MILLWOOD 2-ELEV 2

Kott Lumber Company

S:\CUSTOMERS\GREENPARK

\MINNISALE HOMES\MODELS \MILLWOOD 2\FLOORS\ELEV 2 \MILLWOOD 2-ELEV 2.isl

Building Code NBCC 2010 / OBC

LSD

2012

40

15

480

360

480

360

360

240

480

240

3/4"

SPF Plywood

Nailed & Glued

Stouffville, Ontario

Canada

K2H7V1

Job Path

905-642-4400

Ground Floor

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Thickness

Fastener

Deck

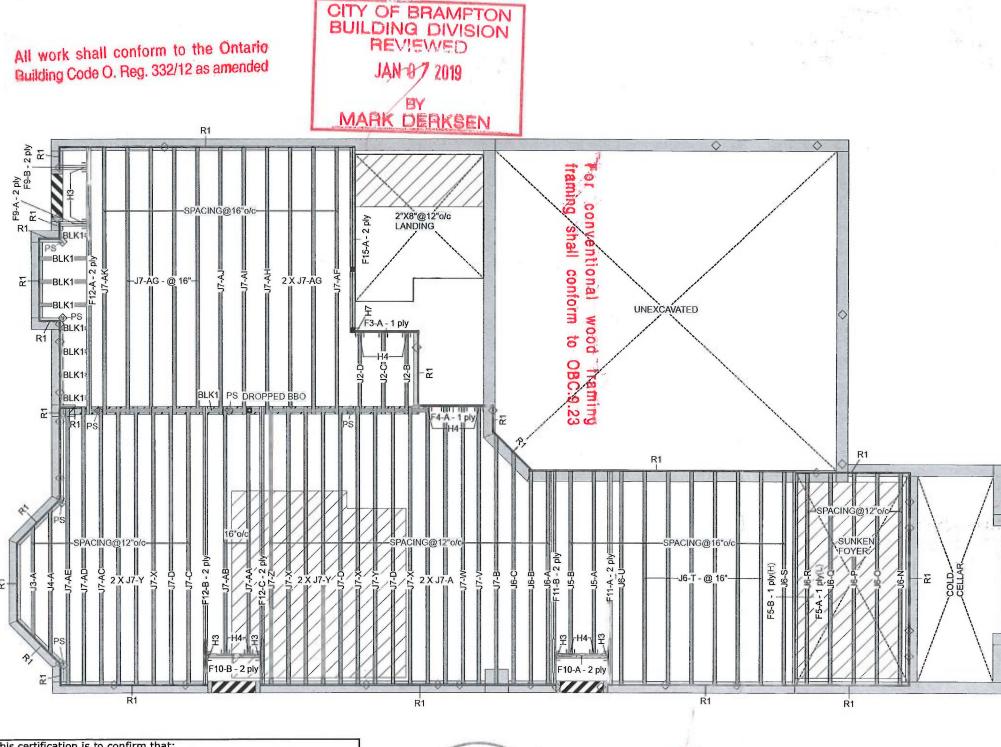
Deflection Girder

Floor

Loads

Live

Dead



This certification is to confirm that:

Version 18.40.162 Powered by iStruct™

Ground Floor

1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.



This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

2. Nascor CCMC - 13535-R

4. CAN/CSA-O86-09

5. CCMC -12787-R APA PR-L310(C)

_,	- (1 10011)							
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	NAS
F5	Forex 2.0E-3000Fb LVL	1.75	11.875			2	14-0-0	1 47 40
F15	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	12-0-0	Layout Name MILLWOOD 2-ELE
F4	Forex 2.0E-3000Fb LVL	1.75	11.875			1	6-0-0	Design Method LSD
F3	Forex 2.0E-3000Fb LVL	1.75	11.875			1	4-0-0	Description
l Joist (Flush)							Created
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	June 25, 2018
F12	NJ	1.5	11.875	3	2	6	16-0-0	
F11	NJ	1.5	11.875	2	2	4	14-0-0	Builder
F10	NJ	1.5	11.875	2	2	4	4-0-0	GREENPARK
F9	NJ	1.5	11.875	2	2	4	2-0-0	Sales Rep
J7	NJH	2.5	11.875			35	16-0-0	RM
J6	NJH	2.5	11.875			16	14-0-0	Designer
J5	NJH	2.5	11.875		- 7	2	12-0-0	RO
J4	HLN	2.5	11.875			1	10-0-0	
J3	NJH	2.5	11.875			1	8-0-0	Shipping
J2	NJH	2.5	11.875			3	6-0-0	Project
Rim Bo	ard							Builder's Project
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	Kott Lumber (
R1	Norbord Rimboard Plus 1.125 X 11.875	1.125	11.875			12	12	14 Anderson Blvd Stouffville, Ontario

Ground Floor LVL/LSL (Flush)

					beam/Girder	Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
НЗ	6	LT2-151188			4 10dx1 1/2	2 10dx1 1/2
H4	11	LT251188			4 10dx1 1/2	2 10dx1 1/2
H7	1	HUCQ1.81/9-				

La

JUNIT	g						
abel	Description	Width	Depth	Qty	Plies	Pcs	Length
BLK1	NJH	2.5	11.875	LinFt		Varies	29-0-0

NOTES:

. Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.

Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls. . Install single-ply flush window header along inside face of

rimboard/rimjoist.

6. Refer to Nascor specifier guide for installation works.

. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.

. Load transfer blocks to be installed under all point loads. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr, Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

Legend

Point Load Support load from Above

Wall Opening

Norbord Rimboard Plus 1.125 X 11.875 NJ 11.875

NJH 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

The floor system must be assembled in accordance to the Nascor September 13, 2018

1. OBC 2012 O.Reg 332/12 as amended

3. LVL CCMC -14056-R

Second Floor

ayout Name MILLWOOD 2-ELEV 2

GREENPARK Sales Rep RM Designer

RO

Shipping Project Builder's Project

Canada K2H7V1

Job Path

905-642-4400

Design Method

Deflection Joist

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

LL Span L/

TL Span L/

LL Cant 2L/

TL Cant 2L/

Decking

Deck

Deflection Girder

Floor

Loads

Live

Dead

Kott Lumber Company

S:\CUSTOMERS\GREENPARK **IMINNISALE HOMESIMODELS** \MILLWOOD 2\FLOORS\ELEV 2

Building Code NBCC 2010 / OBC

LSD

40

15

480

360

480

360

360 240

480

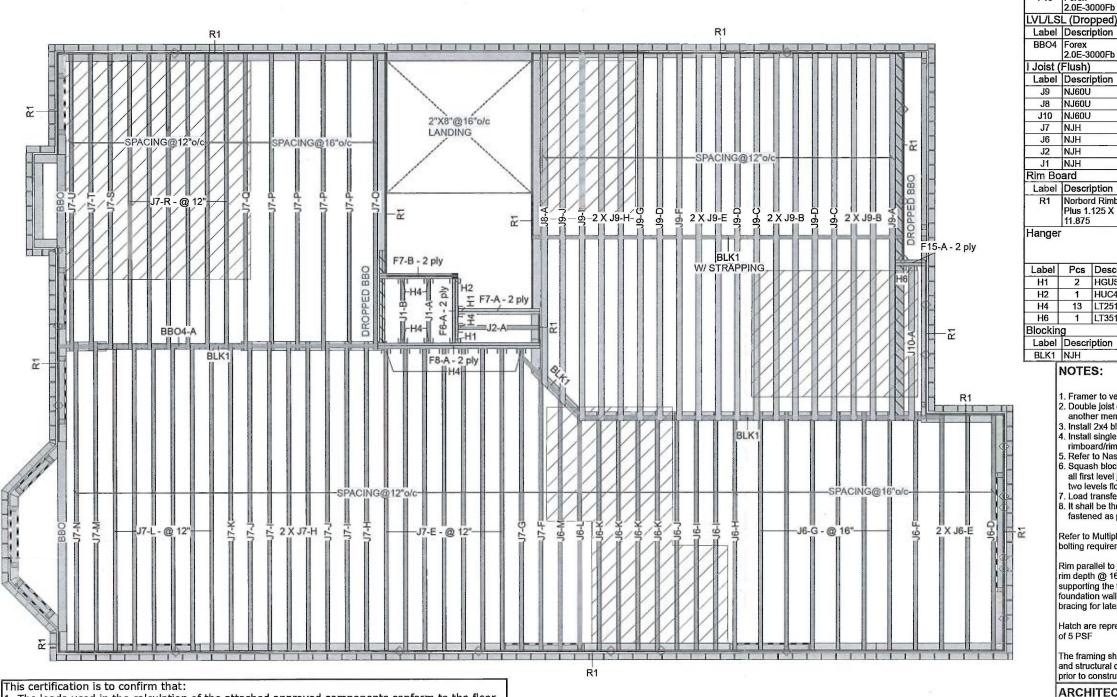
240

SPF Plywood

\MILLWOOD 2-ELEV 2.isl Second Floor

14 Anderson Blvd

Stouffville, Ontario



1. The loads used in the calculation of the attached approved components conform to the floor assembly shown on this layout.

2. The floor joists comply with the Nascor span table for the loads and spacing shown on this layout.

The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply members must be attached together as per the included multiple member connection detail. All other components and structural elements supporting the floor system such as beams, walls, columns and foundation walls and footings including anchorage of components and bracing for lateral stability are the responsibility of others.

Legend

0 1111111 Point Load Support Load from Above Wall Opening NJ 11.875 NJ60U 11.875 NJH 11.875 Forex 2.0E-3000Fb LVL 1.75 X 9.5 (Dropped)

Norbord Rimboard Plus 1.125 X 11.875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

LVL/LS	L (Flush)							MIAC
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	
F8	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	10-0-0	
F7	Forex 2.0E-3000Fb LVL	1.75	11.875	2	2	4	6-0-0	Layout Name MILLWOOD 2-ELE
F6	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	4-0-0	Design Method
F15	Forex 2.0E-3000Fb LVL	1.75	11.875	1	2	2	2-0-0	Description
LVL/LS	L (Dropped)							Created
Label	Description	Width	Depth	Qty	Plies	Pcs	Length	June 25, 2018
BBO4	Forex	1.75	9.5	1	2	2	8-0-0	Builder

BBO4	Forex 2.0E-3000Fb LVL	1.75	9.5	1	2	2	8-0-0
Joist (Flush)					- 5	0
Label	Description	Width	Depth	Qty	Plies	Pcs	Length
J9	NJ60U	3.5	11.875			18	20-0-0
J8	NJ60U	3.5	11.875	4		1	18-0-0
J10	NJ60U	3.5	11.875			1	8-0-0
J7	NJH	2.5	11.875			40	16-0-0
J6	NJH	2.5	11.875			20	14-0-0
J2	NJH	2.5	11.875	· 50		1	6-0-0
J1	NJH	2.5	11.875			2	4-0-0
im Bo	ard						
	D	1442 Pet	D	O1 .	DI:		1

Width Depth Qty Plies Pcs Length Label Description Norbord Rimboard Plus 1.125 X 1.125 11.875 11.875

Hanger

Second Floor

					Beam/Girder	Supported Member
Label	Pcs	Description	Skew	Slope	fasteners	fasteners
H1	2	HGUS410			46 16d	16 16d
H2	1	HUC410 (Min)			14 16d	6 10d
H4	13	LT251188			4 10dx1 1/2	2 10dx1 1/2
H6	1	LT351188			4 10dx1 1/2	2 10dx1 1/2
Blockin	g					

Width Depth | Qty | Plies | Pcs | Length

2.5 11.875 LinFt Varies 40-0-0

NOTES:

- I. Framer to verify dimensions on the architectural drawings.
- 2. Double joist only require filler/backer ply when supporting another member using a face-mounted hanger.
- 3. Install 2x4 blocking @ 24"o/c under parallel non-load bearing walls. 4. Install single-ply flush window header along inside face of rimboard/rimjoist.
- 5. Refer to Nascor specifier guide for installation works.
- i. Squash blocks recommended to be installed at end bearing on all first level joists which support loading from above exceeding two levels floor or roof.
- 7. Load transfer blocks to be installed under all point loads.
 8. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than rim depth @ 16"o/c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load of 5 PSF

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr. Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

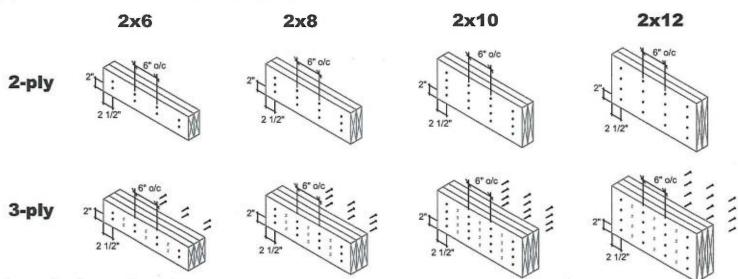
Thickness Nailed & Glued Fastener Vibration Gypsum 1/2" Ceiling: PROFESSIONALE T.L. WISE 100083566 September 13, 2018

Version 18.40.162 Powered by iStruct^{na}

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

MULTIPLE MEMBER CONNECTIONS

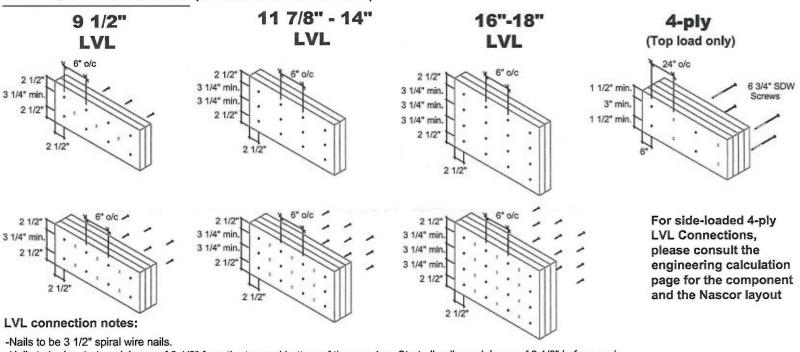
Conventional Connections (for uniform distributed loads)



Conventional connection notes:

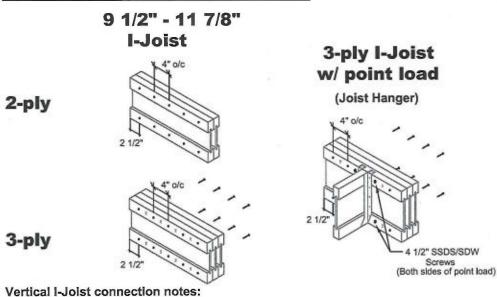
- -Nails to be 3" 10d spiral wire nails.
- -Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Minimum 3 1/4" spacing between rows.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail or screw driven from the opposite side.

Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
- -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
- "X" represents nail driven from the opposite side.

3228 Moodie Drive Ottawa, ON **K2H 7V1** Ph: 613-838-2775

MULTI-PLY CONNECTION DETAILS

Date: November 30, 2016

Scale: NTS

TW0918-058

Engineering Note Page (ENP-2)

REVISION 2009-10-09

Please read all notes prior to installation of the component

DESIGN INFORMATION

This building component is certified as an individual component for the loads and conditions shown on the calculation and drawing page.

The responsibility of the undersigned engineer is <u>only</u> limited to the calculation of this building component for the loads and conditions shown on this drawing.

The responsibility of the undersigned is limited to the verification of the structural capacity of the NASCOR floor joists and LVL beams based on placement as shown on the layout. The loads applied are limited to the gravity effects of the specified loads. The structural integrity of the building and the effect of wind, uplift, seismic, lateral or other forces, calculation of adequate support and anchorage of components, as well as the dimensions and design loads used to calculate components are the responsibility of the overall building designer.

Floor joists and OSB rim board are designed to carry uniformly distributed loads only. Point loads should be transferred through the floor cavity with squash blocks. Structural elements such as walls, posts, connectors, and squash blocks are the responsibility of the overall building designer.

The undersigned engineer disclaims any responsibility for damages as a result of being furnished faulty or incorrect information, specifications and/or designs.

Installation of NASCOR joists is to be carried out in accordance with the current edition of the manufacturer's approved literature available at http://www.nascor.ca.

CODE

This building component is designed in accordance with the National Building Code of Canada, the Ontario Building Code, CCMC and Canadian Standards Association guidelines.

COMPONENT

- 1. The building component used in construction must be the same as indicated on the drawings.
- 2. The building component must be installed and assembled as per specification shown on the drawing and in accordance with the manufacturer's assembly and installation.
- 3. Members consisting of multiple plies must be connected as per the document "Multi-ply Connection Details".
- 4. Pass-thru squash block framing is required at all point loads over bearings.

HANDLING AND INSTALLATION

Do not drill any hole, cut or notch a certified building component without a written preauthorization.



ate, the

MULTIPLE MEMBER CONNECTIONS

Conventional Connections (for uniform distributed loads)

2x8 2x12 2x6 2-ply 3-ply

Conventional connection notes:

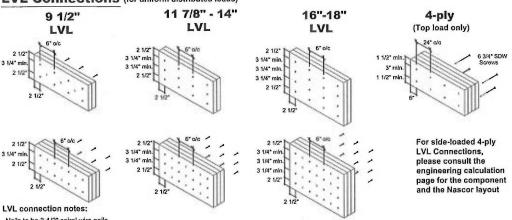
-Nails to be 3" 10d spiral wire nails.

-Nails to be located a minimum of 2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.

-Number of rows and spacing as per details shown, unless noted otherwise.

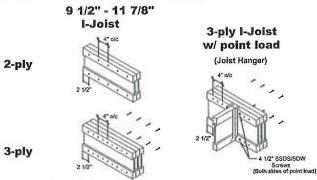
-"X" represents nail driven from the opposite side.

LVL Connections (for uniform distributed loads)



- -Nails to be 3 1/2" spiral wire nails.
 -Nails to be located a minimum of 2 1/2" from the top and bottom of the member. Start all nails a minimum of 2 1/2" in from ends.
- -Nation to be located a finding the control of the most of the mos

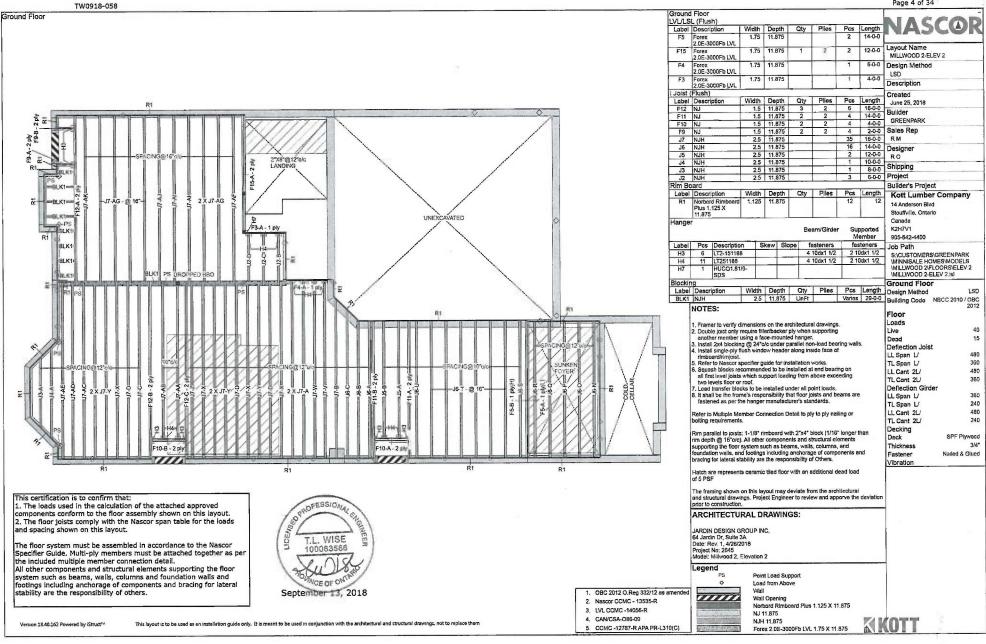
Vertical I-Joist Connections (for uniform distributed loads)



Vertical I-Joist connection notes:

- -Nails to be 3" spiral wire nails.
 -Nails to be located at centre of top and bottom flanges. Start all nails a minimum of 2 1/2" in from ends.
- -Number of rows and spacing as per details shown, unless noted otherwise.
 "X" represents nail driven from the opposite side.

KOTT 3228 Moodle Drive Ottawa, ON K2H 7V1 Ph: 613-838-2775 613-838-4751





Client: **GREENPARK**

Project: Address: Date: 9/7/2018

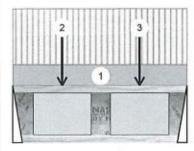
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

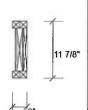
Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor



1 Hanger (LT2-151188) 2 Hanger (LT2-151188)



Wind

0

Page 1 of 1

ember	Intor	mation
,icilibei	111101	madon

Type:	Girder						
Plies:	2						
Moisture Condition:	Dry						
Deflection LL:	360						
Deflection TL:	240						
Importance:	Normal						
General Load							
Floor Live:	40 PSF						
Dead:	15 PSF						

Floor (Residential)

Brg

NBCC 2010 / OBC 2012 No

Load Sharing: Deck: Not Checked Vibration:

Application:

Design Method:

Building Code:

Not Checked

LSD

106 1 282

Unfactored Reactions UNPATTERNED Ib (Uplift)

2	287	108	Ü	U

Dead

Snow

0

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	401 ft-lb	1'4 1/2"	9020 ft-lb	0.044 (4%)	1.25D+1.5L	L
Unbraced	401 ft-lb	1'4 1/2"	5749 ft-lb	0.070 (7%)	1.25D+1.5L	L
Shear	558 lb	2'10 3/4"	3400 lb	0.164 (16%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/38142)	1'5 9/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.002 (L/14284)	1'5 1/2"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.003 (L/10392)	1'5 9/16"	0.140 (L/240)	0.020 (2%)	D+L	L

Bearings and Factored Reactions

Live

bearings and ractored reactions									
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.			
1 - Hanger	2.000"	20%	132 / 423	555	L	1.25D+1.5L			
2 - Hanger	2.000°	21%	135 / 431	566	L	1.25D+1.5L			
	Bearing 1 - Hanger 2 -	Bearing Length 1 - 2.000" Hanger 2 - 2.000"	Bearing Length Cap. 1 - 2.000" 20% Hanger 2 - 2.000" 21%	1 - 2.000" 20% 132 / 423 Hanger 2 - 2.000" 21% 135 / 431	Bearing Length Cap. React D/L lb Total 1 - 2.000" 20% 132 / 423 555 Hanger 2 - 2.000" 21% 135 / 431 566	Bearing Length Cap. React D/L lb Total Ld. Case 1 - 2.000" 20% 132 / 423 555 L Hanger 2 - 2.000" 21% 135 / 431 566 L	Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - 2.000" 20% 132 / 423 555 L 1.25D+1.5L Hanger 2 - 2.000" 21% 135 / 431 566 L 1.25D+1.5L		

Wind

0 PSF

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.



e Rottom	nange braced at bearings	i.						
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	
2	Point	0-10-4		Far Face	87 lb	233 lb	0 lb	
3	Point	2-2-4		Far Face	86 lb	229 lb	0 lb	

Pass-Thru Framing Squash Block is required aball point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Comments

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uoist not to be treated with fire retardant or con

Handling & Installation

- nandling & Installation

 1. Joist flanges must not be cut or drilled

 2. Refer to latest copy of the Joist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-byt fastening details and handling/erection details

 3. Damaged Loists must not be used

 4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 Inches
 For flat roofs

This design i

Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info





Client: **GREENPARK**

Project: Address:

Date: Designer:

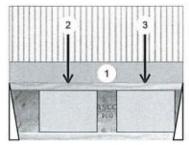
9/7/2018 RO

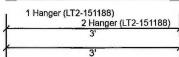
Job Name: MILLWOOD 2-ELEV 1

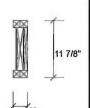
Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor







Page 1 of 1

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 201
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

	Unfacto	red Reaction	S UNPATTER	NED lb (Uplif	t)
	Brg	Live	Dead	Snow	Wind
	1	343	129	0	0
2012	2	404	152	0	0
	l .				

Bearings and Factored Reactions Bearing Length Cap. React D/L lb Total Ld. Case Ld. Comb. 2.000" 25% 161 / 514 675 L 1.25D+1.5L Hanger 2.000" 29% 189 / 606 795 L 1.25D+1.5L Hanger

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	583 ft-lb	1' 1/4"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L
Unbraced	583 ft-lb	1' 1/4"	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L
Shear	788 lb	2'10 3/4"	3400 lb	0.232 (23%)	1.25D+1.5L	L
Perm Defl in.	0.001 (L/27610)	1'1 5/16"	0.093 (L/360)	0.010 (1%)	D	Uniform
LL Defl inch	0.003 (L/10370)	1'1 5/16"	0.093 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.004 (L/7538)	1'1 5/16"	0.140 (L/240)	0.030 (3%)	D+L	L

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the
- 3 Multiple plies must be fastened together as
- 4 Top loads must be supported equally by all
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

as	ne bottom edge s per manufactu plies.	•			September 13, 2018
11	0.140 (L/240)	0.030 (3%)	D+L	L	100083566
IJ	0.093 (L/360)	0.030 (3%)	L	L	0 1.2, 11.02
	0.093 (L/360)	0.010 (1%)	D	Uniform	TI WISE IN
1)	3400 lb	0.232 (23%)	1.25D+1.5L	L	PRO
10	5749 ft-lb	0.101 (10%)	1.25D+1.5L	L	SEBSION.
"	9020 ft-lb	0.065 (6%)	1.25D+1.5L	L	

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-0-0	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	1-0-4		Far Face	130 lb	346 lb	0 lb	0 lb	J7
3	Point	2-4-4		Far Face	110 lb	293 lb	0 lb		ruFraming Squash Block is I at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- Handling & Installation

 1. Blots larges must not be cut or drilled

 2. Refer to latest copy of the Ljoist product information
 defails for framing delais, stiffener tables, web hole
 chart. bridging details, multi-ply fastening details and
 handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

This design i

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length>= 3.5 inches
 For flat roofs p

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.



isDesign™

Client: **GREENPARK**

Project: Address: Date: 9/7/2018

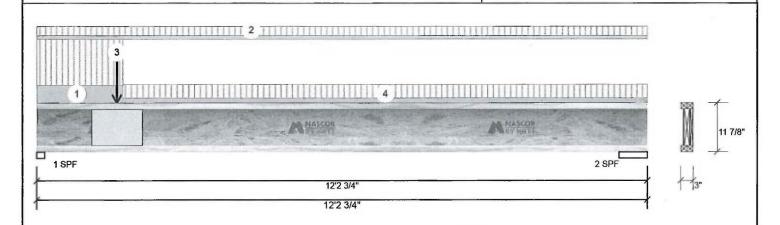
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

11.875" 2-Ply - PASSED F11-A NJ

Level: Ground Floor



Member Inf	ormation						Unfacto	red React	ions L	INPATTERN	IED lb	(Uplift)	
Туре:	Girder		Application	n: Fl	oor (Resident	ial)	Brg	Live		Dead	Sno	W	Wind
Plies:	2		Design M	ethod: LS	SD		1	512		192		0	0
Moisture Cond	ition: Dry		Building (Code: NI	BCC 2010 / O	BC 2012	2	243		91		0	0
Deflection LL:	360		Load Sha	ring: No	0								
Deflection TL:	240		Deck:	Ne	ot Checked								
Importance:	Normal		Vibration:	No	ot Checked								
General Load													•••
Floor Live:	40 PSF						Bearing	s and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	1.875"	38%	240 / 767	1008	L	1.25D+1.5L
							2-SPF	6.875"	14%	114 / 365	479	L	1.25D+1.5L
Analysis Res	sults						_					_	
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	1				1	CESSIO	DAY.
Moment	1602 ft-lb	4'6 5/8"	9020 ft-lb	0.178 (18%)	1.25D+1.5L	L					108	Or	MAL
Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L				,	2 L		181
Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L				/	3/	-	15/
Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform					Ö.	T.L. WIS	SE SEE
LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L					3	1000835	566 "

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	1602 ft-lb	4'6 5/8"	9020 ft-lb	0.178 (18%)	1.25D+1.5L	L
	Unbraced	1602 ft-lb	4'6 5/8"	1617 ft-lb	0.991 (99%)	1.25D+1.5L	L
	Shear	993 lb	1 1/8"	3400 lb	0.292 (29%)	1.25D+1.5L	L
	Perm Defl in.	0.018 (L/7877)	5'6 5/16"	0.388 (L/360)	0.050 (5%)	D	Uniform
	LL Defl inch	0.047 (L/2957)	5'6 5/16"	0.388 (L/360)	0.120 (12%)	L	L
	TL Defl inch	0.065 (L/2150)	5'6 5/16"	0.581 (L/240)	0.110 (11%)	D+L	L
_							

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'7" o.c.
- 5 Rottom flange braced at hearing

DOMOIII	i nanye braceu at bearing	5.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 12-2-12	(Span)0-7-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	108 lb	287 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span)0-11-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
10.71								Pass-Th	ru Framing Squash I

quash Block is required at all point loads over bearings

WCE OF ONT

September 13, 2018

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads. Lumber

Handling & Installation

- I. IJoist flanges must not be cut or drilled
 Refer to latest copy of the IJoist product Information details for framing details, sufferer tables, web hole chart, bridging details, multi-ply fastening details and handlinglerection details
 Demaged IJoists must not be used
 Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeness for point load as shown Minhnum point load bearing length>= 3.5 inches
 For flat roots proporting
 READ ALL NOTES ON

This design is

Manufacturer Info

Nascor by Kott

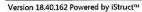
READ ALL NOTES ON THIS PAGE AND ON THE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

Canada K2H7V1

Kott Lumber Company 14 Anderson Blvd, Ontario









Client:

Project: Address: **GREENPARK**

9/7/2018 Date: Designer:

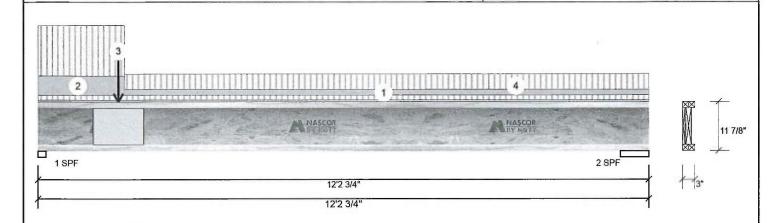
RO

Job Name: MILLWOOD 2-ELEV 1

Project #

2-Ply - PASSED F11-B NJ 11.875"

Level: Ground Floor



Member Inf	ormation						Unfacto	red React	ions (INPATTERN	ED Ib	(Uplift)	
Туре:	Girder		Application	n: Flo	oor (Resident	ial)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design M	ethod: LS	SD		1	471		177		0	0
Moisture Cond	lition: Dry		Building (Code: NE	3CC 2010 / O	BC 2012	2	206		77		0	0
Deflection LL:	360		Load Sha	ring: No)		-						
Deflection TL:	240		Deck:	No	ot Checked								
Importance:	Normal		Vibration:	No	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fact	tored	Reactions			
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
			-				1-SPF	1.875"	35%	221 / 707	928	L	1.25D+1.5L
							2-SPF	6.875"	12%	96 / 308	405	L	1.25D+1.5L
Analysis Res	sults											_	
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	7					EESSIO	
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L					108	Oreano	MALE
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L				- /	3/	OFESSIO	SE SE
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L				- 1	W -	T.L. WIS	SE m
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform				(5	1000835	66 30
					77		1					1000000	100

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1410 ft-lb	4'3 1/4"	9020 ft-lb	0.156 (16%)	1.25D+1.5L	L
Unbraced	1410 ft-lb	4'3 1/4"	1412 ft-lb	0.998 (100%)	1.25D+1.5L	L
Shear	915 lb	1 1/8"	3400 lb	0.269 (27%)	1.25D+1.5L	L
Perm Defl in.	0.015 (L/9004)	5'5 11/16"	0.388 (L/360)	0.040 (4%)	D	Uniform
LL Defl inch	0.041 (L/3379)	5'5 11/16"	0.388 (L/360)	0.110 (11%)	L	L
TL Defl inch	0.057 (L/2457)	5'5 11/16"	0.581 (L/240)	0.100 (10%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 5'11" o.c.

5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	106 lb	282 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 12-2-12	(Span) 0-11-12	Тор	15 PSF	40 PSF	0 PSF		ru Framing Squash Block is at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or correlations.

Handling & Installation

and ling & Installation.

Joist flanges must not be out or drilled.
Refer to latest copy of the Lioist product information details for framing details, stiffener tables, web note chart, bridging details, multiply fastening details and handling/erection details.

Damaged Lioists must not be used.
Design assumes top flange to be laterally restrained by ettached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length= 3.5 inches
 For flat roofs pro

This design is

READ ALL NOTES ON THIS PAGE AND ON THE

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400

WCE OF ON

September 13, 2018





GREENPARK Client:

Project: Address:

9/7/2018 Date:

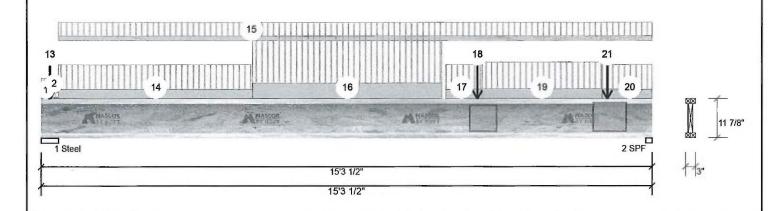
RO Designer:

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED NJ 11.875" F12-A

Level: Ground Floor



Member Infor	mation			Unfactored	Reactions	UNPATTERN	ED lb (Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	2	Design Method:	LSD	1	641	269	0	0
Moisture Conditio	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	539	202	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked	1				
Importance:	Normal	Vibration:	Not Checked	1				
General Load								
Floor Live:	40 PSF			Bearings ar	nd Factored	Reactions		
Dead:	15 PSF			Bearing Ler	ngth Cap	. React D/L lb	Total Ld. Case	Ld. Comb.
				1 - Steel 5.2	50" 38%	337 / 961	1298 L	1.25D+1.5L
				2 - SPF 1.8	75" 40%	253 / 809	1061 L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3724 ft-lb	8' 3/16"	9020 ft-lb	0.413 (41%)	1.25D+1.5L	L
Unbraced	3724 ft-lb	8' 3/16"	3737 ft-lb	0.997 (100%)	1.25D+1.5L	L
Shear	1052 lb	15'2 3/8"	3400 lb	0.309 (31%)	1.25D+1.5L	L
Perm Defl in.	0.062 (L/2891)	7'10 3/8"	0.494 (L/360)	0.120 (12%)	D	Uniform
LL Defl inch	0.164 (L/1083)	7'10 3/8"	0.494 (L/360)	0.330 (33%)	L	L
TL Defl inch	0.226 (L/788)	7'10 3/8"	0.741 (L/240)	0.300 (30%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 3'9" o.c.
- 5 Bottom flange braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 0-5-4	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 0-5-4	(Span)0-8-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	0-2-10		Тор	1 lb	3 lb	0 lb	0 lb	J7
4	Point	0-2-10		Тор	1 lb	4 lb	0 lb	0 lb	J7
5	Point	0-2-10		Тор	1 lb	3 lb	0 ib	dl 0	J7
6	Point	0-2-10		Тор	1 lb	0 lb	0 lb	0 lb	Wall Self Weight
7	Point	0-2-10		Тор	21 lb	56 lb	0 lb	0 lb	J7

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uplist not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- andling & Installation

 Joist flanges must not be cut or drilled
 Refer to latest copy of the Jioist product information
 details for framing details, suffiner tables, web hole
 chart, bridging details, multiply fastening details and
 handling/erection details

 Demaged Loists must not be used
 Design assumes top flange to be laterally restrained
 by attached sheathing or as specified in engineering
 notes.

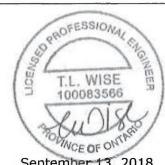
- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing in sthre 3.5 inches
 For flat roofs provide
 READ ALL NOTES O

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

Manufacturer Info

Nascor by Kott

IN THE DESIGN OF THIS COMPONENT. This design is



September 13, 2018



Page 2 of 2



Client: **GREENPARK**

Project: Address: Date: 9/7/2018

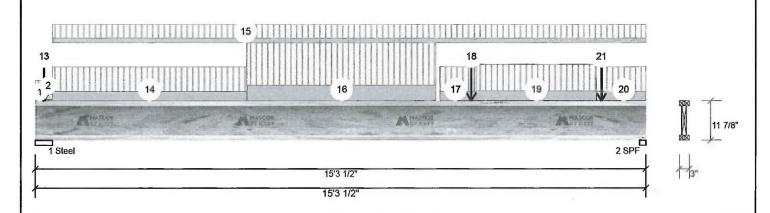
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED NJ 11.875"

Level: Ground Floor



.Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
8	Point	0-2-10		Тор	22 lb	59 lb	0 lb	0 lb	J7
9	Point	0-2-10		Тор	2 lb	5 lb	0 lb	0 lb	J7
10	Point	0-2-10		Тор	20 lb	0 lb	0 lb	0 lb	Wall Self Weight
11	Point	0-2-10		Тор	9 lb	25 lb	0 lb	0 lb	J7
12	Point	0-2-10		Тор	10 lb	26 lb	0 lb	0 lb	J7
13	Point	0-2-10		Тор	9 lb	0 lb	0 lb	0 lb	Wall Self Weight
14	Tie-In	0-5-4 to 5-3-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
15	Tie-In	0-5-4 to 15-3-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
16	Tie-In	5-3-10 to 10-0-8	(Span)2-9-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
17	Tie-In	10-1-10 to 11-0-10	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
18	Point	10-11-2		Far Face	18 lb	49 lb	0 lb	0 lb	F9
19	Tie-In	11-0-10 to 14-0-10	(Span)1-9-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
20	Tie-In	14-0-10 to 15-3-8	(Span)1-7-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
21	Point	14-2-2		Far Face	25 lb	66 lb	0 lb	0 lb	F9

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Unist not to be treated with fire retardant or corr

chemicals

Handling & Installation

1. Note thanges must not be cut or drilled

2. Refer to latest copy of the Ljoist product information details for training details, sufficer tables, web hole chart, bridging details, musti-ply fastening details and handlingerection details

3. Damaged Ljoists must not be used

4. Design assumes top flange to be laterally restrained by altached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.5 Inches
 For flat roofs provide proper drainage to prevent

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott

Client: **GREENPARK**

Project: Address:

9/7/2018 Date:

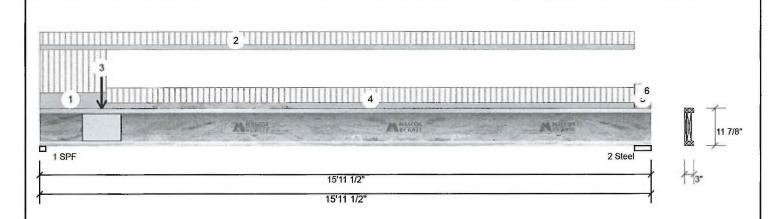
Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED F12-B NJ 11.875"

Level: Ground Floor



Member Info	rmation			Unfactor	red Reac	tions U	INPATTERN	ED lb ((Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	712		267		0	0
Moisture Conditi	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	376		141		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings	and Fac	tored	Reactions			
Dead:	15 PSF	100		Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	53%	334 / 1068	1402	L	1.25D+1.5L
				2 - Steel	5.250"	22%	176 / 563	739	L	1.25D+1.5L

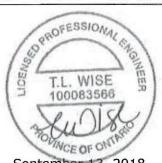
Analysis Results

Α	nalysis	Actual	Location	Allowed	Capacity	Comb.	Case
M	loment	3112 ft-lb	6'11 1/2"	9020 ft-lb	0.345 (34%)	1.25D+1.5L	L
U	nbraced	3112 ft-lb	6'11 1/2"	3135 ft-lb	0.993 (99%)	1.25D+1.5L	L
S	hear	1386 lb	1 1/8"	3400 lb	0.408 (41%)	1.25D+1.5L	L
P	erm Defl in.	0.057 (L/3233)	7'6 13/16"	0.516 (L/360)	0.110 (11%)	D	Uniform
LI	L Defl inch	0.153 (L/1213)	7'6 13/16"	0.516 (L/360)	0.300 (30%)	L	L
T	L Defl inch	0.211 (L/882)	7'6 13/16"	0.774 (L/240)	0.270 (27%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'2" o.c.

5 Bottom flange braced at bearings.



September 13, 2018

3 DORONI I	nange blaced at bealing	Jo							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-6-4	(Span) 0-11-12 to 0-11-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Near Face	129 lb	343 lb	0 lb	0 lb	F10 ru Framing Squash Block is
4	Tie-In	1-8-14 to 15-6-4	(Span)1-1-12 to 1-1-12	Тор	15 PSF	40 PSF	0 PSF	required	at all point loads over bearings
5	Tie-In	15-6-4 to 15-11-8	(Span)0-5-4	Тор	15 PSF	40 PSF	0 PSF	Refersto	Multiple Member Connection
6	Tie-In	15-6-4 to 15-11-8	(Span) 0-10-12	Тор	15 PSF	40 PSF	0 PSF		r ply to ply nailing or bolting

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Lioist not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. Idols flanges must not be cut or drilled

 2. Refer to latest copy of the I-Joist product information details for framing details, sufferier tables, web hole chart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged I-Joists must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing length= 3.5 inches
 For first roofs pro

This design is

READ ALL NOTES ON THIS PAGE AND ON THE CONTAINS SPECIFICATIONS AND CRITERIA USED

Manufacturer Info

Nascor by Kott

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1



905-642-4400

Client: **GREENPARK**

Project: Address:

Date: Designer:

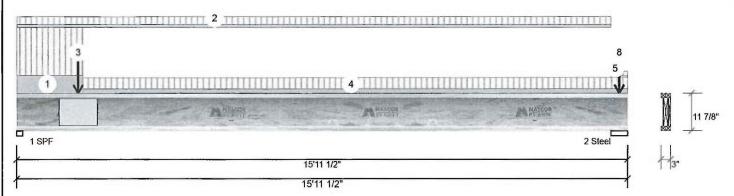
9/7/2018 RO

Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED

Level: Ground Floor



Member Inf	ormation						Unfacto	red Reacti	ions U	NPATTERN	ED lb	(Uplift)	
Туре:	Girder		Application	n: Fl	oor (Resident	ial)	Brg	Live		Dead	Sno	W	Wind
Plies:	2		Design M	ethod: LS	SD		1	649		244		0	0
Moisture Cond	ition: Dry		Building (Code: N	BCC 2010 / O	BC 2012	2	417		179		0	0
Deflection LL:	360		Load Sha	ring: No)								
Deflection TL: Importance: General Load	240 Normal		Deck: Vibration:		ot Checked ot Checked								
Floor Live:	40 PSF						Bearings	and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	1.875"	48%	305 / 973	1278	L	1.25D+1.5L
	1.						2 - Steel	5.250"	25%	223 / 625	849	L	1.25D+1.5L
Analysis Res											123	_	_
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case					/	OFESSIO	NA
Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L					108	-	10
Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L				/	30		SE SEER
Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L				- 1	9	TI WIS	SE m
Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform				1	Ξ.	1000835	66 .0
							1					1000000	A CONTRACTOR OF THE PARTY OF TH

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2271 ft-lb	6'1 5/8"	9020 ft-lb	0.252 (25%)	1.25D+1.5L	L
Unbraced	2271 ft-lb	6'1 5/8"	2271 ft-lb	1.000 (100%)	1.25D+1.5L	L
Shear	1264 lb	1 1/8"	3400 lb	0.372 (37%)	1.25D+1.5L	L
Perm Defl in.	0.042 (L/4432)	7'4 5/8"	0.516 (L/360)	0.080 (8%)	D	Uniform
LL Defl inch	0.112 (L/1664)	7'4 3/4"	0.516 (L/360)	0.220 (22%)	L	L
TL Defl inch	0.154 (L/1210)	7'4 3/4"	0.774 (L/240)	0.200 (20%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top flange must be laterally braced at a maximum of 4'10" o.c.
- 5 Bottom flange braced at bearings.

ODOLLOIN	light braces at bearing	go.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-8-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 15-6-4	(Span)0-6-4 to 0-6-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
3	Point	1-7-6		Far Face	152 lb	404 lb	0 lb	0 lb	F10
4	Tie-In	1-8-14 to 15-11-8	(Span)0-9-4 to 0-9-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
5	Tie-In	15-6-4 to 15-11-8	(Span)0-3-12	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	15-8-14		Тор	32 lb	85 lb	0 lb	0 lb	J7

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upist not to be treated with fire retardant or corrosive

Handling & Installation

- Harduling & Installation

 1. IJoist flarges must not be cut or drilled

 2. Refer to letest copy of the IJoist product information
 details for framing details, stiffener tables, web hot chart, bridging details, multi-ply fastening details and
 handling/erection details

 3. Damaged IJoists must not be used

 4. Design assumes top flange to be laterally restrained
 by attached shealthing or as specified in engineering
 notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation.
 Web stiffeness for point load as shown Minimum point load bearing length>= 3.5 inches
 T. For flat roofs proponding
 READ ALL NOTES ON

Manufacturer Info Nascor by Kott

READ ALL NOTES ON THIS PAGE AND ON THE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT. This design is

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1

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September 13, 2018



Page 2 of 2



Client: **GREENPARK**

Project:

Address:

Date:

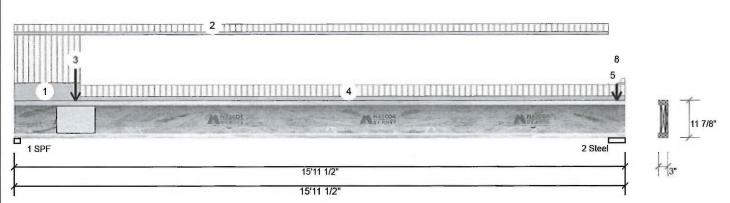
9/7/2018

RO Designer: Job Name: MILLWOOD 2-ELEV 1

Project #:

2-Ply - PASSED

Level: Ground Floor



Continued	from page 1								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
7	Point	15-8-14		Тор	30 lb	80 lb	0 lb	0 lb	J7
8	Point	15-8-14		Тор	22 lb	0 lb	0 lb	0 lb	Wall Self Weight

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the inlended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Upoist not to be treated with fire retardant or corrosive

Handling & Installation

rranding & Installation

1. IJoist flanges must not be cut or drilled

2. Refer to latest copy of the IJoist product information
details for framing details, stiffener tables, web hole
chart, bridging details, multi-py restering details and
handling/erection details

3. Damaged IJoists must not be used

4. Design assumes top flange to be laterally restrained
by attached sheathing or as specified in engineering
notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing lengths—3.6 inches
 For flat roofs provide proper drainage to prevent

This design is valid until 7/10/2021

Manufacturer Info

Nascor by Kott



Client: **GREENPARK**

Project: Address:

Date: 9/10/2018

Designer: RO

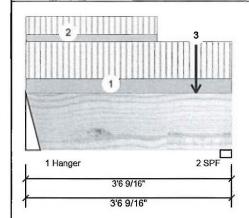
Job Name: MILLWOOD 2-ELEV 1

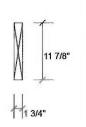
Project #

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





Page 1 of 1

Member Inform	nation						Unfacto	red React	ions U	NPATTERN	ED lb ((Uplift)	
Туре:	Girder		Application	on: Fi	loor (Resident	ial)	Brg	Live		Dead	Sno	W	Wind
Plies:	1		Design M	lethod: L	SD		1	555		218		0	0
Moisture Condition	Dry		Building (Code: N	BCC 2010 / O	BC 2012	2	510		201		0	0
Deflection LL:	360		Load Sha	aring: N	o		_						
Deflection TL:	240		Deck:	N	ot Checked								
Importance:	Normal		Vibration:	: N	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fac	tored I	Reactions			
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - Hanger	3.000"	28%	272 / 833	1106	L	1.25D+1.5L
Analysis Result	s						2-SPF	2.375"	40%	251 / 765	1016	L	1.25D+1.5L
Analysis Act	ual	Location	Allowed	Capacity	Comb.	Case							
Moment 785	ft-lb	1'9 5/16"	17130 ft-lb	0.046 (5%)	1.25D+1.5L	L					/	eessio.	
Unbraced 785	ft-lb	1'9 5/16"	13259 ft-lb	0.059 (6%)	1.25D+1.5L	L					OPE	FESSIO/	141

Actual	Location	Allowed	Capacity	Comb.	Case
785 ft-lb	1'9 5/16"	17130 ft-lb	0.046 (5%)	1.25D+1.5L	L
785 ft-lb	1'9 5/16"	13259 ft-lb	0.059 (6%)	1.25D+1.5L	L
745 lb	2'5 1/16"	5798 lb	0.129 (13%)	1.25D+1.5L	L
0.001 (L/26669)	1'9 7/16"	0.108 (L/360)	0.010 (1%)	D	Uniform
0.004 (L/10472)	1'9 3/8"	0.108 (L/360)	0.030 (3%)	L	L
0.005 (L/7520)	1'9 3/8"	0.161 (L/240)	0.030 (3%)	D+L	L
	785 ft-lb 785 ft-lb 745 lb 0.001 (L/26669) 0.004 (L/10472)	785 ft-lb 1'9 5/16" 785 ft-lb 1'9 5/16" 745 lb 2'5 1/16" 0.001 1'9 7/16" (L/26669) 0.004 1'9 3/8"	785 ft-lb 1'9 5/16" 17130 ft-lb 785 ft-lb 1'9 5/16" 13259 ft-lb 745 lb 2'5 1/16" 5798 lb 0.001 (L/26669) 0.108 (L/360) (L/10472) 1'9 3/8" 0.108 (L/360)	785 ft-lb 1'9 5/16" 17130 ft-lb 0.046 (5%) 785 ft-lb 1'9 5/16" 13259 ft-lb 0.059 (6%) 745 lb 2'5 1/16" 5798 lb 0.129 (13%) 0.001 1'9 7/16" 0.108 (L/360) 0.010 (1%) (L/26669) 0.004 (L/10472) 1'9 3/8" 0.108 (L/360) 0.030 (3%)	785 ft-lb 1'9 5/16" 17130 ft-lb 0.046 (5%) 1.25D+1.5L 785 ft-lb 1'9 5/16" 13259 ft-lb 0.059 (6%) 1.25D+1.5L 745 lb 2'5 1/16" 5798 lb 0.129 (13%) 1.25D+1.5L 0.001 (L/26669) 1'9 7/16" 0.108 (L/360) 0.010 (1/6) D 0.004 (L/10472) 1'9 3/8" 0.108 (L/360) 0.030 (3%) L



September 13, 2018

D	esi	g	n	N	0	tes	i

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Top braced at bearings.
- 4 Bottom braced at bearings

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 3-6-9		Тор	79 PLF	210 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-0-0 to 2-3-3		Near Face	39 PLF	103 PLF	0 PLF	0 PLF	
3	Point	2-11-3		Near Face	33 lb	87 lb	0 lb	0 lb	J2
	Self Weight				5 PLF				ru Framing Square los

uash Block is equired at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- IATIONING & INSTALLATION

 LVL beams must not be cut or drifted

 Refer to manufacturer's product information regarding installation requirements, mutti-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained.

 Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is va

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Project: Address: **GREENPARK**

9/7/2018 Date:

RO Designer:

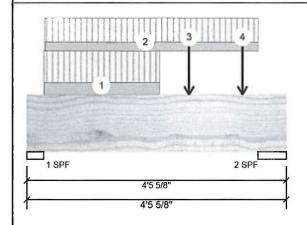
Job Name: MILLWOOD 2-ELEV 1

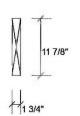
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" - PASSED

Level: Ground Floor





1.25D+1.5L

Page 1 of 1

Member Info	mation			Unfactore	d Reacti	ons UNPATTERN	ED lb (Uplift)	
Туре:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind
Plies:	1	Design Method:	LSD	1	1024	395	0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	1099	424	0	0
Deflection LL:	360	Load Sharing:	No					
Deflection TL:	240	Deck:	Not Checked					
Importance:	Normal	Vibration:	Not Checked					
General Load								
Floor Live:	40 PSF			Bearings a	and Facto	ored Reactions		
Dead:	15 PSF			Bearing L	ength.	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
				1-SPF 3	.500"	54% 493 / 1536	2029 L	1.25D+1.5L

2 - SPF 5.875"

34%

529 / 1649

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
A-07: - 811:111						
Moment	2002 ft-lb	2'1 9/16"	17130 ft-lb	0.117 (12%)	1.25D+1.5L	L
Unbraced	2002 ft-lb	2'1 9/16"	11720 ft-lb	0.171 (17%)	1.25D+1.5L	L
Shear	2314 lb	3' 5/8"	5798 lb	0.399 (40%)	1.25D+1.5L	L
Perm Defl in.	0.004 (L/10769)	2'1 11/16"	0.127 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.011 (L/4138)	2'1 11/16"	0.127 (L/360)	0.090 (9%)	L	L
TL Defl inch	0.015 (L/2989)	2'1 11/16"	0.191 (L/240)	0.080 (8%)	D+L	L



2178 L

September 13, 2018

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-3-8 to 2-3-8		Near Face	120 PLF	319 PLF	0 PLF	0 PLF	
2	Part. Uniform	0-3-12 to 3-11-12		Тор	90 PLF	240 PLF	0 PLF	0 PLF	
3	Point	2-9-8		Near Face	115 lb	305 lb	0 lb	0 lb	J7
4	Point	3-8-8		Near Face	112 lb	300 lb	0 lb	0 lb	J7
	Self Weight				5 PLF			D TI.	eu Framina 9

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Harifuling & Installation

 1. IVI, beams must not be cut or drilled

 2. Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals

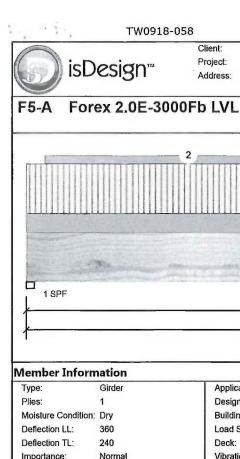
 3. Dameged Beams must not be used
- Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.







GREENPARK

Client:

Project:

Address:

Date:

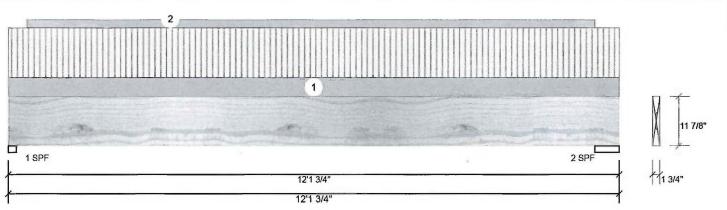
9/7/2018 Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Level: Ground Floor

Project #:

1.750" X 11.875" - PASSED



Member Infor	mation			Unfacto	red Reac	tions U	NPATTERN	ED lb ((Uplift)	
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	w	Wind
Plies:	1	Design Method:	LSD	1	73		67		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	77		70		0	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked	1						
Importance:	Normal	Vibration:	Not Checked	1						
General Load										
Floor Live:	40 PSF			Bearing:	and Fac	tored	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1-SPF	1.875"	10%	83 / 109	192	L	1.25D+1.5L
				2-SPF	5.875"	3%	88 / 115	203	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	553 ft-lb	5'10 7/8"	17130 ft-lb	0.032 (3%)	1.25D+1.5L	L
Unbraced	553 ft-lb	5'10 7/8"	3868 ft-lb	0.143 (14%)	1.25D+1.5L	L
Shear	158 lb	1'1"	5798 lb	0.027 (3%)	1.25D+1.5L	L
Perm Defl ir	n. 0.011 (L/13107)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.012 (L/12101)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
TL Defi inch	0.022 (L/6292)	5'10 7/8"	0.581 (L/240)	0.040 (4%)	D+L	L
			,	,	,	



September 13, 2018

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Top braced at bearings.
- 3 Bottom braced at bearings.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-1-12	(Span)0-7-6	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Part. Uniform	0-4-6 to 11-7-15		Тор	2 PLF	0 PLF	0 PLF	0 PLF	
	Self Weight				5 PLF				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damsged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

This design is

Manufacturer Info

Forex APA: PR-L318

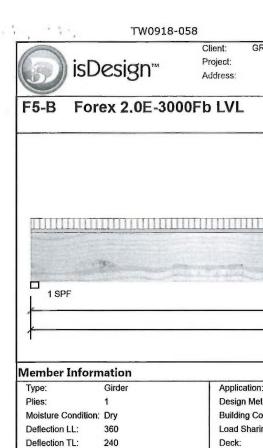
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Ontario Canada K2H7V1 905-642-4400



Version 18.40.162 Powered by iStruct™





15 PSF

GREENPARK

Date:

9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

Level: Ground Floor

Project #:

1.750" X 11.875" - PASSED

2

2 SPF

12'2 3/4 12'2 3/4'

Unfactored	Reactions !	JNPATTER	(Uplift)

Type:	Girder	Application:	Floor (Residential)	Brg	Live	Dead	Snow	Wind	
Plies:	1	Design Method:	LSD	1	67	53	0	0	
Moisture Condition	: Dry	Building Code:	NBCC 2010 / OBC 2012	2	95	66	0	0	
Deflection LL:	360	Load Sharing:	No						
Deflection TL:	240	Deck:	Not Checked						
Importance:	Normal	Vibration:	Not Checked						
General Load									_
Floor Live:	40 PSF			Bearing	s and Factore	d Reactions			

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	1.875"	8%	67 / 101	168	L	1.25D+1.5L
2-SPF	6.875"	3%	82 / 142	224	L	1.25D+1.5L

Analysis Results

Dead:

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	480 ft-lb	5'10 7/8"	17130 ft-lb	0.028 (3%)	1.25D+1.5L	L
Unbraced	480 ft-lb	5'10 7/8"	3868 ft-lb	0.124 (12%)	1.25D+1.5L	L
Shear	137 lb	1'1"	5798 lb	0.024 (2%)	1.25D+1.5L	L
Perm Defl in.	0.008 (L/16496)	5'10 7/8"	0.388 (L/360)	0.020 (2%)	D	Uniform
LL Defl inch	0.011 (L/13065)	5'10 7/8"	0.388 (L/360)	0.030 (3%)	L	L
TL Defl inch	0.019 (L/7290)	5'10 7/8"	0.581 (L/240)	0.030 (3%)	D+L	L

T.L. WISE 100083566 WCE OF ONTE September 13, 2018

Design Notes

1 Girders are designed to be supported on the bottom edge only.

2 Top braced at bearings.

3 Bottom	braced at bearings.								
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 12-2-12	(Span)0-6-14	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	11-9-6 to 12-2-12	(Span)2-6-2	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Self Weight 5 PLF

> Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

Manufacturer Info Forex

APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Client: **GREENPARK**

Project: Address:

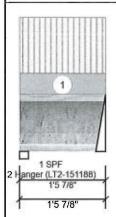
Date: Designer: 9/7/2018 RO

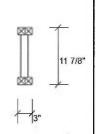
MILLWOOD 2-ELEV 1 Job Name:

Project #:

11.875" 2-Ply - PASSED NJ

Level: Ground Floor





Page 1 of 1

Member Inf	ormation						Unfactored Reactions UNPATTERNED lb (Uplift)						
Туре:	Girder		Applicatio	n: F	loor (Residenti	al)	Brg	Live		Dead	Snov	N	Wind
Plies:	2		Design M	ethod: L	SD		1	48		18		0	0
Moisture Cond	ition: Dry		Building C	Code: N	BCC 2010 / O	BC 2012	2	49		18		0	0
Deflection LL:	360		Load Sha	ring: N	o								
Deflection TL:	240		Deck:	N	ot Checked								
Importance: General Load	Normal		Vibration:	N	ot Checked								
Floor Live:	40 PSF						Bearing	s and Fact	ored	Reactions			
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - SPF	1.875"	4%	23/72	95	L	1.25D+1.5L
Analysis Res	ults						2 - Hanger	2.000"	4%	23 / 73	96	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case						_	
Moment	27 ft-lb	8 7/8"	9020 ft-lb	0.003 (0%)	1.25D+1.5L	L					/	FESSIO/	V.
Unbraced	27 ft-lb	8 7/8"	8539 ft-lb	0.003 (0%)	1.25D+1.5L	L					PR	-	13/2
Shear	83 lb	1 1/8"	3400 lb	0.024 (2%)	1.25D+1.5L	L				/	8/		E 86
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						(5	× L		- 7
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)						[3	3 1	L. WIS	E E
TI Doff inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)							1	0008350	66

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

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ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

NOTES

Caculated Shuctured Designs Is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 Uplish not to be treated with fire retardant or corre

Handling & Installation

- Traindling & Installation

 1. Joist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, stiffener tables, web hole chart, bridging details, multi-ply fastlening details and handling/erection details

 3. Damaged Joist must not be used

 4. Design assumes top flange to be laterally restrained by attached sheathing or as specified in engineering notes.

- Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing leadins 3.5 Inches.
 For Bat roofs provider READ ALL NOTES ON

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED

IN THE DESIGN OF THIS COMPONENT.

Manufacturer Info

Nascor by Kott

This design is



isDesign™

Client: **GREENPARK**

Project: Address:

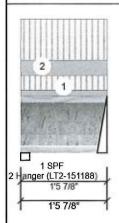
Date: 9/7/2018 Designer:

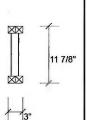
Job Name: MILLWOOD 2-ELEV 1

Project #

11.875" 2-Ply - PASSED NJ

Level: Ground Floor





Page 1 of 1

Member Infor	mation			Unfactored Reactions UNPATTERNED Ib (Uplift)						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Sno	w	Wind
Plies:	2	Design Method:	LSD	1	65		24		0	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	66		25		0	0
Deflection LL:	360	Load Sharing:	No	-						
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearing	s and Fac	tored F	Reactions			
Dead:	15 PSF			Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - SPF	1.875"	5%	31 / 98	128	L	1.25D+1.5L
				2-	2.000"	5%	31/99	130	L	1.25D+1.5L
ınalysis Resul	nalysis Results			Hanger						

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	36 ft-lb	8 7/8"	9020 ft-lb	0.004 (0%)	1.25D+1.5L	L	
	Unbraced	36 ft-lb	8 7/8"	8539 ft-lb	0.004 (0%)	1.25D+1.5L	L	
	Shear	112 lb	1 1/8"	3400 lb	0.033 (3%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/O)	0.000 (0%)			

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top flange braced at bearings.
- 6 Bottom flange braced at bearings.

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September 13, 2018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 1-5-14	(Span)1-1-15	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-5-14	(Span)3-3-0	Тор	15 PSF	40 PSF	0 PSF	0 PSF	

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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Handling & Installation

- Handling & Installation

 1. IJoist flanges must not be cut or drilled

 2. Refer to latest copy of the IJoist product information details for framing details, etiffener tables, web hole tohart, bridging details, multi-ply fastening details and handling/erection details

 3. Damaged IJoist must not be used

 4. Design assumes top flange to be laterally restrained by attached shealthing or as specified in engineering notes.

Provide lateral support at bearing points to avoid lateral displacement and rotation
 Web stiffeners for point load as shown Minimum point load bearing les chib=3.5 inches
 For flat roofs providing

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Manufacturer Info

Nascor by Kott

This design is



Client:

Project:

Address:

GREENPARK

Date: 9/10/2018

Designer: RO

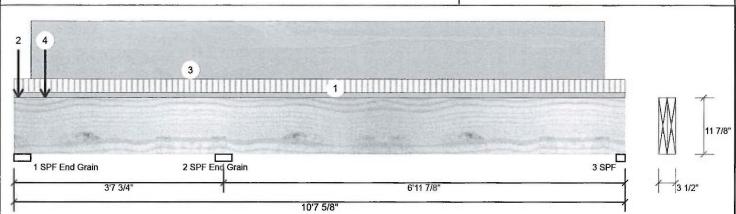
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Ground Floor



Type: G	Girder		
	siidei	Application:	Floor (Residential)
Plies: 2	2	Design Method:	LSD
Moisture Condition: D	Ory	Building Code:	NBCC 2010 / OBC 2012
Deflection LL: 3	360	Load Sharing:	No
Deflection TL: 2	240	Deck:	Not Checked
Importance: N	Normal	Vibration:	Not Checked
General Load			
Floor Live: 4	IO PSF		
Dead: 1	15 PSF		

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Neg Moment	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Unbraced	-617 ft-lb	3'7 3/4"	22269 ft-lb	0.028 (3%)	1.4D	Uniform
Pos Moment	521 ft-lb	7'9"	22269 ft-lb	0.023 (2%)	1.4D	Uniform
Unbraced	521 ft-lb	7'9"	21873 ft-lb	0.024 (2%)	1.4D	Uniform
Shear	421 lb	4'7 5/8"	7537 lb	0.056 (6%)	1.4D	Uniform
Perm Defl in.	0.005 (L/17692)	7'3 7/16"	0.230 (L/360)	0.020 (2%)	D	Uniform

0 999.000 (L/0) 0.000 (0%) LL Defl inch 0.000 (L/999) TL Defl inch 0.006 7'3 3/8" 0.345 (L/240) 0.020 (2%) D+L (L/14724)

Design Notes

Analysis Results

- 1 Performed Secondary Bearing Check (CSA 086-14 6.5.7.3). Assumed point load size: beam width X 4.5.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

Unfactored Reactions UNPATTERNED Ib (Uplift)

ID-a	Live	Dead	Snow	Wind
Brg	Live	Dead	SHOW	AAILIC
1	986	484	0	0
2	176	679	0	0
3	52	259	0	0

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	24%	578 / 1504	2082	L_	1.25D+1.5L
2 - SPF End Grain	3.500"	17%	995 / 0	995	Uniform	1.4D
3-SPF	1.875"	13%	348/0	348	Uniform	1.4D



September 13, 2018

Notes

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Handling & Installation

LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, mutti-ply fastening details, beam strength values, and code

approvals
Damaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

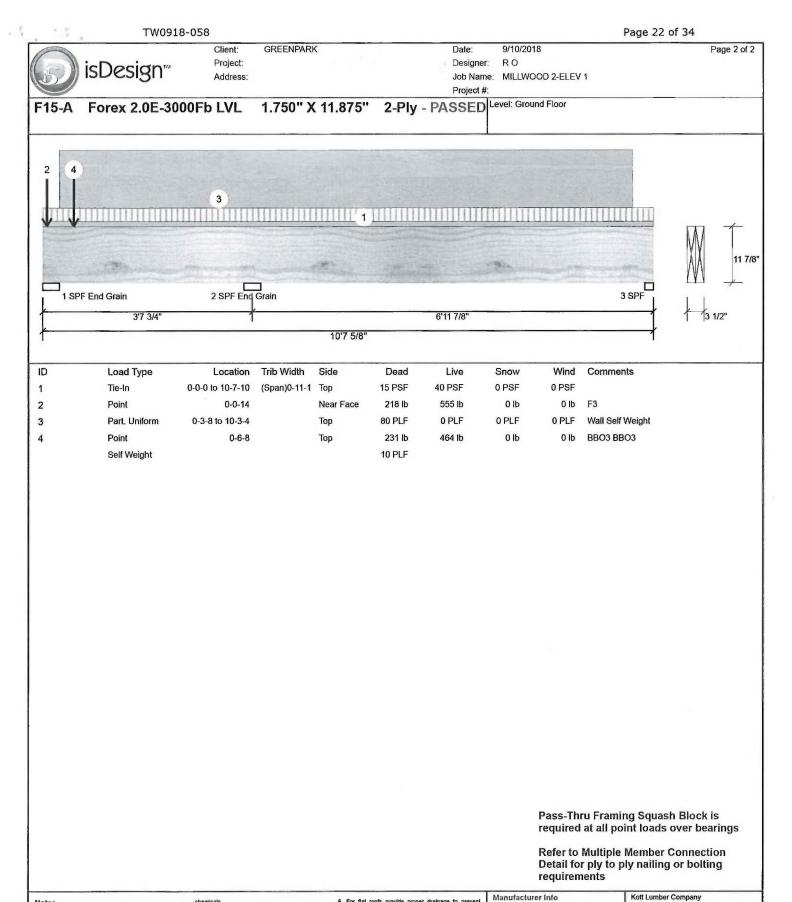
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Manufacturer Info

APA: PR-L318

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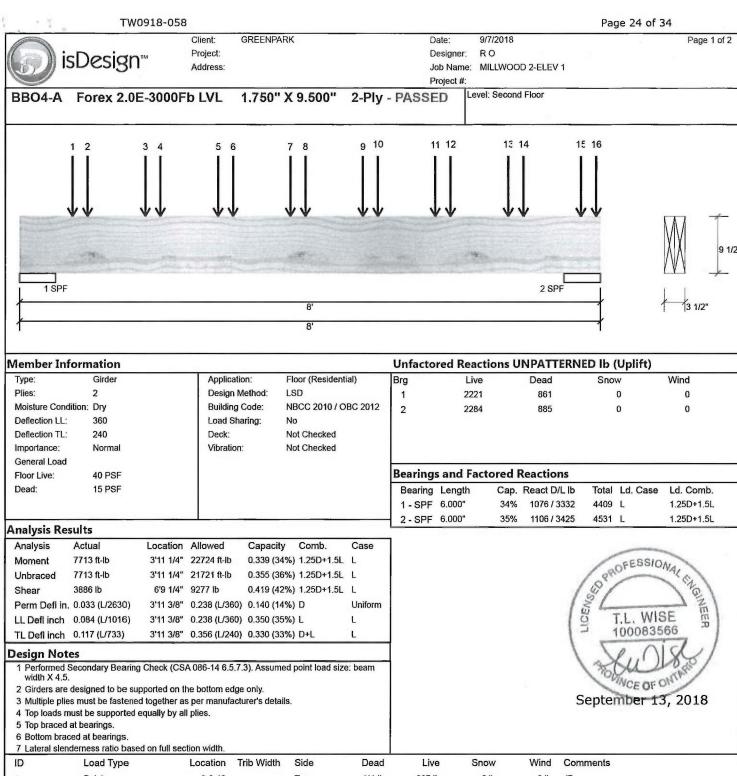
Notes

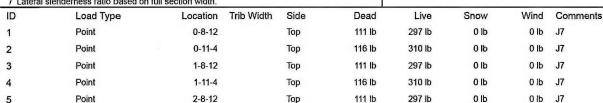
Lumber

6. For flat roofs provide proper drainage to prevent

This design is valid until 7/10/2021

Forex APA: PR-L318





Continued on page 2...

Notes

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Lumber

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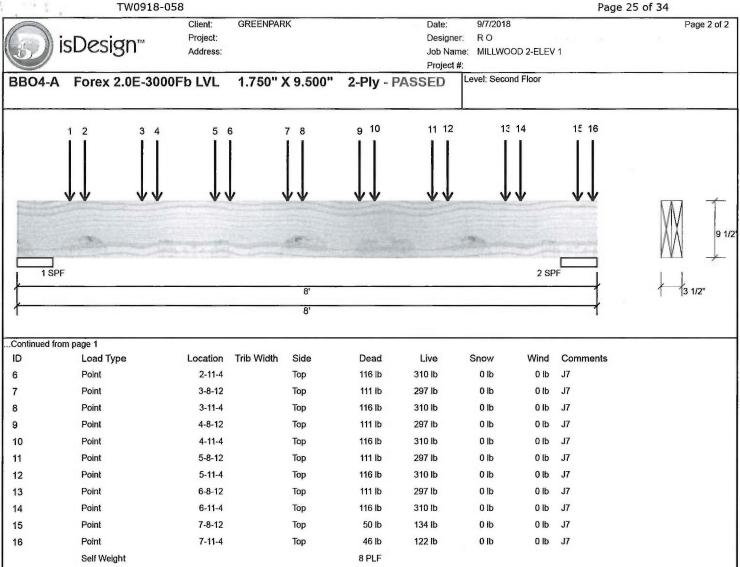
6. For flat roofs provide proper drainage to prevent

Manufacturer Info APA: PR-L318

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Kott Lumber Company 14 Anderson Blvd, Ontario K2H7V1





Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

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Provide tateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 7/10/2021

For flat roofs provide proper drainage to prevent conding

Manufacturer Info

Forex APA: PR-L318



Project: Address: **GREENPARK**

Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

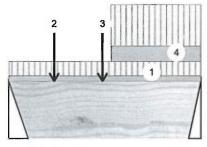
Project #:

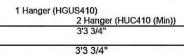
Forex 2.0E-3000Fb LVL

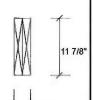
1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor







Wind

0

0

Page 1 of 1

Member Info	rmation			Unfactor	ed React	ions U	NPATTERN	ED lb (Uplift)
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	V
Plies:	2	Design Method:	LSD	1	150		83		0
Moisture Condition	on: Dry	Building Code:	NBCC 2010 / OBC 2012	2	169		88		0
Deflection LL:	360	Load Sharing:	No	1 -					
Deflection TL:	240	Deck:	Not Checked	1					
Importance:	Normal	Vibration:	Not Checked						
General Load									
Floor Live:	40 PSF			Bearings	and Fact	ored F	Reactions		
Dead:	15 PSF			Bearing I	Length	Сар.	React D/L lb	Total	Ld. Case

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Unbraced	246 ft-lb	1'7 1/2"	34261 ft-lb	0.007 (1%)	1.25D+1.5L	L
Shear	246 lb	1'3 1/8"	11596 lb	0.021 (2%)	1.25D+1.5L	L
Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.001 (L/46486)	1'7 3/4"	0.145 (L/240)	0.010 (1%)	D+L	L

ns

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	3%	104 / 225	329	L	1.25D+1.5L
2 - Hanger	2.500"	6%	110 / 253	363	L	1.25D+1.5L

Design Notes

1 Fill all hanger nailing holes.

- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- hased on full section width



/ Laterai	sienderness ratio based	on full section wiath.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 3-3-12	(Span)1-4-8	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-9-7		Near Face	26 lb	70 lb	0 lb	0 lb	J2
3	Point	1-7-8		Near Face	33 lb	35 lb	0 lb	Passill	nru-Framing Squash Block is d at all point loads over bearings
4	Tie-In	1-9-4 to 3-3-12	(Span)	Тор	15 PSF	40 PSF	0 PSF	0 PSF	a at all point loads over bearings
	Self Weight		3-11-13		10 PLF				o Multiple Member Connection or ply to ply nailing or bolting ments

Notes

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Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE **CONTAINS SPECIFICATIONS AND CRITERIA USED** IN THE DESIGN OF THIS COMPONENT.

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT



GREENPARK

Project: Address: Date: 9/7/2018

Designer: RO

Job Name: MILLWOOD 2-ELEV 1

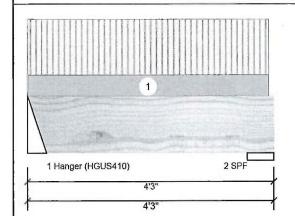
Project #:

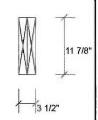
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 1

Member In	formation						Unfacto	red React	tions U	NPATTERN	ED lb ((Uplift)	
Туре:	Girder	v	Applicati	on: F	loor (Residenti	al)	Brg	Live		Dead	Sno	w	Wind
Plies:	2		Design N	Method: L	SD		1	35		33		0	0
Moisture Con-	dition: Dry		Building	Code: N	BCC 2010 / OI	BC 2012	2	35		34		0	0
Deflection LL:	360		Load Sha	aring: N	o								
Deflection TL:	240		Deck:	N	ot Checked		1						
Importance:	Normal		Vibration	: N	ot Checked								
General Load													
Floor Live:	40 PSF						Bearing	s and Fac	tored	Reactions			
Dead:	15 PSF						Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - Hanger	4.000"	1%	41 / 52	93	L	1,25D+1,5L
Analysis Re	sults						2-SPF	5.500°	1%	43 / 53	95	L	1.25D+1.5L
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case						_	
Moment	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L					/	OFESSIC	MA.
Liphrocod	72 ft lb	2' 3/4"	3/1261 ft-lh	0.002 (0%)	1 25D+1 5I	1					1 08	-	10

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L
Unbraced	72 ft-lb	2' 3/4"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L
Shear	36 lb	1'3 1/8"	11596 lb	0.003 (0%)	1.25D+1.5L	L
Perm Defl in	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
LL Defl inch	0.000 (L/999)	0	999,000 (L/0)	0.000 (0%)		
TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		



September 13, 2018

Design Notes

- 1 Fill all hanger nailing holes.
- 2 Girders are designed to be supported on the bottom edge only.
- 3 Multiple plies must be fastened together as per manufacturer's details.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on full section width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-0-0 to 4-1-14	(Span)0-10-1	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
	Colf Maight				10 DI C				

Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

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Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

For flat roofs provide proper drainage to prevent ponding

This design is

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE CONTAINS SPECIFICATIONS AND CRITERIA USED

ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT IN THE DESIGN OF THIS COMPONENT.





Project: Address: **GREENPARK**

Date: 9/7/2018

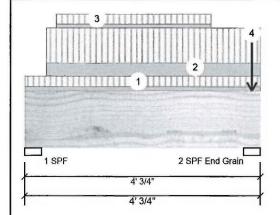
RO Designer:

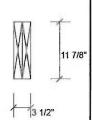
Job Name: MILLWOOD 2-ELEV 1

Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875" 2-Ply - PASSED Level: Second Floor





Wind

Page 1 of 1

Member Inform	nation		
Туре:	Girder	Application:	Floor (Residential)
Plies:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2010 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked
General Load			
Floor Live:	40 PSF		
Dead:	15 PSF		

1	643	260	0	0
2	885	375	0	0
-				

Dead

Snow

Unfactored Reactions UNPATTERNED Ib (Uplift)

Analysis Results Location Allowed Case **Analysis** Actual Capacity Comb. Moment 1215 ft-lb 2' 5/16" 34261 ft-lb 0.035 (4%) 1.25D+1.5L L Unbraced 1215 ft-lb 2' 5/16" 34261 ft-lb 0.035 (4%) 1.25D+1.5L L Shear 711 lb 2'10 1/8" 11596 lb 0.061 (6%) 1.25D+1.5L L Perm Defl in. 0.001 2' 5/16" 0.120 (L/360) 0.010 (1%) D Uniform (L/34561) 0.003 2' 5/16" 0.120 (L/360) 0.030 (3%) L L LL Defl inch (L/13799) TL Defl inch 0.004 (L/9862) 2' 5/16" 0.180 (L/240) 0.020 (2%) D+L

Bearings and Factored Reactions

Live

Brg

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF	3.500"	17%	325 / 965	1289	L	1.25D+1.5L	
2 - SPF End Grain	3.500"	20%	469 / 1328	1797	L	1.25D+1.5L	



- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.

Load Type

Part. Uniform

Part. Uniform

Self Weight

Tie-In

Point



CHOW	AAIIIG	Comments
0 PSF	0 PSF	
0 PLF	0 PLF	
0 PLF		ru Framing Squash Block is
0 lb	required	d at all point loads over bearings
	Refer to	Multiple Member Connection

Detail for ply to ply nailing or bolting

Wind Comments

requirements

Notes

ID

2

3

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Location

0-0-0 to 4-0-12

0-4-8 to 4-0-12

0-6-8 to 3-2-8

3-11-0

Trib Width

(Span)3-7-4

Side

Top

Top

Near Face

Near Face

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Daringed seams into this december of the decem

Dead

15 PSF

90 PLF

25 PLF

10 PLF

This design is

88 lb

Live

40 PSF

240 PLF

68 PLF

169 lb

Manufacturer Info Forex APA: PR-L318

Snow

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

Kott Lumber Company 14 Anderson Blvd, Onlario Canada K2H7V1 905-642-4400

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Client: **GREENPARK**

Project: Address:

Date: 9/7/2018

RO Designer:

Job Name: MILLWOOD 2-ELEV 1

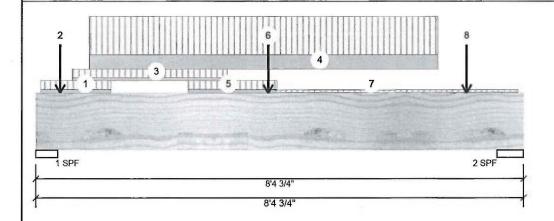
Project #:

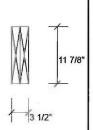
Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED

Level: Second Floor





Page 1 of 2

Member In	formation
-----------	-----------

•	WEIGHDEN THIOTH	ution
	Type:	Girder
	Plies:	2
	Moisture Condition:	Dry
	Deflection LL:	360
	Deflection TL:	240
	Importance:	Normal
	General Load	
	Floor Live:	40 PSF
	Dead:	15 PSF

Application: Design Method:

Building Code:

Vibration:

Floor (Residential) LSD NBCC 2010 / OBC 2012

Load Sharing: No Deck:

Not Checked Not Checked

Unfactored Reactions UNPATTERNED Ib (Uplift)

Brg	Live	Dead	Snow	Wind
1	1594	649	0	0
2	1377	568	0	0

Bearings and Factored Reactions

Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	4.500"	33%	811 / 2391	3202	L	1.25D+1.5L
2-SPF	5.500"	23%	710 / 2066	2776	L	1.25D+1.5L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5966 ft-lb	4'	34261 ft-lb	0.174 (17%)	1.25D+1.5L	L
Unbraced	5966 ft-lb	4'	31511 ft-lb	0.189 (19%)	1.25D+1.5L	L
Shear	3035 lb	7' 1/8"	11596 lb	0.262 (26%)	1.25D+1.5L	L
Perm Defl in.	0.016 (L/5638)	4' 3/4"	0.256 (L/360)	0.060 (6%)	D	Uniform
LL Defl inch	0.040 (L/2325)	4' 13/16"	0.256 (L/360)	0.150 (15%)	L	L
TL Defl inch	0.056 (L/1646)	4' 13/16"	0.384 (L/240)	0.150 (15%)	D+L	L

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width

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Sent	ember 13. 2	018

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Tie-In	0-1-0 to 1-3-8	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
2	Point	0-5-0		Near Face	77 lb	205 lb	0 lb	0 lb	J7
3	Part. Uniform	0-7-8 to 3-3-8		Far Face	25 PLF	68 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-11-0 to 6-11-0		Near Face	115 PLF	308 PLF	0 PLF	0 PLF	
5	Tie-In	2-7-8 to 4-1-12	(Span)3-7-4	Тор	15 PSF	40 PSF	0 PSF	0 PSF	
6	Point	4-0-0		Far Face	83 lb	150 lb	0 lb	0 lb	F6

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals.
- approvals
 Damaged Beams must not be used
 Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

6. For flat roofs provide proper drainage to proporting

Manufacturer Info Forex APA: PR-L318

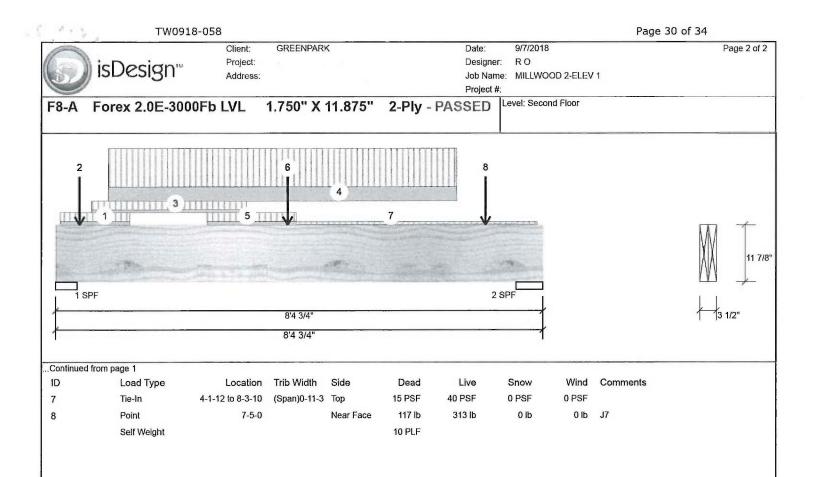
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

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Pass-Thru Framing Squash Block is required at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or continuous.

Handling & Installation

Handling & Installation

1. LVL beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 7/10/2021

Forex

Manufacturer Info

APA: PR-L318



Second Floor

Page 31 of 34

Layout Name

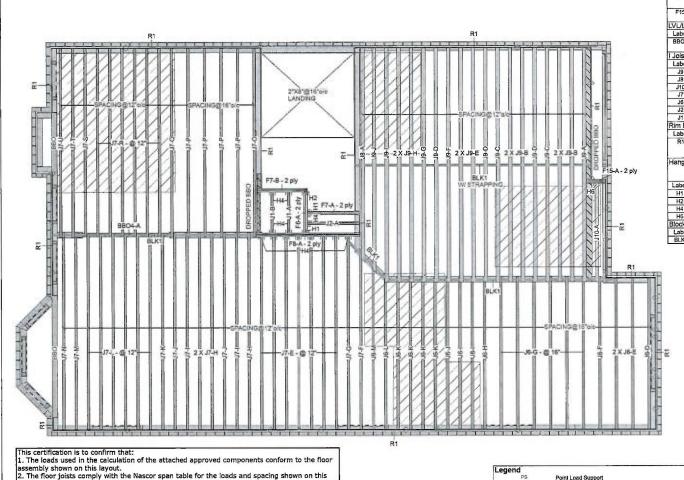
Design Method

150

Description

Created

MILLWOOD 2-ELEV 2



Width Depth Qty Plies Pcs Length F8 1.75 2 2.0E-3000Fb LVI 6-0-0 2.0E-3000Fb LVL F6 11.875 4-0-0 Forex 2.0E-3000Fb LVL 2 F15 Forex 2.0E-3000Fb LVL 1.75 11.875 2-0-0 LVL/LSL (Dropped) Width Depth Qty Plies Pcs Length Label Description BBO4 Forex 9.5 2.0E-3000Fb LVL I Joist (Flush) Pcs Length 18 20-0-0 Label Description Width Depth Qty Plies 3.5 11.875 J9 NJ60U 3.5 11.875 3.5 11.875 J8 NJ60U 1 8-0-0 J10 NJ60U J7 NJH 2.5 11.875 2.5 11.875 2.5 11.875 40 16-0-0 J6 NJH J2 NJH 20 14-0-0 2.5 11.875 J1 NJH Rim Board Pcs Length Width Depth Qty Plies Label Description Norbord Rimboard 1.125 11.875 Plus 1 125 X 11.875 Beam/Girder Supported Member

Label Pcs Description Skow Slope fasteners fasteners 46 16d H1 2 HGUS410 H2 1 HUC410 (Min) H4 13 LT251188 6 10d 4 10dx1 1/2 2 10dx1 1/2 H6 1 LT351188 4 10dx1 1/2 2 10dx1 1/2 Blocking

Label Description
 Width
 Depth
 Qty
 Plies
 Pcs
 Length

 2.5
 11.875
 LinFt
 Varies
 40-0-0
 RIK1 NJH

NOTES:

Second Floor

LVL/LSL (Flush)

Label Description

- f. Framer to verify dimensions on the architectural drawings. 2. Double joist only require filler/backer ply when supporting
- another member using a face-mounted hanger. Install 2x4 blocking @ 24°o/c under parallel non-load bearing walls
- . Install single-ply flush window header along inside face of
- rimboard/rimjoist.

 5. Refer to Nascor specifier guide for installation works.

 6. Squash blocks recommended to be installed at end bearing on
- all first level joists which support loading from above exceeding two levels floor or roof
- Load transfer blocks to be installed under all point loads. 5. It shall be the frame's responsibility that floor joists and beams are fastened as per the hanger manufacturer's standards.

Refer to Multiple Member Connection Detail to ply to ply nailing or bolting requirements.

Rim parallel to joists: 1-1/8" rimboard with 2"x4" block (1/16" longer than nm depth @ 16°o(c). All other components and structural elements supporting the floor system such as beams, walls, columns, and foundation walls, and footings including anchorage of components and bracing for lateral stability are the responsibility of Others.

Hatch are represents ceramic tiled floor with an additional dead load

The framing shown on this layout may deviate from the architectural and structural drawings. Project Engineer to review and apporve the deviation prior to construction.

ARCHITECTURAL DRAWINGS:

JARDIN DESIGN GROUP INC. 64 Jardin Dr. Suite 3A Date: Rev. 1, 4/26/2018 Project No: 2645 Model: Millwood 2, Elevation 2

PS

0

1111111

111111

Point Load Support

Norbord Rimboard Plus 1.125 X 11 875

Forex 2.0E-3000Fb LVL 1.75 X 11.875

Forex 2.0E-3000Fb LVL 1,75 X 9.5 (Dropped)

Load from Above

Wall Opening

NJ60U 11.875

NJH 11 875

NJ 11.875

- 1. OBC 2012 O.Reg 332/12 as amended
- 2. Nascor CCMC 13535-R
- 3. LVL CCMC -14056-R
- 4. CAN/CSA-086-09
- 5. CCMC -12787-R APA PR-L310(C)

POFESSION T.L. WISE 100083566

Celling:

KOT

September 13, 2018

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The floor system must be assembled in accordance to the Nascor Specifier Guide. Multi-ply

members must be attached together as per the included multiple member connection detail.

All other components and structural elements supporting the floor system such as beams,

walls, columns and foundation walls and footings including anchorage of components and

bracing for lateral stability are the responsibility of others.

This layout is to be used as an installation guide only. It is meant to be used in conjunction with the architectural and structural drawings, not to replace them

June 25, 2018 8-0-0 Builder GREENPARK Sales Rep RM 1 18-0-0 Designer RO Shipping 1 6-0-0 Project 2 4-0-0 Builder's Project Kott Lumber Company 14 Anderson Blvd Stouffville, Ontano Canada K2H7V1

905-642-4400 Job Path S;\CUSTOMERS\GREENPARK MINNISALE HOMES/MODELS MILLWOOD 2/FLOORS/ELEV 2

MILLWOOD 2-ELEV 2.isl Second Floor LSD Design Method Building Code NBCC 2010 / OBC

Floor Loads Live 40 Dead 15 Deflection Joist LL Span L/ 480 TL Span L/ 360 480 LL Cant 2L/ TL Cant 2L/ 360 Deflection Girder 360 LL Span L/ TL Span L/ 240 LL Cant 2L/ 480 TL Cant 2L/ 240 Decking SPF Plywood Deck Thickness 5/8" Nailed & Glued Fastener Vibration

WCE OF ONT

Gypsum 1/2"



Project: Address: GREENPARK

9/7/2018 Date:

Designer:

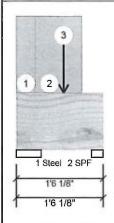
Job Name: MILLWOOD 2-ELEV 2

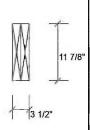
Project #:

Forex 2.0E-3000Fb LVL

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Infor	mation			Unfactore	Unfactored Reactions UNPATTERNED lb (Uplift)					
Type:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snow	V	Wind
Plies:	2	Design Method:	LSD	1	72		103	C)	0
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	61		53	()	0
Deflection LL:	360	Load Sharing:	No							
Deflection TL:	240	Deck:	Not Checked							
Importance:	Normal	Vibration:	Not Checked							
General Load										
Floor Live:	40 PSF			Bearings a	nd Fact	ored Re	eactions			
Dead:	15 PSF			Bearing Le	ength	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 - Steel 5.	250"	2%	129 / 108	237	L	1.25D+1.5L
	_			2 - SPF 2.	375"	3%	66 / 92	158	L	1.25D+1.5L

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	1
	Moment	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	78 ft-lb	10"	33233 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	107 lb	1'4 3/8"	11248 lb	0.009 (1%)	0.9D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
_								_

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width.



September 13, 2018

O Lutturas e	diction index idea bacon c	orr ran bodelor, main.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-4-0		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-4-0 to 1-1-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-10-0		Near Face	50 lb	133 lb	0 lb	0 lb	J10
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

This design

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- Handling & Installation

 1. LVL beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastering details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design essumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent

Manufacturer Info Forex APA: PR-L318

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-2. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





GREENPARK

Project: Address: Date: 9/7/2018

Designer: RO

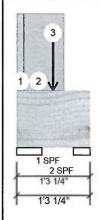
Job Name: MILLWOOD 2-ELEV 3

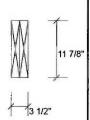
Project #:

Forex 2.0E-3000Fb LVL F15-A

1.750" X 11.875"

2-Ply - PASSED Level: Second Floor





Page 1 of 1

Member Infor	mation			Unfactor	Unfactored Reactions UNPATTERNED Ib (Upli						
Туре:	Girder	Application:	Floor (Residential)	Brg	Live		Dead	Snov	W	Wind	
Plies:	2	Design Method:	LSD	1	108		101		0	0	
Moisture Condition	n: Dry	Building Code:	NBCC 2010 / OBC 2012	2	65		41		0	0	
Deflection LL:	360	Load Sharing:	No								
Deflection TL:	240	Deck:	Not Checked								
Importance:	Normal	Vibration:	Not Checked								
General Load											
Floor Live:	40 PSF			Bearings	and Fact	tored l	Reactions				
Dead:	15 PSF			Bearing	Length	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.	
				1 - SPF	5.250"	3%	126 / 162	288	L	1.25D+1.5L	
				2-SPF 3	3.500"	2%	52 / 98	149	L	1.25D+1.5L	

Analysis Results

	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	
	Moment	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Unbraced	58 ft-lb	7 1/2"	34261 ft-lb	0.002 (0%)	1.25D+1.5L	L	
	Shear	190 lb	1'4 3/8"	11596 lb	0.016 (2%)	1.25D+1.5L	L	
	Perm Defl in.	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
	TL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)			
_								_

Design Notes

- 1 Girders are designed to be supported on the bottom edge only.
- 2 Multiple plies must be fastened together as per manufacturer's details.
- 3 Top loads must be supported equally by all plies.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on full section width



September 13, 2018

O Lateral	sicilaciness ratio basea c	ATTUIL SCOROLL MIGHT.							
ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Part. Uniform	0-0-0 to 0-1-8		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
2	Part. Uniform	0-1-8 to 0-9-12		Тор	80 PLF	0 PLF	0 PLF	0 PLF	Wall Self Weight
3	Point	0-7-8		Far Face	65 lb	173 lb	0 lb	0 lb	J9
	Self Weight				10 PLF				nru Framing Squash Block is d at all point loads over bearings

Refer to Multiple Member Connection Detail for ply to ply nailing or bolting requirements

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the Contractor to ensure the component suitability of the Intended application, and to verify the dimensions and loadis.

Lumber

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corr

Handling & Installation

- Handling & Installation

 1. IVI. beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Forex APA: PR-L318

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