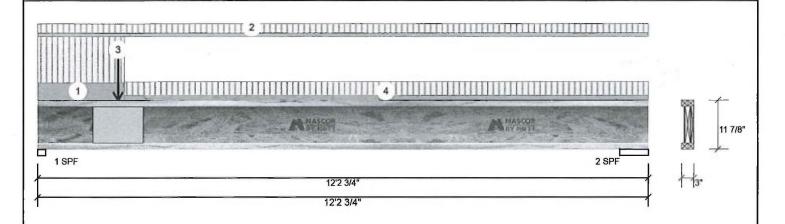
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #

2-Ply - PASSED 11.875"

Level: Ground Floor



∕lember Infor	mation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	512	192	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	243	91	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked	•				
General Load								
Floor Live:	40 PSF			Bearings a	nd Fact	ored Reactions		
Dead:	15 PSF			Bearing Le	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - SPF 1.	875"	38% 240 / 767	1008 L	1.25D+1.5L
				2-SPF 6.	875"	14% 114 / 365	479 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1602 ft-lb	4'6 5/8"	9020 ft-lb	0.178 (18%)	1.25D+1.5L	L
Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L
Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L
Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform
LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L
TL Defl inch	0.065 (L/2150)	5'6 5/16"	0.581 (L/240)	0.110 (11%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'7" o.c.



5 Bottom liange	braced at bearings.				7				
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-12	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	108 lb	287 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
								Pass-Th	ru Framing Squash Block is

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Celculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suifability of the Intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

amuling & installation

Lioist flanges must not be cut or drilled

Refer to latest copy of the Lioist product information
details for framing details, suffiner tables, web hole
chart, bridging details, multi-ply fastening details and
handlingferection details

Damaged Lioists must not be used
Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 Inches

This design is

READ ALL NOTES ON THIS PAGE AND ON THE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1





Client:

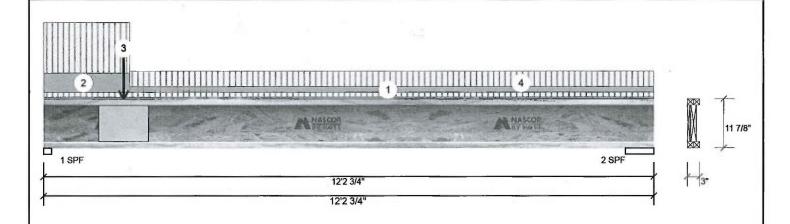
Project: Address: GREENPARK

Date: 9/7/2018 RO Designer:

Job Name: MILLWOOD 2-ELEV 1

2-Ply - PASSED F11-B NJ 11.875"

Level: Ground Floor



Member Inforn	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	471		177		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	206		77		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored R	eactions			
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	35%	221 / 707	928	L	1.25D+1.5L
				2-SPF	6.875"	12%	96 / 308	405	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.041 (L/3379)	5'5 11/16"	0.388 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.057 (L/2457)	5'5 11/16"	0.581 (L/240)	0.100 (10%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'11" o.c.
- 5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	106 lb	282 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Lumber

Handling & Installation

Lioist fianges must not be cut or drifted
Refer to latest copy of the !Joist product information
details for framing details, stiffner tables, web hole
chart, bridging details, multi-ply fastening details and
handlingferection details
Damaged !Joists must not be used
Design assumes top fiange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 Inches

This design is

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400

T.L. WISE

100083566

September 13, 2018



Manufacturer Info

Nascor by Kott



Client: **GREENPARK**

Project: Address: Date: 9/7/2018 Designer:

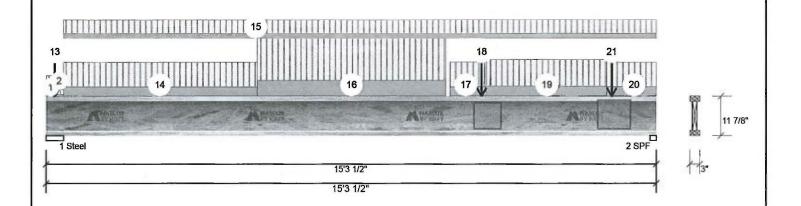
RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED 11.875"

Level: Ground Floor



ation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
2	Design Method:	LSD	1	641		269		0	0
Dry	Building Code:	NBCC 2010 / OBC 2012	2	539		202		0	0
360	Load Sharing:	No							
240	Deck:	Not Checked							
Normal	Vibration:	Not Checked							
40 PSF			Bearings	and Fac	tored	Reactions			
15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
			1 - Steel	5.250"	38%	337 / 961	1298	L	1.25D+1.5L
			2-SPF	1.875"	40%	253 / 809	1061	L	1.25D+1.5L
	240 Normal 40 PSF	Girder Application: Design Method: Building Code: Load Sharing: Deck: Normal Vibration: 40 PSF 15 PSF	Girder Application: Floor (Residential) Design Method: LSD Dry Building Code: NBCC 2010 / OBC 2012 Load Sharing: No Deck: Not Checked Vibration: Not Checked 40 PSF 15 PSF	Girder Application: Floor (Residential) 2 Design Method: LSD 1 Dry Building Code: NBCC 2010 / OBC 2012 360 Load Sharing: No 240 Deck: Not Checked Normal Vibration: Not Checked 40 PSF Bearings 1 - Steel 2 - SPF	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live	Application: Floor (Residential) Brg Live Dead	Application: Floor (Residential) Brg Live Dead Snoon	Application: Floor (Residential) Brg Live Dead Snow

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3724 ft-lb	8' 3/16"	9020 ft-lb	0.413 (41%)	1.25D+1.5L	L
Unbraced	3724 ft-lb	8' 3/16"	3737 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1052 lb	15'2 3/8*	3400 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.062 (L/2891)	7'10 3/8"	0.494 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.164 (L/1083)	7'10 3/8"	0.494 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.226 (L/788)	7'10 3/8"	0.741 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'9" o.c.

:	Bottom liange	braced at bearings.								
- 11	D	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Ē.	Tie-In	0-0-0 to 0-5-4	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	2	Tie-In	0-0-0 to 0-5-4	(Span)0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
4	ı	Point	0-2-10		Тор	1 lb	4 lb	0 lb	0 lb	J7
5	5	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
6	3	Point	0-2-10		Тор	1 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	•	Point	0-2-10		Тор	21 lb	56 lb	0 lb	0 lb	J7

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
 Usoist not to be treated with fire retardant or corrosive
- Handling & Installation
- and ling & installation

 Libels flages must not be cut or drilled

 Refer to latest copy of the Libels product information
 detells for framing details, stiffener tables, web hole
 chart, bridging details, mutil-by fastering details and
 handfling/serection details

 Demaged Libels must not be used

 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs provid

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is

Manufacturer info

Nascor by Kott



Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1



Page 2 of 2



Client:

GREENPARK

Project: Address:

9/7/2018 Date:

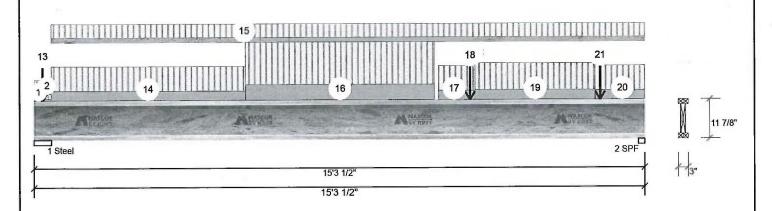
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor



om page 1								
Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
Point	0-2-10		Тор	22 lb	59 lb	0 lb	0 lb	J7
Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J7
Point	0-2-10		Тор	20 lb	0 lb	0 lb	0 lb	Wall Self Weight
Point	0-2-10		Тор	9 lb	25 lb	0 lb	0 lb	J7
Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J7
Point	0-2-10		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
Tie-In	0-5-4 to 5-3-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	0-5-4 to 15-3-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	5-3-10 to 10-0-8	(Span)2-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	10-1-10 to 11-0-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	10-11-2		Far Face	18 lb	49 lb	0 lb	0 lb	F9
Tie-In	11-0-10 to 14-0-10	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Tie-In	14-0-10 to 15-3-8	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
Point	14-2-2		Far Face	25 lb	66 lb	0 lb	0 lb	F9
	Load Type Point Point Point Point Point Point Tie-In Tie-In Tie-In Tie-In Tie-In Tie-In Tie-In	Load Type Location Point 0-2-10 Point 0-2-10 Point 0-2-10 Point 0-2-10 Point 0-2-10 Point 0-2-10 Tie-In 0-5-4 to 5-3-10 Tie-In 0-5-4 to 15-3-8 Tie-In 5-3-10 to 10-0-8 Tie-In 10-1-10 to 11-0-10 Point 10-11-2 Tie-In 11-0-10 to 14-0-10 Tie-In 11-0-10 to 15-3-8	Load Type Location Trib Width Point 0-2-10 Point Tie-In 0-5-4 to 5-3-10 (Span)1-7-15 Tie-In 0-5-4 to 15-3-8 (Span) 0-10-12 (Span)2-9-15 Tie-In 10-1-10 to 11-0-10 (Span)1-7-15 Point 10-11-2 (Span)1-7-15 Tie-In 11-0-10 to 14-0-10 (Span)1-9-8 Tie-In 14-0-10 to 15-3-8 (Span)1-7-15	Load Type Location Trib Width Side Point 0-2-10 Top Tie-In 0-5-4 to 5-3-10 (Span)1-7-15 Top Tie-In 0-5-4 to 15-3-8 (Span) Top Tie-In 5-3-10 to 10-0-8 (Span)2-9-15 Top Tie-In 10-1-10 to 11-0-10 (Span)1-7-15 Top Point 10-11-2 Far Face Tie-In 11-0-10 to 14-0-10 (Span)1-9-8 Top Tie-In 14-0-10 to 15-3-8 (Span)1-7-15 Top	Load Type Location Trib Width Side Dead Point 0-2-10 Top 22 lb Point 0-2-10 Top 2 lb Point 0-2-10 Top 20 lb Point 0-2-10 Top 9 lb Point 0-2-10 Top 10 lb Point 0-2-10 Top 9 lb Tie-In 0-5-4 to 5-3-10 (Span)1-7-15 Top 15 PSF Tie-In 0-5-4 to 15-3-8 (Span) Top 15 PSF Tie-In 5-3-10 to 10-0-8 (Span)2-9-15 Top 15 PSF Tie-In 10-1-10 to 11-0-10 (Span)1-7-15 Top 15 PSF Point 10-11-2 Far Face 18 lb Tie-In 11-0-10 to 14-0-10 (Span)1-9-8 Top 15 PSF Tie-In 14-0-10 to 15-3-8 (Span)1-7-15 Top 15 PSF	Load Type Location Trib Width Side Dead Live Point 0-2-10 Top 22 lb 59 lb Point 0-2-10 Top 2 lb 5 lb Point 0-2-10 Top 20 lb 0 lb Point 0-2-10 Top 9 lb 25 lb Point 0-2-10 Top 9 lb 0 lb Point 0-2-10 Top 9 lb 0 lb Tie-In 0-5-4 to 5-3-10 (Span)1-7-15 Top 15 PSF 40 PSF Tie-In 0-5-4 to 15-3-8 (Span) Top 15 PSF 40 PSF Tie-In 5-3-10 to 10-0-8 (Span)2-9-15 Top 15 PSF 40 PSF Tie-In 10-1-10 to 11-0-10 (Span)1-7-15 Top 15 PSF 40 PSF Point 10-11-2 Far Face 18 lb 49 lb Tie-In 11-0-10 to 14-0-10 (Span)1-7-15 Top 15 PSF 40 PSF Tie-In 14-0-10 to 15-3-8	Load Type Location Trib Width Side Dead Live Snow Point 0-2-10 Top 22 lb 59 lb 0 lb Point 0-2-10 Top 2 lb 5 lb 0 lb Point 0-2-10 Top 20 lb 0 lb 0 lb Point 0-2-10 Top 9 lb 25 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb Point 0-2-10 Top 9 lb 0 lb 0 lb Tie-In 0-5-4 to 5-3-10 (Span)1-7-15 Top 15 PSF 40 PSF 0 PSF Tie-In 5-3-10 to 10-0-8 (Span)2-9-15 Top 15 PSF 40 PSF 0 PSF Tie-In 10-1-10 to 11-0-10 (Span)1-7-15 Top 15 PSF 40 PSF 0 PSF Poi	Load Type Location Trib Width Side Dead Live Snow Wind Point 0-2-10 Top 22 lb 59 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 20 lb 5 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 9 lb 25 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb 0 lb 0 lb Point 0-2-10 Top 10 lb 26 lb 0 lb 0 lb 0 lb Tie-In 0-5-4 to 15-3-8 (Span)1-7-15 Top 15 PSF 40 PSF 0 PSF 0 PSF Tie-In 5-3-10 to 10-0-8 (Span)2-9-15 Top 15 PSF 40 PSF 0 PSF 0 PSF

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimenalons and loads.

Lumber

Handling & Installation

Handling & Installation

1. Joist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information details for framing details, stifferer tables, web hot chart, btdging details, multi-py fastering details and handling/erection details

3. Damaged boiles must not be used

4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 To for flat roofs provide proper drainage to prevent ponding.

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





GREENPARK Client:

Project:

Address:

9/7/2018 Date:

RO Designer:

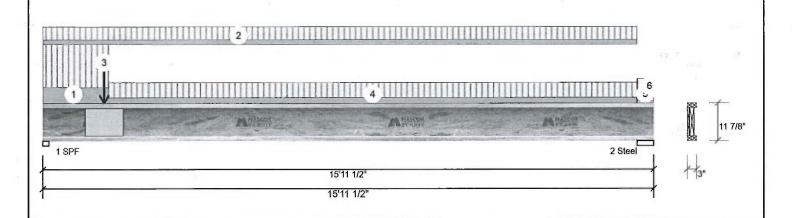
Job Name: MILLWOOD 2-ELEV 1

Unfactored Reactions UNPATTERNED Ib (Uplift)

Project #:

NJ 2-Ply - PASSED F12-B 11.875"

Level: Ground Floor



Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	712	267	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	376	141	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked	1				
Importance:	Normal	Vibration:	Not Checked					
General Load					_			
Floor Live:	40 PSF			Bearings a	and Facto	red Reactions		
Dead:	15 PSF		0.0	Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
		_		1-SPF 1	.875"	53% 334 / 1068	1402 L	1.25D+1.5L
			·	2 - Steel 5	.250"	22% 176 / 563	739 L	1.25D+1.5L

Analysis Results

Member Information

Actual	Location	Allowed	Capacity	Comb.	Case
3112 ft-lb	6'11 1/2"	9020 ft-lb	0.345 (34%)	1.25D+1.5L	L
3112 ft-lb	6'11 1/2"	3135 ft-lb	0.993 (99%)	1.25D+1.5L	L
1386 lb	1 1/8"	3400 lb	0.408 (41%)	1.25D+1.5L	L
0.057 (L/3233)	7'6 13/16"	0.516 (L/360)	0.110 (11%)	D	Uniform
0.153 (L/1213)	7'6 13/16"	0.516 (L/360)	0.300 (30%)	L	L
0.211 (L/882)	7'6 13/16"	0.774 (L/240)	0.270 (27%)	D+L	L
	3112 ft-lb 3112 ft-lb 1386 lb 0.057 (L/3233) 0.153 (L/1213)	8112 ft-lb 6'11 1/2" 8112 ft-lb 6'11 1/2" 1386 lb 1 1/8" 0.057 (L/3233) 7'6 13/16" 0.153 (L/1213) 7'6 13/16"	3112 ft-lb 6'11 1/2" 9020 ft-lb 3112 ft-lb 6'11 1/2" 3135 ft-lb 1386 lb 1 1/8" 3400 lb 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.153 (L/1213) 7'6 13/16" 0.516 (L/360)	3112 ft-lb 6'11 1/2" 9020 ft-lb 0.345 (34%) 3112 ft-lb 6'11 1/2" 3135 ft-lb 0.993 (99%) 1386 lb 1 1/8" 3400 lb 0.408 (41%) 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.300 (30%) 0.153 (L/1213) 7'6 13/16" 0.516 (L/360) 0.300 (30%)	3112 ft-lb 6'11 1/2" 9020 ft-lb 0.345 (34%) 1.25D+1.5L 3112 ft-lb 6'11 1/2" 3135 ft-lb 0.993 (99%) 1.25D+1.5L 3386 lb 1 1/8" 3400 lb 0.408 (41%) 1.25D+1.5L 0.057 (L/3233) 7'6 13/16" 0.516 (L/360) 0.110 (11%) D 0.153 (L/1213) 7'6 13/16" 0.516 (L/360) 0.300 (30%) L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'2" o.c.

5 Bottom flange braced at bearings.



O Dolloin	nunge bracea at bearin	90.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-6-4	(Span) 0-11-12 to 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	129 lb	343 lb	0 lb		F10 u Framing Squash Block is
4	Tie-In	1-8-14 to 15-6-4	(Span)1-1-12 to 1-1-12	Тор	15 PSF	40 PSF	0 PSF	required a	at all point loads over bearings
5	Tie-In	15-6-4 to 15-11-8	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	Refersto M	Multiple Member Connection
6	Tie-In	15-6-4 to 15-11-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	Details for requirement	ply to ply nailing or bolting

Notes

Calculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Usolst not to be treated with fire retardant or corrosive

Handling & Installation

- and ling & Installation.

 Joist flanges must not be cut or drilled.
 Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, muthiply fastening details and handling/erection details.

 Damaged IJoist must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

This design is

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Manufacturer Info Nascor by Kott Canada K2H7V1 905-642-4400





GREENPARK Client:

Project:

Address:

9/7/2018 Date:

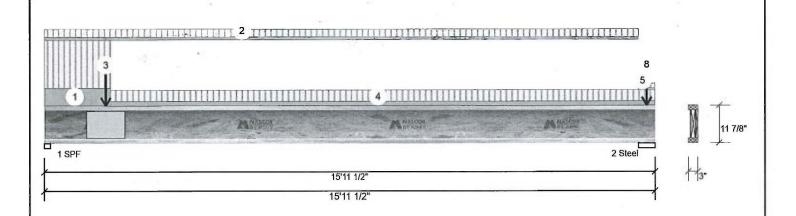
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Unfactored Reactions UNPATTERNED lb (Uplift)

Project #:

11.875" 2-Ply - PASSED Level: Ground Floor



Girder Floor (Residential) Wind Type: Application: Brg Dead Snow Design Method: Plies: 649 244 0 0 Moisture Condition: Dry **Building Code:** NBCC 2010 / OBC 2012 0 0 417 179 2 Deflection LL: 360 Load Sharing: Deflection TL: 240 Deck: Not Checked Importance: Normal Vibration: Not Checked General Load **Bearings and Factored Reactions** 40 PSF Floor Live: 15 PSF Cap. React D/L lb Dead: Bearing Length Total Ld. Case Ld. Comb. 305 / 973 1.25D+1.5L 1 - SPF 1.875" 48% 1278 L 1.25D+1.5L 2 - Steel 5.250" 25% 223 / 625 849 L

Analysis Results

Member Information

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L
	Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L
	Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L
	Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform
	LL Defl inch	0.112 (L/1664)	7'4 3/4"	0.516 (L/360)	0.220 (22%)	L	L
	TL Defl inch	0.154 (L/1210)	7'4 3/4"	0.774 (L/240)	0.200 (20%)	D+L	L
_						_	

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'10" o.c.

O DOLLOTT	flange braced at bearing								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-6-4	(Span)0-6-4 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	152 lb	404 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 15-11-8	(Span)0-9-4 to 0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	15-6-4 to 15-11-8	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	15-8-14		Тор	32 lb	85 lb	0 lb	0 lb	J7

Continued on page 2...

Calculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Use to be treated with fire retardant or corrosive

Handling & Installation

- iandling & Installation

 I Joist Banges must not be cut or drilled

 Refer to latest copy of the IJoist product Information
 detals for framing details, stiffener tables, web hole
 chart, bridging details, multi-pyt fastening details end
 handling/erection details

 Damaged IJoist must not be used

 Design assumes top Bange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

This design is

5. Provide lateral support at bearing points to avoid lateral displacement and rotation.

6. Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches

7. For flat roots proponding READ ALL NOTES ON

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

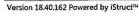
Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1 905-642-4400

T.L. WISE

100083566

September 13, 2018







TW0918-058 Client: **GREENPARK** Date: 9/7/2018 Page 2 of 2 Project: Designer: RO isDesign™ Address: Job Name: MILLWOOD 2-ELEV 1 Project #: Level: Ground Floor 2-Ply - PASSED NJ 11.875" 3 8 5 1 11 7/8" 1 SPF 2 Steel 15'11 1/2" 15'11 1/2" .Continued from page 1 ID Load Type Location Trib Width Side Dead Live Wind Comments 0 lb 0 lb 7 Point 15-8-14 30 lb 80 lb Top J7

22 lb

Top

0 lb

0 lb

Pass-Thru Framing Squash Block is required at all point loads over bearings

Wall Self Weight

0 lb

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

8

Point

Caculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and leadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Holst not to be treated with fire retardant or corrosive

15-8-14

- chemicals
 Handling & Installation

 1. Dots flanges must not be cut or drifled

 2. Refer to latest copy or the Lioist product information
 detals for framing detaits, sufferer tables, web hole
 chart, bridging detaits, multi-ply fastening detaits and
 handling-rection detaits

 3. Damaged Units must not be used

 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For that roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Onlario Canada K2H7V1 905-642-4400



Project:

Address:

Date: 9/10/2018 Designer: RO

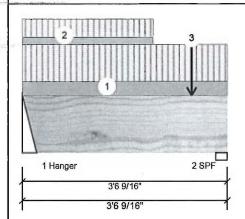
Job Name: MILLWOOD 2-ELEV 1

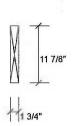
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Inf	ormation				Unfacto	red Reac	tions U	NPATTERN	ED lb (Uplift)			
Туре:	Girder		Applicatio	n: Fl	oor (Resident	ial)	Brg	Live		Dead	Sno	N	Wind
Plies:	1		Design M	ethod: LS	BD		1	555		218		0	0
Moisture Cond	ition: Dry		Building C	Code: NE	BCC 2010 / O	BC 2012	2	510	Ū.	201		0	0
Deflection LL:	360		Load Sha	ring: No)								
Deflection TL:	240		Deck:	No	ot Checked								
Importance:	Normal		Vibration:	No	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - Hanger	3.000"	28%	272 / 833	1106	L	1.25D+1.5L
Analysis Re	sults						2-SPF	2.375"	40%	251 / 765	1016	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case						_	
Moment	785 ft-lb	1'9 5/16"	17130 ft-lb	0.046 (5%)	1.25D+1.5L	L					/	EESSID	
Unbraced	785 ft-lb	1'9 5/16"	13259 ft-lb	0.059 (6%)	1.25D+1.5L	L					ORC	10101	ALA
Shear	745 lb	2'5 1/16"	5798 lb	0.129 (13%)	1.25D+1.5L	L				/	8/		10
Perm Defl in.	0.001 (L/26669)	1'9 7/16"	0.108 (L/360)	0.010 (1%)	D	Uniform				1	3	I MIS	E CHAINE

Design Notes

LL Defl inch

1 Fill all hanger nailing holes.

0.004

(L/10472) TL Defl inch 0.005 (L/7520)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings



September 13, 2018

1 Dottoill	bidood at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part, Uniform	0-0-0 to 3-6-9		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 2-3-3		Near Face	39 PLF	103 PLF	0 PLF	0 PLF	
3	Point	2-11-3		Near Face	33 lb	87 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				ru Framing Squash Block is

L

required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

andiling & Installation
LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-pit
fastening details, beam strength values, and code
approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

1'9 3/8" 0.108 (L/360) 0.030 (3%) L

1'9 3/8" 0.161 (L/240) 0.030 (3%) D+L

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





Address:

Project:

Date:

9/7/2018 Designer: RO

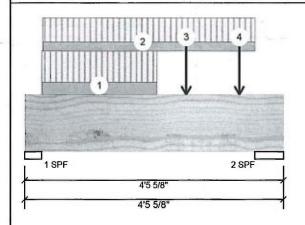
Job Name: MILLWOOD 2-ELEV 1

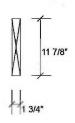
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Inforn	nation			Unfactored Reactions UNPATTERNED Ib (Uplift)							
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind	
Plies:	1	Design Method:	LSD	1	1024		395		0	0	
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	1099		424		0	0	
Deflection LL:	360	Load Sharing:	No	1							
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearings	s and Fac	tored	Reactions				
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	3.500"	54%	493 / 1536	2029	L	1.25D+1.5L	
				2-SPF	5.875"	34%	529 / 1649	2178	L	1.25D+1.5L	

Analysis Results

_							
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	2002 ft-lb	2'1 9/16"	17130 ft-lb	0.117 (12%)	1.25D+1.5L	L
	Unbraced	2002 ft-lb	2'1 9/16"	11720 ft-lb	0.171 (17%)	1.25D+1.5L	L
	Shear	2314 lb	3' 5/8"	5798 lb	0.399 (40%)	1.25D+1.5L	L
	Perm Defl in.	0.004 (L/10769)	2'1 11/16"	0.127 (L/360)	0.030 (3%)	D	Uniform
	LL Defl inch	0.011 (L/4138)	2'1 11/16"	0.127 (L/360)	0.090 (9%)	L	L
	TL Defl inch	0.015 (L/2989)	2'1 11/16"	0.191 (L/240)	0.080 (8%)	D+L	L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings



September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-8 to 2-3-8		Near Face	120 PLF	319 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-3-12 to 3-11-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Point	2-9-8		Near Face	115 lb	305 lb	0 lb	0 lb	J7
4	Point	3-8-8		Near Face	112 lb	300 lb	0 lb	0 lb	J7
	Self Weight				5 PLF			10.0000000	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Lumber

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code.
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrek
 Provide lateral support at bearing points
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED**

IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design is



GREENPARK Client:

Project:

Address:

9/7/2018 Date: Designer: RO

Job Name: MILLWOOD 2-ELEV 1

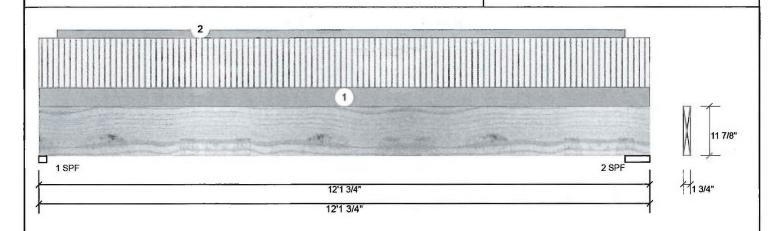
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor

Unfactored Reactions UNPATTERNED Ib (Uplift)



Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	73		67		0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	77		70		0	0
Deflection LL:	360	Load Sharing:	No	1000						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored F	Reactions			3.1
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	10%	83 / 109	192	L	1.25D+1.5L
				2-SPF	5.875"	3%	88 / 115	203	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	553 ft-lb	5'10 7/8"	17130 ft-lb	0.032 (3%)	1.25D+1.5L	L
Unbraced	553 ft-lb	5'10 7/8"	3868 ft-lb	0.143 (14%)	1.25D+1.5L	L
Shear	158 lb	1'1"	5798 lb	0.027 (3%)	1.25D+1.5L	L
Perm Defl in.	0.011 (L/13107)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.012 (L/12101)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.022 (L/6292)	5'10 7/8"	0.581 (L/240)	0.040 (4%)	D+L	L
						-

T.L. WISE 100083566 100083566 WCE OF ONTE September 13, 2018

Design Notes

1 Girders are designed to be supported on the bottom edge only.

2 Top braced at bearings.

3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-1-12	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-6 to 11-7-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

Afforming or installation.

LVL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

tastering details, beam strenger values, and approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Onterio Canada K2H7V1 905-642-4400







Client: GREENPARK

Project: Address:

ject:

Date: 9/7/2018
Designer: R O

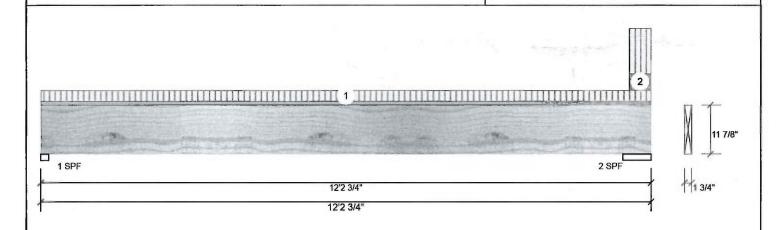
Job Name: MILLWOOD 2-ELEV 1

Project #:

F5-B Forex 2.0E-3000Fb LVL 1.1

1.750" X 11.875" - PASSED

Level: Ground Floor



Member Inforn	nation		- Page -	Unfactored Reactions UNPATTERNED Ib (Uplift)						
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	N	Wind
Plies:	1	Design Method:	LSD	1	67		53		0	0
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012	2	95		66		0	0
Deflection LL:	360	Load Sharing:	No	_						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load		1.000000011001								
Floor Live:	40 PSF			Bearings	and Fac	tored	Reactions			
Dead:	15 PSF	31		Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	1.875"	8%	67 / 101	168	L	1.25D+1.5L
				2-SPF	6.875"	3%	82 / 142	224	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	480 ft-lb	5'10 7/8"	17130 ft-lb	0.028 (3%)	1.25D+1.5L	L
Unbraced	480 ft-lb	5'10 7/8"	3868 ft-lb	0.124 (12%)	1.25D+1.5L	L
Shear	137 lb	1'1"	5798 lb	0.024 (2%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/16496)	5'10 7/8"	0.388 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.011 (L/13065)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.019 (L/7290)	5'10 7/8"	0.581 (L/240)	0.030 (3%)	D+L	L

T.L. WISE 100083566

September 13, 2018

2	eargii	IAOTES	
	4 Cirdo	re are deciar	and to be

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-6-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	11-9-6 to 12-2-12	(Span)2-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural edequecy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the confractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVI beams must not be cut or drilled
 Refer to manufacturer's product information regurding installation requirements, multi-pleastening details, beam strength values, and cod-
- approvals
 3. Damaged Beams must not be used
 4. Design assumes top edge is laterally
- Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





GREENPARK Client:

Project:

Address:

9/7/2018 Date:

Designer: RO

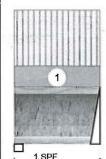
Job Name: MILLWOOD 2-ELEV 1

Project #:

NJ 11.875"

2-Ply - PASSED

Level: Ground Floor



11 7/8"

21	1 SPF anger (LT2-151188)	
1	1'5 7/8"	7
1	1'5 7/8"	7
		_

Member Info	rmation						Unfacto	red Reac	tions U	INPATTERN	ED lb (U	plift)		
Туре:	Girder		Applicatio	n: F	loor (Resident	ial)	Brg	Live		Dead	Snow		Wind	-
Plies:	2		Design M	ethod: L	SD		1	48		18	0		0	
Moisture Conditi	ion: Dry		Building C	Code: N	BCC 2010 / O	BC 2012	2	49	iš.	18	0		0	
Deflection LL:	360		Load Sha	ring: N	io									
Deflection TL:	240		Deck:	N	ot Checked									
Importance:	Normal		Vibration:	N	ot Checked									
General Load														_
Floor Live:	40 PSF						Bearing	s and Fac	tored	Reactions				
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total L	.d. Case	Ld. Comb.	
							1 - SPF	1.875"	4%	23/72	95 L	_	1.25D+1.5L	
Analysis Resu	ults						2 - Hanger	2.000"	4%	23 / 73	96 L	-	1.25D+1.5L	
Analysis A	Actual	Location	Allowed	Capacity	Comb.	Case	C 1 C 1		_		-			
Moment 2	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L	L					101	ESSIO	V	
Unbraced 2	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L	L	1			100	PRO	-	TO V	
Shear 8	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L	L				/	Eg T.		WALCHONEER SEE	
Perm Defl in. 0	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						11	3		m l	
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						13] T.	L. WIS	E M	
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						1	10	00835	90	



1 Fill all hanger nailing holes.

TL Defl inch 0.000 (L/999)

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.

- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended anotication, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation.

 Note langes must not be cut or drilled.
 Refer to latest copy of the Loist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-by frastening details and handling/erection details.

 Damaged Loists must not be used.
 Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. Web stiffeners for point load as shown Minimum point load bearing leaghth 3.5 inches
7. For that roofs provide READ ALL NOTES On

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

WCE OF ONTAR

September 13, 2018







GREENPARK Client:

Project:

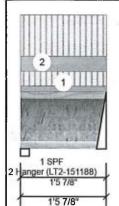
Address:

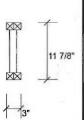
9/7/2018 Date: Designer: RO

> Job Name: MILLWOOD 2-ELEV 1

Project #:

NJ 11.875" 2-Ply - PASSED Level: Ground Floor





Wind 0 0

Ld. Comb. 1.25D+1.5L 1.25D+1.5L

Page 1 of 1

Member Info	ormation						Unfacto	red React	ions U	NPATTERN	ED lb	(Uplift)
Type:	Girder		Application	on: F	loor (Residenti	al)	Brg	Live		Dead	Sno	w
Plies:	2		Design M	iethod: L	SD		1	65		24		0
Moisture Condi	tion: Dry		Building (Code: N	IBCC 2010 / O	BC 2012	2	66		25		0
Deflection LL:	360		Load Sha	ring: N	lo							
Deflection TL:	240		Deck:	N	lot Checked							
Importance:	Normal		Vibration:	N	lot Checked		1					
General Load												
Floor Live:	40 PSF						Bearing:	s and Fac	tored F	Reactions		
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
							1 - SPF	1.875"	5%	31 / 98	128	L
Analysis Res	ults						2 - Hanger	2.000"	5%	31 / 99	130	L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case						
Moment	36 ft-lb	8 7/8"	9020 ft-lb	0.004 (0%)	1.25D+1.5L	L	1				/	FESSIO
Unbraced	36 ft-lb	8 7/8"	8539 ft-lb	0.004 (0%)	1.25D+1.5L	L				-	/ PR	FESSIO

Design Notes

1 Fill all hanger nailing holes.

Perm Defl in. 0.000 (L/999)

LL Defl inch 0.000 (L/999)

TL Defl inch 0.000 (L/999)

2 Girders are designed to be supported on the bottom edge only.

3 Multiple plies must be fastened together as per manufacturer's details.

1 1/8" 3400 lb

0 999.000 (L/0) 0.000 (0%)

0 999.000 (L/0) 0.000 (0%)

0 999.000 (L/0) 0.000 (0%)

4 Top loads must be supported equally by all plies.

5 Top flange braced at bearings.

6 Bottom flange braced at bearings.

112 lb

SSIONAL ENGINEER T.L. WISE 100083566 WCE OF ONTAR

September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

0.033 (3%) 1.25D+1.5L L

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 Unist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

tandling & Installation

I. Joist flanges must not be cut or drilled

Refer to latest copy of the Joist product information
detals for framing details, stiffener tables, web hole
bank, bridging details, multiply fastening details end
handling/erection details

Damaged Joists must not be used

Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rolation
 Web stiffeners for point load as shown Minhmum point load bearing leading 3.5 inches
 For flat roots provided to the point of the

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

This design is

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







GREENPARK Client:

Project:

Address:

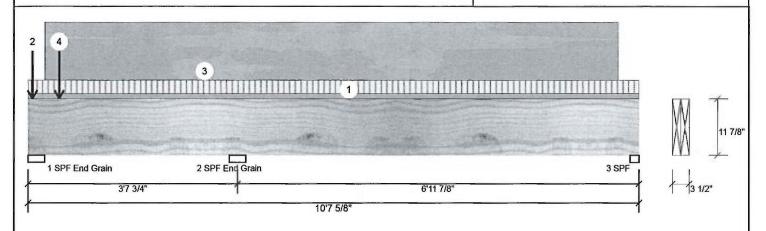
Date: 9/10/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED Level: Ground Floor 1.750" X 11.875" Forex 2.0E-3000Fb LVL



Member Info	rmation			Unfacto	red Reac	tions U	NPATTERN	ED lb ((Uplift)
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w
Plies:	2	Design Method:	LSD	1	986		484		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	176		679		0
Deflection LL:	360	Load Sharing:	No	3	52		259		0
Deflection TL:	240	Deck:	Not Checked	"					
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearing	s and Fac	tored	Reactions		
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case
				1 - SPF	3.500"	24%	578 / 1504	2082	L_

Thury 313 Ite.	, aires					
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Unbraced	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Pos Moment	521 ft-lb	7'9"	22269 ft-lb	0.023 (2%)	1.4D	Uniform
Unbraced	521 ft-lb	7'9"	21873 ft-lb	0.024 (2%)	1.4D	Uniform
Shear	421 lb	4'7 5/8"	7537 lb	0.056 (6%)	1.4D	Uniform
Perm Defl in.	0.005 (L/17692)	7'3 7/16"	0.230 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.000 (1./999)	0	999 000 (1/0)	0.000 (0%)		

TL Defl inch 0.006 (L/14724) Design Notes

Analysis Results

1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.

7'3 3/8" 0.345 (L/240) 0.020 (2%) D+L

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

1	986	484	0	0
2	176	679	0	0
3	52	259	0	0

searning:	s and rac	toreu	keactions			
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	24%	578 / 1504	2082	L_	1.25D+1.5L
2 - SPF End Grain	3.500"	17%	995 / 0	995	Uniform	1.4D
3-SPF	1.875"	13%	348/0	348	Uniform	1.4D



Wind

September 13, 2018

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, it is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended contractor to ensure the component suitability of the intended

Lumber

Dry service conditions, unless noted otherwise
 LVI, not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or or Refer to manufacturer's regarding installation requ
- rastering details, beam strength values approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally rest
 Provide lateral support at bearing poir lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

L

Manufacturer Info

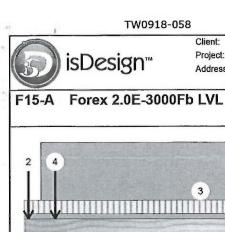
Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE EMP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Onlario Canada K2H7V1 905-642-4400



Page 2 of 2



Client:

Project: Address: GREENPARK

Date: 9/10/2018

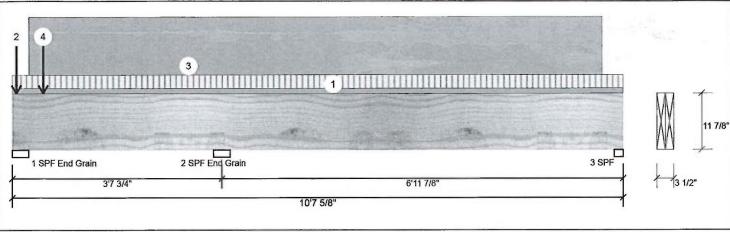
Designer: RO

MILLWOOD 2-ELEV 1

Level: Ground Floor

Job Name: Project #:

1.750" X 11.875" 2-Ply - PASSED



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 10-7-10	(Span)0-11-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-0-14		Near Face	218 lb	555 lb	0 lb	0 lb	F3
3	Part. Uniform	0-3-8 to 10-3-4		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
4	Point	0-6-8		Тор	231 lb	464 lb	0 lb	0 lb	BBO3 BBO3
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

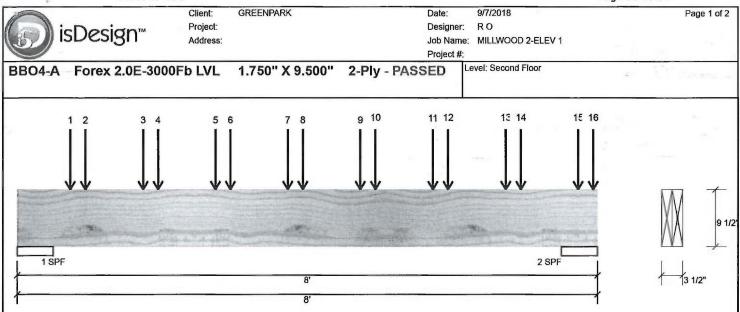
Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation regularements, multi-ply fastening details, beam strength values, and code

Manufacturer Info APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1

This design is valid until 7/10/2021



Type:	Girder		Application:	Floor (Residential)	Brg	Live		Dead	Snov	w	Wind
Plies:	2		Design Method:	LSD	1	2221		861		0	0
Moisture Condition	n: Dry		Building Code:	NBCC 2010 / OBC 2012	2	2284		885		0	0
Deflection LL:	360		Load Sharing:	No	1.50						
Deflection TL:	240		Deck:	Not Checked							
Importance:	Normal		Vibration:	Not Checked							
General Load											
Floor Live:	40 PSF				Bearings	and Fac	tored I	Reactions			
Dead:	15 PSF	9			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
					1-SPF	6.000"	34%	1076 / 3332	4409	L	1.25D+1.5L
					2-SPF	6.000"	35%	1106 / 3425	4531	L	1.25D+1.5L

Analysis Results

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	7713 ft-lb	3'11 1/4"	22724 ft-lb	0.339 (34%)	1.25D+1.5L	L
Unbraced	7713 ft-lb	3'11 1/4"	21721 ft-lb	0.355 (36%)	1.25D+1.5L	L
Shear	3886 lb	6'9 1/4"	9277 lb	0.419 (42%)	1.25D+1.5L	L
Perm Defl in.	0.033 (L/2630)	3'11 3/8"	0.238 (L/360)	0.140 (14%)	D	Uniform
LL Defl inch	0.084 (L/1016)	3'11 3/8"	0.238 (L/360)	0.350 (35%)	L	L
TL Defl inch	0.117 (L/733)	3'11 3/8"	0.356 (L/240)	0.330 (33%)	D+L	L

Design Notes

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- seed on full section width

/ Lateral	sienderness ratio based on	full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Point	0-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7
2	Point	0-11-4		Тор	116 lb	310 lb	0 lb	0 lb	J7
3	Point	1-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7
4	Point	1-11-4		Тор	116 lb	310 lb	0 lb	0 lb	J7
5	Point	2-8-12		Тор	111 lb	297 lb	0 lb	0 lb	J7

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product informals regarding installation requirements, multi-fastering details, beam strength values, and contains the strength values, and contains the strength values.

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is

Manufacturer Info

Unfactored Reactions UNPATTERNED lb (Uplift)

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada 905-642-4400

T.L. WISE 100083566

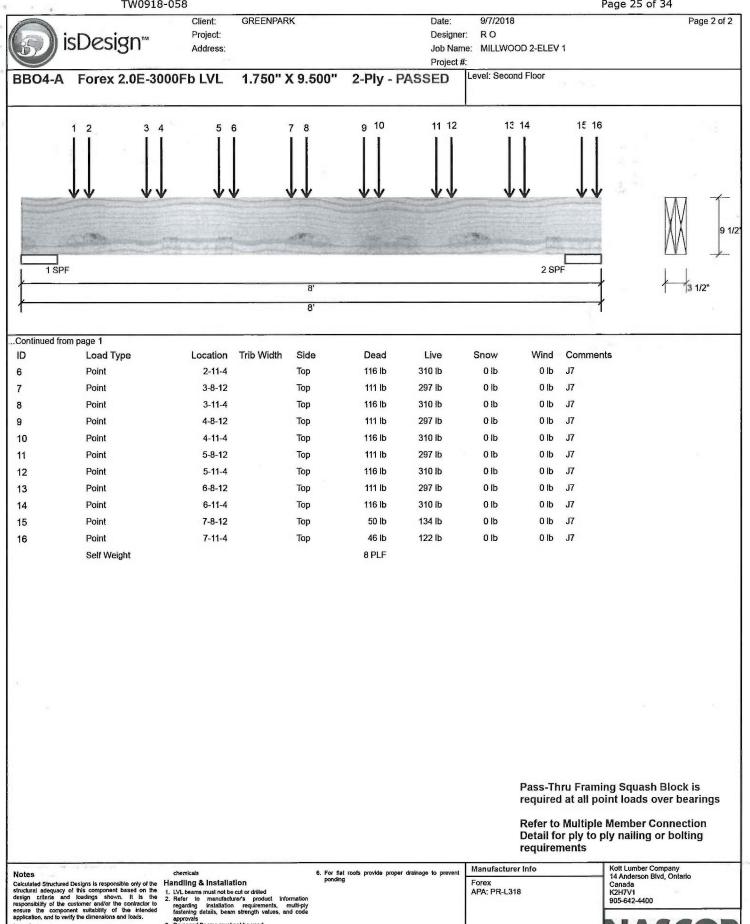
100083566

WCE OF ONTAR

September 13, 2018







This design is valid until 7/10/2021

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Lumber

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Provide lateral support at bearing points to avoid
lateral displacement and rotation



Client: Project: Address: GREENPARK

Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

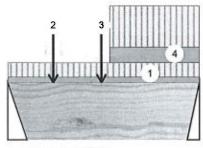
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor



1 Hanger (HGUS410) 2 Hanger (HUC410 (Min)) 3'3 3/4 3'3 3/4'



Page 1 of 1

ı	Type:	Girder	App
ı	Plies:	2	De
ı	Moisture Condition:	Dry	Bui
ı	Deflection LL:	360	Loa
ı	Deflection TL:	240	De
ı	Importance:	Normal	Vib
ı	General Load		

40 PSF

15 PSF

plication: Floor (Residential) esign Method: ilding Code: NBCC 2010 / OBC 2012 ad Sharing:

eck: Not Checked bration: Not Checked **Unfactored Reactions UNPATTERNED Ib (Uplift)**

Brg	Live	Dead	Snow	vvina
1	150	83	0	0
2	169	88	0	0

Analysis Results

Floor Live:

Dead:

Member Information

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	246 lb	1'3 1/8"	11596 lb	0.021 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999,000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/46486)	1'7 3/4"	0.145 (L/240)	0.010 (1%)	D+L	L

rulalysis	riciaai	Location	MOTTEG	Capacity	COMP.	Ouse
Moment	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	246 lb	1'3 1/8"	11596 lb	0.021 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999,000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/46486)	1'7 3/4"	0.145 (L/240)	0.010 (1%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.

Bearings and	Factored	Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	3%	104 / 225	329	L	1.25D+1.5L
2 - Hanger	2.500"	6%	110 / 253	363	L	1.25D+1.5L



September 13, 2018

/ Lateral	sienderness ratio based	on full section width.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-3-12	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-7		Near Face	26 lb	70 lb	0 lb	0 lb	J2
3	Point	1-7-8		Near Face	33 lb	35 lb	0 lb	PassiTh	nry-Framing Squash Block is d at all point loads over bearings
4	Tie-In	1-9-4 to 3-3-12	(Span)	Тор	15 PSF	40 PSF	0 PSF	0 PSF	at an point loads over bearings
	Self Weight		3-11-13		10 PLF				Multiple Member Connection or ply to ply nailing or bolting ments

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LIVI beams must not be cut or drilled Refer to manufacturer's product inform regarding installation requirements, mu fastening details, beam strength values, and approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrain Provide lateral support at bearing points lateral displacement and rotation
- 6. For flat roofs ponding

Manufacturer Info Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



This design



Project: Address:

9/7/2018

Designer: R₀

Job Name: MILLWOOD 2-ELEV 1

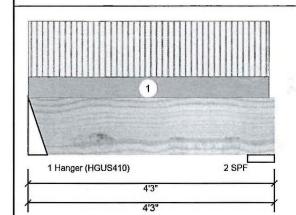
Project #:

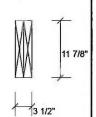
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Member Inf	ormation						Unfacto	red Reacti	ons UNPAT	TERNED Ib (U _l	plift)
Type: Plies: Moisture Cond Deflection LL: Deflection TL: Importance: General Load Floor Live: Dead:	Girder 2		Buildin	Method: I g Code: I sharing: I	Floor (Residen .SD NBCC 2010 / C No Not Checked Not Checked		Brg 1 2	Live 35 35 35 and Facto Length	Deac 33 34 ored Reacti Cap. React	Snow 0 0 0 ons D/L lb Total Lo	Wind 0 0
Analysis Re	eulte						1 - Hanger 2 - SPF	4.000" 5.500"		93 L 13/53 95 L	1.25D+1.5l
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-355	5.500	170		
Moment Unbraced Shear Perm Defl in. LL Defl inch TL Defl inch Oesign Not 1 Fill all hang 2 Girders are 3 Multiple plie 4 Top loads m 5 Top braced 6 Bottom brace	72 ft-lb 72 ft-lb 36 lb 0.000 (L/999) 0.000 (L/999) 0.000 (L/999) es er nailing holes. designed to be st s must be fasteneoust be supported	2' 3/4" 2' 3/4" 1'3 1/8" 0 0 0	34261 ft-lb 34261 ft-lb 11596 lb 999.000 (L/ 999.000 (L/ 999.000 (L/ e bottom ed per manufa plies.	0.002 (0%) 0.002 (0%) 0.003 (0%) 0.000 (0%) 0) 0.000 (0%) 0) 0.000 (0%)	1.25D+1.5L 1.25D+1.5L 1.25D+1.5L	L L				300 M	L. WISE TO NOT THE PROPERTY OF CAMPAGE OF CA
ID 1	Load Type Tie-In Self Weight		Location to 4-1-14	Trib Width (Span)0-10-1	Side Top	Dead 15 PSF 10 PLF	Live 40 PSF	1.51	F 0 PSF Pass-T	, , , , , , , , , , , , , , , , , , , ,	

Notes

NOTES

Calculated Shuctured Designs is responsible only of the shuctural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the linear physician, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is lateraty restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info

Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting

requirements



This design is



Project: Address:

Date:

9/7/2018 Designer: RO

Job Name: MILLWOOD 2-ELEV 1

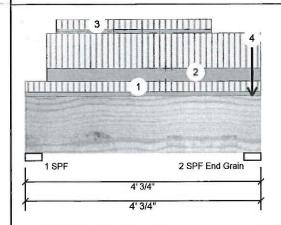
Project #:

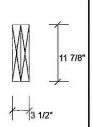
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Wind

Page 1 of 1

Member Info	rmation		
Type:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

Unfactored Reactions UNPATTERNED Ib (Uplift) Brg Live Dead Snow

1	643	260	0	0
2	885	375	0	0

Analysis Results

Γ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
	Unbraced	1215 ft-lb	2' 5/16"	34261 ft-lb	0.035 (4%)	1.25D+1.5L	L
	Shear	711 lb	2'10 1/8"	11596 lb	0.061 (6%)	1.25D+1.5L	L
	Perm Defl in.	0.001 (L/34561)	2' 5/16"	0.120 (L/360)	0.010 (1%)	D	Uniform
	LL Defl inch	0.003 (L/13799)	2' 5/16"	0.120 (L/360)	0.030 (3%)	L	L
l	TL Defl inch	0.004 (L/9862)	2' 5/16"	0.180 (L/240)	0.020 (2%)	D+L	L

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	17%	325 / 965	1289	L	1.25D+1.5L
2 - SPF End Grain	3.500"	20%	469 / 1328	1797	L	1.25D+1.5L



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Load Type

Part. Uniform

Part, Uniform

Self Weight

Tie-In

Point

OROFESSIONAL	
(Sell) Mark	
T.L. WISE 100083566	-
1 Junise	
September 13, 2018	

0 PSF	0 PSF
0 PLF	0 PLF
0 PLF	PasseThru Framing Squash Block is
0 lb	required at all point loads over bearings

Comments

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

ID

2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the cootractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVI, not to be treated with fire retardant or corrosive

Handling & Installation

Location

0-0-0 to 4-0-12

0-4-8 to 4-0-12

0-6-8 to 3-2-8

3-11-0

IATION BY A INSTALLATION IN A CONTROL OF THE ACT OF THE

Trib Width

(Span)3-7-4

Side

Top

Top

Near Face

Near Face

Dead

15 PSF

90 PLF

25 PLF

10 PLF

88 lb

Live

40 PSF

240 PLF

68 PLF

169 lb

Manufacturer Info

Forex APA: PR-L318

Snow

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400







Project: Address:

Date: 9/7/2018 Designer:

Job Name: MILLWOOD 2-ELEV 1

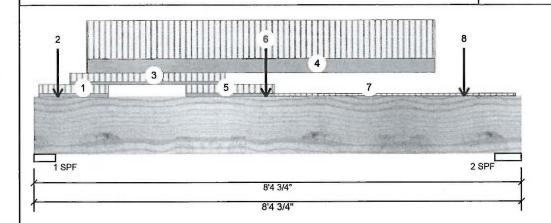
Level: Second Floor

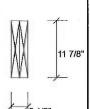
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED





Page 1 of 2

Member	Information
Туре:	Girder

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal
General Load	
Floor Live:	40 PSF

15 PSF

Floor (Residential) Application: Design Method: LSD

Building Code: NBCC 2010 / OBC 2012 Load Sharing: No

Deck: Not Checked Vibration: Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1594	649	0	0
2	1377	568	0	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.500"	33%	811 / 2391	3202	L	1.25D+1.5L
2-SPF	5.500"	23%	710 / 2066	2776	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5966 ft-lb	4'	34261 ft-lb	0.174 (17%)	1.25D+1.5L	L
Unbraced	5966 ft-lb	4'	31511 ft-lb	0.189 (19%)	1.25D+1.5L	L
Shear	3035 lb	7' 1/8"	11596 lb	0.262 (26%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/5638)	4' 3/4"	0.256 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.040 (L/2325)	4' 13/16"	0.256 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.056 (L/1646)	4' 13/16"	0.384 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

OPROFESSIONAL CALL
T.L. WISE
100083566
TO WAGE OF ONTAR!

September 13, 2018

O Lateral	PICTUCALLICSS LOUD DOSCO	off full Section with.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-0 to 1-3-8	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-0		Near Face	77 lb	205 lb	0 lb	0 lb	J7
3	Part. Uniform	0-7-8 to 3-3-8		Far Face	25 PLF	68 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-11-0 to 6-11-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
5	Tie-In	2-7-8 to 4-1-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	4-0-0		Far Face	83 lb	150 lb	0 lb	0 lb	F6

Continued on page 2...

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. If is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Lumber

chemicals

Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

fastening users, and approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

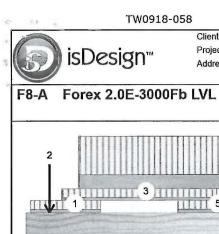
Forex APA: PR-L318

Manufacturer Info

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400





GREENPARK Client:

Project: Address:

9/7/2018

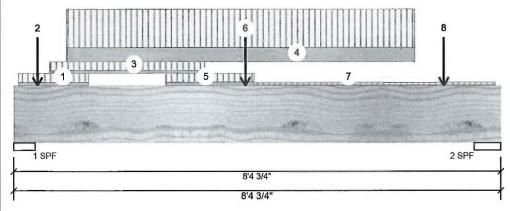
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

Date:

1.750" X 11.875" 2-Ply - PASSED Level: Second Floor



11 7/8"

Page 2 of 2

Continued from	page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Tie-In	4-1-12 to 8-3-10	(Span)0-11-3	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
8	Point	7-5-0		Near Face	117 lb	313 lb	0 lb	0 lb	J7
	Self Weight				10 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Shuctured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the coefractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVI, not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IV beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info

Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

This design is valid until 7/10/2021

Second Floor

Page 31 of 34

Layout Name

Design Method LSD

Description

June 25, 2018

Created

Builder GREENPARK

RM

RO

Sales Rep

Designer

Shipping

Builder's Project

14 Anderson Blvd

Stouffville, Ontario

Kott Lumber Company

S:\GUSTOMERS\GREENPARK \WINNISALE HOMES\WODELS

WILLWOOD 2/FLOORS/ELEV 2 WILLWOOD 2-ELEV 2.isi

Building Code NBCC 2010 / OBC

LSD

40

15

480

350

480

360

360

240

480

240

5/8"

SPF Plywood

Project

Canada

K2H7V1

Job Path

905-642-4400

Second Floor

Design Method

Deflection Joist

Deflection Girder

LL Span L/

TL Span L/

11 Cent 21/ TL Cant 2L/

LL Span L/

TL Span L/ LL Cant 2L/

TL Cant 2L/

Decking

Thickness

loor

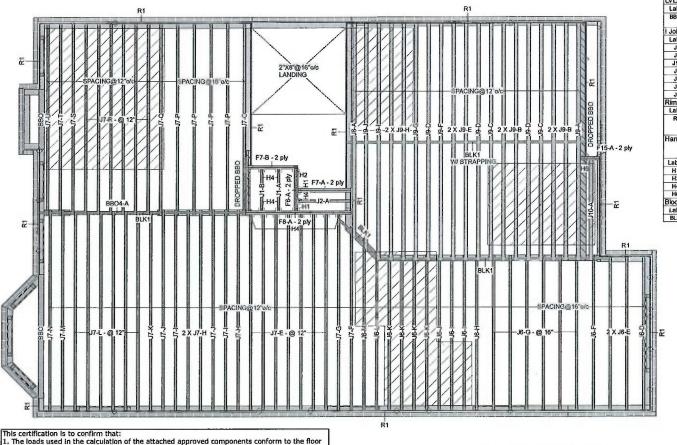
Loads

Live

Dead

MILLWOOD 2-ELEV 2

Plies Pcs Length



Legend

111111

Point Load Support

Norbord Rimboard Plus 1,125 X 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped)

Load from Above

Wali Opening

NJ6011 11 875

NJ 11.875

NJH 11.875

F8 1.75 11.875 2 2 10-0-0 2.0E-3000Fb LVL 6-0-0 Forex 2 0F-3000Fb1VI 4-0-0 F6 Forex 2.0E-3000Fb LVL 1.75 11.875 2 2 2-0-0 F15 Forex 2.0E-3000Fb LVL 1.75 11.875 1 2 2 LVL/LSL (Dropped) Width Depth Qty Piles Pcs Length Label Description BBO4 Forex 2.0E-3000Fb LVL 9.5 2 8-0-0 Joist (Flush) Pcs Length 18 20-0-0 Width Depth Qty Piles Label Description 3.5 11.875 3.5 11.875 J9 NJ60U J8 NJ50U 18-0-0 J10 NJ60U 3.5 11.875 8-0-0 J7 NJH J6 NJH 2.5 11.875 40 16-0-0 25 11875 20 14-0-0 6-0-0 J2 NJH 2.5 11,875 J1 NJH 2.5 11.875 4-0-0 Rim Board Label Description Width Depth Qty Plies Pcs Length Norbord Rimboard 1.125 11.875 R1 Plus 1.125 X 11.875 Hanger Beam/Girder Supported Member Label Pcs Description Skew Slope fasteners fasteners 2 HGUS410 1 HUC410 (M H1 48 16d 16 16d HUC410 (Min) 14 16d 6 10d H4 13 LT251188 H8 1 LT351188 4 10dx1 1/2 2 10dx1 1/2 4 10dx1 1/2 2 10dx1 1/2 Blocking Width Depth Qty Plies Pcs Length 2.5 11.876 LinFt Varies 40-0-0 BLK1 NJH

Width Depth Qty

NOTES:

Second Floor

LVL/LSL (Flush)

- 1. Framer to verify dimensions on the architectural drawings.
- Framer to verny cimensions on the arconectural drawings.
 Double joist only require filter/backer jly when supporting another member using a face-mounted hanger.
 Install 2x4 blocking @ 24*o/c under parallel non-load bearing walls.
 Install single-ply flush window header along inside face of
- rimboard/fimjoist.

 5. Refer to Nascor specifier guide for installation works.

 6. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding
- Load transfer blocks to be installed under all point loads. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or balting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/o). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. GAVOIN DESIGN GROUP INC. 64 Jardin Dr. Sulle 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

Nailed & Glued Fastener Vibration Gypsum 1/2" Ceilina DANOFESSIONAL EN T.L. WISE 100083586 WCE OF ON

September 13, 2018

Version 18.40.162 Powered by IStruct

layout.

assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply

All other components and structural elements supporting the floor system such as beams,

walls, columns and foundation walls and footings including anchorage of components and

bracing for lateral stability are the responsibility of others.

members must be attached together as per the included multiple member connection detail.

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them





Client:

GREENPARK

Project: Address: Date:

9/7/2018 RO

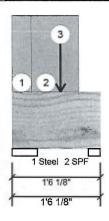
Designer: Job Name: MILLWOOD 2-ELEV 2

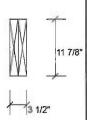
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Inform	nation			Unfactore	d Reactio	ns UNPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	72	103	0	0
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	61	53	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	red Reactions		
Dead:	15 PSF			Bearing L	ength	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - Steel 5	.250"	2% 129 / 108	237 L	1.25D+1.5L
				2-SPF 2	.375"	3% 66 / 92	158 L	1.25D+1.5L

Analysis Results

Г	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	_
	Moment	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
_		-						

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

T.L. WISE
T.L. WISE 100083566
Sulso)
September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments	
1	Part. Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
2	Part. Uniform	0-4-0 to 1-1-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight	
3	Point	0-10-0		Near Face	50 lb	133 lb	0 lb	0 lb	J10	
	Self Weight	Self Weight			10 PLF			Pass-Thru Framing Squash Block is		

required at all point loads over bearings Refer to Multiple Member Connection

Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component besed on the design criticals and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Inlended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 IVL beams must not be cut or drilled
 Rafer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength volues, and code approvels
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained.
 Provide lateral support at bearing points to avoid lateral displecement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED This design IN THE DESIGN OF THIS COMPONENT.



isDesign™

Client:

Project: Address: GREENPARK

Date:

9/7/2018

Page 1 of 1

RO Designer:

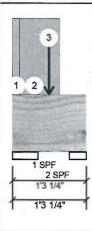
Job Name: MILLWOOD 2-ELEV 3

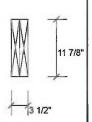
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Member Information				Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	108		101		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	65		41		0	0
Deflection LL:	360	Load Sharing:	No	-						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing:	s and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	5.250"	3%	126 / 162	288	L	1.25D+1.5L
				2-SPF	3.500"	2%	52 / 98	149	L	1.25D+1.5L

Analysis Rosults

•	tildiy bib itte	Tel Co						
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	190 lb	1'4 3/8"	11596 lb	0.016 (2%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
ı	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Load Type

Part, Uniform

Part. Uniform

Self Weight

Point

OFESSIONAL CHAIN	
T.L. WISE 100083566	1
STOUNCE OF ONTHE	
September 13, 2018	

0 PLF Wall Self Weight 0 PLF Wall Self Weight Pass-Thru Framing Squash Block is

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

required at all point loads over bearings

Comments

Wind

Notes

ID

1

2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or con

Handling & Installation

Location

0-7-8

0-0-0 to 0-1-8

0-1-8 to 0-9-12

Trib Width

Side

Top

Top

Far Face

- aged Beams must not be used

Dead

80 PLF

80 PLF

10 PLF

Live

0 PLF

0 PLF

Manufacturer Info

APA: PR-L318

0 PLF

0 PLF

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

