

Building Code O. Reg. 332/12 as amended
All work shall conform to the Ontario Building Code



Layout based on plans provided by
JARDIN DESIGN GROUP INC
Dated April 26/18

CITY OF BRAMPTON
BUILDING DIVISION
REVIEWED
DEC 20 2018
MARK DERKSEN

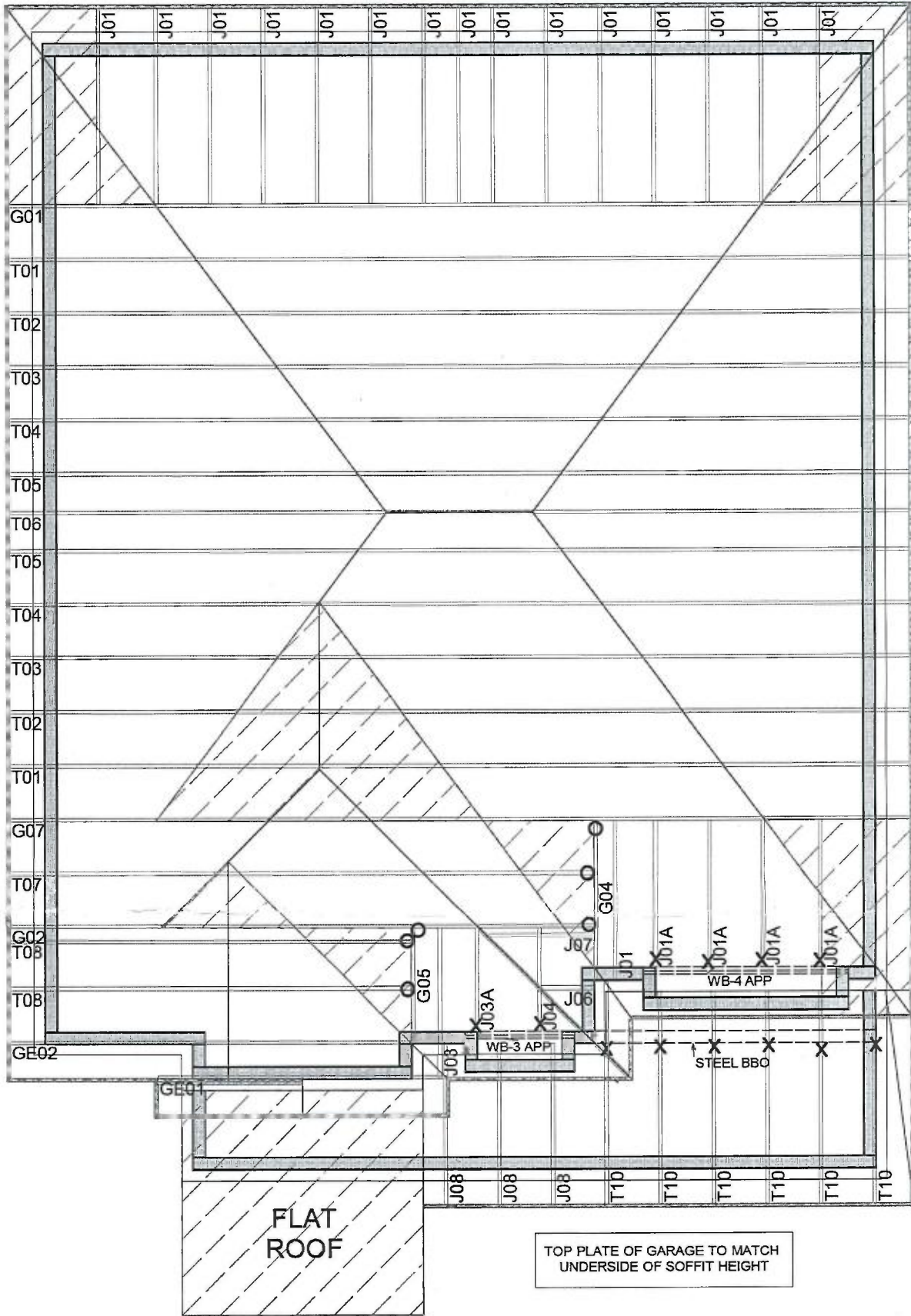
This certification is for the engineering review of structural components shown on this drawing and is limited to the components supplied by KOTT for the loads and conditions shown on the calculation page of each component. The placement of these components on this layout is shown for reference only. Roof trusses must be installed according to manufacturer's guidelines at 24" C/C unless noted otherwise. Conventional framing shown on the layout must be designed and approved by the project engineer. Temporary and permanent bracing is to conform to BCSI guidelines.



September 14, 2018

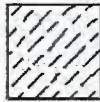
TR - GREENPARK - MINNISALE HOMES - MILLWOOD 1 ELE 3

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IM0918-066



ROOF TRUSS LAYOUT

LOT 5



CONVENTIONAL
FRAMING BY OTHERS

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

DESIGN CRITERIA
SPECIFIED LOADS
TOP CH. LL = 23.3 psf
DL = 3.0 psf
BOT CH. LL = 0.0 psf
DL = 7.0 psf
TOTAL LOAD = 33.3 psf

SPACING @ 24" CENTERS,
THIS TRUSS SYSTEM IS DESIGN FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH PART 9 OF OBC 2012,
BCBC 2012, ABC 2012, CSA 086-09, TPIC 2011

ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	MILLWOOD 1 EL 3
Customer:	GREENPARK
Project:	MINNISALE HOMES
Location:	BRAMPTON
Drawn By:	DM
Date:	9/17/2018

SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS
MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTER THAT CROSS MEET
OVER TRUSSES TO BE 2X4 SPF @24" O/C WITH VERTICAL POST TO THE TRUSS BELOW EACH CROSS POINT.
VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT
AND ROWS OF BRACING DOES NOT EXCEED 6'.

LOT 5

ENGINEERING NOTE PAGE (ENP-1)
PLEASE READ PRIOR TO INSTALLATION

RESPONSIBILITIES

THIS DESIGN IS FOR AN INDIVIDUAL BUILDING COMPONENT AND HAS BEEN BASED ON INFORMATION PROVIDED BY THE DESIGN OFFICE OF KOTT LUMBER. THE UNDERSIGNED ENGINEER DISCLAIMS ANY RESPONSIBILITY FOR DAMAGES AS A RESULT OF FAULTY OR INCORRECT INFORMATION, SPECIFICATION AND/OR DESIGNS FURNISHED TO THE ENGINEER. THE UNDERSIGNED ENGINEER IS ONLY RESPONSIBLE FOR THE STRUCTURAL INTEGRITY OF THIS BUILDING COMPONENT FOR THE CONDITIONS AND LOADS SHOWN ON THIS DRAWING. THE STRUCTURAL INTEGRITY OF THE BUILDING AND THE VERIFICATION OF THE DIMENSIONS AND THE DESIGN LOADS USED ARE THE RESPONSIBILITY OF THE BUILDING DESIGNER.

TRUSSES ARE DESIGNED IN CONFORMANCE WITH THE RELEVANT SECTIONS OF THE NATIONAL BUILDING CODE OF CANADA OR THE CANADIAN CODE FOR FARM BUILDINGS, WHICHEVER APPLIES TO THE BUILDING TYPE INDICATED ON THE DRAWING

IT IS THE RESPONSIBILITY OF KOTT LUMBER TO ENSURE THAT TRUSSES ARE MANUFACTURED IN CONFORMANCE WITH THESE DESIGNS AND WITH THE SPECIFICATIONS OUTLINED BELOW. THE UNDERSIGNED ENGINEER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

USE AND OCCUPANCY

- The building is of the type indicated on the drawing

LOADING

- The truss loading intensity and distribution as well as load transfer mechanism is that indicated on the drawing
- No buildings, trees, parapets or other projections higher than the roof for which the trusses are used are located within a distance less than ten (10) times the difference in height, or five metres (16 ft) whichever is greater, unless the drawing indicates that the snow drifting has been taken into account

HANDLING, INSTALLATION AND BRACING

- The trusses must be handled and installed by a qualified professional as per the supplied document titled *Information for Truss Installers* and the BCSI-B1 and BCSI-B3 Summary Sheets
- The compression chords are laterally braced by continuous rigid diaphragm sheathing or as specified on the drawing
- Temporary and permanent bracing must be installed as indicated on the truss drawing and according to the BCSI-B1 and BCSI-B3 Summary Sheets. Bracing for the lateral stability of the truss is to be provided by the building designer
- **It is recommended that a Professional Engineer's advice be obtained for the bracing of trusses spanning more than 12.37m (40'-7")**

SUPPORTS

- The trusses are to be supported at the bearing points indicated and anchored to the supports where considered necessary by the designer of the overall structure
- Bearing sizes shown are the minimum required to prevent crushing of the truss members and do not necessarily take into account stability of the overall building structure
- Elevation of bearings must be carefully checked and shimmed to alignment for solid bearings
- Adequate wood truss bearing is the responsibility of the building designer.

DIMENSIONS

- Geometry of the truss and dimensions indicated on the drawing are identical to those of the installed truss.



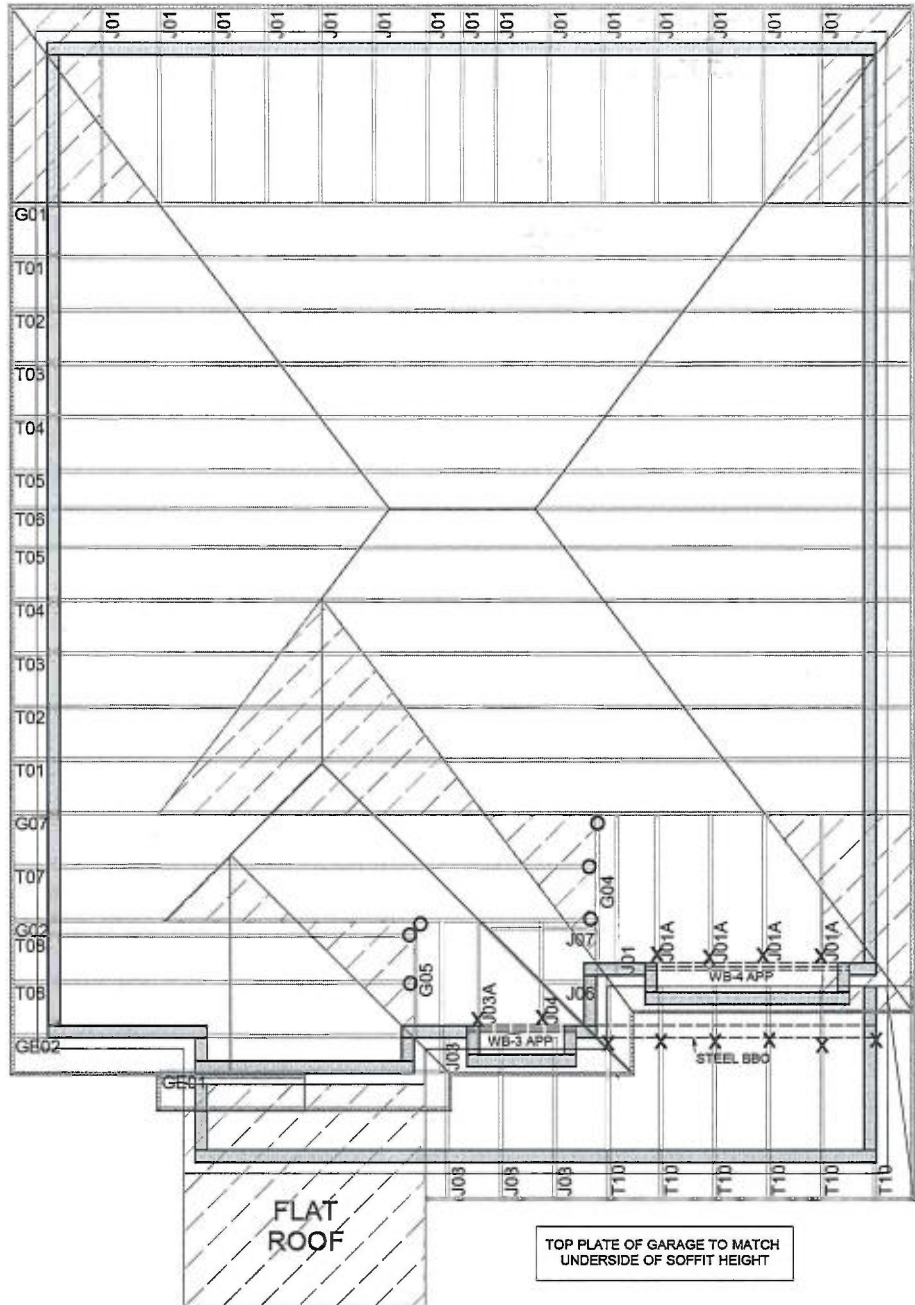


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September 14, 2018



ROOF TRUSS LAYOUT



SIZE AND LOCATION OF CONVENTIONAL FRAMING IS APPROXIMATE. ALL AREAS MAY NOT BE SHOWN. REFER TO ARCHITECTURAL PLANS FOR DETAILS.

ALL CONVENTIONAL FRAMING TO CONFORM WITH PART 9 OF THE O.B.C. ROOF RAFTER THAT CROSS MEET OVER TRUSSES TO BE 2X4 SPF @24" O/C WITH VERTICAL POST TO THE TRUSS BELOW EACH CROSS POINT. VERTICAL POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END POINT AND ROWS OF BRACING DOES NOT EXCEED 6'.

HANGER LEGEND

X - LUS24
- HGUS26

O - LJS26DS
% - HGUS26-2

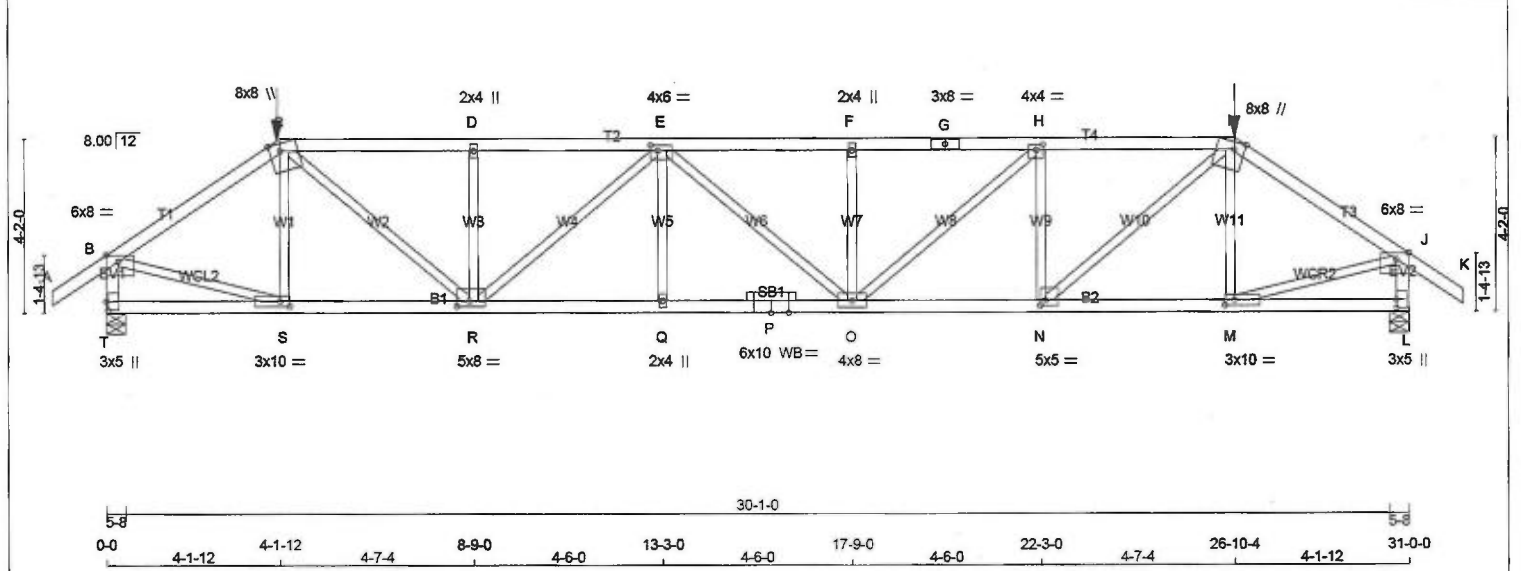
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ALLOWABLE DEFLECTION OF EACH COMPONENT = L/360

Model:	MILLWOOD 1 EL 3
Customer:	GREENPARK
Project:	MINNISALE HOMES
Location:	BRAMPTON
Drawn By:	DM
Date:	9/17/2018



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - G	2x4	DRY	2100F 1.8E
G - I	2x4	DRY	2100F 1.8E
I - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - P	2x4	DRY	2100F 1.8E
P - L	2x4	DRY	2100F 1.8E
ALL WEBS	2x3	DRY	No.2
EXCEPT			SPF
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN
B	TMWw-p	MT20	6.0	8.0
C	TTWw+m	MT20	8.0	8.0
D	TMWw-w	MT20	2.0	4.0
E	TMWwWw-t	MT20	4.0	6.0
F	TMWw-w	MT20	2.0	4.0
G	TS-t	MT20	3.0	8.0
H	TMWw-t	MT20	4.0	4.0
I	TTWw+m	MT20	8.0	8.0
J	TMWw-p	MT20	6.0	8.0
L	BMV1+p	MT20	3.0	5.0
M	BMWw-t	MT20	3.0	10.0
N	BMWw-t	MT20	5.0	5.0
O	BMWwWw-t	MT20	4.0	8.0
P	BS-t	MT20	6.0	10.0
Q	BMWw-w	MT20	2.0	4.0
R	BMWwWw-t	MT20	5.0	8.0
S	BMWwWw-t	MT20	3.0	10.0
T	BMV1+p	MT20	3.0	5.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.
WB - INDICATES BLOCKING REQUIRED

HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 240.5 lbs FACTORED DOWN AT 26-10-4, AND 240.5 lbs FACTORED DOWN AT 4-1-12 ON TOP CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
T	2967	0	2967	0	0	5-8	5-1		
L	2967	0	2967	0	0	5-8	5-1		

UNFACTORED REACTIONS						
1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
T	2081	1458 / 0	0 / 0	0 / 0	0 / 0	622 / 0
L	2081	1458 / 0	0 / 0	0 / 0	0 / 0	622 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 29	-77.4	-77.4 0.11 (1)	10.00	S-C	-549 / 0	0.15 (1)
B-C	-3438 / 0	-77.4	-77.4 0.49 (1)	3.34	C-R	0 / 2732	0.68 (1)
C-D	-4935 / 0	-154.7	-154.7 0.48 (1)	3.52	R-D	-770 / 0	0.21 (1)
D-E	-4935 / 0	-154.7	-154.7 0.48 (1)	3.52	R-E	-1286 / 0	0.79 (1)
E-F	-5904 / 0	-154.7	-154.7 0.48 (1)	3.28	Q-E	0 / 177	0.07 (4)
F-G	-5904 / 0	-154.7	-154.7 0.56 (1)	3.17	E-O	-8 / 0	0.00 (1)
G-H	-5904 / 0	-154.7	-154.7 0.56 (1)	3.17	O-F	-672 / 0	0.19 (1)
H-I	-4938 / 0	-154.7	-154.7 0.49 (1)	3.51	O-H	0 / 1274	0.32 (1)
I-J	-3438 / 0	-77.4	-77.4 0.49 (1)	3.34	N-H	-1607 / 0	0.44 (1)
J-K	0 / 29	-77.4	-77.4 0.11 (1)	10.00	N-I	0 / 2738	0.68 (1)
T-B	-2908 / 0	0.0	0.0 0.33 (1)	4.99	M-I	-552 / 0	0.15 (1)
L-J	-2907 / 0	0.0	0.0 0.33 (1)	4.99	B-S	0 / 2947	0.73 (1)
					M-J	0 / 2947	0.73 (1)
T-S	0 / 0	-35.0	-35.0 0.11 (4)	10.00			
S-R	0 / 2841	-35.0	-35.0 0.30 (1)	10.00			
R-Q	0 / 5910	-35.0	-35.0 0.55 (1)	10.00			
Q-P	0 / 5910	-35.0	-35.0 0.55 (1)	10.00			
P-O	0 / 5910	-35.0	-35.0 0.55 (1)	10.00			
O-N	0 / 4939	-35.0	-35.0 0.47 (1)	10.00			
N-M	0 / 2841	-35.0	-35.0 0.30 (1)	10.00			
M-L	0 / 0	-35.0	-35.0 0.11 (4)	10.00			

FACTORED CONCENTRATED LOADS (LBS)						
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
C	4-1-12	-241	-241	-	FRONT	VERT
I	26-10-4	-241	-241	-	FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 4-1-12
END SETBACK = 6-0-0
END WALL WIDTH = 0-0
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDTL LOADS BASED ON 55% OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/756 (0.49")
CSI: TC=0.58/1.00 (F-H:1), BC=0.55/1.00 (Q-R:1), VB=0.79/1.00 (E-R:1), SSI=0.36/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (S) (INPUT = 0.90)
JSI METAL = 1.00 (P) (INPUT = 1.00)



READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



DESCR

SPF

SPF

SPF

SPF

SPF

DESIGNER

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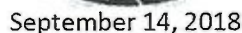
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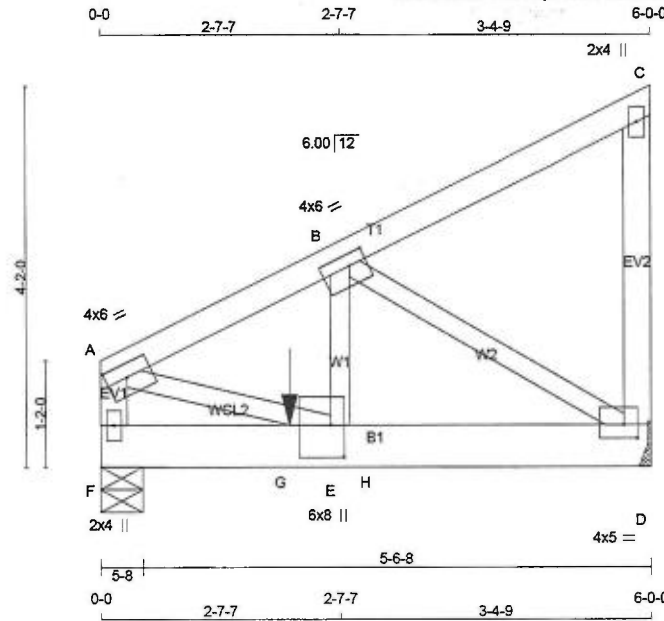
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Scale: 1/2"=1'

TOTAL WEIGHT = 29 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
F - D	2x6	DRY	2100F 1.8E
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0	1.75	2.75
B	TMVW-t	MT20	4.0	6.0	1.50	2.25
C	TMVW-p	MT20	2.0	4.0		
D	BMVW-t	MT20	4.0	5.0	1.75	1.75
E	BMVW-t	MT20	6.0	8.0	4.25	1.75
F	BMV-t	MT20	2.0	4.0		

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1708.4 lbs FACTORED DOWN AT 2-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	1835	1835	0	0
F	1727	1727	0	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. SNOW	MAX/MIN. LIVE	MAX/MIN. PERM. LIVE	WIND	DEAD	SOIL
D	1288	901 / 0	0 / 0	0 / 0	0 / 0	387 / 0	0 / 0
F	1212	848 / 0	0 / 0	0 / 0	0 / 0	364 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	-1938 / 0	-77.4	0.13 (1)	E-B	0 / 1676	0.41 (1)	
B-C	-7 / 0	-77.4	0.13 (1)	B-D	-2045 / 0	0.54 (1)	
D-C	-116 / 0	0.0	0.03 (1)	A-E	0 / 1816	0.45 (1)	
F-A	-1467 / 0	0.0	0.06 (1)				
F-G	0 / 0	-17.5	0.20 (1)				
G-E	0 / 0	-17.5	0.20 (1)				
E-H	0 / 1741	-17.5	0.28 (1)				
H-D	0 / 1741	-445.4	0.28 (1)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1708	-1708		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder

START DISTANCE = 3-0-0

START SPAN CARRIED = 20-6-0

END DISTANCE = 6-0-0

END SPAN CARRIED = 20-6-0

END WALL WIDTH = 5-8

APPLIED TO BACK SIDE OF BOTTOM CHORD.

- ADDTL. LOADS BASED ON 55% OF GSL.

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")

ALLOWABLE DEFL. (TL) = $L/360$ (0.20")

CALCULATED VERT. DEFL. (TL) = $L/999$ (0.04")

CSI: TC=0.18/1.00 (A-F:1), BC=0.28/1.00 (D-E:1),

WB=0.54/1.00 (B-D:1), SSI=0.99/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1887
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.87 (B) (INPUT = 0.90)

JSI METAL=0.52 (D) (INPUT = 1.00)



September 14, 2018

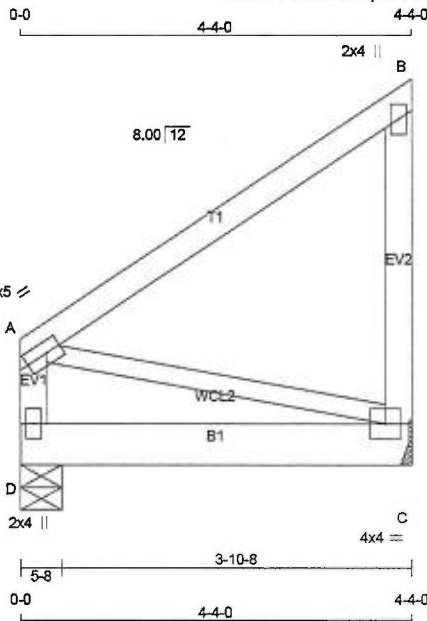
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Scale = 1:24.5

TOTAL WEIGHT = 22 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	
CHORDS			
A - B	2x4 DRY	No.2	
C - B	2x4 DRY	No.2	
D - A	2x4 DRY	No.2	
D - C	2x6 DRY	No.2	
ALL WEBS EXCEPT	2x3 DRY	No.2	
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	3.0	5.0	1.50	2.00
B	TMV+p	MT20	2.0	4.0		
C	BMVW1-t	MT20	4.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
C	781	0	781	0	0	MECHANICAL	
D	781	0	781	0	0	5-8	1-8
A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C TO RESIST THE MAX FACTORED REACTIONS.							

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	548	384 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0
D	548	384 / 0	0 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 0	-77.4	0.27 (1)	10.00	A-C	0 / 0	0.00 (1)
C-B	-168 / 0	0.0	0.04 (1)	7.81			
D-A	-168 / 0	0.0	0.02 (1)	7.81			
D-C	0 / 0	-283.3	-283.3	0.49 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3 PSF
DL = 3.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

GIRDER TYPE: CStdGirder

START DISTANCE = 0-0
START SPAN CARRIED = 13-8-0
END DISTANCE = 4-4-0
END SPAN CARRIED = 13-8-0
END WALL WIDTH = 5-8
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDTL LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/770 (0.07")

CSI: TC=0.27/1.00 (A-B:1), BC=0.49/1.00 (C-D:1),
WB=0.00/1.00 (A-C:1), SSI=0.41/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

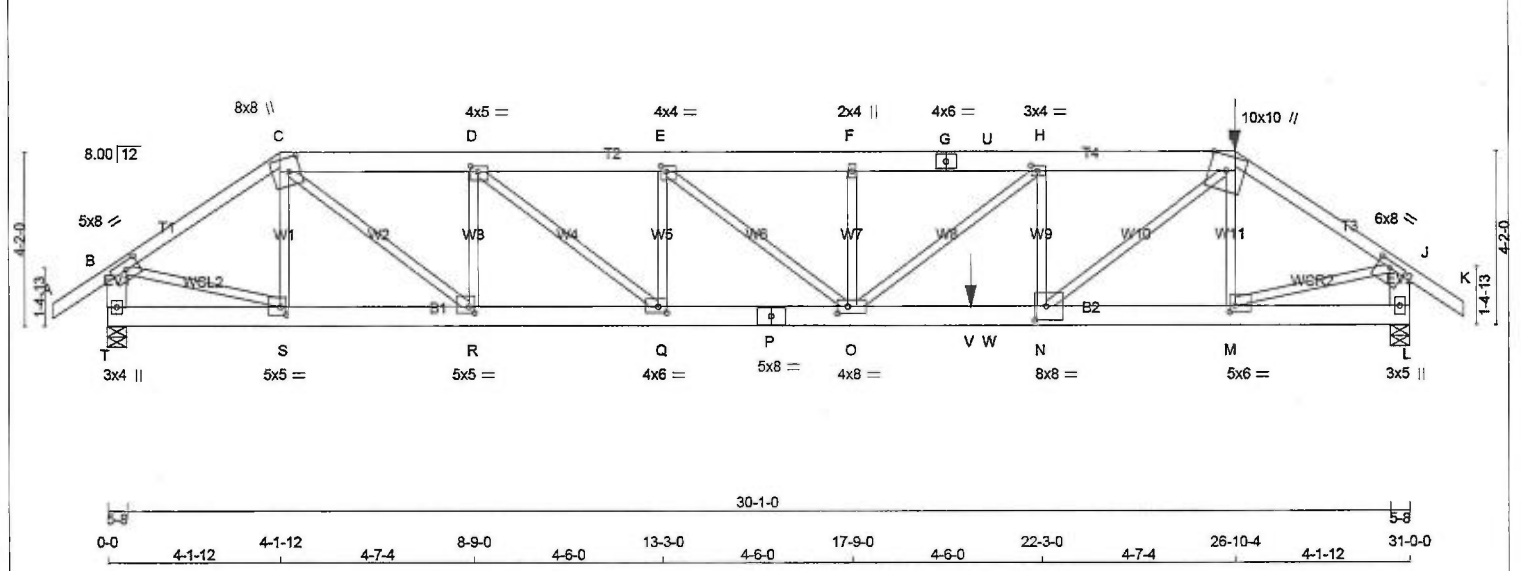


September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Kott, Ottawa, Ontario, IM ID:GvG8oeB5QNkqs?axIV?B3Fz4ISM-TFPIUlwZ?Nm8qFsFCeig4s7fFobGxzzWDsydm8a Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Fri Sep 14 10:04:09 2018 Page 1
-1-3-8 0-0 4-1-12 4-1-12 4-7-4 8-9-0 4-6-0 13-3-0 4-6-0 17-9-0 4-6-0 22-3-0 4-7-4 26-10-4 4-1-12 31-0-0 32-3-8 1-3-8
Scale = 1:52.6



LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - G	2x6	DRY No.2	SPF
G - I	2x6	DRY No.2	SPF
I - K	2x4	DRY No.2	SPF
T - B	2x6	DRY No.2	SPF
L - J	2x6	DRY No.2	SPF
T - P	2x6	DRY No.2	SPF
P - L	2x6	DRY 2100F 1.8E	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.25	3.75
C	TTWW+m	MT20	8.0	8.0	4.00	3.25
D	TMVW-t	MT20	4.0	5.0	1.50	2.00
E	TMVW-t	MT20	4.0	4.0	1.50	1.50
F	TMVW+w	MT20	2.0	4.0		
G	TS-t	MT20	4.0	6.0		
H	TMVW-t	MT20	3.0	4.0	1.50	1.75
I	TTWW+m	MT20	10.0	10.0	4.25	4.75
J	TMVW-t	MT20	6.0	8.0	1.75	3.75
L	BMV1+p	MT20	3.0	5.0		
M	BMVW-t	MT20	5.0	6.0	1.75	1.50
N	BMVW-t	MT20	8.0	8.0	4.00	3.25
O	BMVWV-t	MT20	4.0	8.0	2.00	3.00
P	BS-t	MT20	5.0	8.0		
Q	BMVW-t	MT20	4.0	6.0	1.75	2.50
R	BMVW-t	MT20	5.0	5.0	1.75	1.50
S	BMVW-t	MT20	5.0	5.0	2.25	1.50
T	BMV1+p	MT20	3.0	4.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 240.5 lbs FACTORED DOWN AT 26-10-4 ON TOP CHORD, AND 1835.3 lbs FACTORED DOWN AT 20-6-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2343	2343	0	5-8
DOWN	0	0	0	2-9
HORIZ	3442	3442	0	5-8
UPLIFT	0	0	0	3-12

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE PERM. LIVE WIND DEAD SOIL
T	1642	1184 / 0 0 / 0 0 / 0 478 / 0 0 / 0
L	2414	1695 / 0 0 / 0 0 / 0 719 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 29	-77.4 -77.4 0.11 (1)	10.00	S-C	-448 / 0	0.11 (1)	
B-C	-2739 / 0	-77.4 -77.4 0.41 (1)	3.79	C-R	0 / 2641	0.65 (1)	
C-D	-4324 / 0	-77.4 -77.4 0.25 (1)	3.97	R-D	-1561 / 0	0.38 (1)	
D-E	-5729 / 0	-77.4 -77.4 0.30 (1)	3.46	D-Q	0 / 1820	0.45 (1)	
E-F	-5784 / 0	-77.4 -77.4 0.43 (1)	3.08	Q-E	-1228 / 0	0.30 (1)	
F-G	-6784 / 0	-77.4 -77.4 0.43 (1)	3.08	E-O	0 / 1366	0.34 (1)	
G-U	-6784 / 0	-77.4 -77.4 0.43 (1)	3.08	O-F	-317 / 0	0.08 (1)	
U-H	-6784 / 0	-145.9 -145.9 0.43 (1)	3.08	H-O	0 / 468	0.12 (1)	
H-I	-6423 / 0	-145.9 -145.9 0.53 (1)	3.03	N-H	-976 / 0	0.24 (1)	
I-J	-4130 / 0	-77.4 -77.4 0.59 (1)	2.97	N-I	0 / 3862	0.96 (1)	
J-K	0 / 29	-77.4 -77.4 0.11 (1)	10.00	M-I	-828 / 0	0.20 (1)	
T-B	-2312 / 0	0.0 0.0 0.17 (1)	6.71	B-S	0 / 2348	0.58 (1)	
L-J	-3351 / 0	0.0 0.0 0.24 (1)	5.75	M-J	0 / 3540	0.88 (1)	
T-S	0 / 0	-17.5 -17.5 0.04 (4)	10.00				
S-R	0 / 2273	-17.5 -17.5 0.32 (1)	10.00				
R-Q	0 / 4324	-17.5 -17.5 0.67 (1)	10.00				
Q-P	0 / 5729	-17.5 -17.5 0.85 (1)	10.00				
P-O	0 / 5729	-17.5 -17.5 0.85 (1)	10.00				
O-V	0 / 6423	-17.5 -17.5 0.93 (1)	10.00				
V-W	0 / 6423	-17.5 -17.5 0.93 (1)	10.00				
W-N	0 / 6423	-33.0 -33.0 0.93 (1)	10.00				
N-M	0 / 3423	-33.0 -33.0 0.40 (1)	10.00				
M-L	0 / 0	-33.0 -33.0 0.05 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	26-10-4	-241	-241	---	FRONT	VERT	TOTAL
V	20-6-12	-1835	-1835	---	FRONT	VERT	TOTAL

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***

GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.

LOADS WERE DERIVED FROM USER INPUT

NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL	= 23.3 PSF
DL	= 3.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.09/12 MINIMUM

GIRDER TYPE: CPrimeHip

SIDE SETBACK = 4-1-12

END SETBACK = 6-0-0

END WALL WIDTH = 5-8

CORNER FRAMING TYPE: CONVENTIONAL

END JACK TYPE: CONVENTIONAL

APPLIED TO FRONT SIDE

- ADDTL LOADS BASED ON 55 % OF GSL

- LOADS APPLIED TO FIRST 10-0-0 OF SPAN MEASURED FROM THE RIGHT.

*** NON STANDARD GIRDER ***

ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 066-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")

CALCULATED VERT. DEFL.(LL) = L/999 (0.29")

ALLOWABLE DEFL.(TL) = L/360 (1.03")

CALCULATED VERT. DEFL.(TL) = L/721 (0.52")

CSI: TC=0.59/1.00 (I-J:1), BC=0.93/1.00 (N-O:1), WB=0.96/1.00 (H-N:1), SSI=0.82/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR (PSI)	SECTION (PL)
MT20	618 354 1667 822 2284 1656

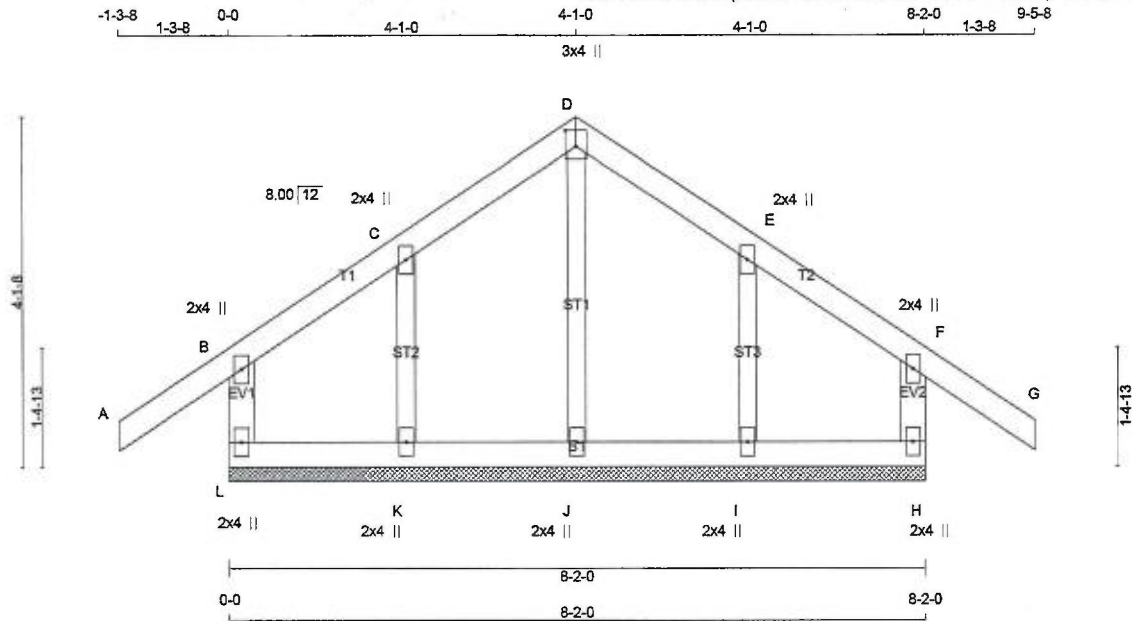
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.90 (R) (INPUT = 0.90)

JSI METAL = 0.95 (P) (INPUT = 1.00)





Scale = 1:25.9

TOTAL WEIGHT = 33 lb

LUMBER				
N. L. G. A.	RULES			LUMBER
CHORDS	SIZE			
L - B	2x4	DRY		No.2
A - D	2x4	DRY		No.2
D - G	2x4	DRY		No.2
H - F	2x4	DRY		No.2
L - H	2x4	DRY		No.2
ALL WEBS	2x3	DRY		No.2
ALL GABLE WEBS	2x3	DRY		No.2
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2-0-0 OC.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
C	TTW+w	MT20	2.0	4.0	
D	TTW+p	MT20	3.0	4.0	2.25 1.50
E	TMW+w	MT20	2.0	4.0	
F	TMV+p	MT20	2.0	4.0	
H	BMV1+p	MT20	2.0	4.0	
I, J, K	BMW1+w	MT20	2.0	4.0	
L	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		VERT. LOAD	LC1				
FR-TO		FROM	TO		FR-TO		
L-B	-193 / 0	0.0	0.0	0.01 (1)	J-D	-149 / 0	0.04 (1)
A-B	0 / 29	-77.4	-77.4	0.10 (1)	K-C	-154 / 0	0.03 (1)
B-C	-18 / 0	-77.4	-77.4	0.04 (1)	I-E	-154 / 0	0.03 (1)
C-D	-11 / 0	-77.4	-77.4	0.04 (1)			
D-E	-11 / 0	-77.4	-77.4	0.04 (1)			
E-F	-18 / 0	-77.4	-77.4	0.04 (1)			
F-G	0 / 29	-77.4	-77.4	0.10 (1)			
H-F	-193 / 0	0.0	0.0	0.01 (1)			
L-K	0 / 15	-17.5	-17.5	0.02 (4)			
K-J	0 / 9	-17.5	-17.5	0.02 (4)			
J-I	0 / 9	-17.5	-17.5	0.02 (4)			
I-H	0 / 15	-17.5	-17.5	0.02 (4)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (F-G:1), BC=0.02/1.00 (H-I:4), WB=0.04/1.00 (D-J:1), SSI=0.07/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

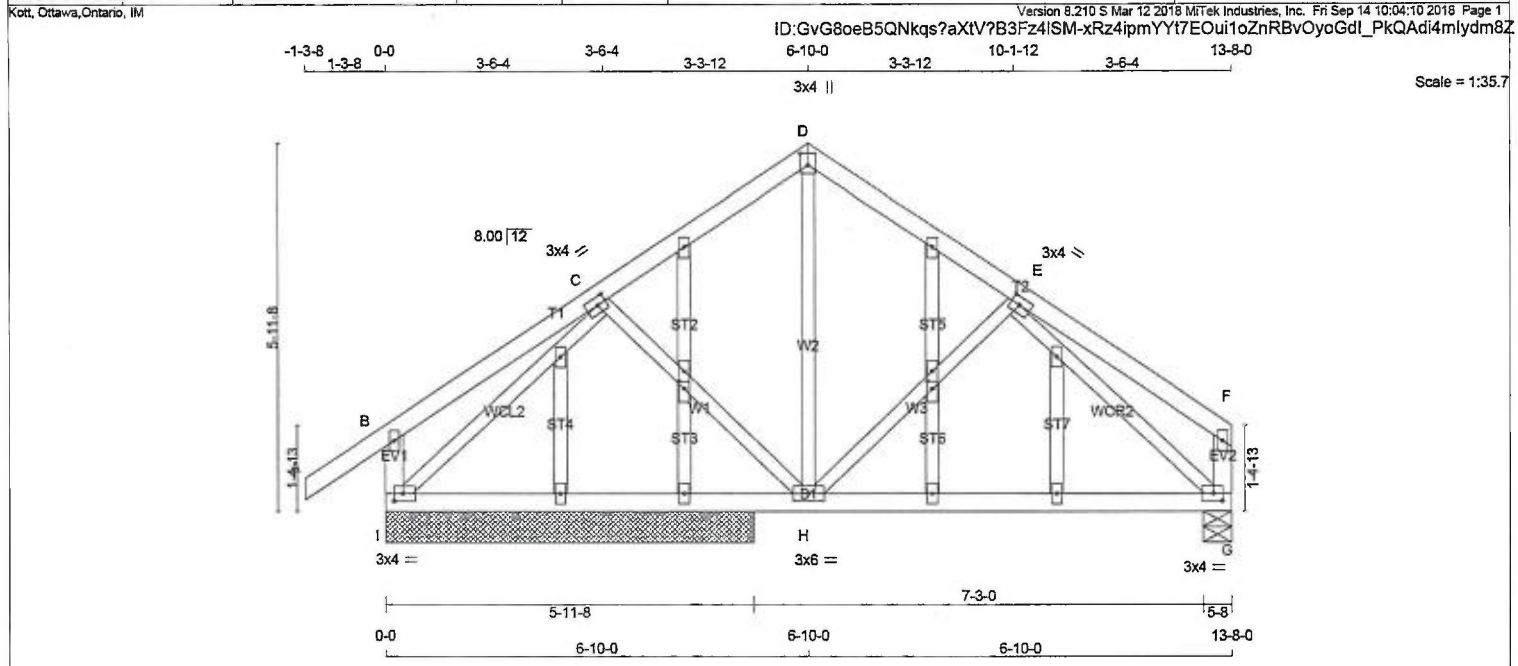
JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.06 (D) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
I - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
C	TMVW-t	MT20	3.0	4.0	1.50	1.75
D	TTW+p	MT20	3.0	4.0	2.25	1.50
E	TMVW-t	MT20	3.0	4.0	1.50	1.75
F	TMV+p	MT20	2.0	4.0		
G	BMVW1-t	MT20	3.0	4.0	1.50	1.75
H	BMVW1-t	MT20	3.0	6.0		
I	BMVW1-t	MT20	3.0	4.0	1.50	1.75
J	NP+ww	MT20	2.0	4.0	1.50	1.00
K, L, M, N, O, P, Q, R, S						
J	NP+ww	MT20	2.0	4.0		
Q	NP+ww	MT20	2.0	4.0	1.50	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

DESCR.	SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	SPF	VERT 754	DOWN 754	0	5-11-8
I	SPF	HORZ 0	HORZ 0	0	10-15
G	SPF	UPLIFT 648	UPLIFT 648	0	1-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	527	SNOW 382 / 0
I	455	LIVE 318 / 0
G		PERM. LIVE 0 / 0
		WIND 0 / 0
		DEAD 145 / 0
		SOIL 137 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	CSF (LC)
FR-TO				FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	C-H	-138 / 7	0.05 (1)
B-C	0 / 16	-77.4	-77.4 0.14 (1)	10.00	H-D	0 / 340	0.08 (1)
C-D	-509 / 0	-77.4	-77.4 0.11 (1)	6.25	H-E	-138 / 6	0.05 (1)
D-E	-509 / 0	-77.4	-77.4 0.11 (1)	6.25	I-C	-713 / 0	0.26 (1)
E-F	0 / 16	-77.4	-77.4 0.14 (1)	10.00	E-G	-713 / 0	0.26 (1)
I-B	-208 / 0	0.0	0.0 0.02 (1)	7.81			
G-F	-102 / 0	0.0	0.0 0.01 (1)	7.81			
I-H	0 / 508	-17.5	-17.5 0.27 (4)	10.00			
H-G	0 / 509	-17.5	-17.5 0.27 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.3	PSF
DL	3.0	PSF
BOT CH.	LL	PSF
DL	0.0	PSF
DL	7.0	PSF
TOTAL LOAD	33.3	PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.46")

CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.14/1.00 (B-C:1), BC=0.27/1.00 (G-H:4), WB=0.28/1.00 (E-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667	822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)

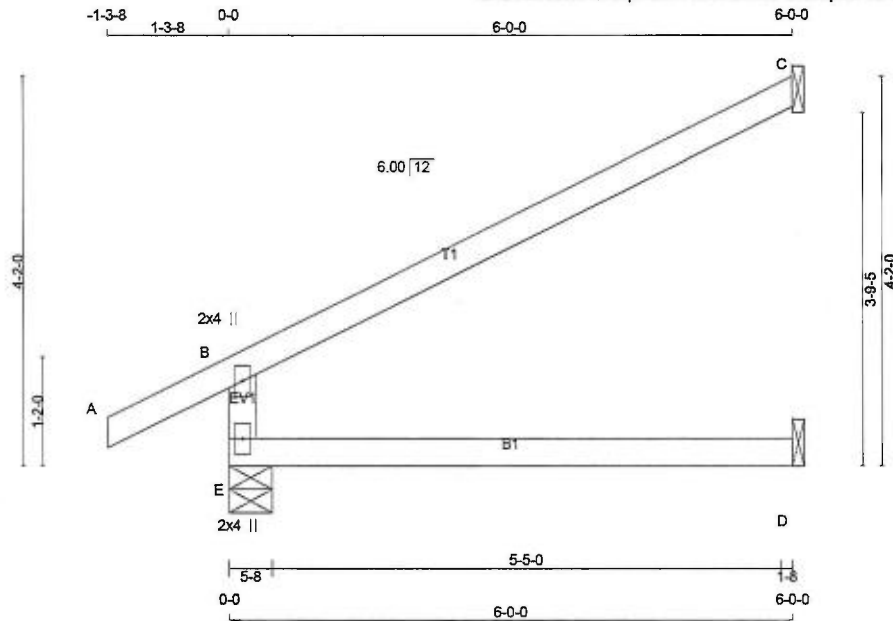
JSI METAL= 0.25 (C) (INPUT = 1.00)



September 14, 2018

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Scale = 1:23.6

TOTAL WEIGHT = 16 X 17 = 274 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
E - B 2x4 DRY No.2
A - C 2x4 DRY No.2
E - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMV+p MT20 2.0 4.0
E BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX
E	457	0	457	0	0	5-8	1-8		
C	174	0	174	0	0	1-8	1-8		
D	43	0	49	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	318	238 / 0	0 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	118	105 / 0	0 / 0	0 / 0	0 / 0	14 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	FR-TO
E-B	-395 / 0	0.0	0.0 0.13 (4)	E-B	7.81	10.00	
A-B	0 / 23	-77.4	-77.4 0.10 (1)	A-B	6.25	10.00	
B-C	-28 / 0	-77.4	-77.4 0.47 (1)	B-C	6.25	10.00	
E-D	0 / 0	-17.5	-17.5 0.13 (4)	E-D	10.00	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.47/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



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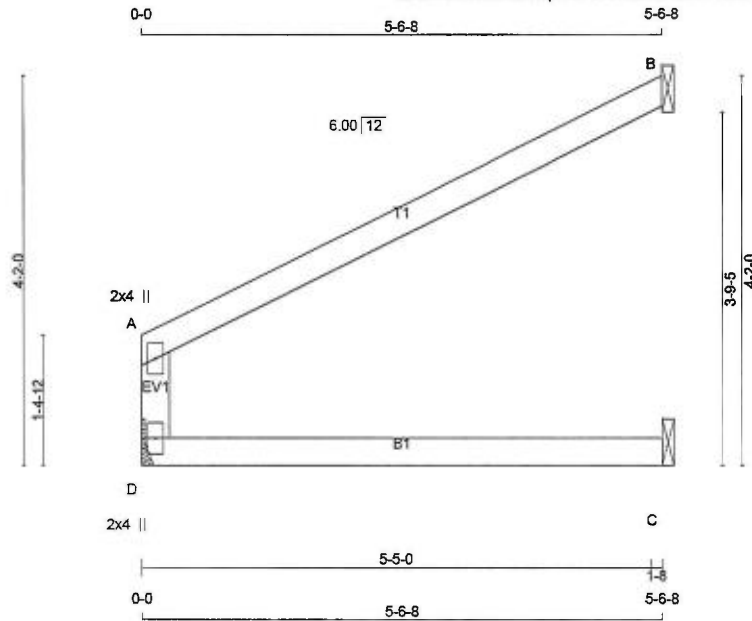
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Scale = 1:23.6

TOTAL WEIGHT = 4 X 15 = 58 lb [M]F

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	UP/LIFT
D	263	0	263	0
B	197	0	197	0
C	66	0	66	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
COMBINED	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL	
D	184	129/0	0/0	0/0	55/0	0/0	
B	134	116/0	0/0	0/0	18/0	0/0	
C	50	13/0	0/0	0/0	37/0	0/0	

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. LC2	MAX. UNBRAC LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO				
D-A	-232/0	0.0	0.0	0.12 (1)	7.81		
A-B	-8/1	-77.4	-77.4	0.34 (1)	10.00		
D-C	0/0	-17.5	-17.5	0.17 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/829$ (0.08")

CSI: TC=0.34/1.00 (A-B:1), BC=0.17/1.00 (C-D:1), WB=0.00/1.00 (n/a:0), SS=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.13 (A) (INPUT = 0.90)
JSI METAL= 0.05 (A) (INPUT = 1.00)

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



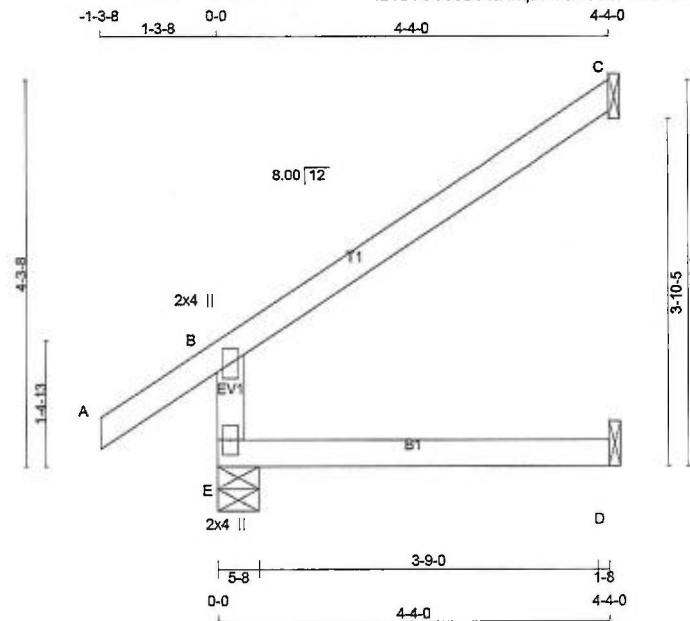
September 14, 2018

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.	Page 12 of 28
IM0918-066	J03	1	1	TR - GREENPARK - MINNISALE HOMES - MILLWOOD 1 ELE 3		IM0918-066

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Scale = 1:24.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0		
E	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	GROSS REACTION	BRG	BRG
E	359	0	359	0	5-8
C	126	0	126	0	1-8
D	32	0	36	0	1-8

SEE MITTEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	249	190 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0
C	85	76 / 0	0 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	28	0 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	(PLF)	CSI (LC)	UNBRAC	FR-TO	MEMB.	FORCE (LBS)	FROM	TO	(PLF)	CSI (LC)
E-B	-318 / 0	0.0	0.0	0.06 (4)	7.81									
A-B	0 / 29	-77.4	-77.4	0.10 (1)	10.00									
B-C	-23 / 0	-77.4	-77.4	0.25 (1)	6.25									
E-D	0 / 0	-17.5	-17.5	0.07 (4)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF	
	BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF	
TOTAL LOAD	=	33.3	PSF		

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.25/1.00 (B-C:1), BC=0.07/1.00 (D-E:4),
WB=0.00/1.00 (n/a:0), SSI=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

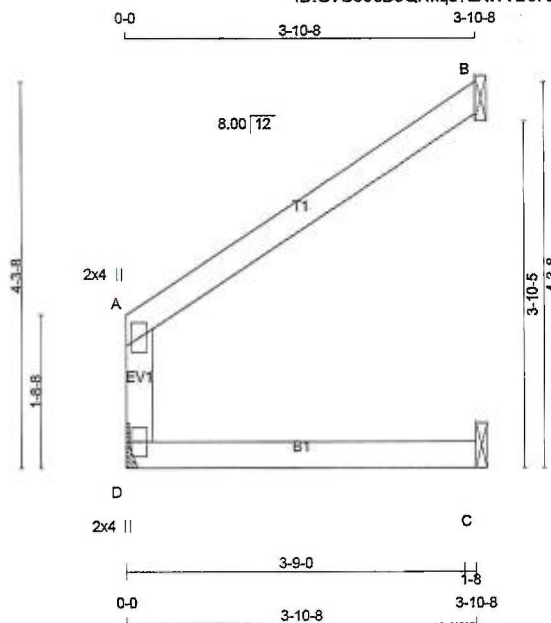
JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





Scale = 1:24.5

LUMBER
N. L. G. A. RULES
CHORDS SIZE DRY LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMV+p MT20 2.0 4.0
D BMV1+p MT20 2.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
D	184	0	184	0	MECHANICAL	
B	139	0	139	0	1-8	1-8
C	45	0	45	0	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	129	90 / 0	0 / 0	0 / 0	0 / 0	39 / 0	0 / 0
B	95	82 / 0	0 / 0	0 / 0	0 / 0	13 / 0	0 / 0
C	34	8 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS	
	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. LOAD (LC1)	MAX. CS1 (LC)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO						
D-A	-161/0	0.0	0.0	0.05 (1)	7.81	
A-B	-8/0	-77.4	-77.4	0.16 (1)	10.00	
D-C	0/0	-17.5	-17.5	0.08 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.16/1.00 (A-B-1), BC=0.08/1.00 (C-D-1),
WB=0.00/1.00 (n/a:0), SSI=0.10/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

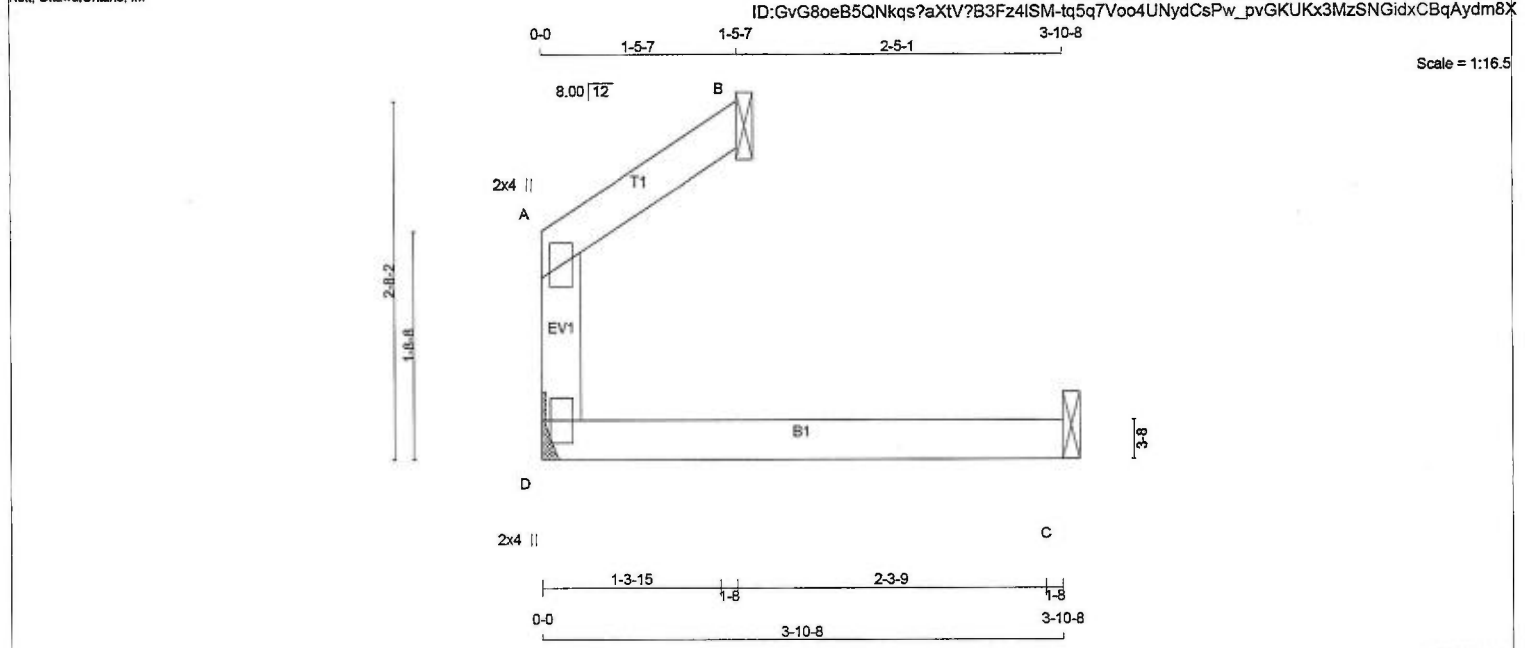
JSI GRIP= 0.10 (A) (INPUT = 0.90)
JSI METAL = 0.04 (A) (INPUT = 1.00)



September 14, 2018

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LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DESCR.
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
D	86	0	86	0	MECHANICAL	
B	63	0	63	0	1-8	1-8
C	31	0	34	0	1-8	1-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT D TO RESIST THE MAX FACTORED REACTIONS.

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	62	35/0	0/0	0/0	0/0	27/0	0/0
B	44	32/0	0/0	0/0	0/0	11/0	0/0
C	25	1/0	0/0	0/0	0/0	25/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	WEBS		MEMB. MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
			FROM	TO		
FR-TO						
D-A	-50/4	0.0	0.0	0.03 (4)	7.81	
A-B	0/5	-77.4	-77.4	0.04 (1)	10.00	
D-C	0/0	-17.5	-17.5	0.06 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.01")

CSI: TC=0.04/1.00 (A-B:1), BC=0.06/1.00 (C-D:4), WS=0.00/1.00 (n/a:0), SSI=0.05/1.00 (C-D:4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

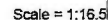
JSI GRIP= 0.03 (A) (INPUT = 0.90)
JSI METAL = 0.01 (A) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 6 lb

DESCR
SPF
SPF
SPF

DRY: SEASONED LUMBER

DIMENSIONS, SUPPORTS, AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) B . C

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D. B.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
D-A	-78 / 0	0.0	0.01 (1)		7.81		
A-B	-2 / 0	-77.4	-77.4 0.04 (1)		10.00		
D-C	0 / 0	-17.5	-17.5 0.02 (4)		10.00		

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
-------	---------------------------------	-----------------

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBCC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.02/1.00 (C-D:4), WB=0.00/1.00 (n/a:0), SSI=0.05/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

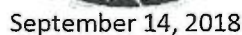
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

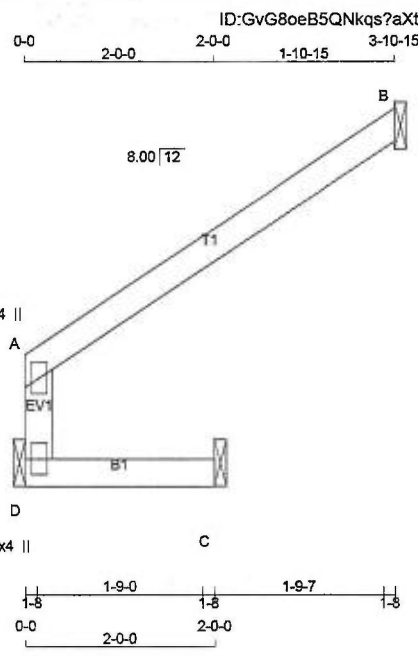
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.05 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
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IS AN INTEGRAL PART OF THIS DRAWING AS IT
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IN THE DESIGN OF THIS COMPONENT.**





Scale = 1:23.4

TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
D - A 2x4 DRY No.2
A - B 2x4 DRY No.2
D - C 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	2.0	4.0		
D	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	SPF	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
		VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
JT									
D	SPF	152	0	152	0	0	1-8	1-8	1-8
B	SPF	134	0	134	0	0	1-8	1-8	1-8
C	SPF	52	0	52	0	0	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) D, B, C

UNFACTORED REACTIONS							
	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	106	81 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	91	80 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	37	21 / 0	0 / 0	0 / 0	0 / 0	16 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
D-A	-169 / 0	0.0	0.0	0.08 (1)	7.81				
A-B	-10 / 0	-77.4	-77.4	0.15 (1)	10.00				
D-C	0 / 0	-17.5	-17.5	0.09 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.15/1.00 (A-B:1), BC=0.09/1.00 (C-D:1),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

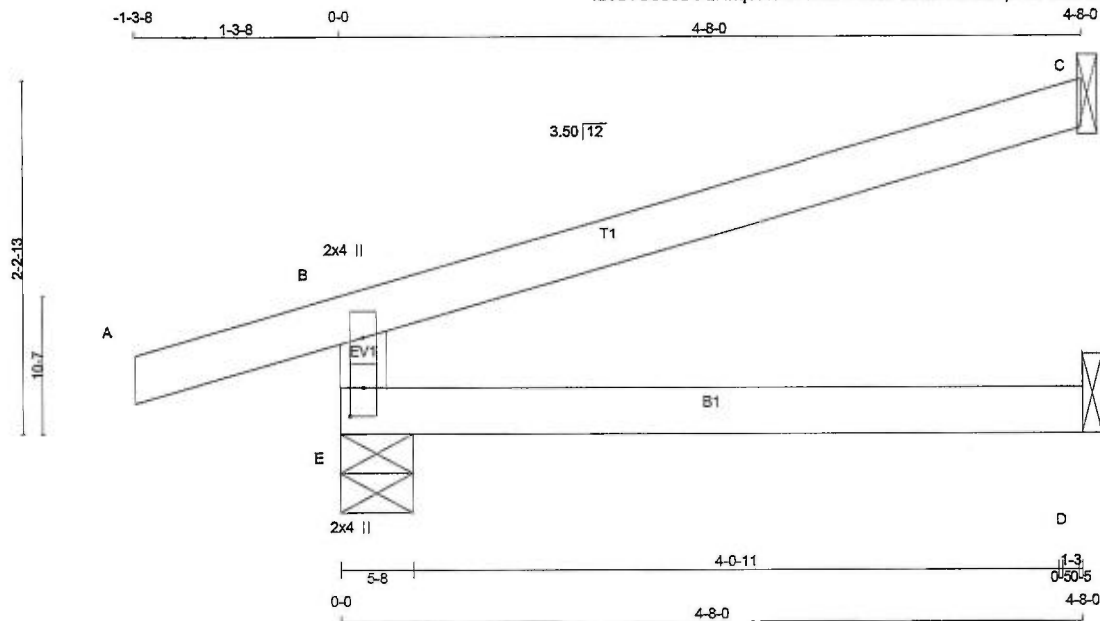
JSI GRIP= 0.11 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 3 X 13 = 39 lb

LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	2.0	4.0		
E BMV1+p	MT20	2.0	4.0	2.25	1.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	377	0	377	0	5-8	1-8
C	135	0	135	0	1-8	1-8
D	33	0	37	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37579H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	282	198 / 0	0 / 0	0 / 0	0 / 0	64 / 0	0 / 0
C	92	82 / 0	0 / 0	0 / 0	0 / 0	11 / 0	0 / 0
D	27	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO					
E-B	-329 / 0	0.0	0.0	0.08 (4)	7.81			
A-B	0 / 14	-77.4	-77.4	0.10 (1)	10.00			
B-C	-13 / 0	-77.4	-77.4	0.29 (1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.08 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.
(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.01")

CSI: TC=0.29/1.00 (B-C:1), BC=0.08/1.00 (D-E:4), WB=0.00/1.00 (n/a:0), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.18 (B) (INPUT=0.90)
JSI METAL=0.06 (E) (INPUT=1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.

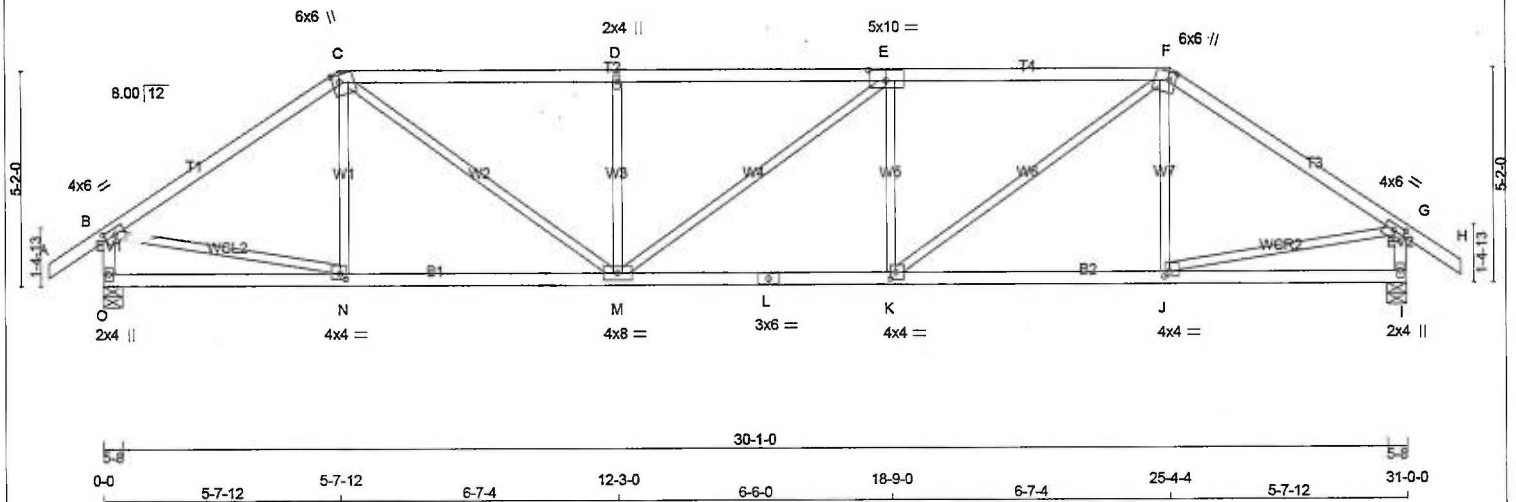


Kott, Ottawa, Ontario, IM

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Fri Sep 14 10:04:13 2018 Page 1

ID:GvG8oeB5QNks7aXIV7B3Fz4ISM-L0fDKroRroVpFMRCUjK8pY0LSclBITssbXkNdydm8W

Scale = 1:52.6



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	4.0	6.0	1.75	3.00
C	CTWW+m	MT20	6.0	6.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TSWW-l	MT20	5.0	10.0	3.00	5.00
F	CTWW+m	MT20	6.0	6.0	2.25	2.00
G	TMWV-t	MT20	4.0	6.0	1.75	3.00
I	BMV1+p	MT20	2.0	4.0		
J	BMWW-l	MT20	4.0	4.0	1.50	1.50
K	BMWW-l	MT20	4.0	4.0	2.00	1.50
L	BS-t	MT20	3.0	6.0		
M	BMWWV-t	MT20	4.0	4.0	8.0	
N	BMWW-l	MT20	4.0	4.0	1.50	1.50
O	BMV1+p	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
O	1577	0	1577	0	5-8	2-11	
I	1577	0	1577	0	5-8	2-11	

UNFACTORED REACTIONS							
1ST CASE	MAX/MIN COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
O	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
I	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	N-C	-145 / 54	0.06 (1)
B-C	-1702 / 0	-77.4	-77.4 0.55 (1)	4.48	C-M	0 / 1114	0.25 (1)
C-D	-2308 / 0	-77.4	-77.4 0.64 (1)	3.89	M-D	-543 / 0	0.21 (1)
D-E	-2308 / 0	-77.4	-77.4 0.64 (1)	3.86	M-E	-11 / 0	0.01 (1)
E-F	-2305 / 0	-77.4	-77.4 0.68 (1)	3.86	K-E	-548 / 0	0.22 (1)
F-G	-1702 / 0	-77.4	-77.4 0.55 (1)	4.48	K-F	0 / 1110	0.25 (1)
G-H	0 / 29	-77.4	-77.4 0.10 (1)	10.00	J-F	-148 / 54	0.08 (1)
O-B	-1537 / 0	0.0	0.0 0.16 (1)	6.64	B-N	0 / 1439	0.32 (1)
I-G	-1537 / 0	0.0	0.0 0.16 (1)	6.64	J-G	0 / 1439	0.32 (1)
O-N	0 / 0	-17.5	-17.5 0.15 (4)	10.00			
N-M	0 / 1411	-17.5	-17.5 0.31 (1)	10.00			
M-L	0 / 2317	-17.5	-17.5 0.44 (1)	10.00			
L-K	0 / 2317	-17.5	-17.5 0.44 (1)	10.00			
K-J	0 / 1411	-17.5	-17.5 0.31 (1)	10.00			
J-I	0 / 0	-17.5	-17.5 0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.23")

CSI: TC=0.66/1.00 (E-F:1), BC=0.44/1.00 (K-M:1), WB=0.32/1.00 (B-N:1), SS=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

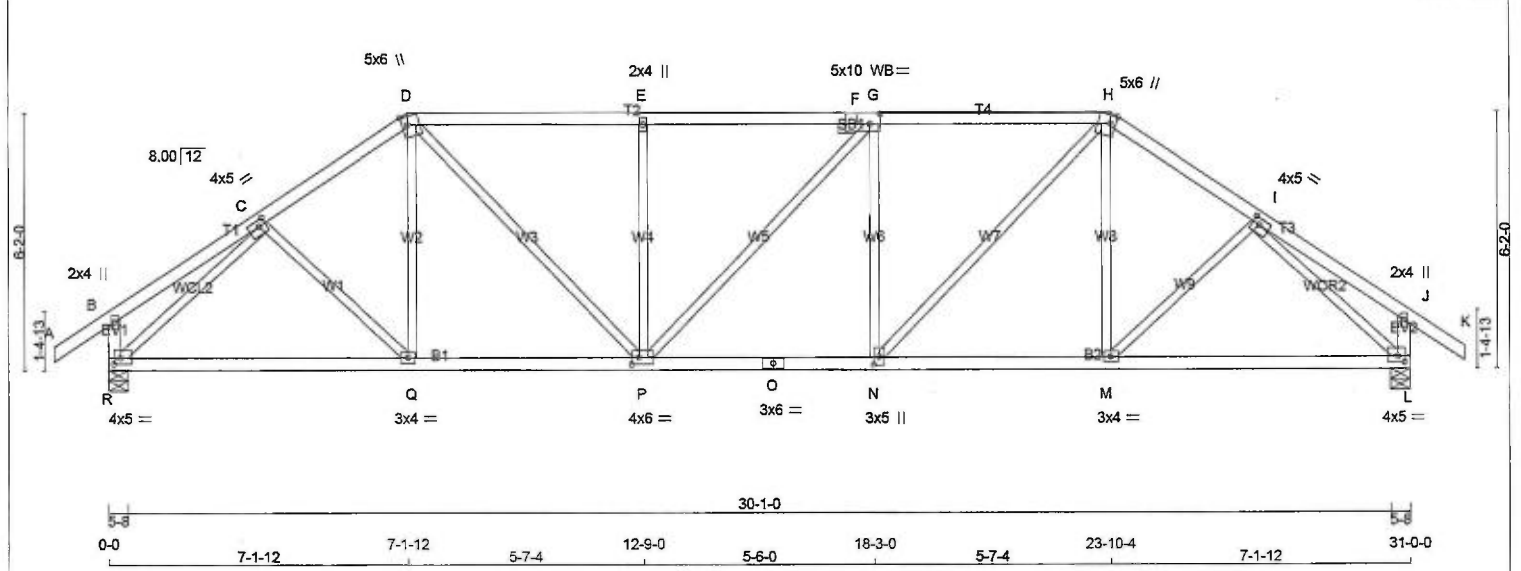
JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.67 (L) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2

ALL WEBS EXCEPT
2x3 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	2.0	4.0	
C	TMVW+1	MT20	4.0	5.0	1.75
D	TTWW+m	MT20	5.0	6.0	2.50
E	TMVW+w	MT20	2.0	4.0	
F	TSWW-1	MT20	5.0	10.0	3.00
G	TTWW+m	MT20	5.0	6.0	2.50
H	TMVW+1	MT20	4.0	5.0	1.75
J	TMV+p	MT20	2.0	4.0	
L	BMVW+1-t	MT20	4.0	5.0	1.75
M	BMVW+1	MT20	3.0	4.0	
N	BMVW+1	MT20	3.0	5.0	2.25
O	BS-t	MT20	3.0	6.0	
P	BMVW+1	MT20	4.0	6.0	2.00
Q	BMVW+1	MT20	3.0	4.0	
R	BMVW+1-t	MT20	4.0	5.0	1.75

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	1577	0	1577	0	5-8	1-12
L	1577	0	1577	0	5-8	1-12

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM/LIVE			
R	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
L	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.43 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO					
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	C-Q 0 / 67 0.02 (4)
B-C	0 / 16	-77.4	-77.4 0.14 (1)	10.00	Q-D 0 / 103 0.04 (4)
C-D	-1893 / 0	-77.4	-77.4 0.18 (1)	4.94	D-P 0 / 763 0.17 (1)
D-E	-1922 / 0	-77.4	-77.4 0.41 (1)	4.44	P-E -464 / 0 0.28 (1)
E-F	-1922 / 0	-77.4	-77.4 0.41 (1)	4.43	G-P -2 / 0 0.00 (1)
F-G	-1922 / 0	-77.4	-77.4 0.41 (1)	4.43	N-G -465 / 0 0.28 (1)
G-H	-1923 / 0	-77.4	-77.4 0.41 (1)	4.43	N-H 0 / 765 0.17 (1)
H-I	-1893 / 0	-77.4	-77.4 0.18 (1)	4.94	M-H 0 / 103 0.04 (4)
I-J	0 / 16	-77.4	-77.4 0.14 (1)	10.00	M-I 0 / 67 0.02 (4)
J-K	0 / 29	-77.4	-77.4 0.10 (1)	10.00	R-C -1879 / 0 0.74 (1)
R-B	-216 / 0	0.0	0.0 0.02 (1)	7.61	I-L -1879 / 0 0.74 (1)
L-J	-216 / 0	0.0	0.0 0.02 (1)	7.61	
R-Q	0 / 1349	-17.5	-17.5 0.33 (1)	10.00	
Q-P	0 / 1394	-17.5	-17.5 0.33 (1)	10.00	
P-O	0 / 1923	-17.5	-17.5 0.36 (1)	10.00	
O-N	0 / 1923	-17.5	-17.5 0.36 (1)	10.00	
N-M	0 / 1394	-17.5	-17.5 0.33 (1)	10.00	
M-L	0 / 1348	-17.5	-17.5 0.32 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.41/1.00 (G-H:1), BC=0.38/1.00 (N-P:1), WB=0.74/1.00 (C-R:1), SSI=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1567
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

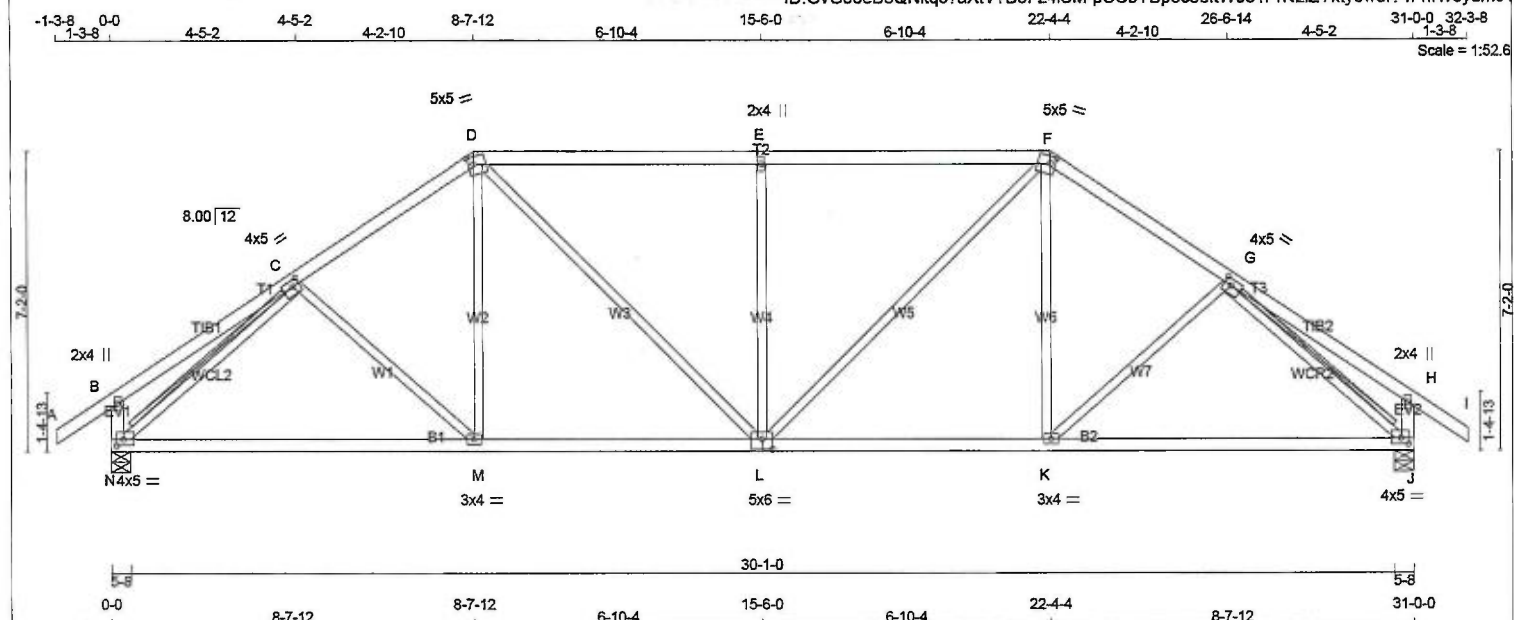
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = $2 \times 130 = 260$ lb

LUMBER			
N. L. G. A.	RULES		
CHORDS	SIZE	LUMBER	DESCR
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY-SEASONED LUMBER			

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
B	TMV+p	MT20	2.0	4.0	
C	TMVW+t	MT20	4.0	5.0	1.75 2.00
D	TMVW+m	MT20	5.0	5.0	2.00 1.50
E	TMV+w	MT20	2.0	4.0	
F	TTVW+m	MT20	5.0	5.0	2.00 1.50
G	TMVW+t	MT20	4.0	5.0	1.75 2.00
H	TMV+p	MT20	2.0	4.0	
J	BMVWV+t	MT20	4.0	5.0	1.75 2.00
K	BMVW+t	MT20	3.0	4.0	
L	BSVWVW+t	MT20	5.0	6.0	3.00 3.00
M	BMVW+t	MT20	3.0	4.0	
N	BMVWV+t	MT20	4.0	5.0	1.75 2.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER							
BEARINGS							
FACTORED GROSS REACTION			MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	1577	0	1577	0	0	5-8	1-12
J	1577	0	1577	0	0	5-8	1-12

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N. J.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

2x4 DRY SPF No.2 T-BRACE AT C-N, G-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED L1 C1 (LBS)	MAX. UNBRAC. CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (L/C)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0/29	-77.4	-77.4	0.10 (1)	10.00	C-M	-55/44
B-C	0/21	-77.4	-77.4	0.22 (1)	10.00	M-D	0/200
C-D	-1648/0	-77.4	-77.4	0.21 (1)	4.98	D-L	0/524
D-E	-1726/0	-77.4	-77.4	0.53 (1)	4.48	L-E	-852/0
E-F	-177.0	-77.4	-77.4	0.55 (1)	4.46	J-F	0/524
F-G	-1648/0	-77.4	-77.4	0.21 (1)	4.98	K-F	0/200
G-H	0/21	-77.4	-77.4	0.22 (1)	10.00	K-G	-55/44
H-I	0/29	-77.4	-77.4	0.10 (1)	10.00	N-C	-1891/0
N-B	-236/0	0.0	0.0	0.02 (1)	7.81	G-J	-1891/0
J-H	-236/0	0.0	0.0	0.02 (1)	7.81		
N-M	0/1393	-17.5	-17.5	0.39 (4)	10.00		
M-L	0/1355	-17.5	-17.5	0.40 (4)	10.00		
L-K	0/1355	-17.5	-17.5	0.40 (4)	10.00		
K-J	0/1393	-17.5	-17.5	0.39 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBCC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.53/1.00 (E-F:1), BC=0.40/1.00 (L-M:4),
WB=0.58/1.00 (E-L:1), SSI=0.26/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

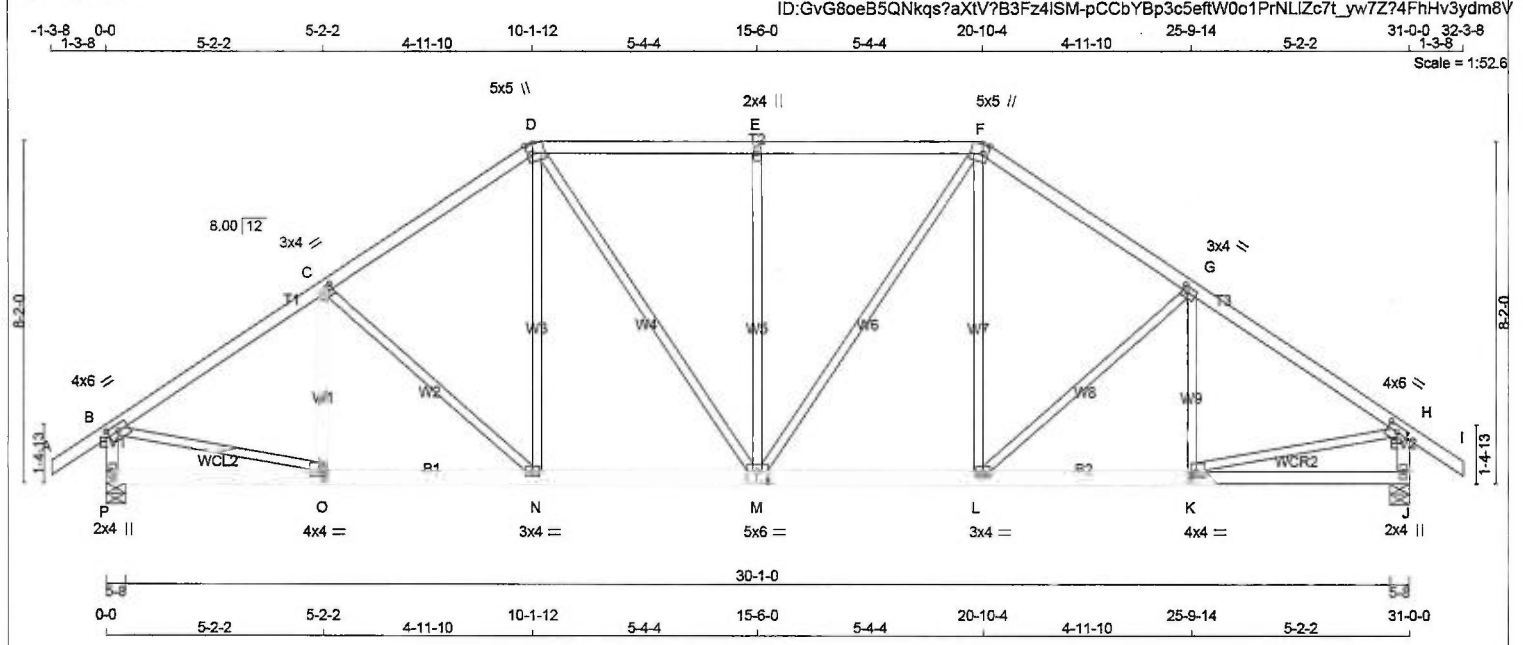
JSI GRIP= 0.88 (D) (INPUT = 0.90)
JSI METAL= 0.56 (C) (INPUT = 1.00)



September 14, 2018

**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**





LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER

A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-t	MT20	4.0	6.0	1.75 3.00
C	TMVW-t	MT20	3.0	4.0	1.50 1.50
D	TTWW+m	MT20	5.0	5.0	2.50 1.50
E	TMVW-w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	5.0	2.50 1.50
G	TMVW-t	MT20	3.0	4.0	1.50 1.50
H	TMVW-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMVW-t	MT20	4.0	4.0	1.50 1.50
L	BMVW-t	MT20	3.0	4.0	
M	BSVWV-t	MT20	5.0	6.0	3.00 3.00
N	BMVW-t	MT20	3.0	4.0	
O	BMVW-t	MT20	4.0	4.0	1.50 1.50
P	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

DESCR.	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
P	SPF	VERT 1577	DOWN 1577	0	5-8
J	SPF	HORZ 1577	UP/LIFT 1577	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE PERMILIVE WIND DEAD SOIL
P	1104	786 / 0	0 / 0 0 / 0 318 / 0 0 / 0
J	1104	786 / 0	0 / 0 0 / 0 318 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	O-C	-210 / 22	0.07 (1)
B-C	-1727 / 0	-77.4	-77.4 0.30 (1)	4.79	C-N	-240 / 0	0.21 (1)
C-D	-1565 / 0	-77.4	-77.4 0.29 (1)	4.99	N-D	0 / 252	0.06 (1)
D-E	-1485 / 0	-77.4	-77.4 0.31 (1)	5.05	D-M	0 / 360	0.08 (1)
E-F	-1485 / 0	-77.4	-77.4 0.31 (1)	5.05	M-E	-505 / 0	0.65 (1)
F-G	-1565 / 0	-77.4	-77.4 0.29 (1)	4.99	G-H	0 / 360	0.08 (1)
G-H	-1727 / 0	-77.4	-77.4 0.30 (1)	4.79	H-I	0 / 252	0.06 (1)
H-I	0 / 29	-77.4	-77.4 0.10 (1)	10.00	L-G	-240 / 0	0.21 (1)
P-B	-1538 / 0	0.0	0.0 0.18 (1)	6.64	K-G	-210 / 22	0.07 (1)
J-H	-1538 / 0	0.0	0.0 0.16 (1)	6.64	B-O	0 / 1489	0.34 (1)
P-O	0 / 0	-17.5	-17.5 0.10 (4)	10.00	K-H	0 / 1489	0.34 (1)
O-N	0 / 1458	-17.5	-17.5 0.29 (1)	10.00			
N-M	0 / 1282	-17.5	-17.5 0.28 (1)	10.00			
M-L	0 / 1282	-17.5	-17.5 0.28 (1)	10.00			
L-K	0 / 1458	-17.5	-17.5 0.29 (1)	10.00			
K-J	0 / 0	-17.5	-17.5 0.10 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 23.3	PSF
	DL = 3.0	PSF
BOT CH.	LL = 0.0	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.31/1.00 (D-E:1), BC=0.29/1.00 (N-O:1), WB=0.65/1.00 (E-M:1), SSI=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)



September 14, 2018

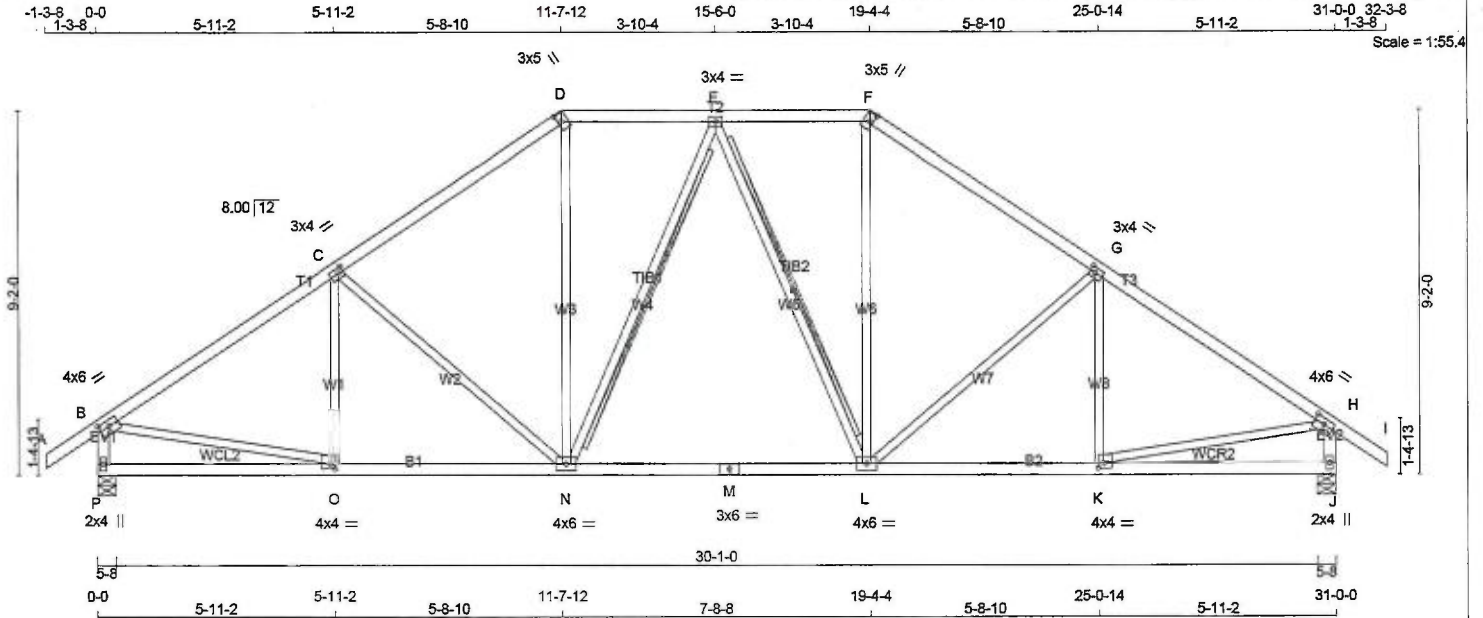
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Kott, Ottawa, Ontario, IM

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LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-t	MT20	4.0	8.0	1.75 3.00
C	TMWV-t	MT20	3.0	4.0	1.50 1.50
D	TTW-h	MT20	3.0	5.0	2.50 1.00
E	TMWV-t	MT20	3.0	4.0	
F	TTW-h	MT20	3.0	5.0	2.50 1.00
G	TMWV-t	MT20	3.0	4.0	1.50 1.50
H	TMWV-t	MT20	4.0	6.0	1.75 3.00
J	BMV1+p	MT20	2.0	4.0	
K	BMWVW-t	MT20	4.0	4.0	1.50 1.50
L	BMWVW-t	MT20	4.0	6.0	
M	BS-t	MT20	3.0	6.0	
N	BMWVW-t	MT20	4.0	6.0	
O	BMWVW-t	MT20	4.0	4.0	1.50 1.50
P	BMV1+p	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING

DESIGNER BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	1577	0	1577	0	5-8	2-11
J	1577	0	1577	0	5-8	2-11

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0
J	1104	786 / 0	0 / 0	0 / 0	0 / 0	318 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT E-N, E-L

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 29	-77.4	-77.4 0.10 (1)	10.00	O-C	-163 / 44 0.07 (1)	
B-C	-1739 / 0	-77.4	-77.4 0.41 (1)	4.65	C-N	-344 / 0 0.41 (1)	
C-D	-1488 / 0	-77.4	-77.4 0.38 (1)	4.97	N-D	0 / 524 0.12 (1)	
D-E	-1217 / 0	-77.4	-77.4 0.15 (1)	5.65	E-E	-193 / 0 0.15 (1)	
E-F	-1217 / 0	-77.4	-77.4 0.15 (1)	5.65	E-L	-193 / 0 0.15 (1)	
F-G	-1488 / 0	-77.4	-77.4 0.38 (1)	4.97	L-F	0 / 524 0.12 (1)	
G-H	-1739 / 0	-77.4	-77.4 0.41 (1)	4.65	L-G	-344 / 0 0.41 (1)	
H-I	0 / 29	-77.4	-77.4 0.10 (1)	10.00	K-G	-163 / 44 0.07 (1)	
P-B	-1532 / 0	0.0	0.0 0.16 (1)	6.65	B-O	0 / 1496 0.34 (1)	
J-H	-1532 / 0	0.0	0.0 0.16 (1)	6.65	K-O	0 / 1496 0.34 (1)	
P-O	0 / 0	-17.5	-17.5 0.13 (4)	10.00			
O-N	0 / 1472	-17.5	-17.5 0.33 (1)	10.00			
N-M	0 / 1292	-17.5	-17.5 0.30 (1)	10.00			
M-L	0 / 1292	-17.5	-17.5 0.30 (1)	10.00			
L-K	0 / 1472	-17.5	-17.5 0.33 (1)	10.00			
K-J	0 / 0	-17.5	-17.5 0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	23.3	PSF
	DL =	3.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	33.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.06")
ALLOWABLE DEFL.(TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.16")

CSI: TC=0.41/1.00 (B-C-1), BC=0.33/1.00 (N-O-1), WB=0.41/1.00 (C-N-1), SSI=0.18/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1567
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)



September 14, 2018

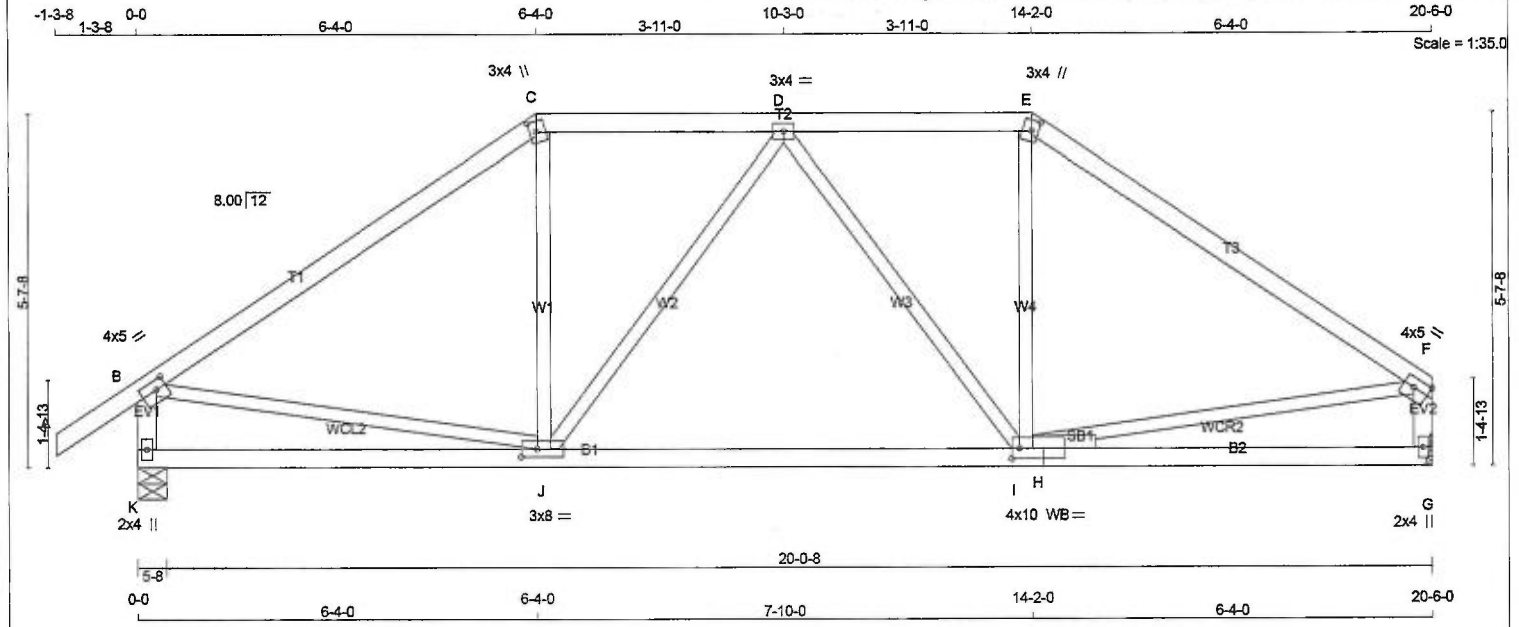
READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.



Kott, Ottawa, Ontario, IM

Version 8.210 S Mar 12 2018 MiTek Industries, Inc. Fri Sep 14 10:04:16 2018 Page 1

ID:GvG8oeB5QNkqs?axIV?B3Fz4ISM-mbKLytrJ7juN6pAA9qrQAewthgkO9MIYZAOzydm8T



TOTAL WEIGHT = 83 lb [M/F]

LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
K - B	2x4	DRY	No.2
G - F	2x4	DRY	No.2
K - H	2x4	DRY	No.2
H - G	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-t	MT20	4.0	5.0	1.75 2.00
C	TTW+m	MT20	3.0	4.0	2.00 1.25
D	TMVW-t	MT20	3.0	4.0	
E	TTW+m	MT20	3.0	4.0	2.00 1.25
F	TMVW-t	MT20	4.0	5.0	1.75 Edge
G	BMV1+p	MT20	2.0	4.0	
H	BSWVW-I	MT20	4.0	10.0	2.00 1.50
I	BMVWVW-I	MT20	3.0	8.0	1.50 3.00
K	BMV1+p	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

WB - INDICATES BLOCKING REQUIRED

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1079	0	1079	0	0	5-8	1-13
G	972	0	972	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
K	755	541 / 0	0 / 0	0 / 0	0 / 0	213 / 0	0 / 0
G	682	477 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
FR-TO				FR-TO			
A-B	0 / 29	-77.4	0.10 (1)	J-C	0 / 183	0.04 (4)	
B-C	-883 / 0	-77.4	0.42 (1)	J-D	-225 / 0	0.18 (1)	
C-D	-823 / 0	-77.4	0.15 (1)	D-I	-225 / 0	0.18 (1)	
D-E	-823 / 0	-77.4	0.15 (1)	I-E	0 / 183	0.04 (4)	
E-F	-883 / 0	-77.4	0.42 (1)	B-J	0 / 829	0.19 (1)	
K-B	-1034 / 0	0.0	0.11 (1)	I-F	0 / 829	0.19 (1)	
G-F	-828 / 0	0.0	0.10 (1)				
K-J	0 / 0	-17.5	-17.5 0.20 (4)				
J-I	0 / 952	-17.5	-17.5 0.28 (4)				
I-H	0 / 0	-17.5	-17.5 0.20 (4)				
H-G	0 / 0	-17.5	-17.5 0.20 (4)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 23.3 PSF
DL = 3.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 33.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.68")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.68")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.42/1.00 (E-F:1), BC=0.26/1.00 (I-J:4), WB=0.19/1.00 (F-I:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES				
PLATE	GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	MAX	MIN
MT20	618	354	1667	822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

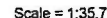
JSI GRIP= 0.87 (C) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)



September 14, 2018

READ ALL NOTES ON THIS PAGE AND ON THE ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE IS AN INTEGRAL PART OF THIS DRAWING AS IT CONTAINS SPECIFICATIONS AND CRITERIA USED IN THE DESIGN OF THIS COMPONENT.





TOTAL WEIGHT = 2 X 57 = 115 lb

DESCR
SPF
SPF
SPF
SPF
SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT G TO RESIST THE MAX FACTORED REACTIONS.

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) !

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

LOADING
TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:				
TOP CH.	LL	=	23.3	PSF
	DL	=	3.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	33.3	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9, NBC
2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 27.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.3 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.46")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.46")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.14/1.00 (B-C:1), BC=0.27/1.00 (G-H:4),
WB=0.26/1.00 (E-G:1), SSI=0.11/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

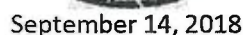
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

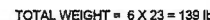
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.25 (E) (INPUT = 1.00)



**READ ALL NOTES ON THIS PAGE AND ON THE
ENGINEERING NOTE PAGE ENP-1. THE NOTE PAGE
IS AN INTEGRAL PART OF THIS DRAWING AS IT
CONTAINS SPECIFICATIONS AND CRITERIA USED
IN THE DESIGN OF THIS COMPONENT.**



 KOTT

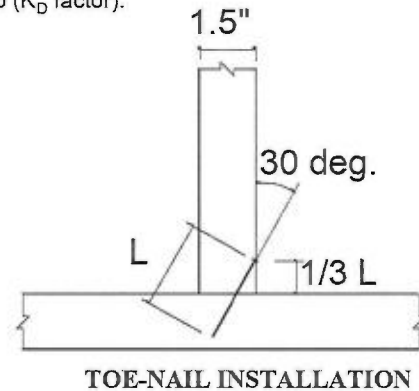
BEARING ANCHORAGE BY TOE-NAILS FOR LATERAL CAPACITY

B37579H1

NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL LATERAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON WIRE	3.00	0.144	132	147
	3.25	0.144	132	147
	3.50	0.160	159	177

NOTES:

1. Rafter and ceiling members may be anchored to top and bottom chords of girder truss by toe-nailing rafter and ceiling members to girder chords provided the reaction does not exceed the lateral capacities in the table. Hangers (specified by others) are required for reactions higher than the maximum toe-nail capacity. Reactions are based on factored loads.
2. Toe nail capacities shown in the table are for one toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.4.1.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in tables below.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately $1/3$ the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See next page for nailing on bearing plate).
7. For loads due to wind the nail lateral capacity in this table may be multiplied by 1.15 (K_D factor).
8. Lumber must be dry ($< 19\%$ moisture content) at the time of nail installation.
9. Nail values in this table comply with CSA O86-09, section 10.9.4
10. This design is not valid after April 30, 2019.



Nail type	Common wire	Common spiral	Common wire
Nail dia. (in)	0.160	0.152	0.144
	(3.5" nail)		(3" and 3.25" nail)
LUMBER SIZE	MAXIMUM NUMBER OF TOE-NAILS		
2X4 SPF	2	2	3
2X4 D. Fir	2	2	2
2X6 SPF	4	4	4
2X6 D. Fir	3	3	3



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

PEO
Certificate No. 10889485



April 26, 2017

BEARING ANCHORAGE BY TOE-NAILS FOR WIND LOADING

B37579H2

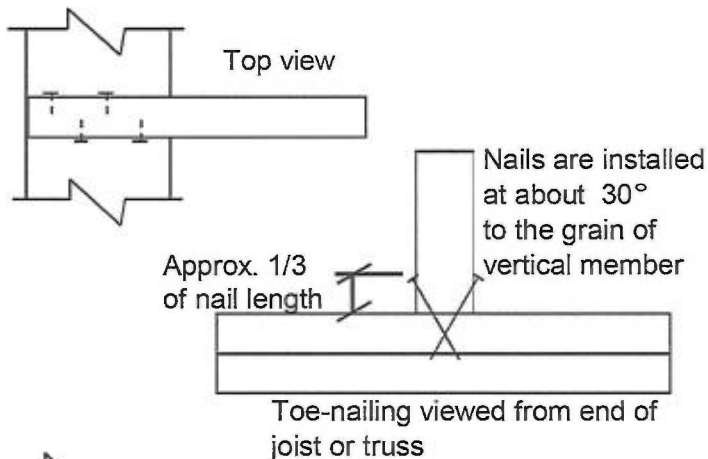
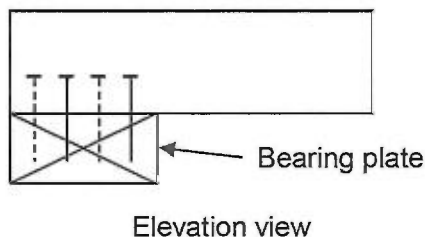
NAIL TYPE	LENGTH (IN)	DIAMETER (IN)	NAIL WITHDRAWAL CAPACITY (LB)	
			S-P-F	D. FIR
COMMON	3.00	0.144	30	42
WIRE	3.25	0.144	32	45
	3.50	0.160	38	52

Note: If using truss with D. Fir lumber and S-P-F bearing plate, use values in table for S-P-F.

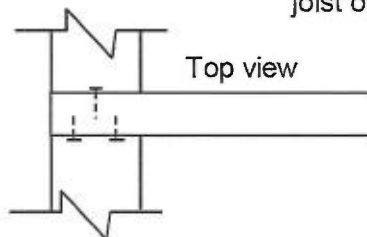
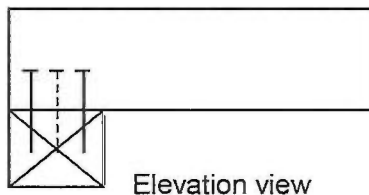
NOTES:

1. Truss chord, rafter, or ceiling members may be anchored to bearing plate by toe-nails, provided that the actual factored uplift force due to **wind or earthquake** load does not exceed the withdrawal capacities in the table. Hangers (specified by others) are required for uplift forces that are higher than the maximum toe-nail withdrawal capacity.
2. Toe nail capacities shown in the table are for **one** toe-nail. For additional toe-nails multiply values in table by the number of toe-nails used. Toe-nail capacities take into account toe-nailing factor J_A in CSA O86-09, section 10.9.5.2.
3. For 9- 3/4 gauge 3.25" common wire gun nails (diameter = 0.120") use 3" common spiral nail values.
4. Maximum number of toe-nails allowed depends on the lumber size & species to be toe-nailed to supporting member and nail diameter, as shown in table above.
5. Nail values in table are based on the following relative lumber densities: $G = 0.42$ (SPF), $G = 0.49$ (D. Fir).
6. Toe-nails shall be driven at approximately 1/3 the nail length from the edge of the joist/truss chord and driven at an angle of 30° to the grain of the member (See drawing on detail B37579H1).
7. Lumber must be dry (< 19% moisture content) at the time of nail installation.
8. Nail values in this table comply with CSA O86-09, section 10.9.5
9. This design is not valid after April 30, 2019

Toe-nailing on 2x6 Bearing Plate



Toe-nailing on 2x4 Bearing Plate



MiTek Canada Inc
100 Industrial Rd.
Bradford, Ontario L3Z 3G7

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Certificate No. 10889485



April 26, 2017