

10/12 ROOF PITCH
UNLESS OTHERWISE
SPECIFIED

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
GS LOAD 1.8 KPA
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

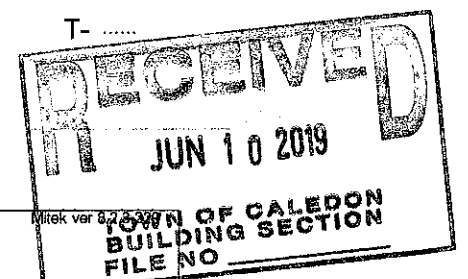
ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART 9 OF THE OBC. LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO BE
2"X4" SPF @24" o.c. WITH A
2"X4" SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS AND
BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

HARDWARE
HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V)

BM1:2-2X10

DENOTES
CONVENTIONAL
FRAMING



M12237



Job Track: 50033

Plan Log: 201018

Layout ID: 402975

Builder / Location:

GREEN PARK HOMES / CALEDON

Project: LAMBERT LANE PH.2

Date: 5/8/2019

Sales: Mario DiCano

Designer: JG

Model / Elevation:

BARTON 3/2- REAR UPGRADE

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



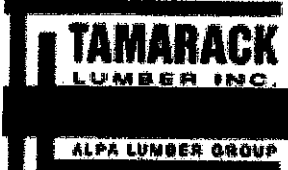
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 3
 Lot #:
 Elevation: 2- REAR UPGRADE

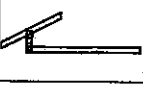
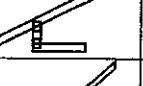
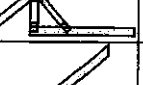
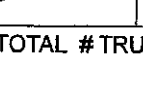
Job Track: 50033
 PlanLog: 201018
 Layout ID: 402975
 Ref #
 Page: 1 of 2
 Date: 05/06/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T9 Common	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	198.87 132.00		
	1	G9 GABLE	10 / 12	11-02-00	6-03-08	2 x 4	1-05-00 1-03-08	1-07-11 1-07-11	54.45 37.00		
	1 2-ply	T21 Hip Girder	6 / 12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	252.71 158.00		
	1	T22 Hip	6 / 12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	109.31 68.83		
	1	T23 Hip	6 / 12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	110.98 70.67		
	1	T24 Hip	6 / 12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	112.6 70.67		
	11	T25 Common	6 / 12	27-00-00	7-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1199.12 740.67		
	1 2-ply	T28 Hip Girder	10 / 12	27-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	266.28 170.33		
	2	T29 Hip	10 / 12	27-00-00	6-06-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	250.96 158.67		
	2	T30 Hip	10 / 12	27-00-00	8-02-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	260.71 165.33		
	2	T31 Hip	10 / 12	27-00-00	9-10-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	269.72 171.33		
	1	V20 Valley	6 / 12	15-07-01	3-10-12	2 x 4			44.6 29.67		
	1	V21 Valley	6 / 12	11-07-01	2-10-12	2 x 4			28.16 17.50		
	1	V22 Valley	6 / 12	7-07-01	1-10-12	2 x 4			16.65 10.67		

		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: BARTON Lot #: Elevation: 2- REAR UPGRADE	Job Track: 50033 PlanLog: 201018 Layout ID: 402975 Ref # Page: 2 of 2 Date: 05/06/2019 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	2	J2 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	6	J7 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	91.36 58.00		
	1	J21 Jack-Open Girder	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J22 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		
	7	J23 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= 64

TOTAL BFT OF ALL TRUSSES= 2289.33

BFT.

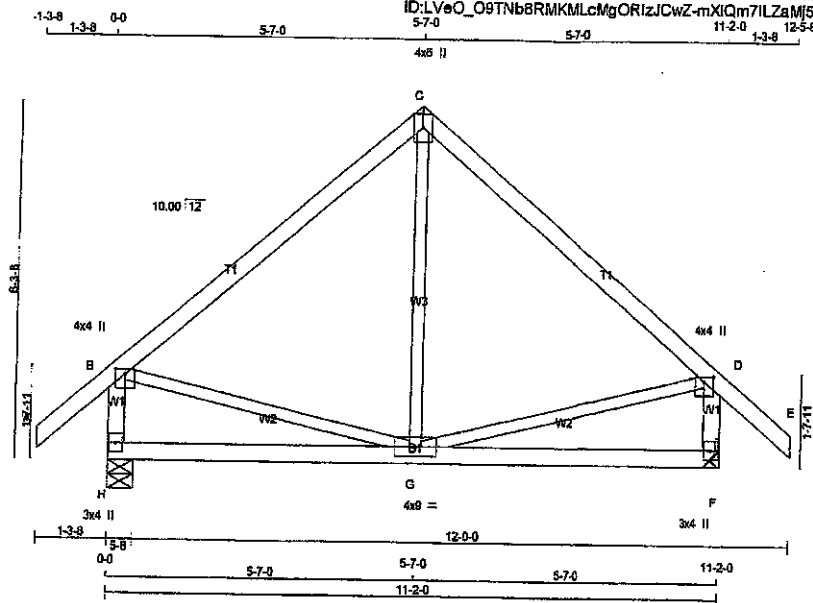
TOTAL WEIGHT OF ALL TRSSES 3625.15 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 2

JOB NAME 402973	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1:37.5

LUMBER

N. L. G. A. RULES	SIZE	LUMBER
CHORDS		
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
H - B	2x4	DRY No.2
F - D	2x4	DRY No.2
H - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT
 DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	6.0	Edge	
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMVWW+t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
H	927	0	927	0
F	927	0	927	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	884	405/0	117/0	0/0	0/0	162/0	0/0
F	884	405/0	117/0	0/0	0/0	162/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO					FR-TO		
A-B	0/45	-102.1	-102.1 0.14 (1)	10.00	G-C	0/248	0.08 (3)
B-C	-510/0	-102.1	-102.1 0.41 (1)	8.25	B-G	0/403	0.08 (1)
C-D	-510/0	-102.1	-102.1 0.41 (1)	8.25	G-D	0/403	0.08 (1)
D-E	0/45	-102.1	-102.1 0.14 (1)	10.00			
H-B	-845/0	0.0	0.0 0.08 (1)	7.81			
F-D	-845/0	0.0	0.0 0.08 (1)	7.81			
H-G	0/0	-38.5	-38.5 0.27 (3)	10.00			
G-F	0/0	-38.5	-38.5 0.27 (3)	10.00			

TOTAL WEIGHT = 2 X 50 = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 20.1 PSF
 DL = 8.0 PSF
 BOT. CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-08, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.37")
 CALCULATED VERT. DEFL. (LL) = $L/989$ (0.03")
 ALLOWABLE DEFL. (TL) = $L/360$ (0.37")
 CALCULATED VERT. DEFL. (TL) = $L/989$ (0.05")

CSI: TC=0.41/1.00 (B-C:1), BC=0.27/1.00 (G-H:3), WB=0.09/1.00 (D-G:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

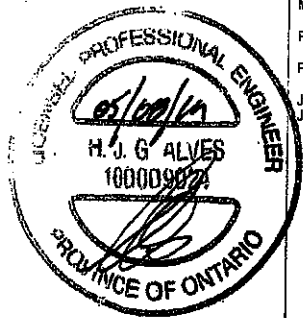
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 789 1967 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.78 (D) (INPUT = 0.90)
 JSI METAL = 0.21 (D) (INPUT = 1.00)



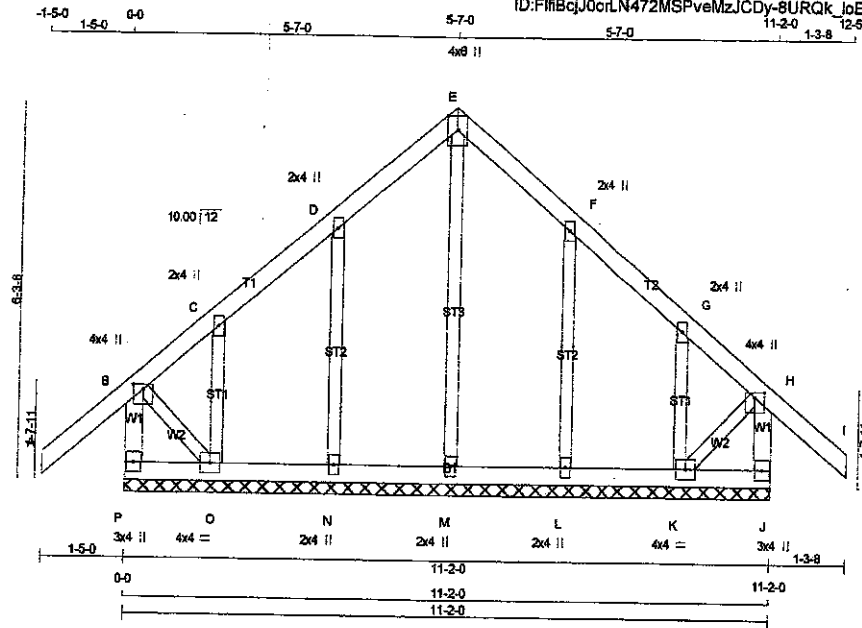
DRWG NO. TAM 171/0807
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	G9	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 08:55:57 2019 Page 1
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Scale = 1:37.1



TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - I	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
P - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JF TYPE	PLATES	W	LEN	Y	X
B TMWV+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMW+w	MT20	2.0	4.0		
E TTW+p	MT20	4.0	6.0	Edge	
H TMWV+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMWV1-i	MT20	4.0	4.0		
L, M, N					
L BMW1+w	MT20	2.0	4.0		
O BMWV1-i	MT20	4.0	4.0		
P BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	LC1 CS1 (LC)	MAX. UNBRACED LENGTH (FT)	WEBS MEMB. MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO						
P-B	-331 / 0	0.0	0.0	0.04 (1)	7.81	
A-B	0 / 49	-102.1	-102.1	0.17 (1)	10.00	
B-C	-69 / 0	-102.1	-102.1	0.16 (1)	6.25	
C-D	-3 / 0	-102.1	-102.1	0.07 (1)	10.00	
D-E	-25 / 0	-102.1	-102.1	0.07 (1)	6.25	
E-F	-25 / 0	-102.1	-102.1	0.07 (1)	6.25	
F-G	-4 / 0	-102.1	-102.1	0.13 (1)	6.25	
G-H	-60 / 0	-102.1	-102.1	0.13 (1)	10.00	
H-I	0 / 45	-102.1	-102.1	0.14 (1)	10.00	
J-H	-305 / 0	0.0	0.0	0.03 (1)	7.81	
P-O	0 / 0	-38.5	-38.5	0.02 (3)	10.00	
O-N	0 / 13	-38.5	-38.5	0.02 (3)	10.00	
N-M	0 / 8	-38.5	-38.5	0.02 (3)	10.00	
M-L	0 / 8	-38.5	-38.5	0.02 (3)	10.00	
L-K	0 / 13	-38.5	-38.5	0.02 (3)	10.00	
K-J	0 / 0	-38.5	-38.5	0.02 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.17/1.00 (A-B:1), BC=0.02/1.00 (N-O:3), WB=0.09/1.00 (E-M:1), SS1=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

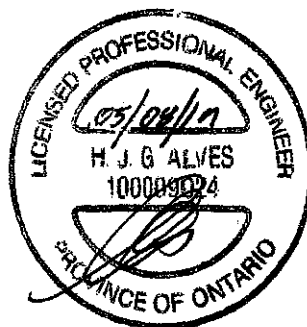
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)



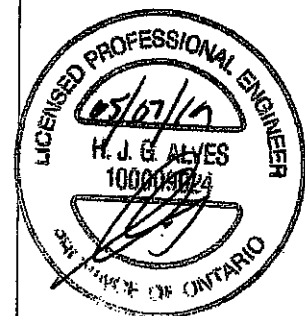
DRWG NO. TAM 11910920
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T21	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue May 7 17:48:13 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	10-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
P	12-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
Q	14-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
R	16-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
S	18-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
T	20-1-4	-123	-123	-	FRONT	VERT	TOTAL	-	-
U	1-11-4	-55	-74	-	FRONT	VERT	TOTAL	-	-
V	3-11-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
W	8-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
X	10-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
Y	12-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
Z	14-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AA	18-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AB	20-1-4	-55	-70	-	FRONT	VERT	TOTAL	-	-
AC	23-0-12	-55	-70	-	FRONT	VERT	TOTAL	-	-
AD	25-0-12	-55	-74	-	FRONT	VERT	TOTAL	-	-

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

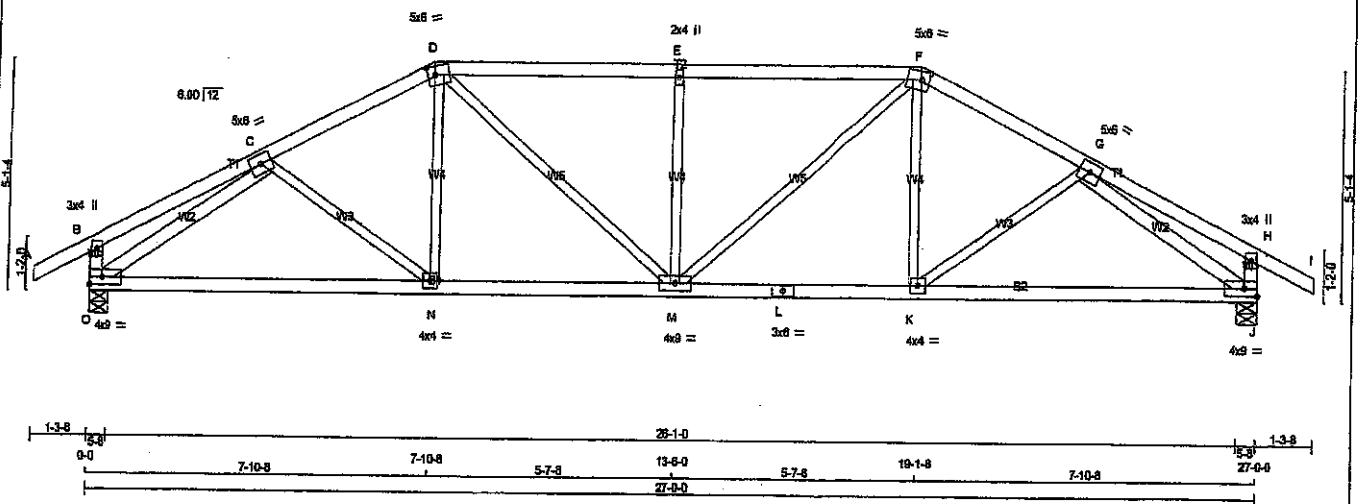


ENGINEER TAM 1910779
STRUCTURAL
CITY OF BURLINGTON

7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T22	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:48:14 2018 Page 1
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Scale = 1:48.6



TOTAL WEIGHT = 109 lb
(MTF)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
O - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
O - L	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
O - C	2x4 DRY	No.2	SPF	
G - J	2x4 DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVtp	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0		
D	TMVW-m	MT20	5.0	6.0	2.25	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-m	MT20	5.0	6.0	2.25	2.00
G	TMVW-t	MT20	5.0	6.0		
H	TMVtp	MT20	3.0	4.0		
J	BMVW-1	MT20	4.0	9.0		Edge
K	BMVW-t	MT20	4.0	4.0		
L	BS-1	MT20	3.0	6.0		
M	BMVW-1	MT20	4.0	9.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-1	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT	REQD
JT	GROSS REACTION	GROSS REACTION				BRG	BRG
O	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	2037	0	2037	0	0	5-8	5-8
	2037	0	2037	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
O	1514	863 / 0	284 / 0	0 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	C-N	-3 / 138	0.03 (3)
B-C	0 / 19	-102.1	-102.1	0.23 (1)	N-D	0 / 375	0.08 (2)
C-D	-2533 / 0	-102.1	-102.1	0.25 (1)	D-M	0 / 847	0.15 (1)
D-E	-2746 / 0	-102.1	-102.1	0.52 (1)	M-E	-702 / 0	0.27 (1)
E-F	-2746 / 0	-102.1	-102.1	0.52 (1)	F-G	0 / 847	0.15 (1)
F-G	-2533 / 0	-102.1	-102.1	0.25 (1)	G-H	0 / 375	0.08 (2)
G-H	0 / 19	-102.1	-102.1	0.23 (1)	H-I	-3 / 138	0.03 (3)
H-I	0 / 31	-102.1	-102.1	0.13 (1)	I-J	-2788 / 0	0.77 (1)
I-J	-299 / 0	0.0	0.0	0.03 (1)	J-K	-2788 / 0	0.77 (1)
J-K	-299 / 0	0.0	0.0	0.03 (1)	K-L		
K-L					L-M		
L-M					M-N		
M-N					N-O		
N-O					O-P		
O-P					P-Q		
P-Q					Q-R		
Q-R					R-S		
R-S					S-T		
S-T					T-U		
T-U					U-V		
U-V					V-W		
V-W					W-X		
W-X					X-Y		
X-Y					Y-Z		
Y-Z					Z-A		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/LC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/889 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/889 (0.30")
CS1: TO=0.52/1.00 (D-E-1), BC=0.71/1.00 (K-M-2),
WB=0.77/1.00 (G-J-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVEHEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	518	354	1687
	788	1987	1688

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

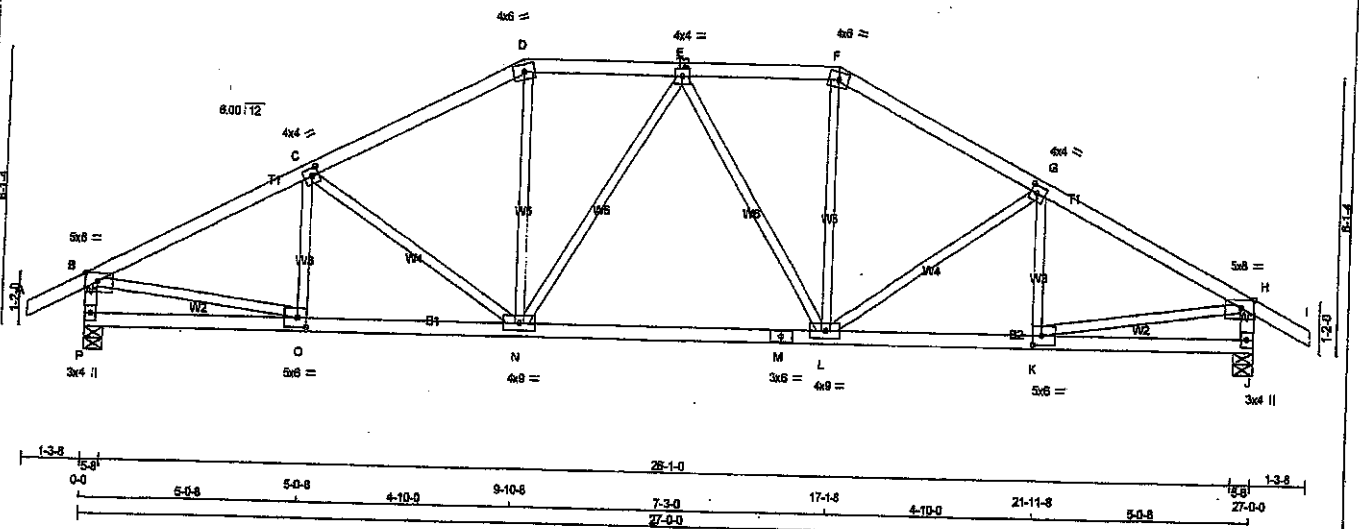
JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL = 0.73 (L) (INPUT = 1.00)



DESIGN BY 17/10/18
STRUCTURAL
COUNCIL OF ONTARIO

JOB NAME 402939	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:48:15 2019 Page 1
 ID: yH3luQzwKBAxQAh7hsf8URzKCTp-mudYMxSHMFB0sn5fuDfchKjyKoPloXW1YE00VzIqdU
 1-3-8 0-0 5-0-8 5-0-8 4-10-0 9-10-8 3-7-8 13-8-0 3-7-8 17-1-8 4-10-0 21-11-8 5-0-8 27-0-0 28-3-8 1-3-8
 Scale = 1/48.6



TOTAL WEIGHT = 111 lb
 [MIF]

LUMBER
 N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-l	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	6.0	
E	TMVW-l	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	6.0	
G	TMVW-l	MT20	4.0	4.0	2.00 1.75
H	TMVW-p	MT20	5.0	8.0	Edge 3.50
J	BMV-l-p	MT20	3.0	4.0	
K	BMV-l-p	MT20	5.0	8.0	2.50 2.25
L	BMVW-l	MT20	4.0	9.0	
M	BS-l	MT20	3.0	6.0	
N	BMVW-l	MT20	4.0	9.0	
O	BMVW-l	MT20	5.0	8.0	2.50 2.25
P	BMV-l-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	2037	0	2037	0	5-8	5-8
J	2037	0	2037	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
P	1514	863/0	284/0	0/0	0/0	367/0	0/0
J	1514	863/0	284/0	0/0	0/0	367/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (L/C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (L/C)	
FR-TO		FROM TO		FR-TO			
A-B	0/31	-102.1 -102.1	0.13 (1)	10.00	C-C	-221/84	0.05 (1)
B-C	-2863/0	-102.1 -102.1	0.41 (1)	3.88	C-N	-410/0	0.25 (1)
C-D	-2337/0	-102.1 -102.1	0.38 (1)	4.14	N-D	0/1698	0.18 (1)
D-E	-2078/0	-102.1 -102.1	0.20 (1)	4.53	E-E	-258/0	0.22 (1)
E-F	-2078/0	-102.1 -102.1	0.20 (1)	4.53	F-F	0/898	0.22 (1)
F-G	-2337/0	-102.1 -102.1	0.38 (1)	4.14	L-F	-410/0	0.25 (1)
G-H	-2863/0	-102.1 -102.1	0.41 (1)	3.88	L-G	-221/84	0.05 (1)
H-I	0/31	-102.1 -102.1	0.13 (1)	10.00	K-G	0/2438	0.55 (1)
P-B	-1951/0	0.0 0.0	0.20 (1)	8.03	B-O	0/2438	0.55 (1)
J-H	-1951/0	0.0 0.0	0.20 (1)	8.03	K-H	0/2438	0.55 (1)
P-O	0/0	-38.5 -38.5	0.16 (3)	10.00			
O-N	0/2404	-38.5 -38.5	0.56 (1)	10.00			
N-M	0/2211	-38.5 -38.5	0.53 (2)	10.00			
M-L	0/2211	-38.5 -38.5	0.53 (2)	10.00			
L-K	0/2404	-38.5 -38.5	0.56 (1)	10.00			
K-J	0/0	-38.5 -38.5	0.16 (3)	10.00			

DESIGN CRITERIA
 SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2018, OBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.23")

CSI TO=0.41/1.00 (G-H-1), BC=0.98/1.00 (K-L-1), WB=0.55/1.00 (H-K-1), SS=0.22/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

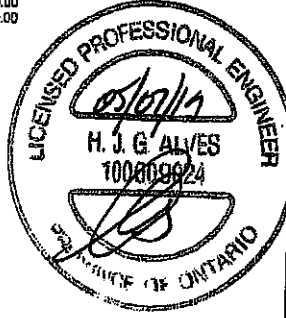
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 818 354 1687 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.85 (Q) (INPUT = 0.80)

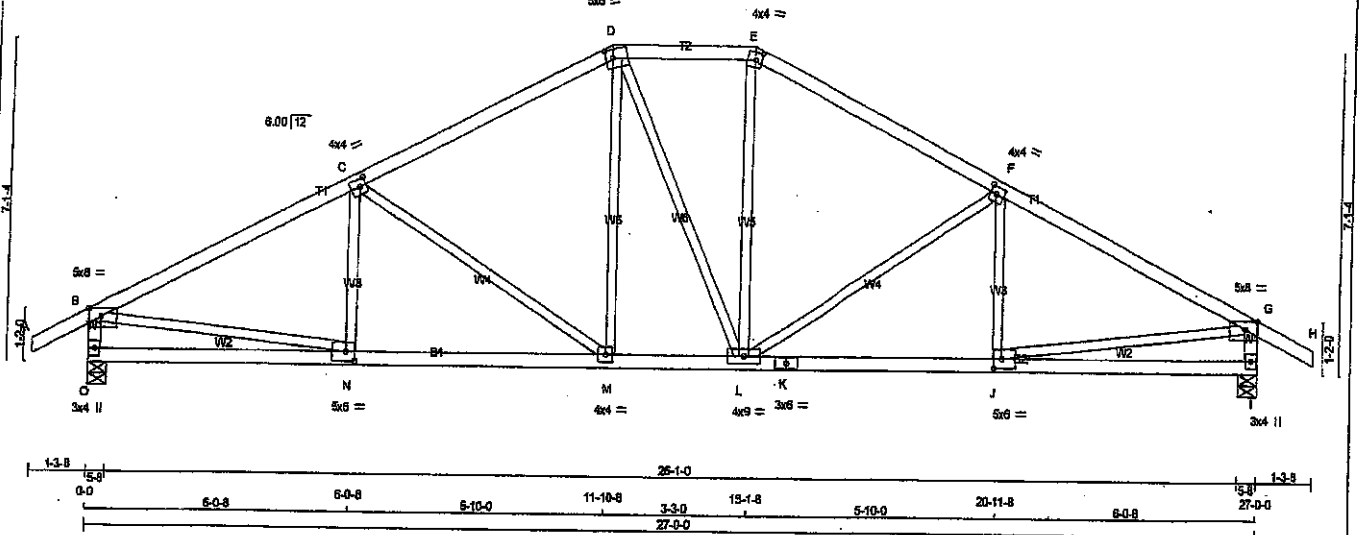
JSI METAL= 0.85 (M) (INPUT = 1.00)



ENCL. NO. TAM 17910781
 STRUCTURAL
 FOR EYE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T24	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue May 7 17:48:16 2019 Page 1
 ID: yH3luQzwKBaXQAh7hsf8URzKCTp-E4BwZfKbU8p9RzNPSCpqSQLf89JGC_xZzdqdT
 1-3-8 0-0 6-0-8 6-0-8 5-10-0 11-10-8 3-3-0 15-1-8 5-10-0 20-11-8 6-0-8 27-0-0 1-3-8
 Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 113 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	5.0	6.0	2.25	2.00
E	TTW-m	MT20	4.0	4.0	2.00	1.75
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	TMVW-p	MT20	5.0	8.0	Edge	3.50
I	BMV-t+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0	2.50	2.25
K	BS-t	MT20	3.0	6.0		
L	BMVW-t	MT20	4.0	9.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	5.0	6.0	2.50	2.25
O	BMV-t+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG	IN-SX
JT VERT	2037	0	2037	0
O VERT	2037	0	2037	0
I VERT	2037	0	2037	0
JT HORZ	0	0	0	0
O HORZ	0	0	0	0
I HORZ	0	0	0	0

UNFACTORED REACTIONS

1ST LCASE	MAX MIN COMPONENT REACTIONS	FACTORED	MAX MIN COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM LIVE
O	1514	863 / 0	284 / 0
I	1514	863 / 0	284 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	FACTORED	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (LBS)	MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (LBS)
FR-TO				FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	N-C	-84 / 185
B-C	-2694 / 0	-102.1	-102.1	0.61 (1)	3.63	C-M	-692 / 0
C-D	-2111 / 0	-102.1	-102.1	0.53 (1)	4.11	M-D	0 / 537
D-E	-1868 / 0	-102.1	-102.1	0.18 (1)	4.78	D-L	0 / 4
E-F	-2113 / 0	-102.1	-102.1	0.53 (1)	4.11	L-E	0 / 541
F-G	-2693 / 0	-102.1	-102.1	0.61 (1)	3.63	L-F	-699 / 0
G-H	0 / 31	-102.1	-102.1	0.13 (1)	10.00	J-F	-86 / 183
O-B	-1942 / 0	0.0	0.0	0.20 (1)	8.04	B-N	0 / 2482
I-G	-1942 / 0	0.0	0.0	0.20 (1)	8.04	J-G	0 / 2481
C-N	0 / 0	-38.5	-38.5	0.27 (3)	10.00		
N-M	0 / 2438	-38.5	-38.5	0.58 (2)	10.00		
M-L	0 / 1866	-38.5	-38.5	0.40 (1)	10.00		
L-K	0 / 2437	-38.5	-38.5	0.57 (2)	10.00		
K-J	0 / 2437	-38.5	-38.5	0.57 (2)	10.00		
J-I	0 / 0	-38.5	-38.5	0.27 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPC 2011, TPC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = 1 / 999 (0.12")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = 1 / 999 (0.20")

CSI: TC=0.61/1.00 (B-C-1), BC=0.58/1.00 (M-N-2),
 WB=0.67/1.00 (C-M-1), SS=0.28/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

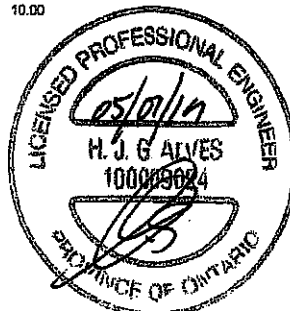
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

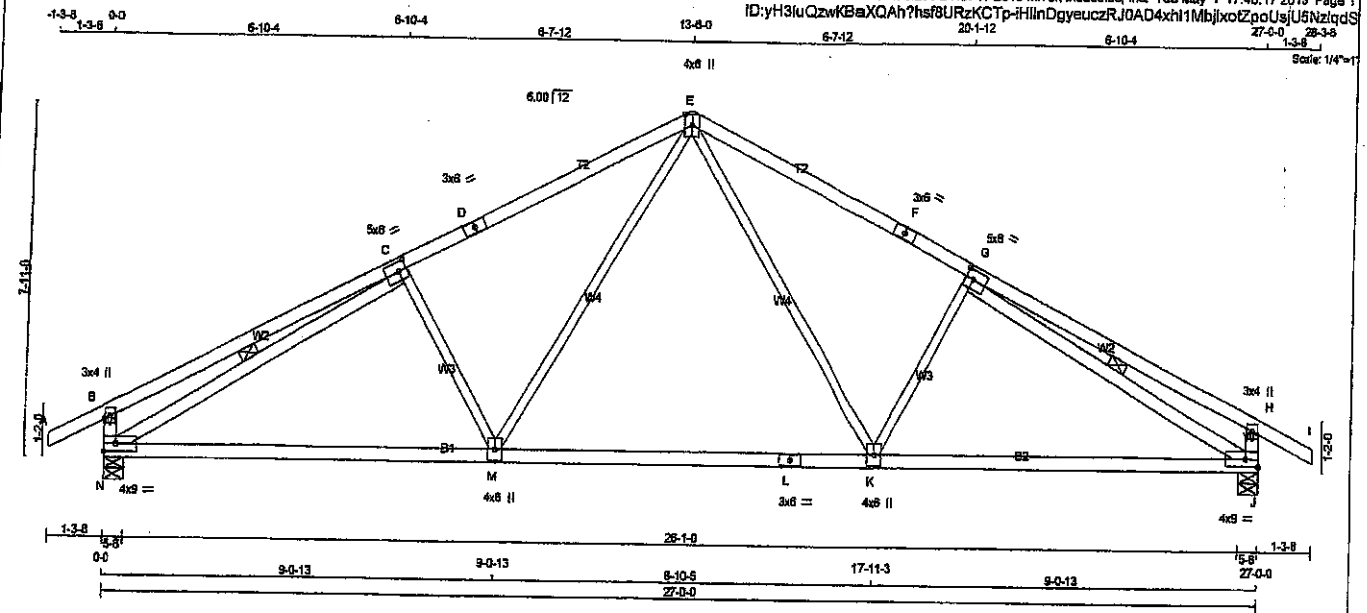
JSI GRIP= 0.89 (E) (INPUT = 0.90)
 JSI METAL = 0.71 (K) (INPUT = 1.00)



REVISION 1 04/07/22
 STRUCTURAL
 CIVIL ENGINEER

JOB NAME 402939	TRUSS NAME T25	QUANTITY 8	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitak Industries, Inc. Tue May 7 17:48:17 2018 Page 1
 ID: yH3iuQzwKBAxQA7hsf8URzKCTp-iHlinDgyeuczRJ0AD4xh1MbjixotZpoUsjU5NzldS
 27-0-0 28-3-8 1-3-8
 Scale: 1/4"=1'



TOTAL WEIGHT = 8 X 109 = 872 lb
 (M/F)

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
N - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
N - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMWW+1	MT20	5.0	6.0	2.50	2.25
D TS-1	MT20	3.0	6.0		
E TTWW+p	MT20	4.0	6.0	Edge	
F TS-1	MT20	3.0	6.0		
G TMWW+1	MT20	5.0	6.0	2.50	2.25
H TMV+p	MT20	3.0	4.0		
I BMVW+1	MT20	4.0	6.0	Edge	
K BMVW+1	MT20	4.0	6.0		
L BS-1	MT20	3.0	6.0		
M BMVW+1	MT20	4.0	6.0		
N BMVW+1	MT20	4.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
N	2037	0	0	2037	0	5-8	5-8		
J	2037	0	0	2037	0	5-8	5-8		

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
N	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-N, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 31	-102.1 -102.1	0.13 (1)	E-K	0 / 937	0.21 (1)	
B-C	0 / 37	-102.1 -102.1	0.78 (1)	K-G	-493 / 7	0.19 (1)	
C-D	-2435 / 0	-102.1 -102.1	0.78 (1)	M-E	0 / 937	0.21 (1)	
D-E	-2435 / 0	-102.1 -102.1	0.78 (1)	C-M	-493 / 7	0.19 (1)	
E-F	-2435 / 0	-102.1 -102.1	0.78 (1)	N-C	-2813 / 0	0.84 (1)	
F-G	-2435 / 0	-102.1 -102.1	0.78 (1)	G-J	-2813 / 0	0.84 (1)	
G-H	0 / 37	-102.1 -102.1	0.78 (1)				
H-I	0 / 31	-102.1 -102.1	0.13 (1)				
N-B	-400 / 0	0.0 0.0	0.04 (1)				
J-H	-400 / 0	0.0 0.0	0.04 (1)				
N-M	0 / 2383	-38.5 -38.5	0.85 (2)				
M-L	0 / 1874	-38.5 -38.5	0.75 (2)				
L-K	0 / 1874	-38.5 -38.5	0.75 (2)				
K-J	0 / 2383	-38.5 -38.5	0.85 (2)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 8 OF BCBC 2018, CBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/898 (0.27")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/708 (0.46")

CSL TO=0.78/1.00 (G-H:1), BC=0.85/1.00 (J-K:2), WB=0.84/1.00 (G-J:1), SS=0.30/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

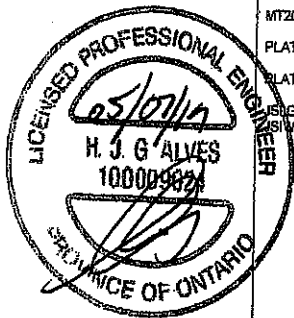
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1686

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SLIP GRIP= 0.88 (G) (INPUT = 0.90)
 SLIP METAL= 0.69 (G) (INPUT = 1.00)



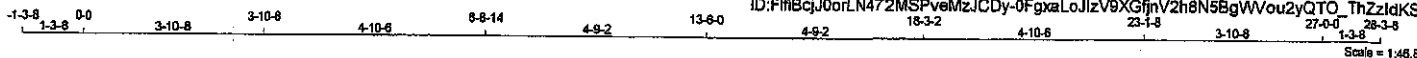
DATE: 11/07/23
 11/07/23
 (1) ONLY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T28	1	2	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID: FfBcJ0orLN472MSPveMzJCdy-0FgxaLoJizV9XGfnVzh8N5BgVVou2yQTO_ThZzldKS



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - E	2x4	DRY	No.2	SPF		
E - H	2x4	DRY	No.2	SPF		
H - J	2x4	DRY	No.2	SPF		
R - B	2x6	DRY	No.2	SPF		
K - I	2x6	DRY	No.2	SPF		
R - O	2x6	DRY	No.2	SPF		
O - K	2x6	DRY	No.2	SPF		

ALL WEBS EXCEPT 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C 1	12	SIDE(61.0)
C - E 1	12	SIDE(61.0)
E - H 1	12	SIDE(0.0)
H - J 1	12	TOP
R - B 2	12	TOP
K - I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R - O 2	12	SIDE(183.1)
O - K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTVWW+m	MT20	5.0	8.0	Edge	1.25
D	TMVWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMVW-w	MT20	2.0	4.0		
G	TMVWW-t	MT20	4.0	4.0		
H	TTVWW+m	MT20	5.0	8.0	Edge	1.25
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1-p	MT20	3.0	6.0		
L	M, P, Q					
L	BMVWW-t	MT20	5.0	6.0		
N	BMVWW-t	MT20	5.0	8.0		
O	BS-t	MT20	5.0	6.0		
R	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	DOWN	UP	IN-SX
R	3067	0	0	5-8
K	2763	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	2274	1311	0	412	0	0	0
R	2274	1311	0	412	0	0	0
K	2047	1188	0	368	0	0	0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 MAX. CSI (LC)	MAX. LENGTH UNBRAC	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO					FR-TO		
A-B	0/45	-102.1	-102.1	0.08 (1)	10.00	Q-C	-406/44
B-C	-3032/0	-102.1	-102.1	0.19 (1)	5.12	C-P	0/2393
C-S	-4075/0	-102.1	-102.1	0.37 (1)	4.34	P-D	-1438/0
S-T	-4075/0	-102.1	-102.1	0.37 (1)	4.34	D-N	0/919
T-D	-4075/0	-102.1	-102.1	0.37 (1)	4.34	N-F	-616/0
D-U	-4743/0	-102.1	-102.1	0.40 (1)	4.04	F-G	0/1001
U-E	-4743/0	-102.1	-102.1	0.40 (1)	4.04	M-G	-1226/0
E-V	-4743/0	-102.1	-102.1	0.40 (1)	4.04	M-H	0/2681
V-F	-4743/0	-102.1	-102.1	0.40 (1)	4.04	L-H	-509/0
F-W	-4743/0	-102.1	-102.1	0.29 (1)	4.18	B-Q	0/2430
W-G	-4743/0	-102.1	-102.1	0.29 (1)	4.18	L-I	0/2148
G-H	-4015/0	-102.1	-102.1	0.26 (1)	4.51		
H-I	-2680/0	-102.1	-102.1	0.18 (1)	5.38		
I-J	0/45	-102.1	-102.1	0.08 (1)	10.00		
R-B	-3002/0	0.0	0.0	0.11 (1)	7.81		
K-I	-2692/0	0.0	0.0	0.10 (1)	7.81		
R-X	0/0	-38.5	-38.5	0.05 (2)	10.00		
X-Q	0/0	-38.5	-38.5	0.05 (2)	10.00		
Q-Y	0/2314	-38.5	-38.5	0.19 (1)	10.00		
Y-Z	0/2314	-38.5	-38.5	0.19 (1)	10.00		
Z-P	0/2314	-38.5	-38.5	0.19 (1)	10.00		
P-AA	0/4075	-38.5	-38.5	0.36 (1)	10.00		
AA-O	0/4075	-38.5	-38.5	0.36 (1)	10.00		
O-AB	0/4075	-38.5	-38.5	0.36 (1)	10.00		
AB-N	0/4075	-38.5	-38.5	0.36 (1)	10.00		
N-AC	0/4015	-38.5	-38.5	0.53 (1)	10.00		
AC-AD	0/4015	-38.5	-38.5	0.53 (1)	10.00		
AD-M	0/4015	-38.5	-38.5	0.53 (1)	10.00		
M-L	0/2041	-38.5	-38.5	0.23 (1)	10.00		
L-K	0/0	-38.5	-38.5	0.03 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-274	-274	—	BACK	VERT	TOTAL	—
Q	3-11-4	-36	-46	—	BACK	VERT	TOTAL	—
S	5-11-4	-96	-96	—	BACK	VERT	TOTAL	—
T	7-11-4	-96	-96	—	BACK	VERT	TOTAL	—
U	9-11-4	-96	-96	—	BACK	VERT	TOTAL	—
V	11-11-4	-96	-96	—	BACK	VERT	TOTAL	—

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.40/1.00 (D-F-1), BC=0.53/1.00 (M-N-1).
WB=0.33/1.00 (H-M-1), SSI=0.19/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.89 (C) (INPUT = 0.90)
SI METAL= 0.65 (O) (INPUT = 1.00)

DWG NO. TAM 1710924
STRUCTURAL
COMPONENT ONLY

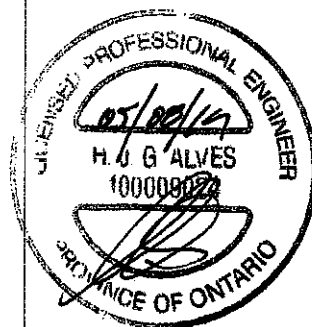
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	T28	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 08:56:01 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	13-11-4	-96	-96	—	BACK	VERT	TOTAL	—	—
X	1-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
Y	5-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
Z	7-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
AA	9-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
AB	11-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
AC	13-11-4	-36	-46	—	BACK	VERT	TOTAL	—	—
AD	15-8-8	-745	-745	—	BACK	VERT	TOTAL	—	—

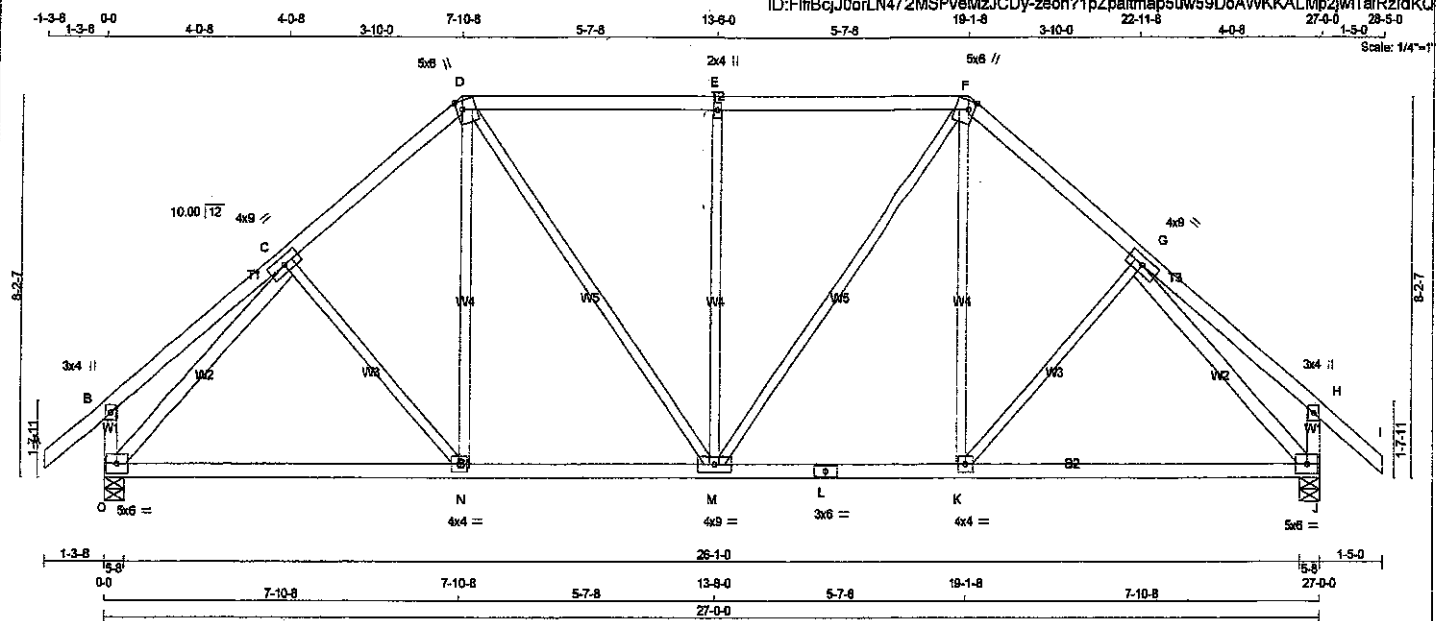
THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



DWG NO. TAM 1910924
STRUCTURAL
COMPONENT ONLY 7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T30	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 08:56:03 2019 Page 1
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TOTAL WEIGHT = 2 X 130 = 261 lb [MIF]

LUMBER	SIZE	DRY	No.2	DESCR.
N.L.G.A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	9.0		
D	TTWW+m	MT20	5.0	6.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	6.0	2.25	1.50
G	TMWW-t	MT20	4.0	9.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORZ	UPLIFT
O	2040	0	2040	0
J	2053	0	2053	0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1516	865 / 0	284 / 0	0 / 0	0 / 0	388 / 0	0 / 0
J	1525	872 / 0	284 / 0	0 / 0	0 / 0	389 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	UNBRAC	MEMB.	MAX. FACTORED	FORCE	MAX
			(LBS)	(PLF)		(LC)		LENGTH			(LBS)	(LC)
FR-TO									FR-TO			
A-B	0 / 45	-102.1	-102.1	0.14	(1)	10.00	C-N	-28 / 84	0.02	(3)		
B-C	0 / 29	-102.1	-102.1	0.25	(1)	10.00	N-D	0 / 393	0.09	(2)		
C-D	-1815 / 0	-102.1	-102.1	0.24	(1)	4.76	D-M	0 / 515	0.12	(1)		
D-E	-1672 / 0	-102.1	-102.1	0.45	(1)	4.61	M-E	-700 / 0	0.92	(1)		
E-F	-1672 / 0	-102.1	-102.1	0.45	(1)	4.61	M-F	0 / 515	0.12	(1)		
F-G	-1815 / 0	-102.1	-102.1	0.24	(1)	4.76	K-F	0 / 393	0.09	(2)		
G-H	0 / 29	-102.1	-102.1	0.25	(1)	10.00	K-G	-28 / 84	0.02	(3)		
H-I	0 / 49	-102.1	-102.1	0.17	(1)	10.00	O-C	-2133 / 0	0.93	(1)		
O-B	-295 / 0	0.0	0.0	0.03	(1)	7.81	G-J	-2133 / 0	0.93	(1)		
J-H	-368 / 0	0.0	0.0	0.03	(1)	7.81						
O-N	0 / 1387	-38.5	-38.5	0.59	(2)	10.00						
N-M	0 / 1373	-38.5	-38.5	0.59	(2)	10.00						
M-L	0 / 1373	-38.5	-38.5	0.59	(2)	10.00						
L-K	0 / 1373	-38.5	-38.5	0.59	(2)	10.00						
K-J	0 / 1387	-38.5	-38.5	0.59	(2)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.16")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.27")

CSI: TO=0.45/1.00 (D-E-1), BC=0.59/1.00 (K-M-2),
WE=0.93/1.00 (G-J-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

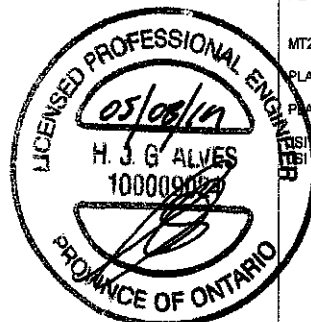
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CSI GRIP= 0.88 (C) (INPUT = 0.90)
CSI METAL= 0.46 (L) (INPUT = 1.00)

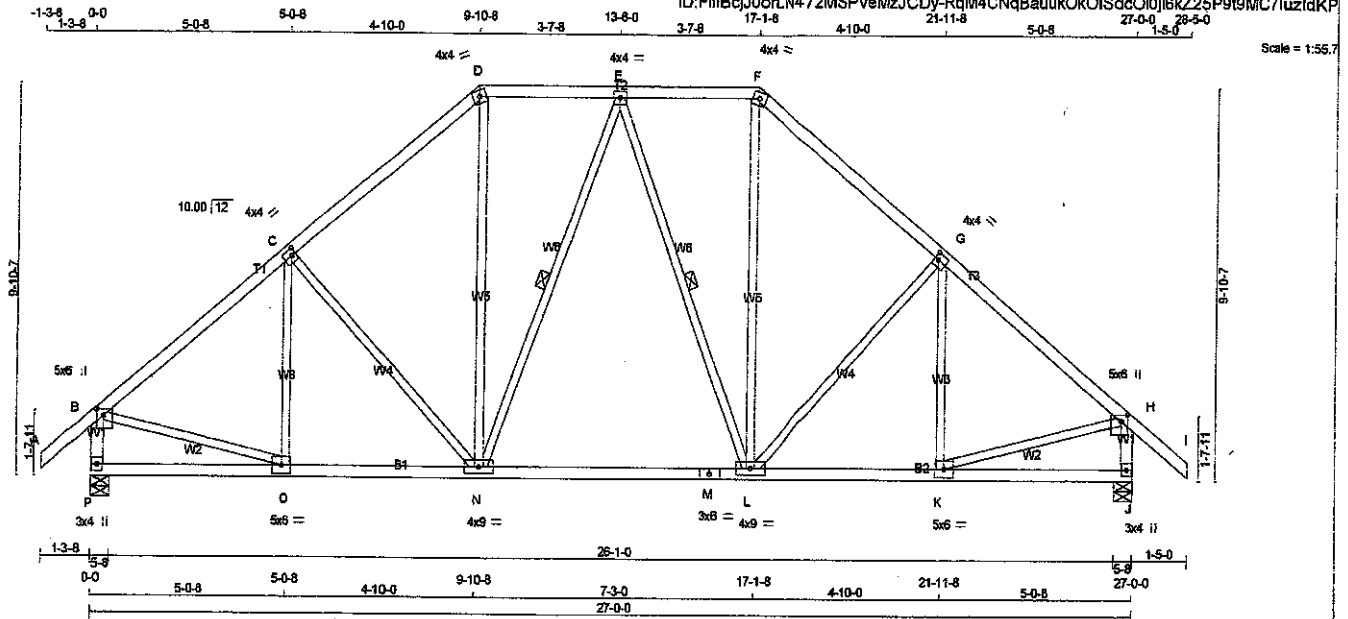


DRWG NO. TAM 1910.926
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T31	2	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID: F1BcJ0orL472MSPveMzJCdy-RqM4CNqBauukOkOISdcO10j6kZ25P919MC7IuzIdKP



TOTAL WEIGHT = 2 X 135 = 270 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - D 2x4 DRY No.2

D - F 2x4 DRY No.2

F - H 2x4 DRY No.2

P - B 2x4 DRY No.2

J - H 2x4 DRY No.2

P - M 2x4 DRY No.2

M - J 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER.

LUMBER

DESCR.

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED GROSS REACTION INPUT RECD BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

P 2040 0 2040 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

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J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

J 2053 0 2053 0 0 5-8 5-8

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")

CALCULATED VERT. DEFL.(LL) = L/998 (0.09")

ALLOWABLE DEFL.(TL) = L/360 (0.90")

CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.39/1.00 (G-H-1), BC=0.43/1.00 (K-L-2),

WB=0.37/1.00 (G-L-1), SSI=0.19/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

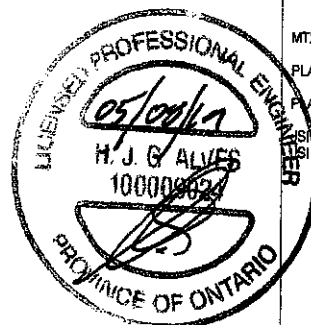
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

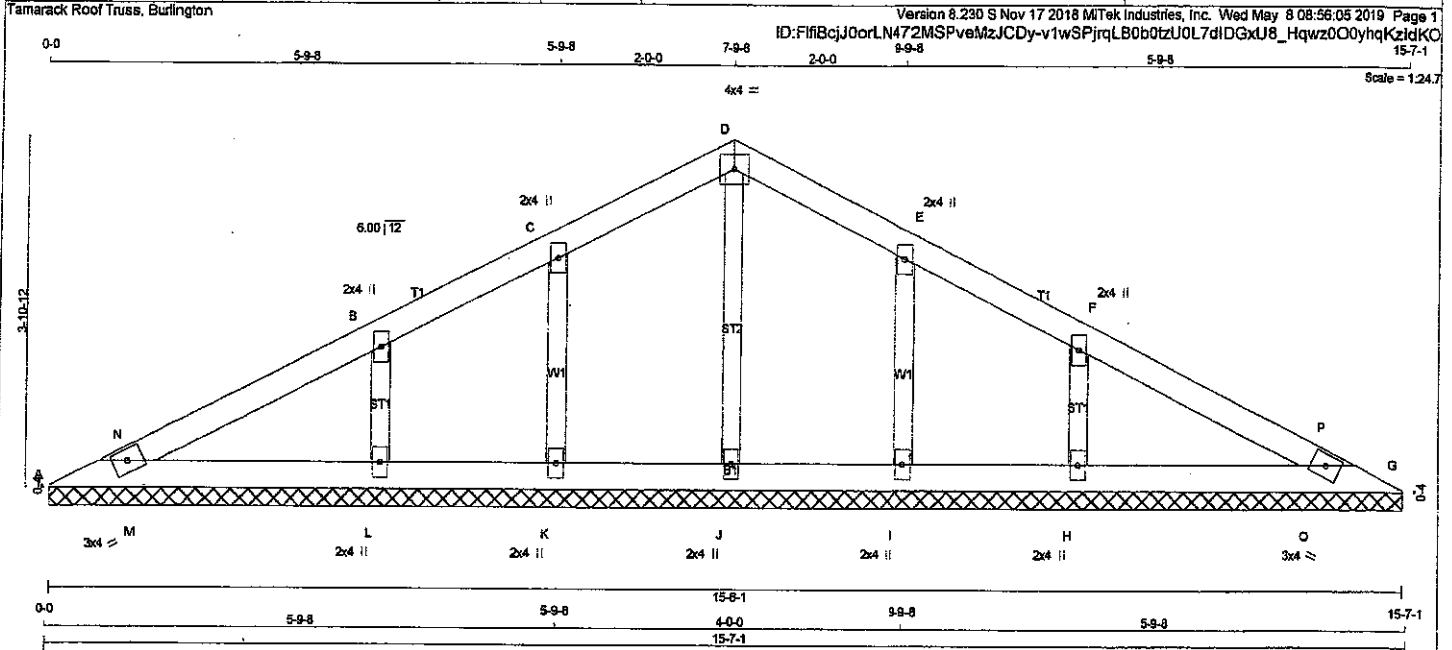
SLIP GRIP= 0.87 (B) (INPUT = 0.90)

SLIP METAL= 0.42 (B) (INPUT = 1.00)



DWG NO. TAM 1910927
STRUCTURAL
COMPONENT ONLY

JOB NAME 402975	TRUSS NAME V20	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TBM1-h	MT20	3.0	4.0	
B, C, E, F				
B TMW+w	MT20	2.0	4.0	
D TTW-p	MT20	4.0	4.0	
G TBM1-h	MT20	3.0	4.0	
H, I, J, K, L				
H BMW1+w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	167	0	167	0	15-6-1	15-6-1
G	167	0	167	0	15-6-1	15-6-1
J	417	0	417	0	15-6-1	15-6-1
L	489	0	489	0	15-6-1	15-6-1
H	489	0	489	0	15-6-1	15-6-1
K	216	0	216	0	15-6-1	15-6-1
I	216	0	216	0	15-6-1	15-6-1

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	124	70/0	24/0	0/0	0/0	30/0	0/0
G	124	70/0	24/0	0/0	0/0	30/0	0/0
J	314	164/0	70/0	0/0	0/0	80/0	0/0
L	374	204/0	77/0	0/0	0/0	93/0	0/0
H	374	204/0	77/0	0/0	0/0	93/0	0/0
K	159	94/0	28/0	0/0	0/0	38/0	0/0
I	159	94/0	28/0	0/0	0/0	38/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, L, H, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO					
FR-TO								
A-N	0/110	-102.1	-102.1	0.08 (1)	10.00	J-D	-314/0	0.07 (1)
N-B	0/153	-102.1	-102.1	0.15 (1)	10.00	L-B	-314/0	0.05 (1)
B-C	0/131	-102.1	-102.1	0.15 (1)	10.00	H-F	-314/0	0.05 (1)
C-D	0/143	-102.1	-102.1	0.08 (1)	10.00	K-C	-189/0	0.03 (1)
D-E	0/143	-102.1	-102.1	0.08 (1)	10.00	I-E	-189/0	0.03 (1)
E-F	0/131	-102.1	-102.1	0.15 (1)	10.00	M-N	-81/37	0.00 (1)
F-P	0/153	-102.1	-102.1	0.15 (1)	10.00	O-P	-81/37	0.00 (1)
P-G	0/110	-102.1	-102.1	0.08 (1)	10.00			
A-M	-129/0	-38.5	-38.5	0.11 (1)	6.25			
M-L	-116/0	-38.5	-38.5	0.11 (1)	6.25			
L-K	-129/0	-38.5	-38.5	0.10 (1)	6.25			
K-J	-134/0	-38.5	-38.5	0.03 (2)	6.25			
J-I	-134/0	-38.5	-38.5	0.03 (2)	6.25			
I-H	-129/0	-38.5	-38.5	0.10 (1)	6.25			
H-O	-116/0	-38.5	-38.5	0.11 (1)	6.25			
O-G	-129/0	-38.5	-38.5	0.11 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (B-N-1), BC=0.11/1.00 (L-M-1), WB=0.07/1.00 (D-J-1), SSI=0.13/1.00 (B-N-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

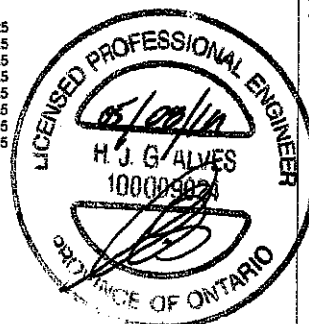
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (F) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)

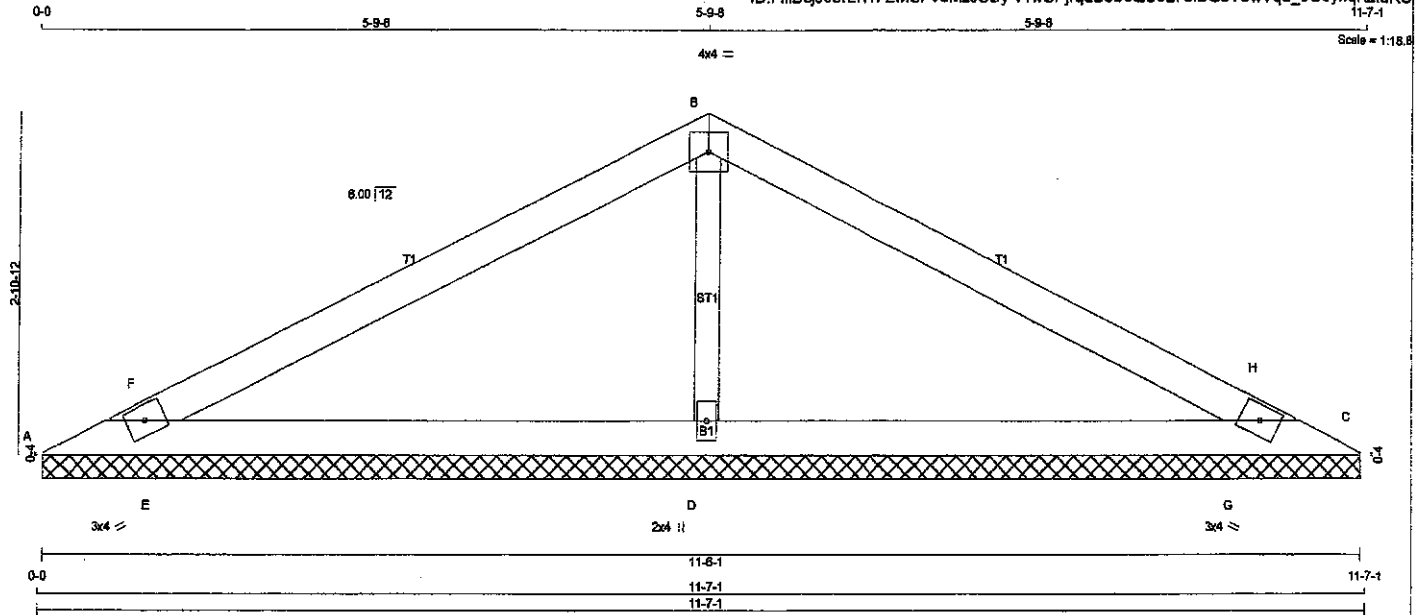


DRWG NO. TAM 71910928
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	V21	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: FfIBcjJ0orLN472MSPveMzJCDy-v1wSPjrqLB0b0tzU0L7dIDGsY8wVqu_0C0yHqKzldKC



TOTAL WEIGHT = 28 lb
[M]

LUMBER				DESCR.	
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	SPF	
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
A	TBM1-h	MT20	3.0	4.0	
B	TTW-p	MT20	4.0	4.0	
C	TBM1-h	MT20	3.0	4.0	
D	BMW1-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	38	0	38	0	11-6-1	11-6-1
C	38	0	38	0	11-6-1	11-6-1
D	1545	0	1545	0	11-6-1	11-6-1

UNFACTORED REACTIONS

JT	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
A	29	11/0	9/0	0/0	0/0	8/0	0/0
C	29	11/0	9/0	0/0	0/0	8/0	0/0
D	1151	648/0	223/0	0/0	0/0	282/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-F	0/792	-102.1	-102.1	0.23 (1)	D-B	-1174/0	0.20 (1)
F-B	0/801	-102.1	-102.1	0.47 (1)	E-F	-279/32	0.00 (1)
B-H	0/801	-102.1	-102.1	0.47 (1)	G-H	-279/32	0.00 (1)
H-C	0/792	-102.1	-102.1	0.23 (1)			
A-E	-757/0	-38.5	-38.5	0.28 (1)			
E-D	-722/0	-38.5	-38.5	0.35 (1)			
D-G	-722/0	-38.5	-38.5	0.35 (1)			
G-C	-757/0	-38.5	-38.5	0.28 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (B-F:1), BC=0.35/1.00 (D-E:1), WB=0.20/1.00 (B-D:1), SSI=0.18/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

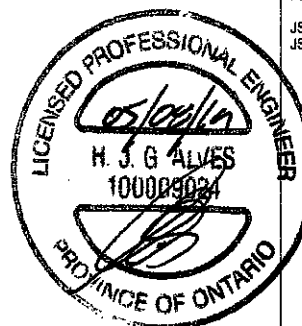
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)

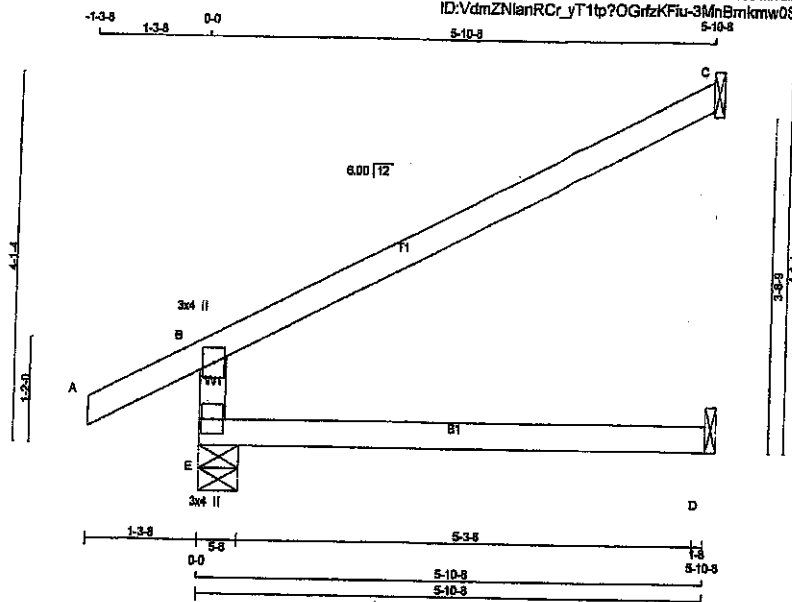


DWG NO. TAM 1910929
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J1	QUANTITY 19	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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ID:VdmZNlanRCr_yT1tp?OGrfzKFlu-3MnBmkmw0SbQL4YK_XAlqd5QSVtyJbkBMM7ZNLzlgj

Scale = 1:23.6



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	646	0	646	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	184	128 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED CS1 (LC)
FR-TO		FROM TO				
E-B	-514 / 0	0.0	0.0	0.22 (3)	7.81	
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
B-C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25	
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00	

TOTAL WEIGHT = 19 X 17 = 319 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/942$ (0.07")

CS1: TO=0.80/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (A-B:0), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	394	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

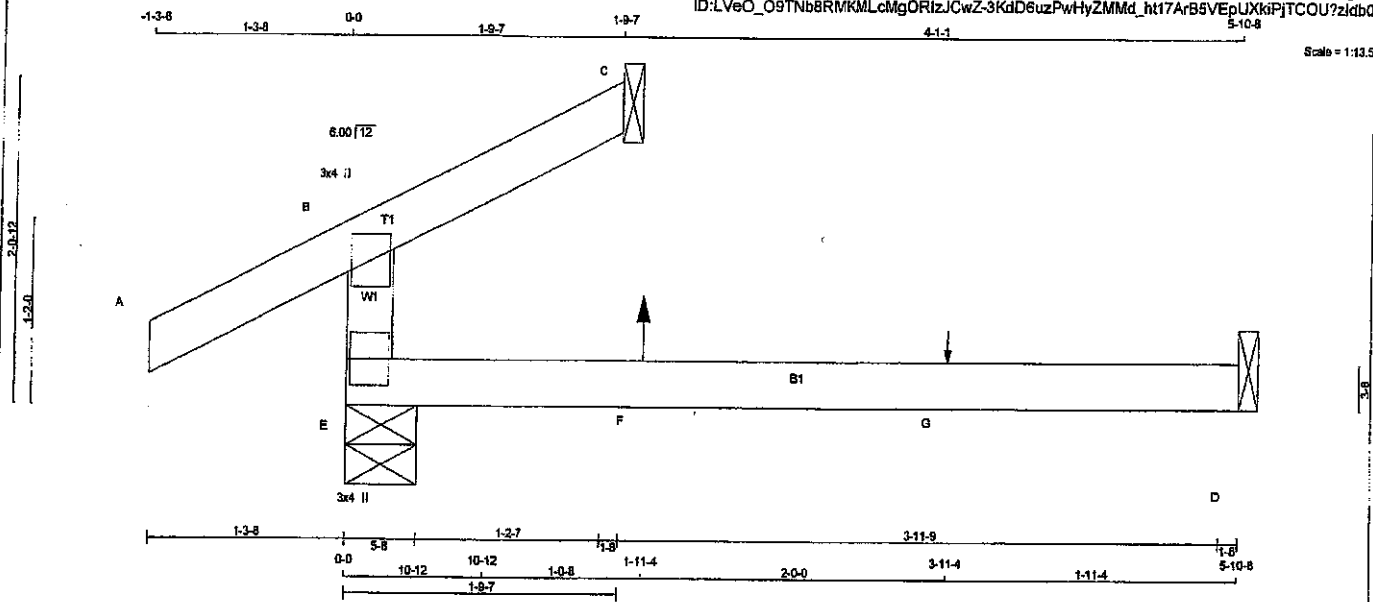
JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



DRWG NO. TAM **1910753**
STRUCTURAL
COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID: L.VeO_09TNb8RMKMLcMgORtZJCwZ-3KdD6uzPwHyZMMd_ht17ArB5VEpUXkPjTCOU?2idb0
 5-10-8



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	352	0	352	0	5-8	5-8
C	94	0	97	0	1-8	1-8
D	94	0	123	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LOOSE		MAX. MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	259	156 / 0	42 / 0	0 / 0	0 / 0	61 / 0	0 / 0
C	76	24 / 0	28 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	86	0 / -3	53 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO					
E-B	-229 / 26	0.0 0.0	0.18 (3)	7.81			
A-B	0 / 31	-102.1 -102.1	0.15 (1)	10.00			
B-C	0 / 27	-102.1 -102.1	0.16 (3)	10.00			
E-F	0 / 0	-38.5 -38.5	0.23 (3)	10.00			
F-G	0 / 0	-38.5 -38.5	0.23 (3)	10.00			
G-D	0 / 0	-38.5 -38.5	0.23 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11-4	8	1	13	FRONT	VERT	TOTAL	-	-
G	3-11-4	1	1	-	FRONT	VERT	TOTAL	-	-

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/839 (0.06")

CSK TC=0.18/1.00 (B-E3), BC=0.23/1.00 (D-E3), VB=0.00/1.00 (n/a0), SSI=0.14/1.00 (D-E3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

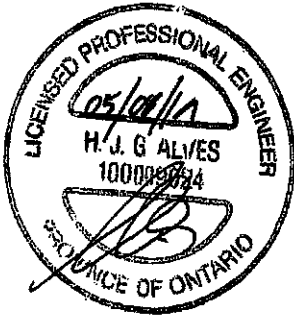
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 554 1667 788 1967 1956

PLATE PLACEMENT TOL. = 0.250 inches

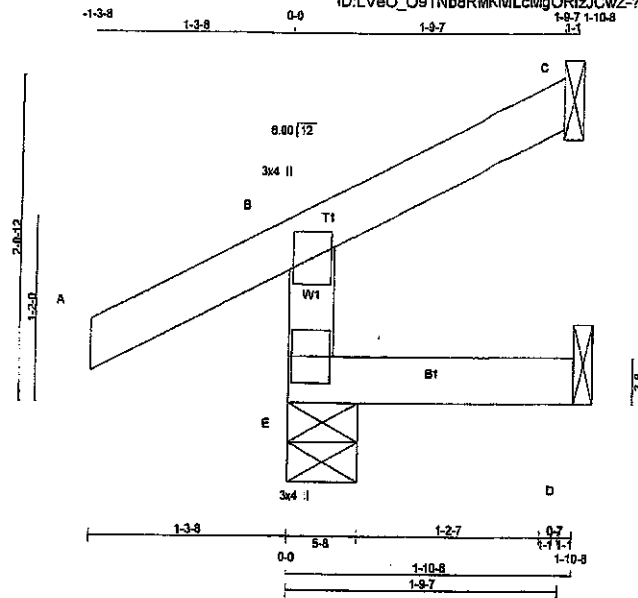
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.90)
 JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 17910798
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1:13.5

TOTAL WEIGHT = 2 X 7 = 14 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
E	BMV+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	318	0	318	0	5-8	5-8
C	51	0	51	0	1-8	1-8
D	24	0	41	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	226	160 / 0	20 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	36	27 / -21	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	24	0 / -9	18 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED UNBRACED LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
E-B	-270 / 0	0.0	0.0	7.81
A-B	0 / 31	-102.1	-102.1	10.00
B-C	-18 / 0	-102.1	-102.1	6.25
E-D	0 / 0	-38.5	-38.5	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA D88-08, CSA D88-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.13/1.00 (A-B-1), BC=0.04/1.00 (D-E-5),
WB=0.00/1.00 (n/a-0), SSI=0.10/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

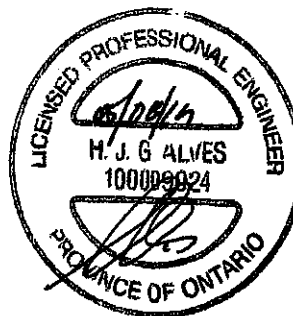
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

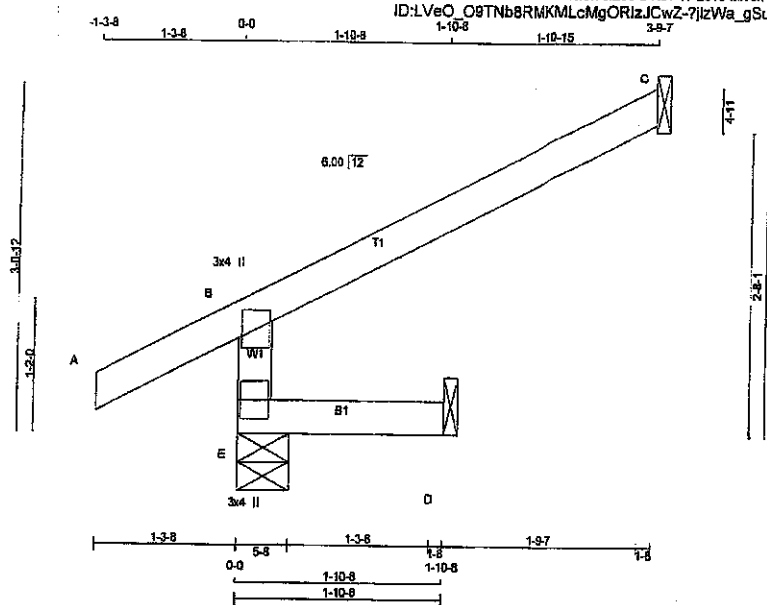


DRWG NO. TAM 091 0800
STRUCTURAL
COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

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ID: LveO_O8TNb8RMKMLcMgORlZJCwZ-7jZWa_gSuCGbgnNol3bFGHQ?1X8?dCIBnhVZuzldo

Scale = 1:18.7



LUMBER			
N.L.G.A. RULES	SIZE	DRY	LUMBER
CHORDS	2x4	DRY	No.2
E - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
E - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	SMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
E	420	0	420	0	0	5-8	5-8	1-8	1-8
C	145	0	145	0	0	1-8	1-8	1-8	1-8
D	32	0	41	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS		1ST LOASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
E	297	216 / 0	22 / 0	0 / 0	59 / 0
C	100	83 / 0	0 / 0	0 / 0	17 / 0
D	29	0 / 0	18 / 0	0 / 0	12 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. FACTORED (LBS)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
E-B	-380 / 0	0.0	0.0	0.01 (3)	7.81
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00
B-C	-22 / 0	-102.1	-102.1	0.25 (1)	6.25
E-D	0 / 0	-38.5	-38.5	0.03 (3)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 2 X 10 = 19 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/899 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/899 (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.03/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

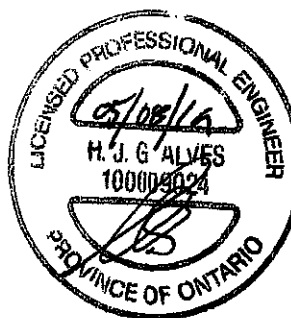
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PLI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

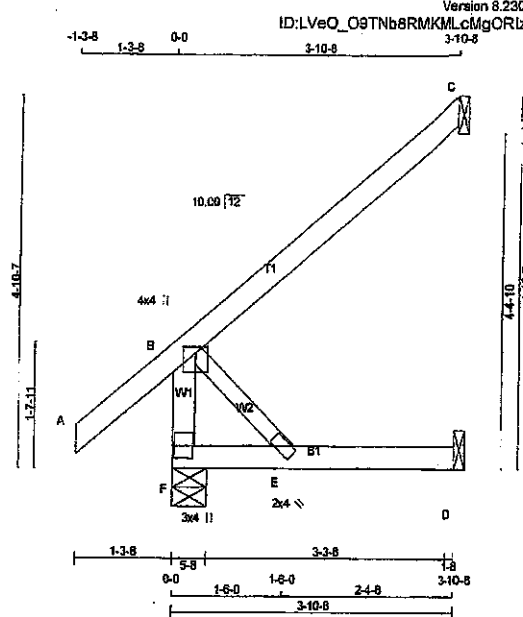
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.80)
JSI METAL= 0.11 (B) (INPUT = 1.00)



DRWG NO. TAM 1910801
STRUCTURAL
COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J7	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1:27.9

TOTAL WEIGHT = 4 X 15 = 61 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMVW+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
F	414	0	414	0
C	198	0	198	0
D	75	0	95	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
F	301	193 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	136	113 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	88	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM	TO	FR-TO			
F-B	-339 / 0	0.0	0.0	0.04 (1)	7.81	8-E	0 / 0
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00		
B-C	0 / 0	-102.1	-102.1	0.26 (1)	10.00		
F-E	0 / 0	-38.5	-38.5	0.13 (3)	10.00		
E-D	0 / 0	-38.5	-38.5	0.14 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.28/1.00 (B-C:1), 8C=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

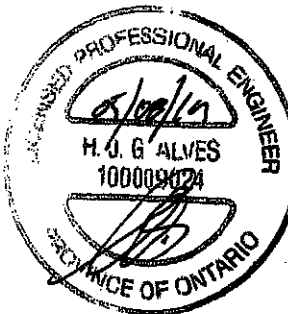
NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	618	354
	1967	788
	1987	1896

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

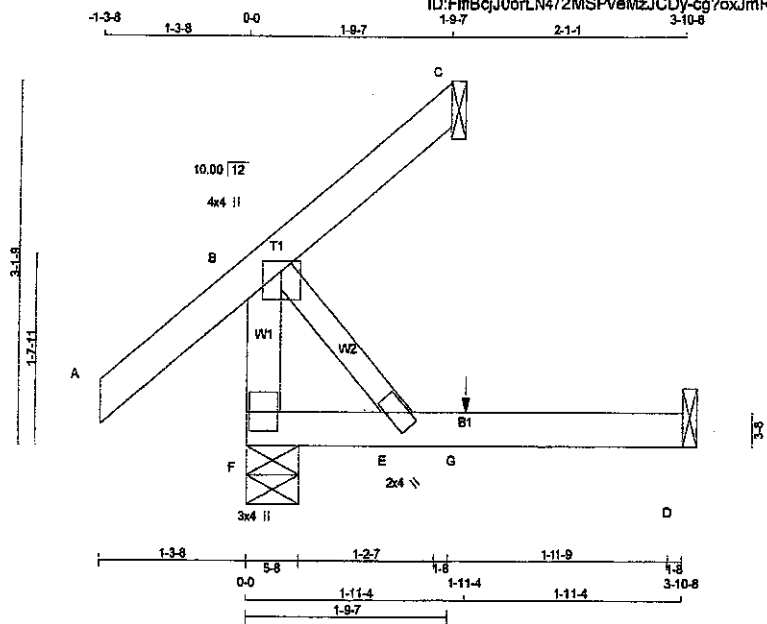
JSI GRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DRWG NO. TAM 1010803
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	J21	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:55:58 2019 Page 1
ID: FmBcjJ0orLN472MSPveMzJCDy-cg?oxJmR?27agox86MV_WITkDJaGhnN_nQlp4EzIdKV



TOTAL WEIGHT = 12 lb

LUMBER				DESCR.	
N.L.G.A. RULES	CHORDS	SIZE	LUMBER		
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV+w	MT20	2.0	4.0		
F BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	362	0	362	0	0	5-8	5-8
C	36	0	36	0	-45	1-8	1-8
D	75	0	95	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	265	164 / 0	41 / 0	0 / 0	0 / 0	61 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	68	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED				WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX	MAX. CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO				FR-TO			
F-B	-288 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 45	-102.1	-102.1	0.15 (1)	10.00				
B-C	-35 / 0	-102.1	-102.1	0.15 (1)	8.25				
F-E	0 / 0	-38.5	-38.5	0.12 (2)	10.00				
E-G	0 / 0	-38.5	-38.5	0.14 (3)	10.00				
G-D	0 / 0	-38.5	-38.5	0.14 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)		FACE		DIR.		TYPE		HEEL		CONN.	
JT	LOC.	LC1	MAX-	MAX+	FRONT	VERT	TOTAL				
G	1-11-4	1	1	-							

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CS1: TC=0.15/1.00 (A-B:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SS1=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

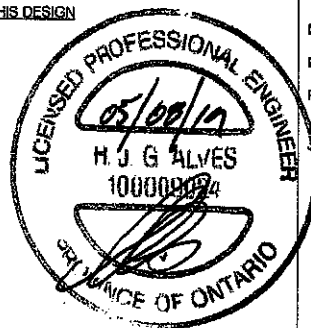
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

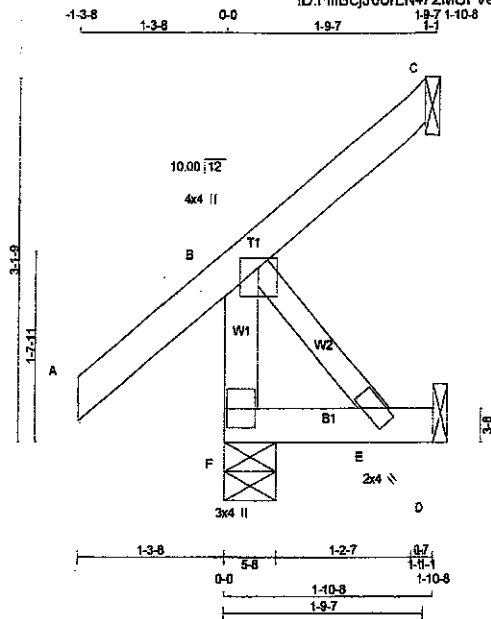
SI GRIP= 0.22 (B) (INPUT = 0.90)
SI METAL= 0.06 (B) (INPUT = 1.00)



JOB NO. TAM 71910921
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	J22	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:55:59 2019 Page 1
ID: FfBcjJ0orLN472MSPveMzJCDy-4tYB9fm3mLFRIyWKf40D2y0v8jy8QEd804VNcgzldKkU



Scale = 1:19.1

TOTAL WEIGHT = 10 lb

LUMBER	SIZE	DRY	LUMBER	DESCR.
N.L.G.A. RULES				
CHORDS	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW+p	MT20	4.0	4.0	1.00 2.00
E BMW+w	MT20	2.0	4.0	
F BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	RECRD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
F 324	324	0	5-8
C 36	36	0	1-8
D 36	48	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS	MAX/MIN. COMPONENT REACTIONS
1ST LCASE	
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F 230	184 / 0 20 / 0 0 / 0 0 / 0 47 / 0 0 / 0
C 25	21 / -31 0 / 0 0 / 0 0 / 0 4 / 0 0 / 0
D 33	0 / 0 20 / 0 0 / 0 0 / 0 13 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	WEBS
MAX. FACTORED	MAX. FACTORED
MEMB. FORCE (LBS)	MEMB. FORCE (LBS)
FR-TO	FR-TO
FROM TO	FROM TO
VERT. LOAD LC1	VERT. LOAD LC1
MAX. CS1 (LC)	MAX. CS1 (LC)
UNBRACED LENGTH	UNBRACED LENGTH
F-B -288 / 0	0.0 0.0 0.03 (1) 7.81
A-B 0 / 45	-102.1 -102.1 0.14 (1) 10.00
B-C -35 / 0	-102.1 -102.1 0.13 (1) 6.25
F-E 0 / 0	-38.5 -38.5 0.03 (3) 10.00
E-D 0 / 0	-38.5 -38.5 0.03 (3) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CS1: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

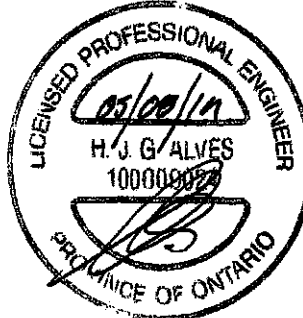
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

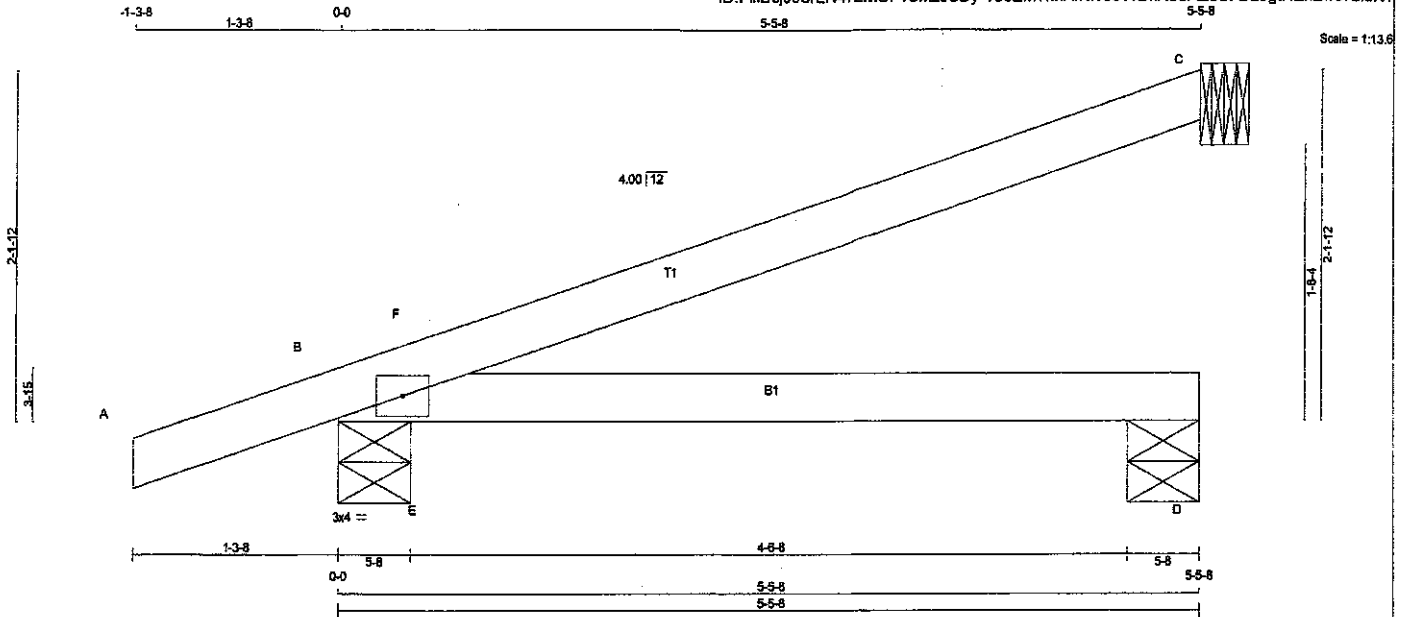
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.22 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)



DRWG NO. TAM 1910922
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	J23	7	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:58:00 2019 Page 1						
ID: FfBcJ0orLN472MSPveMzJCDy-Y36ZM?nhXfNiv65WdNXSbAZ0c7Dz9gtHEkEw87zldKT						



TOTAL WEIGHT = 7 X 14 = 101 LB

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
C	250	0	250	0	5-8	5-8	5-8	5-8
B	520	0	520	0	5-8	5-8	5-8	5-8
D	133	0	145	0	5-8	5-8	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	381	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS		
MAX. FACTORED		FACTORED			MAX. FACTORED		
MEMB.	FORCE	VERT. LOAD	LC1	MAX.	MEMB.	FORCE	
	(LBS)	(PLF)	CSI (LC)	UNBRAC		CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	E-F	-254 / 122
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25		0.00 (1)
F-C	0 / 6	-102.1	-102.1	0.42 (1)	10.00		
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00		
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/842$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/488$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DRWG NO. TAM 719/09123
STRUCTURAL
COMPONENT ONLY



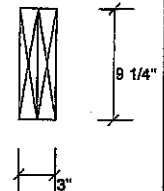
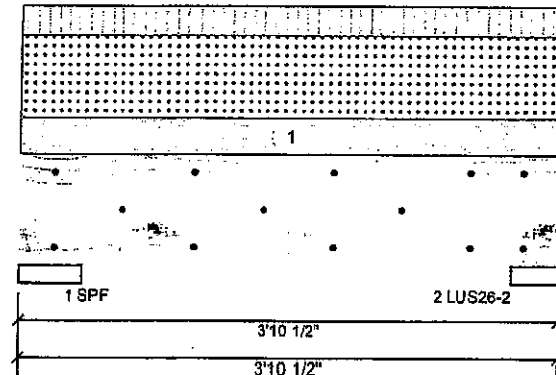
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 201018
Project #:

Page 1 of 2

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	138	171	388	0
2	130	160	364	0

Bearings and Factored Reactions

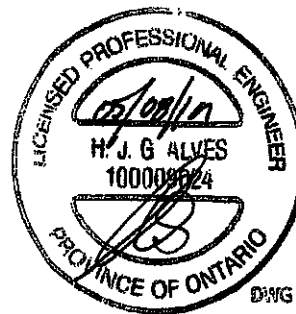
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	9% 214 / 721	935 L	1.25D+1.5S +L
2 - LUS26-2	4.000"	12% 201 / 876	876 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	601 ft-lb	2'	6039 ft-lb	0.100 (10%)	1.25D+1.5S +L	L
Unbraced	601 ft-lb	2'	5734 ft-lb	0.105 (10%)	1.25D+1.5S +L	L
Shear	750 lb	12"	3984 lb	0.188 (19%)	1.25D+1.5S +L	L
LL Defl inch	0.002 (L/19241)	2'	0.107 (L/360)	0.020 (2%)	S+0.5L	L
TL Defl inch	0.003 (L/14002)	2'	0.107 (L/360)	0.030 (3%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1010811
STRUCTURAL
COMPOSITE PLATE 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-7-0	Near Face	13 PSF	10.5 PSF	28.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3288 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

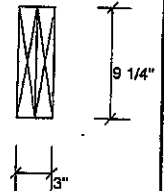
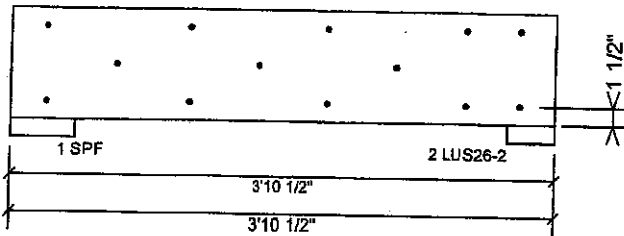
Client:
Project:
Address:

Date: 5/8/2019
Designer:
Job Name: 201018
Project #:

Page 2 of 2

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	68.7 %
Load	233.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17910811
STRUCTURAL
COMPONENT ONLY 4/2

Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (800) 335-1115
	TAMARACK LUMBER INC ALMA LUMBER GROUP

This design is valid until 12/11/2021



THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



TECH-NOTES

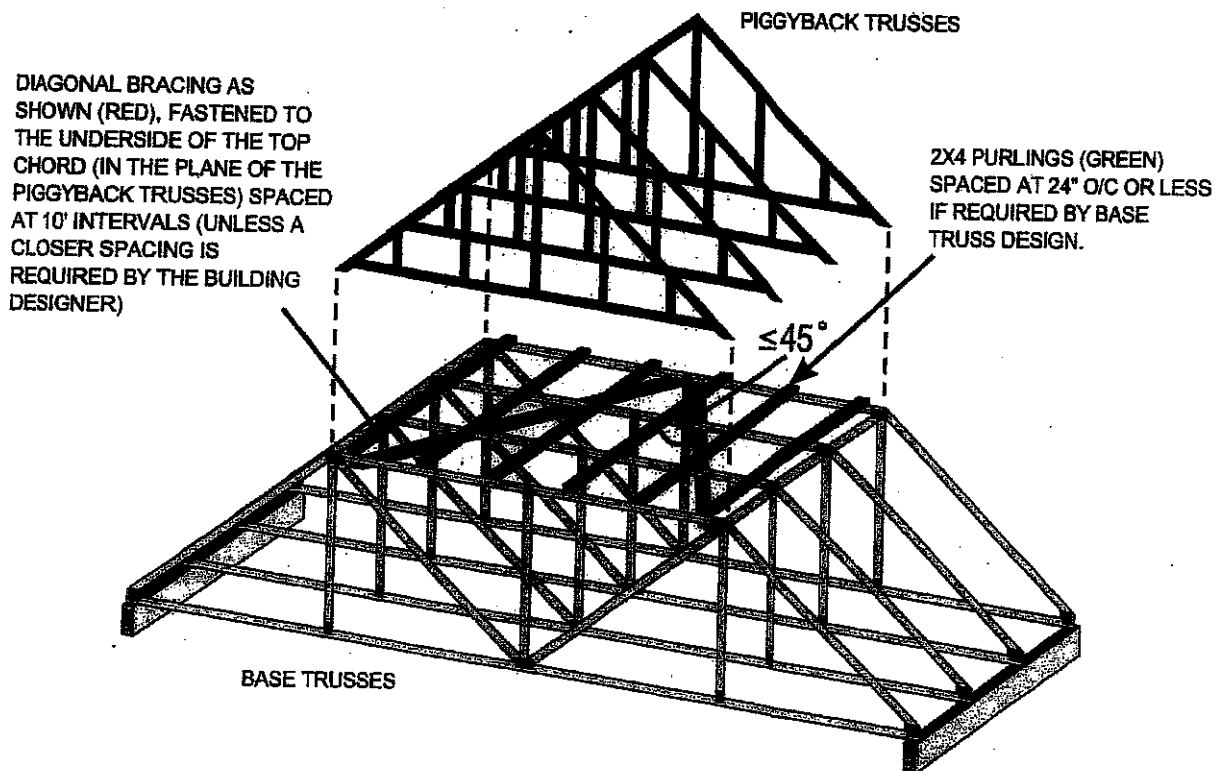
TN 15-001
Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committees and although there may be professional engineers involved in development, the information contained in the tech-note is not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

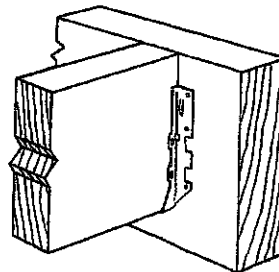
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

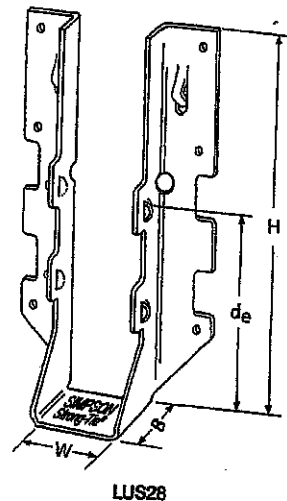
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

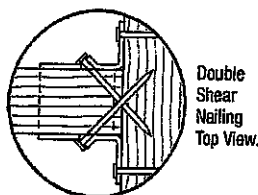
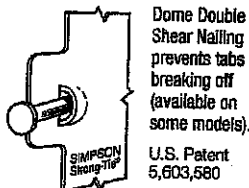


Typical LUS Installation



Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist		D.Fir-L		S-P-F	
									Uplift	Normal	Uplift	Normal
									(K _p =1.15)	(K _p =1.00)	(K _p =1.15)	(K _p =1.00)
LUS24	18	1 1/8	3 1/8	1 1/4	1 1/8	(4) 10d	(2) 10d		710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d		835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 1/4	3 3/8	(4) 10d	(4) 10d		1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d		1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 1/4	3 3/8	(6) 10d	(6) 10d		1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d		1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/8	(8) 10d	(4) 10d		1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d		2580	4500	2320	3195
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d		2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



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HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

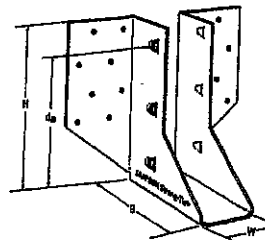
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

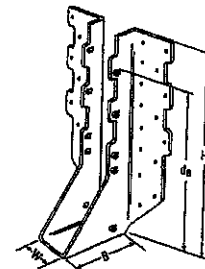
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

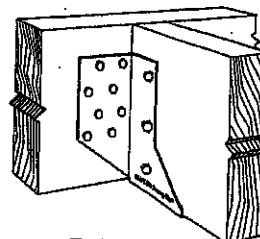
- See current catalogue for options



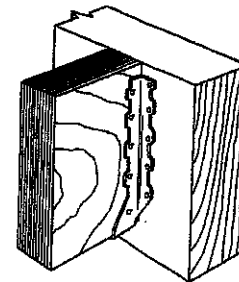
LJS26DS



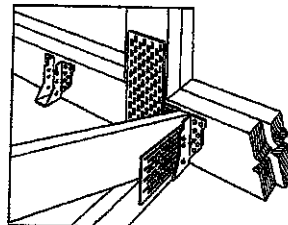
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



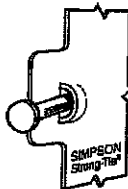
Typical HUS
Installation



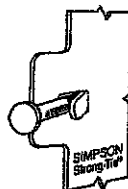
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _p =1.00)	Uplift (K _u =1.15)	Normal (K _p =1.00)
LJS26DS	18	1½	5	3½	4½	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1½	5½	3	3½	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1½	7½	3	6½	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1½	9½	3	7½	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1½	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

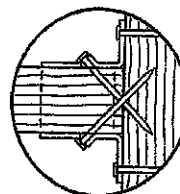
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

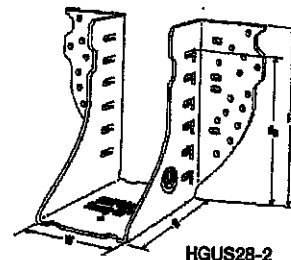
Material: 12 gauge

Finish: G90 galvanized

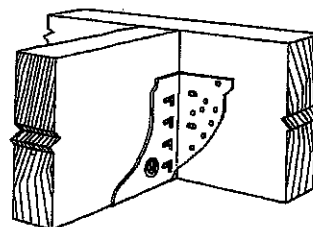
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

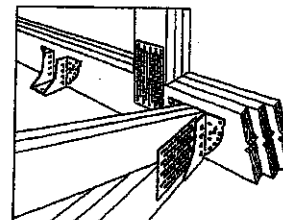
- See current catalogue for options



HGUS28-2



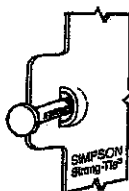
Typical HGUS
Installation



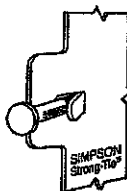
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1 1/8	5 3/8	5	4 1/2	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3 3/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 3/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210-2	12	3 3/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645

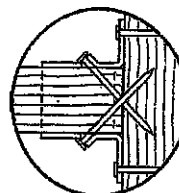
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



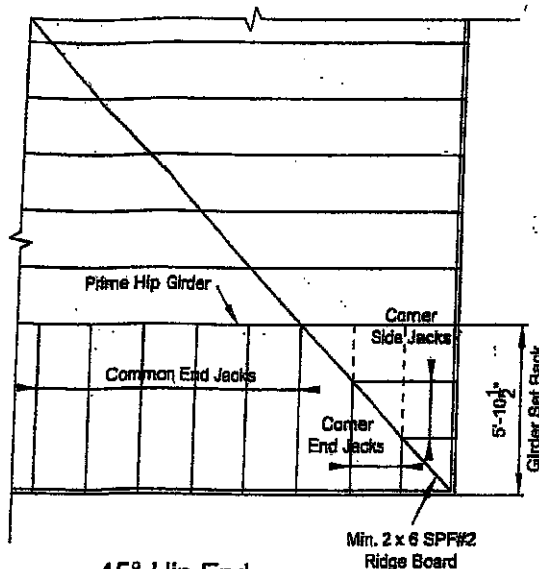
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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45° Hip End

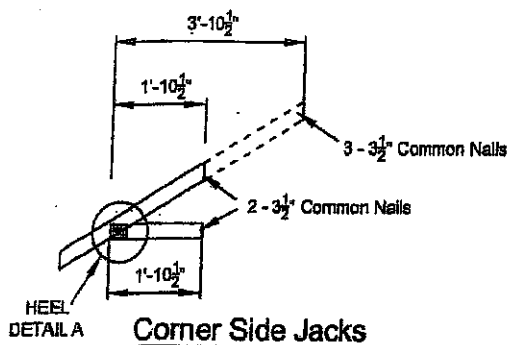
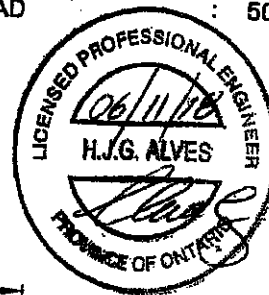
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

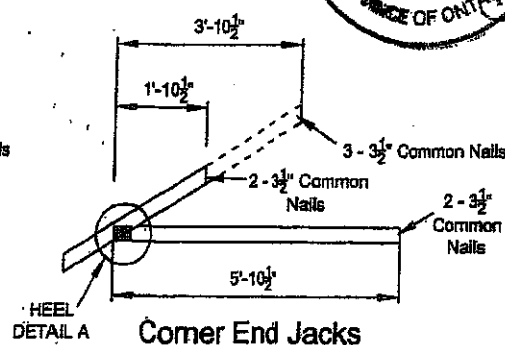
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

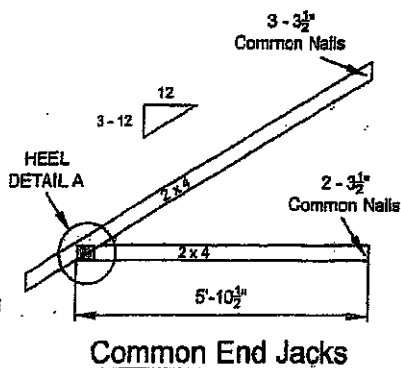
TOTAL LOAD : 50.5 P.S.F



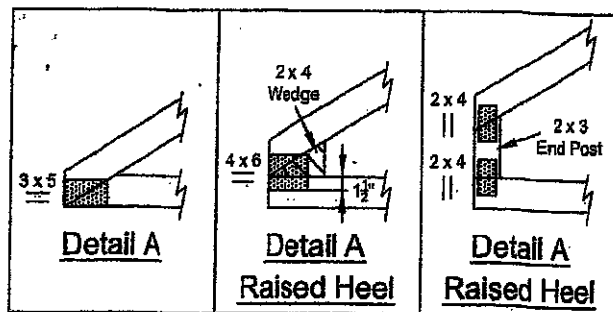
Corner Side Jacks



Corner End Jacks

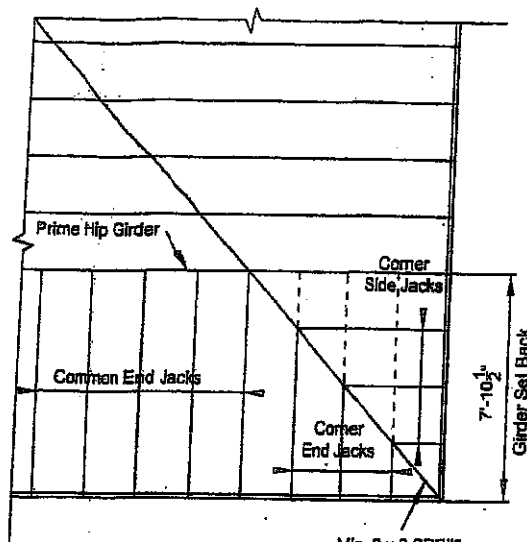


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800218



45° Hip End

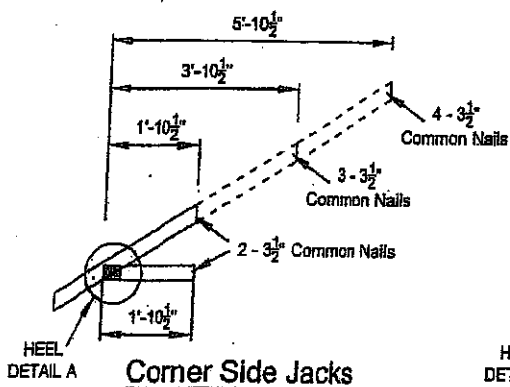
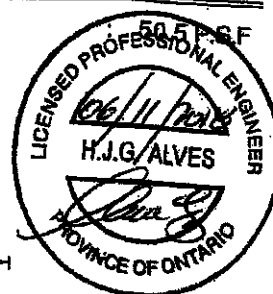
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

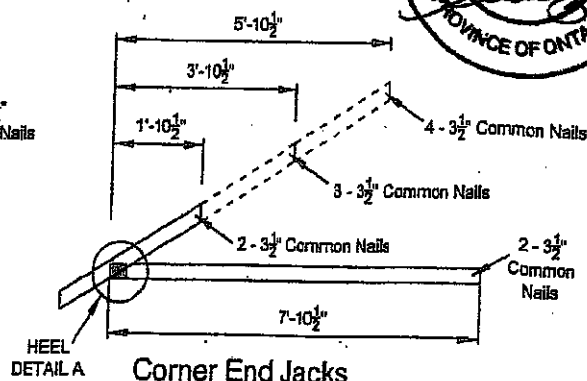
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

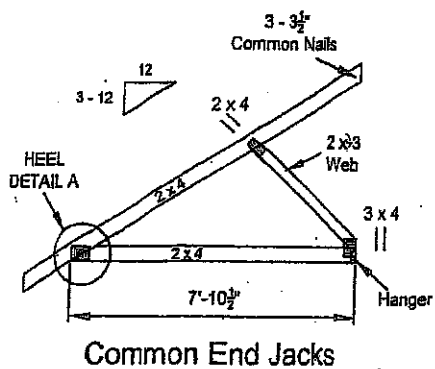
TOTAL LOAD



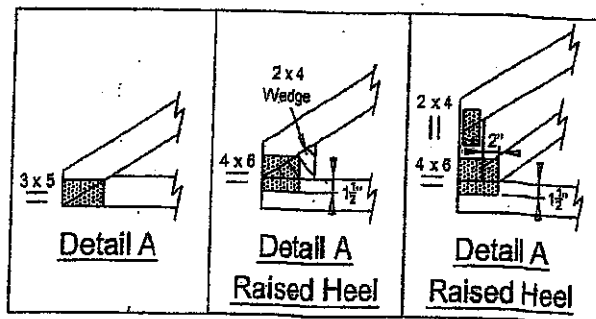
Corner Side Jacks



Corner End Jacks



Common End Jacks

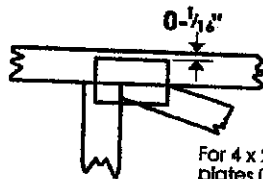
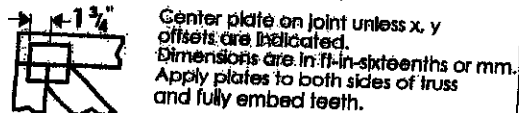


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



*Plate location details available in MiTek software or upon request.

PLATE SIZE

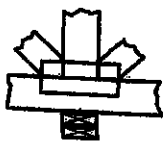
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



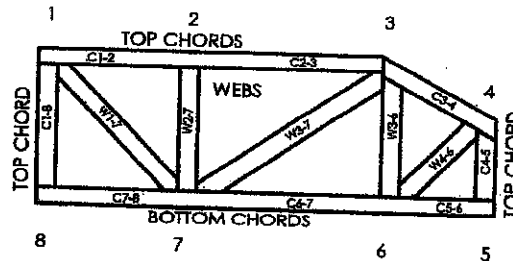
BEARING



Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses Design Standard for Bracing.
DSB-89: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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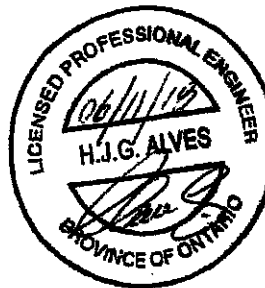
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with the retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE

CONSTRUCTED TO MEET OR

EXCEED THE PROVISIONS OF

THE ONTARIO BUILDING CODE

T-1900218

Feb 09, 2018