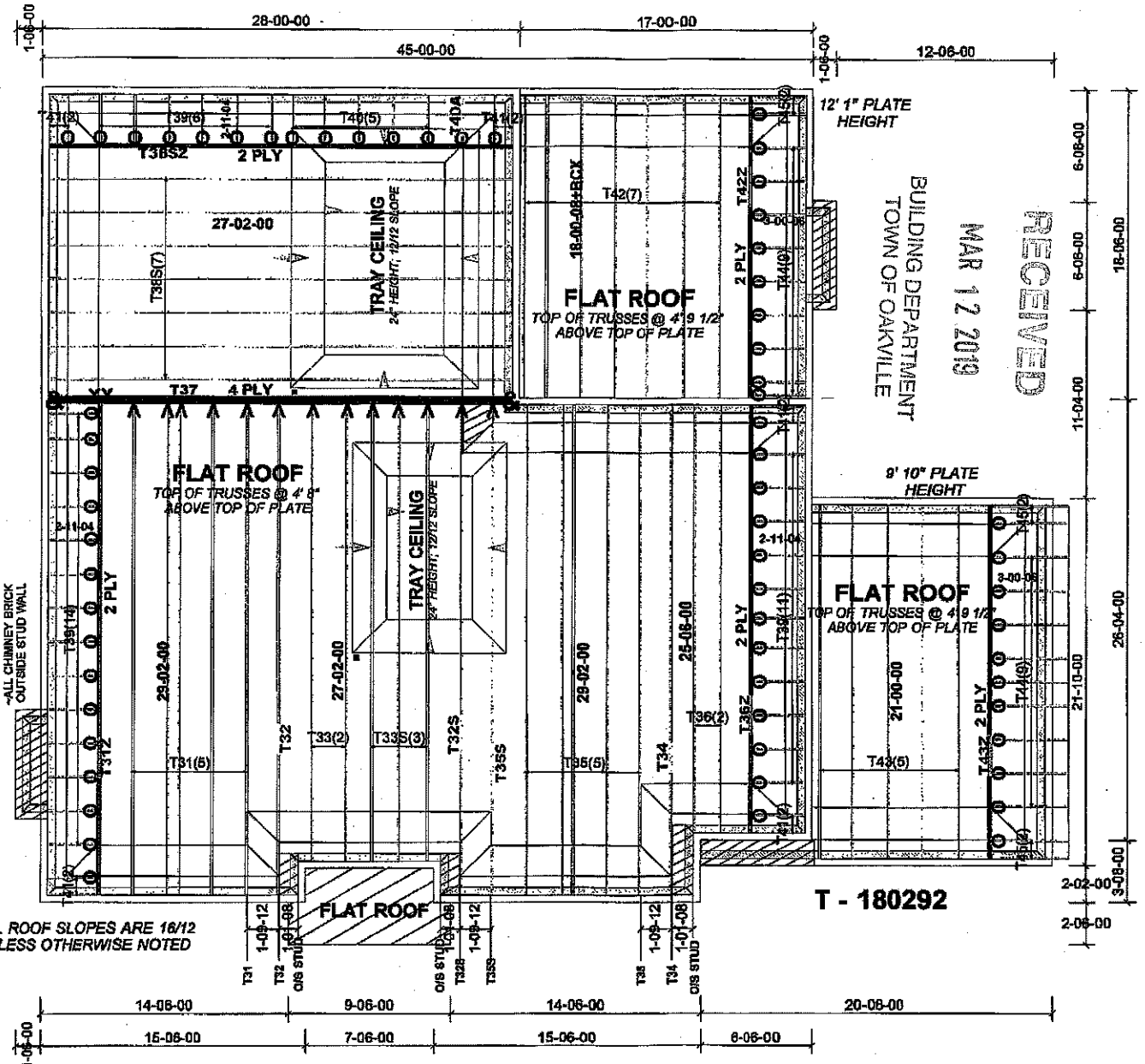
SITE COPY

19-0250



BUILDING DEPARTMENT
TOWN OF OAKVILLE

RECEIVED
MAR 12 2019

ASPHALT SHINGLES
0" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
HEEL: SEE HEEL DIAGRAM

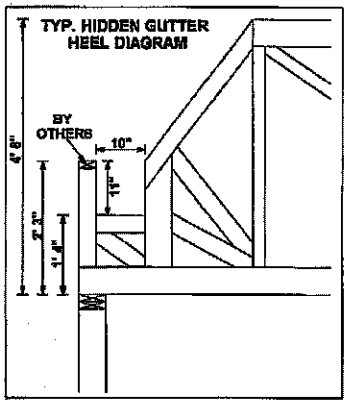
ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT.POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DENOTES: CONVENTIONAL FRAMING

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD Ss=39.3psf Sr=8.4psf
DESIGN LOADS
TCDL (15psf)
BCLL (10.5psf)
BCDL (7psf)

OK-DAS.
Mar 26/19

HARDWARE:
LUS24 - (O)
LJS26DS - (V)
LUS26-2 - (W)
HGUS26-2 - (XX)
CP4-6 - (&)



m11,470

TAMARACK LUMBER INC. ALPA LUMBER GROUP	Job Track: 45487	Builder / Location: GREEN YORK HOMES	Model / Elevation: GEORGIAN 3 / B -LOT 7
	Layout ID: 292709	Project: TIBURTINO	
	Plan Log: 95790	Date: 05/28/18 Designer: colinh	

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTLILZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

DATE	03/05/18
SALES REP	Rick

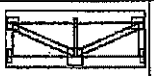
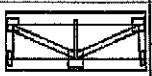
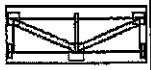
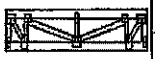
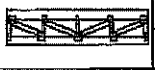
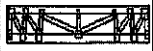
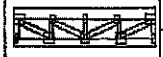
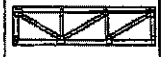
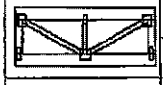
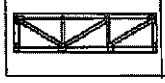
TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 45487 LAYOUT ID: 292707 LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO SUB-BUILDER:
MODEL: GEORGIAN 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T1 FLAT	0.00 / 0.00	23-02-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	788.41 / 491.19		
	1	G1 GABLE	0.00 / 0.00	23-02-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	76.18 / 50.00		
	2	T2 FLAT GIRDER	0.00 / 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	91.12 / 60.00		
	1 / 3 Ply	T2Z FLAT GIRDER	0.00 / 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	136.68 / 90.00		
	1 / 3 Ply	T2Z2 FLAT GIRDER	0.00 / 0.00	09-03-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	136.68 / 90.00		
	3	T3 FLAT	0.00 / 0.00	05-07-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	72.45 / 48.00		
	6	T4 FLAT	0.00 / 0.00	19-08-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	578.22 / 352.02		
	1	G4 GABLE	0.00 / 0.00	19-08-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	65.62 / 42.67		
	1 / 2 Ply	T5 FLAT GIRDER	0.00 / 0.00	19-08-00	02-02-00	2 X 8	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	226.44 / 141.34		
	2 / 2 Ply	T6 FLAT GIRDER	0.00 / 0.00	13-00-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	278.68 / 181.32		
	1	G7 GABLE	0.00 / 0.00	13-00-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	46.55 / 30.33		
	1 / 2 Ply	T8 FLAT	0.00 / 0.00	15-06-00	02-02-00	2 X 8	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	176.98 / 112.00		
	1	G8 GABLE	0.00 / 0.00	15-06-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	53.54 / 35.33		
	1 / 3 Ply	T10 FLAT GIRDER	0.00 / 0.00	14-04-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	238.38 / 159.99		
	2 / 2 Ply	T11 FLAT GIRDER	0.00 / 0.00	13-08-08	02-02-00	2 X 8	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	316.40 / 205.32		
	1	T12 FLAT	0.00 / 0.00	13-08-08	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	50.30 / 32.17		
	1 / 2 Ply	T13 FLAT	0.00 / 0.00	09-00-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	88.96 / 56.00		
	6	T14 FLAT	0.00 / 0.00	13-07-08	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	300.24 / 193.02		

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

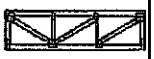
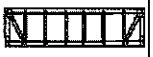
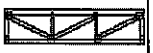
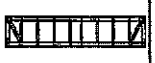
Delivery Shiplist

DATE	03/05/18
SALES REP	Rick

JOB TRACK: 45487 LAYOUT ID: 292707 LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO SUB-BUILDER:
MODEL: GEORGIAN 3 ELEVATION: A

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	1 / 3 Ply	T15 FLAT GIRDER	0.00 / 0.00	13-07-08	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	237.54 / 156.00		
	1	G16 GABLE	0.00 / 0.00	05-04-08	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	24.00 / 17.00		
	1 / 3 Ply	T17 FLAT GIRDER	0.00 / 0.00	16-04-00	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	281.82 / 179.01		
	5	T20 FLAT	0.00 / 0.00	17-05-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	315.80 / 205.00		
	3	T21 FLAT	0.00 / 0.00	13-09-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	151.29 / 96.51		
	1	G21 GABLE	0.00 / 0.00	13-09-00	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	48.26 / 31.67		
	5	T22 FLAT	0.00 / 0.00	15-09-08	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	283.25 / 181.65		
	1	G22X GABLE	0.00 / 0.00	17-05-08	02-02-00	2 X 4	2 X 4	00-00-00 / 00-00-00	02-02-00 / 02-02-00	59.29 / 39.00		
	1 / 2 Ply	T23 FLAT GIRDER	0.00 / 0.00	15-09-08	02-02-00	2 X 6	2 X 6	00-00-00 / 00-00-00	02-02-00 / 02-02-00	164.48 / 101.34		

TOTAL # TRUSS= **76** TOTAL BFT OF ALL TRUSSES= **3377.88** BFT. TOTAL WEIGHT OF ALL TRUSSES= **5287.56** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
29	Hangers	LJS26DS	
10	Hangers	HGUS26-2	
2	Hangers	HGUS28-3	
2	Beam	1 3/4" x 16" LVL 2.0E	16-00-00

TOTAL # ITEMS= **43**

TAMARACK LUMBER INC
ALPA LUMBER GROUP

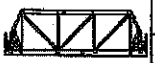
Delivery Shiplist

DATE	05/28/18
SALES REP	Rick

JOB TRACK: 45487 LAYOUT ID: 292709 LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO SUB-BUILDER:
MODEL: GEORGIAN 3 ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK / TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	5	T31 ROOF	16.00 / 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	800.20 / 493.35		
	1 / 2 Ply	T31Z ROOF	16.00 / 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	320.08 / 197.34		
	1	T32 ROOF	16.00 / 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	159.08 / 96.67		
	1	T32S ROOF	16.00 / 12.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	190.23 / 120.83		
	2	T33 ROOF	16.00 / 0.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	302.72 / 185.00		
	3	T33S ROOF	16.00 / 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-08-00	525.81 / 334.50		
	1	T34 ROOF	16.00 / 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	158.04 / 99.67		
	5	T35 ROOF	16.00 / 0.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	813.30 / 503.35		
	1	T35S ROOF	16.00 / 12.00	29-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	182.74 / 116.50		
	2	T36 ROOF	16.00 / 0.00	25-06-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	284.90 / 177.34		
	1 / 2 Ply	T36Z ROOF	16.00 / 0.00	25-06-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	284.90 / 177.34		
	1 / 4 Ply	T37 ROOF	16.00 / 0.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	722.20 / 465.32		
	7	T38S ROOF	16.00 / 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	1206.66 / 775.81		
	1 / 2 Ply	T38SZ ROOF	16.00 / 12.00	27-02-00	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	344.76 / 221.66		
	31	T39 ROOF	16.00 / 0.00	02-11-04	04-08-00	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 04-08-00	673.32 / 459.73		
	5	T40 ROOF	16.00 / 0.00	02-11-04	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-02-08	127.65 / 93.35		
	1	T40A ROOF	16.00 / 0.00	02-11-04	04-08-00	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 04-02-08	26.60 / 19.33		
	10	T41 FLAT	0.00 / 0.00	02-11-04	02-01-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 01-04-00	113.20 / 75.00		

DATE	05/28/18
SALES REP	Rick

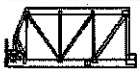
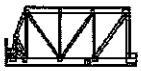
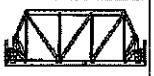
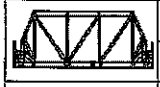
TAMARACK LUMBER INC
ALPA LUMBER GROUP

Delivery Shiplist

JOB TRACK: 45487	LAYOUT ID: 292709	LOCATION: OAKVILLE
BUILDER: GREEN YORK HOMES / TIBURTINO		SUB-BUILDER:
MODEL: GEORGIAN 3		ELEVATION: B

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY / PLY	MARK TYPE	PITCH TC / BC	SPAN	TRUSS HEIGHT	LUMBER TOP	LUMBER BOT	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY: / REMARKS
	7	T42 ROOF	16.00 / 0.00	18-00-08	04-09-08	2 X 4	2 X 8	00-00-00 / 00-00-00	01-04-00 / 00-07-04	845.53 / 511.00		
	1 / 2 Ply	T42Z ROOF	16.00 / 0.00	18-00-08	04-09-08	2 X 4	2 X 8	00-00-00 / 00-00-00	01-04-00 / 00-07-04	241.58 / 146.00		
	5	T43 ROOF	16.00 / 0.00	21-00-00	04-09-08	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	620.40 / 395.85		
	1 / 2 Ply	T43Z ROOF	16.00 / 0.00	21-00-00	04-09-08	2 X 4	2 X 6	00-00-00 / 00-00-00	01-04-00 / 01-04-00	248.16 / 158.34		
	18	T44 ROOF	16.00 / 0.00	03-00-06	04-09-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 04-09-08	400.86 / 291.06		
	6	T45 FLAT	0.00 / 0.00	03-00-06	02-01-08	2 X 4	2 X 4	00-00-00 / 00-00-00	01-04-00 / 01-04-00	69.66 / 49.02		

TOTAL # TRUSS= **125.00** TOTAL BFT OF ALL TRUSSES= **6163.36** BFT. TOTAL WEIGHT OF ALL TRUSSES= **9662.58** LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
63	Hangers	LUS24	
13	Hangers	LJS26DS	
1	Hangers	HGUS26-2	
2	Crush Plates	CP4-6	

TOTAL # ITEMS= **79.00**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T1	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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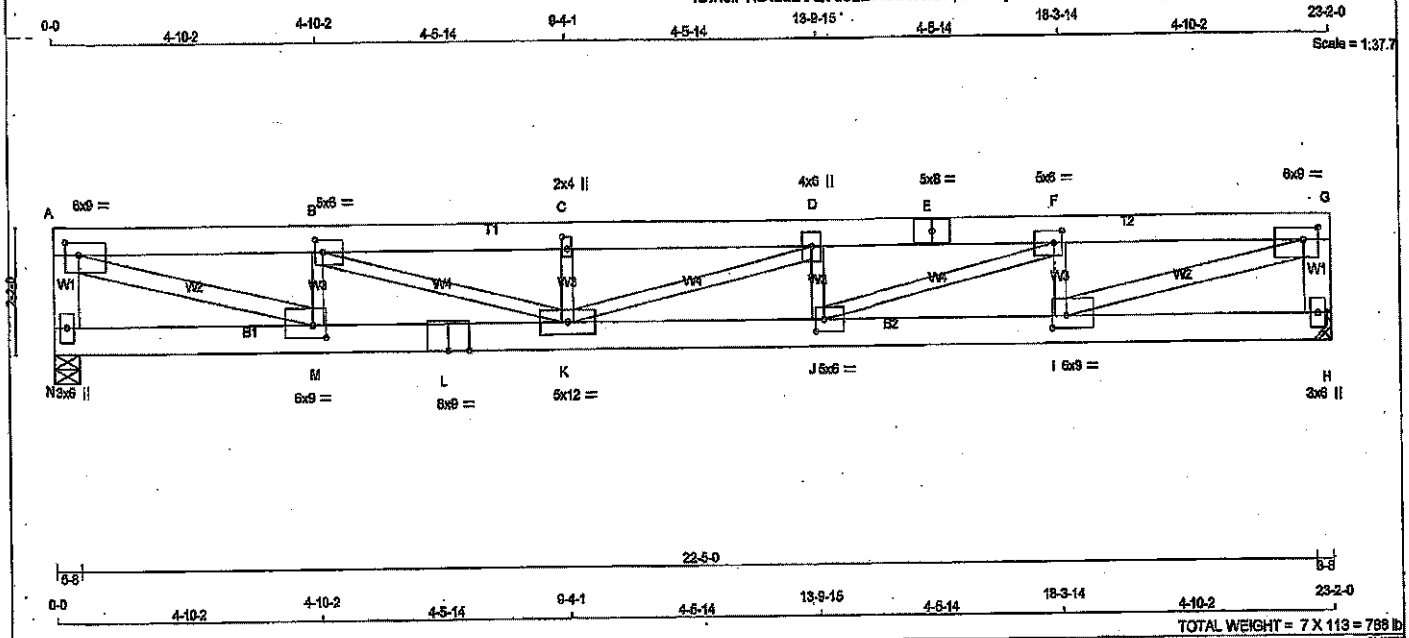

TOTAL WEIGHT = 7 X 113 = 788 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	2100F 1.8E	SPF
E - G	2x6	DRY	2100F 1.8E	SPF
H - G	2x6	DRY	No.2	SPF
N - L	2x6	DRY	2100F 1.8E	SPF
L - H	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
I - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	2.50	3.00
B	TMWW-t	MT20	5.0	6.0	2.50	1.75
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TMWW+t	MT20	4.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWW-t	MT20	5.0	6.0	2.50	1.75
G	TMWW-t	MT20	6.0	9.0	2.50	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMWW-t	MT20	6.0	9.0	2.50	3.00
J	BMWW-t	MT20	5.0	6.0	2.50	1.75
K	BMWWW-t	MT20	5.0	12.0		
L	BS-t	MT20	6.0	9.0		
M	BMWW-t	MT20	6.0	9.0	2.50	3.00
N	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1800	0	1800	0	0	5-8	5-8
H	1800	0	1800	0	0	HANGER BY OTHERS	IN-SX
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1447	694 / 0	243 / 0	0 / 0	0 / 0	510 / 0	0 / 0
H	1447	694 / 0	243 / 0	0 / 0	0 / 0	510 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
N-A	-1716 / 0	0.0	0.0	0.16 (1)	6.77	6.46
A-B	-4079 / 0	-127.4	-127.4	0.21 (1)	4.46	3.84
B-C	-5786 / 0	-127.4	-127.4	0.24 (1)	3.84	3.80
C-D	-5786 / 0	-127.4	-127.4	0.27 (1)	3.80	3.84
D-E	-5794 / 0	-127.4	-127.4	0.23 (1)	3.84	4.46
E-F	-5794 / 0	-127.4	-127.4	0.23 (1)	3.84	6.77
F-G	-4077 / 0	-127.4	-127.4	0.21 (1)	4.46	
H-G	-1716 / 0	0.0	0.0	0.16 (1)	6.77	
N-M	0 / 0	-28.0	-28.0	0.07 (3)	10.00	
M-L	0 / 4079	-28.0	-28.0	0.35 (1)	10.00	
L-K	0 / 4079	-28.0	-28.0	0.35 (1)	10.00	
K-J	0 / 5794	-28.0	-28.0	0.48 (1)	10.00	
J-I	0 / 4077	-28.0	-28.0	0.35 (1)	10.00	
I-H	0 / 0	-28.0	-28.0	0.07 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-G	0 / 4323	0.93 (1)
A-M	0 / 4325	0.93 (1)
I-F	-1308 / 0	0.25 (1)
M-B	-1308 / 0	0.25 (1)
J-F	0 / 1637	0.55 (1)
B-K	0 / 1626	0.55 (1)
J-D	-541 / 0	0.10 (1)
K-C	-539 / 0	0.10 (1)
K-D	-9 / 0	0.00 (1)

RECEIVED

MAR 12 2019

BUILDING DEPARTMENT
TOWN OF OAKVILLE

LICENSED PROFESSIONAL ENGINEER
2/3/18
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF	
	DL =	15.0	PSF	
BOT CH.	LL =	10.5	PSF	
	DL =	7.0	PSF	
TOTAL LOAD	=	62.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/ 620 (0.45")

CSI: TC=0.27/1.00 (C-D:1) , BC=0.48/1.00 (J-K:1) , WB=0.93/1.00 (A-M:1) , SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.99 (A) (INPUT = 1.00)

DWG NO. TAM | 2087-8
STRUCTURAL
COMPONENT ONLY

SITE COPY

19-0250

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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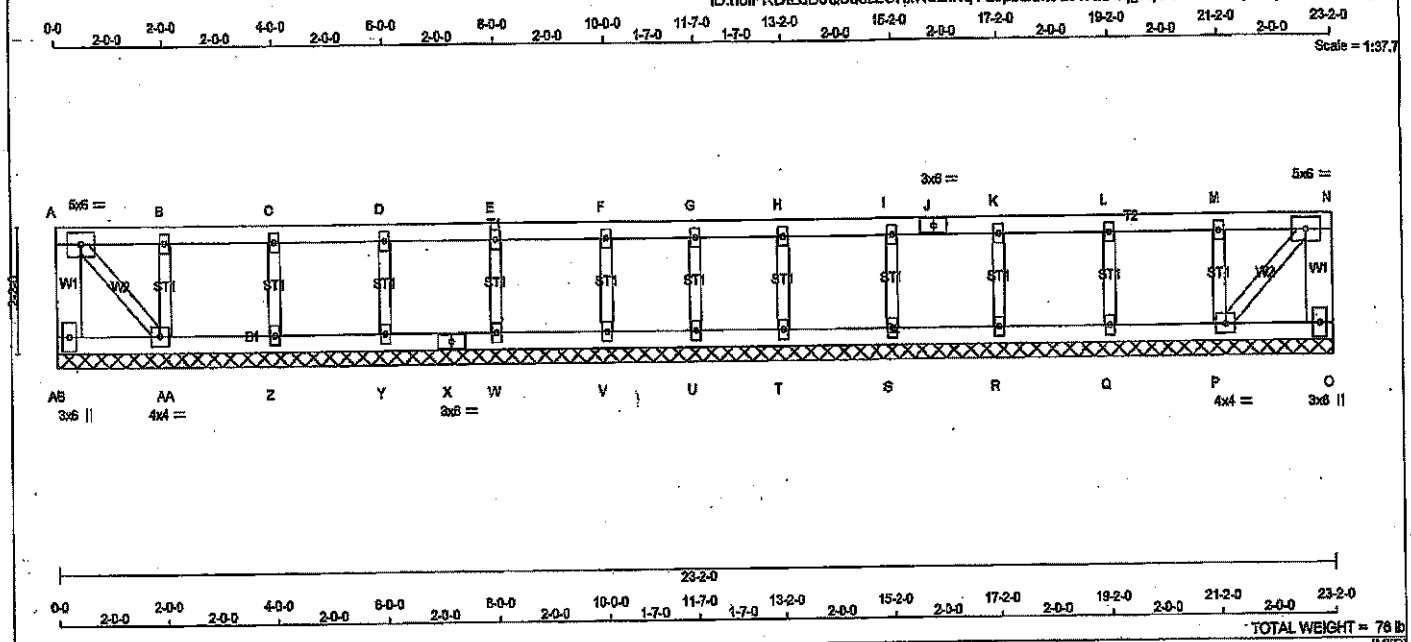

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AB- A	2x6	DRY	No.2	SPF
A - J	2x4	DRY	No.2	SPF
J - N	2x4	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
AB- X	2x4	DRY	No.2	SPF
X - O	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 CC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B, C, D, E, F, G, H, I, K, L, M					
B	TMW+w	MT20	2.0	4.0	
J	TS-t	MT20	3.0	6.0	
N	TMWW-t	MT20	5.0	6.0	
O	BMV1+p	MT20	3.0	6.0	
P	BMWW1-t	MT20	4.0	4.0	
Q, R, S, T, U, V, W, Y, Z					
Q	BMW1+w	MT20	2.0	4.0	
X	BS-t	MT20	3.0	6.0	
AA	BMWW1-t	MT20	4.0	4.0	
AB	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX UNBRAC LENGTH
AB-A	-98 / 0	0.0	0.0	0.01 (1)	7.81	
A-B	0 / 4	-127.4	-127.4	0.09 (1)	10.00	
B-C	0 / 4	-127.4	-127.4	0.09 (1)	10.00	
C-D	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
D-E	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
E-F	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
F-G	0 / 4	-127.4	-127.4	0.07 (1)	10.00	
G-H	0 / 4	-127.4	-127.4	0.07 (1)	10.00	
H-I	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
I-J	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
J-K	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
K-L	0 / 4	-127.4	-127.4	0.08 (1)	10.00	
L-M	0 / 4	-127.4	-127.4	0.09 (1)	10.00	
M-N	0 / 4	-127.4	-127.4	0.09 (1)	10.00	
O-N	-98 / 0	0.0	0.0	0.01 (1)	7.81	
AB-AA	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
AA-Z	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
Z-Y	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
Y-X	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
X-W	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
W-V	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
V-U	-4 / 0	-28.0	-28.0	0.03 (3)	10.00	
U-T	-4 / 0	-28.0	-28.0	0.03 (3)	10.00	
T-S	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
S-R	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
R-Q	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
Q-P	-4 / 0	-28.0	-28.0	0.04 (3)	10.00	
P-O	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-G	-186 / 0	0.04 (1)
F-M	-286 / 0	0.06 (1)
Q-L	-248 / 0	0.05 (1)
R-K	-255 / 0	0.05 (1)
S-I	-260 / 0	0.05 (1)
T-H	-232 / 0	0.05 (1)
AA-B	-286 / 0	0.06 (1)
Z-C	-248 / 0	0.05 (1)
Y-D	-255 / 0	0.05 (1)
W-E	-260 / 0	0.05 (1)
V-F	-232 / 0	0.05 (1)
A-AA	-5 / 0	0.00 (1)
P-N	-5 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (B-C:1) , BC=0.04/1.00 (Z-AA:3) , WB=0.06/1.00 (M-P:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1587	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (AA) (INPUT = 0.90)
JSI METAL= 0.07 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12107-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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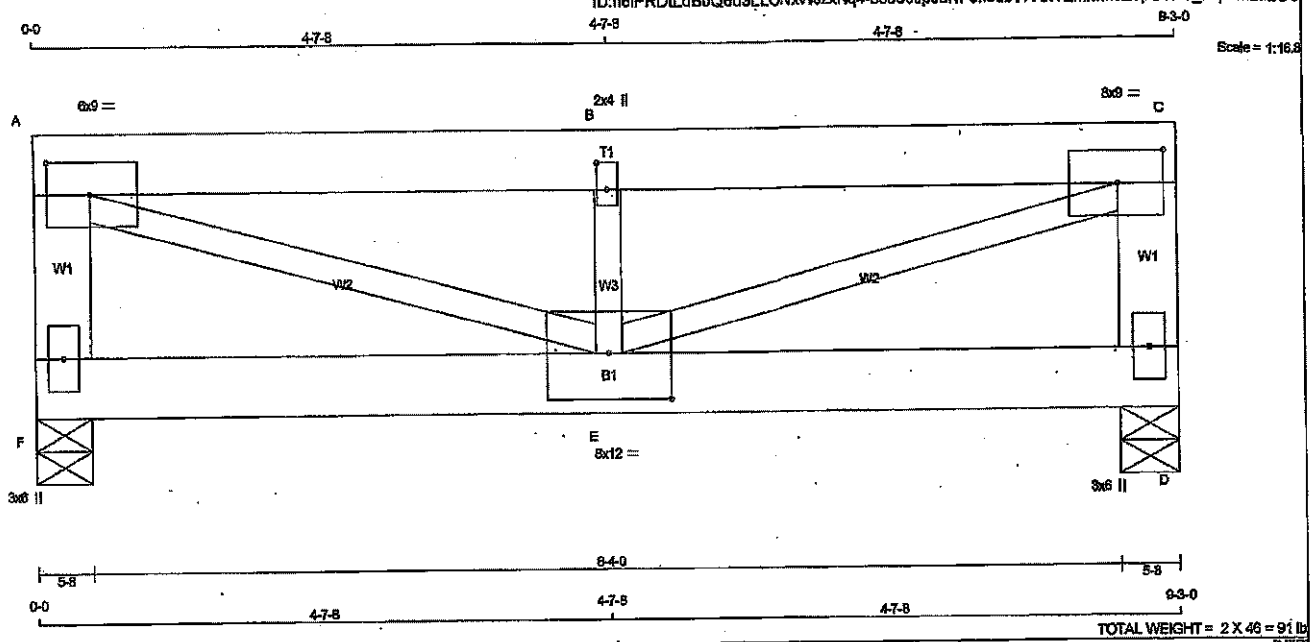

TOTAL WEIGHT = 2 X 46 = 91 lb

MiTek

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	4.25
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMWW-t	MT20	6.0	9.0	3.00	4.25
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	12.0	4.25	6.00
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	719	0	719	0	0	5-8	5-8
D	719	0	719	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	578	277 / 0	97 / 0	0 / 0	0 / 0	204 / 0	0 / 0
D	576	277 / 0	97 / 0	0 / 0	0 / 0	204 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-844 / 0	0.0	0.0	0.08 (1)	7.81	
A-B	-1070 / 0	-127.4	-127.4	0.23 (1)	6.19	
B-C	-1070 / 0	-127.4	-127.4	0.23 (1)	6.19	
D-C	-844 / 0	0.0	0.0	0.08 (1)	7.81	
F-E	0 / 0	-28.0	-28.0	0.11 (3)	10.00	
E-D	0 / 0	-28.0	-28.0	0.11 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 1140	0.34 (1)
E-B	-662 / 0	0.13 (1)
E-C	0 / 1140	0.34 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.23/1.00 (B-C:1) , BC=0.11/1.00 (E-F:3) , WB=0.34/1.00 (A-E:1) , SSI=0.27/1.00 (A-B:1)

DOL-LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.62 (R) (INPUT = 0.80)
JSI METAL= 0.24 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 120828
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T2Z	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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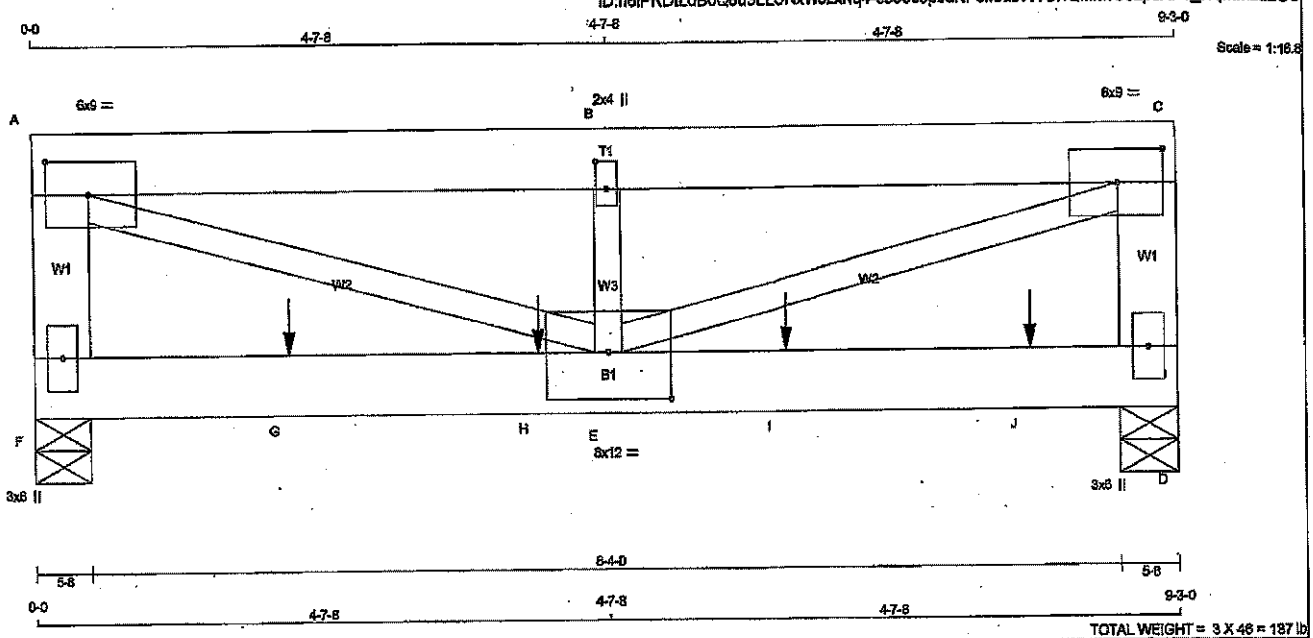

TOTAL WEIGHT = 3 X 46 = 137 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F-A	2	12	TOP
A-C	2	12	TOP
C-D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	6	SIDE(492.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	8	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	4.25
B	TMWtw	MT20	2.0	4.0	2.50	1.00
C	TMWW-t	MT20	6.0	9.0	3.00	4.25
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	8.0	12.0	4.25	6.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1772.1 lbs FACTORED DOWN AT 2-0-12, 1772.1 lbs FACTORED DOWN AT 4-0-12, AND 1772.1 lbs FACTORED DOWN AT 6-0-12, AND 1772.1 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3928	0	3928	0	0	5-8	5-8
D	4598	0	4598	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	3135	1534 / 0	500 / 0	0 / 0	0 / 0	1101 / 0	0 / 0
D	3670	1797 / 0	584 / 0	0 / 0	0 / 0	1289 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-3020 / 0	0.0	0.0	0.09 (1)	7.81	
A-B	-7106 / 0	-127.4	-127.4	0.23 (1)	4.56	
B-C	-7106 / 0	-127.4	-127.4	0.23 (1)	4.56	
D-C	-3020 / 0	0.0	0.0	0.09 (1)	7.81	
F-G	0 / 0	-28.0	-28.0	0.54 (1)	10.00	
G-H	0 / 0	-28.0	-28.0	0.54 (1)	10.00	
H-E	0 / 0	-28.0	-28.0	0.54 (1)	10.00	
E-I	0 / 0	-28.0	-28.0	0.55 (1)	10.00	
I-J	0 / 0	-28.0	-28.0	0.55 (1)	10.00	
J-D	0 / 0	-28.0	-28.0	0.55 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 7575	0.76 (1)
E-B	-386 / 0	0.03 (1)
E-C	0 / 7575	0.76 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1772	-1772	---	FRONT	VERT	TOTAL
H	4-0-12	-1772	-1772	---	FRONT	VERT	TOTAL
I	6-0-12	-1772	-1772	---	FRONT	VERT	TOTAL
J	8-0-12	-1772	-1772	---	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
3526
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 653 (0.12")

CSI: TC=0.23/1.00 (A-B:1) , BC=0.55/1.00 (D-E:1) , WB=0.76/1.00 (A-E:1) , SSI=0.74/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.53 (A) (INPUT = 1.00)

DWG NO. TAM 12009-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T2Z2	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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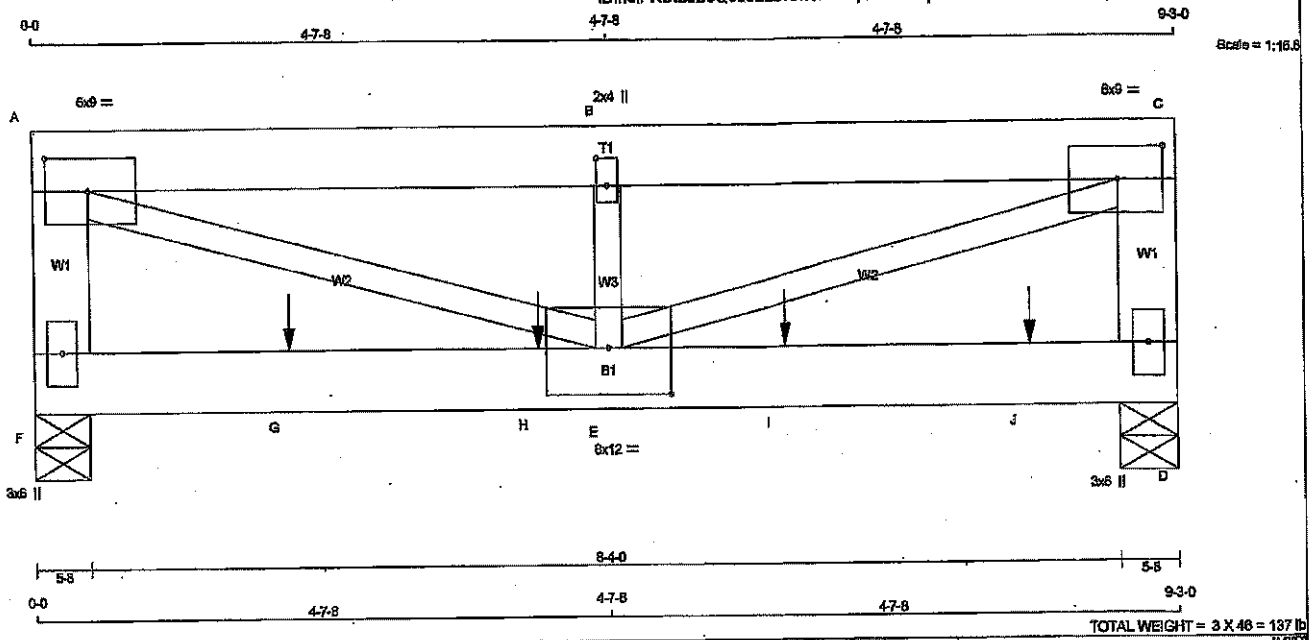

TOTAL WEIGHT = 3 X 46 = 137 lb

[M][R]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F- A	2	12	TOP
A- C	2	12	TOP
C- D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F- D	2	7	SIDE(391.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	4.25
B	TMWw	MT20	2.0	4.0	2.60	1.00
C	TMWW-t	MT20	6.0	9.0	3.00	4.25
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	8.0	12.0	4.25	6.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1500.2 lbs FACTORED DOWN AT 2-0-12, 1500.2 lbs FACTORED DOWN AT 4-0-12, AND 1500.2 lbs FACTORED DOWN AT 6-0-12, AND 1500.2 lbs FACTORED DOWN AT 8-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	3435	0	3435	0	0	5-8	5-8
D	4003	0	4003	0	0	5-8	5-8

UNFACTORED REACTIONS

MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	2739	1944 / 0	435 / 0	0 / 0	0 / 0	862 / 0	0 / 0
D	3191	1567 / 0	503 / 0	0 / 0	0 / 0	1120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.86 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-2556 / 0	0.0	0.0	0.08 (1)	7.81	
A- B	-6180 / 0	-127.4	-127.4	0.20 (1)	4.86	
B- C	-6180 / 0	-127.4	-127.4	0.20 (1)	4.86	
D- C	-2555 / 0	0.0	0.0	0.08 (1)	7.81	
F- G	0 / 0	-28.0	-28.0	0.46 (1)	10.00	
G- H	0 / 0	-28.0	-28.0	0.46 (1)	10.00	
H- E	0 / 0	-28.0	-28.0	0.46 (1)	10.00	
E- I	0 / 0	-28.0	-28.0	0.47 (1)	10.00	
I- J	0 / 0	-28.0	-28.0	0.47 (1)	10.00	
J- D	0 / 0	-28.0	-28.0	0.47 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 6588	0.66 (1)
E- B	-434 / 0	0.03 (1)
E- C	0 / 6588	0.66 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	2-0-12	-1500	-1500	—	BACK	VERT	TOTAL
H	4-0-12	-1500	-1500	—	BACK	VERT	TOTAL
I	6-0-12	-1500	-1500	—	BACK	VERT	TOTAL
J	8-0-12	-1500	-1500	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.31")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.31")
CALCULATED VERT. DEFL.(TL) = L/ 969 (0.10")

CSI: TC=0.20/1.00 (A-B:1) , BC=0.47/1.00 (D-E:1) , WB=0.66/1.00 (A-E:1) , SSI=0.63/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (C) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM N090-88
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T3	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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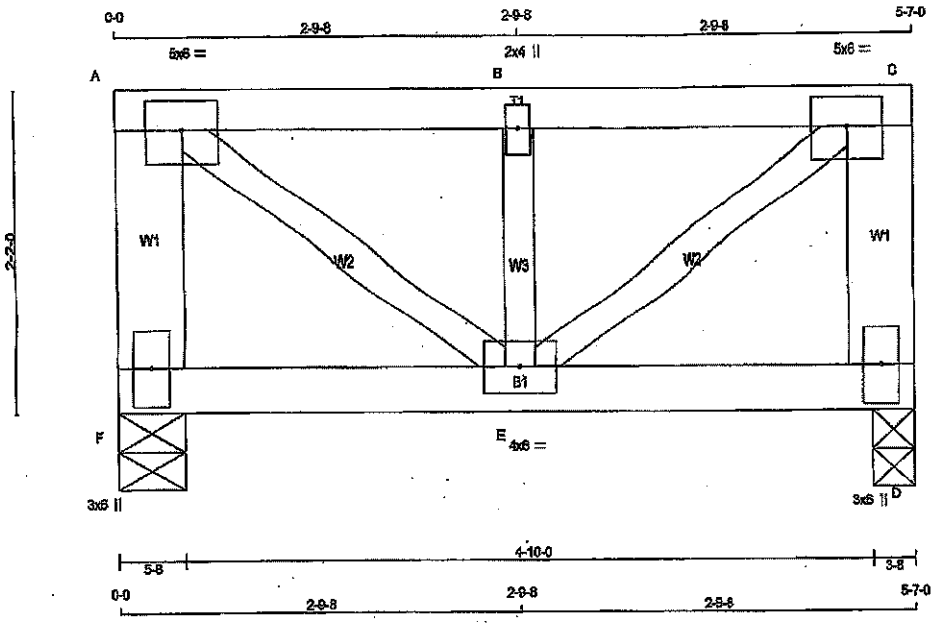

TOTAL WEIGHT = 3 X 24 = 72 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B	TMWH+w	MT20	2.0	4.0	
C	TMWW-t	MT20	5.0	6.0	
D	BMV1+p	MT20	3.0	6.0	
E	BMWWW-t	MT20	4.0	6.0	
F	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	434	0	434	0	0	5-8	5-8
D	434	0	434	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	349	167 / 0	59 / 0	0 / 0	0 / 0	123 / 0	0 / 0
D	349	167 / 0	59 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-399 / 0	0.0	0.0	0.04 (1)	7.81
A-B	-385 / 0	-127.4	-127.4	0.20 (1)	6.25
B-C	-385 / 0	-127.4	-127.4	0.20 (1)	6.25
D-C	-399 / 0	0.0	0.0	0.04 (1)	7.81
F-E	0 / 0	-28.0	-28.0	0.08 (3)	10.00
E-D	0 / 0	-28.0	-28.0	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 464	0.14 (1)
E-B	-432 / 0	0.09 (1)
E-C	0 / 464	0.14 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.20/1.00 (B-C:1) , BC=0.08/1.00 (E-F:3) , WB=0.14/1.00 (A-E:1) , SSI=0.22/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3510
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12071-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T4	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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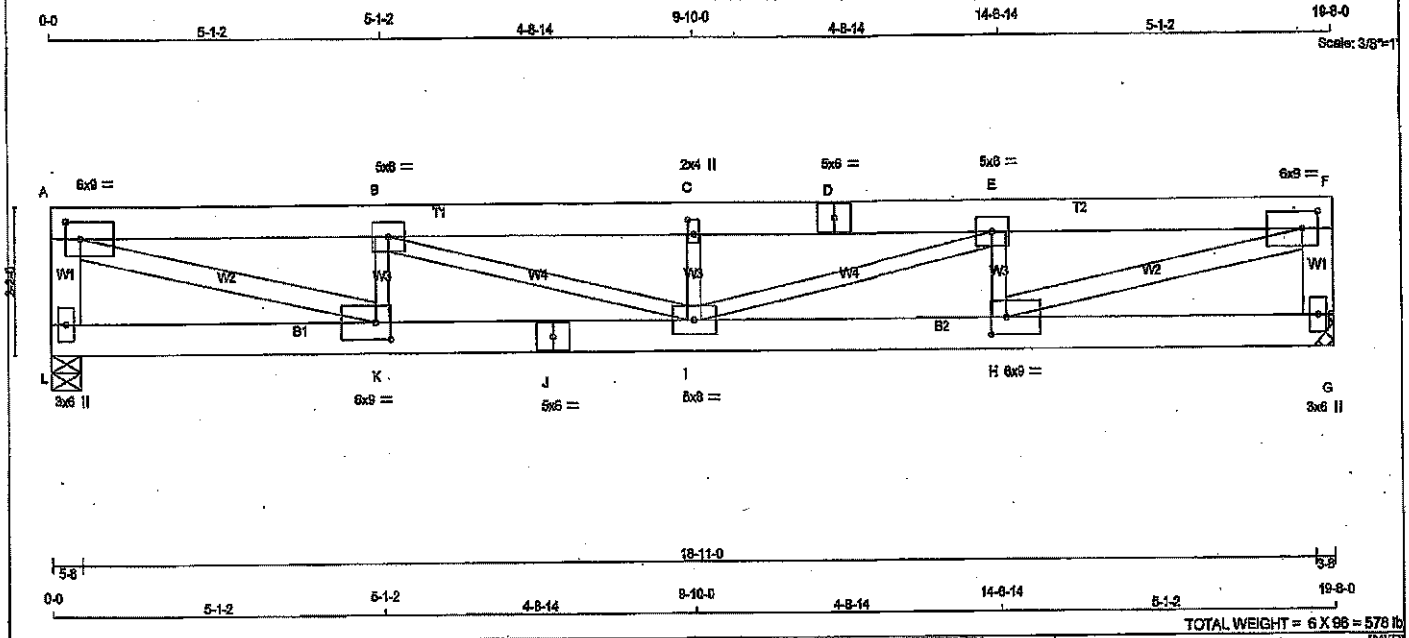

TOTAL WEIGHT = 6 X 96 = 576 lb

[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
H - F	2x4	DRY	No.2	SPF
A - K	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	3.00	2.75
B	TMWW-t	MT20	6.0	6.0		
C	TMW+w	MT20	2.0	4.0	2.50	1.00
D	TS-t	MT20	5.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMWW-t	MT20	6.0	9.0	3.00	2.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	6.0	9.0	3.00	2.75
I	BMWWW-t	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	2.75
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1528	0	1528	0	0	5-8	5-8
G	1528	0	1528	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1229	589 / 0	207 / 0	0 / 0	0 / 0	433 / 0	0 / 0
G	1229	589 / 0	207 / 0	0 / 0	0 / 0	433 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-A	-1445 / 0	0.0	0.0	0.13 (1)	7.24	
A-B	-3459 / 0	-127.4	-127.4	0.41 (1)	3.70	
B-C	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	
C-D	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	
D-E	-4361 / 0	-127.4	-127.4	0.40 (1)	3.36	
E-F	-3459 / 0	-127.4	-127.4	0.41 (1)	3.70	
G-F	-1445 / 0	0.0	0.0	0.13 (1)	7.24	
L-K	0 / 0	-28.0	-28.0	0.14 (3)	10.00	
K-J	0 / 3459	-28.0	-28.0	0.69 (1)	10.00	
J-I	0 / 3459	-28.0	-28.0	0.69 (1)	10.00	
I-H	0 / 3459	-28.0	-28.0	0.69 (1)	10.00	
H-G	0 / 0	-28.0	-28.0	0.14 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-F	0 / 3648	0.78 (1)
A-K	0 / 3648	0.78 (1)
H-E	-1024 / 0	0.20 (1)
K-B	-1024 / 0	0.20 (1)
I-E	0 / 959	0.29 (1)
B-I	0 / 959	0.29 (1)
I-C	-538 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 728 (0.32")

CSI: TC=0.41/1.00 (A-B:1) , BC=0.69/1.00 (I-K:1) , WB=0.78/1.00 (A-K:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (A) (INPUT = 0.90)
JSI METAL= 0.66 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12092-08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G4	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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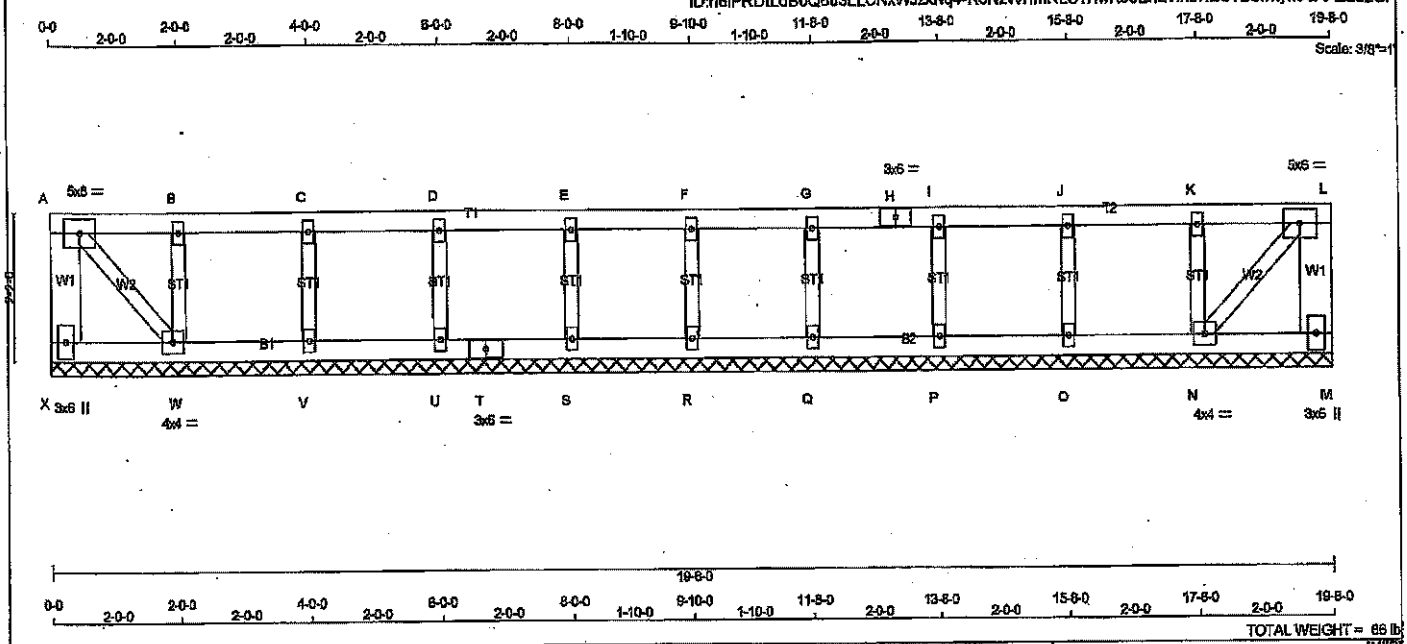

TOTAL WEIGHT = 66 lb

[M][R]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
X - A	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - L	2x4	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
X - T	2x4	DRY	No.2	SPF
T - M	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B, C, D, E, F, G, I, J, K					
B	TMW+w	MT20	2.0	4.0	
H	TS-t	MT20	3.0	6.0	
L	TMWW-t	MT20	5.0	8.0	
M	BMV1+p	MT20	3.0	6.0	
N	BMWW1-t	MT20	4.0	4.0	
O, P, Q, R, S, U, V					
O	BMW1+w	MT20	2.0	4.0	
T	SS-t	MT20	3.0	6.0	
W	BMWW1-t	MT20	4.0	4.0	
X	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
X- A	-97 / 0	0.0	0.0	0.01 (1)	7.81
A- B	0 / 4	-127.4	-127.4	0.09 (1)	10.00
B- C	0 / 4	-127.4	-127.4	0.09 (1)	10.00
C- D	0 / 4	-127.4	-127.4	0.08 (1)	10.00
D- E	0 / 4	-127.4	-127.4	0.08 (1)	10.00
E- F	0 / 4	-127.4	-127.4	0.07 (1)	10.00
F- G	0 / 4	-127.4	-127.4	0.07 (1)	10.00
G- H	0 / 4	-127.4	-127.4	0.08 (1)	10.00
H- I	0 / 4	-127.4	-127.4	0.08 (1)	10.00
I- J	0 / 4	-127.4	-127.4	0.08 (1)	10.00
J- K	0 / 4	-127.4	-127.4	0.09 (1)	10.00
K- L	0 / 4	-127.4	-127.4	0.09 (1)	10.00
M- L	-97 / 0	0.0	0.0	0.01 (1)	7.81
X- W	0 / 0	-28.0	-28.0	0.04 (3)	10.00
W- V	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
V- U	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
U- T	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
T- S	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
S- R	-4 / 0	-28.0	-28.0	0.03 (3)	10.00
R- Q	-4 / 0	-28.0	-28.0	0.03 (3)	10.00
Q- P	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
P- O	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
O- N	-4 / 0	-28.0	-28.0	0.04 (3)	10.00
N- M	0 / 0	-28.0	-28.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- F	-228 / 0	0.06 (1)
N- K	-288 / 0	0.08 (1)
O- J	-247 / 0	0.05 (1)
P- I	-259 / 0	0.05 (1)
Q- G	-245 / 0	0.05 (1)
W- B	-288 / 0	0.06 (1)
V- C	-247 / 0	0.05 (1)
U- D	-259 / 0	0.05 (1)
S- E	-245 / 0	0.05 (1)
A- W	-6 / 0	0.00 (1)
N- L	-6 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (K-L:1) , BC=0.04/1.00 (N-O:3) , WB=0.08/1.00 (K-N:1) , SSI=0.16/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	922	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (W) (INPUT = 0.60)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12408

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T5	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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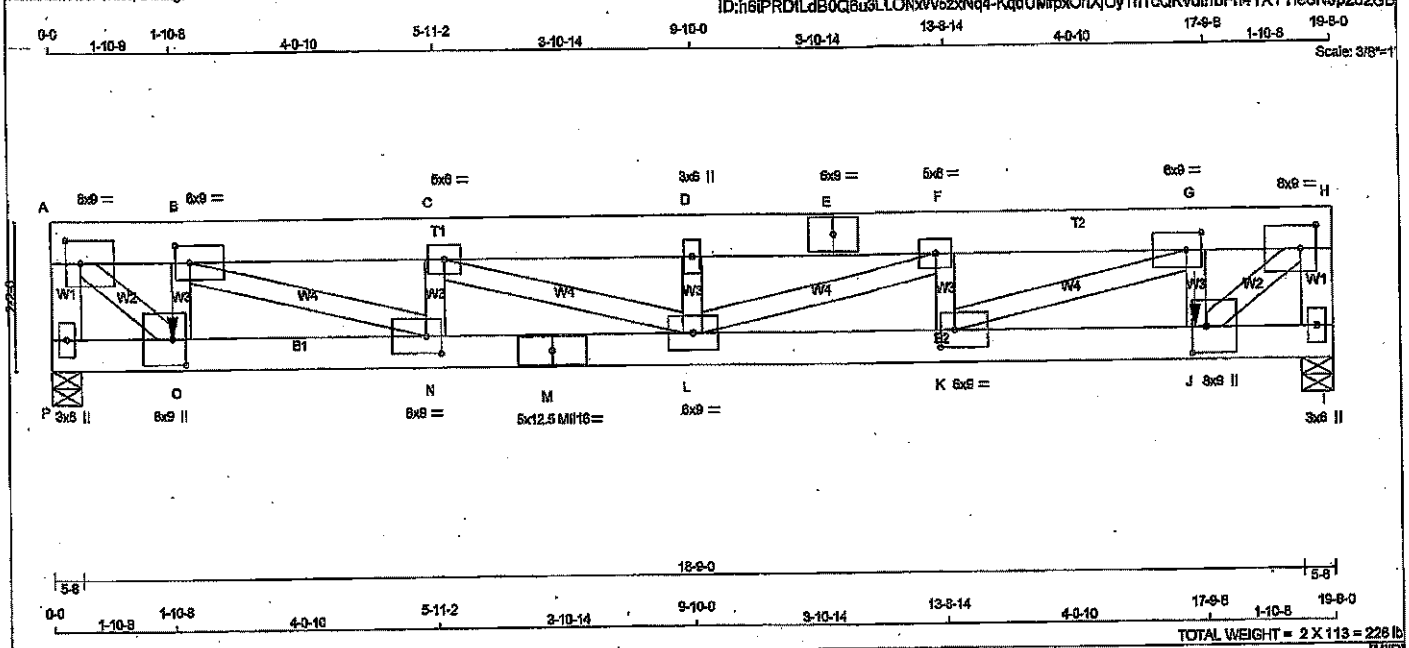

TOTAL WEIGHT = 2 X 113 = 226 lb

[MIR]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - E	2x8	DRY	No.2	SPF
E - H	2x8	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	2100F 1.8E	SPF
M - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
A - O	2x4	DRY	2100F 1.8E	SPF
J - H	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
P-A	2	12	TOP
H-I	2	12	TOP
E-A	2	9	SIDE(179.7)
E-H	2	9	SIDE(179.7)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
P-M	2	12	SIDE(148.3)
M-I	2	12	SIDE(148.3)
WEBS : (0.122"X3") SPIRAL NAILS			
B-O	2	2	SIDE(257.0)
G-J	2	2	SIDE(257.0)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	8.0	9.0	4.00	2.75
B	TMWW-t	MT20	6.0	9.0	3.00	2.75
C	TMWW-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TS-t	MT20	6.0	9.0		
F	TMWW-t	MT20	5.0	6.0		
G	TMWW-t	MT20	6.0	9.0	3.00	2.75
H	TMWW-t	MT20	8.0	9.0	4.00	2.75
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	8.0	9.0	4.50	2.50
K	BMWW-t	MT20	6.0	9.0	3.00	2.75
L	BMWWW-t	MT20	6.0	9.0		
M	BS-t	MII16	5.0	12.5		
N	BMWW-t	MT20	6.0	9.0	3.00	2.75
O	BMWW+t	MT20	8.0	9.0	4.50	2.50
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	5869	0	5869	0	0	5-8	5-8
I	5869	0	5869	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX.MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	4720	2284 / 0	776 / 0	0 / 0	0 / 0	1860 / 0	0 / 0
I	4720	2284 / 0	776 / 0	0 / 0	0 / 0	1860 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-A	-5876 / 0	0.0	0.0	0.29 (1)	5.37	
A-B	-7334 / 0	-127.4	-127.4	0.21 (1)	4.27	
B-C	-13883 / 0	-486.8	-486.8	0.61 (1)	2.79	
C-D	-16245 / 0	-486.8	-486.8	0.84 (1)	2.23	
D-E	-16245 / 0	-486.8	-486.8	0.84 (1)	2.23	
E-F	-16245 / 0	-486.8	-486.8	0.84 (1)	2.23	
F-G	-13883 / 0	-486.8	-486.8	0.61 (1)	2.79	
G-H	-7334 / 0	-127.4	-127.4	0.21 (1)	4.27	
I-H	-5876 / 0	0.0	0.0	0.29 (1)	5.37	
P-O	0 / 0	-28.0	-28.0	0.02 (3)	10.00	
O-N	0 / 7334	-28.0	-28.0	0.38 (1)	10.00	
N-M	0 / 13883	-28.0	-28.0	0.62 (1)	10.00	
M-L	0 / 13883	-28.0	-28.0	0.62 (1)	10.00	
L-K	0 / 13883	-28.0	-28.0	0.62 (1)	10.00	
K-J	0 / 7334	-28.0	-28.0	0.38 (1)	10.00	
J-I	0 / 0	-28.0	-28.0	0.02 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-O	0 / 9435	0.52 (1)
O-B	-4157 / 0	0.31 (1)
J-G	-4157 / 0	0.31 (1)
J-H	0 / 9435	0.52 (1)
K-G	0 / 7100	0.84 (1)
B-N	0 / 7100	0.84 (1)
K-F	-2761 / 0	0.21 (1)
N-C	-2761 / 0	0.21 (1)
L-F	0 / 2561	0.30 (1)
C-L	0 / 2561	0.30 (1)
L-O	-1866 / 0	0.14 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.
J	17-7-8	-1554	-1554	—	BACK	TOTAL
O	1-10-8	-1554	-1554	—	BACK	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

GIRDER TYPE: CSI\Girder
START DISTANCE = 1-10-8
START SPAN CARRIED = 11-3-0
END DISTANCE = 17-7-8
END SPAN CARRIED = 11-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/594 (0.40")

CSI: TC=0.84/1.00 (C-D:1) , BC=0.62/1.00 (L-N:1) , WB=0.84/1.00 (G-K:1) , SSI=0.43/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.96 (A) (INPUT = 1.00)

pg 1/2

DWG NO. TAM 12093-18

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T5	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1553.5 lbs FACTORED DOWN AT 1-10-8, AND 1553.5 lbs FACTORED DOWN AT 17-7-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12093-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T6	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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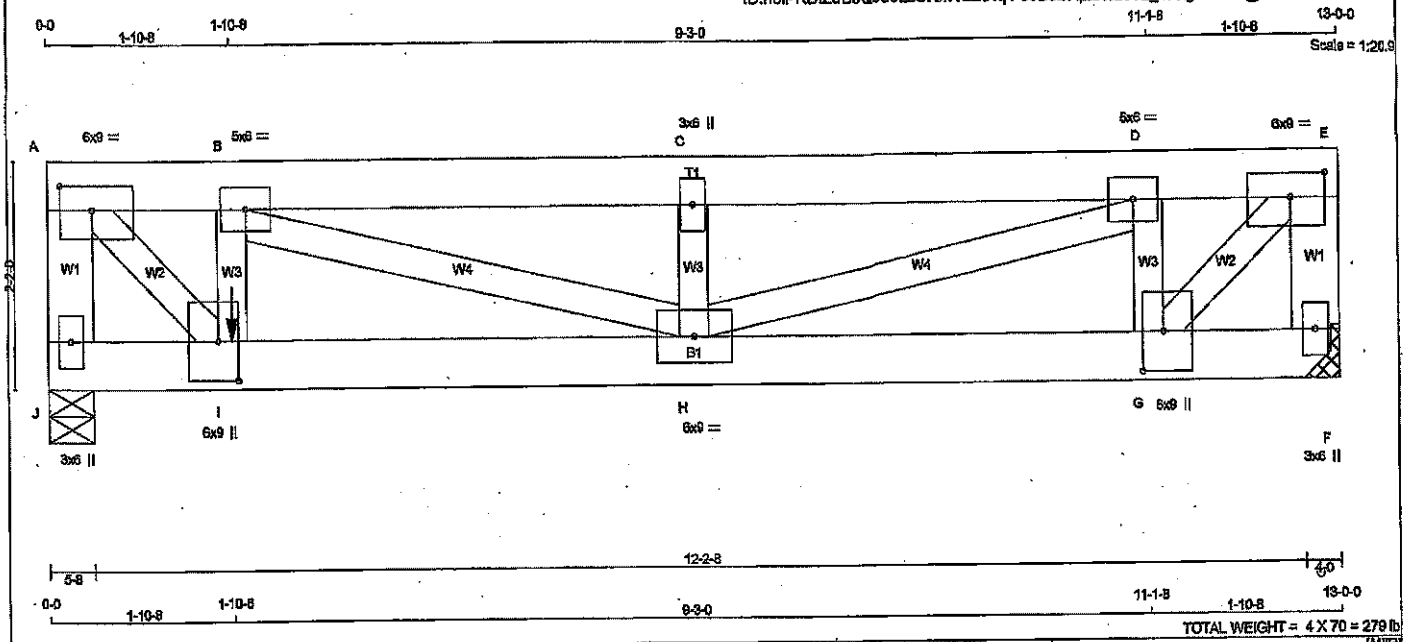

TOTAL WEIGHT = 4 X 70 = 279 lb

[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J- A	2	12	TOP
A- E	2	12	TOP
E- F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J- F	2	3	SIDE(686.7)
WEBS : (0.122"X3") SPIRAL NAILS			
B- I	2	2	SIDE(346.8)
2x4	1	6	
D- G	2	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	2.75	4.00
B	TMWW-t	MT20	5.0	6.0		
C	TMW+w	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	6.0		
E	TMWW-t	MT20	6.0	9.0	2.75	4.00
F	BMV1+p	MT20	3.0	6.0		
G	BMWW+t	MT20	6.0	9.0	4.50	2.50
H	BMWWW-t	MT20	6.0	9.0		
I	BMWW+t	MT20	6.0	9.0	4.50	2.50
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3961.6 lbs FACTORED DOWN AT 1-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	4400	0	4400	0	0	5-8	5-8
F	1582	0	1582	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	3527	1705 / 0	580 / 0	0 / 0	0 / 0	1241 / 0	0 / 0
F	1270	612 / 0	211 / 0	0 / 0	0 / 0	447 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-4239 / 0	0.0	0.0	0.21 (1)	6.19	
A- B	-4468 / 0	-127.4	-127.4	0.10 (1)	4.79	
B- C	-4128 / 0	-127.4	-127.4	0.21 (1)	4.82	
C- D	-4128 / 0	-127.4	-127.4	0.14 (1)	4.90	
D- E	-1724 / 0	-127.4	-127.4	0.10 (1)	6.25	
F- E	-1590 / 0	0.0	0.0	0.08 (1)	7.61	
J- I	0 / 0	-28.0	-28.0	0.12 (1)	10.00	
I- H	0 / 4468	-28.0	-28.0	0.52 (1)	10.00	
H- G	0 / 1724	-28.0	-28.0	0.19 (1)	10.00	
G- F	0 / 0	-28.0	-28.0	0.05 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- I	0 / 6044	0.71 (1)
I- B	-193 / 13	0.01 (1)
G- D	-1416 / 0	0.11 (1)
G- E	0 / 2332	0.27 (1)
H- C	-622 / 0	0.05 (1)
B- H	-363 / 0	0.07 (1)
H- D	0 / 2563	0.30 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	1-10-8	-3962	-3962	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.21/1.00 (A-J:1) , BC=0.52/1.00 (H-I:1) , WB=0.71/1.00 (A-I:1) , SSI=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.90)
JSI METAL= 0.65 (I) (INPUT = 1.00)

DWG NO. TAM 12094 B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G7	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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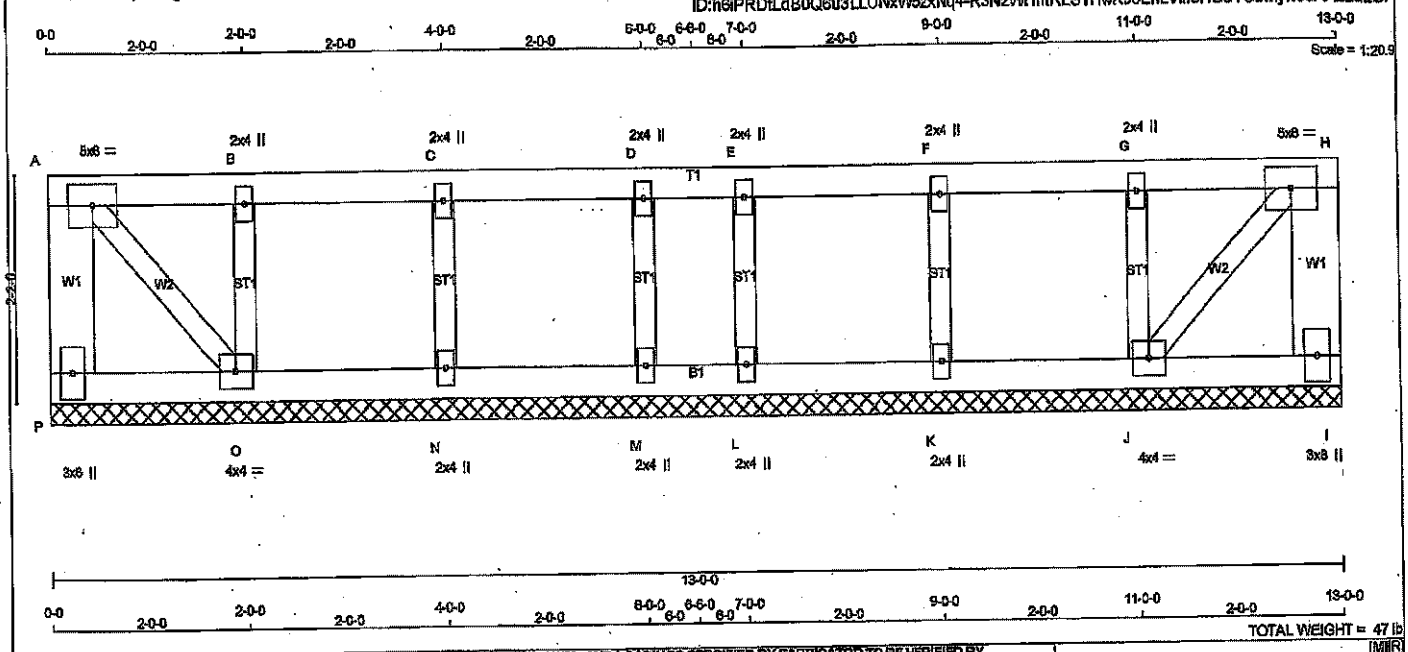

TOTAL WEIGHT = 47 lb

[MIR]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		
B, C, D, E, F, G						
B	TMWW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMWW1-t	MT20	4.0	4.0		
K, L, M, N						
K	BMWV1+w	MT20	2.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
P-A	-98 / 0	0.0	0.0	0.01 (1)	7.81
A-B	0 / 6	-127.4	-127.4	0.09 (1)	10.00
B-C	0 / 6	-127.4	-127.4	0.09 (1)	10.00
C-D	0 / 6	-127.4	-127.4	0.08 (1)	10.00
D-E	0 / 6	-127.4	-127.4	0.05 (1)	10.00
E-F	0 / 6	-127.4	-127.4	0.06 (1)	10.00
F-G	0 / 6	-127.4	-127.4	0.09 (1)	10.00
G-H	0 / 6	-127.4	-127.4	0.09 (1)	10.00
I-H	-96 / 0	0.0	0.0	0.01 (1)	7.81
P-O	0 / 0	-26.0	-26.0	0.04 (3)	10.00
O-N	-6 / 0	-26.0	-26.0	0.04 (3)	10.00
N-M	-6 / 0	-26.0	-26.0	0.04 (3)	10.00
M-L	-6 / 0	-26.0	-26.0	0.02 (3)	10.00
L-K	-6 / 0	-26.0	-26.0	0.04 (3)	10.00
K-J	-6 / 0	-26.0	-26.0	0.04 (3)	10.00
J-I	0 / 0	-26.0	-26.0	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-G	-284 / 0	0.06 (1)
K-F	-258 / 0	0.05 (1)
L-E	-184 / 0	0.04 (1)
O-B	-284 / 0	0.06 (1)
N-C	-258 / 0	0.05 (1)
M-D	-184 / 0	0.04 (1)
A-O	-8 / 0	0.00 (1)
J-H	-8 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (A-B:1) , BC=0.04/1.00 (N-O:3) , WB=0.06/1.00 (B-C:1) , SSI=0.16/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (J) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3/13
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12109-B
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T8	1	2	TRUSS DESC.	

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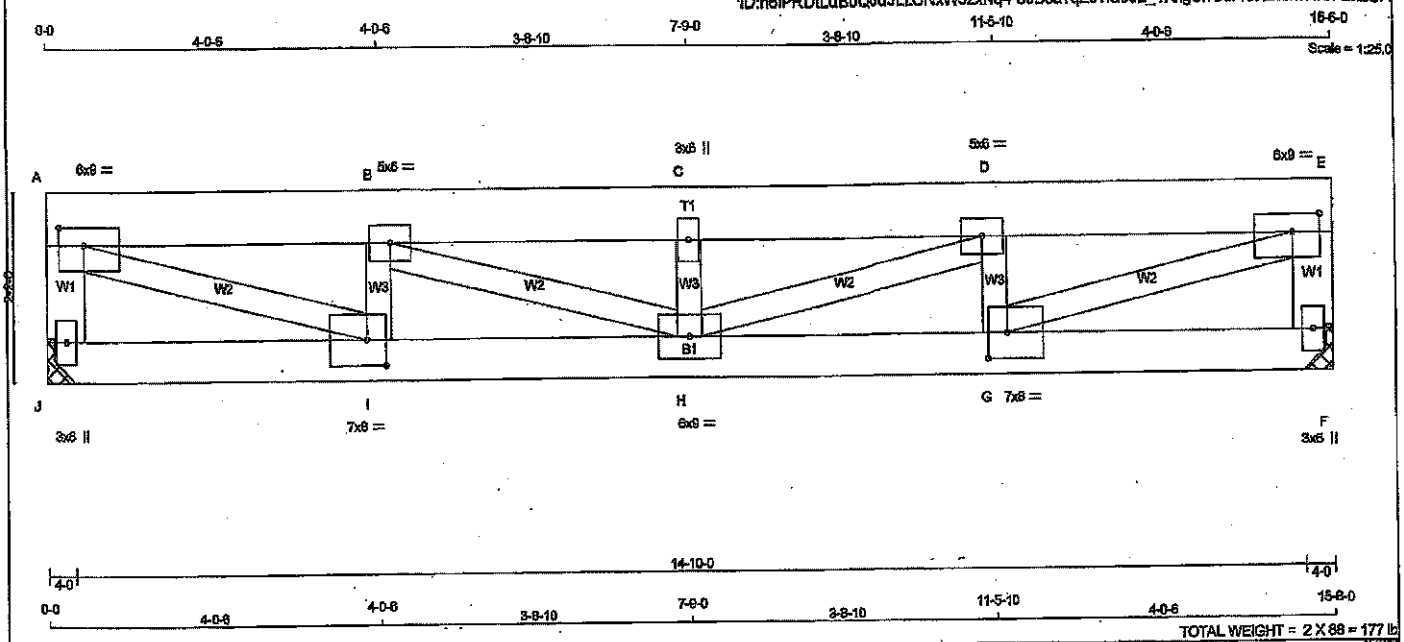

TOTAL WEIGHT = 2 X 88 = 177 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x6	DRY	No.2	SPF
A - E	2x8	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS		
J- A 2	12	TOP
E- F 2	12	TOP
A- E 2	9	SIDE(179.7)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS		
J- F 2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS		
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	2.50	3.75
B	TMWW-t	MT20	5.0	8.0		
C	TMW+w	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	8.0		
E	TMWW-t	MT20	6.0	9.0	2.50	3.75
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	8.0	3.50	2.75
H	BMWWW-t	MT20	6.0	9.0		
I	BMWW-t	MT20	7.0	8.0	3.50	2.75
J	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	FACTORED UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	3990	0	3990	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
F	3990	0	3990	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	3207	1539 / 0	539 / 0	0 / 0	0 / 0	1130 / 0	0 / 0
F	3207	1539 / 0	539 / 0	0 / 0	0 / 0	1130 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.62 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J- A	-3888 / 0	0.0	0.0	0.19 (1)	6.42	
A- B	-7162 / 0	-486.8	-486.8	0.40 (1)	4.08	
B- C	-9279 / 0	-486.8	-486.8	0.42 (1)	3.62	
C- D	-9279 / 0	-486.8	-486.8	0.42 (1)	3.62	
D- E	-7162 / 0	-486.8	-486.8	0.40 (1)	4.08	
F- E	-3888 / 0	0.0	0.0	0.19 (1)	6.42	
J- I	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
I- H	0 / 7162	-28.0	-28.0	0.76 (1)	10.00	
H- G	0 / 7162	-28.0	-28.0	0.76 (1)	10.00	
G- F	0 / 0	-28.0	-28.0	0.13 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- E	0 / 7729	0.91 (1)
A- I	0 / 7729	0.91 (1)
G- D	-2876 / 0	0.22 (1)
I- B	-2876 / 0	0.22 (1)
H- D	0 / 2312	0.27 (1)
B- H	0 / 2312	0.27 (1)
H- C	-1730 / 0	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

GIRDER TYPE: CSIdGirder
START DISTANCE = 0-0
START SPAN CARRIED = 11-3-0
END DISTANCE = 15-6-0
END SPAN CARRIED = 11-3-0
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF TOP CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 36.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.52")
CALCULATED VERT. DEFL.(LL)= L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.52")
CALCULATED VERT. DEFL.(TL)= L/ 938 (0.20")

CSI: TC=0.42/1.00 (B-C:1) , BC=0.76/1.00 (H-I:1) , WB=0.91/1.00 (A-I:1) , SSI=0.39/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.60
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.91 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12095-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G8	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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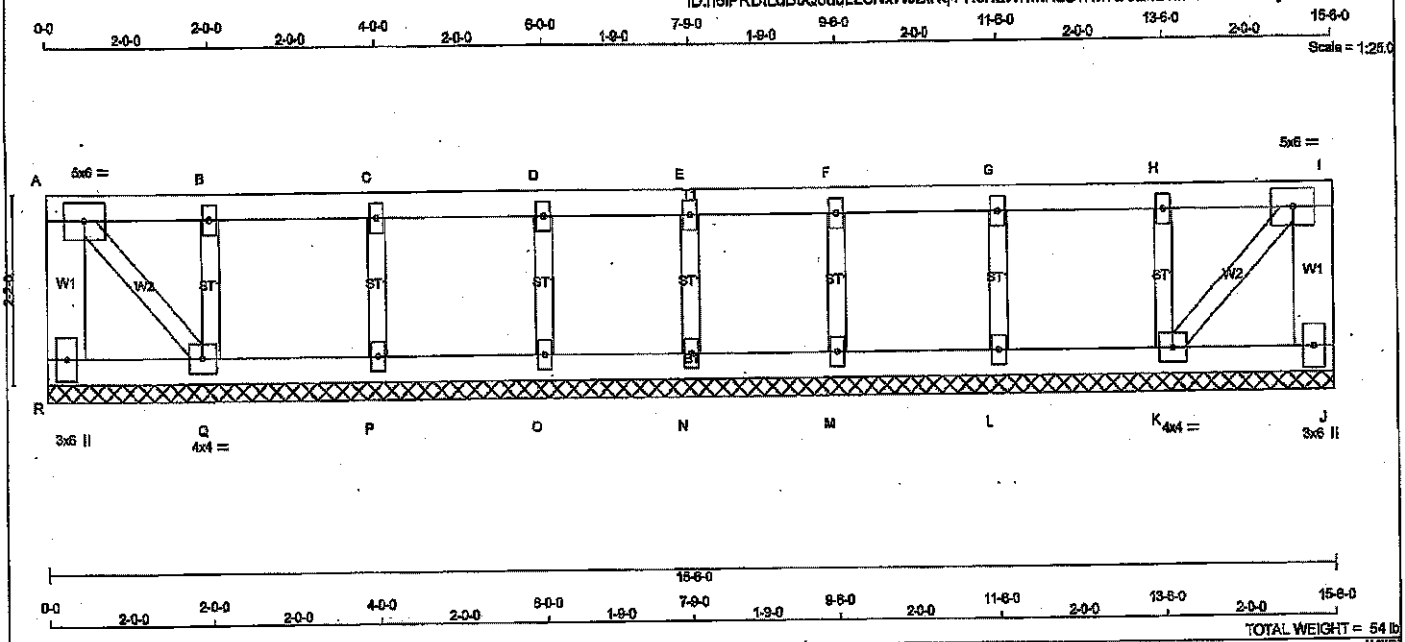

TOTAL WEIGHT = 54 lb

MiTek

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
R - A	2x6	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
R - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	5.0	6.0	
B, C, D, E, F, G, H					
B	TMW+w	MT20	2.0	4.0	
I	TMVW-t	MT20	5.0	6.0	
J	BMV1+p	MT20	3.0	6.0	
K	BMWW1-t	MT20	4.0	4.0	
L, M, N, O, P					
L	BMW1+w	MT20	2.0	4.0	
Q	BMWW1-t	MT20	4.0	4.0	
R	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
R- A	-97 / 0	0.0	0.0	0.01 (1)	7.61	
A- B	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
B- C	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
C- D	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
D- E	0 / 5	-127.4	-127.4	0.07 (1)	10.00	
E- F	0 / 5	-127.4	-127.4	0.07 (1)	10.00	
F- G	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
G- H	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
H- I	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
J- I	-97 / 0	0.0	0.0	0.01 (1)	7.61	
R- Q	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
Q- P	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
P- O	-5 / 0	-28.0	-28.0	0.03 (3)	10.00	
O- N	-5 / 0	-28.0	-28.0	0.03 (3)	10.00	
N- M	-5 / 0	-28.0	-28.0	0.03 (3)	10.00	
M- L	-5 / 0	-28.0	-28.0	0.03 (3)	10.00	
L- K	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
K- J	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N- E	-213 / 0	0.04 (1)
K- H	-286 / 0	0.05 (1)
L- G	-251 / 0	0.05 (1)
M- F	-242 / 0	0.05 (1)
Q- B	-286 / 0	0.05 (1)
P- C	-251 / 0	0.05 (1)
O- D	-242 / 0	0.05 (1)
A- Q	-7 / 0	0.00 (1)
K- I	-7 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (H-I:1) , BC=0.04/1.00 (K-L:3) , WB=0.06/1.00 (H-K:1) , SSI=0.16/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1856

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.23 (K) (INPUT = 0.90)
SSI METAL= 0.07 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12/10 -8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T10	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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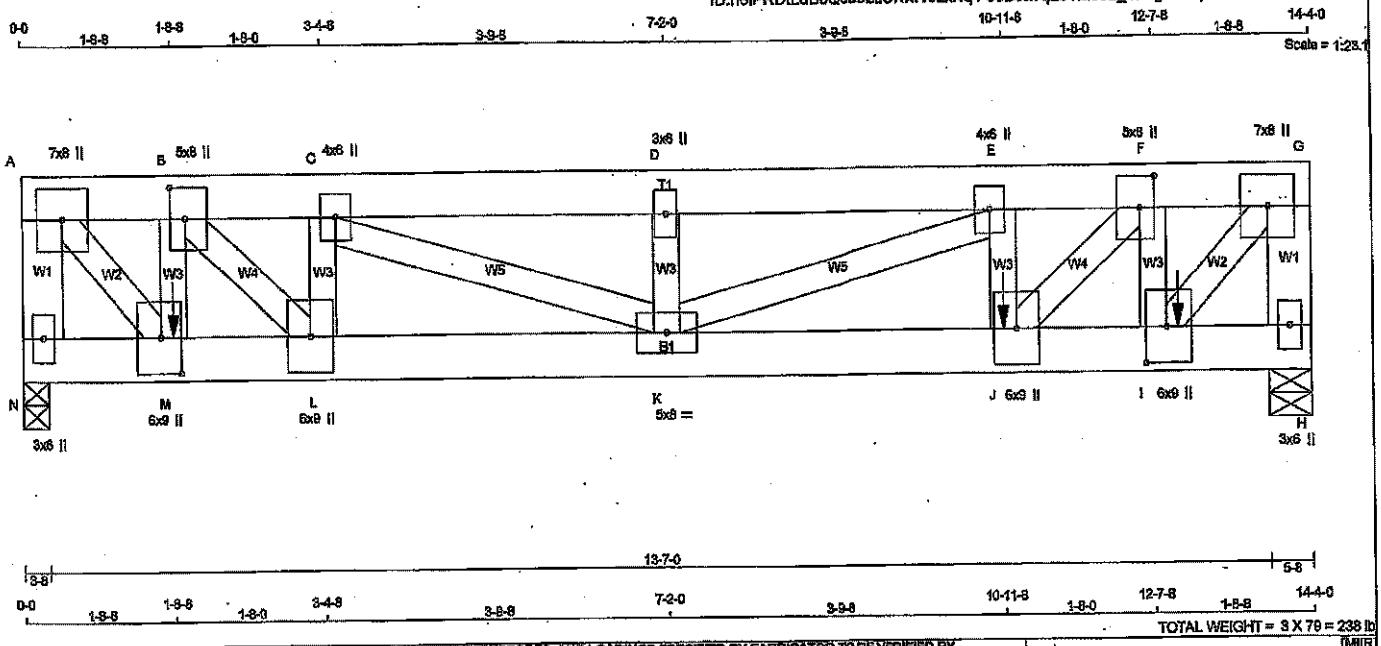

TOTAL WEIGHT = 3 X 79 = 238 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
N- A	2	12	TOP
A- G	2	12	TOP
G- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- H	3	4	SIDE(1167.4)
WEBS : (0.122"X3") SPIRAL NAILS			
B- M	2	4	SIDE(175.0)
E- J	2	4	SIDE(175.0)
2x4	2	4	
M- A	1	6	
L- B	1	6	
K- C	1	6	
D- K	1	6	
K- E	1	6	
J- F	1	6	
I- G	1	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	7.0	6.0		
B	TMWW+t	MT20	5.0	6.0	4.00	2.00
C	TMWW+t	MT20	4.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMWW+t	MT20	4.0	6.0		
F	TMWW+t	MT20	5.0	6.0	4.00	2.00
G	TMWW+p	MT20	7.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	6.0	9.0	4.50	2.75
J	BMWW+t	MT20	6.0	9.0		
K	BMWWW-t	MT20	5.0	8.0		
L	BMWW+t	MT20	6.0	9.0		
M	BMWW+t	MT20	6.0	9.0	4.50	2.75
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	6794	0	6794	0	0	3-8	3-8
H	6427	0	6427	0	0	5-8	5-8

UNFACTORED REACTIONS
MAX./MIN. COMPONENT REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	5297	2762 / 0	689 / 0	0 / 0	0 / 0	1845 / 0	0 / 0
H	5026	2599 / 0	674 / 0	0 / 0	0 / 0	1752 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.87 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-6472 / 0	0.0	0.0	0.20 (1)	6.19	
A- B	-6110 / 0	-127.4	-127.4	0.16 (1)	4.93	
B- C	-8007 / 0	-127.4	-127.4	0.19 (1)	4.39	
C- D	-10125 / 0	-127.4	-127.4	0.17 (1)	4.00	
D- E	-10125 / 0	-127.4	-127.4	0.28 (1)	3.94	
E- F	-10883 / 0	-127.4	-127.4	0.21 (1)	3.67	
F- G	-8060 / 0	-127.4	-127.4	0.12 (1)	4.99	
H- G	-6330 / 0	0.0	0.0	0.19 (1)	6.24	
N- M	0 / 0	-28.0	-28.0	0.15 (1)	10.00	
M- L	0 / 6110	-28.0	-28.0	0.47 (1)	10.00	
L- K	0 / 8007	-28.0	-28.0	0.48 (1)	10.00	
K- J	0 / 10883	-28.0	-28.0	0.71 (1)	10.00	
J- I	0 / 8060	-28.0	-28.0	0.47 (1)	10.00	
I- H	0 / 0	-28.0	-28.0	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX.-FACTORED FORCE (LBS)	MAX CSI (LC)
A- M	0 / 8641	0.62 (1)
M- B	-1643 / 0	0.06 (1)
B- L	0 / 2717	0.19 (1)
L- C	-1562 / 0	0.07 (1)
C- K	0 / 2328	0.17 (1)
K- D	-533 / 0	0.02 (1)
K- E	-612 / 0	0.05 (1)
J- E	-6 / 48	0.00 (3)
J- F	0 / 6620	0.47 (1)
I- F	-4815 / 0	0.22 (1)
I- G	0 / 8570	0.61 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	12-10-12	-1037	-1037	—	BACK	VERT	TOTAL
J	10-11-8	-4978	-4978	—	BACK	VERT	TOTAL
M	1-8-8	-4978	-4978	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBOC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.48")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.48")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.23/1.00 (D-E:1) , BC=0.71/1.00 (J-K:1) , WB=0.62/1.00 (A-M:1) , SSI=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (A) (INPUT = 0.90)
JSI METAL= 0.63 (L) (INPUT = 1.00)

DWG NO. TAM 120960

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T10	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 4978.0 lbs FACTORED DOWN AT 1-8-8, AND 4978.0 lbs FACTORED DOWN AT 10-11-8, AND 1037.2 lbs FACTORED DOWN AT 12-10-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
3518
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12096-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T11	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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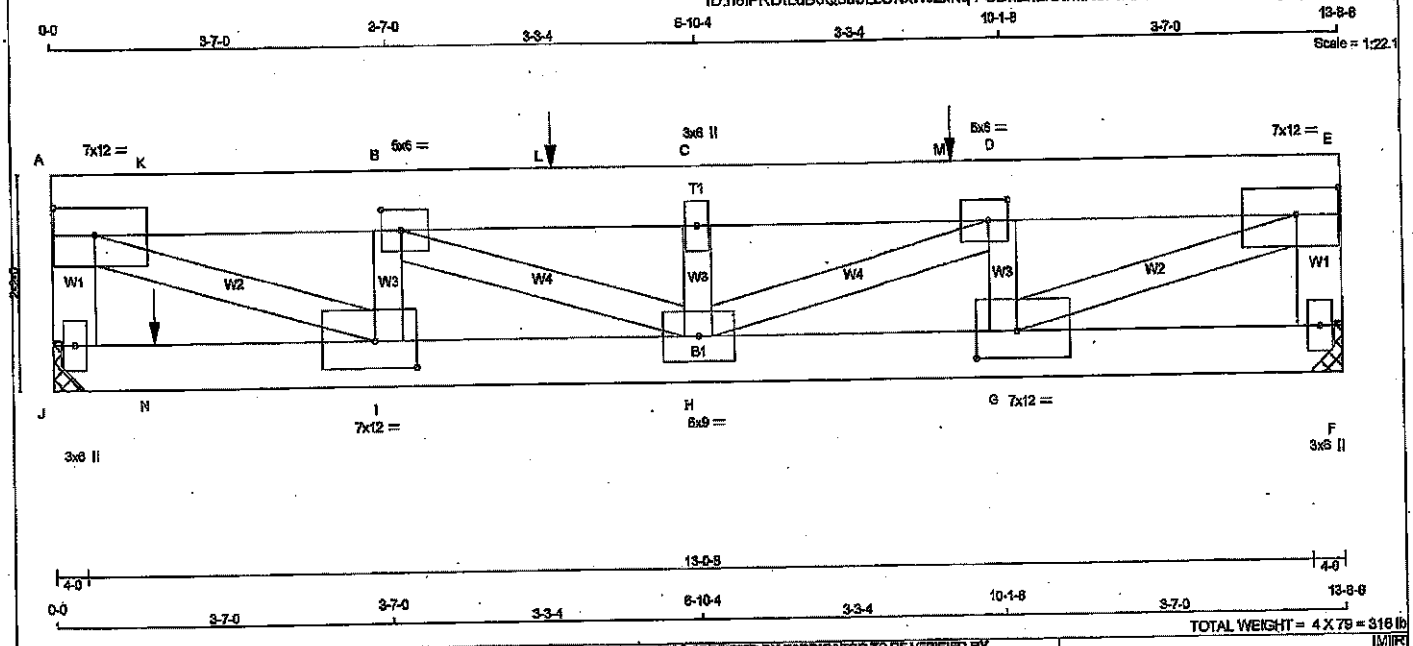

TOTAL WEIGHT = 4 X 79 = 316 lb

[M]R

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	No.2	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
G - E	2x4	DRY	2100F 1.8E	SPF
A - I	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J-A	2	12	TOP
E-F	2	12	TOP
A-E	2	12	SIDE(143.8)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-F	2	12	SIDE(56.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	7.0	12.0	3.25	5.25
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TMW+w	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	7.0	12.0	3.25	5.25
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MT20	7.0	12.0	3.25	5.25
H	BMWWW-t	MT20	6.0	9.0		
I	BMWW-t	MT20	7.0	12.0	3.25	5.25
J	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1402.5 lbs FACTORED DOWN AT 5-4-0, AND 1402.5 lbs FACTORED DOWN AT 9-7-0 ON TOP CHORD, AND 671.3 lbs FACTORED DOWN AT 1-1-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	5006	0	5006	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	
F	5038	0	5038	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	3890	2046 / 0	491 / 0	0 / 0	0 / 0	1353 / 0	0 / 0
F	3903	2070 / 0	477 / 0	0 / 0	0 / 0	1358 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.76 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-4415 / 0	0.0	0.0	0.21 (1)	6.08
A-K	-8699 / 0	-127.4	-127.4	0.29 (1)	3.88
K-B	-8699 / 0	-414.9	-414.9	0.29 (1)	3.88
B-L	-11258 / 0	-414.9	-414.9	0.73 (1)	2.76
L-C	-11258 / 0	-414.9	-414.9	0.73 (1)	2.76
C-M	-11258 / 0	-414.9	-414.9	0.52 (1)	3.17
M-D	-11258 / 0	-414.9	-414.9	0.52 (1)	3.17
D-E	-6900 / 0	-414.9	-414.9	0.34 (1)	3.76
F-E	-4806 / 0	0.0	0.0	0.23 (1)	5.87
J-N	0 / 0	-28.0	-28.0	0.17 (1)	10.00
N-I	0 / 0	-92.0	-92.0	0.17 (1)	10.00
I-H	0 / 8899	-92.0	-92.0	0.42 (1)	10.00
H-G	0 / 8900	-92.0	-92.0	0.44 (1)	10.00
G-F	0 / 0	-92.0	-92.0	0.08 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 9783	0.54 (1)
A-I	0 / 9562	0.53 (1)
G-D	-3753 / 0	0.28 (1)
I-S	-3361 / 0	0.25 (1)
H-D	0 / 2636	0.31 (1)
B-H	0 / 2881	0.34 (1)
H-G	-2337 / 0	0.18 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	5-4-0	-413	-462	—	FRONT	VERT	DEAD
L	5-4-0	-990	-990	—	FRONT	VERT	SNOW
M	9-7-0	-413	-462	—	FRONT	VERT	DEAD
M	9-7-0	-990	-990	—	FRONT	VERT	SNOW
N	1-1-0	-671	-671	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 948 (0.17")

CSI: TC=0.73/1.00 (B-C:1) , BC=0.44/1.00 (G-H:1) , WB=0.54/1.00 (E-G:1) , SSI=0.72/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.69 (G) (INPUT = 1.00)

DWG NO. TAM 12097-B

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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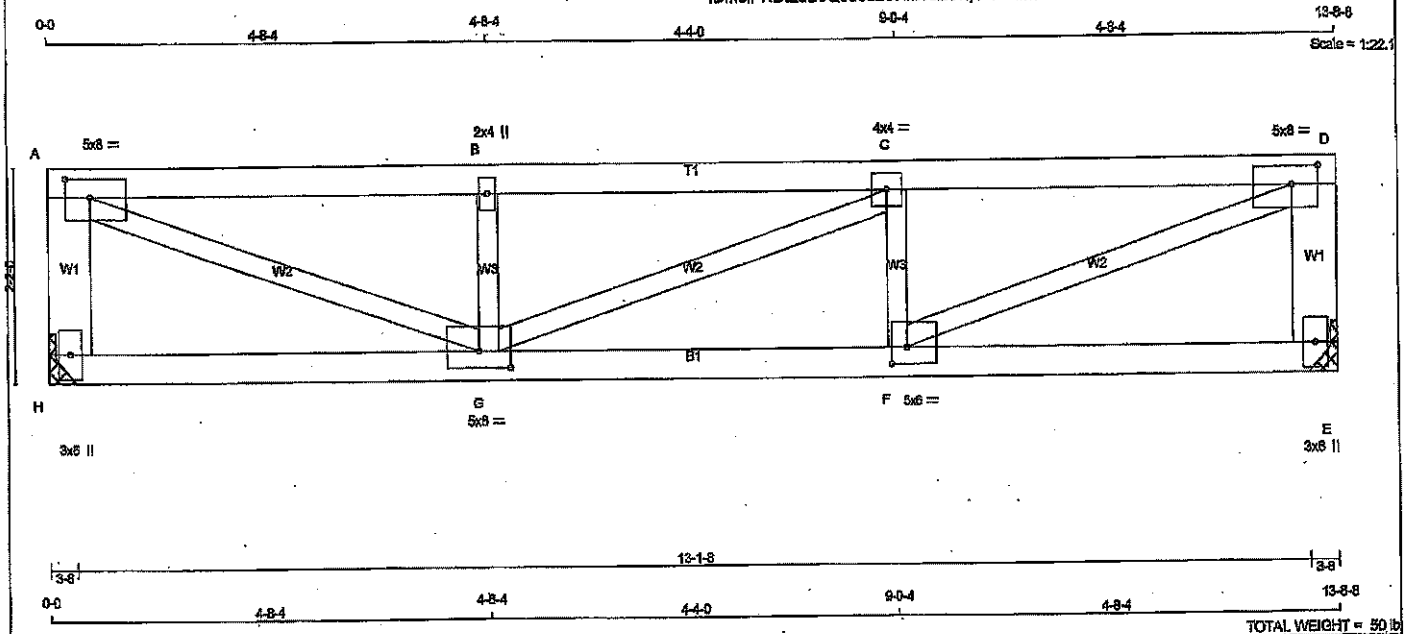

TOTAL WEIGHT = 50 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.25	3.25
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	2.25	3.25
E	BMV1+p	MT20	3.0	6.0		
F	BMWW-t	MT20	5.0	6.0	2.00	2.00
G	BMWWW-t	MT20	5.0	8.0	2.00	4.00
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1065	0	1065	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
E	1065	0	1065	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	856	411 / 0	144 / 0	0 / 0	0 / 0	302 / 0	0 / 0
E	856	411 / 0	144 / 0	0 / 0	0 / 0	302 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-A	-1004 / 0	0.0	0.0	0.10 (1)	7.81	
A-B	-1865 / 0	-127.4	-127.4	0.57 (1)	3.71	
B-C	-1886 / 0	-127.4	-127.4	0.53 (1)	3.76	
C-D	-1869 / 0	-127.4	-127.4	0.56 (1)	3.71	
E-D	-1005 / 0	0.0	0.0	0.10 (1)	7.81	
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
G-F	0 / 1889	-28.0	-28.0	0.48 (1)	10.00	
F-E	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	0 / 2034	0.61 (1)
A-G	0 / 2031	0.61 (1)
F-C	-623 / 0	0.12 (1)
G-B	-623 / 0	0.12 (1)
G-C	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 899 (0.13")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.48/1.00 (F-G:1) , WB=0.61/1.00 (D-F:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.69 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3518
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12098-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T13	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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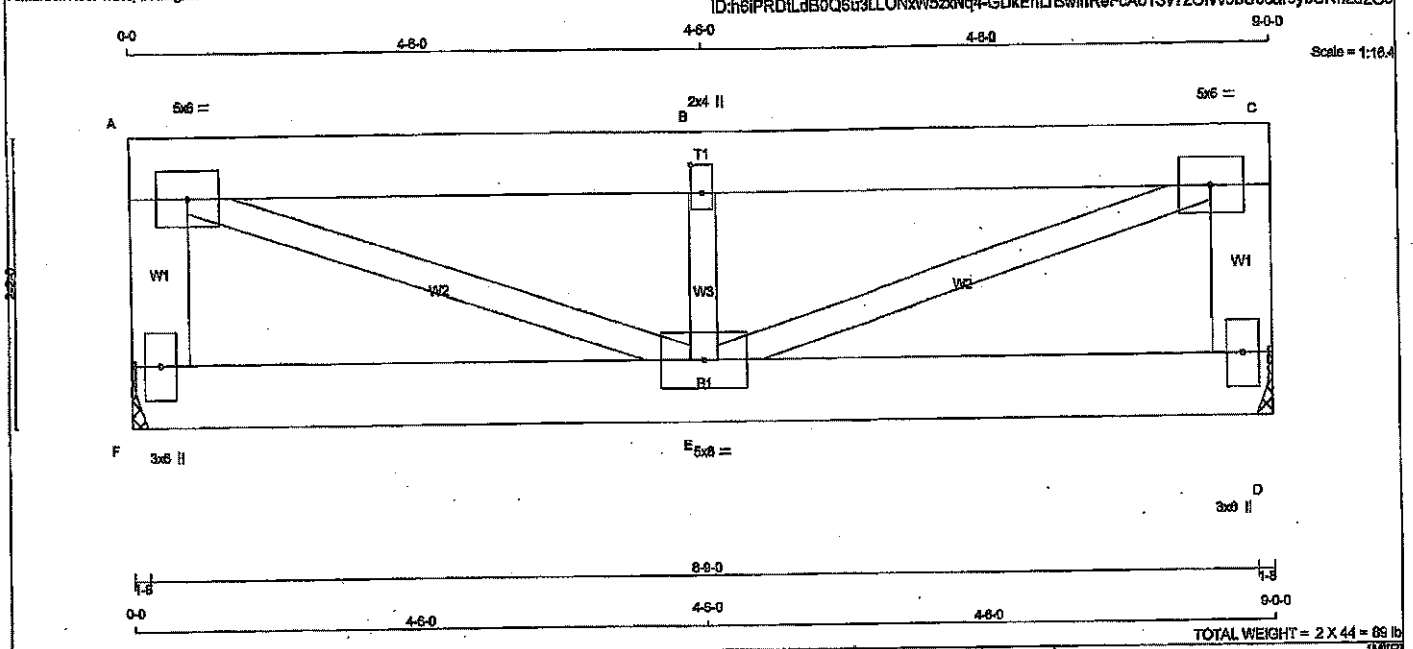

TOTAL WEIGHT = 2 X 44 = 89 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x6	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x6	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F-A	2	12	TOP
A-C	2	12	TOP
C-D	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		
B	TMW+w	MT20	2.0	4.0	2.50	1.00
C	TMWW-t	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMWWW-t	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	699	0	699	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	
D	699	0	699	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 1-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	562	270 / 0	95 / 0	0 / 0	0 / 0	196 / 0	0 / 0
D	562	270 / 0	95 / 0	0 / 0	0 / 0	196 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-626 / 0	0.0	0.0	0.03 (1)	7.81
A-B	-1012 / 0	-127.4	-127.4	0.10 (1)	6.25
B-C	-1012 / 0	-127.4	-127.4	0.10 (1)	6.25
D-C	-626 / 0	0.0	0.0	0.03 (1)	7.81
F-E	0 / 0	-28.0	-28.0	0.05 (3)	10.00
E-D	0 / 0	-28.0	-28.0	0.05 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-E	0 / 1082	0.16 (1)
E-B	-663 / 0	0.09 (1)
E-C	0 / 1082	0.16 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL. =	30.0	PSF
	DL. =	15.0	PSF
BOT CH.	LL. =	10.5	PSF
	DL. =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.9 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.10/1.00 (A-B:1) , BC=0.05/1.00 (E-F:3) , WB=0.16/1.00 (A-E:1) , SSI=0.13/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI: GRIP= 0.52 (E) (INPUT = 0.80)
CSI: METAL= 0.17 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3518
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 20099-08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T14	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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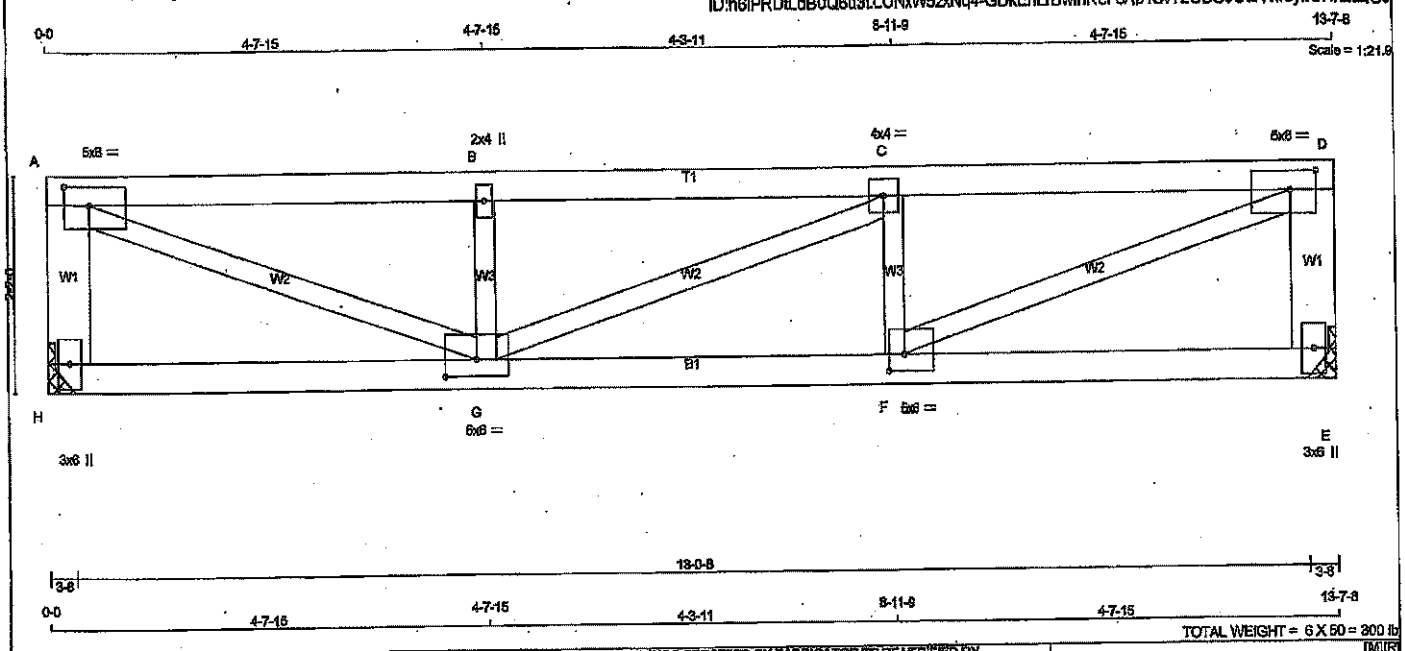

TOTAL WEIGHT = 6 X 50 = 300 lb

[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.25	3.25
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	2.25	3.25
E	BMV1+p	MT20	3.0	6.0		
F	BMWW-t	MT20	5.0	6.0	2.00	2.00
G	BMWWW-t	MT20	5.0	8.0	2.00	4.00
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1059	0	1059	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	
E	1059	0	1059	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	851	408 / 0	143 / 0	0 / 0	0 / 0	300 / 0	0 / 0
E	851	408 / 0	143 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-A	-996 / 0	0.0	0.0	0.10 (1)	7.91	
A-B	-1863 / 0	-127.4	-127.4	0.56 (1)	3.74	
B-C	-1863 / 0	-127.4	-127.4	0.62 (1)	3.79	
C-D	-1866 / 0	-127.4	-127.4	0.56 (1)	3.74	
E-D	-999 / 0	0.0	0.0	0.10 (1)	7.91	
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
G-F	0 / 1888	-28.0	-28.0	0.47 (1)	10.00	
F-E	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	0 / 2011	0.80 (1)
A-G	0 / 2009	0.80 (1)
F-C	-819 / 0	0.12 (1)
G-B	-819 / 0	0.12 (1)
G-C	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/989 (0.07")
ALLOWABLE DEFL.(TL)= L/350 (0.45")
CALCULATED VERT. DEFL.(TL) = L/969 (0.13")

CSI: TC=0.56/1.00 (A-B:1) , BC=0.47/1.00 (F-G:1) , WB=0.80/1.00 (D-F:1) , SSI=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1657	622	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.62 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12100 -08
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T15	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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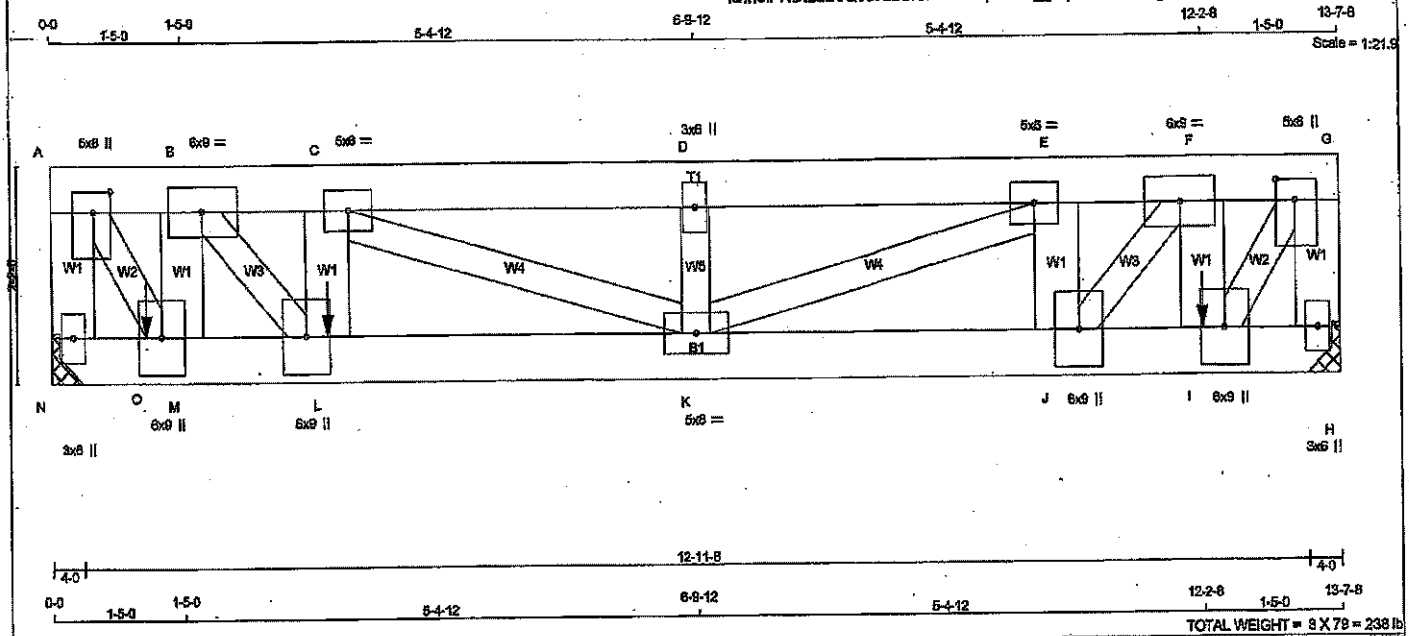

TOTAL WEIGHT = 3 X 79 = 238 lb

(MIR)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
N - A	2x6	DRY	No.2	SPF
A - G	2x6	DRY	No.2	SPF
H - G	2x6	DRY	No.2	SPF
N - H	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
M - B	2x6	DRY	No.2	SPF
I - F	2x6	DRY	No.2	SPF
L - C	2x6	DRY	No.2	SPF
J - E	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
N- A	2	12	TOP
A- G	2	12	TOP
G- H	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- H	3	4	SIDE(1187.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
F- I	3	5	SIDE(188.3)
G- L	3	5	SIDE(188.3)
2x6	3	5	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	5.0	8.0	2.50	2.25
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	5.0	6.0		
D	TMW+w	MT20	3.0	6.0		
E	TMWW-t	MT20	5.0	6.0		
F	TMWW-t	MT20	6.0	9.0		
G	TMWW+p	MT20	5.0	8.0	2.50	2.25
H	BMV1+p	MT20	3.0	6.0		
I, J, L, M						
I	BMWW+t	MT20	6.0	9.0		
K	BMWWW-t	MT20	5.0	8.0		
N	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	6492	0	6492	0	0	IN-SX	IN-SX
						HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	
H	6714	0	6714	0	0	HANGER BY OTHERS	
						MIN. SEAT SIZE: 4-0	

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	5041	2624 / 0	661 / 0	0 / 0	0 / 0	1756 / 0	0 / 0
H	5218	2743 / 0	660 / 0	0 / 0	0 / 0	1816 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
N- A	-8143 / 0	0.0	0.0	0.19 (1)	6.32	
A- B	-4890 / 0	-127.4	-127.4	0.09 (1)	5.45	
B- C	-9186 / 0	-127.4	-127.4	0.16 (1)	4.17	
C- D	-8734 / 0	-127.4	-127.4	0.19 (1)	4.23	
D- E	-8734 / 0	-127.4	-127.4	0.14 (1)	4.29	
E- F	-8707 / 0	-127.4	-127.4	0.18 (1)	4.74	
F- G	-4923 / 0	-127.4	-127.4	0.15 (1)	5.37	
H- G	-8322 / 0	0.0	0.0	0.19 (1)	6.25	
N- O	0 / 0	-28.0	-28.0	0.09 (1)	10.00	
O- M	0 / 0	-28.0	-28.0	0.09 (1)	10.00	
M- L	0 / 4890	-28.0	-28.0	0.39 (1)	10.00	
L- K	0 / 9186	-28.0	-28.0	0.62 (1)	10.00	
K- J	0 / 8707	-28.0	-28.0	0.40 (1)	10.00	
J- I	0 / 4923	-28.0	-28.0	0.41 (1)	10.00	
I- H	0 / 0	-28.0	-28.0	0.16 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- M	0 / 7660	0.55 (1)
M- B	-4828 / 0	0.14 (1)
I- F	-1572 / 0	0.05 (1)
I- G	0 / 7713	0.55 (1)
L- C	-44 / 24	0.00 (3)
J- E	-1557 / 0	0.05 (1)
K- D	-538 / 0	0.02 (1)
B- L	0 / 6413	0.46 (1)
C- K	-494 / 0	0.04 (1)
K- E	0 / 2218	0.16 (1)
J- F	0 / 2662	0.19 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	12-2-8	-5010	-5010	—	BACK	VERT	TOTAL
L	2-11-8	-5010	-5010	—	BACK	VERT	TOTAL
O	1-0-4	-1037	-1037	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.45")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.45")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.19/1.00 (C-D:1) , BC=0.62/1.00 (K-L:1) , WB=0.55/1.00 (G-I:1) , SSI=0.20/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.63 (J) (INPUT = 1.00)

P6 / 6

DWG NO. TAM M01 -18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T15	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1037.2 lbs FACTORED DOWN AT 1-0-4, AND 5010.5 lbs FACTORED DOWN AT 2-11-8, AND 5010.5 lbs FACTORED DOWN AT 12-2-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12101-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G16	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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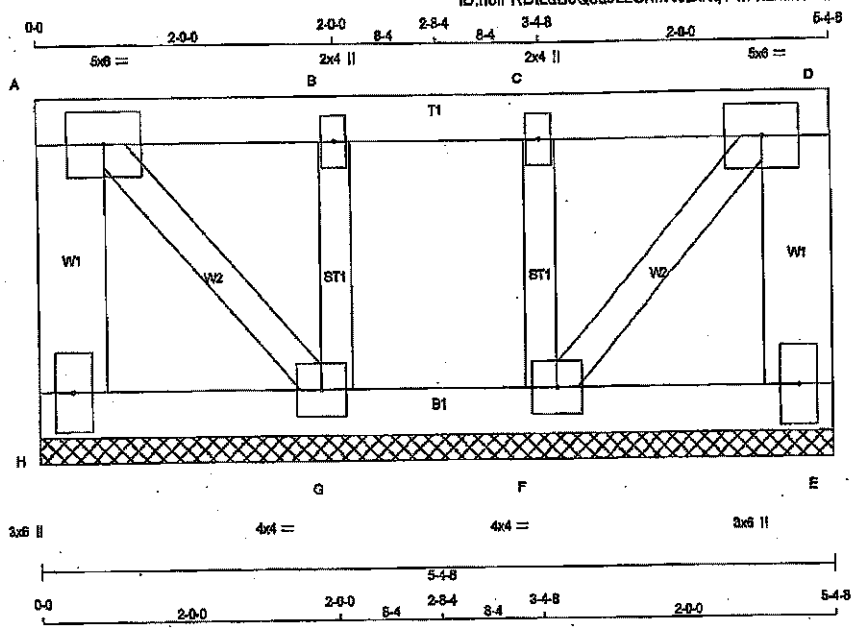

TOTAL WEIGHT = 24 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMWW-t	MT20	5.0	6.0	
B	TMW+w	MT20	2.0	4.0	
C	TMW+w	MT20	2.0	4.0	
D	TMWW-t	MT20	5.0	6.0	
E	BMV1+p	MT20	3.0	6.0	
F	BMWW1-t	MT20	4.0	4.0	
G	BMWW1-t	MT20	4.0	4.0	
H	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-A	-98 / 0	0.0	0.0	0.01 (1)	7.81	
A-B	0 / 10	-127.4	-127.4	0.08 (1)	10.00	
B-C	0 / 10	-127.4	-127.4	0.08 (1)	10.00	
C-D	0 / 10	-127.4	-127.4	0.08 (1)	10.00	
E-D	-98 / 0	0.0	0.0	0.01 (1)	7.81	
H-G	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
G-F	-10 / 0	-28.0	-28.0	0.03 (3)	6.25	
F-E	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	-235 / 0	0.05 (1)
G-B	-235 / 0	0.05 (1)
A-G	-14 / 0	0.00 (1)
F-D	-14 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN.O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (A-B:1) , BC=0.04/1.00 (G-H:3) , WB=0.05/1.00 (B-G:1) , SSI=0.15/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (F) (INPUT = 0.80)
JSI METAL= 0.06 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12117-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T17	1	3	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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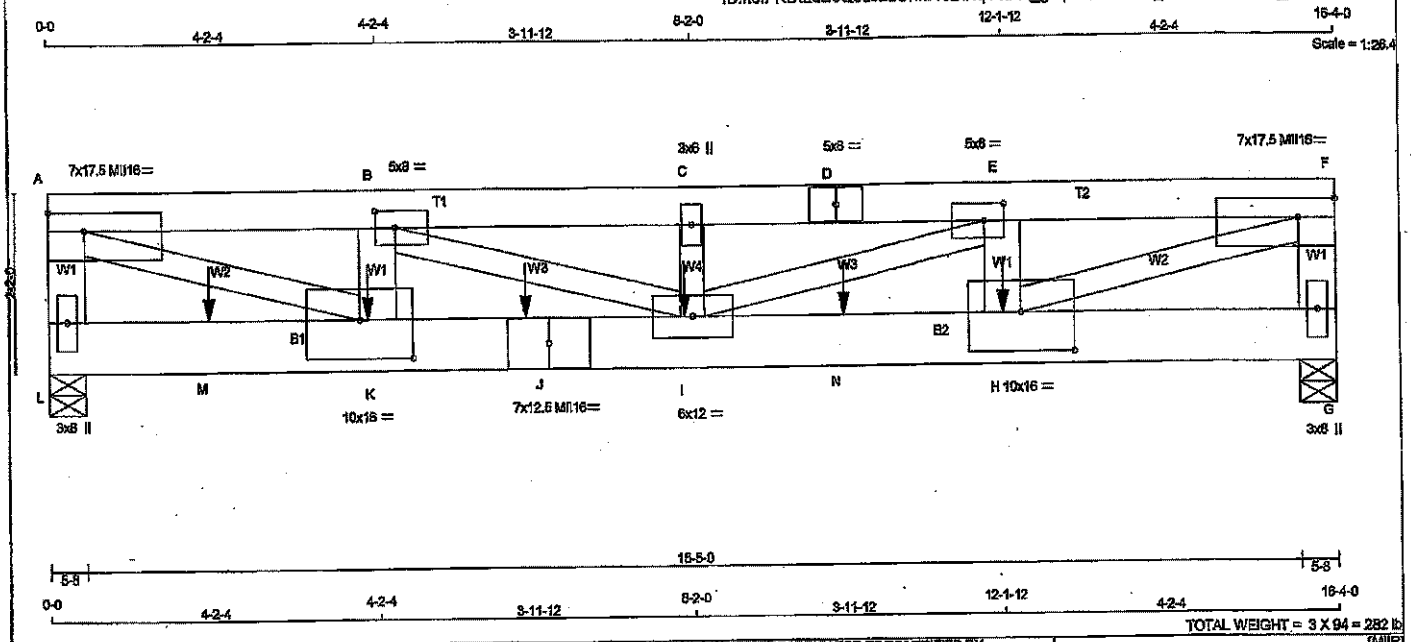

TOTAL WEIGHT = 3 X 94 = 282 lb (MIR)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x6	DRY	1950F 1.7E	SPF
J - G	2x6	DRY	1950F 1.7E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
A - K	2x4	DRY	2100F 1.8E	SPF
K - B	2x6	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
H - F	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
L- A	2	12	TOP
A- D	2	12	TOP
D- F	2	12	TOP
F- G	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
L- J	4	4	SIDE(1579.4)
J- G	4	4	SIDE(1579.4)
WEBS : (0.122"X3") SPIRAL NAILS			
2x4	1	6	
2x6	2	4	
E- H	2	4	SIDE(131.9)

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MII16	7.0	17.5	2.75	Edge
B	TMWW-t	MT20	5.0	8.0	2.50	3.00
C	TMW+w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	8.0		
E	TMWW-t	MT20	5.0	8.0	2.50	3.00
F	TMWW-t	MII16	7.0	17.5	2.75	Edge
G	BMV1+p	MT20	3.0	8.0		
H	BMWW-t	MT20	10.0	16.0	5.50	8.00
I	BMWWW-t	MT20	6.0	12.0		
J	BS-t	MII16	7.0	12.5		
K	BMWW-t	MT20	10.0	16.0	5.50	8.00
L	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	6128	0	6128	0	0	5-8	5-8
G	7999	0	7999	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	4844	2433 / 0	715 / 0	0 / 0	0 / 0	1695 / 0	0 / 0
G	6284	3212 / 0	878 / 0	0 / 0	0 / 0	2194 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- A	-5467 / 0	0.0	0.0	0.16 (1)	6.62	
A- B	-13237 / 0	-127.4	-127.4	0.28 (1)	3.46	
B- C	-19079 / 0	-127.4	-127.4	0.53 (1)	2.67	
C- D	-19079 / 0	-127.4	-127.4	0.55 (1)	2.59	
D- E	-19079 / 0	-127.4	-127.4	0.58 (1)	2.59	
E- F	-18167 / 0	-127.4	-127.4	0.51 (1)	2.75	
G- F	-7467 / 0	0.0	0.0	0.22 (1)	5.83	
L- M	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
M- K	0 / 0	-28.0	-28.0	0.13 (1)	10.00	
K- J	0 / 13237	-28.0	-28.0	0.42 (1)	10.00	
J- I	0 / 13237	-28.0	-28.0	0.42 (1)	10.00	
I- N	0 / 18167	-28.0	-28.0	0.59 (1)	10.00	
N- H	0 / 18167	-28.0	-28.0	0.59 (1)	10.00	
H- G	-0 / 0	-28.0	-28.0	0.19 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- K	0 / 14211	0.47 (1)
K- B	-2984 / 0	0.09 (1)
B- I	0 / 6316	0.45 (1)
I- C	-461 / 0	0.02 (1)
I- E	0 / 988	0.07 (1)
H- E	-743 / 0	0.02 (1)
H- F	0 / 19503	0.65 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
H	12-1-12	-6434	-6434	—	FRONT	VERT	TOTAL
I	8-0-12	-1031	-1031	—	FRONT	VERT	TOTAL
J	8-0-12	-1031	-1031	—	FRONT	VERT	TOTAL
K	4-0-12	-1031	-1031	—	FRONT	VERT	TOTAL
M	2-0-12	-1031	-1031	—	FRONT	VERT	TOTAL
N	10-0-12	-1031	-1031	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.54")
CALCULATED VERT. DEFL.(LL) = L/899 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.54")
CALCULATED VERT. DEFL.(TL) = L/651 (0.20")

CSI: TC=0.58/1.00 (C-E:1) , BC=0.59/1.00 (H-I:1) , WB=0.65/1.00 (F-H:1) , SSI=0.20/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.98 (K) (INPUT = 1.00)

DWG NO. TAM P102-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T17	1	3	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1030.7 lbs FACTORED DOWN AT 2-0-12, 1030.7 lbs FACTORED DOWN AT 4-0-12, 1030.7 lbs FACTORED DOWN AT 6-0-12, 1030.7 lbs FACTORED DOWN AT 8-0-12, AND 1030.7 lbs FACTORED DOWN AT 10-0-12, AND 6433.7 lbs FACTORED DOWN AT 12-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
25B
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 121028
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T20	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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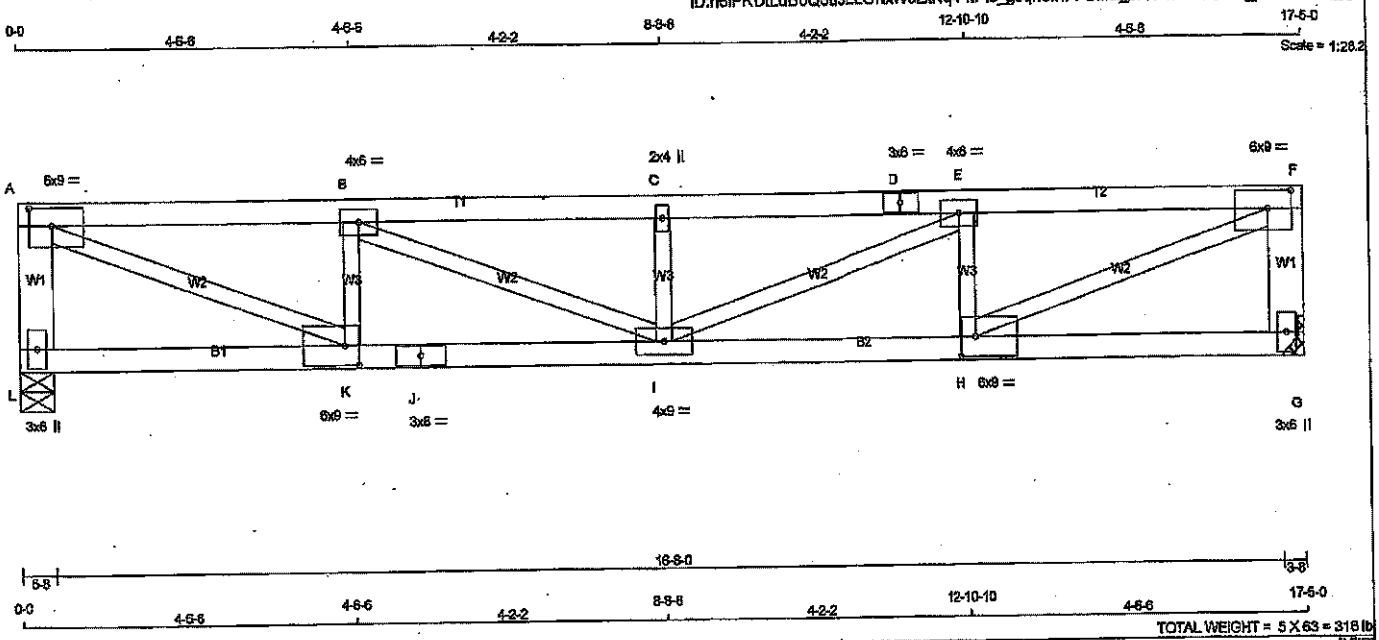

TOTAL WEIGHT = 5 X 63 = 318 lb

[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x6	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	6.0	9.0	2.75	3.75
B	TMWW-t	MT20	4.0	6.0		
C	TMWHw	MT20	2.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	6.0		
F	TMWW-t	MT20	6.0	9.0	2.75	3.75
G	BMV1+p	MT20	3.0	6.0		
H	BMWW-t	MT20	6.0	9.0	3.00	2.25
I	BMWWW-t	MT20	4.0	9.0		
J	BS-t	MT20	3.0	8.0		
K	BMWW-t	MT20	6.0	9.0	3.00	2.25
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1353	0	1353	0	0	5-8	5-8
G	1353	0	1353	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1088	522 / 0	183 / 0	0 / 0	0 / 0	383 / 0	0 / 0
G	1089	522 / 0	183 / 0	0 / 0	0 / 0	383 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASE(S): (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
L-A	-1294 / 0	0.0	0.0	0.13 (1)	7.55	
A-B	-2550 / 0	-127.4	-127.4	0.61 (1)	3.20	
B-C	-3202 / 0	-127.4	-127.4	0.69 (1)	2.74	
C-D	-3202 / 0	-127.4	-127.4	0.69 (1)	2.74	
D-E	-3202 / 0	-127.4	-127.4	0.69 (1)	2.74	
E-F	-2550 / 0	-127.4	-127.4	0.61 (1)	3.20	
G-F	-1294 / 0	0.0	0.0	0.13 (1)	7.55	
L-K	0 / 0	-28.0	-28.0	0.19 (3)	10.00	
K-J	0 / 2550	-28.0	-28.0	0.64 (1)	10.00	
J-I	0 / 2550	-28.0	-28.0	0.64 (1)	10.00	
I-H	0 / 2550	-28.0	-28.0	0.64 (1)	10.00	
H-G	0 / 0	-28.0	-28.0	0.19 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-F	0 / 2760	0.83 (1)
A-K	0 / 2760	0.83 (1)
H-E	-925 / 0	0.19 (1)
K-B	-925 / 0	0.19 (1)
I-E	0 / 715	0.21 (1)
B-I	0 / 715	0.21 (1)
I-C	-478 / 0	0.10 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.56")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.56")
CALCULATED VERT. DEFL.(TL) = L/ 807 (0.26")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.64/1.00 (I-K:1) , WB=0.83/1.00 (A-K:1) , SSI=0.35/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.92 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12103-8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T21	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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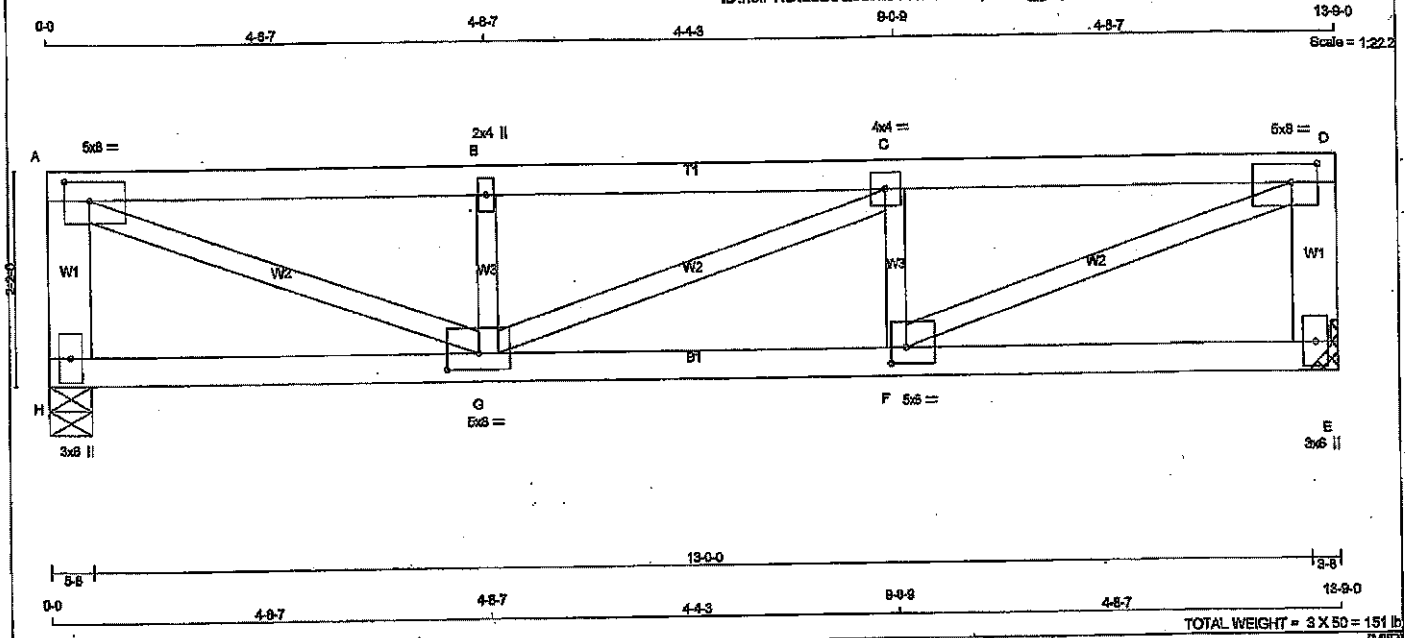

TOTAL WEIGHT = 3 X 50 = 151 lb

[MIR]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.25	3.25
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	2.25	3.25
E	BMV1+p	MT20	3.0	6.0		
F	BMWWW-t	MT20	5.0	6.0	2.00	2.00
G	BMWWW-t	MT20	5.0	8.0	2.00	4.00
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1068	0	1068	0	0	5-8	5-8
E	1068	0	1068	0	0	HANGER BY OTHERS MIN. SEAT SIZE: 3-8	

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	859	412 / 0	144 / 0	0 / 0	0 / 0	303 / 0	0 / 0
E	859	412 / 0	144 / 0	0 / 0	0 / 0	303 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-A	-1008 / 0	0.0	0.0	0.10 (1)	7.81	
A-B	-1898 / 0	-127.4	-127.4	0.57 (1)	3.69	
B-C	-1898 / 0	-127.4	-127.4	0.53 (1)	3.74	
C-D	-1900 / 0	-127.4	-127.4	0.57 (1)	3.69	
E-D	-1008 / 0	0.0	0.0	0.10 (1)	7.81	
H-G	0 / 0	-28.0	-28.0	0.21 (3)	10.00	
G-F	0 / 1900	-28.0	-28.0	0.48 (1)	10.00	
F-E	0 / 0	-28.0	-28.0	0.21 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	0 / 2046	0.61 (1)
A-G	0 / 2043	0.61 (1)
F-C	-625 / 0	0.13 (1)
G-B	-625 / 0	0.13 (1)
G-C	-3 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 **IN. C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 969 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.48/1.00 (F-G:1) , WB=0.61/1.00 (D-F:1) , SS1=0.36/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.63 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 17104-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G21	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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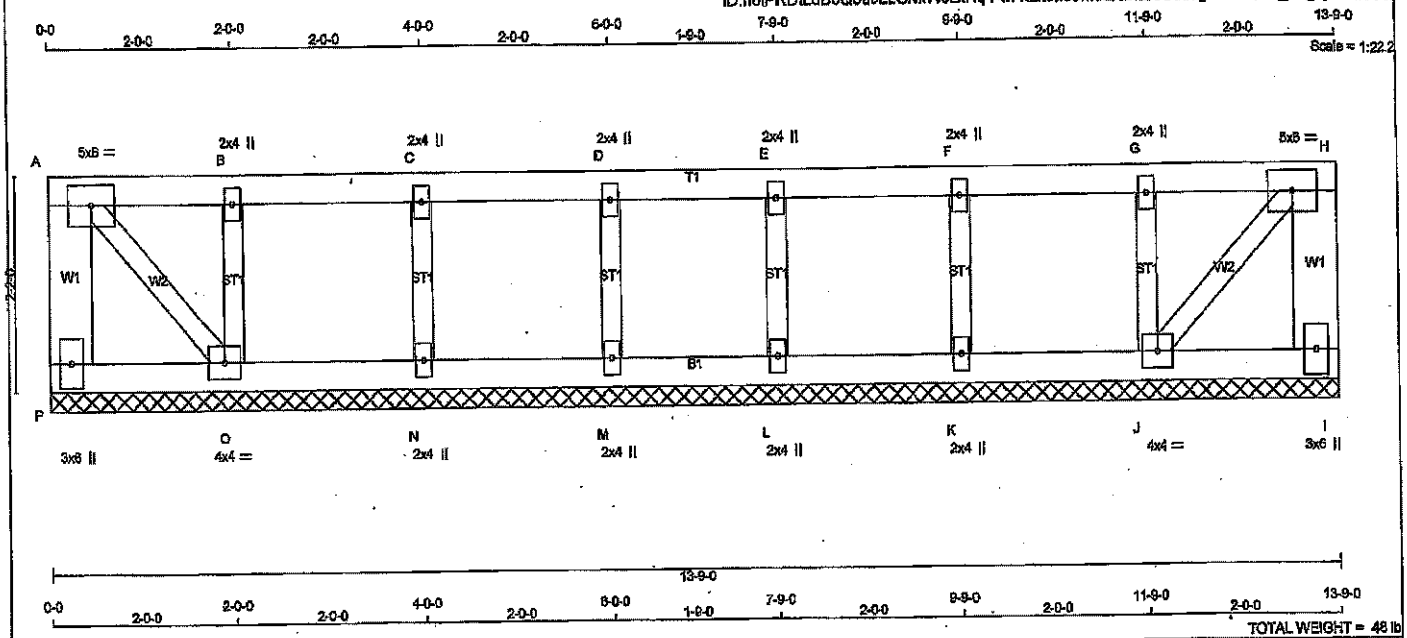

TOTAL WEIGHT = 48 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
P - A	2x6	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
I - H	2x6	DRY	No.2	SPF
P - I	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-t	MT20	5.0	6.0		
B, C, D, E, F, G						
B	TMW+w	MT20	2.0	4.0		
H	TMW-t	MT20	5.0	6.0		
I	BMV1+p	MT20	3.0	6.0		
J	BMW/W1-t	MT20	4.0	4.0		
K, L, M, N						
K	BMW1+w	MT20	2.0	4.0		
O	BMW/W1-t	MT20	4.0	4.0		
P	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
P- A	-96 / 0	0.0	0.0	0.01 (1)	7.81	
A- B	0 / 6	-127.4	-127.4	0.09 (1)	10.00	
B- C	0 / 6	-127.4	-127.4	0.09 (1)	10.00	
C- D	0 / 6	-127.4	-127.4	0.08 (1)	10.00	
D- E	0 / 6	-127.4	-127.4	0.07 (1)	10.00	
E- F	0 / 6	-127.4	-127.4	0.08 (1)	10.00	
F- G	0 / 6	-127.4	-127.4	0.09 (1)	10.00	
G- H	0 / 6	-127.4	-127.4	0.09 (1)	10.00	
I- H	-96 / 0	0.0	0.0	0.01 (1)	7.81	
P- O	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
O- N	-6 / 0	-28.0	-28.0	0.04 (3)	10.00	
N- M	-6 / 0	-28.0	-28.0	0.03 (3)	10.00	
M- L	-6 / 0	-28.0	-28.0	0.03 (3)	10.00	
L- K	-6 / 0	-28.0	-28.0	0.03 (3)	10.00	
K- J	-6 / 0	-28.0	-28.0	0.04 (3)	10.00	
J- I	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- G	-285 / 0	0.06 (1)
K- F	-252 / 0	0.05 (1)
L- E	-237 / 0	0.05 (1)
O- B	-285 / 0	0.06 (1)
N- C	-252 / 0	0.05 (1)
M- D	-237 / 0	0.05 (1)
A- O	-6 / 0	0.00 (1)
J- H	-6 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (F-G:1) , BC=0.04/1.00 (J-K:3) , WB=0.06/1.00 (G-J:1) , SSI=0.16/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (J) (INPUT = 0.90)
JSI METAL= 0.07 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
3318
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12112-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T22	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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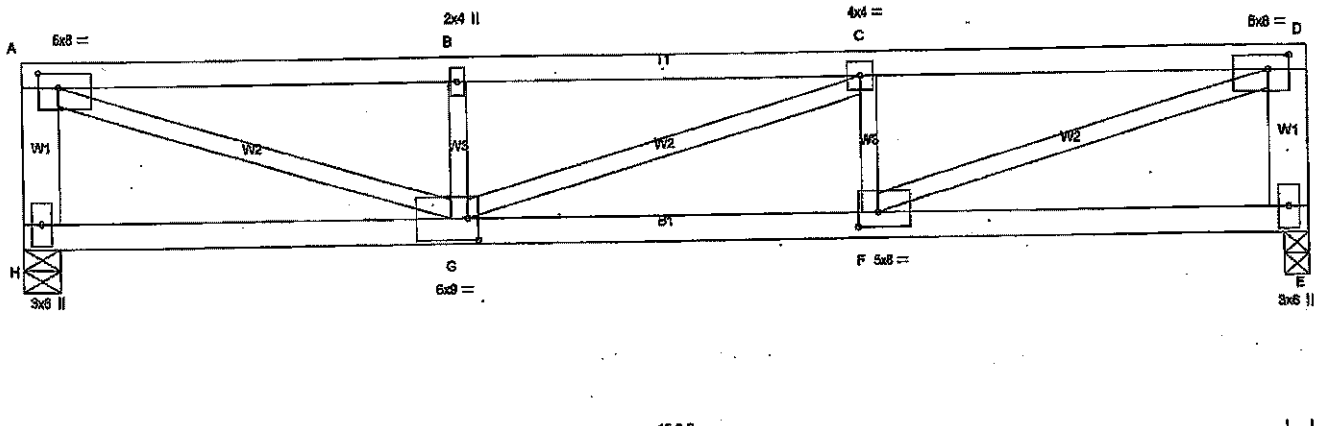

15-0-8

8-8 3-8

0-0 5-4-9 5-4-9 5-0-5 10-4-15 5-4-9 15-9-8

TOTAL WEIGHT = 5 X 57 = 283 lb

[MIR

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	8.0	2.00	3.00
B	TMW+w	MT20	2.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TMWW-t	MT20	5.0	8.0	2.00	3.00
E	BMV1+p	MT20	3.0	6.0		
F	BMWW-t	MT20	5.0	8.0	2.00	3.00
G	BMWWW-t	MT20	6.0	9.0	3.00	1.50
H	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1227	0	1227	0	0	5-8	5-8
E	1227	0	1227	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	986	473 / 0	166 / 0	0 / 0	0 / 0	347 / 0	0 / 0
E	986	473 / 0	166 / 0	0 / 0	0 / 0	347 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
H-A	-1156 / 0	0.0	0.0	0.11 (1)	7.81
A-B	-2502 / 0	-127.4	-127.4	0.85 (1)	2.86
B-C	-2502 / 0	-127.4	-127.4	0.79 (1)	2.93
C-D	-2504 / 0	-127.4	-127.4	0.85 (1)	2.87
E-D	-1156 / 0	0.0	0.0	0.11 (1)	7.81
H-G	0 / 0	-28.0	-28.0	0.27 (3)	10.00
G-F	0 / 2504	-28.0	-28.0	0.64 (1)	10.00
F-E	0 / 0	-28.0	-28.0	0.27 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-D	0 / 2651	0.80 (1)
A-G	0 / 2649	0.79 (1)
F-C	-720 / 0	0.14 (1)
G-B	-719 / 0	0.14 (1)
G-C	-2 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 38.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/ 835 (0.23")

CSI: TC=0.85/1.00 (A-B:1) , BC=0.64/1.00 (F-G:1) , WB=0.80/1.00 (D-F:1) , SSI=0.42/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (F) (INPUT = 0.90)
JSI METAL= 0.85 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
2/16
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12105-8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	G22X	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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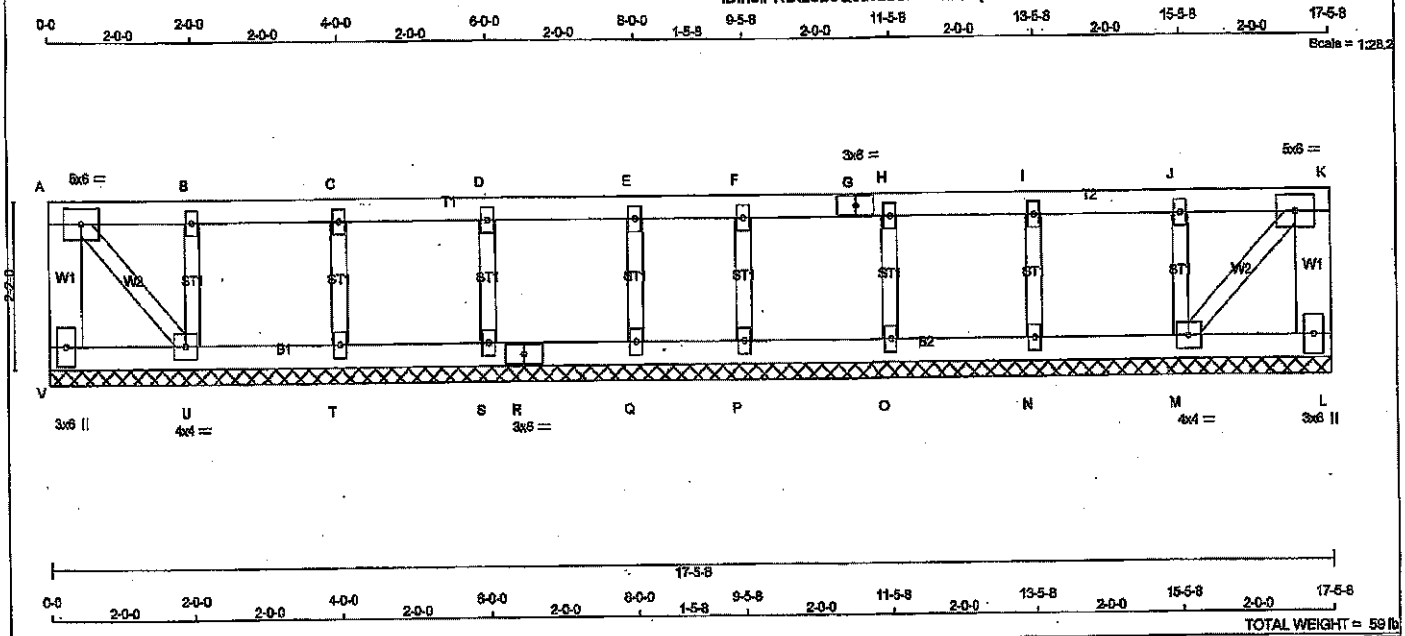

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
V - A	2x6	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - K	2x4	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
V - R	2x4	DRY	No.2	SPF
R - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		
B, C, D, E, F, H, I, J						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
K	TMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	6.0		
M	BMWW1-t	MT20	4.0	4.0		
N, O, P, Q, S, T						
N	BMW1+w	MT20	2.0	4.0		
R	BS-t	MT20	3.0	6.0		
U	BMWW1-t	MT20	4.0	4.0		
V	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
V-A	-97 / 0	0.0	0.0	0.01 (1)	7.81	
A-B	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
B-C	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
C-D	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
D-E	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
E-F	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
F-G	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
G-H	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
H-I	0 / 5	-127.4	-127.4	0.08 (1)	10.00	
I-J	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
J-K	0 / 5	-127.4	-127.4	0.09 (1)	10.00	
L-K	-97 / 0	0.0	0.0	0.01 (1)	7.81	
V-U	0 / 0	-28.0	-28.0	0.04 (3)	10.00	
U-T	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
T-S	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
S-R	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
R-Q	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
Q-P	-5 / 0	-28.0	-28.0	0.03 (3)	10.00	
P-O	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
O-N	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
N-M	-5 / 0	-28.0	-28.0	0.04 (3)	10.00	
M-L	0 / 0	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-J	-287 / 0	0.06 (1)
N-I	-246 / 0	0.05 (1)
O-H	-264 / 0	0.05 (1)
P-F	-214 / 0	0.04 (1)
U-B	-287 / 0	0.06 (1)
T-C	-246 / 0	0.05 (1)
S-D	-264 / 0	0.05 (1)
Q-E	-214 / 0	0.04 (1)
A-U	-7 / 0	0.00 (1)
M-K	-7 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.09/1.00 (J-K:1) , BC=0.04/1.00 (M-N:3) , WB=0.06/1.00 (J-M:1) , SSI=0.16/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.61 (U) (INPUT = 0.90)
JSI METAL= 0.07 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12113-48
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T23	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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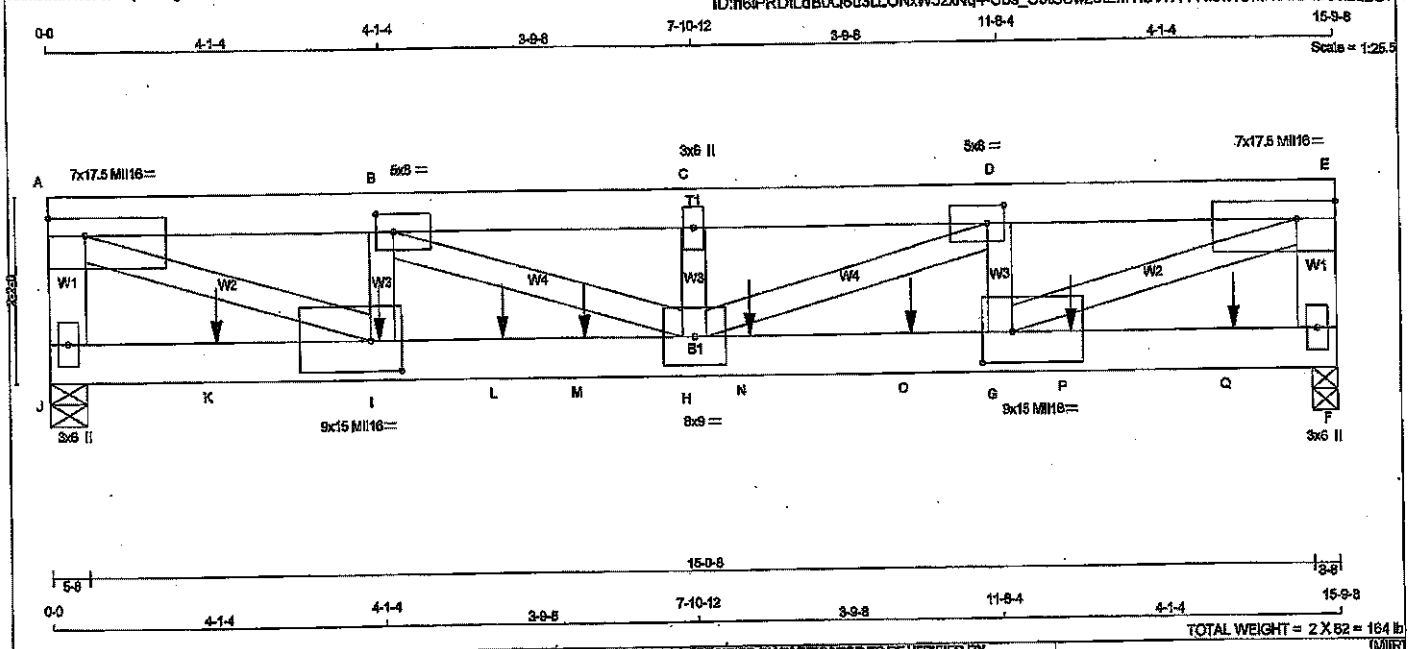

TOTAL WEIGHT = 2 X 82 = 164 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
J - A	2x6	DRY	No.2	SPF
A - E	2x6	DRY	2100F 1.8E	SPF
F - E	2x6	DRY	No.2	SPF
J - F	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
G - E	2x4	DRY	2100F 1.8E	SPF
A - I	2x4	DRY	2100F 1.8E	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
J-A	2	12	TOP
A-E	2	12	TOP
E-F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
J-F	2	12	SIDE(148.3)
WEBS : (0.122"X3") SPIRAL NAILS			
I-B	1	2	SIDE(123.1)
2x4	1	8	
G-D	1	2	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MII16	7.0	17.5	2.50	Edge
B	TMWW-t	MT20	5.0	8.0	2.50	2.50
C	TMW+w	MT20	3.0	6.0		
D	TMWW-t	MT20	5.0	8.0	2.50	2.50
E	TMWW-t	MII16	7.0	17.5	2.50	Edge
F	BMV1+p	MT20	3.0	6.0		
G	BMWW-t	MII16	9.0	15.0	4.25	4.50
H	BMWWW-t	MT20	8.0	9.0		
I	BMWW-t	MII16	9.0	15.0	4.25	4.50
J	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING
BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	5770	0	5770	0	0	5-8	5-8
F	6432	0	6432	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	4590	2268 / 0	712 / 0	0 / 0	0 / 0	1610 / 0	0 / 0
F	5120	2525 / 0	799 / 0	0 / 0	0 / 0	1796 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
J-A	-5316 / 0	0.0	0.0	0.26 (1)	5.62	
A-B	-11981 / 0	-127.4	-127.4	0.24 (1)	3.72	
B-C	-15515 / 0	-127.4	-127.4	0.37 (1)	3.19	
C-D	-15515 / 0	-127.4	-127.4	0.37 (1)	3.19	
D-E	-12097 / 0	-127.4	-127.4	0.25 (1)	3.71	
F-E	-5353 / 0	0.0	0.0	0.27 (1)	5.60	
J-K	0 / 0	-28.0	-28.0	0.23 (1)	10.00	
K-I	0 / 0	-28.0	-28.0	0.23 (1)	10.00	
I-L	0 / 11981	-28.0	-28.0	0.76 (1)	10.00	
L-M	0 / 11981	-28.0	-28.0	0.76 (1)	10.00	
M-H	0 / 11981	-28.0	-28.0	0.76 (1)	10.00	
H-N	0 / 12087	-28.0	-28.0	0.63 (1)	10.00	
N-O	0 / 12087	-28.0	-28.0	0.63 (1)	10.00	
O-G	0 / 12087	-28.0	-28.0	0.63 (1)	10.00	
G-P	0 / 0	-28.0	-28.0	0.35 (1)	10.00	
P-Q	0 / 0	-28.0	-28.0	0.35 (1)	10.00	
Q-F	0 / 0	-28.0	-28.0	0.35 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-E	0 / 13071	0.72 (1)
A-I	0 / 12977	0.71 (1)
G-D	-2021 / 0	0.15 (1)
I-B	-2085 / 0	0.16 (1)
H-D	0 / 3782	0.45 (1)
B-H	0 / 3877	0.46 (1)
H-C	-415 / 0	0.09 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
I	4-0-12	-1040	-1040	—	BACK	VERT	TOTAL
K	2-0-12	-1040	-1040	—	BACK	VERT	TOTAL
L	5-6-12	-1040	-1040	—	BACK	VERT	TOTAL
M	6-6-12	-1325	-1325	—	BACK	VERT	TOTAL
N	8-6-12	-1325	-1325	—	BACK	VERT	TOTAL
O	10-6-12	-1325	-1325	—	BACK	VERT	TOTAL
P	12-6-12	-1325	-1325	—	BACK	VERT	TOTAL
Q	14-6-12	-1325	-1325	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KAFSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/639 (0.30")

CSI: TC=0.37/1.00 (C-D:1) , BC=0.76/1.00 (H-I:1) , WB=0.72/1.00 (E-G:1) , SSI=0.75/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.94 (G) (INPUT = 1.00)

DWG NO. TAM 17406.8
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292707	T23	1	2	TRUSS DESC.	

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1040.4 lbs FACTORED DOWN AT 2-0-12, 1040.4 lbs FACTORED DOWN AT 4-0-12, 1040.4 lbs FACTORED DOWN AT 5-8-12, 1325.3 lbs FACTORED DOWN AT 6-8-12, 1325.3 lbs FACTORED DOWN AT 8-8-12, 1325.3 lbs FACTORED DOWN AT 10-8-12, AND 1325.3 lbs FACTORED DOWN AT 12-8-12, AND 1325.3 lbs FACTORED DOWN AT 14-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12406-8
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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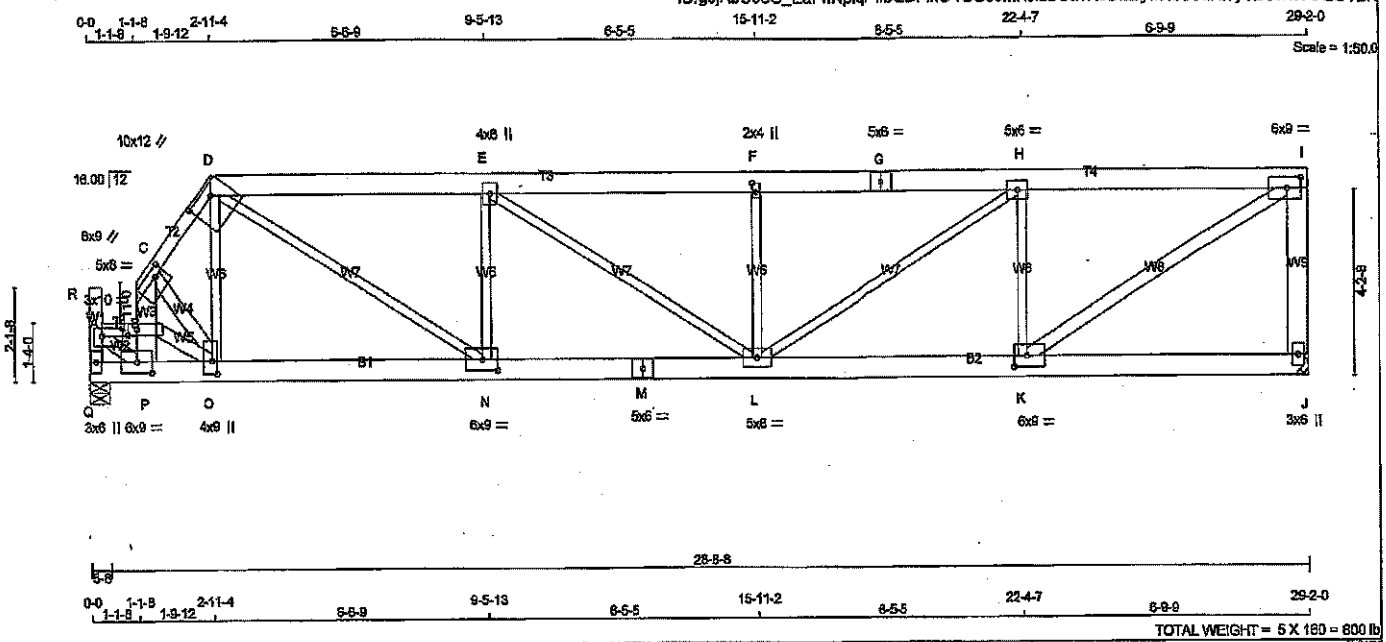

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - R	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - C	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - P	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	5.0	8.0	2.25	5.75
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.00	2.75
D	TTWW-h	MT20	10.0	12.0	2.50	7.00
E	TMWW-t	MT20	4.0	6.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	6.0	6.0		
I	TMVW-t	MT20	6.0	9.0	3.00	3.75
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	3.75
L	BMWWW-t	MT20	5.0	8.0		
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	6.0	9.0	3.00	4.25
O	BMWWW+t	MT20	4.0	8.0	3.50	1.50
P	BMVW-t	MT20	6.0	8.0	3.00	4.25
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2266	0	2266	0	0	5-8	5-8
J	2266	0	2266	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1622	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0
J	1622	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.09 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Q-A	-1890 / 0	0.0	0.0	0.26 (1)	5.40	
A-R	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-2549 / 0	-127.4	-127.4	0.17 (1)	3.70	
C-D	-2440 / 0	-127.4	-127.4	0.19 (1)	3.75	
D-E	-3575 / 0	-127.4	-127.4	0.64 (1)	3.38	
E-F	-3969 / 0	-127.4	-127.4	0.67 (1)	3.17	
F-G	-3969 / 0	-127.4	-127.4	0.72 (1)	3.09	
G-H	-3969 / 0	-127.4	-127.4	0.72 (1)	3.09	
H-I	-2962 / 0	-127.4	-127.4	0.64 (1)	3.65	
J-I	-2165 / 0	0.0	0.0	0.55 (1)	6.16	
P-B	-2539 / 0	0.0	0.0	0.64 (1)	4.69	
B-C	-3250 / 0	0.0	0.0	0.67 (1)	4.10	
Q-P	0 / 0	-28.0	-28.0	0.43 (1)	10.00	
P-O	0 / 1861	-28.0	-28.0	0.72 (1)	10.00	
O-N	0 / 1459	-28.0	-28.0	0.36 (1)	10.00	
N-M	0 / 3575	-28.0	-28.0	0.67 (1)	10.00	
M-L	0 / 3575	-28.0	-28.0	0.67 (1)	10.00	
L-K	0 / 2962	-28.0	-28.0	0.55 (1)	10.00	
K-J	0 / 0	-28.0	-28.0	0.22 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
O-D	0 / 248	0.09 (3)
A-P	0 / 3122	0.87 (1)
B-O	-1505 / 0	0.22 (1)
C-O	0 / 1480	0.32 (1)
K-I	0 / 3484	0.76 (1)
D-N	0 / 2515	0.75 (1)
K-H	-1623 / 0	0.60 (1)
N-E	-1191 / 0	0.44 (1)
L-H	0 / 1203	0.38 (1)
E-L	0 / 470	0.14 (1)
L-F	-749 / 0	0.28 (1)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSI: TC=0.72/1.00 (F-H:1) , BC=0.72/1.00 (O-P:1) , WB=0.75/1.00 (D-N:1) , SSI=0.55/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (A) (INPUT = 0.90)
JSI METAL= 0.84 (M) (INPUT = 1.00)

DWG NO. TAM28035-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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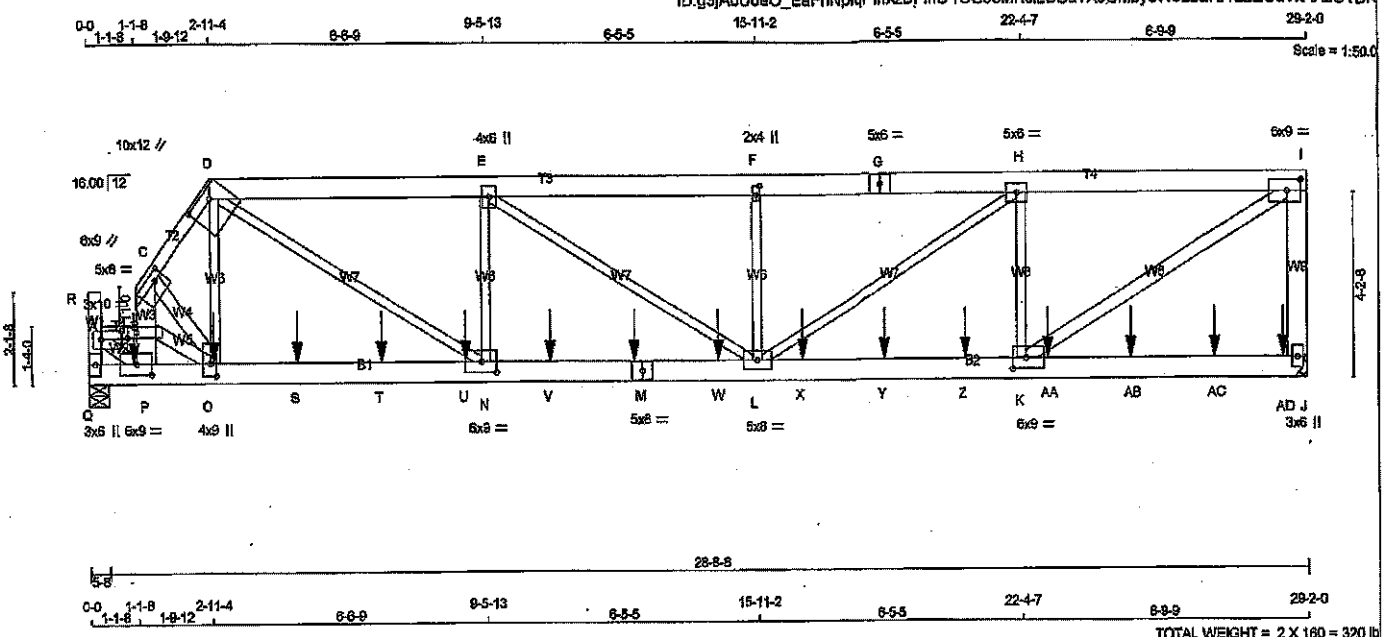

TOTAL WEIGHT = 2 X 160 = 320 lb

[MiTek]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - R	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - C	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - P	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
Q- R	1	12	TOP
A- B	1	12	TOP
C- D	1	12	TOP
D- G	2	12	TOP
G- I	2	12	TOP
I- J	2	12	TOP
C- P	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
Q- M	2	12	SIDE(137.3)
M- J	2	12	SIDE(137.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	5.0	8.0	2.25	5.75
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.00	2.75
D	TTWW-h	MT20	10.0	12.0	2.50	7.00
E	TMWW+t	MT20	4.0	8.0		
F	TMW+w	MT20	2.0	4.0	2.50	1.00
G	TS-t	MT20	5.0	6.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMVW-t	MT20	6.0	9.0	3.00	3.75
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	3.75
L	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	3779	0	3779	0	0	5-8	5-8
J	3883	0	3883	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	2942	1541 / 0	378 / 0	0 / 0	0 / 0	1023 / 0	0 / 0
J	3021	1594 / 0	387 / 0	0 / 0	0 / 0	1051 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- A	-3120 / 0	0.0	0.0	0.24 (1)	5.81	O- D	0 / 698	0.11 (1)
A- R	0 / 0	0.0	0.0	0.00 (1)	10.00	A- P	0 / 5251	0.62 (1)
A- B	-4286 / 0	-127.4	-127.4	0.13 (1)	3.99	B- O	-2507 / 0	0.20 (1)
C- D	-4200 / 0	-127.4	-127.4	0.14 (1)	4.02	C- O	0 / 2571	0.30 (1)
D- E	-5973 / 0	-127.4	-127.4	0.37 (1)	4.00	K- I	0 / 5846	0.69 (1)
E- F	-6660 / 0	-127.4	-127.4	0.39 (1)	3.79	D- N	0 / 4120	0.88 (1)
F- G	-6660 / 0	-127.4	-127.4	0.42 (1)	3.75	K- H	-2084 / 0	0.40 (1)
G- H	-6660 / 0	-127.4	-127.4	0.42 (1)	3.75	N- E	-1371 / 0	0.27 (1)
H- I	-4970 / 0	-127.4	-127.4	0.35 (1)	4.32	L- H	0 / 2016	0.33 (1)
J- I	-3433 / 0	0.0	0.0	0.44 (1)	6.74	E- L	0 / 821	0.14 (1)
P- B	-4293 / 0	0.0	0.0	0.59 (1)	5.13	L- F	-751 / 0	0.15 (1)
B- C	-5539 / 0	0.0	0.0	0.62 (1)	4.51			
Q- P	0 / 0	-28.0	-28.0	0.43 (1)	10.00			
P- O	0 / 3118	-28.0	-28.0	0.70 (1)	10.00			
O- S	0 / 2508	-28.0	-28.0	0.43 (1)	10.00			
S- T	0 / 2508	-28.0	-28.0	0.43 (1)	10.00			
T- U	0 / 2508	-28.0	-28.0	0.43 (1)	10.00			
U- N	0 / 2508	-28.0	-28.0	0.43 (1)	10.00			
N- V	0 / 5973	-28.0	-28.0	0.70 (1)	10.00			
V- M	0 / 5973	-28.0	-28.0	0.70 (1)	10.00			
M- W	0 / 5973	-28.0	-28.0	0.70 (1)	10.00			
W- L	0 / 5973	-28.0	-28.0	0.70 (1)	10.00			
L- X	0 / 4970	-28.0	-28.0	0.72 (1)	10.00			
X- Y	0 / 4970	-28.0	-28.0	0.72 (1)	10.00			
Y- Z	0 / 4970	-28.0	-28.0	0.72 (1)	10.00			
Z- K	0 / 4970	-28.0	-28.0	0.72 (1)	10.00			
K-AA	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
AA-AB	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
AB-AC	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
AC-AD	0 / 0	-28.0	-28.0	0.29 (1)	10.00			
AD- J	0 / 0	-28.0	-28.0	0.29 (1)	10.00			

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.27")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.72/1.00 (K-L:1) , WB=0.69/1.00 (I-K:1) , SSI=0.52/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.80 (M) (INPUT = 1.00)

per 1/2

DWG NO. TAM28036-18
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T31Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BS-t	MT20	5.0	6.0		
N	BMWW-t	MT20	6.0	9.0	3.00	4.25
O	BMWWW+t	MT20	4.0	9.0	3.50	1.50
P	BMVW-t	MT20	6.0	9.0	3.00	4.25
Q	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 13-0-0, 208.8 lbs FACTORED DOWN AT 15-0-0, 208.8 lbs FACTORED DOWN AT 17-0-0, 208.8 lbs FACTORED DOWN AT 19-0-0, 208.8 lbs FACTORED DOWN AT 21-0-0, 208.8 lbs FACTORED DOWN AT 23-0-0, 208.8 lbs FACTORED DOWN AT 25-0-0, AND 208.8 lbs FACTORED DOWN AT 27-0-0, AND 214.9 lbs FACTORED DOWN AT 28-7-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	13-0-0	-209	-209	—	BACK	VERT	TOTAL
O	3-0-0	-209	-209	—	BACK	VERT	TOTAL
P	1-0-12	-200	-200	—	BACK	VERT	TOTAL
S	5-0-0	-209	-209	—	BACK	VERT	TOTAL
T	7-0-0	-209	-209	—	BACK	VERT	TOTAL
U	9-0-0	-209	-209	—	BACK	VERT	TOTAL
V	11-0-0	-209	-209	—	BACK	VERT	TOTAL
W	15-0-0	-209	-209	—	BACK	VERT	TOTAL
X	17-0-0	-209	-209	—	BACK	VERT	TOTAL
Y	19-0-0	-209	-209	—	BACK	VERT	TOTAL
Z	21-0-0	-209	-209	—	BACK	VERT	TOTAL
AA	23-0-0	-209	-209	—	BACK	VERT	TOTAL
AB	25-0-0	-209	-209	—	BACK	VERT	TOTAL
AC	27-0-0	-209	-209	—	BACK	VERT	TOTAL
AD	28-7-4	-215	-215	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28036-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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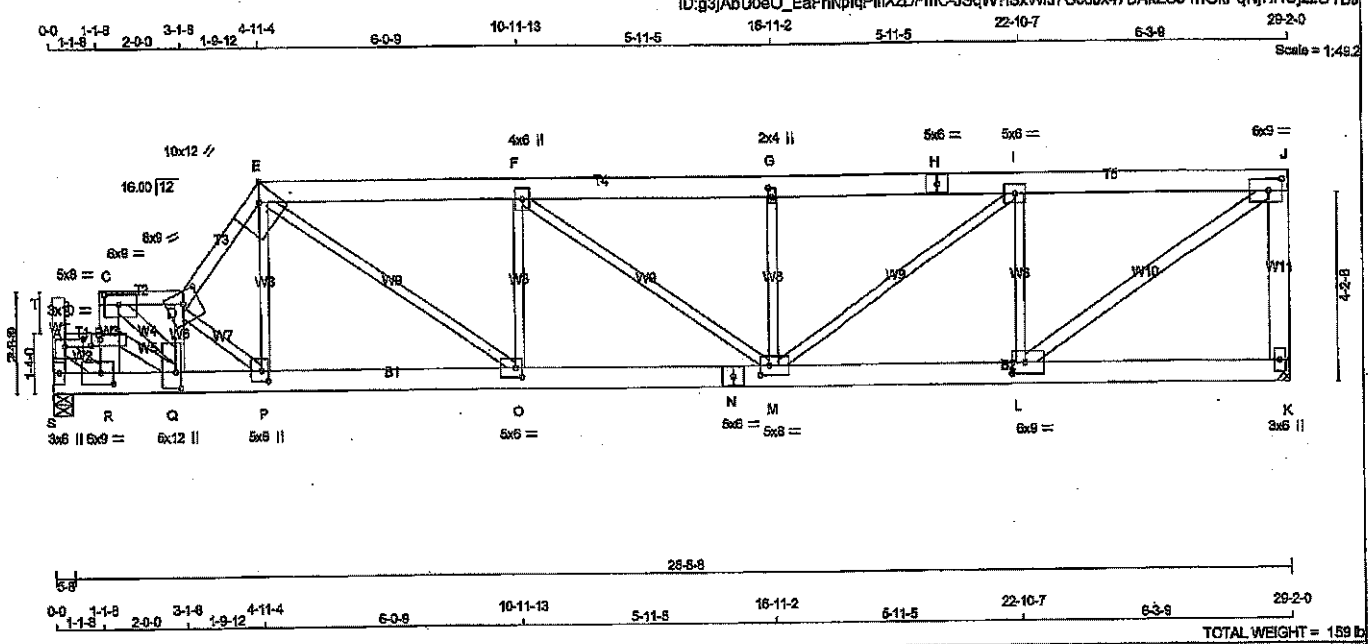

TOTAL WEIGHT = 159 lb

(MIR)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - T	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
R - C	2x6	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
K - J	2x6	DRY	No.2	SPF
S - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - R	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
C - Q	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	5.0	8.0	2.00	5.25
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.75	4.00
D	TTWW-m	MT20	6.0	9.0	3.25	4.50
E	TTWW-h	MT20	10.0	12.0	Edge	4.50
F	TMWW+t	MT20	4.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	6.0		
J	TMWW-t	MT20	6.0	9.0	3.00	3.75
K	BMW1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	6.0	9.0	3.00	3.75
M	BMWWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	5.0	8.0		
O	BMWW-t	MT20	5.0	6.0	2.50	1.75
P	BMWW+t	MT20	5.0	6.0	2.75	2.00
Q	BMWWW+t	MT20	5.0	12.0	4.25	1.50
R	BMWW-t	MT20	6.0	9.0	3.00	3.75
S	BMW1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2266	0	2266	0	0	5-8	5-8
K	2266	0	2266	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT K.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1822	674 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0
K	1822	674 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.97 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
S-A	-1969 / 0	0.0	0.0	0.27 (1)	5.28	
A-T	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-2689 / 0	-127.4	-127.4	0.18 (1)	3.59	
R-B	-2172 / 0	0.0	0.0	0.86 (1)	4.01	
B-C	-2536 / 0	0.0	0.0	0.87 (1)	3.63	
C-D	-3040 / 0	-127.4	-127.4	0.26 (1)	3.32	
D-E	-3619 / 0	-127.4	-127.4	0.35 (1)	2.97	
E-F	-3798 / 0	-127.4	-127.4	0.58 (1)	3.39	
F-G	-3884 / 0	-127.4	-127.4	0.57 (1)	3.34	
G-H	-3884 / 0	-127.4	-127.4	0.61 (1)	3.27	
H-I	-3884 / 0	-127.4	-127.4	0.61 (1)	3.27	
I-J	-2791 / 0	-127.4	-127.4	0.53 (1)	3.90	
K-J	-2191 / 0	0.0	0.0	0.55 (1)	6.15	
S-R	0 / 0	-28.0	-28.0	0.31 (1)	10.00	
R-Q	0 / 1723	-28.0	-28.0	0.58 (1)	10.00	
Q-P	0 / 3331	-28.0	-28.0	0.77 (1)	10.00	
P-O	0 / 2229	-28.0	-28.0	0.42 (1)	10.00	
O-N	0 / 3798	-28.0	-28.0	0.69 (1)	10.00	
N-M	0 / 3798	-28.0	-28.0	0.69 (1)	10.00	
M-L	0 / 2791	-28.0	-28.0	0.52 (1)	10.00	
L-K	0 / 0	-28.0	-28.0	0.19 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-R	0 / 3294	0.71 (1)
Q-D	-2127 / 0	0.42 (1)
D-P	-1569 / 0	0.25 (1)
P-E	0 / 1352	0.41 (1)
B-Q	-843 / 0	0.12 (1)
C-Q	0 / 3225	0.69 (1)
L-J	0 / 3357	0.72 (1)
E-O	0 / 1910	0.57 (1)
L-I	-1667 / 0	0.62 (1)
O-F	-920 / 0	0.34 (1)
M-I	0 / 1340	0.40 (1)
F-M	0 / 106	0.03 (1)
M-G	-890 / 0	0.28 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.32")

CSI: TC=0.67/1.00 (B-C:1) , BC=0.77/1.00 (P-Q:1) , WB=0.72/1.00 (J-L:1) , SSI=0.81/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (A) (INPUT = 0.90)
JSI METAL= 0.82 (N) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28037 18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T32S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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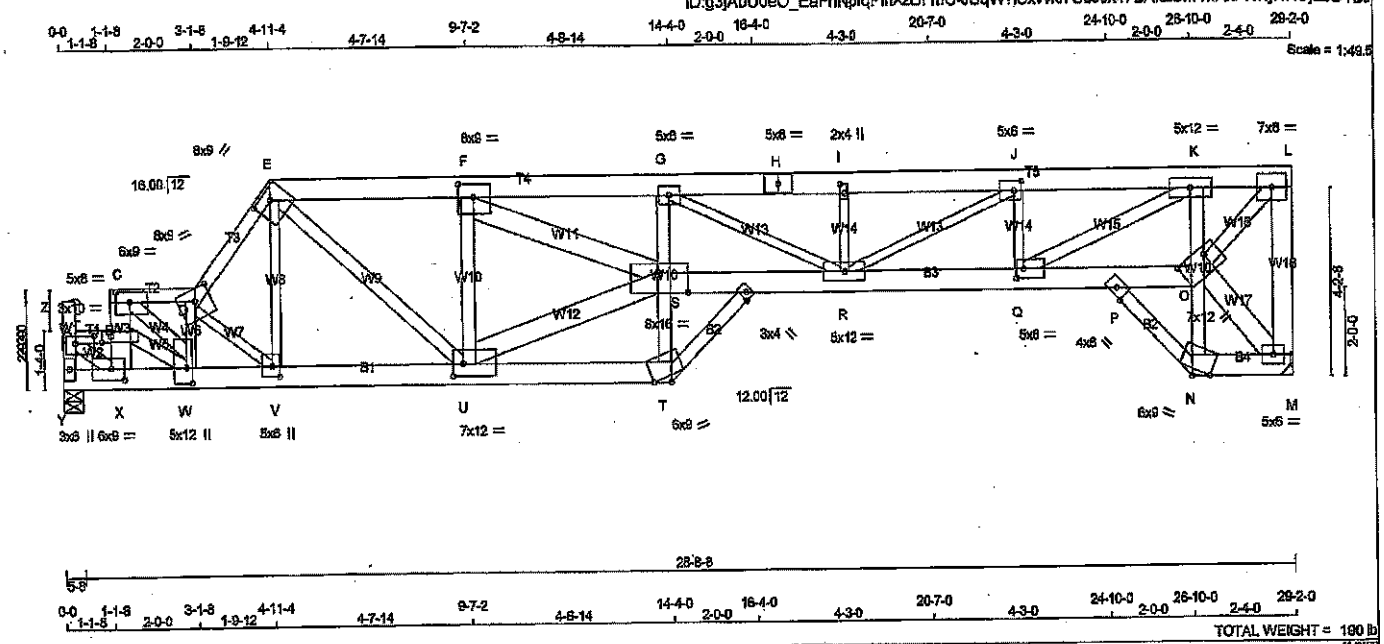

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - Z	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
X - C	2x6	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - L	2x6	DRY	No.2	SPF
M - L	2x6	DRY	No.2	SPF
Y - T	2x6	DRY	No.2	SPF
S - O	2x6	DRY	2100F 1.8E	SPF
P - N	2x4	DRY	No.2	SPF
N - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - D	2x3	DRY	No.2	SPF
D - V	2x3	DRY	No.2	SPF
V - E	2x3	DRY	No.2	SPF
O - M	2x6	DRY	No.2	SPF
R - I	2x3	DRY	No.2	SPF
Q - J	2x3	DRY	No.2	SPF
G - R	2x3	DRY	No.2	SPF
R - J	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
F - S	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.00	5.25
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.75	4.00
D	TTWW-m	MT20	8.0	9.0	3.25	4.50
E	TTWW-h	MT20	8.0	9.0	2.50	4.50
F	TMWW-t	MT20	8.0	9.0	3.50	4.25
G	TMWW-t	MT20	5.0	6.0		
H	TS-t	MT20	5.0	8.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TMWW-t	MT20	5.0	8.0	2.50	2.00
K	TMWW-t	MT20	5.0	12.0		
L	TMWV-t	MT20	7.0	8.0		
M	BMVW1-t	MT20	5.0	6.0		
N	BBW-m	MT20	6.0	9.0	Edge	
O	BWMWW^-w	MT20	7.0	12.0	2.00	8.00
P	BMB-h	MT20	4.0	6.0	Edge	
Q	BMWW-t	MT20	5.0	8.0	2.50	2.00
R	BMWWW-t	MT20	5.0	12.0		
S	BWMWW^-t	MT20	8.0	16.0	5.75	6.50
T						
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	7.0	12.0	3.25	2.75
V	BMWW+t	MT20	5.0	6.0	2.75	2.00
W	BMWWW+t	MT20	5.0	12.0	4.25	1.50
X	BMVW-t	MT20	6.0	9.0	3.00	3.75
Y	BMV1+p	MT20	3.0	6.0		
AA	NP-h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2266	0	2266	0	0	5-8	5-8
M	2266	0	2266	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT M.

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD OK

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0
M	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.11 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 5.01 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y-A	-1986 / 0	0.0	0.0	0.27 (1)	5.26	
A-Z	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-2685 / 0	-127.4	-127.4	0.16 (1)	3.59	
X-B	-2187 / 0	0.0	0.0	0.86 (1)	3.98	
B-C	-2547 / 0	0.0	0.0	0.87 (1)	3.60	
C-D	-3051 / 0	-127.4	-127.4	0.26 (1)	3.31	
D-E	-3590 / 0	-127.4	-127.4	0.35 (1)	2.99	
E-F	-3514 / 0	-127.4	-127.4	0.37 (1)	3.73	
F-G	-7631 / 0	-127.4	-127.4	0.84 (1)	2.11	
G-H	-8893 / 0	-127.4	-127.4	0.73 (1)	2.36	
H-I	-8893 / 0	-127.4	-127.4	0.73 (1)	2.36	
I-J	-6893 / 0	-127.4	-127.4	0.66 (1)	2.46	
J-K	-5195 / 0	-127.4	-127.4	0.43 (1)	3.08	
K-L	-1858 / 0	-127.4	-127.4	0.19 (1)	5.07	
M-L	-2165 / 0	0.0	0.0	0.54 (1)	6.18	
Y-X	0 / 0	-28.0	-28.0	0.32 (1)	10.00	
X-W	0 / 1718	-28.0	-28.0	0.59 (1)	10.00	
W-V	0 / 3335	-28.0	-28.0	0.75 (1)	10.00	
V-U	0 / 2210	-28.0	-28.0	0.40 (1)	10.00	
U-T	0 / 65	-28.0	-28.0	0.12 (3)	10.00	
S-R	0 / 7896	-28.0	-28.0	0.59 (1)	10.00	
R-Q	0 / 5195	-28.0	-28.0	0.43 (1)	10.00	
Q-P	0 / 2108	-28.0	-28.0	0.20 (1)	10.00	
P-O	0 / 2063	0.0	0.0	0.20 (1)	10.00	
P-N	0 / 67	-28.0	-28.0	0.04 (3)	10.00	
N-M	-7 / 67	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-X	0 / 3289	0.70 (1)
W-D	-2082 / 0	0.41 (1)
D-V	-1599 / 0	0.38 (1)
V-E	0 / 1289	0.39 (1)
E-U	0 / 1777	0.38 (1)
U-F	-2597 / 0	0.69 (1)
T-S	0 / 92	0.28 (1)
S-G	-62 / 93	0.25 (1)
N-O	0 / 101	0.09 (1)
O-K	-2121 / 0	0.20 (1)
O-M	-95 / 0	0.01 (1)
O-L	0 / 2809	0.60 (1)
R-I	-561 / 0	0.12 (1)
Q-J	-1414 / 0	0.29 (1)
G-R	-901 / 0	0.44 (1)
R-J	0 / 1916	0.57 (1)
Q-K	0 / 3474	0.74 (1)
U-S	0 / 3612	0.69 (1)
F-S	0 / 4487	0.83 (1)
B-W	-835 / 0	0.12 (1)
C-W	0 / 3233	0.69 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 549 (0.64")

CSI: TC=0.87/1.00 (B-C:1) , BC=0.75/1.00 (V-W:1) , WB=0.74/1.00 (K-Q:1) , SSI=0.81/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1687	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (J) (INPUT = 0.90)
JSI METAL= 1.00 (S) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 28038-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T33	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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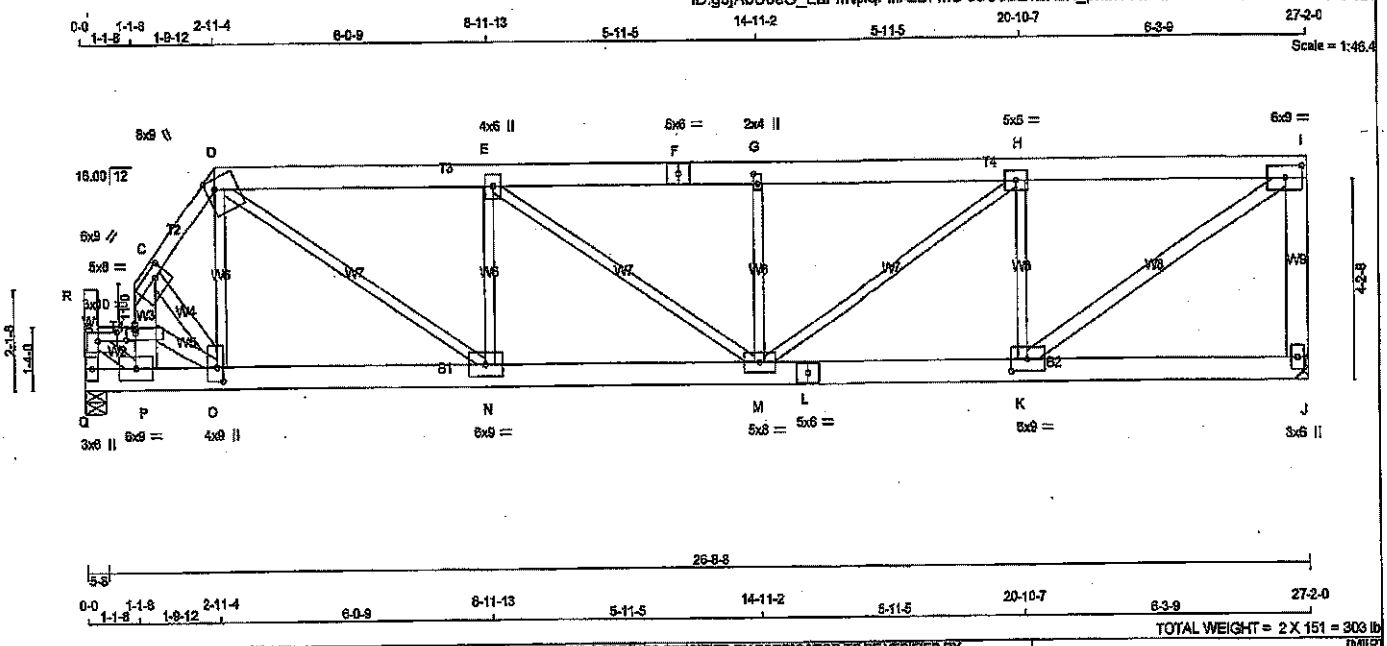

[M][R]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Q - R	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
J - I	2x6	DRY	No.2	SPF
P - C	2x6	DRY	No.2	SPF
Q - L	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - P	2x4	DRY	No.2	SPF
B - O	2x4	DRY	No.2	SPF
C - O	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	5.0	8.0	2.25	5.00
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	8.0	9.0	2.50	2.25
E	TMWW+t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TMWW-t	MT20	5.0	6.0		
I	TMVW-t	MT20	6.0	9.0	3.00	4.25
J	BMV1+p	MT20	3.0	6.0		
K	BMWW-t	MT20	6.0	9.0	3.00	4.25
L	BS-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	5.0	8.0		
N	BMWW-t	MT20	6.0	9.0		
O	BMWWW+t	MT20	4.0	9.0	3.50	1.50
P	BMVW-t	MT20	6.0	9.0		
Q	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2111	0	2111	0	0	5-8	5-8
J	2111	0	2111	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1697	814 / 0	285 / 0	0 / 0	0 / 0	598 / 0	0 / 0
J	1697	814 / 0	285 / 0	0 / 0	0 / 0	598 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
Q-A	-1762 / 0	0.0	0.0	0.24 (1)	5.57	
A-R	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-2388 / 0	-127.4	-127.4	0.15 (1)	3.83	
C-D	-2249 / 0	-127.4	-127.4	0.17 (1)	3.91	
D-E	-3125 / 0	-127.4	-127.4	0.61 (1)	3.75	
E-F	-3435 / 0	-127.4	-127.4	0.53 (1)	3.57	
F-G	-3435 / 0	-127.4	-127.4	0.53 (1)	3.57	
G-H	-3435 / 0	-127.4	-127.4	0.58 (1)	3.50	
H-I	-2558 / 0	-127.4	-127.4	0.52 (1)	4.06	
J-I	-2035 / 0	0.0	0.0	0.51 (1)	6.33	
P-B	-2339 / 0	0.0	0.0	0.59 (1)	5.00	
B-C	-2999 / 0	0.0	0.0	0.61 (1)	4.42	
Q-P	0 / 0	-28.0	-28.0	0.40 (1)	10.00	
P-O	0 / 1781	-28.0	-28.0	0.67 (1)	10.00	
O-N	0 / 1345	-28.0	-28.0	0.32 (1)	10.00	
N-M	0 / 3125	-28.0	-28.0	0.59 (1)	10.00	
M-L	0 / 2558	-28.0	-28.0	0.48 (1)	10.00	
L-K	0 / 2558	-28.0	-28.0	0.48 (1)	10.00	
K-J	0 / 0	-28.0	-28.0	0.19 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-D	0 / 232	0.08 (3)
A-P	0 / 2902	0.62 (1)
B-O	-1403 / 0	0.20 (1)
C-O	0 / 1360	0.29 (1)
K-I	0 / 3077	0.66 (1)
D-N	0 / 2168	0.65 (1)
K-H	-1514 / 0	0.56 (1)
N-E	-1081 / 0	0.40 (1)
M-H	0 / 1074	0.32 (1)
E-M	0 / 390	0.11 (1)
M-G	-660 / 0	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.67/1.00 (O-P:1) , WB=0.66/1.00 (I-K:1) , SSI=0.51/1.00 (B-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.84 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28035-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T33S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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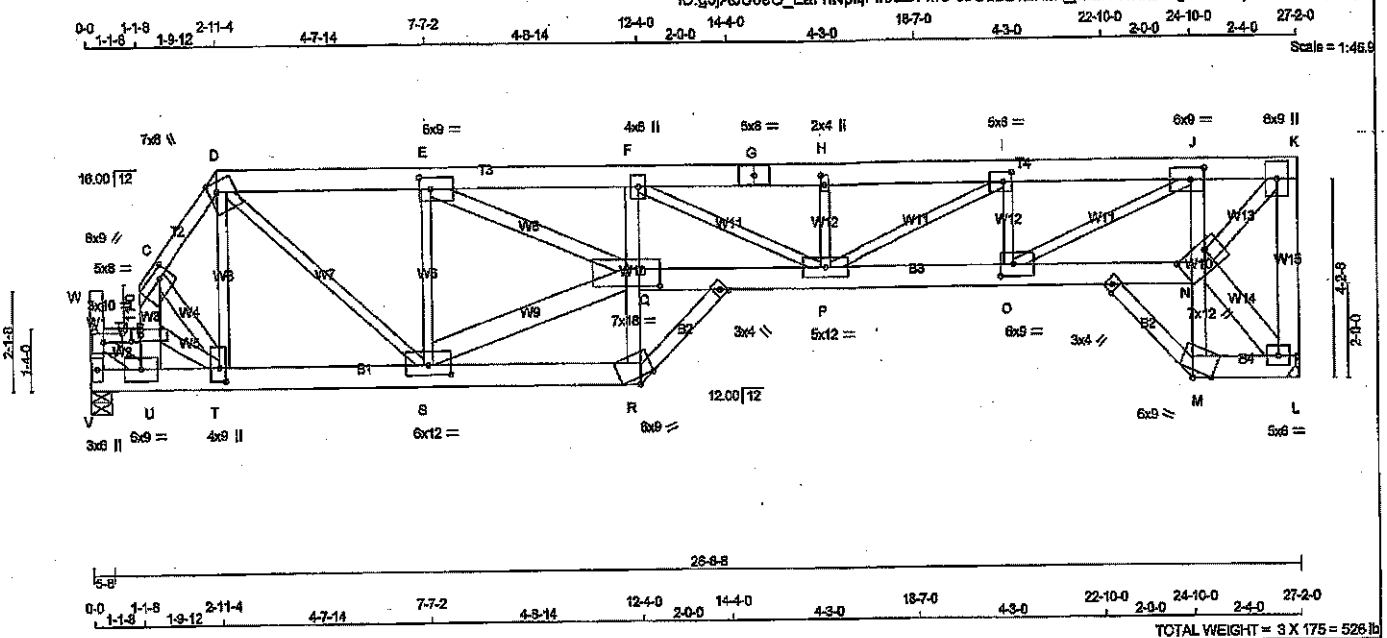

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
V - W	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - K	2x6	DRY	No.2	SPF
L - K	2x6	DRY	No.2	SPF
U - C	2x6	DRY	No.2	SPF
V - R	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	2100F 1.8E	SPF
M - L	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
R - F	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
E - Q	2x4	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
A - U	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
C - T	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	5.00
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMWW-t	MT20	6.0	9.0	3.00	3.00
F	TMWW+t	MT20	4.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMW+w	MT20	2.0	4.0	2.50	1.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMWW-t	MT20	6.0	9.0	3.00	3.50
K	TMWW+p	MT20	6.0	9.0		
L	BMVW1-t	MT20	5.0	6.0		
M, M, R, R						
M						
M	BBW-m	MT20	6.0	9.0	Edge	
N	BWMWW*-w	MT20	7.0	12.0	2.00	8.00
O	BMWW-t	MT20	6.0	9.0	3.00	3.50
P	BMWWW-t	MT20	5.0	12.0		
Q	BWMWW*-I	MT20	7.0	18.0	4.75	9.00
R	BBW-m	MT20	6.0	9.0	Edge	
S	BMWWW-t	MT20	6.0	12.0	2.50	6.00
T	BMWWW+t	MT20	4.0	9.0	3.50	1.50
U	BMVW-t	MT20	6.0	9.0		
V	BMV1+p	MT20	3.0	6.0		
X	NP+h	MT20	3.0	4.0	Edge	
Y	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2111	0	2111	0	0	5-8	5-8
L	2117	0	2117	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT 1.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1698	814 / 0	286 / 0	0 / 0	0 / 0	598 / 0	0 / 0
L	1705	814 / 0	290 / 0	0 / 0	0 / 0	601 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 5.21 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
V- A	-1770 / 0	0.0	0.0	0.24 (1)	5.56	
A- W	0 / 0	0.0	0.0	0.00 (1)	10.00	
A- B	-2379 / 0	-127.4	-127.4	0.15 (1)	3.83	
C- D	-2230 / 0	-127.4	-127.4	0.17 (1)	3.92	
D- E	-2792 / 0	-127.4	-127.4	0.32 (1)	4.17	
E- F	-6543 / 0	-127.4	-127.4	0.68 (1)	2.49	
F- G	-6168 / 0	-127.4	-127.4	0.62 (1)	2.62	
G- H	-6166 / 0	-127.4	-127.4	0.62 (1)	2.62	
H- I	-6166 / 0	-127.4	-127.4	0.57 (1)	2.70	
I- J	-4710 / 0	-127.4	-127.4	0.42 (1)	3.23	
J- K	-1767 / 0	-127.4	-127.4	0.22 (1)	5.11	
L- K	-2067 / 0	0.0	0.0	0.52 (1)	6.28	
U- B	-2302 / 0	0.0	0.0	0.58 (1)	5.04	
B- C	-2970 / 0	0.0	0.0	0.61 (1)	4.44	
V- U	0 / 0	-28.0	-28.0	0.39 (1)	10.00	
U- T	0 / 1743	-28.0	-28.0	0.66 (1)	10.00	
T- S	0 / 1334	-28.0	-28.0	0.28 (1)	10.00	
S- R	0 / 68	-28.0	-28.0	0.11 (3)	10.00	
Q- P	0 / 6810	-28.0	-28.0	0.51 (1)	10.00	
P- O	0 / 4710	-28.0	-28.0	0.39 (1)	10.00	
O- N	0 / 2018	-28.0	-28.0	0.18 (1)	10.00	
M- L	0 / 19	-28.0	-28.0	0.03 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- D	0 / 184	0.08 (3)
D- S	0 / 1980	0.59 (1)
S- E	-2437 / 0	0.91 (1)
R- Q	0 / 85	0.27 (1)
Q- F	-243 / 0	0.26 (1)
M- N	0 / 52	0.09 (1)
N- J	-1950 / 0	0.18 (1)
S- Q	0 / 3029	0.48 (1)
E- Q	0 / 4086	0.88 (1)
N- L	-28 / 0	0.00 (1)
N- K	0 / 2702	0.53 (1)
P- H	-577 / 0	0.12 (1)
O- I	-1259 / 0	0.26 (1)
F- P	-498 / 0	0.24 (1)
P- I	0 / 1642	0.49 (1)
O- J	0 / 3029	0.91 (1)
A- U	0 / 2815	0.62 (1)
B- T	-1418 / 0	0.20 (1)
C- T	0 / 4842	0.89 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 609 (0.54")

CSI: TC=0.68/1.00 (E-F:1) , BC=0.66/1.00 (T-U:1) , WB=0.91/1.00 (J-O:1) , SSI=0.51/1.00 (B-U:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.98 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280490

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T34	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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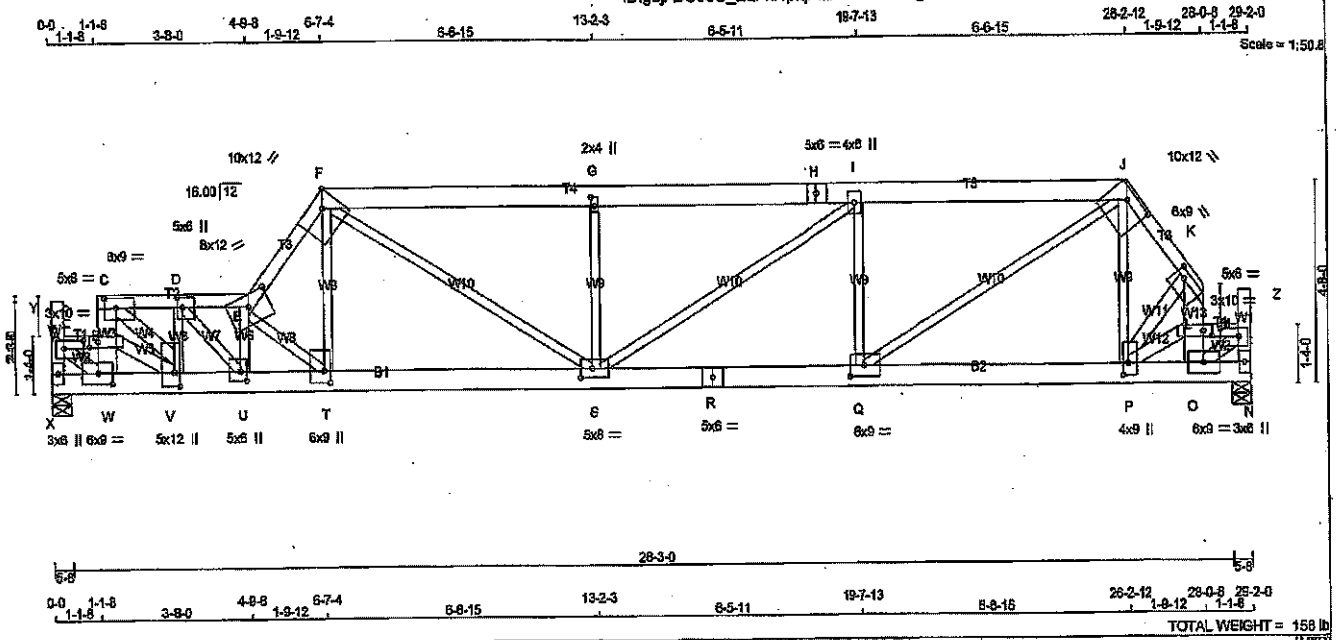

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
X - Y	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
W - C	2x6	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - Z	2x4	DRY	No.2	SPF
X - R	2x6	DRY	2100F 1.8E	SPF
R - N	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
A - W	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
P - L	2x4	DRY	No.2	SPF
P - K	2x4	DRY	No.2	SPF
B - V	2x4	DRY	No.2	SPF
G - V	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	5.75
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.75	3.50
D	TMWW+t	MT20	5.0	6.0	2.75	1.50
E	TTWW-m	MT20	8.0	12.0	3.25	6.00
F	TTWW-h	MT20	10.0	12.0	Edge	4.50
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TS-t	MT20	5.0	6.0		
I	TMWW+t	MT20	4.0	6.0		
J	TTWW-h	MT20	10.0	12.0	2.50	7.00
K	TMVW-t	MT20	6.0	9.0	2.00	2.75
L	TVMW-I	MT20	3.0	10.0	1.50	2.50
M	TVMW-I	MT20	5.0	8.0	2.25	5.25
N	BMV1+p	MT20	3.0	6.0		
O	BMWW-t	MT20	6.0	9.0		
P	BMWWW+t	MT20	4.0	9.0	3.25	1.50
Q	BMWW-t	MT20	6.0	9.0	3.00	4.25
R	BS-t	MT20	5.0	6.0		
S	BMWWW-t	MT20	5.0	8.0	2.50	3.25
T	BMWW+t	MT20	6.0	9.0	3.25	1.75
U	BMWW+t	MT20	5.0	6.0	2.50	1.75
V	BMWWW+t	MT20	5.0	12.0	3.75	1.50
W	BMVW-t	MT20	6.0	9.0	3.00	4.25
X	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2266	0	2266	0	0	5-8	5-8
N	2266	0	2266	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0
N	1822	874 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.51 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
X-A	-1877 / 0	0.0	0.0	0.26 (1)	5.42	
A-Y	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-2531 / 0	-127.4	-127.4	0.17 (1)	3.71	
W-B	-2465 / 0	0.0	0.0	0.88 (1)	3.73	
B-C	-2749 / 0	0.0	0.0	0.87 (1)	3.48	
C-D	-3223 / 0	-127.4	-127.4	0.28 (1)	3.22	
D-E	-4566 / 0	-127.4	-127.4	0.45 (1)	2.51	
E-F	-4477 / 0	-127.4	-127.4	0.49 (1)	2.52	
F-G	-4004 / 0	-127.4	-127.4	0.64 (1)	3.20	
G-H	-4004 / 0	-127.4	-127.4	0.64 (1)	3.20	
H-I	-4004 / 0	-127.4	-127.4	0.64 (1)	3.20	
I-J	-3570 / 0	-127.4	-127.4	0.60 (1)	3.42	
J-K	-2447 / 0	-127.4	-127.4	0.19 (1)	3.75	
O-L	-2812 / 0	0.0	0.0	0.62 (1)	4.68	
L-K	-3289 / 0	0.0	0.0	0.66 (1)	4.14	
L-M	-2445 / 0	-127.4	-127.4	0.18 (1)	3.77	
N-M	-1817 / 0	0.0	0.0	0.25 (1)	5.49	
M-Z	0 / 0	0.0	0.0	0.00 (1)	10.00	
X-W	0 / 0	-28.0	-28.0	0.24 (1)	10.00	
W-V	0 / 1575	-28.0	-28.0	0.34 (1)	10.00	
V-U	0 / 3349	-28.0	-28.0	0.48 (1)	10.00	
U-T	0 / 4680	-28.0	-28.0	0.57 (1)	10.00	
T-S	0 / 2773	-28.0	-28.0	0.23 (1)	10.00	
S-R	0 / 3570	-28.0	-28.0	0.29 (1)	10.00	
R-Q	0 / 3570	-28.0	-28.0	0.29 (1)	10.00	
Q-P	0 / 1463	-28.0	-28.0	0.17 (1)	10.00	
P-O	0 / 1781	-28.0	-28.0	0.40 (1)	10.00	
O-N	0 / 0	-28.0	-28.0	0.26 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-W	0 / 3100	0.66 (1)
U-E	-2036 / 0	0.40 (1)
E-T	-2713 / 0	0.61 (1)
T-F	0 / 2305	0.69 (1)
P-J	0 / 258	0.09 (3)
O-M	0 / 2695	0.64 (1)
P-L	-1435 / 0	0.21 (1)
P-K	0 / 1523	0.33 (1)
V-D	-1533 / 0	0.30 (1)
D-U	0 / 1669	0.55 (1)
B-V	-674 / 0	0.10 (1)
C-V	0 / 3458	0.74 (1)
Q-J	0 / 2501	0.75 (1)
F-S	0 / 1461	0.44 (1)
Q-I	-1187 / 0	0.44 (1)
S-G	-893 / 0	0.33 (1)
S-I	0 / 517	0.18 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.19")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 666 (0.36")

CSI: TC=0.87/1.00 (B-C:1) , BC=0.57/1.00 (T-U:1) , WB=0.75/1.00 (J-Q:1) , SSI=0.80/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.90 (O) (INPUT = 0.90)
METAL= 0.91 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28041-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T35	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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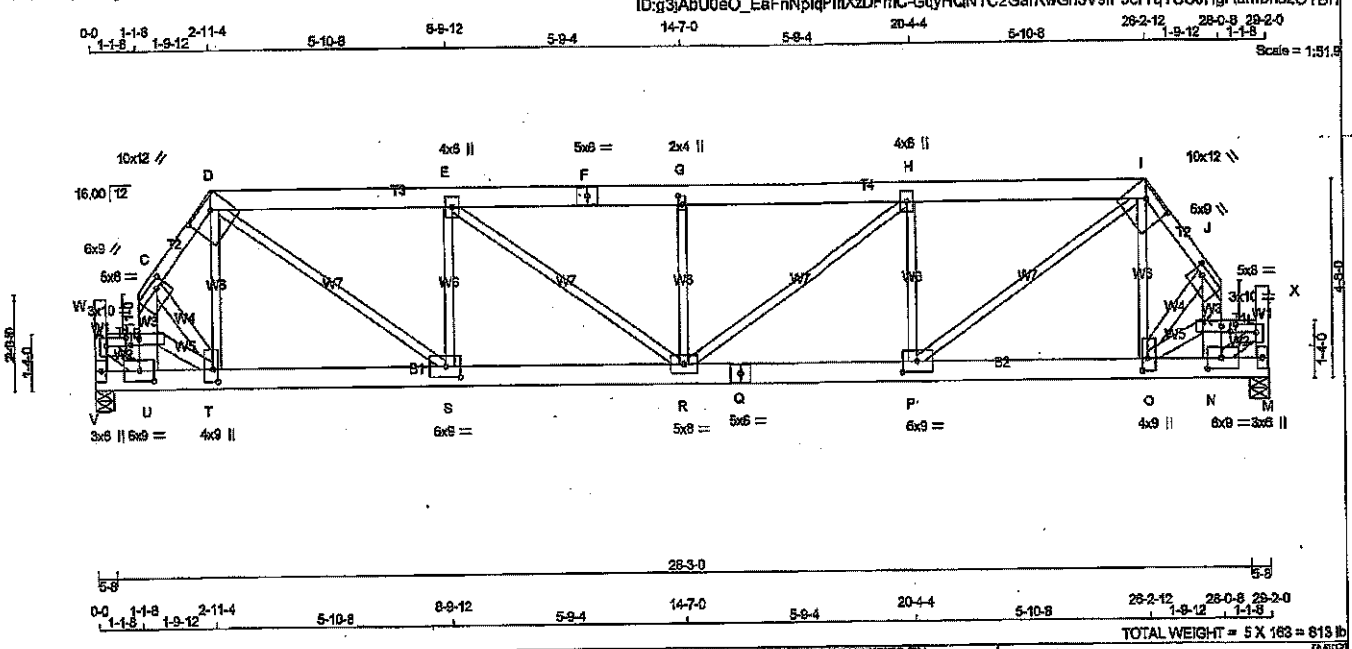

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
V - W	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - I	2x6	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
K - L	2x4	DRY	No.2	SPF
M - X	2x4	DRY	No.2	SPF
U - C	2x6	DRY	No.2	SPF
V - Q	2x6	DRY	No.2	SPF
Q - M	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
A - U	2x4	DRY	No.2	SPF
B - T	2x4	DRY	No.2	SPF
C - T	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
O - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	5.0	8.0	2.25	6.00
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.00	2.75
D	TTWW-h	MT20	10.0	12.0	2.75	7.25
E	TMWW-t	MT20	4.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TMWW-t	MT20	4.0	6.0		
I	TTWW-h	MT20	10.0	12.0	2.75	7.25
J	TMVW-t	MT20	6.0	9.0	2.00	2.75
K	TVMW-t	MT20	3.0	10.0	1.50	2.50
L	TVMW-t	MT20	5.0	6.0	2.25	6.00
M	BMV1+p	MT20	3.0	6.0		
N	BMVW-t	MT20	6.0	9.0	3.00	4.25
O	BMWWW-t	MT20	4.0	9.0	3.50	1.50
P	BMWW-t	MT20	6.0	9.0	3.00	4.25
Q	BS-t	MT20	5.0	6.0		
R	BMWWW-t	MT20	5.0	8.0		
S	BMWW-t	MT20	6.0	9.0	3.00	4.25
T	BMWWW-t	MT20	4.0	9.0	3.50	1.50
U	BMVW-t	MT20	6.0	9.0	3.00	4.25
V	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	2266	0	2266	0	0	5-8	5-8
M	2266	0	2266	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	1822	674 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0
M	1822	674 / 0	306 / 0	0 / 0	0 / 0	642 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
V- A	-1893 / 0	0.0	0.0	0.26 (1)	5.40	
A- W	0 / 0	0.0	0.0	0.00 (1)	10.00	
A- B	-2553 / 0	-127.4	-127.4	0.17 (1)	3.69	
C- D	-2431 / 0	-127.4	-127.4	0.19 (1)	3.77	
D- E	-3411 / 0	-127.4	-127.4	0.50 (1)	3.63	
E- F	-3987 / 0	-127.4	-127.4	0.55 (1)	3.33	
F- G	-3987 / 0	-127.4	-127.4	0.55 (1)	3.33	
G- H	-3987 / 0	-127.4	-127.4	0.55 (1)	3.33	
H- I	-3411 / 0	-127.4	-127.4	0.50 (1)	3.63	
I- J	-2431 / 0	-127.4	-127.4	0.19 (1)	3.77	
N- K	-2518 / 0	0.0	0.0	0.64 (1)	4.70	
K- J	-3236 / 0	0.0	0.0	0.66 (1)	4.11	
K- L	-2553 / 0	-127.4	-127.4	0.17 (1)	3.69	
M- L	-1893 / 0	0.0	0.0	0.26 (1)	5.40	
L- X	0 / 0	0.0	0.0	0.00 (1)	10.00	
U- B	-2518 / 0	0.0	0.0	0.64 (1)	4.70	
B- C	-3236 / 0	0.0	0.0	0.66 (1)	4.11	
V- U	0 / 0	-28.0	-28.0	0.43 (1)	10.00	
U- T	0 / 1866	-28.0	-28.0	0.72 (1)	10.00	
T- S	0 / 1454	-28.0	-28.0	0.34 (1)	10.00	
S- R	0 / 3411	-28.0	-28.0	0.63 (1)	10.00	
R- Q	0 / 3411	-28.0	-28.0	0.63 (1)	10.00	
Q- P	0 / 3411	-28.0	-28.0	0.63 (1)	10.00	
P- O	0 / 1454	-28.0	-28.0	0.34 (1)	10.00	
O- N	0 / 1866	-28.0	-28.0	0.72 (1)	10.00	
N- M	0 / 0	-28.0	-28.0	0.43 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- D	0 / 222	0.08 (3)
O- I	0 / 222	0.08 (3)
N- L	0 / 3128	0.67 (1)
P- I	0 / 2406	0.72 (1)
D- S	0 / 2406	0.72 (1)
P- H	-1253 / 0	0.47 (1)
S- E	-1253 / 0	0.47 (1)
R- H	0 / 712	0.21 (1)
E- R	0 / 712	0.21 (1)
R- G	-881 / 0	0.25 (1)
A- U	0 / 3128	0.67 (1)
B- T	-1511 / 0	0.22 (1)
C- T	0 / 1472	0.32 (1)
O- K	-1511 / 0	0.22 (1)
O- J	0 / 1472	0.32 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 39.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/ 899 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.72/1.00 (T-U:1) , WB=0.72/1.00 (I-P:1) , SSI=0.55/1.00 (K-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (U) (INPUT = 0.90)
JSI METAL= 0.75 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28042-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T35S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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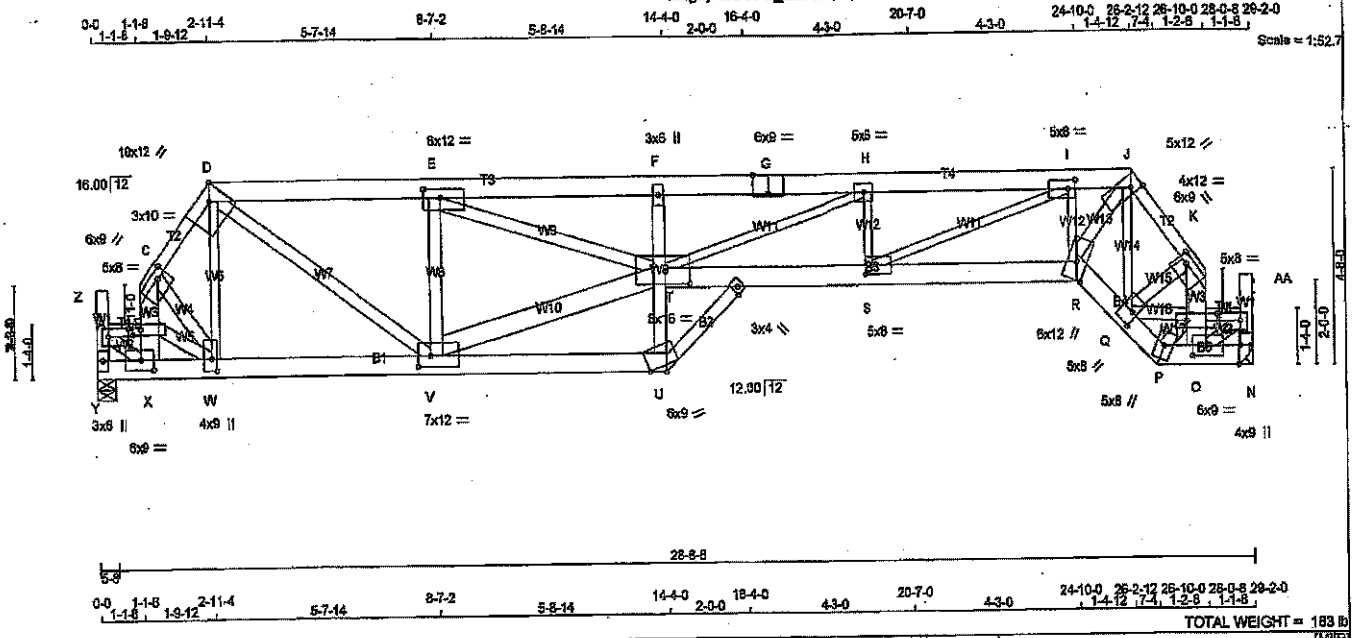

[M]R

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - Z	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	2100F 1.8E	SPF
G - J	2x6	DRY	2100F 1.8E	SPF
J - K	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - AA	2x4	DRY	No.2	SPF
X - C	2x6	DRY	No.2	SPF
Y - U	2x6	DRY	No.2	SPF
T - R	2x6	DRY	2100F 1.8E	SPF
R - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
W - D	2x3	DRY	No.2	SPF
R - I	2x3	DRY	No.2	SPF
V - T	2x6	DRY	No.2	SPF
Q - J	2x3	DRY	No.2	SPF
S - H	2x3	DRY	No.2	SPF
S - I	2x3	DRY	No.2	SPF
T - H	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	6.00
B	TVMW-I	MT20	9.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.00	3.00
D	TTWW-h	MT20	10.0	12.0	Edge	4.50
E	TMWW-t	MT20	6.0	12.0	2.50	5.00
F	TMVW+w	MT20	3.0	6.0		
G	TS-t	MT20	6.0	9.0	Edge	4.50
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	8.0	2.50	2.25
J	TTWW+h	MT20	5.0	12.0	3.25	1.75
K	TMVW-t	MT20	6.0	9.0	2.00	2.75
L	TVMWW-t	MT20	4.0	12.0	2.00	8.75
M	TVMW-I	MT20	5.0	8.0	2.25	6.00
N	BMV1+t	MT20	4.0	9.0	Edge	0.50
O	BMVW-t	MT20	6.0	9.0	3.00	4.00
P	BBW+m	MT20	5.0	8.0		
Q	BMWWW+t	MT20	5.0	8.0	3.75	1.50
R	BBWW+m	MT20	6.0	12.0	5.00	3.00
S	BMWW-t	MT20	5.0	8.0	2.50	2.25
T	BWMWWW*-I	MT20	8.0	16.0	4.50	7.25
U						
U	BBW-m	MT20	6.0	9.0	Edge	
V	BMWWW-t	MT20	7.0	12.0	3.25	3.75
W	BMWWW+t	MT20	4.0	6.0	3.50	1.50
X	BMVW-t	MT20	6.0	9.0	3.00	4.00
Y	BMV1+p	MT20	3.0	6.0		
AB	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Y	2266	0	2266	0	0	5-8	5-8
N	2266	0	2266	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT N.

ALLOW FOR 0.3" OF HORIZONTAL MOVEMENT DUE TO TOTAL LOAD

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Y	1921	874 / 0	308 / 0	0 / 0	0 / 0	641 / 0	0 / 0
N	1822	874 / 0	306 / 0	0 / 0	0 / 0	641 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Y

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.61 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FROM	TO	FACTORED VERT. LOAD LC1 (PLF)		MAX CSI (LC)	MAX. UNBRAC LENGTH
Y-A	-1901 / 0	0.0	0.0	0.26 (1)		5.39	
A-Z	0 / 0	0.0	0.0	0.00 (1)		10.00	
A-B	-2584 / 0	-127.4	-127.4	0.17 (1)		3.99	
C-D	-2409 / 0	-127.4	-127.4	0.18 (1)		3.78	
D-E	-3347 / 0	-127.4	-127.4	0.27 (1)		4.75	
E-F	-7400 / 0	-127.4	-127.4	0.43 (1)		3.27	
F-G	-7462 / 0	-127.4	-127.4	0.39 (1)		3.30	
G-H	-7462 / 0	-127.4	-127.4	0.39 (1)		3.30	
H-I	-6648 / 0	-127.4	-127.4	0.29 (1)		3.58	
I-J	-3950 / 0	-127.4	-127.4	0.14 (1)		4.58	
J-K	-3095 / 0	-127.4	-127.4	0.25 (1)		3.34	
O-L	-2707 / 0	0.0	0.0	0.45 (1)		5.00	
L-K	-3460 / 0	0.0	0.0	0.49 (1)		4.40	
L-M	-2568 / 0	-127.4	-127.4	0.17 (1)		3.88	
N-M	-1883 / 0	0.0	0.0	0.26 (1)		5.41	
M-AA	0 / 0	0.0	0.0	0.00 (1)		10.00	
X-B	-2475 / 0	0.0	0.0	0.63 (1)		4.75	
B-C	-3201 / 0	0.0	0.0	0.86 (1)		4.14	
Y-X	0 / 0	-28.0	-28.0	0.42 (1)		10.00	
X-W	0 / 1878	-28.0	-28.0	0.71 (1)		10.00	
W-V	0 / 1441	-28.0	-28.0	0.32 (3)		10.00	
V-U	0 / 78	-28.0	-28.0	0.18 (3)		10.00	
T-S	0 / 6648	-28.0	-28.0	0.51 (1)		10.00	
S-R	0 / 4031	-28.0	-28.0	0.34 (1)		10.00	
R-Q	0 / 2362	-28.0	-28.0	0.48 (1)		10.00	
Q-P	0 / 1935	-28.0	-28.0	0.41 (1)		10.00	
P-O	0 / 2098	-28.0	-28.0	0.77 (1)		10.00	
O-N	0 / 0	-28.0	-28.0	0.44 (1)		10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W-D	0 / 176	0.06 (3)
D-V	0 / 2394	0.61 (1)
V-E	-2441 / 0	0.65 (1)
U-T	0 / 114	0.29 (1)
T-F	-574 / 0	0.29 (1)
R-I	-1654 / 0	0.34 (1)
O-M	0 / 3146	0.67 (1)
V-T	0 / 3538	0.58 (1)
E-T	0 / 4276	0.92 (1)
B-W	-1527 / 0	0.22 (1)
C-W	0 / 1451	0.31 (1)
A-X	0 / 3141	0.67 (1)
Q-J	-1042 / 0	0.29 (1)
R-J	0 / 4036	0.87 (1)
Q-L	-909 / 0	0.13 (1)
P-L	-1158 / 0	0.16 (1)
Q-K	0 / 1712	0.37 (1)
S-H	-1040 / 0	0.24 (1)
S-I	0 / 2893	0.59 (1)
T-H	0 / 1680	0.26 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 5.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.32")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/574 (0.61")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.77/1.00 (O-P:1) , WB=0.92/1.00 (E-T:1) , SSI=0.55/1.00 (B-X:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.92 (T) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM28043-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T36	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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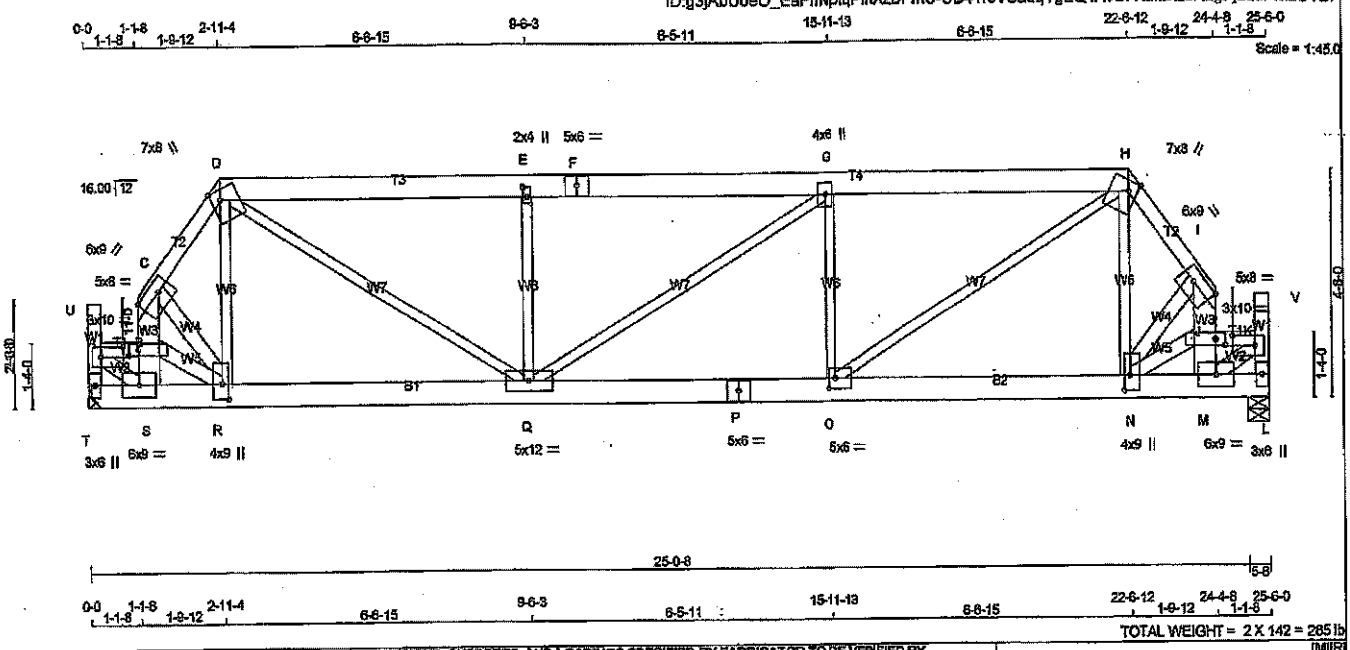

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - U	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
L - V	2x4	DRY	No.2	SPF
S - C	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.50	5.75
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	Edge	5.75
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW+t	MT20	4.0	6.0		
H	TTWW+m	MT20	7.0	8.0	2.50	2.25
I	TMW-t	MT20	6.0	9.0	Edge	5.75
J	TVMW-I	MT20	3.0	10.0	1.50	2.50
K	TVMW-I	MT20	5.0	8.0	2.50	5.75
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	6.0	9.0		
N	BMWWW+t	MT20	4.0	9.0	3.75	1.50
O	BMWW-t	MT20	5.0	6.0	2.50	1.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	12.0		
R	BMWWW+t	MT20	4.0	9.0	3.75	1.50
S	BMVW-t	MT20	6.0	9.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	1981	0	1981	0	0	MECHANICAL	
L	1981	0	1981	0	0	5-8	5-8

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT T.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1593	764 / 0	288 / 0	0 / 0	0 / 0	561 / 0	0 / 0
L	1593	764 / 0	288 / 0	0 / 0	0 / 0	561 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.79 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
T- A	-1651 / 0	0.0	0.0	0.23 (1)	5.73
A- U	0 / 0	0.0	0.0	0.00 (1)	10.00
A- B	-2210 / 0	-127.4	-127.4	0.14 (1)	3.97
C- D	-2105 / 0	-127.4	-127.4	0.16 (1)	4.03
D- E	-2927 / 0	-127.4	-127.4	0.54 (1)	3.81
E- F	-2927 / 0	-127.4	-127.4	0.55 (1)	3.79
F- G	-2927 / 0	-127.4	-127.4	0.55 (1)	3.79
G- H	-2933 / 0	-127.4	-127.4	0.55 (1)	3.79
H- I	-2102 / 0	-127.4	-127.4	0.16 (1)	4.04
M- J	-2200 / 0	0.0	0.0	0.55 (1)	5.25
J- I	-2811 / 0	0.0	0.0	0.57 (1)	4.65
J- K	-2212 / 0	-127.4	-127.4	0.14 (1)	3.97
L- K	-1651 / 0	0.0	0.0	0.23 (1)	5.73
K- V	0 / 0	0.0	0.0	0.00 (1)	10.00
S- B	-2205 / 0	0.0	0.0	0.55 (1)	5.24
B- C	-2814 / 0	0.0	0.0	0.57 (1)	4.65
T- S	0 / 0	-28.0	-28.0	0.36 (1)	10.00
S- R	0 / 1515	-28.0	-28.0	0.63 (1)	10.00
R- Q	0 / 1258	-28.0	-28.0	0.32 (1)	10.00
Q- P	0 / 2933	-28.0	-28.0	0.56 (1)	10.00
P- O	0 / 2933	-28.0	-28.0	0.56 (1)	10.00
O- N	0 / 1257	-28.0	-28.0	0.32 (1)	10.00
N- M	0 / 1616	-28.0	-28.0	0.63 (1)	10.00
M- L	0 / 0	-28.0	-28.0	0.36 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- D	0 / 261	0.09 (3)
N- H	0 / 259	0.09 (3)
M- K	0 / 2709	0.56 (1)
A- S	0 / 2708	0.56 (1)
B- R	-1308 / 0	0.19 (1)
C- R	0 / 1274	0.27 (1)
N- J	-1310 / 0	0.19 (1)
N- I	0 / 1272	0.27 (1)
O- H	0 / 1980	0.59 (1)
D- Q	0 / 1980	0.59 (1)
O- G	-900 / 0	0.34 (1)
Q- E	-896 / 0	0.33 (1)
Q- G	-6 / 0	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.63/1.00 (R-S:1) , WB=0.60/1.00 (H-O:1) , SSI=0.48/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.90 (K) (INPUT = 0.90)
METAL= 0.68 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28044/18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T36Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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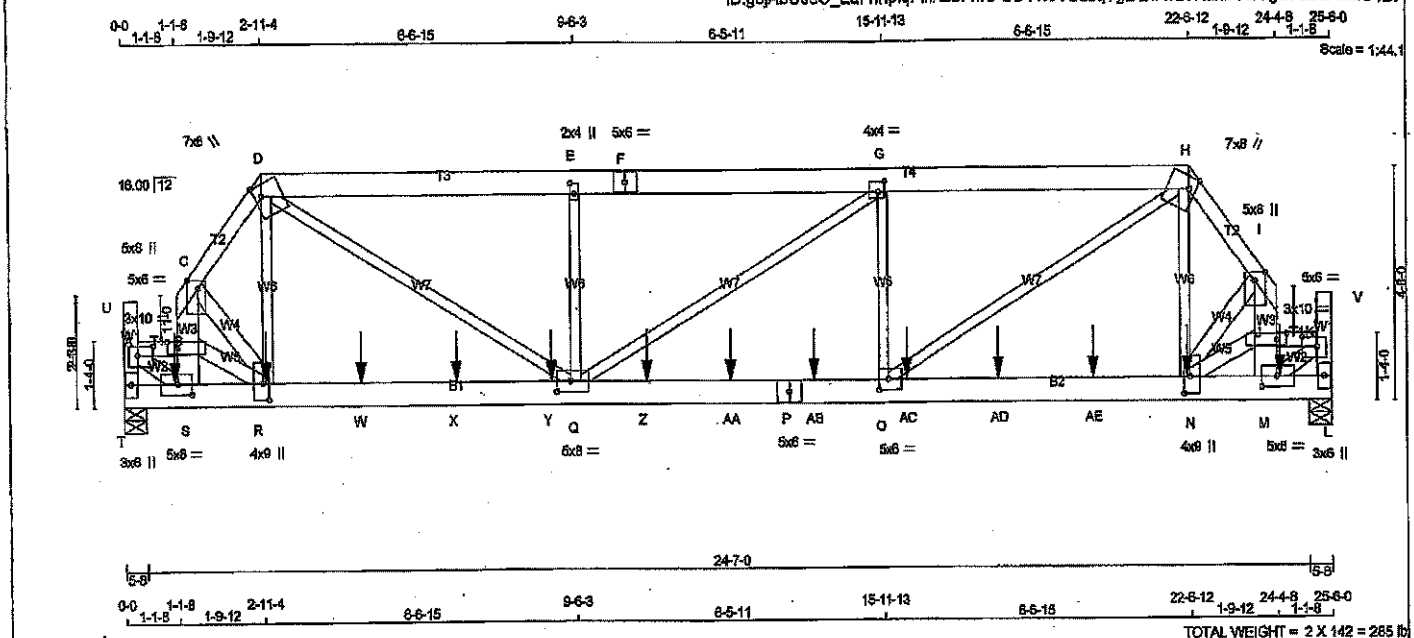

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
T - U	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - I	2x4	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
L - V	2x4	DRY	No.2	SPF
S - C	2x6	DRY	No.2	SPF
T - P	2x6	DRY	No.2	SPF
P - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
A - S	2x4	DRY	No.2	SPF
B - R	2x4	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
N - J	2x4	DRY	No.2	SPF
N - I	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
T-U	1	12	TOP
A-B	1	12	TOP
C-D	1	12	TOP
H-I	1	12	TOP
J-K	1	12	TOP
V-L	1	12	TOP
D-F	2	12	TOP
F-H	2	12	TOP
I-M	2	12	TOP
C-S	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-P	2	12	SIDE(137.3)
P-L	2	12	SIDE(137.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	6.0	2.25	3.75
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWV+p	MT20	5.0	8.0	2.00	2.75
D	TTWW+m	MT20	7.0	8.0	2.75	1.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	3330	0	3330	0	0	5-8	5-8
L	3330	0	3330	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	2591	1358 / 0	332 / 0	0 / 0	0 / 0	901 / 0	0 / 0
L	2591	1358 / 0	332 / 0	0 / 0	0 / 0	901 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
T-A	-2743 / 0	0.0	0.0	0.21 (1)	6.14	
A-U	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-3754 / 0	-127.4	-127.4	0.11 (1)	4.25	
C-D	-3669 / 0	-127.4	-127.4	0.12 (1)	4.28	
D-E	-4962 / 0	-127.4	-127.4	0.33 (1)	4.35	
E-F	-4963 / 0	-127.4	-127.4	0.31 (1)	4.38	
F-G	-4963 / 0	-127.4	-127.4	0.31 (1)	4.38	
G-H	-4969 / 0	-127.4	-127.4	0.33 (1)	4.35	
H-I	-3667 / 0	-127.4	-127.4	0.12 (1)	4.29	
M-J	-3763 / 0	0.0	0.0	0.51 (1)	5.63	
J-I	-4846 / 0	0.0	0.0	0.53 (1)	5.01	
J-K	-3768 / 0	-127.4	-127.4	0.11 (1)	4.25	
L-K	-2744 / 0	0.0	0.0	0.21 (1)	6.14	
K-V	0 / 0	0.0	0.0	0.00 (1)	10.00	
S-B	-3767 / 0	0.0	0.0	0.51 (1)	5.63	
B-C	-4849 / 0	0.0	0.0	0.53 (1)	5.01	
T-S	0 / 0	-28.0	-28.0	0.38 (1)	10.00	
S-R	0 / 2732	-28.0	-28.0	0.61 (1)	10.00	
R-W	0 / 2187	-28.0	-28.0	0.40 (1)	10.00	
W-X	0 / 2187	-28.0	-28.0	0.40 (1)	10.00	
X-Y	0 / 2187	-28.0	-28.0	0.40 (1)	10.00	
Y-Q	0 / 2187	-28.0	-28.0	0.40 (1)	10.00	
Q-Z	0 / 4969	-28.0	-28.0	0.63 (1)	10.00	
Z-AA	0 / 4969	-28.0	-28.0	0.63 (1)	10.00	
AA-P	0 / 4969	-28.0	-28.0	0.63 (1)	10.00	
P-AB	0 / 4969	-28.0	-28.0	0.63 (1)	10.00	
AB-O	0 / 4969	-28.0	-28.0	0.63 (1)	10.00	
O-AC	0 / 2186	-28.0	-28.0	0.40 (1)	10.00	
AC-AD	0 / 2186	-28.0	-28.0	0.40 (1)	10.00	
AD-AE	0 / 2186	-28.0	-28.0	0.40 (1)	10.00	
AE-N	0 / 2186	-28.0	-28.0	0.40 (1)	10.00	
N-M	0 / 2733	-28.0	-28.0	0.61 (1)	10.00	
M-L	0 / 0	-28.0	-28.0	0.38 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-D	0 / 706	0.12 (1)
N-H	0 / 701	0.12 (1)
M-K	0 / 4601	0.54 (1)
A-S	0 / 4599	0.54 (1)
B-R	-2198 / 0	0.17 (1)
C-R	0 / 2244	0.26 (1)
N-J	-2199 / 0	0.17 (1)
N-I	0 / 2241	0.26 (1)
O-H	0 / 3309	0.55 (1)
D-Q	0 / 3293	0.54 (1)
O-G	-891 / 0	0.17 (1)
Q-E	-890 / 0	0.17 (1)
Q-G	-8 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	24-5-4	-200	-200	—	FRONT	VERT	TOTAL
N	22-6-0	-209	-209	—	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.85")
CALCULATED VERT. DEFL.(LL) = L/ 998 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.85")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.18")

CSI: TC=0.53/1.00 (B-C:1) , BC=0.63/1.00 (O-Q:1) , WB=0.55/1.00 (H-Q:1) , SSI=0.45/1.00 (B-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)
JSI METAL= 0.62 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2 B0458
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T36Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	TMW+w	MT20	2.0	4.0	2.50	1.00
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	4.0	4.0	2.50	1.75
H	TTWW+m	MT20	7.0	8.0	2.75	1.75
I	TMWW+p	MT20	5.0	8.0	2.00	2.75
J	TVMW-l	MT20	3.0	10.0	1.50	2.50
K	TVMW-l	MT20	5.0	6.0	2.25	3.75
L	BMV1+p	MT20	3.0	6.0		
M	BMVW-l	MT20	5.0	8.0	2.50	3.75
N	BMWWW+t	MT20	4.0	9.0	4.00	1.50
O	BMWW-t	MT20	5.0	6.0	2.50	2.50
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0	2.50	3.50
R	BMWWW+t	MT20	4.0	9.0	4.00	1.50
S	BMVW-t	MT20	5.0	8.0	2.50	3.75
T	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 12-9-0, 208.8 lbs FACTORED DOWN AT 14-6-0, 208.8 lbs FACTORED DOWN AT 16-6-0, 208.8 lbs FACTORED DOWN AT 18-6-0, 208.8 lbs FACTORED DOWN AT 20-6-0, AND 208.8 lbs FACTORED DOWN AT 22-6-0, AND 200.3 lbs FACTORED DOWN AT 24-5-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
R	3-0-0	-209	-209	—	FRONT	VERT	TOTAL
S	1-0-12	-200	-200	—	FRONT	VERT	TOTAL
W	5-0-0	-209	-209	—	FRONT	VERT	TOTAL
X	7-0-0	-209	-209	—	FRONT	VERT	TOTAL
Y	9-0-0	-209	-209	—	FRONT	VERT	TOTAL
Z	11-0-0	-209	-209	—	FRONT	VERT	TOTAL
AA	12-9-0	-209	-209	—	FRONT	VERT	TOTAL
AB	14-6-0	-209	-209	—	FRONT	VERT	TOTAL
AC	16-6-0	-209	-209	—	FRONT	VERT	TOTAL
AD	18-6-0	-209	-209	—	FRONT	VERT	TOTAL
AE	20-6-0	-209	-209	—	FRONT	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28048-B
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T37	1	4	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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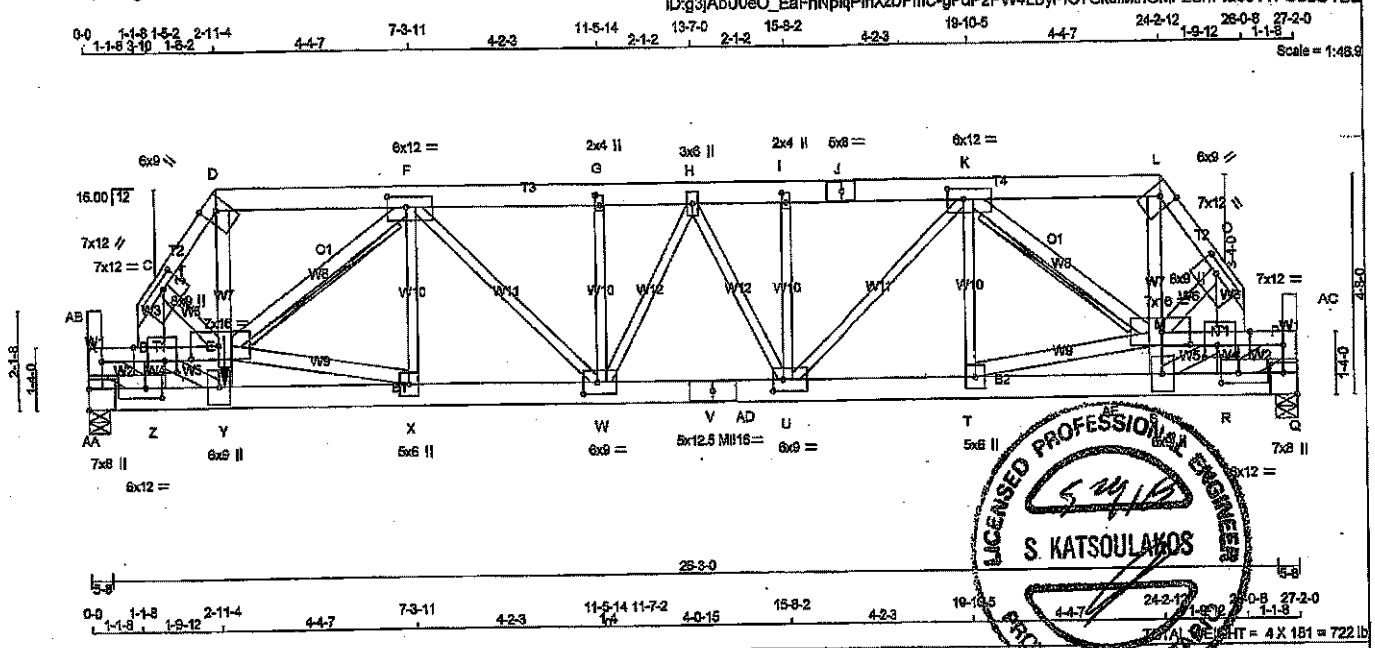

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
AA- AB	2x4	DRY	No.2	SPF
A - E	2x4	DRY	2100F 1.8E	SPF
C - D	2x4	DRY	No.2	SPF
D - J	2x6	DRY	No.2	SPF
J - L	2x6	DRY	No.2	SPF
L - O	2x4	DRY	No.2	SPF
M - P	2x4	DRY	2100F 1.8E	SPF
Q - AC	2x4	DRY	No.2	SPF
B - C	2x6	DRY	No.2	SPF
N - O	2x6	DRY	No.2	SPF
AA- V	2x6	DRY	2100F 1.8E	SPF
V - Q	2x6	DRY	2100F 1.8E	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
R - N	2x6	DRY	No.2	SPF
R - P	2x6	DRY	2100F 1.8E	SPF
Z - B	2x6	DRY	No.2	SPF
A - Z	2x6	DRY	2100F 1.8E	SPF
T - K	2x3	DRY	No.2	SPF
X - F	2x3	DRY	No.2	SPF
U - K	2x3	DRY	No.2	SPF
F - W	2x3	DRY	No.2	SPF
U - I	2x3	DRY	No.2	SPF
W - G	2x3	DRY	No.2	SPF
W - H	2x3	DRY	No.2	SPF
H - U	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 4 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
AA- AB	1	12	TOP
A- E	1	12	TOP
C- D	1	12	TOP
L- O	1	12	TOP
M- P	1	12	TOP
AC- Q	1	12	TOP
D- J	2	12	TOP
J- L	2	12	TOP
C- B	2	12	TOP
O- N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
AA- V	3	4	SIDE(826.2)
V- Q	3	4	SIDE(826.2)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
Y- D	1	4	SIDE(1084.6)
2x4	1	-	
E- C	1	6	
M- O	1	6	
N- S	1	6	
B- Y	1	6	
E- F	1	6	
K- M	1	6	
X- E	1	6	
T- M	1	6	
2x6	2	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
AA	17124	0	17124	0	0	5-8	5.8
Q	16883	0	16883	0	0	5-8	5.8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
AA	13677	6682 / 0	2191 / 0	0 / 0	0 / 0	4805 / 0	0 / 0
Q	13561	6522 / 0	2284 / 0	0 / 0	0 / 0	4774 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) AA, Q

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x6 DRY SPF No.2 T-BRACE AT E-F, K-M

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
AA- A	-14989 / 0	0.0	0.0	0.61 (1)	3.31	
A-AB	0 / 0	0.0	0.0	0.00 (1)	10.00	
A- B	-22420 / 0	-127.4	-127.4	0.84 (1)	2.33	
B- E	-20271 / 0	0.0	0.0	0.63 (1)	3.17	
C- D	-16870 / 0	-127.4	-127.4	0.51 (1)	2.35	
D- F	-10772 / 0	-127.4	-127.4	0.16 (1)	4.23	
F- G	-27468 / 0	-127.4	-127.4	0.70 (1)	2.28	
G- H	-27468 / 0	-127.4	-127.4	0.84 (1)	2.31	
H- I	-27021 / 0	-127.4	-127.4	0.82 (1)	2.35	
I- J	-27021 / 0	-127.4	-127.4	0.86 (1)	2.32	
J- K	-27021 / 0	-127.4	-127.4	0.68 (1)	2.32	
K- L	-9480 / 0	-127.4	-127.4	0.13 (1)	4.48	
L- O	-14849 / 0	-127.4	-127.4	0.40 (1)	2.84	
M- N	-18437 / 0	0.0	0.0	0.56 (1)	3.46	
N- P	-21176 / 0	-127.4	-127.4	0.59 (1)	2.48	
Q- P	-14160 / 0	0.0	0.0	0.58 (1)	3.46	
P-AC	0 / 0	0.0	0.0	0.00 (1)	10.00	
B- C	-22063 / 0	0.0	0.0	0.38 (1)	4.39	
N- O	-19432 / 0	0.0	0.0	0.36 (1)	4.70	
AA- Z	0 / 0	-28.0	-28.0	0.46 (1)	10.00	
Z- Y	0 / 22420	-28.0	-28.0	0.87 (1)	10.00	
Y- X	0 / 18686	-1129.6	-1129.6	0.57 (1)	10.00	
X- W	0 / 22913	-1129.6	-1129.6	0.85 (1)	10.00	
W- V	0 / 27402	-1129.6	-1129.6	0.68 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Y- E	0 / 10721	0.68 (1)
E- D	0 / 13163	0.81 (1)
S- M	0 / 7879	0.52 (1)
M- L	0 / 11550	0.72 (1)
C- E	0 / 14181	0.76 (1)
M- O	0 / 12482	0.67 (1)
R- N	-16094 / 0	0.94 (1)
S- N	-4555 / 0	0.16 (1)
R- P	0 / 25129	0.40 (1)
Z- B	-18918 / 0	0.40 (1)
A- Z	0 / 28605	0.42 (1)
B- Y	-4048 / 0	0.14 (1)
E- F	-15320 / 0	0.60 (1)
K- M	-15544 / 0	0.61 (1)
T- K	0 / 3534	0.27 (1)
X- F	0 / 4084	0.31 (1)
U- K	0 / 7407	0.56 (1)
F- W	0 / 6461	0.48 (1)
U- I	-87 / 52	0.01 (1)
W- G	-89 / 51	0.01 (1)
E- X	0 / 4332	0.23 (1)
T- M	0 / 4877	0.26 (1)
W- H	0 / 147	0.01 (1)
H- U	-856 / 0	0.10 (1)

[MIR]

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

GIRDER TYPE: CStdGirder
START DISTANCE = 3-0-12
START SPAN CARRIED = 30-4-4
END DISTANCE = 14-8-0
END SPAN CARRIED = 30-4-4
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 14-8-0
START SPAN CARRIED = 28-4-4
END DISTANCE = 23-0-0
END SPAN CARRIED = 28-4-4
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

GIRDER TYPE: CStdGirder
START DISTANCE = 23-0-0
START SPAN CARRIED = 30-4-4
END DISTANCE = 27-2-0
END SPAN CARRIED = 30-4-4
END WALL WIDTH = 0-0
APPLIED TO FRONT SIDE OF BOTTOM CHORD.
- ADDT'L LOADS BASED ON 55 % OF GSL.

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD. 18

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 20096

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T37	1	4	TRUSS DESC.	

IN ADDITION, PRE-DRILL ONE 0.56" DIAM. HOLE IN EACH CHORD PANEL AND INSTALL 0.50" DIAM. ASTM A307 BOLTS WITH WASHERS, BOTH SIDES. FOR OTHER BOLT TYPES SEE CSAO86 3.3.2.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-l	MT20	7.0	12.0	Edge	8.75
B	TMVWW+p	MT20	8.0	9.0	3.00	3.25
C	TMVW-t	MT20	7.0	12.0	2.00	4.75
D	TTW+h	MT20	6.0	9.0	Edge	
E	TWMWWW*-l	MT20	7.0	16.0	3.25	7.50
F	TMWWW-t	MT20	6.0	12.0	2.75	5.00
G	TMW+w	MT20	2.0	4.0	2.50	1.00
H	TMWW+t	MT20	3.0	6.0		
I	TMW+w	MT20	2.0	4.0	2.50	1.00
J	TS-t	MT20	5.0	8.0		
K	TMWWW-t	MT20	6.0	12.0	2.75	4.50
L	TTW+h	MT20	6.0	9.0	Edge	
M	TWMWWW*-l	MT20	7.0	16.0	3.25	7.50
N	TMVWW+p	MT20	8.0	9.0	3.00	3.25
O	TMVW-t	MT20	7.0	12.0	2.00	4.75
P	TVMW-l	MT20	7.0	12.0	Edge	8.75
Q	BMV1+p	MT20	7.0	8.0	5.50	Edge
R	BMWW-t	MT20	6.0	12.0	2.50	4.50
S	BMWW+t	MT20	6.0	9.0		
T	BMWW+t	MT20	5.0	8.0		
U	BMWWW-t	MT20	6.0	9.0	3.00	2.75
V	BS-t	MII16	5.0	12.5		
W	BMWWW-t	MT20	6.0	9.0	3.00	3.75
X	BMWW+t	MT20	5.0	6.0		
Y	BMWW+t	MT20	6.0	9.0		
Z	BMWW-t	MT20	6.0	12.0	2.50	4.50
AA	BMV1+p	MT20	7.0	8.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3883.3 lbs FACTORED DOWN AT 3-0-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
V-AD	0 / 27402	-1129.6	-1129.6	0.68 (1)	10.00	
AD-U	0 / 27402	-1051.9	-1051.9	0.68 (1)	10.00	
U-T	0 / 21799	-1051.9	-1051.9	0.60 (1)	10.00	
T-AE	0 / 17043	-1051.9	-1051.9	0.52 (1)	10.00	
AE-S	0 / 17043	-1129.6	-1129.6	0.52 (1)	10.00	
S-R	0 / 21178	-1129.6	-1129.6	0.80 (1)	10.00	
R-Q	0 / 0	-1129.6	-1129.6	0.42 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
Y	3-0-12	-1314	-1471	—	FRONT	VERT	DEAD
Y	3-0-12	-194	-581	—	FRONT	VERT	LIVE
Y	3-0-12	-2376	-2376	—	FRONT	VERT	SNOW

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.20")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 888 (0.36")

CSI: TC=0.70/1.00 (F-G:1) , BC=0.87/1.00 (Y-Z:1) , WB=0.81/1.00 (D-E:1) , SSI=0.74/1.00 (R-S:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656
MII16	473	276	2341	1245	4454	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (R) (INPUT = 0.90)
JSI METAL= 0.93 (Z) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM2 [illegible]
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T38S	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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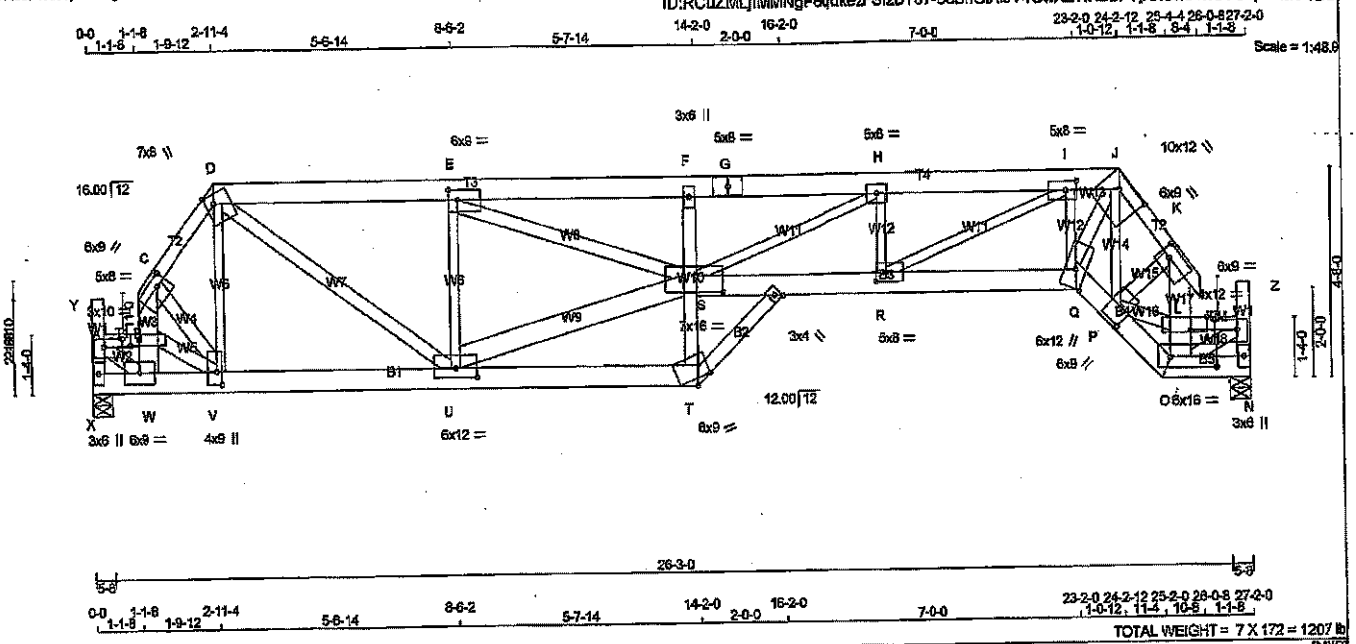

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
X - Y	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - Z	2x4	DRY	No.2	SPF
W - C	2x6	DRY	No.2	SPF
X - T	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
V - D	2x3	DRY	No.2	SPF
U - E	2x3	DRY	No.2	SPF
Q - I	2x3	DRY	No.2	SPF
P - J	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
R - H	2x3	DRY	No.2	SPF
S - H	2x3	DRY	No.2	SPF
R - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	5.00
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMWW-t	MT20	6.0	9.0	2.75	2.50
F	TMW+w	MT20	3.0	6.0		
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	8.0	2.50	3.00
J	TTWW-h	MT20	10.0	12.0	3.00	7.50
K	TMW-t	MT20	6.0	9.0	2.75	2.50
L	TVMW-I	MT20	4.0	12.0	Edge	4.50
M	TVMW-I	MT20	6.0	9.0	2.75	6.25
N	BMV1+p	MT20	3.0	6.0		
O	BBVW-I	MT20	6.0	16.0	2.75	12.50
P	BMWWW+t	MT20	6.0	9.0	4.50	2.75
Q	BBWW+m	MT20	6.0	12.0	5.00	3.00
R	BMWW-t	MT20	5.0	8.0	2.50	3.00
S	BWMWWW-I	MT20	7.0	18.0	4.50	7.25
T						
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.50	6.00
V	BMWWW+t	MT20	4.0	9.0	3.50	1.50
W	BMVW-t	MT20	6.0	9.0		
X	BMVt+p	MT20	3.0	6.0		
AA	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	2111	0	2111	0	0	5-8	5-8
N	2111	0	2111	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	1697	814 / 0	285 / 0	0 / 0	0 / 0	596 / 0	0 / 0
N	1697	814 / 0	285 / 0	0 / 0	0 / 0	596 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
X-A	-1771 / 0	0.0	0.0	0.24 (1)	5.56
A-Y	0 / 0	0.0	0.0	0.00 (1)	10.00
A-B	-2380 / 0	-127.4	-127.4	0.15 (1)	3.83
C-D	-2228 / 0	-127.4	-127.4	0.17 (1)	3.93
D-E	-3014 / 0	-127.4	-127.4	0.50 (1)	3.81
E-F	-6416 / 0	-127.4	-127.4	0.64 (1)	2.28
F-G	-6480 / 0	-127.4	-127.4	0.66 (1)	2.51
G-H	-6460 / 0	-127.4	-127.4	0.68 (1)	2.51
H-I	-5615 / 0	-127.4	-127.4	0.50 (1)	2.90
I-J	-3372 / 0	-127.4	-127.4	0.19 (1)	3.99
J-K	-3067 / 0	-127.4	-127.4	0.23 (1)	3.34
O-L	-4461 / 0	0.0	0.0	0.57 (1)	3.75
L-K	-3666 / 0	0.0	0.0	0.55 (1)	4.21
L-M	-3943 / 0	-127.4	-127.4	0.35 (1)	2.83
N-M	-2084 / 0	0.0	0.0	0.29 (1)	5.17
M-Z	0 / 0	0.0	0.0	0.00 (1)	10.00
W-B	-2295 / 0	0.0	0.0	0.58 (1)	5.05
B-C	-2964 / 0	0.0	0.0	0.61 (1)	4.45
X-W	0 / 0	-28.0	-28.0	0.39 (1)	10.00
W-V	0 / 1744	-28.0	-28.0	0.66 (1)	10.00
V-U	0 / 1331	-28.0	-28.0	0.31 (3)	10.00
U-T	0 / 74	-28.0	-28.0	0.17 (3)	10.00
S-R	0 / 5615	-28.0	-28.0	0.43 (1)	10.00
R-Q	0 / 3445	-28.0	-28.0	0.30 (1)	10.00
Q-P	0 / 2390	-28.0	-28.0	0.45 (1)	10.00
P-O	0 / 2972	-28.0	-28.0	0.54 (1)	10.00
O-N	0 / 0	-28.0	-28.0	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V-D	0 / 179	0.06 (3)
D-U	0 / 2126	0.46 (1)
U-E	-2196 / 0	0.82 (1)
T-S	0 / 112	0.28 (1)
S-F	-529 / 0	0.28 (1)
Q-I	-1469 / 0	0.31 (1)
O-M	0 / 4414	0.95 (1)
P-J	-861 / 0	0.22 (1)
Q-J	0 / 3584	0.77 (1)
U-S	0 / 3189	0.50 (1)
R-H	-1035 / 0	0.21 (1)
S-H	0 / 982	0.29 (1)
R-I	0 / 2416	0.73 (1)
E-S	0 / 3593	0.77 (1)
P-L	-1757 / 0	0.24 (1)
P-K	0 / 1687	0.40 (1)
B-V	-1420 / 0	0.20 (1)
A-W	0 / 2916	0.62 (1)
C-V	0 / 1339	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.91")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.29")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 588 (0.56")

CSI: TC=0.84/1.00 (E-F:1) , BC=0.66/1.00 (V-W:1) , WB=0.95/1.00 (M-O:1) , SSI=0.51/1.00 (B-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.90 (S) (INPUT = 0.90)
METAL= 0.98 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280478
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T38SZ	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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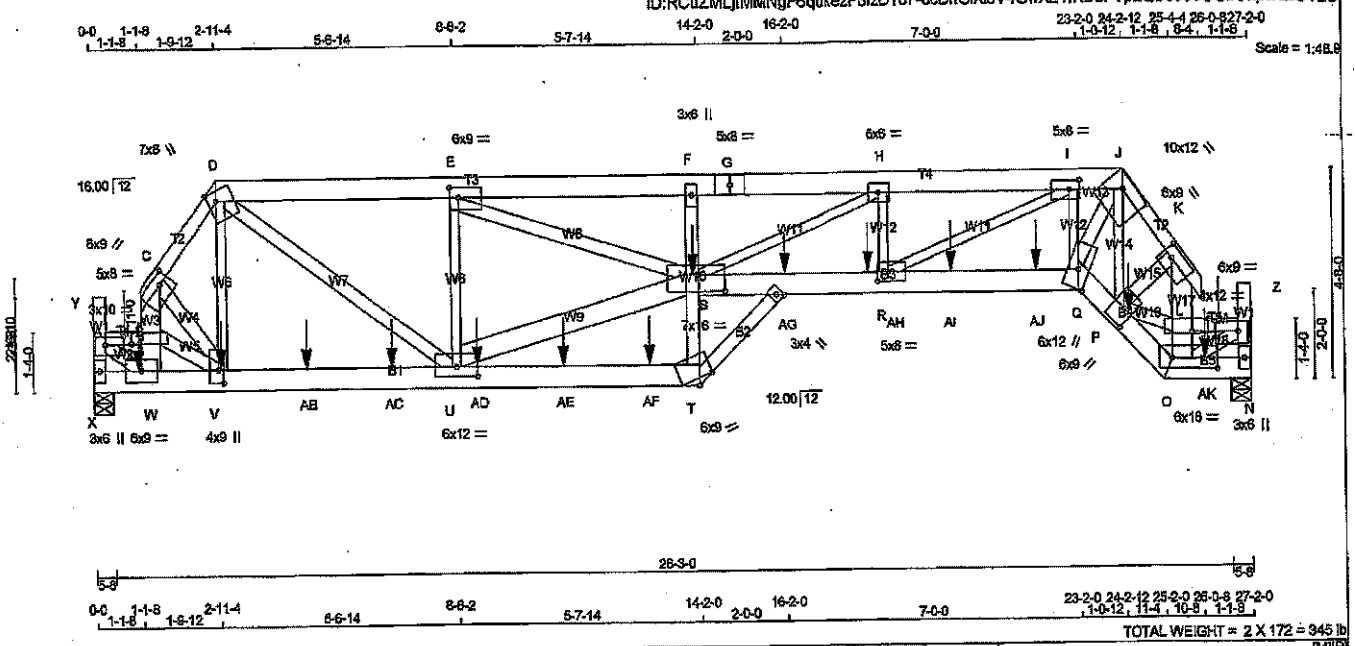

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
X - Y	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - J	2x6	DRY	No.2	SPF
J - K	2x4	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
L - M	2x4	DRY	No.2	SPF
N - Z	2x4	DRY	No.2	SPF
W - C	2x6	DRY	No.2	SPF
X - T	2x6	DRY	No.2	SPF
S - Q	2x6	DRY	2100F 1.8E	SPF
Q - O	2x6	DRY	No.2	SPF
O - N	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
V - D	2x3	DRY	No.2	SPF
U - E	2x3	DRY	No.2	SPF
Q - I	2x3	DRY	No.2	SPF
P - J	2x3	DRY	No.2	SPF
U - S	2x6	DRY	No.2	SPF
R - H	2x3	DRY	No.2	SPF
S - H	2x3	DRY	No.2	SPF
R - I	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
X-Y	1	12	TOP
A-B	1	12	TOP
C-D	1	12	TOP
J-K	1	12	TOP
L-M	1	12	TOP
Z-N	1	12	TOP
D-G	2	12	TOP
G-J	2	12	TOP
K-O	2	12	TOP
C-W	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
X-T	2	12	SIDE(137.3)
S-Q	2	12	SIDE(0.0)
Q-O	2	12	SIDE(137.3)
O-N	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
X	3421	0	3421	0	0	5-8	5-8
N	3183	0	3183	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
X	2671	1388 / 0	353 / 0	0 / 0	0 / 0	930 / 0	0 / 0
N	2502	1277 / 0	351 / 0	0 / 0	0 / 0	873 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
X- A	-2854 / 0	0.0	0.0	0.22 (1)	6.04	
A- Y	0 / 0	0.0	0.0	0.00 (1)	10.00	
A- B	-3911 / 0	-127.4	-127.4	0.11 (1)	4.17	
C- D	-3683 / 0	-127.4	-127.4	0.12 (1)	4.28	
D- E	-4867 / 0	-127.4	-127.4	0.30 (1)	4.42	
E- F	-10051 / 0	-127.4	-127.4	0.54 (1)	2.98	
F- G	-10152 / 0	-127.4	-127.4	0.49 (1)	3.00	
G- H	-10152 / 0	-127.4	-127.4	0.49 (1)	3.00	
H- I	-8516 / 0	-127.4	-127.4	0.33 (1)	3.44	
I- J	-5009 / 0	-127.4	-127.4	0.11 (1)	4.57	
J- K	-4591 / 0	-127.4	-127.4	0.15 (1)	3.86	
O- L	-6585 / 0	0.0	0.0	0.45 (1)	4.54	
L- K	-5475 / 0	0.0	0.0	0.43 (1)	5.00	
L- M	-5897 / 0	-127.4	-127.4	0.23 (1)	3.36	
N- M	-3067 / 0	0.0	0.0	0.23 (1)	5.68	
M- Z	0 / 0	0.0	0.0	0.00 (1)	10.00	
W- B	-3692 / 0	0.0	0.0	0.52 (1)	5.66	
B- C	-4844 / 0	0.0	0.0	0.54 (1)	4.99	
X- W	0 / 0	-28.0	-28.0	0.36 (1)	10.00	
W- V	0 / 2864	-28.0	-28.0	0.61 (1)	10.00	
V-AB	0 / 2198	-28.0	-28.0	0.40 (1)	10.00	
AB-AC	0 / 2198	-28.0	-28.0	0.40 (1)	10.00	
AC- U	0 / 2198	-28.0	-28.0	0.40 (1)	10.00	
U-AD	0 / 116	-28.0	-28.0	0.22 (1)	10.00	
AD-AE	0 / 116	-28.0	-28.0	0.22 (1)	10.00	
AE-AF	0 / 116	-28.0	-28.0	0.22 (1)	10.00	
AF- T	0 / 116	-28.0	-28.0	0.22 (1)	10.00	
S-AG	0 / 8516	-28.0	-28.0	0.38 (1)	10.00	
AG-AH	0 / 8516	-28.0	-28.0	0.38 (1)	10.00	
AH- R	0 / 8516	-28.0	-28.0	0.38 (1)	10.00	
R-AI	0 / 5110	-28.0	-28.0	0.24 (1)	10.00	
AI-AJ	0 / 5110	-28.0	-28.0	0.24 (1)	10.00	
AJ- Q	0 / 5110	-28.0	-28.0	0.24 (1)	10.00	
Q- P	0 / 3605	-28.0	-28.0	0.36 (1)	10.00	
P- O	0 / 4469	-28.0	-28.0	0.44 (1)	10.00	
O-AK	0 / 0	-28.0	-28.0	0.05 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
V- D	0 / 606	0.08 (1)
D- U	0 / 3370	0.40 (1)
U- E	-2926 / 0	0.57 (1)
T- S	0 / 467	0.28 (1)
S- F	-476 / 0	0.23 (1)
Q- I	-2075 / 0	0.23 (1)
Q- M	0 / 6601	0.78 (1)
P- J	-1173 / 0	0.16 (1)
Q- J	0 / 5274	0.62 (1)
U- S	0 / 5133	0.44 (1)
R- H	-1431 / 0	0.16 (1)
S- H	0 / 1826	0.30 (1)
R- I	0 / 3795	0.63 (1)
E- S	0 / 5490	0.65 (1)
P- L	-2653 / 0	0.20 (1)
P- K	0 / 2824	0.33 (1)
B- V	-2329 / 0	0.18 (1)
A- W	0 / 4791	0.56 (1)
C- V	0 / 2223	0.26 (1)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL.	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.61")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.23")
ALLOWABLE DEFL.(TL)= L/360 (0.91")
CALCULATED VERT. DEFL.(TL) = L/ 757 (0.43")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.61/1.00 (V-W:1) , WB=0.78/1.00 (M-O:1) , SSI=0.48/1.00 (B-W:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM280487B

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T38SZ	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-l	MT20	6.0	8.0	2.25	5.00
B	TVMW-l	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMWW-t	MT20	6.0	9.0	2.75	2.50
F	TMW+w	MT20	3.0	8.0		
G	TS-t	MT20	5.0	8.0		
H	TMWW-t	MT20	5.0	6.0		
I	TMWW-t	MT20	5.0	8.0	2.50	3.00
J	TTWW-h	MT20	10.0	12.0	3.00	7.50
K	TMVW-t	MT20	6.0	9.0	2.75	2.50
L	TVMW-l	MT20	4.0	12.0	Edge	4.50
M	TVMW-i	MT20	6.0	9.0	2.75	6.25
N	BMV1+p	MT20	3.0	6.0		
O	BBVW-l	MT20	6.0	16.0	2.75	12.50
P	BMWWW+t	MT20	6.0	9.0	4.50	2.75
Q	BBWW+m	MT20	6.0	12.0	5.00	3.00
R	BMWW-t	MT20	5.0	6.0	2.50	3.00
S	BWMWWW"-l	MT20	7.0	16.0	4.50	7.25
T						
T						
T	BBW-m	MT20	6.0	9.0	Edge	
U	BMWWW-t	MT20	6.0	12.0	2.50	6.00
V	BMWWW-t	MT20	4.0	9.0	3.50	1.50
W	BMWW-t	MT20	6.0	9.0		
X	BMV1+p	MT20	3.0	6.0		
AA	NP+h	MT20	3.0	4.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 200.3 lbs FACTORED DOWN AT 1-0-12, 208.8 lbs FACTORED DOWN AT 3-0-0, 208.8 lbs FACTORED DOWN AT 5-0-0, 208.8 lbs FACTORED DOWN AT 7-0-0, 208.8 lbs FACTORED DOWN AT 9-0-0, 208.8 lbs FACTORED DOWN AT 11-0-0, 208.8 lbs FACTORED DOWN AT 13-0-0, 172.4 lbs FACTORED DOWN AT 14-0-6, 161.2 lbs FACTORED DOWN AT 16-2-0, 161.2 lbs FACTORED DOWN AT 24-2-0, 202.5 lbs FACTORED DOWN AT 26-1-4, 77.4 lbs FACTORED DOWN AT 18-2-0, AND 77.4 lbs FACTORED DOWN AT 20-2-0, AND 77.4 lbs FACTORED DOWN AT 22-2-0 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX. CSI (LC)
AK- N	0 / 0	-28.0	-28.0	0.05 (1)	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
MAX. UNBRAC LENGTH FR-TO		
10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
P	24-2-0	-161	-161	---	BACK	VERT	TOTAL
T	14-0-6	-172	-172	---	BACK	VERT	TOTAL
V	3-0-0	-209	-209	---	BACK	VERT	TOTAL
W	1-0-12	-200	-200	---	BACK	VERT	TOTAL
AB	5-0-0	-209	-209	---	BACK	VERT	TOTAL
AC	7-0-0	-209	-209	---	BACK	VERT	TOTAL
AD	9-0-0	-209	-209	---	BACK	VERT	TOTAL
AE	11-0-0	-209	-209	---	BACK	VERT	TOTAL
AF	13-0-0	-209	-209	---	BACK	VERT	TOTAL
AG	16-2-0	-161	-161	---	BACK	VERT	TOTAL
AH	18-2-0	-77	-77	---	BACK	VERT	TOTAL
AI	20-2-0	-77	-77	---	BACK	VERT	TOTAL
AJ	22-2-0	-77	-77	---	BACK	VERT	TOTAL
AK	26-1-4	-202	-202	---	BACK	VERT	TOTAL

JSI GRIP= 0.74 (A) (INPUT = 0.90)
JSI METAL= 0.73 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280486
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T39	31	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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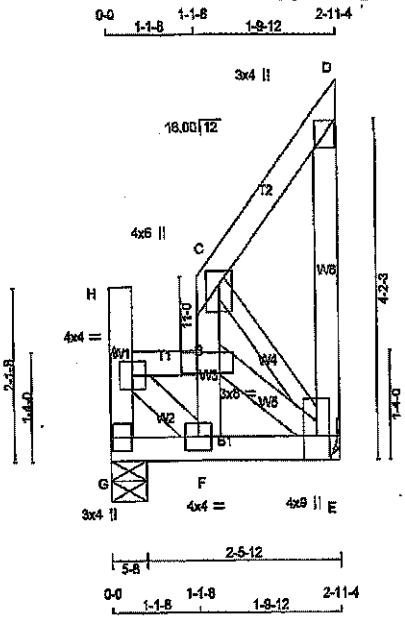

TOTAL WEIGHT = 31 X 22 = 673 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - H	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x4	DRY	No.2	SPF
EXCEPT				
C - E	2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TWMW*-I	MT20	3.0	8.0	1.50	2.50
C	TMWW+w	MT20	4.0	6.0	2.00	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1+p	MT20	4.0	9.0	Edge	
F	BMWW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	236	0	236	0	0	5-8	5-8
E	237	0	237	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	191	93 / 0	31 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	190	92 / 0	31 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 7.81 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
G-A	-219 / 0	0.0	0.0	0.02 (1)	7.81	
A-H	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-157 / 0	-127.4	-127.4	0.03 (1)	6.25	
C-D	0 / 0	-127.4	-127.4	0.08 (1)	10.00	
E-D	-109 / 0	0.0	0.0	0.03 (1)	7.81	
G-F	0 / 0	-28.0	-28.0	0.01 (3)	10.00	
F-E	0 / 156	-28.0	-28.0	0.04 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-89 / 5	0.01 (1)
B-C	-107 / 0	0.01 (1)
A-F	0 / 203	0.03 (1)
B-E	-191 / 0	0.02 (1)
C-E	-4 / 3	0.00 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.06/1.00 (C-D:1) , BC=0.04/1.00 (E-F:1) , WB=0.03/1.00 (A-F:1) , SSI=0.07/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.21 (F) (INPUT = 0.60)
ISI METAL= 0.08 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM280649-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T40	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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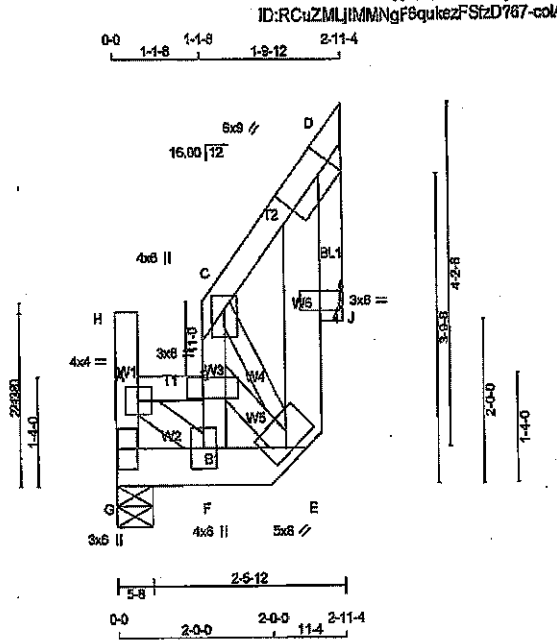

TOTAL WEIGHT = 5 X 26 = 128 lb

(M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - H	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TVMW-I	MT20	3.0	6.0	1.50	2.50
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	TMVK1-I	MT20	6.0	9.0	Edge	2.50
E	BMVWWm	MT20	5.0	8.0	1.00	2.75
F	BMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	6.0		
I	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	222	0	222	0	0	5-8	5-8
J(D)	189	0	189	0	0	MECHANICAL	

(** SEE "BEARING NOTE" **)

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J(D).

UNFACTORED REACTIONS

1ST LCASE - MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	178	86 / 0	30 / 0	0 / 0	0 / 0	63 / 0	0 / 0
J(D)	152	73 / 0	26 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
G-A	-164 / 0	0.0	0.0	0.02 (1)	7.81	
A-H	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-197 / 0	-127.4	-127.4	0.03 (1)	6.25	
C-D	-19 / 0	-127.4	-127.4	0.03 (1)	6.25	
E-I	0 / 117	0.0	0.0	0.03 (1)	10.00	
I-D	0 / 117	0.0	0.0	0.03 (1)	10.00	
F-B	-105 / 0	0.0	0.0	0.01 (1)	7.81	
B-C	-108 / 0	0.0	0.0	0.01 (1)	7.81	
G-F	0 / 0	-28.0	-28.0	0.02 (1)	10.00	
F-E	0 / 127	-28.0	-28.0	0.03 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	-143 / 0	0.01 (1)
C-E	0 / 4	0.00 (1)
A-F	0 / 171	0.03 (1)
D-J	-190 / 0	0.02 (1)
I-J	0 / 21	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 89.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.03/1.00 (C-D:1) , BC=0.03/1.00 (E-F:1) , WB=0.03/1.00 (A-F:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.08 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28050-18
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T40A	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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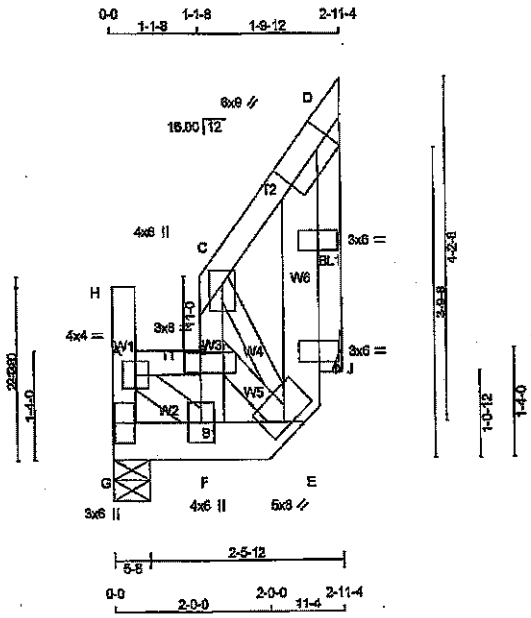

TOTAL WEIGHT = 27 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - H	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
G - E	2x6	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

BEARING NOTE: GAP BETWEEN INSIDE OF TOP CHORD BEARING AND FIRST DIAGONAL OR VERTICAL WEB SHALL NOT EXCEED 0.5 INCHES.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TVMW-I	MT20	3.0	8.0	1.50	2.50
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	TMVK1-t	MT20	6.0	8.0	Edge	2.50
E	BMVWWm	MT20	5.0	8.0	1.00	2.75
F	BMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	8.0		
I	KP-p	MT20	3.0	8.0		
I	KP-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	222	0	222	0	0	5-8	5-8
J(D)	189	0	189	0	0	MECHANICAL	

(** SEE "BEARING NOTE" **)

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT J(D).

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	178	86 / 0	30 / 0	0 / 0	0 / 0	63 / 0	0 / 0
J(D)	152	73 / 0	26 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
G-A	-184 / 0	0.0	0.0	0.02 (1)	7.81
A-H	0 / 0	0.0	0.0	0.00 (1)	10.00
A-B	-137 / 0	-127.4	-127.4	0.03 (1)	6.25
C-D	-18 / 0	-127.4	-127.4	0.03 (1)	6.25
E-I	0 / 117	0.0	0.0	0.02 (1)	10.00
I-D	0 / 117	0.0	0.0	0.02 (1)	10.00
F-B	-105 / 0	0.0	0.0	0.01 (1)	7.81
B-C	-106 / 0	0.0	0.0	0.01 (1)	7.81
G-F	0 / 0	-28.0	-28.0	0.02 (1)	10.00
F-E	0 / 127	-28.0	-28.0	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	-143 / 0	0.01 (1)
C-E	0 / 4	0.00 (1)
A-F	0 / 171	0.03 (1)
D-J	-190 / 0	0.04 (1)
I-J	0 / 15	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.03/1.00 (C-D:1) , BC=0.03/1.00 (E-F:1) , WB=0.04/1.00 (D-J:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	622	2284	1858

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (A) (INPUT = 0.90)
JSI METAL= 0.06 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28051-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T41	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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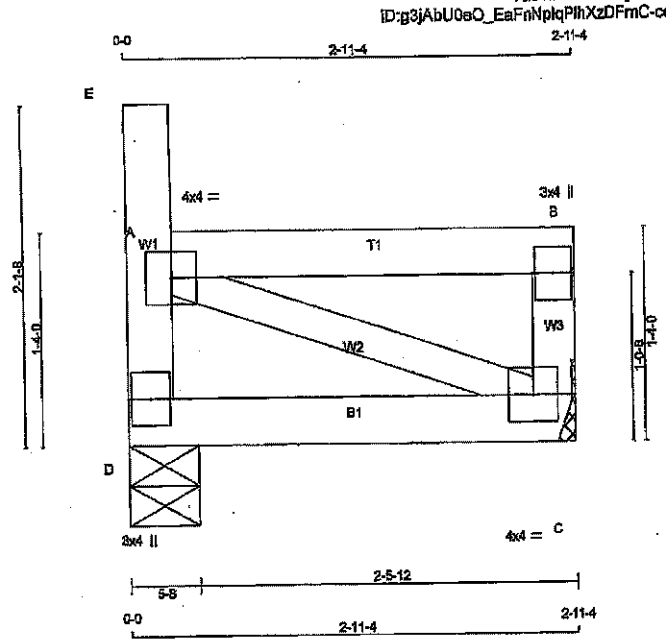

TOTAL WEIGHT = 10 X 11 = 113 lb

[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - E	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TVMW-I	MT20	4.0	4.0	
B	TMV+p	MT20	3.0	4.0	
C	BMVW1-t	MT20	4.0	4.0	
D	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	228	0	228	0	0	5-8	5-8
C	228	0	228	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	184	88 / 0	31 / 0	0 / 0	0 / 0	65 / 0	0 / 0
C	184	88 / 0	31 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
D- A	-187 / 0	0.0	0.0	0.03 (1)	7.81	
A- E	0 / 0	0.0	0.0	0.00 (1)	10.00	
A- B	0 / 0	-127.4	-127.4	0.25 (1)	10.00	
C- B	-187 / 0	0.0	0.0	0.03 (1)	7.81	
D- C	0 / 0	-28.0	-28.0	0.11 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- C	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.25/1.00 (A-B:1) , BC=0.11/1.00 (C-D:3) , WB=0.00/1.00 (A-C:1) , SSI=0.19/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.34 (A) (INPUT = 0.80)
JSI METAL= 0.09 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM2 80528
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T42	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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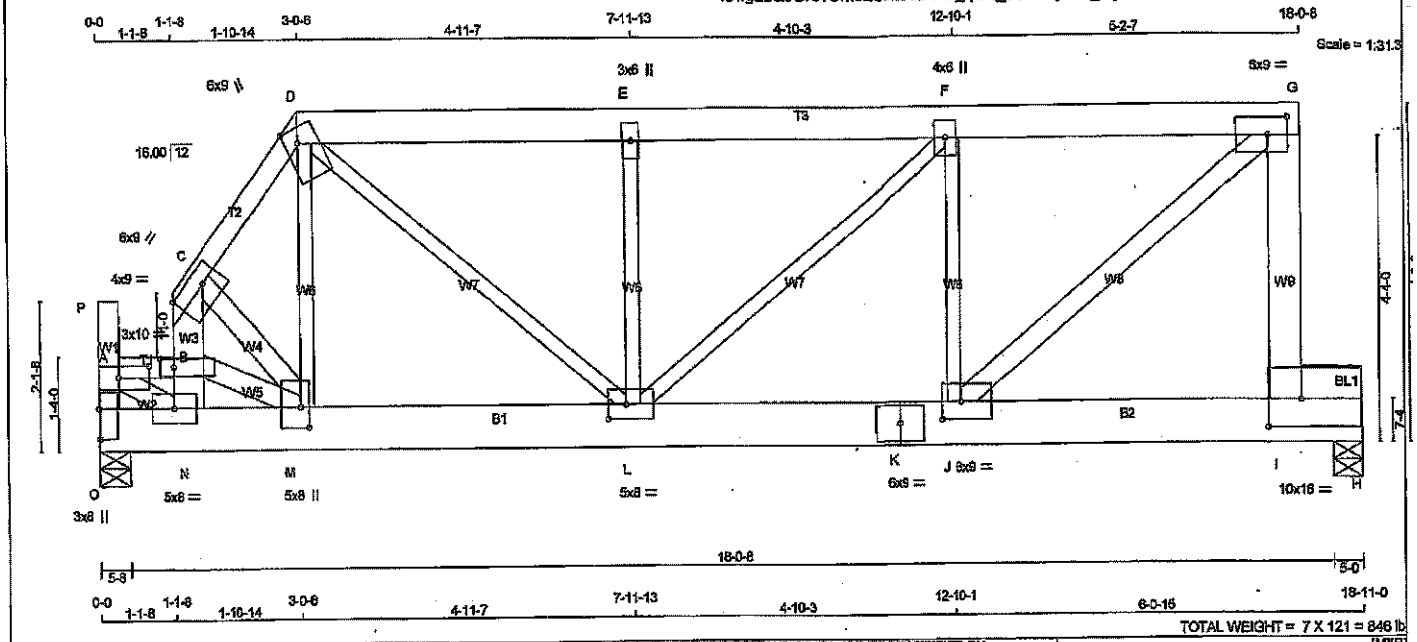

TOTAL WEIGHT = 7 X 121 = 846 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - P	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
N - C	2x6	DRY	No.2	SPF
O - K	2x8	DRY	1950F 1.7E	SPF
K - H	2x8	DRY	1950F 1.7E	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-t	MT20	4.0	9.0	2.00	5.50
B	TVMW-t	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	Edge	5.75
D	TTWW+m	MT20	6.0	9.0	2.50	2.25
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMWW-t	MT20	6.0	9.0	3.00	3.50
I	BMWK*-t	MT20	10.0	16.0	4.75	5.75
J	BMWW-t	MT20	6.0	9.0	3.00	3.50
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	5.0	6.0	2.50	3.25
M	BMWWW+t	MT20	6.0	6.0	3.50	1.50
N	BMW-t	MT20	6.0	8.0		
O	BMV1+p	MT20	3.0	8.0	5.25	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	1956	0	1956	0	0	5-8	5-8
H	2107	0	2107	0	0	5-0	5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1506	891 / 0	199 / 0	0 / 0	0 / 0	417 / 0	0 / 0
H	1608	986 / 0	199 / 0	0 / 0	0 / 0	423 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
O-A	-1286 / 0	0.0	0.0	0.19 (1)	6.31	
A-P	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-1834 / 0	-127.4	-127.4	0.13 (1)	4.27	
C-D	-2115 / 0	-127.4	-127.4	0.19 (1)	3.95	
D-E	-2323 / 0	-200.3	-200.3	0.49 (1)	4.20	
E-F	-2323 / 0	-200.3	-200.3	0.52 (1)	4.14	
F-G	-2292 / 0	-200.3	-200.3	0.53 (1)	4.16	
N-B	-2468 / 0	0.0	0.0	0.54 (1)	4.95	
B-C	-2845 / 0	0.0	0.0	0.56 (1)	4.61	
O-N	0 / 0	-28.0	-28.0	0.29 (1)	10.00	
N-M	0 / 1274	-28.0	-28.0	0.37 (1)	10.00	
M-L	0 / 1261	-28.0	-28.0	0.17 (1)	10.00	
L-K	0 / 2292	-28.0	-28.0	0.30 (1)	10.00	
K-J	0 / 2292	-28.0	-28.0	0.30 (1)	10.00	
J-I	-130 / 0	-28.0	-28.0	0.74 (1)	6.25	
I-H	0 / 0	-155.4	-155.4	0.40 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-G	-2418 / 0	0.43 (1)
M-D	0 / 279	0.09 (3)
B-M	-954 / 0	0.15 (1)
C-M	0 / 1331	0.31 (1)
A-N	0 / 2192	0.52 (1)
J-G	0 / 3129	0.74 (1)
D-L	0 / 1999	0.46 (1)
J-F	-1135 / 0	0.44 (1)
L-E	-1049 / 0	0.41 (1)
L-F	0 / 41	0.01 (1)

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.66/1.00 (B-C:1) , BC=0.74/1.00 (I-J:1) , WB=0.74/1.00 (G-J:1) , SSI=0.94/1.00 (H-I:1)

.DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAROS
PROVINCE OF ONTARIO

DWG NO. TAM 2 80531B
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T42	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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ID:fgZzQJGrcTUmcES8Ls3usrzD_qd-4_JYhRYyd6K_9rirPIG3UQuviFWac1XYYWDv?WzC1BB

JSI GRIP= 0.89 (M) (INPUT = 0.90)
JSI METAL= 0.62 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 2805318
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T42Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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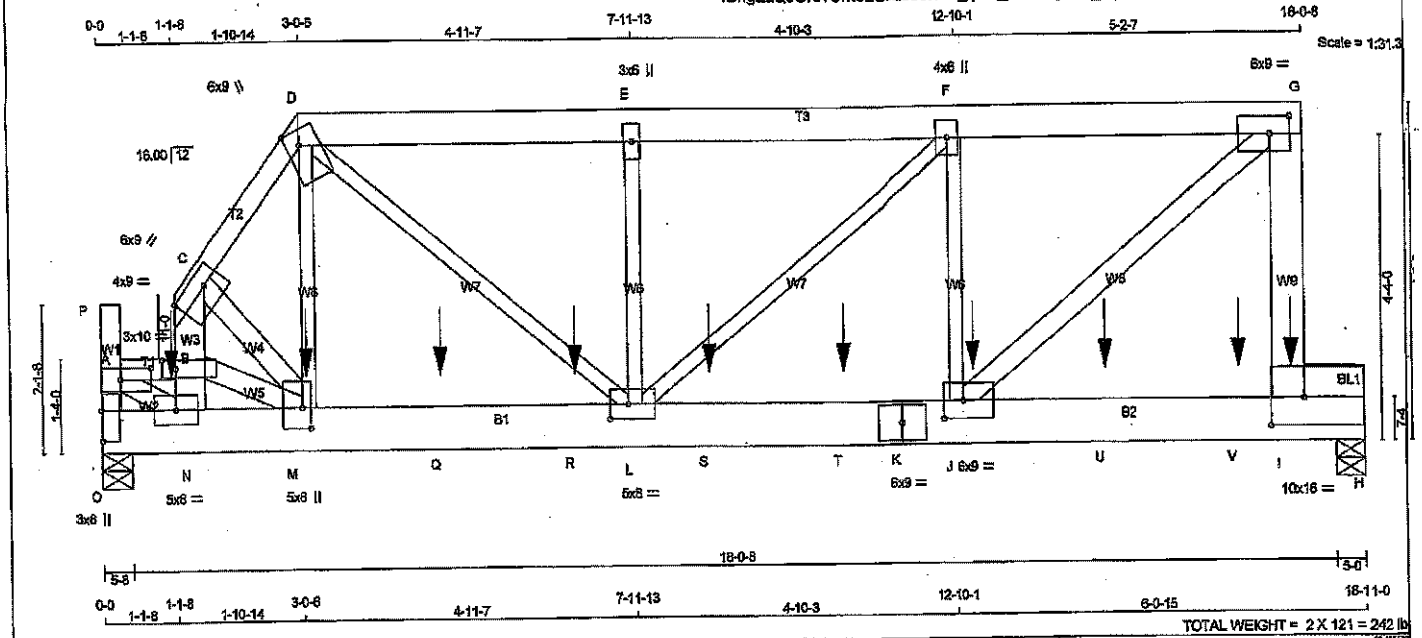

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
O - P	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
N - C	2x6	DRY	No.2	SPF
O - K	2x8	DRY	1950F 1.7E	SPF
K - H	2x8	DRY	1950F 1.7E	SPF
BEARING BLOCKS				
BL1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
B - M	2x4	DRY	No.2	SPF
C - M	2x4	DRY	No.2	SPF
A - N	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
O-P	1	12	TOP
A-B	1	12	TOP
C-D	1	12	TOP
D-G	2	12	SIDE(36.4)
C-N	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
O-K	2	12	SIDE(137.3)
K-H	2	12	SIDE(137.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	
2x6	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	9.0	2.00	5.50
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-I	MT20	6.0	9.0	Edge	5.75
D	TTWW+m	MT20	6.0	9.0	2.50	2.25
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	4.0	6.0		
G	TMWW-t	MT20	6.0	9.0	3.00	3.50
I	BMWK-I	MT20	10.0	16.0	4.75	5.75
J	BMWW-t	MT20	6.0	9.0	3.00	3.50
K	BS-t	MT20	6.0	9.0		
L	BMWWW-t	MT20	5.0	8.0	2.50	3.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2787	0	2787	0	0	5-8	5-8
H	2976	0	2976	0	0	5-0	5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX.MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2116	1320 / 0	250 / 0	0 / 0	0 / 0	547 / 0	0 / 0
H	2247	1435 / 0	256 / 0	0 / 0	0 / 0	557 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
O-A	-1817 / 0	0.0	0.0	0.14 (1)	7.24
A-P	0 / 0	0.0	0.0	0.00 (1)	10.00
A-B	-2647 / 0	-127.4	-127.4	0.07 (1)	4.96
C-D	-3026 / 0	-127.4	-127.4	0.10 (1)	4.66
D-E	-3240 / 0	-200.3	-200.3	0.25 (1)	5.25
E-F	-3240 / 0	-200.3	-200.3	0.25 (1)	5.25
F-G	-3188 / 0	-200.3	-200.3	0.27 (1)	5.26
N-B	-3437 / 0	0.0	0.0	0.38 (1)	6.16
B-C	-4025 / 0	0.0	0.0	0.38 (1)	5.76
O-N	0 / 0	-28.0	-28.0	0.21 (1)	10.00
N-M	0 / 1843	-28.0	-28.0	0.27 (1)	10.00
M-Q	0 / 1800	-28.0	-28.0	0.11 (1)	10.00
Q-R	0 / 1800	-28.0	-28.0	0.11 (1)	10.00
R-L	0 / 1800	-28.0	-28.0	0.11 (1)	10.00
L-S	0 / 3188	-28.0	-28.0	0.24 (1)	10.00
S-T	0 / 3188	-28.0	-28.0	0.24 (1)	10.00
T-K	0 / 3188	-28.0	-28.0	0.24 (1)	10.00
K-J	0 / 3188	-28.0	-28.0	0.24 (1)	10.00
J-U	-171 / 0	-28.0	-28.0	0.53 (1)	6.25
U-V	-171 / 0	-28.0	-28.0	0.53 (1)	6.25
V-I	-171 / 0	-28.0	-28.0	0.53 (1)	6.25
I-H	0 / 0	-155.4	-155.4	0.28 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-G	-3192 / 0	0.28 (1)
M-D	0 / 659	0.11 (1)
B-M	-1385 / 0	0.11 (1)
C-M	0 / 1903	0.22 (1)
A-N	0 / 3162	0.37 (1)
J-G	0 / 4342	0.51 (1)
D-L	0 / 1898	0.31 (1)
J-F	-1148 / 0	0.22 (1)
L-E	-1044 / 0	0.20 (1)
L-F	0 / 69	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
I	17-10-4	-128	-136	—	BACK	VERT	TOTAL
J	13-1-2	-170	-170	—	BACK	VERT	TOTAL
M	3-1-2	-188	-188	—	BACK	VERT	TOTAL
N	1-0-12	-183	-183	—	BACK	VERT	TOTAL
Q	5-1-2	-170	-170	—	BACK	VERT	TOTAL
R	7-1-2	-170	-170	—	BACK	VERT	TOTAL
S	9-1-2	-170	-170	—	BACK	VERT	TOTAL
T	11-1-2	-170	-170	—	BACK	VERT	TOTAL
U	15-1-2	-170	-170	—	BACK	VERT	TOTAL
V	17-1-2	-171	-171	—	BACK	VERT	TOTAL

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN.C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.63")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.63")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.53/1.00 (I-J:1) , WB=0.51/1.00 (G-J:1) , SSI=0.66/1.00 (H-I:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	822	2284	1659

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 292709
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T42Z	1	2	TRUSS DESC.	

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
M	BMWWW+t	MT20	5.0	8.0	3.50	1.50
N	BMVW-t	MT20	5.0	8.0		
O	BMV1+p	MT20	3.0	8.0	5.25	0.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 183.3 lbs FACTORED DOWN AT 1-0-12, 187.6 lbs FACTORED DOWN AT 3-1-2, 170.0 lbs FACTORED DOWN AT 5-1-2, 170.0 lbs FACTORED DOWN AT 7-1-2, 170.0 lbs FACTORED DOWN AT 9-1-2, 170.0 lbs FACTORED DOWN AT 11-1-2, 170.0 lbs FACTORED DOWN AT 13-1-2, 170.0 lbs FACTORED DOWN AT 15-1-2, AND 171.0 lbs FACTORED DOWN AT 17-1-2, AND 138.5 lbs FACTORED DOWN AT 17-10-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

JSI GRIP= 0.64 (M) (INPUT = 0.90)
JSI METAL= 0.46 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280548
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T43	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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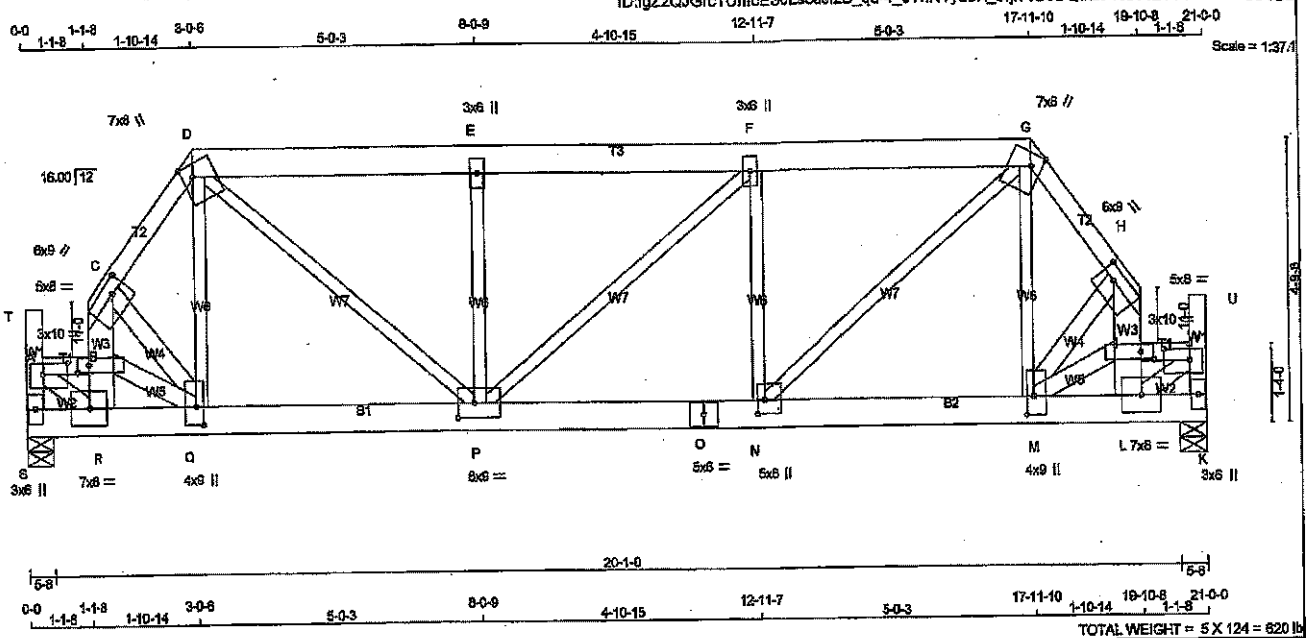

TOTAL WEIGHT = 5 X 124 = 620 lb

[MIR]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - T	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - U	2x4	DRY	No.2	SPF
R - C	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - J	2x4	DRY	No.2	SPF
A - R	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
C - Q	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	5.25
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMVW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMW+w	MT20	3.0	6.0		
F	TMWW+t	MT20	3.0	6.0		
G	TTWW+m	MT20	7.0	8.0	2.50	2.25
H	TMVW-t	MT20	6.0	9.0	2.25	3.00
I	TVMW-I	MT20	3.0	10.0	1.50	2.50
J	TVMW-I	MT20	5.0	8.0	2.25	5.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	7.0	8.0		
M	BMWWW+t	MT20	4.0	9.0	3.75	1.50
N	BMWW+t	MT20	5.0	8.0	2.75	1.50
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	9.0	3.00	3.50
Q	BMWWW+t	MT20	4.0	9.0	3.75	1.50
R	BMVW-t	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2176	0	2176	0	0	5-8	5-8
K	2176	0	2176	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1675	992 / 0	221 / 0	0 / 0	0 / 0	462 / 0	0 / 0
K	1675	992 / 0	221 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
S-A	-1824 / 0	0.0	0.0	0.27 (1)	5.43
A-T	0 / 0	0.0	0.0	0.00 (1)	10.00
A-B	-2456 / 0	-127.4	-127.4	0.19 (1)	3.70
C-D	-2312 / 0	-127.4	-127.4	0.21 (1)	3.78
D-E	-2757 / 0	-200.3	-200.3	0.53 (1)	3.85
E-F	-2757 / 0	-200.3	-200.3	0.53 (1)	3.85
F-G	-2771 / 0	-200.3	-200.3	0.53 (1)	3.85
G-H	-2307 / 0	-127.4	-127.4	0.21 (1)	3.78
L-I	-2365 / 0	0.0	0.0	0.66 (1)	4.70
I-H	-3022 / 0	0.0	0.0	0.69 (1)	4.10
I-J	-2458 / 0	-127.4	-127.4	0.19 (1)	3.69
K-J	-1826 / 0	0.0	0.0	0.28 (1)	5.42
J-U	0 / 0	0.0	0.0	0.00 (1)	10.00
R-B	-2372 / 0	0.0	0.0	0.66 (1)	4.70
B-C	-3028 / 0	0.0	0.0	0.69 (1)	4.10
S-R	0 / 0	-28.0	-28.0	0.44 (1)	10.00
R-Q	0 / 1605	-28.0	-28.0	0.76 (1)	10.00
Q-P	0 / 1382	-28.0	-28.0	0.34 (1)	10.00
P-O	0 / 2771	-28.0	-28.0	0.57 (1)	10.00
O-N	0 / 2771	-28.0	-28.0	0.57 (1)	10.00
N-M	0 / 1380	-28.0	-28.0	0.33 (1)	10.00
M-L	0 / 1608	-28.0	-28.0	0.75 (1)	10.00
L-K	0 / 0	-28.0	-28.0	0.44 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-D	0 / 210	0.08 (3)
M-G	0 / 205	0.08 (3)
I-J	0 / 3012	0.71 (1)
A-R	0 / 3009	0.71 (1)
B-Q	-1455 / 0	0.23 (1)
C-Q	0 / 1354	0.32 (1)
M-I	-1458 / 0	0.23 (1)
M-H	0 / 1350	0.32 (1)
N-G	0 / 1637	0.81 (1)
D-P	0 / 1616	0.80 (1)
N-F	-1071 / 0	0.44 (1)
P-E	-1067 / 0	0.44 (1)
P-F	-18 / 0	0.02 (1)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.69/1.00 (B-C:1) , BC=0.76/1.00 (Q-R:1) , WB=0.71/1.00 (J-L:1) , SSI=0.69/1.00 (B-R:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

DWG NO. TAM 2605598
STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T43	5	1	TRUSS DESC.	

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.87 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28055-18
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T43Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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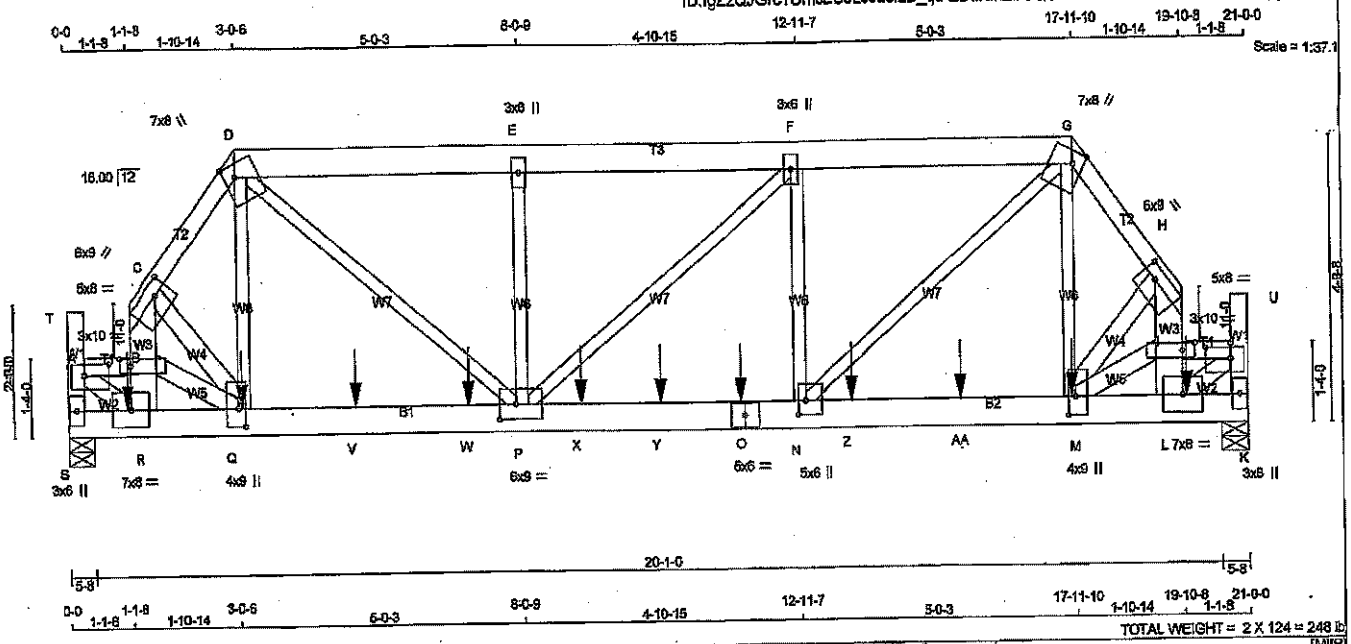

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - T	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - G	2x6	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - U	2x4	DRY	No.2	SPF
R - C	2x6	DRY	No.2	SPF
S - O	2x6	DRY	No.2	SPF
Q - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
A - R	2x4	DRY	No.2	SPF
B - Q	2x4	DRY	No.2	SPF
C - Q	2x4	DRY	No.2	SPF
M - I	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
S-T	1	12	TOP
A-B	1	12	TOP
C-D	1	12	TOP
G-H	1	12	TOP
I-J	1	12	TOP
U-K	1	12	TOP
D-G	2	12	SIDE(36.4)
H-L	2	12	TOP
C-R	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-O	2	12	SIDE(137.3)
O-K	2	12	SIDE(137.3)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	5.0	8.0	2.25	5.25
B	TVMW-I	MT20	3.0	10.0	1.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.25	3.00
D	TTWW+m	MT20	7.0	8.0	2.50	2.25
E	TMW+w	MT20	3.0	8.0		
F	TMWW+t	MT20	3.0	5.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	3317	0	3317	0	0	5-8	5-8
K	3318	0	3318	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2523	1492 / 0	280 / 0	0 / 0	0 / 0	752 / 0	0 / 0
K	2524	1492 / 0	280 / 0	0 / 0	0 / 0	752 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
S-A	-2766 / 0	0.0	0.0	0.21 (1)	6.12	
A-T	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-3787 / 0	-127.4	-127.4	0.11 (1)	4.24	
C-D	-3564 / 0	-127.4	-127.4	0.12 (1)	4.34	
D-E	-4176 / 0	-200.3	-200.3	0.28 (1)	4.72	
E-F	-4177 / 0	-200.3	-200.3	0.25 (1)	4.74	
F-G	-4192 / 0	-200.3	-200.3	0.27 (1)	4.72	
G-H	-3561 / 0	-127.4	-127.4	0.12 (1)	4.34	
L-I	-3546 / 0	0.0	0.0	0.50 (1)	5.80	
I-H	-4607 / 0	0.0	0.0	0.52 (1)	5.16	
I-J	-3791 / 0	-127.4	-127.4	0.11 (1)	4.24	
K-J	-2769 / 0	0.0	0.0	0.21 (1)	6.12	
J-U	0 / 0	0.0	0.0	0.00 (1)	10.00	
R-B	-3552 / 0	0.0	0.0	0.50 (1)	5.80	
B-C	-4611 / 0	0.0	0.0	0.52 (1)	5.16	
S-R	0 / 0	-28.0	-28.0	0.35 (1)	10.00	
R-Q	0 / 2785	-28.0	-28.0	0.59 (1)	10.00	
Q-V	0 / 2128	-28.0	-28.0	0.30 (1)	10.00	
V-W	0 / 2128	-28.0	-28.0	0.30 (1)	10.00	
W-P	0 / 2128	-28.0	-28.0	0.30 (1)	10.00	
P-X	0 / 4192	-28.0	-28.0	0.51 (1)	10.00	
X-Y	0 / 4192	-28.0	-28.0	0.51 (1)	10.00	
Y-O	0 / 4192	-28.0	-28.0	0.51 (1)	10.00	
O-N	0 / 4192	-28.0	-28.0	0.51 (1)	10.00	
N-Z	0 / 2125	-28.0	-28.0	0.31 (1)	10.00	
Z-AA	0 / 2125	-28.0	-28.0	0.31 (1)	10.00	
AA-M	0 / 2125	-28.0	-28.0	0.31 (1)	10.00	
M-L	0 / 2789	-28.0	-28.0	0.59 (1)	10.00	
L-K	0 / 0	-28.0	-28.0	0.35 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-D	0 / 648	0.09 (1)
M-G	0 / 535	0.09 (1)
L-J	0 / 4644	0.55 (1)
A-R	0 / 4639	0.55 (1)
B-Q	-2248 / 0	0.18 (1)
C-Q	0 / 2089	0.25 (1)
M-I	-2250 / 0	0.18 (1)
M-H	0 / 2088	0.25 (1)
N-G	0 / 2730	0.45 (1)
D-P	0 / 2708	0.45 (1)
N-F	-1061 / 0	0.22 (1)
P-E	-1059 / 0	0.22 (1)
P-F	-21 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
L	19-11-4	-208	-208	---	FRONT	VERT	TOTAL
M	17-10-14	-208	-208	---	FRONT	VERT	TOTAL
O	11-10-14	-208	-208	---	FRONT	VERT	TOTAL
Q	3-1-2	-208	-208	---	FRONT	VERT	TOTAL
R	1-0-12	-208	-208	---	FRONT	VERT	TOTAL
V	5-1-2	-208	-208	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO LOADCASE(S): 1,2

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.70")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.70")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.52/1.00 (H-I:1) , BC=0.59/1.00 (Q-R:1) , WB=0.55/1.00 (J-L:1) , SSI=0.44/1.00 (H-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 292709

STRUCTURAL COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T43Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
G	TTWW+m	MT20	7.0	8.0	2.50	2.25
H	TMVW-t	MT20	6.0	9.0	2.25	3.00
I	TVMW-t	MT20	3.0	10.0	1.50	2.50
J	TVMW-t	MT20	5.0	8.0	2.25	5.25
K	BMV1+p	MT20	3.0	6.0		
L	BMVW-t	MT20	7.0	9.0		
M	BMWWW+t	MT20	4.0	9.0	3.75	1.50
N	BMWW+t	MT20	5.0	6.0	2.75	1.50
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	9.0	3.00	3.50
Q	BMWWW+t	MT20	4.0	9.0	3.75	1.50
R	BMVW-t	MT20	7.0	8.0		
S	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 207.5 lbs FACTORED DOWN AT 1-0-12, 207.5 lbs FACTORED DOWN AT 3-1-2, 207.5 lbs FACTORED DOWN AT 5-1-2, 207.5 lbs FACTORED DOWN AT 7-1-2, 207.5 lbs FACTORED DOWN AT 9-1-2, 207.5 lbs FACTORED DOWN AT 10-6-0, 207.5 lbs FACTORED DOWN AT 11-10-14, 207.5 lbs FACTORED DOWN AT 13-10-14, 207.5 lbs FACTORED DOWN AT 15-10-14, AND 207.5 lbs FACTORED DOWN AT 17-10-14, AND 207.5 lbs FACTORED DOWN AT 19-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
W	7-1-2	-208	-208	—	FRONT	VERT	TOTAL
X	9-1-2	-208	-208	—	FRONT	VERT	TOTAL
Y	10-6-0	-208	-208	—	FRONT	VERT	TOTAL
Z	13-10-14	-208	-208	—	FRONT	VERT	TOTAL
AA	15-10-14	-208	-208	—	FRONT	VERT	TOTAL

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.51 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280568
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T44	18	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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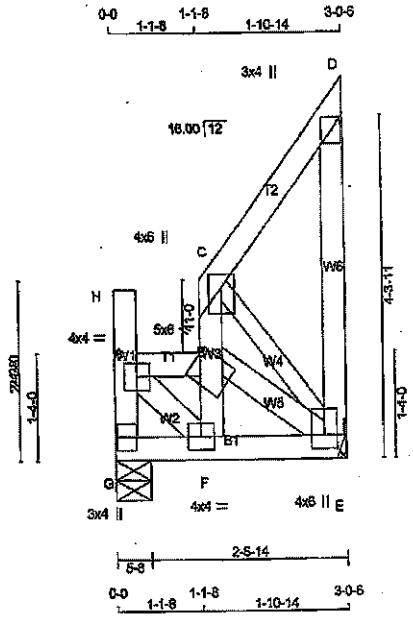

TOTAL WEIGHT = 18 X 22 = 401 lb

[MII(F)]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
G - H	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
F - C	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TVMW+w	MT20	5.0	6.0	1.00	2.50
C	TMVW+p	MT20	4.0	6.0	2.00	2.00
D	TMV+p	MT20	3.0	4.0		
E	BMVWW1+p	MT20	4.0	6.0	2.00	2.00
F	BMVW-t	MT20	4.0	4.0		
G	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
G	236	0	236	0	0	5-8	5-8
E	236	0	236	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT E.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	169	91 / 0	32 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	169	91 / 0	32 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
G-A	-217 / 0	0.0	0.0	0.02 (1)	7.81	
A-H	0 / 0	0.0	0.0	0.00 (1)	10.00	
A-B	-166 / 0	-127.4	-127.4	0.03 (1)	6.25	
C-D	0 / 0	-127.4	-127.4	0.07 (1)	10.00	
E-D	-112 / 0	0.0	0.0	0.04 (1)	7.81	
F-B	-95 / 3	0.0	0.0	0.01 (1)	7.81	
B-C	-100 / 0	0.0	0.0	0.01 (1)	7.81	
G-F	0 / 0	-28.0	-28.0	0.01 (3)	10.00	
F-E	0 / 155	-28.0	-28.0	0.04 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-F	0 / 214	0.03 (1)
B-E	-168 / 0	0.02 (1)
C-E	-17 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	30.0	PSF
	DL =	15.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN ALL FLAT SECTIONS BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.07/1.00 (C-D:1) , BC=0.04/1.00 (E-F:3) , WB=0.03/1.00 (A-F:1) , SSI=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.60

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (A) (INPUT = 0.90)
JSI METAL= 0.07 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 28057-8
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
292709	T45	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version: 8.210 S May 18 2018 MiTek Industries, Inc. Mon May 28 09:14:27 2018 Page 1
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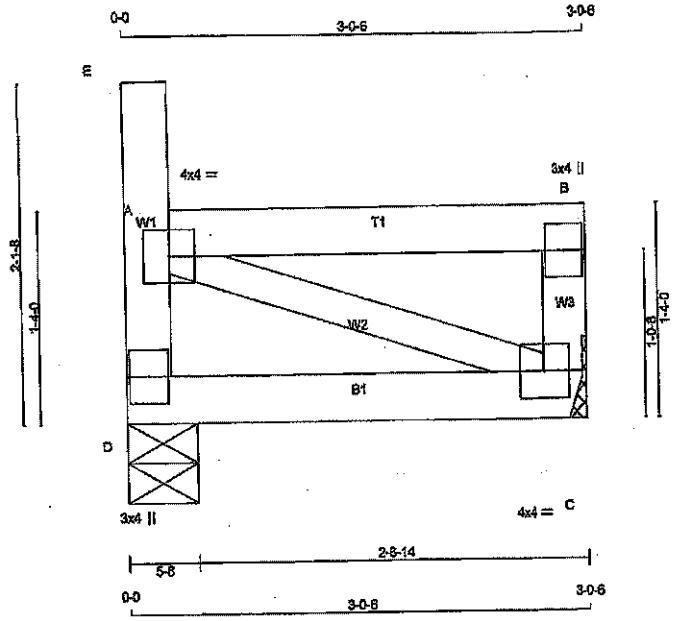


TOTAL WEIGHT = 6 X 12 = 70 lb
[M][R]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
D - E	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
C - B	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TVMW-I	MT20	4.0	4.0		
B	TMV+p	MT20	3.0	4.0		
C	BMVW1-I	MT20	4.0	4.0		
D	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

PROVIDE ADEQUATE DRAINAGE TO PREVENT PONDING

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	236	0	236	0	0	5-8	5-8
C	236	0	236	0	0	MECHANICAL	

A SUITABLE MECHANICAL CONNECTION IS REQUIRED AT JOINT C.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	189	91 / 0	32 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	189	91 / 0	32 / 0	0 / 0	0 / 0	67 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
D-A	-193 / 0	0.0	0.0	0.03 (1)	7.81
A-E	0 / 0	0.0	0.0	0.00 (1)	10.00
A-B	0 / 0	-127.4	-127.4	0.27 (1)	10.00
C-B	-193 / 0	0.0	0.0	0.03 (1)	7.81
D-C	0 / 0	-28.0	-28.0	0.12 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-C	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	30.0	PSF
	DL	=	15.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	62.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012 , BCBC 2012 , ABC 2014
- CSA 086-09
- TPIC 2011

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 39.3 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 30.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.27/1.00 (A-B:1) , BC=0.12/1.00 (C-D:3) , WB=0.00/1.00 (A-C:1) , SSI=0.20/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50
FLAT ROOF FACTOR = 0.75

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	822	2264	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (A) (INPUT = 0.90)
JSI METAL= 0.09 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
S KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 280889
STRUCTURAL COMPONENT ONLY

Boise Cascade

Double 1-3/4" x 16" VERSA-LAM® 2.0 2800 DF

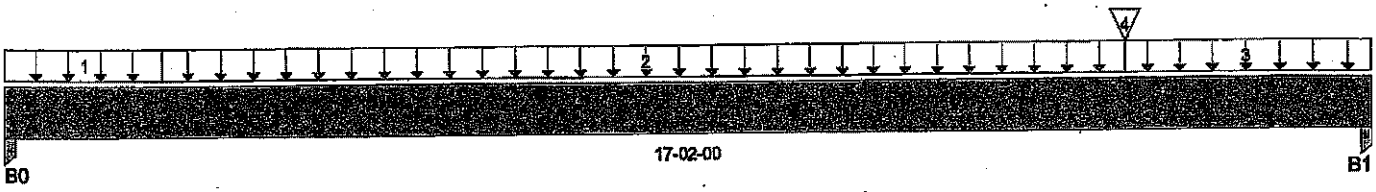
Floor Beam\B1

Dry | 1 span | No cantilevers | 0/12 slope (deg)

March 5, 2018 07:53:45

BC CALC® Design Report

Build 6536		File Name:	GEORGIAN 3
Job Name:	GREEN YORK HOMES - TIBURTINO	Description:	Designs\B1
Address:		Specifier:	
City, Province, Postal Code:	OAKVILLE,	Designer:	colinh
Customer:		Company:	
Code reports:	CCMC 12472-R	Misc:	



Total Horizontal Product Length = 17-02-00

Reaction Summary (Down / Uplift) (lbs)

Bearing	Live	Dead	Snow	Wind
B0, 5-1/2"	751 / 0	1,770 / 0	2,289 / 0	
B1, 3-1/2"	1,091 / 0	2,766 / 0	3,827 / 0	

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
1	TRUSSES1	Unf. Area (lb/ft^2)	L	00-00-00	02-00-00	11	22	30		03-11-12
2	TRUSSES2	Unf. Area (lb/ft^2)	L	02-00-00	14-01-12	11	22	30		08-01-04
3	SPAN	Unf. Area (lb/ft^2)	L	14-01-12	17-02-00	11	22	30		02-00-00
4	T15	Conc. Pt. (lbs)	L	14-01-12	14-01-12	661	1,816	2,743		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	31,252 ft-lbs	56,137 ft-lbs	55.7%	5	10-04-03
End Shear	9,462 lbs	19,488 lbs	48.6%	5	15-06-08
Total Load Defl.	L/413 (0.48")	0.827"	58.1%	13	09-00-04
Live Load Defl.	L/681 (0.292")	0.551"	52.9%	17	09-00-04
Max Defl.	0.48"	1"	48%	13	09-00-04
Span / Depth	12.4	n/a	n/a		00-00-00

Bearing Supports		Dim. (L x W)	Demand	Demand/ Resistance Support	Demand/ Resistance Member	Material
B0	Post	5-1/2" x 3-1/2"	6,022 lbs	n/a	25.6%	Unspecified
B1	Post	3-1/2" x 3-1/2"	9,744 lbs	n/a	65.2%	Unspecified

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets Code minimum (L/360) Live load deflection criteria.
Design meets User specified (1") Maximum Total load deflection criteria.
Calculations assume member is fully braced.
Resistance Factor phi has been applied to all presented results per CSA O86.
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.
CONFORMS TO OBC 2012
Design based on Dry Service Condition.
Importance Factor : Normal Part code : Part 9

LICENSED PROFESSIONAL ENGINEER
3/18
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM 12135-1B
STRUCTURAL COMPONENT ONLY

Boise Cascade

Double 1-3/4" x 16" VERSA-LAM® 2.0 2800 DF

Floor Beam\B1

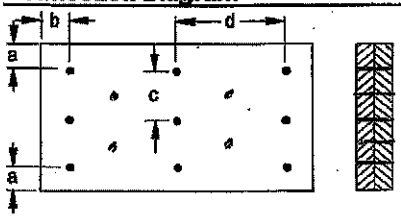
Dry | 1 span | No cantilevers | 0/12 slope (deg)

March 5, 2018 07:53:45

BC CALC® Design Report

Build 6536		File Name:	GEORGIAN 3
Job Name:	GREEN YORK HOMES - TIBURTINO	Description:	Designs\B1
Address:		Specifier:	
City, Province, Postal Code:	OAKVILLE,	Designer:	colinh
Customer:		Company:	
Code reports:	CCMC 12472-R	Misc:	

Connection Diagram



5 ROWS

a minimum = 2" c = 6"
b minimum = 3" d = 6"

Calculated Side Load = 541.0 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.
Connectors are: 16d Nails
3-1/2" ARDOX SPIRAL

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.

LICENSED PROFESSIONAL ENGINEER
3578
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO. TAM12135
STRUCTURAL COMPONENT ONLY

TECHNICAL BULLETIN

CP Crush Plates – Bearing Enhancers

SIMPSON Strong-Tie®

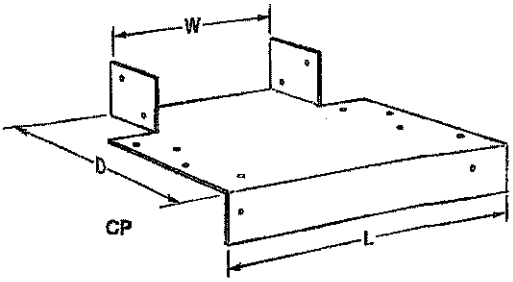
The CP transfers load from the truss or girder to plates for bearing limited conditions. Replaces nail-on scabs or in some cases, an additional ply when needed for bearing.

Material: See table

Finish: G90 galvanized

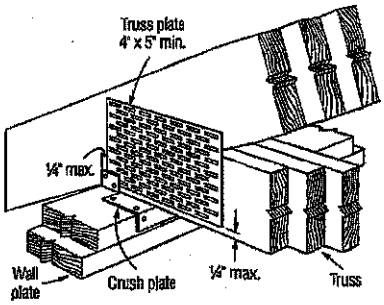
Design:

- Factored resistances are in accordance with CSA O86-14 assuming Q_r/A_b and Q'_r/A'_b = 812 psi for D.Fir-L and 615 psi for S-P-F. See clauses 6.5.4 and 7.5.9 TPIC 2014 when compression loads are applied to both sides of truss chord members at bearing locations.

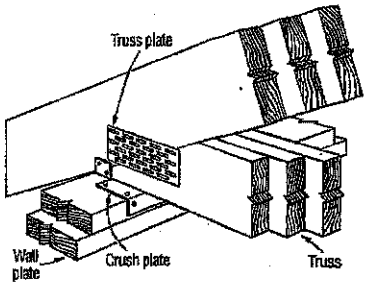


Installation:

- Use all specified fasteners.
- Nails: 10d = 0.148" dia. x 3" long common wire, 10d x 1½" = 0.148" dia. x 1½" long.
- For Case 1, truss plates must be located a maximum of ¼" from the underside of the truss chord and a maximum of ¼" from the edge of the wall plates in accordance with the reinforcing requirements of 7.5.9 TPIC 2014.



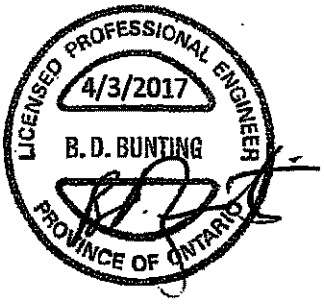
Case 1



Case 2

Model No.	Ga.	Wall Plate	Dimensions (in.) W	D	L	Fasteners Wall Plate	Truss	Factored Resistance (lb.) D.Fir-L Uplift (K_D=1.15)	D.Fir-L Bearing (K_D=1.00)	S-P-F Uplift (K_D=1.15)	S-P-F Bearing (K_D=1.00)
CASE 1 (Truss Plate Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	5965	225	4515
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	11390	225	9030
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	17895	225	13545
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	23860	225	18065
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	9370	225	7095
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	18740	225	14190
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	28110	225	21285
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	37495	225	28390
CASE 2 (No Reinforcement)											
CP1-4	20	2x4	1⅝	3½	4½	(6) 10d	(4) 10d x 1½"	225	4685	225	3550
CP2-4	16		3¼	3½	5¾	(6) 10d	(4) 10d x 1½"	225	9370	225	7100
CP3-4	16		4¾	3½	7½	(6) 10d	(4) 10d x 1½"	225	14055	225	10650
CP4-4	12		6½	3½	9½	(6) 10d	(4) 10d x 1½"	225	18750	225	14195
CP1-6	20	2x6	1⅝	5½	4½	(10) 10d	(4) 10d x 1½"	225	7365	225	5575
CP2-6	16		3¼	5½	5¾	(10) 10d	(4) 10d x 1½"	225	14730	225	11150
CP3-6	16		4¾	5½	7½	(10) 10d	(4) 10d x 1½"	225	22095	225	16725
CP4-6	12		6½	5½	9½	(10) 10d	(4) 10d x 1½"	225	29460	225	22305

1. Factored bearing resistances assume wall plate and truss are the same species. For a mixed species system use S-P-F values.



LIMIT STATES DESIGN

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T-SPECCP17 3/17 exp. 6/19

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TECHNICAL BULLETIN

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

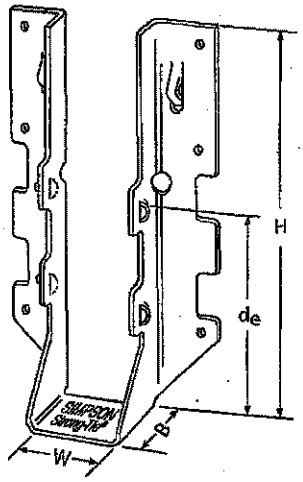
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

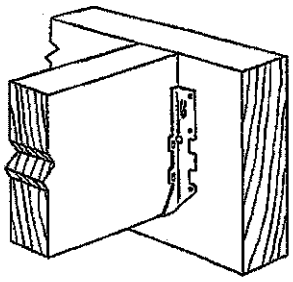
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



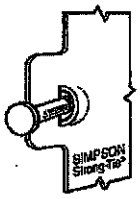
LUS28



Typical LUS Installation

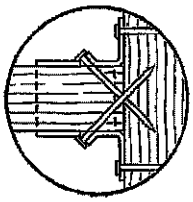
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_a[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1 9/16	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 9/16	4 3/4	1 3/4	3 5/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 5/8	4 3/16	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 9/16	6 5/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 5/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 9/16	7 13/16	1 3/4	3 7/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 5/8	8 9/16	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_a is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/5/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECLUS17 3/17 exp. 6/19

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TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

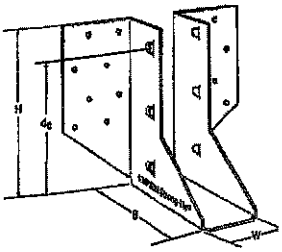
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

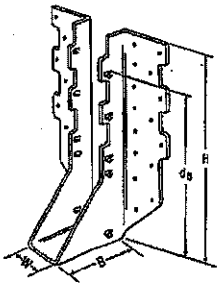
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

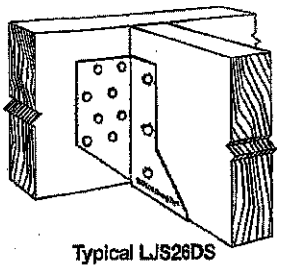
- See current catalogue for options



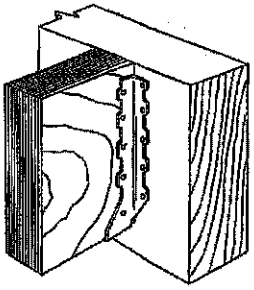
LJS26DS



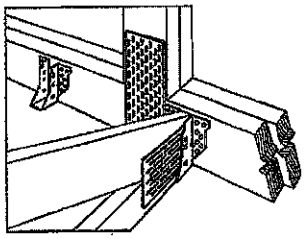
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



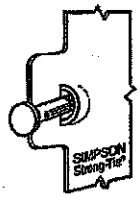
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

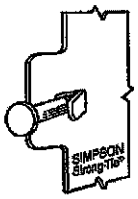
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L.		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.	Uplift (K_D=1.15) lb.	Normal (K_D=1.00) lb.
LJS26DS	18	1⅝	5	3½	4⅝	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

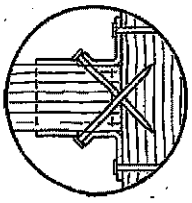


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/3/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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T-SPECHUS17 3/17 exp. 6/19

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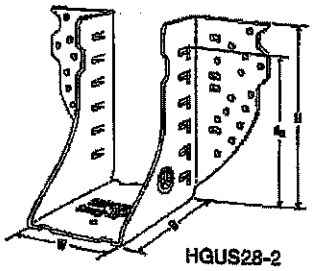
strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.



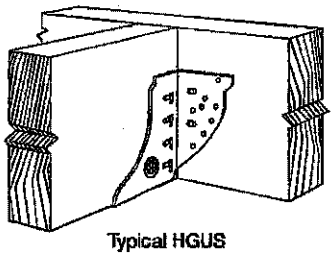
HGUS28-2

Material: 12 gauge

Finish: G90 galvanized

Design:

- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



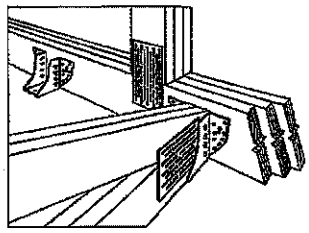
Typical HGUS Installation

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

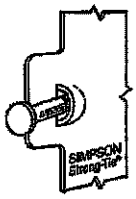
- See current catalogue for options



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

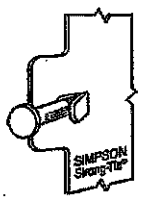
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_s[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS26	12	1⅝	5⅜	5	4 5/32	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 5/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 15/16	6½	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 9/16	5 7/16	4	4⅛	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1⅝	7⅛	5	6⅛	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 5/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 15/16	7¼	4	6⅜	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 9/16	7 3/16	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 5/16	9 3/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 15/16	9¼	4	8⅜	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 9/16	9 9/16	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 9/16	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 9/16	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_s is the distance from the seat of the hanger to the highest joist nail.

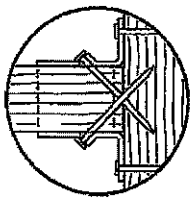


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
4/10/2017
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

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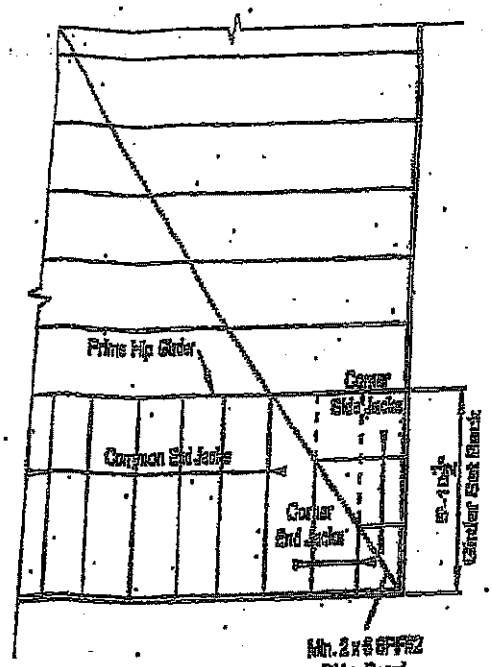
MICRO CITY
ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2x4 SPF#2

BOTTOM CHORD : 2x4 SPF#2

WEBS : 2x3 SPF#2

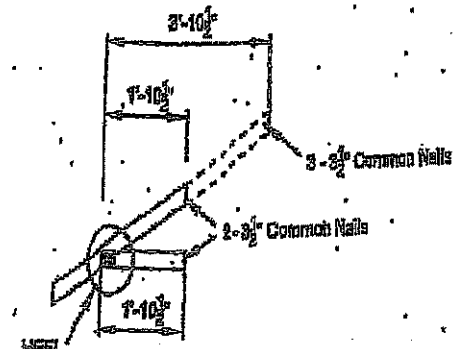
UNLESS OTHERWISE SHOWN

DESIGN LOAD:

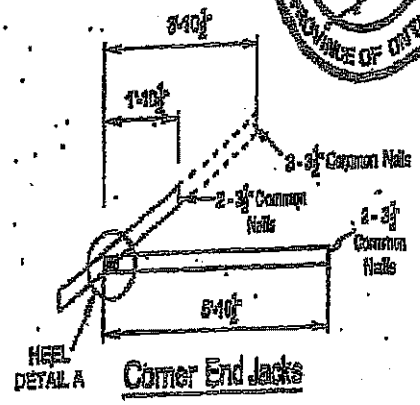
TOP CHORD LIVE LOAD	: 39.4 P.S.F.
TOP CHORD DEAD LOAD	: 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	: 10.5 P.S.F.
BOTTOM CHORD DEAD LOAD	: 7.0 P.S.F
TOTAL LOAD	: 59.9 P.S.F.

DWG NO TAM 2495.14
STRUCTURAL
COMPONENT ONLY

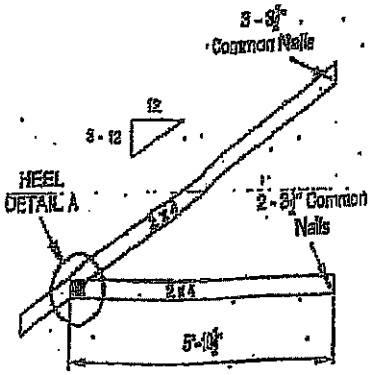
LICENSED PROFESSIONAL ENGINEER
1-20-14
S. KATSOULAKOS
PROVINCE OF ONTARIO



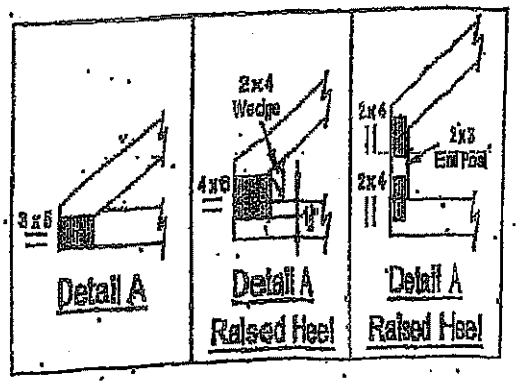
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

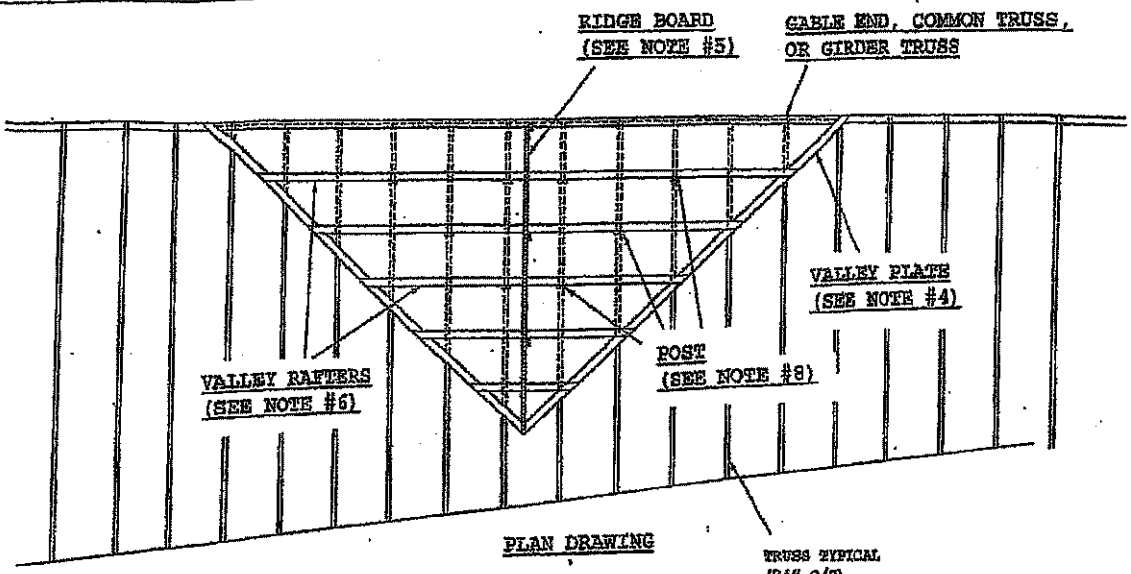
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

MICRO CITY ENGINEERING SERVICES INC.

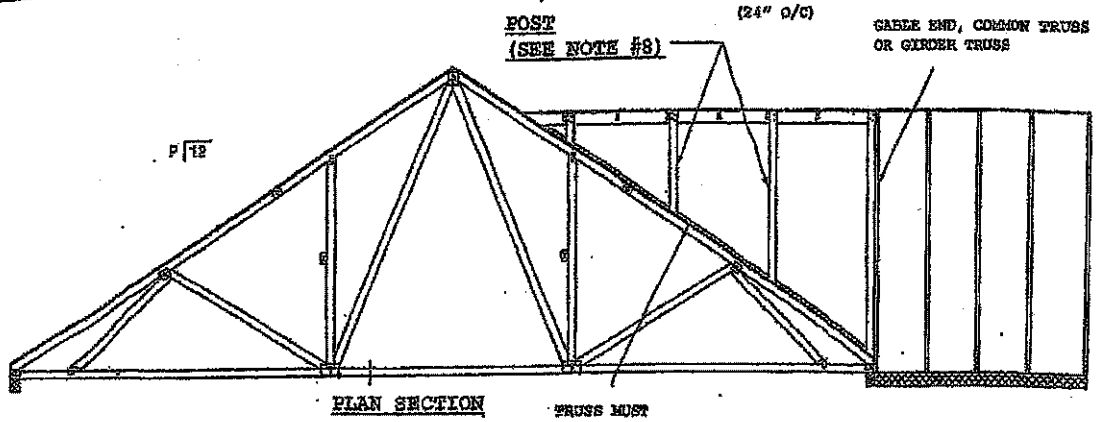
TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1M0

CONVENTIONAL VALLEY FRAMING DETAIL



PLAN DRAWING



PLAN SECTION

GENERAL SPECIFICATIONS:

(1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
(2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
(3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
(4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
(5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
(6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
(7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
(8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
(9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

(10) 48" O/C (MAXIMUM POST SPACING.
(11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
(12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
(13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
(14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
(15) BASE TRUSS SPACING (24" O/C MAX.)
(16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
(17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
(18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.

LICENSED PROFESSIONAL ENGINEER
3-5-74
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PROVINCE OF ONTARIO

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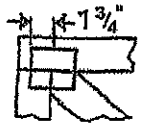
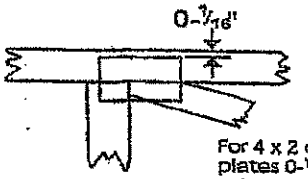
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-3/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in MiTek software or upon request.

PLATE SIZE

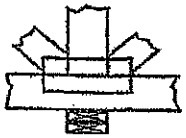
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

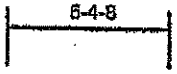
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

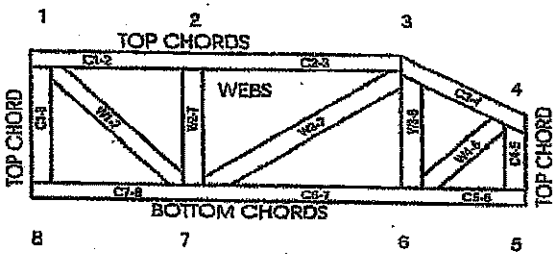
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

· CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)
RR #1, Po Box 61
Glencoe, Ontario
N0L 1M0
(519) 287 - 2242; Fax: (519) 287 - 5750 (Call)

LICENSED PROFESSIONAL ENGINEER
1-20-79
S. KATSOULAKOS
PROVINCE OF ONTARIO

DWG NO [illegible] 14
STRUCTURAL
COMPONENT ONLY

Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be totally familiar with all aspects of truss erection prior to proceeding on any truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is not meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpic.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.