

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

DESIGN LOADS:		
GS LOAD	1.8	KPA
TC DEAD	6	PSF
BC LIVE	10.5	PSF
BC DEAD	7	PSF

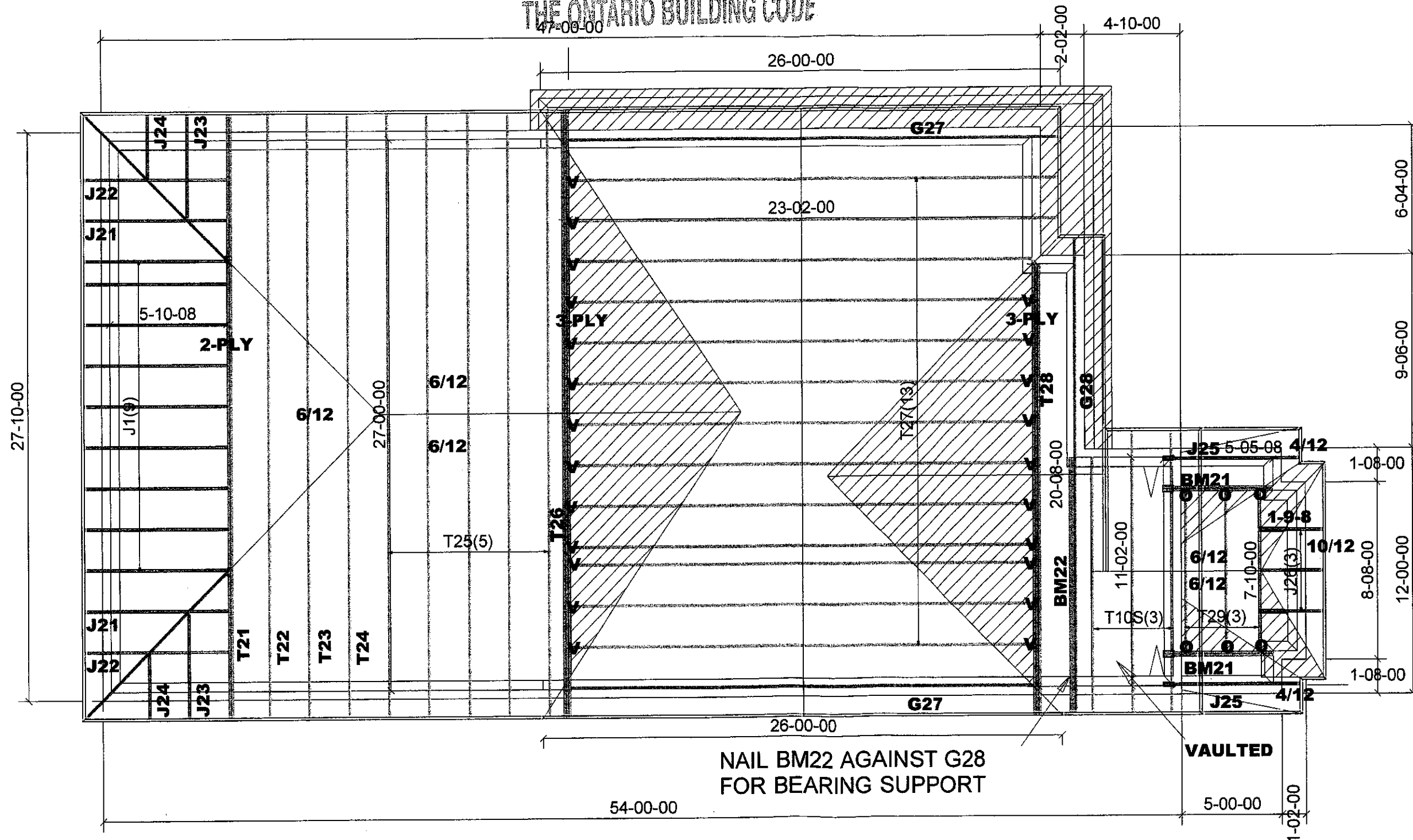
TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

BM21,22: 2-2X10

DENOTES
CONVENTIONAL
FRAMING



RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



MI 2248



Builder / Location:		
GREEN PARK HOMES / CALEDON		
Project: LAMBERT LANE PH.2		
Date: 5/6/2019	Sales: Mario DiCano	Designer: J

Model / Elevation:
BARTON 2/2

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DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 2
 Lot #:
 Elevation: 2

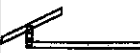
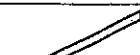
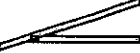
Job Track: 50033
 PlanLog: 201008
 Layout ID: 402947
 Ref #
 Page: 1 of 2
 Date: 05/06/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	3	T10S Scissor	9 /12 7 /12	11-02-00	5-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	145.11 94.50		
	1 2-ply	T21 Hip Girder	6 /12	27-00-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	217.92 138.00		
	1	T22 Hip	6 /12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	109.31 68.83		
	1	T23 Hip	6 /12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	110.98 70.67		
	1	T24 Hip	6 /12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	112.6 70.67		
	5	T25 Common	6 /12	27-00-00	7-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	545.05 336.67		
	1 3-ply	T26 Common Girder	6 /12	27-00-00	7-11-00	2 x 4 2 x 6	1-05-00 1-05-00	1-02-00 1-02-00	383.04 234.00		
	13	T27 Common	9 /12	23-02-00	10-02-08	2 x 4		1-06-04 1-06-04	1396.45 875.33		
	2	G27 GABLE	9 /12	23-02-00	10-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	236.55 154.67		
	1 3-ply	T28 Common Girder	9 /12	20-08-00	9-03-04	2 x 4 2 x 6	1-05-00 1-05-00	1-06-04 1-06-04	324.72 199.50		
	1	G28 GABLE	9 /12	20-08-00	9-03-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	115.75 74.50		
	3	T29 Common	6 /12	7-10-00	2-06-04	2 x 4		6-12 6-12	66.4 43.00		
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	2	J21 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		

		DELIVERY SHIPLIST	
	Lumber Yard:	TAMARACK LUMBER	Job Track: 50033
	Builder:	GREEN PARK HOMES	PlanLog: 201008
	Project:	LAMBERT LANE PH.2	Layout ID: 402947
	Location:	CALEDON	Ref #
	Model:	BARTON 2	Page: 2 of 2
	Lot #:		Date: 05/06/2019
Elevation:	2	Designer:	
			Sales Rep: Mario DiCano

Roof Trusses

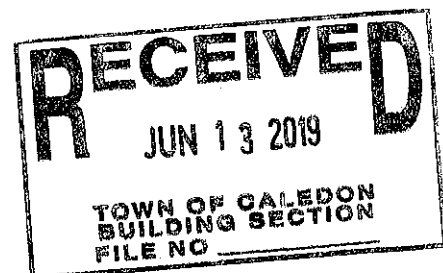
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	J22 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J23 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	2	J24 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J25 GABLE	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	28.9 18.67		
	3	J26 Jack-Open	10 /12	1-09-08	2-11-07	2 x 4	1-03-08	1-00-07 2-06-06	22.79 16.00		

TOTAL # TRUSS= 60 TOTAL BFT OF ALL TRUSSES= 2544.34 BFT. TOTAL WEIGHT OF ALL TRSSES 4051.32 LBS

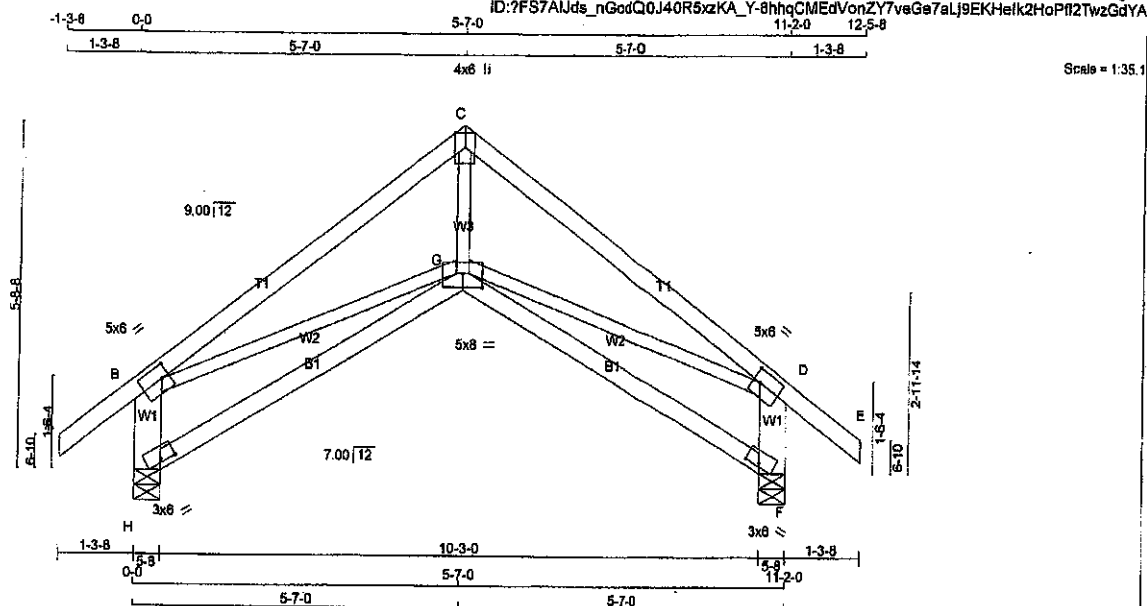
HARDWARE

QTY	TYPE	MODEL	LENGTH
23	Hardware	LJS26DS	
6	Hardware	LUS24	

TOTAL NUMBER OF ITEMS= 29



JOB NAME 402946	TRUSS NAME T10S	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 3 X 50 = 150 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - B	2x4	DRY	No.2	SPF		
H - B	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
H - G	2x4	DRY	No.2	SPF		
G - F	2x4	DRY	No.2	SPF		

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	8.0	2.50	1.50
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW-t	MT20	5.0	6.0	2.50	1.50
F	BVM1-i	MT20	3.0	6.0	0.50	3.00
G	BBWWW-p	MT20	5.0	8.0	2.75	4.00
H	BVM1-i	MT20	3.0	6.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
H	928	0	0	5-8
F	928	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	883	404 / 0	117 / 0	0 / 0	0 / 0	182 / 0
F	883	404 / 0	117 / 0	0 / 0	0 / 0	182 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX.	MAX.	UNBRACED	MEMB.	WEBS	MAX. FACTORED
		FORCE	(PLF)			CSI (LC)	LENGTH	FR-TO			FORCE
FR-TO											
A-B	0 / 42								G-C	0 / 819	0.18 (1)
B-C	-1157 / 0								B-G	0 / 974	0.22 (1)
C-D	-1157 / 0								G-D	0 / 974	0.22 (1)
D-E	0 / 42										
H-B	-819 / 0										
F-D	-819 / 0										
H-G	0 / 0										
G-F	0 / 0										

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 28.1	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = L/916 (0.15")

CSI: TC=0.44/1.00 (B-C:1), BC=0.28/1.00 (G-H:3),
WB=0.22/1.00 (D-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

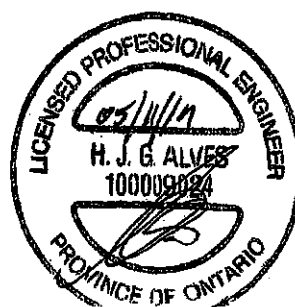
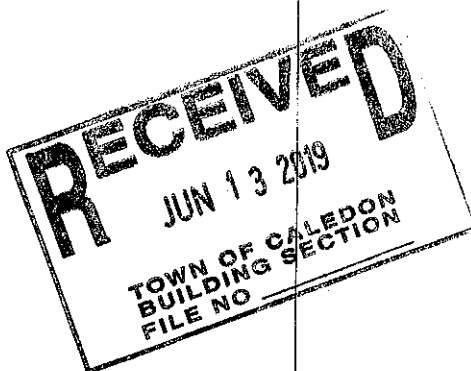
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1687 788 1987 1656	

PLATE PLACEMENT TOL = 0.250 inches

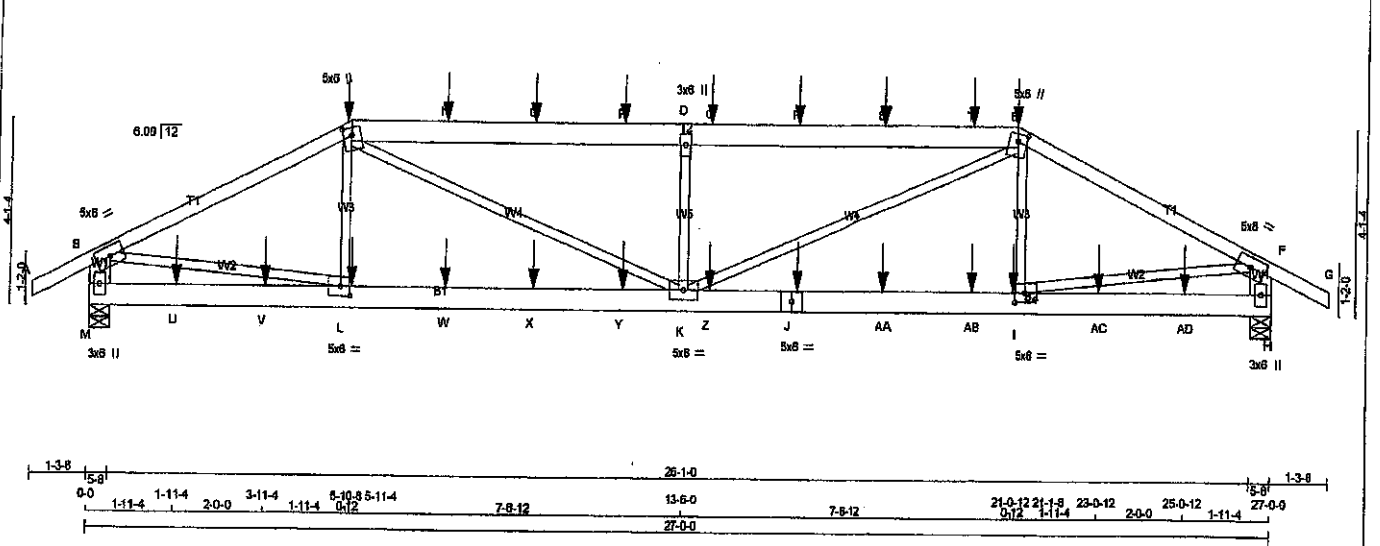
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.62 (B) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

DRWG NO. TAM 17911524
STRUCTURAL
COMPONENT ONLY



JOB NAME 402939	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 5 Nov 17 2018 Mitak Industries, Inc. Tue May 7 17:48:13 2019 Page 1 ID: yH3luQzwKBaXQAh7hsfBURzKCTp-qVvnxrdSag8XylIP_FslaBCy48h4xqRDaeIHyzclqdVW 27-0-0 28-3-8 1-3-8 Scale = 1:48.8	



LUMBER	N. L. & A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C 1 12	SIDE(61.0)	
E-G 1 12	SIDE(61.0)	
C-E 2 12	SIDE(61.0)	
M-B 2 12	TOP	
H-F 2 12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
M-J 2 12	SIDE(183.1)	
J-H 2 12	SIDE(183.1)	
WEBS: (0.122"x3") SPIRAL NAILS		
L-C 1 8	SIDE(1.4)	
I-E 1 8	SIDE(1.4)	
2x3 1 6		

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-t	MT20	5.0	8.0		
C TTWW+m	MT20	5.0	6.0	2.00	2.50
D TMVW-t	MT20	3.0	8.0		
E TTWW+m	MT20	5.0	6.0	2.00	2.50
F TMVW-t	MT20	5.0	8.0		
H BMV1+p	MT20	3.0	8.0		
I BMVW-t	MT20	5.0	6.0	2.50	2.75
J BS-t	MT20	5.0	6.0		
K BMVW-t	MT20	5.0	8.0		
L BMVW-t	MT20	5.0	8.0	2.50	2.75
M BMVW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX	BRG
M	3270	0	3270	0	0	5-8	5-8		
H	3325	0	3325	0	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
M	2443	1351 / 0	487 / 0	0 / 0	0 / 0	605 / 0	0 / 0	0 / 0
H	2485	1373 / 0	497 / 0	0 / 0	0 / 0	618 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX. (PLF)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0 / 31	-102.1	-102.1 0.07 (1)	10.00	L-C	-90 / 347	0.04 (3)
B-C	-4802 / 0	-102.1	-102.1 0.54 (1)	3.92	C-K	0 / 2416	0.30 (1)
C-N	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20	K-D	-1599 / 0	0.19 (1)
N-O	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20	D-E	0 / 2317	0.29 (1)
O-P	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20	E-F	-71 / 381	0.05 (3)
P-Q	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20	F-L	0 / 4343	0.54 (1)
Q-R	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20	L-F	0 / 4433	0.55 (1)
R-S	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20			
S-T	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20			
T-E	-4883 / 0	-102.1	-102.1 0.48 (1)	4.20			
E-F	-4902 / 0	-102.1	-102.1 0.55 (1)	3.88			
F-G	0 / 31	-102.1	-102.1 0.07 (1)	10.00			
M-B	-3144 / 0	0.0	0.0 0.11 (1)	7.78			
H-F	-3201 / 0	0.0	0.0 0.11 (1)	7.74			
M-U	0 / 0	-38.5	-38.5 0.13 (3)	10.00			
U-V	0 / 0	-38.5	-38.5 0.13 (3)	10.00			
V-L	0 / 0	-38.5	-38.5 0.13 (3)	10.00			
L-W	0 / 4307	-38.5	-38.5 0.38 (1)	10.00			
W-X	0 / 4307	-38.5	-38.5 0.38 (1)	10.00			
X-Y	0 / 4307	-38.5	-38.5 0.38 (1)	10.00			
Y-K	0 / 4307	-38.5	-38.5 0.38 (1)	10.00			
K-Z	0 / 4398	-38.5	-38.5 0.39 (1)	10.00			
Z-J	0 / 4398	-38.5	-38.5 0.39 (1)	10.00			
J-AA	0 / 4398	-38.5	-38.5 0.39 (1)	10.00			
AA-AB	0 / 4398	-38.5	-38.5 0.39 (1)	10.00			
AB-I	0 / 4398	-38.5	-38.5 0.39 (1)	10.00			
I-AC	0 / 0	-38.5	-38.5 0.13 (3)	10.00			
AC-AD	0 / 0	-38.5	-38.5 0.13 (3)	10.00			
AD-H	0 / 0	-38.5	-38.5 0.13 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
5-10-8	-473	-473	-	-	FRONT	VERT	TOTAL	-	-
21-1-8	-473	-473	-	-	FRONT	VERT	TOTAL	-	-
21-0-12	-55	-70	-	-	FRONT	VERT	TOTAL	-	-
18-1-4	-55	-70	-	-	FRONT	VERT	TOTAL	-	-
5-11-4	-55	-70	-	-	FRONT	VERT	TOTAL	-	-
8-1-4	-123	-123	-	-	FRONT	VERT	TOTAL	-	-

TOTAL WEIGHT = 2 X 126 = 253 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.80")
CALCULATED VERT. DEFL. (LL) = L/899 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.80")
CALCULATED VERT. DEFL. (TL) = L/989 (0.22")

CSI: TC=0.55/1.00 (E-F-1), BC=0.38/1.00 (I-K-1), WB=0.55/1.00 (F-1-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

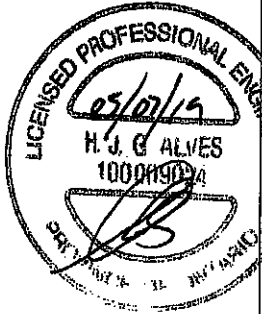
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
618 354 1657 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

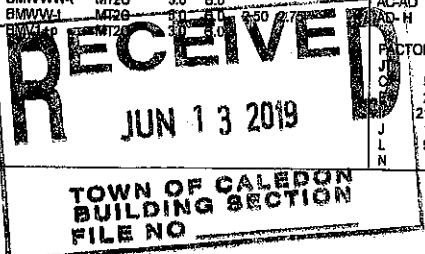
SI GRIP=0.88 (L) (INPUT = 0.90)
SI METAL=0.55 (I) (INPUT = 1.00)



DATE NO. TAM 17910779

1/2

CONTINUED ON PAGE 2



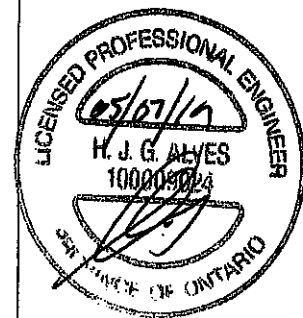
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T21	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:48:13 2019 Page 2
 ID:ylH3luQzwKBsXQAh?hsfBURzKCTp-qVVnxrdSag6XyilP FslaBCy48h4xgRDaeIHyczdW

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	10-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
P	12-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
Q	14-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
R	16-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
S	18-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
T	20-1-4	-123	-123	---	FRONT	VERT	TOTAL	---	---
U	1-11-4	-55	-74	---	FRONT	VERT	TOTAL	---	---
V	3-11-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
W	8-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
X	10-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
Y	12-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
Z	14-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
AA	16-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	20-1-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	23-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	25-0-12	-55	-74	---	FRONT	VERT	TOTAL	---	---

THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE ONTARIO BUILDING CODE

RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO _____



ENGINE NO. TAM 1910779
 STRUCTURAL
 CONTRACT ONLY

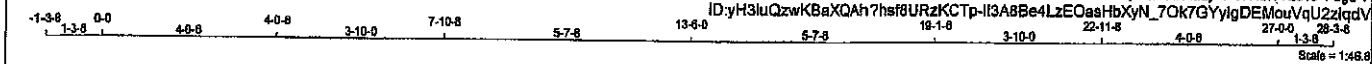
3/2

JOB NAME 402939	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID: yH3luQzwKBaXQAh7hsf8URzKCTp-l3A8Be4LzEOasHbXyN_7OK7GYyigDEMouVqU2zldv



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+1	MT20	5.0	6.0	
D	TTWw+m	MT20	5.0	6.0	2.25 2.00
E	TTWw+w	MT20	2.0	4.0	
F	TTWw+m	MT20	5.0	6.0	2.25 2.00
G	TMVW+1	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW+1	MT20	4.0	9.0	Edge
K	BMVW+1	MT20	4.0	9.0	
L	BS-1	MT20	3.0	4.0	
M	BMVWw+1	MT20	4.0	9.0	
N	BMVW+1	MT20	4.0	9.0	
O	BMVW+1	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	RECRD		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2037	0	2037	0	0	5-8	5-8
J	2037	0	2037	0	0	5-8	5-8

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS			
O	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FACTORED
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	(LBS)	CS1 (LC)	
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	C-N	-3 / 138	0.03 (3)	
B-C	0 / 19	-102.1	-102.1	0.23 (1)	10.00	N-D	0 / 375	0.08 (2)	
C-D	-2533 / 0	-102.1	-102.1	0.25 (1)	4.13	D-M	0 / 647	0.15 (1)	
D-E	-2746 / 0	-102.1	-102.1	0.52 (1)	3.68	M-E	-702 / 0	0.27 (1)	
E-F	-2746 / 0	-102.1	-102.1	0.52 (1)	4.13	F-G	0 / 647	0.15 (1)	
F-G	-2533 / 0	-102.1	-102.1	0.25 (1)	4.13	K-F	0 / 375	0.08 (2)	
G-H	0 / 19	-102.1	-102.1	0.23 (1)	10.00	K-G	-3 / 138	0.03 (3)	
H-I	0 / 31	-102.1	-102.1	0.13 (1)	10.00	C-C	-2768 / 0	0.77 (1)	
I-J	-299 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2768 / 0	0.77 (1)	
J-H	-299 / 0	0.0	0.0	0.03 (1)	7.81				
O-N	0 / 2250	-38.5	-38.5	0.70 (2)	10.00				
N-M	0 / 2254	-38.5	-38.5	0.71 (2)	10.00				
M-L	0 / 2254	-38.5	-38.5	0.71 (2)	10.00				
L-K	0 / 2254	-38.5	-38.5	0.71 (2)	10.00				
K-J	0 / 2250	-38.5	-38.5	0.70 (2)	10.00				

TOTAL WEIGHT = 109 lb
(M/F)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.90")
CALCULATED VERT. DEFL.(LL) = L/989 (0.18")
ALLOWABLE DEFL.(TL) = L/380 (0.90")
CALCULATED VERT. DEFL.(TL) = L/989 (0.30")

CS1: TC=0.52/1.00 (D-E-1), BC=0.71/1.00 (K-M-2), WB=0.77/1.00 (G-J-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

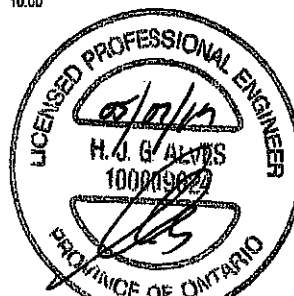
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	618	354	1687	788	1987

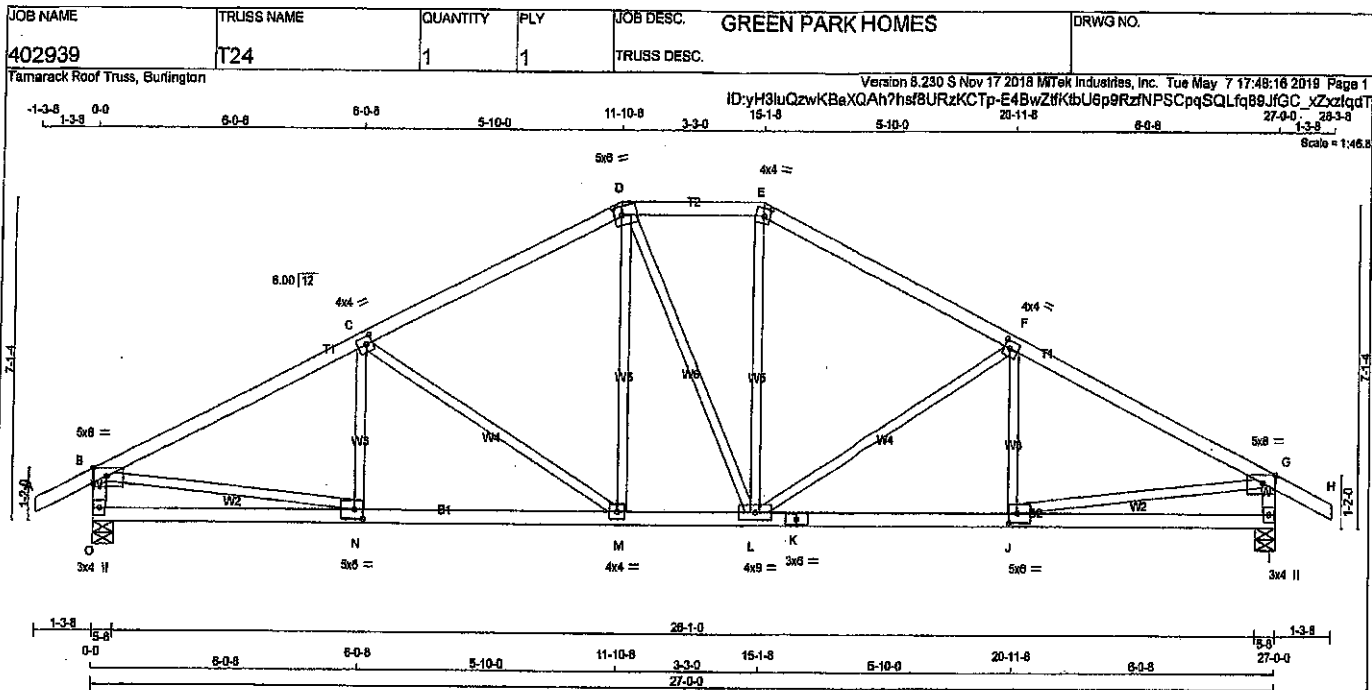
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.73 (L) (INPUT = 1.00)



DESIGN BY **19/10/20**
STRUCTURAL
CHECKED BY



LUMBER					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR
A - D		2x4	DRY	No.2	SPF
D - E		2x4	DRY	No.2	SPF
E - H		2x4	DRY	No.2	SPF
O - B		2x4	DRY	No.2	SPF
I - G		2x4	DRY	No.2	SPF
O - K		2x4	DRY	No.2	SPF
K - I		2x4	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT					
DRY: SEASONED LUMBER					

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTVW-m	MT20	5.0	6.0	2.25	2.00
E	TTVW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
I	BMV1+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0	2.50	2.25
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0	2.50	2.25
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	2037	0	2037	0	5-8	5-8
I	2037	0	2037	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS				
	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
O	1514	893 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
I	1514	893 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0 / 31	-102.1	-102.1 0.13 (1)	10.00	N-C	-84 / 185	0.04 (3)
B-C	-2694 / 0	-102.1	-102.1 0.81 (1)	3.63	C-M	-892 / 0	0.87 (1)
C-D	-2111 / 0	-102.1	-102.1 0.53 (1)	4.11	M-D	0 / 537	0.12 (1)
D-E	-1888 / 0	-102.1	-102.1 0.18 (1)	4.76	D-L	0 / 4	0.00 (1)
E-F	-2113 / 0	-102.1	-102.1 0.53 (1)	4.11	L-E	0 / 541	0.12 (1)
F-G	-2693 / 0	-102.1	-102.1 0.81 (1)	3.63	L-F	-899 / 0	0.86 (1)
G-H	0 / 31	-102.1	-102.1 0.13 (1)	10.00	J-F	-86 / 183	0.04 (3)
O-B	-1942 / 0	0.0	0.0 0.20 (1)	6.04	B-N	0 / 2462	0.55 (1)
I-G	-1942 / 0	0.0	0.0 0.20 (1)	6.04	J-G	0 / 2461	0.55 (1)
O-N	0 / 0	-38.5	-38.5 0.27 (3)	10.00			
N-M	0 / 2438	-38.5	-38.5 0.58 (2)	10.00			
M-L	0 / 1806	-38.5	-38.5 0.40 (1)	10.00			
L-K	0 / 2437	-38.5	-38.5 0.57 (2)	10.00			
K-J	0 / 2437	-38.5	-38.5 0.57 (2)	10.00			
J-I	0 / 0	-38.5	-38.5 0.27 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.12")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.20")

CSI: TC=0.61/1.00 (B-C-1), BC=0.58/1.00 (M-N-2), WB=0.67/1.00 (C-M-1), SS=0.28/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

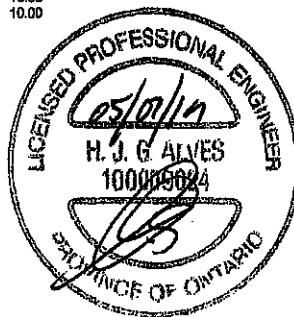
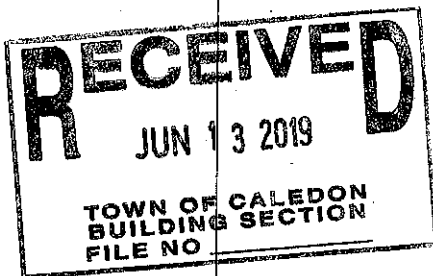
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1897 789 1997 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (E) (INPUT = 0.80)
JSI METAL= 0.71 (K) (INPUT = 1.00)

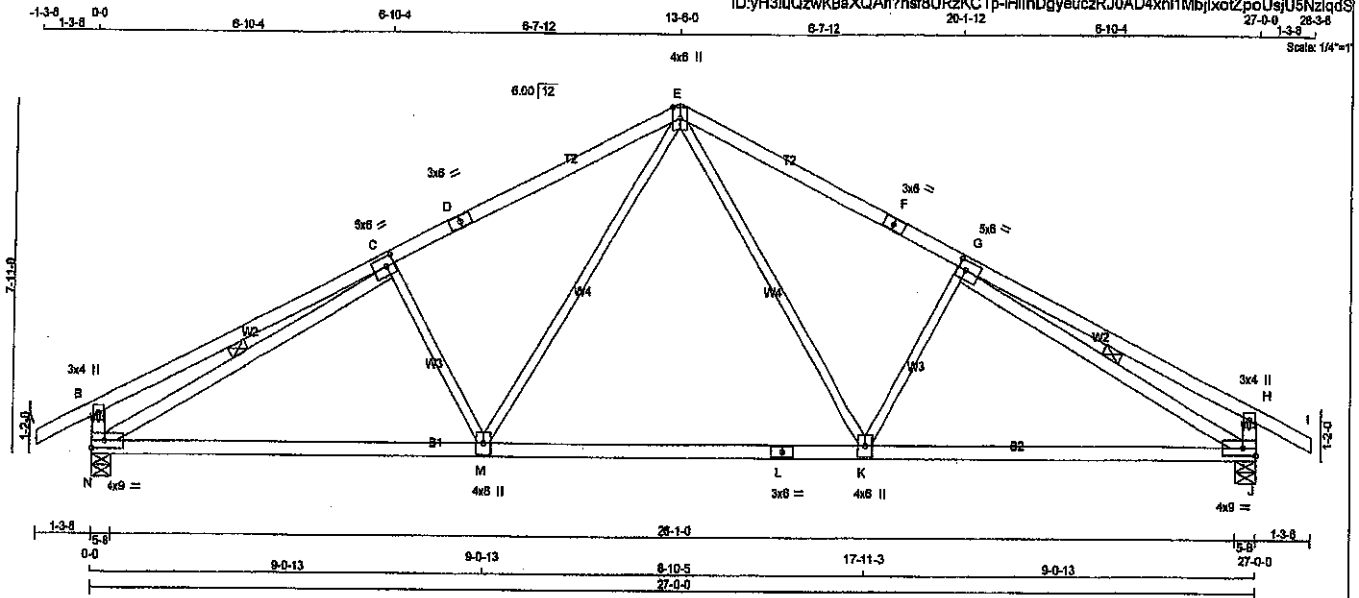


DESIGN BY: TAM 794/0782
CHECKED BY: [Signature]
DATE: 05/07/19

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T25	8	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID: yH3luQzwKBaXQA4h7hsf8URzKCTp-iHInDgyeuczRJ0AD4xh1MbixotZpoUajU5NzldS



TOTAL WEIGHT = 8 X 108 = 872 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+I	MT20	5.0	6.0	2.50 2.25
D	TS-I	MT20	3.0	6.0	
E	TTVW+p	MT20	4.0	6.0	Edge
F	TS-I	MT20	3.0	6.0	
G	TMVW+I	MT20	5.0	6.0	2.50 2.25
H	TMV+p	MT20	3.0	4.0	
J	BMVW+I	MT20	4.0	9.0	Edge
K	BMVW+I	MT20	4.0	6.0	
L	BS-I	MT20	3.0	6.0	
M	BMVW+I	MT20	4.0	6.0	
N	BMVW+I	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
N	2037	0	2037	0
J	2037	0	2037	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
N	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	VERT. LOAD	LC1	MAX	MAX	WEBS	MAX. FACTORED	FORCE	MAX
		(LBS)	(PLF)	(LC)	UNBRAC	LENGTH		(LBS)	(LBS)	(LC)
FR-TO			FROM	TO			FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		E-K	0 / 937	0.21 (1)	
B-C	0 / 37	-102.1	-102.1	0.78 (1)	10.00		K-G	-493 / 7	0.19 (1)	
C-D	-2435 / 0	-102.1	-102.1	0.78 (1)	3.54		M-E	0 / 937	0.21 (1)	
D-E	-2435 / 0	-102.1	-102.1	0.78 (1)	3.54		C-M	-493 / 7	0.19 (1)	
E-F	-2435 / 0	-102.1	-102.1	0.78 (1)	3.54		N-C	-2813 / 0	0.84 (1)	
F-G	-2435 / 0	-102.1	-102.1	0.78 (1)	3.54		G-J	-2813 / 0	0.84 (1)	
G-H	0 / 37	-102.1	-102.1	0.78 (1)	10.00					
H-I	0 / 31	-102.1	-102.1	0.13 (1)	10.00					
N-B	-400 / 0	0.0	0.0	0.04 (1)	7.81					
J-H	-400 / 0	0.0	0.0	0.04 (1)	7.81					
N-M	0 / 2383	-38.5	-38.5	0.85 (2)	10.00					
M-L	0 / 1674	-38.5	-38.5	0.75 (2)	10.00					
L-K	0 / 1674	-38.5	-38.5	0.75 (2)	10.00					
K-J	0 / 2383	-38.5	-38.5	0.85 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/699 (0.27")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/705 (0.48")

CSI: TC=0.78/1.00 (G-H:1), BC=0.85/1.00 (J-K:2), WB=0.84/1.00 (G-J:1), SSI=0.30/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

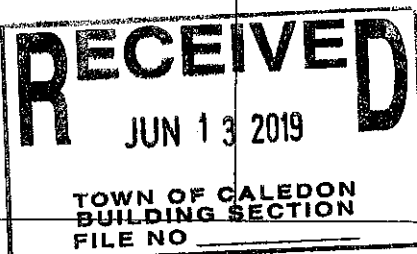
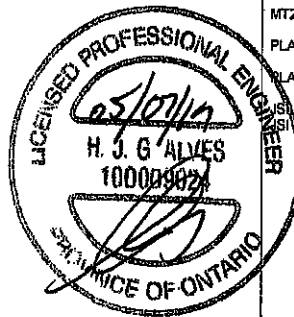
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

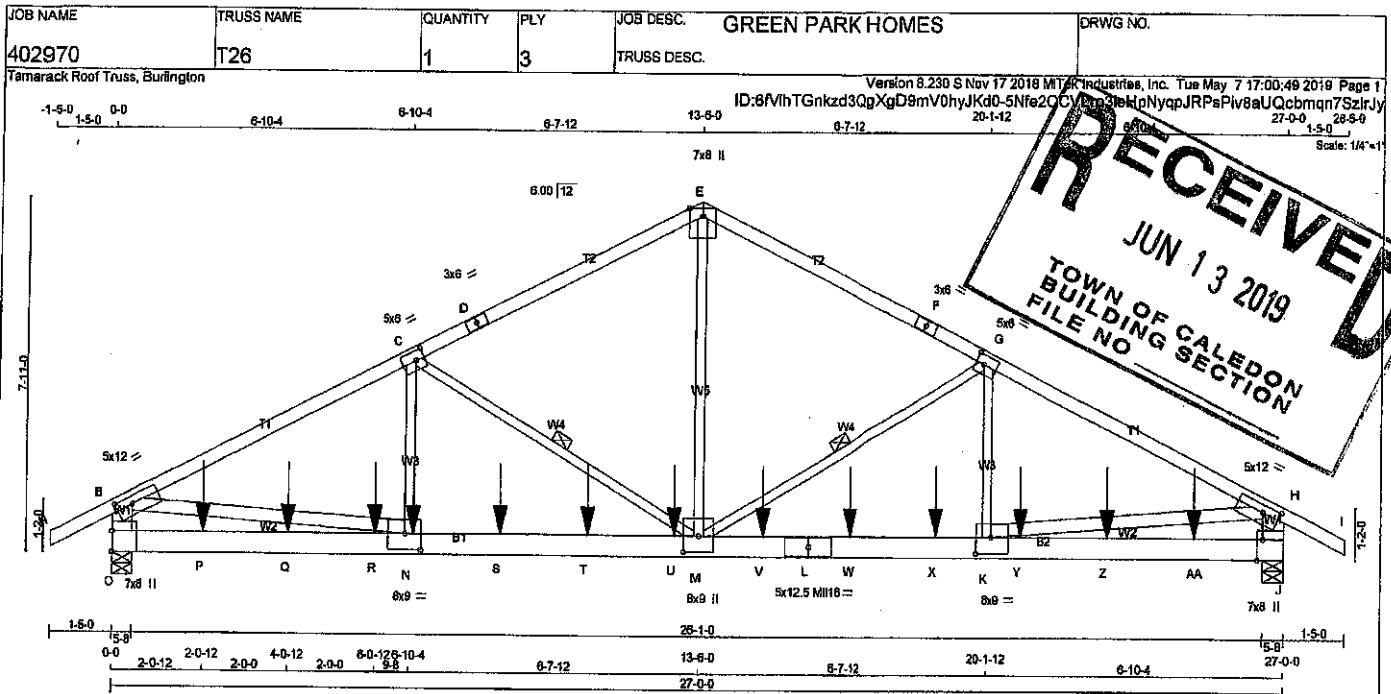
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP= 0.89 (G) (INPUT = 0.80)
METAL= 0.89 (G) (INPUT = 1.00)



17/10/2019
17/10/2019
17/10/2019



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	1850F 1.5E	SPF
D - E	2x4	1850F 1.5E	SPF
E - F	2x4	1850F 1.5E	SPF
F - I	2x4	1850F 1.5E	SPF
O - B	2x6	No.2	SPF
J - H	2x6	No.2	SPF
O - L	2x6	1850F 1.5E	SPF
L - J	2x6	1850F 1.5E	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
B - N 2x4 DRY No.2 SPF
K - H 2x4 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-D 1 12	TOP	
E-F 1 12	TOP	
F-I 1 12	TOP	
O-B 2 12	TOP	
J-H 2 12	TOP	
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
O-L 2 9	SIDE(608.4)	
L-J 2 9	SIDE(414.9)	
WEBS: (0.122"x3") SPIRAL NAILS		
2x3 1 6		
2x4 1 6		

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT PLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-I	MT20	5.0	12.0	2.00	4.50
C TMWW-I	MT20	5.0	8.0	2.50	2.25
D TS-I	MT20	3.0	8.0		
E TTW+P	MT20	7.0	8.0	Edge	
F TS-I	MT20	3.0	8.0		
G TMWW-I	MT20	5.0	8.0	2.50	2.25
H TMWW-I	MT20	5.0	12.0	2.00	4.50
J BMV1+I	MT20	7.0	8.0	Edge	1.50
K BMWW-I	MT20	8.0	9.0	4.25	4.25
L BS-I	MT16	5.0	12.5		
M BMWWWW-I	MT20	8.0	9.0	4.25	4.00
N BMWW-I	MT20	8.0	9.0	4.25	4.25
O BMV1+I	MT20	7.0	8.0	5.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
O	12825	0	12825	0	5-8	5-8
J	12152	0	12152	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
O	9363	5344 / 0	1761 / 0	0 / 0	0 / 0	2278 / 0	0 / 0
J	9032	5144 / 0	1695 / 0	0 / 0	0 / 0	2193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.02 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-M, C-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 MAX (PLF)	MAX. UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 34	-102.1 -102.1 0.04 (1)	10.00	M-E 0 / 11488 0.88 (1)
B-C	-19251 / 0	-102.1 -102.1 0.89 (1)	3.02	M-G -5315 / 0 0.73 (1)
C-D	-13489 / 0	-102.1 -102.1 0.49 (1)	3.66	K-G 0 / 4274 0.32 (1)
D-E	-13489 / 0	-102.1 -102.1 0.49 (1)	3.66	C-M -8154 / 0 0.65 (1)
E-F	-13489 / 0	-102.1 -102.1 0.49 (1)	3.66	N-C 0 / 5079 0.38 (1)
F-G	-13489 / 0	-102.1 -102.1 0.49 (1)	3.66	B-N 0 / 17352 0.93 (1)
G-H	-18458 / 0	-102.1 -102.1 0.83 (1)	3.09	K-H 0 / 16838 0.89 (1)
H-I	0 / 34	-102.1 -102.1 0.04 (1)	10.00	
O-B	-11011 / 0	0.0 0.0 0.24 (1)	5.58	
J-H	-10575 / 0	0.0 0.0 0.23 (1)	5.68	
O-P	0 / 0	-38.5 -38.5 0.50 (1)	10.00	
P-Q	0 / 0	-38.5 -38.5 0.50 (1)	10.00	
Q-R	0 / 0	-38.5 -38.5 0.50 (1)	10.00	
R-N	0 / 0	-38.5 -38.5 0.50 (1)	10.00	
N-S	0 / 17242	-38.5 -38.5 0.81 (1)	10.00	
S-T	0 / 17242	-38.5 -38.5 0.81 (1)	10.00	
T-U	0 / 17242	-38.5 -38.5 0.81 (1)	10.00	
U-M	0 / 17242	-38.5 -38.5 0.81 (1)	10.00	
M-V	0 / 16533	-38.5 -38.5 0.94 (1)	10.00	
V-L	0 / 16533	-38.5 -38.5 0.94 (1)	10.00	
L-W	0 / 16533	-38.5 -38.5 0.94 (1)	10.00	
W-X	0 / 16533	-38.5 -38.5 0.94 (1)	10.00	
X-K	0 / 16533	-38.5 -38.5 0.94 (1)	10.00	
K-Y	0 / 0	-38.5 -38.5 0.53 (1)	10.00	
Y-Z	0 / 0	-38.5 -38.5 0.53 (1)	10.00	
Z-AA	0 / 0	-38.5 -38.5 0.53 (1)	10.00	
AA-J	0 / 0	-38.5 -38.5 0.53 (1)	10.00	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	6-11-4	-1591	-1591		FRONT	VERT	TOTAL		
P	2-0-12	-1591	-1591		FRONT	VERT	TOTAL		
Q	4-0-12	-1591	-1591		FRONT	VERT	TOTAL		
R	6-0-12	-1591	-1591		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/786 (0.42")

CSI: TC=0.88/1.00 (B-C:1), BC=0.84/1.00 (K-M:1), WB=0.93/1.00 (B-N:1), SSI=0.70/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION (PSI)	(PLI)	(PLI)
MT20	618	354	1667	788	1587
MT16	438	276	2341	1245	4283

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.96 (K) (INPUT = 1.00)

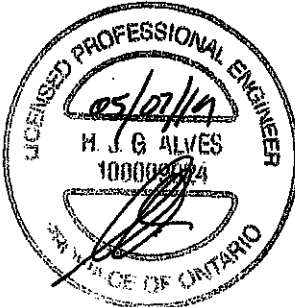
DRWG NO. TAM 1910736
STRUCTURAL
COMPONENT ONLY

JOB NAME 402970	TRUSS NAME T26	QUANTITY 1	PLY 3	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:00:49 2019 Page 2 ID:8fVlhTGnkzd3QgXgD9mV0hyJKd0-5Nfe2OCVLro3leHpNyqpJRPpPiv8aUQcbmqn7SzlrJy	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	8-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
S	10-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
T	12-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
U	14-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
V	16-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
W	18-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
X	20-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
Y	22-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
Z	24-11-4	-1591	-1591	—	FRONT	VERT	TOTAL	—	—
AA									

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

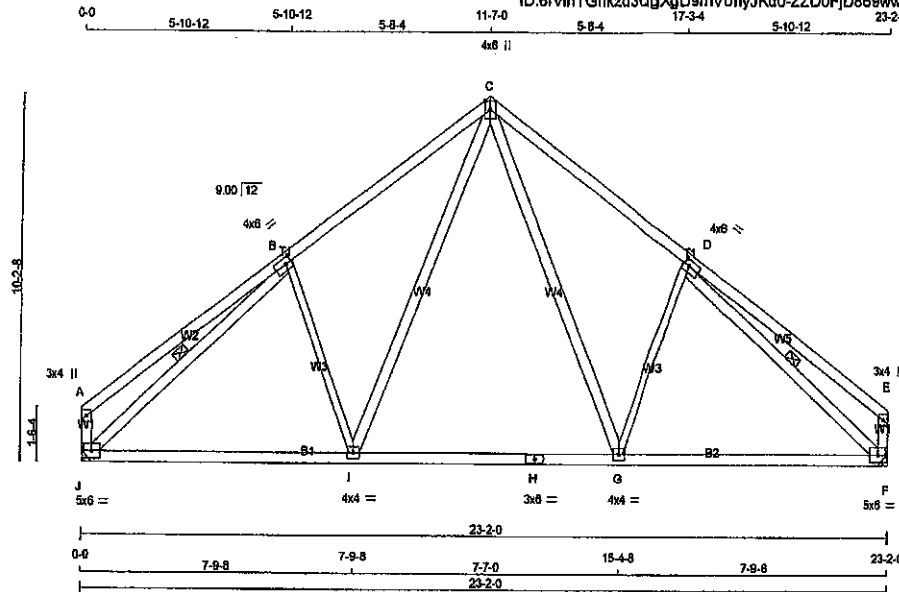


DWG NO. TAM **1910736**
 STRUCTURAL
 COMPONENT ONLY **3/6**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402970	T27	13	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:00:50 2019 Page 1
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Scale = 1:59.2

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
J - A	2x4	DRY	No.2
F - E	2x4	DRY	No.2
J - H	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS	2x4	DRY	No.2
EXCEPT			
G - D	2x3	DRY	No.2
B - I	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMV+p	MT20	3.0	4.0	
B	TMVW+1	MT20	4.0	6.0	2.00 2.00
C	TTVW+p	MT20	4.0	6.0	Edge
D	TMVW+1	MT20	4.0	6.0	2.00 2.00
E	TMV+p	MT20	3.0	4.0	
F	BMVW+1	MT20	5.0	6.0	
G	BMVW+1	MT20	4.0	4.0	
H	BS+1	MT20	3.0	6.0	
I	BMVW+1	MT20	4.0	4.0	
J	BMVW+1	MT20	5.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
J	1628	0	1628	0	0
F	1628	0	1628	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J, F. MINIMUM BEARING LENGTH AT JOINT J = 3'-8", JOINT F = 3'-8".

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1217	673 / 0	243 / 0	0 / 0	0 / 0	301 / 0	0 / 0
F	1217	673 / 0	243 / 0	0 / 0	0 / 0	301 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.71 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF B-J, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 142	-102.1 -102.1	0.57 (1)	10.00	C-G	0 / 728	0.12 (1)
B-C	-1560 / 0	-102.1 -102.1	0.49 (1)	4.71	G-D	-379 / 12	0.23 (1)
C-D	-1555 / 0	-102.1 -102.1	0.50 (1)	4.71	D-E	0 / 728	0.12 (1)
D-E	0 / 143	-102.1 -102.1	0.57 (1)	10.00	E-F	-366 / 10	0.24 (1)
J-A	-228 / 0	0.0 0.0	0.02 (1)	7.81	J-B	-1853 / 0	0.56 (1)
F-E	-221 / 0	0.0 0.0	0.02 (1)	7.81	D-F	-1859 / 0	0.56 (1)
J-I	0 / 1341	-38.5 -38.5	0.57 (2)	10.00			
I-H	0 / 949	-38.5 -38.5	0.52 (2)	10.00			
H-G	0 / 949	-38.5 -38.5	0.52 (2)	10.00			
G-F	0 / 1343	-38.5 -38.5	0.57 (2)	10.00			

TOTAL WEIGHT = 13 X 112 = 1456 lb (MIF)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.77")
CALCULATED VERT. DEFL.(LL) = L/989 (0.15")
ALLOWABLE DEFL.(TL) = L/360 (0.77")
CALCULATED VERT. DEFL.(TL) = L/989 (0.24")

CSI: TC=0.57/1.00 (D-E:1), BC=0.57/1.00 (F-G:2), WB=0.58/1.00 (D-F:1), SS=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

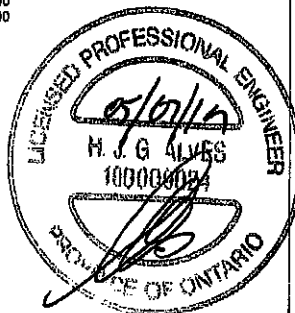
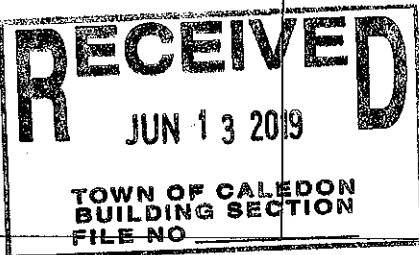
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 768 1987 1656

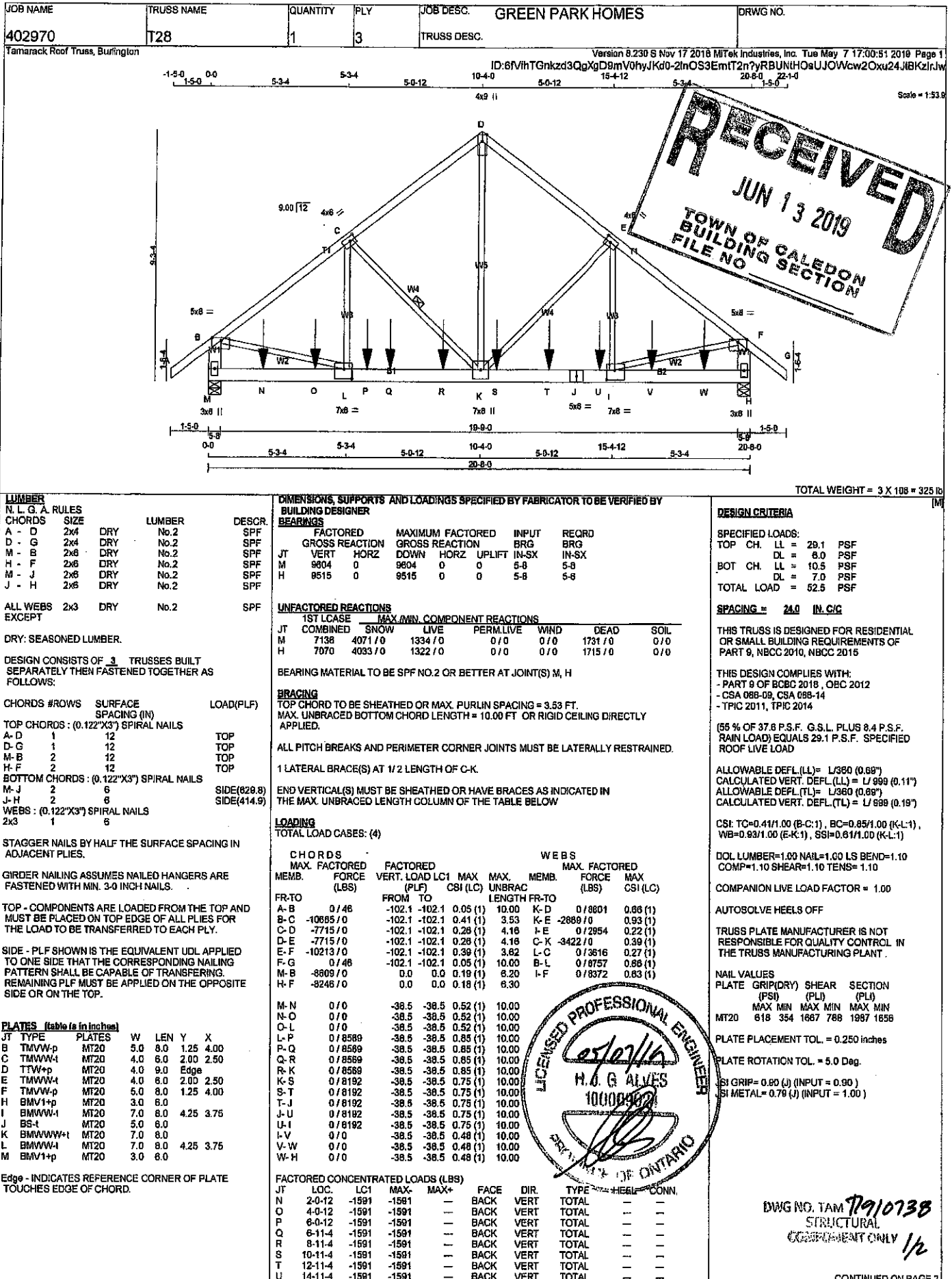
PLATE PLACEMENT TOL. = 0.250 inches

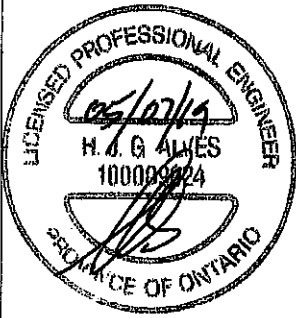
PLATE ROTATION TOL. = 5.0 Deg.

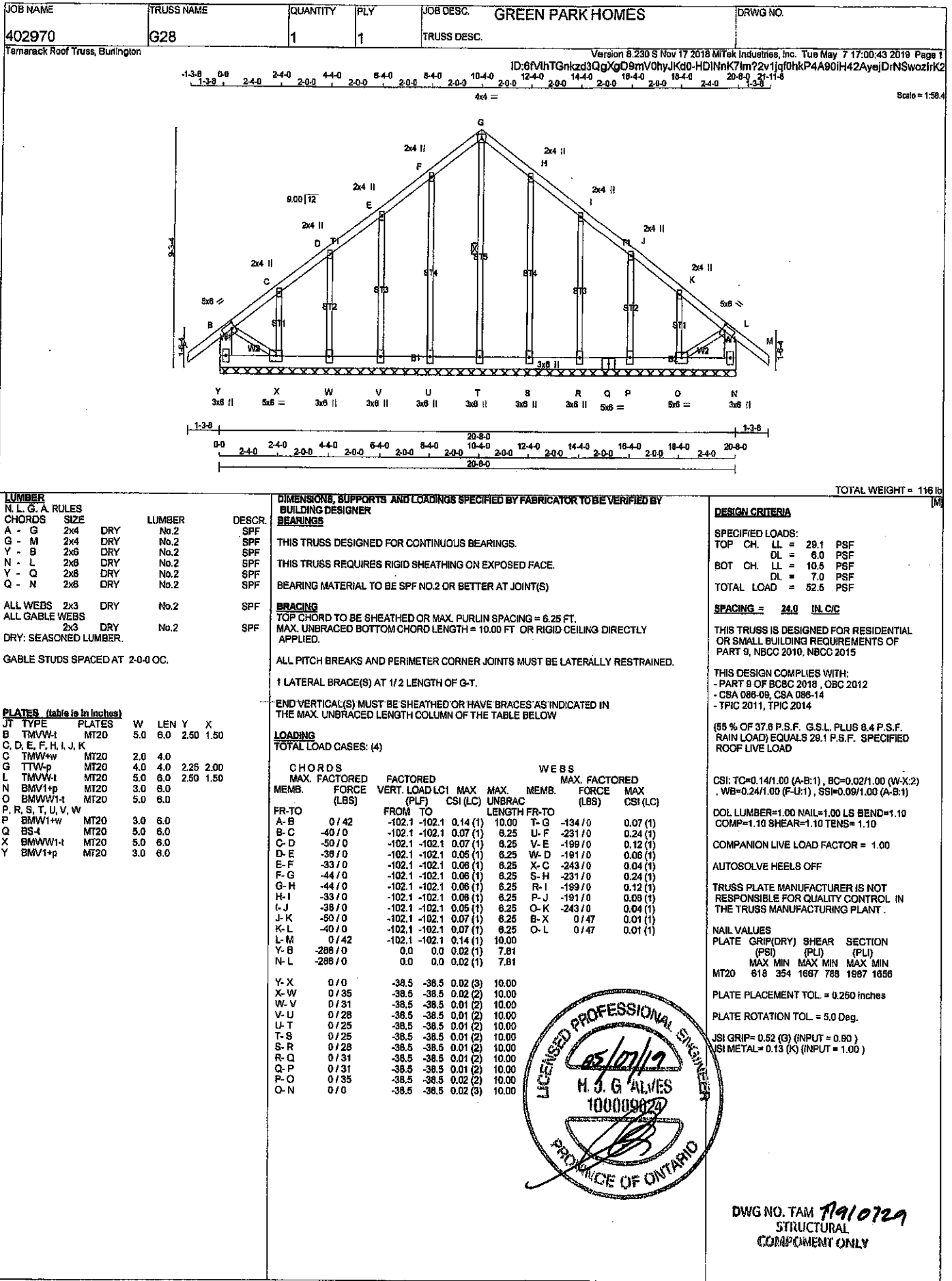
JSI GRIP= 0.90 (D) (INPUT = 0.90)
JSI METAL= 0.49 (B) (INPUT = 1.00)



DWG NO. TAM 1910737
STRUCTURAL
QUANTITY ONLY

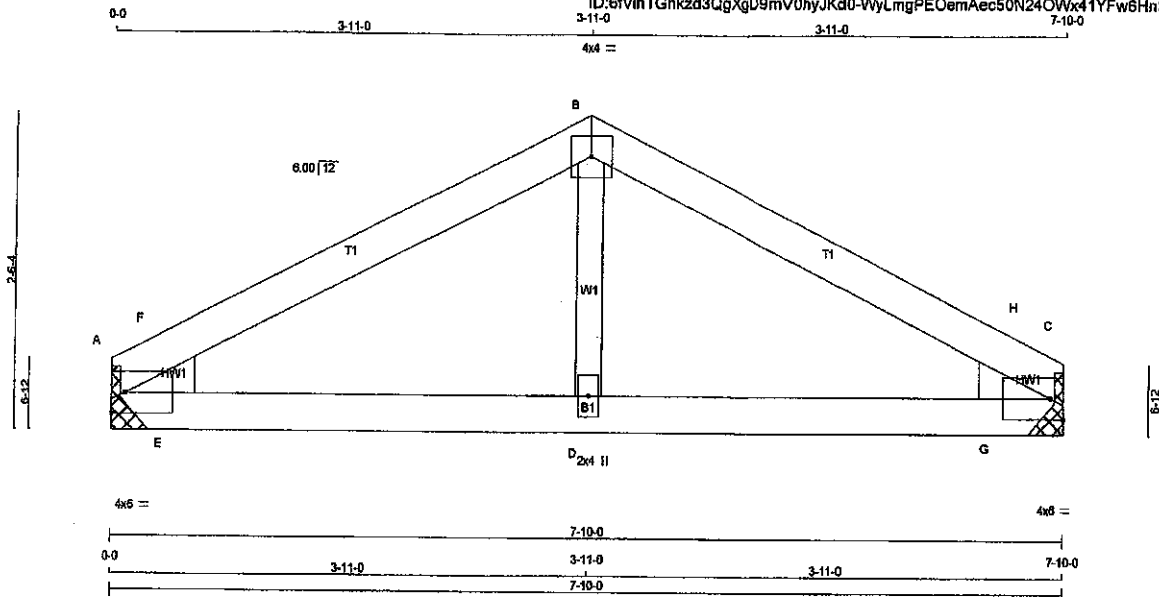


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.																																								
402970	T28	1	3	TRUSS DESC.																																										
Tamarack Roof Truss, Burlington Version 8.280 S Nov 17 2018 Mittek Industries, Inc. Tue May 7 17:00:51 2019 Page 2																																														
ID: 6VhTGnkzd3QgXgD8mV0hyJKd0-2nOS3EmtT2n7yRBUNIH0aUJOWcw2Oxu24JIBKzfrJw																																														
<table border="1"> <thead> <tr> <th colspan="10">FACTORED CONCENTRATED LOADS (LBS)</th> </tr> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr> <td>V</td> <td>16-11-4</td> <td>-1591</td> <td>-1591</td> <td>—</td> <td>BACK</td> <td>VERT</td> <td>TOTAL</td> <td>—</td> <td>—</td> </tr> <tr> <td>W</td> <td>18-11-4</td> <td>-1591</td> <td>-1591</td> <td>—</td> <td>BACK</td> <td>VERT</td> <td>TOTAL</td> <td>—</td> <td>—</td> </tr> </tbody> </table>							FACTORED CONCENTRATED LOADS (LBS)										JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	V	16-11-4	-1591	-1591	—	BACK	VERT	TOTAL	—	—	W	18-11-4	-1591	-1591	—	BACK	VERT	TOTAL	—	—
FACTORED CONCENTRATED LOADS (LBS)																																														
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																																					
V	16-11-4	-1591	-1591	—	BACK	VERT	TOTAL	—	—																																					
W	18-11-4	-1591	-1591	—	BACK	VERT	TOTAL	—	—																																					
 DWG NO. TAM 7791073B STRUCTURAL COMPONENT ONLY																																														



JOB NAME 402970	TRUSS NAME T29	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MTek Industries, Inc. Tue May 7 17:00:52 2019 Page 1
ID:6VhTGkzd3QgXgD9mV0hyJKd0-VyLmgPEOemAec50N24OWx41YFw6Hn3s2HK3RknzlrJv



TOTAL WEIGHT = 3 X 22 = 66 lb
(MT)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF
B - C	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - B	TMBH1-I	4.0	8.0		Edge
B - C	TTWV-p	4.0	4.0		
C - D	TMBH1-I	4.0	8.0		Edge
D - E	BMVW-w	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	HEEL WEDGE
JT VERT					
A	551	0	0	MECHANICAL	2x4 L
C	551	0	0	MECHANICAL	2x4 R

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT A, C. MINIMUM BEARING LENGTH AT JOINT A = 3'-8", JOINT C = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
A	412	228 / 0 82 / 0 0 / 0 0 / 0 102 / 0 0 / 0
C	412	228 / 0 82 / 0 0 / 0 0 / 0 102 / 0 0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-F	-655 / 0	-102.1 -102.1	0.07 (1)	D-B	0 / 238	0.05 (2)	
F-B	-588 / 0	-102.1 -102.1	0.15 (1)	E-F	-83 / 89	0.00 (1)	
B-H	-588 / 0	-102.1 -102.1	0.15 (1)	G-H	-83 / 89	0.00 (1)	
H-C	-655 / 0	-102.1 -102.1	0.07 (1)				
A-E	0 / 527	-38.5 -38.5	0.17 (1)				
E-D	0 / 527	-38.5 -38.5	0.20 (1)				
D-G	0 / 527	-38.5 -38.5	0.20 (1)				
G-C	0 / 527	-38.5 -38.5	0.17 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (B-H:1), BC=0.20/1.00 (D-G:1), WB=0.05/1.00 (B-D:2), SS=0.13/1.00 (B-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

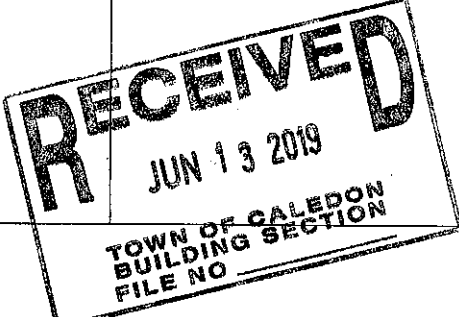
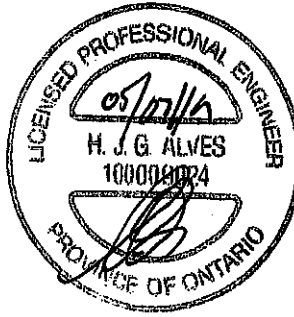
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687 788 1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (A) (INPUT = 0.90)
JSI METAL= 0.14 (A) (INPUT = 1.00)

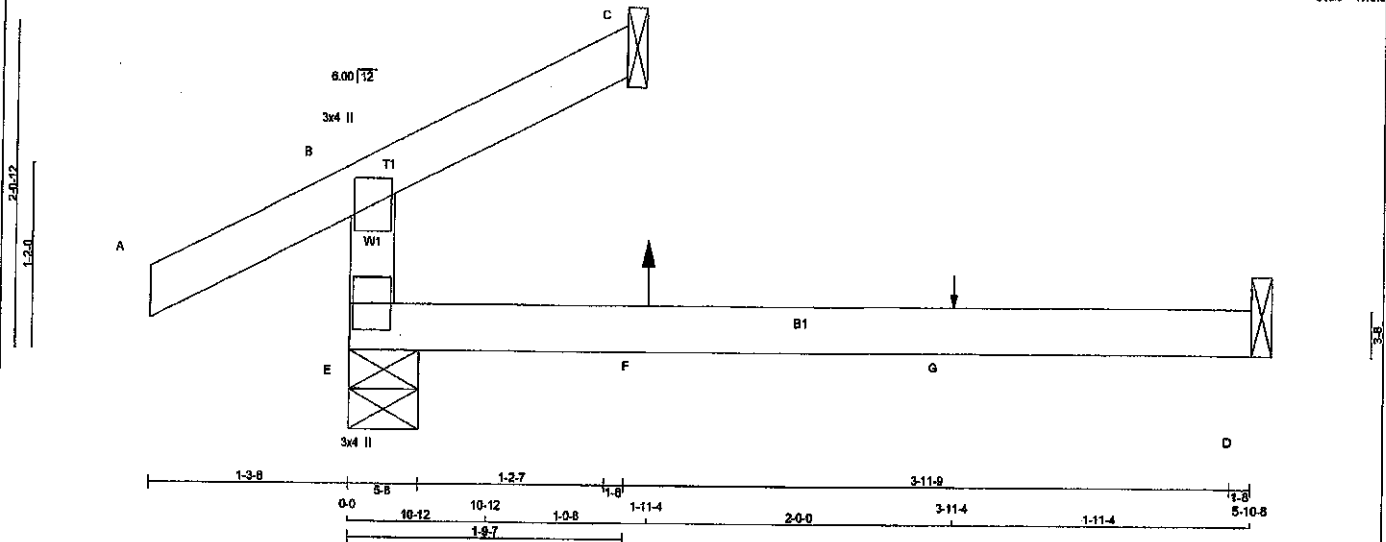


DRWG NO. TAM 09/0739
STRUCTURAL
CONSULTANT ONLY

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

JOB NAME 402970	TRUSS NAME J22	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:00:45 2019 Page 1
ID: 8fVhTGnkzd3QgXgD9mV0hyJKd0-DbQ7C097IdldG1_186mt9bELU5jCevy0g8sZ7hzlrK0
5-10-8
Scale = 1:13.5



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No. 2
A - C	2x4	DRY	No. 2
E - D	2x4	DRY	No. 2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQRD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
E	352	0	352	0	5-8
C	84	0	97	0	1-8
D	84	0	123	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	259	158 / 0	42 / 0	0 / 0	0 / 0	61 / 0	0 / 0
C	78	24 / 0	28 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	86	0 / -3	53 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO. 2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
FR-TO	(LBS)	(PLF)	CS1 (LC)	UNBRAC	CS1 (LC)
E-B	-229 / 26	0.0	0.0	0.18 (3)	7.81
A-B	0 / 31	-102.1	-102.1	0.15 (1)	10.00
B-C	0 / 27	-102.1	-102.1	0.18 (3)	10.00
E-F	0 / 0	-38.5	-38.5	0.23 (3)	10.00
F-G	0 / 0	-38.5	-38.5	0.23 (3)	10.00
G-D	0 / 0	-38.5	-38.5	0.23 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	8	1	13	BACK	VERT	TOTAL	-	-
G	3-11-4	1	1	-	BACK	VERT	TOTAL	-	-

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/839$ (0.08")

CS1: TO=0.18/1.00 (B-E-3), BC=0.23/1.00 (D-E-3), WB=0.00/1.00 (V-E-0), SS=0.14/1.00 (D-E-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1659

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DRWG NO. TAM 0731
STRUCTURAL
COMPONENT ONLY

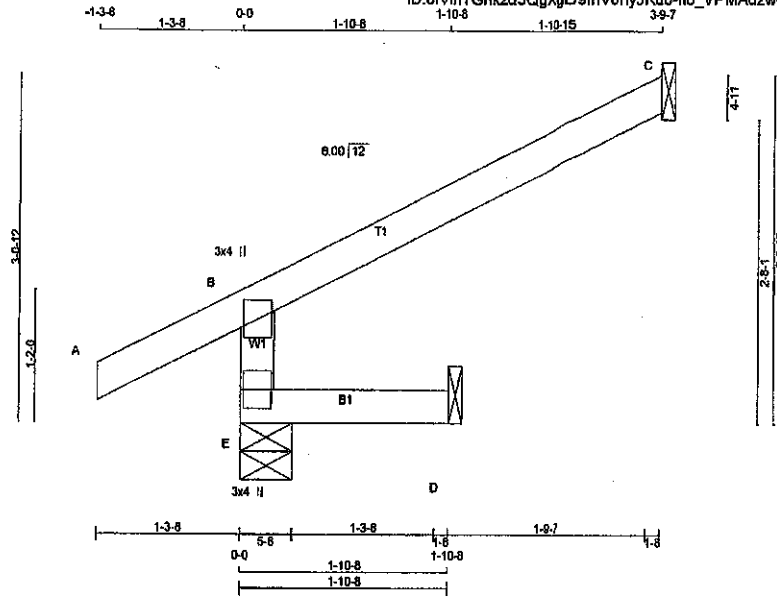
RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 402970	TRUSS NAME J23	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:00:46 2019 Page 1
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Scale = 1:18.7

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	420	0	420	0	5-8	5-8
C	145	0	145	0	1-8	1-8
D	32	0	41	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	287	216 / 0	22 / 0	0 / 0	0 / 0	59 / 0	0 / 0
C	100	83 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	29	0 / 0	18 / 0	0 / 0	0 / 0	12 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO				
E-B	-380 / 0	0.0	0.0	0.01 (3)	7.81		
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		
B-C	-22 / 0	-102.1	-102.1	0.25 (1)	8.25		
E-D	0 / 0	-38.5	-38.5	0.03 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.18")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.25/1.00 (B-C:1), BC=0.03/1.00 (D-E:3), WB=0.00/1.00 (H-A:0), SS1=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

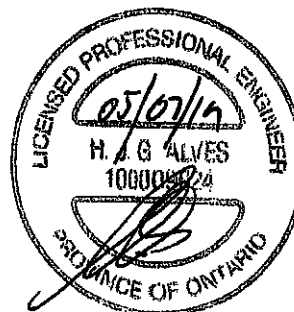
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354 1897 788 1897 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



DRWG NO. TAM 1910732
STRUCTURAL
COMPONENT ONLY

JOB NAME 402970	TRUSS NAME J25	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:00:47 2018 Page 1 ID:6fVhTGnkzd3QgD9mV0hyJKd0-9_YtdlBFpEYLWK8QFXoLE0KdLvNO6oRJ8SLg3ZzlrK	

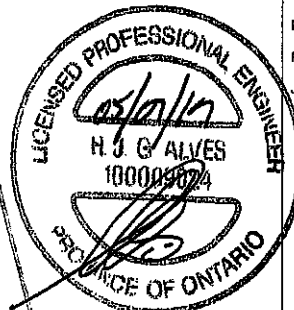
Scale = 1:13.6

TOTAL WEIGHT = 2 X 14 = 28 lb

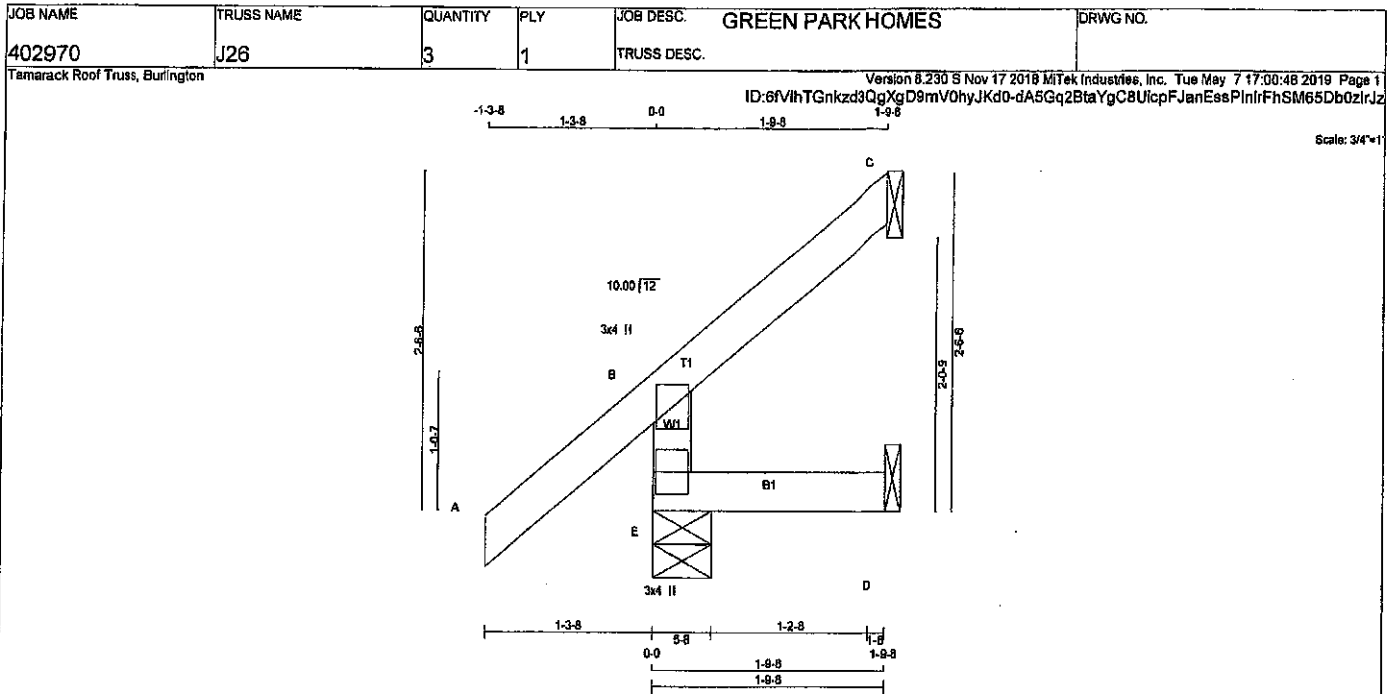
LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF B - D 2x4 DRY No.2 SPF DRY: SEASONED LUMBER. GABLE STUDS SPACED AT 2-0-0 OC.				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS. THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE. BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.				DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.1 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.18") CALCULATED VERT. DEFL.(LL) = L/842 (0.08") ALLOWABLE DEFL.(TL) = L/360 (0.18") CALCULATED VERT. DEFL.(TL) = L/499 (0.13") CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.28/1.00 (B-E:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1697 768 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP= 0.31 (B) (INPUT = 0.80) JSI METAL= 0.09 (B) (INPUT = 1.00)																																																													
PLATES (table is in inches) JT TYPE PLATES W LEN Y X B TMB14 MT20 3.0 4.0				LOADING TOTAL LOAD CASES: (4) <table border="1"> <thead> <tr> <th colspan="3">CHORDS</th> <th colspan="3">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>LC1 MAX CSI (LC)</th> <th>MAX. UNBRACED LENGTH</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX. CSI (LC)</th> </tr> </thead> <tbody> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td></td> <td>FR-TO</td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0/20</td> <td>-102.1 -102.1</td> <td>0.13 (1)</td> <td>10.00</td> <td>E-F</td> <td>-254/122</td> <td>0.00 (1)</td> </tr> <tr> <td>B-F</td> <td>-51/0</td> <td>-102.1 -102.1</td> <td>0.13 (3)</td> <td>6.25</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F-C</td> <td>0/6</td> <td>-102.1 -102.1</td> <td>0.42 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>B-E</td> <td>0/0</td> <td>-38.5 -38.5</td> <td>0.28 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>E-D</td> <td>0/0</td> <td>-38.5 -38.5</td> <td>0.31 (1)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> </tbody> </table>				CHORDS			WEBS			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO		FROM TO			FR-TO			A-B	0/20	-102.1 -102.1	0.13 (1)	10.00	E-F	-254/122	0.00 (1)	B-F	-51/0	-102.1 -102.1	0.13 (3)	6.25				F-C	0/6	-102.1 -102.1	0.42 (1)	10.00				B-E	0/0	-38.5 -38.5	0.28 (1)	10.00				E-D	0/0	-38.5 -38.5	0.31 (1)	10.00			
CHORDS			WEBS																																																																		
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)																																																														
FR-TO		FROM TO			FR-TO																																																																
A-B	0/20	-102.1 -102.1	0.13 (1)	10.00	E-F	-254/122	0.00 (1)																																																														
B-F	-51/0	-102.1 -102.1	0.13 (3)	6.25																																																																	
F-C	0/6	-102.1 -102.1	0.42 (1)	10.00																																																																	
B-E	0/0	-38.5 -38.5	0.28 (1)	10.00																																																																	
E-D	0/0	-38.5 -38.5	0.31 (1)	10.00																																																																	

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO



DWG NO. TAM 09/10734
 STRUCTURAL
 COMPONENT ONLY



TOTAL WEIGHT = 3 X 8 = 23 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY No.2

A - C 2x4 DRY No.2

E - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	322 0	322 0	5-8	5-8
C	52 0	52 0	1-8	1-8
D	20 0	39 0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	229	184 / 0	19 / 0	0 / 0	0 / 0	48 / 0	0 / 0	0 / 0
C	36	28 / -19	1 / 0	0 / 0	0 / 0	7 / 0	0 / 0	0 / 0
D	21	0 / -12	17 / 0	0 / 0	0 / 0	10 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED HORIZ. LOAD (PL)	MAX. FACTORED UNBRACED LENGTH (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)
E-B	-273 / 0	0.0	0.0	0.05 (5)	7.81			
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00			
B-C	-26 / 0	-102.1	-102.1	0.10 (1)	6.25			
E-D	0 / 0	-38.5	-38.5	0.05 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: $TC=0.14/1.00$ (A-B:1), $BC=0.05/1.00$ (D-E:6), $WB=0.00/1.00$ (H/A:0), $SSI=0.08/1.00$ (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

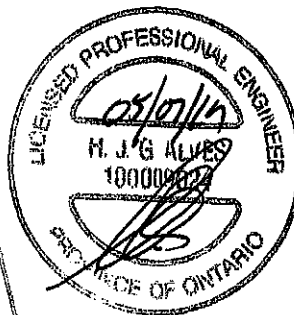
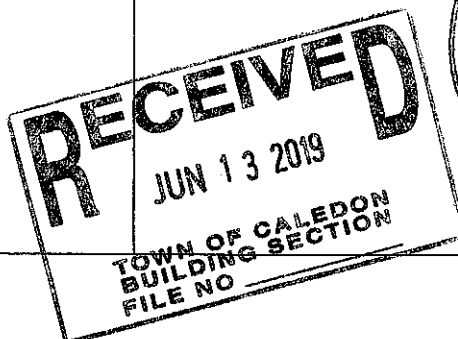
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.13 (B) (INPUT = 0.90)
JSI METAL = 0.10 (B) (INPUT = 1.00)

DWG NO. TAM 17910735
STRUCTURAL
COMPONENT ONLY





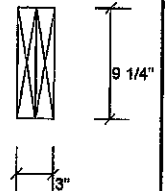
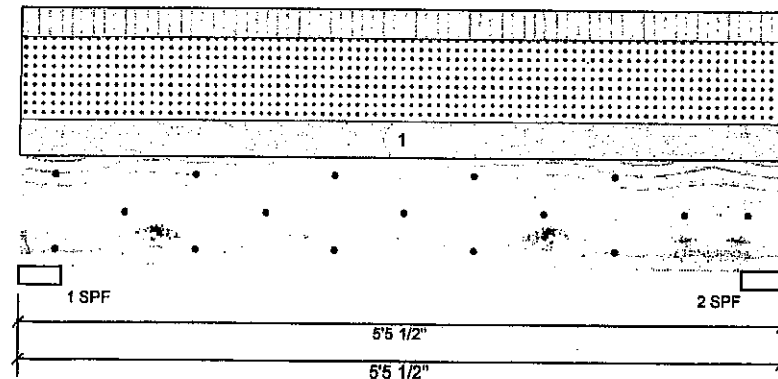
isDesign™

Client:
Project:
Address:Date: 5/7/2019
Designer:
Job Name: 201008
Project #:

Page 3 of 4

BM21 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	141	174	396	0
2	141	174	396	0

Bearings and Factored Reactions

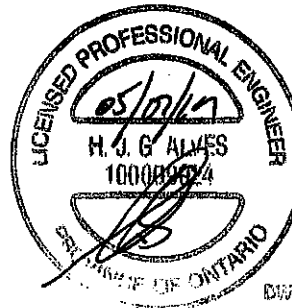
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	15%	218 / 735	953	L	1.25D+1.5S +L
2 - SPF	3.500"	15%	218 / 735	953	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1091 ft-lb	2'8 3/4"	6039 ft-lb	0.181 (18%)	1.25D+1.5S +L	L
Unbraced	1091 ft-lb	2'8 3/4"	5299 ft-lb	0.206 (21%)	1.25D+1.5S +L	L
Shear	873 lb	4'5 1/2"	3984 lb	0.218 (22%)	1.25D+1.5S +L	L
LL Defl inch	0.009 (L/6806)	2'8 3/4"	0.167 (L/360)	0.050 (5%)	S+0.5L	L
TL Defl inch	0.012 (L/4953)	2'8 3/4"	0.167 (L/360)	0.070 (7%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DRWG NO. TAM 7110751
STRUCTURAL
CHECKED ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		4-11-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

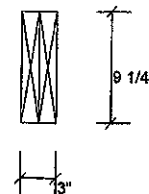
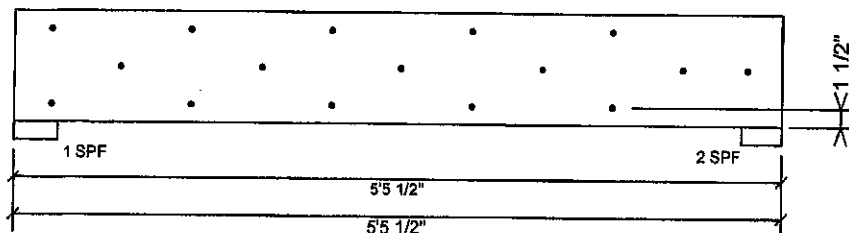
Client:
Project:
Address:

Date: 5/7/2019
Designer:
Job Name: 201008
Project #:

Page 4 of 4

BM21 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

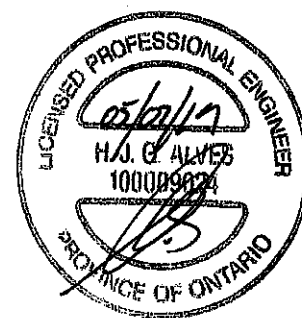
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	45.5 %
Load	174.5 PLF
Yield Limit per Foot	383.4 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DESIGNED BY: P910751
STRUCTURAL
CHECKED BY: 3/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021

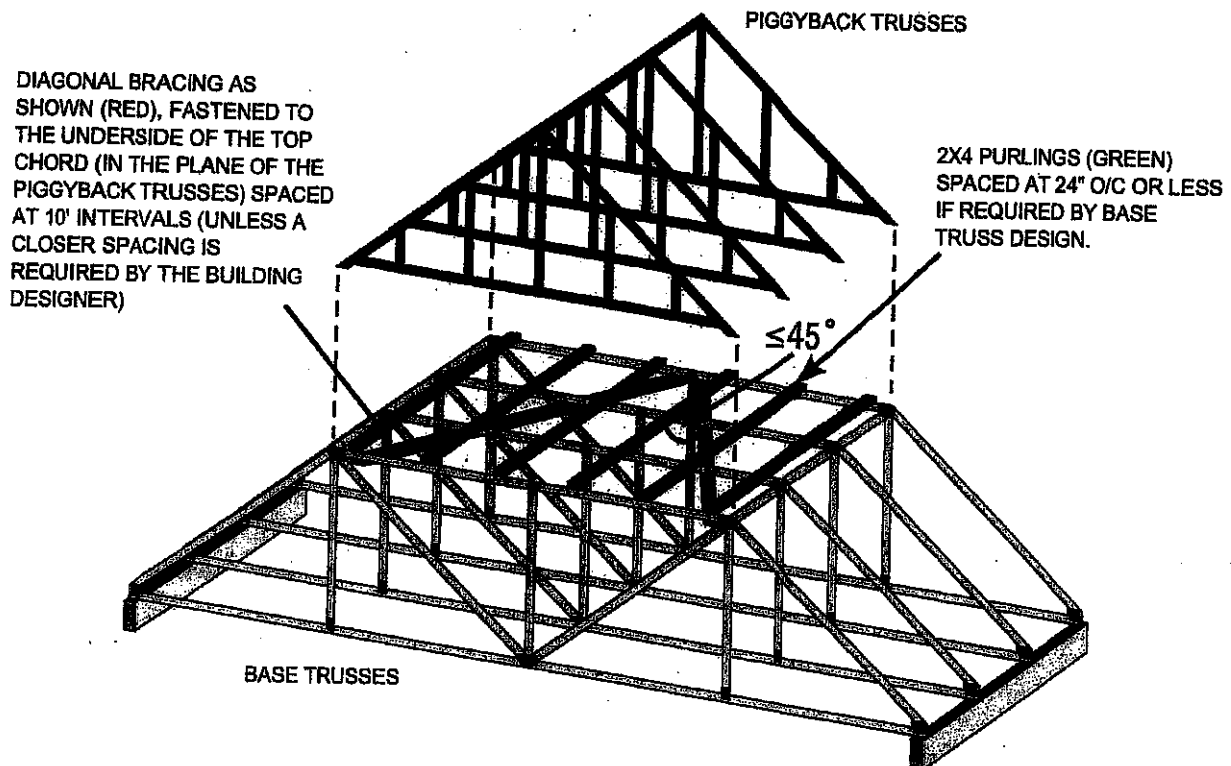


Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

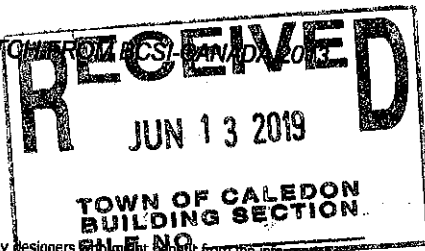


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

SKETCH FROM OBCSI-CANADA 2013



LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

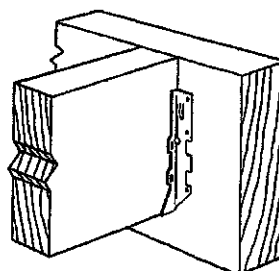
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

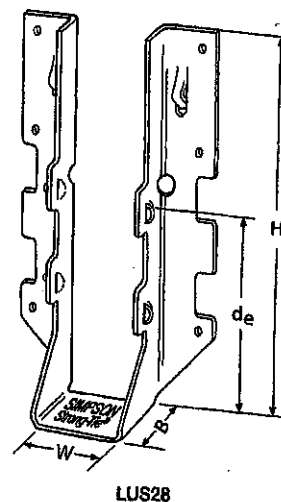
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



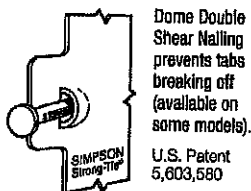
Typical LUS Installation



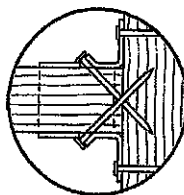
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/2	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/2	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/4	1 1/4	3/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/2	1 1/4	3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

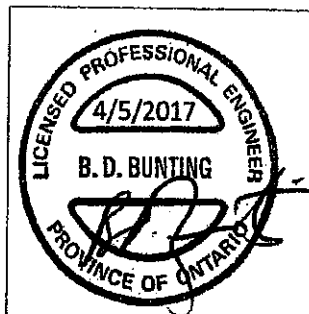
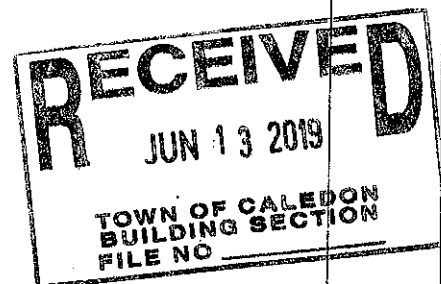
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



U.S. PATENT
DESIGN

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ASTM A1063-17

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

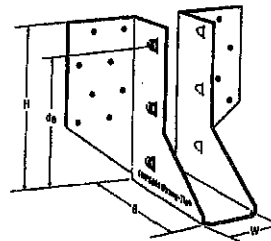
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

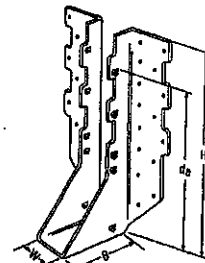
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

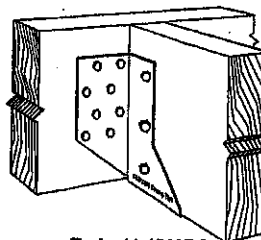
- See current catalogue for options



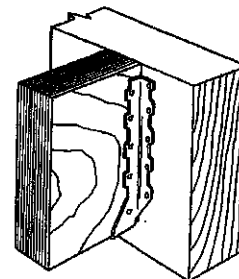
LJS26DS



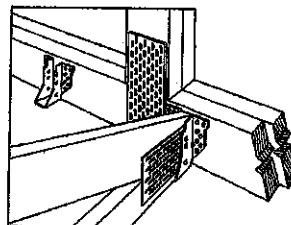
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



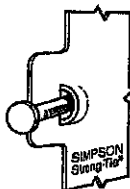
Typical HUS
Installation



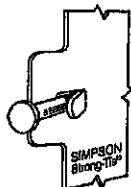
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15) lb.	Normal (K _u =1.00) lb.	Uplift (K _u =1.15) lb.	Normal (K _u =1.00) lb.
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(8) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	8 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

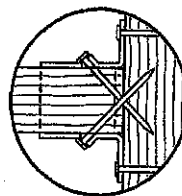
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,803,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



(800) 999-5099
strongtie.com

JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

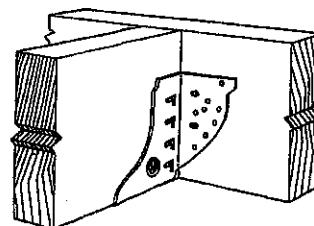
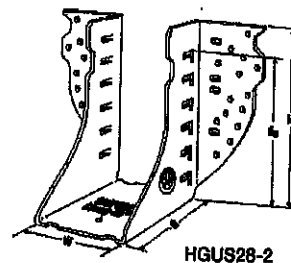
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

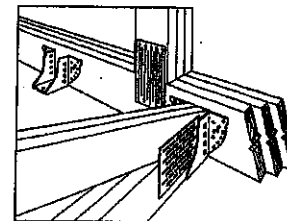
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



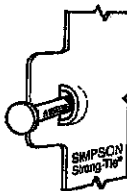
Typical HGUS Installation



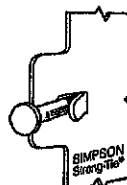
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fr-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

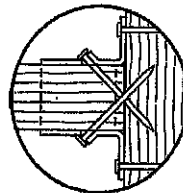
1. d_o is the distance from the seat of the hanger to the highest joist nail.



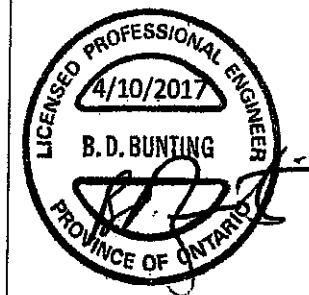
Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



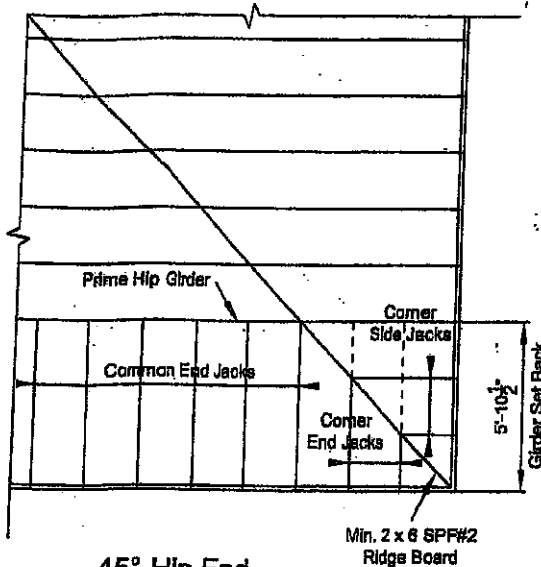
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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45° Hip End

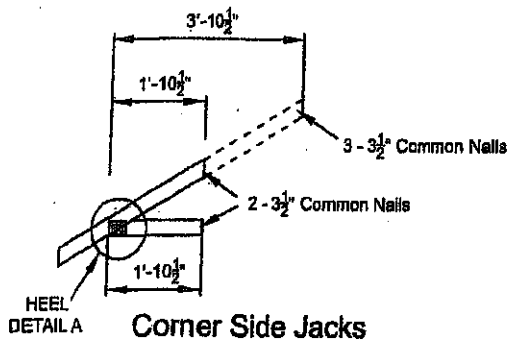
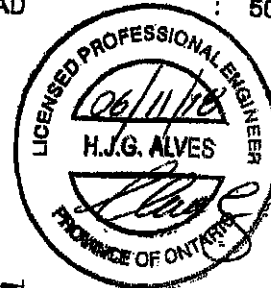
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

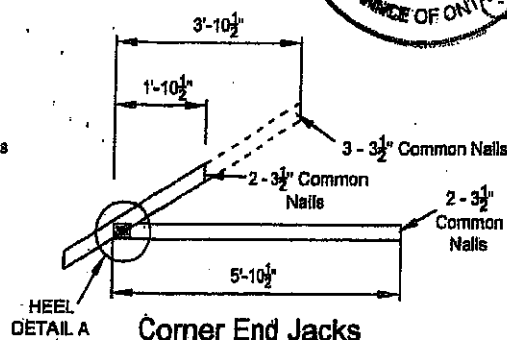
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

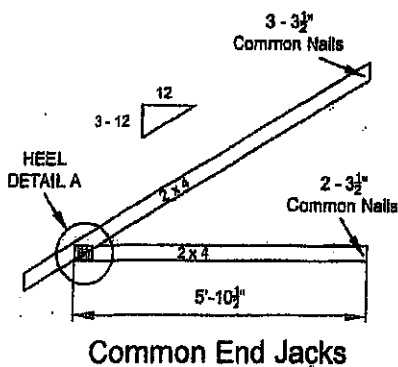
TOTAL LOAD : 50.5 P.S.F



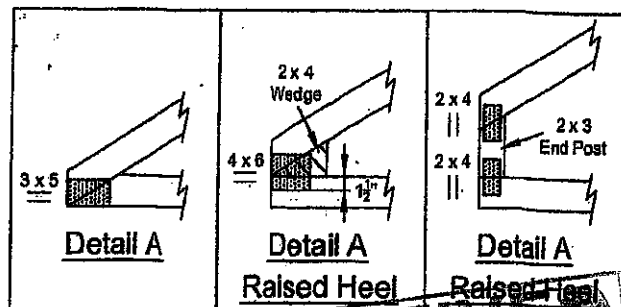
Corner Side Jacks



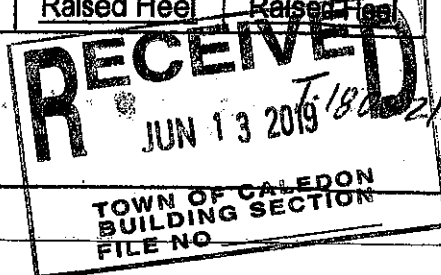
Corner End Jacks

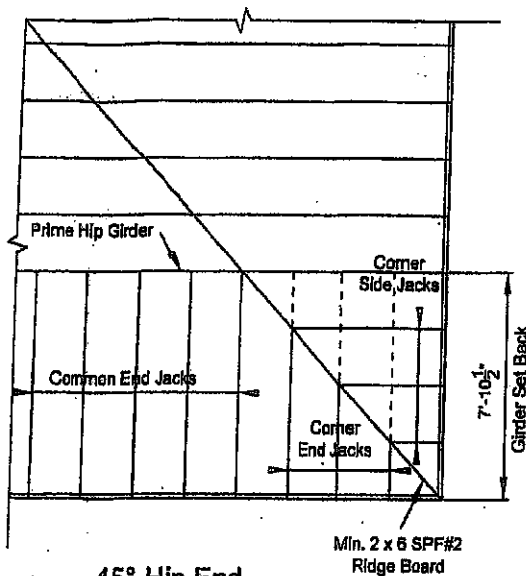


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)





45° Hip End

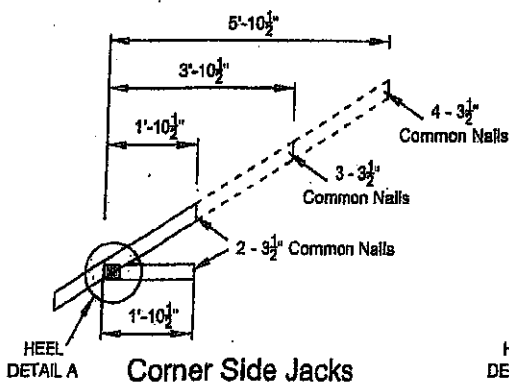
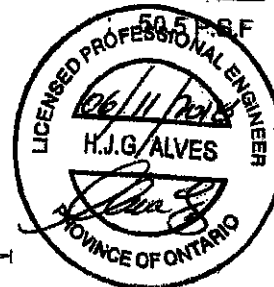
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

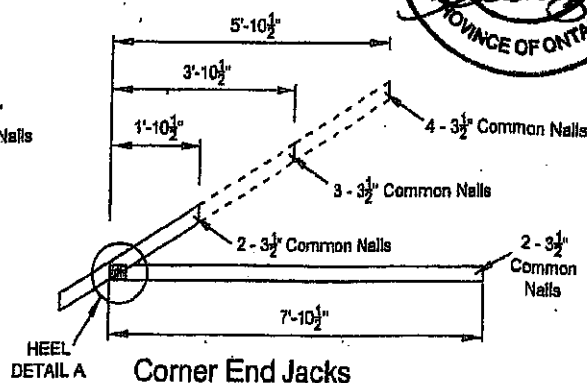
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

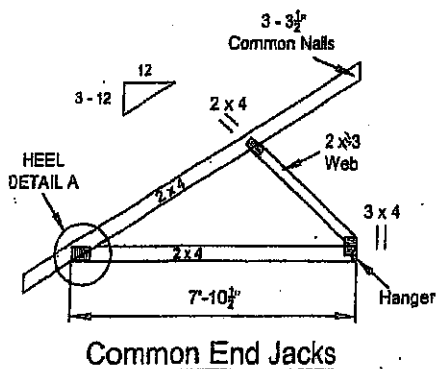
TOTAL LOAD



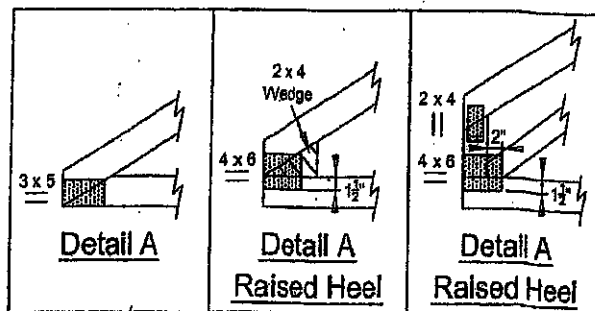
Corner Side Jacks



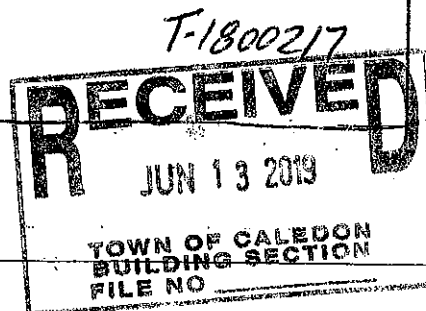
Corner End Jacks



Common End Jacks

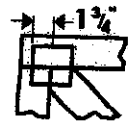


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

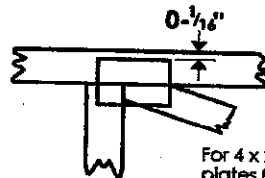


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-3/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



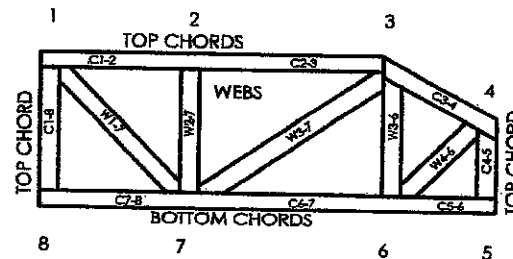
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MI-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

