

		Products				Connector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	· 1	30	4	H1	IUS2.56/9.5
J1DJ	14-00-00	9 1/2" NI-40x	2	8	5	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	4	6	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	21	6	H1	IUS2.56/9.5
J3DJ	10-00-00	9 1/2" NI-40x	2	4	3	H3	HUS1.81/10
J4	8-00-00	9 1/2" NI-40x	1	2	1	H3	HUS1.81/10
J5	6-00-00	9 1/2" NI-40x	1	3	1	H4	HGUS410
J6	4-00-00	9 1/2" NI-40x	1	3			
B7	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B4	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B5	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B6	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B1	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			



**BUILDER:** GREEN PARK HOMES

**SITE:** SECONDO VALES ESTATES

**MODEL: NEWBERRY 2A** 

**ELEVATION: 1** 

LOT:

**CITY: EAST GWILLIMBURY** 

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

NOTES:

REFER TO THE **NORDIC INSTALLATION**GUIDE FOR PROPER STORAGE AND

INSTALLATION.

SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

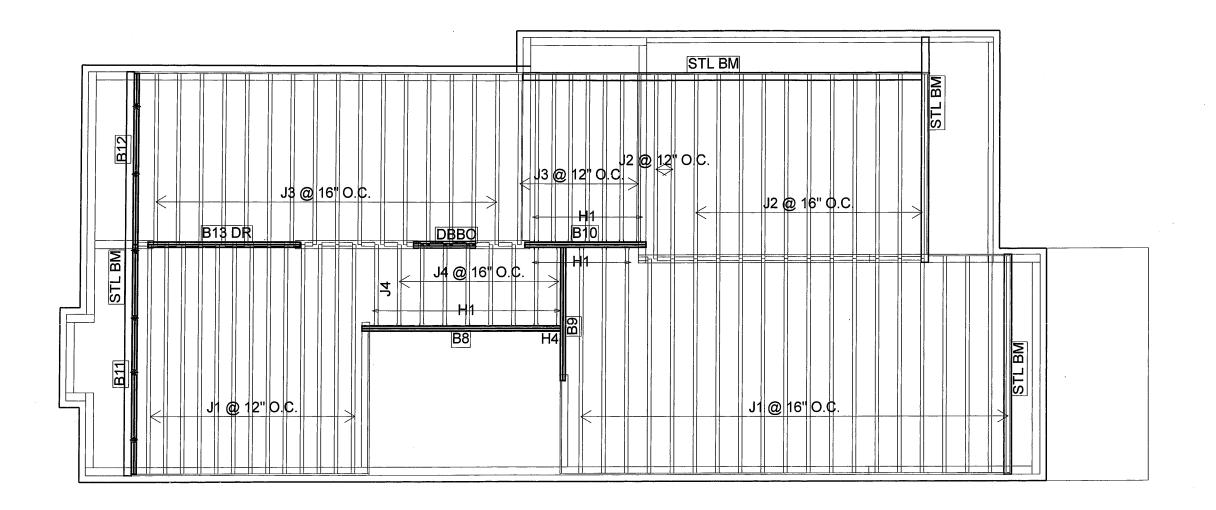
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft<sup>2</sup> TILED AREAS: 20 lb/ft

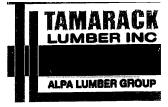
SUBFLOOR: 3/4" GLUED AND NAILED

**DATE**: 2017-10-26

1st FLOOR



		Products			C	Connector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	1	33	1	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	13	21	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	24	1	H4	HGUS410
J4	6-00-00	9 1/2" NI-40x	1	9			
B11	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B12	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B10	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
В9	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B13 DR	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2			



**BUILDER:** GREEN PARK HOMES

SITE: SECONDO VALES ESTATES

**MODEL: NEWBERRY 2A** 

**ELEVATION:** 1

LOT:

CITY: EAST GWILLIMBURY

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

### NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE** FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

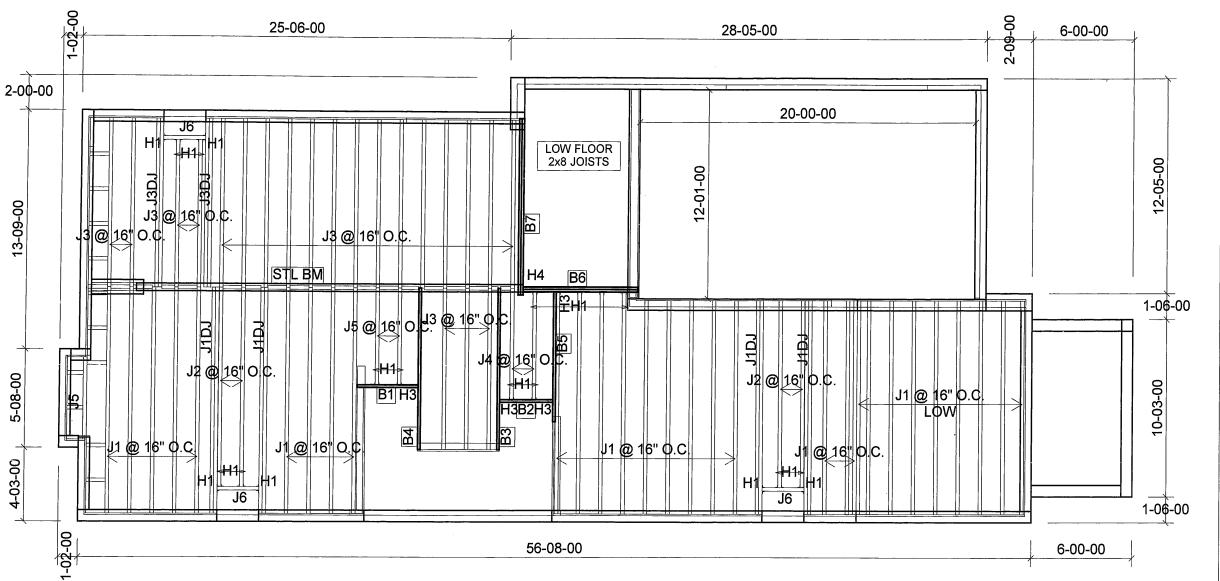
### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft<sup>2</sup> TILED AREAS: 20 lb/ft

**SUBFLOOR:** 5/8" GLUED AND NAILED

**DATE:** 2017-10-26

### 2nd FLOOR



		Products			(	Connector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	1	30	4	H1	IUS2.56/9.5
J1DJ	14-00-00	9 1/2" NI-40x	2	8	5	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	4	6	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	21	6	H1	IUS2.56/9.5
J3DJ	10-00-00	9 1/2" NI-40x	2	4	3	H3	HUS1.81/10
J4	8-00-00	9 1/2" NI-40x	1	2	1	H3	HUS1.81/10
J5	6-00-00	9 1/2" NI-40x	1	3	1	H4	HGUS410
J6	4-00-00	9 1/2" NI-40x	1	3		-	
B7	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B4	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B5	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B6	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B1	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			



**BUILDER: GREENPARK HOMES** 

**SITE: SECONDO VALES ESTATES** 

**MODEL: NEWBERRY 2A** 

**ELEVATION**: 2

LOT:

**CITY:** EAST GWILLIMBURY

SALESMAN: M D **DESIGNER:** AJ **REVISION**: Ibv

NOTES:

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND

INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH **BLOCKS** REQ'D UNDER CONCENTRATED LOADS, SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7, TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

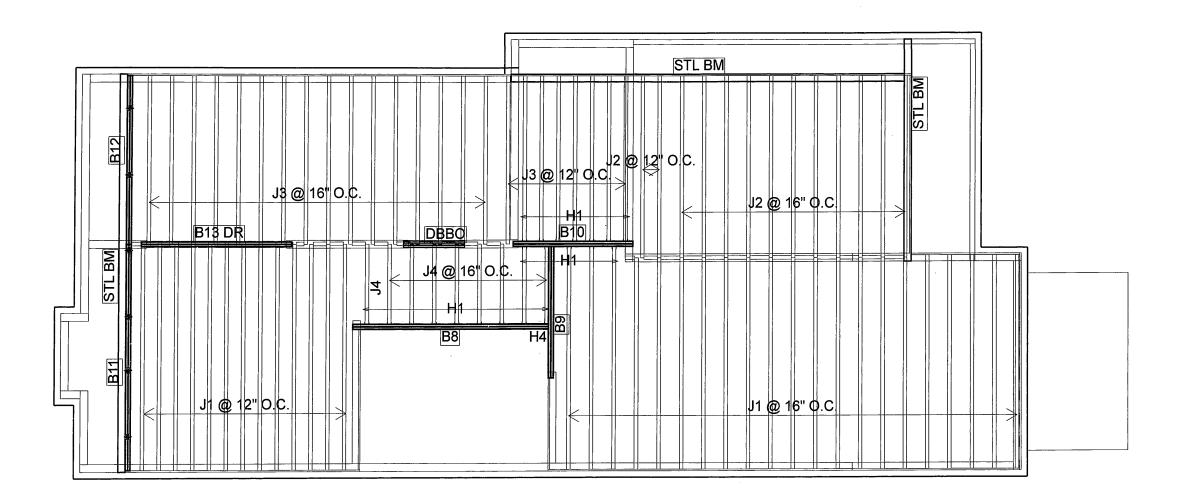
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft<sup>2</sup>

**SUBFLOOR: 3/4" GLUED AND NAILED** 

**DATE:** 2017-10-26

1st FLOOR



		Products			C	onnector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	1	34	1	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	13	21	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	24	1	H4	HGUS410
J4	6-00-00	9 1/2" NI-40x	1	9			
B11	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B12	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B10	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B9	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B13 DR	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2			



**BUILDER:** GREENPARK HOMES

**SITE**: SECONDO VALES ESTATES

**MODEL:** NEWBERRY 2A

**ELEVATION**: 2

LOT:

**CITY:** EAST GWILLIMBURY

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

### NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE FOR PROPER STORAGE AND** INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

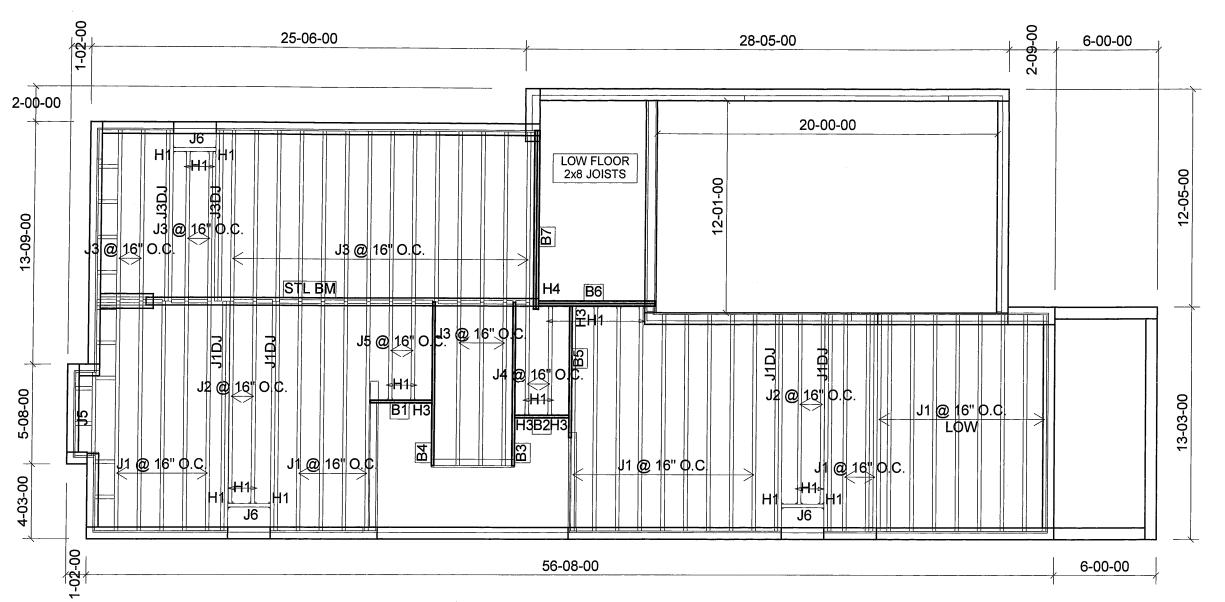
### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft<sup>2</sup> TILED AREAS: 20 lb/ft

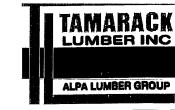
SUBFLOOR: 5/8" GLUED AND NAILED

**DATE**: 2017-10-26

### 2nd FLOOR



		Products				Connector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	1	30	4	H1	IUS2.56/9.5
J1DJ	14-00-00	9 1/2" NI-40x	2	8	5	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	4	6	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	21	6	H1	IUS2.56/9.5
J3DJ	10-00-00	9 1/2" NI-40x	2	4	3	H3	HUS1.81/10
J4	8-00-00	9 1/2" NI-40x	1	2	1	H3	HUS1.81/10
J5	6-00-00	9 1/2" NI-40x	1	3	1	H4	HGUS410
J6	4-00-00	9 1/2" NI-40x	1	3			
B7	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B3	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B4	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B5	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B6	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B1	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			
B2	4-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	1	1			



**BUILDER: GREEN PARK HOMES** 

**SITE:** SECONDO VALES ESTATES

**MODEL:** NEWBERRY 2A

**ELEVATION**: 3

LOT:

**CITY:** EAST GWILLIMBURY

SALESMAN: M D **DESIGNER**: AJ **REVISION**: lbv

**NOTES:** 

REFER TO THE NORDIC INSTALLATION GUIDE FOR PROPER STORAGE AND

INSTALLATION.

**SQUASH BLOCKS** OF 2x4, 2x6, 2x8 #2 S.P.F REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH **BLOCKS REQ'D UNDER CONCENTRATED** LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD CUT OPENINGS SEE FIGURE 7. TABLES 1 & 2. CERAMIC TILE APPLICATION AS PER O.B.C 9.30.6.

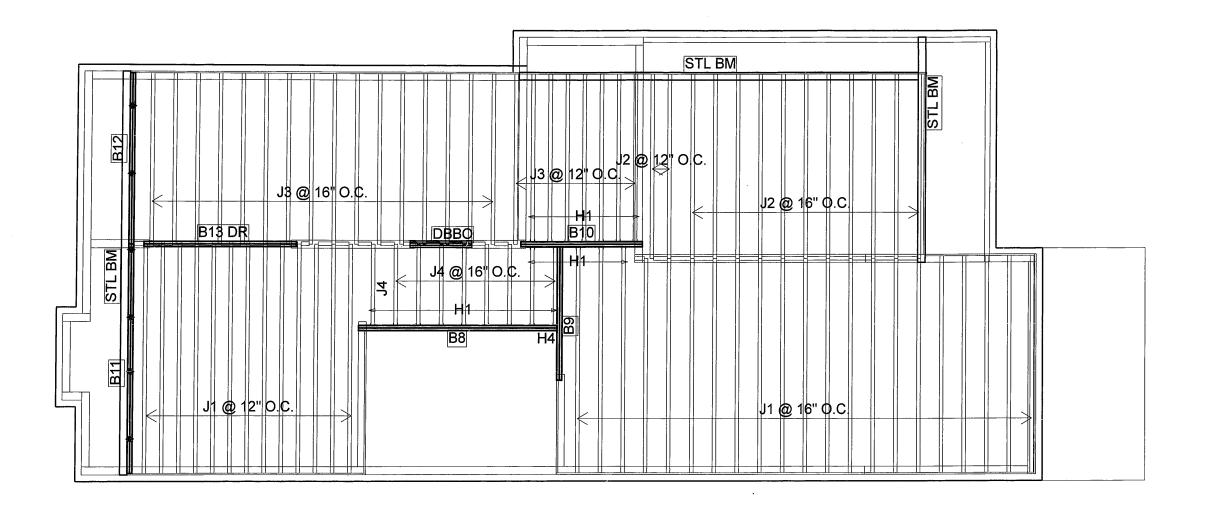
LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft TILED AREAS: 20 lb/ft<sup>2</sup>

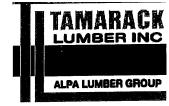
**SUBFLOOR:** 3/4" GLUED AND NAILED

**DATE:** 2017-10-26

### 1st FLOOR



		Products	-		C	Connector	Summary
PlotID	Length	Product	Plies	Net Qty	Qty	Manuf	Product
J1	14-00-00	9 1/2" NI-40x	1	34	1	H1	IUS2.56/9.5
J2	12-00-00	9 1/2" NI-40x	1	13	21	H1	IUS2.56/9.5
J3	10-00-00	9 1/2" NI-40x	1	24	1	H4	HGUS410
J4	6-00-00	9 1/2" NI-40x	1	9			
B11	14-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B8	12-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B12	10-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B10	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B9	8-00-00	1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP	2	2			
B13 DR	10-00-00	1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP	2	2			



**BUILDER: GREEN PARK HOMES** 

**SITE**: SECONDO VALES ESTATES

**MODEL: NEWBERRY 2A** 

**ELEVATION**: 3

LOT:

**CITY: EAST GWILLIMBURY** 

SALESMAN: M D DESIGNER: AJ REVISION: Ibv

### NOTES:

REFER TO THE NORDIC INSTALLATION **GUIDE** FOR PROPER STORAGE AND INSTALLATION. SQUASH BLOCKS OF 2x4, 2x6, 2x8 #2 S.P.F. REQ'D UNDER INTERIOR UNIFORM LOAD BEARING WALLS. MULTIPLE SQUASH BLOCKS REQ'D UNDER CONCENTRATED LOADS. SEE FIGURE 1. CANTILEVERED JOISTS INCLUDING CANT' OVER BRICK REQ. I-JOIST BLOCKING ALONG BEARING AND RIMBOARD CLOSURE AT ENDS. SEE FIGURE 7 TABLES 4 & 5 FOR REINFORCEMENT REQUIREMENTS. FOR HOLES INCLUDING DUCT CHASE AND FIELD **CUT OPENINGS** SEE FIGURE 7 TABLES 1 & 2 OF THE INSTALLATION GUIDE. CERAMIC TILE APPLICATION AS PER O.B.C. 9.30.6

### LOADING:

DESIGN LOADS: L/480.000 LIVE LOAD: 40.0 lb/ft<sup>2</sup> DEAD LOAD: 15.0 lb/ft<sup>2</sup> TILED AREAS: 20 lb/ft

**SUBFLOOR: 5/8" GLUED AND NAILED** 

**DATE**: 2017-10-26

### 2nd FLOOR



### Boise Cascade Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B7(i1210)

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

October 26, 2017 10:03:22

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

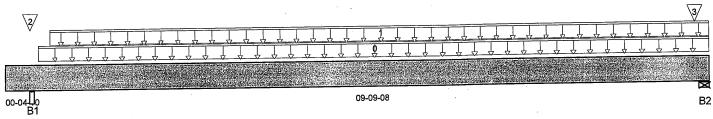
CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B7(i1210)

Specifier: Designer: Company:

Misc:



Total Horizontal Product Length = 10-02-02

Reaction Summary (Down /	Uplift) (lbs) Live	De ad	Snow	Wind	·
B1, 5-1/4"	1,756 / 1	1,397/0			
B2, 3-1/2"	2,465/6	1,553 / 0			

	1.0					Live	Dead	Snow	Wind	i rib.
	ad Summary g Description	Load Type	Re f.	. Start	En d	1.00	0.65	1.00	1.15	
0	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-05-10	10-02-02	12	5			n/a
4	6(i 107)	Unf. Lin. (lb/ft)	Ē	00-07-10	10-02-02		81			n/a
1	` '	Conc. Pt. (lbs)	Ē	00-04-04	00-04-04	1,687	936			n/a
∠ 3	- 6(i107)	Conc. Pt. (lbs)	Ē	09-11-08	09-11-08	2,402	1,080			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	1,498 ft-lbs	16,515 ft-lbs	9.1%	0	05-02-08
Neg. Moment	-129 ft-lbs	-25,408 ft-lbs	0.5%	1	00-04-10
End Shear	518 lbs	7.521 lbs	6.9%	0	09-01-02
Cont. Shear	594 lbs	7,521 lbs	7.9%	0	01-04-12
Total Load Defl.	L/999 (0.04")	n/a	n/a	. 13	05-02-08
Live Load Defl.	L/999 (0.005")	n/a	n/a	17	05-02-08
	2xL/1,998 (-0.0		n/a	13	00-00-00
Total Neg. Defl.	0.04"	n/a	n/a	13	05-02-08
Max Defl. Span / Depth	12.1	n/a	n/a		00-00-00

				De mand/ Re sistance		
Bear	ing Supports	Dim . (L x W)	Demand	Support	Member	Material
B1	Beam	5-1/4" x 3-1/2"	4,381 lbs	44.6%	19.5%	Unspecified
B2	Wall/Plate	3-1/2" x 3-1/2"	5,638 lbs	86.2%	37.7%	Unspecified

**Notes** 



DWG NO . TAM 60323.17 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B7(i1210)

Dry | 2 spans | Left cantilever | 0/12 slope (deg)

October 26, 2017 10:03:22

BC CALC® Design Report

Build 5033 Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B7(i121-

Designer:

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

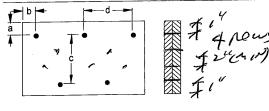
O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Cantilevers require sheathed bottom flanges, blocking at cantilever support and closure at

**Connection Diagram** 



a minimum = 2" b minimum = 3"

Calculated Side Load = 194.5 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

ARDOX SPIKAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.



COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B1(i1215)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:10

BC CALC® Design Rep

Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B1(i1215)

Specifier: Designer: Company.

Misc:

$\overline{\mathbb{V}}$	2/	3/
⊠ B0	03-07	-08 B1

Total Horizontal Product Length = 03-07-08

Reaction Summary	(Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2"	652/0	423/0			
B1	536/0	259/0			

Lood Cummon				Live	Dead	Snow Wind	Trib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 STAIR	Unf. Lin. (lb/ft)	L 00-00-00	03-07-08	240	120		n/a
1 7(i154)	Conc. Pt. (lbs)	L 00-02-12	00-02-12	16	116		n/a
2 J5(i1167)	Conc. Pt. (lbs)	L 01-02-00	01-02-00	152	57		n/a
3 J5(i1169)	Conc. Pt. (lbs)	L 02-06-00	02-06-00	146	55		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	894 ft-1bs	12,704 ft-lbs	7%	1	01-11-14
End Shear	657 lbs	5,785 lbs	11. <del>4</del> %	1	01-03-00
Total Load Defl.	L/999 (0.005")	n/a	n/a	4	01-11-07
Live Load Defl.	L/999 (0.003")	n/a	n/a	5	01-11-07
Max Defl.	0.005"	n/a	n/a	4	01-11-07
Span / Depth	3.9	n/a	n/a		00-00-00

Bear	ring Supports	Dim . (L x W)	De man d	De mand/ Re sistance Support	De man d/ Resistance Member	Material
B0	Wall/Plate	5-1/2" x 1-3/4"	1,506 lbs	29.3%	12.8%	Unspecified
B1	Hanger	2" x 1-3/4"	1,128 lbs	· n/a	26.4%	HUS1.81/10

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWO NO. TAM 603241 STRUCTURAL COMPONENT ONLY

CONFORMS TO OBC 2012



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B2(i1212)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:11

BC CALC® Design Report

\*

Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B2(i1212)

Specifier: Designer: Company:

Misc:

	<b>1</b>	'
Modelli S. State S		
B0	03-02-00	B1

Total Horizontal Product Length = 03-02-00

Reaction Summary (D	own / Uplift) (lbs)			
Be aring	Live	De ad	Snow	Wind
B0	470/0	222/0		
B1	384/0	183/0		

Load Summary			Live	Dead	Snow Wind	Trib.
Tag Description	Load Type	Ref. Start	En d 1.00	0.65	1.00 1.15	
0 J4(i991)	Conc. Pt. (lbs)	L 00-08-08	00-08-08 389	177		n/a
1 J4(i1148)	Conc. Pt. (lbs)	L 02-00-08	02-00-08 465	212		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	816 ft-lbs	12,704 ft-lbs	6.4%	1	02-00-08
End Shear	799 lbs	5,785 lbs	13.8%	1	02-02-08
Total Load Defl.	L/999 (0.003")	n/a	n/a	4	01-07-03
Live Load Defl.	L/999 (0.002")	n/a	n/a	5	01-07-03
Max Defl.	0.003"	n/a	n/a	4	01-07-03
Span / Depth	3.7	n/a	n/a		00-00-00

				Demand/ Resistance	Demand/ Resistance	
Bear	ing Supports	Dim. (L x W)	Demand	Support	Member	Material
B0	Hanger	2" x 1-3/4"	982 lbs	n/a	23%	HUS1.81/10
B1	Hanger	2" x 1-3/4"	805 lbs	n/a	18.8%	HUS1.81/10

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B5(i1213)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:13

BC CALC® Design Report

ort

Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B5(i1213)

Specifier: Designer: Company:

Misc:

1		3/2/2																_										
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																												<u>:/</u>
<b>X</b>												07-0	06-0	00														B1

Total Horizontal Product Length = 07-06-00

Reaction Summary	(Down / Uplift) (lbs)				
Be aring	Live	De ad	Snow	Wind	
B0, 3-1/2"	743/0	466/0			
B1	170/0	89 / 0			

1.0	ad Summary					Live	Dead	Snow	Wind	Trib.
	g Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
ō	FC1 Floor Material	Unf. Lin. (lb/ft)	L	01-01-08	07-06-00	27	10			n/a
1	8(i 106)	Conc. Pt. (lbs)	L	00-01-12	00-01-12	218	203			n/a
2	B2(i1212)	Conc. Pt. (lbs)	L	01-02-06	01-02-06	387	184			n/a
3	STAIR	Conc. Pt. (lbs)	L	01-02-06	01-02-06	127	63			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	1,071 ft-lbs	12,704 ft-lbs	8.4%	1	01-05-06
End Shear	1,106 lbs	5,785 lbs	19.1%	1	01-01-00
Total Load Defl.	L/999 (0.026")	n/a	n/a	4	03-06-07
Live Load Defl.	L/999 (0.017")	n/a	n/a	5	03-06-07
Max Defl.	0.026"	n/a	n/a	4	03-06-07
Span / Depth	9.1	n/a	n/a		00-00-00

				De mand/ Re sistance	Demand/ Resistance	
Beari	ng Supports	Dim . (L x W)	Demand	Support	Member	Material
B0	Wall/Plate	3-1/2" x 1-3/4"	1,697 lbs	51.9%	22.7%	Unspecified
B1	Hanger	2" x 1-3/4"	365 lbs	n/a	8.6%	HUS1.81/10

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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DWG ND. TAM 60326-17 STRUGTURAL COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B4(i1216)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:15

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B4(i1216)

Specifier: Designer: Company.

Msc:

	3	
<u> </u>		2
×	09-03-14	 B1

Total Horizontal Product Length = 09-03-14

Reaction Summary (Down / Uplift) (lbs)										
Bearing	Live	De ad	Snow	Wind						
B0, 4-3/8"	511/0	242/0								
B1. 2-5/8"	426/0	198/0								

	ad Common mo					Live	Dead	Snow	Wind	Trib.
Load Summary Tag Description		Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-03-14	29	11			n/a
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	03-06-14	3				n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	Ĺ	03-06-14	09-03-14	24	9			n/a
3	B1 (i1215)	Conc. Pt. (lbs)	Ĺ	03-07-12	03-07-12	515	237			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,112 ft-lbs	12,704 ft-lbs	24.5%	1	03-07-12
End Shear	988 lbs	5,785 lbs	17.1%	1	01-01-14
Total Load Defl.	L/999 (0.108")	n/a	n/a	4	04-06-04
Live Load Defl.	L/999 (0.074")	n/a	n/a	5	04-06-04
Max Defl.	0.108"	n/a	n/a	4	04-06-04
Span / Depth	11.2	n/a	n/a		00-00-00

				De mand/ Resistance	Demand/ Resistance	
Beari	ng Supports	Dim.(LxW)	De m an d	Support	Member	Material
B0	Wall/Plate	4-3/8" x 1-3/4"	1,068 lbs	26.1%	11.4%	Unspecified
B1	Beam	2-5/8" x 1-3/4"	887 lbs	36.2%	15.8%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

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CONFORMS TO OBG 2012



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1217)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:17

BC CALC® Design Report

\*\*\*

Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

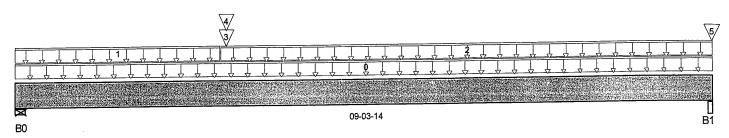
CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i1217)

Specifier: Designer: Company:

Misc:



Total Horizontal Product Length = 09-03-14

Reaction Summary (Dow	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 4-3/8" B1, 2-5/8"	486/0 314/0	244/0 168/0			

	1.0					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Re	f. Start	En d	1.00	0.65	1.00	1.15	
0	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	09-03-14	11	4			n/a
1	FC1 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	02-08-14	3				n/a
2	FC1 Floor Material	Unf. Lin. (lb/ft)	L	02-08-14	09-03-14	16	6			n/a
3	B2(i1212)	Conc. Pt. (lbs)	L	02-09-12	02-09-12	467	220			n/a
4	STÀIR	Conc. Pt. (lbs)	L	02-09-12	02-09-12		38			n/a
5	3(i294)	Conc. Pt. (lbs)	L	09-03-10	09-03-10	42	28			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	2.463 ft-lbs	12,704 ft-lbs	19.4%	1	02-09-12
End Shear	995 lbs	5,785 lbs	17.2%	1	01-01-14
Total Load Defl.	L/999 (0.083")	n/a	n/a	4	04-04-05
Live Load Defl.	L/999 (0.055")	n/a	n/a	5	04-04-05
Max Defl.	0.083"	n/a	n/a	4	04-04-05
Span / Depth	11.2	n/a	n/a		00-00-00

Bearing Supports		Di (1 × 10/)	Demand	De mand/ Resistance Support	De mand/ Resistance Member	Material
		Dim.(L x W)	Demand			
B0	Wall/Plate	4-3/8" x 1-3/4"	1,033 lbs	25.3%	11.1%	Unspecified
B1	Beam	2-5/8" x 1-3/4"	681 lbs	27.7%	12.1%	Unspecified

Notes

S. KATSOULAKOS G

WE NO. TAW 6326-STRUCTURAL COMPONENT ONLY



### Single 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B3(i1217)

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:17

**Build 5033** 

Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Arnmdl

Description: Designs\Flush Beams\Basment\Flush Beams\B3(i121)

Specifier: Designer:

Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Disclosure

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### Boise Cascade Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i1214)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:20

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

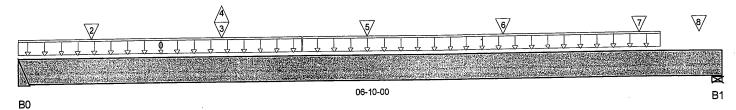
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i1214)

Specifier:

Designer: Company.

Misc:



Total Horizontal Product Length = 06-10-00

Reaction Summary (D	own / Uplift) ( lbs ) Live	De ad	Snow	Wind	
BO	890/1	545/0			
B1, 5-1/2"	3,010/0	1,372/0			

	10					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
0	WALL	Unf. Lin. (lb/ft)	L	00-00-00	02-09-00		60			n/a
4	STAIR	Unf. Lin. (lb/ft)	L	02-09-00	06-03-00	240	120			n/a
1	J4(i1148)	Conc. Pt. (lbs)	Ĺ	00-08-08	00-08-08	162	61			n/a
3	•	Conc. Pt. (lbs)	Ī	01-11-10	01-11-10	261	119			n/a
-	#	Conc. Pt. (lbs)	ī	01-11-10	01-11-10	-1				n/a
4	- 14 (34479)	Conc. Pt. (lbs)	ī	03-04-08	03-04-08	349	131			n/a
5	J1(i1172)	Conc. Pt. (lbs)	ī	04-08-08	04-08-08	354	133			n/a
6	J1(i1062)	Conc. Pt. (lbs)	ī	06-00-08	06-00-08	337	126			n/a
7 8	J1 (i1147) 14 (i839)	Conc. Pt. (lbs)	Ĺ	06-07-04	06-07-04		682			n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4,306 ft-lbs	25,408 ft-lbs	16.9%	1	03-04-08
End Shear	2,615 lbs	11,571 lbs	22.6%	1	05-07-00
Total Load Defl.	L/999 (0.042")	n/a	n/a	6	03-04-08
Live Load Defl.	L/999 (0.028")	n/a	n/a	8	03-04-08
Max Defl.	0.042"	n/a	n/a	6	03-04-08
Span / Depth	8	n/a	n/a		00-00-00

Reari	ng Supports	Dim . (L x W)	De man d	De mand/ Resistance Support	De mand/ Resistance Member	Material
B0	Hanger	2" x 3-1/2"	2,016 lbs	n/a	23.6%	HGUS410
B1	Wall/Plate	5-1/2" x 3-1/2"	6,231 lbs	60.6%	26.5%	Un specified

COMPONENT ONLY

Page 1 of 2

**Notes** 



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP Basment\Flush Beams\B6(i1214)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:20

BC CALC® Design Report



**Build 5033** 

Job Name:

Address:

City, Province, Postal Code:,

Customer: Code reports: CCMC 12472-R

File Name: NEWBERRY2Ammd!

Description: Designs\Flush Beams\Basment\Flush Beams\B6(i121-

Specifier:

Designer. Company:

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Design based on Dry Service Condition.

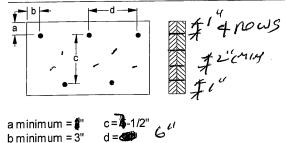
Importance Factor: Normal Part code: Part 9

### Disclosure

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### Connection Diagram



Calculated Side Load = 425.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d 7 Nails

312" ARDOX SPIRAL

COMPONENT ONLY



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13 DR(i945)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:24

BC CALC® Design Report



Build 5033 Job Name:

Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B13

Specifier:

Designer: Company:

Misc:

1/	2/	3	4	7
V	V	v	ŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢŢ	
	e de la companya de			
<b>X</b>			08-11-02	×
B0				B <sub>1</sub>

Reaction Summary (Down / Uplift) ( lbs )									
Be aring	Live	De ad	Snow	Wind					
B0, 3-5/8"	2,135/0	854/0							
B1 3-1/2"	1.932 / 0	778/0							

١٥	ad Summary					Live	Dead	Snow	Wind	Trib.
	a Description	Load Type	Ref	. Start	En d	1.00	0.65	1.00	1.15	
0	Smoothed Load	Unf. Lin. (lb/ft)	L	02-05-04	08-01-04	448	168			n/a
1	-	Conc. Pt. (lbs)	L	00-03-04	00-03-04	507	190			n/a
2	J1 (i953)	Conc. Pt. (lbs)	L	01-01-04	01-01-04	256	96			n/a
3	-	Conc. Pt. (lbs)	L	01-11-04	01-11-04	507	190			n/a
4	J3(i980)	Conc. Pt. (lbs)	L	08-05-04	08-05-04	251	94			n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Dem and	Resistance	Resistance	Case	
Pos. Moment	8,002 ft-lbs	38,727 ft-lbs	20.7%	1	04-05-04
End Shear	3,225 lbs	14,464 lbs	22.3%	1	01-03-08
Total Load Defl.	L/999 (0.073")	n/a	n/a	4	04-05-04
Live Load Defl.	L/999 (0.052")	n/a	n/a	5	04-05-04
Max Defl.	0.073"	n/a	n/a	4	04-05-04
Span / Depth	8.5	n/a	n/a		00-00-00

				Resistance	Resistance	
Bear	ing Supports	Dim.(L x W)	Demand	Support	Member	Material
B0	Wall/Plate	3-5/8" x 3-1/2"	4,270 lbs	41.4%	27.6%	Unspecified
B1	Wall/Plate	3-1/2" x 3-1/2"	3,870 lbs	38.9%	25.9%	Unspecified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume unbraced length of Top: 00-02-12, Bottom: 00-02-12.

Resistance Factor phi has been applied to all presented results per CSA O86.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAN 6033017
STRUCTURAL

COMPONENT ONLY



### Double 1-3/4" x 11-7/8" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B13 DR(i945)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:24

BC CALC® Design Report

Build 5033 Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

ue.,

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

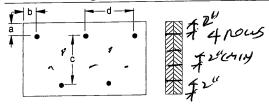
Description: Designs\Dropped Beams\1st Floor\Dropped Beams\B1

Specifier: Designer:

Company:

Misc:

### **Connection Diagram**



a minimum = 2"

b minimum = 3" d = 6 Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Member has no side loads.

Connectors are: 16d . 7 . Nails

3%" ARDOX SPIRAL

### Disclosure

Completeness and accuracy of input must be verified by anyone w ho w ould rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered w ood products must be in accordance w ith current installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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ALLJOIST®, BC RIM BOARD™, BCI®,
BOISE GLULAM™, SIMPLE FRAMING
SYSTEM®, VERSA-LAM®, VERSA-RIM
PLUS®, VERSA-RIM®,
VERSA-STRAND®, VERSA-STUD® are
trademarks of Boise Cascade Wood
Products L.L.C.





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1219)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:26

BC CALC® Design Report

**Build 5033** Job Name:

Address: City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

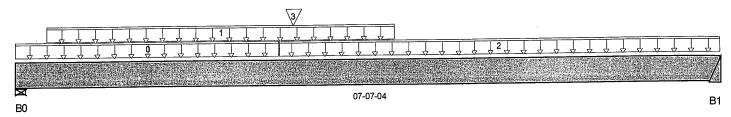
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B9(i1219)

Specifier: Designer:

Company.

Misc:



Total Horizontal Product I	_ength = 0	07-07-04
----------------------------	------------	----------

Reaction Summary (Dov	vn / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 4"	544/0	451/0			
B1	365/0	260/0			

						Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type	Re f.	Start	En d	1.00	0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L C	00-00-00	02-10-00	24	9			n/a
4	WALL	Unf. Lin. (lb/ft)	ī	00-04-00	04-01-00		60			n/a
;	•	` '	ī	2-10-00	07-07-04	27	10			n/a
2		` '				712	339			n/a
2	FC2 Floor Material B8 (i1218)	Unf. Lin. (lb/ft) Conc. Pt. (lbs)		)2-10-00 )2-11-12	07-07-04 02-11-12		10 339			

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	3,206 ft-lbs	25,408 ft-lbs	12.6%	1	02-11-12
End Shear	1.252 lbs	11,571 lbs	10.8%	1	01-01-08
Total Load Defl	L/999 (0.036")	n/a	n/a	4	03-08-00
Live Load Defl.	L/999 (0.021")	n/a	n/a	5	03-08-00
Max Defl.	0.036"	n/a	n/a	4	03-08-00
Span / Depth	9.1	n/a	n/a		00-00-00

Bearing Supports				Demand/ Resistance	Demand/ Resistance	Material	
		Dim . (L x W)	De man d	Support	Member		
B0	Wall/Plate	4" x 3-1/2"	1,379 lbs	18.4%	8.1%	Un spe cified	
B1	Hanger	2" x 3-1/2"	872 lbs	n/a	10.2%	IUS2.56/9.5	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B9(i1219)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:26

BC CALC® Design Report

Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

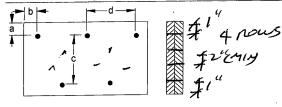
Description: Designs\Flush Beams\1st Floor\Flush Beams\B9(i1215

Specifier: Designer:

Company.

Misc:

### **Connection Diagram**



1

a minimum = 1" c = 1-1/2" u b minimum = 3" d = 4"

Calculated Side Load = 196.2 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

· Nails

3½" ARDOX SPIRAL

### Disclosure

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DWG NO.TAM 65/1-STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i893)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:28

BC CALC® Design Report

Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

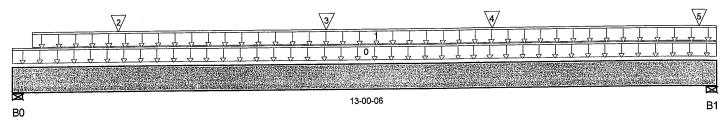
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i893)

Specifier: Designer:

Company:

Misc:



Total Horizontal Product Length = 13-00-06

Reaction Summary (D	Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 4-3/8"	356/0	938/0	427/0		
B1. 2-3/4"	413/0	1,012 / 0	541/0		

	! C					Live	Dead	Snow	Wind	Trib.
	ad Summary Description	Load Type Ref. Star		f. Start	Start End ′		0.65	1.00	1.15	
0	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-00	13-00-06	20	7			n/a
1	User Load	Unf. Lin. (lb/ft)	L	00-04-06	13-00-06		100			n/a
2	Us er Load	Conc. Pt. (lbs)	L	01-11-06	01-11-06	127	115	242		n/a
2	Us er Load	Conc. Pt. (lbs)	L	05-09-06	05-09-06	127	115	242		n/a
4	Us er Load	Conc. Pt. (lbs)	L	08-10-06	08-10-06	127	115	242		n/a
5		Conc. Pt. (lbs)	L	12-08-06	12-08-06	127	115	242		n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	4,148 ft-lbs	16,515 ft-lbs	25.1%	0	06-04-05
End Shear	1.855 lbs	11,571 lbs	16%	13	01-01-14
Total Load Defl.	L/546 (0.276")	0.628"	44%	45	06-06-10
Live Load Defl.	L/999 (0.108")	n/a	n/a	61	06-06-10
Max Defl.	0.276"	n/a	n/a	45	06-06-10
Span / Depth	15.9	n/a	n/a		00-00-00

Poor	ing Supports	Dim . (L × W)	Demand	De mand/ Resistance Support	De mand/ Resistance Member	Material
B0	ing Supports Wall/Plate	4-3/8" x 3-1/2"	1,313 lbs	24.7%	10.8%	Unspecified
B1	Wall/Plate	2-3/4" x 3-1/2"	2,283 lbs	44.4%	19.4%	Unspecified

Notes





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B11(i893)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:28

BC CALC® Design Report



**Build 5033** 

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B11(i89;

Specifier: Designer:

Company.

Misc:

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

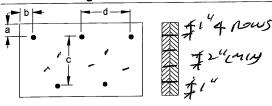
Resistance Factor phi has been applied to all presented results per CSA O86. BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

### Connection Diagram



a minimum = 2" -1/2" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d ARDUX SPIKAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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> POLYMICE OF ONLY DWG NO. TAN 6033217



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B8(i1218)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:30

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

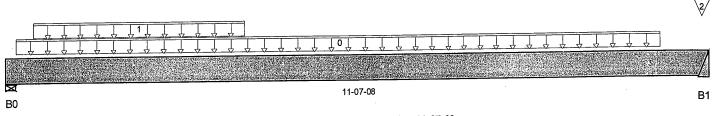
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\88(i1218)

Specifier: Designer:

Company:

Misc:



Total Horizontal Product Length = 11-07-08

Reaction Summary (I	Down / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 5-1/2"	1,289 / 0	629/0			
B1	715/0	341/0			

				Live	Dead	Snow Wind	i rib.
Load Summary Tag Description	Load Type	Ref. Start	En d	1.00	0.65	1.00 1.15	
0 Smoothed Load	Unf. Lin. (lb/ft)	L 00-02-02	10-10-02	101	38		n/a
•	Unf. Lin. (lb/ft)	L 00-05-08	03-11-08	240	120		n/a
1 STAR	Conc. Pt. (lbs)	L 11-06-02	11-06-02		30		n/a
2 J4(i823)	Conc. Pt. (ips)	L 11-00-02	11 00 02	0.	••		

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5,119 ft-lbs	25,408 ft-lbs	20.1%	. 1	04-10-02
End Shear	2.164 lbs	11,571 lbs	18.7%	1	01-03-00
Total Load Defl.	L/835 (0.16")	0.556"	28.7%	4	05-08-02
Live Load Defl.	L/999 (0.108")	n/a	· n/a	5	05-08-02
Max Defl.	0.16"	n/a	n/a	4	05-08-02
Span / Depth	14.1	n/a	n/a		00-00-00

				Resistance			
Bearing Supports		Dim.(LxW)	Demand	Support	Member	Material	
B0	Wall/Plate	5-1/2" x 3-1/2"	2,720 lbs	26.5%	11.6%	Unspecified	
B1	Hanger	2" x 3-1/2"	1,499 lbs	n/a	17.6%	HGUS410	

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Hanger Manufacturer: Unassigned

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA

O86.

CONFORMS TO OBC 2012

Design based on Dry Service Condition. Importance Factor: Normal Part code: Part 9





### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B8(i1218)

October 26, 2017 10:03:30

BC CALC® Design Report



Dry | 1 span | No cantilevers | 0/12 slope (deg)

Build 5033

Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

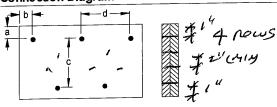
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B8(i1218

Specifier: Designer:

Company. Misc:

**Connection Diagram** 



a minimum = 2" b minimum = 3"

Calculated Side Load = 196.6 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are:

. Nails 'ARDOX SPIRAL

### Disclosure

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TO OF OF

DWG NO. TAM 60333 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1209)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:33

BC CALC® Design Report



Build 5033 Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

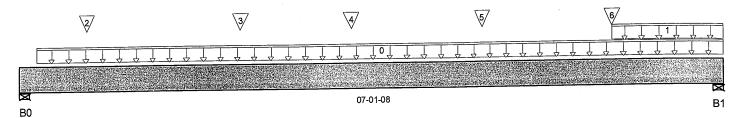
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B10(i1209)

Specifier:

Designer: Company:

Misc:



Total Horizontal Product Length = 07-01-08

Reaction Summary (Down	n / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 3-1/2"	1,356 / 0	625/0			
B1.5-1/2"	1,555 / 0	656/0			

	ad Summary	Load Type	Ret	f. Start	En d	Live 1.00	Dead 0.65	Snow 1.00	Wind 1.15	Trib.
0 1 2 3	Smoothed Load FC2 Floor Material J4(i153)	Unf. Lin. (lb/ft) Unf. Lin. (lb/ft) Conc. Pt. (lbs) Conc. Pt. (lbs)	L L L	00-02-02 06-00-02 00-08-02 02-02-13 03-04-02	07-01-08 07-01-08 00-08-02 02-02-13 03-04-02	32 127 427	73 48 280 119			n/a n/a n/a n/a n/a
4 5 6	J1 (i201) J1 (i91) J1 (i202)	Conc. Pt. (lbs) Conc. Pt. (lbs) Conc. Pt. (lbs)	L L	04-08-02 06-00-02	04-08-02	344	129 121			n/a n/a

Controls Summary	Factored Demand	Factored Resistance	Demand / Resistance	Load Case	Location
Pos. Moment	5.038 ft-lbs	25,408 ft-lbs	19.8%	1	03-04-02
End Shear	2.630 lbs	11,571 lbs	22.7%	1	05-10-08
Total Load Defl.	L/999 (0.053")	n/a	n/a	4	03-05-02
Live Load Defl.	L/999 (0.037")	n/a	n/a	5	03-06-02
Max Defl.	0.053"	n/a	n/a	4	03-05-02
Span / Denth	8.2	n/a	n/a		00-00-00

		5. (1 M)	D and	De mand/ Resistance	Demand/ Resistance Member	Material
Bear	earing Supports Dim. (LxW) Demand Support					
B0	Wall/Plate	3-1/2" x 3-1/2"	2,814 lbs	43%	18.8%	Un spe dified
B1	Wall/Plate	5-1/2" x 3-1/2"	3,152 lbs	30.7%	13.4%	Un spe cified

Notes



DWG NO. TAM 633417 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\...\B10(i1209)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:33

BC CALC® Design Report

City, Province, Postal Code:,

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1stFloor\Flush Beams\B10(i12(

Specifier:

Company:

Designer:

Misc:

Customer: Code reports:

**Build 5033** 

Job Name:

Address:

CCMC 12472-R

Design meets Code minimum (L/240) Total load deflection criteria. Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA O86.

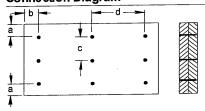
BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

O86.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

**Connection Diagram** 



c = 2-3/4" a minimum = 2"  $b \min = 3$ "

Calculated Side Load = 445.9 lb/ft

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record.

Connectors are: 16d 🧳 : Nails

34" ARDOX SPIRAL

### Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

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### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B12(i906)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:36

BC CALC® Design Report

**Build 5033** Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

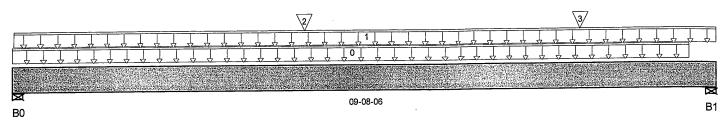
CCMC 12472-R

File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i906)

Specifier: Designer: Company.

Misc:



Total Horizontal Product Length = 09-08-06

Reaction Summary (De	own / Uplift) (lbs) Live	De ad	Snow	Wind	
B0, 2-3/4"	223/0	658/0	182/0		
B1, 4-3/8"	289/0	695/0	302/0		

١٠	ad Summanı					Live	Dead	Snow	Wind	Trib.
	ad Summary g Description	Load Type	Ref. Start		En d	1.00	0.65	1.00	1.15	
0	Us er Load	Unf. Lin. (lb/ft)	L	00-00-00	09-04-00		100			n/a
1	FC2 Floor Material	Unf. Lin. (lb/ft)	L	00-00-04	09-08-06	27	10			n/a
2	User Load	Conc. Pt. (lbs)	L	04-00-00	04-00-00	127	115	242		n/a
3	Us er Load	Conc. Pt. (lbs)	L	07-10-00	07-10-00	127	115	242		n/a

	Factored	Factored	Demand /	Load	Location
Controls Summary	Demand	Resistance	Resistance	Case	
Pos. Moment	2,222 ft-lbs	16,515 ft-lbs	13.5%	0	04-07-03
End Shear	1,323 lbs	11,571 lbs	11.4%	13	08-06-08
Total Load Defl.	L/999 (0.078")	n/a	n/a	45	04-10-01
Live Load Defl.	L/999 (0.029")	n/a	n/a	61	04-10-01
Max Defl.	0.078"	n/a	n/a	45	04-10-01
Span / Depth	11.7	n/a	n/a		00-00-00

Roar	ing Supports	Dim.(L x W)	De man d	Resistance Support	Resistance Member	Material
B0	Wall/Plate Wall/Plate	2-3/4" x 3-1/2"	922 lbs	27.6%	12.1%	Un specified
B1		4-3/8" x 3-1/2"	973 lbs	18.3%	8%	Un specified

### Notes

Design meets Code minimum (L/240) Total load deflection criteria.

Design meets Code minimum (L/360) Live load deflection criteria.

Calculations assume member is fully braced.

Resistance Factor phi has been applied to all presented results per CSA 086.

BC CALC® analysis is based on Canadian Limit States Design, as per NBCC 2010 and CSA CONFORMS TO OBC 2012

Unbalanced snow loads determined from building geometry were used in selected product's verification.

Design based on Dry Service Condition.

Importance Factor: Normal Part code: Part 9

DWG NO. TAM 60332 STRUCTURAL COMPONENT ONLY



### Double 1-3/4" x 9-1/2" VERSA-LAM® 2.0 3100 SP 1st Floor\Flush Beams\B12(i906)

Dry | 1 span | No cantilevers | 0/12 slope (deg)

October 26, 2017 10:03:36

BC CALC® Design Report



**Build 5033** Job Name: Address:

City, Province, Postal Code:,

Customer:

Code reports:

CCMC 12472-R

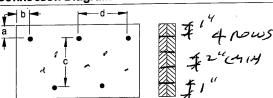
File Name: NEWBERRY 2Ammdl

Description: Designs\Flush Beams\1st Floor\Flush Beams\B12(i906

Specifier: Designer: Company.

Misc:

**Connection Diagram** 



a minimum = ?" b minimum = 3"

Connection design assumes point load is top-loaded. For connection design of side-loaded point loads, please consult a technical representative or professional of Record. Member has no side loads.

Connectors are: 16d // Nails

312" ARDOX SPIRAL

### Disclosure

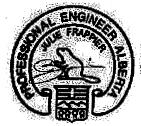
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call 1-800-964-6999 before installation.

BC CALO®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD $^{\mathsf{TM}}$ , BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Cascade Wood Products L.L.C.





Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare		1	1/2" Gypsum Ceiling				
Depth	Series		On Cen	tre Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	Ni-20	15'-1"	14'-2"	13'-9"	N/A	15'-7"	14'-8"	14'-2"	N/A		
	N!-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	NI-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17' <b>-</b> 3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11.770	NI-70	19'-6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17'-9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20' <b>-</b> 5"	18'-11"	18'-1"	N/A	21'-2"	19'-7"	18' <del>-</del> 9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19' <del>-</del> 8"	N/A		
	NI-80	21'-11"	20'-3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
10	NI-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23'-5"	22'-4"	N/A		

			Mid-Spa	n Blocking		Mid-	Span Blocking a	nd 1/2" Gypsum	Ceiling		
Depth	Series		On Cent	re Spacing			On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24'		
	NI-20	16'-8"	15'-3"	14'-5"	N/A	16'-8"	15'-3"	14'-5"	N/A		
	NI-40x	17'-11"	16'-11"	16'-1"	N/A	18'-5"	17'-1"	16'-1"	N/A		
9-1/2"	NI-60	18'-2"	17'-1"	16'-4"	N/A	18'-7"	17'-4"	16'-4"	N/A		
	NI-70	19'-2"	17'-10"	17'-2"	N/A	19'-7"	18'-3"	17'-7"	N/A		
	NI-80	19'-5"	18'-0"	17'-4"	N/A	19'-10"	18'-5"	17'-8"	N/A		
	NI-20	19'-6"	18'-1"	17'-3"	N/A	19'-11"	18'-3"	17'-3"	N/A		
	NI-40x	21'-0"	19'-6"	18'-8"	N/A	21'-7"	20' <del>-</del> 2"	19'-2"	N/A		
11-7/8"	NI-60	21'-4"	19'-9"	18'-11"	N/A	21'-11"	20'-4"	19'-6"	N/A		
11 7/0	NI-70	22'~6"	20'-10"	19'-11"	N/A	23'-0"	21'-5"	20'-5"	N/A		
	NI-80	22' <b>-</b> 9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-8"	N/A		
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A		
	NI-40x	23'-7"	21'-11"	20'-11"	N/A	24'-3"	22'-7"	21'-7"	N/A		
	NI-60	24'-0"	22' <b>-</b> 3"	21'-3"	N/A	24'-8"	22'-11"	21'-11"	N/A		
14"	NI-70	25 <b>'-</b> 3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-11"	N/A		
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23'-2"	N/A		
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A		
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	25'-3"	24'-2"	N/A		
16"	NI-70	27'-9"	25'-8"	24'-6"	N/A	28'-5"	26'-5"	25'-2"	N/A		
	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A		
	NI-90x	29'-0"	26'-10"	25'-7"	N/A	29'-7"	27'-5"	26'-2"	N/A		

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 15 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







				are		1/2" Gypsum Ceiling On Centre Spacing				
Depth	Series		On Cent	re Spacing						
		12"	16"	19.2"	24"	12"	16"	<b>/</b> 19.2"	24"	
	NI-20	15'-10"	15'-0"	14'-5"	13'-5"	16'-4"	15'-5"	14'-6"	13'-5"	
	NI-40x	17'-0"	16'-0"	15'-5"	14'-9"	17'-5"	16'-5"	15'-10"	15'-2"	
9-1/2"	NI-60	17'-2"	16'-2"	15'-7"	14'-11"	17'-6"	16'-7"	15'-11"	15'-3"	
	N!-70	18'-0"	16'-11"	16'-3"	15'-7"	18'-5"	17'-3"	16'-7"	15'-11"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	<b>17'-5</b> "	16'-9"	16'-1"	
	NI-20	17'-10"	16'-10"	16'-2"	15'-6"	18'-6"	17'-4"	16'-9"	16'-1"	
	NI-40x	19'-4"	17'-11"	17'-3"	16'-6"	19'-11"	18'-6"	17'-9"	17'-0"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16'-9"	20'-2"	18'-9"	17'-11"	17'-2"	
11-7/0	NI-70	20'-9"	19' <b>-</b> 2"	18'-3"	17'-5"	21'-4"	19 <b>'-</b> 9"	18'-10"	17'-10"	
•	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	NI-40x	21'-5"	19'-10"	18'-11"	17'-11"	22'-1"	20'-6"	19'-7"	18'-7"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"	
	NI-80	23' <b>-</b> 5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22'-3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"	
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25' <del>-</del> 9"	23'-10"	22'-9"	21'-6"	
10	NI-80	25'-6"	23'-6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

			Mid-Spa	n Blocking		Mid-S	pan Blocking ar	nd 1/2" Gypsum	Ceiling	
Depth	Series		On Cent	re Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	16'-10"	15'-5"	14'-6"	13'-5"	16'-10"	15'-5"	14'-6"	13'-5"	
	NI-40x	18'-8"	17'-2"	16'-3"	15'-2"	18'-10"	17'-2"	16'-3"	15'-2"	
9-1/2"	NI-60	18'-11"	17'-6"	16'-6"	15'-5"	19'-2"	17'-6"	16'-6"	15'-5"	
	NI-70	20'-0"	18'-7"	17'-9"	16'-7"	20'-5"	18'-11"	17'-10"	16'-7"	
	NI-80	20'-3"	18'-10"	17'-11"	16'-10"	20'-8"	19'-3"	18'-2"	16'-10"	
	NI-20	20'-1"	18'-5"	17'-5"	16'-2"	20'-1"	18'-5"	17'-5"	16'-2"	
44 7/01	NI-40x	21'-10"	20'-4"	19'-4"	17'-8"	22'-5"	20'-6"	19'-4"	17'-8"	
	NI-60	22'-1"	20'-7"	19'-7"	18'-4"	22'-8"	20'-10"	19'-8"	18'-4"	
11-7/8"	NI-70	23'-4"	21'-8"	20'-8"	19'-7"	23'-10"	22'-3"	21'-2"	19'-9"	
	NI-80	23'-7"	21'-11"	20'-11"	19'-9"	24'-1"	22'-6"	21' <del>-</del> 5"	20'-0"	
	NI-90x	24'-3"	22'-6"	21'-6"	20'-4"	24'-8"	23'-0"	22'-0"	20'-9"	
	NI-40x	24'-5"	22'-9"	21'-8"	19'-5"	25'-1"	23'-2"	21'-9"	19'-5"	
	NI-60	24'-10"	23'-1"	22'-0"	20'-10"	25'-6"	23'-8"	22'-4"	20'-10"	
14"	NI-70	26'-1"	24'-3"	23'-2"	21'-10"	26'-8"	24'-11"	23' <b>-</b> 9"	22'-4"	
	NI-80	26'-6"	24'-7"	23'-5"	22'-2"	27'-1"	25'-3"	24'-1"	22'-9"	
	NI-90x	27'-3"	25'-4"	24'-1"	22'-9"	27'-9"	25'-11"	24'-8"	23'-4"	
	NI-60	27' <b>-</b> 3"	25'-5"	24'-2"	22'-10"	28'-0"	26'-2"	24'-9"	23'-1"	
l6"	NI-70	28' <b>-</b> 8"	26'-8"	25'-4"	23'-11"	29'-3"	27'-4"	26'-1"	24'-8"	
LO	NI-80 .	29'-1"	27'-0"	25'-9"	24'-4"	29 <b>'-</b> 8"	27'-9"	26'-5"	25'-0"	
	NI-90x	29'-11"	27'-10"	26'-6"	25'-0"	30'-6"	28'-5"	27'-2"	25 <b>'-</b> 8"	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 5/8" OSB G&N Sheathing







				Bare		1	1/2" Gypsum Ceiling On Centre Spacing				
Depth	Series		On Cent	re Spacing							
		12"	16"	19.2"	24"	12"	16"	19.2"	24"		
	NI-20	15'-1"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A		
	NI-40x	16'-1"	15'-2"	14'-8"	N/A	16'-7"	15'-7"	15'-1"	N/A		
9-1/2"	NI-60	16'-3"	15'-4"	14'-10"	N/A	16'-8"	15'-9"	15'-3"	N/A		
	NI-70	17'-1"	16'-1"	15'-6"	N/A	17'-5"	16'-5"	15'-10"	N/A		
	NI-80	17'-3"	16'-3"	15'-8"	N/A	17'-8"	16'-7"	16'-0"	N/A		
	NI-20	16'-11"	16'-0"	15'-5"	N/A	17'-6"	16'-6"	16'-0"	N/A		
	Ni-40x	18'-1"	17'-0"	16'-5"	N/A	18'-9"	17'-6"	16'-11"	N/A		
11-7/8"	NI-60	18'-4"	17'-3"	16'-7"	N/A	19'-0"	17'-8"	17'-1"	N/A		
11-7/0	Ni-70	19' <b>-</b> 6"	18'-0"	17'-4"	N/A	20'-1"	18'-7"	17 <b>'-</b> 9"	N/A		
	NI-80	19'-9"	18'-3"	17'-6"	N/A	20'-4"	18'-10"	17'-11"	N/A		
	NI-90x	20'-4"	18'-9"	17'-11"	N/A	20'-10"	19'-3"	18'-5"	N/A		
-	NI-40x	20'-1"	18'-7"	17'-10"	N/A	20'-10"	19'-4"	18'-6"	N/A		
	NI-60	20'-5"	18'-11"	18'-1"	N/A	21'-2"	19 <b>'-</b> 7"	18'-9"	N/A		
14"	NI-70	21'-7"	20'-0"	19'-1"	N/A	22'-3"	20'-7"	19'-8"	N/A		
	NI-80	21'-11"	20' <del>-</del> 3"	19'-4"	N/A	22'-7"	20'-11"	20'-0"	N/A		
	NI-90x	22'-7"	20'-11"	19'-11"	N/A	23'-3"	21'-6"	20'-6"	N/A		
	NI-60	22'-3"	20'-8"	19'-9"	N/A	23'-1"	21'-5"	20'-6"	N/A		
16"	NI-70	23'-6"	21'-9"	20'-9"	N/A	24'-3"	22'-5"	21'-5"	N/A		
10	N!-80	23'-11"	22'-1"	21'-1"	N/A	24'-8"	22'-10"	21'-9"	N/A		
	NI-90x	24'-8"	22'-9"	21'-9"	N/A	25'-4"	23' <b>-</b> 5"	22'-4"	N/A		

	Mid-Span Blocking					Mid-S	Span Blocking ar	nd 1/2" Gypsum	Ceiling	
Depth	Series		On Cent	re Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-1"	13'-3"	N/A	15'-7"	14'-1"	13'-3"	N/A	
	NI-40x	17'-9"	16'-1"	15'-1"	N/A	17'-9"	16'-1"	15'-1"	N/A	
9-1/2"	NI-60	18'-1"	16'-4"	15'-4"	N/A	18'-1"	16'-4"	15'-4"	N/A	
	NI-70	19'-2"	17'-10"	16'-9"	N/A	19'-7"	17'-10"	16'-9"	N/A	
	NI-80	19'-5"	18'-0"	17'-1"	N/A	19'-10"	18'-3"	17'-1"	N/A	
	NI-20	18'-9"	17'-0"	16'-0"	N/A	18'-9"	17'-0"	16'-0"	N/A	
44.7/01	NI-40x	21'-0"	19'-3"	17'-9"	N/A	21'-3"	19'-3"	17'-9"	N/A	
	NI-60	21'-4"	19'-8"	18'-5"	N/A	21'-8"	19'-8"	18' <b>-</b> 5"	N/A	
11-7/8"	NI-70	22'-6"	20'-10"	19'-11"	N/A	23'-0"	21'-4"	20'-0"	N/A	
	NI-80	22'-9"	21'-1"	20'-1"	N/A	23'-3"	21'-7"	20'-5"	N/A	
	NI-90x	23'-4"	21'-8"	20'-8"	N/A	23'-10"	22'-2"	21'-2"	N/A	
	NI-40x	23'-7"	21'-5"	19'-6"	N/A	24'-1"	21'-5"	19'-6"	N/A	
	NI-60	24'-0"	22'-3"	21'-0"	N/A	24'-8"	22'-5"	21'-0"	N/A	
14"	NI-70	25'-3"	23'-4"	22'-3"	N/A	25'-10"	24'-0"	22'-9"	N/A	
	NI-80	25'-7"	23'-8"	22'-7"	N/A	26'-2"	24'-4"	23' <b>-</b> 2"	N/A	
	NI-90x	26'-4"	24'-4"	23'-3"	N/A	26'-10"	24'-11"	23'-9"	N/A	
	NI-60	26'-5"	24'-6"	23'-4"	N/A	27'-2"	24'-10"	23'-4"	N/A	
16"	NI-70	27'-9"	25' <b>-</b> 8"	24'-6"	N/A	28'-5"	26'-5"	25' <b>-</b> 2"	N/A	
16"	NI-80	28'-2"	26'-1"	24'-10"	N/A	28'-10"	26'-9"	25'-6"	N/A	
	NI-90x	29'-0"	26 <b>'-1</b> 0"	25' <b>-</b> 7"	N/A	29'-7"	27'-5"	26'-2"	N/A	

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.



Live Load = 40 psf, Dead Load = 30 psf Simple Spans, L/480 Deflection Limit 3/4" OSB G&N Sheathing







			B	are		1	1/2" Gyp	sum Ceiling		
Depth	Series		On Cent	re Spacing		On Centre Spacing				
		12"	16"	19.2"	24"	12"	16"	19.2"	24"	
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"	
	NI-40x	17'-0"	16'-0"	15'-1"	13'-11"	17'-5"	16'-1"	15'-1"	13'-11"	
9-1/2"	NI-60	17'-2"	16'-2"	15'-5"	14'-3"	17'-6"	16'-5"	15'-5"	14'-3"	
	NI-70	18'-0"	16'-11"	16'-3"	15'-6"	18'-5"	17'-3"	16'-7"	15'-6"	
	NI-80	18'-3"	17'-1"	16'-5"	15'-9"	18'-8"	17'-5"	16'-9"	15'-10"	
	NI-20	17'-10"	16'-10"	16'-0"	14'-10"	18'-6"	17'-1"	16'-0"	14'-10"	
	NI-40x	19'-4"	17'-11"	17'-3"	15'-10"	19'-11"	18'-6"	17'-9"	15'-10"	
11-7/8"	NI-60	19'-7"	18'-2"	17'-5"	16' <b>-</b> 9"	20'-2"	18'-9"	17'-11"	17'-1"	
11-7/6	NI-70	20'-9"	19'-2"	18'-3"	17'-5"	21'-4"	19'-9"	18'-10"	17'-10"	
	NI-80	21'-1"	19'-5"	18'-6"	17'-7"	21'-7"	20'-0"	19'-0"	18'-0"	
	NI-90x	21'-8"	20'-0"	19'-1"	18'-0"	22'-2"	20'-6"	19'-6"	18'-6"	
	NI-40x	21'-5"	19'-10"	18'-11"	17'-5"	22'-1"	20'-6"	19'-6"	17'-5"	
	NI-60	21'-10"	20'-2"	19'-3"	18'-2"	22'-5"	20'-10"	19'-11"	18'-10"	
14"	NI-70	23'-0"	21'-3"	20'-3"	19'-2"	23'-8"	21'-11"	20'-10"	19'-9"	
	NI-80	23'-5"	21'-7"	20'-7"	19'-5"	24'-0"	22'-3"	21'-2"	20'-0"	
	NI-90x	24'-1"	22 <b>'-</b> 3"	21'-2"	20'-0"	24'-8"	22'-10"	21'-9"	20'-7"	
	NI-60	23'-9"	22'-0"	20'-11"	19'-10"	24'-6"	22'-9"	21'-8"	20'-6"	
16"	NI-70	25'-1"	23'-2"	22'-0"	20'-10"	25' <b>-</b> 9"	23'-10"	22'-9"	21'-6"	
10	NI-80	25'-6"	23' <del>-</del> 6"	22'-4"	21'-2"	26'-1"	24'-2"	23'-1"	21'-10"	
	NI-90x	26'-4"	24'-3"	23'-1"	21'-10"	26'-11"	24'-11"	23'-8"	22'-5"	

			Mid-Spa	ın Blocking		Mid-	Span Blocking ar	nd 1/2" Gypsum	Ceiling
Depth	Series		On Cent	re Spacing			On Cent	re Spacing	
		12"	16"	19.2"	24"	12"	16"	19.2"	24"
	NI-20	15'-7"	14'-2"	13'-4"	12'-4"	15'-7"	14'-2"	13'-4"	12'-4"
	NI-40x	17'-9"	16'-1"	15'-1"	13'-11"	17'-9"	16'-1"	15'-1"	13'-11"
9-1/2"	NI-60	18'-1"	16'-5"	15'-5"	14'-3"	18'-1"	16'-5"	15' <b>-</b> 5"	14'-3"
	NI-70	19'-10"	17'-11"	16'-9"	15'-6"	19'-10"	17'-11"	16'-9"	15'-6"
	NI-80	20'-2"	18'-3"	17'-1"	15'-10"	20'-2"	18'-3"	17'-1"	15'-10"
	NI-20	18'-10"	17'-1"	16'-0"	14'-10"	18'-10"	17'-1"	16'-0"	14'-10"
	NI-40x	21'-3"	19' <b>-</b> 3"	17' <del>-</del> 9"	15'-10"	21'-3"	19'-3"	17'-9"	15'-10"
11-7/8"	NI-60	21' <b>-</b> 9"	19' <b>-</b> 8"	18'-5"	17'-1"	21'-9"	19'-8"	18'-5"	17'-1"
11-7/0	NI-70	23'-4"	21'-5"	20'-1"	18'-6"	23'-8"	21'-5"	20'-1"	18'-6"
	NI-80	23'-7"	21'-10"	20'-5"	18'-11"	24'-1"	21'-10"	20'-5"	18'-11"
	NI-90x	24'-3"	22'-6"	21'-3"	19'-7"	24'-8"	22'-7"	21'-3"	19'-7"
	NI-40x	24'-2"	21'-5"	19'-6"	17'-5"	24'-2"	21'-5"	19'-6"	17'-5"
	NI-60	24'-9"	22'-5"	21'-0"	19'-6"	24'-9"	22'-5"	21'-0"	19'-6"
14"	NI-70	26'-1"	24'-3"	22' <del>-</del> 9"	21'-0"	26'-8"	24'-3"	22'-9"	21'-0"
	NI-80	26'-6"	24'-7"	23'-3"	21'-6"	27'-1"	24'-10"	23'-3"	21'-6"
	NI-90x	27'-3"	25'-4"	24' <b>-</b> 1"	22'-4"	27'-9"	25'-10"	24'-3"	22'-4"
	NI-60	27'-3"	24'-11"	23'-5"	21'-7"	27'-6"	24'-11"	23'-5"	21'-7"
16"	NI-70	28' <b>-</b> 8"	26'-8"	25'-3"	23'-4"	29' <b>-</b> 3"	26'-11"	25' <b>-</b> 3"	23'-4"
10	NI-80	29'-1"	27'-0"	25 <b>'-</b> 9"	23'-10"	29'-8"	27'-6"	25'-10"	23'-10"
	NI-90x	29'-11"	27'-10"	26'-6"	24'-10"	30'-6"	28' <b>-</b> 5"	26'-11"	24'-10"

<sup>1.</sup> Maximum clear span applicable to simple-span residential floor construction with a design live load of 40 psf and dead load of 30 psf. The ultimate limit states are based on the factored loads of 1.50L + 1.25D. The serviceability limit states include the consideration for floor vibration, a live load deflection limit of L/480 and a total load deflection limit of L/240.

<sup>2.</sup> Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 3/4 inch for a joist spacing of 24 inches or less. The composite floor may include 1/2 inch gypsum ceiling and/or one row of blocking at mid-span with strapping. Strapping shall be minimum 1x4 inch strap applied to underside of joists at blocking line or 1/2 inch gypsum ceiling attached to joists.

<sup>3.</sup> Minimum bearing length shall be 1-3/4 inches for the end bearings.

<sup>4.</sup> Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.

<sup>5.</sup> This span chart is based on uniform loads. For applications with other than uniformly distributed loads, an engineering analysis may be required based on the use of the design properties. Tables are based on Limit States Design per CSA O86-09, NBC 2010, and OBC 2012.

<sup>6.</sup> Joists shall be laterally supported at supports and continuously along the compression edge. Refer to technical documentation for installation guidelines and construction details. Nordic I-joists are listed in CCMC evaluation report 13032-R and APA Product Report PR-L274C.

braced and sheathed. l-joists are not stable until completely installed, and will not carry any load until fully

## Avoid Accidents by Following these Important Guidelines:

- Brace and nail each I-joist as it is installed, using hangers, blocking panels, rim board, and/or cross-bridging at joist ends. When I-joists are applied continuous blocking will be required at the interior support. over interior supports and a load-bearing wall is planned at that location,
- When the building is completed, the floor sheathing will provide lateral support for the top flanges of the I-joists. Until this sheathing is applied, temporary bracing, often called struts, or temporary sheathing must be applied to prevent l-joist rollover or buckling.

braced, or serious inju-

ries can result.

until fully fastened and Do not walk on I-joists

- Temporary bracing or struts must be 1x4 inch minimum, at least 8 feet long and spaced no more than 8 feet on centre, and must be secured with a minimum of two 2-1/2" nails fastened to the top surface of each I-joist. Nail the bracing to a lateral restraint at the end of each bay. Lap ends of adjoining bracing over at least two 1-joists.
- Or, sheathing (temporary or permanent) can be nailed to the top flange of the first 4 feet of I-joists at the end of the bay.
- 3. For cantilevered I-joists, brace top and bottom flanges, and brace ends with closure panels, rim board, or cross-bridging.
- 4. Install and fully nail permanent sheathing to each I-joist before placing loads on the floor system. Then, stack building materials over beams or walls only.

concentrated loads from building materials.

Once sheathed, do not

Never stack building

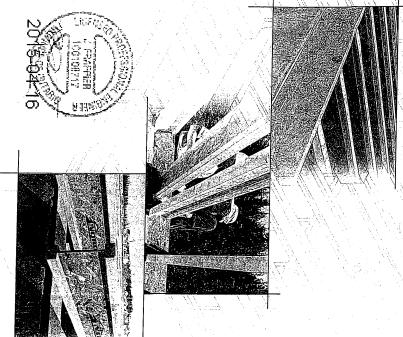
over-stress I-joist with materials over unsheathed I-joists.

Never install a damaged I-joist.

Improper storage or installation, failure to follow applicable building codes, failure to follow span ratings for Nordic I-joists, failure to follow allowable hole sizes and locations, or failure to use web stiffeners when required can result in serious accidents. Follow these installation guidelines carefully.

## ENGINEERED WOOD





Distributed by:



## STORAGE AND HANDLING GUIDELINES

- 1. Bundle wrap can be slippery when wet. Avoid walking on wrapped
- Store, stack, and handle I-joists vertically and level only.
- 3. Always stack and handle Lioists in the upright position only.
- Protect I-joists from weather, and use spacers to separate bundles.

4. Do not store I-joists in direct contact with the ground and/or flatwise

- Bundled units should be kept intact until time of installation.
- 7. When handling I-joists with a crane on the job site, take a few simple precautions to prevent damage to the I-joists and injury
- Pick I-joists in bundles as shipped by the supplier.
- Orient the bundles so that the webs of the I-joists are vertical.
- Pick the bundles at the 5th points, using a spreader bar if necessary,
- 8. Do not handle t-joists in a horizontal orientation.
- 9. NEVER USE OR TRY TO REPAIR A DAMAGED 1-JOIST.





### MAXIMUM FLOOR SPANS

- Maximum **clear** spans applicable to simple-span or multiple-span residential floor construction with a design or more of the adjacent span. 1.25D. The serviceability limit states include the consideration for floor vibration and a live load deflection limit of L/480. for multiple-span applications, the end spans shall be 40% live load of 40 psf and dead load of 15 psf. The ultimate limit states are based on the factored loads of 1.50L +
- Spans are based on a composite floor with glued-nailed oriented strand board (OSB) sheathing with a minimum thickness of 5/8 inch for a joist spacing of 19.2 inches or less, or 3/4 inch for joist spacing of 24 inches. Adhesive of gypsum and/or a row of blocking at mid-span. Standard. No concrete topping or bridging element was assumed. Increased spans may be achieved with the used shall meet the requirements given in CGBS-71.26
- Minimum bearing length shall be 1-3/4 inches for the end bearings, and 3-1/2 inches for the intermediate bearings.
- Bearing stiffeners are not required when I-joists are used with the spans and spacings given in this table, except as required for hangers.
- 5. This span chart is based on uniform loads. For applications be required based on the use of the design properties. with other than uniform loads, an engineering analysis may
- 6. Tables are based on Limit States Design per CAN/CSA O86-09 Standard, and NBC 2010.
- 7. SI units conversion: 1 inch = 25.4 mm 1 foot = 0.305 m

### SIMPLE AND MULTIPLE SPANS **MAXIMUM FLOOR SPANS FOR NORDIC I-JOISTS**

Joist Depth	Joist Series		Simple On centr	spans e spacing			Multiple On centr	e spans e spacing
		12"	16"	19.2	24"	12"	16"	19.2*
			14:24 14:24	13:9:		15.00 M	544	
	- 10	10.01	50	14-10	4-11	177	617	
			1651	15:6	5.0		724	
			16:0	15'5'	15.6		200	
			To de	16-7	6.6	2000	18.0	6 :
				77.4	5			9
				17.10				9.
		200	11817	17-10	To do	22/2	20-6	19
			2010	19-1			20-11 22-11	200
				19:4"	1995		22 5 22 10	00
	18 2		20	19-9"	20-0	7510	200	22:0
S	: :, :	200		20-9 21-1		20.00	24.0	313
		5 (2)	22.6	21-5	8) 8)	28	24110	N.

## CCMC EVALUATION REPORT 13032-R

### **I-JOIST HANGERS**

- Hangers shown illustrate the three to support 1-joists. most commonly used metal hangers
- 2. All nailing must meet the hanger manufacturer's recommendations.
- Hangers should be selected based on the joist depth, flange width and load capacity based on the maximum spans.
- Web stiffeners are required when the brace the top flange of the 1-joist. sides of the hangers do not laterally







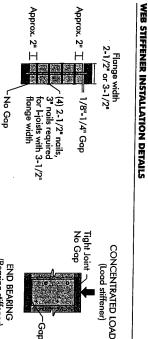
Face Mount

### WEB STIFFENERS

### RECOMMENDATIONS:

- Lipist properties table found of the *Lipist* Construction Guide (C101). The gap between the stiffener and the flange is at the top. reactions greater than shown in the engineered applications with factored A bearing stiffener is required in all
- stiffener and flange is at the top. support, the top flange. The gap between the the I-joist is supported in a hanger and the sides of the hanger do not extend up to, and A bearing stiffener is required when
- and the flange is at the bottom. by the code. The gap between the stiffener adjusted for other load durations as permitted standard term load duration, and may be tip and the support. These values are for cantilever, anywhere between the cantilever than 2,370 lbs is applied to the top flange where a factored concentrated load greater A load stiffener is required at locations between supports, or in the case of a

SI units conversion: 1 inch = 25.4 mm



### See table below for web stiffener size requirements

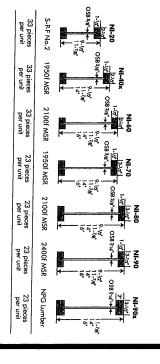
(Bearing stiffener)

3-1/2"	2-1/2"	Flange Width
1-1/2" x 2-5/16" minimum width	1" x 2-5/16" minimum width	Web Stiffener Size Each Side of Web

STIFFENER SIZE REQUIREMENTS

### No Gap Tight Join 0

### NORDIC I-JOIST SERIES



manufacturing process. Every phase of the operation, from totals to the products to adhere to strict quality control procedures throughout the Chantiers Chibougamau Ltd. harvests its own trees, which enables. Narrylg finished product, reflects our commitment to quality.

Nordic Engineered Wood I-joists use only finger-jointed black spruce longer span carrying capacity. lumber in their flanges, ensuring consistent quality, superior strength time. Heldeller

2015-04-16

### INSTALLING NORDIC I-JOISTS

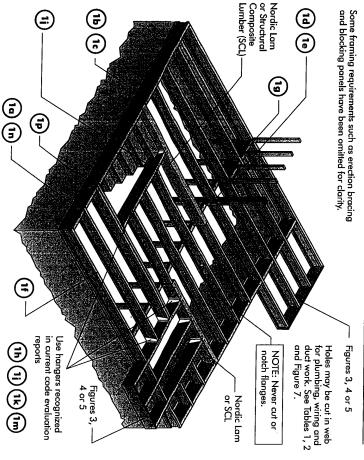
- 1. Before laying out floor system components, verify that I-joist flange widths match hanger widths. If not, contratyour
- 2. Except for cutting to length, I-joist flanges should never be cut, drilled, or notched.
- 3. Install I-joists so that top and bottom flanges are within 1/2 inch of true vertical alignment.

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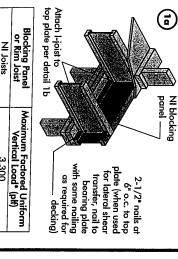
- 4. I-joists must be anchored securely to supports before floor sheathing is attached, and supports for multiple அளில்கதாய்
- 5. Minimum bearing lengths: 1-3/4 inches for end bearings and 3-1/2 inches for intermediate bearings 2015-04-16
- 6. When using hangers, seat I-joists firmly in hanger bottoms to minimize settlement.
- Leave a 1/16-inch gap between the I-joist end and a header.
- the top surface of the top flange. Normal concentrated loads include track lighting fixtures, audio equipment and security cameras. Never suspend unusual or heavy loads from the Ljoist's bottom flange. Whenever possible, suspend all concentrated loads from the top of the Ljoist. Or, attach the load to blocking that has been securely fastened to the Concentrated loads greater than those that can normally be expected in residential construction should only be applied to
- 9. Never install Lipists where they will be permanently exposed to weather, or where they will remain in direct contact with
- 10. Restrain ends of floor joists to prevent rollover. Use rim board, rim joists or 1-joist blocking panels.
- 11. For I-joists installed over and beneath bearing walls, use full depth blocking panels, rim board, or squash blocks (cripple members) to transfer gravity loads through the floor system to the wall or foundation below.
- 12. Due to shrinkage, common framing lumber set on edge may never be used as blocking or rim boards. Hoist blocking panels or other engineered wood products – such as rim board – must be cut to fit between the Ljoists, and an I-Joist-compatible depth selected.
- 13. Provide permanent lateral support of the bottom flange of all I-joists at interior supports of multiple-span joists. Similarly, structure, the gypsum wallboard ceiling provides this lateral support. Until the final finished ceiling is applied, temporary bracing or struts must be used. support the bottom flange of all camilevered Lioists at the end support next to the cantilever extension. In the completed
- 14. If square-edge panels are used, edges must be supported between Ljoists with 2x4 blocking. Glue panels to blocking to minimize squeaks. Blocking is not required under structural finish flooring, such as wood strip flooring, or if a separate underlayment layer is installed
- 15. Nail spacing: Space nails installed to the flange's top face in accordance with the applicable building code requirements or approved building plans.

One 2-1/2"

## TYPICAL NORDIC I-JOIST FLOOR FRAMING AND CONSTRUCTION DETAILS

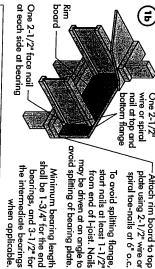


All nails shown in the above details are assumed to be common wire nails unless otherwise noted. 3" (0.122" dia.) common spiral nails may be substituted for 2-1/2" (0.128" dia.) common wire nails. Framing lumber assumed to be Spruce-Pine-Fir No. 2 or better. Individual components not shown to scale for clarity



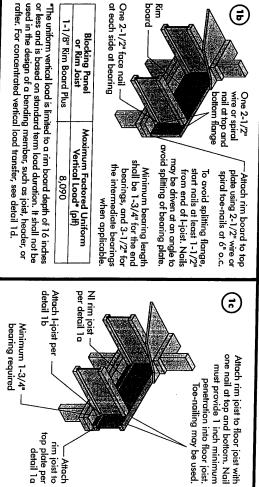
or Rim Joist	Vertical Load* (p#)
NI Joists	3,300
*The uniform vertical load	*The uniform vertical load is limited to a joist depth of 16
Inches or less and is based in the	Inches or less and is based on standard term load duration. It shall not be used in the design of a bending member
such as joist, header, or r	such as joist, header, or rafter. For concentrated vertical

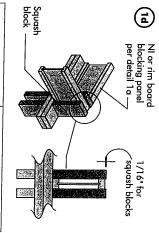
load transfer, see detail 1d.



8,090	1-1/8" Rim Board Plus
Maximum Factored Uniform	Blocking Panel
Vertical Load* (pH)	or Rim Joist

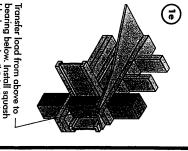
rafter. For concentrated vertical load transfer, see detail



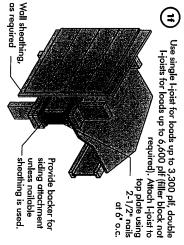


1-1/8" Rim Board Plus / 300	2x Lumber 5,500	3-1/2" wide	Pair of Squash Blocks Pair of Sc
			Maximum Factored Vertical per Pair of Squash Blocks (lbs)
6 600	8,500	5-1/2" wide	/ertical per cks (lbs)

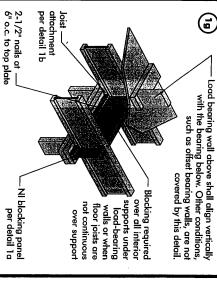
Provide lateral bracing per detail la, lb, or lc

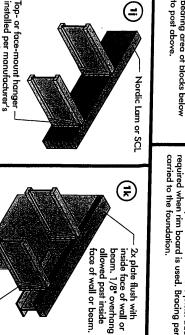


to post above, bearing area of blocks below blocks per detail 1d. Match bearing below. Install squash



Rim board may be used in lieu of L-joists. Backer is not required when rim board is used. Bracing per code shall be





(T)

 $\Xi$ 

Top-mount hanger installed per \_\_\_

detail 1p Filler block per

support the top flange, bearing stiffeners shall be used

Note: Unless hanger sides laterally

beams, see the manutacturer's

For nailing schedules for multiple

recommendations.

recommendations

support the top flange, bearing Note: Unless hanger sides laterally stiffeners shall be used manutacturer's recommendations

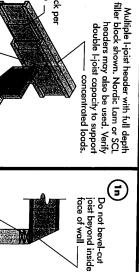
Backer block attached per —

recommendations

manufacturer's install hanger per

detail 1h. Nail with twelve 3" nails,

clinch when possible.

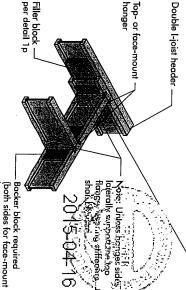


I-joist per detail 1b Note: Blocking required

support, not shown at bearing for lateral

tor clarity.

 $\bigcirc$ Use twelve 3" nails, clinched when possible. Maximum factored resistance for hanger for this detail = 1,620 lbs. backer block will fit. Clinch. Install backer tight to top flange. additional 3" nails through the webs and filler block where the Before installing a backer block to a double I-joist, drive three Backer block (use if hanger load exceeds 360 lbs)



For hanger capacity see hanger manufacturer's recommendations. Verify double I-joist capacity to support concentrated loads.

hangers)

BACKER BLOCKS (Blocks must be long enough to permit required nailing without splitting)

3-1/2"	2-1/2"	Flange Width
1-1/2"	1"	Material Thickness Required*
7-1/4"	5-1/2"	Minimum Depth**

- better for solid sawn lumber and wood structural panels conforming to CAN/CSA-O325 or CAN/CSA-O437 Standard. Minimum grade for backer block material shall be S-P-F No. 2 or
- For face-mount hangers use net joist depth minus 3-1/4" for joists with 1-1/2" thick flanges. For 2" thick flanges use net depth minus 4-1/4"



**(** 

Filler block

- 2. Leave a 1/8 to 1/4-inch gap between top of filler block and bottom of top I-joist prevent damage to web/flange connection.
- Filler block is required between joists for full length of span.
- 4. Nail joists together with two rows of 3" are required. can be clinched, only two nails per foot Total of four nails per foot required. If na possible) on each side of the double I-jo nails at 12 inches o.c. (clinched when
- -1/8" to 1/4" gap between top flange The maximum factored load that may be applied to one side of the double joist using this detail is 860 lbf/ft. Verify double

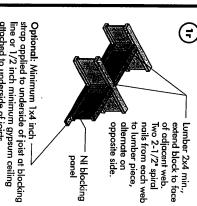
and filler block

opposite face by 6" Offset nails from

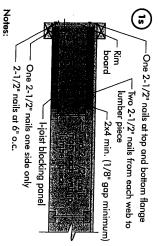
### DOUBLE I-JOIST CONSTRUCTION FILLER BLOCK REQUIREMENTS FOR

Maximum support capacity = 1,620 lbs.

ō	Flange Size	Joist Depth	Filler Block Size
	2-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	2-1/8" x 6" 2-1/8" x 8" 2-1/8" x 10" 2-1/8" x 12"
uis ist	3-1/2"× 1-1/2"	9-1/2" 11-7/8" 14" 16"	မ္ မ္ မ္ × × × × 12
	3-1/2"× 2"	11-7/8" 14" 16"	"11 × "8 "9 × "7 "7 × "8

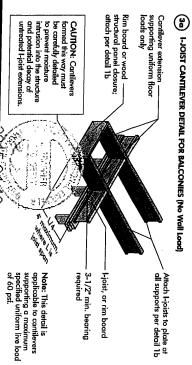


attached to underside of joists



- In some local codes, blocking is prescriptively required in the starter joist. Where required, see local code requirements the first joist space (or first and second joist space) next to for spacing of the blocking
- All nails are common spiral in this detail

## CANTILEVER DETAILS FOR BALCONIES (NO WALL LOAD)





Full depth backer block with 1/8" gap between block and top flange of i-joist. See detail 1 h. Nail with 2 rows of 3" nails at 6" o.c. and clinch.

2x8 min. Nail to backer block and joist with 2 rows of 3" nails at 6" o.c. and clinch. (Cantilever nails may be used to attach backer block if length of nail is sufficient to allow clinching.)

Attach I-joists to plate at all supports per detail 1b

Cantilever extension supporting uniform floor loads only

3-1/2" min. bearing required

### cantilevers supporting a maximum specified uniform live load of 60 psf. Note: This detail is applicable to Lumber or wood structural panel closure l-joist, or rim board

# CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

FIGURE 4 (continued) below for NI requirements at reinforcement See table

> Roof truss span

<u>ا</u>م maximum

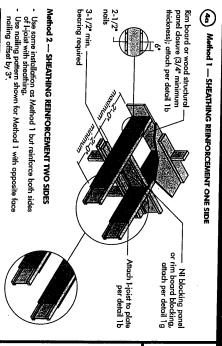
truss

Roof truss. span

Roof trusses

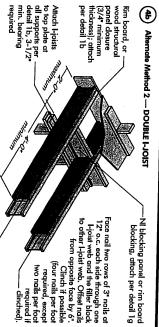
\_\_\_\_\_13'-0" maximum Jack trusses -2'-0" maximum cantilever

> the cantilevered floor joists, requirements for a span of 26 ft. shall be permitted to the I-joist reinforcement For hip roofs with the jack trusses running parallel to



Note: Canadian softwood plywood sheathing or equivalent (minimum thickness 3/4") required on sides of joist. Depth shall match the full height of the joist. Nail with 2-1/2" nails at 6" o.c., top and bottom flange. Install with face grain horizontal. Attach I-joist to plate at all supports per detail 1b. Verify reinforced I-joist capacity.

Alternate Method 2 — DOUBLE I-JOIST



N = No reinforcement required.
1 = NI reinforced with 3/4" wood structural

width window or door openings

Block Lipists together with filler blocks for the full length of the reinforcement. \rightarrow For Lipist flange widths greater than 3 inches place an additional row of 3\* nails along the centreline of the reinforcing panel from each side. Clinch when possible.

CANTILEVER REIN

CAMILLEVE	REINTO	RCEMENT MI	THODS ALLOWED	₩						
JOIST DEPTH (in.)	ROOF TRUSS SPAN (#)	LL = 30 psf JOIST SP 12 16	psf, DL = 15 psi SPACING (in.) 5 19.2	sf 24	ROOF LOADING LL = 40 psf, JOIST SPA 12 16	(UNFACTORE) DL = 15 psf .CING (in.) 19.2 24	ارة = ۲. ار=	= 50 psf, [ OIST SPAC	DL = 15 p CING (in.)	st st
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6	26 28 30	72.77 22.72	-zzz		ZZZZ = ZZZZ	2 2 2 2	zzzz	z×	3N2=X	<××××
	38			2	2.2		zz	<b>3</b> 1	۷,	×
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	336 386 386		ZZZZ	<u>دورد</u> دورو	7777 2222	2 - Z	zzzz	22	<u> </u>	*0001
	26 28		22	zz-		ZZ	zzz	z z -	2 2 2	<b>x</b>
	32 32 4	222.	ZZZZ	-222		*****	zzzz	zzzz	<b>-</b> Z	s N = 4 -
	40 42		ZZZ			N 22	zzz	-22	113	וממ×

For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c., additional joist beneath the opening's cripple studs may be required.

3. Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 pst and dead load of 15 psf, and a live load deflection limit of 1/480. Use 12" o.c. requirements for lesser spacing.

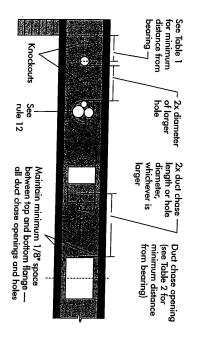
4. For conventional roof construction using a ridge beam, he floor fines Span column above is equivalent to the distance between the supporting wall and the ridge beam. When the root is fromed using a ridge board, the Roof Truss Span is equivalent to the distance between the root is fromed using a ridge board. distance between the supporting walls as if a

Cantilevered joists supporting girder trusses or roof beams may require additional

## RULES FOR CUTTING HOLES AND DUCT CHASE OPENINGS.

- The distance between the inside edge of the support and the centreline of any hole or duct chase opening shall be in compliance with the requirements of Table 1 or 2, respectively.
- L-joist top and bottom flanges must NEVER be cut, notched, or otherwise modified
- ω Whenever possible, field-cut holes should be centred on the middle of the web.
- 4. The maximum size hole or the maximum depth of a duct chase opening that can be cut into an I-joist web shall equal the clear distance between the flanges of between the top or bottom of the hole or opening and the adjacent I-joist flange. the I-joist minus 1/4 inch. A minimum of 1/8 inch should always be maintained
- Ċ The sides of square holes or longest sides of rectangular holes should not exceed 3/4 of the diameter of the maximum round hole permitted at that location.
- ٥ Where more than one hole is necessary, the distance between adjacent hole opening shall be sized and located in compliance with the requirements of Tables 1 and 2, respectively langest rectangular hole or duct chase opening) and each hole and duct chase size of the largest square hole (or twice the length of the longest side of the edges shall exceed twice the diameter of the largest round hole or twice the
- A knockout is **not** considered a hole, may be utilized anywhere it occurs, and may be ignored for purposes of calculating minimum distances between holes and/or duct chase openings.
- œ Holes measuring 1-1/2 inches or smaller shall be permitted anywhere in a cantilevered section of a joist. Holes of greater size may be permitted subject to
- % A 1-1/2 inch hole or smaller can be placed anywhere in the web provided that it meets the requirements of rule number 6 above.
- 11. Limit three maximum size holes per span, of which one may be a duct chase 70. All holes and duct chase openings shall be cut in a workman-like manner in accordance with the restrictions listed above and as illustrated in Figure 7.
- 12. A group of round holes at approximately the same location shall be permitted if they meet the requirements for a single round hole circumscribed around them.

### FIELD-CUT HOLE LOCATOR FIGURE 7



A knockout is NOT considered a hole, may be utilized wherever it occurs and may be ignored for purposes of calculating minimum distances

> electrical or small plumbing lines. They field-cut holes preferable to use knockouts instead of length of the I-joist. Where possible, it is spaced 15 inches on centre along the are 1-1/2 inches in diameter, and are for the contractor's convenience to install Knockouts are prescored holes provided **Never** drill, cut or notch the flange, or



should be cut with a Holes in webs over-cut the web

the corners, as this can cause unnecessary stress concentrations. Slightly rounding the holes is another good method to and then making the cuts between diameter hole in each of the four corners the rectangular hole by drilling a 1-inch the corners is recommended. for rectangular holes, avoid over-cutting Starting

ninimize damage to the I-joist

## LOCATION OF CIRCULAR HOLES IN JOIST WEBS

Simple or Multiple Span for Dead Loads up to 15 psf and Live Loads up to 40 psf

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( ) ( )		re-re-ce		Span
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- Laboration in the second or rejust spacing of 24 incres on centre or less.
  I fole location distance is measured from inside face of supports to centre of hole.
  Distances in this chart are based on uniformly loaded joists.

### OPTIONAL:

The above table is based on the I-joists used at their maximum span. If the I-joists are placed at less than their full maximum span (see Maximum free Spans). The minimum distance from the centreline of the hole to the face of any support (D) as given above may be reduced as follows:

Dreduced = Lactual × D SAF

Where: Dreduced = Distance from the inside face of any support to centre of hole, reduced for less-than-maximum span applications (fit. The jeptice) distance shall not be less than 6 inches from the face of the support to edge of the hole.

Ş actual. The actual measured span distance between the inside faces of supports (ff)

Span Adjustment Factor given in this table.

The minimum distance from the inside face of any support to centre of hole from this table H<u>ractual</u> is greater than 1, use 1 in the above calculation for <u>hadual</u> SAF

201990升

ABLE 2

## DUCT CHASE OPENING SIZES AND LOCATIONS — Simple Span Only

Joist Depth	Joist Series	Minim	ım distan	ice from i	nside fac Duct ct	e of any s ase leng	upport to th (in.)	centre a	f openin	g (ff-in.)
		8	10	12	14	16	18	20	22	24
			y.	6		5.8	0.1	6.6	7.11	7.5
		55.1	53	5 10	637		7.5	8.0	813	9 @ C
		100	5.0		77.75	10.0	7.0		8.5	8.6
						000		0 to 1	io I	0.9
						88		8	5	2
						8721	9.7	0		10.0
	i. Va		e G			2001	10.7	111.2	12.0	12-8
							8-018	60	12:17	12:3
			0.00		67			9114	12.4	12:11
f.				, Alak	9) p			k io o K	333	14-10
			111.2	110.8	2.0	12.61 2.10		36.	20	14.10
								The second secon	The second secon	

- . Above table may be used for Lioist spacing of 24 inches on centre or less.

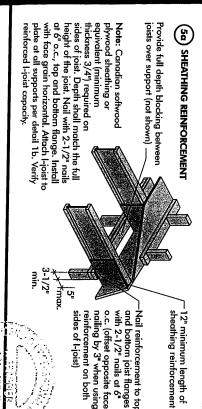
  Duct chase opening location distance is measured from inside face of supports to centre of opening.

  The above table is based on simple-span joists only. For other applications, contact your lacal distributor.

  Distances are based on uniformly loaded floor joists that meet the span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. For other applications, contact your local distributor.

# BRICK CANTILEVER DETAILS FOR VERTICAL BUILDING OFFSET (CONCENTRATED WALL LOAD)

FIGURE 5 (continued)



nailing by 3" when using o.c. (offset opposite face and bottom joist flanges with 2-1/2" nails at 6" Nail reinforcement to top reinforcement on both

> cantilever, requirements at reinforcement below for NI See table

### (not shown for clarity) - Attach I-joist to plate at all supports per detail 1b. - 3-1/2" minimum I-joist structural panel closure (3/4" minimum thickness), Provide full depth blocking between joists over support Bearing walls

attach per detail 1b.

Rim board or wood

Notes:

(F

SET-BACK DETAIL

### (5c) SET-BACK CONNECTION

bearing required.

girder joist per detail 5c. Attach joists to a ax

through joist web and web of girder using 2-1/2" nails. Alternate for opposite side. (2x6 S-P-F No. 2 or better) nailed Vertical solid sawn blocks —



Attach double I-joist per detail 1p, if required

bottom flanges. nails, toe-nail at top and Nail joist end using 3" Hanger may be

used in lieu of solid sawn blocks N = No reinforcement required.
 N = NI reinforced with 3/4" wood structural

2. Maximum design load shall be: 15 psf roof dead load, 55 psf floor total load, and 80 plf panel on both sides, or double I-joist. X = Try a deeper joist or closer spacing. panel on one side only.
2 = NI reinforced with 3/4" wood structural wall load. Wall load is based on 3'-0"

naximum width window or door openings.

additional joists beneath the opening's cripple For larger openings, or multiple 3'-0" width openings spaced less than 6'-0" o.c.,

studs may be required.
Table applies to joists 12" to 24" o.c. that meet the floor span requirements for a design live load of 40 psf and dead load of 15 psf, and a live load deflection limit of L/480. Use 12" o.c. requirements for lesser spacing.

7 2'-0" Lmaximum -5" maximum cantilever TUSS Girder -Roof trusses Roof truss span <del>\_\_\_\_ 13'\_0" maximum</del> cantilever Jack trusses 2<u>'</u>-0 5" maximum

Roof truss span

trusses running parallel to the cantilevered floor joists, be used 26 ft. shall be permitted to the I-joist reinforcement For hip roofs with the jack requirements for a span of

## BRICK CANTILEVER REINFORCEMENT METHODS ALLOWED

TSICIL	ROOF					ROOF LOA	DING (UN	(FACTO	Ê				
DEPTH (in.)	SPAN (ff)	or ='T1	30 psf, DIST SPA	DL = 15 CING (in.	psf )	LL = 4 JOIS	0 psf, DL =	= 15 psf G (in.)		Of =	= 50 psf, [ JOIST SPAC	DL = 15 p CING (in.)	ısf
	14		X	17. <i>2</i>	X	2	16 15	19.2	24	12	16	19.2	24
	28 30		××	<b>&lt;</b> ×	×	3131	<b>(X)</b>		× 2	×	××	××	××
	2 <b>ω</b>	S KS	×	(X	:×	(2) (2)			××	××	××	××	××
	36	2	××	X	××	×	×		××	××	××	××	××
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	38 40		×'n	××	××	3-			( <b>X</b> )	) N	:×>	××	××
	26 28	zz		2	××				X	Z	2	××	××
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6	34 3	zz	<b>5</b> 5	××	××				( <b>X</b> )		×>	××	××
	38 40	zz	901	<×>	<×>		(×) (×)		××	2-	××	××	××
	42	1	2	X	×	1.3			** 	2	××	××	××

- 4. For conventional roof construction using a When the roof is framed using a ridge board, distance between the supporting walls as if a the Roof Truss Span is equivalent to the ridge beam, the Roof Truss Span column truss is used. the supporting wall and the ridge beam. above is equivalent to the distance between
- Cantilevered joists supporting girder trusses or roof beams may require additional reinforcing.

## INSTALLING THE GLUED FLOOR SYSTEM

- 1. Wipe any mud, dirt, water, or ice from I-joist flanges before gluing.
- 2. Snap a chalk line across the I-joists four feet in from the wall for panel edge alignment and as a boundary for spreading glue.
- Spread only enough glue to lay one or two panels at a time, or follow specific recommendations from
- 4. Lay the first panel with tongue side to the wall, and nail in place. This protects the tongue of the next panel from damage when tapped into place with a block and sledgehammer.
- Apply a continuous line of glue (about 1/4-inch diameter) to the top flange of a single I-joist. Apply glue in a winding pattern on wide areas, such as with double I-joists.
- 6. Apply two lines of glue on I-joists where panel ends butt to assure proper gluing of each end.
- 7. After the first row of panels is in place, spread glue in the groove of one or two panels at a time a thinner line (1/8 inch) than used on I-joist flanges. before laying the next row. Glue line may be continuous or spaced, but avoid squeeze-out by applying

2-1/2" toe-nails at 6" o.c. (typical) —

٦

2X LEDGER TO RIM BOARD ATTACHMENT DETAIL

- 8. Tap the second row of panels into place, using a block to protect groove edges
- Stagger end joints in each succeeding row of panels. A 1/8-inch space between all end joints and 1/8-inch at all edges, including T&G edges, is recommended. (Use a spacer tool or an 2-1/2" common nail to assure accurate and consistent spacing.)
- 10. Complete all nailing of each panel before glue sets. Check the manufacturer's recommendations for cure time. (Warm weather accelerates glue setting.) Use 2" ring- or screw-shank nails for panels 3/4-inch thick or less, and 2-1/2" ring- or screw-shank nails for thicker panels. Space nails per the finished deck can be walked on right away and will carry construction loads without damage to the table below. Closer nail spacing may be required by some codes, or for diaphragm construction. The

## FASTENERS FOR SHEATHING AND SUBFLOORING(1)

×	23		Maxi Jo Spo (ii
4	0	8	aximum Joist pacing (in.)
3/4	5/8	5/8	Minimum Panel Thickness (in.)
			s C
22	2	2	Nice or Iral Nails
1-3/4*	1-3/4"	1-3/4"	ail Size and Ring Threa Nails or Screws
-	4	-	nd Typs read s s
2"	2	2"	Staples
6"	6"	6"	Maximur of Fas Edges
12"	12"	12"	m Spacing steners Interm, Supports

- 1. Fasteners of sheathing and subflooring shall conform to the above table.
- Staples shall not be less than 1/16-inch in diameter or thickness, with not less than a 3/8-inch crown driven with the crown parallel to framing.
- 3. Flooring screws shall not be less than 1/8-inch in diameter.
- 4. Special conditions may impose heavy traffic and concentrated loads that require construction in excess
- 5. Use only adhesives conforming to CAN/CGSB-71.26 Standard, Adhesives for Field-Gluing Plywood to Lumber Framing for Floor System, applied in accordance with the manufacturer's recommendations. If OSB panels with sealed surfaces and edges are to be used, use only solvent-based glues; check with panel manufacturer.

Ref.: NRC-CNRC, National Building Code of Canada 2010, Table 9.23.3.5

IMPORTANT NOTE:
Floor sheathing must be field glued to the I-joist flanges in order to achieve the maximum spans shown in this document. If sheathing is nailed only, I-joist spans must be verified with

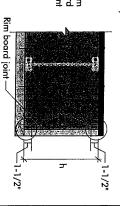
## RIM BOARD INSTALLATION DETAILS

## (8a) ATTACHMENT DETAILS WHERE RIM BOARDS ABUT

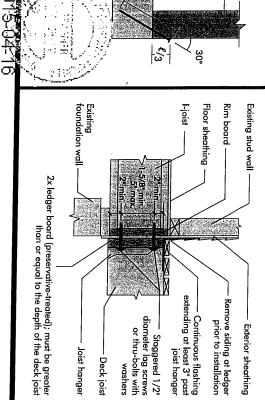
Rim board Joint Between Floor Joists

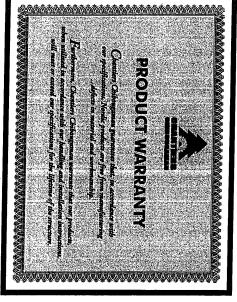
**Rim board Joint at Corner** 

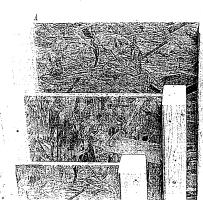




### æ Top or Rim board sole plate TOE-NAIL CONNECTION AT RIM BOARD







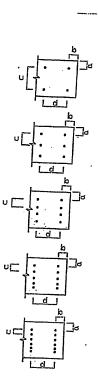
### MICRO CITY

### Engineering services inc.

TEL: (519) 287 - 2242

R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, NOL 1MO

	LVL HEADER AND CONVENTIONAL						
	LUM	OETAILS					
	DETAIL NUMBER	NUMBER OF ROWS	SPACING (INCHES o/c				
	. A	2.	12				
	В	2	8				
	С	2	6				
	D	2	4				
Manual F	1A	3	12				
F	1B	3	8				
	1C	3	. 6				
	1D	. 3:	4				
	2A	4	. 12				
	2B	4	8 .				
	2C	4	6				
	2D	4	4				
	3A	5	12				
1	3B	5	8				
ŀ	3C	5	6				
L	3D	5,	4				
L	4A	6	12				
Ľ	4B	6	8				
Ŀ	4C	6	6				
4D		6	4				



### NOTES:

- (1) MINIMUM LUMBER EDGE DISTANCE "a" = 1"
  - (2) MINIMUM LUMBER END DISTANCE "b" = 2"
  - (3) MINIMUM NAIL ROW SPACING "c" = 2"
  - (4) STAGGER NAILS "d/2" BETWEEN PLIES FOR MULTI-PLY MEMBERS (3 PLY OR MORE)
  - (5) ALL NAILS ARE 3-1/2" ARDOX SPIRAL NAILS
  - (6) DO NOT USE AIR-DRIVEN NAILS



DNG NO TANNICOI. 14
STRUCTURAL
COMPONENT ONLY
TO BE USED ONLY
WITH BEAM CALCS
BEARING THE
STAMP BELOWS

PROVICE NATLING
DETAIL № × SEE
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