



Job Track	45246
Layout ID:	290362
Plan Log:	96216

Builder / Location

GREENPARK HOMES / EAST GWILLIMBURY

PINEBROOK 2 / 1

Model / Elevation

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.
26

T - 180020

ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 O.C. WITH A VERT.POST TO THE TRF UNDERNEATH AT EACH CROSS PT VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

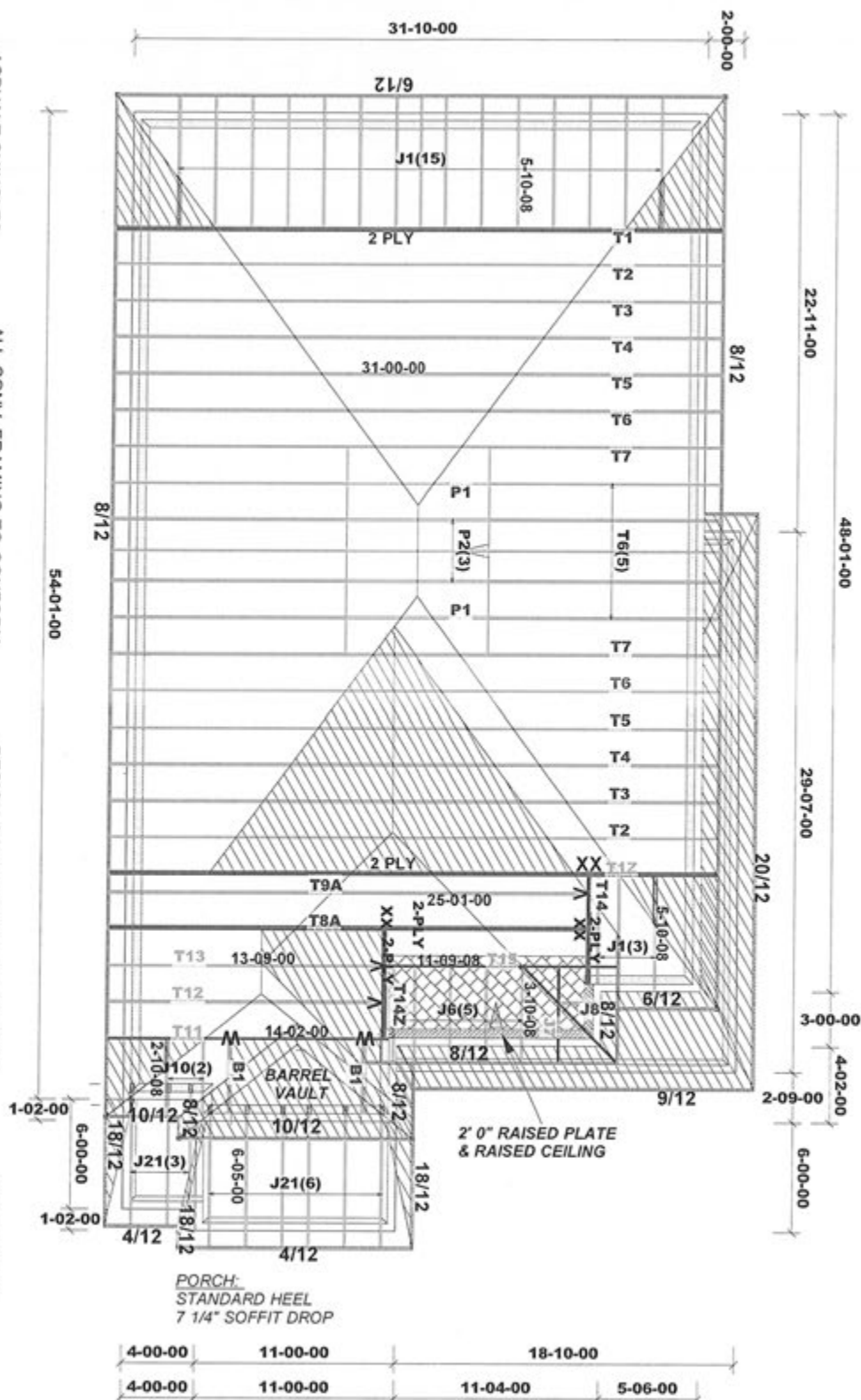
DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD $S_s=48$, 1psf $S_r=8$, 4psf
DESIGN LOADS

HARDWARE:
LJS26DS - (V)
LUS26-2 - (W)
HGUS26-2 - (XX)



DENOTES:
CONVENTIONAL
FRAMING

BEAMS:
B1: 2 - 2x10 SPF #2



PORCH:
STANDARD HEEL
7 1/4" SOFFIT DROP

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED OR REIDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 7.5.0

Minokver 7.5.0

THIS TRUSS DESIGNED FOR FLAT SOLAR PANEL ARRAY TO BE INSTALLED ALONG TOP CHORDS HAVING A COMBINED DEAD LOAD (SOLAR PANEL ARRAY) NOT EXCEEDING 5.0 PSF AND IN ACCORDANCE WITH CURRENT EDITION OF TPIC. THIS IS A LOAD READY DECLARATION ONLY AND NOT AN INSTALLATION READY DECLARATION. A SEPARATE INSTALLATION DETAIL IS REQUIRED BY LICENSED PROFESSIONAL ENGINEER ONCE A SOLAR ARRAY LAYOUT IS ESTABLISHED, AND AVAILABLE ON A ROOF BY ROOF BASIS.

M11,354

Job Track	45246	Builder / Location	GREENPARK HOMES / EAST GWILLIMBURY
Layout ID	290364	Model / Elevation	PINEBROOK 2/2
Plan Log	96216	Project	SECONDO VALES ESTATES INC
Date	1/18/2018	Designer	colin.vg

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Milek ver. 7.5.0

ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

T - 180020

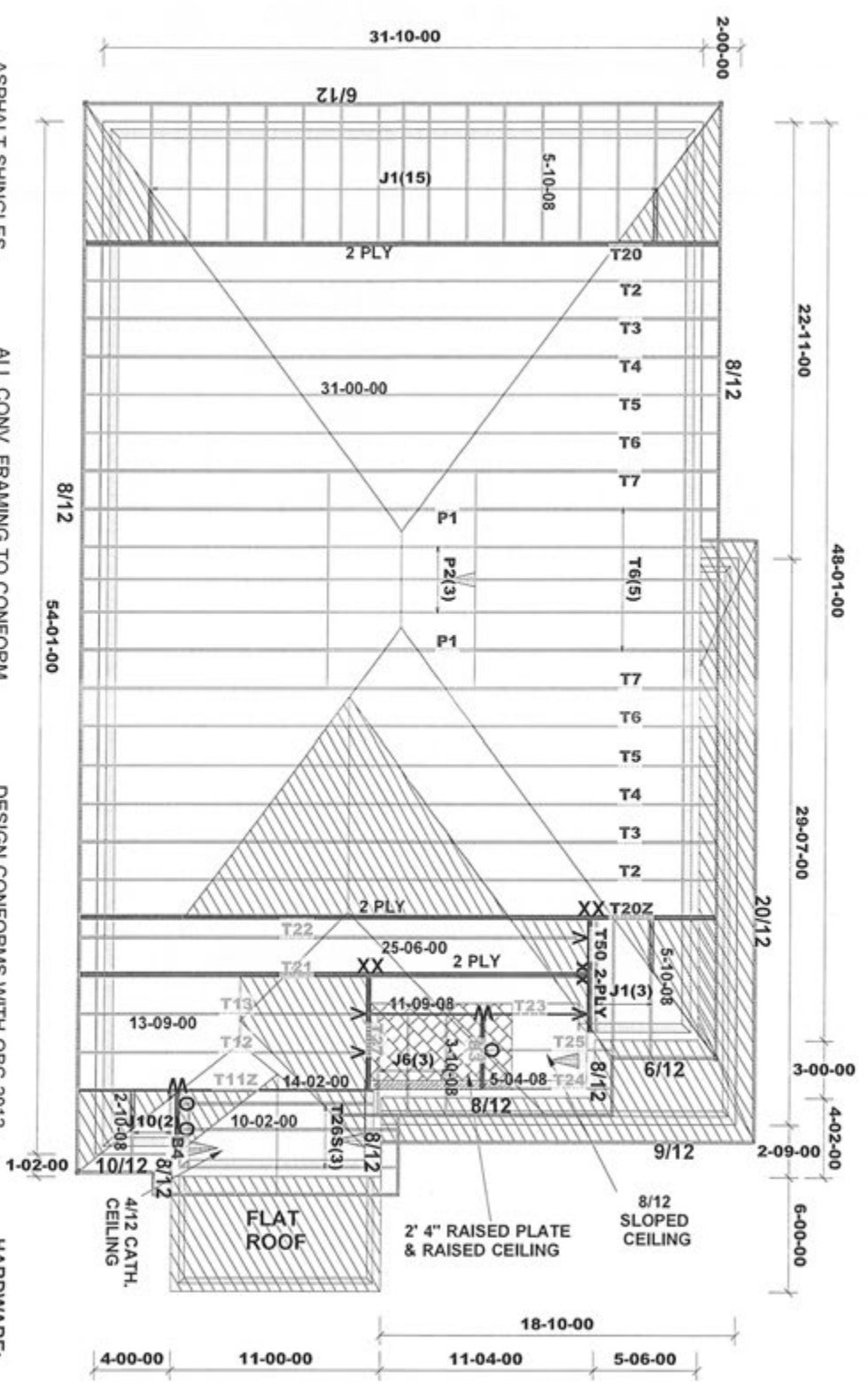
ALL CONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 24" O.C. WITH A VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012 OCCUPANCY: RESIDENTIAL | PART: 9 GROUND SNOW LOAD $S_s=48, 1\text{psf}$ $S_r=8, 4\text{psf}$ DESIGN LOADS
TCDL (8psf)
BCLL (10.5psf)
BCDL (7psf)

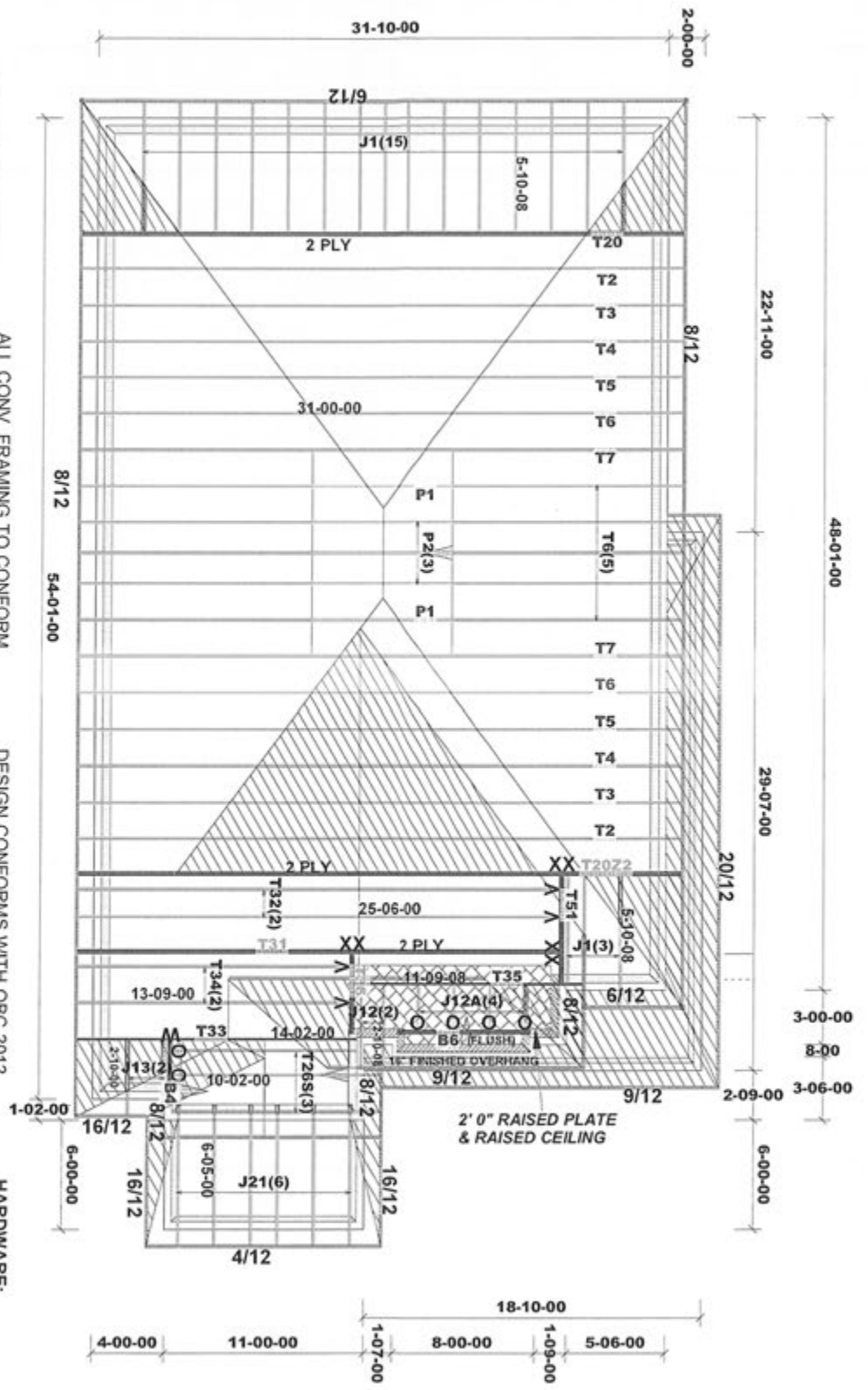
NOTES:
CONVENTIONAL FRAMING

HARDWARE:
LUS24 - (O)
LUS26DS - (V)
LUS26-2 - (W)
HGUS26-2 - (XX)

BEAMS:
B2, B3, B4: 2 - 2x10 SPF #2



M11,354



ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C. 

ALL ONV. FRAMING TO CONFORM WITH PART 9 OF O.B.C. 2012 ROOF RAFTERS THAT CROSS OVER OR MEET TRUSSES TO BE 2X4 SPF #2 2 O.C. WITH A VERT.POST TO THE TR UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DIST. BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL I PART: 9
GROUND SNOW LOAD $S_s=48$, 1psf $S_f=8$, 4psf
DESIGN LOADS
TCDL (8psf)
BCLL (10.5psf)
BCDL (7psf)

DENOTES:
CONVENTIONAL
FRAMING

BEAMS:
B4, B5, B6: 2 - 2x10 SPF #2

GREENPARK HOMES / EAST GWILLIMBURY

PINEBROOK 2 / 3

TAMARACK A DIVISION OF LAYCO	
Job Track: 45246	
Layout ID: 290366	
Plan Log: 96216	
Builder / Location:	
GREENPARK HOMES / EAST GWILLIMBURY	
Model / Elevation:	
PINEBROOK 2 / 3	
Project: SECONDO VALES ESTATES INC.	
Date: 1/18/2018 Designer: colin.vg	
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC.. SHALL NOT BE REPRODUCED, PUBLISHED, FOR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
Mitek ver 7.5.0	

Miltek ver 7.5.0



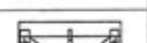
Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 290362 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALES ESTA SUB-BUILDER:
 MODEL: PINEBROOK 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	1 2 Ply	T1 HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.44 185.34		
	1 2 Ply	T1Z HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	299.44 185.34		
	2	T2 HIP	8.00 0.00	31-00-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	262.42 165.00		
	2	T3 HIP	8.00 0.00	31-00-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	263.78 165.34		
	2	T4 HIP	8.00 0.00	31-00-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	259.64 163.66		
	2	T5 HIP	8.00 0.00	31-00-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	279.92 175.34		
	7	T6 PIGGYBACK	8.00 0.00	31-00-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	979.58 611.38		
	2	T7 HIP	8.00 0.00	31-00-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	297.40 185.34		
	1 2 Ply	T8A HIP GIRDER	8.00 0.00	25-01-00	07-03-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 03-08-02	263.28 163.34		
	1	T9A HIP	8.00 0.00	25-01-00	08-07-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-08-02	119.99 76.00		
	1	T11 HIP GIRDER	8.00 0.00	14-02-00	04-00-07	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	71.51 47.00		
	1	T12 HIP	8.00 0.00	13-09-00	05-08-07	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	65.10 43.17		
	1	T13 COMMON	8.00 0.00	13-09-00	06-01-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	58.38 37.67		
	1 2 Ply	T14 FLAT GIRDER	0.00 0.00	05-10-08	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	57.80 37.34		
	1 2 Ply	T14Z FLAT GIRDER	0.00 0.00	05-10-08	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	57.80 37.34		
	1	T15 HALF HIP	8.00 0.00	11-09-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	52.08 33.67		
	2	P1 PIGGYBACK	8.00 0.00	06-04-13	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	39.56 26.66		
	3	P2 PIGGYBACK	8.00 0.00	06-04-13	02-06-01	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	58.89 36.99		



Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK: 45246 LAYOUT ID: 290362 LOCATION: EAST GWILLIMBURY
 BUILDER: GREENPARK / SECONDO VALES ESTA SUB-BUILDER:
 MODEL: PINEBROOK 2 ELEVATION: 1

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	18	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	305.10 192.06		
	5	J6 JACK-OPEN	8.00 0.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-13	70.85 45.00		
	1	J7 JACK-OPEN	8.00 0.00	03-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -02-01-01	01-04-13 00-03-08	9.95 6.00		
	1	J8 JACK-OPEN	8.00 0.00	01-10-08	02-07-02	2 X 4	2 X 4	01-03-08 -00-01-01	01-04-13 00-03-08	7.67 4.67		
	2	J10 JACK-OPEN	10.00 0.00	02-10-08	04-00-07	2 X 4	2 X 4	01-03-08 00-00-00	01-07-11 04-00-07	25.20 16.66		
	9	J21 JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	169.65 109.53		

TOTAL # TRUSS= 73.00

TOTAL BFT OF ALL TRUSSES=

2749.84 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4374.43 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
3	Hangers	LJS26DS	
2	Hangers	LUS26-2	

TOTAL # ITEMS= 8.00



Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK: 45246	LAYOUT ID: 290364	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALES ESTA	SUB-BUILDER:	
MODEL: PINEBROOK 2	ELEVATION: 2	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	2	T2 HIP	8.00 0.00	31-00-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	262.42 165.00		
	2	T3 HIP	8.00 0.00	31-00-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	263.78 165.34		
	2	T4 HIP	8.00 0.00	31-00-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	259.64 163.66		
	2	T5 HIP	8.00 0.00	31-00-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	279.92 175.34		
	7	T6 PIGGYBACK	8.00 0.00	31-00-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	979.58 611.38		
	2	T7 HIP	8.00 0.00	31-00-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	297.40 185.34		
	1	T11Z HIP GIRDER	8.00 0.00	14-02-00	04-00-07	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	71.51 47.00		
	1	T12 HIP	8.00 0.00	13-09-00	05-08-07	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	65.10 43.17		
	1	T13 COMMON	8.00 0.00	13-09-00	06-01-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	58.38 37.67		
	1 2 Ply	T20 HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	300.08 185.34		
	1 2 Ply	T20Z HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	300.08 185.34		
	1 2 Ply	T21 HIP GIRDER	8.00 0.00	25-06-00	07-07-13	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	287.12 182.66		
	1	T22 HIP	8.00 0.00	25-06-00	08-11-13	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	118.65 74.50		
	1	T23 HALF HIP	8.00 8.00	11-09-08	06-03-13	2 X 4	2 X 6	00-00-00 00-00-00	01-04-13 03-11-13	57.23 38.17		
	1	T24 HALF HIP	8.00 8.00	05-04-08	03-08-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-04-13	23.54 16.00		
	1	T25 MONOPITCH	8.00 8.00	05-04-08	04-11-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 02-07-13	26.12 18.67		
	3	T26S SCISSOR	8.00 4.00	10-02-00	04-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.07 83.49		
	1 2 Ply	T27 FLAT GIRDER	0.00 0.00	05-10-08	02-04-00	2 X 6	2 X 6	00-00-00 00-00-00	02-04-00 02-04-00	60.36 37.34		



Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK: 45246	LAYOUT ID: 290364	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALES ESTA	SUB-BUILDER:	
MODEL: PINEBROOK 2	ELEVATION: 2	

ROOF TRUSSES

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	1	T50 JACK-CLOSED	6.00	05-10-08	04-01-04	2 X 4	2 X 6	00-00-00	01-02-00	57.62		
	2 Ply		0.00					00-00-00	04-01-04	37.00		
	2	P1 PIGGYBACK	8.00	06-04-13	01-10-08	2 X 4	2 X 4	00-00-00	00-04-07	42.74		
			0.00					00-00-00	00-04-07	28.66		
	3	P2 PIGGYBACK	8.00	06-04-13	02-06-01	2 X 4	2 X 4	00-00-00	00-04-07	58.89		
			0.00					00-00-00	00-04-07	36.99		
	18	J1 JACK-OPEN	6.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00	01-02-00	305.10		
			0.00					00-00-00	04-01-04	192.06		
	3	J6 JACK-OPEN	8.00	03-10-08	03-11-13	2 X 4	2 X 4	01-03-08	01-04-13	42.51		
			0.00					00-00-00	03-11-13	27.00		
	2	J10 JACK-OPEN	10.00	02-10-08	04-00-07	2 X 4	2 X 4	01-03-08	01-07-11	25.20		
			0.00					00-00-00	04-00-07	16.66		

TOTAL # TRUSS= 65.00

TOTAL BFT OF ALL TRUSSES=

2753.78 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4374.04 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
2	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 10.00



Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK:45246	LAYOUT ID: 290366	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALES ESTA	SUB-BUILDER:	
MODEL: PINEBROOK 2	ELEVATION: 3	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY PLY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
						TOP	BOT					
	2	T2 HIP	8.00 0.00	31-00-00	05-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	262.42 165.00		
	2	T3 HIP	8.00 0.00	31-00-00	06-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	263.78 165.34		
	2	T4 HIP	8.00 0.00	31-00-00	07-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	259.64 163.66		
	2	T5 HIP	8.00 0.00	31-00-00	08-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	279.92 175.34		
	7	T6 PIGGYBACK	8.00 0.00	31-00-00	09-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	979.58 611.38		
	2	T7 HIP	8.00 0.00	31-00-00	10-01-04	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	297.40 185.34		
	1 2 Ply	T20 HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	300.08 185.34		
	1 2 Ply	T20Z2 HIP GIRDER	8.00 0.00	31-00-00	04-01-04	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	300.08 185.34		
	3	T26S SCISSOR	8.00 4.00	10-02-00	04-09-08	2 X 4	2 X 4	01-03-08 01-03-08	01-04-13 01-04-13	131.07 83.49		
	1 2 Ply	T31 COMMON	8.00 0.00	25-06-00	10-10-13	2 X 4	2 X 6	01-03-08 00-00-00	01-04-13 03-04-13	278.24 168.66		
	2	T32 COMMON	8.00 0.00	25-06-00	10-10-13	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-04-13	242.54 149.66		
	1	T33 COMMON	8.00 0.00	14-02-00	06-01-08	2 X 4	2 X 6	01-03-08 01-03-08	01-04-13 01-04-13	73.88 48.67		
	2	T34 COMMON	8.00 0.00	13-09-00	06-01-08	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 01-08-02	116.76 75.34		
	1	T35 HALF HIP	8.00 0.00	11-09-08	03-11-02	2 X 4	2 X 4	01-03-08 00-00-00	01-04-13 03-11-02	51.11 33.00		
	1 2 Ply	T36 FLAT GIRDER	0.00 0.00	04-05-00	02-00-00	2 X 6	2 X 6	00-00-00 00-00-00	02-00-00 02-00-00	43.08 29.34		
	1 2 Ply	T51 FLAT GIRDER	0.00 0.00	05-10-08	03-02-13	2 X 6	2 X 6	00-00-00 00-00-00	03-02-13 03-02-13	67.66 43.00		
	2	P1 PIGGYBACK	8.00 0.00	06-04-13	01-10-08	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	42.74 28.66		
	3	P2 PIGGYBACK	8.00 0.00	06-04-13	02-06-01	2 X 4	2 X 4	00-00-00 00-00-00	00-04-07 00-04-07	58.89 36.99		



Delivery Shiplist

DATE	01/12/18
SALES REP	Mario

JOB TRACK:45246	LAYOUT ID: 290366	LOCATION: EAST GWILLIMBURY
BUILDER: GREENPARK / SECONDO VALES ESTA	SUB-BUILDER:	
MODEL: PINEBROOK 2	ELEVATION: 3	

ROOF TRUSSES

ROOF TRUSS SPACING:24.0 IN. O.C. (TYP.)

PROFILE	QTY	MARK TYPE	PITCH TC BC	SPAN	TRUSS HEIGHT	LUMBER		OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY: REMARKS
	PLY					TOP	BOT					
	18	J1 JACK-OPEN	6.00 0.00	05-10-08	04-01-04	2 X 4	2 X 4	01-05-00 00-00-00	01-02-00 04-01-04	305.10 192.06		
	2	J12 JACK-OPEN	9.00 0.00	02-10-08	03-11-02	2 X 4	2 X 4	01-07-08 00-00-00	01-09-04 03-11-02	26.00 16.66		
	4	J12A JACK-OPEN	9.00 0.00	02-05-08	03-11-02	2 X 4	2 X 4	02-00-08 00-00-00	02-01-00 03-11-02	51.56 33.32		
	2	J13 JACK-OPEN	16.00 0.00	02-10-00	06-01-08	2 X 4	2 X 4	01-03-08 00-00-00	02-04-02 06-01-08	31.42 20.34		
	6	J21 JACK-OPEN	4.00 0.00	06-05-00	02-05-10	2 X 4	2 X 4	01-03-08 00-00-00	00-03-15 02-05-10	113.10 73.02		

TOTAL # TRUSS= 73.00

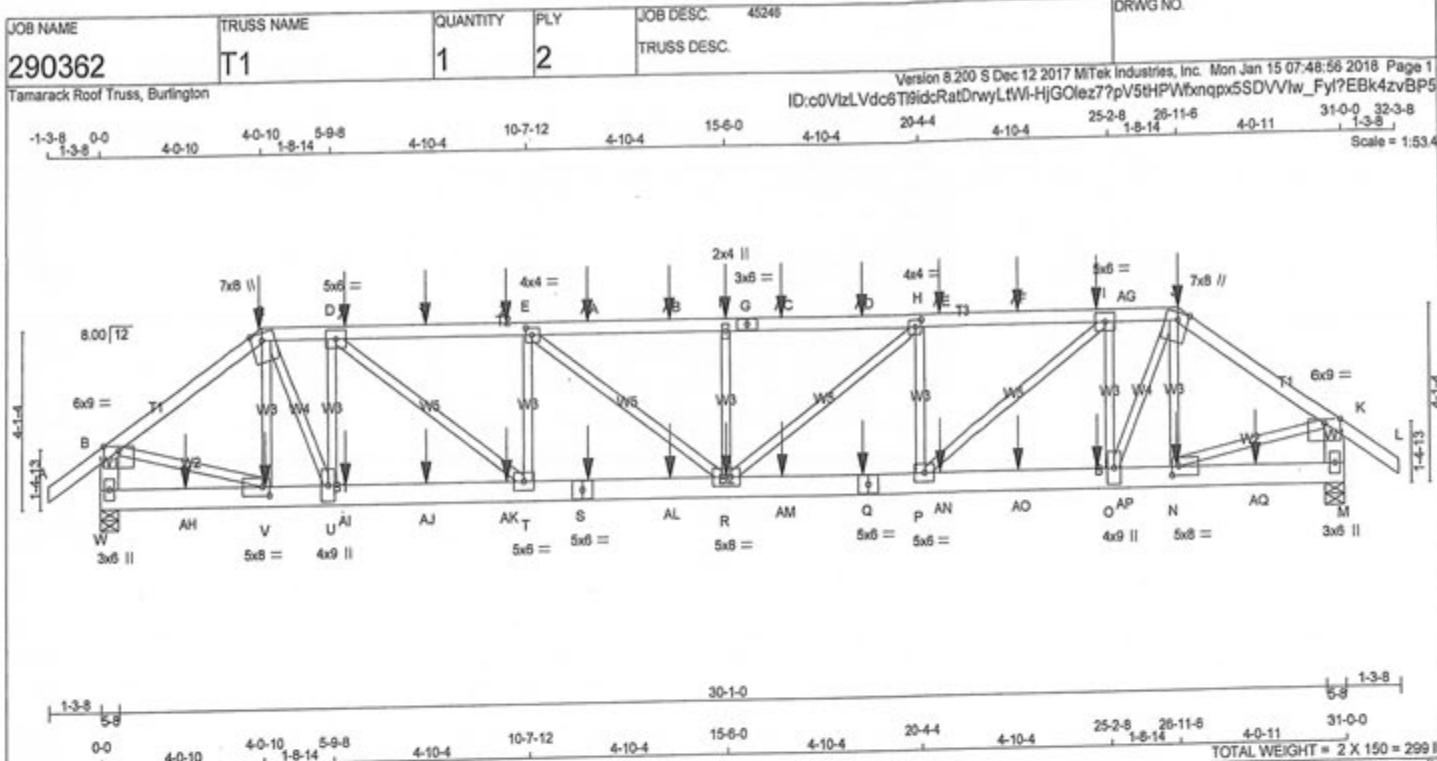
TOTAL BFT OF ALL TRUSSES=

2868.95 BFT. TOTAL WEIGHT OF ALL TRUSSES= 4576.05 LBS.

HARDWARE

QTY	ITEM TYPE	MODEL	LENGTH FT-IN-16
3	Hangers	HGUS26-2	
4	Hangers	LJS26DS	
6	Hangers	LUS24	
1	Hangers	LUS26-2	

TOTAL # ITEMS= 14.00



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - G	2x4	DRY	No.2 SPF
G - J	2x4	DRY	No.2 SPF
J - L	2x4	DRY	No.2 SPF
W - B	2x6	DRY	No.2 SPF
M - K	2x6	DRY	No.2 SPF
S - Q	2x6	DRY	No.2 SPF
Q - M	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (61.0)
C - G	12	SIDE (61.0)
G - J	12	SIDE (61.0)
J - L	12	SIDE (61.0)
W - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
W - S	12	SIDE (183.1)
S - Q	12	SIDE (183.1)
Q - M	12	SIDE (183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
V - C	6	SIDE (20.3)
N - J	6	SIDE (20.3)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	2.00	3.25
D	TMWV-i	MT20	5.0	6.0		
E	TMWV-i	MT20	4.0	4.0	2.00	1.75
F	TMWV-w	MT20	2.0	4.0		
G	TS-i	MT20	3.0	6.0		
H	TMWV-i	MT20	4.0	4.0	2.00	1.75
I	TMWV-i	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	2.00	3.25
K	TMWV-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWV-i	MT20	5.0	8.0	2.50	2.00
O	BMWV-i	MT20	4.0	9.0		
P	BMWV-i	MT20	5.0	6.0		
Q	BS-4	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
W	4201	0	4201	0	5-8	5-8
M	4201	0	4201	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
W	3304	1934 / 0	550 / 0	0 / 0	0 / 0	819 / 0	0 / 0
M	3304	1934 / 0	550 / 0	0 / 0	0 / 0	819 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO			FR-TO		
A-B	0 / 47	-124.4	-124.4	0.09 (1)	10.00	V-C	-792 / 0	0.10 (1)
B-C	-4912 / 0	-124.4	-124.4	0.31 (1)	4.08	C-U	0 / 3003	0.37 (1)
C-D	-5328 / 0	-124.4	-124.4	0.29 (1)	3.93	U-D	-2595 / 0	0.33 (1)
D-X	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02	D-T	0 / 3087	0.38 (1)
X-Y	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02	T-E	-1634 / 0	0.21 (1)
Y-Z	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02	E-R	0 / 1026	0.13 (1)
Z-E	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02	R-F	-1031 / 0	0.13 (1)
E-AA	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	R-H	0 / 1026	0.13 (1)
AA-AB	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	P-H	-1634 / 0	0.21 (1)
AB-F	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	P-I	0 / 3087	0.38 (1)
F-G	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	O-I	-2595 / 0	0.33 (1)
G-AC	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	O-J	0 / 3003	0.37 (1)
AC-AD	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	N-J	-792 / 0	0.10 (1)
AD-H	-8591 / 0	-124.4	-124.4	0.70 (1)	2.81	B-V	0 / 4195	0.52 (1)
H-AE	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02	N-K	0 / 4195	0.52 (1)
AE-AF	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02			
AF-AG	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02			
AG-I	-7778 / 0	-124.4	-124.4	0.63 (1)	3.02			
I-J	-5328 / 0	-124.4	-124.4	0.29 (1)	3.93			
J-K	-4912 / 0	-124.4	-124.4	0.31 (1)	4.08			
K-L	0 / 47	-124.4	-124.4	0.09 (1)	10.00			
W-B	-4135 / 0	0.0	0.0	0.15 (1)	7.01			
M-K	-4135 / 0	0.0	0.0	0.15 (1)	7.01			

MADE PROFESSIONAL

1588

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
CALCULATED VERT. DEFL. (LL) = L/999 (0.24")
ALLOWABLE DEFL. (TL) = L/360 (1.03")
CALCULATED VERT. DEFL. (TL) = L/921 (0.40")

CS: TC=0.70/1.00 (E-F:1), BC=0.58/1.00 (P-R:1),
WB=0.52/1.00 (K-N:1), SS=0.23/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.65 (Q) (INPUT = 1.00)



DWG NO. TAM 3617-18
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-1	MT20	5.0	8.0		
S	BS-1	MT20	5.0	6.0		
T	BMWW-1	MT20	5.0	6.0		
U	BMWW-1	MT20	4.0	9.0		
V	BMWW-1	MT20	5.0	8.0	2.50	2.00
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 545.6 lbs FACTORED DOWN AT 28-11-5, 545.6 lbs FACTORED DOWN AT 4-0-11, 149.7 lbs FACTORED DOWN AT 6-1-6, 149.7 lbs FACTORED DOWN AT 8-1-6, 149.7 lbs FACTORED DOWN AT 10-1-6, 149.7 lbs FACTORED DOWN AT 12-1-6, 149.7 lbs FACTORED DOWN AT 14-1-6, 149.7 lbs FACTORED DOWN AT 15-8-0, 149.7 lbs FACTORED DOWN AT 18-10-10, 149.7 lbs FACTORED DOWN AT 18-10-10, 149.7 lbs FACTORED DOWN AT 20-10-10, AND 149.7 lbs FACTORED DOWN AT 22-10-10, AND 149.7 lbs FACTORED DOWN AT 24-10-10 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, 69.9 lbs FACTORED DOWN AT 6-1-6, 69.9 lbs FACTORED DOWN AT 8-1-6, 69.9 lbs FACTORED DOWN AT 10-1-6, 69.9 lbs FACTORED DOWN AT 12-1-6, 69.9 lbs FACTORED DOWN AT 14-1-6, 69.9 lbs FACTORED DOWN AT 15-8-0, 69.9 lbs FACTORED DOWN AT 18-10-10, 69.9 lbs FACTORED DOWN AT 18-10-10, 69.9 lbs FACTORED DOWN AT 20-10-10, 69.9 lbs FACTORED DOWN AT 22-10-10, 69.9 lbs FACTORED DOWN AT 24-10-10, AND 69.9 lbs FACTORED DOWN AT 26-10-10, AND 69.9 lbs FACTORED DOWN AT 28-10-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 (LBS)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LBS)
FR-TO		FROM	TO				
AP-O	0/5330	-28.0	-28.0	0.41 (1)	10.00		
O-N	0/4058	-28.0	-28.0	0.28 (1)	10.00		
N-AQ	0/0	-28.0	-28.0	0.05 (3)	10.00		
AQ-M	0/0	-28.0	-28.0	0.05 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX*	FACE	DIR.	TYPE
C	4-0-11	-63	-70	---	FRONT	VERT	DEAD
C	4-0-11	-159	-159	---	FRONT	VERT	TOTAL
C	4-0-11	-324	-324	---	FRONT	VERT	SNOW
F	15-8-0	-150	-150	---	FRONT	VERT	TOTAL
J	26-11-5	-63	-70	---	FRONT	VERT	DEAD
J	26-11-5	-159	-159	---	FRONT	VERT	TOTAL
J	26-11-5	-324	-324	---	FRONT	VERT	SNOW
N	26-10-10	-40	-70	---	FRONT	VERT	TOTAL
Q	18-10-10	-40	-70	---	FRONT	VERT	TOTAL
R	15-6-0	-40	-70	---	FRONT	VERT	TOTAL
S	12-1-6	-40	-70	---	FRONT	VERT	TOTAL
V	4-1-6	-40	-70	---	FRONT	VERT	TOTAL
X	6-1-6	-150	-150	---	FRONT	VERT	TOTAL
Y	8-1-6	-150	-150	---	FRONT	VERT	TOTAL
Z	10-1-6	-150	-150	---	FRONT	VERT	TOTAL
AA	12-1-6	-150	-150	---	FRONT	VERT	TOTAL
AB	14-1-6	-150	-150	---	FRONT	VERT	TOTAL
AC	16-10-10	-150	-150	---	FRONT	VERT	TOTAL
AD	18-10-10	-150	-150	---	FRONT	VERT	TOTAL
AE	20-10-10	-150	-150	---	FRONT	VERT	TOTAL
AF	22-10-10	-150	-150	---	FRONT	VERT	TOTAL
AG	24-10-10	-150	-150	---	FRONT	VERT	TOTAL
AH	2-1-6	-40	-70	---	FRONT	VERT	TOTAL
AI	6-1-6	-40	-70	---	FRONT	VERT	TOTAL
AJ	8-1-6	-40	-70	---	FRONT	VERT	TOTAL
AK	10-1-6	-40	-70	---	FRONT	VERT	TOTAL
AL	14-1-6	-40	-70	---	FRONT	VERT	TOTAL
AM	16-10-10	-40	-70	---	FRONT	VERT	TOTAL
AN	20-10-10	-40	-70	---	FRONT	VERT	TOTAL
AO	22-10-10	-40	-70	---	FRONT	VERT	TOTAL
AP	24-10-10	-40	-70	---	FRONT	VERT	TOTAL
AQ	28-10-10	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 3617-18
 STRUCTURAL COMPONENT ONLY

ID:c0VizLVdc6Ti9IdcRatDrwyLrWi-lvqny__mm6dyVQ_Del3L9eehvq9fNr6_fziRWzV8

Scale = 1:53.4



CONTINUED ON PAGE 2



DWG NO. TAM 3610-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-1	MT20	5.0	8.0		
S	BS-1	MT20	5.0	6.0		
T	BMWW-1	MT20	5.0	6.0		
U	BMWW-1	MT20	4.0	9.0		
V	BMWW-1	MT20	5.0	8.0	2.50	2.00
W	BMV1+p	MT20	3.0	6.0		

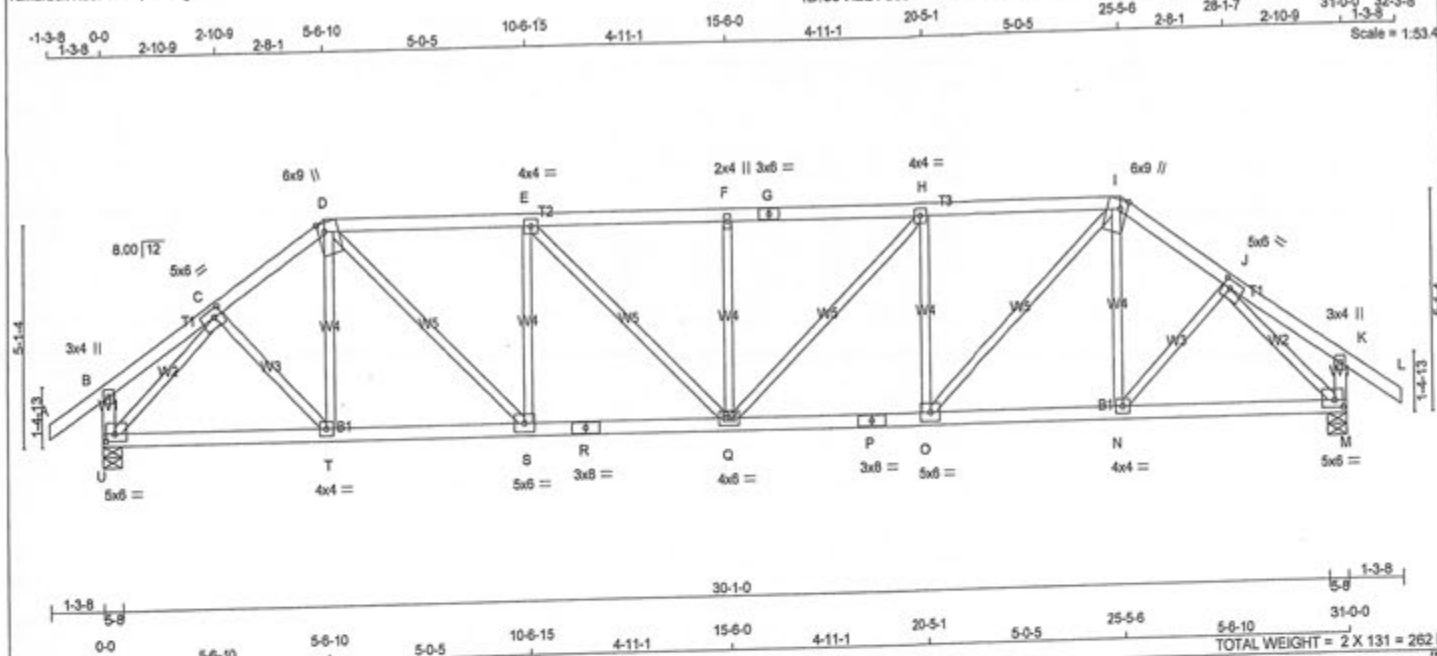
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 386.5 lbs FACTORED DOWN AT 4-0-11, AND 159.1 lbs FACTORED DOWN AT 4-0-11, AND 149.7 lbs FACTORED DOWN AT 5-3-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, AND 69.9 lbs FACTORED DOWN AT 5-3-4, AND 3796.3 lbs FACTORED DOWN AT 5-9-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 3618 -18
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.25	2.25
D	TTVW+m	MT20	6.0	9.0	Edge	2.00
E	TMVW-1	MT20	4.0	4.0		
F	TMVW+m	MT20	2.0	4.0		
G	TS-1	MT20	3.0	6.0		
H	TMVW-1	MT20	4.0	4.0		
I	TTVW+m	MT20	6.0	9.0	Edge	2.00
J	TMVW-1	MT20	5.0	6.0	2.25	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW-1	MT20	5.0	6.0	2.00	2.50
N	BMVW-1	MT20	4.0	4.0		
O	BMVW-1	MT20	5.0	6.0		
P	BS-1	MT20	3.0	8.0		
Q	BMVW-1	MT20	4.0	6.0		
R	BS-1	MT20	3.0	8.0		
S	BMVW-1	MT20	5.0	6.0		
T	BMVW-1	MT20	4.0	4.0		
U	BMVW-1	MT20	5.0	6.0	2.00	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORIZ	DOWN	HORIZ		
U	2533	0	2533	0	5-8	5-8
M	2533	0	2533	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN SNOW		LIVE	PERM LIVE	WIND	DEAD	SOIL
	VERT	HORIZ	DOWN	UP					
U	1987	1174 / 0	326 / 0	0 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0
M	1987	1174 / 0	326 / 0	0 / 0	0 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO							
A-B	0 / 47	-124.4 -124.4	0.17 (1)	10.00	C-T	0 / 323	0.07 (1)		
B-C	0 / 16	-124.4 -124.4	0.12 (1)	10.00	T-D	-58 / 131	0.03 (3)		
C-D	-2744 / 0	-124.4 -124.4	0.21 (1)	4.02	D-S	0 / 1700	0.38 (1)		
D-E	-3492 / 0	-124.4 -124.4	0.69 (1)	3.10	S-E	-1046 / 0	0.40 (1)		
E-F	-3852 / 0	-124.4 -124.4	0.74 (1)	2.91	E-Q	0 / 502	0.11 (1)		
F-G	-3852 / 0	-124.4 -124.4	0.74 (1)	2.91	Q-F	-564 / 0	0.22 (1)		
G-H	-3852 / 0	-124.4 -124.4	0.74 (1)	2.91	H-Q	0 / 502	0.11 (1)		
H-I	-3492 / 0	-124.4 -124.4	0.69 (1)	3.10	Q-H	-1046 / 0	0.40 (1)		
I-J	-2744 / 0	-124.4 -124.4	0.21 (1)	4.02	O-I	0 / 1700	0.38 (1)		
J-K	0 / 16	-124.4 -124.4	0.12 (1)	10.00	N-I	-58 / 131	0.03 (3)		
K-L	0 / 47	-124.4 -124.4	0.17 (1)	10.00	N-J	0 / 323	0.07 (1)		
U-B	-314 / 0	0.0 0.0	0.03 (1)	7.81	U-C	-2964 / 0	0.79 (1)		
M-K	-314 / 0	0.0 0.0	0.03 (1)	7.81	J-M	-2964 / 0	0.79 (1)		
U-T	0 / 2037	-28.0 -28.0	0.44 (1)	10.00					
T-S	0 / 2263	-28.0 -28.0	0.47 (1)	10.00					
S-R	0 / 3493	-28.0 -28.0	0.63 (1)	10.00					
R-Q	0 / 3493	-28.0 -28.0	0.63 (1)	10.00					
Q-P	0 / 3493	-28.0 -28.0	0.63 (1)	10.00					
P-O	0 / 3493	-28.0 -28.0	0.63 (1)	10.00					
O-N	0 / 2263	-28.0 -28.0	0.47 (1)	10.00					
N-M	0 / 2037	-28.0 -28.0	0.44 (1)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN.C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.03")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.21")
 ALLOWABLE DEFL.(TL) = $L/360$ (1.03")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.34")

CSI: TC=0.74/1.00 (F-H-1), BC=0.63/1.00 (O-Q-1),
 WB=0.79/1.00 (J-M-1), SS=0.29/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667 822 2284 1656

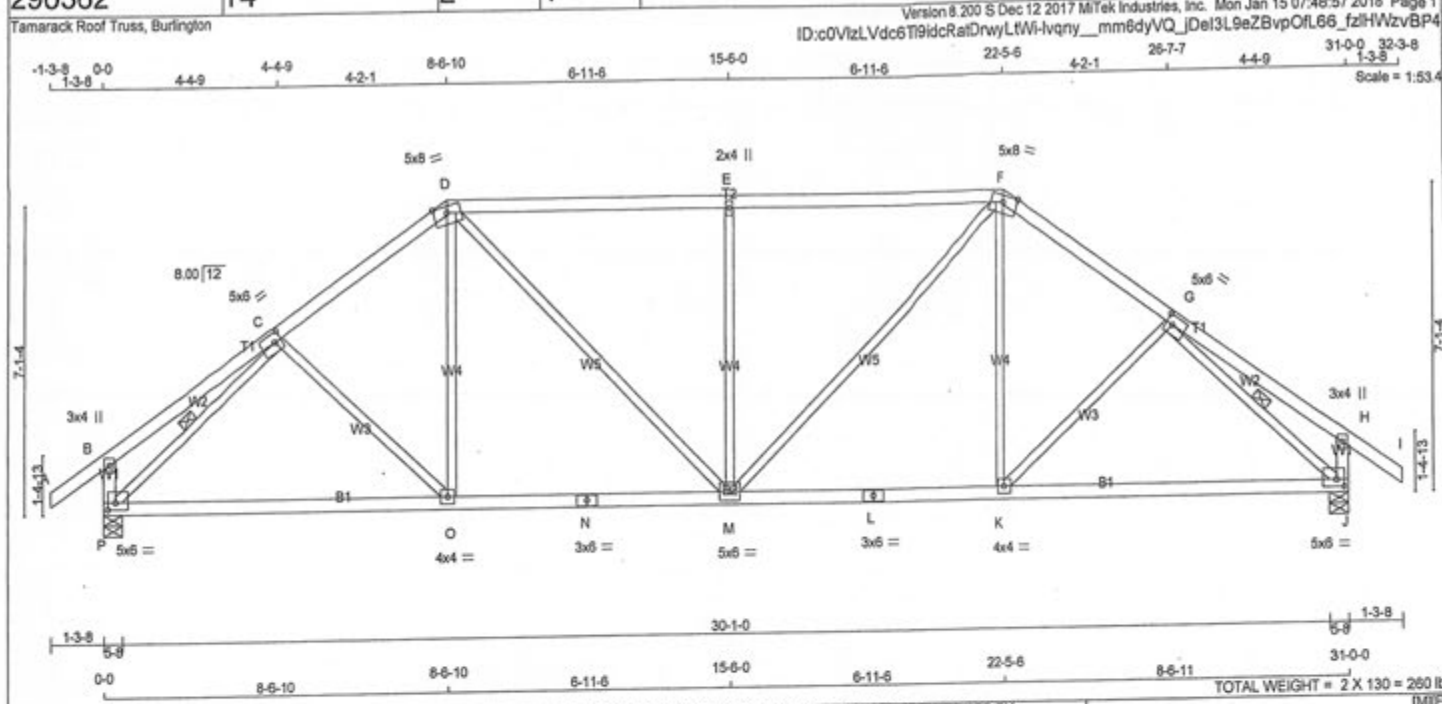
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
 JSI METAL= 0.93 (R) (INPUT = 1.00)



DRWG NO. TAM 3619-08
 STRUCTURAL COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No. 2	SPF
D - F	2x4	DRY	No. 2	SPF
F - I	2x4	DRY	No. 2	SPF
P - B	2x4	DRY	No. 2	SPF
J - H	2x4	DRY	No. 2	SPF
P - N	2x4	DRY	No. 2	SPF
N - L	2x4	DRY	No. 2	SPF
L - J	2x4	DRY	No. 2	SPF

ALL WEBS 2x3 DRY No. 2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	
G	TMVW-t	MT20	5.0	6.0	2.50	2.00
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0	2.00	2.25
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.00	2.25

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
P	2533	0	2533	0	5-8	5-8
J	2533	0	2533	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM. LIVE	WIND			
P	1987	1174 / 0	326 / 0	0 / 0	0 / 0	487 / 0	0 / 0
J	1987	1174 / 0	326 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) P, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.75 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MAX. FACTORED		FACTORED		MAX. UNBRACED LENGTH	MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)		MEMB.	FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 147	-124.4 -124.4	0.17 (1)	10.00	C-O	-78 / 115	0.05 (1)	
B-C	0 / 33	-124.4 -124.4	0.35 (1)	10.00	O-D	0 / 410	0.09 (3)	
C-D	-2654 / 0	-124.4 -124.4	0.36 (1)	3.94	D-M	0 / 866	0.19 (1)	
D-E	-2601 / 0	-124.4 -124.4	0.94 (1)	2.75	M-E	-1062 / 0	0.93 (1)	
E-F	-2601 / 0	-124.4 -124.4	0.94 (1)	2.75	M-F	0 / 866	0.19 (1)	
F-G	-2654 / 0	-124.4 -124.4	0.36 (1)	3.94	K-F	0 / 410	0.09 (3)	
G-H	0 / 33	-124.4 -124.4	0.35 (1)	10.00	K-G	-78 / 115	0.05 (1)	
H-I	0 / 147	-124.4 -124.4	0.17 (1)	10.00	P-C	-3038 / 0	0.74 (1)	
P-B	-377 / 0	0.0 0.0	0.04 (1)	7.81	G-J	-3038 / 0	0.74 (1)	
J-H	-377 / 0	0.0 0.0	0.04 (1)	7.81				
P-O	0 / 2236	-28.0 -28.0	0.71 (2)	10.00				
O-N	0 / 2182	-28.0 -28.0	0.72 (2)	10.00				
N-M	0 / 2182	-28.0 -28.0	0.72 (2)	10.00				
M-L	0 / 2182	-28.0 -28.0	0.72 (2)	10.00				
L-K	0 / 2182	-28.0 -28.0	0.72 (2)	10.00				
K-J	0 / 2236	-28.0 -28.0	0.71 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.03")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.23")
 ALLOWABLE DEFL. (TL) = $L/360$ (1.03")
 CALCULATED VERT. DEFL. (TL) = $L/947$ (0.39")

CSI: TC=0.94/1.00 (E-F:1), BC=0.72/1.00 (K-M:2), WB=0.93/1.00 (E-M:1), SSI=0.42/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

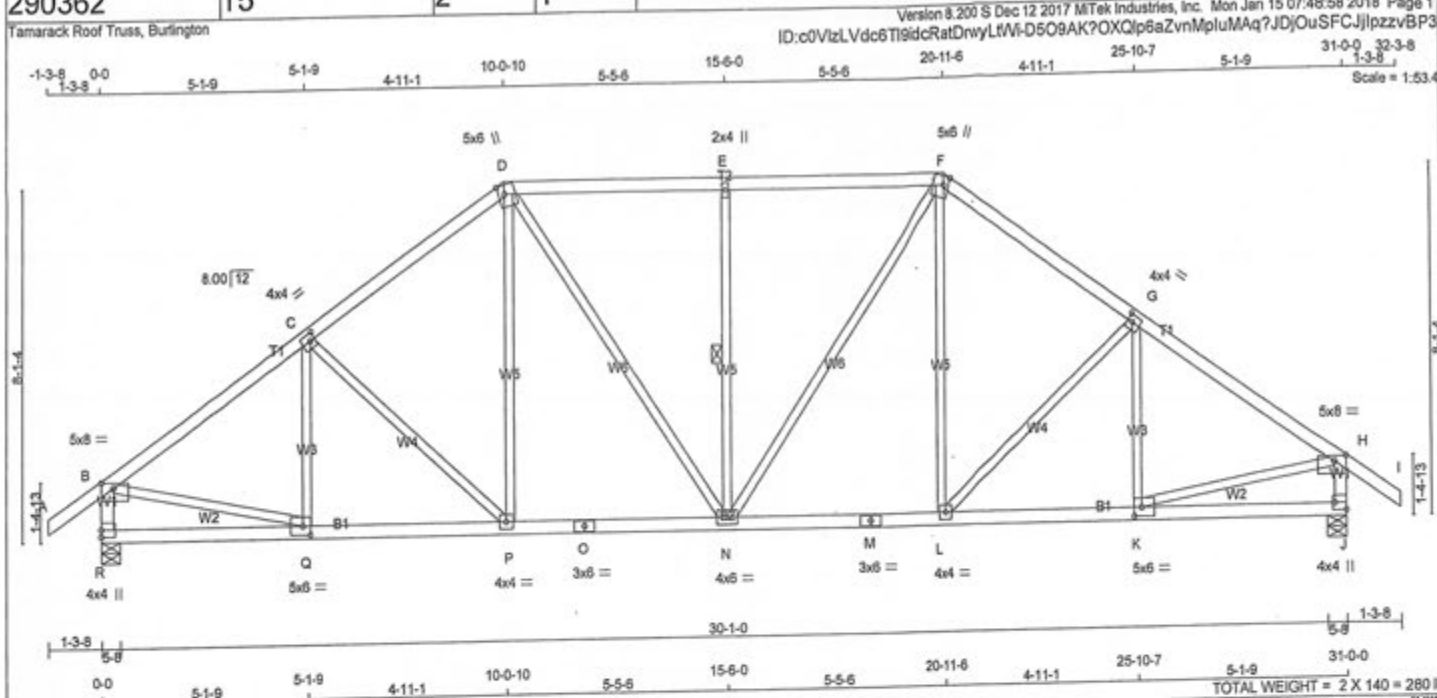
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
 JSI METAL= 0.75 (G) (INPUT = 1.00)



DRWG NO. TAM 3621 -18
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW+m	MT20	5.0	6.0	Edge	1.75
E	TMVW+m	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	Edge	1.75
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMVW-t	MT20	4.0	4.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.25
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2533	0	2533	0	5-8	5-8
J	2533	0	2533	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
R	1987	1174 / 0	326 / 0	0 / 0	0 / 0	487 / 0	0 / 0
J	1987	1174 / 0	326 / 0	0 / 0	0 / 0	487 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.67 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
 1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-NL

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO				
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	Q-C	-343 / 54 0.12 (1)
B-C	-2773 / 0	-124.4	-124.4	0.53 (1)	3.67	C-P	-375 / 0 0.32 (1)
C-D	-2523 / 0	-124.4	-124.4	0.50 (1)	3.86	P-D	0 / 410 0.09 (2)
D-E	-2407 / 0	-124.4	-124.4	0.55 (1)	3.83	D-N	0 / 595 0.13 (1)
E-F	-2407 / 0	-124.4	-124.4	0.55 (1)	3.83	N-E	-826 / 0 0.34 (1)
F-G	-2523 / 0	-124.4	-124.4	0.50 (1)	3.86	N-F	0 / 595 0.13 (1)
G-H	-2773 / 0	-124.4	-124.4	0.53 (1)	3.67	L-F	0 / 410 0.09 (2)
H-I	0 / 47	-124.4	-124.4	0.17 (1)	10.00	L-G	-375 / 0 0.32 (1)
I-B	-2470 / 0	0.0	0.0	0.25 (1)	5.45	K-G	-343 / 54 0.12 (1)
J-H	-2470 / 0	0.0	0.0	0.25 (1)	5.45	B-Q	0 / 2392 0.54 (1)
						K-H	0 / 2392 0.54 (1)
R-Q	0 / 0	-28.0	-28.0	0.18 (3)	10.00		
Q-P	0 / 2340	-28.0	-28.0	0.46 (1)	10.00		
P-O	0 / 2067	-28.0	-28.0	0.42 (1)	10.00		
O-N	0 / 2067	-28.0	-28.0	0.42 (1)	10.00		
N-M	0 / 2067	-28.0	-28.0	0.42 (1)	10.00		
M-L	0 / 2067	-28.0	-28.0	0.42 (1)	10.00		
L-K	0 / 2340	-28.0	-28.0	0.46 (1)	10.00		
K-J	0 / 0	-28.0	-28.0	0.18 (3)	10.00		

REGISTERED PROFESSIONAL ENGINEER

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360 (1.03')$
 CALCULATED VERT. DEFL. (LL) = $L/999 (0.12')$
 ALLOWABLE DEFL. (TL) = $L/360 (1.03')$
 CALCULATED VERT. DEFL. (TL) = $L/999 (0.20')$

CSI: TC=0.55/1.00 (E-F:1), BC=0.46/1.00 (K-L:1), WB=0.54/1.00 (H-K:1), SS=0.33/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

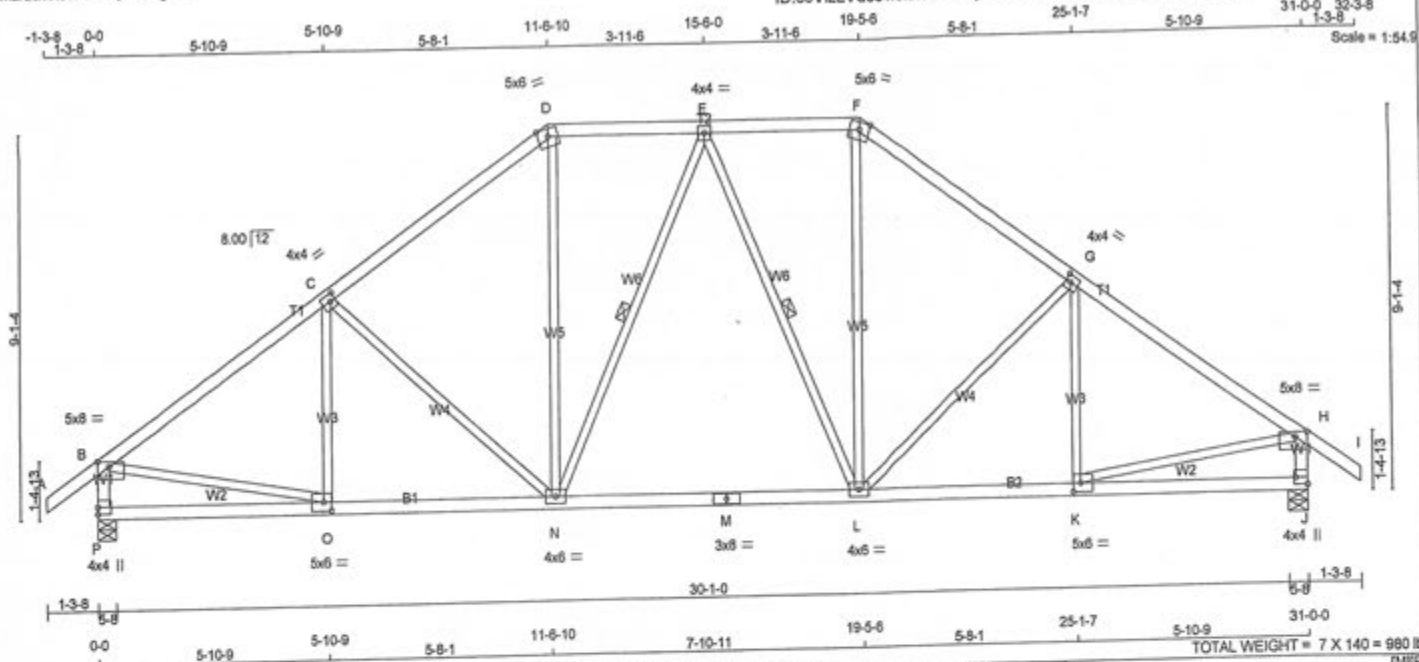
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)
 JSI METAL= 0.58 (Q) (INPUT = 1.00)



DRWG NO. TAM 3622-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - D 2x4 DRY No.2 SPF
 D - F 2x4 DRY No.2 SPF
 F - I 2x4 DRY No.2 SPF
 P - B 2x4 DRY No.2 SPF
 J - H 2x4 DRY No.2 SPF
 P - M 2x4 DRY No.2 SPF
 M - J 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	5.0	6.0	Edge	
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	5.0	6.0	Edge	
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	6.0	2.50	2.25
L	BMVW-t	MT20	4.0	6.0		
M	BS-t	MT20	3.0	8.0		
N	BMVW-t	MT20	4.0	6.0		
O	BMVW-t	MT20	5.0	6.0	2.50	2.25
P	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD IN-SX
P	2533 0	2533 0	5-8	5-8
J	2533 0	2533 0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
P	1987	1174 / 0	326 / 0	0 / 0	487 / 0
J	1987	1174 / 0	326 / 0	0 / 0	487 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.42 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 47	-124.4 -124.4	0.17 (1)	10.00	O-C	-268 / 106 0.11 (1)	
B-C	-2794 / 0	-124.4 -124.4	0.71 (1)	3.42	C-N	-542 / 0 0.64 (1)	
C-D	-2399 / 0	-124.4 -124.4	0.64 (1)	3.72	N-D	0 / 844 0.19 (1)	
D-E	-1963 / 0	-124.4 -124.4	0.27 (1)	4.56	E-L	-319 / 0 0.21 (1)	
E-F	-1963 / 0	-124.4 -124.4	0.27 (1)	4.53	L-F	0 / 844 0.19 (1)	
F-G	-2399 / 0	-124.4 -124.4	0.64 (1)	3.72	G-H	-542 / 0 0.64 (1)	
G-H	-2794 / 0	-124.4 -124.4	0.71 (1)	3.42	L-G	-268 / 106 0.11 (1)	
H-I	0 / 47	-124.4 -124.4	0.17 (1)	10.00	K-G	-268 / 106 0.11 (1)	
P-B	-2462 / 0	0.0 0.0	0.25 (1)	5.46	B-O	0 / 2403 0.54 (1)	
J-H	-2462 / 0	0.0 0.0	0.25 (1)	5.46	K-H	0 / 2403 0.54 (1)	
P-O	0 / 0	-28.0 -28.0	0.22 (3)	10.00			
O-N	0 / 2363	-28.0 -28.0	0.54 (1)	10.00			
N-M	0 / 2091	-28.0 -28.0	0.50 (2)	10.00			
M-L	0 / 2091	-28.0 -28.0	0.50 (2)	10.00			
L-K	0 / 2363	-28.0 -28.0	0.54 (1)	10.00			
K-J	0 / 0	-28.0 -28.0	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.03")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.15")
 ALLOWABLE DEFL. (TL) = L/360 (1.03")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.71/1.00 (G-H-I), BC=0.54/1.00 (K-L-I), WB=0.64/1.00 (G-L-I), SSI=0.29/1.00 (G-H-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PLI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 616 354 1667 822 2284 1656

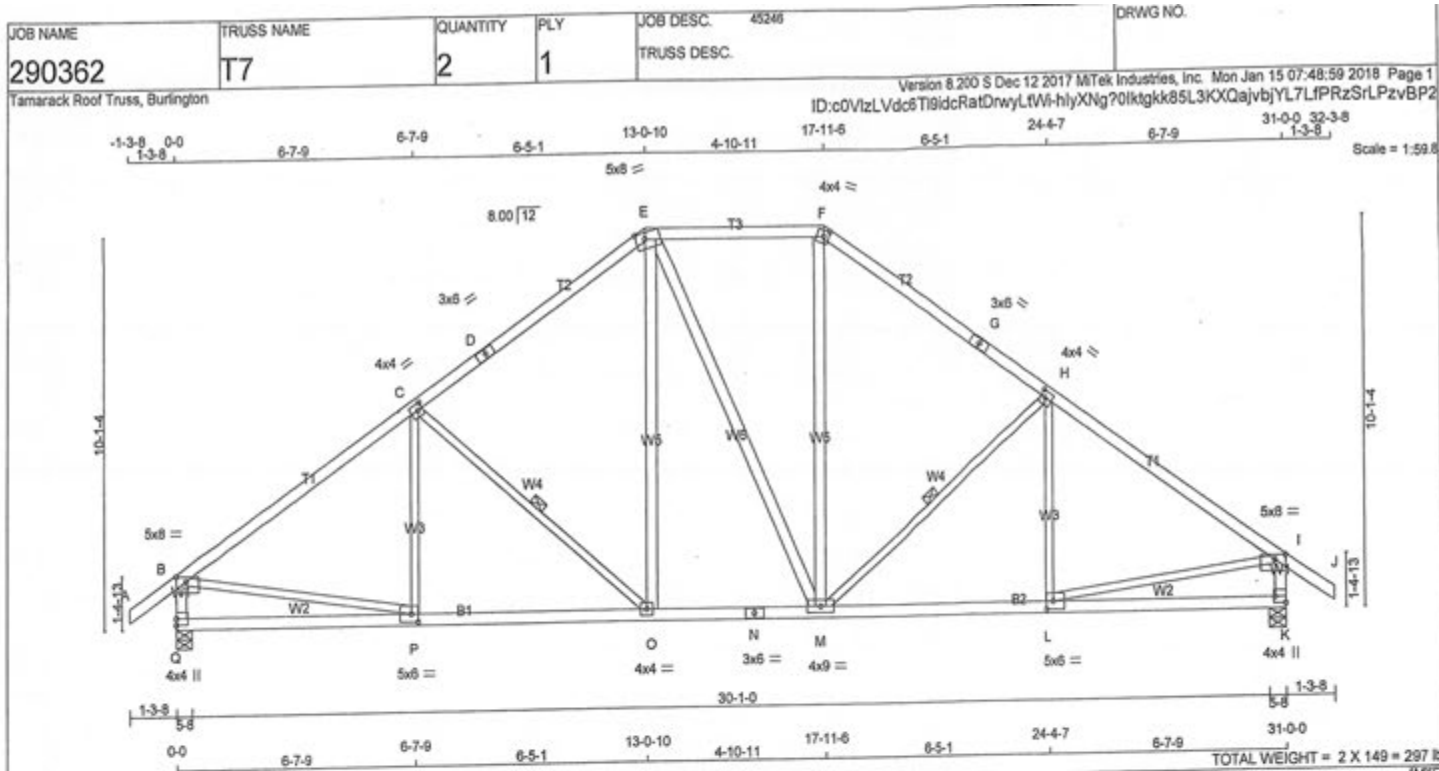
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
 JSI METAL= 0.72 (M) (INPUT = 1.00)



DRWG NO. TAM 3623-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

CHORDS	SIZE	DRY	No.2
O - E	2x4	DRY	No.2
E - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTVW-m	MT20	5.0	8.0	2.00	3.00
F	TTVW-m	MT20	4.0	4.0	2.00	1.75
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
K	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMVW-t	MT20	5.0	6.0	2.50	2.25
M	BMVW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.25
Q	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
Q	2533	0	2533	0
K	2533	0	2533	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM/LIVE	
Q	1987	1174 / 0	326 / 0	0 / 0	487 / 0
K	1987	1174 / 0	326 / 0	0 / 0	487 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.99 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)
FR-TO		FROM	TO	FR-TO	
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00
B-C	-2802 / 0	-124.4	-124.4	0.94 (1)	2.99
C-D	-2245 / 0	-124.4	-124.4	0.83 (1)	3.47
D-E	-2245 / 0	-124.4	-124.4	0.83 (1)	3.47
E-F	-1830 / 0	-124.4	-124.4	0.43 (1)	4.49
F-G	-2246 / 0	-124.4	-124.4	0.83 (1)	3.47
G-H	-2246 / 0	-124.4	-124.4	0.83 (1)	3.47
H-I	-2802 / 0	-124.4	-124.4	0.94 (1)	2.99
I-J	0 / 47	-124.4	-124.4	0.17 (1)	10.00
Q-B	-2457 / 0	0.0	0.0	0.25 (1)	5.46
K-I	-2457 / 0	0.0	0.0	0.25 (1)	5.46
Q-P	0 / 0	-28.0	-28.0	0.34 (3)	10.00
P-O	0 / 2376	-28.0	-28.0	0.56 (2)	10.00
O-N	0 / 1829	-28.0	-28.0	0.39 (1)	10.00
N-M	0 / 1829	-28.0	-28.0	0.39 (1)	10.00
M-L	0 / 2376	-28.0	-28.0	0.56 (2)	10.00
L-K	0 / 0	-28.0	-28.0	0.34 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:

- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.12")
ALLOWABLE DEFL.(TL) = $L/360$ (1.03")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.22")

CSI: TC=0.94/1.00 (B-C:1), BC=0.56/1.00 (O-P:2), WB=0.54/1.00 (B-P:1), SSI=0.33/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 822	2284 1656

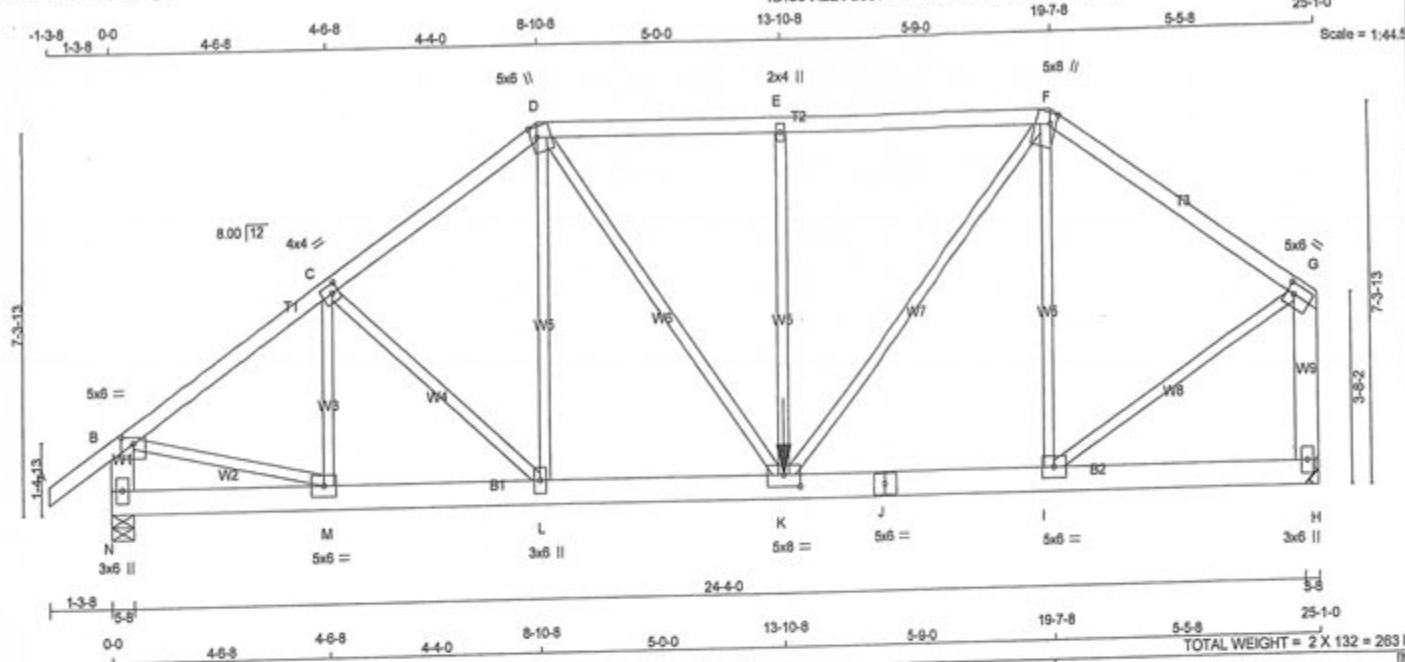
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.59 (P) (INPUT = 1.00)



DWG NO. TAM 3624-18
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - D 2x4 DRY No.2 SPF
 D - F 2x4 DRY No.2 SPF
 F - G 2x4 DRY No.2 SPF
 N - B 2x6 DRY No.2 SPF
 H - G 2x6 DRY No.2 SPF
 N - J 2x6 DRY No.2 SPF
 J - H 2x6 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS

A-D	1	12	TOP
D-F	1	12	TOP
F-G	1	12	TOP
N-B	2	12	TOP
H-G	2	12	TOP

BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS

N-J	2	12	SIDE(183.1)
J-H	2	12	TOP

WEBS : (0.122"x3") SPIRAL NAILS

E-K	1	6	SIDE(422.8)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50 3.00
C	TMVW-l	MT20	4.0	4.0	2.00 1.50
D	TTWW+m	MT20	5.0	6.0	2.50 1.50
E	TMVW-w	MT20	2.0	4.0	
F	TTWW+m	MT20	5.0	8.0	2.50 1.50
G	TMVW-l	MT20	5.0	6.0	2.25 1.75
H	BMV1+p	MT20	3.0	6.0	
I	BMVW-l	MT20	5.0	6.0	
J	BS-l	MT20	5.0	6.0	
K	BMVWV-l	MT20	5.0	8.0	2.75 4.00
L	BMVW-h	MT20	3.0	6.0	
M	BMVW-l	MT20	5.0	6.0	
N	BMV1+p	MT20	3.0	6.0	

HANGERS NOTES
 1)

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
N	3120	0	3195	0	0	5-8	5-8
H	3195	0	3195	0	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX	MIN	COMPONENT REACTIONS
N	2443	1450 / 0	395 / 0	0 / 0
H	2519	1489 / 0	427 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. MEMB. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO								
A-B	0 / 47	-124.4	-124.4	0.09 (1)	10.00	M-C	-587 / 0	0.08 (1)
B-C	-3558 / 0	-124.4	-124.4	0.29 (1)	4.70	C-L	-86 / 0	0.03 (1)
C-D	-3557 / 0	-124.4	-124.4	0.28 (1)	4.70	L-D	0 / 312	0.04 (2)
D-E	-3824 / 0	-124.4	-124.4	0.40 (1)	4.40	D-K	0 / 1528	0.19 (1)
E-F	-3824 / 0	-124.4	-124.4	0.42 (1)	4.40	K-E	-802 / 0	0.36 (1)
F-G	-2683 / 0	-124.4	-124.4	0.43 (1)	5.06	K-F	0 / 2521	0.31 (1)
N-B	-3040 / 0	0.0	0.0	0.11 (1)	7.81	I-F	-1082 / 0	0.49 (1)
H-G	-3144 / 0	0.0	0.0	0.23 (1)	7.78	B-M	0 / 3060	0.38 (1)
						I-G	0 / 2587	0.32 (1)
N-M	0 / 0	-28.0	-28.0	0.04 (2)	10.00			
M-L	0 / 2989	-28.0	-28.0	0.24 (1)	10.00			
L-K	0 / 2929	-28.0	-28.0	0.24 (1)	10.00			
K-J	0 / 2213	-28.0	-28.0	0.20 (1)	10.00			
J-I	0 / 2213	-28.0	-28.0	0.20 (1)	10.00			
I-H	0 / 0	-28.0	-28.0	0.07 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
K	13-10-8	-2323	-2323	---	FRONT	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.84")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
 ALLOWABLE DEFL.(TL) = L/360 (0.84")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.43/1.00 (F-G:1), BC=0.24/1.00 (K-L:1), WB=0.49/1.00 (F-I:1), SSI=0.18/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX	MIN	MAX
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

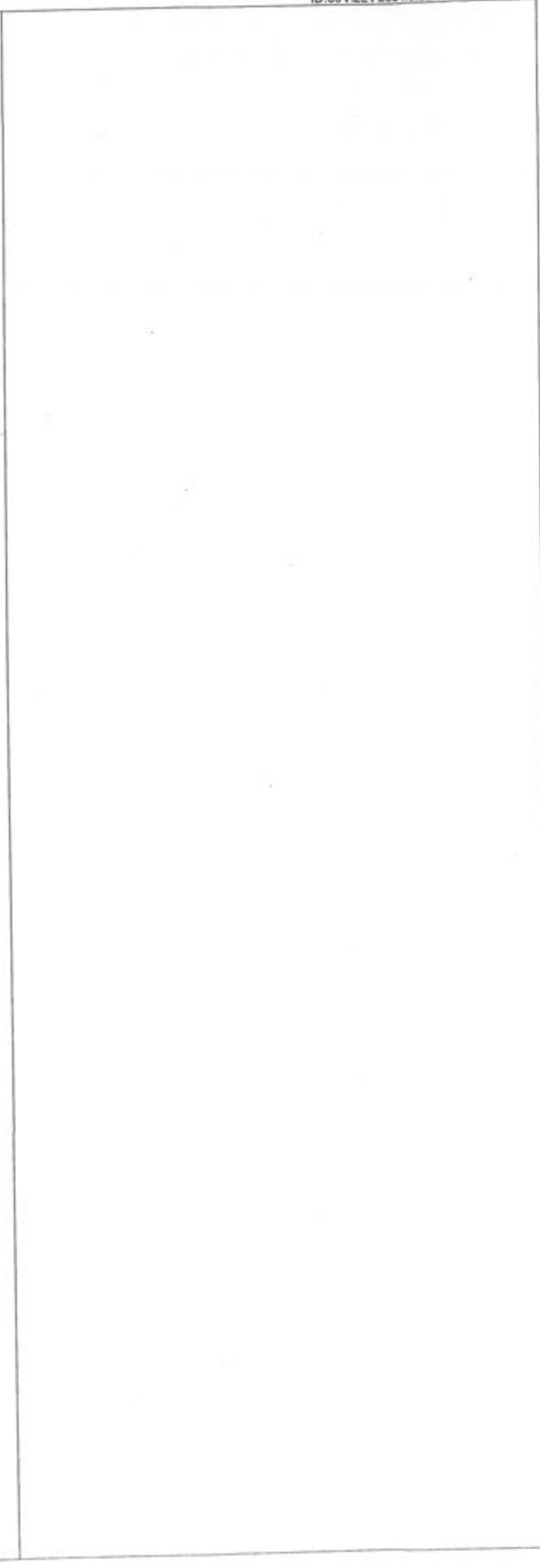
PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.87 (F) (INPUT = 0.90)
 JSI METAL= 0.37 (M) (INPUT = 1.00)



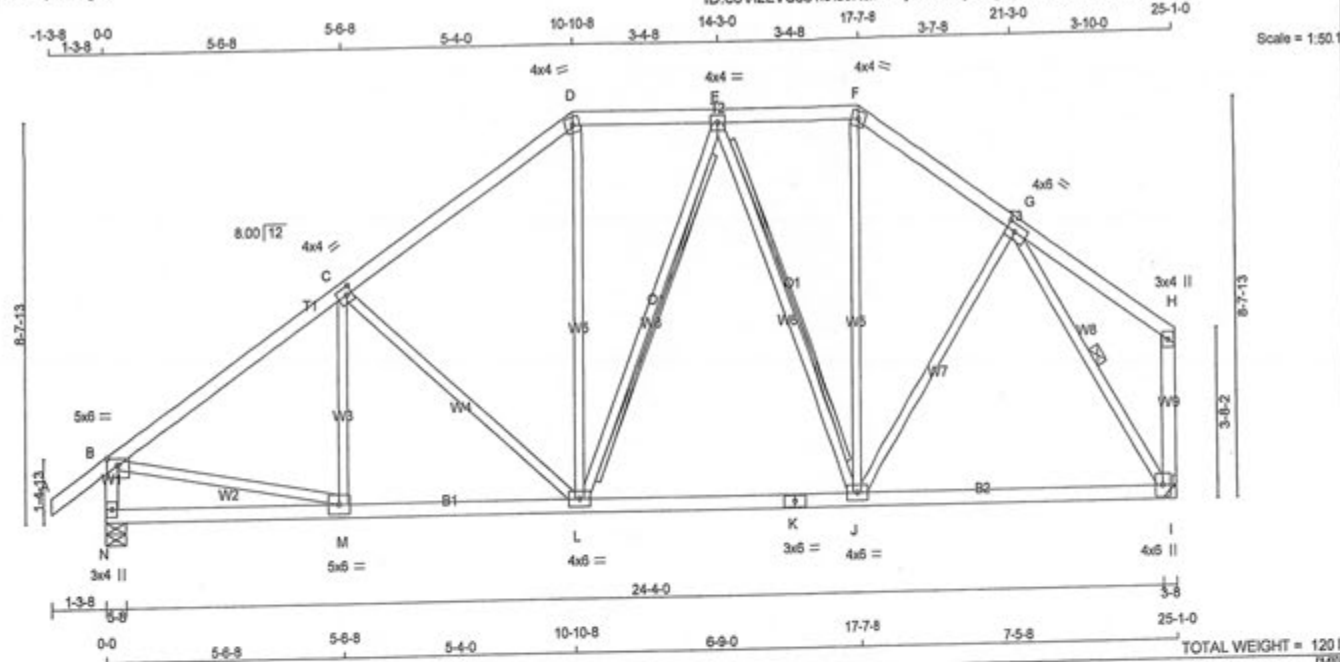
DWG NO. TAM 3625-18
 STRUCTURAL
 COMPONENT ONLY

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2322.5 lbs FACTORED DOWN AT 13-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



LICENSED PROFESSIONAL ENGINEER
 11578
 S. KATSOULAKOS
 PROVINCE OF ONTARIO

PKZ
 DWG NO. TAM **3625-18**
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.75	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	6.0	2.00	2.50
H	TMV-p	MT20	3.0	4.0		
I	BMVW1-p	MT20	4.0	6.0		
J	BMVW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	6.0		
M	BMVW-t	MT20	5.0	6.0		
N	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	2082	0	2082	0	5-8	5-8
I	1911	0	1911	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	1830	988 / 0	263 / 0	0 / 0	0 / 0	398 / 0	0 / 0
I	1512	873 / 0	263 / 0	0 / 0	0 / 0	376 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.02 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-I.
 2x4 DRY SPF No.2 1-BRACE AT E-L, E-J

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	FR-TO	
FR-TO		FROM TO			FR-TO				
A-B	0 / 47	-124.4 -124.4	0.17 (1)	10.00	M-C	-191 / 140	0.07 (1)		
B-C	-2161 / 0	-124.4 -124.4	0.56 (1)	4.02	C-L	-562 / 0	0.57 (1)		
C-D	-1746 / 0	-124.4 -124.4	0.52 (1)	4.44	L-D	0 / 527	0.12 (1)		
D-E	-1419 / 0	-124.4 -124.4	0.19 (1)	5.29	E-J	-3 / 60	0.01 (3)		
E-F	-1226 / 0	-124.4 -124.4	0.18 (1)	5.60	J-F	-534 / 0	0.34 (1)		
F-G	-1499 / 0	-124.4 -124.4	0.23 (1)	5.13	J-F	0 / 472	0.11 (1)		
G-H	0 / 29	-124.4 -124.4	0.27 (1)	10.00	J-G	0 / 307	0.07 (1)		
H-B	-2017 / 0	0.0 0.0	0.21 (1)	5.94	B-M	0 / 1870	0.42 (1)		
I-H	-181 / 0	0.0 0.0	0.04 (1)	7.81	G-I	-1953 / 0	0.62 (1)		
N-M	0 / 0	-28.0 -28.0	0.22 (3)	10.00					
M-L	0 / 1835	-28.0 -28.0	0.39 (2)	10.00					
L-K	0 / 1421	-28.0 -28.0	0.53 (2)	10.00					
K-J	0 / 1421	-28.0 -28.0	0.53 (2)	10.00					
J-I	0 / 1053	-28.0 -28.0	0.49 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 BOT CH. LL = 10.5 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 088-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.84")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
 ALLOWABLE DEFL. (TL) = L/360 (0.84")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CS1: TC=0.56/1.00 (B-C:1), BC=0.53/1.00 (J-L:2), WB=0.62/1.00 (G-I:1), SSI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (M) (INPUT = 0.90)
 JSI METAL= 0.46 (M) (INPUT = 1.00)



DRWG NO. TAM 3626-18
 STRUCTURAL
 COMPONENT ONLY

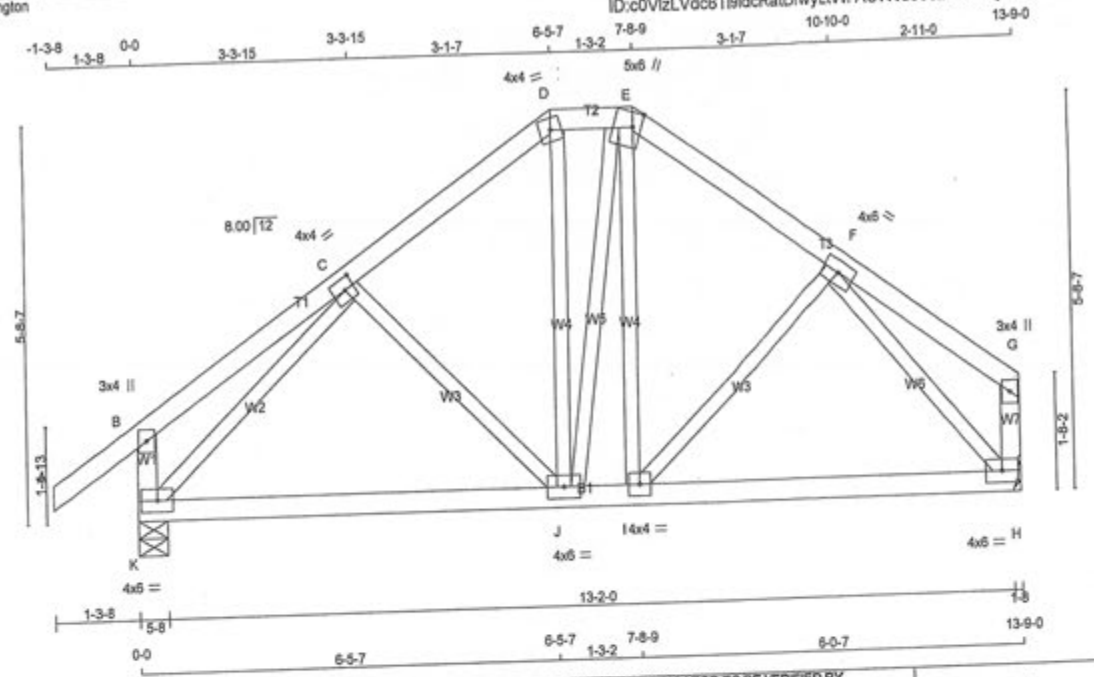


PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	165

PLATE PLACEMENT TOL = 0.250 inches
 PLATE ROTATION TOL = 5.0 Deg.
 JSI GRIP = 0.89 (K) (INPUT = 0.90)
 JSI METAL = 0.51 (K) (INPUT = 1.00)



DWG NO. TAM 3627-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE DRY LUMBER DESCR. LUMBER DESCR. LUMBER DESCR.

A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-1	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.50 1.50
F	TMWW-1	MT20	4.0	6.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW1-1	MT20	4.0	6.0	
I	BMVW1-1	MT20	4.0	4.0	
J	BMVW1-1	MT20	4.0	6.0	
K	BMVW1-1	MT20	4.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
K	1218	0	0	5-8
H	1048	0	0	5-8

HANGER BY OTHERS
 MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

	1ST CASE	MAX MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
K	945	574 / 0	144 / 0	0 / 0	0 / 0	228 / 0
H	829	478 / 0	144 / 0	0 / 0	0 / 0	206 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MAX. FACTORED UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)
FR-TO						FR-TO			
A-B	0 / 147	-124.4	-124.4	0.17 (1)	10.00	C-J	-187 / 25	0.07 (1)	
B-C	0 / 125	-124.4	-124.4	0.20 (1)	10.00	J-D	0 / 223	0.05 (2)	
C-D	-845 / 0	-124.4	-124.4	0.16 (1)	6.25	J-E	0 / 58	0.01 (2)	
D-E	-685 / 0	-124.4	-124.4	0.03 (1)	6.25	I-E	0 / 157	0.04 (1)	
E-F	-831 / 0	-124.4	-124.4	0.14 (1)	6.25	I-F	-91 / 56	0.03 (1)	
F-G	0 / 125	-124.4	-124.4	0.18 (1)	10.00	K-C	-1156 / 0	0.38 (1)	
G-H	-326 / 0	0.0	0.0	0.03 (1)	7.81	F-H	-1123 / 0	0.33 (1)	
H-G	-129 / 0	0.0	0.0	0.01 (1)	7.81				
K-J	0 / 616	-28.0	-28.0	0.37 (3)	10.00				
J-I	0 / 675	-28.0	-28.0	0.38 (3)	10.00				
I-H	0 / 737	-28.0	-28.0	0.32 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA C86-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
 ALLOWABLE DEFL.(TL) = L/360 (0.46")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.20/1.00 (B-C-1), BC=0.38/1.00 (I-J-3), WB=0.38/1.00 (C-K-1), SSI=0.17/1.00 (J-K-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	822	2284	1656

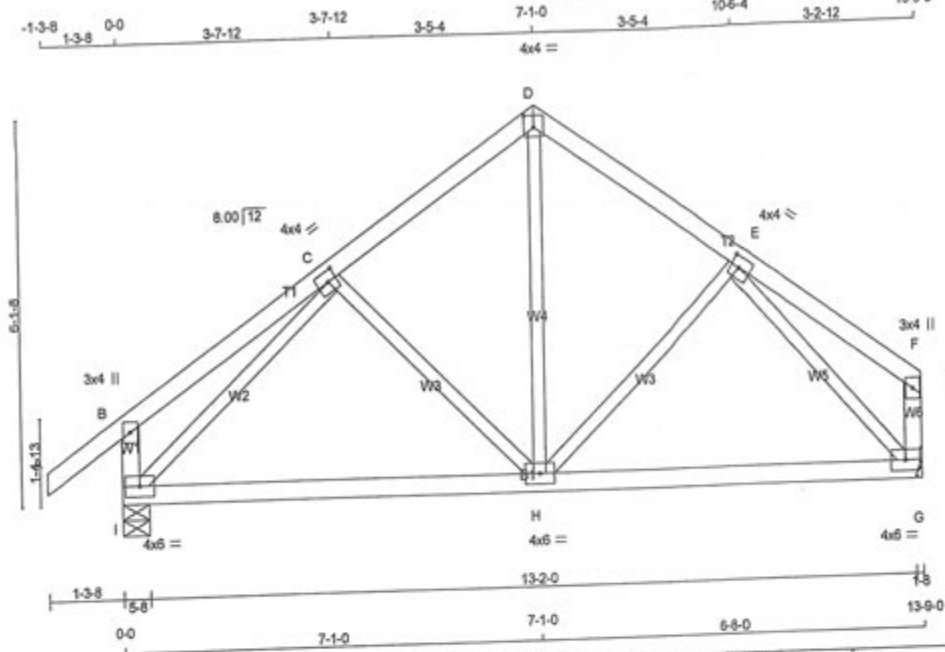
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (C) (INPUT = 0.90)
 JSI METAL= 0.41 (C) (INPUT = 1.00)



DWG NO. TAM 3628-18
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 58 lb (M/F)

LUMBER	N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
I - G.	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMV+p MT20	3.0	4.0		
C TMWW-t MT20	4.0	4.0	2.00	1.75
D TTW-p MT20	4.0	4.0	2.25	2.00
E TMWW-t MT20	4.0	4.0	2.00	1.75
F TMV+p MT20	3.0	4.0		
G BMVW1-t MT20	4.0	6.0		
H BMVWW-t MT20	4.0	6.0		
I BMVW1-t MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	DOWN	5-8	5-8
I	1218 0	1218 0	0	0
G	1048 0	1048 0	0	0

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	SNOW	LIVE			
I	948 574 / 0	144 / 0	0 / 0	228 / 0	0 / 0
G	829 478 / 0	144 / 0	0 / 0	206 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)
MEMB.						MEMB.				
FR-TO		FROM	TO			FR-TO		FROM	TO	
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	C-H	-246 / 12	0.10 (1)		
B-C	0 / 28	-124.4	-124.4	0.25 (1)	10.00	H-D	0 / 515	0.12 (1)		
C-D	-805 / 0	-124.4	-124.4	0.19 (1)	6.25	H-E	-153 / 44	0.06 (1)		
D-E	-802 / 0	-124.4	-124.4	0.17 (1)	6.25	I-C	-1149 / 0	0.44 (1)		
E-F	0 / 28	-124.4	-124.4	0.22 (1)	10.00	E-G	-1126 / 0	0.38 (1)		
I-B	-340 / 0	0.0	0.0	0.04 (1)	7.81					
G-F	-143 / 0	0.0	0.0	0.02 (1)	7.81					
I-H	0 / 824	-28.0	-28.0	0.48 (3)	10.00					
H-G	0 / 757	-28.0	-28.0	0.48 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 34.8 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 60.3 PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.46")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
 ALLOWABLE DEFL. (TL) = L/360 (0.46")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.25/1.00 (B-C-1), BC=0.48/1.00 (H-I-3), WB=0.44/1.00 (C-I-1), SSI=0.18/1.00 (H-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1667	822 2284 1656

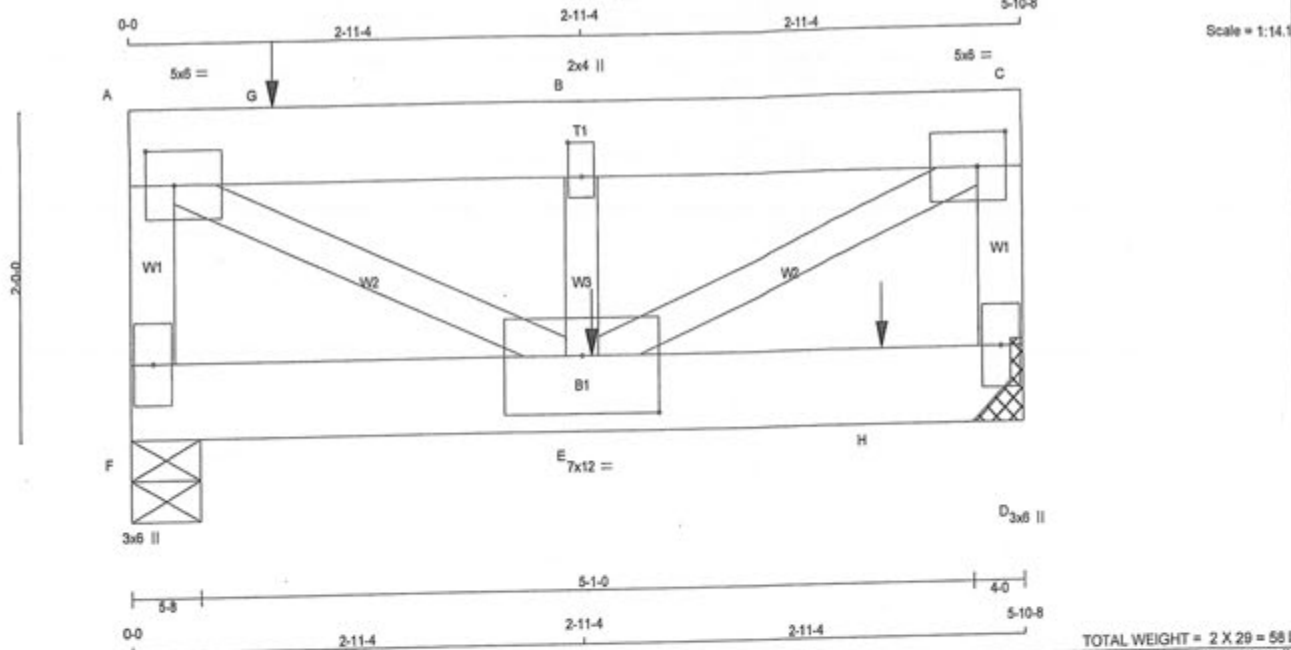
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
 JSI METAL= 0.40 (C) (INPUT = 1.00)



DWG NO. TAM 3629 -18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
F-A	1	12	TOP
C-D	1	12	TOP
A-C	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
F-D	2	4	SIDE(671.4)
WEBS: (0.122"x3") SPIRAL NAILS			
B-E	1	2	SIDE(154.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMW-1	MT20	5.0	6.0	2.50	2.25
B	TMW-1	MT20	2.0	4.0	2.50	1.00
C	TMW-1	MT20	5.0	6.0	2.50	2.25
D	BMV1-p	MT20	3.0	6.0		
E	BMW-1	MT20	7.0	12.0	4.25	6.00
F	BMV1-p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1311.3 lbs FACTORED DOWN AT 11-4 ON TOP CHORD, AND 3167.8 lbs FACTORED DOWN AT 3-0-0, AND 1883.9 lbs FACTORED DOWN AT 4-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ		
F	3434	0	3434	0	5-8	5-8
D	3824	0	3824	0	5-8	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 4-0

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
F	2700	1584 / 0	449 / 0	0 / 0	0 / 0	687 / 0	0 / 0
D	3004	1769 / 0	497 / 0	0 / 0	0 / 0	738 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED		
		VERT. LOAD (PLF)	LC1	MAX. CSI (LC)			FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO			FR-TO			
F-A	-3323 / 0	0.0	0.0	0.19 (1)	6.39	A-E	0 / 4694	0.58 (1)	
A-G	-4157 / 0	-124.4	-124.4	0.40 (1)	5.08	E-B	-652 / 0	0.05 (1)	
G-B	-4157 / 0	-124.4	-124.4	0.40 (1)	5.08	E-C	0 / 4694	0.58 (1)	
B-C	-4157 / 0	-124.4	-124.4	0.11 (1)	5.49				
D-C	-2430 / 0	0.0	0.0	0.14 (1)	7.23				
F-E	0 / 0	-28.0	-28.0	0.08 (1)	10.00				
E-H	0 / 0	-28.0	-28.0	0.48 (1)	10.00				
H-D	0 / 0	-28.0	-28.0	0.48 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
E	3-0-0	-3168	-3168	---	BACK	VERT	TOTAL
G	11-4	-1311	-1311	---	TOP	VERT	TOTAL
H	4-11-4	-1884	-1884	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 34.8	PSF
	DL	= 8.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD = 60.3 PSF			

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.05")

CSI: TC=0.40/1.00 (A-B-1), BC=0.48/1.00 (D-E-1), WB=0.58/1.00 (A-E-1), SSI=0.45/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

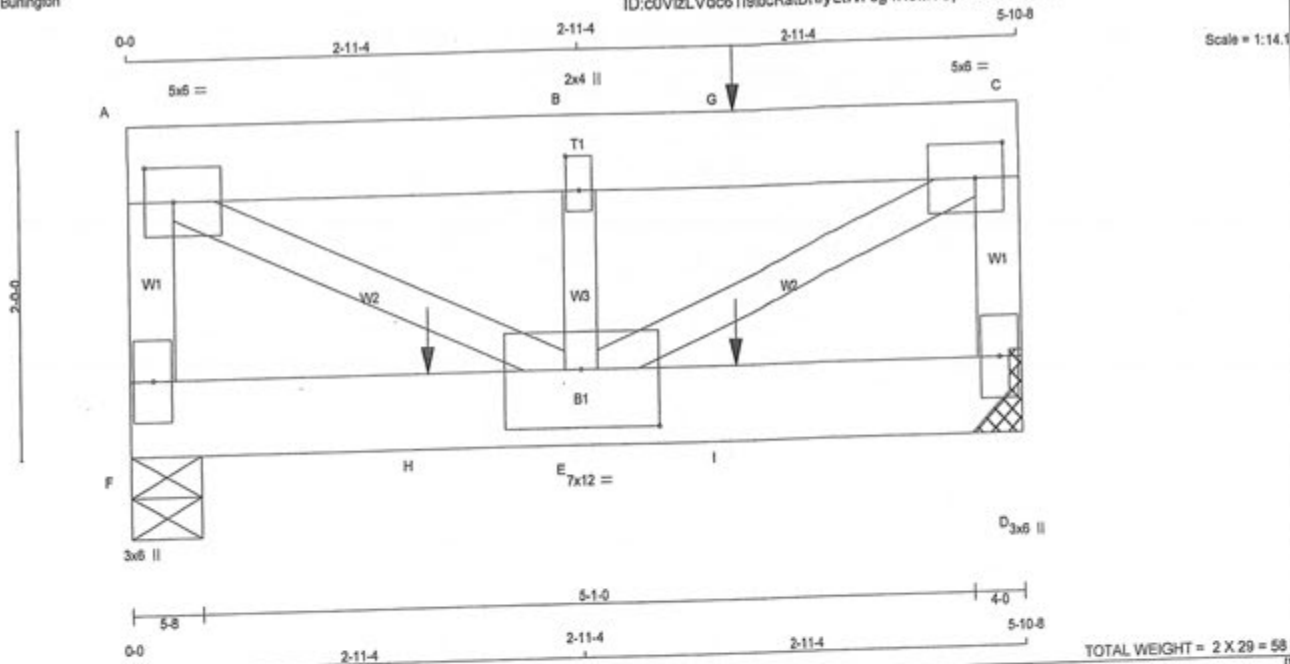
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.52 (A) (INPUT = 1.00)



DWG NO. TAM 3630 -18
STRUCTURAL
COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 F - A 2x4 DRY No.2 SPF
 A - C 2x6 DRY No.2 SPF
 D - C 2x4 DRY No.2 SPF
 F - D 2x6 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
C - D 1	12	TOP
A - C 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
F - D 2	12	SIDE (0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-I	MT20	5.0	6.0	2.50	2.25
B	TMVW+I	MT20	2.0	4.0	2.50	1.00
C	TMVW-I	MT20	5.0	6.0	2.50	2.25
D	BMV1+p	MT20	3.0	6.0		
E	BMVWVW-I	MT20	7.0	12.0	4.25	6.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1318.0 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND 1019.6 lbs FACTORED DOWN AT 1-11-4, AND 1019.6 lbs FACTORED DOWN AT 3-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
F	1902	0	0	5-8
D	2350	0	0	5-8

HANGER BY OTHERS
 MIN. SEAT SIZE: 4-0

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
F	1496	878/0	250/0	0/0
D	1854	1078/0	316/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.07 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LBS)	MAX. FACTORED LC2 (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LBS)
FR-TO		FROM	TO			FR-TO		
F-A	-1574/0	0.0	0.0	0.09 (1)	7.81	A-E	0/3055	0.38 (1)
A-B	-2705/0	-124.4	-124.4	0.04 (1)	6.25	E-B	-1292/0	0.10 (1)
B-G	-2705/0	-124.4	-124.4	0.35 (1)	6.07	E-C	0/3055	0.38 (1)
G-C	-2705/0	-124.4	-124.4	0.35 (1)	6.07			
D-C	-2023/0	0.0	0.0	0.12 (1)	7.75			
F-H	0/0	-28.0	-28.0	0.21 (1)	10.00			
H-E	0/0	-28.0	-28.0	0.21 (1)	10.00			
E-I	0/0	-28.0	-28.0	0.21 (1)	10.00			
I-D	0/0	-28.0	-28.0	0.21 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
G	3-11-4	-1318	-1318	---	TOP	VERT	TOTAL
H	1-11-4	-1020	-1020	---	BACK	VERT	TOTAL
I	3-11-4	-1020	-1020	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH	LL	= 34.8	PSF
DL	= 8.0	PSF	
BOT CH	LL	= 10.5	PSF
DL	= 7.0	PSF	
TOTAL LOAD	= 60.3	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
 ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.35/1.00 (B-C-1), BC=0.21/1.00 (E-F-1), WB=0.38/1.00 (A-E-1), SSI=0.36/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
	822	2284	1656

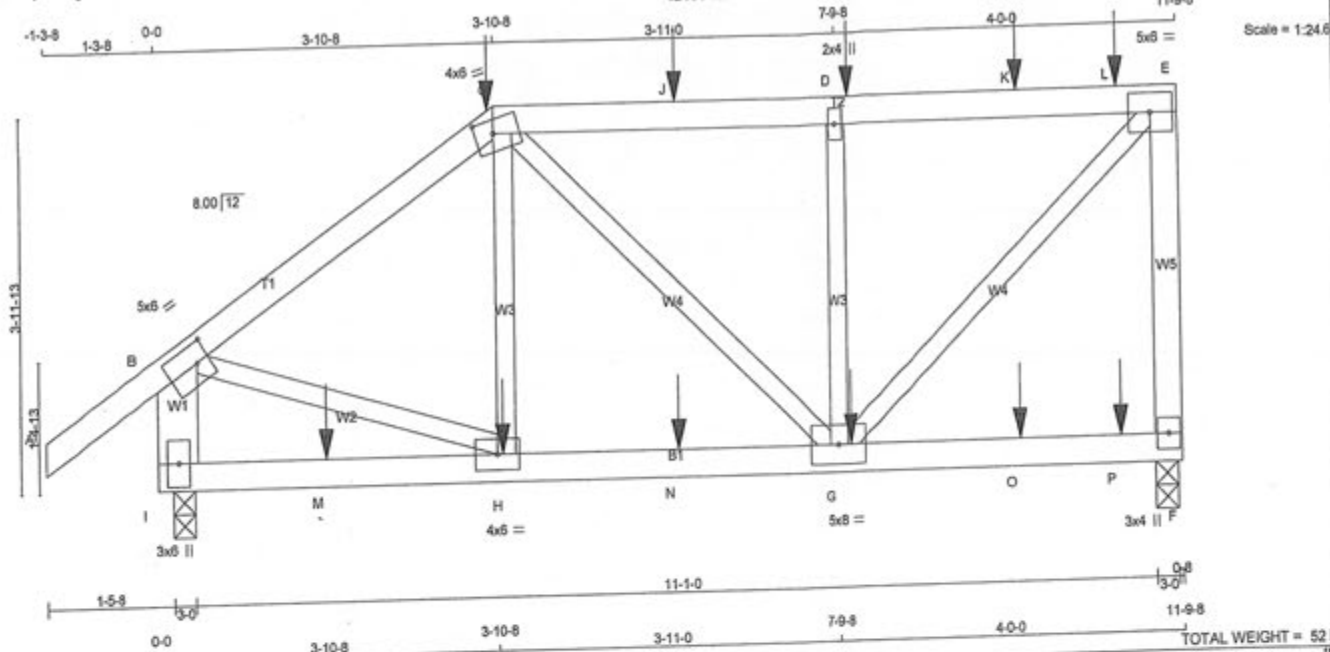
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (C) (INPUT = 0.90)
 JSI METAL = 0.34 (A) (INPUT = 1.00)



DWG NO. TAM 3631 -18
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - E 2x4 DRY No.2 SPF
 F - E 2x4 DRY No.2 SPF
 I - B 2x6 DRY No.2 SPF
 I - F 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0	2.50	1.75
C	TTWW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW-1	MT20	2.0	4.0		
E	TMVW-1	MT20	5.0	6.0		
F	BMV1-p	MT20	3.0	4.0		
G	BMVWV-1	MT20	5.0	6.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMV1-p	MT20	3.0	6.0		

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 271.2 lbs FACTORED DOWN AT 3-10-8, 116.8 lbs FACTORED DOWN AT 5-11-4, 116.8 lbs FACTORED DOWN AT 7-11-4, AND 116.8 lbs FACTORED DOWN AT 9-11-4, AND 136.0 lbs FACTORED DOWN AT 11-1-4 ON TOP CHORD, AND 35.3 lbs FACTORED DOWN AT 1-11-4, 45.9 lbs FACTORED DOWN AT 3-11-4, 45.9 lbs FACTORED DOWN AT 5-11-4, 45.9 lbs FACTORED DOWN AT 7-11-4, AND 45.9 lbs FACTORED DOWN AT 9-11-4, AND 53.6 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	UPLIFT	IN-SX
JT	1442	0	0	3-0
F	1432	0	0	3-0

UNFACTORED REACTIONS

	1ST LCASE COMBINED	MAX MIN SNOW	COMPONENT LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	1139	660 / 0	196 / 0	0 / 0	0 / 0	284 / 0	0 / 0
F	1116	659 / 0	174 / 0	0 / 0	0 / 0	273 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.18 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 47	-124.4 -124.4	0.18 (1)	H-C	-113 / 167	0.04 (3)	
B-C	-1238 / 0	-124.4 -124.4	0.39 (1)	C-G	0 / 103	0.03 (1)	
C-J	-1100 / 0	-124.4 -124.4	0.52 (1)	G-D	-909 / 0	0.23 (1)	
D-D	-1100 / 0	-124.4 -124.4	0.52 (1)	G-E	0 / 1498	0.37 (1)	
D-K	-1100 / 0	-124.4 -124.4	0.52 (1)	B-H	0 / 1065	0.26 (1)	
K-L	-1100 / 0	-124.4 -124.4	0.52 (1)				
L-E	-1100 / 0	-124.4 -124.4	0.52 (1)				
F-E	-1358 / 0	0.0 0.0	0.35 (1)				
I-B	-1380 / 0	0.0 0.0	0.10 (1)				
I-M	0 / 0	-28.0 -28.0	0.15 (3)	10.00			
M-H	0 / 0	-28.0 -28.0	0.15 (3)	10.00			
H-N	0 / 1025	-28.0 -28.0	0.28 (2)	10.00			
N-G	0 / 1025	-28.0 -28.0	0.28 (2)	10.00			
G-O	0 / 0	-28.0 -28.0	0.19 (3)	10.00			
O-P	0 / 0	-28.0 -28.0	0.19 (3)	10.00			
P-F	0 / 0	-28.0 -28.0	0.19 (3)	10.00			

LICENSED PROFESSIONAL

1178

S. KATSOUKAKOS

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX*	FACE	OR	TYPE
JT	3-10-8	-271	-271	---	BACK	VERT	TOTAL
C	7-11-4	-117	-117	---	BACK	VERT	TOTAL
D	7-11-4	-26	-46	---	BACK	VERT	TOTAL
G	3-11-4	-26	-46	---	BACK	VERT	TOTAL
H	5-11-4	-117	-117	---	BACK	VERT	TOTAL
J	9-11-4	-117	-117	---	BACK	VERT	TOTAL
K	11-1-4	-136	-136	---	BACK	VERT	TOTAL
L	1-11-4	-14	-35	---	BACK	VERT	TOTAL
M	5-11-4	-26	-46	---	BACK	VERT	TOTAL
N	9-11-4	-26	-46	---	BACK	VERT	TOTAL
O	11-1-4	-31	-54	---	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
 ALLOWABLE DEFL.(TL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.52/1.00 (D-E-1), BC=0.28/1.00 (G-H-2), WB=0.37/1.00 (E-G-1), SS=0.35/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

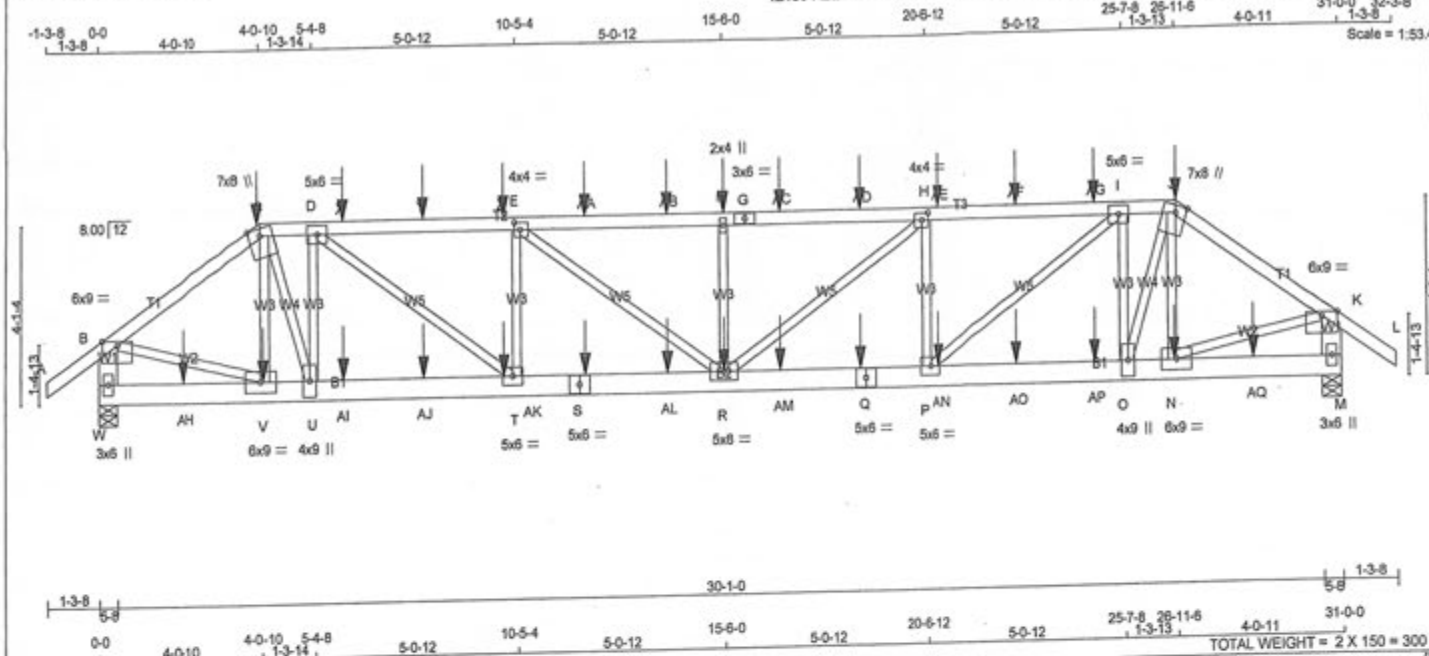
PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	818	354	1667
	822	2284	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.84 (H) (INPUT = 0.90)
 JSI METAL= 0.37 (B) (INPUT = 1.00)

DWG NO. TAM 3632-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2 SPF
C - G	2x4	DRY	No.2 SPF
G - J	2x4	DRY	No.2 SPF
J - L	2x4	DRY	No.2 SPF
W - B	2x6	DRY	No.2 SPF
M - K	2x6	DRY	No.2 SPF
W - S	2x6	DRY	No.2 SPF
S - Q	2x6	DRY	No.2 SPF
Q - M	2x6	DRY	No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A - C 1	12	SIDE(51.0)
C - G 1	12	SIDE(51.0)
G - J 1	12	SIDE(51.0)
J - L 1	12	SIDE(51.0)
W - B 2	12	TOP
M - K 2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
W - S 2	12	SIDE(183.1)
S - Q 2	12	SIDE(183.1)
Q - M 2	12	SIDE(183.1)
WEBS : (0.122'X3') SPIRAL NAILS		
V - C 1	6	SIDE(20.3)
N - J 1	6	SIDE(20.3)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3.0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	9.0	Edge	
C	TTWW+m	MT20	7.0	8.0	2.00	3.25
D	TMVW-l	MT20	5.0	6.0		
E	TMVW-l	MT20	4.0	4.0	2.00	1.75
F	TMVW+w	MT20	2.0	4.0		
G	TS-l	MT20	3.0	6.0		
H	TMVW-l	MT20	4.0	4.0	2.00	1.75
I	TMVW-l	MT20	5.0	6.0		
J	TTWW+m	MT20	7.0	8.0	2.00	3.25
K	TMVW-p	MT20	6.0	9.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMVW-l	MT20	6.0	9.0		
O	BMVW-l	MT20	4.0	9.0		
P	BMVW-l	MT20	5.0	6.0		
Q	BS-l	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	HORZ	UP/LIFT	IN-SX
JT	4201	0	0	5-8
W	4201	0	0	5-8
M	4201	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX /MIN	COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
JT	3304	1934 / 0	550 / 0	0 / 0	0 / 0	819 / 0
W	3304	1934 / 0	550 / 0	0 / 0	0 / 0	819 / 0
M	3304	1934 / 0	550 / 0	0 / 0	0 / 0	819 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) W, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					V-C			
A-B	0 / 47	-124.4	-124.4	0.09 (1)	10.00	-725 / 0	0.09 (1)	
B-C	-4928 / 0	-124.4	-124.4	0.31 (1)	4.08	C-U	0 / 2921	0.36 (1)
C-D	-5049 / 0	-124.4	-124.4	0.31 (1)	4.00	U-D	-2708 / 0	0.34 (1)
D-X	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00	D-T	0 / 3314	0.41 (1)
X-Y	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00	T-E	-1702 / 0	0.22 (1)
Y-Z	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00	E-R	0 / 1095	0.14 (1)
Z-E	-7720 / 0	-124.4	-124.4	0.74 (1)	2.77	R-F	-1071 / 0	0.14 (1)
E-AA	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	F-H	0 / 1095	0.14 (1)
AA-AB	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	H-P	-1702 / 0	0.22 (1)
AB-F	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	P-I	0 / 3314	0.41 (1)
F-G	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	I-O	-2708 / 0	0.34 (1)
G-AC	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	O-J	0 / 2921	0.36 (1)
AC-AD	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	J-N	-725 / 0	0.09 (1)
AD-H	-8601 / 0	-124.4	-124.4	0.74 (1)	2.77	N-V	0 / 4209	0.52 (1)
H-AE	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00	V-K	0 / 4209	0.52 (1)
AE-AF	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00			
AF-AG	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00			
AG-I	-7720 / 0	-124.4	-124.4	0.67 (1)	3.00			
I-J	-5049 / 0	-124.4	-124.4	0.31 (1)	4.00			
J-K	-4928 / 0	-124.4	-124.4	0.31 (1)	4.08			
K-L	0 / 47	-124.4	-124.4	0.09 (1)	10.00			
W-B	-4148 / 0	0.0	0.0	0.15 (1)	7.00			
M-K	-4148 / 0	0.0	0.0	0.15 (1)	7.00			



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.03")
CALCULATED VERT. DEFL.(LL) = L/999 (0.25")
ALLOWABLE DEFL.(TL) = L/360 (1.03")
CALCULATED VERT. DEFL.(TL) = L/907 (0.41")

CSI: TC=0.74/1.00 (E-F:1), BC=0.58/1.00 (R-T:1),
WB=0.52/1.00 (K-N:1), SS=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (G) (INPUT = 0.90)
JSI METAL= 0.65 (Q) (INPUT = 1.00)

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWH-1	MT20	5.0	8.0		
S	BS-1	MT20	5.0	6.0		
T	BMWWH-1	MT20	5.0	6.0		
U	BMWWH-1	MT20	4.0	9.0		
V	BMWWH-1	MT20	6.0	9.0		
W	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 545.6 lbs FACTORED DOWN AT 26-11-5, 545.6 lbs FACTORED DOWN AT 4-0-11, 149.7 lbs FACTORED DOWN AT 6-1-6, 149.7 lbs FACTORED DOWN AT 8-1-6, 149.7 lbs FACTORED DOWN AT 10-1-6, 149.7 lbs FACTORED DOWN AT 12-1-6, 149.7 lbs FACTORED DOWN AT 14-1-6, 149.7 lbs FACTORED DOWN AT 15-6-0, 149.7 lbs FACTORED DOWN AT 16-10-10, 149.7 lbs FACTORED DOWN AT 18-10-10, 149.7 lbs FACTORED DOWN AT 20-10-10, AND 149.7 lbs FACTORED DOWN AT 22-10-10, AND 149.7 lbs FACTORED DOWN AT 24-10-10 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, 69.9 lbs FACTORED DOWN AT 6-1-6, 69.9 lbs FACTORED DOWN AT 8-1-6, 69.9 lbs FACTORED DOWN AT 10-1-6, 69.9 lbs FACTORED DOWN AT 12-1-6, 69.9 lbs FACTORED DOWN AT 14-1-6, 69.9 lbs FACTORED DOWN AT 15-6-0, 69.9 lbs FACTORED DOWN AT 16-10-10, 69.9 lbs FACTORED DOWN AT 18-10-10, 69.9 lbs FACTORED DOWN AT 20-10-10, 69.9 lbs FACTORED DOWN AT 22-10-10, 69.9 lbs FACTORED DOWN AT 24-10-10, AND 69.9 lbs FACTORED DOWN AT 26-10-10, AND 69.9 lbs FACTORED DOWN AT 28-10-10 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

LOADING

TOTAL LOAD CASES: (4)

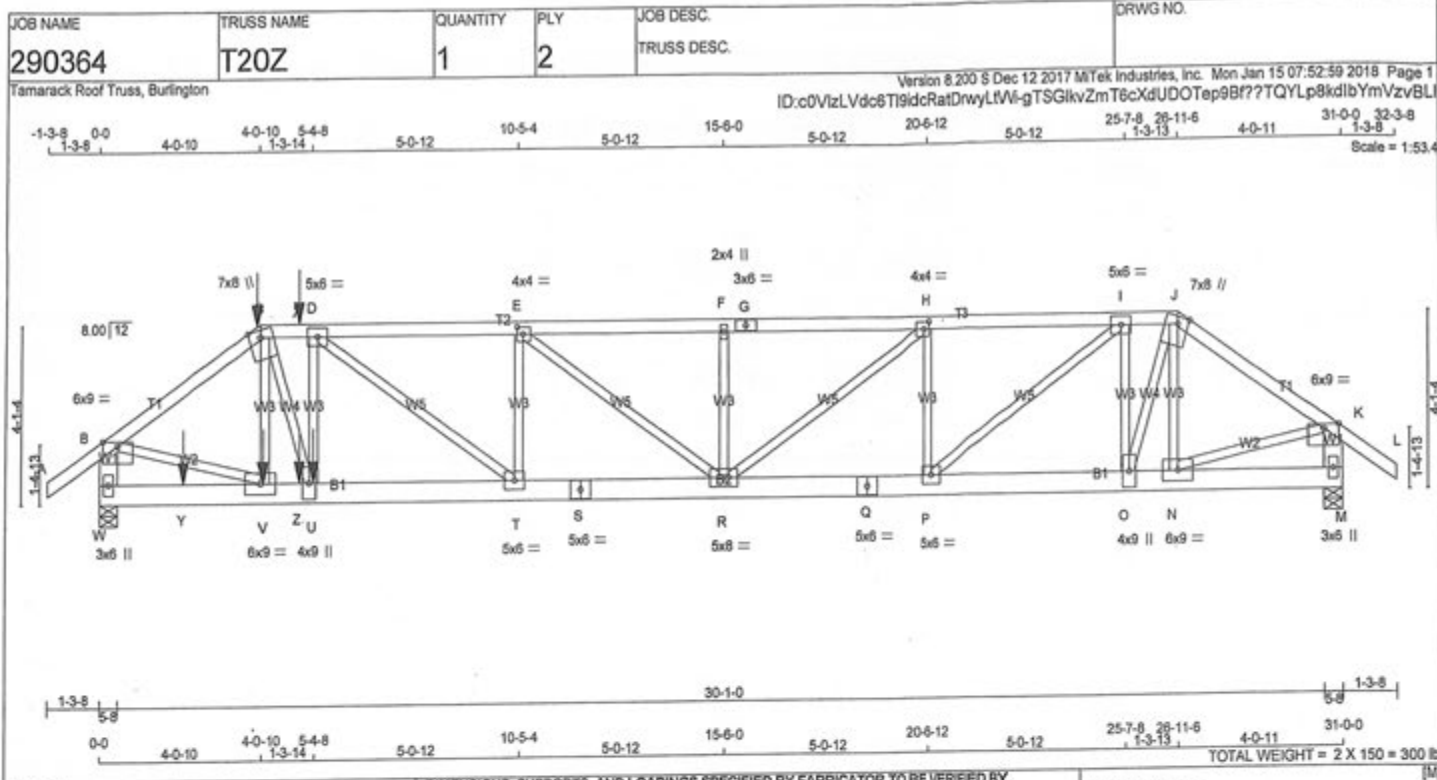
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB. UNBRAC. LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO				
AP-O	0 / 5051	-28.0	-28.0	0.39 (1)	10.00		
O-N	0 / 4073	-28.0	-28.0	0.30 (1)	10.00		
N-AQ	0 / 0	-28.0	-28.0	0.05 (3)	10.00		
AQ-M	0 / 0	-28.0	-28.0	0.05 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
C	4-0-11	-63	-70	---	FRONT	VERT	DEAD
C	4-0-11	-159	-159	---	FRONT	VERT	TOTAL
C	4-0-11	-324	-324	---	FRONT	VERT	SNOW
F	15-6-0	-150	-150	---	FRONT	VERT	TOTAL
J	26-11-5	-63	-70	---	FRONT	VERT	DEAD
J	26-11-5	-159	-159	---	FRONT	VERT	TOTAL
J	26-11-5	-324	-324	---	FRONT	VERT	SNOW
N	26-10-10	-40	-70	---	FRONT	VERT	TOTAL
Q	18-10-10	-40	-70	---	FRONT	VERT	TOTAL
R	15-6-0	-40	-70	---	FRONT	VERT	TOTAL
S	12-1-6	-40	-70	---	FRONT	VERT	TOTAL
V	4-1-6	-40	-70	---	FRONT	VERT	TOTAL
X	6-1-6	-150	-150	---	FRONT	VERT	TOTAL
Y	8-1-6	-150	-150	---	FRONT	VERT	TOTAL
Z	10-1-6	-150	-150	---	FRONT	VERT	TOTAL
AA	12-1-6	-150	-150	---	FRONT	VERT	TOTAL
AB	14-1-6	-150	-150	---	FRONT	VERT	TOTAL
AC	16-10-10	-150	-150	---	FRONT	VERT	TOTAL
AD	18-10-10	-150	-150	---	FRONT	VERT	TOTAL
AE	20-10-10	-150	-150	---	FRONT	VERT	TOTAL
AF	22-10-10	-150	-150	---	FRONT	VERT	TOTAL
AG	24-10-10	-150	-150	---	FRONT	VERT	TOTAL
AH	2-1-6	-40	-70	---	FRONT	VERT	TOTAL
AI	6-1-6	-40	-70	---	FRONT	VERT	TOTAL
AJ	8-1-6	-40	-70	---	FRONT	VERT	TOTAL
AK	10-1-6	-40	-70	---	FRONT	VERT	TOTAL
AL	14-1-6	-40	-70	---	FRONT	VERT	TOTAL
AM	16-10-10	-40	-70	---	FRONT	VERT	TOTAL
AN	20-10-10	-40	-70	---	FRONT	VERT	TOTAL
AO	22-10-10	-40	-70	---	FRONT	VERT	TOTAL
AP	24-10-10	-40	-70	---	FRONT	VERT	TOTAL
AQ	28-10-10	-40	-70	---	FRONT	VERT	TOTAL



DWG NO. TAM 3636-18
STRUCTURAL
COMPONENT ONLY



Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-1	MT20	5.0	8.0		
S	BS-1	MT20	5.0	6.0		
T	BMWW-1	MT20	5.0	6.0		
U	BMWW-1	MT20	4.0	9.0		
V	BMWW-1	MT20	6.0	9.0		
W	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 386.5 lbs FACTORED DOWN AT 4-0-11, AND 159.1 lbs FACTORED DOWN AT 4-0-11, AND 151.8 lbs FACTORED DOWN AT 5-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-6, 69.9 lbs FACTORED DOWN AT 4-1-6, AND 69.9 lbs FACTORED DOWN AT 5-0-4, AND 4049.9 lbs FACTORED DOWN AT 5-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 3637 -18
STRUCTURAL
COMPONENT ONLY



CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
290366	T20Z2	1	2	TRUSS DESC.	
Version 8.200 5 Dec 12 2017 Mitek Industries, Inc. Mon Jan 15 07:57:43 2018 Page 2					
ID:c0VtZLVdc6Tl9ldcRatDnwyLfVI-L6cdMrlJukuKbGCP16kQg4QXWxL9uGxxXUOaj1zvBGs					
Tamarack Roof Truss, Burlington					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWW-t	MT20	5.0	8.0		
U	BMWW-t	MT20	4.0	9.0	4.50	1.75
V	BMWW-t	MT20	6.0	9.0		
W	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 159.1 lbs FACTORED DOWN AT 4-0-11, AND 151.8 lbs FACTORED DOWN AT 5-0-4 ON TOP CHORD, AND 69.9 lbs FACTORED DOWN AT 2-1-8, 69.9 lbs FACTORED DOWN AT 4-1-8, AND 69.9 lbs FACTORED DOWN AT 5-0-4, AND 4185.8 lbs FACTORED DOWN AT 5-4-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 3646
STRUCTURAL
COMPONENT ONLY



CONTINUED ON PAGE 2

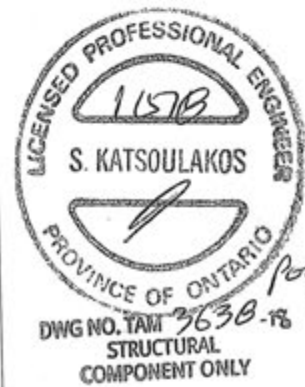
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
290364	T21	1	2	TRUSS DESC.	

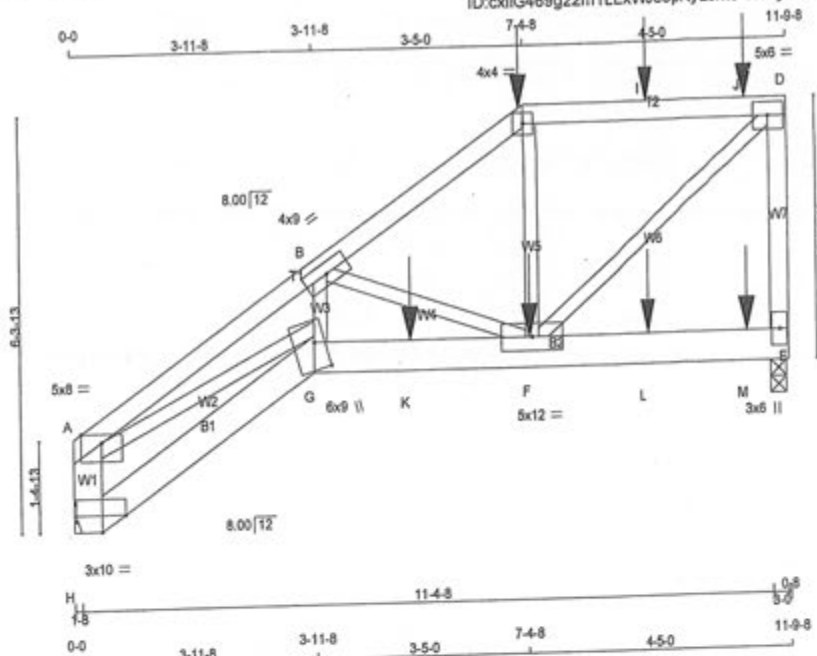
Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Mon Jan 15 07:53:00 2018 Page 2
ID: cxiIG469g22m1LExWJe3pKyLsm8-8f0ey4wBXnET8m3QyA92iOCEDsr74KXtsyK6lxzvBLH

Tamarack Roof Truss, Burlington

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2348.5 lbs FACTORED DOWN AT 13'-10" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.





LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	No.2	DESCR	SPF
H - A	2x6	DRY	No.2	SPF			
A - C	2x4	DRY	No.2	SPF			
C - D	2x4	DRY	No.2	SPF			
E - D	2x4	DRY	No.2	SPF			
H - G	2x6	DRY	No.2	SPF			
G - E	2x6	DRY	No.2	SPF			
ALL WEBS	2x3	DRY	No.2	SPF			
DRY: SEASONED LUMBER.							

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge		
B	TMVW-t	MT20	4.0	9.0			
C	TTW-t	MT20	4.0	4.0			
D	TMVW-t	MT20	5.0	6.0			
E	BMV-t	MT20	3.0	6.0			
F	BMVW-t	MT20	5.0	12.0			
G	BMVW-t	MT20	6.0	9.0	5.00	2.00	
H	BMV-t	MT20	3.0	10.0	Edge	4.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 126.0 lbs FACTORED DOWN AT 7-4-8, AND 116.6 lbs FACTORED DOWN AT 9-5-4, AND 136.0 lbs FACTORED DOWN AT 11-1-4 ON TOP CHORD, AND 462.1 lbs FACTORED DOWN AT 5-6-0, 45.9 lbs FACTORED DOWN AT 7-5-4, AND 45.9 lbs FACTORED DOWN AT 9-5-4, AND 53.6 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	1240	0	1240	0
E	1481	0	1481	0

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE
H	971	575 / 0	156 / 0	0 / 0
E	1163	652 / 0	191 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.19 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. (LC)	MAX. UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
MEMB.		FROM TO			MEMB.		
FR-TO					FR-TO		
H-A	-1185 / 0	0.0	0.0	0.09 (1)	A-G	0 / 3143	0.78 (1)
A-B	-3557 / 0	-124.4	-124.4	0.56 (1)	G-B	0 / 1368	0.34 (1)
B-C	-1419 / 0	-124.4	-124.4	0.21 (1)	B-F	-1837 / 0	0.47 (1)
C-I	-1165 / 0	-124.4	-124.4	0.78 (1)	F-C	0 / 226	0.08 (3)
I-J	-1165 / 0	-124.4	-124.4	0.78 (1)	F-D	0 / 1520	0.38 (1)
J-D	-1165 / 0	-124.4	-124.4	0.78 (1)			
E-D	-1415 / 0	0.0	0.0	0.33 (1)			
H-G	0 / 0	-28.0	-28.0	0.08 (3)			
G-K	0 / 2867	-28.0	-28.0	0.65 (1)			
K-F	0 / 2867	-28.0	-28.0	0.65 (1)			
F-L	0 / 0	-28.0	-28.0	0.12 (2)			
L-M	0 / 0	-28.0	-28.0	0.12 (2)			
M-E	0 / 0	-28.0	-28.0	0.12 (2)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	7-4-8	-126	-126		BACK	VERT	TOTAL
F	7-5-4	-26	-46		BACK	VERT	TOTAL
I	9-5-4	-117	-117		BACK	VERT	TOTAL
J	11-1-4	-136	-136		BACK	VERT	TOTAL
K	5-6-0	-462	-462		BACK	VERT	TOTAL
L	9-5-4	-26	-46		BACK	VERT	TOTAL
M	11-1-4	-31	-54		BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, CBC 2012, ABC 2014
- CSA 085-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/757 (0.19")

CS1: TC=0.78/1.00 (C-D:1), BC=0.65/1.00 (F-G:1), WB=0.78/1.00 (A-G:1), SS1=0.38/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

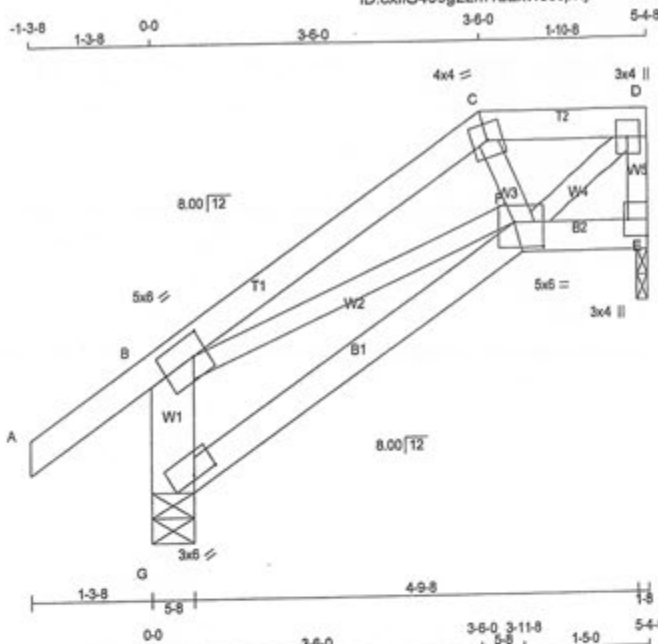
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 822 2284 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.99 (G) (INPUT = 1.00)

DWG NO. TAM 3640 -18-
STRUCTURAL
COMPONENT ONLY



Scale = 1:23.1

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
G - B	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
E - D	2x3	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	580	0	580	0	5-8	5-8
E	410	0	410	0	1-8	1-8

UNFACTORED REACTIONS

JT	MAX/MIN. COMPONENT REACTIONS						
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
G	442	283 / 0	56 / 0	0 / 0	0 / 0	103 / 0	0 / 0
E	324	187 / 0	56 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. C-F	WEBS MAX. FACTORED FORCE (LBS)	
		FROM	TO				FORCE (LBS)	CS1 (LC)
G-B	-525 / 0	0.0	0.0	0.04 (1)	7.81	C-F	-80 / 53	0.01 (3)
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	F-D	0 / 455	0.10 (1)
B-C	-466 / 0	-124.4	-124.4	0.27 (1)	6.25	B-F	0 / 407	0.09 (1)
C-D	-362 / 0	-124.4	-124.4	0.07 (1)	6.25			
E-D	-389 / 0	0.0	0.0	0.05 (1)	7.81			
G-F	0 / 0	-28.0	-28.0	0.14 (3)	10.00			
F-E	0 / 0	-28.0	-28.0	0.02 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CS1: TC=0.27/1.00 (B-C:1), BC=0.14/1.00 (F-G:3), WB=0.10/1.00 (D-F:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667

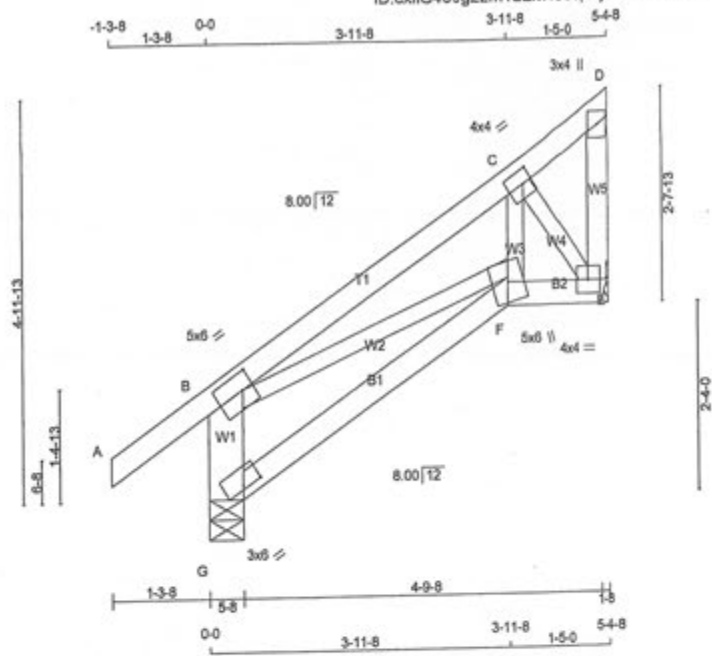
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.79 (F) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



DWG NO. TAM 3641-13
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
G - B	2x6	DRY	No.2
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - F	2x4	DRY	No.2
F - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0	2.50	1.75
C	TMVW-1	MT20	4.0	4.0	2.00	1.50
D	TMV+p	MT20	3.0	4.0		
E	BMVW-1	MT20	4.0	4.0		
F	BMVW+m	MT20	5.0	6.0		
G	BVM-1	MT20	3.0	6.0	0.50	3.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
G	580	0	580	0	5-8	5-8
E	410	0	410	0	5-8	5-8

HANGER BY OTHERS
 MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
G	442	283 / 0	56 / 0	0 / 0	0 / 0	103 / 0	0 / 0
E	324	197 / 0	56 / 0	0 / 0	0 / 0	81 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)		
FR-TO		FROM TO			FR-TO				
G-B	-525 / 0	0.0	0.0	0.04 (1)	7.81	B-F	0 / 443	0.10 (1)	
A-B	0 / 147	-124.4	-124.4	0.17 (1)	10.00	F-C	0 / 218	0.05 (2)	
B-C	-482 / 0	-124.4	-124.4	0.25 (1)	6.25	C-E	-806 / 0	0.09 (1)	
C-D	-76 / 0	-124.4	-124.4	0.25 (1)	6.25				
E-D	0 / 56	0.0	0.0	0.01 (1)	10.00				
G-F	0 / 0	-28.0	-28.0	0.15 (3)	10.00				
F-E	0 / 406	-28.0	-28.0	0.07 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2012, CBC 2012, ABC 2014
 - CSA 080-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.25/1.00 (B-C:1), BC=0.15/1.00 (F-G:3), WB=0.10/1.00 (B-F:1), SSI=0.19/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	
MT20	618	354	1667 822 2284 1656

PLATE PLACEMENT TOL = 0.250 inches

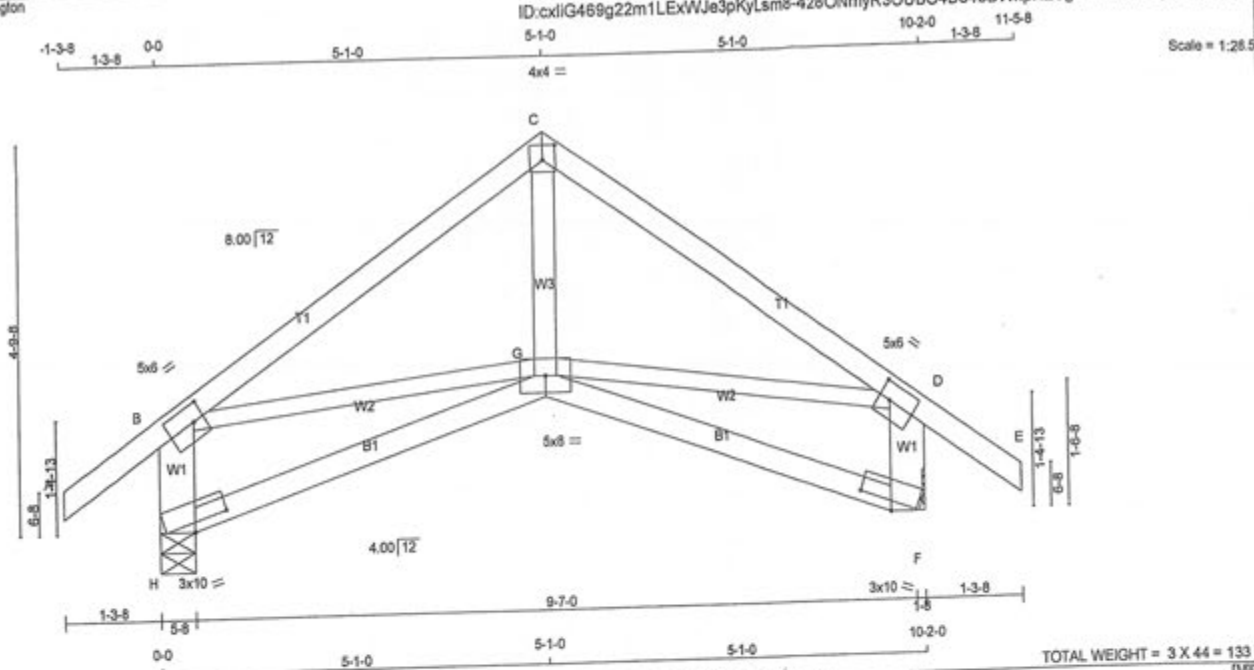
PLATE ROTATION TOL = 5.0 Deg

JSI GRIP= 0.47 (E) (INPUT = 0.90)
 JSI METAL= 0.18 (B) (INPUT = 1.00)



DWG NO. TAM 3642-18
 STRUCTURAL
 COMPONENT ONLY

Tamarack Roof Truss, Burlington



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
H - B	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
G - C	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	945	0	0	5-8
F	945	0	0	5-8

HANGER BY OTHERS
MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
H	730	449 / 0	107 / 0	0 / 0	0 / 0	174 / 0	0 / 0	0 / 0
F	730	449 / 0	107 / 0	0 / 0	0 / 0	174 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.13 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	FORCE	MAX
MEMB.	(LBS)	(PLF)	(LBS)	(LBS)	(LBS)	(LBS)	FR-TO	(LBS)	(LBS)
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	6.13	G-C	0 / 334	0.06 (3)
B-C	-811 / 0	-124.4	-124.4	0.42 (1)	6.13	6.13	B-G	0 / 677	0.15 (1)
C-D	-811 / 0	-124.4	-124.4	0.42 (1)	6.13	6.13	G-D	0 / 677	0.15 (1)
D-E	0 / 47	-124.4	-124.4	0.17 (1)	10.00	6.13			
H-B	-874 / 0	0.0	0.0	0.06 (1)	7.81	7.81			
F-D	-874 / 0	0.0	0.0	0.06 (1)	7.81	7.81			
H-G	0 / 0	-28.0	-28.0	0.24 (3)	10.00	10.00			
G-F	0 / 0	-28.0	-28.0	0.24 (3)	10.00	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 085-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.34")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
 ALLOWABLE DEFL. (TL) = $L/360$ (0.34")
 CALCULATED VERT. DEFL. (TL) = $L/999$ (0.09")

CSI: TC=0.42/1.00 (B-C-1), BC=0.24/1.00 (G-H-3), WB=0.15/1.00 (B-G-1), SSI=0.21/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

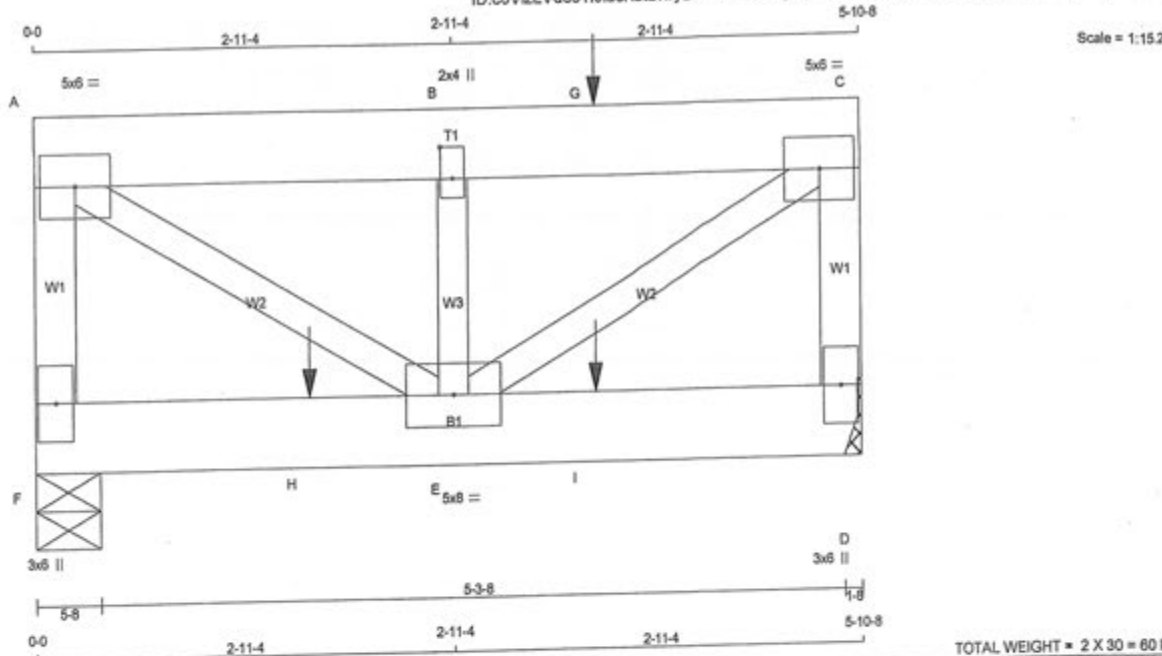
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.74 (D) (INPUT = 0.90)
 JSI METAL= 0.27 (B) (INPUT = 1.00)



DWG NO. TAM 3643-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
F-A	1	12	TOP
C-D	1	12	TOP
A-C	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
F-D	2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0		
B	TMVW-1	MT20	2.0	4.0	2.50	1.00
C	TMVW-1	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVW-1	MT20	5.0	8.0		
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1356.3 lbs FACTORED DOWN AT 3-11-4 ON TOP CHORD, AND 1019.6 lbs FACTORED DOWN AT 1-11-4, AND 1019.6 lbs FACTORED DOWN AT 3-11-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
F	1914 0	1914 0	5-8	5-8
D	2376 0	2376 0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS
F	1503	885 / 0	248 / 0
D	1970	1094 / 0	313 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO	F-A	-1611 / 0	0.0	0.0	10.10 (1)	7.81	A-E	0 / 2761	0.34 (1)
	A-B	-2327 / 0	-124.4	-124.4	0.07 (1)	6.25	E-B	-1373 / 0	0.11 (1)
	B-G	-2327 / 0	-124.4	-124.4	0.34 (1)	6.25	E-C	0 / 2761	0.34 (1)
	G-C	-2327 / 0	-124.4	-124.4	0.34 (1)	6.25			
	D-C	-2073 / 0	0.0	0.0	10.12 (1)	7.68			
	F-H	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
	H-E	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
	E-I	0 / 0	-28.0	-28.0	0.20 (1)	10.00			
	I-D	0 / 0	-28.0	-28.0	0.20 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
G	3-11-4	-1356	-1356	—	TOP	VERT	TOTAL
H	1-11-4	-1020	-1020	—	BACK	VERT	TOTAL
I	3-11-4	-1020	-1020	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 34.8 PSF
DL = 8.0 PSF

BOT CH. LL = 10.5 PSF
DL = 7.0 PSF

TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CS: TC=0.34/1.00 (B-C-1), BC=0.20/1.00 (D-E-1), WB=0.34/1.00 (A-E-1), SSI=0.38/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 822 2284 1656

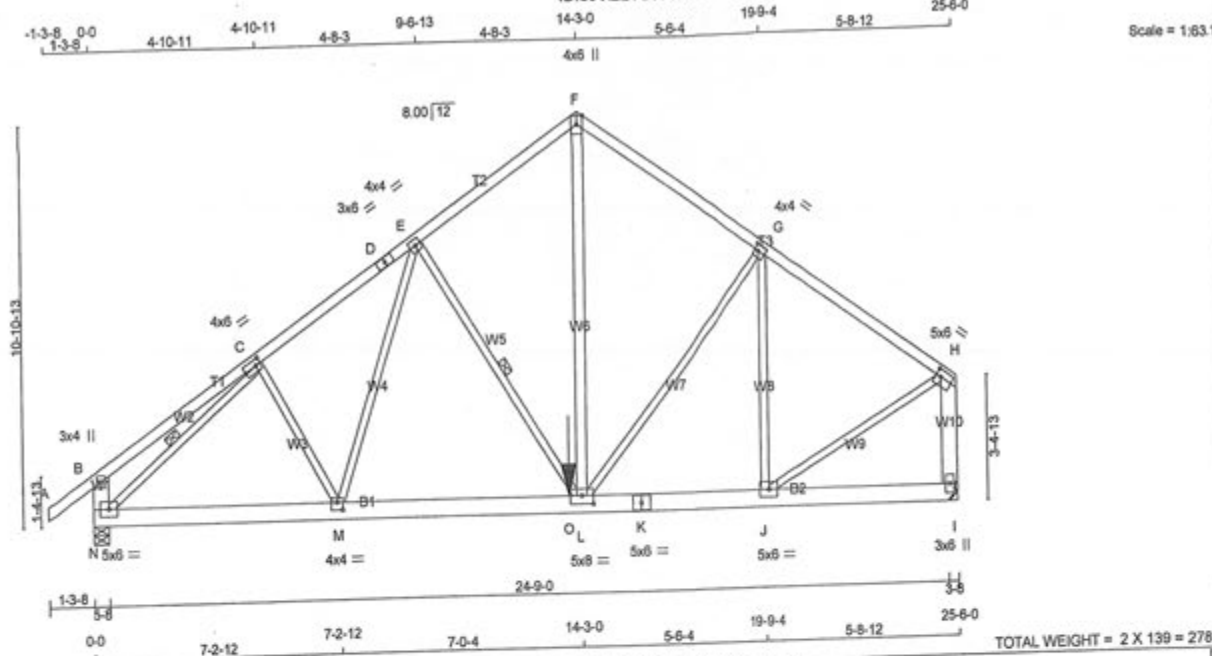
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.77 (E) (INPUT = 0.90)
JSI METAL= 0.29 (A) (INPUT = 1.00)



DWG NO. TAM 3644-18
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	SIZE	DRY	NO. 2	DESCR.
CHORDS					
A - D	2x4	DRY	No. 2	SPF	
D - F	2x4	DRY	No. 2	SPF	
F - H	2x4	DRY	No. 2	SPF	
N - B	2x6	DRY	No. 2	SPF	
I - H	2x6	DRY	No. 2	SPF	
N - K	2x6	DRY	No. 2	SPF	
K - I	2x6	DRY	No. 2	SPF	

ALL WEBS	2x3	DRY	No. 2	SPF	
EXCEPT					
L - F	2x4	DRY	No. 2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-F 1	12	TOP
F-H 1	12	TOP
N-B 2	12	TOP
I-H 2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
N-K 2	12	SIDE (183.1)
K-I 2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1	6	
L-F 1	6	SIDE (386.2)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3/8 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-I	MT20	4.0	6.0	2.00	1.75
D	TS-I	MT20	3.0	6.0		
E	TMWW-I	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMWW-I	MT20	4.0	4.0	2.00	1.50
H	TMWW-I	MT20	5.0	6.0	2.25	1.75
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-I	MT20	5.0	6.0		
K	BS-I	MT20	5.0	8.0	2.75	4.00
L	BMWW-I	MT20	5.0	8.0	2.50	2.00
M	BMWW-I	MT20	4.0	4.0		
N	BMWW-I	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
N	3108	0	3108	0	5-8	5-8
I	3130	0	3130	0	HANGER BY OTHERS	MIN. SEAT SIZE: 3-8

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
N	2434	1448 / 0	393 / 0	0 / 0	0 / 0	595 / 0	0 / 0
I	2437	1440 / 0	418 / 0	0 / 0	0 / 0	609 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.70 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LBS)	MAX. MEMB. UNBRACED LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO						
A-B	0 / 47	-124.4	-124.4	0.09 (1)	10.00	C-M -11 / 142
B-C	0 / 29	-124.4	-124.4	0.19 (1)	10.00	M-E 0 / 364
C-D	-3580 / 0	-124.4	-124.4	0.29 (1)	4.70	E-L -740 / 0
D-E	-3580 / 0	-124.4	-124.4	0.29 (1)	4.70	L-F 0 / 2810
E-F	-3013 / 0	-124.4	-124.4	0.26 (1)	5.05	F-G 0 / 246
F-G	-3029 / 0	-124.4	-124.4	0.46 (1)	4.80	G-H -1161 / 0
G-H	-2750 / 0	-124.4	-124.4	0.45 (1)	5.00	H-I -3970 / 0
H-I	-415 / 0	0.0	0.0	0.01 (1)	7.81	I-J 0 / 2625
I-J	-3042 / 0	0.0	0.0	0.19 (1)	7.81	

N-M	0 / 2986	-28.0	-28.0	0.25 (1)	10.00
M-O	0 / 2884	-28.0	-28.0	0.36 (1)	10.00
O-L	0 / 2884	-28.0	-28.0	0.36 (1)	10.00
L-K	0 / 2326	-28.0	-28.0	0.32 (1)	10.00
K-J	0 / 2326	-28.0	-28.0	0.32 (1)	10.00
J-I	0 / 0	-28.0	-28.0	0.06 (2)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DR
O	13-10-8	-2182	-2182		FRONT	VER



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.65")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.65")
CALCULATED VERT. DEFL. (TL) = L/999 (0.14")

CSI: TC=0.46/1.00 (F-G:1), BC=0.36/1.00 (L-M:1), WB=0.55/1.00 (C-N:1), SS=0.73/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (M) (INPUT = 0.90)
JSI METAL= 0.49 (C) (INPUT = 1.00)

DRWG NO. TAM 3647
STRUCTURAL COMPONENT ONLY

JOB NAME 290366	TRUSS NAME T31	QUANTITY 1	PLY 2	JOB DESC. TRUSS DESC.	DRWG NO.
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Version 8.200 S Dec 12 2017 Mitek Industries, Inc. Mon Jan 15 07:57:44 2018 Page 2
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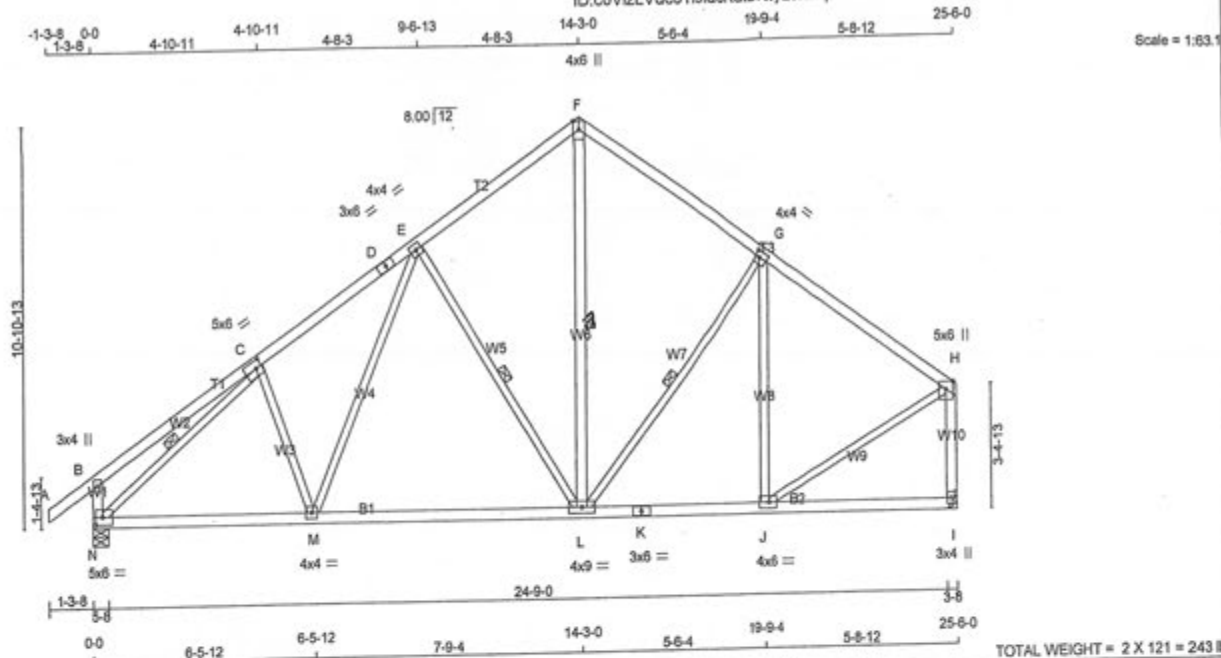
HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 2182.0 lbs FACTORED DOWN AT 13'-10" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.



DWG NO. TAM 3647 -18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 2 X 121 = 243 lb

LUMBER		N. L. G. A. RULES		LUMBER		DESCR	
CHORDS	SIZE						
A - D	2x4	DRY	No.2				SPF
D - F	2x4	DRY	No.2				SPF
F - H	2x4	DRY	No.2				SPF
H - B	2x4	DRY	No.2				SPF
I - H	2x4	DRY	No.2				SPF
N - K	2x4	DRY	No.2				SPF
K - I	2x4	DRY	No.2				SPF
ALL WEBS		2x3	DRY	No.2			SPF
EXCEPT							
L - F	2x4	DRY	No.2				SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - B	TMV+p	MT20	3.0	4.0		
B - C	TMVW-1	MT20	5.0	6.0	2.50	2.00
C - D	TS-t	MT20	3.0	6.0		
D - E	TMVW-1	MT20	4.0	4.0		
E - F	TTW+p	MT20	4.0	6.0	Edge	
F - G	TMVW-1	MT20	4.0	4.0	2.00	1.50
G - H	TMVW+p	MT20	5.0	6.0	Edge	
H - I	BMV1+p	MT20	3.0	4.0		
I - J	BMVW-1	MT20	4.0	6.0		
J - K	BS-t	MT20	3.0	6.0		
K - L	BMVW-1	MT20	4.0	9.0		
L - M	BMVW-1	MT20	4.0	4.0		
M - N	BMVW-1	MT20	5.0	6.0	2.50	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	UP	DOWN
N	2114	0	0	0
I	1943	0	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX / MIN	COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM	LIVE
N	1655	983 / 0	268 / 0	0 / 0	0 / 0
I	1537	867 / 0	268 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.34 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L, G-L, C-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO						FR-TO			
A-B	0 / 47	-124.4	-124.4	0.17 (1)	10.00	C-M	-192 / 71	0.07 (1)	
B-C	0 / 30	-124.4	-124.4	0.35 (1)	10.00	M-E	0 / 434	0.10 (1)	
C-D	-2107 / 0	-124.4	-124.4	0.36 (1)	4.34	E-L	-785 / 0	0.42 (1)	
D-E	-2107 / 0	-124.4	-124.4	0.36 (1)	4.34	L-F	0 / 1059	0.17 (1)	
E-F	-1432 / 0	-124.4	-124.4	0.35 (1)	5.07	L-G	-266 / 0	0.14 (1)	
F-G	-1448 / 0	-124.4	-124.4	0.54 (1)	4.73	J-G	-552 / 0	0.51 (1)	
G-H	-1553 / 0	-124.4	-124.4	0.56 (1)	4.58	N-C	-2448 / 0	0.68 (1)	
H-I	-413 / 0	0.0	0.0	0.04 (1)	7.81	J-H	0 / 1511	0.34 (1)	
I-H	-1875 / 0	0.0	0.0	0.37 (1)	6.13				
N-M	0 / 1825	-28.0	-28.0	0.54 (2)	10.00				
M-L	0 / 1589	-28.0	-28.0	0.52 (2)	10.00				
L-K	0 / 1331	-28.0	-28.0	0.41 (2)	10.00				
K-J	0 / 1331	-28.0	-28.0	0.41 (2)	10.00				
J-I	0 / 0	-28.0	-28.0	0.21 (3)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.85")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
 ALLOWABLE DEFL.(TL) = L/360 (0.85")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.56/1.00 (G-H:1), BC=0.54/1.00 (M-N:2), WB=0.68/1.00 (C-N:1), SSI=0.29/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

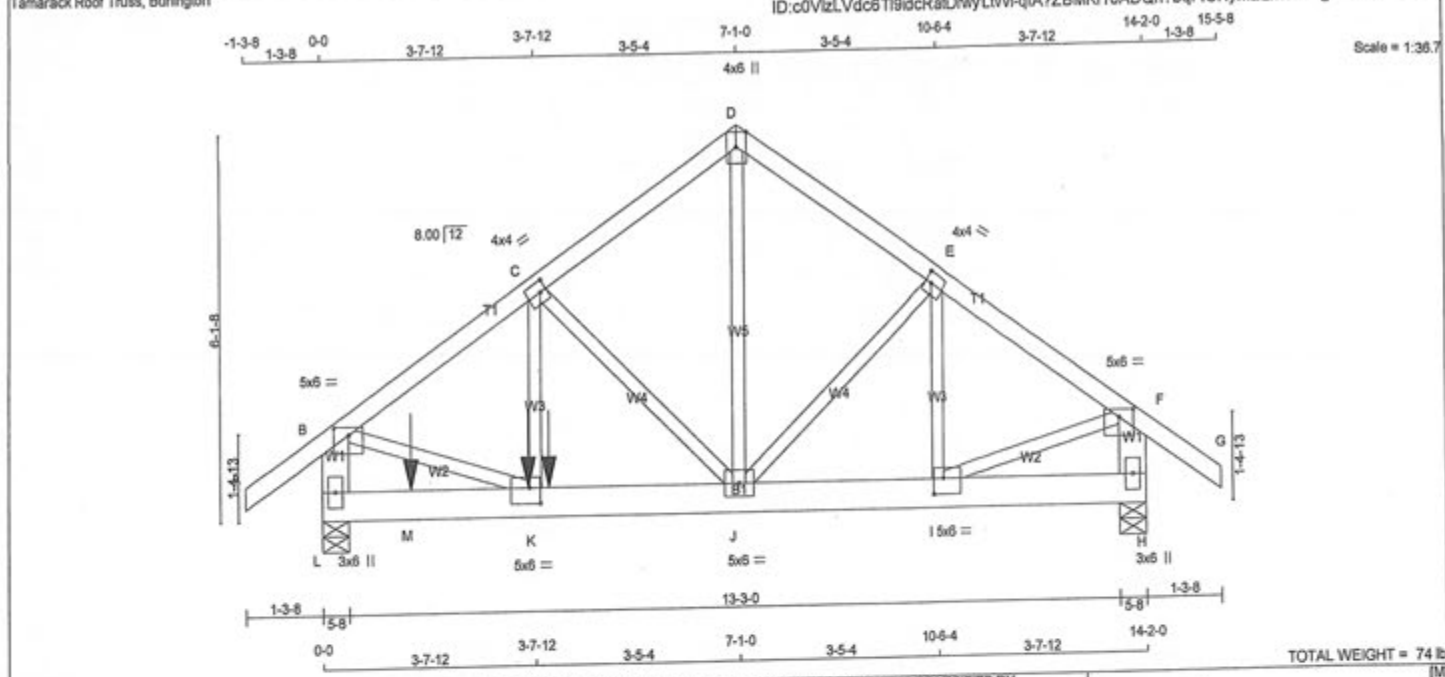
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

PLATE PLACEMENT | OL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
 JSI METAL= 0.62 (C) (INPUT = 1.00)

DRWG NO. TAM 3648-13
 STRUCTURAL
 COMPONENT ONLY



LUMBER				DESCR.	
N. L. G. A. RULES					
CHORDS	SIZE	LUMBER			
A - D	2x4	DRY	No.2	SPF	
D - G	2x4	DRY	No.2	SPF	
L - B	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
L - H	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMVW-p	MT20	5.0	6.0	1.50 3.00
C	TMVW-i	MT20	4.0	4.0	2.00 1.25
D	TTW+p	MT20	4.0	6.0	Edge
E	TMVW-i	MT20	4.0	4.0	2.00 1.25
F	TMVW-p	MT20	5.0	6.0	1.50 3.00
H	BMV1+p	MT20	3.0	6.0	
I	BMVW-i	MT20	5.0	6.0	3.00 2.25
J	BMVW-i	MT20	5.0	6.0	
K	BMVW-i	MT20	5.0	6.0	3.00 2.25
L	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 20.4 lbs FACTORED DOWN AT 1-6-4, AND 20.4 lbs FACTORED DOWN AT 3-6-4, AND 1204.4 lbs FACTORED DOWN AT 3-10-8 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	
L	2164	0	2164	0	0	5-8	5-8
H	1564	0	1564	0	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST CASE	MAX. MIN.	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE				
L	1670	1030 / 0	242 / 0	0 / 0	0 / 0	398 / 0	0 / 0
H	1211	741 / 0	179 / 0	0 / 0	0 / 0	290 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

CHORDS						WEBS					
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (L/C)	MAX. UNBRACED LENGTH	FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (L/C)			
FR-TO											
A-B	0 / 47	-124.4	-124.4	0.18 (1)	10.00	J-D	0 / 1096	0.27 (1)			
B-C	-2177 / 0	-124.4	-124.4	0.34 (1)	4.24	J-E	-228 / 0	0.09 (1)			
C-D	-1310 / 0	-124.4	-124.4	0.28 (1)	5.27	I-E	-273 / 32	0.05 (1)			
D-E	-1312 / 0	-124.4	-124.4	0.30 (1)	5.25	C-J	-1079 / 0	0.42 (1)			
E-F	-1450 / 0	-124.4	-124.4	0.31 (1)	5.04	K-C	0 / 737	0.18 (1)			
F-G	0 / 47	-124.4	-124.4	0.18 (1)	10.00	B-K	0 / 1899	0.47 (1)			
L-B	-2074 / 0	0.0	0.0	0.15 (1)	7.00	I-F	0 / 1274	0.32 (1)			
H-F	-1504 / 0	0.0	0.0	0.11 (1)	7.81						
L-M	0 / 0	-28.0	-28.0	0.09 (1)	10.00						
M-K	0 / 0	-28.0	-28.0	0.09 (1)	10.00						
K-J	0 / 1832	-28.0	-28.0	0.32 (1)	10.00						
J-I	0 / 1229	-28.0	-28.0	0.20 (1)	10.00						
I-H	0 / 0	-28.0	-28.0	0.05 (2)	10.00						

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
K	3-6-4	-12	-20	-	FRONT	VERT	TOTAL
K	3-10-8	-1204	-1204	-	FRONT	VERT	TOTAL
M	1-6-4	-12	-20	-	FRONT	VERT	TOTAL

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD
 ALLOWABLE DEFL.(LL)= L/360 (0.47")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
 ALLOWABLE DEFL.(TL)= L/360 (0.47")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.34/1.00 (B-C:1), BC=0.32/1.00 (J-K:1), WB=0.47/1.00 (B-K:1), SSI=0.20/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION		
	(PSI)	(PLI)	(PLI)		
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	822	2284 1656

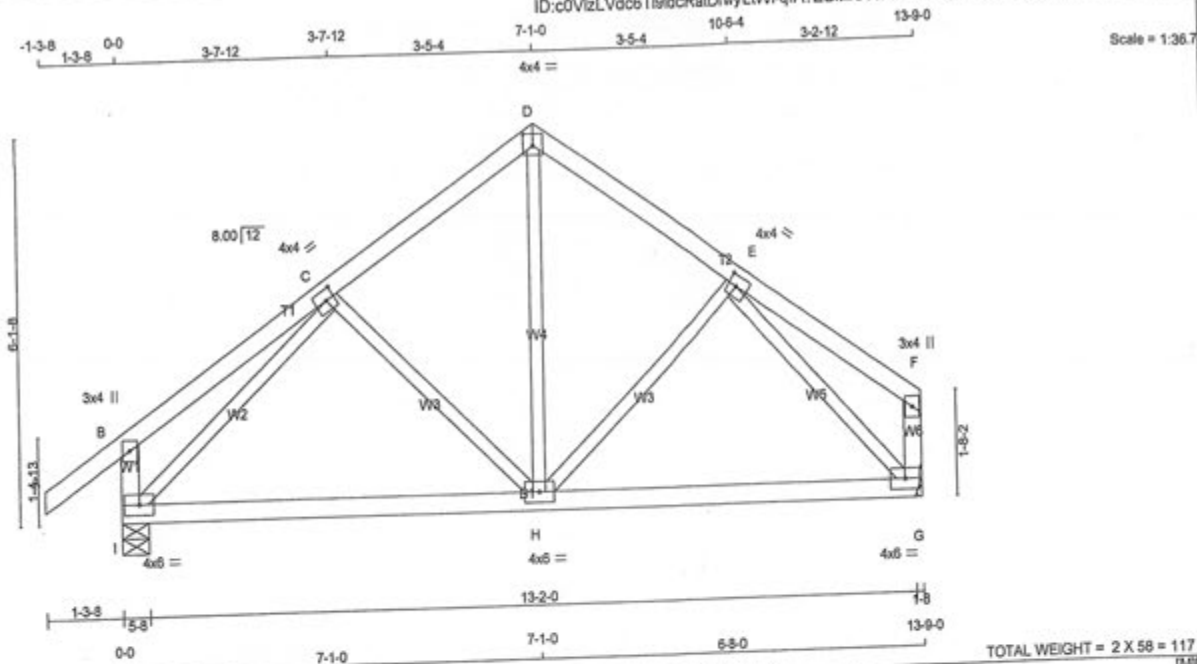
PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (i) (INPUT = 0.90)
 JSI METAL= 0.46 (i) (INPUT = 1.00)

DWG NO. TAM 3649-13
 STRUCTURAL
 COMPONENT ONLY



DRWG NO. TAM 3649-13
STRUCTURAL
COMPONENT ONLY



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
I - B	2x4	DRY	No.2	SPF	
G - F	2x4	DRY	No.2	SPF	
I - G	2x4	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-1	MT20	4.0	4.0	2.00	1.75
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-1	MT20	4.0	4.0	2.00	1.75
F	TMV+p	MT20	3.0	4.0		
G	BMVW1-1	MT20	4.0	6.0		
H	BMVWW1-1	MT20	4.0	6.0		
I	BMVW1-1	MT20	4.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
JT	1218	0	1218	0	5-8	5-8
G	1048	0	1048	0	HANGER BY OTHERS	MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN LIVE		PERM.LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
JT	945	574 / 0	144 / 0	0 / 0	0 / 0	0 / 0	228 / 0	0 / 0
G	829	478 / 0	144 / 0	0 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)!

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED					
MEMB.	FORCE	VERT. LOAD LC1	MAX	MAX.	MEMB.	FORCE	MAX		
	(LBS)	(PLF)	CSI (LC)	UNBRAC		(LBS)	CSI (LC)		
FR-TO		FROM TO		LENGTH	FR-TO				
A-B	0 / 47	-124.4 -124.4	0.17 (1)	10.00	C-H	-246 / 12	0.10 (1)		
B-C	0 / 28	-124.4 -124.4	0.25 (1)	10.00	H-D	0 / 515	0.12 (1)		
C-D	-805 / 0	-124.4 -124.4	0.19 (1)	6.25	H-E	-153 / 44	0.06 (1)		
D-E	-802 / 0	-124.4 -124.4	0.17 (1)	6.25	I-C	-1149 / 0	0.44 (1)		
E-F	0 / 28	-124.4 -124.4	0.22 (1)	10.00	E-G	-1126 / 0	0.38 (1)		
I-B	-340 / 0	0.0 0.0	0.04 (1)	7.81					
G-F	-143 / 0	0.0 0.0	0.02 (1)	7.81					
I-H	0 / 824	-28.0 -28.0	0.48 (3)	10.00					
H-G	0 / 757	-28.0 -28.0	0.48 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA C88-09
- TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.46")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.14")

CSI: TC=0.25/1.00 (B-C-1), BC=0.48/1.00 (H-I-3), WB=0.44/1.00 (C-I-1), SSI=0.18/1.00 (H-I-3)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)		SHEAR (PLI)	SECTION (PLI)	
	MAX	MIN		MAX	MIN
MT20	616	354	1667	822	2284

PLATE PLACEMENT TOL. = 0.250 inches

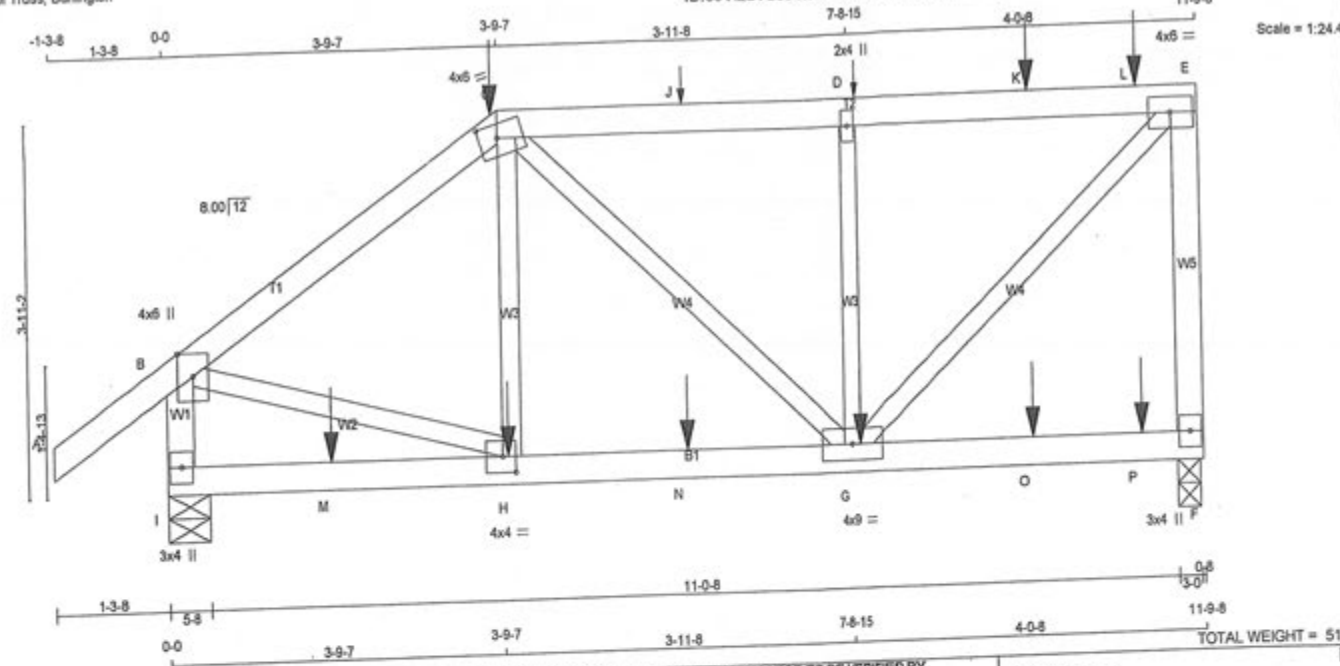
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (E) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DWG NO. TAM 3650 -18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



Scale = 1:24.4

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
C	TTVW-m	MT20	4.0	6.0	1.75	2.50
D	TMVW+r	MT20	2.0	4.0		
E	TMVW-l	MT20	4.0	6.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMVWV-l	MT20	4.0	9.0		
H	BMVW-l	MT20	4.0	4.0	2.00	1.75
I	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

HANGERS NOTES

1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 145.3 lbs FACTORED DOWN AT 3-9-7, 1.0 lbs FACTORED DOWN AND 200.6 lbs FACTORED UP AT 3-9-7, 1.0 lbs FACTORED DOWN AND 200.6 lbs FACTORED UP AT 5-10-3, 1.0 lbs FACTORED DOWN AND 200.6 lbs FACTORED UP AT 7-10-3, AND 54.4 lbs FACTORED DOWN AT 9-10-3, AND 73.8 lbs FACTORED DOWN AT 11-1-4 ON TOP CHORD, AND 11.2 lbs FACTORED DOWN AT 1-10-3, 11.2 lbs FACTORED DOWN AT 3-10-3, 11.2 lbs FACTORED DOWN AT 5-10-3, 11.2 lbs FACTORED DOWN AT 7-10-3, AND 21.4 lbs FACTORED DOWN AT 9-10-3, AND 29.1 lbs FACTORED DOWN AT 11-1-4 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT	VERT	HORZ	UPLIFT	
F	1095	0	0	3-0
I	1199	0	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN SNOW	MAX/MIN LIVE	PERM/LIVE	WIND	DEAD	SOIL
F	868	501 / 0	151 / 0	0 / 0	0 / 0	216 / 0	0 / 0
I	928	568 / 0	137 / 0	0 / 0	0 / 0	223 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, I

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.84 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	FACTORED (PLF)	VERT. LOAD LC1	MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CSI (LC)
FR-TO										
A-B	0 / 47	-124.4 -124.4	0.18 (1)	10.00	H-C	-87 / 128	0.04 (7)			
B-C	-946 / 0	-124.4 -124.4	0.36 (1)	5.84	C-G	0 / 75	0.02 (1)			
C-J	-838 / 0	-124.4 -124.4	0.41 (1)	5.99	G-D	-665 / 0	0.17 (1)			
J-D	-838 / 0	-124.4 -124.4	0.42 (1)	5.99	G-E	0 / 1127	0.28 (1)			
D-K	-838 / 0	-124.4 -124.4	0.42 (1)	5.99	B-H	0 / 815	0.20 (1)			
K-L	-838 / 0	-124.4 -124.4	0.42 (1)	5.99						
L-E	-838 / 0	0.0	0.0	2.25 (1)	7.71					
F-E	-1029 / 0	0.0	0.0	0.13 (1)	7.39					
I-B	-1151 / 0	0.0	0.0	0.13 (1)	7.39					
I-M	0 / 0	-28.0	-28.0	0.13 (7)	10.00					
M-H	0 / 0	-28.0	-28.0	0.13 (7)	10.00					
H-N	0 / 783	-28.0	-28.0	0.23 (7)	10.00					
N-G	0 / 783	-28.0	-28.0	0.23 (7)	10.00					
G-O	0 / 0	-28.0	-28.0	0.15 (7)	10.00					
O-P	0 / 0	-28.0	-28.0	0.15 (7)	10.00					
P-F	0 / 0	-28.0	-28.0	0.15 (7)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.
C	3-9-7	-24	-27	201	FRONT	VERT
C	3-9-7	1	1	201	BACK	VERT
C	3-9-7	-122	-122	201	FRONT	VERT
C	3-9-7	1	1	201	BACK	VERT
D	7-10-3	-6	-11	201	FRONT	VERT
G	7-10-3	-6	-11	201	BACK	VERT
H	3-10-3	-6	-11	201	FRONT	VERT
J	5-10-3	-6	-11	201	BACK	VERT
K	9-10-3	-54	-54	201	FRONT	VERT
L	11-1-4	-74	-74	201	BACK	VERT
M	1-10-3	-6	-11	201	FRONT	VERT
N	5-10-3	-6	-11	201	BACK	VERT
O	9-10-3	-12	-21	201	FRONT	VERT
P	11-1-4	-17	-29	201	BACK	VERT



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
 ALLOWABLE DEFL.(TL) = L/360 (0.39")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.42/1.00 (D-E:1), BC=0.23/1.00 (G-H:7), WB=0.28/1.00 (E-G:1), SS=0.31/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

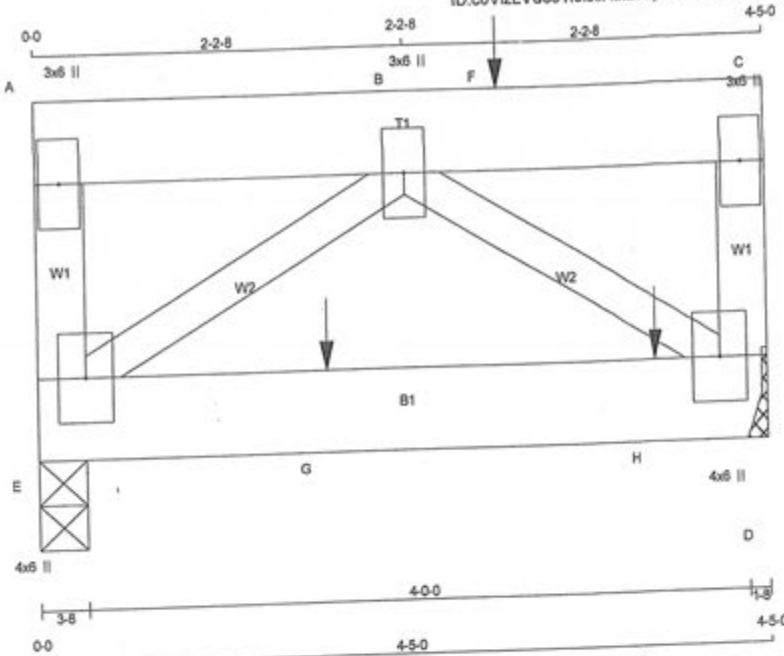
PLATE	GRIP(DRY)	SHEAR	SECTION
MT20	618	354	1667
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (B) (INPUT = 0.90)
 JSI METAL= 0.29 (H) (INPUT = 1.00)

DRWG NO. TAM 3651 -18
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 2 X 22 = 44 lb

LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 E - A 2x4 DRY No.2 SPF
 A - C 2x4 DRY No.2 SPF
 D - C 2x4 DRY No.2 SPF
 E - D 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS SURFACE SPACING (IN) LOAD (PLF)

TOP CHORDS : (0.122"x3") SPIRAL NAILS
 E-A 1 12 TOP
 C-D 1 12 TOP
 A-C 2 12 TOP
 BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS
 E-D 2 12 SIDE (0.0)
 WEBS : (0.122"x3") SPIRAL NAILS
 2x3 1 6

NAILED TO BE DRIVEN FROM ONE SIDE ONLY.
 GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 30 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	6.0		
B	TMWW+t	MT20	3.0	6.0		
C	TMV+p	MT20	3.0	6.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW1+p	MT20	4.0	6.0		

HANGERS NOTES
 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 972.1 lbs FACTORED DOWN AT 2-9-4 ON TOP CHORD, AND 1019.6 lbs FACTORED DOWN AT 1-8-12, AND 1024.0 lbs FACTORED DOWN AT 3-8-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG UPLIFT	REQD BRG IN-SX
JT	1479	0	1479	0	3-8
E	1479	0	1479	0	3-8
D	2210	0	2210	0	3-8

HANGER BY OTHERS
 MIN. SEAT SIZE: 1-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX MIN SNOW	MAX MIN LIVE	PERM LIVE	WIND	DEAD	SOIL
JT	1165	680 / 0	197 / 0	0 / 0	0 / 0	289 / 0	0 / 0
E	1165	680 / 0	197 / 0	0 / 0	0 / 0	289 / 0	0 / 0
D	1743	1015 / 0	296 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED VERT. LOAD LC1 (LC)	MAX. MEMB. UNBRACED LENGTH FR-TO	WEBS	
					MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LC)
FR-TO						
E-A	-81 / 0	0.0	0.00 (1)	7.81	E-B	-971 / 0
A-B	0 / 0	-124.4	-124.4 0.05 (1)	10.00	B-D	-971 / 0
B-F	0 / 0	-124.4	-124.4 0.14 (1)	10.00		
F-C	0 / 0	-124.4	-124.4 0.14 (1)	10.00		
D-C	-329 / 0	0.0	0.02 (1)	7.81		
E-G	0 / 796	-28.0	-28.0 0.57 (1)	10.00		
G-H	0 / 796	-28.0	-28.0 0.57 (1)	10.00		
H-D	0 / 796	-28.0	-28.0 0.57 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
JT	2-9-4	-972	-972		TOP	VERT	TOTAL
G	1-8-12	-1020	-1020		BACK	VERT	TOTAL
H	3-8-12	-1024	-1024		BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12
 THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55% OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
 ALLOWABLE DEFL. (TL) = L/360 (0.19")
 CALCULATED VERT. DEFL. (TL) = L/750 (0.07")

CSI: TC=0.14/1.00 (B-C-1), BC=0.57/1.00 (D-E-1), WB=0.09/1.00 (B-E-1), SS=0.44/1.00 (D-E-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 822 2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.48 (E) (INPUT = 0.90)
 SSI METAL= 0.13 (E) (INPUT = 1.00)



DWG NO. TAM 3652-18
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 2 X 29 = 58 lb

DESIGN CONSISTS OF 2 TRUSSES BUILT
SEPARATELY THEN FASTENED TOGETHER AS
FOLLOWS:

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

HANGERS NOTES

- 1) SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 1212.9 lbs FACTORED DOWN AT 11'-4" AND 3366.5 lbs FACTORED DOWN AT 3'-0" AND 2086.4 lbs FACTORED DOWN AT 4'-11" ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
E	3-0-0	-3366	-3366	—	BACK	VERT	TOTAL
G	11-4	-1213	-1213	—	BACK	VERT	TOTAL
H	4-11-4	-2086	-2086	—	BACK	VERT	TOTAL



DESIGN CRITERIA

SPECIFIED LOADS:

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF OBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.15/1.00 (A-F:1), BC=0.69/1.00 (D-E:1),
WB=0.50/1.00 (B-D:1), SSI=0.46/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	822	2284	1656

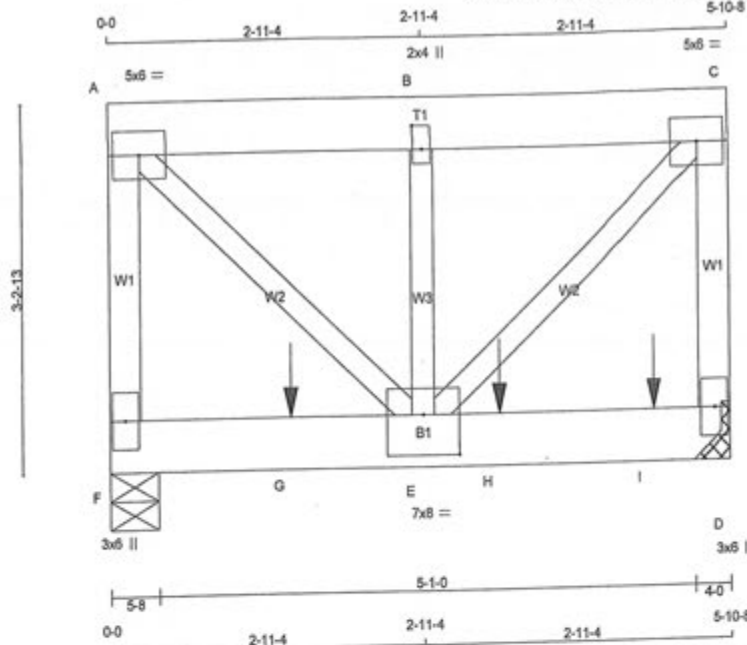
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.87 (B) (INPUT = 0.90)
JSI METAL= 0.46 (A) (INPUT = 1.00)

DWG NO. TAM 3645-18
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	No. 2	DESCR.	SPF
F - A	2x4	DRY	No. 2	SPF			
A - C	2x6	DRY	No. 2	SPF			
D - C	2x4	DRY	No. 2	SPF			
F - D	2x6	DRY	No. 2	SPF			
ALL WEBS	2x3	DRY	No. 2	SPF			
DRY: SEASONED LUMBER.							

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
F - A 1	12	TOP
C - D 1	12	TOP
A - C 2	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
F - D 2	12	SIDE (0.0)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-1	MT20	5.0	6.0		
B	TMVW-1	MT20	2.0	4.0	2.50	1.00
C	TMVW-1	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-1	MT20	7.0	8.0	4.25	4.00
F	BMV1+p	MT20	3.0	6.0		

HANGERS NOTES

- SPECIAL HANGER(S) OR CONNECTION(S) REQUIRED TO SUPPORT CONCENTRATED LOAD(S) 3102.1 lbs FACTORED DOWN AT 1-8-8, AND 1914.8 lbs FACTORED DOWN AT 3-7-12, AND 1918.6 lbs FACTORED DOWN AT 5-1-12 ON BOTTOM CHORD. DESIGN FOR UNSPECIFIED CONNECTION(S) IS DELEGATED TO THE BUILDING DESIGNER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	HORZ	DOWN	UPLIFT
F	3612	0	3612	0
D	4218	0	4218	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX / MIN SNOW	LIVE	PERM. LIVE	DEAD	SOIL
F	2836	1672 / 0	468 / 0	0 / 0	696 / 0	0 / 0
D	3314	1951 / 0	549 / 0	0 / 0	814 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CS1 (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						FR-TO			
F-A	-2710 / 0	0.0	0.0	0.21 (1)	6.93	A-E	0 / 3578	0.44 (1)	
A-B	-2601 / 0	-124.4	-124.4	0.10 (1)	6.25	E-B	-226 / 24	0.02 (1)	
B-C	-2601 / 0	-124.4	-124.4	0.10 (1)	6.25	E-C	0 / 3578	0.44 (1)	
D-C	-2710 / 0	0.0	0.0	0.21 (1)	6.93				
F-G	0 / 0	-28.0	-28.0	0.55 (1)	10.00				
G-E	0 / 0	-28.0	-28.0	0.55 (1)	10.00				
E-H	0 / 0	-28.0	-28.0	0.47 (1)	10.00				
H-I	0 / 0	-28.0	-28.0	0.47 (1)	10.00				
I-D	0 / 0	-28.0	-28.0	0.47 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
G	1-8-8	-3102	-3102	---	BACK	VERT	TOTAL
H	3-7-12	-1915	-1915	---	BACK	VERT	TOTAL
I	5-1-12	-1919	-1919	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 34.8	PSF
	DL = 8.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	= 60.3	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF OBC 2012, CBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.04")

CS1: TC=0.21/1.00 (A-F:1), BC=0.55/1.00 (E-F:1), WB=0.44/1.00 (A-E:1), SS=0.80/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

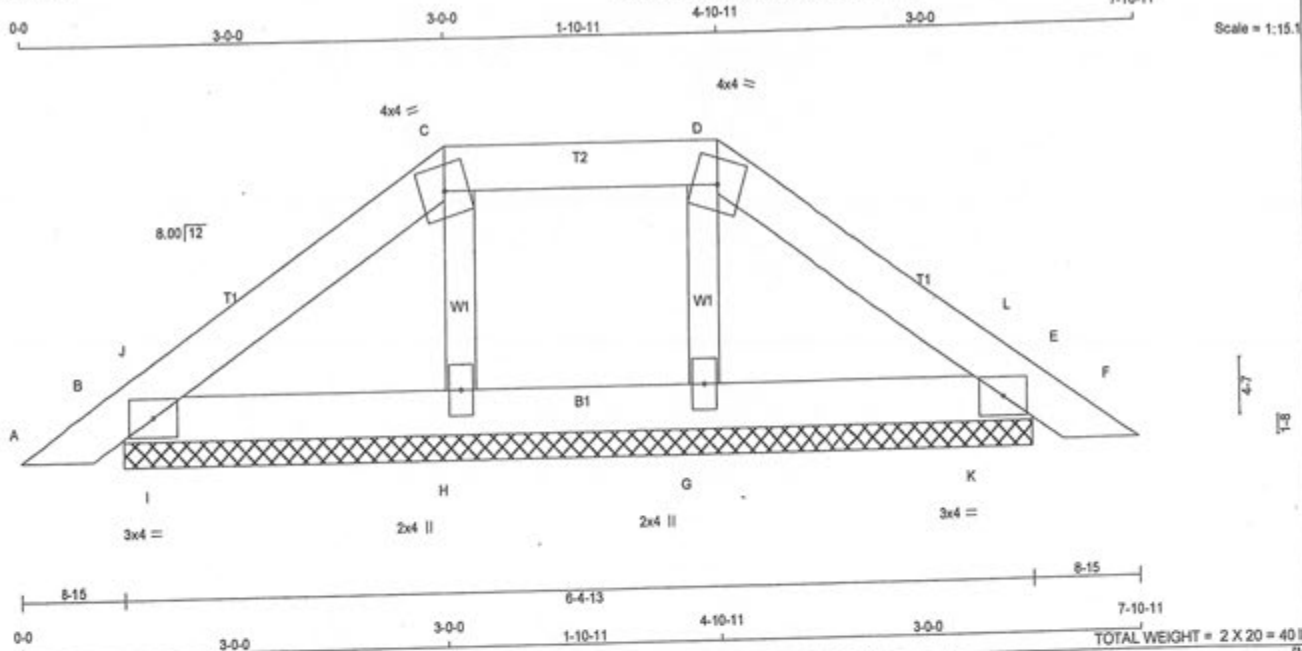
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
 JSI METAL= 0.32 (C) (INPUT = 1.00)



DWG NO. TAM 3653-112
 STRUCTURAL
 COMPONENT ONLY



LUMBER
 N. L. G. A. RULES
 CHORDS SIZE LUMBER DESCR.
 A - C 2x4 DRY No.2 SPF
 C - D 2x4 DRY No.2 SPF
 D - F 2x4 DRY No.2 SPF
 B - E 2x4 DRY No.2 SPF
 ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ UPLIFT		
B	292	0	292	0	6-4-13	6-4-13
E	292	0	292	0	6-4-13	6-4-13
H	278	0	278	0	6-4-13	6-4-13
G	278	0	278	0	6-4-13	6-4-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	219	145/0	24/0	0/0	0/0	49/0	0/0
E	219	145/0	24/0	0/0	0/0	49/0	0/0
H	224	123/0	44/0	0/0	0/0	57/0	0/0
G	224	123/0	44/0	0/0	0/0	57/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 CSI (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)	
					H-C	G-D
FR-TO						
A-B	0/21	-124.4	-124.4	0.04 (1)	10.00	H-C -191/0 0.03 (1)
B-J	-69/0	-124.4	-124.4	0.01 (3)	6.25	G-D -191/0 0.03 (1)
J-C	-82/0	-124.4	-124.4	0.08 (1)	6.25	I-J -160/17 0.00 (1)
C-D	-53/0	-124.4	-124.4	0.08 (1)	6.25	K-L -160/17 0.00 (1)
D-L	-82/0	-124.4	-124.4	0.08 (1)	6.25	
L-E	-69/0	-124.4	-124.4	0.01 (3)	6.25	
E-F	0/21	-124.4	-124.4	0.04 (1)	10.00	
B-I	0/66	-28.0	-28.0	0.08 (1)	10.00	
I-H	0/66	-28.0	-28.0	0.04 (1)	10.00	
H-G	0/53	-28.0	-28.0	0.04 (1)	10.00	
G-K	0/66	-28.0	-28.0	0.08 (1)	10.00	
K-E	0/66	-28.0	-28.0	0.08 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 34.8 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2012, BCBC 2012, ABC 2014
 - CSA 086-09
 - TPIC 2011

(55 % OF 48.1 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (C-D-1), BC=0.08/1.00 (G-K-1), WB=0.03/1.00 (D-G-1), SS=0.13/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 822 2284 1656

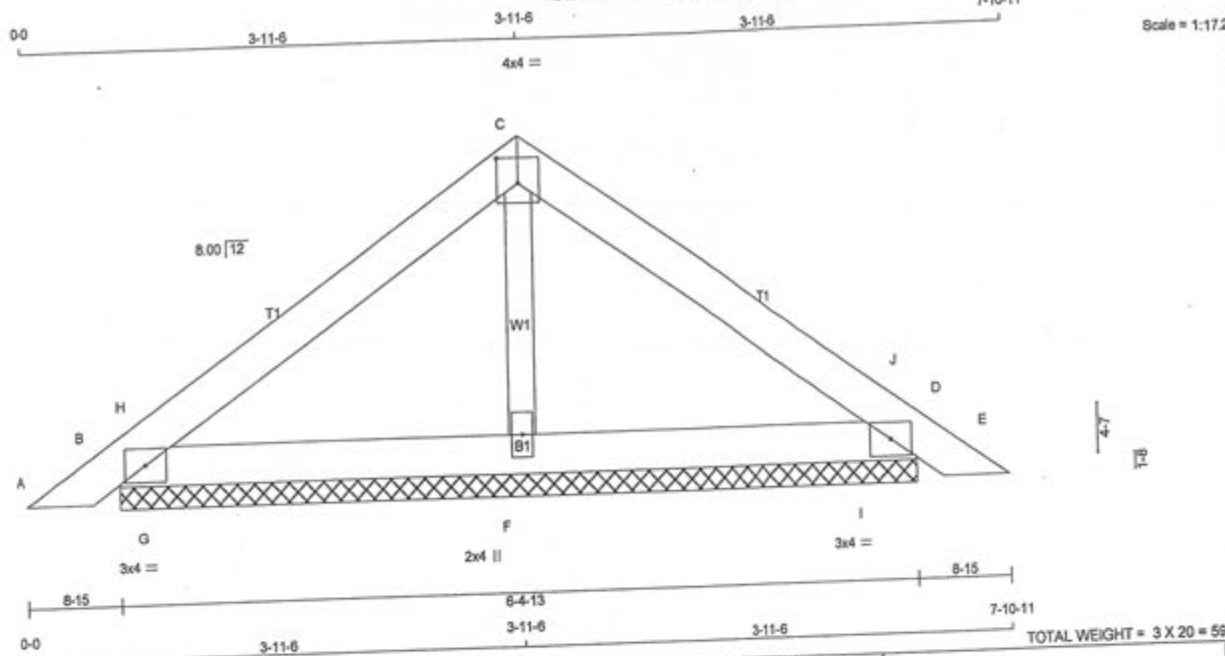
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (E) (INPUT = 0.90)
 JSI METAL= 0.05 (E) (INPUT = 1.00)



DWG NO. TAM 3633-18
 STRUCTURAL
 COMPONENT ONLY



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY; SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-J	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TMB1-I	MT20	3.0	4.0	
F	BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	374	0	374	0	6-4-13	6-4-13
D	374	0	374	0	6-4-13	6-4-13
F	392	0	392	0	6-4-13	6-4-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
		SNOW	LIVE			
B	281	187 / 0	31 / 0	0 / 0	63 / 0	0 / 0
D	281	187 / 0	31 / 0	0 / 0	63 / 0	0 / 0
F	324	184 / 0	73 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS	
					MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO						
A-B	0 / 21	-124.4 -124.4	0.04 (1)	10.00	F-C	-181 / 0
B-H	-82 / 0	-124.4 -124.4	0.06 (1)	6.25	G-H	-345 / 3
H-C	-142 / 0	-124.4 -124.4	0.15 (1)	6.25	I-J	-345 / 3
C-J	-142 / 0	-124.4 -124.4	0.15 (1)	6.25		
J-D	-82 / 0	-124.4 -124.4	0.06 (1)	6.25		
D-E	0 / 21	-124.4 -124.4	0.04 (1)	10.00		
B-G	0 / 111	-28.0 -28.0	0.15 (1)	10.00		
G-F	0 / 111	-28.0 -28.0	0.15 (1)	10.00		
F-I	0 / 111	-28.0 -28.0	0.15 (1)	10.00		
I-D	0 / 111	-28.0 -28.0	0.15 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 34.8 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 60.3 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2012, BCBC 2012, ABC 2014
- CSA 086-09
- TPIC 2011

(55 % OF 48.1 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 34.8 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.15/1.00 (C-J-1), BC=0.15/1.00 (F-I-1),
WD=0.03/1.00 (C-F-1), SSI=0.26/1.00 (B-G-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 0.50

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 822	2284 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.9C)
JSI METAL = 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 3634 -18
STRUCTURAL
COMPONENT ONLY

LUS – Double Shear Joist Hangers

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

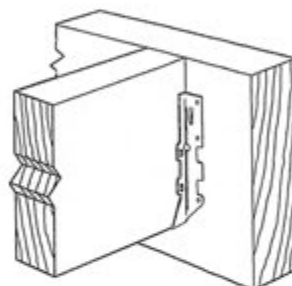
Finish: G90 galvanized

Design:

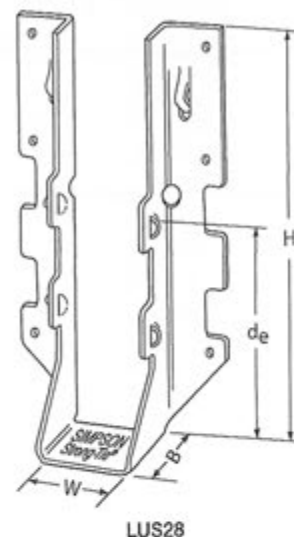
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



Typical LUS Installation



Options:

- These hangers cannot be modified

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1½	3½	1¼	1½	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4¾	1¼	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4¾	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4¾	4¾	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6¾	1¼	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4¾	6¾	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1¼	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4¾	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until June 30, 2019, and reflects information available as of March 1, 2017. This information is updated periodically and should not be relied upon after June 30, 2019. Contact Simpson Strong-Tie for current information and limited warranty or see strongtie.com.

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T-SPECLUS17 3/17 exp. 6/19

(800) 999-5099
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HGUS – Double Shear Joist Hangers

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

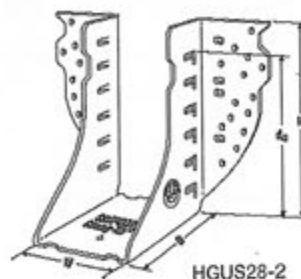
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

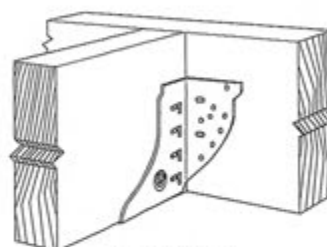
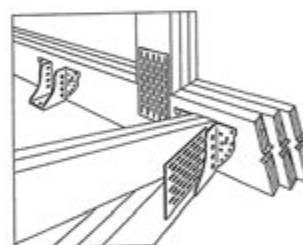
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



HGUS28-2

Typical HGUS
InstallationTypical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d ₁ ¹	Face	Joist	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
HGUS26	12	1½	5½	5	4½ ₃₂	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6½	7½	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6½	9½	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6½	10½	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6½	12½	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

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T-SPECHGUS17 3/17 exp. 6/19

(800) 999-5099
strongtie.com

HUS/LJS – Double Shear Joist Hangers

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

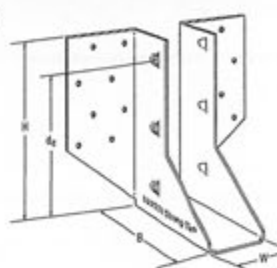
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

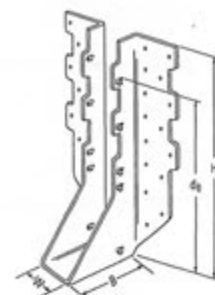
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

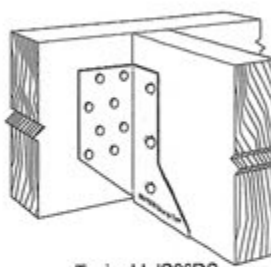
- See current catalogue for options



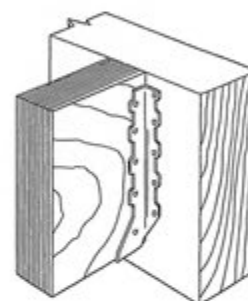
LJS26DS



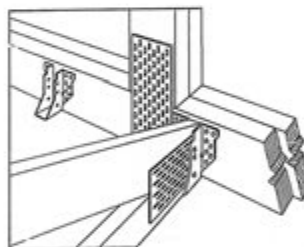
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)	Uplift (K ₀ =1.15)	Normal (K ₀ =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 3/8	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/8	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	.8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



**LIMIT
STATES
DESIGN**

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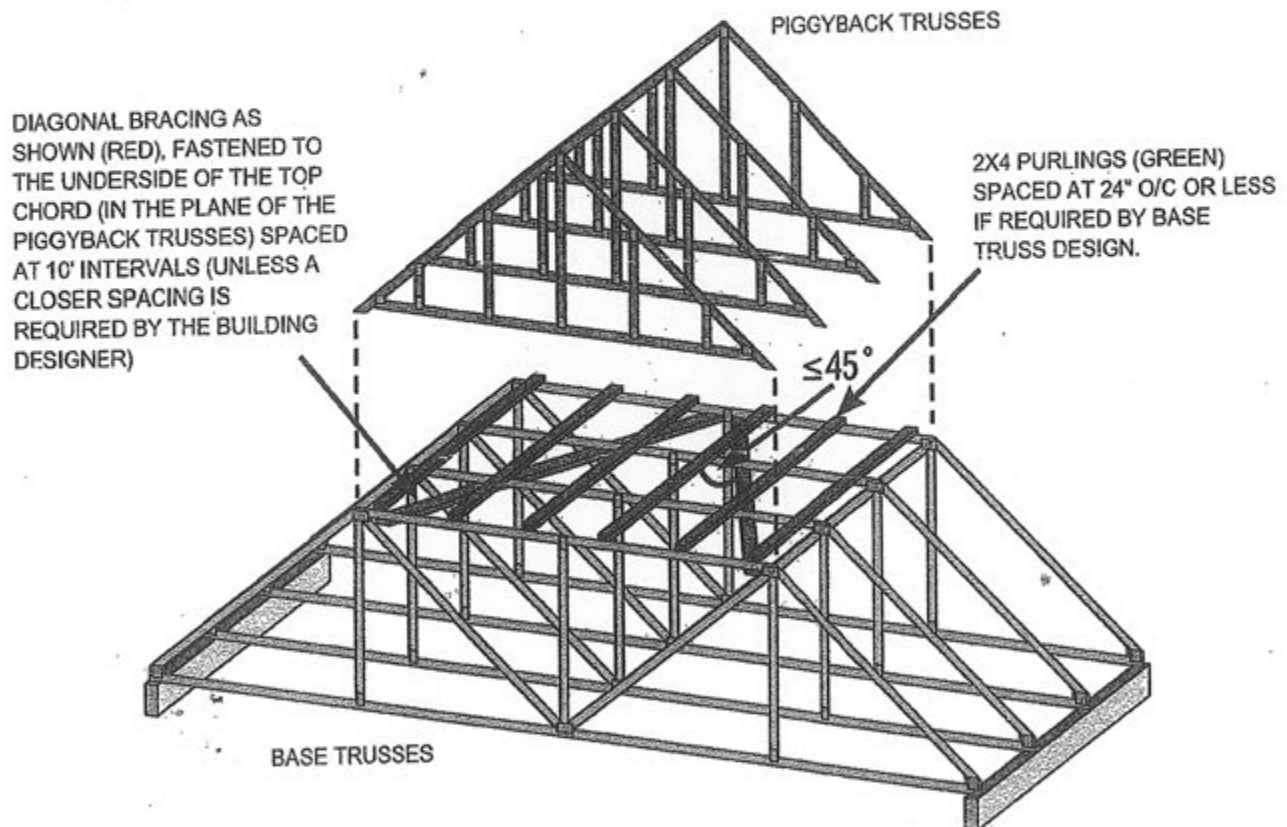
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Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

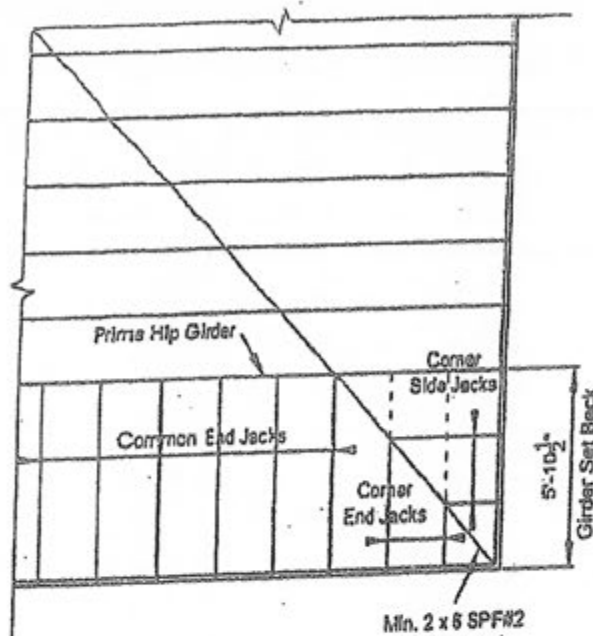
SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

MICRO CITY ENGINEERING SERVICES INC.

TEL: (519) 287 - 2242



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2

BOTTOM CHORD : 2 x 4 SPF#2

WEBS : 2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.

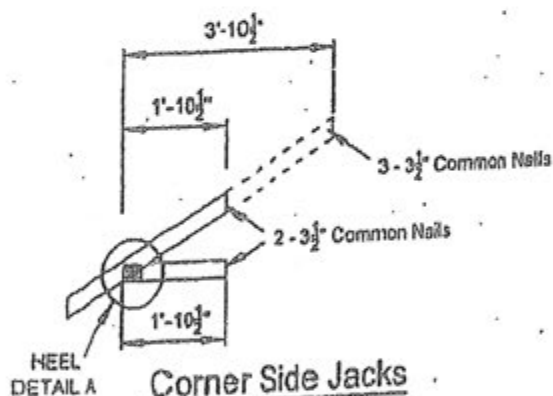
TOP CHORD DEAD LOAD : 3.0 P.S.F.

BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.

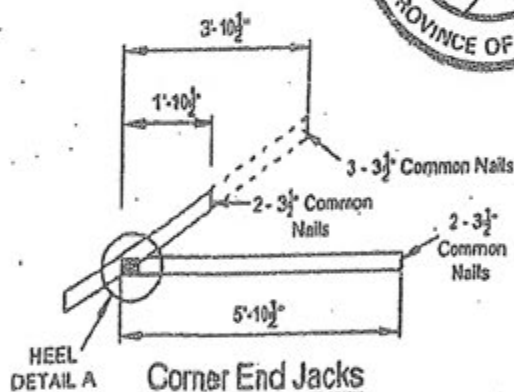
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 44.8 P.S.F.

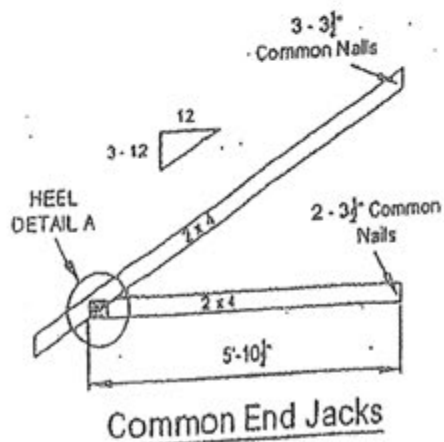
DWG NO T&M 3495.14
STRUCTURAL
COMPONENT ONLY



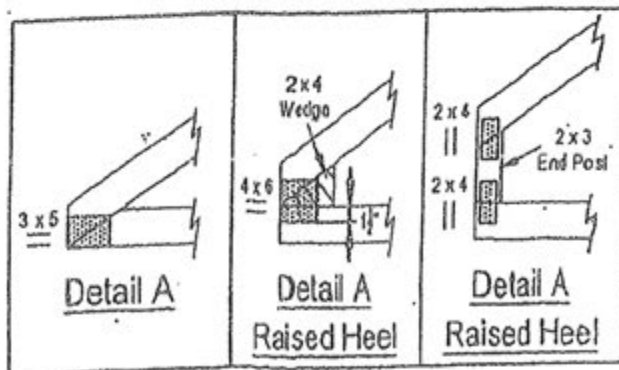
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

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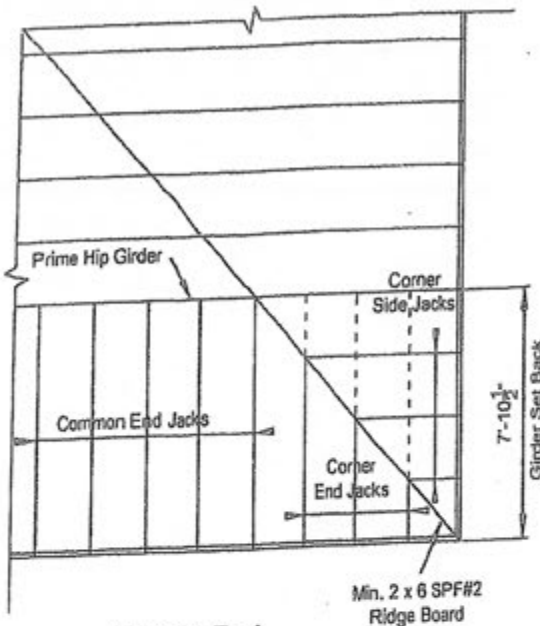
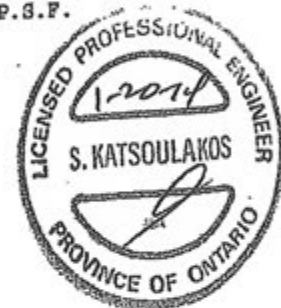
R.R. #1, P.O. BOX 61, GLENCOE, ONTARIO, N0L 1M0

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

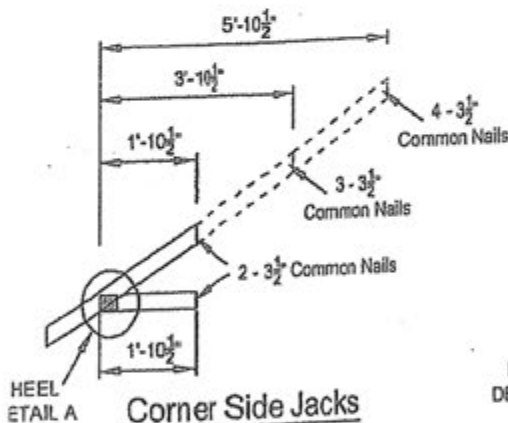
DESIGN LOAD:

TOP CHORD LIVE LOAD : 34.8 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 44.8 P.S.F.

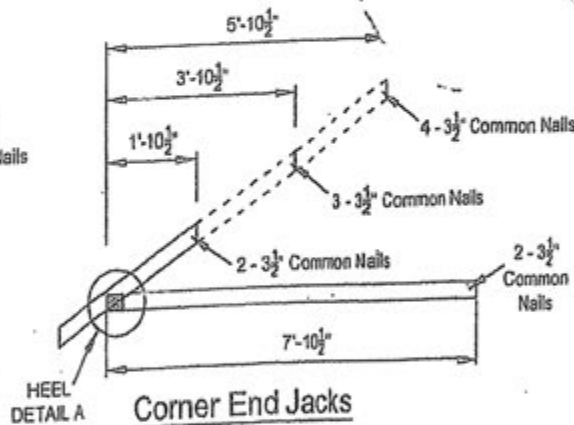


45° Hip End

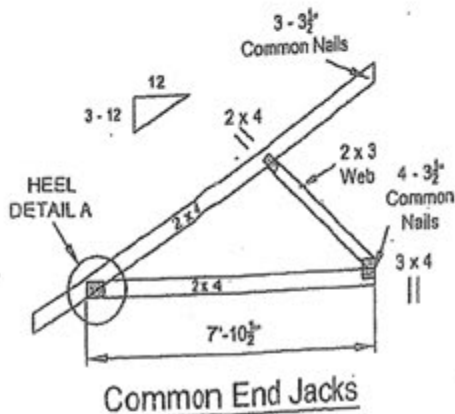
DWG NO T&M 3503.14
STRUCTURAL
COMPONENT ONLY



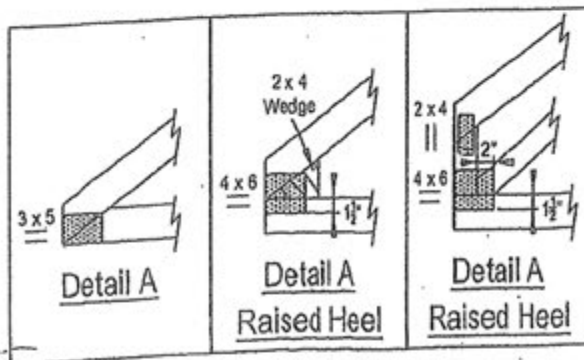
Corner Side Jacks



Corner End Jacks



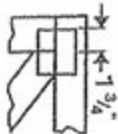
Common End Jacks



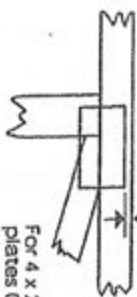
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (LIMIT STATES DESIGN)
(TO BE INCLUDED AND USED AS PART OF A FULL TRUSS ENGINEERING PACKAGE)

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

* Plate location details available in Mitek software or upon request.

PLATE SIZE

4 X 4

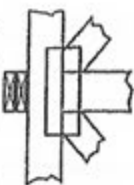
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

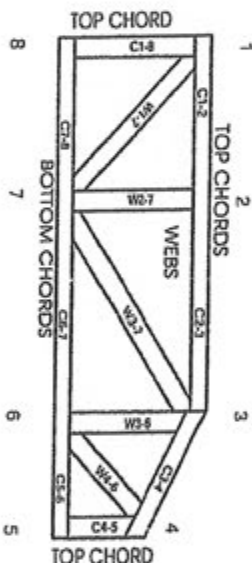


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:
TPIC:
Truss Design Procedures and Specifications
for Light Metal Plate Connected Wood Trusses
DSB-89:
Design Standard for Bracing.
BCSI:
Building Component Safety Information,
Guide to Good Practice for Handling,
Installing & Bracing of Metal Plate
Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10318-L, 13270-L, 12691-R

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Mitek
POWER TO PERFORM.™

Mitek Engineering Reference Sheet Mtl-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

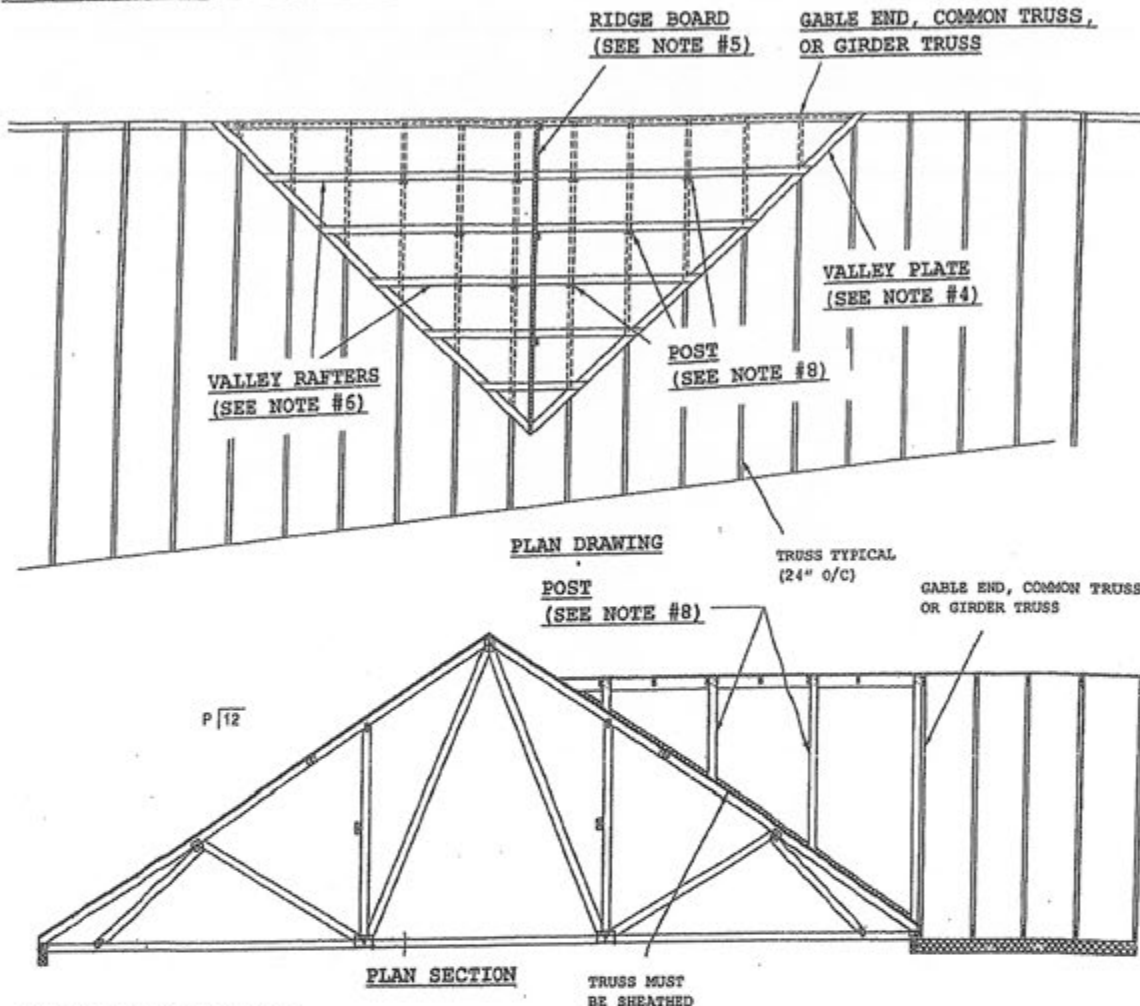
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and waste at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or pultrus provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

MICRO CITY ENGINEERING SERVICES INC.

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CONVENTIONAL VALLEY FRAMING DETAIL



GENERAL SPECIFICATIONS:

- (1) WITH THE BASE TRUSSES ERECTED (INSTALLED), APPLY SHEATHING TOP CHORD OF SUPPORTING (BASE) TRUSSES.
- (2) BRACE BOTTOM CHORD AND WEB MEMBERS AS PER PRE-ENGINEERED TRUSS DESIGNS.
- (3) DEFINE VALLEY RIDGE BY RUNNING A LEVEL STRING FROM THE INTERSECTING RIDGE OF THE (a) GABLE END, (b) GIRDER TRUSS OR (c) COMMON TRUSS TO THE ROOF SHEATHING.
- (4) INSTALL 2 X 6 VALLEY PLATES ON FLAT. FASTEN TO EACH SUPPORTING TRUSS WITH (2) 16d (3.5" X 0.131") NAILS.
- (5) SET A 2 X 6 #2 RIDGE BOARD (MAX. 10'-0" RIDGE) OR 2 X 8 #2 SPF RIDGE BOARD (MAX. 20'-0" RIDGE). SUPPORT RIDGE BOARD WITH 2 X 4 POSTS SPACED 48" O/C. BEVEL BOTTOM OF POST TO SET EVENLY ON THE SHEATHING. FASTEN POST TO RIDGE WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST TO ROOF SHEATHING WITH (3) 10d (3" X 0.131") TOE-NAILS.
- (6) FRAME VALLEY RAFTERS FROM VALLEY PLATE TO RIDGE BOARD. MAXIMUM RAFTER SPACING IS 24" O/C. FASTEN VALLEY RAFTER TO RIDGE BEAM WITH (3) 16d (3.5" X 0.131") TOE-NAILS. FASTEN VALLEY RAFTER TO VALLEY PLATE WITH (3) 16d (3.5" X 0.131") TOE-NAILS.
- (7) SUPPORT THE VALLEY RAFTERS WITH 2 X 4 POSTS AT 48" O/C (OR LESS) ALONG EACH RAFTER. INSTALL POSTS IN A STAGGERED PATTERN AS SHOWN ON PLAN DRAWING. ALIGN POSTS WITH TRUSSES BELOW. FASTEN VALLEY RAFTER TO POST WITH (4) 10d (3" X 0.131") NAILS. FASTEN POST THROUGH SHEATHING TO SUPPORTING TRUSSES WITH (2) 16d (3.5" X 0.131") NAILS.
- (8) POSTS SHALL BE 2 X 4 #2 SPF OR BETTER. POSTS EXCEEDING 75" IN HEIGHT SHALL BE INCREASED TO 4 X 4 #2 SPF, OR BETTER, OR BE PRE-ASSEMBLED TWO (2) PLY 2 X 4 #2 SPF OR BETTER FASTENED TOGETHER WITH 2 ROWS OF 10d (3" X 0.131") NAILS AT 6" O/C.
- (9) MAINTAIN A MINIMUM 3/4" LUMBER EDGE DISTANCE WHEN NAILING. NAIL SPACING SHOULD APPROXIMATE A MINIMUM 1-3/4" O/C OR MORE UNLESS NOTED OTHERWISE. ALL CONSTRUCTION TO CONFORM TO ONTARIO BUILDING CODE (CURRENT ADDITION) AT ALL TIMES.

NOTES:

- (10) 48" O/C (MAXIMUM POST SPACING).
- (11) ROOF LIVE LOAD = 34.8 PSF (MAX.)
- (12) ROOF DEAD LOAD = 10.0 PSF (MAX.)
- (13) PART 9 APPLICATION ONLY (ONTARIO BUILDING CODE)
- (14) PART 4 APPLICATION ONLY (ONTARIO BUILDING CODE) WITH APPROVED REVIEW BY LICENSED PROFESSIONAL ENGINEER.
- (15) BASE TRUSS SPACING (24" O/C MAX.)
- (16) ALL PRE-ENGINEERED BASE TRUSS COMPONENTS TO BE SEALED BY LICENSED PROFESSIONAL ENGINEER AND THIS DETAIL TO BE VERIFIED AND APPROVED BY SAME WHEN RIDGE BOARD LENGTH EXCEEDS 12'-0".
- (17) ALL BASE TRUSSES: P = 4 (4/12) - MINIMUM.
- (18) ALL VALLEY RAFTERS: P = 4 (4/12) - MINIMUM.



DWG NO T&M 6305.14
STRUCTURAL
COMPONENT ONLY

Micro City Engineering Services Inc.
(BCIN: 26064; FIRM BCIN: 29991)

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Responsibilities:

Micro City Engineering Services is responsible for the design of trusses as individual components.

It is the responsibilities of others to ascertain that the design loads utilized on this (these) drawing(s) meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdiction over such decisions.

All dimensions are to be verified by the owner, contractor, architect, or other authority having input over such decisions prior to truss component manufacture. At no time shall Micro City Engineering Services Inc. or its employees be responsible for dimension errors.

Micro City Engineering Services Inc. bears no responsibility for the erection of any truss components. Persons erecting truss components are cautioned to seek professional advice regarding temporary and permanent bracing systems and to be **totally** familiar with all aspects of truss erection prior to proceeding on **any** truss component erection job. Any bracing shown on Micro City Engineering Services Inc. or Tamarack Roof Trusses Inc. sealed or unsealed truss component drawings is specified for the single truss component in question and is identified as an integral part of the design for that particular truss component but is **not** meant to represent the only required bracing for that particular truss component when installed as a component in a series of truss components in a roof truss system.

It is the truss manufacturer's responsibility to ensure that trusses are manufactured in accordance with Micro City Engineering Services Inc. specifications outlined below:

SPECIFICATIONS:

Truss components sealed by Micro City Engineering Services Inc. must conform to the relevant sections of the current Building Code of Ontario and Canada (Part 4 or Part 9) or the current Farm Building Code of Canada in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the Truss Plate Institute of Canada (TPIC). All unit lumber and nailing stresses identified on truss component design drawings and/or used in the design of individual truss components shall conform to the current CSA Wood Design standard identified in the current Building Code and TPIC Design Standards.

The lumber used to manufacture any truss component is to conform to the specified size and grade identified on the truss drawing.

The lumber used in the manufacture of any truss component is not to exceed 19% during its service use unless specifically noted on the truss drawing.

The lumber used in the manufacture of any truss component is not to be treated with any chemicals during its service life unless specifically noted on the truss drawing.

Connector plates shall be applied to both faces of the truss component at each joint and shall be positioned exactly as specified.

The top chord of any truss component is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the sealed truss component drawing but not exceeding 24" o/c (Part 9 design) and not exceeding 48" o/c (Part 4 or Agricultural design).

When a truss component is to be installed with no rigid ceiling attached directly to the bottom chord, then the bottom chord is to be laterally braced at intervals not exceeding 3m (or 10'-0").

All sealed or unsealed truss component drawings provided by Micro City Engineering Services Inc. Or Tamarack Roof Trusses Inc. should be read in conjunction with the following:

Warning-Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473C rev 10-'08 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of the building designer - not the truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection, and bracing, consult TPIC Appendix G - Minimum Quality Manufacturing Criteria available from www.tpica.ca and BCSI Building Component Safety Information available from the Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA, 22314.