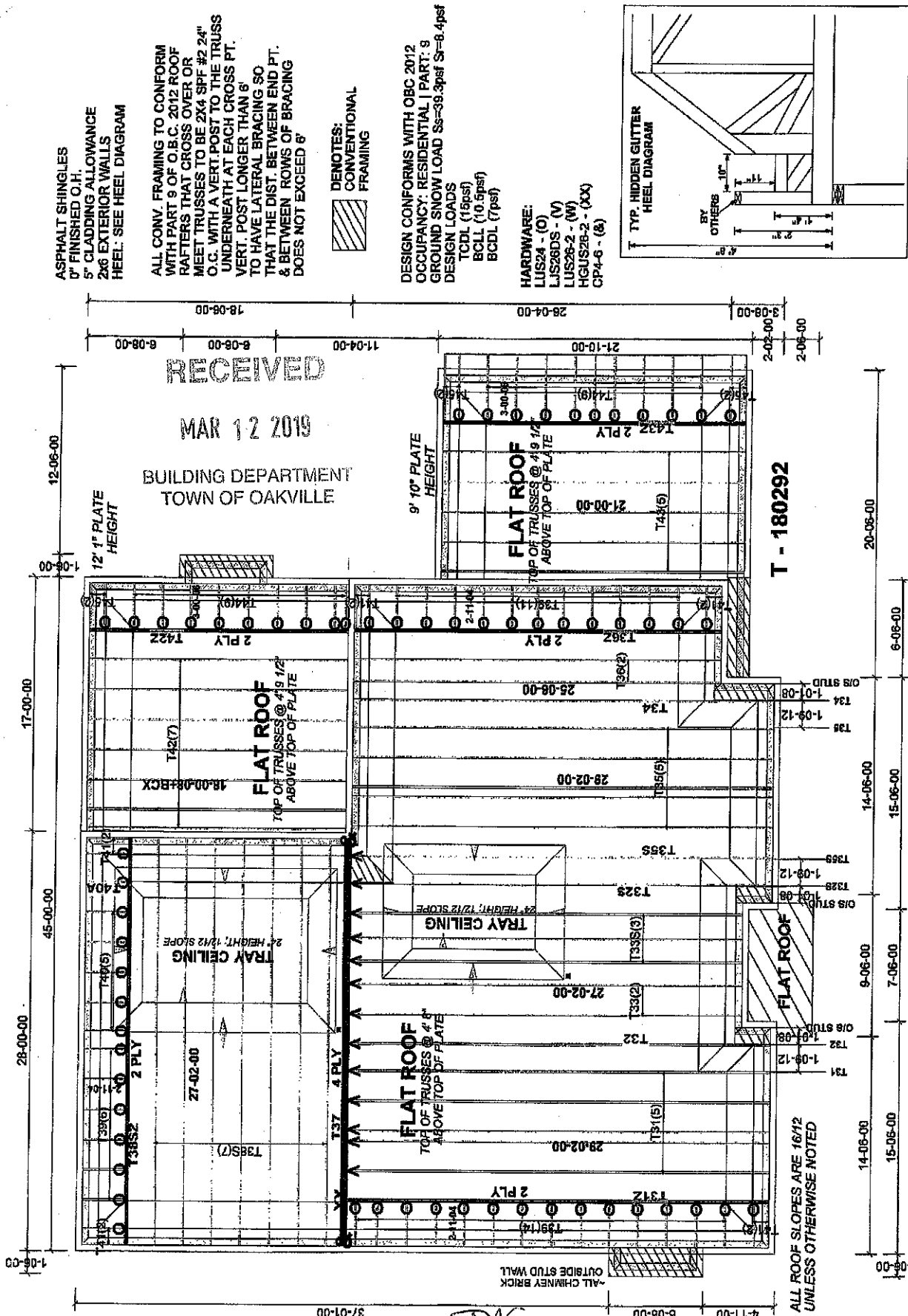


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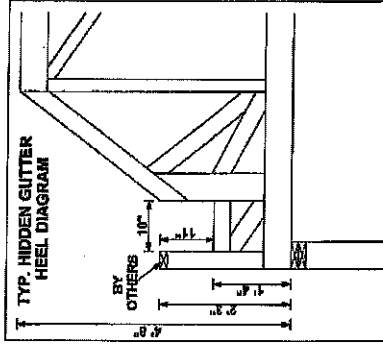
ASPHALT SHINGLES
5" FINISHED O.H.
2x6 EXTERIOR WALLS
HEEL: SEE HEEL DIAGRAM

ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. 2012 ROOF
RAFTERS THAT CROSS OVER OR
MEET TRUSSES TO BE 2x4 SPF #2 24"
O.C. WITH A VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6"
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

DENOTES:
CONVENTIONAL
FRAMING

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL / PART: 9
GROUND SNOW LOAD $S_s = 35.3 \text{ psf}$ $S_i = 8.4 \text{ psf}$
DESIGN LOADS
TCDL (15psf)
BCDL (10.5psf)
BCDL (7psf)

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
LUS26-2 - (W)
HGUS26-2 - (XX)
CP4-G - (6)



GREEN YORK HOMES Project: TIBURTINO Date: 05/28/18 Designer: calish		Builder / Location: GREEN YORK HOMES	Model / Elevation: GEORGIAN 3 / B LOT 1
Job Track: 45487 Layout ID: 292709 Plan Log: 95790	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.		





Delivery Shiplist

DATE	03/05/18
SALES REP	Rick

JOB TRACK: 45487 LAYOUT ID: 292707 LOCATION: OAKVILLE
 BUILDER: GREEN YORK HOMES / TIBURTINO SUB-BUILDER:
 MODEL: GEORGIAN 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	7	T1 FLAT	0.00 0.00	23-02-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	788.41 491.19		
	1	G1 GABLE	0.00 0.00	23-02-00	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	76.18 50.00		
	2	T2 FLAT GIRDER	0.00 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	91.12 60.00		
	1 3 Ply	T2Z FLAT GIRDER	0.00 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	136.68 90.00		
	1 3 Ply	T2Z2 FLAT GIRDER	0.00 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	136.68 90.00		
	3	T3 FLAT	0.00 0.00	05-07-00	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	72.45 48.00		
	6	T4 FLAT	0.00 0.00	19-08-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	578.22 352.02		
	1	G4 GABLE	0.00 0.00	19-08-00	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	65.62 42.67		
	1 2 Ply	T5 FLAT GIRDER	0.00 0.00	19-08-00	02-02-00	2 X 8	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	226.44 141.34		
	2 2 Ply	T6 FLAT GIRDER	0.00 0.00	13-00-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	278.68 181.32		
	1	G7 GABLE	0.00 0.00	13-00-00	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	46.65 30.33		
	1 2 Ply	T8 FLAT	0.00 0.00	15-05-00	02-02-00	2 X 8	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	176.98 112.00		
	1	G8 GABLE	0.00 0.00	15-06-00	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	53.54 35.33		
	1 3 Ply	T10 FLAT GIRDER	0.00 0.00	14-04-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	238.38 159.99		
	2 2 Ply	T11 FLAT GIRDER	0.00 0.00	13-08-08	02-02-00	2 X 8	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	316.40 205.32		
	1	T12 FLAT	0.00 0.00	13-08-08	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	50.30 32.17		
	1 2 Ply	T13 FLAT	0.00 0.00	09-00-00	02-02-00	2 X 6	2 X 6	00-00-00 00-00-00	02-02-00 02-02-00	88.96 56.00		
	6	T14 FLAT	0.00 0.00	13-07-08	02-02-00	2 X 4	2 X 4	00-00-00 00-00-00	02-02-00 02-02-00	300.24 193.02		

RECEIVED

MAR 12 2019

BUILDING DEPARTMENT
TOWN OF OAKVILLE

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19-0247



Delivery Shiplist

DATE	03/05/18
SALES REP	Rick

JOB TRACK: 45487	LAYOUT ID: 292707	LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO	SUB-BUILDER:	
MODEL: GEORGIAN 3	ELEVATION: A	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER TOP BOT	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	1	T15	0.00	13-07-08	02-02-00	2 X 6 2 X 6	00-00-00	02-02-00	237.54		
	3 Ply	FLAT GIRDER	0.00				00-00-00	02-02-00	156.00		
	1	G16	0.00	05-04-08	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	24.00		
		GABLE	0.00				00-00-00	02-02-00	17.00		
	1	T17	0.00	18-04-00	02-02-00	2 X 6 2 X 8	00-00-00	02-02-00	281.82		
	3 Ply	FLAT GIRDER	0.00				00-00-00	02-02-00	179.01		
	5	T20	0.00	17-05-00	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	315.80		
		FLAT	0.00				00-00-00	02-02-00	205.00		
	3	T21	0.00	13-09-00	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	151.29		
		FLAT	0.00				00-00-00	02-02-00	96.51		
	1	G21	0.00	13-09-00	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	48.26		
		GABLE	0.00				00-00-00	02-02-00	31.67		
	5	T22	0.00	15-09-08	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	283.25		
		FLAT	0.00				00-00-00	02-02-00	181.65		
	1	G22X	0.00	17-05-08	02-02-00	2 X 4 2 X 4	00-00-00	02-02-00	59.29		
		GABLE	0.00				00-00-00	02-02-00	39.00		
	1	T23	0.00	15-09-08	02-02-00	2 X 6 2 X 6	00-00-00	02-02-00	164.48		
	2 Ply	FLAT GIRDER	0.00				00-00-00	02-02-00	101.34		

TOTAL # TRUSS= 76

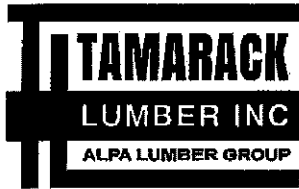
TOTAL BFT OF ALL TRUSSES=

3377.88 BFT. TOTAL WEIGHT OF ALL TRUSSES= 5287.56 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
29	Hangers	LJS26DS	
10	Hangers	HGUS28-2	
2	Hangers	HGUS28-3	
2	Beam	1 3/4" x 16" LVL 2.0E	18-00-00

TOTAL # ITEMS= 43



Delivery Shiplist

DATE	05/28/18
SALES REP	Rick

JOB TRACK: 45487	LAYOUT ID: 292709	LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO	SUB-BUILDER:	
MODEL: GEORGIAN 3	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	5	T31 ROOF	16.00 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	800.20 493.35		
	1 2 Ply	T31Z ROOF	16.00 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	320.08 197.34		
	1	T32 ROOF	16.00 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	159.08 96.67		
	1	T32S ROOF	16.00 12.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	190.23 120.83		
	2	T33 ROOF	16.00 0.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	302.72 185.00		
	3	T33S ROOF	16.00 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-08-00	525.81 334.50		
	1	T34 ROOF	16.00 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	158.04 99.67		
	5	T35 ROOF	16.00 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	813.30 503.35		
	1	T35S ROOF	16.00 12.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	182.74 116.50		
	2	T36 ROOF	16.00 0.00	25-06-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	284.90 177.34		
	1 2 Ply	T36Z ROOF	16.00 0.00	25-06-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	284.90 177.34		
	1 4 Ply	T37 ROOF	16.00 0.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	722.20 465.32		
	7	T38S ROOF	16.00 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	1206.66 775.81		
	1 2 Ply	T38SZ ROOF	16.00 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	344.76 221.66		
	31	T39 ROOF	16.00 0.00	02-11-04	04-08-00	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 04-08-00	673.32 459.73		
	5	T40 ROOF	16.00 0.00	02-11-04	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-02-08	127.65 93.35		
	1	T40A ROOF	16.00 0.00	02-11-04	04-08-00	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 04-02-08	26.60 19.33		
	10	T41 FLAT	0.00 0.00	02-11-04	02-01-08	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 01-04-00	113.20 75.00		



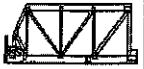
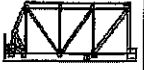
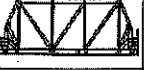
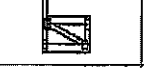
Delivery Shiplist

DATE	05/28/18
SALES REP	Rick

JOB TRACK: 45487	LAYOUT ID: 292709	LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO	SUB-BUILDER:	
MODEL: GEORGIAN 3	ELEVATION: B	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	7	T42 ROOF	16.00 0.00	18-00-08	04-09-08	2 X 4	2 X 8	00-00-00 00-00-00	01-04-00 00-07-04	845.53 511.00		
	1 2 Ply	T42Z ROOF	16.00 0.00	18-00-08	04-09-08	2 X 4	2 X 8	00-00-00 00-00-00	01-04-00 00-07-04	241.58 146.00		
	5	T43 ROOF	16.00 0.00	21-00-00	04-09-08	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	620.40 395.85		
	1 2 Ply	T43Z ROOF	16.00 0.00	21-00-00	04-09-08	2 X 4	2 X 6	00-00-00 00-00-00	01-04-00 01-04-00	248.16 158.34		
	18	T44 ROOF	16.00 0.00	03-00-06	04-09-08	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 04-09-08	400.86 291.06		
	6	T45 FLAT	0.00 0.00	03-00-06	02-01-08	2 X 4	2 X 4	00-00-00 00-00-00	01-04-00 01-04-00	69.66 49.02		

TOTAL # TRUSS= 125.00

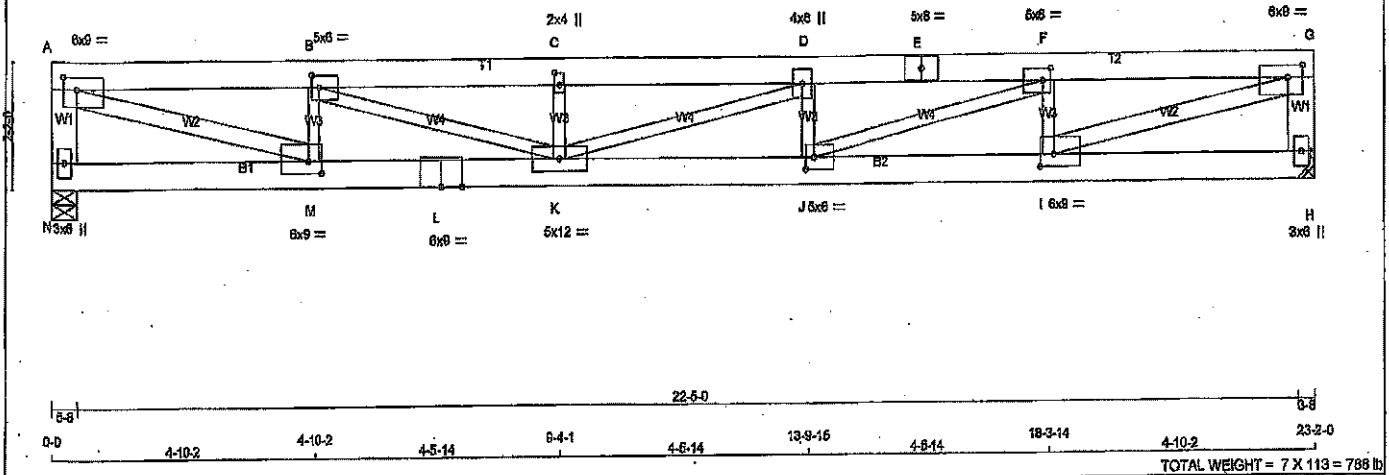
TOTAL BFT OF ALL TRUSSES=

6163.36 BFT. TOTAL WEIGHT OF ALL TRUSSES= 9662.58 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
63	Hangers	LUS24	
13	Hangers	LJS26DS	
1	Hangers	HGUS26-2	
2	Crush Plates	CP4-6	

TOTAL # ITEMS= 79.00



LUMBER N. L. G. A. RULES CHORDS SIZE N - A 2x6 DRY No.2 SPF A - E 2x6 DRY 2100F 1.8E SPF E - G 2x6 DRY 2100F 1.8E SPF H - G 2x6 DRY No.2 SPF N - L 2x6 DRY 2100F 1.8E SPF L - H 2x6 DRY 2100F 1.8E SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT I - G 2x4 DRY No.2 SPF A - M 2x4 DRY No.2 SPF DRY: SEASONED LUMBER.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 30.0 PSF DL = 15.0 PSF BOT CH. LL = 10.0 PSF DL = 7.0 PSF TOTAL LOAD = 62.5 PSF SPACING = 24.0 IN. C/C LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF NBC 2012, BCBC 2012, ABC 2014 - CSA 088-09 - TRC 2011 (65% OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.77") CALCULATED VERT. DEFL.(LL) = L/899 (0.23") ALLOWABLE DEFL.(TL) = L/360 (0.77") CALCULATED VERT. DEFL.(TL) = L/820 (0.45") CSL TC=0.27/1.00 (C-D:1), BC=0.48/1.00 (J-K:1), WB=0.93/1.00 (A-M:1), SS=0.27/1.00 (A-B:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.60 FLAT ROOF FACTOR = 0.75 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1867 822 2294 1856 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.89 (F) (INPUT = 0.90) JSI METAL= 0.99 (A) (INPUT = 1.00)			
PLATES (table in inches) JT TYPE PLATES W LEN Y X A TMWW-1 MT20 6.0 9.0 2.50 3.00 B TMWW-1 MT20 5.0 6.0 2.50 1.75 C TMWW-1 MT20 2.0 4.0 2.50 1.00 D TMWW-1 MT20 4.0 6.0 E TS-1 MT20 5.0 6.0 F TMWW-1 MT20 5.0 6.0 2.50 1.75 G TMWW-1 MT20 6.0 9.0 2.50 3.00 H BMV1+p MT20 3.0 6.0 I BMWW-1 MT20 6.0 9.0 2.50 3.00 J BMWW-1 MT20 5.0 6.0 2.50 1.75 K BMWW-1 MT20 5.0 12.0 L BS-1 MT20 6.0 9.0 M BMWW-1 MT20 6.0 9.0 2.50 3.00 N BMV1+p MT20 3.0 6.0				UNFACTORED REACTIONS 1ST LOASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL N 1447 894 / 0 243 / 0 0 / 0 510 / 0 0 / 0 H 1447 894 / 0 243 / 0 0 / 0 510 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE VERT. LOAD LC1 MAX MAX. MEMB. FORCE MAX MEMB. (LBS) (PLF) CSI (LC) LENGTH FR-TO (LBS) CSI (LC) FR-TO N-A -1716 / 0 0.0 0.0 0.16 (1) 6.77 I-G 0 / 4323 0.93 (1) A-B -4079 / 0 -127.4 -127.4 0.21 (1) 4.46 A-M 0 / 4325 0.93 (1) B-C -5786 / 0 -127.4 -127.4 0.24 (1) 3.84 I-F -1308 / 0 0.25 (1) C-D -5786 / 0 -127.4 -127.4 0.27 (1) 3.80 M-B -1308 / 0 0.25 (1) D-E -5794 / 0 -127.4 -127.4 0.23 (1) 3.84 J-F 0 / 1837 0.95 (1) E-F -5794 / 0 -127.4 -127.4 0.23 (1) 3.84 B-K 0 / 1826 0.65 (1) F-G -4077 / 0 -127.4 -127.4 0.21 (1) 4.46 J-D -541 / 0 0.10 (1) G-H -1716 / 0 0.0 0.0 0.16 (1) 6.77 K-C -539 / 0 0.10 (1) K-D -8 / 0 0.00 (1) N-M 0 / 0 -28.0 -28.0 0.07 (3) 10.00 M-L 0 / 4079 -28.0 -28.0 0.35 (1) 10.00 L-K 0 / 4079 -28.0 -28.0 0.35 (1) 10.00 K-J 0 / 5794 -28.0 -28.0 0.46 (1) 10.00 J-I 0 / 4077 -28.0 -28.0 0.35 (1) 10.00 I-H 0 / 0 -28.0 -28.0 0.07 (3) 10.00			



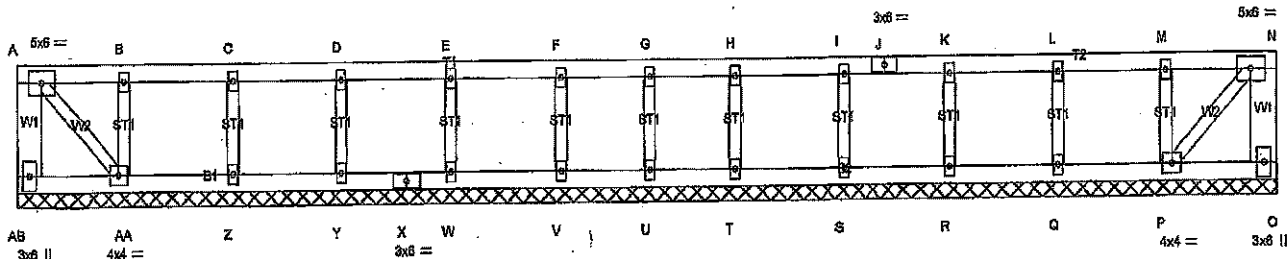
DRWG NO. TAM / 2007-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 292707	TRUSS NAME G1	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Version 8.200 8 Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 13:48:13 2018 Page 1
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Vanaraack Roof Truss, Burlington

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2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0 1-7-0 1-7-0 2-0-0 2-0-0 2-0-0 2-0-0 2-0-0
Scale = 1:37.7



LUMBER			
N. L. C. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
AB - A	2x6	DRY	No.2
A - J	2x4	DRY	No.2
J - N	2x4	DRY	No.2
O - N	2x6	DRY	No.2
AB - X	2x4	DRY	No.2
X - O	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table 1a in inches)			
JT TYPE	PLATES	W	LEN Y X
A TMWV-1	MT20	5.0	8.0
B, C, D, E, F, G, H, I, K, L, M			
B TMWV-w	MT20	2.0	4.0
J TS-1	MT20	3.0	8.0
N TMWV-1	MT20	5.0	8.0
O BMV1+p	MT20	3.0	6.0
P BMWV1-1	MT20	4.0	4.0
Q, R, S, T, U, V, W, Y, Z			
Q BMW1+w	MT20	2.0	4.0
X BS-1	MT20	3.0	6.0
AA BMWV1-1	MT20	4.0	4.0
AB BMV1+p	MT20	3.0	8.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CS1 (LC)	MAX. MEMB. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			U-G	-186/0	0.04 (1)
AB-A	-98/0	0.0	0.0 0.01 (1)	7.81	P-M	-288/0	0.08 (1)
A-B	0/4	-127.4 -127.4	0.08 (1)	10.00	Q-L	-248/0	0.05 (1)
B-C	0/4	-127.4 -127.4	0.08 (1)	10.00	R-K	-255/0	0.08 (1)
C-D	0/4	-127.4 -127.4	0.08 (1)	10.00	S-I	-280/0	0.05 (1)
D-E	0/4	-127.4 -127.4	0.08 (1)	13.00	T-H	-232/0	0.05 (1)
E-F	0/4	-127.4 -127.4	0.07 (1)	10.00	AA-B	-288/0	0.08 (1)
F-G	0/4	-127.4 -127.4	0.07 (1)	10.00	Z-C	-248/0	0.05 (1)
G-H	0/4	-127.4 -127.4	0.07 (1)	10.00	Y-D	-285/0	0.05 (1)
H-I	0/4	-127.4 -127.4	0.08 (1)	10.00	W-E	-250/0	0.05 (1)
I-J	0/4	-127.4 -127.4	0.08 (1)	10.00	V-F	-232/0	0.05 (1)
J-K	0/4	-127.4 -127.4	0.08 (1)	10.00	A-AA	-5/0	0.00 (1)
K-L	0/4	-127.4 -127.4	0.08 (1)	10.00	P-N	-5/0	0.00 (1)
L-M	0/4	-127.4 -127.4	0.09 (1)	10.00			
M-N	0/4	-127.4 -127.4	0.09 (1)	10.00			
O-N	-98/0	0.0	0.0 0.01 (1)	7.81			
AB-AA	0/0	-28.0 -28.0	0.04 (3)	10.00			
AA-Z	-4/0	-28.0 -28.0	0.04 (3)	10.00			
Z-Y	-4/0	-28.0 -28.0	0.04 (3)	10.00			
Y-X	-4/0	-28.0 -28.0	0.04 (3)	10.00			
X-W	-4/0	-28.0 -28.0	0.04 (3)	10.00			
W-V	-4/0	-28.0 -28.0	0.04 (3)	10.00			
V-U	-4/0	-28.0 -28.0	0.03 (3)	10.00			
U-T	-4/0	-28.0 -28.0	0.03 (3)	10.00			
T-S	-4/0	-28.0 -28.0	0.04 (3)	10.00			
S-R	-4/0	-28.0 -28.0	0.04 (3)	10.00			
R-Q	-4/0	-28.0 -28.0	0.04 (3)	10.00			
Q-P	-4/0	-28.0 -28.0	0.04 (3)	10.00			
P-O	0/0	-28.0 -28.0	0.04 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH LL = 30.0 PSF

DL = 15.0 PSF

BOT CH LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00H2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TFC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.00/1.00 (B-C:1), BC=0.04/1.00 (Z-AA:3)

WB=0.08/1.00 (M-P:1), SS=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION

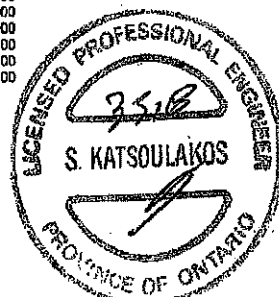
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1567	822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (AA) (INPUT = 0.90)

JSI METAL= 0.07 (M) (INPUT = 1.00)



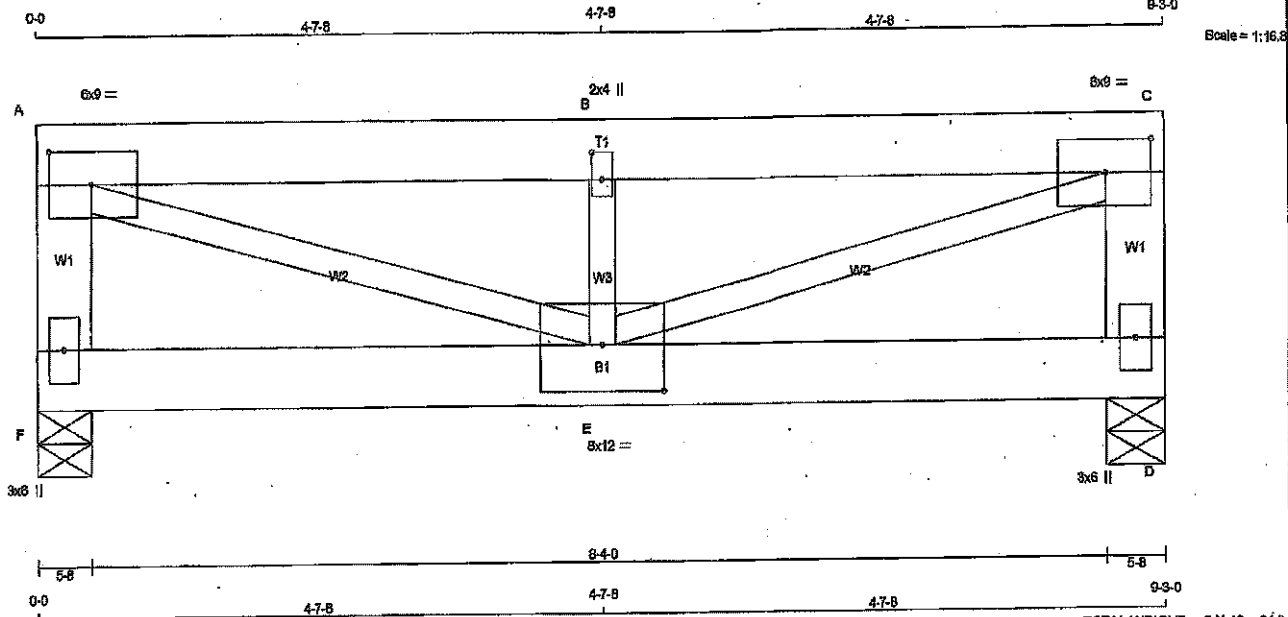
DRWG NO. TAM 1407-8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T2	2	1	TRUSS DESC.	

Remarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 48 = 96 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - A	2x6	DRY	No.2
A - C	2x6	DRY	No.2
D - C	2x6	DRY	No.2
F - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	6.0	9.0	3.00 4.25
B	TMVW-1	MT20	2.0	4.0	2.50 1.00
C	TMVW-1	MT20	6.0	9.0	3.00 4.25
D	BMV1+p	MT20	3.0	6.0	
E	BMVW-1	MT20	6.0	12.0	4.25 6.00
F	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
F	719	0	719	0	0	5-8	5-8	5-8	5-8
D	719	0	719	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	578	277 / 0	97 / 0	0 / 0	0 / 0	204 / 0	0 / 0
D	578	277 / 0	97 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM	TO	FR-TO			
F-A	-844 / 0	0.0	0.0 0.08 (1)	A-E	0 / 1140	0.34 (1)	
A-B	-1070 / 0	-127.4	-127.4 0.23 (1)	E-B	-682 / 0	0.13 (1)	
B-C	-1070 / 0	-127.4	-127.4 0.23 (1)	E-C	0 / 1140	0.34 (1)	
D-C	-844 / 0	0.0	0.0 0.08 (1)				
F-E	0 / 0	-28.0	-28.0 0.11 (3)				
E-D	0 / 0	-28.0	-28.0 0.11 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0	PSF
DL	= 15.0	PSF
BOT CH.	LL = 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCSC 2012, ABC 2014
- CSA 086-09
- TFC 2011

(55 % OF 89.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CS1: TC=0.23/1.00 (B-C:1), BC=0.11/1.00 (E-F:3), WB=0.34/1.00 (A-E:1), SB=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 L3 BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

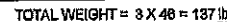
PLATE ROTATION TOL. = 5.0 Deg.

J&I GRIP= 0.62 (B) (INPUT = 0.60)
J&I MSTAL= 0.24 (A) (INPUT = 1.00)



DWG NO. TAM 1206618
STRUCTURAL
COMPONENT ONLY

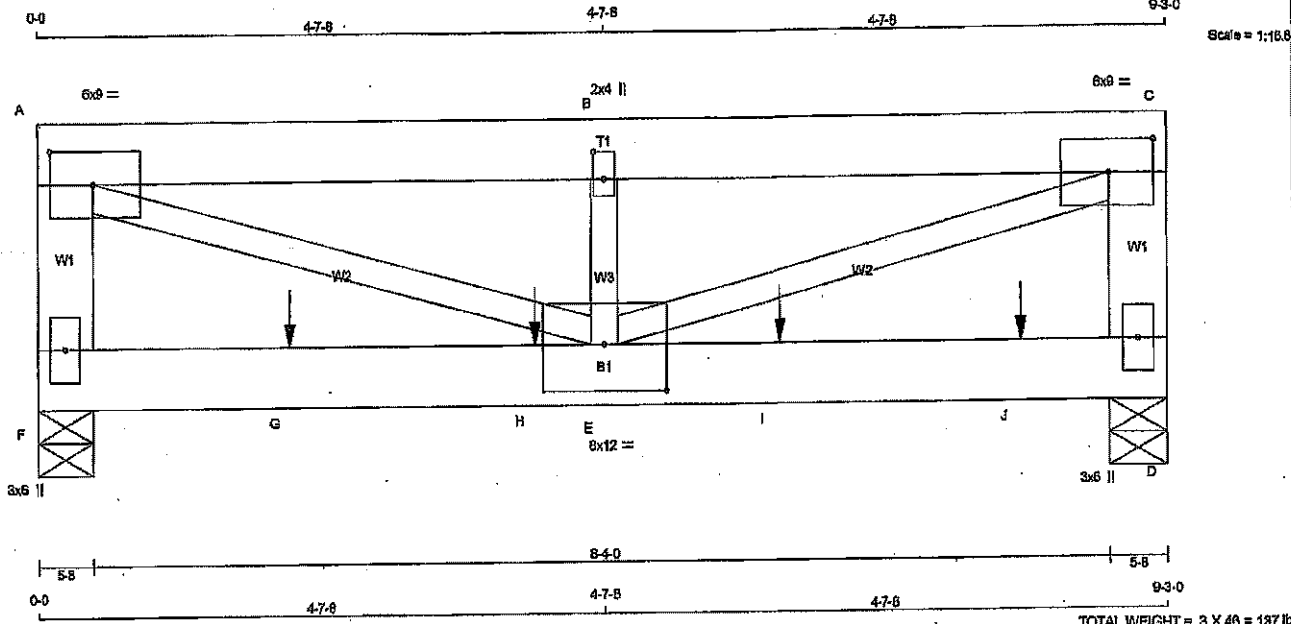
Version 6.200 S Jan 6 2018 Mitek Industries, Inc. Mon Jan 5 13:45:17 2018 Page 1
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DWG NO. TAM 12089-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 292707	TRUSS NAME T2ZZ	QUANTITY 1	PLY 3	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamirack Roof Truss, Burlington Version 8.280 S Jan 6 2018 MITek Industries, Inc. Mon Mar 5 13:48:17 2018 Page 1
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LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x8	DRY	No.2	SPF
A - C	2x8	DRY	No.2	SPF
D - C	2x8	DRY	No.2	SPF
F - D	2x8	DRY	No.2	SPF

ALL WEBS 2x8 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS: (0.122'x3') SPIRAL NAILS	SPACING (IN)	LOAD (PLF)
F-A	12	TOP
A-C	12	TOP
C-D	12	TOP

BOTTOM CHORDS: (0.122'x3') SPIRAL NAILS

F-D	SPACING (IN)	LOAD (PLF)
F-D	7	SIDE (391.3)

WEBS: (0.122'x3') SPIRAL NAILS

2x8	SPACING (IN)
2x8	6

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-4	MT20	6.0	9.0	3.00	4.25
B	TMW-4	MT20	2.0	4.0	2.50	1.00
C	TMW-4	MT20	6.0	9.0	3.00	4.25
D	BMV-4p	MT20	3.0	6.0		
E	BMV-4p	MT20	8.0	12.0	4.25	6.00
F	BMV-4p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1500.2 lbs FACTORED DOWN AT 2-0-12, 1500.2 lbs FACTORED DOWN AT 4-0-12, AND 1500.2 lbs FACTORED DOWN AT 8-0-12, AND 1500.2 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
F	3485	3485	0	5-8
D	4003	4003	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	WIND	DEAD	SOIL
F	2739	1344 / 0	433 / 0	0 / 0	862 / 0
D	3191	1567 / 0	593 / 0	0 / 0	1120 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
F-A	-2855 / 0	0.0 0.0	0.08 (1)	7.81	A-E	0 / 6568	0.66 (1)
A-B	-6180 / 0	-127.4 -127.4	0.20 (1)	4.88	E-B	-434 / 0	0.03 (1)
B-C	-6180 / 0	-127.4 -127.4	0.20 (1)	4.88	E-C	0 / 6568	0.66 (1)
D-C	-2855 / 0	0.0 0.0	0.08 (1)	7.81			
F-G	0 / 0	-28.0 -28.0	0.48 (1)	10.00			
G-H	0 / 0	-28.0 -28.0	0.48 (1)	10.00			
H-E	0 / 0	-28.0 -28.0	0.48 (1)	10.00			
E-I	0 / 0	-28.0 -28.0	0.47 (1)	10.00			
I-J	0 / 0	-28.0 -28.0	0.47 (1)	10.00			
J-D	0 / 0	-28.0 -28.0	0.47 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1500	-1500		BACK	VERT	TOTAL
H	4-0-12	-1500	-1500		BACK	VERT	TOTAL
I	6-0-12	-1500	-1500		BACK	VERT	TOTAL
J	8-0-12	-1500	-1500		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, AEC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 30.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/380 (0.31")
CALCULATED VERT. DEFL. (LL) = 1/888 (0.05")
ALLOWABLE DEFL. (TL) = 1/380 (0.31")
CALCULATED VERT. DEFL. (TL) = 1/599 (0.10")

CSI: TO=0.20/1.00 (A-B-1), BC=0.47/1.00 (D-E-1), WB=0.66/1.00 (A-E-1), SS=0.63/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

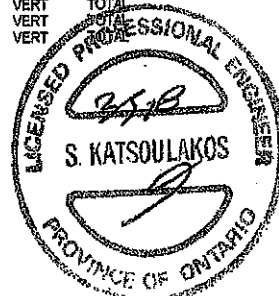
PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MAX	MIN	MAX	MIN

MT20 618 854 1687 622 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

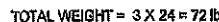
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.75 (C) (INPUT = 0.80)
JSI METAL = 0.48 (A) (INPUT = 1.00)



DRWG NO. TAM 12090-38
STRUCTURAL
COMPONENT ONLY

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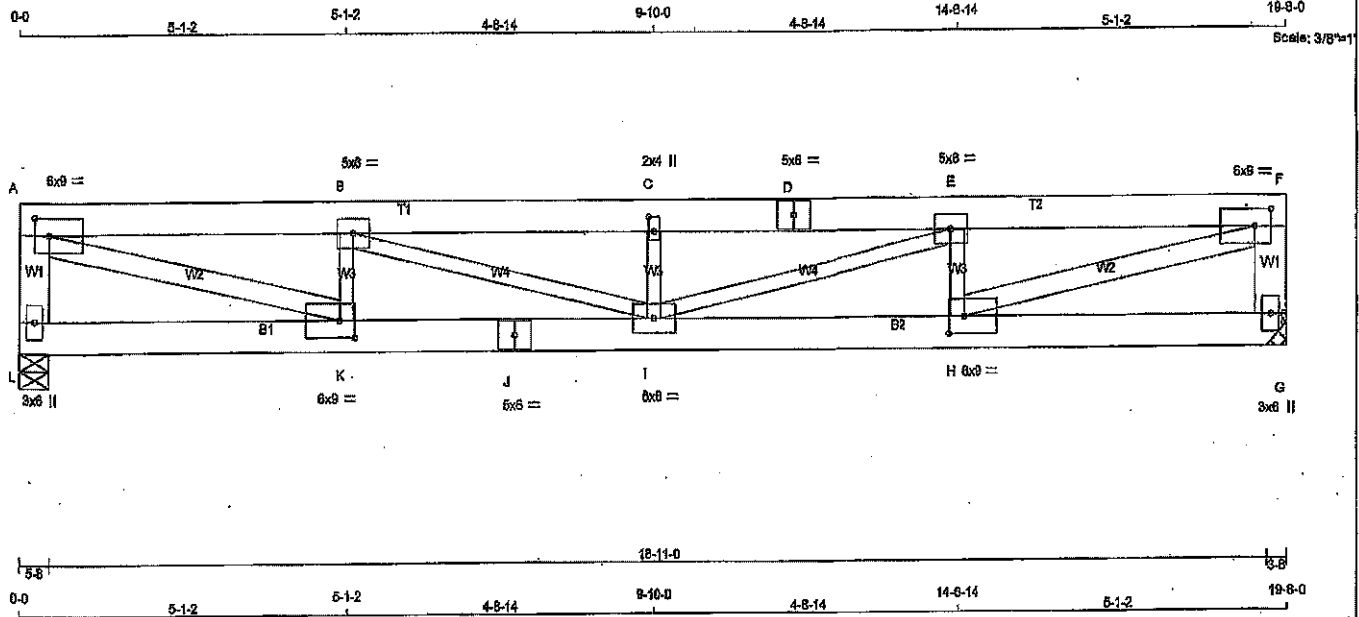
DWG NO. TAM 12071-10
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T4	6	1	TRUSS DESC.	

Tannerack Roof Truss, Burlington

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TOTAL WEIGHT = 6 X 96 = 576 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
L - A	2x6	DRY	No.2
A - D	2x6	DRY	No.2
D - F	2x6	DRY	No.2
G - F	2x6	DRY	No.2
L - J	2x6	DRY	No.2
J - G	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
H - F	2x4	DRY	No.2
A - K	2x4	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW-1	MT20	8.0	6.0	3.00 2.75
B	TMVW-1	MT20	6.0	6.0	
C	TMVW-1	MT20	2.0	4.0	2.50 1.00
D	TS-1	MT20	5.0	6.0	
E	TMVW-1	MT20	5.0	6.0	
F	TMVW-1	MT20	6.0	6.0	3.00 2.75
G	BMV1-p	MT20	3.0	6.0	
H	BMVW-1	MT20	6.0	6.0	3.00 2.75
I	BMVW-1	MT20	5.0	6.0	
J	BS-1	MT20	5.0	6.0	
K	BMVW-1	MT20	6.0	6.0	3.00 2.75
L	BMV1-p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION	GROSS REACTION	DOWN	HORZ	UP/LIFT	IN-SX	IN-SX	
JT	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	IN-SX
L	1528	0	1528	0	0	5-8	5-8
G	1528	0	1528	0	0		

HANGER BY OTHERS

MIN. SEAT SIZE: 3/8"

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1229	599 / 0	207 / 0	0 / 0	0 / 0	433 / 0	0 / 0
G	1229	599 / 0	207 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
L-A	-1445 / 0	0.0	0.0	0.13 (1)	7.24	H-F	0 / 3648
A-B	-3459 / 0	-127.4	-127.4	0.41 (1)	3.70	A-K	0 / 3648
B-C	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	H-E	-1024 / 0
C-D	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	K-B	-1024 / 0
D-E	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	I-E	0 / 859
E-F	-3459 / 0	-127.4	-127.4	0.41 (1)	3.70	B-I	0 / 859
G-F	-1445 / 0	0.0	0.0	0.13 (1)	7.24	I-G	-538 / 0
L-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00		
K-J	0 / 3459	-28.0	-28.0	0.69 (1)	10.00		
J-I	0 / 3459	-28.0	-28.0	0.69 (1)	10.00		
I-H	0 / 3459	-28.0	-28.0	0.69 (1)	10.00		
H-G	0 / 0	-28.0	-28.0	0.14 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0	PSF
DL = 15.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 62.5	PSF	

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 066-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.66")
CALCULATED VERT. DEFL.(LL) =	L/599 (0.17")
ALLOWABLE DEFL.(TL) =	L/360 (0.66")
CALCULATED VERT. DEFL.(TL) =	L/728 (0.32")

CSI: TC=0.41/1.00 (A-B:1), BC=0.69/1.00 (I-K:1), WB=0.78/1.00 (A-K:1), SS=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687
	822	2264	1659

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

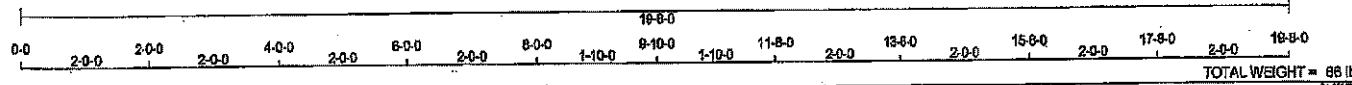
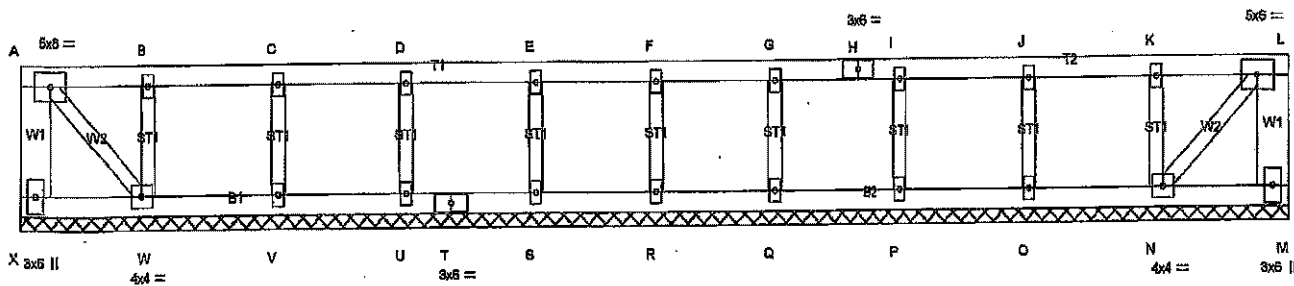
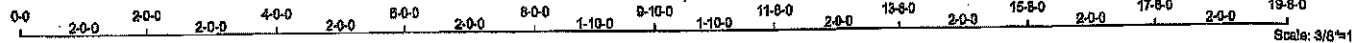
JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.86 (J) (INPUT = 1.00)



DRWG NO. TAM 12092-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G4	1	1	TRUSS DESC.	

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 ID:h6PRDILd80Q8u3LLONxW5zxNg4-R3NzWHmRLS1HwK00EnLVn8HBsYB0wky08AA2zdZGF
 Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 86 lb
 (M/R)

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE DRY LUMBER DESCR. SPF

X - A	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
X - T	2x4	DRY	No.2	SPF
T - M	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 ALL GABLE WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.
 GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
BEARINGS
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA
SPECIFIED LOADS:
 TOP CH. LL = 30.0 PSF
 DL = 15.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 62.5 PSF
SPACING = 24.0 IN. G/C
 LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 6, NBCC 2010

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0		
B, C, D, E, F, G, I, J, K						
B	TMVW-w	MT20	2.0	4.0		
H	TS-1	MT20	3.0	6.0		
L	TMVW-1	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		
N	BMVW-1	MT20	4.0	4.0		
O, P, Q, R, S, U, V						
O	BMV1+w	MT20	2.0	4.0		
T	BS-1	MT20	3.0	6.0		
W	BMVW-1	MT20	4.0	4.0		
X	BMV1+p	MT20	3.0	6.0		

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED CSI (LC)	MAX. FACTORED CSI (LC)
FR-TO		FROM		FR-TO			
X-A	-87.10	0.0	0.01 (1)	7.81	R-F	-228.10	0.05 (1)
A-B	0.14	-127.4	-127.4	0.09 (1)	10.00	N-K	-266.10
B-C	0.14	-127.4	-127.4	0.09 (1)	10.00	O-J	-247.10
C-D	0.14	-127.4	-127.4	0.09 (1)	10.00	P-I	-259.10
D-E	0.14	-127.4	-127.4	0.09 (1)	10.00	Q-G	-245.10
E-F	0.14	-127.4	-127.4	0.07 (1)	10.00	W-B	-269.10
F-G	0.14	-127.4	-127.4	0.07 (1)	10.00	V-C	-247.10
G-H	0.14	-127.4	-127.4	0.08 (1)	10.00	U-D	-259.10
H-I	0.14	-127.4	-127.4	0.08 (1)	10.00	S-E	-245.10
I-J	0.14	-127.4	-127.4	0.08 (1)	10.00	A-W	-8.10
J-K	0.14	-127.4	-127.4	0.09 (1)	10.00	N-L	-8.10
K-L	0.14	-127.4	-127.4	0.09 (1)	10.00		
M-L	-87.10	0.0	0.01 (1)	7.81			
X-W	0.10	-28.0	-28.0	0.04 (3)	10.00		
W-V	-4.10	-28.0	-28.0	0.04 (3)	10.00		
V-U	-4.10	-28.0	-28.0	0.04 (3)	10.00		
U-T	-4.10	-28.0	-28.0	0.04 (3)	10.00		
T-S	-4.10	-28.0	-28.0	0.04 (3)	10.00		
S-R	-4.10	-28.0	-28.0	0.03 (3)	10.00		
R-Q	-4.10	-28.0	-28.0	0.03 (3)	10.00		
Q-P	-4.10	-28.0	-28.0	0.04 (3)	10.00		
P-O	-4.10	-28.0	-28.0	0.04 (3)	10.00		
O-N	-4.10	-28.0	-28.0	0.04 (3)	10.00		
N-M	0.10	-28.0	-28.0	0.04 (3)	10.00		

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011
 (55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD
 CSI: TC=0.09/1.00 (K-L:1), SC=0.04/1.00 (N-O:3), WB=0.08/1.00 (K-N:1), GSI=0.16/1.00 (K-L:1)
 DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10
 COMPANION LIVE LOAD FACTOR = 0.50
 FLAT ROOF FACTOR = 0.75
 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 622 2284 1858
 PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.
 JSI GRIP=0.81 (W) (INPUT = 0.80)
 JSI METAL=0.07 (B) (INPUT = 1.00)
 DWG NO. TAM 12408-18
 STRUCTURAL COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T5	1	2	TRUSS DESC.	
Version 8.200 8 Jan 6 2018 MITek Industries, Inc. Mon Mar 6 13:46:16 2018 Page 2 ID:heIPRDILdBOQ6uLLONxW8zxNg4-KgdUMfoxChXIOyTnTcQRvdlhbFn4YXYie6NjzdzG9					
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1553.5 lbs FACTORED DOWN AT 1-10-8, AND 1553.5 lbs FACTORED DOWN AT 17-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.					
 DWG NO. TAM 12093-8 STRUCTURAL COMPONENT ONLY					

Tenlarack Roof Truss, Burlington



HANGERS NOTES
1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3961.6 lbs FACTORED DOWN AT 1-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

CHORDS				WEBS				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (FLF)	MAX. FACTORED CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO			FR-TO			
J-A	-4239 / 0	0.0	0.0	0.21 (1)	6.19	A-I	0 / 8044	0.71 (1)
A-B	-4468 / 0	-127.4	-127.4	0.10 (1)	4.79	I-B	-193 / 13	0.01 (1)
B-C	-4128 / 0	-127.4	-127.4	0.21 (1)	4.82	G-D	-1416 / 0	0.11 (1)
D-E	-4128 / 0	-127.4	-127.4	0.14 (1)	4.90	G-E	0 / 2352	0.27 (1)
C-D	-1724 / 0	-127.4	-127.4	0.10 (1)	6.26	H-C	-622 / 0	0.06 (1)
F-E	-1590 / 0	0.0	0.0	0.08 (1)	7.81	B-H	-389 / 0	0.07 (1)
						H-D	0 / 2563	0.30 (1)
J-I	0 / 0	-28.0	-28.0	0.12 (1)	10.00			
I-H	0 / 1466	-28.0	-28.0	0.52 (1)	10.00			
H-G	0 / 1724	-28.0	-28.0	0.19 (1)	10.00			
G-F	0 / 0	-28.0	-28.0	0.06 (8)	10.00			

A circular professional seal for a Licensed Professional Engineer in the Province of Ontario. The seal features the text "LICENSED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. In the center, the name "S. KATSOULAKOS" is printed, with the handwritten license number "3518" above it. A diagonal line is drawn across the center of the seal.

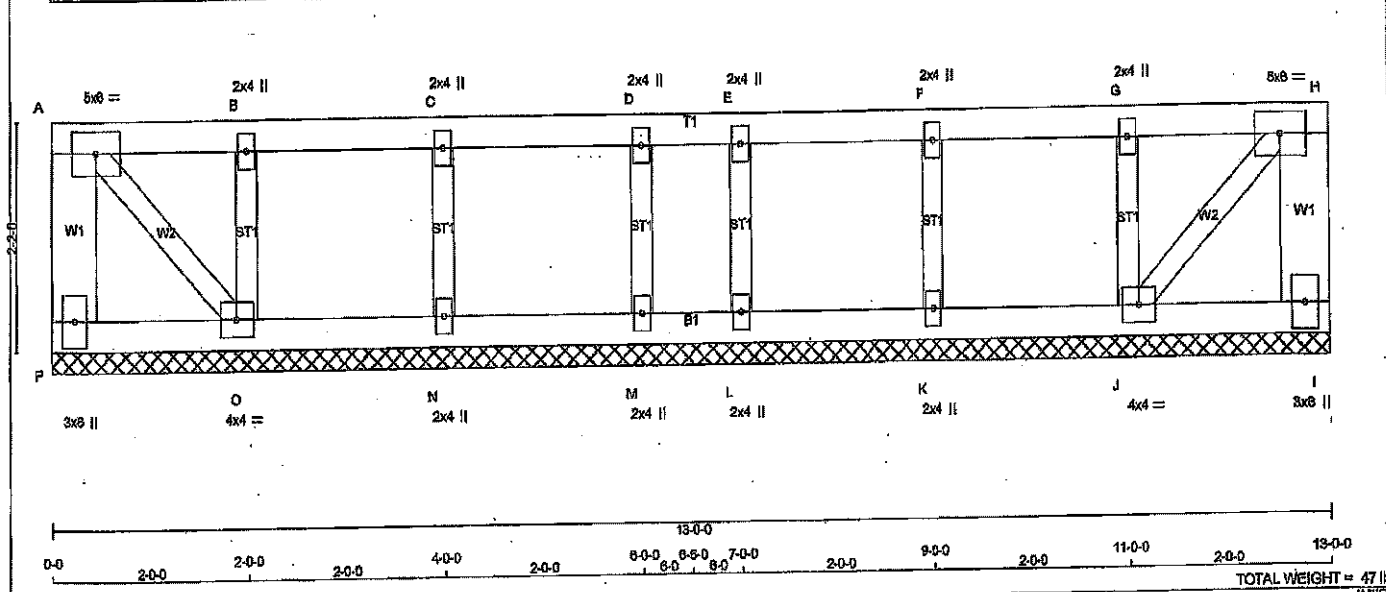
JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.65 (I) (INPUT = 1.00)

DWG NO. TAM 12094-B
STRUCTURAL
COMPONENT ONLY

JOB NAME 292707	TRUSS NAME G7	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamrack Roof Truss, Burlington

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ID:hGIPRDLdBOQ8u3LLONxW6zNq4-R3NzWHmRLS1HwK90EnLvn8HCeYCowlyn08AA2zd2GF



TOTAL WEIGHT = 47 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
P - A 2x6 DRY No.2
A - H 2x4 DRY No.2
I - H 2x6 DRY No.2
P - I 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMW4	MT20	5.0	6.0	
B, C, D, E, F, G				
B TMW4	MT20	2.0	4.0	
H TMW4	MT20	5.0	6.0	
I BMV1+p	MT20	3.0	6.0	
J BMWV1-i	MT20	4.0	4.0	
K, L, M, N				
K BMW1+w	MT20	2.0	4.0	
O BMWV1-i	MT20	4.0	4.0	
P BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO				FR-TO		
P-A	-98/0	0.0	0.0 (1)	J-G	-284/0	0.05 (1)
A-B	0/6	-127.4	-127.4 (1)	K-F	-258/0	0.05 (1)
B-C	0/6	-127.4	-127.4 (1)	L-E	-184/0	0.04 (1)
C-D	0/8	-127.4	-127.4 (1)	O-B	-284/0	0.05 (1)
D-E	0/6	-127.4	-127.4 (1)	N-C	-258/0	0.05 (1)
E-F	0/6	-127.4	-127.4 (1)	M-D	-184/0	0.04 (1)
F-G	0/8	-127.4	-127.4 (1)	A-O	-9/0	0.00 (1)
G-H	0/8	-127.4	-127.4 (1)	J-H	-9/0	0.00 (1)
H-I	-98/0	0.0	0.0 (1)			
P-Q	0/0	-28.0	-28.0 (3)			
Q-N	-9/0	-28.0	-28.0 (3)			
N-M	-9/0	-28.0	-28.0 (3)			
M-L	-9/0	-28.0	-28.0 (3)			
L-K	-9/0	-28.0	-28.0 (3)			
K-J	-9/0	-28.0	-28.0 (3)			
J-I	0/0	-28.0	-28.0 (3)			

DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF
SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.08/1.00 (A-B:1), BC=0.04/1.00 (N-O:3), WB=0.09/1.00 (B-O:1), SS=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

**COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75**

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

**JSI GRIP= 0.23 (J) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 1.00)**



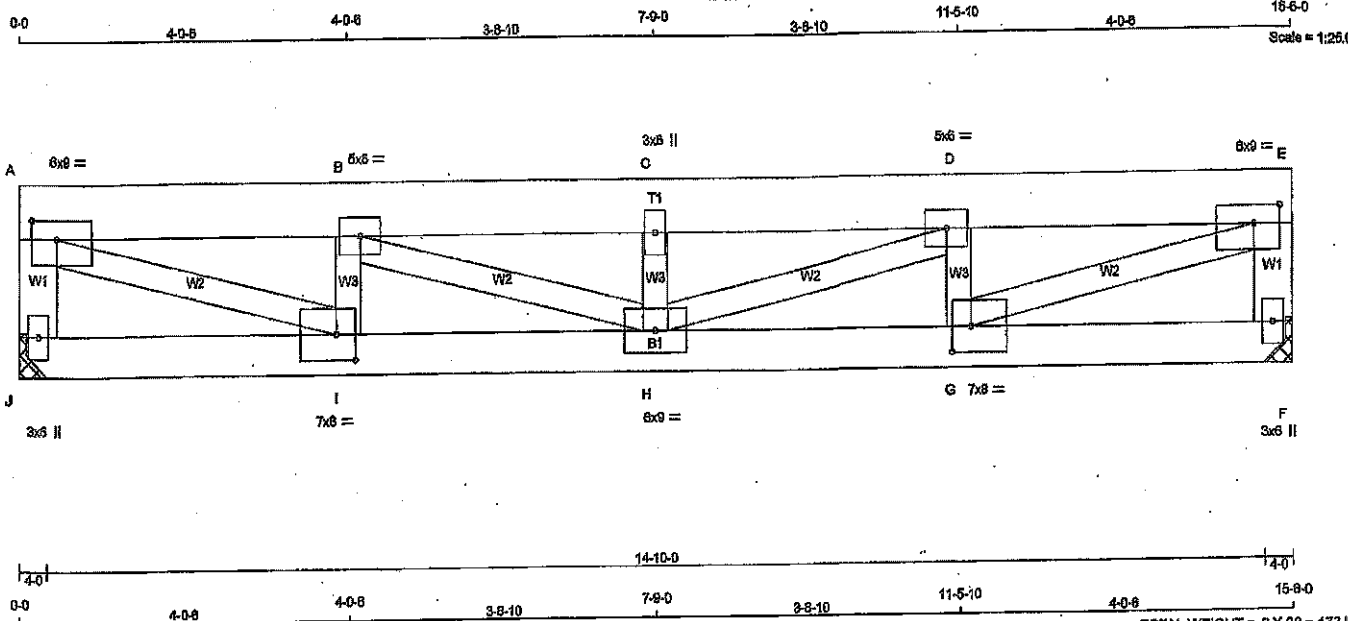
**DRWG NO. TAM 12409-18
STRUCTURAL
COMPONENT ONLY**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T8	1	2	TRUSS DESC.	

Remarck Roof Truss, Burlington

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ID:h9PRDILdBOG8u3LLONxW5zxNq4-o0B6a7q297a062_1KxgSrr3uf48HzdhwlxrfZdzQA



TOTAL WEIGHT = 2 X 88 = 177 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
J - A	2x6 DRY	No.2	SPF		
A - E	2x6 DRY	No.2	SPF		
E - F	2x6 DRY	No.2	SPF		
F - F	2x6 DRY	No.2	SPF		
ALL WEBS	2x4 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
J - A	2	12
E - F	2	12
A - E	2	0
J - F	2	12
WEBS: (0.122"x3") SPIRAL NAILS	2x4	1

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	6.0	8.0	2.50	3.75
B	TMVW-1	MT20	5.0	6.0		
C	TMVW-1	MT20	3.0	6.0		
D	TMVW-1	MT20	5.0	6.0		
E	TMVW-1	MT20	6.0	8.0	2.50	3.75
F	BMV1+p	MT20	3.0	6.0		
G	BMVW-1	MT20	7.0	8.0	3.50	2.75
H	BMVW-1	MT20	6.0	8.0		
I	BMVW-1	MT20	7.0	8.0	3.50	2.75
J	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
J	3990	0	3990	0
F	3990	0	3990	0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	3207	1539 / 0	539 / 0	0 / 0	0 / 0	1130 / 0	0 / 0
F	3207	1539 / 0	539 / 0	0 / 0	0 / 0	1130 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LOAD (LC)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LOAD (LC)
FR-TO					FR-TO		
J-A	-3698 / 0	0.0	0.0	0.19 (1)	G-E	0 / 7729	0.91 (1)
A-B	-7162 / 0	-488.8	-488.8	0.40 (1)	A-I	0 / 7729	0.91 (1)
B-C	-9279 / 0	-488.8	-488.8	0.42 (1)	G-D	-2878 / 0	0.22 (1)
C-D	-9279 / 0	-488.8	-488.8	0.42 (1)	I-B	-2878 / 0	0.22 (1)
D-E	-7162 / 0	-488.8	-488.8	0.40 (1)	H-D	0 / 2312	0.27 (1)
E-F	-3698 / 0	0.0	0.0	0.19 (1)	B-H	0 / 2312	0.27 (1)
J-I	0 / 0	-28.0	-28.0	0.13 (1)	H-C	-1730 / 0	0.13 (1)
I-H	0 / 77162	-28.0	-28.0	0.76 (1)			
H-G	0 / 77162	-28.0	-28.0	0.76 (1)			
G-F	0 / 0	-28.0	-28.0	0.13 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH	LL = 30.0	PSF
DL = 15.0	PSF	
BOT CH	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 62.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.09/12

GIRDER TYPE: CSI Girders

START DISTANCE = 0-0

END DISTANCE = 11-8-0

END DISTANCE = 15-8-0

END WALL WIDTH = 11-3-0

APPLIED TO FRONT SIDE OF TOP CHORD.

- ADDTL. LOADS BASED ON 55 % OF GSL

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCO 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCO 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.52")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.10")
ALLOWABLE DEFL. (TL) = 1/360 (0.52")
CALCULATED VERT. DEFL. (TL) = 1/938 (0.20")

CSI: TC=0.42/1.00 (B-C), BC=0.78/1.00 (H-I),
WB=0.81/1.00 (A-I), SS=0.39/1.00 (A-S)

DOL LUMBER=1.00 NAIL=1.00 L.S. BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1887	822 2284 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.91 (I) (INPUT = 1.00)

DRWG NO. TAM 10295-18
STRUCTURAL
COMPONENT ONLY



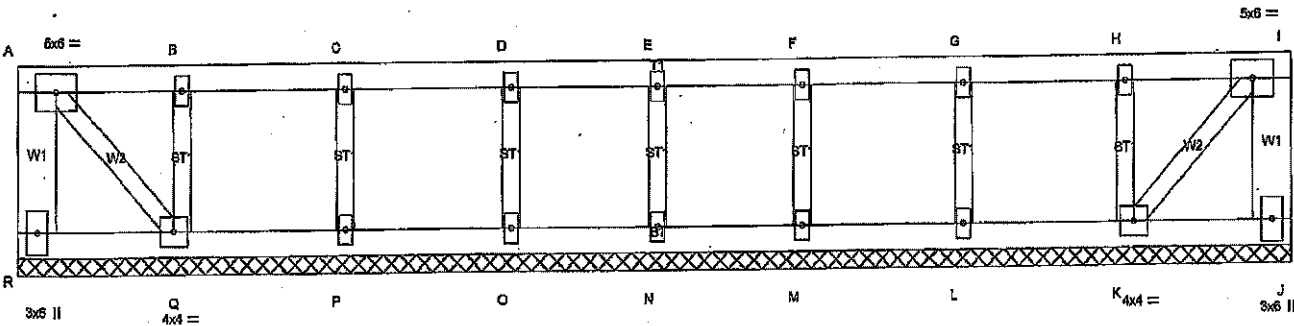
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:h6IPRdldB0Q8u3LLONxW5zNq4-R3NzWHmRLS1HwK90EnLVIn8HBeYBcwkm08AA2dzGF

0-0 2-0-0 2-0-0 2-0-0 4-0-0 2-0-0 6-0-0 1-9-0 7-9-0 1-9-0 9-8-0 2-0-0 11-6-0 2-0-0 13-6-0 2-0-0 15-6-0
Scale = 1:20.0



0-0 2-0-0 2-0-0 2-0-0 4-0-0 2-0-0 6-0-0 1-9-0 7-9-0 1-9-0 9-8-0 2-0-0 11-6-0 2-0-0 13-6-0 2-0-0 15-6-0
TOTAL WEIGHT = 54 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	R - A	2x6	DRY No.2	SPF
	A - I	2x4	DRY No.2	SPF
	J - I	2x6	DRY No.2	SPF
	R - J	2x4	DRY No.2	SPF
	ALL WEBS	2x3	DRY No.2	SPF
	ALL GABLE WEBS	2x3	DRY No.2	SPF
	DRY: SEASONED LUMBER.			
	GABLE STUDS SPACED AT	2-0-0 OC.		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally Restrained.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)
FR-TO							
R-A	-97.0	0.0	0.0	0.01 (1)	7.81	N-E	-213.0
A-B	0.0	-127.4	-127.4	0.09 (1)	10.00	K-H	-285.0
B-C	0.0	-127.4	-127.4	0.09 (1)	10.00	L-G	-251.0
C-D	0.0	-127.4	-127.4	0.08 (1)	10.00	M-F	-242.0
D-E	0.0	-127.4	-127.4	0.07 (1)	10.00	Q-B	-288.0
E-F	0.0	-127.4	-127.4	0.07 (1)	10.00	P-C	-251.0
F-G	0.0	-127.4	-127.4	0.08 (1)	10.00	O-D	-242.0
G-H	0.0	-127.4	-127.4	0.09 (1)	10.00	A-Q	-7.0
H-I	0.0	-127.4	-127.4	0.09 (1)	10.00	K-I	-7.0
J-I	-97.0	0.0	0.0	0.01 (1)	7.81		
R-Q	0.0	-28.0	-28.0	0.04 (3)	10.00		
Q-P	-5.0	-28.0	-28.0	0.04 (3)	10.00		
P-O	-5.0	-28.0	-28.0	0.03 (3)	10.00		
O-N	-5.0	-28.0	-28.0	0.03 (3)	10.00		
N-M	-5.0	-28.0	-28.0	0.03 (3)	10.00		
M-L	-5.0	-28.0	-28.0	0.03 (3)	10.00		
L-K	-5.0	-28.0	-28.0	0.04 (3)	10.00		
K-J	0.0	-28.0	-28.0	0.04 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 089-08
- TPC 2011

(65% OF 30.0 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.08/1.00 (H-I); EC=0.04/1.00 (K-L-3); WB=0.06/1.00 (H-K-1); SS=0.16/1.00 (H-I)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

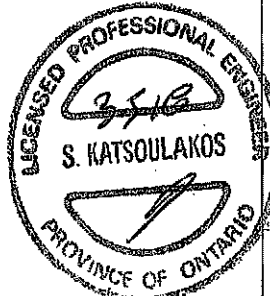
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 622 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 8.0 Deg.

SI GRIP= 0.23 (K) (INPUT = 0.90)
SI METAL= 0.07 (H) (INPUT = 1.00)



DWG NO. TAM 1410-8
STRUCTURAL
COMPONENT ONLY

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ID:heIPRdLdBOQ6u3LLONxW5zxNq4-o0Bsa7qZ97fa062_1KxySrr8qf4wH2DhwlsxFzdZGA



DWG NO. TAM 12096
STRUCTURAL
COMPONENT ONE

HANGERS NOTES

1. CONTINUED ON PAGE 2

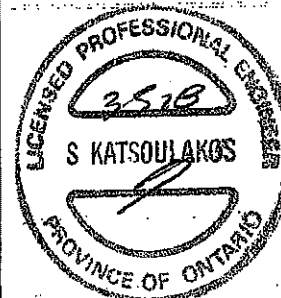
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T10	1	3	TRUSS DESC.	

Tamlaeck Roof Truss, Burlington

Version 6.200 S Jan 6 2018 MITek Industries, Inc. Mon Mar 5 13:48:19 2018 Page 2
ID:h0IPRDILd80Q6u3LLONxW5zyNg4-o0Bsa?oZ97fa062_1Kq9m8qf4wH2DhwlsxfFzd2GA

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4978.0 lbs FACTORED DOWN AT 1-8-8, AND 4978.0 lbs FACTORED DOWN AT 10-11-8, AND 1037.2 lbs FACTORED DOWN AT 12-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM12096-18
STRUCTURAL
COMPONENT ONLY



DEBCE
SPF
SPF
SPF
SPF

SPF

SPF
SPF

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

LOAD(PLF)
TOP
TOP
SIDE(148.
SIDE(58.0

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

25	5.25
50	2.50
50	2.50
25	5.25
25	5.25
25	5.25

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1402.5 lbs FACTORED DOWN AT 5-4-0, AND 1402.5 lbs FACTORED DOWN AT 9-7-0 ON TOP CHORD, AND 571.3 lbs FACTORED DOWN AT 1-1-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

BUILDING DESIGNER: PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQ'D
	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
JT							
J	5008	0	5008	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	H-5X MIN. SEAT SIZE: 4-0
F	5038	0	6038	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

AD	SOIL
0	0/0
0	0/0

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.78 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

TOTAL LOAD CASES: (4)

STORY	MAX CSI (LC)
3	0.54 (1)
2	0.63 (1)
	0.28 (1)
	0.25 (1)
8	0.31 (1)
1	0.34 (1)
	0.16 (1)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	5-4-0	-413	-462	-	FRONT	VERT	DEAD
L	5-4-0	-480	-980	-	FRONT	VERT	SINGING
M	9-7-0	-413	-482	-	FRONT	VERT	DEAD
M	9-7-0	-980	-980	-	FRONT	VERT	SINGING
N	1-1-0	-871	-571	-	BACK	VERT	TOTAL

FRONT	VERT	SNOW
FRONT	VERT	SNOW
FRONT	VERT	SNOW
BACK	VERT	TOTAL

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.6	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=		62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- GSA 088-09
- TPIG 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/380$ (0.46")
CALCULATED VERT. DEFL(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL)= $L/380$ (0.46")
CALCULATED VERT. DEFL(TL) = $L/948$ (0.17")

CSI: TC=0.73/1.00 (B-C:1), BC=0.44/1.00 (G-H:1),
WB=0.54/1.00 (E-G:1), SS=0.72/1.00 (C-D:1)

DOE LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY)		SHEAR		SECTION
	{PSI}		{PLI}		{PLI}
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1667	822	2284 1651

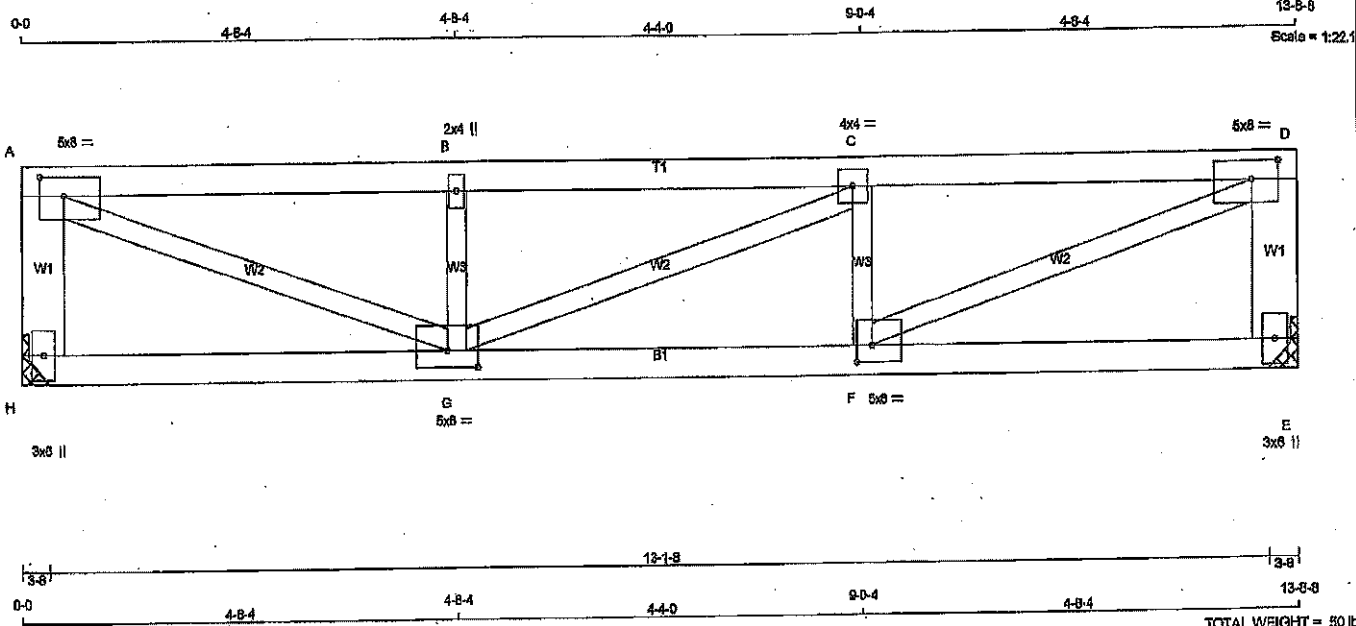
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (1) (INPUT = 0.90)
JSI METAL = 0.89 (3) (INPUT = 1.00)
DWG NO. TAM 12097-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T12	1	1	TRUSS DESC:	

Tantrac Roof Truss, Burlington

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ID:h6IPRDldB0Q6u3LLONxW5zXNq4-GDKEnLrBwlnReFcAb1Sv720BJ3Uo0Var9ybUNhxdzG6



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	H - A	2x6	DRY	No.2	SPF
	A - D	2x4	DRY	No.2	SPF
	E - D	2x6	DRY	No.2	SPF
	H - E	2x4	DRY	No.2	SPF
	ALL WEBS	2x3	DRY	No.2	SPF
	DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMMVW-1	MT20	5.0	8.0	2.25	3.25
B	TMMVW-1	MT20	2.0	4.0		
C	TMMVW-1	MT20	4.0	4.0		
D	TMMVW-1	MT20	5.0	8.0	2.25	3.25
E	BMV1+p	MT20	3.0	6.0		
F	BMVW-1	MT20	5.0	6.0	2.00	2.00
G	BMVWVW-1	MT20	5.0	8.0	2.00	4.00
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	1085	0	0	0
E	1085	0	0	0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	888	411/0	144/0	0/0	0/0	302/0	0/0
E	888	411/0	144/0	0/0	0/0	302/0	0/0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)
FR-TO									
H-A	-1004/0		0.0	0.0	0.10 (1)	7.81	F-D	0/2034	0.81 (1)
A-B	-1885/0		-127.4	-127.4	0.67 (1)	3.71	A-G	0/2031	0.81 (1)
B-C	-1885/0		-127.4	-127.4	0.63 (1)	3.76	F-C	-623/0	0.12 (1)
C-D	-1889/0		-127.4	-127.4	0.56 (1)	3.71	G-B	-623/0	0.12 (1)
E-D	-1005/0		0.0	0.0	0.10 (1)	7.81	G-C	-3/0	0.00 (1)
H-G	0/0		-28.0	-28.0	0.21 (3)	10.00			
G-F	0/1889		-28.0	-28.0	0.48 (1)	10.00			
F-E	0/0		-28.0	-28.0	0.21 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH LL = 30.0 PSF
DL = 15.0 PSF
BOT CH LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 068-09
- TPC 2011

(55% OF 30.0 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.57/1.00 (A-B:1), BC=0.48/1.00 (F-G:1), WB=0.51/1.00 (D-F:1), SS=0.35/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 622 2284 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

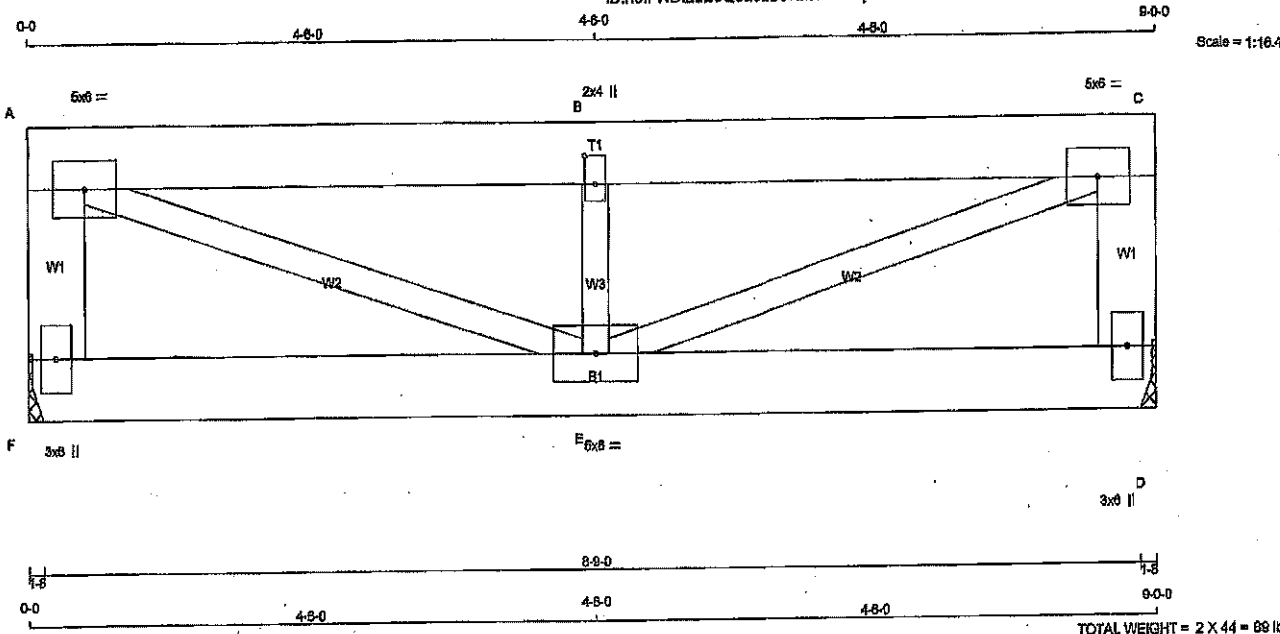
JSI GRIP= 0.89 (D) (INPUT = 0.80)
JSI METAL= 0.63 (F) (INPUT = 1.00)



DRWG NO. TAM 12098-58
STRUCTURAL
COMPONENT ONLY

JOB NAME 292707	TRUSS NAME T13	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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ID:h8lPRDlLd80Q6u3LLONxW5zxNq4-GDkEnLr8wlnReFeAb1Sv72OIW3bU0car3ybUNhZdzG9



LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x8	DRY	No.2	SPF
A - C	2x8	DRY	No.2	SPF
D - C	2x8	DRY	No.2	SPF
F - D	2x8	DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF				
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F-A 2 12		TOP
A-C 2 12		TOP
C-D 2 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F-D 2 12		TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMW-4	MT20	5.0	8.0
B	TMW-4	MT20	2.0	4.0 2.50 1.00
C	TMW-4	MT20	5.0	8.0
D	BMV1+4	MT20	3.0	6.0
E	BMW-4	MT20	5.0	8.0
F	BMV1+4	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING									
BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	IN-SX	IN-SX	UPLIFT	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8
F	899	0	699	0	0	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8
D	899	0	699	0	0	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	662	270 / 0	95 / 0	0 / 0	0 / 0	198 / 0	0 / 0
D	562	270 / 0	95 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				
F-A	-828 / 0	0.0	0.0	0.03 (1)
A-B	-1012 / 0	-127.4	-127.4	0.10 (1)
B-C	-1012 / 0	-127.4	-127.4	0.10 (1)
D-C	-828 / 0	0.0	0.0	0.03 (1)
F-E	0 / 0	-28.0	-28.0	0.05 (3)
E-D	0 / 0	-28.0	-28.0	0.05 (3)

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH.	LL = 30.0 PSF
DL	15.0 PSF
BOT CH.	LL = 10.5 PSF
DL	7.0 PSF
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010.

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.30")
CALCULATED VERT. DEFL. (LL) = L/599 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.30")
CALCULATED VERT. DEFL. (TL) = L/599 (0.02")

CSE TO=0.10/1.00 (A-B:1), BO=0.05/1.00 (E-F:3),
WB=0.16/1.00 (A-E:1), SS=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

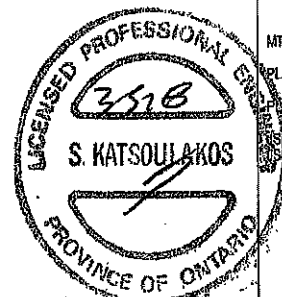
COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

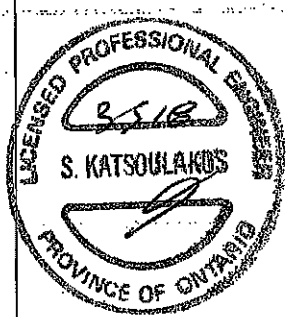
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PS) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 516 354 1667 822 2284 1858

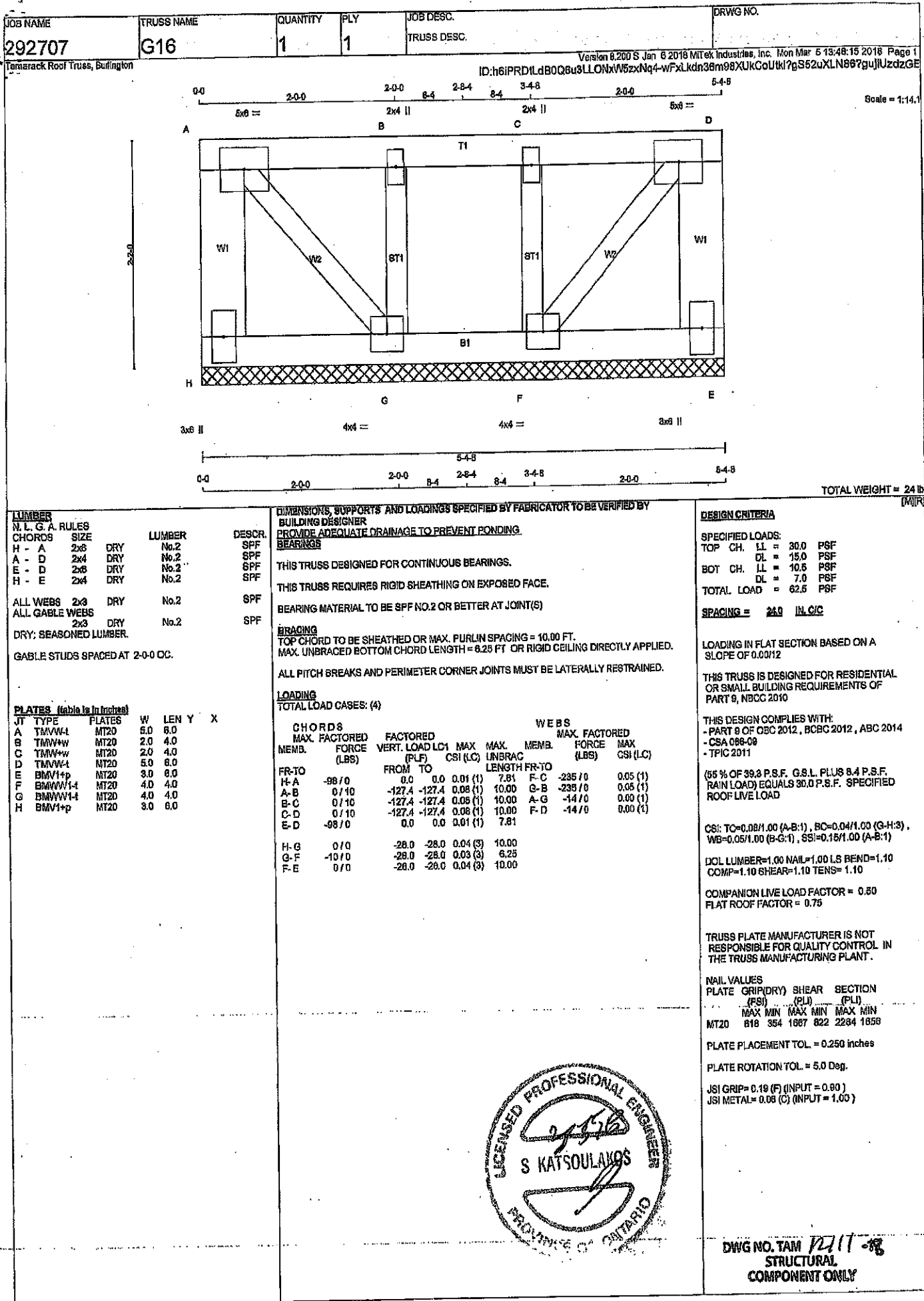
PLATE PLACEMENT TOL. = 0.250 inches
PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.52 (E) (INPUT = 0.80)
METAL = 0.17 (C) (INPUT = 1.00)



DWG NO. TAM 1099-88
STRUCTURAL
COMPONENT ONLY

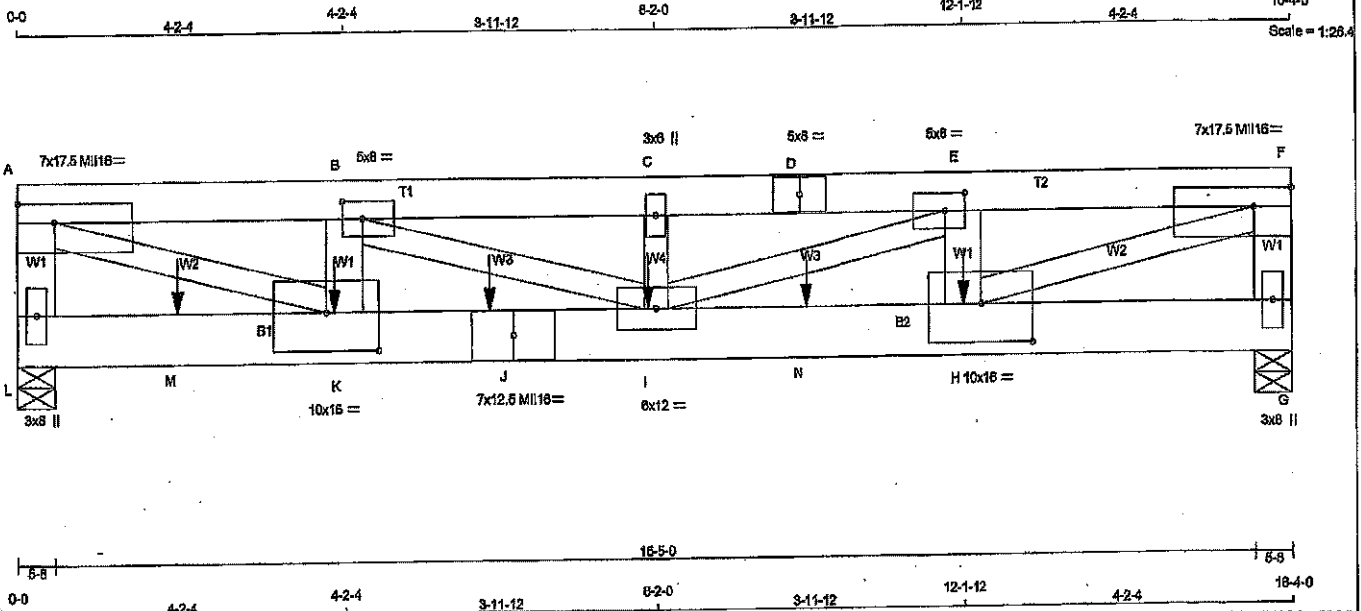
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T15	1	3	TRUSS DESC.	
Version 8.200 S Jan 6 2016 Mittek Industries, Inc. Mon Mar 6 13:48:21 2018 Page 2 ID:h0IFRDldB0Q0u3LLONxW5zxNq4-kPic_gsqhcnwIFPBM8l 8XGwRITosizm_QcL1w8zdzQ8					
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1037.2 lbs FACTORED DOWN AT 1-D-4, AND 6010.5 lbs FACTORED DOWN AT 2-11-8, AND 6010.5 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.					
 DWG NO. TAM/2101-18 STRUCTURAL COMPONENT ONLY					



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T17	1	3	TRUSS DESC.	

Ramarack Roof Truss, Burlington

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ID:h6IPRDLD80Q3u3LLONxV6zxNq4-kPic_gsqhowlFPBMB81_8XGwLoToNxD_Ocl1w8zdZGB8



LUMBER	SIZE	DRY	No.2	SPF
N. L. G. A. RULES				
CHORDS				
L - A	2x8	DRY	No.2	SPF
A - D	2x8	DRY	No.2	SPF
D - F	2x8	DRY	No.2	SPF
G - F	2x8	DRY	No.2	SPF
L - J	2x8	DRY	1850F 1.7E	SPF
J - G	2x8	DRY	1850F 1.7E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
A - K	2x4	DRY	2100F 1.8E	SPF
K - B	2x8	DRY	No.2	SPF
H - E	2x8	DRY	No.2	SPF
H - F	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
L-A 2	12	TOP
A-D 2	12	TOP
D-F 2	12	TOP
F-G 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-J 4	4	SIDE (1678.4)
J-G 4	4	SIDE (1678.4)
WEBS : (0.122"x3") SPIRAL NAILS		
2x4 1	6	
2x8 2	4	SIDE (131.9)
E-H 2	4	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

ORDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
A TMVW-4	MT16	7.0	17.5	2.75 Edge
B TMVW-4	MT20	5.0	8.0	2.50 3.00
C TMVW-4	MT20	3.0	6.0	
D TS-4	MT20	5.0	8.0	
E TMVW-4	MT20	5.0	8.0	2.50 3.00
F TMVW-4	MT16	7.0	17.5	2.75 Edge
G BMV1+p	MT20	3.0	6.0	
H BMVW-4	MT20	10.0	16.0	5.50 8.00
I BMVW-4	MT20	6.0	12.0	
J BS-4	MT16	7.0	12.5	
K BMVW-4	MT20	10.0	16.0	5.50 8.00
L BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD IN-SX
JT VERT	6128	6128	0	5-8
L 6128	0	0	0	5-8
G 7899	0	7899	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT COMBINED	SNOW	LIVE	PERM. LIVE		
L 4844	2433 / 0	715 / 0	0 / 0	1695 / 0	0 / 0
G 6284	3212 / 0	878 / 0	0 / 0	2184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.59 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
L-A	-5467 / 0	0.0	0.0 0.16 (1)	A-K	0 / 14211
A-B	-13237 / 0	-127.4	-127.4 0.28 (1)	K-B	-2984 / 0
B-C	-19079 / 0	-127.4	-127.4 0.53 (1)	B-I	0 / 8316
C-D	-19079 / 0	-127.4	-127.4 0.58 (1)	I-C	-481 / 0
D-E	-19079 / 0	-127.4	-127.4 0.58 (1)	E-I	0 / 888
E-F	-19167 / 0	-127.4	-127.4 0.51 (1)	H-E	-743 / 0
G-F	-7487 / 0	0.0	0.0 0.22 (1)	H-F	0 / 19503
L-M	0 / 0	-28.0	-28.0 0.13 (1)		
M-K	0 / 0	-28.0	-28.0 0.13 (1)		
K-J	0 / 13237	-28.0	-28.0 0.42 (1)		
J-I	0 / 13237	-28.0	-28.0 0.42 (1)		
I-N	0 / 18167	-28.0	-28.0 0.59 (1)		
N-H	0 / 18167	-28.0	-28.0 0.59 (1)		
H-G	0 / 0	-28.0	-28.0 0.19 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
H	12-1-12	-6434	-6434		FRONT	VERT	TOTAL
I	8-0-12	-1031	-1031		FRONT	VERT	TOTAL
J	8-0-12	-1031	-1031		FRONT	VERT	TOTAL
K	4-0-12	-1031	-1031		FRONT	VERT	TOTAL
M	2-0-12	-1031	-1031		FRONT	VERT	TOTAL
N	10-0-12	-1031	-1031		FRONT	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0 PSF
	DL = 15.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(65 % OF 36.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.54")
CALCULATED VERT. DEFL. (LL) = $L/588$ (0.16")
ALLOWABLE DEFL. (TL) = $L/360$ (0.54")
CALCULATED VERT. DEFL. (TL) = $L/651$ (0.30")

CSI: TO=0.59/1.00 (C-E-1), BC=0.59/1.00 (H-I-1), WB=0.65/1.00 (F-H-1), SSI=0.20/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1687 822	2284 1656
MT16	473 276	2341 1245	4454 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (S) (INPUT = 0.80)
JSI METAL = 0.88 (K) (INPUT = 1.00)

DWG NO. TAM 14102-08
STRUCTURAL
COMPONENT ONLY
CONTINUED ON PAGE 2

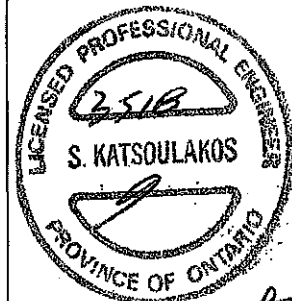
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T17	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1030.7 lbs FACTORED DOWN AT 2-0-12, 1030.7 lbs FACTORED DOWN AT 4-0-12, 1030.7 lbs FACTORED DOWN AT 6-0-12, 1030.7 lbs FACTORED DOWN AT 8-0-12, AND 1030.7 lbs FACTORED DOWN AT 10-0-12, AND 6433.7 lbs FACTORED DOWN AT 12-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



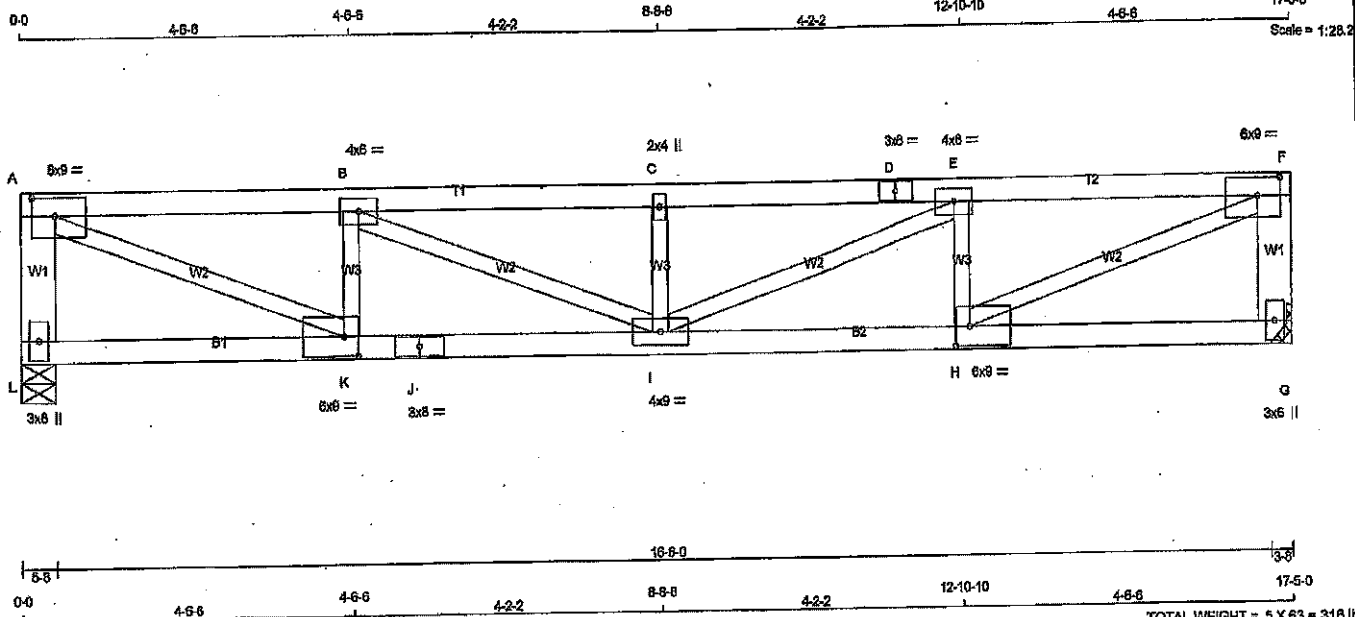
DWG NO. TAM 12102-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T20	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 5 X 63 = 315 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF	
A - D	2x6	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
G - F	2x6	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	
ALL WEBS	2x6	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TM/VW-1	MT20	6.0	9.0	2.75	3.75
B	TM/VW-1	MT20	4.0	6.0		
C	TM/VW-1	MT20	2.0	4.0		
D	TS-1	MT20	3.0	6.0		
E	TM/VW-1	MT20	4.0	6.0		
F	TM/VW-1	MT20	6.0	9.0	2.75	3.75
G	BM/V1-p	MT20	3.0	6.0		
H	BM/VW-1	MT20	6.0	9.0	3.00	2.25
I	BM/VW-1	MT20	4.0	6.0		
J	BS-1	MT20	3.0	6.0		
K	BM/VW-1	MT20	6.0	9.0	3.00	2.25
L	BM/V1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
L	1353	0	5-8	5-8
G	1353	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1088	522 / 0	183 / 0	0 / 0	0 / 0	383 / 0	0 / 0	0 / 0
G	1088	522 / 0	183 / 0	0 / 0	0 / 0	383 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. GSI (LC)	MAX. UNBRACED LENGTH (FR-TO)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. GSI (LC)
FR-TO						FR-TO			
L-A	-1294 / 0	0.0	0.0	0.13 (1)	7.65	H-F	0 / 2760	0.83 (1)	
A-B	-2550 / 0	-127.4	-127.4	0.81 (1)	3.20	A-K	0 / 2760	0.83 (1)	
B-C	-3202 / 0	-127.4	-127.4	0.88 (1)	2.74	H-E	-825 / 0	0.19 (1)	
C-D	-3202 / 0	-127.4	-127.4	0.88 (1)	2.74	K-B	-825 / 0	0.19 (1)	
D-E	-3202 / 0	-127.4	-127.4	0.88 (1)	2.74	I-E	0 / 715	0.21 (1)	
E-F	-2560 / 0	-127.4	-127.4	0.81 (1)	3.20	B-I	0 / 715	0.21 (1)	
G-F	-1294 / 0	0.0	0.0	0.13 (1)	7.65	I-C	-478 / 0	0.10 (1)	
L-K	0 / 0	-28.0	-28.0	0.19 (3)	10.00				
K-J	0 / 2550	-28.0	-28.0	0.84 (1)	10.00				
J-I	0 / 2550	-28.0	-28.0	0.84 (1)	10.00				
I-H	0 / 2550	-28.0	-28.0	0.84 (1)	10.00				
H-G	0 / 0	-28.0	-28.0	0.19 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
DL	=	15.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	82.5	PSF	

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 30.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.58")
CALCULATED VERT. DEFL.(LL) = L/899 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (0.58")
CALCULATED VERT. DEFL.(TL) = L/807 (0.26")

CSI: TC=0.69/1.00 (B-C-1), SC=0.64/1.00 (A-K-1),
WS=0.63/1.00 (A-K-1), SS=0.35/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1697 822 2284 1668

PLATE PLACEMENT TOL. = 0.250 inches

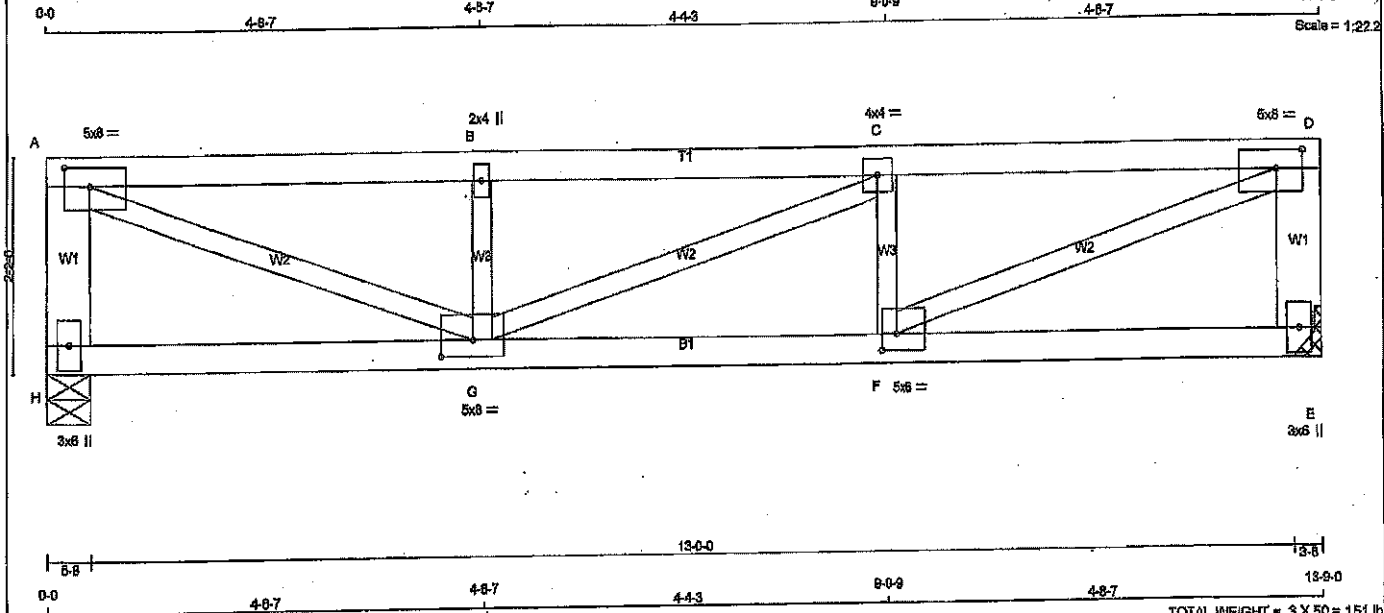
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.92 (J) (INPUT = 1.00)



DWG NO. TAM-12103-8
STRUCTURAL
COMPONENT ONLY

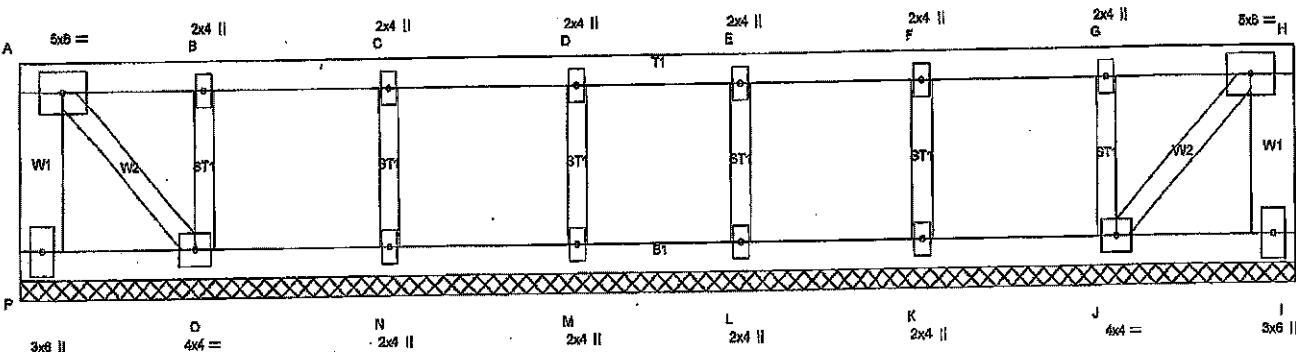
JOB NAME 292707	TRUSS NAME T21	QUANTITY 3	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Version 8.200 S Jan 6 2018 MITek Industries, Inc. Mon Mar 5 13:49:21 2018 Page 1 ID:h6IPROldB0Q6u3LLOHxW5zxNq4-kPic_gsqhcvlFPBM8l_8XGwM_Tq_ljn_OcL1w8dzG8					
Tamarack Roof Truss, Burlington					



LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. SPF H - A 2x6 DRY No.2 SPF A - D 2x4 DRY No.2 SPF E - D 2x6 DRY No.2 SPF H - E 2x4 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF DRY, SEASONED LUMBER.				PLATES (table in inches) JT TYPE PLATES W LEN Y X A TMWV4 MT20 5.0 8.0 2.25 3.25 B TMWV4 MT20 2.0 4.0 C TMWV4 MT20 4.0 4.0 D TMWV4 MT20 5.0 8.0 2.25 3.25 E BMV1+p MT20 3.0 8.0 F BMWV4 MT20 5.0 6.0 2.00 2.00 G BMWV4 MT20 5.0 8.0 2.00 4.00 H BMV1+p MT20 3.0 8.0				UNFACTORED REACTIONS 1ST CASE MAX./MIN. COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL H 889 412/0 144/0 0/0 0/0 308/0 0/0 E 889 412/0 144/0 0/0 0/0 308/0 0/0				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 30.0 PSF DL = 15.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 62.5 PSF SPACING = 24.0 IN./C LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.0012 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010 THIS DESIGN COMPLIES WITH: - PART 9 OF OSC 2012, BCBC 2012, ABC 2014 - CSA 088-09 - TRC 2011 (55% OF 20.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.46") CALCULATED VERT. DEFL.(LL) = L/999 (0.07") ALLOWABLE DEFL.(TL) = L/360 (0.46") CALCULATED VERT. DEFL.(TL) = L/999 (0.13") CSI: TC=0.57/1.00 (A-B:1), BC=0.48/1.00 (F-G:1), WB=0.61/1.00 (D-F:1), SS=0.36/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 0.50 FLAT ROOF FACTOR = 0.75 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 822 2284 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.90 (D) (INPUT = 0.90) JSI METAL= 0.83 (F) (INPUT = 1.00)				LOADING TOTAL LOAD CASES: (4) CHORDS MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD (LBS) MAX. FACTORED VERT. LOAD (LBS) MAX. FACTORED VERT. LOAD (LBS) MAX. FACTORED VERT. LOAD (LBS) FR-TO H-A -1088/0 0.0 0.0 0.10 (1) 7.81 F-D 0/2043 0.81 (1) A-B -1888/0 -127.4 -127.4 0.57 (1) 3.69 A-G 0/2043 0.81 (1) B-C -1888/0 -127.4 -127.4 0.53 (1) 3.74 F-C -825/0 0.13 (1) C-D -1800/0 -127.4 -127.4 0.57 (1) 3.80 G-B -825/0 0.13 (1) E-D -1088/0 0.0 0.0 0.10 (1) 7.81 G-C -3/0 0.00 (1) H-G 0/0 -28.0 -28.0 0.21 (3) 10.00 G-F 0/1800 -28.0 -28.0 0.48 (1) 10.00 F-E 0/0 -28.0 -28.0 0.21 (3) 10.00				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H BRACING TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.								DRWG NO. TAW 1404-8 STRUCTURAL COMPONENT ONLY			
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JOB NAME 292707	TRUSS NAME G21	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.200 5 Jan 6 2018 Mitek Industries, Inc. Mon Mar 5 13:48:16 2018 Page 1 ID:h6IPRDLdBOQ6u3LLONxV6zxNg4-wFxLkdn36m96XUKCoUkl?gSx2uQLN_87gUjUzdZGE	

0-0 2-0-0 2-0-0 2-0-0 4-0-0 2-0-0 6-0-0 1-0-0 7-9-0 2-0-0 9-9-0 2-0-0 11-9-0 2-0-0 13-9-0
Scale = 1:22.2



0-0 2-0-0 2-0-0 2-0-0 4-0-0 2-0-0 6-0-0 1-0-0 7-9-0 2-0-0 9-9-0 2-0-0 11-9-0 2-0-0 13-9-0
TOTAL WEIGHT = 48 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
P - A	2x6 DRY	No.2	SPF
A - H	2x4 DRY	No.2	SPF
I - H	2x6 DRY	No.2	SPF
P - I	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
ALL GABLE WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
 PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.
 BEARINGS
 THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
 THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
 BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO		FROM TO		FR-TO		FROM TO	
P-A	-96/0	0.0 0.0 0.01 (1)	7.81	J-G	-285/0	0.06 (1)	0.06 (1)
A-B	0/6	-127.4 -127.4 0.08 (1)	10.00	K-F	-252/0	0.06 (1)	0.06 (1)
B-C	0/6	-127.4 -127.4 0.08 (1)	10.00	L-E	-237/0	0.06 (1)	0.06 (1)
C-D	0/6	-127.4 -127.4 0.08 (1)	10.00	O-B	-285/0	0.06 (1)	0.06 (1)
D-E	0/6	-127.4 -127.4 0.07 (1)	10.00	N-C	-252/0	0.06 (1)	0.06 (1)
E-F	0/6	-127.4 -127.4 0.06 (1)	10.00	M-D	-237/0	0.06 (1)	0.06 (1)
F-G	0/6	-127.4 -127.4 0.08 (1)	10.00	A-O	-8/0	0.03 (1)	0.03 (1)
G-H	0/6	-127.4 -127.4 0.08 (1)	10.00	J-H	-8/0	0.03 (1)	0.03 (1)
H-I	-96/0	0.0 0.0 0.01 (1)	7.81				
P-O	0/0	-28.0 -28.0 0.04 (3)	10.00				
O-N	-6/0	-28.0 -28.0 0.04 (3)	10.00				
N-M	-6/0	-28.0 -28.0 0.03 (3)	10.00				
M-L	-6/0	-28.0 -28.0 0.03 (3)	10.00				
L-K	-6/0	-28.0 -28.0 0.03 (3)	10.00				
K-J	-6/0	-28.0 -28.0 0.04 (3)	10.00				
J-I	0/0	-28.0 -28.0 0.04 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 30.0 PSF
 DL = 15.0 PSF
 BOT CH. LL = 10.6 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 38.2 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TC=0.09/1.00 (F-G-1), BC=0.04/1.00 (J-K-3), WB=0.08/1.00 (G-J-1), SS=0.16/1.00 (G-H-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60
 FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.260 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (J) (INPUT = 0.80)
 JSI METAL= 0.07 (G) (INPUT = 1.00)



DRWG NO. TAM/21/12-18
 STRUCTURAL
 COMPONENT ONLY

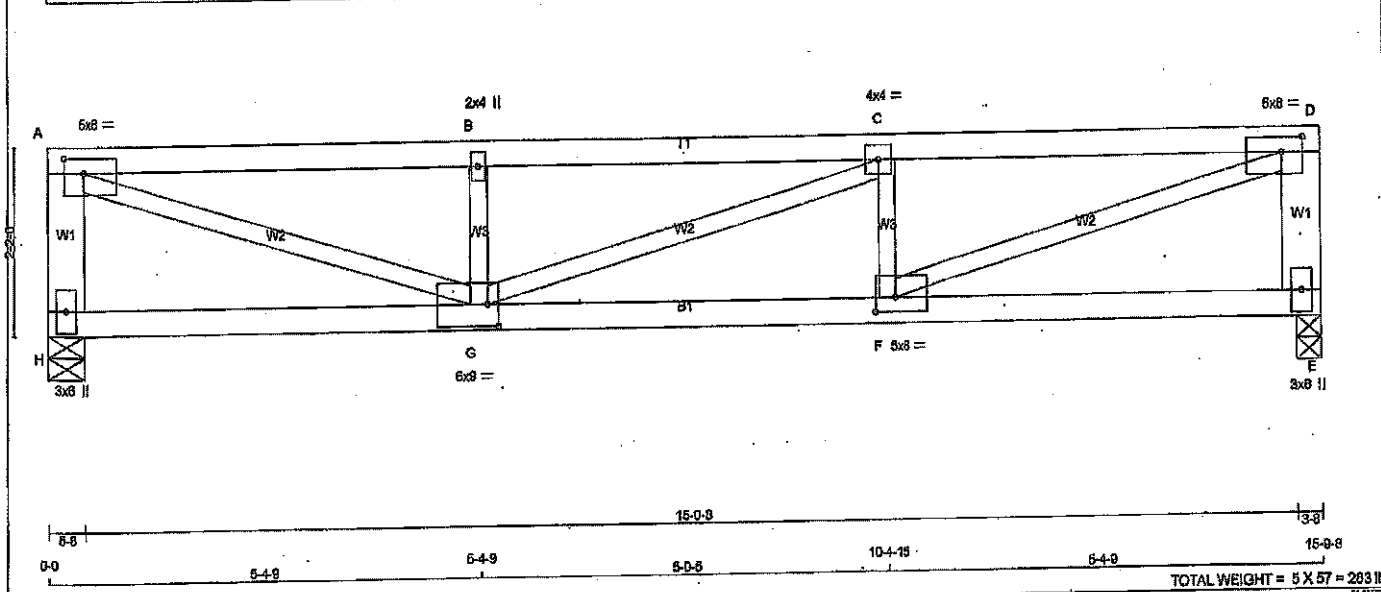


Table with 4 columns: LUMBER, CHORDS, SIZE, LUMBER, DESCR. It lists various lumber specifications and descriptions.

Table with 7 columns: PLATES (table in inches), JT, TYPE, PLATES, W, LEN, Y, X. It lists plate specifications for different joints.

Table with 8 columns: DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER. It lists various dimensions and loadings.

Table with 8 columns: UNFACTORED REACTIONS, 1ST CASE, MAX. MIN. COMPONENT REACTIONS. It lists unreacted reactions for the first case.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING TOTAL LOAD CASES: (4)

Table with 12 columns: CHORDS, MAX. FACTORED, FORCE, VERT. LOAD, LC1, MAX, MAX, UNBRAC, MEMB., FORCE, MAX, FACTORED. It lists various loading and force data for chords and members.

Table with 2 columns: DESIGN CRITERIA, SPECIFIED LOADS. It lists design criteria and specified loads.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH: - PART 9 OF OBC 2012, BCBC 2012, ABC 2014 - CSA 088-09 - TRC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53") CALCULATED VERT. DEFL.(LL) = 1/899 (0.12") ALLOWABLE DEFL.(TL)= L/360 (0.53") CALCULATED VERT. DEFL.(TL) = 1/835 (0.23")

CSI: TC=0.85/1.00 (A-B:1), BC=0.84/1.00 (F-G:1), WB=0.50/1.00 (D-F:1), SS=0.42/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50 FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

Table with 6 columns: NAIL VALUES, PLATE: GRIP(DRY) SHEAR SECTION. It lists nail values for different plate types.

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.80 (F) (INPUT = 0.80) JSI METAL = 0.85 (D) (INPUT = 1.00)

DRWG NO. TAM 12405-08 STRUCTURAL COMPONENT ONLY



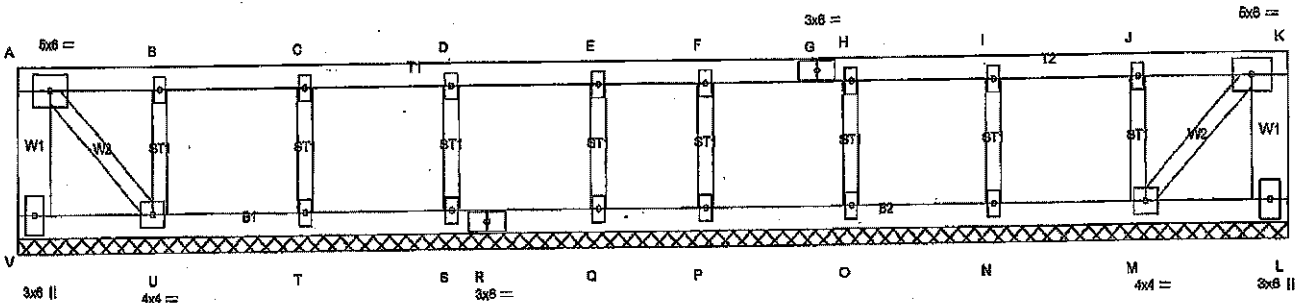
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G22X	1	1	TRUSS DESC.	

Jamareck Roof Truss, Burlington

Version 8.200 S Jan 6 2016 Mitek Industries, Inc. Mon Mar 5 13:48:15 2016 Page 1

ID:h8IPRDLdBOQ6u3LLONXW5zxNq4-wPxl.kdn36m98XUkCoUKi7gSx2uQLN_67guUjUdzdGE

0-0 2-0 2-0 2-0 4-0 2-0 6-0 2-0 8-0 1-5 9-5 2-0 11-5 2-0 13-5 2-0 15-5 2-0 17-5
Scale = 1:28.8



0-0 2-0 2-0 2-0 4-0 2-0 6-0 2-0 8-0 1-5 9-5 2-0 11-5 2-0 13-5 2-0 15-5 2-0 17-5
TOTAL WEIGHT = 69 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
V - A	2x6	DRY	No.2	SPF	
A - G	2x4	DRY	No.2	SPF	
G - K	2x4	DRY	No.2	SPF	
L - K	2x6	DRY	No.2	SPF	
V - R	2x4	DRY	No.2	SPF	
R - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					
GABLE STUDS SPACED AT	2-0-0	OC.			

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - TMVW-1	MT20	5.0	6.0			
B, C, D, E, F, H, I, J						
B - TMVW-1	MT20	2.0	4.0			
G - TS-1	MT20	3.0	6.0			
K - TMVW-1	MT20	5.0	6.0			
L - BMV1+p	MT20	3.0	6.0			
M - BMVW-1	MT20	4.0	4.0			
N, O, P, Q, S, T						
N - BMV1+w	MT20	2.0	4.0			
R - BS-1	MT20	3.0	6.0			
U - BMVW-1	MT20	4.0	4.0			
V - BMV1+p	MT20	3.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT FONDING.
BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L)	
FR-TO				FR-TO			
V-A	-97.10	0.0	0.01 (1)	M-J	-287.10	0.05 (1)	
A-B	0.15	-127.4	-127.4 0.09 (1)	N-I	-248.10	0.05 (1)	
B-C	0.15	-127.4	-127.4 0.09 (1)	O-H	-284.10	0.05 (1)	
C-D	0.15	-127.4	-127.4 0.09 (1)	P-F	-214.10	0.04 (1)	
D-E	0.15	-127.4	-127.4 0.09 (1)	U-B	-287.10	0.05 (1)	
E-F	0.15	-127.4	-127.4 0.09 (1)	T-C	-248.10	0.05 (1)	
F-G	0.15	-127.4	-127.4 0.09 (1)	S-D	-284.10	0.05 (1)	
G-H	0.15	-127.4	-127.4 0.09 (1)	Q-E	-214.10	0.04 (1)	
H-I	0.15	-127.4	-127.4 0.09 (1)	A-U	-7.10	0.00 (1)	
I-J	0.15	-127.4	-127.4 0.09 (1)	M-K	-7.10	0.00 (1)	
J-K	0.15	-127.4	-127.4 0.09 (1)				
L-K	-97.10	0.0	0.01 (1)				
V-U	0.10	-28.0	-28.0 0.04 (3)				
U-T	-5.10	-28.0	-28.0 0.04 (3)				
T-S	-5.10	-28.0	-28.0 0.04 (3)				
S-R	-5.10	-28.0	-28.0 0.04 (3)				
R-Q	-5.10	-28.0	-28.0 0.04 (3)				
Q-P	-5.10	-28.0	-28.0 0.04 (3)				
P-O	-5.10	-28.0	-28.0 0.04 (3)				
O-N	-5.10	-28.0	-28.0 0.04 (3)				
N-M	-5.10	-28.0	-28.0 0.04 (3)				
M-L	0.10	-28.0	-28.0 0.04 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 688-09
- TPC 2011

(55% OF 39.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSK TC=0.09/1.00 (J-K); BC=0.04/1.00 (M-N); WB=0.09/1.00 (J-M); SS=0.16/1.00 (J-K)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 364 1687 622 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

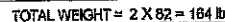
PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.81 (U) (INPUT = 0.80)
JSI METAL= 0.07 (J) (INPUT = 1.00)



DWG NO. TAW/2113-18
STRUCTURAL
COMPONENT ONLY

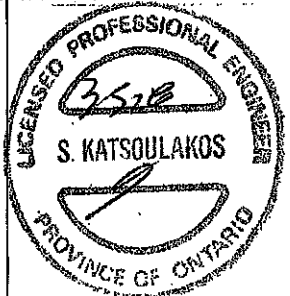
Version 8.200 S Jan 6 2016 Mitek Industries, Inc. Mon Mar 5 13:48:22 2016 Page 1
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HANGERS NOTES
51

A circular professional engineer seal for the Province of Ontario. The outer ring contains the text "REGISTERED PROFESSIONAL ENGINEER" at the top and "PROVINCE OF ONTARIO" at the bottom. The center of the seal features a large stylized "S" and "E" forming a circle. Inside this circle, the number "3518" is handwritten. Below the number, the word "TYPE" is printed, followed by "TOM SOULAKOS" in bold capital letters. Below the name, the word "TOTAL" is printed, followed by a large handwritten "S" or "5". At the bottom of the seal, the text "NO. 110-10-10" is visible.

JSI GRIP= 0.88 (I) (INPUT = 0.80)
JSI METAL= 0.94 (G) (INPUT = 1.00)

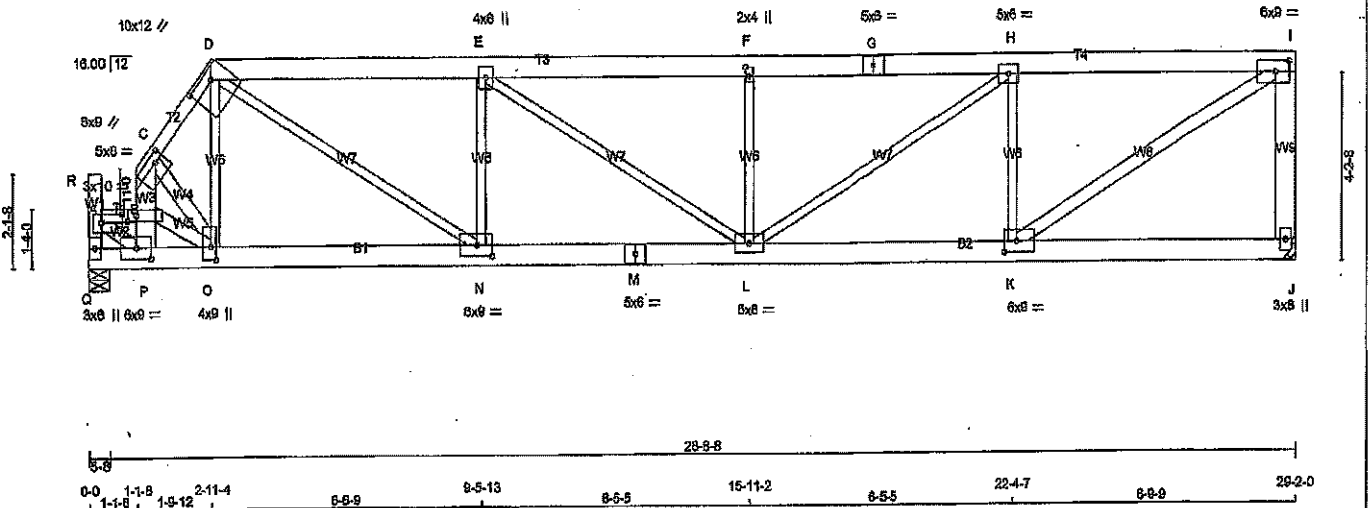
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T23	1	2	TRUSS DESC.	
Tamarack Roof Truss, Burlington					
Version 8.200 S Jan 6 2018 Mitek Industries, Inc. Mon Mar 6 13:46:22 2018 Page 2					
ID:h6PRDILdBOQ6u3LLONxW5zxNq4-Cbs_C0tSSw29tZmYISVN4TTastGrUMN8dG4bSazdzG7					
HANGERS NOTES 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1040.4 lbs FACTORED DOWN AT 2-0-12, 1040.4 lbs FACTORED DOWN AT 4-0-12, 1040.4 lbs FACTORED DOWN AT 6-8-12, 1325.3 lbs FACTORED DOWN AT 8-8-12, 1325.3 lbs FACTORED DOWN AT 10-6-12, 1325.3 lbs FACTORED DOWN AT 12-6-12, AND 1325.3 lbs FACTORED DOWN AT 14-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.					
 DWG NO. TAM 11406-8 STRUCTURAL COMPONENT ONLY					

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31	5	1	TRUSS DESC.	

Tamarrack Roof Truss, Burlington

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ID:g3jAbU0eQ_EaFnNpIqPihXzDFmC-rGG8oMRJLBGaTX8OMbydW0JSdR7y1EUdYxAXzC1BK

0-0 1-1-8 2-11-4 8-6-9 9-5-13 6-5-5 15-11-2 8-5-5 22-4-7 8-9-9 29-2-0
1-1-8 1-9-12
Scale = 1:50.0



LUMBER	N L G A RULES	CHORDS	SIZE	LUMBER	DESCR.
Q - R	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
G - I	2x6	DRY	No.2	SPF	
J - I	2x6	DRY	No.2	SPF	
P - C	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	

ALL WEBS EXCEPT	SIZE	DRY	No.2	SPF
A - P	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25 5.75
B	TVMW-I	MT20	3.0	10.0	1.50 2.50
C	TVMW-I	MT20	6.0	8.0	2.00 2.75
D	TTVMW-h	MT20	10.0	12.0	2.50 7.00
E	TVMW-I	MT20	4.0	6.0	
F	TVMW-w	MT20	2.0	4.0	2.50 1.00
G	TS-I	MT20	5.0	6.0	
H	TVMW-I	MT20	5.0	6.0	
I	TVMW-I	MT20	6.0	9.0	3.00 3.75
J	BMV-I+p	MT20	3.0	6.0	
K	BMWV-I	MT20	6.0	9.0	3.00 3.75
L	BMWV-I	MT20	5.0	8.0	
M	BS-I	MT20	5.0	6.0	
N	BMWV-I	MT20	6.0	9.0	3.00 4.25
O	BMWV-I	MT20	4.0	9.0	3.50 1.50
P	BMV-I	MT20	6.0	9.0	3.00 4.25
Q	BMV-I+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
Q	2268	0	0	5-8
J	2268	0	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1822	874 / 0	308 / 0	0 / 0	0 / 0	642 / 0	0 / 0
J	1822	874 / 0	308 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (UNBRAC)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO			FR-TO		
Q-A	-1896 / 0	0.0	0.0	0.28 (1)	5.40	C-D	0 / 248	0.09 (3)
A-R	0 / 0	0.0	0.0	0.00 (1)	10.00	A-P	0 / 3122	0.87 (1)
A-B	-2549 / 0	-127.4	-127.4	0.17 (1)	3.70	B-O	-1805 / 0	0.22 (1)
C-D	-2440 / 0	-127.4	-127.4	0.19 (1)	3.75	C-O	0 / 1480	0.32 (1)
D-E	-3575 / 0	-127.4	-127.4	0.84 (1)	3.38	K-I	0 / 3484	0.75 (1)
E-F	-3869 / 0	-127.4	-127.4	0.67 (1)	3.17	D-N	0 / 2515	0.75 (1)
F-G	-3869 / 0	-127.4	-127.4	0.72 (1)	3.08	K-H	-1823 / 0	0.60 (1)
G-H	-3869 / 0	-127.4	-127.4	0.72 (1)	3.09	N-E	-1191 / 0	0.44 (1)
H-I	-2982 / 0	-127.4	-127.4	0.84 (1)	3.65	L-H	0 / 1203	0.38 (1)
J-I	-2185 / 0	0.0	0.0	0.65 (1)	6.18	E-L	0 / 470	0.14 (1)
P-B	-2536 / 0	0.0	0.0	0.64 (1)	4.89	L-F	-749 / 0	0.26 (1)
B-C	-3250 / 0	0.0	0.0	0.67 (1)	4.10			
Q-P	0 / 0	-28.0	-28.0	0.43 (1)	10.00			
P-O	0 / 1861	-28.0	-28.0	0.72 (1)	10.00			
O-N	0 / 1459	-28.0	-28.0	0.35 (1)	10.00			
N-M	0 / 3575	-28.0	-28.0	0.67 (1)	10.00			
M-L	0 / 3575	-28.0	-28.0	0.67 (1)	10.00			
L-K	0 / 2582	-28.0	-28.0	0.65 (1)	10.00			
K-J	0 / 0	-28.0	-28.0	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0	PSF
DL	15.0	PSF
BOT CH.	LL = 10.5	PSF
DL	7.0	PSF
TOTAL LOAD	62.5	PSF

SPACING = 24.0 IN. G/G

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF NBC 2012, CBC 2012, ABC 2014
- CSA C88-09
- TPC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.72/1.00 (F-H-I), SC=0.72/1.00 (O-P-I),
WB=0.72/1.00 (O-N-I), SS=0.56/1.00 (B-P-I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2264 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.80)
JSI METAL= 0.84 (M) (INPUT = 1.00)



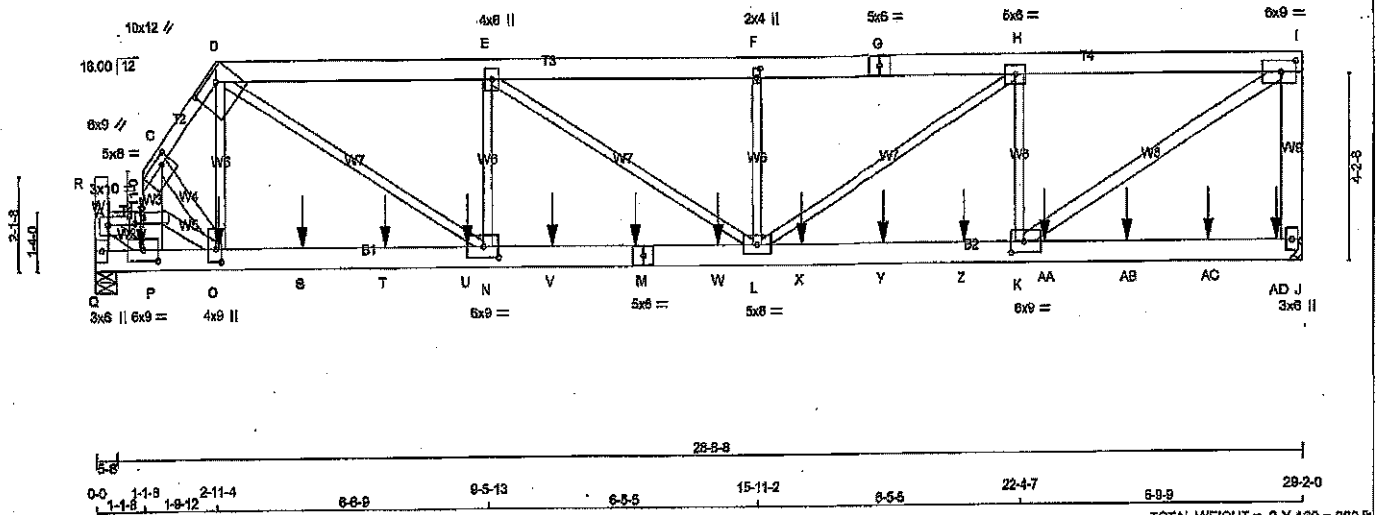
DRWG NO. TAM28035-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31Z	1	2	TRUSS DESC.	

Temarack Roof Truss, Burlington

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ID:g3jAbU0eO_EaFnNpIqPhXzDFmC-rGG8oMRJILBGaTXBOMbydWOL2dRr7z2EUdYyAXzC1BK

0-0 1-1-8 2-11-4 6-6-9 9-5-13 6-5-5 15-11-2 6-5-5 22-4-7 6-9-9 28-2-0
1-1-8 1-8-12 2-11-4 6-6-9 9-5-13 6-5-5 15-11-2 6-5-5 22-4-7 6-9-9 28-2-0
Scale = 1:50.0



LUMBER	SIZE	DESCR.
N. L. G. A. RULES		
CHORDS		
Q - R	2x4 DRY No.2	SPF
A - B	2x4 DRY No.2	SPF
C - D	2x4 DRY No.2	SPF
D - G	2x6 DRY No.2	SPF
G - I	2x6 DRY No.2	SPF
J - L	2x6 DRY No.2	SPF
P - C	2x6 DRY No.2	SPF
Q - M	2x6 DRY No.2	SPF
M - J	2x6 DRY No.2	SPF
ALL WEBS	2x3 DRY No.2	SPF
EXCEPT		
A - P	2x4 DRY No.2	SPF
B - O	2x4 DRY No.2	SPF
C - O	2x4 DRY No.2	SPF
K - I	2x4 DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
Q-R	12	TOP
A-B	12	TOP
C-D	12	TOP
D-G	12	TOP
G-I	12	TOP
J-L	12	TOP
P-C	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Q-M	12	SIDE (137.3)
N-J	12	SIDE (137.3)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	
2x4	8	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TMWVH	MT20	5.0	8.0	2.25 5.75
B TMWVH	MT20	3.0	10.0	1.50 2.50
C TMWVH	MT20	6.0	9.0	2.00 2.75
D TMWVH	MT20	10.0	12.0	2.50 7.00
E TMWVH	MT20	4.0	8.0	
F TMWVH	MT20	2.0	4.0	2.50 1.00
G TS-t	MT20	5.0	6.0	
H TMWVH	MT20	6.0	6.0	
I TMWVH	MT20	6.0	9.0	3.00 3.75
J BMV1+p	MT20	3.0	6.0	
K BMWVH	MT20	6.0	9.0	3.00 3.75
L BMWVH	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
Q	3778	0	0	5-8
J	3883	0	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	2942	1541/0	378/0	0/0	0/0	1023/0	0/0
J	3021	1684/0	387/0	0/0	0/0	1051/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.75 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (FT-LB)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED MOMENT (FT-LB)
FR-TO					FR-TO		
Q-A	-3120/0	0.0	0.0	0.24 (1)	Q-D	0/688	0.11 (1)
A-R	0/0	0.0	0.0	0.00 (1)	A-P	0/5251	0.62 (1)
A-B	-4288/0	-127.4	-127.4	0.13 (1)	B-O	-2507/0	0.20 (1)
C-D	-4200/0	-127.4	-127.4	0.14 (1)	C-O	0/2571	0.30 (1)
D-E	-6973/0	-127.4	-127.4	0.37 (1)	K-I	0/5846	0.69 (1)
E-F	-6690/0	-127.4	-127.4	0.39 (1)	D-N	0/4120	0.68 (1)
F-G	-6680/0	-127.4	-127.4	0.42 (1)	K-H	-2064/0	0.40 (1)
G-H	-6680/0	-127.4	-127.4	0.42 (1)	N-E	-1871/0	0.27 (1)
H-I	-4970/0	-127.4	-127.4	0.36 (1)	L-H	0/2016	0.33 (1)
J-I	-3493/0	0.0	0.0	0.44 (1)	E-L	0/521	0.14 (1)
P-B	-4293/0	0.0	0.0	0.59 (1)	L-F	-751/0	0.16 (1)
B-C	-5539/0	0.0	0.0	0.62 (1)			
Q-P	0/0	-28.0	-28.0	0.43 (1)			
P-O	0/3113	-28.0	-28.0	0.70 (1)			
O-S	0/2508	-28.0	-28.0	0.43 (1)			
S-T	0/2508	-28.0	-28.0	0.43 (1)			
T-U	0/2508	-28.0	-28.0	0.43 (1)			
U-N	0/2508	-28.0	-28.0	0.43 (1)			
N-V	0/5973	-28.0	-28.0	0.70 (1)			
V-M	0/5973	-28.0	-28.0	0.70 (1)			
M-W	0/5973	-28.0	-28.0	0.70 (1)			
W-L	0/5973	-28.0	-28.0	0.70 (1)			
L-X	0/4970	-28.0	-28.0	0.72 (1)			
X-Y	0/4970	-28.0	-28.0	0.72 (1)			
Y-Z	0/4970	-28.0	-28.0	0.72 (1)			
Z-K	0/4970	-28.0	-28.0	0.72 (1)			
K-AA	0/0	-28.0	-28.0	0.29 (1)			
AA-AB	0/0	-28.0	-28.0	0.29 (1)			
AB-AC	0/0	-28.0	-28.0	0.29 (1)			
AC-AD	0/0	-28.0	-28.0	0.29 (1)			
AD-J	0/0	-28.0	-28.0	0.29 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0	PSF
DL = 15.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 62.5	PSF	

SPACING = 240 IN. O.C.

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, ECBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(88 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LINE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
CALCULATED VERT. DEFL. (LL) = L/999 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (0.97")
CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.62/1.00 (B-C:1), SC=0.72/1.00 (K-L:1),
WB=0.69/1.00 (J-K:1), SB=0.62/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1867 822 2284 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.80)
JSI METAL= 0.80 (M) (INPUT = 1.00)

DRWG NO. TAM2B036-8
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:g3lAbU0eO_EaFnNpIdPlhXzDFmC-rGG8oMRJLBGaTX6QMbydW0L2dRr?z2EUdYxAXzC1BK

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
M BS-1	MT20	5.0	6.0		
N BMWW-t	MT20	6.0	9.0	3.00	4.25
O BMWWW-t	MT20	4.0	9.0	3.50	1.50
P BMWW-t	MT20	6.0	9.0	3.00	4.25
Q BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 13-0-0, 208.8 lbs FACTORED DOWN AT 15-0-0, 208.8 lbs FACTORED DOWN AT 17-0-0, 208.8 lbs FACTORED DOWN AT 19-0-0, 208.8 lbs FACTORED DOWN AT 21-0-0, 208.8 lbs FACTORED DOWN AT 23-0-0, 208.8 lbs FACTORED DOWN AT 25-0-0, AND 208.8 lbs FACTORED DOWN AT 27-0-0, AND 214.9 lbs FACTORED DOWN AT 28-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

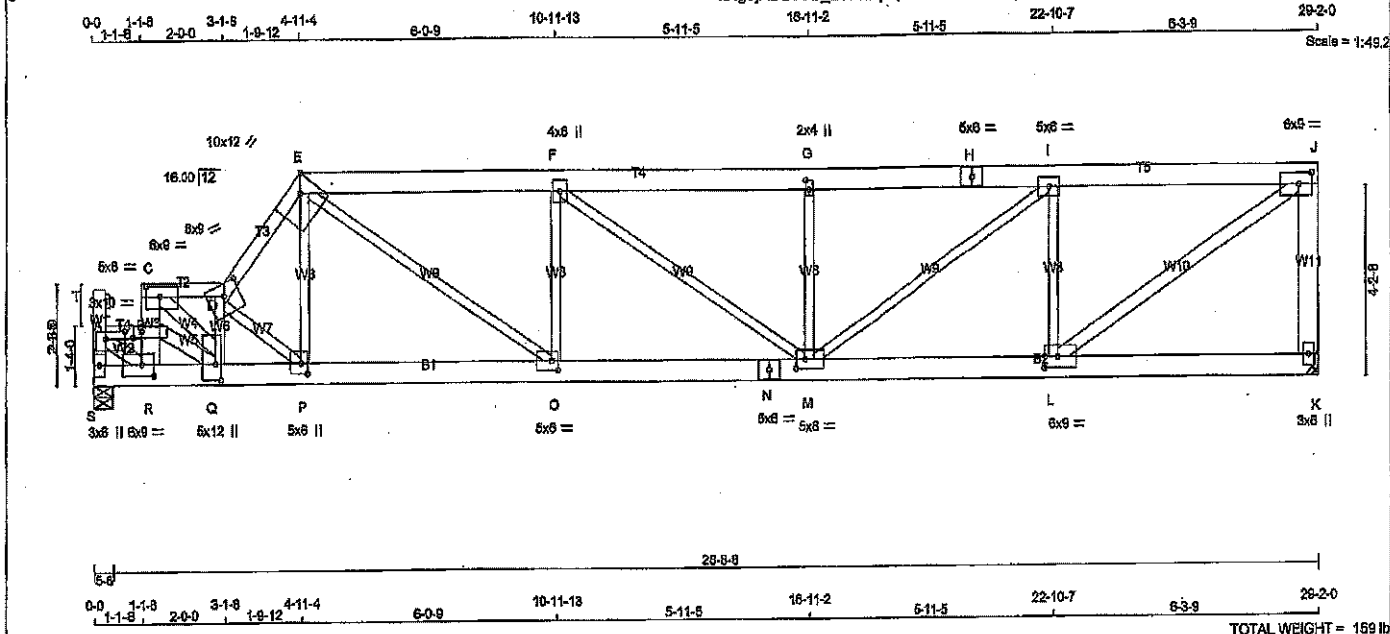
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	13-0-0	-209	-209	---	BACK	VERT	TOTAL
O	3-0-0	-209	-209	---	BACK	VERT	TOTAL
P	1-0-12	-200	-200	---	BACK	VERT	TOTAL
S	5-0-0	-209	-209	---	BACK	VERT	TOTAL
T	7-0-0	-209	-209	---	BACK	VERT	TOTAL
U	9-0-0	-209	-209	---	BACK	VERT	TOTAL
V	11-0-0	-209	-209	---	BACK	VERT	TOTAL
W	15-0-0	-209	-209	---	BACK	VERT	TOTAL
X	17-0-0	-209	-209	---	BACK	VERT	TOTAL
Y	19-0-0	-209	-209	---	BACK	VERT	TOTAL
Z	21-0-0	-209	-209	---	BACK	VERT	TOTAL
AA	23-0-0	-209	-209	---	BACK	VERT	TOTAL
AB	25-0-0	-209	-209	---	BACK	VERT	TOTAL
AC	27-0-0	-209	-209	---	BACK	VERT	TOTAL
AD	28-7-4	-215	-215	---	BACK	VERT	TOTAL



DWG NO. TAM 28036-18
STRUCTURAL
COMPONENT ONLY

JOB NAME 292709	TRUSS NAME T32	QUANTITY 1	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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TOTAL WEIGHT = 169 lb (MTR)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
S - T	2x4	DRY	No.2
A - B	2x4	DRY	No.2
R - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
K - J	2x6	DRY	No.2
S - N	2x6	DRY	No.2
N - K	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
A - R	2x4	DRY	No.2
B - Q	2x4	DRY	No.2
C - Q	2x4	DRY	No.2
L - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (tablets in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TMVW-H	MT20	5.0	8.0 2.00 6.25
B	TMVW-H	MT20	3.0	10.0 1.50 2.50
C	TMVW-H	MT20	8.0	9.0 2.75 4.00
D	TTVW-H	MT20	8.0	9.0 3.25 4.50
E	TTVW-H	MT20	10.0	12.0 Edge 4.50
F	TMVW-H	MT20	4.0	8.0
G	TMVW-H	MT20	2.0	4.0 2.50 1.00
H	TS-I	MT20	5.0	6.0
I	TMVW-H	MT20	5.0	6.0
J	TMVW-H	MT20	6.0	9.0 3.00 3.75
K	BMV-I+p	MT20	3.0	6.0
L	BMVW-H	MT20	6.0	9.0 3.00 3.75
M	BMVW-H	MT20	5.0	8.0 2.50 2.50
N	BS-I	MT20	5.0	8.0
O	BMVW-H	MT20	5.0	6.0 2.50 1.75
P	BMVW-H	MT20	5.0	6.0 2.75 2.00
Q	BMVW-H	MT20	5.0	12.0 4.25 1.50
R	BMVW-H	MT20	6.0	9.0 3.00 3.75
S	BMV-I+p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING					
BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
S	2266	0	2266	0	5-8
K	2266	0	2266	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K.

UNFACTORED REACTIONS

1ST CASE MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	1822	674/0	308/0	0/0	0/0	642/0	0/0
K	1822	674/0	308/0	0/0	0/0	642/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
S-A	-1689/0	0.0	0.0 0.27 (1)	A-R	0/3284	0.71 (1)	
A-T	0/0	0.0	0.0 0.00 (1)	Q-D	-2127/0	0.42 (1)	
A-B	-2689/0	-127.4	-127.4 0.19 (1)	D-P	-1689/0	0.35 (1)	
R-B	-2172/0	0.0	0.0 0.89 (1)	P-E	0/1352	0.41 (1)	
B-C	-2539/0	0.0	0.0 0.87 (1)	B-Q	-948/0	0.12 (1)	
C-D	-3040/0	-127.4	-127.4 0.25 (1)	C-Q	0/3225	0.69 (1)	
D-E	-3619/0	-127.4	-127.4 0.35 (1)	L-J	0/3357	0.72 (1)	
E-F	-3798/0	-127.4	-127.4 0.59 (1)	E-O	0/1910	0.57 (1)	
F-G	-3884/0	-127.4	-127.4 0.57 (1)	L-I	-1687/0	0.62 (1)	
G-H	-3884/0	-127.4	-127.4 0.61 (1)	O-F	-920/0	0.34 (1)	
H-I	-3884/0	-127.4	-127.4 0.61 (1)	M-I	0/1340	0.40 (1)	
I-J	-2781/0	-127.4	-127.4 0.63 (1)	F-M	0/1105	0.03 (1)	
K-J	-2191/0	0.0	0.0 0.55 (1)	M-G	-960/0	0.28 (1)	
S-R	0/0	-28.0	-28.0 0.31 (1)				
R-Q	0/1723	-28.0	-28.0 0.59 (1)				
Q-P	0/3331	-28.0	-28.0 0.77 (1)				
P-O	0/2220	-28.0	-28.0 0.42 (1)				
O-N	0/3788	-28.0	-28.0 0.69 (1)				
N-M	0/3788	-28.0	-28.0 0.69 (1)				
M-L	0/2791	-28.0	-28.0 0.52 (1)				
L-K	0/0	-28.0	-28.0 0.19 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (0.97")
CALCULATED VERT. DEFL. (TL) = 1/959 (0.32")

CS: TC=0.87/1.00 (B-C:1), BO=0.77/1.00 (P-Q:1), WB=0.72/1.00 (J-L:1), SS=0.61/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

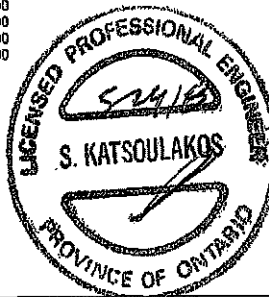
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI), (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 822 2264 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (A) (INPUT = 0.90)
JSI METAL = 0.82 (N) (INPUT = 1.00)



DWG NO. TAM 28037-18
STRUCTURAL COMPONENT ONLY

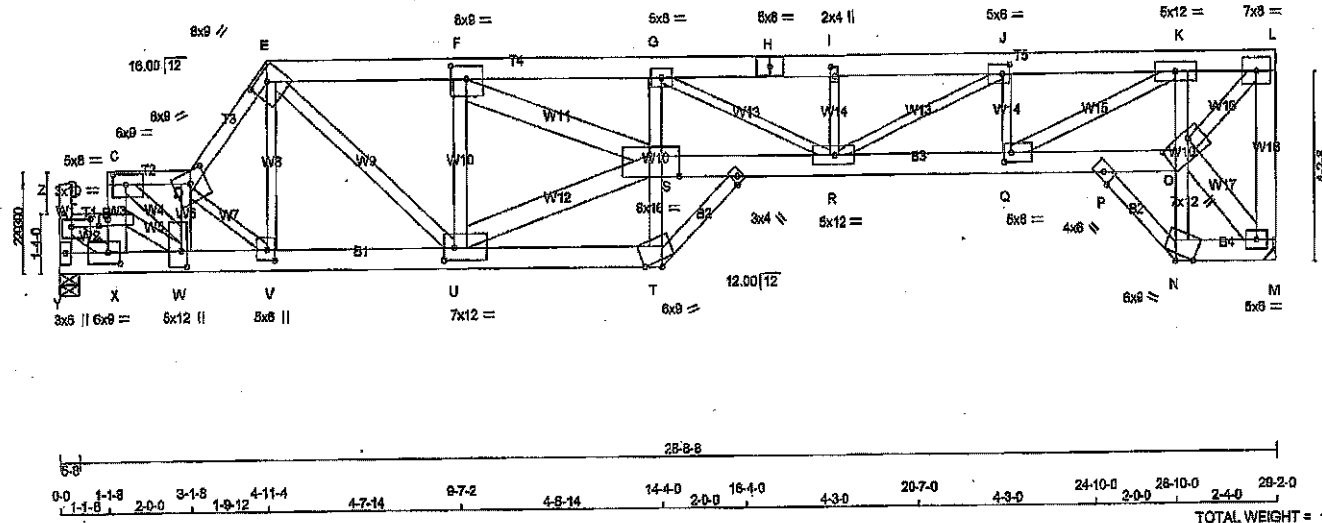
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T32S	1	1	TRUSS DESC.	

Tamrack Roof Truss, Burlington

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ID: g3jAbU0eO_EaFnNplqPInXzDFmCJSqW7ISxWJ7C6Sjx47BAkZSm1nXkPRNHUJzzC1Bj

0-0 1-1.8 2-0.0 3-1.8 4-11.4 4-7-14 9-7-2 4-8-14 14-4-0 16-4-0 2-0-0 4-3-0 20-7-0 4-3-0 24-10-0 2-0-0 28-10-0 2-4-0 29-2-0
Scale = 1/48.6



TOTAL WEIGHT = 190 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
Y - Z	2x4	DRY	No.2
A - B	2x4	DRY	No.2
X - C	2x6	DRY	No.2
C - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x6	DRY	No.2
H - L	2x6	DRY	No.2
M - L	2x6	DRY	No.2
Y - T	2x6	DRY	No.2
S - O	2x6	DRY	2100F 1.8E
P - N	2x4	DRY	No.2
N - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x4	DRY	No.2
W - D	2x3	DRY	No.2
D - V	2x3	DRY	No.2
V - E	2x3	DRY	No.2
O - M	2x3	DRY	No.2
R - I	2x3	DRY	No.2
Q - J	2x3	DRY	No.2
G - R	2x3	DRY	No.2
R - J	2x3	DRY	No.2
U - S	2x3	DRY	No.2
F - S	2x6	DRY	No.2

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	DOWN	HORIZ	IN-SX	IN-SX
Y	2288	0	0	5-8	5-8
M	2288	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS					
	1ST LOSE	MAX. MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
Y	1822	874/0	308/0	0/0	0/0
M	1822	874/0	308/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 2.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 5.01 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO		FROM TO	CSI (L.C)	UNBRACED LENGTH FR-TO			
Y-A	-1986/0	0.0	0.0 0.27 (1)	5.28	A-X	0/3289	0.70 (1)
A-Z	0/0	0.0	0.0 0.00 (1)	10.00	W-D	-2082/0	0.41 (1)
A-B	-2685/0	-127.4	-127.4 0.18 (1)	3.59	D-V	-1569/0	0.88 (1)
X-B	-2187/0	0.0	0.0 0.88 (1)	3.98	V-E	0/1289	0.59 (1)
B-C	-2547/0	0.0	0.0 0.87 (1)	3.60	E-U	0/1777	0.38 (1)
C-D	-3051/0	-127.4	-127.4 0.28 (1)	3.31	U-F	-2597/0	0.69 (1)
D-E	-3590/0	-127.4	-127.4 0.35 (1)	2.88	F-S	0/92	0.28 (1)
E-F	-3514/0	-127.4	-127.4 0.37 (1)	3.73	S-G	-62/93	0.25 (1)
F-G	-7831/0	-127.4	-127.4 0.84 (1)	2.11	N-O	0/101	0.09 (1)
G-H	-8893/0	-127.4	-127.4 0.73 (1)	2.36	O-K	-2121/0	0.20 (1)
H-I	-8893/0	-127.4	-127.4 0.73 (1)	2.36	K-M	-65/9	0.01 (1)
I-J	-8893/0	-127.4	-127.4 0.68 (1)	2.46	L-N	0/2809	0.60 (1)
J-K	-5185/0	-127.4	-127.4 0.43 (1)	3.08	R-I	-591/0	0.12 (1)
K-L	-1859/0	-127.4	-127.4 0.19 (1)	6.07	Q-J	-1414/0	0.29 (1)
M-L	-2165/0	0.0	0.0 0.64 (1)	6.18	G-R	-901/0	0.44 (1)
Y-X	0/0	-28.0	-28.0 0.32 (1)	10.00	R-J	0/1918	0.57 (1)
X-W	0/1718	-28.0	-28.0 0.59 (1)	10.00	Q-K	0/3474	0.74 (1)
W-V	0/2335	-28.0	-28.0 0.75 (1)	10.00	U-S	0/3842	0.89 (1)
V-U	0/2210	-28.0	-28.0 0.40 (1)	10.00	F-S	0/1472	0.12 (1)
U-T	0/85	-28.0	-28.0 0.12 (3)	10.00	B-W	-885/0	0.12 (1)
S-R	0/7898	-28.0	-28.0 0.59 (1)	10.00	C-V	0/2335	0.75 (1)
R-Q	0/5195	-28.0	-28.0 0.43 (1)	10.00			
Q-P	0/2108	-28.0	-28.0 0.20 (1)	10.00			
P-O	0/2093	0.0	0.0 0.20 (1)	10.00			
P-N	0/67	-28.0	-28.0 0.04 (3)	10.00			
N-M	-7187	-28.0	-28.0 0.04 (3)	10.00			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMWV-I	MT20	5.0	8.0	2.00 5.25
B	TMWV-I	MT20	3.0	10.0	1.50 2.50
C	TMWV-I	MT20	6.0	9.0	2.75 4.00
D	TTWV-m	MT20	8.0	9.0	3.25 4.50
E	TTWV-h	MT20	8.0	9.0	2.50 4.50
F	TMWV-I	MT20	8.0	9.0	3.50 4.25
G	TMWV-I	MT20	5.0	8.0	
H	TS-I	MT20	5.0	8.0	
I	TMWV-w	MT20	2.0	4.0	2.50 1.00
J	TMWV-I	MT20	5.0	8.0	2.50 2.00
K	TMWV-I	MT20	5.0	12.0	
L	TMWV-I	MT20	7.0	8.0	
M	BMWV-I	MT20	5.0	8.0	
N	BBW-m	MT20	6.0	9.0	Edge
O	BBWV-w	MT20	7.0	12.0	2.00 8.00
P	BBW-h	MT20	4.0	8.0	Edge
Q	BMWV-I	MT20	5.0	8.0	2.50 2.00
R	BMWV-I	MT20	5.0	12.0	
S	BBWV-I	MT20	6.0	16.0	5.75 8.50
T					
U	BSW-m	MT20	8.0	9.0	Edge
V	BMWV-I	MT20	7.0	12.0	3.25 2.75
W	BMWV-I	MT20	5.0	6.0	2.75 2.00
X	BMWV-I	MT20	6.0	12.0	4.25 1.50
Y	BMV-I	MT20	6.0	9.0	3.00 3.75
AA	NP-h	MT20	3.0	6.0	
			3.0	4.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/998 (0.33")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/548 (0.84")

CSI: TC=0.87/1.00 (B-C-1), BC=0.75/1.00 (V-W-1), WB=0.74/1.00 (K-Q-1), SS=0.81/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

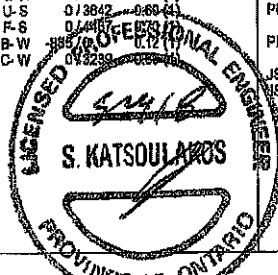
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1687 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 1.00 (S) (INPUT = 1.00)



DRWG NO. TAM 26038-18
STRUCTURAL COMPONENT ONLY

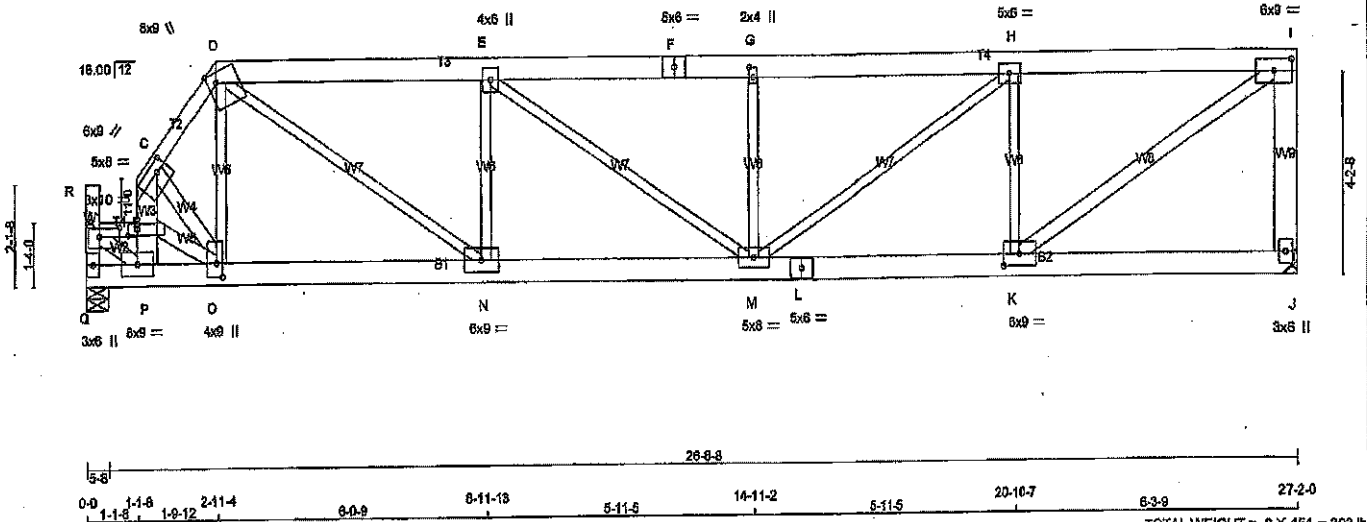
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:g3jAbUDeO_EaFnNpIqPihXzDFmC-oeOuD2TZHzR_pmtVVneQb6heR80T10Xox12FQzC1B1

0-0 1-1-8 1-8-12 2-11-4 6-0-9 8-11-13 5-11-5 14-11-2 5-11-5 20-10-7 6-3-9 27-2-0
1-1-8 1-8-12 2-11-4 6-0-9 8-11-13 5-11-5 14-11-2 5-11-5 20-10-7 6-3-9 27-2-0
Scale = 1:40.4



TOTAL WEIGHT = 2 X 161 = 303 lb

LUMBER	N.L. G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
Q - R	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
J - I	2x6	DRY	No.2	SPF	
P - C	2x6	DRY	No.2	SPF	
Q - L	2x6	DRY	No.2	SPF	
L - J	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	
A - P	2x4	DRY	No.2	SPF	
B - O	2x4	DRY	No.2	SPF	
C - J	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	8.0	2.25 5.00
B	TMVW-1	MT20	3.0	10.0	1.50 2.50
C	TMVW-1	MT20	6.0	9.0	2.25 3.00
D	TMVW+m	MT20	8.0	9.0	2.50 2.25
E	TMVW+1	MT20	4.0	6.0	
F	TS-1	MT20	5.0	6.0	
G	TMVW+1	MT20	2.0	4.0	2.50 1.00
H	TMVW-1	MT20	5.0	6.0	
I	TMVW-1	MT20	6.0	9.0	3.00 4.25
J	BMV1+p	MT20	3.0	6.0	
K	BMVW-1	MT20	6.0	9.0	3.00 4.25
L	BS-1	MT20	6.0	8.0	
M	BMVW+1	MT20	6.0	9.0	
N	BMVW+1	MT20	6.0	9.0	
O	BMVW+1	MT20	4.0	8.0	3.50 1.50
P	BMVW-1	MT20	6.0	9.0	
Q	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
Q	2111	0	2111	0
J	2111	0	2111	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
Q	1697	814 / 0	285 / 0	0 / 0	598 / 0
J	1697	814 / 0	285 / 0	0 / 0	598 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED	FORCE	MAX
				(LBS)							(LBS)	
FR-TO		FROM	TO						FR-TO			
Q-A	-1762 / 0	0.0	0.0	0.24 (1)	5.57	C-D	0 / 232	0.06 (3)				
A-R	0 / 0	0.0	0.0	0.00 (1)	10.30	A-P	0 / 2902	0.82 (1)				
A-B	-2398 / 0	-127.4	-127.4	0.16 (1)	3.83	B-O	-1403 / 0	0.20 (1)				
C-D	-2249 / 0	-127.4	-127.4	0.17 (1)	3.91	C-O	0 / 1980	0.28 (1)				
D-E	-3125 / 0	-127.4	-127.4	0.61 (1)	3.75	K-I	0 / 3077	0.66 (1)				
E-F	-3435 / 0	-127.4	-127.4	0.53 (1)	3.57	D-N	0 / 2168	0.66 (1)				
F-G	-3435 / 0	-127.4	-127.4	0.53 (1)	3.57	K-H	-1514 / 0	0.66 (1)				
G-H	-3435 / 0	-127.4	-127.4	0.58 (1)	3.60	N-E	-1081 / 0	0.40 (1)				
H-I	-2558 / 0	-127.4	-127.4	0.51 (1)	4.08	M-H	0 / 1074	0.32 (1)				
I-J	-2035 / 0	0.0	0.0	0.61 (1)	6.33	E-M	0 / 380	0.11 (1)				
P-B	-2339 / 0	0.0	0.0	0.59 (1)	5.00	M-G	-680 / 0	0.28 (1)				
B-C	-2999 / 0	0.0	0.0	0.81 (1)	4.42							
Q-P	0 / 0	-28.0	-28.0	0.40 (1)	10.00							
P-O	0 / 1731	-28.0	-28.0	0.67 (1)	10.00							
O-N	0 / 1345	-28.0	-28.0	0.32 (1)	10.00							
N-M	0 / 3125	-28.0	-28.0	0.59 (1)	10.00							
M-L	0 / 2558	-28.0	-28.0	0.48 (1)	10.00							
L-K	0 / 2558	-28.0	-28.0	0.48 (1)	10.00							
K-J	0 / 0	-28.0	-28.0	0.19 (3)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. G/G

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 098-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.61")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.61/H.00 (B-C:1), BC=0.87/H.00 (O-P:1),
WB=0.68/H.00 (I-K:1), SS=0.61/H.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1856

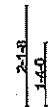
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.80)
JSI METAL= 0.84 (A) (INPUT = 1.00)



DRWG NO. TAM 20059-18
STRUCTURAL
COMPONENT ONLY



DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT L.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.
MAX. UNBRACED INTERIOR CHORD LENGTH = 5.21 FT
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. LCL PLF	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO							
V-A	-1770 / 0	0.0	0.0	0.24 (1)	5.58	D-D	0 / 164	0.06 (3)	
A-W	0 / 0	0.0	0.0	0.02 (1)	10.00	T-S	0 / 1980	0.89 (1)	
A-B	-2379 / 0	-127.4	-127.4	0.15 (1)	3.89	S-E	-2437 / 0	0.91 (1)	
C-D	-2230 / 0	-127.4	-127.4	0.17 (1)	3.92	R-Q	0 / 165	0.27 (1)	
D-E	-2782 / 0	-127.4	-127.4	0.32 (1)	4.17	M-F	-243 / 0	0.26 (1)	
E-F	-6540 / 0	-127.4	-127.4	0.68 (1)	2.48	N-N	0 / 52	0.09 (1)	
F-G	-6168 / 0	-127.4	-127.4	0.62 (1)	2.82	N-J	-1950 / 0	0.18 (1)	
G-H	-6168 / 0	-127.4	-127.4	0.62 (1)	2.52	S-Q	0 / 3029	0.48 (1)	
H-I	-6168 / 0	-127.4	-127.4	0.57 (1)	2.70	E-Q	0 / 4068	0.88 (1)	
I-J	-4710 / 0	-127.4	-127.4	0.42 (1)	3.23	N-L	-28 / 0	0.12 (1)	
J-K	-1767 / 0	-127.4	-127.4	0.22 (1)	5.11	H-K	-577 / 0	0.12 (1)	
L-K	-2037 / 0	0.0	0.0	0.52 (1)	6.28	P-H	-577 / 0	0.26 (1)	
U-B	-2802 / 0	0.0	0.0	0.59 (1)	5.04	O-I	-1259 / 0	0.24 (1)	
B-C	-2970 / 0	0.0	0.0	0.61 (1)	4.44	F-F	-498 / 0	0.24 (1)	
						P-I	0 / 1842	0.49 (1)	
V-U	0 / 0	-28.0	-28.0	0.39 (1)	10.00	Q-J	0 / 3029	0.91 (1)	
U-T	0 / 1743	-28.0	-28.0	0.56 (1)	10.00	A-U	0 / 2915	0.82 (1)	
T-S	0 / 1334	-28.0	-28.0	0.28 (1)	10.00	B-T	-1418 / 0	0.20 (1)	
S-R	0 / 188	-28.0	-28.0	0.11 (3)	10.00	C-T	0 / 182	0.89 (1)	
Q-P	0 / 6810	-28.0	-28.0	0.51 (1)	10.00				
P-O	0 / 4710	-28.0	-28.0	0.39 (1)	10.00				
Q-N	0 / 2018	-28.0	-28.0	0.19 (1)	10.00				
M-L	0 / 18	-28.0	-28.0	0.03 (3)	10.00				

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT
OFF.

(55 % OF 30.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/899 (0.28")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/609 (0.54")

CSI: TC=0.69/1.00 (E-F:1), BC=0.66/1.00 (T-U:1),
WB=0.91/1.00 (J-Q:1), SSI=0.51/1.00 (B-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

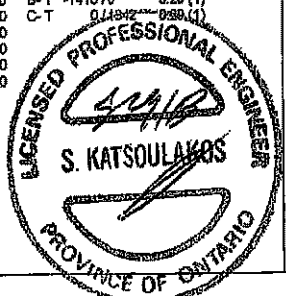
NAIL VALUES					
PLATE GRIP(DRY)		SHEAR		SECTION	
(PSI)		(PLI)		(PLI)	
MAX	MIN	MAX	MIN	MAX	MIN

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.90 (Q) (INPUT = 1.00)

DWG NO. TAM 28040-18
STRUCTURAL
COMPONENT ONLY

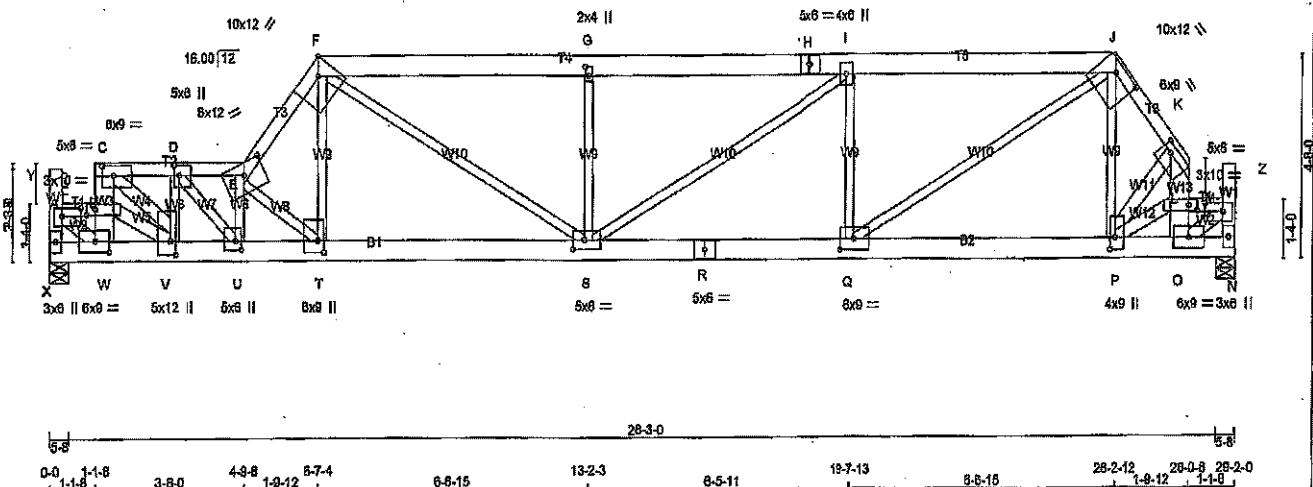
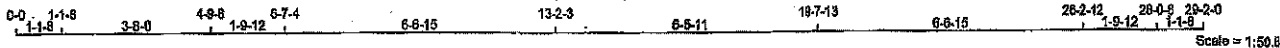


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	CRWG NO.
292709	T34	1	1			

TenTrack Roof Truss, Burlington

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ID:g3JAbU0eO_EaFnNplgPhXzDFmC-GnyHQNTC2GaRwGh3V9fF9eolqVvCJagAambnszC1BH



LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
X - Y	2x4	DRY	No.2
A - B	2x4	DRY	No.2
W - C	2x6	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
J - K	2x4	DRY	No.2
O - K	2x6	DRY	No.2
L - M	2x4	DRY	No.2
N - Z	2x4	DRY	No.2
X - R	2x6	DRY	2100F 1.8E
R - N	2x6	DRY	2100F 1.8E
ALL WEBS EXCEPT			
A - W	2x4	DRY	No.2
O - M	2x4	DRY	No.2
P - L	2x4	DRY	No.2
P - K	2x4	DRY	No.2
B - V	2x4	DRY	No.2
C - V	2x4	DRY	No.2

DRY, SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TVMW-1	MT20	6.0	8.0 2.25 5.75
B	TVMW-1	MT20	6.0	10.0 1.50 2.50
C	TVMW-1	MT20	6.0	8.0 2.75 3.50
D	TVMW-1	MT20	6.0	8.0 2.75 1.50
E	TTWW-m	MT20	8.0	12.0 3.25 6.00
F	TTWW-h	MT20	10.0	12.0 Edge 4.50
G	TTWW-w	MT20	2.0	4.0 2.50 1.00
H	TS-1	MT20	6.0	8.0
I	TVMW-1	MT20	4.0	8.0
J	TTWW-h	MT20	10.0	12.0 2.50 7.00
K	TVMW-1	MT20	8.0	8.0 2.00 2.75
L	TVMW-1	MT20	3.0	10.0 1.50 2.50
M	TVMW-1	MT20	6.0	8.0 2.25 5.25
N	BMV1-p	MT20	3.0	8.0
O	BMVW-1	MT20	8.0	8.0
P	BMVW-1	MT20	4.0	8.0 3.25 1.50
Q	BMVW-1	MT20	6.0	8.0 3.00 4.25
R	BS-1	MT20	6.0	8.0
S	BMVW-1	MT20	6.0	8.0 2.50 3.25
T	BMVW-1	MT20	6.0	8.0 3.25 1.75
U	BMVW-1	MT20	6.0	8.0 2.50 1.75
V	BMVW-1	MT20	5.0	12.0 3.75 1.50
W	BMVW-1	MT20	6.0	8.0 3.00 4.25
X	BMV1-p	MT20	3.0	8.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
X	2268	0	2268	0	0
N	2268	0	2268	0	0

UNFACTORED REACTIONS						
	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
X	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0
N	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	FACTORED	MAX
	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)		FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)
FR-TO				FR-TO			
X-A	-1677 / 0	0.0	0.0 0.28 (1)	A-W	0 / 3100	0.86 (1)	0.86 (1)
A-Y	0 / 0	0.0	0.0 0.00 (1)	U-E	-2036 / 0	0.40 (1)	0.40 (1)
A-B	-2531 / 0	-127.4	-127.4 0.17 (1)	E-T	-2713 / 0	0.61 (1)	0.61 (1)
W-B	-2465 / 0	0.0	0.0 0.88 (1)	T-F	0 / 2305	0.89 (1)	0.89 (1)
B-C	-2748 / 0	0.0	0.0 0.87 (1)	F-J	0 / 2358	0.09 (3)	0.09 (3)
C-D	-3223 / 0	-127.4	-127.4 0.28 (1)	O-M	0 / 2965	0.84 (1)	0.84 (1)
D-E	-4585 / 0	-127.4	-127.4 0.45 (1)	P-L	-1435 / 0	0.21 (1)	0.21 (1)
E-F	-4477 / 0	-127.4	-127.4 0.48 (1)	F-K	0 / 1523	0.33 (1)	0.33 (1)
F-G	-4004 / 0	-127.4	-127.4 0.64 (1)	V-D	-1533 / 0	0.28 (1)	0.28 (1)
G-H	-4004 / 0	-127.4	-127.4 0.64 (1)	D-U	0 / 1669	0.58 (1)	0.58 (1)
H-I	-4004 / 0	-127.4	-127.4 0.64 (1)	B-V	-874 / 0	0.19 (1)	0.19 (1)
I-J	-3570 / 0	-127.4	-127.4 0.60 (1)	C-V	0 / 3458	0.74 (1)	0.74 (1)
J-K	-2447 / 0	-127.4	-127.4 0.19 (1)	Q-W	0 / 2591	0.78 (1)	0.78 (1)
O-L	-2612 / 0	0.0	0.0 0.62 (1)	F-S	0 / 1481	0.44 (1)	0.44 (1)
L-K	-3289 / 0	0.0	0.0 0.85 (1)	Q-I	-1187 / 0	0.44 (1)	0.44 (1)
L-M	-2445 / 0	-127.4	-127.4 0.18 (1)	S-G	-893 / 0	0.33 (1)	0.33 (1)
N-M	-1817 / 0	0.0	0.0 0.25 (1)	B-I	0 / 1517	0.18 (1)	0.18 (1)
M-Z	0 / 0	0.0	0.0 0.00 (1)				

X-W	0 / 0	-28.0	-28.0 0.24 (1)	10.00
W-V	0 / 1575	-28.0	-28.0 0.34 (1)	10.00
V-U	0 / 3349	-28.0	-28.0 0.48 (1)	10.00
U-T	0 / 4680	-28.0	-28.0 0.57 (1)	10.00
T-S	0 / 2773	-28.0	-28.0 0.23 (1)	10.00
S-R	0 / 3570	-28.0	-28.0 0.29 (1)	10.00
R-Q	0 / 3570	-28.0	-28.0 0.29 (1)	10.00
Q-P	0 / 1483	-28.0	-28.0 0.17 (1)	10.00
P-O	0 / 1781	-28.0	-28.0 0.40 (1)	10.00
O-N	0 / 0	-28.0	-28.0 0.28 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 30.0 PSF
DL = 18.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-08
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.97")
CALCULATED VERT. DEFL. (LL) = 1/888 (0.19")
ALLOWABLE DEFL. (TL) = 1/960 (0.97")
CALCULATED VERT. DEFL. (TL) = 1/688 (0.36")

CS1: TC=0.87/1.00 (B-C-1), BC=0.57/1.00 (T-U-1), WB=0.75/1.00 (J-Q-1), SSI=0.80/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

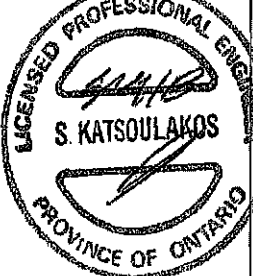
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 822 2284 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

PLATE GRIP=0.90 (O) (INPUT = 0.90)
METAL=0.81 (F) (INPUT = 1.00)



DWG NO. TAM 2804-19
STRUCTURAL
COMPONENT ONLY

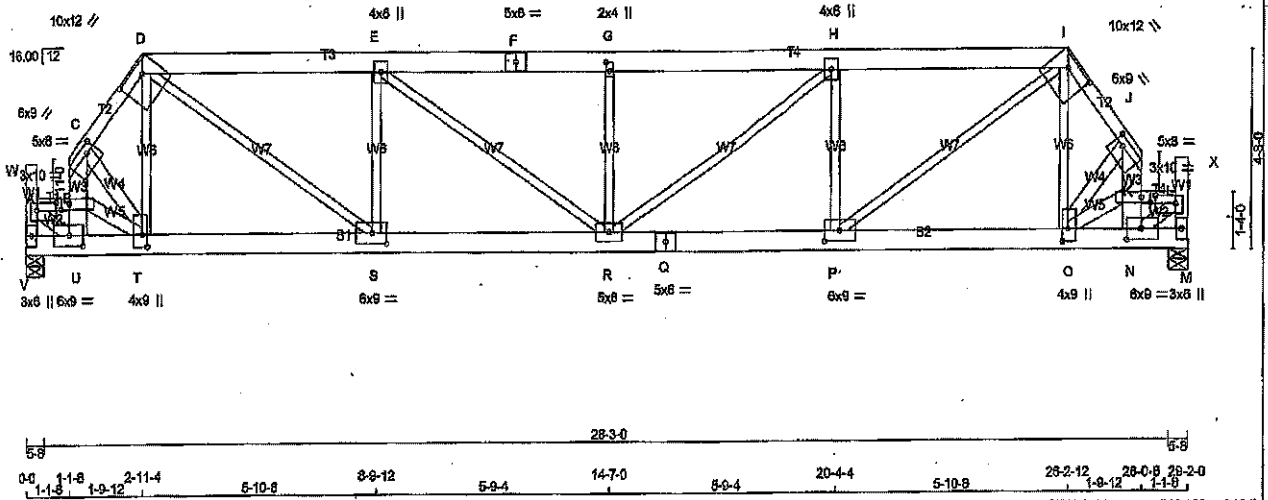
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T35	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:g3jAbU0eO_EaFnNplqPhXzDFmC-GcyHQNTC2GarRwGh3V9f9erYqTSCJHgAambnszC1BH

0-0 1-1-8 2-11-4 8-10-8 8-8-12 5-9-4 14-7-0 5-9-4 20-4-4 5-10-8 28-2-12 28-0-8 28-2-0
1-1-8 1-9-12 5-10-8 8-9-12 5-9-4 14-7-0 5-9-4 20-4-4 5-10-8 28-2-12 28-0-8 28-2-0
Scale = 1:51.9



TOTAL WEIGHT = 5 X 163 = 813 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
V - W	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x6	DRY	No.2	SPF	
F - I	2x6	DRY	No.2	SPF	
I - J	2x4	DRY	No.2	SPF	
N - J	2x6	DRY	No.2	SPF	
K - L	2x4	DRY	No.2	SPF	
M - X	2x4	DRY	No.2	SPF	
U - C	2x6	DRY	No.2	SPF	
V - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
N - L	2x4	DRY	No.2	SPF	
A - U	2x4	DRY	No.2	SPF	
B - T	2x4	DRY	No.2	SPF	
C - T	2x4	DRY	No.2	SPF	
O - K	2x4	DRY	No.2	SPF	
O - J	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
V	2266	0	0	5-8
M	2266	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
V	1822	874 / 0	306 / 0	0 / 0	642 / 0
M	1822	874 / 0	306 / 0	0 / 0	642 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
V-A	-1693 / 0	0.0	0.0	0.28 (1)	5.40	T-D	0 / 222
A-W	0 / 0	0.0	0.0	0.00 (1)	10.00	O-I	0 / 222
A-B	-2563 / 0	-127.4	-127.4	0.17 (1)	3.69	N-L	0 / 3128
C-D	-2431 / 0	-127.4	-127.4	0.19 (1)	3.77	P-I	0 / 2406
D-E	-3411 / 0	-127.4	-127.4	0.50 (1)	3.83	D-S	0 / 2406
E-F	-3987 / 0	-127.4	-127.4	0.55 (1)	3.33	P-H	-1253 / 0
F-G	-3987 / 0	-127.4	-127.4	0.55 (1)	3.33	S-E	-1253 / 0
G-H	-8907 / 0	-127.4	-127.4	0.55 (1)	3.33	R-H	0 / 712
H-I	-3411 / 0	-127.4	-127.4	0.60 (1)	3.63	E-R	0 / 712
I-J	-2431 / 0	-127.4	-127.4	0.19 (1)	3.77	R-G	-581 / 0
N-K	-2818 / 0	0.0	0.0	0.84 (1)	4.70	A-U	0 / 3128
K-J	-3236 / 0	0.0	0.0	0.86 (1)	4.11	B-T	-1511 / 0
K-L	-2553 / 0	-127.4	-127.4	0.17 (1)	3.69	C-T	0 / 1472
M-L	-1693 / 0	0.0	0.0	0.28 (1)	5.40	O-K	-1511 / 0
L-X	0 / 0	0.0	0.0	0.00 (1)	10.00	O-J	0 / 1472
U-B	-2818 / 0	0.0	0.0	0.84 (1)	4.70		
B-C	-3236 / 0	0.0	0.0	0.86 (1)	4.11		
V-U	0 / 0	-28.0	-28.0	0.43 (1)	10.00		
U-T	0 / 1866	-28.0	-28.0	0.72 (1)	10.00		
T-S	0 / 1454	-28.0	-28.0	0.34 (1)	10.00		
S-R	0 / 3411	-28.0	-28.0	0.83 (1)	10.00		
R-Q	0 / 3411	-28.0	-28.0	0.83 (1)	10.00		
Q-P	0 / 3411	-28.0	-28.0	0.83 (1)	10.00		
P-O	0 / 1454	-28.0	-28.0	0.34 (1)	10.00		
O-N	0 / 1866	-28.0	-28.0	0.72 (1)	10.00		
N-M	0 / 0	-28.0	-28.0	0.43 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	30.0	PSF
DL	=	15.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 088-08
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/860 (0.97")
CALCULATED VERT. DEFL. (LL) = L/999 (0.16")
ALLOWABLE DEFL. (TL) = L/860 (0.97")
CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TO=0.89/1.00 (B-C:1), BC=0.72/1.00 (T-U:1),
WB=0.72/1.00 (P-I), SSI=0.55/1.00 (K-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

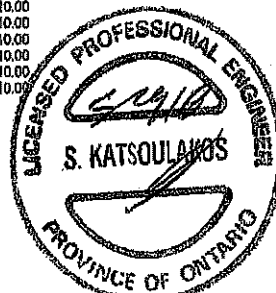
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(PY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1687 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.75 (C) (INPUT = 1.00)

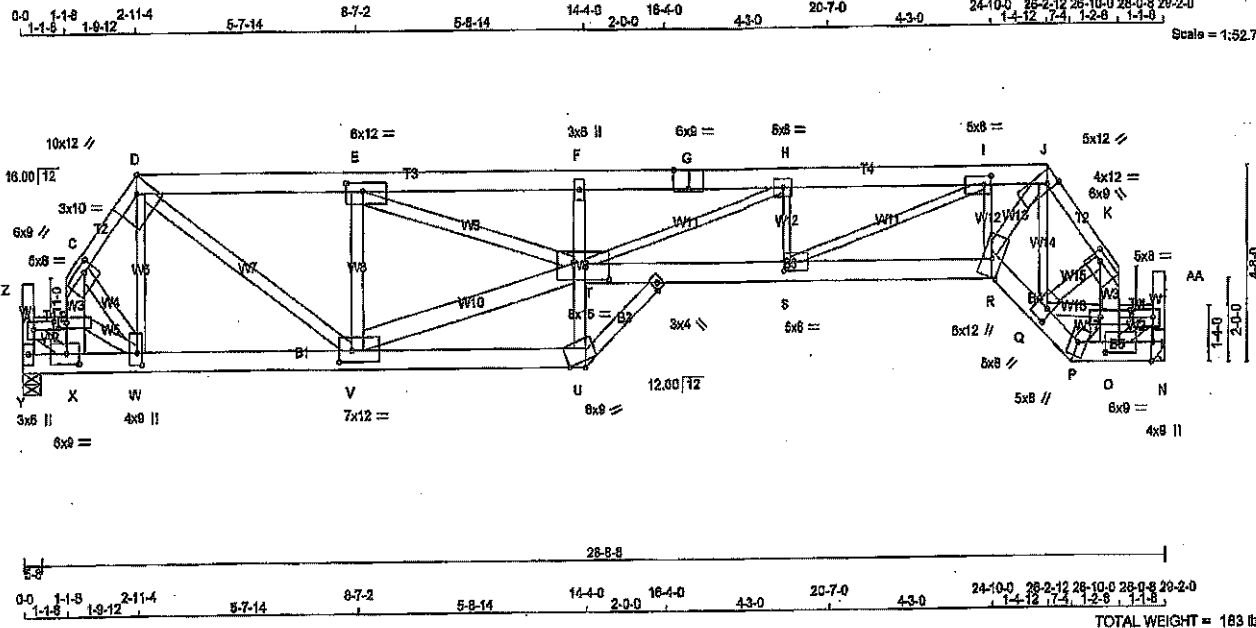


DRWG NO. TAM 28042-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T35S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.210 S May 18 2018 Mitek Industries, Inc. Mon May 28 09:14:21 2018 Page 1
ID:g3jAbU0eD_EaFmHplqPlnXzDFmC-k1VWdUqpai34rudCgunM80KEooUpFEW8KizC1BG



TOTAL WEIGHT = 183.10

LUMBER	SIZE	DRY	NO.2	SPF
Y - Z	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	2100F 1.8E	SPF
G - J	2x6	DRY	2100F 1.8E	SPF
J - K	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - AA	2x4	DRY	No.2	SPF
X - C	2x6	DRY	No.2	SPF
Y - U	2x6	DRY	No.2	SPF
T - R	2x6	DRY	2100F 1.8E	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - D	2x3	DRY	No.2	SPF
R - I	2x3	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
Q - J	2x3	DRY	No.2	SPF
S - H	2x3	DRY	No.2	SPF
T - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-H	MT20	5.0	8.0	2.25	6.00
B	TVMW-H	MT20	3.0	10.0	1.50	2.50
C	TMVW-H	MT20	6.0	9.0	2.00	3.00
D	TTVW-H	MT20	10.0	12.0	Edge	4.50
E	TMVW-H	MT20	8.0	12.0	2.50	5.00
F	TMVW-H	MT20	3.0	8.0		
G	TS-H	MT20	8.0	9.0	Edge	4.50
H	TMVW-H	MT20	5.0	6.0		
I	TMVW-H	MT20	5.0	8.0	2.50	2.25
J	TTVW-H	MT20	5.0	12.0	3.25	1.75
K	TMVW-H	MT20	6.0	8.0	2.00	2.75
L	TVMWV-H	MT20	4.0	12.0	2.00	8.75
M	TVMW-H	MT20	5.0	8.0	2.25	6.00
N	BMV-H	MT20	4.0	9.0	Edge	0.80
O	BMVW-H	MT20	8.0	9.0	3.00	4.00
P	BBW-H	MT20	5.0	8.0		
Q	BMVWV-H	MT20	5.0	8.0	3.75	1.50
R	BBW-H	MT20	6.0	12.0	5.00	3.00
S	BMVW-H	MT20	5.0	8.0	2.50	2.25
T	BMVWV-H	MT20	8.0	18.0	4.50	7.25
U						
U						
U	BBW-H	MT20	6.0	9.0	Edge	
V	BMVWV-H	MT20	7.0	12.0	3.25	3.75
W	BMVWV-H	MT20	4.0	6.0	3.50	1.50
X	BMV-H	MT20	6.0	9.0	3.00	4.00
Y	BMV-H	MT20	3.0	6.0		
AB	NH-H	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Y	2286	0	0	5-8
N	2286	0	0	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

	1ST LOASE	MAX MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Y	1821	874/0	308/0
N	1822	874/0	308/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.61 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX FACTORED	FACTORED	VERT. LOAD	MAX	MAX	MEMB.	MAX	MAX	MAX
MEMB.	FORCE	FORCE	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FORCE	MAX	CS1 (LC)
FR-TO		FROM TO				FR-TO			
Y-A	-1901/0	0.0	0.0	0.26 (1)	5.39	W-O	0/176	0.06 (3)	
A-Z	0/0	0.0	0.0	0.00 (1)	10.00	D-V	0/2394	0.51 (1)	
A-B	-2584/0	-127.4	-127.4	0.17 (1)	3.69	V-E	-2441/0	0.85 (1)	
C-D	-2408/0	-127.4	-127.4	0.18 (1)	3.78	U-T	0/114	0.29 (1)	
D-E	-3347/0	-127.4	-127.4	0.27 (1)	4.75	T-F	-574/0	0.29 (1)	
E-F	-7400/0	-127.4	-127.4	0.43 (1)	3.27	R-I	-1654/0	0.34 (1)	
F-G	-7482/0	-127.4	-127.4	0.36 (1)	3.30	C-M	0/3148	0.87 (1)	
G-H	-7482/0	-127.4	-127.4	0.39 (1)	3.30	V-T	0/3538	0.58 (1)	
H-I	-6846/0	-127.4	-127.4	0.29 (1)	3.68	E-T	0/4276	0.92 (1)	
I-J	-3960/0	-127.4	-127.4	0.14 (1)	4.58	B-W	-1627/0	0.22 (1)	
J-K	-3035/0	-127.4	-127.4	0.26 (1)	3.34	C-W	0/1451	0.31 (1)	
K-L	-2707/0	0.0	0.0	0.45 (1)	5.00	A-X	0/3141	0.67 (1)	
L-M	-3480/0	0.0	0.0	0.49 (1)	4.40	Q-J	-1042/0	0.29 (1)	
N-M	-2588/0	-127.4	-127.4	0.17 (1)	3.68	R-J	0/14038	0.97 (1)	
M-AA	0/0	0.0	0.0	0.26 (1)	5.41	Q-K	-908/0	0.13 (1)	
X-B	-2475/0	0.0	0.0	0.63 (1)	4.75	P-L	-1168/0	0.15 (1)	
B-C	-3201/0	0.0	0.0	0.65 (1)	4.14	S-I	-1040/0	0.27 (1)	
						S-I	0/2538	0.26 (1)	
Y-X	0/0	-28.0	-28.0	0.42 (1)	10.00	T-H	0/2538	0.26 (1)	
X-Y	0/1878	-28.0	-28.0	0.71 (1)	10.00				
W-V	0/1441	-28.0	-28.0	0.32 (3)	10.00				
V-U	0/78	-28.0	-28.0	0.18 (3)	10.00				
T-S	0/6848	-28.0	-28.0	0.61 (1)	10.00				
S-R	0/4031	-28.0	-28.0	0.34 (1)	10.00				
R-Q	0/2382	-28.0	-28.0	0.48 (1)	10.00				
Q-P	0/1935	-28.0	-28.0	0.41 (1)	10.00				
P-O	0/2093	-28.0	-28.0	0.77 (1)	10.00				
O-N	0/0	-28.0	-28.0	0.44 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0	PSF
DL	= 15.0	PSF
BOT CH.	LL = 10.5	PSF
DL	= 7.0	PSF
TOTAL LOAD	= 62.5	PSF

SPACING = 24.0 IN. GIG

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-08
- TPI 2011

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
CALCULATED VERT. DEFL. (LL) = L/589 (0.32")
ALLOWABLE DEFL. (TL) = L/360 (0.87")
CALCULATED VERT. DEFL. (TL) = L/674 (0.81")

CS1: TO=0.88/1.00 (B-C:1), BC=0.77/1.00 (O-P:1),
WB=0.92/1.00 (E-T:1), SS1=0.55/1.00 (B-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

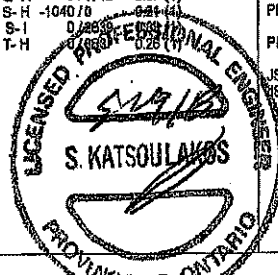
NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

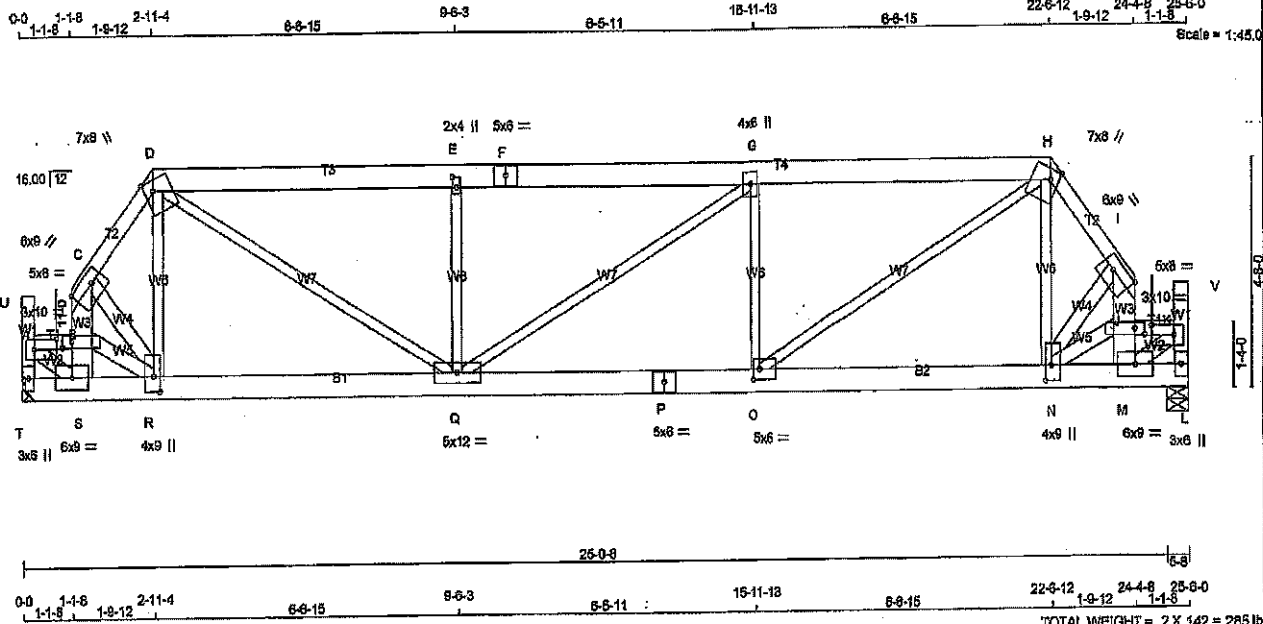
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.80 (C) (INPUT=0.80)
JSI METAL=0.92 (T) (INPUT=1.00)



DWG NO. TAM28043-18
STRUCTURAL COMPONENT ONLY

JOB NAME 292709	TRUSS NAME T36	QUANTITY 2	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
Version 3.210 5 May 18 2018 M/Tek Industries, Inc. Mon May 28 09:14:22 2018 Page 1 ID: g3JAbU0eO_EaFnNpIqPihXzDFmC-CD41r3VSAuqYgEQ4Av87KakDZeALgFjzduFisZC1BF					
Tamarack Roof Truss, Burlington					



LUMBER	N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
T - U	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
L - V	2x4	DRY	No.2	SPF
S - C	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
M - K	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
C - J	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW4	MT20	5.0	8.0	2.50	5.75
B	TVMW4	MT20	3.0	10.0	1.50	2.50
C	TVMW4	MT20	6.0	9.0	Edge	5.75
D	TTWW4+m	MT20	7.0	8.0	2.50	2.25
E	TVMW4	MT20	2.0	4.0	2.50	1.00
F	TS-1	MT20	5.0	8.0		
G	TVMW4	MT20	4.0	8.0		
H	TTWW4+m	MT20	7.0	8.0	2.50	2.25
I	TVMW4	MT20	6.0	9.0	Edge	5.75
J	TVMW4	MT20	3.0	10.0	1.50	2.50
K	TVMW4	MT20	6.0	8.0	2.50	6.75
L	BMV1+p	MT20	3.0	6.0		
M	BMVW4	MT20	6.0	9.0		
N	BMVW4+H	MT20	4.0	9.0	3.75	1.50
O	BMVW4	MT20	5.0	6.0	2.50	1.75
P	BS-1	MT20	5.0	6.0		
Q	BMVW4+H	MT20	5.0	12.0		
R	BMVW4+H	MT20	4.0	9.0	3.75	1.50
S	BMVW4	MT20	6.0	9.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
T	1981	0	1981	0
L	1981	0	1981	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT T.

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL		
T	1593	764 / 0	268 / 0	0 / 0	0 / 0	551 / 0	0 / 0		
L	1593	764 / 0	268 / 0	0 / 0	0 / 0	551 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. HORIZ. LOAD (LC2)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LC2)
FR-TO		FROM TO			FR-TO		
T-A	-1651 / 0	0.0	0.0	0.23 (1)	5.73	R-D	0 / 261
A-U	0 / 0	0.0	0.0	0.00 (1)	10.00	N-H	0 / 259
A-B	-210 / 0	-127.4	-127.4	0.14 (1)	3.87	M-K	0 / 2708
C-D	-2105 / 0	-127.4	-127.4	0.16 (1)	4.03	A-S	0 / 2708
D-E	-2627 / 0	-127.4	-127.4	0.54 (1)	3.81	B-R	-1308 / 0
E-F	-2627 / 0	-127.4	-127.4	0.55 (1)	3.79	C-R	0 / 1274
F-G	-2627 / 0	-127.4	-127.4	0.55 (1)	3.79	N-J	-1310 / 0
G-H	-2633 / 0	-127.4	-127.4	0.55 (1)	3.79	N-I	0 / 1272
H-I	-2102 / 0	-127.4	-127.4	0.16 (1)	4.04	O-H	0 / 1690
M-J	-2200 / 0	0.0	0.0	0.55 (1)	5.25	D-Q	0 / 1590
J-I	-2611 / 0	0.0	0.0	0.57 (1)	4.66	O-G	-800 / 0
J-K	-2212 / 0	-127.4	-127.4	0.23 (1)	5.73	Q-E	-898 / 0
L-K	-1651 / 0	0.0	0.0	0.23 (1)	5.73	Q-G	-8 / 0
K-V	0 / 0	0.0	0.0	0.00 (1)	10.00		
S-B	-2205 / 0	0.0	0.0	0.55 (1)	5.24		
B-C	-2614 / 0	0.0	0.0	0.57 (1)	4.66		

T-S	0 / 0	-28.0	-28.0	0.39 (1)	10.00
S-R	0 / 1615	-28.0	-28.0	0.63 (1)	10.00
R-Q	0 / 1258	-28.0	-28.0	0.32 (1)	10.00
Q-P	0 / 2833	-28.0	-28.0	0.58 (1)	10.00
P-O	0 / 2833	-28.0	-28.0	0.58 (1)	10.00
O-N	0 / 1257	-28.0	-28.0	0.32 (1)	10.00
N-M	0 / 1615	-28.0	-28.0	0.63 (1)	10.00
M-L	0 / 0	-28.0	-28.0	0.39 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/G

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA S806-06
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 30.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.85")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.11")
ALLOWABLE DEFL.(TL) = $L/360$ (0.85")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSL TO = 0.57 / 1.00 (B-C), BC = 0.83 / 1.00 (R-S-1), WB = 0.80 / 1.00 (H-C-1), SB = 0.48 / 1.00 (B-S-1)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10
COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

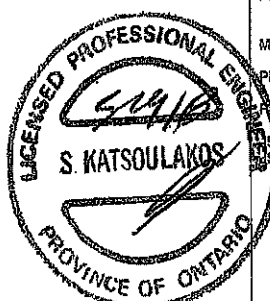
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	318	354	1667
	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.90 (K) (INPUT = 0.90)
METAL = 0.68 (P) (INPUT = 1.00)



DWG NO. TAM 28044/18
STRUCTURAL
COMPONENT ONLY

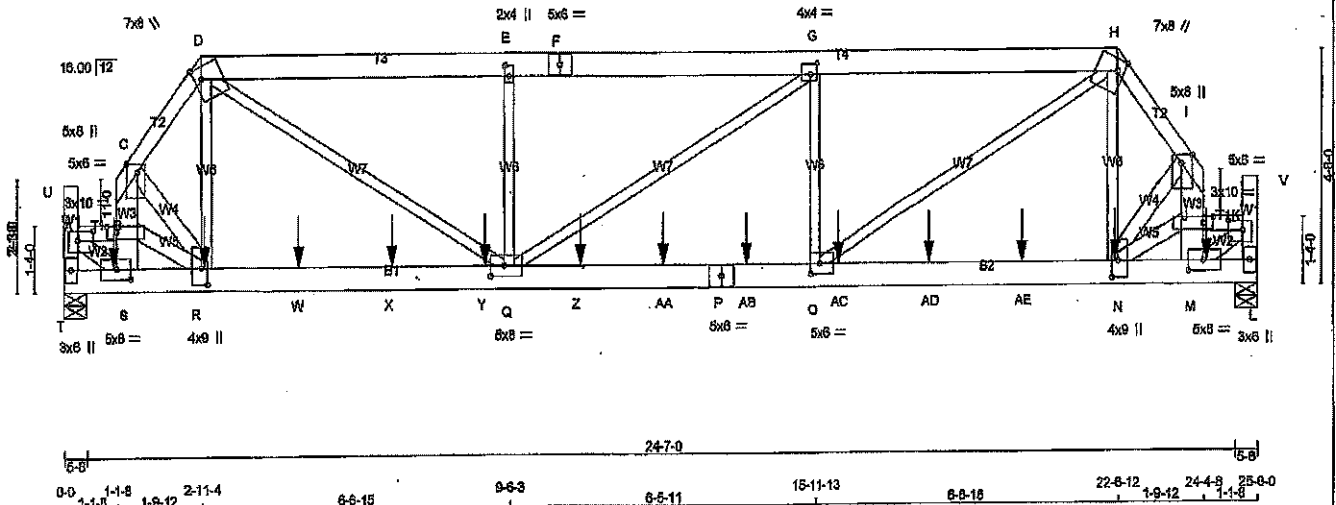
JOB NAME 292709	TRUSS NAME T36Z	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tarbrack Roof Truss, Burlington

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ID:g3jAbU0eO_EaFnNplqPInXzDFmC-CD41r3VSeuqYgEQ4AvB7KakD5eAigGXzduFisizC1BF

0-0 1-1-8 2-11-4 6-6-15 9-6-3 6-5-11 15-11-13 6-5-15 22-6-12 24-4-8 25-6-0
1-1-8 1-9-12 1-1-8 Scale = 1/44.1



TOTAL WEIGHT = 2 X 142 = 285 lb

CHORDS	SIZE	LUMBER	DESCR.
T - U	2x4	DRY No.2	SPF
A - B	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
D - E	2x6	DRY No.2	SPF
F - H	2x6	DRY No.2	SPF
H - I	2x4	DRY No.2	SPF
M - I	2x6	DRY No.2	SPF
J - K	2x4	DRY No.2	SPF
L - V	2x4	DRY No.2	SPF
S - C	2x6	DRY No.2	SPF
T - P	2x6	DRY No.2	SPF
P - L	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
A - S	2x4	DRY No.2	SPF
C - R	2x4	DRY No.2	SPF
O - R	2x4	DRY No.2	SPF
N - J	2x4	DRY No.2	SPF
N - I	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'x3') SPIRAL NAILS		
T-U 1	12	TOP
A-B 1	12	TOP
C-D 1	12	TOP
D-E 1	12	TOP
F-H 1	12	TOP
H-I 1	12	TOP
J-K 1	12	TOP
V-L 1	12	TOP
D-F 2	12	TOP
F-H 2	12	TOP
L-M 2	12	TOP
C-S 2	12	TOP
BOTTOM CHORDS : (0.122'x3') SPIRAL NAILS		
T-P 2	12	SIDE(137.3)
P-L 2	12	SIDE(137.3)
WEBS : (0.122'x3') SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMMW-H	MT20	5.0	8.0	2.25	3.75
B TMMW-H	MT20	3.0	10.0	1.50	2.50
C TMMW+P	MT20	5.0	8.0	2.00	2.75
D TTVW+M	MT20	7.0	8.0	2.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
JT	VERT	DOWN	UP	IN-SX
T	3380	0	0	5-8
L	3330	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	2591	1356 / 0	332 / 0	0 / 0	0 / 0	901 / 0	0 / 0
L	2591	1356 / 0	332 / 0	0 / 0	0 / 0	901 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO		
T-A	-2743 / 0	0.0	0.21 (1)	R-D	0 / 706	0.12 (1)
A-U	0 / 0	0.0	0.00 (1)	N-H	0 / 701	0.12 (1)
A-B	-3754 / 0	-127.4	-127.4 0.11 (1)	M-K	0 / 4601	0.64 (1)
C-D	-3868 / 0	-127.4	-127.4 0.12 (1)	A-S	0 / 4599	0.64 (1)
D-E	-4862 / 0	-127.4	-127.4 0.33 (1)	B-R	-2168 / 0	0.17 (1)
E-F	-4863 / 0	-127.4	-127.4 0.31 (1)	C-R	0 / 2244	0.26 (1)
F-G	-4863 / 0	-127.4	-127.4 0.31 (1)	N-J	-2168 / 0	0.17 (1)
G-H	-4869 / 0	-127.4	-127.4 0.33 (1)	C-H	0 / 2241	0.26 (1)
H-I	-3867 / 0	-127.4	-127.4 0.12 (1)	Q-H	0 / 3303	0.65 (1)
M-J	-3763 / 0	0.0	0.0 0.51 (1)	D-Q	0 / 3293	0.64 (1)
J-I	-4848 / 0	0.0	0.0 0.53 (1)	Q-G	-691 / 0	0.17 (1)
J-K	-3766 / 0	-127.4	-127.4 0.11 (1)	Q-E	-680 / 0	0.17 (1)
L-K	-2744 / 0	0.0	0.0 0.21 (1)	Q-G	-8 / 0	0.01 (1)
K-V	0 / 0	0.0	0.0 0.00 (1)			
S-B	-3767 / 0	0.0	0.0 0.51 (1)			
B-C	-4840 / 0	0.0	0.0 0.53 (1)			
T-S	0 / 0	-28.0	-28.0 0.38 (1)			
S-R	0 / 2732	-28.0	-28.0 0.61 (1)			
R-W	0 / 2187	-28.0	-28.0 0.40 (1)			
W-X	0 / 2187	-28.0	-28.0 0.40 (1)			
X-Y	0 / 2187	-28.0	-28.0 0.40 (1)			
Y-Q	0 / 2187	-28.0	-28.0 0.40 (1)			
Q-Z	0 / 14969	-28.0	-28.0 0.63 (1)			
Z-AA	0 / 14969	-28.0	-28.0 0.63 (1)			
AA-P	0 / 14969	-28.0	-28.0 0.63 (1)			
P-AB	0 / 14969	-28.0	-28.0 0.63 (1)			
AB-O	0 / 14969	-28.0	-28.0 0.63 (1)			
O-AC	0 / 2186	-28.0	-28.0 0.40 (1)			
AC-AD	0 / 2186	-28.0	-28.0 0.40 (1)			
AD-AE	0 / 2186	-28.0	-28.0 0.40 (1)			
AE-N	0 / 2186	-28.0	-28.0 0.40 (1)			
N-M	0 / 2733	-28.0	-28.0 0.61 (1)			
M-L	0 / 0	-28.0	-28.0 0.38 (1)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	DIR.	TYPE
M	24-5-4	-200	-200	-	FRONT	VERT	TOTAL
N	22-6-0	-200	-200	-	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 30.0 PSF
DL	= 15.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.0042

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 36.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.85")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.85")
CALCULATED VERT. DEFL. (TL) = L/999 (0.10")

CSI: TC=0.831/0.0 (B-C:1), BC=0.63/1.00 (C-Q:1),
WB=0.55/1.00 (H-C:1), SS=0.45/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 818 354 1957 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.60)
JSI METAL= 0.62 (P) (INPUT = 1.00)

DRWG NO. TAM 2.804518
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T36Z	1	2	TRUSS DESC.	

Tenarack Roof Truss, Burlington

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ID:q3AbU0eO EaFnPlqPihXzDFmC-CD41r3VSauYgEQ4AvB7KakD5eAlgGXzduFisIzC1BF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	TMW+H	MT20	2.0	4.0	2.50	1.00
F	T8-I	MT20	5.0	6.0		
G	TMWW-I	MT20	4.0	4.0	2.50	1.75
H	TTWW+m	MT20	7.0	8.0	2.75	1.75
I	TMWW+p	MT20	5.0	8.0	2.00	2.75
J	TMWW-I	MT20	3.0	10.0	1.50	2.50
K	TMWW-I	MT20	5.0	6.0	2.25	3.75
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-I	MT20	5.0	8.0	2.50	3.75
N	BMWWWW-I	MT20	4.0	9.0	4.00	1.50
O	BMWW-I	MT20	5.0	6.0	2.50	2.50
P	BS-I	MT20	5.0	6.0		
Q	BMWWWW-I	MT20	5.0	8.0	2.50	3.50
R	BMWWWW-I	MT20	4.0	9.0	4.00	1.50
S	BMWW-I	MT20	5.0	8.0	2.50	3.75
T	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	3-0-0	-209	-209	—	FRONT	VERT	TOTAL
S	1-0-12	-200	-200	—	FRONT	VERT	TOTAL
W	5-0-0	-209	-209	—	FRONT	VERT	TOTAL
X	7-0-0	-208	-208	—	FRONT	VERT	TOTAL
Y	9-0-0	-209	-209	—	FRONT	VERT	TOTAL
Z	11-0-0	-209	-209	—	FRONT	VERT	TOTAL
AA	12-9-0	-209	-209	—	FRONT	VERT	TOTAL
AB	14-6-0	-209	-209	—	FRONT	VERT	TOTAL
AC	16-6-0	-209	-209	—	FRONT	VERT	TOTAL
AD	18-6-0	-208	-208	—	FRONT	VERT	TOTAL
AE	20-6-0	-209	-209	—	FRONT	VERT	TOTAL

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 12-9-0, 208.8 lbs FACTORED DOWN AT 14-8-0, 208.8 lbs FACTORED DOWN AT 16-8-0, 208.8 lbs FACTORED DOWN AT 18-8-0, 208.8 lbs FACTORED DOWN AT 20-8-0, AND 208.8 lbs FACTORED DOWN AT 22-8-0, AND 200.3 lbs FACTORED DOWN AT 24-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 28045-18
STRUCTURAL
COMPONENT ONLY

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

DRWG NO.

292709

T37

1

4

TRUSS DESC.

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Tamarack Roof Truss, Burlington

ID:q3JAbU0eQ EaFnNpIdPihXzDFmC-qPdP2PV4LBypIQ7GkdIMnGMF2SnPfa8eY7FOBzC1BE

IN ADDITION, PRE-DRILL ONE 0.88" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSAO88 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FORCE (LBS)

MAX. FACTORED

VERT. LOAD

LC1

MAX.

MAX.

MEMB. FORCE (LBS)

MAX. FACTORED

CS1 (LC)

UNBRAC

FR-TO

FROM TO

LENGTH

FR-TO

FR-TO

V-AD

AD-U

U-T

T-AE

AE-S

S-R

R-Q

FROM TO

-1129.6-1129.6

-1051.9-1051.9

-1051.9-1051.9

-1051.9-1051.9

-1129.6-1129.6

-1129.6-1129.6

-1129.6-1129.6

LENGTH

10.00

10.00

10.00

10.00

10.00

10.00

10.00

FACTORED CONCENTRATED LOADS (LBS)

LOC.

LC1

MAX.

MAX.

FACE

DIR.

TYPE

Y

3-0-12

-1314

-1471

FRONT

VERT

DEAD

Y

3-0-12

-184

-581

FRONT

VERT

LIVE

Y

3-0-12

-2378

-2378

FRONT

VERT

SNOW

ALLOWABLE DEFL.(L) = L/360 (0.91")

CALCULATED VERT. DEFL.(L) = L/989 (0.20")

ALLOWABLE DEFL.(TL) = L/380 (0.91")

CALCULATED VERT. DEFL.(TL) = L/888 (0.38")

CS1: TC=0.70/1.00 (F-G:1), EC=0.87/1.00 (Y-Z:1), WS=0.51/1.00 (D-E:1), SS=0.74/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1867 822 2284 1858

MM16 478 276 2341 1245 4454 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 3.0 Deg.

JSI GRIP = 0.90 (R) (INPUT = 0.80)

JSI METAL = 0.93 (Z) (INPUT = 1.00)

PLATES (table is in inches)

JT TYPE

PLATES

W

LEN

Y

X

A

TMVW-I

MT20

7.0

12.0

Edge

8.75

B

TMVWV+p

MT20

8.0

9.0

3.00

3.25

C

TMVW-I

MT20

7.0

12.0

2.00

4.75

D

TTW+h

MT20

6.0

9.0

Edge

E

TMVWVWV-I

MT20

7.0

16.0

3.25

7.50

F

TMVWVW-I

MT20

6.0

12.0

2.75

5.00

G

TMVW-w

MT20

2.0

4.0

2.50

1.00

H

TMVW-I

MT20

3.0

6.0

I

TMVW-w

MT20

2.0

4.0

2.50

1.00

J

TS-I

MT20

5.0

8.0

K

TMVWVW-I

MT20

6.0

12.0

2.75

4.50

L

TTW+h

MT20

6.0

9.0

Edge

M

TMVWVWV-I

MT20

7.0

16.0

3.25

7.50

N

TMVWVW+p

MT20

8.0

9.0

3.00

3.25

O

TMVW-I

MT20

7.0

12.0

2.00

4.75

P

TMVW-I

MT20

7.0

12.0

Edge

8.75

Q

BMV1+p

MT20

7.0

8.0

5.60

Edge

R

BMVW-I

MT20

6.0

12.0

2.50

4.50

S

BMVWV+I

MT20

8.0

9.0

T

BMVWV+I

MT20

5.0

8.0

U

BMVWVW-I

MT20

8.0

9.0

3.00

2.75

V

BS-I

MM16

5.0

12.5

W

BMVWVW-I

MT20

6.0

9.0

3.00

3.75

X

BMVWV+I

MT20

5.0

6.0

Y

BMVWV+I

MT20

6.0

9.0

Z

BMVW-I

MT20

8.0

12.0

2.50

4.50

AA

BMV1+p

MT20

7.0

8.0

5.60

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3883.3 lbs FACTORED DOWN AT 3-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER

S KATSOULAKOS

PROVINCE OF ONTARIO

DRWG NO. TAM2604618

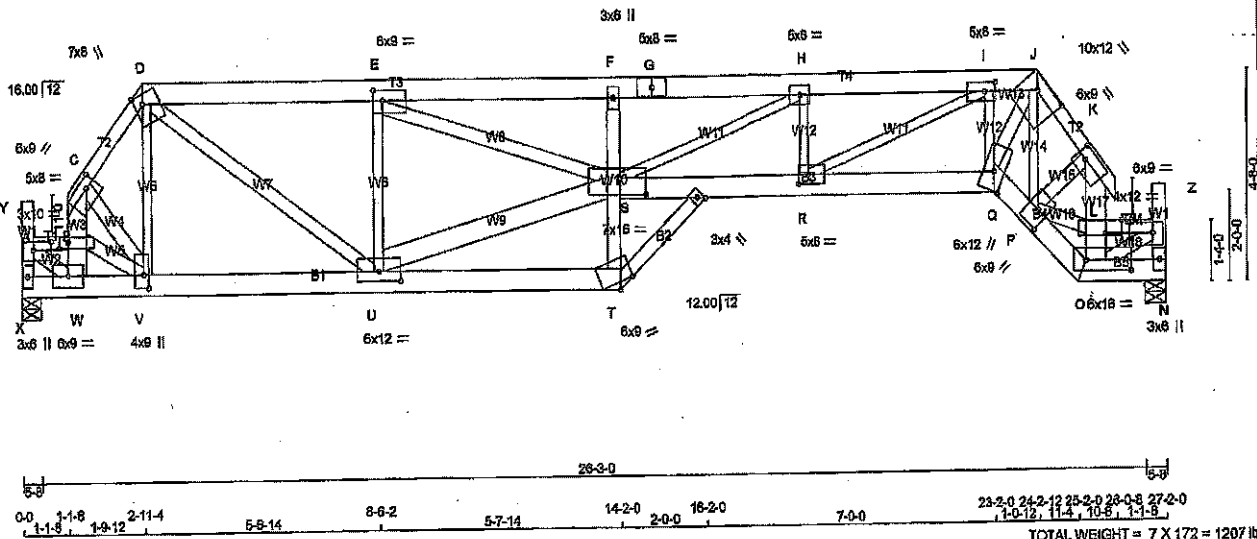
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T38S	7	1	TRUSS DESC.	

Tennessack Roof Truss, Burlington

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ID:RCuZMLjMMNFG6uqezFSzD767-8cBnGIXB4GwXZTIKDBP7pUlsrB4nG5CkpwdzC1BD

0-0 1-1-8 1-9-12 2-11-4 5-6-14 8-6-2 5-7-14 14-2-0 2-0-0 16-2-0 7-0-0 23-2-0 24-2-12 25-4-4 26-0-27-2-0 1-0-12 1-1-8 8-4 1-1-8 Scale = 1/48.0



LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR
CHORDS					
X - Y	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
G - J	2x6	DRY	No.2	SPF	
J - K	2x4	DRY	No.2	SPF	
O - K	2x6	DRY	No.2	SPF	
L - M	2x4	DRY	No.2	SPF	
N - Z	2x4	DRY	No.2	SPF	
W - C	2x6	DRY	No.2	SPF	
X - T	2x6	DRY	No.2	SPF	
S - Q	2x6	DRY	2100F 1.6E	SPF	
Q - O	2x6	DRY	No.2	SPF	
O - N	2x6	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
V - D	2x3	DRY	No.2	SPF	
U - E	2x3	DRY	No.2	SPF	
Q - I	2x3	DRY	No.2	SPF	
P - J	2x3	DRY	No.2	SPF	
U - S	2x6	DRY	No.2	SPF	
R - H	2x3	DRY	No.2	SPF	
S - I	2x6	DRY	No.2	SPF	
R - I	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2111	2111	0	5-8
N VERT	2111	2111	0	5-8
JT HORZ	0	0	0	5-8
N HORZ	0	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT COMBINED	1697	814 / 0	285 / 0	PERM LIVE WIND DEAD SOIL
N COMBINED	1697	814 / 0	285 / 0	0 / 0 586 / 0 0 / 0 586 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LIVE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LIVE (LBS)	MAX. LIVE (LBS)
FR-TO		FROM TO		FR-TO			
X-A	-1771 / 0	0.0	0.0 0.24 (1)	5-8	V-D	0 / 179	0.06 (3)
A-Y	0 / 0	0.0	0.0 0.00 (1)	10.00	D-U	0 / 2128	0.46 (1)
A-B	-2380 / 0	-127.4	-127.4 0.15 (1)	3.83	U-E	-2188 / 0	0.82 (1)
C-D	-2228 / 0	-127.4	-127.4 0.17 (1)	3.93	T-S	0 / 112	0.28 (1)
D-E	-3014 / 0	-127.4	-127.4 0.50 (1)	3.81	S-F	-528 / 0	0.28 (1)
E-F	-8416 / 0	-127.4	-127.4 0.84 (1)	2.28	Q-I	-1468 / 0	0.31 (1)
F-G	-6480 / 0	-127.4	-127.4 0.68 (1)	2.51	C-M	0 / 4414	0.95 (1)
G-H	-6480 / 0	-127.4	-127.4 0.68 (1)	2.51	P-J	-861 / 0	0.32 (1)
H-I	-5815 / 0	-127.4	-127.4 0.50 (1)	2.90	Q-J	0 / 3594	0.77 (1)
I-J	-3372 / 0	-127.4	-127.4 0.19 (1)	3.89	U-S	0 / 1869	0.50 (1)
J-K	-3087 / 0	-127.4	-127.4 0.23 (1)	3.34	R-H	-1035 / 0	0.21 (1)
O-L	-4461 / 0	0.0	0.0 0.57 (1)	3.75	S-H	0 / 992	0.29 (1)
L-K	-3886 / 0	0.0	0.0 0.55 (1)	4.21	R-I	0 / 2418	0.73 (1)
L-M	-3943 / 0	-127.4	-127.4 0.35 (1)	2.83	E-S	0 / 3598	0.77 (1)
N-M	-2084 / 0	0.0	0.0 0.29 (1)	5.17	P-L	-1767 / 0	0.24 (1)
M-Z	0 / 0	0.0	0.0 0.00 (1)	10.00	P-K	0 / 1867	0.40 (1)
W-B	-2286 / 0	0.0	0.0 0.58 (1)	5.05	B-V	-1420 / 0	0.20 (1)
B-C	-2984 / 0	0.0	0.0 0.81 (1)	4.45	A-W	0 / 2916	0.82 (1)
					C-V	0 / 1338	0.29 (1)
X-W	0 / 0	-28.0	-28.0 0.39 (1)	10.00			
W-V	0 / 1744	-28.0	-28.0 0.66 (1)	10.00			
V-U	0 / 1331	-28.0	-28.0 0.31 (3)	10.00			
U-T	0 / 174	-28.0	-28.0 0.17 (3)	10.00			
S-R	0 / 5815	-28.0	-28.0 0.43 (1)	10.00			
R-Q	0 / 3445	-28.0	-28.0 0.30 (1)	10.00			
Q-P	0 / 2360	-28.0	-28.0 0.45 (1)	10.00			
P-O	0 / 2872	-28.0	-28.0 0.64 (1)	10.00			
O-N	0 / 0	-28.0	-28.0 0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 30.0 PSF
DL = 15.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00H/2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.81")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (0.81")
CALCULATED VERT. DEFL.(TL) = L/588 (0.56")

CSL TC=0.84/1.00 (E-F:1), BC=0.68/1.00 (V-W:1),
WB=0.95/1.00 (M-C:1), SS=0.51/1.00 (B-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.90 (S) (INPUT = 0.80)
METAL = 0.96 (E) (INPUT = 1.00)

PLATES (table is in inches)

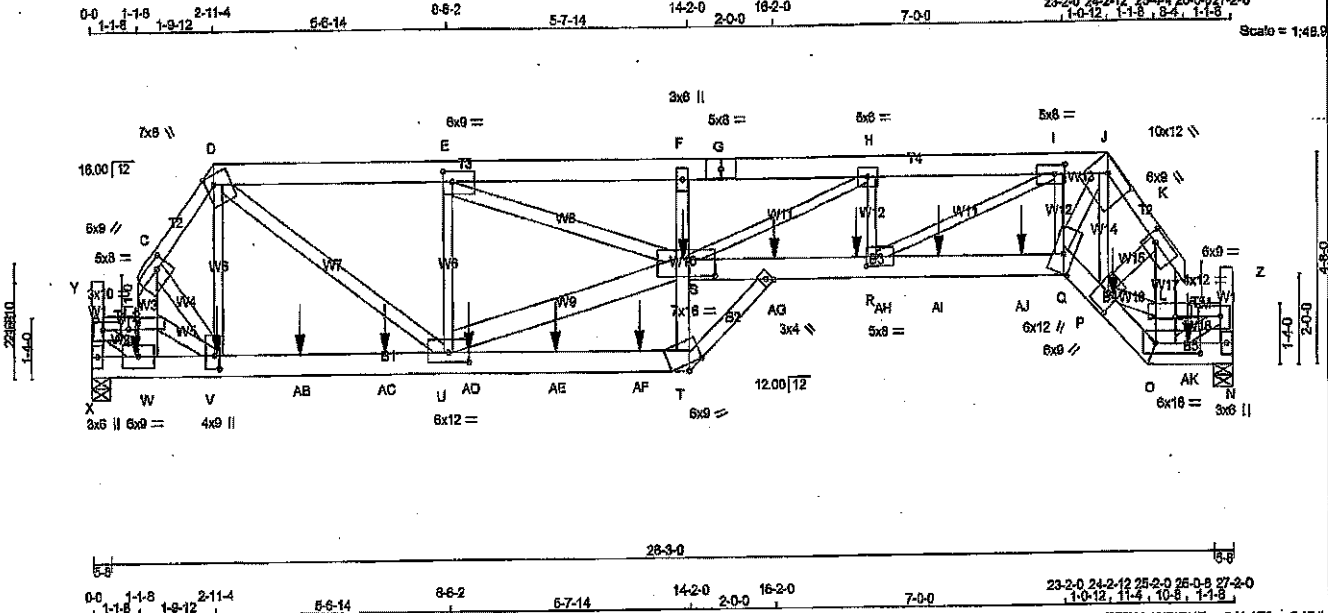
JT TYPE	PLATES	W	LEN	Y	X
A TVMW-I	MT20	5.0	8.0	2.25	5.00
B TVMW-I	MT20	3.0	10.0	1.50	2.50
C TVMW-I	MT20	6.0	9.0	2.25	3.00
D TTVMW-m	MT20	7.0	8.0	2.50	2.25
E TVMW-I	MT20	6.0	8.0	2.75	2.50
F TVMW-v	MT20	3.0	8.0		
G TS-I	MT20	5.0	8.0		
H TVMW-I	MT20	5.0	6.0		
I TVMW-I	MT20	5.0	8.0	2.50	3.00
J TTVMW-h	MT20	10.0	12.0	3.00	7.50
K TVMW-I	MT20	6.0	9.0	2.75	2.50
L TVMW-I	MT20	4.0	12.0	Edge	4.50
M TVMW-I	MT20	6.0	9.0	2.75	6.25
N BMV-I+p	MT20	3.0	6.0		
O BBV-I	MT20	6.0	16.0	2.75	12.50
P BMVWV-h	MT20	6.0	9.0	4.50	2.75
Q BBVW-m	MT20	6.0	12.0	5.00	3.00
R BMVW-I	MT20	6.0	8.0	2.50	3.00
S BMVWVW-I	MT20	7.0	18.0	4.50	7.25
T					
T BBV-m	MT20	6.0	9.0	Edge	
U BMVWVW-I	MT20	6.0	12.0	2.50	8.00
V BMVWVW-I	MT20	4.0	9.0	3.50	1.50
W BMVW-I	MT20	6.0	9.0		
X BMV-I+p	MT20	3.0	6.0		
AA NP-h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DRWG NO. TAM280428
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T38SZ	1	2	TRUSS DESC.	
Version 8.210 8 May 16 2018 Mitex Industries, Inc. Mon May 28 09:14:24 2018 Page 1					
ID:RCuZMLJIMNgF6qukeZF5fzD767-8cBnGIX6V4GwXZTKIDbP7pZQSSs586OG5CkpwdzC1BD					
23-2-0 24-2-12 25-4-4 26-0-927-2-0					
1-0-12, 1-1-8, 1-8-1, 1-1-8					
Scale = 1/4" = 8'-0"					



JOB NAME 292709	TRUSS NAME T38SZ	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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ID:RCU2MLIMMNgF6qukerFSzD767-c0IAT5XKsoC7Xh8fe2kyCLkArCKiZePKsUMT3zC1BC

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	6.0	8.0	2.25	6.00
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TVMW-I	MT20	6.0	8.0	2.25	3.00
D	TVMW-I	MT20	7.0	8.0	2.50	2.25
E	TVMW-I	MT20	6.0	9.0	2.75	2.50
F	TVMW-I	MT20	3.0	8.0		
G	TS-I	MT20	6.0	8.0		
H	TVMW-I	MT20	5.0	6.0		
I	TVMW-I	MT20	5.0	8.0	2.50	3.00
J	TVMW-I	MT20	10.0	12.0	3.00	7.50
K	TVMW-I	MT20	6.0	9.0	2.75	2.50
L	TVMW-I	MT20	4.0	12.0	Edge	4.50
M	TVMW-I	MT20	8.0	9.0	2.75	8.25
N	BMV1-p	MT20	3.0	8.0		
O	BSVW-I	MT20	6.0	16.0	2.75	12.60
P	BSVW-I	MT20	6.0	9.0	4.50	2.75
Q	BSVW-I	MT20	6.0	12.0	6.00	3.00
R	BSVW-I	MT20	5.0	8.0	2.50	3.00
S	BSVW-I	MT20	7.0	16.0	4.50	7.25
T						
U	BMV-m	MT20	8.0	8.0	Edge	
V	BMVW-I	MT20	6.0	12.0	2.50	6.00
W	BMVW-I	MT20	4.0	8.0	3.50	1.50
X	BMVW-I	MT20	6.0	8.0		
AA	NP-I	MT20	3.0	4.0	Edge	

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
AK-N	0/0	-28.0 -28.0	0.05 (1)				

FACTORED CONCENTRATED LOADS (LBS)

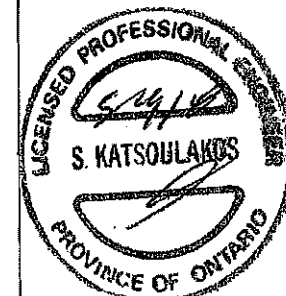
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	24-2-0	-161	-181		BACK	VERT	TOTAL
T	14-0-8	-172	-172		BACK	VERT	TOTAL
V	3-0-0	-209	-209		BACK	VERT	TOTAL
W	1-0-12	-200	-200		BACK	VERT	TOTAL
AB	5-0-0	-209	-209		BACK	VERT	TOTAL
AC	7-0-0	-209	-209		BACK	VERT	TOTAL
AD	9-0-0	-209	-209		BACK	VERT	TOTAL
AE	11-0-0	-209	-209		BACK	VERT	TOTAL
AF	13-0-0	-209	-209		BACK	VERT	TOTAL
AG	15-2-0	-161	-161		BACK	VERT	TOTAL
AH	18-2-0	-77	-77		BACK	VERT	TOTAL
AI	20-2-0	-77	-77		BACK	VERT	TOTAL
AJ	22-2-0	-77	-77		BACK	VERT	TOTAL
AK	28-1-4	-202	-202		BACK	VERT	TOTAL

JSI GRIP = 0.74 (A) (INPUT = 0.90)
JSI METAL = 0.73 (E) (INPUT = 1.00)

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 208.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 13-0-0, 172.4 lbs FACTORED DOWN AT 14-0-8, 161.2 lbs FACTORED DOWN AT 15-2-0, 161.2 lbs FACTORED DOWN AT 18-2-0, 202.6 lbs FACTORED DOWN AT 20-2-0, 77.4 lbs FACTORED DOWN AT 22-2-0, AND 77.4 lbs FACTORED DOWN AT 28-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



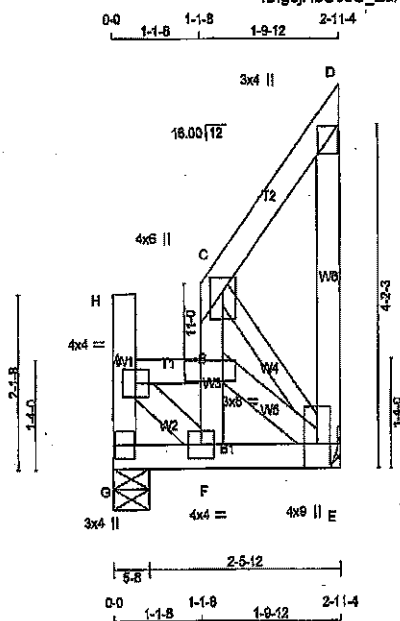
DRWG NO. TAM 28048
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T39	31	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID: g3JAbU0eO_EaFnNplqPihXzDFmC-coIAT5XKsoC7Xh8fs2qyCLsrlGwPKsUMT3zC1BC



Scale = 1:27.0

TOTAL WEIGHT = 31 X 22 = 673 lb

LUMBER	N.L. G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
G - H	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x4	DRY	No.2	SPF	
EXCEPT					
C - E	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TVMW-I	MT20	3.0	8.0	1.50	2.50
C	TVMW-w	MT20	4.0	8.0	2.00	2.00
D	TVMW-p	MT20	3.0	4.0		
E	BMVWV1+p	MT20	4.0	9.0	Edge	
F	BMVW-I	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
G	238	0	238	0
E	237	0	237	0

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E.

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM.LIVE
G	181	93/0	31/0	0/0
E	180	92/0	31/0	0/0

BEARING MATERIAL TO BE 6PF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.
		FORCE	FORCE	(PLF)	(LC)	UNBRAC	LENGTH			FORCE	FORCE	(PLF)	(LC)	UNBRAC	LENGTH
FR-TO															
G-A		-219/0	0.0	0.0	0.02 (1)	7.81		F-B		-99/5	0.01 (1)				
A-H		0/0	0.0	0.0	0.03 (1)	10.00		B-C		-107/0	0.01 (1)				
A-B		-157/0	-127.4	-127.4	0.03 (1)	6.25		A-F		0/203	0.03 (1)				
C-D		0/0	-127.4	-127.4	0.08 (1)	10.00		B-E		-191/0	0.02 (1)				
E-D		-108/0	0.0	0.0	0.03 (1)	7.81		C-E		-4/3	0.00 (3)				
G-F		0/0	-28.0	-28.0	0.01 (3)	10.00									
F-E		0/156	-28.0	-28.0	0.04 (1)	10.00									

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	62.5	PSF	

SPACING = 24.0 IN./C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.00")

CSI: TC=0.06/1.00 (C-D:1), BC=0.04/1.00 (E-F:1),
WB=0.03/1.00 (A-F:1), SH=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1657 822 2284 1855

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.21 (F) (INPUT = 0.80)
SI METAL=0.08 (A) (INPUT = 1.00)



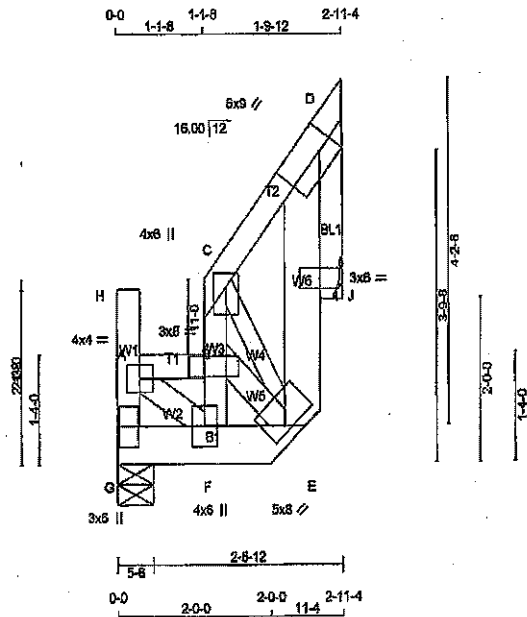
DRWG NO. TAM2009-8
STRUCTURAL
COMPONENT ONLY

JOB NAME 292709	TRUSS NAME T40	QUANTITY 5	PLY 1	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:RCuZMLJIMMNgF8quksZF8fzD787-colAT5XKsoC7Xh8f2tqyCLsBrLN6NPKsUMT3zC1BC



Scale = 1:27.0

TOTAL WEIGHT = 5 X 26 = 128 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
G - H	2x4	DRY	No.2
A - B	2x4	DRY	No.2
C - D	2x4	DRY	No.2
E - D	2x6	DRY	No.2
F - C	2x4	DRY	No.2
G - E	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x4	DRY	No.2
ALL WEBS			
EXCEPT	2x4	DRY	No.2
C - E	2x4	DRY	No.2
DRY: SEASONED LUMBER.			
BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TVMW-I	MT20	4.0	4.0
B	TVMW-I	MT20	3.0	8.0 1.50 2.50
C	TMVW-I	MT20	4.0	6.0 2.00 2.00
D	TMVW-I	MT20	6.0	9.0 Edge 2.50
E	BMVW-W	MT20	6.0	8.0 1.00 2.75
F	BMVW-W	MT20	4.0	6.0
G	BMVW-I	MT20	3.0	6.0
I	KP-p	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	222	0	222	0	0	5.8	5.8
J(D)	189	0	189	0	0	MECHANICAL	

(** SEE "BEARING NOTE" **)

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J(D).

UNFACTORED REACTIONS

1ST LOOSE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	178	86 / 0	30 / 0	0 / 0	0 / 0	53 / 0
J(D)	152	73 / 0	26 / 0	0 / 0	0 / 0	54 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED				WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO		FROM TO		LENGTH		FR-TO			
G-A	-184 / 0	0.0	0.0	0.02 (1)	7.81	B-E	-143 / 0	0.01 (1)	
A-H	0 / 0	0.0	0.0	0.00 (1)	10.00	C-E	0 / 4	0.00 (1)	
A-B	-137 / 0	-127.4	-127.4	0.03 (1)	6.25	A-F	0 / 171	0.03 (1)	
C-D	-19 / 0	-127.4	-127.4	0.03 (1)	6.25	D-J	-190 / 0	0.02 (1)	
E-I	0 / 117	0.0	0.0	0.03 (1)	10.00	I-J	0 / 21	0.00 (1)	
I-D	0 / 117	0.0	0.0	0.03 (1)	10.00				
F-B	-105 / 0	0.0	0.0	0.01 (1)	7.81				
B-C	-108 / 0	0.0	0.0	0.01 (1)	7.81				
G-F	0 / 0	-28.0	-28.0	0.02 (1)	10.00				
F-E	0 / 127	-28.0	-28.0	0.03 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	30.0	PSF
DL	=	15.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2012, CBC 2012, ABC 2014
- CSA 088-09
- TFC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(58 % OF 89.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/699 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/699 (0.00")

CSI: TO=0.03/1.00 (C-D:1), BC=0.03/1.00 (E-F:1),
WB=0.03/1.00 (A-F:1), SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.60

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1887 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

DRWG NO. TAM 28050-18
STRUCTURAL
COMPONENT ONLY



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ID:RCuZMLUMMNcF8aukeZFsfzD767-colAT6XKscC7Xh8fs2lqyCLsBrLNUNPKsUMT3zc1BC



TOTAL WEIGHT = 27 lb

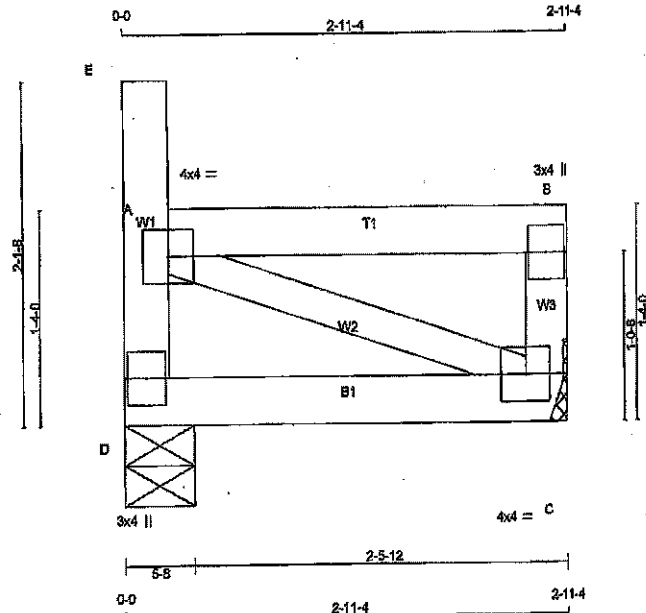
DWG NO. TAM 20051-18
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T41	10	1	TRUSS DESC.	

Tangerack Roof Truss, Burlington

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ID:g3jAbU0eO_EaFnNpIqPlhXzDFmC-colAT6XKscC7Xh8fs2lqyCLpmrk5loPKsUMT3zC1BC



Scale = 1:13.7

TOTAL WEIGHT = 10 X 11 = 113 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
D - E	2x4	DRY	No.2	SPF		
A - B	2x4	DRY	No.2	SPF		
C - B	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW4	MT20	4.0	4.0			
B	TVMW4	MT20	3.0	4.0			
C	BMW4-1	MT20	4.0	4.0			
D	BMW1-1	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
D	228 0	228 0	0	5-8
C	228 0	228 0	0	MECHANICAL

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
D	184	88/0	31/0	0/0	0/0	65/0	0/0	0/0
C	184	88/0	31/0	0/0	0/0	65/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO		FR-TO				
D-A	-187/0	0.0	0.0	0.03 (1)	7.81	A-C	0/0	0.00 (1)
A-E	0/0	0.0	0.0	0.00 (1)	10.00			
A-B	0/0	-127.4	-127.4	0.25 (1)	10.00			
C-B	-187/0	0.0	0.0	0.03 (1)	7.81			
D-C	0/0	-28.0	-28.0	0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 30.0 PSF
DL = 15.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, SBC 2012, ABC 2014
- CSA 086-08
- TRIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.10")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.10")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.01")

CSI: TC=0.25/1.00 (A-B:1), BC=0.11/1.00 (C-D:3),
WB=0.00/1.00 (A-C:1), SS=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 822 2284 1656

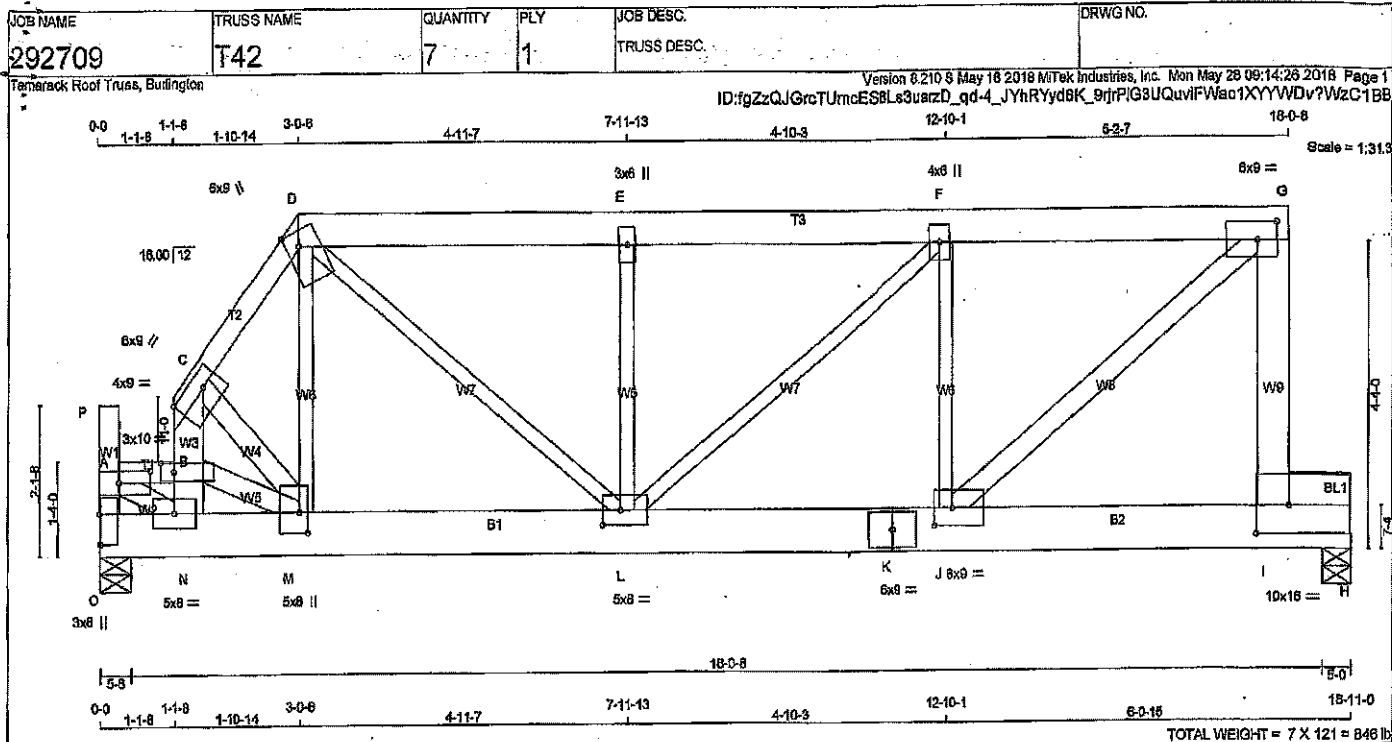
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (A) (INPUT = 0.60)
JSI METAL= 0.09 (A) (INPUT = 1.00)



DWG NO. T41-2052-18
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
O - P	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - G	2x6	DRY	No.2	SPF	
N - C	2x6	DRY	No.2	SPF	
O - K	2x6	DRY	1950F 1.7E	SPF	
K - H	2x6	DRY	1950F 1.7E	SPF	

BEARING BLOCKS	BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	J - G	2x6	DRY	No.2	SPF
	B - M	2x4	DRY	No.2	SPF
	C - N	2x4	DRY	No.2	SPF
	A - N	2x4	DRY	No.2	SPF
	J - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	4.0	9.0	2.00	6.50
B	TMVW-I	MT20	3.0	10.0	1.50	2.60
C	TMVW-I	MT20	6.0	9.0	Edge	5.75
D	TMVW+m	MT20	6.0	9.0	2.60	2.25
E	TMVW-w	MT20	3.0	6.0		
F	TMVW-w	MT20	4.0	6.0		
G	TMVW-I	MT20	6.0	9.0	3.00	3.50
I	BMVW-I	MT20	10.0	16.0	4.75	5.75
J	BMVW-I	MT20	6.0	9.0	3.00	3.50
K	BS-I	MT20	6.0	9.0		
L	BMVW-w	MT20	5.0	6.0	2.50	3.25
M	BMVW-w	MT20	5.0	6.0	3.50	1.50
N	BMVW-I	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	6.0	5.25	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	1995	0	1995	0	5-6	5-6
H	2107	0	2107	0	5-0	5-0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1506	881/0	199/0	0/0	0/0	417/0	0/0
H	1606	986/0	199/0	0/0	0/0	423/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
O-A	-1288/0	0.0	0.0 0.19 (1)	I-G	-2418/0	0.43 (1)	
A-P	0/0	0.0	0.0 0.00 (1)	M-D	0/279	0.08 (3)	
A-B	-1834/0	-127.4	-127.4 0.13 (1)	B-M	-864/0	0.15 (1)	
C-D	-2119/0	-127.4	-127.4 0.19 (1)	C-M	0/1331	0.31 (1)	
D-E	-2323/0	-200.3	-200.3 0.49 (1)	A-N	0/2192	0.52 (1)	
E-F	-2323/0	-200.3	-200.3 0.52 (1)	J-G	0/3128	0.74 (1)	
F-G	-2323/0	-200.3	-200.3 0.53 (1)	D-L	0/1399	0.48 (1)	
N-B	-2498/0	0.0	0.0 0.54 (1)	J-F	-1135/0	0.44 (1)	
B-C	-2845/0	0.0	0.0 0.68 (1)	L-E	-1048/0	0.41 (1)	
				L-F	0/44	0.01 (1)	
O-N	0/0	-28.0	-28.0 0.29 (1)				
N-M	0/1274	-28.0	-28.0 0.37 (1)				
M-L	0/1281	-28.0	-28.0 0.17 (1)				
L-K	0/2282	-28.0	-28.0 0.30 (1)				
K-J	0/2282	-28.0	-28.0 0.30 (1)				
J-I	-130/0	-28.0	-28.0 0.74 (1)				
I-H	0/0	-155.4	-155.4 0.40 (1)				

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	62.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBCG 2012, ABC 2014
- CSA 088-09
- TPIC 2011

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/380 (0.63")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.10")
ALLOWABLE DEFL.(TL) = 1/380 (0.63")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.17")

CSL TO=0.58/1.00 (B-C:1), 8C=0.74/1.00 (I-J:1),
WB=0.74/1.00 (G-J:1), SSI=0.94/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1697 622 2284 1658

PLATE PLACEMENT TOL = 0.250 inches

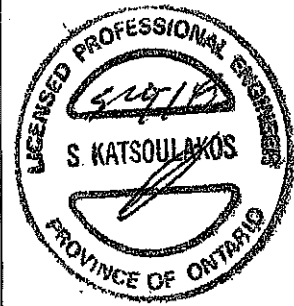
PLATE ROTATION TOL = 5.0 Deg.



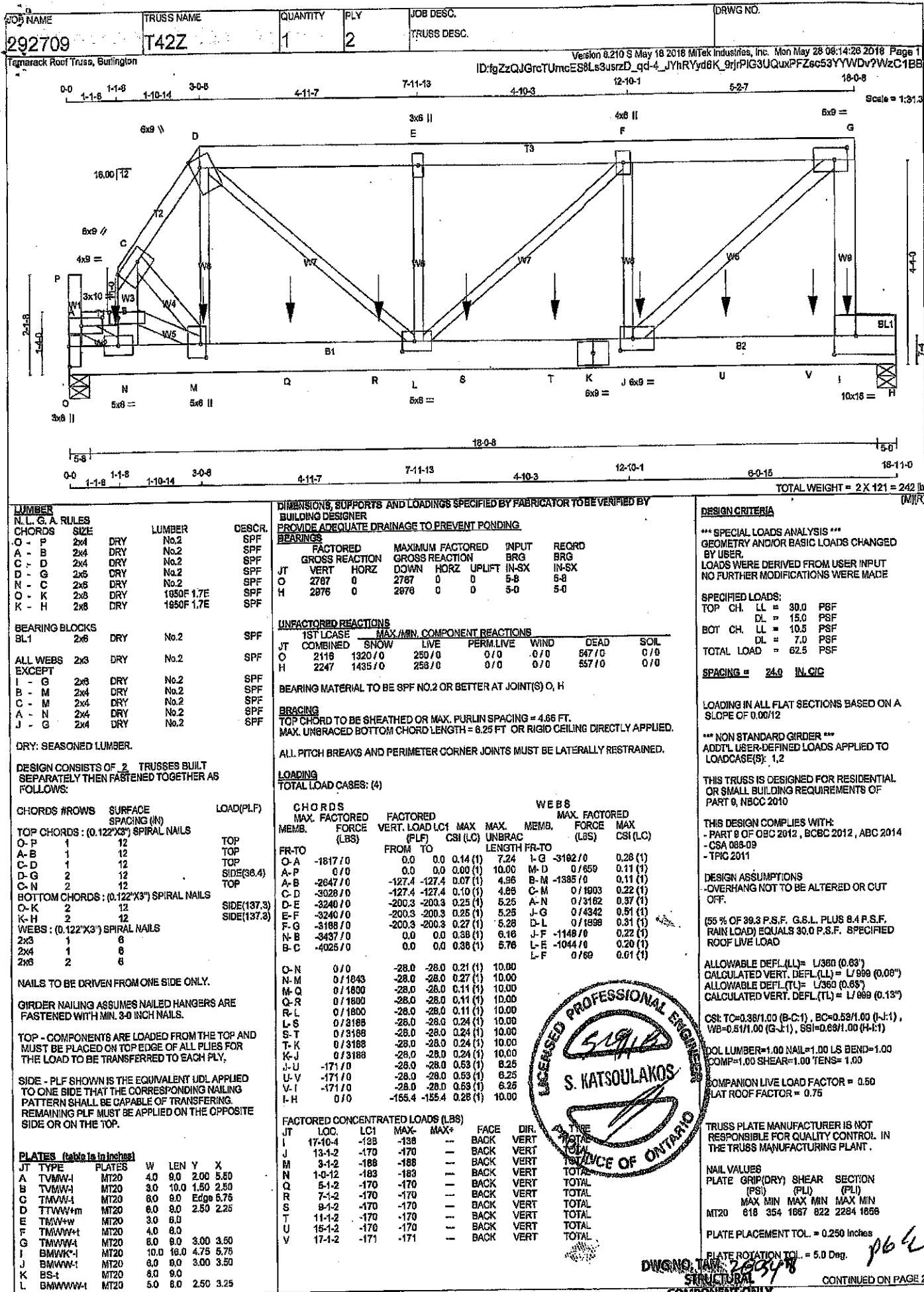
DWG NO. TAM 2605318
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T42	7	1	TRUSS DESC.	
Version 6.210 \$ May 18 2018 MITek Industries, Inc. Mon May 28 09:14:26 2018 Page 2 Tapered Roof Truss, Burlington ID:fgZzQJGrcTUmceS8Ls3usrzD_qd-4 JYhRYyd8K 8nrfIG8UQuvFWac1XYVWDv?VWzC1B8					
JSI GRIP= 0.89 (M) (INPUT = 0.80) JSI METAL= 0.62 (K) (INPUT = 1.00)					



DWG NO. TAM 2805318
 STRUCTURAL
 COMPONENT ONLY



JOB NAME 292709	TRUSS NAME T42Z	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.210 5 May 18 2018 Mitak Industries, Inc. Mon May 28 09:14:28 2018 Page 2
ID:fpZzQJGrcTUmcE88Ls3usrzD qd-4 JYhRYvd6K 9ntrPIG3UQuXPFZsc53YYWDv7WzC1BB

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMVWW+H	MT20	5.0	8.0	3.50	1.50
N	BMVWH	MT20	5.0	5.0		
O	BMV1+p	MT20	3.0	8.0	5.25	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S)
REQUIRED TO SUPPORT CONCENTRATED
LOAD(S) 183.3 lbs FACTORED DOWN AT 1-0-12,
187.6 lbs FACTORED DOWN AT 3-1-2, 170.0 lbs
FACTORED DOWN AT 5-1-2, 170.0 lbs
FACTORED DOWN AT 7-1-2, 170.0 lbs
FACTORED DOWN AT 9-1-2, 170.0 lbs
FACTORED DOWN AT 11-1-2, 170.0 lbs
FACTORED DOWN AT 13-1-2, 170.0 lbs
FACTORED DOWN AT 15-1-2, AND 171.0 lbs
FACTORED DOWN AT 17-1-2, AND 138.5 lbs
FACTORED DOWN AT 17-10-4 ON BOTTOM
CHORD. DESIGN FOR UNSPECIFIED
CONNECTION(S) IS DELEGATED TO THE
BUILDING DESIGNER.

JSI GRIP= 0.64 (M) (INPUT = 0.90)
JSI METAL= 0.48 (K) (INPUT = 1.00)



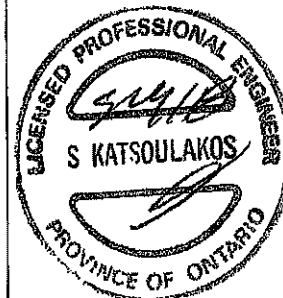
DWG NO. TAM 28054-8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T43	5	1	TRUSS DESC.	

Tenarack Roof Truss, Burlington

Version 0.210 S May 16 2018 Mitek Industries, Inc. Mon May 28 09:14:27 2018 Page 2
 ID:fgZzQJGrTUmcES8Ls3uazrD od-ZBtwunZbOQSm71zTnl1dR2LfaYKUCinAzTXyzC1BA

JSI GRIP= 0.90 (J) (INPUT = 0.90)
 JSI METAL= 0.87 (J) (INPUT = 1.00)



DWG NO. TAM 28055-18
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 292709	TRUSS NAME T43Z	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Version 8.210 S May 10 2016 Mitek Industries, Inc. Mon May 28 08:14:27 2018 Page 2
ID:fgZzQJGrcTUmcES8Ls3uerzD_gd-ZBtwunZbQQSm7ltzTnl1dR44fu3KKknAztXyzC1BA

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TTWW+m	MT20	7.0	8.0	2.50	2.25
H	TMVW-I	MT20	8.0	9.0	2.25	3.00
I	TMVW-I	MT20	3.0	10.0	1.50	2.50
J	TMVW-I	MT20	5.0	8.0	2.25	5.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-I	MT20	7.0	8.0		
M	BMVWWH	MT20	4.0	9.0	3.75	1.50
N	BMVWWH	MT20	6.0	8.0	2.75	1.50
O	BS-I	MT20	5.0	6.0		
P	BMVWW-I	MT20	6.0	9.0	3.00	3.50
Q	BMVWWH	MT20	4.0	9.0	3.75	1.50
R	BMVW-I	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 207.5 lbs FACTORED DOWN AT 1-0-12, 207.5 lbs FACTORED DOWN AT 3-1-2, 207.5 lbs FACTORED DOWN AT 5-1-2, 207.5 lbs FACTORED DOWN AT 7-1-2, 207.5 lbs FACTORED DOWN AT 9-1-2, 207.5 lbs FACTORED DOWN AT 11-10-14, 207.5 lbs FACTORED DOWN AT 13-10-14, 207.5 lbs FACTORED DOWN AT 15-10-14, AND 207.5 lbs FACTORED DOWN AT 17-10-14, AND 207.5 lbs FACTORED DOWN AT 19-11-14 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

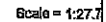
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	7-1-2	-208	-208	---	FRONT	VERT	TOTAL
X	9-1-2	-208	-208	---	FRONT	VERT	TOTAL
Y	10-8-0	-208	-208	---	FRONT	VERT	TOTAL
Z	13-10-14	-208	-208	---	FRONT	VERT	TOTAL
AA	15-10-14	-208	-208	---	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)



DRWG NO. TAM 2805678
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 1B X 22 = 401 lb

PLATES (table is in inches)		W	LEN	Y	X
JT	TYPE	PLATES			
A	TVMW-I	MT20	4.0	4.0	
B	TVMW+w	MT20	5.0	5.0	1.00 2.50
C	TVMW+p	MT20	4.0	6.0	2.00 2.00
D	TMV+p	MT20	3.0	4.0	
E	BMVWW-I+p	MT20	4.0	6.0	2.00 2.00
F	BMVW-I	MT20	4.0	4.0	
G	BMV-I+p	MT20	3.0	4.0	

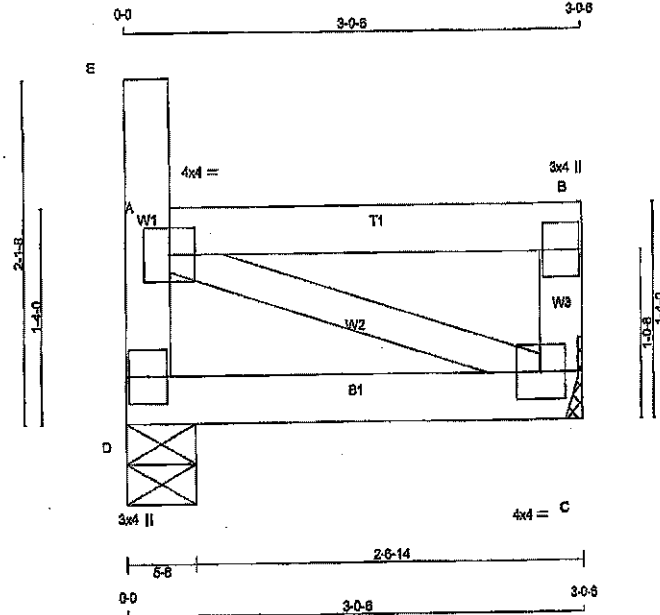
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO			FR-TO		
G-A	-217.0	0.0	0.0	0.02 (1)	A-F	0.214	0.03 (1)
A-H	0.0	0.0	0.0	0.00 (1)	B-E	-168.0	0.02 (1)
A-B	-188.0	-127.4	-127.4	0.03 (1)	C-E	-17.0	0.00 (1)
C-D	0.0	-127.4	-127.4	0.07 (1)			
E-D	-112.0	0.0	0.0	0.04 (1)			
F-B	-95.3	0.0	0.0	0.01 (1)			
B-C	-100.0	0.0	0.0	0.01 (1)			
G-F	0.0	-28.0	-28.0	0.01 (3)			
F-E	0.155	-28.0	-28.0	0.04 (3)			



DWG NO. TAM-20057-18
STRUCTURAL
COMPONENT ONLY

Tall-srack Roof Truss, Burlington

Scale = 1:13.7



TOTAL WEIGHT = 6 X 12 = 70 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
D - E	2x4	DRY	No.2	SPF		
A - B	2x4	DRY	No.2	SPF		
C - B	2x4	DRY	No.2	SPF		
D - C	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	JT VERT	IN-SX	IN-SX
D 238 0	238 0	5-8	5-8
C 238 0	238 0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOL.
JT COMBINED	SNOW	LIVE	PERM. LIVE	
D 189	91/0	32/0	0/0	0/0
C 189	91/0	32/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	WEBS
MAX. FACTORED	MAX. FACTORED
MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
FR-TO	FR-TO
D-A	-193/0
A-E	0/0
A-B	0/0
C-B	-193/0
D-C	0/0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 30.0 PSF
DL	= 15.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/380 (0.19")
 CALCULATED VERT. DEFL. (LL) = 1/899 (0.01")
 ALLOWABLE DEFL. (TL) = 1/380 (0.19")
 CALCULATED VERT. DEFL. (TL) = 1/899 (0.01")

CSI: TO=0.27/1.00 (A-B:1), BC=0.12/1.00 (C-D:3),
 WB=0.00/1.00 (A-C:1), 8SI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50
 FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PS) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1887 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.35 (A) (INPUT = 0.90)
 JSI METAL = 0.09 (A) (INPUT = 1.00)



DWG NO. TAM 280428
 STRUCTURAL
 COMPONENT ONLY



Double 1-3/4" x 16" VERSA-LAM® 2.0 2800 DF

Floor Beam\B1

March 5, 2018 07:53:45

Dry | 1 span | No cantilevers | 0/12 slope (deg)

BC CALC® Design Report



Build 6536

Job Name:

GREEN YORK HOMES - TIBURTINO

Address:

City, Province, Postal Code: OAKVILLE,

Customer:

Code reports:

CCMC 12472-R

File Name: GEORGIAN 3

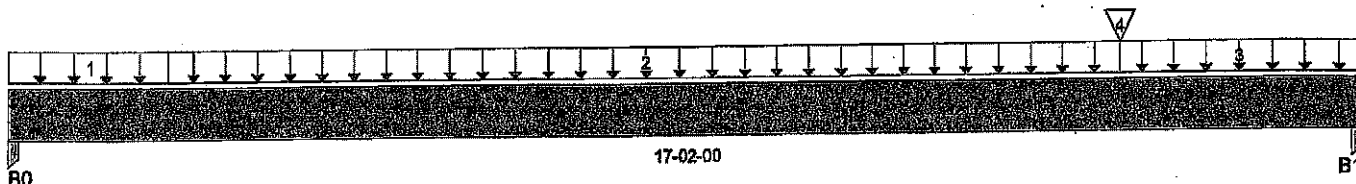
Description: Designs\B1

Specifier:

Designer: colinh

Company:

Misc:



Total Horizontal Product Length = 17-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 5-1/2"	751 / 0	1,770 / 0	2,289 / 0	
B1, 3-1/2"	1,091 / 0	2,766 / 0	3,827 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	TRUSSES1	Unf. Area (lb/ft^2)	L	00-00-00	02-00-00	11	22	30		03-11-12
2	TRUSSES2	Unf. Area (lb/ft^2)	L	02-00-00	14-01-12	11	22	30		08-01-04
3	SPAN	Unf. Area (lb/ft^2)	L	14-01-12	17-02-00	11	22	30		02-00-00
4	T15	Conc. Pt. (lbs)	L	14-01-12	14-01-12	661	1,816	2,743		n/a

Controls Summary

	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	31,252 ft-lbs	56,137 ft-lbs	55.7%	5	10-04-03
End Shear	9,462 lbs	19,488 lbs	48.6%	5	15-08-08
Total Load Defl.	L/413 (0.48")	0.827"	58.1%	13	09-00-04
Live Load Defl.	L/681 (0.292")	0.551"	52.9%	17	09-00-04
Max Defl.	0.48"	1"	48%	13	09-00-04
Span / Depth	12.4	n/a	n/a		00-00-00

Bearing Supports

	Dim. (L x W)	Demand	Demand / Resistance Support	Demand / Resistance Member	Material
B0 Post	5-1/2" x 3-1/2"	6,022 lbs	n/a	25.6%	Unspecified
B1 Post	3-1/2" x 3-1/2"	9,744 lbs	n/a	65.2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Design meets User specified (1") Maximum Total load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor : Normal Part code : Part 9



DWG NO. TAM 12135-18
STRUCTURAL
COMPONENT ONLY



Double 1-3/4" x 16" VERSA-LAM® 2.0 2800 DF

Floor Beam\B1

Dry | 1 span | No cantilevers | 0/12 slope (deg)

March 5, 2018 07:53:45

BC CALC® Design Report



Build 6536

Job Name:

GREEN YORK HOMES - TIBURTINO

Address:

City, Province, Postal Code: OAKVILLE,

Customer:

Code reports:

CCMC 12472-R

File Name: GEORGIAN 3

Description: Designs\B1

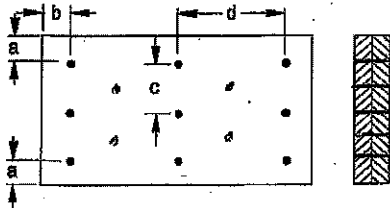
Specifier:

Designer: colinh

Company:

Misc:

Connection Diagram



5 ROWS

a minimum = 2" c = 6"
b minimum = 3" d = 6"

Calculated Side Load = 541.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d Nails

3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



DWG NO. TAM/2135-18
STRUCTURAL
COMPONENT ONLY

TECHNICAL BULLETIN

CP Crush Plates – Bearing Enhancers

SIMPSON
Strong-Tie

The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

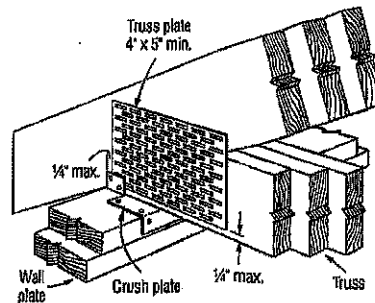
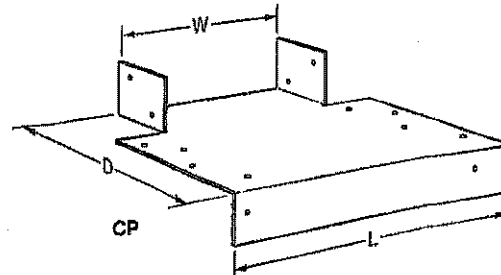
Finish: G90 galvanized

Design:

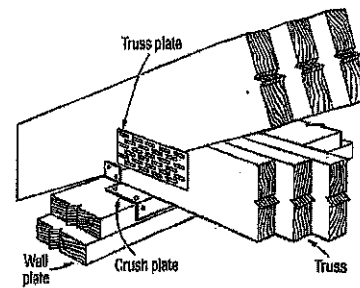
- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_p and $Q'_r/A'_p = 812$ psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.)			Fasteners		Factored Resistance (lb.)			
			W	D	L	Wall Plate	Truss	D.Fir-L		S-P-F	
								Uplift	Bearing	Uplift	Bearing
CASE 1 (Truss Plate Reinforcement)											
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515
CP2-4	16		3¼	3½	5½	(6) 10d	(4) 10d x 1½"	225	11390	225	9030
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095
CP2-6	16		3¼	5½	5½	(10) 10d	(4) 10d x 1½"	225	18740	225	14190
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390
CASE 2 (No Reinforcement)											
CP1-4	20	2x4	1½	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550
CP2-4	16		3¼	3½	5½	(6) 10d	(4) 10d x 1½"	225	9370	225	7100
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195
CP1-6	20	2x6	1½	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575
CP2-6	16		3¼	5½	5½	(10) 10d	(4) 10d x 1½"	225	14730	225	11160
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPEC-CP17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

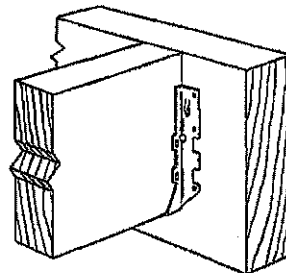
Finish: G90 galvanized

Design:

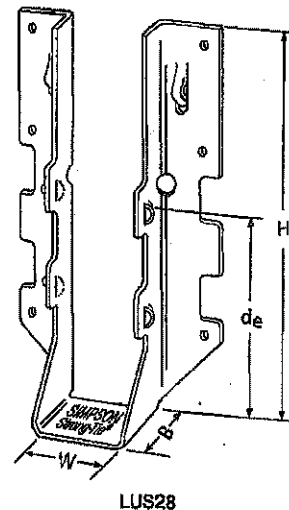
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

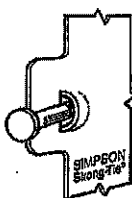


Typical LUS
Installation

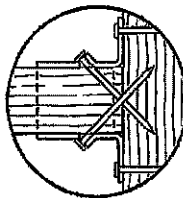


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/4	1 1/4	1 1/4	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	8 1/4	1 1/4	3 1/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	8 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/4	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

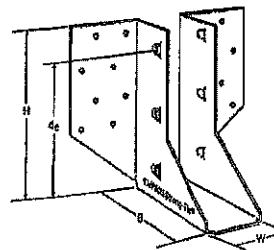
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

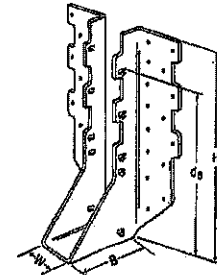
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

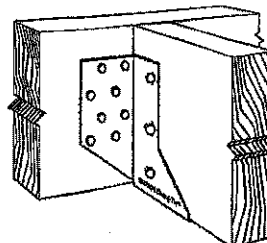
- See current catalogue for options



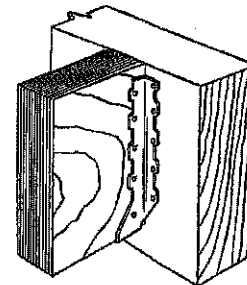
LJS26DS



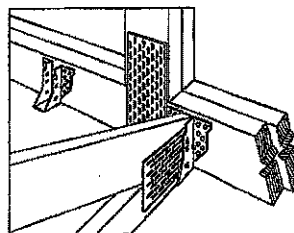
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



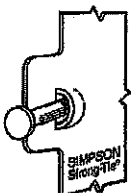
Typical HUS
Installation



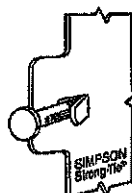
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2875	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	18	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	8450	4010	5200

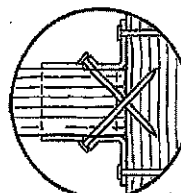
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tab
breaking off
(available on
some models).
U.S. Patent
5,803,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



**LIMIT
STATES
DESIGN**

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECHUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers



All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

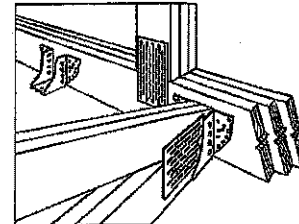
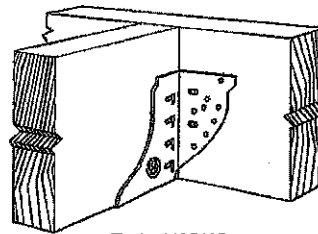
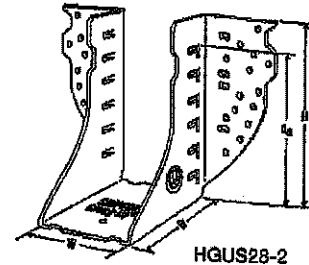
- Factored resistances are in accordance with CSA C86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

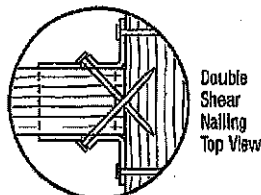
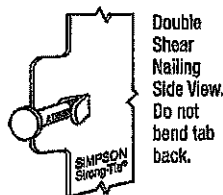
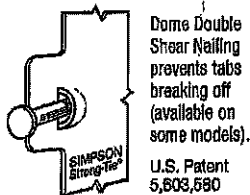
Options:

- See current catalogue for options



Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)	Uplift (K ₁ =1.15)	Normal (K ₂ =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10845
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.



This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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(800) 999-5099
strongtie.com

MICRO CITY

ENGINEERING SERVICES INC.

TEL: (610) 287-2242

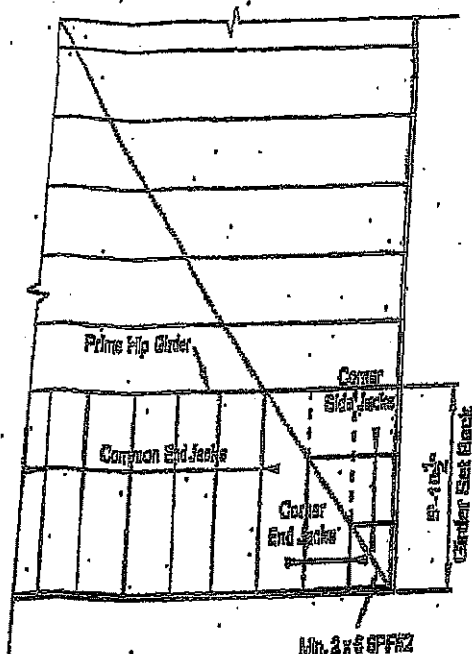
LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 1x6 SPF#2

UNLESS OTHERWISE SHOWN



45° Hip End

DESIGN LOADS:

TOP CHORD LIVE LOAD : 39.4 P.S.F.

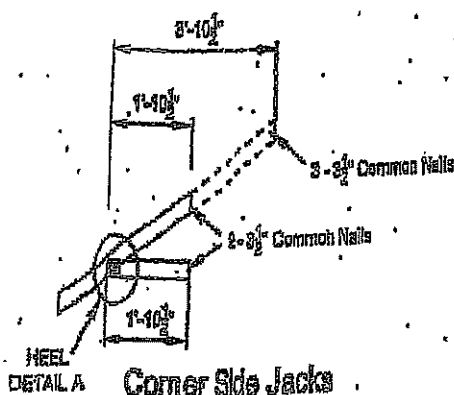
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 10.5 P.S.F.

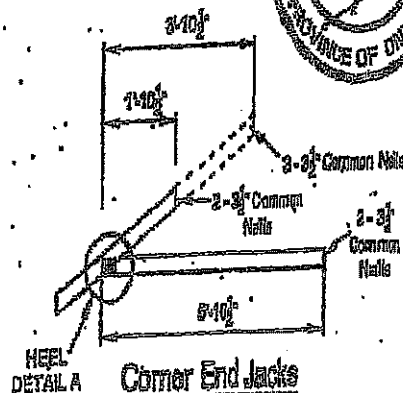
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 59.9 P.S.F.

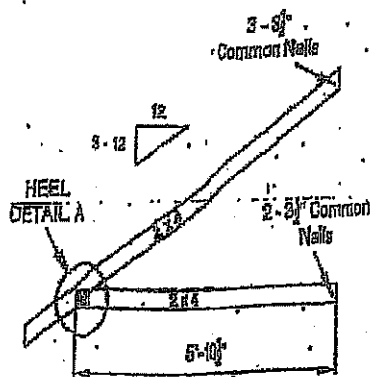
DATE: JAN 24 1995
STRUCTURAL
COMPONENT ONLY



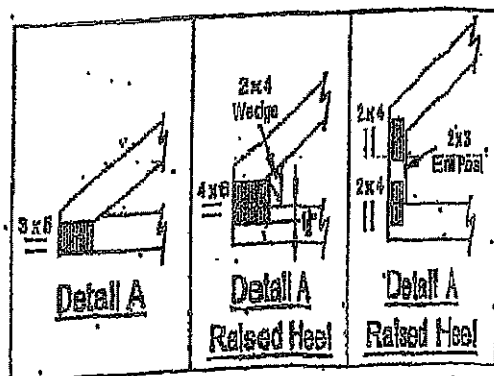
Corner Side Jacks



Corner End Jacks



Common End Jacks



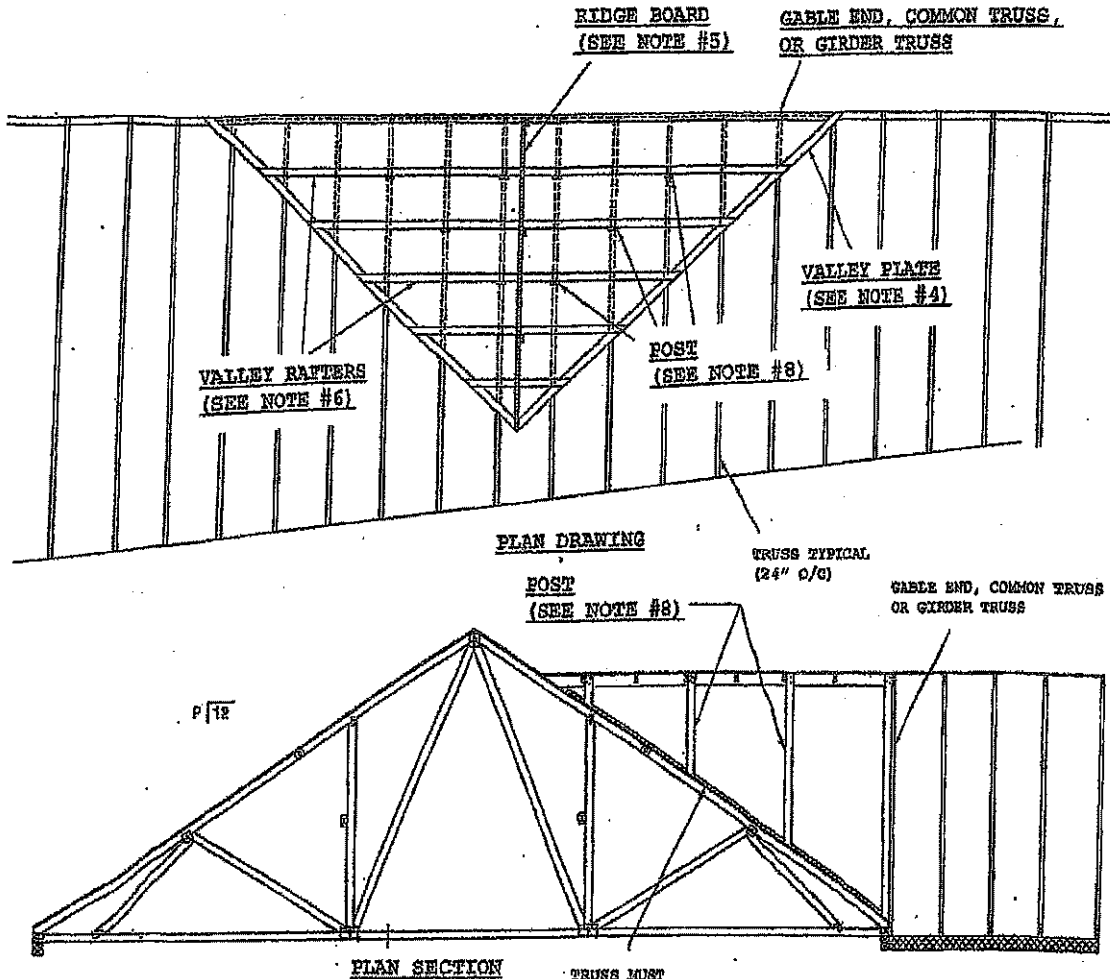
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATE DESIGN)
FOR THE TRUSS AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (A) GABLE END, (B) GIRDER TRUSS OR (C) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT, FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATES TO RIDGE BOARD, MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/8" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

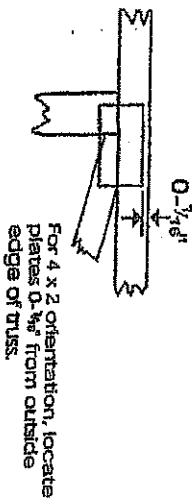
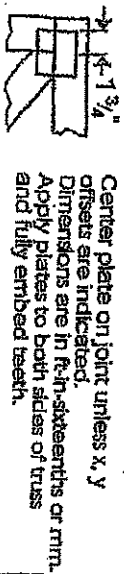
- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 3 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

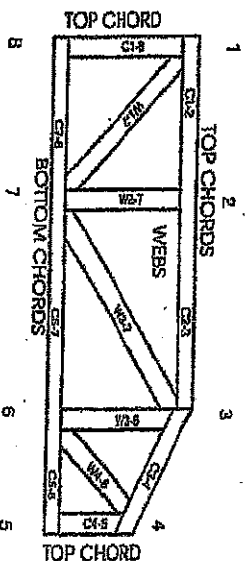


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing, Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plates
BCSI: Connected Wood Trusses

Numbering System

6-4-8 dimensions shown in feet-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.
CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11896-L, 10819-L, 13270-L, 12691-R

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Mitek Engineering Reference Sheet MIE-7472C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knees and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be safely protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 15% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pultrus provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing or less, if no ceiling is installed, unless otherwise noted.
15. Corrections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

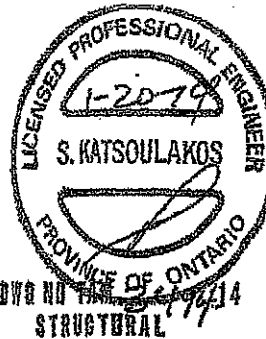
Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

RR #1, Po Box 61

Glencoe, Ontario

NOL 1M0

(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)



Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.