

**9/12 ROOF PITCH
UNLESS OTHERWISE SPECIFIED**

REVIEWED BY J. S. SNELL
DATE JULY 23/19
FILE # BARTON 1 - ELEV 1, 2

BM1,2:2-2X10

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO _____

12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:

GS LOAD	1.8	KPA
TC DEAD	6	PSF
BC LIVE	10.5	PSF
BC DEAD	7	PSF

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4" SPF @24"o.c. WITH A 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance between end points and between rows of bracing does not exceed 6'.

**TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)**

HARDWARE

HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V)
LUS24(O)

**DENOTES
CONVENTIONAL
FRAMING**

T- ...

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

APPLICANT COPY

50-08-00

52-00-00

M12244

Job Track: **50033**Plan Log: **201007**

Layout ID: 402938

Builder / Location:

GREEN PARK HOMES / CALEDON

Project: **LAMBERT LANE PH.2**

Date: 5/3/2019

Sales: **Mario DiCano**

Designer: JG

Model / Elevation:

BARTON 1/1

Mitek ver 8.2.3.229

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



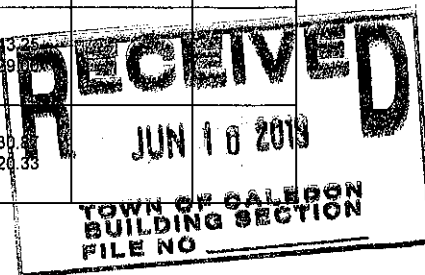
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 1
 Lot #:
 Elevation: 1

Job Track: 50033
 PlanLog: 201007
 Layout ID: 402938
 Ref #
 Page: 1 of 2
 Date: 05/03/2019
 Designer:
 Sales Rep: Mario DiCano

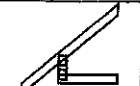
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9 /12	27-00-00	4-01-08	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.67 182.00		
	1 2-ply	T12 Hip Girder	9 /12	27-00-00	4-01-08	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	289.67 182.00		
	2	T2 Hip	9 /12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	221.33 142.00		
	2	T3 Hip	9 /12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	242.71 157.33		
	2	T4 Hip	9 /12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	245.08 159.00		
	2	T5 Hip	9 /12	27-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	259.05 166.67		
	6	T6 Hip	9 /12	27-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	777.67 492.00		
	2	T7 Hip	9 /12	27-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	264.11 169.33		
	2	T9 Common	9 /12	8-10-00	4-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	79 53.67		
	1	G9 GABLE	9 /12	8-10-00	4-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	40.94 27.83		
	1	T10 Hip Girder	9 /12	11-02-00	4-05-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	61.14 40.00		
	3	T11 Common	9 /12	11-02-00	5-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	144.14 95.00		
	1 2-ply	T12 Jack-Closed Girder	9 /12	3-10-08	3-01-00	2 x 4 2 x 6		1-06-04 3-01-00	13.51 29.00		
	1	T13 Common	6 /12	8-05-08	3-02-03	2 x 4 2 x 6	1-03-08	6-12 9-00	30.00 26.33		



DELIVERY SHIPLIST											
	Lumber Yard:		TAMARACK LUMBER						Job Track:		50033
	Builder:		GREEN PARK HOMES						PlanLog:		201007
	Project:		LAMBERT LANE PH.2						Layout ID:		402938
	Location:		CALEDON						Ref #		
	Model:		BARTON 1						Page:		2 of 2
	Lot #:								Date:		05/03/2019
Elevation:		1							Designer:		
									Sales Rep:		Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T13Z Common Girder	6 /12	8-05-08	3-02-03	2 x 4 2 x 6	1-03-08	6-12 9-00	30.87 20.33		
	2	PB1 Piggyback	9 /12	6-09-05	2-00-00	2 x 4			35.06 25.33		
	2	PB2 Piggyback	9 /12	6-09-05	2-06-08	2 x 4			31.67 21.33		
	19	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	319.1 202.67		
	2	J2 Jack-Open	9 /12	2-01-00	3-01-00	2 x 4	1-03-08	1-06-04 3-01-00	20.35 15.33		
	2	J5 Jack-Open	4 /12	5-06-08	2-07-00	2 x 4	1-03-08	3-15 2-02-02	29.29 18.67		
	4	J7 Jack-Open	10 /12	2-00-08	3-01-14	2 x 4	1-03-08	1-00-07 2-08-14	32.99 24.00		

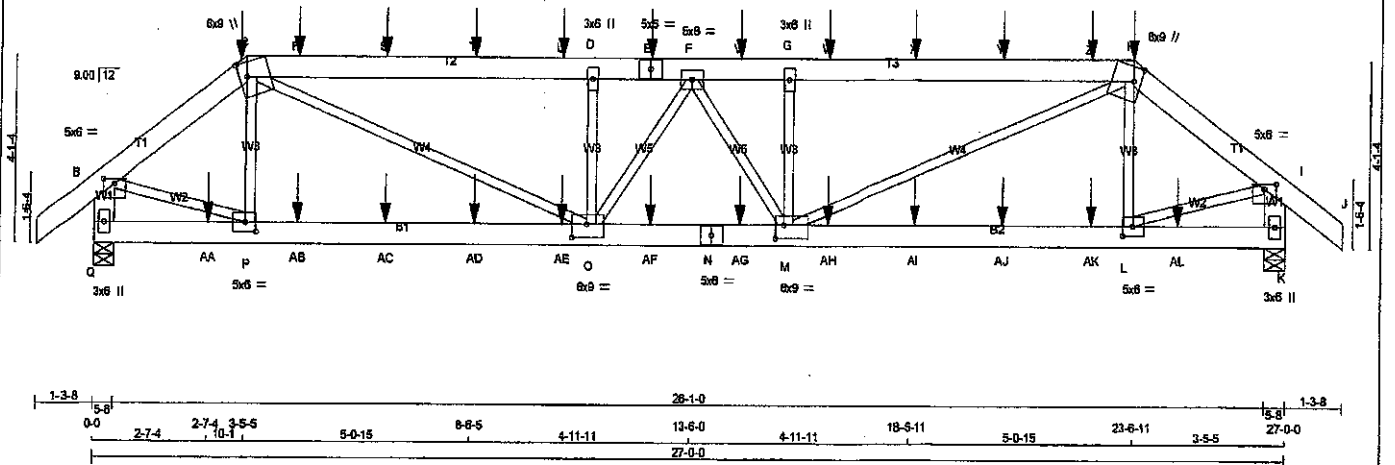
TOTAL # TRUSS= 62 TOTAL BFT OF ALL TRUSSES= 2243.82 BFT. TOTAL WEIGHT OF ALL TRSSES 3487.94 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
4	Hardware	LUS24	
1	Hardware	LUS26-2	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= 9

JOB NAME 402938	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue May 7 17:41:37 2018 Page 1 ID:VdmZNIanRCr_yT1tp?OGrfzKFiU-QKa4qRq2r?DJRrIn4luXgJgVW69mGwVdK2Z2lqj					
1-3.8 0-0 3-5.5 3-5.5 5-0-15 8-8-5 4-11-11 13-6-0 4-11-11 18-5-11 4-1-9 22-7-4 23-6-11 3-5-5 1-3-8 Scale = 1:48.8					



LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES					
A - C	2x6	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
J - K	2x6	DRY	No.2	SPF	
K - L	2x6	DRY	No.2	SPF	
L - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 2 12		SIDE(122.0)
C-E 2 12		SIDE(61.0)
E-H 2 12		SIDE(61.0)
H-J 2 12		SIDE(122.0)
J-K 2 12		TOP
K-L 2 12		TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
Q-N 2 12		SIDE(0.0)
N-K 2 12		SIDE(0.0)

WEBS : (0.122"x3") SPIRAL NAILS
2x3 1 6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.25	3.00
C	TTWW-m	MT20	8.0	9.0	3.75	2.00
D	TMW-v	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMW-w-t	MT20	5.0	6.0		
G	TMW-v	MT20	3.0	6.0		
H	TTWW-m	MT20	8.0	9.0	3.75	2.00
I	TMW-v	MT20	5.0	8.0	1.25	3.00
K	BMV1-p	MT20	3.0	8.0		
L	BMW-v-t	MT20	5.0	8.0	2.50	2.75
M	BMW-v-t	MT20	6.0	9.0	3.50	2.25
N	BS-t	MT20	5.0	8.0		
O	BMW-v-t	MT20	6.0	9.0	3.50	4.25
P	BMW-v-t	MT20	5.0	8.0	2.50	2.75
Q	BMV1-p	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
Q	3276	0	0	3276	0	0	0	5-8	5-8
K	3292	0	0	3292	0	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX. MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
Q	2437	1381 / 0	482 / 0	0 / 0	594 / 0
K	2449	1388 / 0	485 / 0	0 / 0	597 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

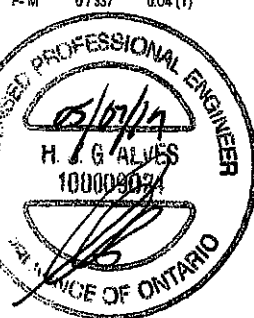
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		VERT. LOAD LC1		MAX. CS1 (LC)		UNBRAC		MEMB.		MAX. FACTORED		FORCE		MAX. CS1 (LC)	
FR-TO	MEMB.	FORCE (LBS)	VERT. (PLF)	FROM	TO	FROM	TO	FROM	TO	FROM	TO	FR-TO	MEMB.	FORCE (LBS)	VERT. (PLF)	FROM	TO	FROM	TO
A-B	0 / 44		-102.1	-102.1	0.04 (1)	10.00						P-C	-310 / 212	0.04 (1)					
B-C	-3581 / 0		-102.1	-102.1	0.07 (1)	5.87						C-O	0 / 3469	0.43 (1)					
C-R	-5994 / 0		-102.1	-102.1	0.44 (1)	4.40						O-D	-1371 / 0	0.16 (1)					
R-S	-5994 / 0		-102.1	-102.1	0.44 (1)	4.40						M-G	-1371 / 0	0.16 (1)					
S-T	-5994 / 0		-102.1	-102.1	0.44 (1)	4.40						M-H	0 / 3458	0.43 (1)					
T-U	-5994 / 0		-102.1	-102.1	0.44 (1)	4.40						L-H	-310 / 215	0.04 (1)					
U-D	-5994 / 0		-102.1	-102.1	0.44 (1)	4.40						B-P	0 / 2965	0.37 (1)					
D-E	-5995 / 0		-102.1	-102.1	0.35 (1)	4.46						L-I	0 / 2978	0.37 (1)					
E-F	-5995 / 0		-102.1	-102.1	0.35 (1)	4.46						O-F	0 / 332	0.04 (1)					
F-V	-5997 / 0		-102.1	-102.1	0.36 (1)	4.46						F-M	0 / 337	0.04 (1)					
V-G	-5997 / 0		-102.1	-102.1	0.36 (1)	4.46													
G-W	-5997 / 0		-102.1	-102.1	0.44 (1)	4.40													
W-X	-5997 / 0		-102.1	-102.1	0.44 (1)	4.40													
X-Y	-5997 / 0		-102.1	-102.1	0.44 (1)	4.40													
Y-Z	-5997 / 0		-102.1	-102.1	0.44 (1)	4.40													
Z-H	-5997 / 0		-102.1	-102.1	0.44 (1)	4.40													
H-I	-3596 / 0		-102.1	-102.1	0.07 (1)	5.85													
I-J	0 / 44		-102.1	-102.1	0.04 (1)	10.00													
Q-B	-3294 / 0		0.0	0.0	0.12 (1)	7.65													
K-I	-3306 / 0		0.0	0.0	0.12 (1)	7.64													

Q-AA	0 / 0	-38.5	-38.5	0.16 (2)	19.00
AA-P	0 / 0	-38.5	-38.5	0.16 (2)	19.00
P-AB	0 / 2844	-38.5	-38.5	0.32 (2)	10.00
AB-AC	0 / 2844	-38.5	-38.5	0.32 (2)	10.00
AC-AD	0 / 2844	-38.5	-38.5	0.32 (2)	10.00
AD-AE	0 / 2844	-38.5	-38.5	0.32 (2)	10.00
AE-O	0 / 2844	-38.5	-38.5	0.32 (2)	10.00
O-AF	0 / 5823	-38.5	-38.5	0.41 (1)	10.00
AF-N	0 / 5823	-38.5	-38.5	0.41 (1)	10.00
N-AG	0 / 5823	-38.5	-38.5	0.41 (1)	10.00
AG-M	0 / 5823	-38.5	-38.5	0.41 (1)	10.00
M-AH	0 / 2857	-38.5	-38.5	0.32 (2)	10.00
AH-AI	0 / 2857	-38.5	-38.5	0.32 (2)	10.00
AI-AJ	0 / 2857	-38.5	-38.5	0.32 (2)	10.00
AJ-AK	0 / 2857	-38.5	-38.5	0.32 (2)	10.00
AK-L	0 / 2857	-38.5	-38.5	0.32 (2)	10.00
L-AL	0 / 0	-38.5	-38.5	0.16 (2)	10.00
AL-K	0 / 0	-38.5	-38.5	0.16 (2)	10.00



DRWG NO. TAM 1910757
STRUCTURAL
CONNECTION ONLY

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.60")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.60")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CS1: TC=0.44/1.00 (C-D:1), BC=0.41/1.00 (M-C:1)
WB=0.43/1.00 (C-O:1), SS=0.25/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (TNSI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

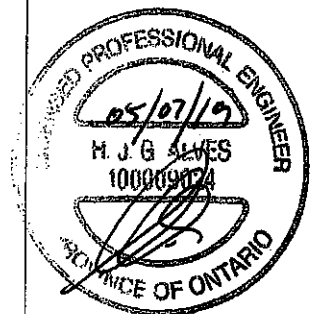
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (I) (INPUT = 0.90)
JSI METAL= 0.53 (N) (INPUT = 1.00)

JOB NAME 402938	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:41:38 2019 Page 2 ID:VdmZNlanRCr_vT1tp?OGrfzKFiu-uW8T1nrgclLa37?ULnG74uLUQwtLuDW4kHata?zlgjh	

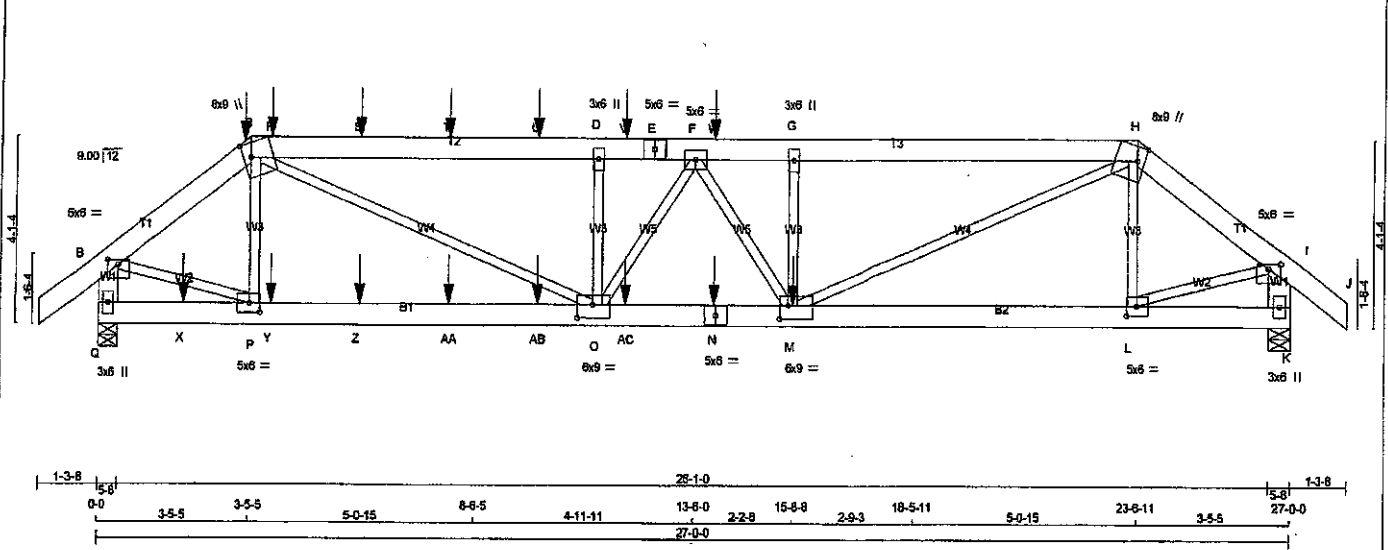
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-44	-49	—	FRONT	VERT	DEAD	—	—
C	3-5-5	-251	-251	—	FRONT	VERT	SNOW	—	—
E	12-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
H	23-6-11	-44	-49	—	FRONT	VERT	DEAD	—	—
H	23-6-11	-251	-251	—	FRONT	VERT	SNOW	—	—
R	4-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
S	6-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
T	8-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
U	10-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
V	14-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
W	16-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
X	18-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
Y	20-7-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
Z	22-7-4	-125	-125	—	FRONT	VERT	TOTAL	—	—
AA	2-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	4-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC	6-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AD	8-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AE	10-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AF	12-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AG	14-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AH	16-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AI	18-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AJ	20-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AK	22-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AL	24-7-4	-55	-70	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM 11710759
STRUCTURAL
COMMENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T1Z	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington	Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:39 2018 Page 1
1-3-8 0-0 3-5-5 3-5-5 11-4 5-11-4 2-0-0 2-0-0 7-11-4 8-8-5 4-11-11 13-6-0 4-11-11 18-5-11 5-0-15 23-6-11 3-5-5 27-0-0 1-3-8	ID:VdmZNIanRCr_yT1p?OGfzKFiu-MirF7sJNcTRh9aguVoMc5teMJApdcRzXJQ7SzIqjg
Scale = 1:48.8	



CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x6	DRY	No.2	SPF
Q - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
Q - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS EXCEPT
2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C 2 12		SIDE(122.0)
C - E 2 12		SIDE(61.0)
E - H 2 12		SIDE(0.0)
H - J 2 12		TOP
Q - B 2 12		TOP
K - I 2 12		TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Q - N 2 12		SIDE(0.0)
N - K 2 12		SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		SIDE(303.2)
G - M 1 6		SIDE(303.2)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMWV-p MT20	5.0	6.0	1.25	3.00
C TTWVW+m MT20	8.0	9.0	3.75	2.00
D TMWV-w MT20	3.0	6.0		
E TS-t MT20	5.0	6.0		
F TMWVW-t MT20	5.0	6.0		
G TMWV-w MT20	3.0	6.0		
H TTWVW+m MT20	8.0	9.0	3.75	2.00
I TMWV-p MT20	5.0	6.0	1.25	3.00
K BMV1+p MT20	3.0	6.0		
L BMWVW-t MT20	5.0	6.0	2.50	2.75
M BMWVWV-t MT20	6.0	9.0	3.50	2.25
N BS-t MT20	5.0	6.0		
O BMWVWV-t MT20	6.0	9.0	3.50	4.25
P BMWVW-t MT20	5.0	6.0	2.50	2.75
Q BMV1+p MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT 3868 0	DOWN 3868 0	5-8	5-8
Q	HORZ 3528 0	UPLIFT 3528 0	5-8	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
Q	2866 1860 / 0	517 / 0 0 / 0 688 / 0 0 / 0
K	2611 1522 / 0	465 / 0 0 / 0 624 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO									
A-B	0 / 44	-102.1	-102.1	0.04 (1)	10.00	P-C	-450 / 170	0.05 (1)	0.57 (1)
B-C	-4288 / 0	-102.1	-102.1	0.08 (1)	5.47	C-O	0 / 4640	0.57 (1)	0.16 (1)
C-R	-7617 / 0	-102.1	-102.1	0.49 (1)	3.93	O-D	-1315 / 0	0.09 (1)	0.68 (1)
R-S	-7617 / 0	-102.1	-102.1	0.49 (1)	3.93	M-G	-783 / 0	0.09 (1)	0.06 (1)
S-T	-7617 / 0	-102.1	-102.1	0.49 (1)	3.93	L-H	0 / 5487	0.68 (1)	0.06 (1)
T-U	-7617 / 0	-102.1	-102.1	0.49 (1)	3.93	B-P	-3551 / 0	0.44 (1)	0.08 (1)
U-D	-7617 / 0	-102.1	-102.1	0.49 (1)	3.93	L-I	0 / 3551	0.44 (1)	0.08 (1)
D-V	-7617 / 0	-102.1	-102.1	0.37 (1)	4.01	O-P	-262 / 0	0.03 (1)	0.08 (1)
V-E	-7617 / 0	-102.1	-102.1	0.37 (1)	4.01	F-M	0 / 262	0.03 (1)	0.08 (1)
E-F	-7617 / 0	-102.1	-102.1	0.37 (1)	4.01				
F-W	-8068 / 0	-102.1	-102.1	0.22 (1)	4.10				
W-G	-8068 / 0	-102.1	-102.1	0.39 (1)	3.97				
G-H	-8068 / 0	-102.1	-102.1	0.39 (1)	3.97				
H-I	-3890 / 0	-102.1	-102.1	0.08 (1)	5.68				
I-J	0 / 44	-102.1	-102.1	0.04 (1)	10.00				
Q-B	-3880 / 0	0.0	0.0	0.14 (1)	7.18				
K-I	-3550 / 0	0.0	0.0	0.13 (1)	7.43				
Q-X	0 / 0	-38.5	-38.5	0.16 (2)	10.00				
X-P	0 / 0	-38.5	-38.5	0.16 (2)	10.00				
P-Y	0 / 3403	-38.5	-38.5	0.36 (1)	10.00				
Y-Z	0 / 3403	-38.5	-38.5	0.36 (1)	10.00				
Z-AA	0 / 3403	-38.5	-38.5	0.36 (1)	10.00				
AA-AB	0 / 3403	-38.5	-38.5	0.36 (1)	10.00				
AB-O	0 / 3403	-38.5	-38.5	0.36 (1)	10.00				
O-AC	0 / 7753	-38.5	-38.5	0.59 (1)	10.00				
AC-N	0 / 7753	-38.5	-38.5	0.59 (1)	10.00				
N-M	0 / 7753	-38.5	-38.5	0.59 (1)	10.00				
M-L	0 / 3085	-38.5	-38.5	0.32 (1)	10.00				
L-K	0 / 0	-38.5	-38.5	0.13 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	3-5-5	-44	-49		FRONT	VERT	DEAD		
C	3-5-5	-251	-251		FRONT	VERT	SNOW		
M	15-8-8	-1884	-1884		BACK	VERT	TOTAL		
N	13-11-4	-65	-70		BACK	VERT	TOTAL		
R	3-11-4	-149	-149		BACK	VERT	TOTAL		
S	5-11-4	-123	-123		BACK	VERT	TOTAL		
T	7-11-4	-123	-123		BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/999 (0.17")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.49/1.00 (C-D:1), BC=0.59/1.00 (M-O:1)
WB=0.68/1.00 (H-M:1), SS=0.24/1.00 (C-D:1)

SOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

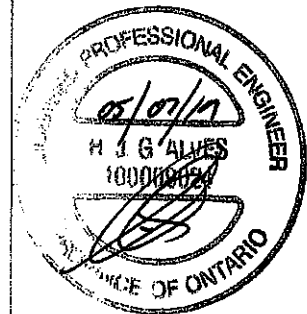
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (O) (INPUT = 0.90)
JSI METAL= 0.76 (N) (INPUT = 1.00)

JOB NAME 402938	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:39 2019 Page 2 ID:VdmZNIanRCr_vT1tp?QGfzKFu-MiirF7sJNcTRh9aguVoMc5teMJApdcDzxJQ7SzIqg	

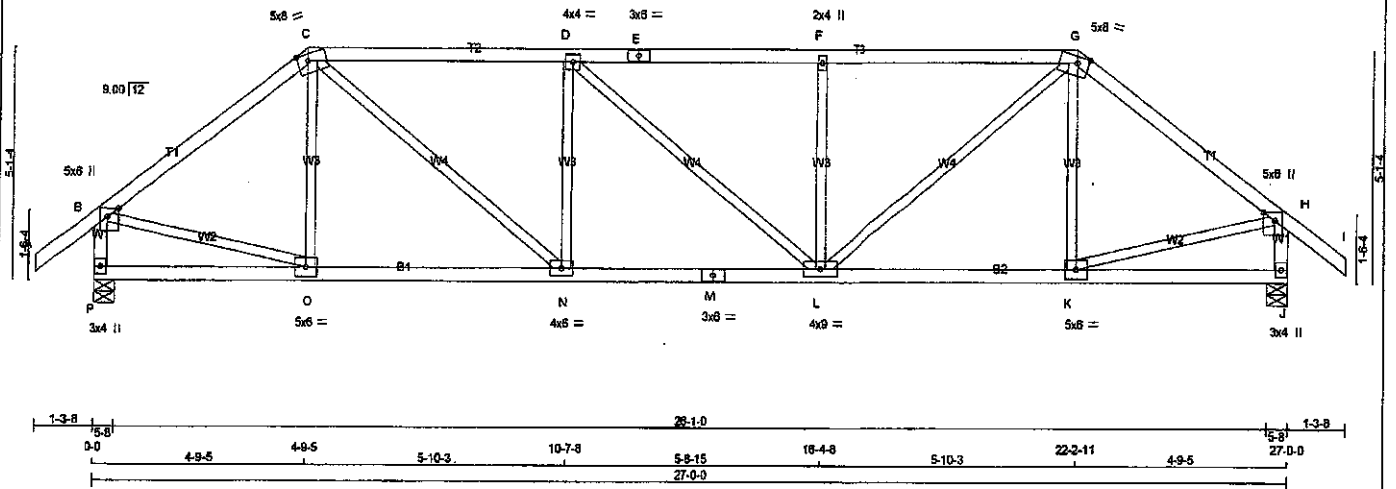
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
U	9-11-4	-123	-123	—	BACK	VERT	TOTAL	—	—
V	11-11-4	-123	-123	—	BACK	VERT	TOTAL	—	—
W	13-11-4	-123	-123	—	BACK	VERT	TOTAL	—	—
X	1-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
Y	3-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
Z	5-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AA	7-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AB	9-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AC	11-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



DWG NO. TAM 1910760
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME 402938	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	
Version 8.230 8 Nov 17 2018 Mitak Industries, Inc. Tue May 7 17:41:40 2019 Page 1 ID:VdmZNIanRCr_yT1tp?OGrtzKFu-qvGDSTsx8wbIJ9ISCJb9JQmVjWdM8zNCb3_fuzlqj					
-1-3-8 0-0 4-9-5 4-9-5 5-10-3 10-7-8 5-8-15 15-4-8 5-10-3 22-2-11 4-9-5 27-0-0 1-3-8 Scale = 1/4" = 1'-0"					



TOTAL WEIGHT = 2 X 111 = 221 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
B - H	2x4	DRY	No.2
H - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
P	2039	0	2039	0	5-8
J	2039	0	2039	0	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	MAX/MIN. COMPONENT REACTIONS	MAX/MIN. COMPONENT REACTIONS	MAX/MIN. COMPONENT REACTIONS	MAX/MIN. COMPONENT REACTIONS	MAX/MIN. COMPONENT REACTIONS
P	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	388 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.59 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	O-C	-159 / 110	0.06 (1)
B-C	-1985 / 0	-102.1	-102.1 0.52 (1)	4.22	C-N	0 / 1355	0.30 (1)
C-D	-2630 / 0	-102.1	-102.1 0.68 (1)	3.59	N-D	-640 / 0	0.25 (1)
D-E	-2628 / 0	-102.1	-102.1 0.65 (1)	3.59	D-L	-2 / 0	0.00 (1)
E-F	-2628 / 0	-102.1	-102.1 0.65 (1)	3.59	L-F	-639 / 0	0.25 (1)
F-G	-2628 / 0	-102.1	-102.1 0.65 (1)	3.81	L-G	0 / 1353	0.30 (1)
G-H	-1986 / 0	-102.1	-102.1 0.52 (1)	4.22	K-G	-157 / 110	0.06 (1)
H-I	0 / 42	-102.1	-102.1 0.14 (1)	10.00	B-O	0 / 1832	0.37 (1)
P-B	-1967 / 0	0.0	0.0 0.21 (1)	6.00	K-H	0 / 1832	0.37 (1)
J-H	-1967 / 0	0.0	0.0 0.21 (1)	6.00			
P-O	0 / 0	-38.5	-38.5 0.20 (3)	10.00			
O-N	0 / 1582	-38.5	-38.5 0.41 (2)	10.00			
N-M	0 / 2630	-38.5	-38.5 0.55 (1)	10.00			
M-L	0 / 2630	-38.5	-38.5 0.55 (1)	10.00			
L-K	0 / 1582	-38.5	-38.5 0.41 (2)	10.00			
K-J	0 / 0	-38.5	-38.5 0.20 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.68/1.00 (C-D:1), BC=0.55/1.00 (L-N:1),
WB=0.37/1.00 (H-K:1), SSI=0.28/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

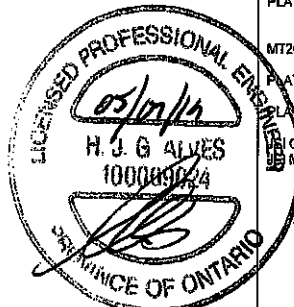
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

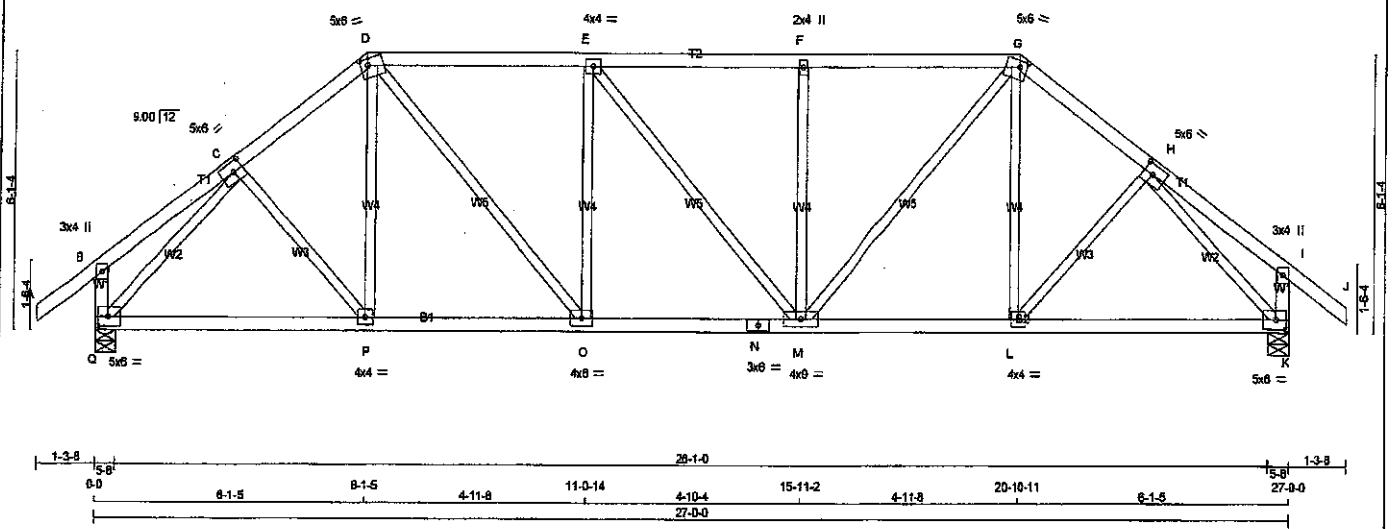
MIN GRIP= 0.88 (G) (INPUT = 0.90)
MIN METAL= 0.91 (M) (INPUT = 1.00)



DRWG NO. TAM 19/10761
STRUCTURAL
CONNECTION ONLY

JOB NAME 402938	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 3 Nov 17 2018 MITek Industries, Inc. Tue May 7 17:41:41 2019 Page 1
 ID: VdmZNIanRCr_yT1tp?OGrfzKFiu-H5ptfpZuDJ9wSk30wqghVz?w7U5VBWRFoXBKzIqje
 1-3-8 0-0 3-1-15 3-1-15 2-1-17 6-1-5 4-11-8 11-0-14 4-10-4 15-11-2 4-11-8 20-10-11 2-11-7 23-10-1 27-0-0 1-3-8
 Scale = 1:48.8



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	5.0	6.0	2.50 2.50
D	TTWW+m	MT20	5.0	6.0	1.75 2.00
E	TMVW+1	MT20	4.0	4.0	
F	TMVW+m	MT20	2.0	4.0	
G	TTWW+m	MT20	5.0	6.0	1.75 2.00
H	TMVW+1	MT20	5.0	6.0	2.50 2.50
I	TMV+p	MT20	3.0	4.0	
K	BMVW+1	MT20	5.0	6.0	2.50 2.75
L	BMVW+1	MT20	4.0	4.0	
M	BMVW+1	MT20	4.0	4.0	
N	BS-1	MT20	3.0	6.0	
O	BMVW+1	MT20	4.0	6.0	
P	BMVW+1	MT20	4.0	4.0	
Q	BMVW+1	MT20	5.0	6.0	2.50 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
Q	2039 0	2039 0	5-8	5-8	
K	2039 0	2039 0	5-8	5-8	

UNFACTORED REACTIONS					
JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS			
Q	1515	884 / 0	284 / 0	0 / 0	368 / 0
K	1515	884 / 0	284 / 0	0 / 0	368 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	C-P	0 / 178	0.04 (2)	
B-C	0 / 18	-102.1 -102.1	0.13 (1)	P-D	0 / 205	0.05 (3)	
C-D	-1988 / 0	-102.1 -102.1	0.18 (1)	D-O	0 / 938	0.21 (1)	
D-E	-2184 / 0	-102.1 -102.1	0.42 (1)	Q-E	-540 / 0	0.32 (1)	
E-F	-2182 / 0	-102.1 -102.1	0.42 (1)	E-M	-3 / 0	0.00 (1)	
F-G	-2182 / 0	-102.1 -102.1	0.42 (1)	M-F	-539 / 0	0.32 (1)	
G-H	-1988 / 0	-102.1 -102.1	0.18 (1)	M-G	0 / 835	0.21 (1)	
H-I	0 / 18	-102.1 -102.1	0.13 (1)	L-G	0 / 205	0.05 (3)	
I-J	0 / 42	-102.1 -102.1	0.14 (1)	L-H	0 / 178	0.04 (2)	
Q-B	-266 / 0	0.0 0.0	0.03 (1)	Q-C	-2226 / 0	0.75 (1)	
K-I	-266 / 0	0.0 0.0	0.03 (1)	H-K	-2226 / 0	0.75 (1)	
Q-P	0 / 1466	-38.5 -38.5	0.43 (2)				
P-O	0 / 1574	-38.5 -38.5	0.45 (2)				
O-N	0 / 2184	-38.5 -38.5	0.43 (1)				
N-M	0 / 2184	-38.5 -38.5	0.43 (1)				
M-L	0 / 1574	-38.5 -38.5	0.46 (2)				
L-K	0 / 1466	-38.5 -38.5	0.44 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
 ALLOWABLE DEFL. (TL) = L/360 (0.90")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.42/1.00 (D-E-1), BC=0.46/1.00 (L-M-2), WB=0.75/1.00 (H-K-1), SS=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

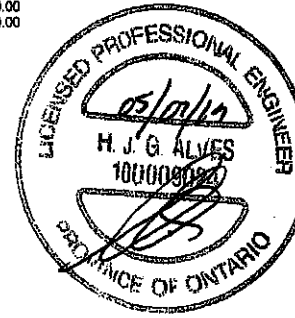
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1867 768 1867 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

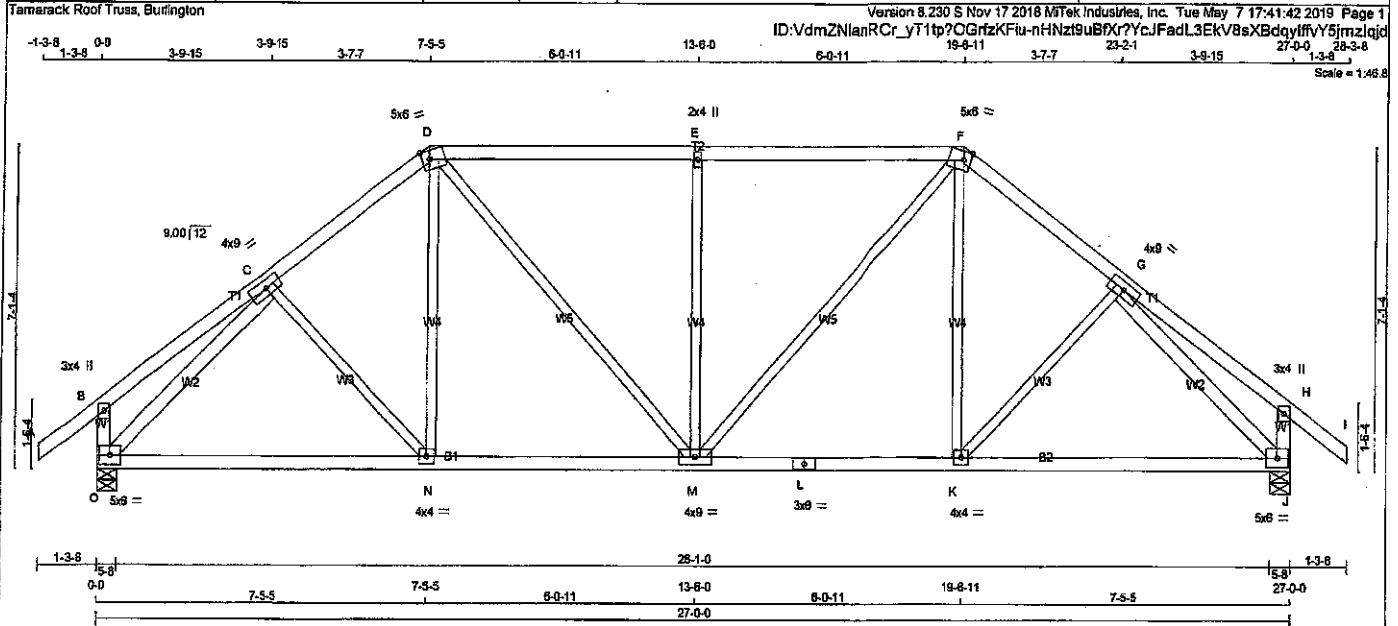
ISI GRIP= 0.87 (K) (INPUT = 0.90)

ISI METAL= 0.67 (N) (INPUT = 1.00)



DRWG NO. TAM T1910762
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 402938	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TAMERACK ROOF TRUSS, BURLINGTON				TRUSS DESC.	



TOTAL WEIGHT = 2 X 123 = 246 lb
[M/F]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No. 2	SPF	
D - F	2x4	DRY	No. 2	SPF	
F - I	2x4	DRY	No. 2	SPF	
O - B	2x4	DRY	No. 2	SPF	
J - H	2x4	DRY	No. 2	SPF	
O - L	2x4	DRY	No. 2	SPF	
L - J	2x4	DRY	No. 2	SPF	
ALL WEBS	2x3	DRY	No. 2	SPF	
EXCEPT					
O - C	2x4	DRY	No. 2	SPF	
G - J	2x4	DRY	No. 2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-1	MT20	4.0	9.0		
D TTWW-m	MT20	5.0	6.0	Edge 2.00	
E TMVW-w	MT20	2.0	4.0		
F TTWW-m	MT20	5.0	6.0	Edge 2.00	
G TMVW-1	MT20	4.0	9.0		
H TMV+p	MT20	3.0	4.0		
J BMVW-1	MT20	5.0	6.0		
K BMVW-1	MT20	4.0	4.0		
L BS-1	MT20	3.0	6.0		
M BMVW-1	MT20	4.0	9.0		
N BMVW-1	MT20	4.0	4.0		
O BMVW-1	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	UPLIFT	IN-SX
O	2039	0	2039	0
J	2039	0	2039	0

UNFACTORED REACTIONS

1ST LOASE	MAX./MIN.	COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
O	1515	824 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH (FR-TO)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	C-N	0 / 113	0.03 (3)	10.00
B-C	0 / 25	-102.1	-102.1 0.22 (1)	N-D	0 / 351	0.08 (2)	10.00
C-D	-1948 / 0	-102.1	-102.1 0.21 (1)	D-M	0 / 626	0.14 (1)	4.65
D-E	-1855 / 0	-102.1	-102.1 0.54 (1)	M-E	-756 / 0	0.66 (1)	4.20
E-F	-1855 / 0	-102.1	-102.1 0.54 (1)	M-F	0 / 626	0.14 (1)	4.20
F-G	-1948 / 0	-102.1	-102.1 0.21 (1)	K-F	0 / 351	0.08 (2)	4.65
G-H	0 / 25	-102.1	-102.1 0.22 (1)	K-G	0 / 113	0.03 (3)	10.00
H-I	0 / 42	-102.1	-102.1 0.14 (1)	O-C	-2234 / 0	0.76 (1)	7.81
O-B	-288 / 0	0.0	0.0 0.03 (1)	G-J	-2234 / 0	0.76 (1)	7.81
J-H	-288 / 0	0.0	0.0 0.03 (1)				
O-N	0 / 1524	-38.5	-38.5 0.57 (2)				10.00
N-M	0 / 1539	-38.5	-38.5 0.58 (2)				10.00
M-L	0 / 1539	-38.5	-38.5 0.58 (2)				10.00
L-K	0 / 1539	-38.5	-38.5 0.58 (2)				10.00
K-J	0 / 1524	-38.5	-38.5 0.57 (2)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 52.5	PSF	

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.90")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.13")
ALLOWABLE DEFL. (TL) = $L/360$ (0.90")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.22")

CSI: TC=0.54/1.00 (D-E:1), BC=0.58/1.00 (K-M:2), WB=0.78/1.00 (G-J:1), SSI=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

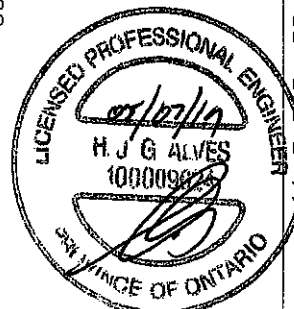
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JBI GRIP= 0.89 (C) (INPUT = 0.90)
JBI METAL= 0.56 (L) (INPUT = 1.00)



DRWG NO. TAM **T410763**
STRUCTURAL
CHECK ONLY

[illegible]

JOB NAME 402938	TRUSS NAME T6	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
--------------------	------------------	---------------	----------	-------------------------------	----------

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:43 2019 Page 1

ID:VdmZNianRCr_yT1tp?OGfzKFiu-FUXL4UvpQrzs9muR7Lslnx2LixaBZVXpuZHeGDzlgjc

Scale = 1:54.9

LUMBER

N. L. G. A. RULES	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
P - H	2x4	DRY	No.2	SPF
J - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVV+p	MT20	5.0	8.0	Edge	3.00
C TMVV+I	MT20	4.0	4.0		2.00 1.50
D TTW-m	MT20	4.0	4.0		
E TMVV+I	MT20	4.0	4.0		
F TTW-m	MT20	4.0	4.0		
G TMVV+I	MT20	4.0	4.0		2.00 1.50
H TMVV+p	MT20	5.0	8.0	Edge	3.00
J BMV1+p	MT20	3.0	4.0		
K BMVV-I	MT20	5.0	8.0		
L BMVV+I	MT20	4.0	4.0		
M BS-I	MT20	3.0	8.0		
N BMVV+I	MT20	4.0	4.0		
O BMVV-I	MT20	5.0	8.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQD BRG IN-SX
P	2039 0	2039 0	5-8	5-8
J	2039 0	2039 0	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL FITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-C	-175 / 94	0.08 (1)
B-C	-2017 / 0	-102.1 -102.1	0.41 (1)	4.37	C-N	-379 / 0	0.37 (1)
C-D	-1758 / 0	-102.1 -102.1	0.38 (1)	4.84	N-D	0 / 701	0.16 (1)
D-E	-1382 / 0	-102.1 -102.1	0.15 (1)	5.39	N-E	-215 / 0	0.13 (1)
E-F	-1382 / 0	-102.1 -102.1	0.15 (1)	5.39	E-L	-215 / 0	0.13 (1)
F-G	-1758 / 0	-102.1 -102.1	0.38 (1)	4.84	L-F	0 / 701	0.16 (1)
G-H	-2017 / 0	-102.1 -102.1	0.41 (1)	4.37	L-G	-379 / 0	0.37 (1)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	K-G	-175 / 94	0.08 (1)
P-B	-1955 / 0	0.0	0.0 0.20 (1)	6.02	B-O	0 / 1685	0.38 (1)
J-H	-1955 / 0	0.0	0.0 0.20 (1)	6.02	K-H	0 / 1685	0.38 (1)
P-O	0 / 0	-38.5	-38.5 0.17 (3)	10.00			
O-N	0 / 1642	-38.5	-38.5 0.43 (2)	10.00			
N-M	0 / 1458	-38.5	-38.5 0.40 (2)	10.00			
M-L	0 / 1458	-38.5	-38.5 0.40 (2)	10.00			
L-K	0 / 1642	-38.5	-38.5 0.43 (2)	10.00			
K-J	0 / 0	-38.5	-38.5 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 088-14

- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")

CALCULATED VERT. DEFL.(LL) = L/999 (0.08")

ALLOWABLE DEFL.(TL)= L/360 (0.90")

CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.41/1.00 (G:H:1), BC=0.43/1.00 (K:L:2), WS=0.38/1.00 (H-K:1), SS=0.20/1.00 (G:H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

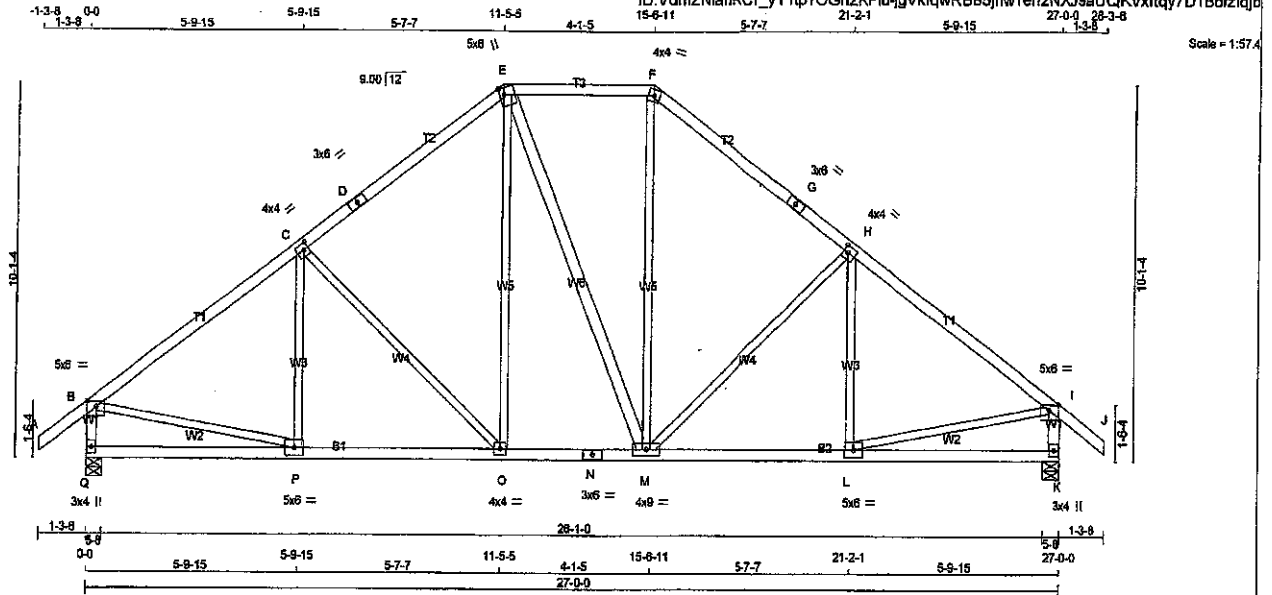
MAX MIN MAX MIN MAX MIN

MT20 616 354 1667 798 1987 1656

JOB NAME 402938	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:41:44 2019 Page 1

ID:VdmZNlanRCr_yT1tp?OGfzKfU-jgVklqwrB85jnwTeh2NXJ8aUQKvxtqy7D1Bofzljlb



TOTAL WEIGHT = 2 X 132 = 264 lb

LUMBER			
N L G A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.75	3.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-p	MT20	5.0	6.0	1.75	3.25
K	BMV1+p	MT20	3.0	4.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
Q	2039	0	2039	0	0
K	2039	0	2039	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0
K	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO			
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00
B-C	-2028 / 0	-102.1 -102.1 0.54 (1)	4.20
C-D	-1645 / 0	-102.1 -102.1 0.49 (1)	4.62
D-E	-1645 / 0	-102.1 -102.1 0.49 (1)	4.62
E-F	-1268 / 0	-102.1 -102.1 0.24 (1)	5.43
F-G	-1647 / 0	-102.1 -102.1 0.49 (1)	4.61
G-H	-1647 / 0	-102.1 -102.1 0.49 (1)	4.61
H-I	-2027 / 0	-102.1 -102.1 0.54 (1)	4.20
I-J	0 / 42	-102.1 -102.1 0.14 (1)	10.00
Q-B	-1949 / 0	0.0 0.0 0.20 (1)	6.03
K-I	-1949 / 0	0.0 0.0 0.20 (1)	6.03
Q-P	0 / 0	-38.5 -38.5 0.25 (3)	10.00
P-O	0 / 1856	-38.5 -38.5 0.46 (2)	10.00
O-N	0 / 1285	-38.5 -38.5 0.31 (1)	10.00
N-M	0 / 1285	-38.5 -38.5 0.31 (1)	10.00
M-L	0 / 1655	-38.5 -38.5 0.46 (2)	10.00
L-K	0 / 0	-38.5 -38.5 0.25 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/689 (0.12")

CSI: TC=0.54/H.00 (B-C:1), BC=0.46/H.00 (O-P:2), WB=0.70/H.00 (C-O:1), SS=0.23/H.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

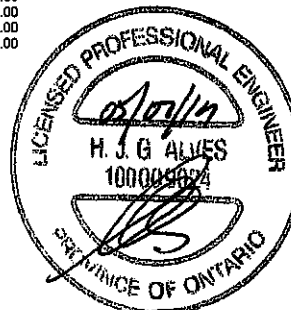
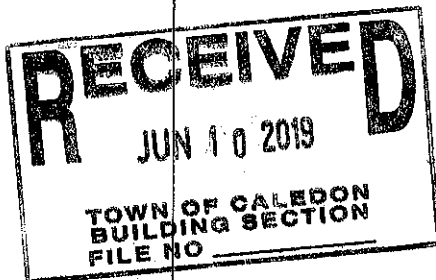
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1896

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

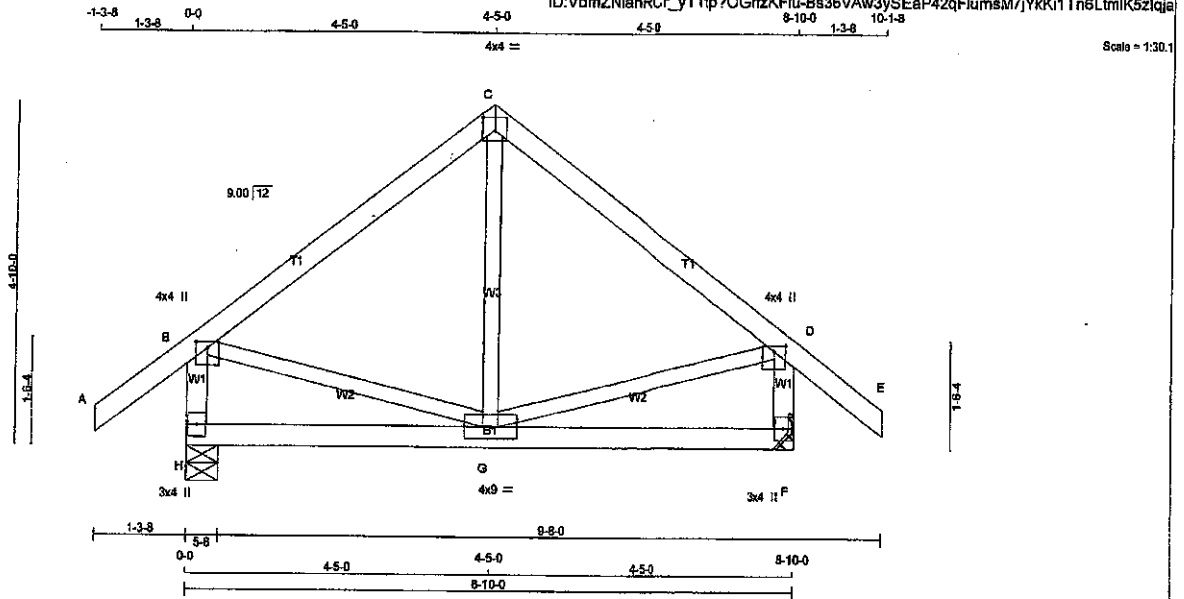
JSI GRIP= 0.89 (B) (INPUT = 0.80)
JSI METAL = 0.41 (P) (INPUT = 1.00)



DRWG NO. TAM T7910766
STRUCTURAL
CHECK ONLY

JOB NAME 402938	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:41:45 2019 Page 1
ID:VdmZNlanRCr_yT1p?OGrfzKFiU-Bs38VAw3ySEaP42qFlumsM7jYkK11Tn6LtmIK5zIqja



TOTAL WEIGHT = 2 X 39 = 79 lb
(M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	762	0	5-8	5-8
F	762	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LCASE	MAX MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
H	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0
F	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (LC)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)
FR-TO					FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 180	0.04 (3)
B-C	-405 / 0	-102.1	-102.1	0.26 (1)	6.25	B-G	0 / 335	0.08 (1)
C-D	-405 / 0	-102.1	-102.1	0.26 (1)	6.25	G-D	0 / 335	0.08 (1)
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
H-B	-697 / 0	0.0	0.0	0.07 (1)	7.81			
F-D	-697 / 0	0.0	0.0	0.07 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.29")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/980 (0.29")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSK: TC=0.26/1.00 (C-D-1), EC=0.17/1.00 (F-G-3), WB=0.08/1.00 (D-G-1), SS=0.14/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

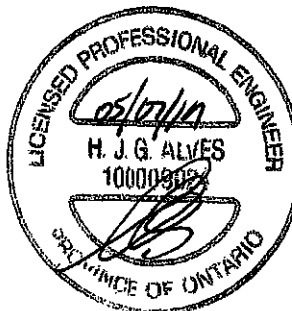
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

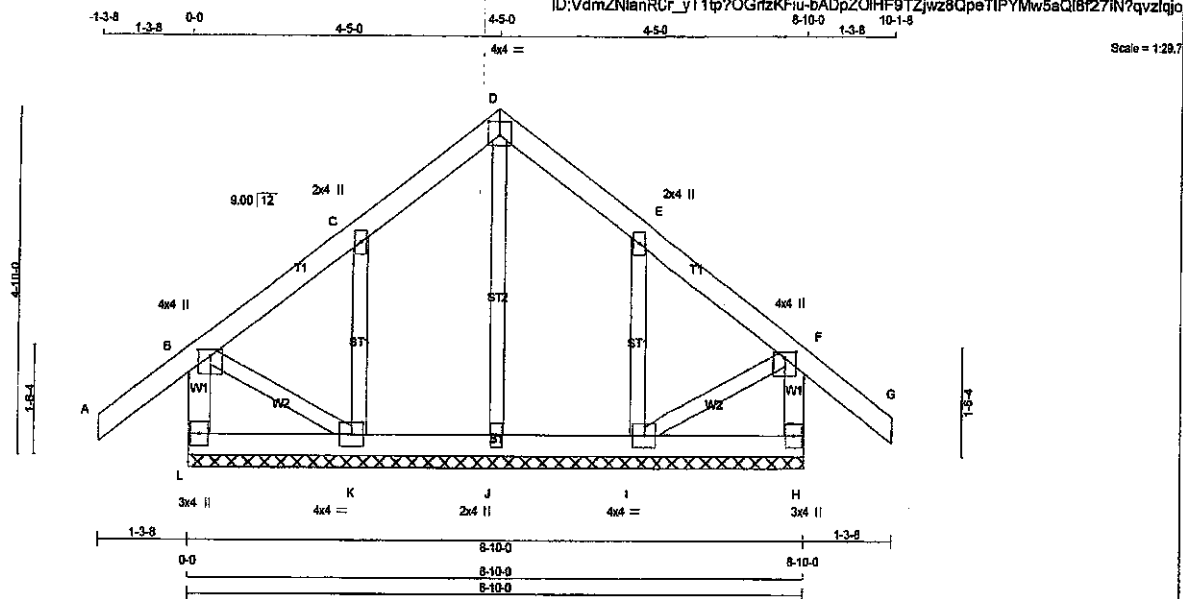


DRWG NO. TAM 1910767
STRUCTURAL
CHECKED ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402938	G9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:31 2019 Page 1
ID:VdmZNlanRCr_yT1p?OGfzKFiU-bADpZOIHf9TZjwz8QpeTIPYmw5aQl8f27IN?qvzljao



TOTAL WEIGHT = 41 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0\"/>

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVH+p	MT20	4.0	4.0	1.00	2.00
C	TMVH+w	MT20	2.0	4.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVH+w	MT20	2.0	4.0		
F	TMVH+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMV1+w	MT20	4.0	4.0		
J	BMV1+w	MT20	2.0	4.0		
K	BMV1+w	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
L-B	-259 / 0	0.0	0.0	J-D	-137 / 0	0.05 (1)	0.05 (1)
A-B	0 / 42	-102.1	-102.1	K-C	-275 / 0	0.05 (1)	0.05 (1)
B-C	-10 / 0	-102.1	-102.1	I-E	-275 / 0	0.05 (1)	0.05 (1)
C-D	-33 / 0	-102.1	-102.1	B-K	0 / 23	0.01 (1)	0.01 (1)
D-E	-33 / 0	-102.1	-102.1	I-F	0 / 23	0.01 (1)	0.01 (1)
E-F	-10 / 0	-102.1	-102.1				
F-G	0 / 42	-102.1	-102.1				
H-F	-259 / 0	0.0	0.0				
L-K	0 / 0	-38.5	-38.5				
K-J	0 / 12	-38.5	-38.5				
J-I	0 / 12	-38.5	-38.5				
I-H	0 / 0	-38.5	-38.5				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.14/1.00 (F-G-1), BC=0.04/1.00 (J-K-3), WB=0.05/1.00 (C-K-1), SS=0.09/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1887

PLATE PLACEMENT TOL = 0.250 inches

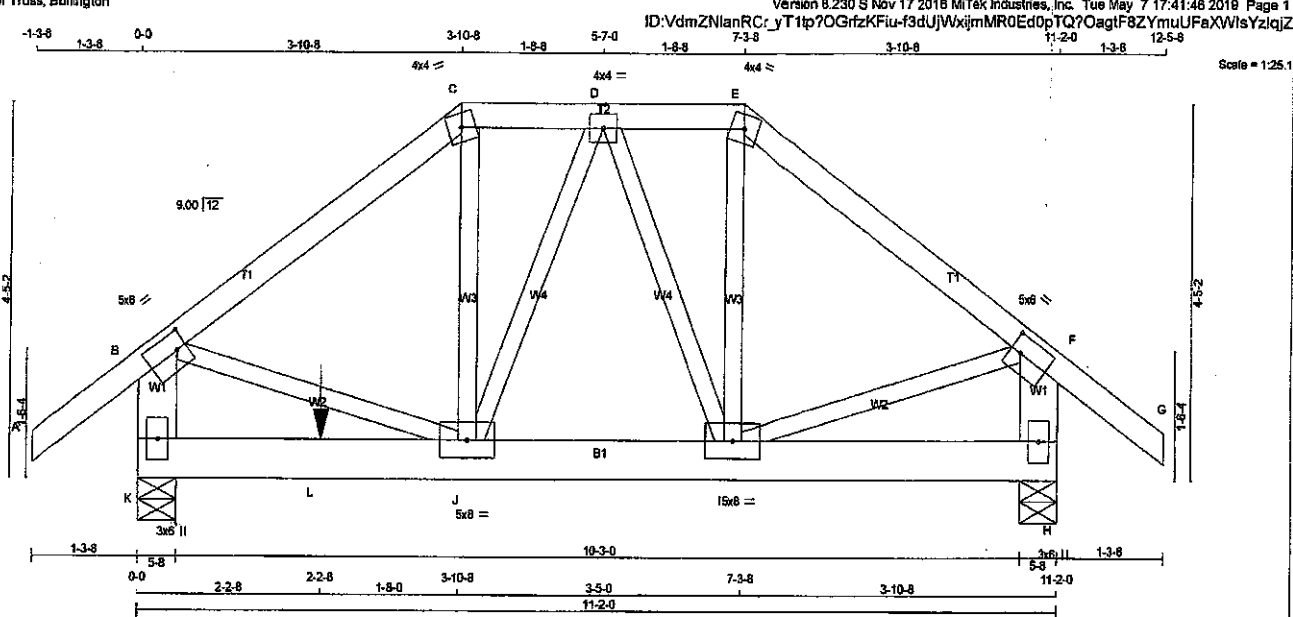
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (F) (INPUT = 0.60)
JSI METAL = 0.15 (E) (INPUT = 1.00)



DRWG NO. TAM 11910752
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 61 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
K - B	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
K - H	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-I	MT20	5.0	6.0	2.50	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-I	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-I	MT20	5.0	6.0	2.50	1.50
H	BMV1-p	MT20	3.0	6.0		
I	BMVWW-I	MT20	5.0	8.0		
J	BMVWW-I	MT20	5.0	8.0		
K	BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX		
K	1894	0	1894	0	0	5-8	5-8		
H	1115	0	1115	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1240	766 / 0	189 / 0	0 / 0	0 / 0	286 / 0	0 / 0
H	821	484 / 0	135 / 0	0 / 0	0 / 0	192 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-102.1	-102.1	0.15 (1)	J-C	0 / 413	0.10 (1)
B-C	-1160 / 0	-102.1	-102.1	0.32 (1)	I-E	0 / 227	0.08 (2)
C-D	-938 / 0	-102.1	-102.1	0.05 (1)	B-J	0 / 961	0.24 (1)
D-E	-672 / 0	-102.1	-102.1	0.05 (1)	I-F	0 / 691	0.17 (1)
E-F	-834 / 0	-102.1	-102.1	0.31 (1)	J-D	0 / 284	0.07 (1)
F-G	0 / 42	-102.1	-102.1	0.15 (1)	D-I	-455 / 0	0.15 (1)
K-B	-1295 / 0	0.0	0.0	0.09 (1)			
H-F	-1027 / 0	0.0	0.0	0.07 (1)			
K-L	0 / 0	-38.5	-38.5	0.58 (1)			
L-J	0 / 0	-38.5	-38.5	0.58 (1)			
J-I	0 / 838	-38.5	-38.5	0.41 (1)			
I-H	0 / 0	-38.5	-38.5	0.08 (2)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	2-2-8	-958	-958		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.32/1.00 (B-C:1), BC=0.58/1.00 (J-K:1), WB=0.24/1.00 (B-J:1), SSI=0.47/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

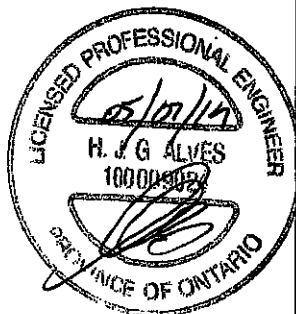
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 786 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

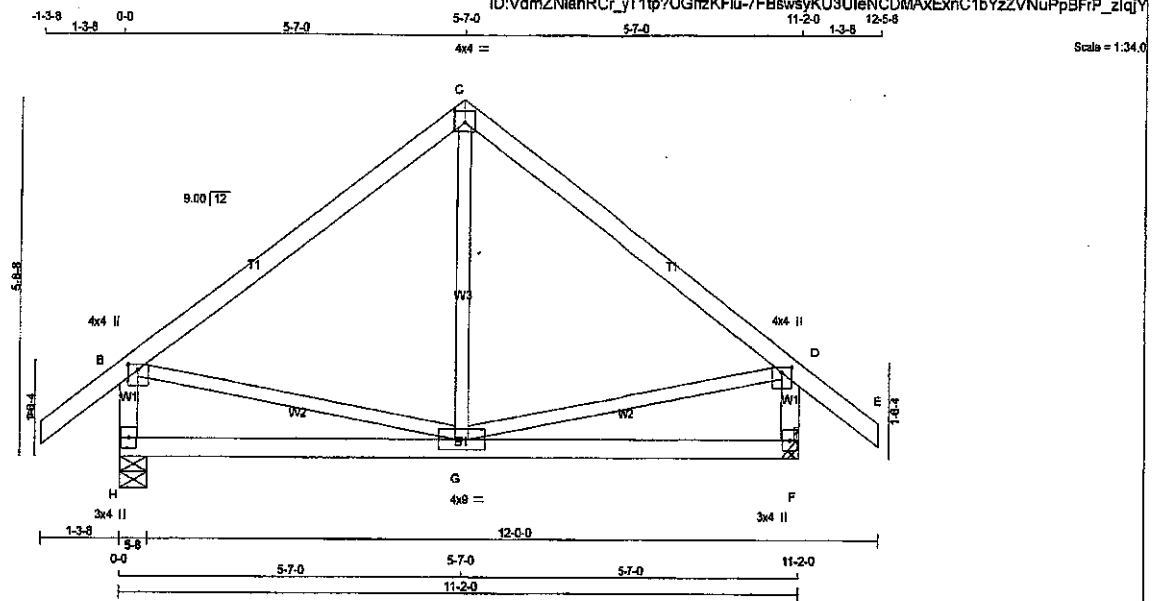
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (F) (INPUT = 0.90)
JSI METAL = 0.34 (F) (INPUT = 1.00)



DRWG NO. TAM 71710768
STRUCTURAL
CONSULTANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T11	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 3 X 48 = 144 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
H - B	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW-1	MT20	4.0	8.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT
H	928	0	928	0
F	928	0	928	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

1ST LCASE	MAX	MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
H	683	404 / 0	117 / 0
F	683	404 / 0	117 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX	MEMB.	MAX. FACTORED
	FORCE	(PLF)	CS1 (LC)	UNBRAC	LENGTH		FORCE
FR-TO	(LBS)	FROM	TO	CS1 (LC)	FR-TO		(LBS)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 246
B-C	-543 / 0	-102.1	-102.1	0.41 (1)	6.25	B-G	0 / 444
C-D	-543 / 0	-102.1	-102.1	0.41 (1)	6.25	G-D	0 / 444
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
H-B	-844 / 0	0.0	0.0	0.09 (1)	7.81		
F-D	-844 / 0	0.0	0.0	0.09 (1)	7.81		
H-G	0 / 0	-38.5	-38.5	0.27 (3)	10.00		
G-F	0 / 0	-38.5	-38.5	0.27 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.03")
ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.05")

CS1: TC=0.41/1.00 (B-C:1), BC=0.27/1.00 (F-G:3), WB=0.10/1.00 (D-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

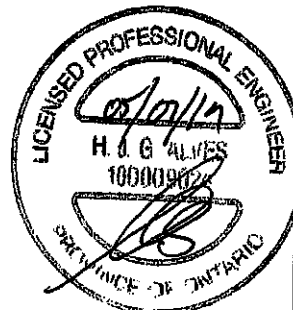
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.81 (D) (INPUT = 0.90)
JSI METAL = 0.21 (B) (INPUT = 1.00)



ISSUED: TAM 11910769
STRUCTURAL
CALCULATIONS ONLY

Tamarack Roof Truss, Burlington

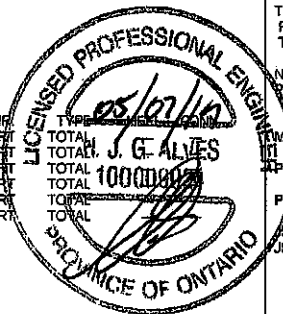


TOTAL WEIGHT = $2 \times 22 = 43$ lb

JT	TYPE	PLATES	W	LEN	Y	X
A	TWVW+p	MT20	4.0	4.0	1.00	2.00
B	TRVW+m	MT20	5.0	6.0	2.25	1.50
C	TV+p	MT20	3.0	4.0		
D	BWVW1+p	MT20	4.0	6.0		
E	BWVW-l	MT20	5.0	6.0		
F	BWV1+p	MT20	3.0	6.0		

JT	LOC.	LC1	MAX-	MAX+	FACE	DIF
B	2-1-0	-104	-104	—	BACK	VERT
E	2-1-12	-2	-2	—	BACK	VERT
G	3-1-12	-18	-18	—	BACK	VERT
H	8-12	-729	-729	—	FRONT	VERT
I	2-8-12	-723	-723	—	FRONT	VERT
J	3-1-12	-7	-9	—	BACK	VERT

JSI GRIP= 0.30 (A) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



BALDWIN TAM 77910770
 STRUCTURAL
 CIVIL ENGINEERING

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T13	1	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:41:48 2019 Page 1
ID:VdmZNlanRCr_yT1tp?OGrfzKFIu-bRIE7CzyFNc9GXmPwuSTU7IGCcX0ErjY2r?PxQzlqjX

Scale = 1:18.3

TOTAL WEIGHT = 31 lb

<u>LUMBER</u>			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
B - D	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	6.0	1.75	1.50
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-l	MT20	5.0	6.0		Edge
E	BMVW+w	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

LOADING BEARINGS

FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	HEEL
B	733	0	733	0	0	5-8	5-8	2x4 L
D	595	0	595	0	0	MECHANICAL		2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	540	324 / 0	89 / 0	0 / 0	0 / 0	126 / 0	0 / 0
D	444	248 / 0	88 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S). B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED				
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0 / 24	-102.1	-102.1	0.13 (f)	10.00	E-C	0 / 279	0.08 (2)
B-G	-835 / 0	-102.1	-102.1	0.10 (f)	6.25	F-G	-244 / 2	0.00 (1)
G-C	-850 / 0	-102.1	-102.1	0.17 (f)	6.25	H-I	-73 / 128	0.00 (1)
C-I	-855 / 0	-102.1	-102.1	0.14 (f)	6.25			
I-D	-713 / 0	-102.1	-102.1	0.13 (f)	8.25			
B-F	0 / 572	-38.5	-38.5	0.19 (f)	10.00			
F-E	0 / 572	-38.5	-38.5	0.19 (f)	10.00			
E-H	0 / 572	-38.5	-38.5	0.14 (f)	10.00			
H-D	0 / 572	-38.5	-38.5	0.10 (f)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP	CH.	LL	=	29.1	PSF
		DL	=	6.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= $L/360$ (0.28")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL)= $L/360$ (0.28")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.17/1.00 (C-G:1), BC=0.19/1.00 (E-F:1),
WB=0.06/1.00 (C-E:2), SSI=0.16/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

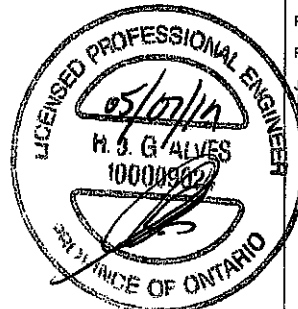
NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT2D	618	354	1867	768	1987	1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.70 (D) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)



DIVE NO. TAM 779/0771
STRUCTURAL
CUT & SHEET ONLY

JOB NAME TRUSS NAME QUANTITY PLY JOB DESC. GREEN PARK HOMES DRWG NO.

402938 T13Z 1 1 TRUSS DESC.

Tamarack Roof Truss, Burlington Version 8.230 S Nov 17 2016 MITek Industries, Inc. Tue May 7 17:41:49 2019 Page 1
ID:VdmZNianRCr_yT1tp?OGrfzKFlu-3dIdL.YzaOhk0tH.LbUzbIOCIQJLgqzGxhGVkyTszqjVw
Scale = 1:16.3

Diagram showing truss structure with dimensions and labels (A-H, T1-T2, W1-W2, F, K, L, M, H). Dimensions include spans (e.g., 1-3-8, 1-3-8, 0-0, 3-11-4, 3-11-4, 4-5-0, 4-0-8, 8-5-8) and heights (e.g., 2-9-4, 6-5-2, 6-0-0, 6-4).

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	No.2	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
B - D	2x6	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN Y	X
B	TMBH1-m MT20	4.0	6.0	1.75 1.50
C	TTW-p MT20	4.0	4.0	
D	TMBH1-m MT20	5.0	6.0	2.25 Edge
E	BMW+w MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
B	808	808	0	5-8	2x4 L
D	680	680	0	MECHANICAL	2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	581	367 / 0	89 / 0	0 / 0	0 / 0	135 / 0	0 / 0
D	504	291 / 0	81 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.18 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FR-TO	FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX	LC1 CS1 (LC)	MAX. UNBRAC LENGTH PR-TO	MEMB.	FR-TO	FORCE (LBS)	MAX. CS1 (LC)
A-B	0 / 24	-102.1	-102.1	0.15 (1)	10.00		E-C	0 / 261	0.06 (2)	
B-G	-791 / 0	-102.1	-102.1	0.15 (1)	8.25	F-G	-218 / 28	0.00 (1)		
G-J	-792 / 0	-102.1	-102.1	0.20 (1)	8.25	H-I	0 / 141	0.00 (1)		
J-K	-792 / 0	-102.1	-102.1	0.20 (1)	8.25					
K-L	-797 / 0	-102.1	-102.1	0.21 (1)	6.25					
L-D	-808 / 0	-102.1	-102.1	0.20 (1)	6.18					
B-F	0 / 689	-38.5	-38.5	0.22 (1)	10.00					
F-K	0 / 689	-38.5	-38.5	0.22 (1)	10.00					
K-L	0 / 689	-38.5	-38.5	0.22 (1)	10.00					
L-E	0 / 689	-38.5	-38.5	0.22 (1)	10.00					
E-M	0 / 689	-38.5	-38.5	0.15 (1)	10.00					
M-H	0 / 689	-38.5	-38.5	0.15 (1)	10.00					
H-D	0 / 689	-38.5	-38.5	0.11 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-0	-23	-25	--	FRONT	VERT	DEAD	--	--
C	4-5-0	-129	-129	--	FRONT	VERT	SNOW	--	--
H	7-11-4	-5	-7	--	BACK	VERT	TOTAL	--	--
J	3-11-4	-3	-3	--	BACK	VERT	TOTAL	--	--
K	1-11-4	1	1	--	BACK	VERT	TOTAL	--	--
L	3-11-4	1	1	--	BACK	VERT	TOTAL	--	--
M	5-11-4	1	1	--	BACK	VERT	TOTAL	--	--

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

*** NON STANDARD GIRDER ***
ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 089-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/998 (0.02")

CSI: TC=0.21/1.00 (C-1), BC=0.22/1.00 (B-F), WS=0.08/1.00 (C-E,2), SS=0.17/1.00 (C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.85 (D) (INPUT = 0.80)
JSI METAL=0.21 (B) (INPUT = 1.00)

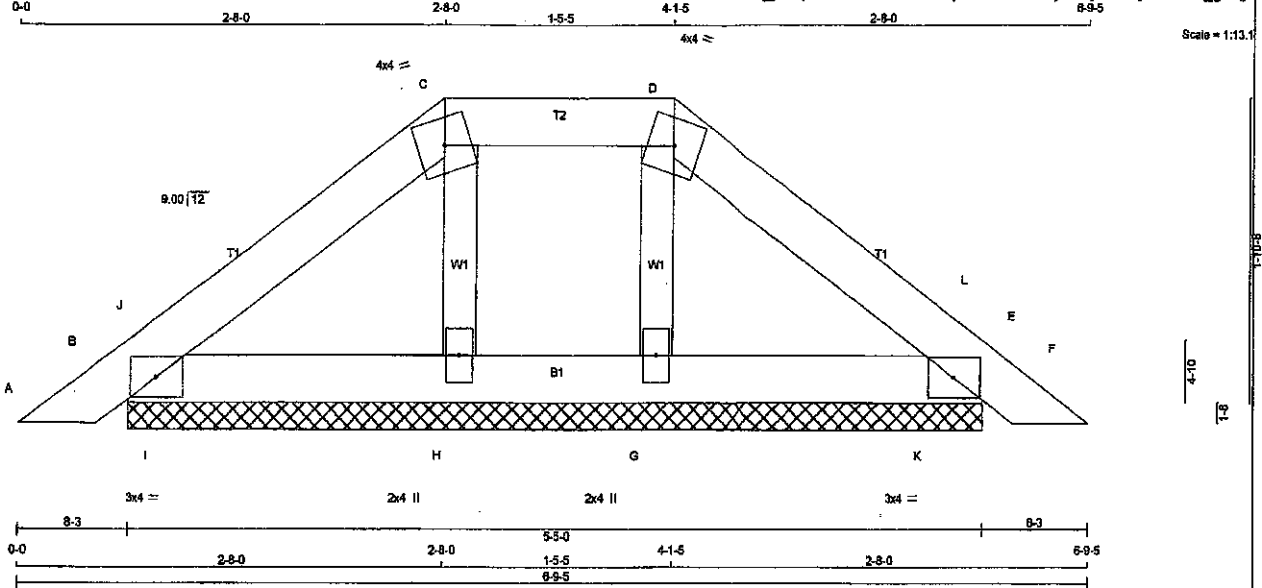
DATE: 05/07/19
H. J. G. ALVES
100009621
REGISTERED PROFESSIONAL ENGINEER
PROFESSIONAL SOCIETY OF ON

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402938	PB1	2	1	TRUSS DESC.		

Temerack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 M&T Industries, Inc. Tue May 7 17:41:35 2019 Page 1

ID:VdmZNIanRCr_yT1tp?OGrfzKFiu-UxSKPlpJNz?CXHvfjQSFJ3lixDhyCd2JLD_gzljkl



TOTAL WEIGHT = 2 X 18 = 35 lb

<u>LUMBER</u>						
N. L. G. A. RULES						
CHORDS		SIZE	LUMBER		DESCR.	
A - C	2x4	DRY	No.2	SPF		
C - D	2x4	DRY	No.2	SPF		
D - F	2x4	DRY	No.2	SPF		
B - E	2x4	DRY	No.2	SPF		
ALL WEBS		2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-1	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-1	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
B	233	0	233	0	0	5-5-0	5-5-0	5-5-0	5-5-0
E	233	0	233	0	0	5-5-0	5-5-0	5-5-0	5-5-0
H	208	0	208	0	0	5-5-0	5-5-0	5-5-0	5-5-0
G	208	0	208	0	0	5-5-0	5-5-0	5-5-0	5-5-0

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS					
		1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD
B	169	111/0	21/0	0/0	0/0	37/0	0/0
E	169	111/0	21/0	0/0	0/0	37/0	0/0
H	158	81/0	38/0	0/0	0/0	41/0	0/0
G	158	81/0	38/0	0/0	0/0	41/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM TO		FR-TO			
A-B	0/17	-102.1 -102.1	0.03 (1)	H-C	-121/0	0.02 (1)	10.00
B-J	-64/0	-102.1 -102.1	0.01 (3)	G-D	-121/0	0.02 (1)	6.25
J-C	-85/0	-102.1 -102.1	0.05 (1)	I-J	-103/22	0.00 (1)	6.25
C-D	-43/0	-102.1 -102.1	0.04 (1)	K-L	-103/22	0.00 (1)	6.25
D-L	-65/0	-102.1 -102.1	0.05 (1)				
L-E	-64/0	-102.1 -102.1	0.01 (3)				
E-F	0/17	-102.1 -102.1	0.03 (1)				
B-I	0/51	-38.5 -38.5	0.05 (1)				
I-H	0/51	-38.5 -38.5	0.05 (1)				
H-G	0/43	-38.5 -38.5	0.04 (1)				
G-K	0/51	-38.5 -38.5	0.05 (1)				
K-E	0/51	-38.5 -38.5	0.05 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. GIG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-L:1), BC=0.05/1.00 (G-K:1), WB=0.02/1.00 (D-G:1), SS=0.09/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

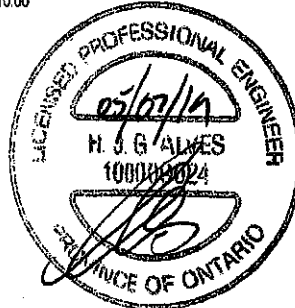
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	616 354	1667 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)

JSI METAL= 0.04 (B) (INPUT = 1.00)



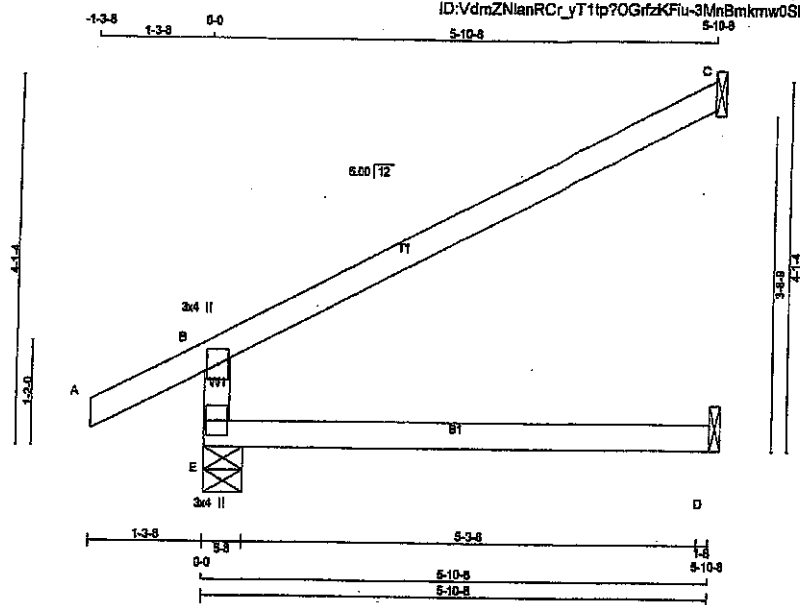
DWG NO. TAM T910757
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J1	QUANTITY 19	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
--------------------	------------------	----------------	----------	-------------------------------	-------------	----------

Tamersack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mtek Industries, Inc. Tue May 7 17:41:32 2019 Page 1
ID:VdmZNlanRCr_yT1tp7OGfzKFlu-3MnBmkmw0SbQL4YK_XAiqd5QSVtyUbkBMM7ZNLzdqjn

Scale = 1/2" = 1'-0"



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	648	0	648	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MTEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
E	473	282 / 0	72 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		MAX. UNBRACED LENGTH	MEMB.	FACTORED	
		VERT. LOAD (PLF)	LC1 MAX. CSI (LC)			MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO		
E-B	-514 / 0	0.0	0.0	0.22 (3)	7.81		
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		
B-C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25		
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00		

TOTAL WEIGHT = 19 X 17 = 319 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/899 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/842 (0.07")

CSI: TC=0.60/1.00 (B-C:1), BC=0.22/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), SSI=0.28/1.00 (B-C:1)

DOIL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

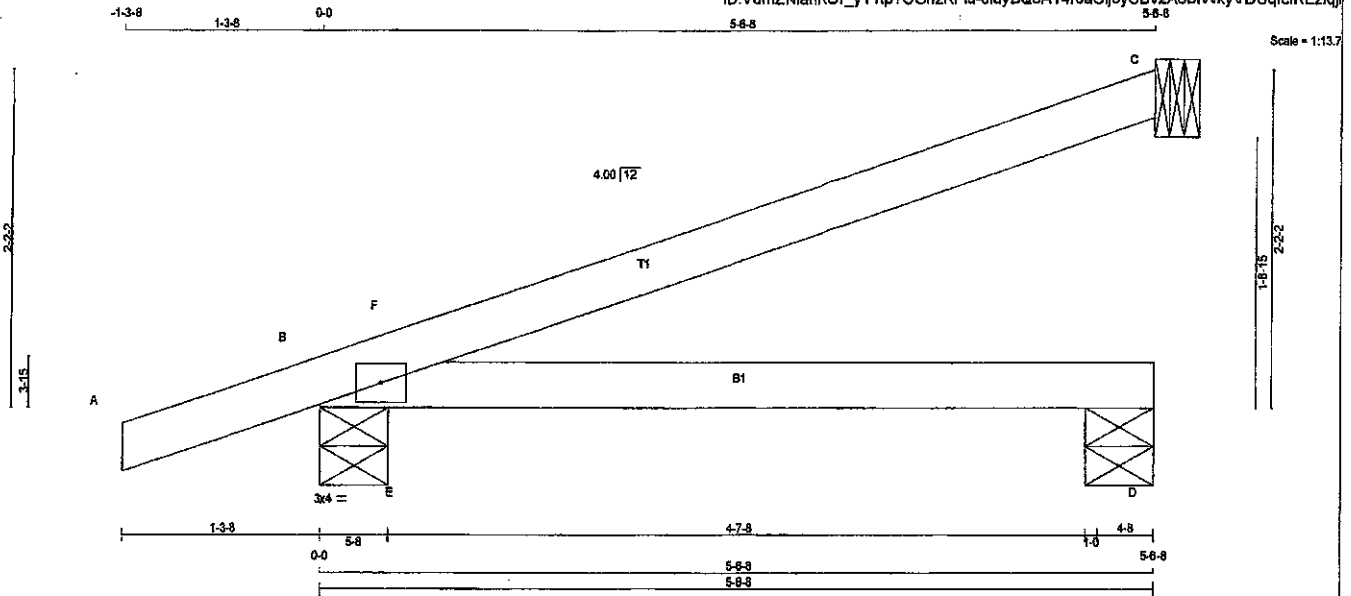
JSI GRIP= 0.21 (E) (INPUT = 0.80)
JSI METAL= 0.14 (B) (INPUT = 1.00)



DRWG NO. TAM 1910153
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:34 2019 Page 1
ID:VdmZNlanRCr_yT1tp7OGfzKFiu-0luyBQoAY4r8aOj6yCBv2AobIVkyVDUqfcREzlgj



TOTAL WEIGHT = 2 X 15 = 29 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY

LUMBER
No. 2
No. 2

DESCR.
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X
B TMB1-1 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
C	254	0	4-8	4-8
B	528	0	5-8	5-8
D	135	0	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
C	178	138 / 0	9 / 0	0 / 0	34 / 0	0 / 0
B	385	239 / 0	58 / 0	0 / 0	88 / 0	0 / 0
D	113	25 / 0	49 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 20	-102.1 -102.1	0.13 (1)	10.00	E-F	-261 / 125	0.00 (1)
B-F	-53 / 0	-102.1 -102.1	0.13 (3)	6.25			
F-C	0 / 6	-102.1 -102.1	0.43 (1)	10.00			
B-E	0 / 0	-38.5 -38.5	0.27 (1)	10.00			
E-D	0 / 0	-38.5 -38.5	0.32 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA 088-09, CSA 088-14
- TRC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/806 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/468 (0.14")

CSI: TC=0.43/1.00 (C-F:1), BC=0.32/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

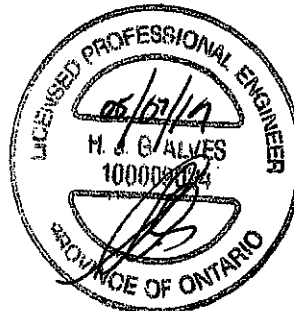
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

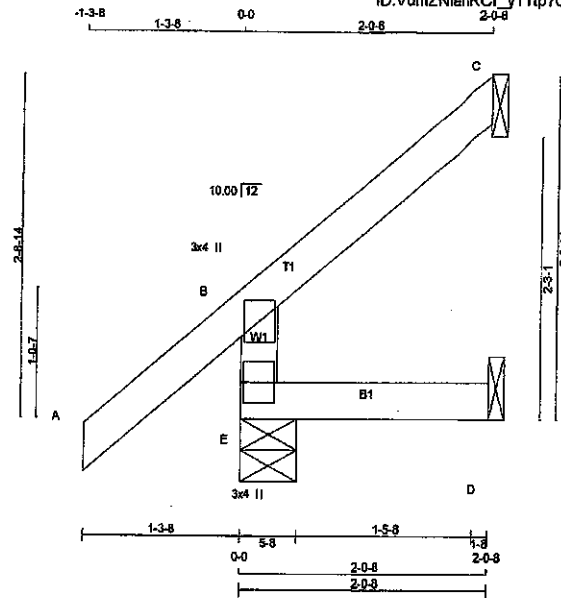
JSI GRIP= 0.31 (B) (INPUT = 0.60)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DRWG NO. TAM 1910755
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J7	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:35 2019 Page 1
ID: VdmZNlenRCr_yT1tp?OGrfzKFiu-UxSKPipoJNz?CXHvffjQSFj1txYhyTd2JLD_gztlqjk



Scale = 1:17.0

TOTAL WEIGHT = 4 X 8 = 33 lb

LUMBER	N. L. G. A. RULES
CHORDS	SIZE
E - B	2x4 DRY
A - C	2x4 DRY
E - D	2x4 DRY

LUMBER	DESCR.
No.2	SPF
No.2	SPF

DESCR.	SPF
SPF	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
	VERT	DOWN	IN-SX	IN-SX
E	316	0	5-8	5-8
C	79	0	1-8	1-8
D	34	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
E	225	154/10	24/10	0/0	0/0	48/0
C	54	45/10	0/0	0/0	0/0	9/0
D	31	0/0	19/0	0/0	0/0	13/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (PLF)	MAX. LENGTH FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LENGTH FR-TO
E-B		-271/0	0.0	0.0	0.02 (3)	7.81			
A-B		0/45	-102.1	-102.1	0.14 (1)	10.00			
B-C		-16/0	-102.1	-102.1	0.07 (1)	6.25			
E-D		0/0	-38.5	-38.5	0.03 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSK TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), SS=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

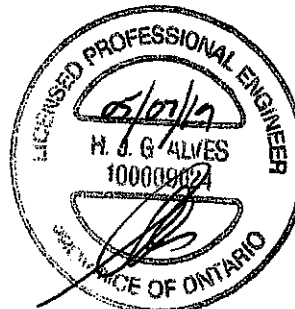
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1887 789

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DRWG NO. TAM 11910753
STRUCTURAL
COMPONENT ONLY



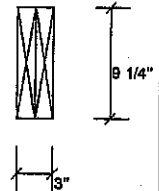
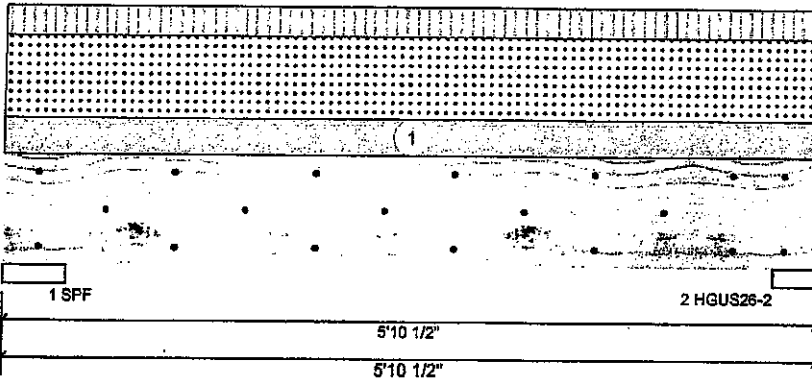
isDesign™

Client:
Project:
Address:Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 1 of 4

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Roof (Residential)
 Slope: 0/12
 Design Method: LSD
 Building Code: NBCC 2015
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	207	257	583	0
2	199	246	558	0

Bearings and Factored Reactions

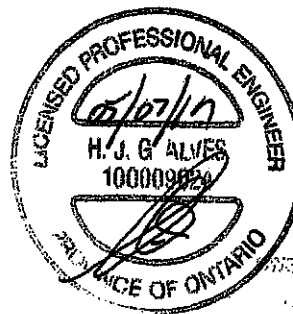
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	321 / 1081	1402 L
2 - HGUS...	4.000"	18%	308 / 1036	1344 L
				1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1585 ft-lb	3'	6039 ft-lb	0.262 (26%)	1.25D+1.5S +L	L
Unbraced	1585 ft-lb	3'	5236 ft-lb	0.303 (30%)	1.25D+1.5S +L	L
Shear	1217 lb	12"	3984 lb	0.306 (31%)	1.25D+1.5S +L	L
LL Defl inch	0.014 (L/4497)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.019 (L/3273)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



17910792

STRUCTURAL

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-7-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
 3269 North Service Rd., ON
 Canada
 L7N3G2
 (905) 336-1115



This design is valid until 12/11/2021





isDesign™

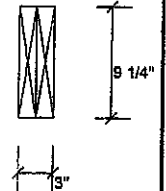
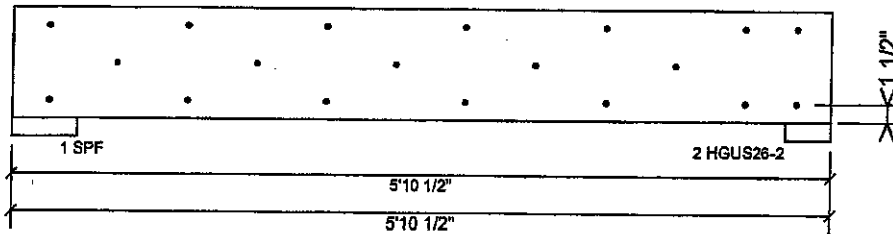
Client:
Project:
Address:

Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 2 of 4

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

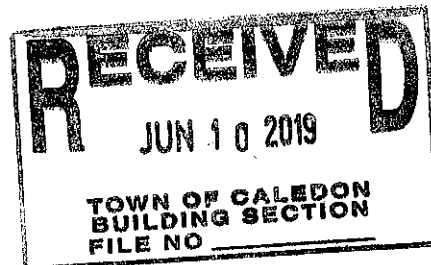
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	88.7 %
Load	293.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



1910792
TAMARACK
LUMBER INC.
3/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 355-1115



This design is valid until 12/11/2021





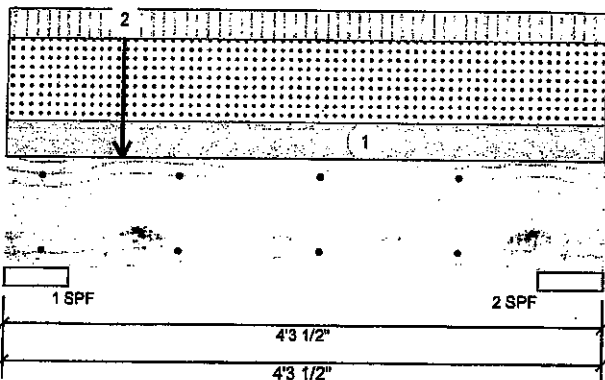
isDesign™

Client:
Project:
Address:Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

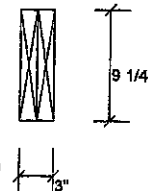
Page 3 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	149	192	449	0
2	81	101	232	0

Bearings and Factored Reactions

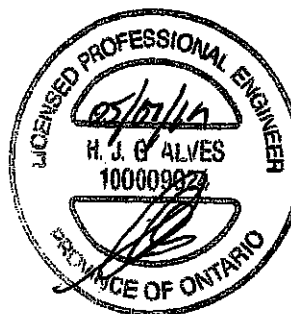
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	10%	239 / 822	1062 L
2 - SPF	5.500"	5%	127 / 430	557 L
				1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	504 ft-lb	1'9"	6039 ft-lb	0.084 (8%)	1.25D+1.5S	L
Unbraced	504 ft-lb	1'9"	5676 ft-lb	0.089 (9%)	1.25D+1.5S	L
Shear	691 lb	1'2"	3984 lb	0.173 (17%)	1.25D+1.5S	L
LL Defl inch	0.002 (L/20503)	2' 3/4"	0.117 (L/360)	0.020 (2%)	S+0.5L	L
TL Defl inch	0.003 (L/14977)	2' 3/4"	0.117 (L/360)	0.020 (2%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all piles.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



ENGINEER TAM 17910793
STRUCTURAL
100009822

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		3-1-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	0-10-2		Top	121 lb	91 lb	291 lb	0 lb	

Manufacturer Info

Tamarack Roof Trusses
3259 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

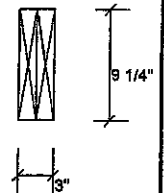
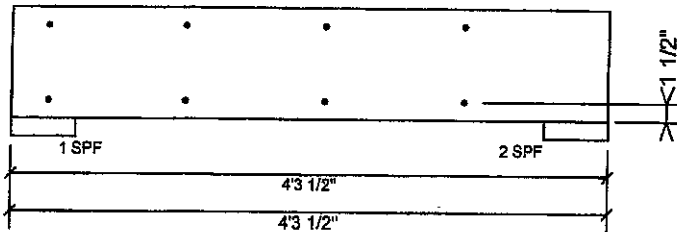
Client:
Project:
Address:

Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 4 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	42.8 %
Load	109.5 PLF
Yield Limit per Foot	255.6 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DATE OF TAM 17/10/2019
STRUCTURAL
ALPHA LUMBER GROUP

Manufacturer Info	Tamarack Roof Trusses 3268 North Service Rd., ON Canada L7N3G2 (905) 335-1115
	TAMARACK LUMBER INC ALPHA LUMBER GROUP

This design is valid until 12/11/2021



TECH-NOTES

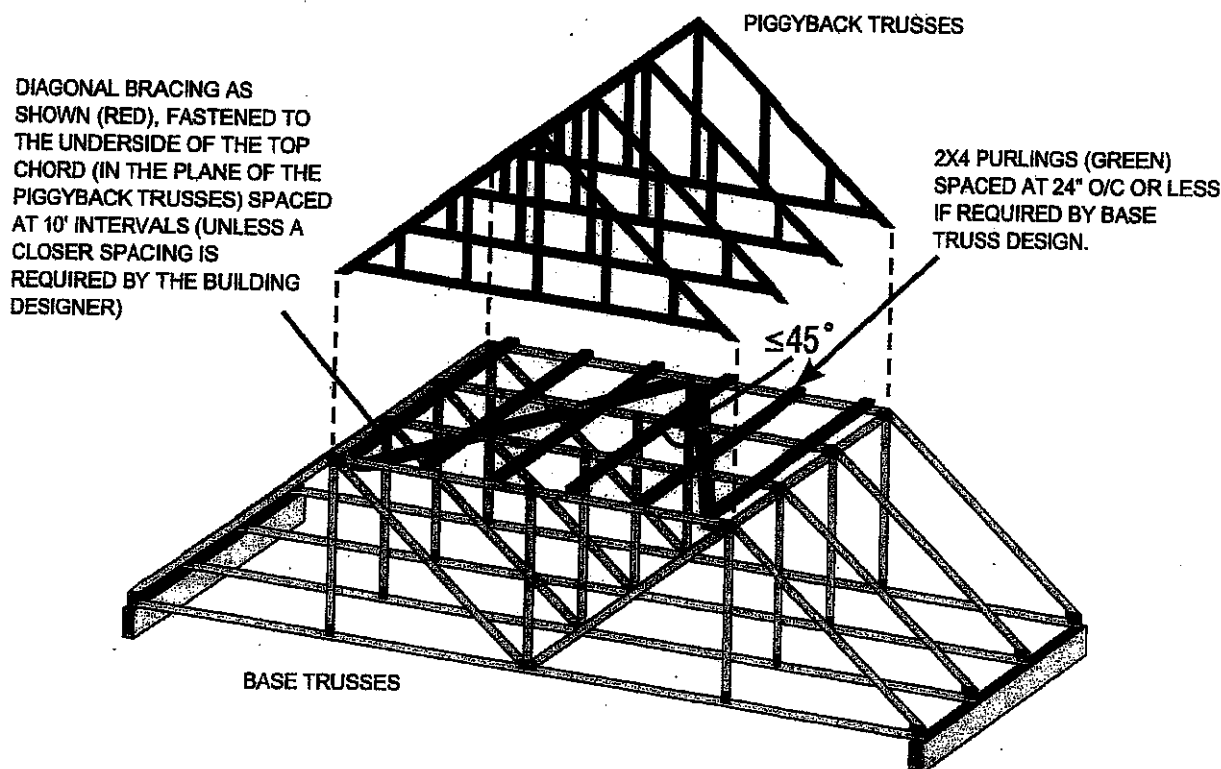
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

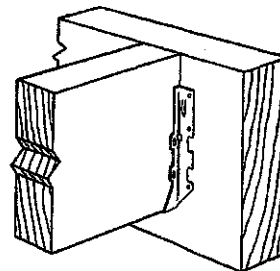
Finish: G90 galvanized

Design:

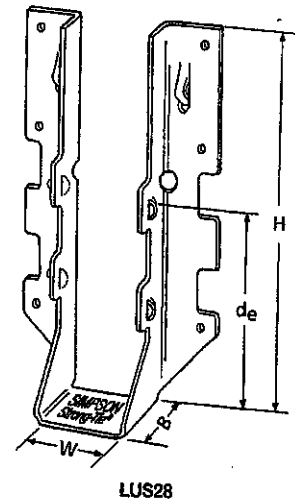
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



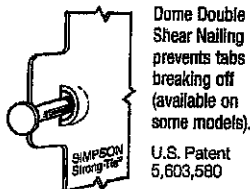
Typical LUS
Installation



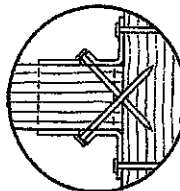
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 1/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 1/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

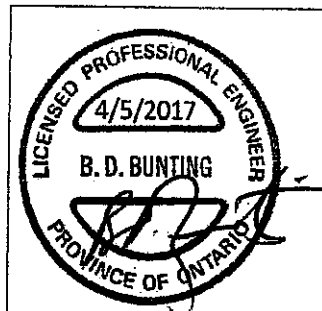
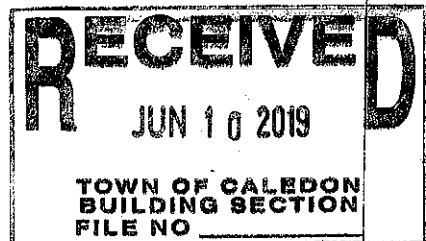
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



DESIGN

For more information, visit us online at www.simpsonstrongtie.com or call 1-800-999-5099. We are committed to providing you with the highest quality products and service. Our products are designed to meet or exceed all applicable codes and standards. We are proud to be a part of the Simpson Strong-Tie family.

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

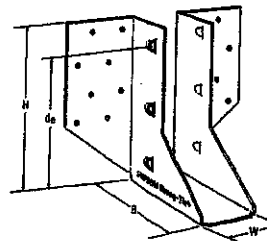
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

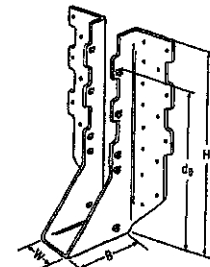
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

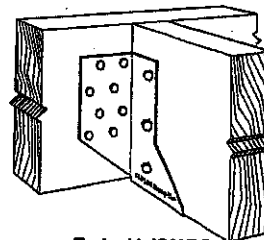
- See current catalogue for options



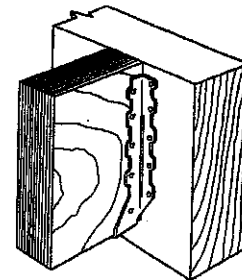
LJS26DS



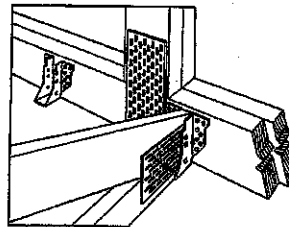
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



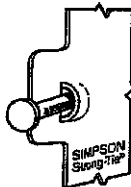
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

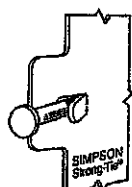
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K ₁ =1.15) lb.	Normal (K ₂ =1.00) lb.	Uplift (K ₁ =1.15) lb.	Normal (K ₂ =1.00) lb.
LJS26DS	18	1 1/8	5	3 1/2	4 3/4	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	18	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	18	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/4	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d₁ is the distance from the seat of the hanger to the highest joist nail.

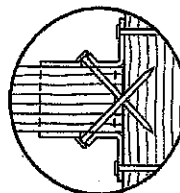


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,803,580

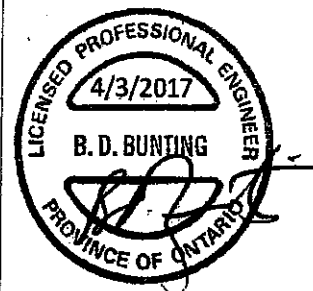


Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



(800) 999-5099
strongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

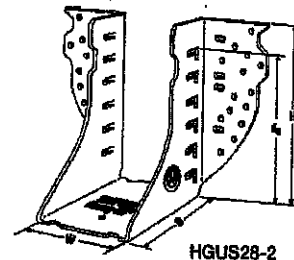
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

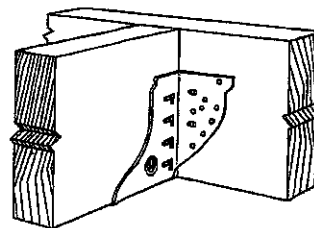
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

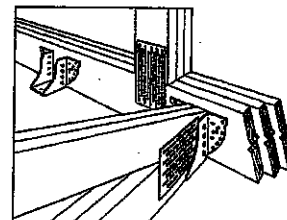
- See current catalogue for options



HGUS28-2



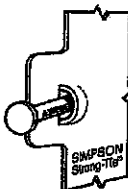
Typical HGUS
Installation



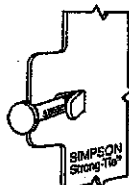
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁ ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/2	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/2	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/2	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/2	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/2	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/2	(66) 16d	(22) 16d	10130	16400	7195	11645

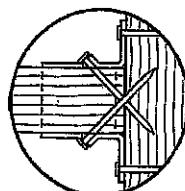
1. d₁ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,803,580



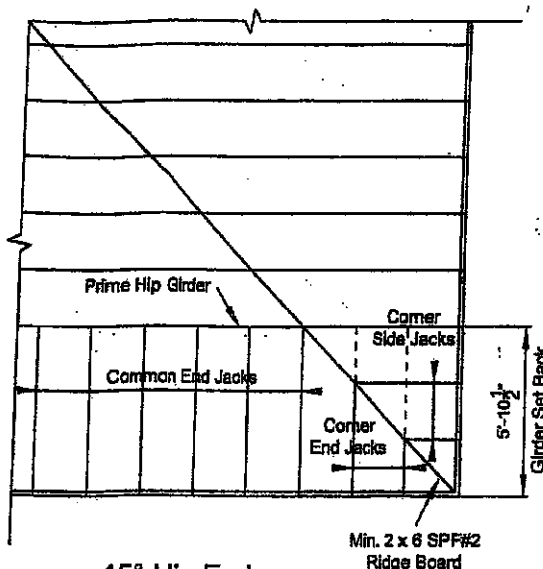
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



(800) 999-5099
strongtie.com



45° Hip End

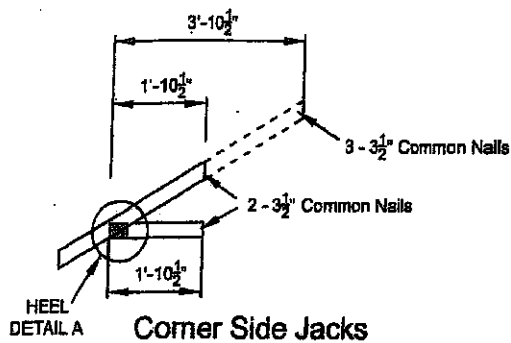
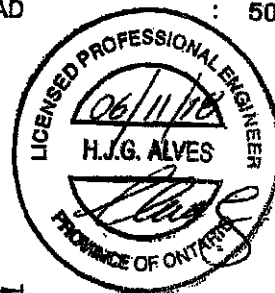
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

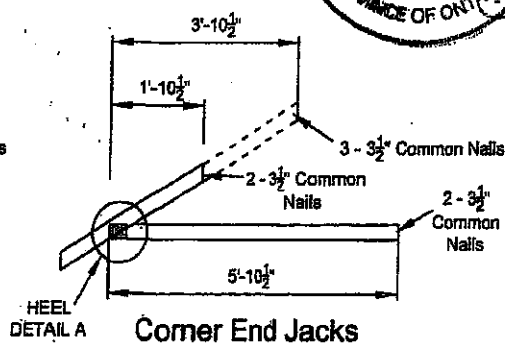
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

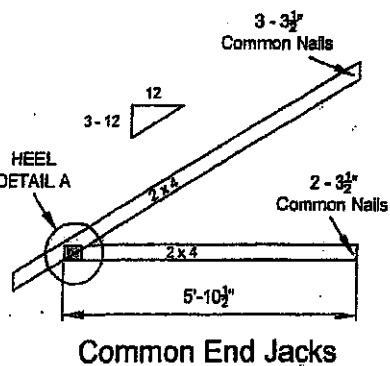
TOTAL LOAD : 50.5 P.S.F.



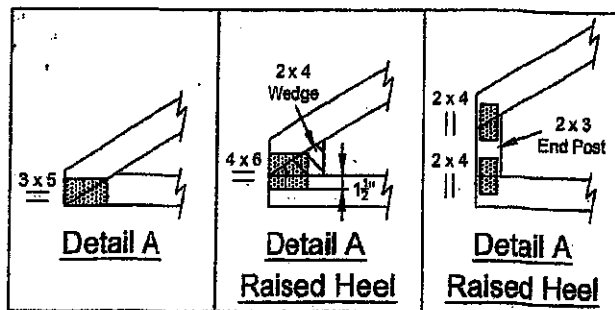
Corner Side Jacks



Corner End Jacks

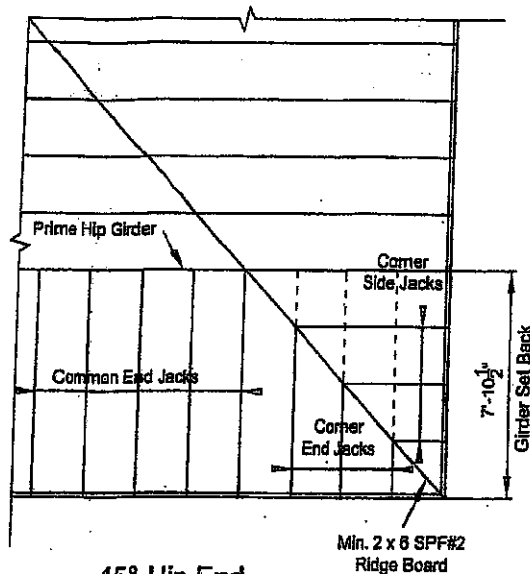


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9.0 B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

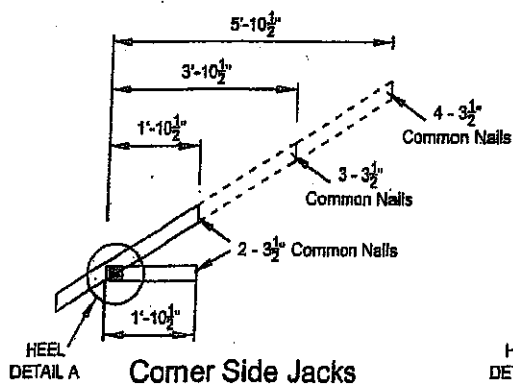
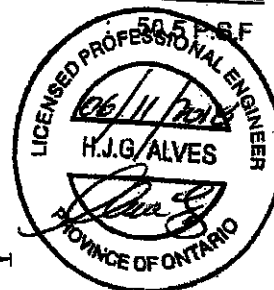
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

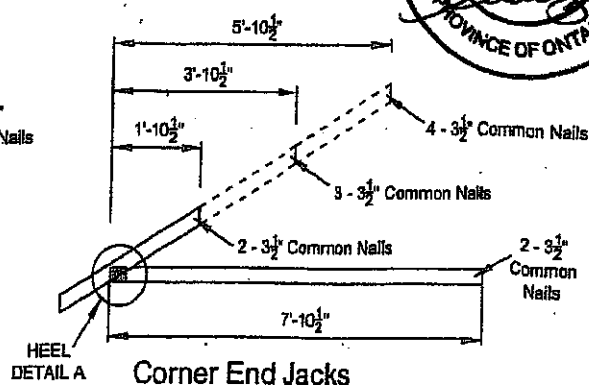
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

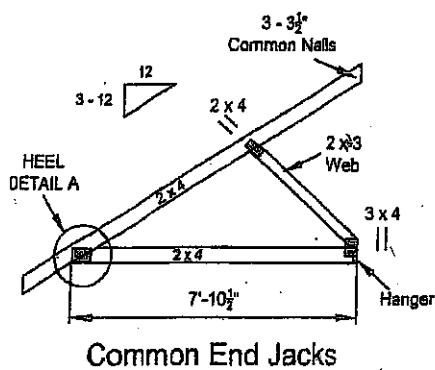
TOTAL LOAD



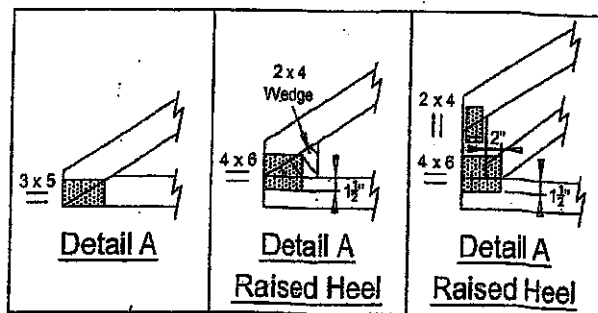
Corner Side Jacks



Corner End Jacks



Common End Jacks

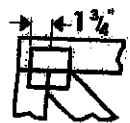


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

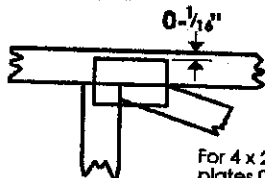
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/2" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

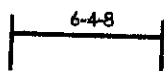


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

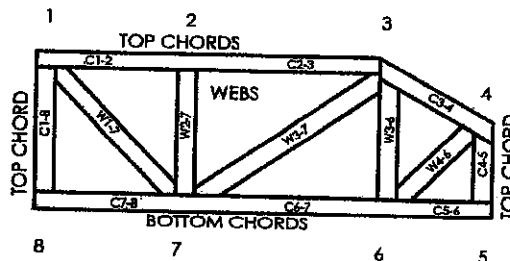
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

T-1800218

Feb 09, 2018