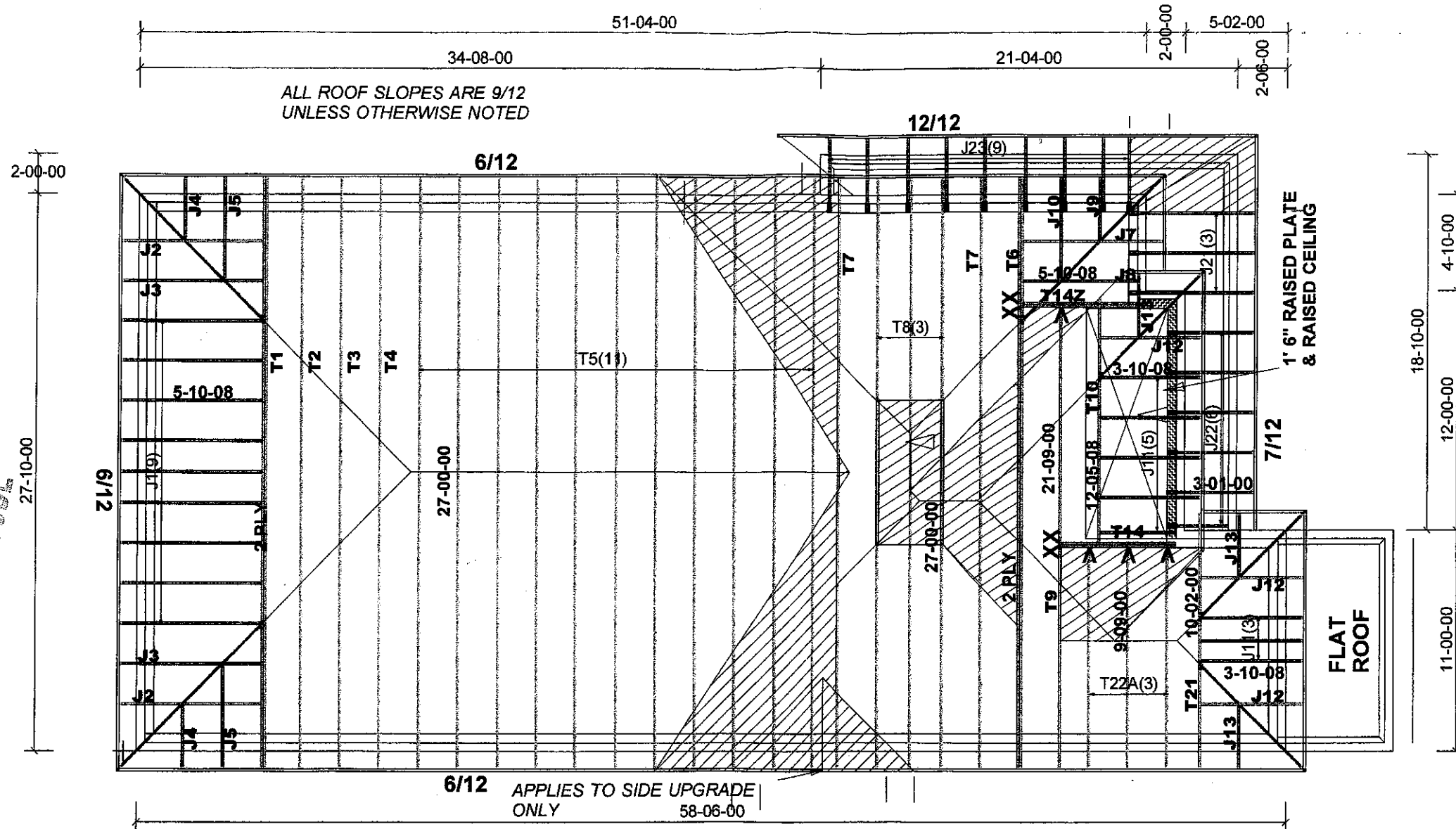


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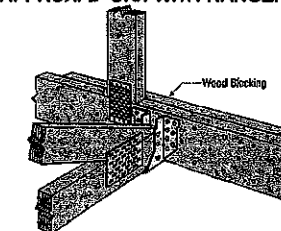
ASPHALT SHINGLES
12" FINISHED O.H.
5" CLADDING ALLOWANCE
2x6 EXTERIOR WALLS
2x6 FASCIA BOARD
HEEL: R.T.M.C.

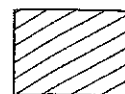
ALL CONV. FRAMING TO CONFORM
WITH PART 9 OF O.B.C. 2012 ROOF
RAFTERS THAT CROSS OVER OR
MEET TRUSSES TO BE 2X4 SPF #2 24"
O.C. WITH A VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS PT.
VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DIST. BETWEEN END PT.
& BETWEEN ROWS OF BRACING
DOES NOT EXCEED 6'

DESIGN CONFORMS WITH OBC 2012
OCCUPANCY: RESIDENTIAL | PART: 9
GROUND SNOW LOAD $S_s = 37.6$ psf $S_r = 8.4$ psf HGUS26-2 - (XX)
DESIGN LOADS
TCDL (6 psf)
BCLL (10.5 psf)
BCDL (7 psf)

HARDWARE:
LJS26DS - (V)
T22A(3)

HANGERS APPLIED TO A 1-PLY GIRDERS
REQUIRE A BLOCK SCABBED BEHIND.
APPROX. 2' O.C. WITH HANGER



 **DENOTES:**
CONVENTIONAL
FRAMING

M12268



Job Track: **50120**
Plan Log: **200142**
Layout ID: **400341**

Builder / Location:
Greenpark / CALEDON

Project: **Lamberts Lane Home Corp.**

Date: 4/30/2019

Sales: **Mario DiCano**

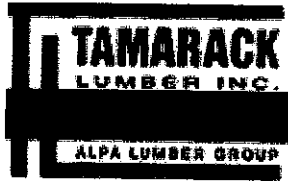
Designer: **Brian**

Model / Elevation:

Brentwood 3A/ 2 STD OR With Side Upgrade

Model ver 8.2.3.229

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: CALEDON
 Model: Brentwood 3A
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200142
 Layout ID: 400341
 Ref #
 Page: 2 of 3
 Date: 04/30/2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 98.00		
	2	J2 Jack-Open Girder	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 / 12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 / 12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 / 12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	1	J7 Jack-Open Girder	9 / 12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	13.87 9.67		
	1	J8 Jack-Open	9 / 12	5-10-08	4-04-05	2 x 4	1-03-08	1-06-04 5-11-02	17.07 11.50		
	1	J9 Jack-Open	9 / 12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	9.31 7.00		
	1	J10 Jack-Open	9 / 12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	12.22 8.33		
	8	J11 Jack-Open	9 / 12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	119.31 81.33		
	3	J12 Jack-Open Girder	9 / 12	1-09-07	2-10-05	2 x 4	1-03-08 2-01-01	1-06-04 2-10-05	34.78 25.00		
	3	J13 Jack-Open	8 / 12	1-09-07	2-07-02	2 x 4	1-03-08 1-01	1-04-13 2-07-02	26.71 17.00		
	3	J21 Jack-Open	7 / 12	5-01-08	4-01-00	2 x 4	1-03-08	4-05 3-03-14	43.53 28.00		
	6	J22 Jack-Open	7 / 12	3-01-08	2-11-00	2 x 4	1-03-08	4-05 2-01-14	57.53 40.00		

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DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: CALEDON
 Model: Brentwood 3A
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200142
 Layout ID: 400341
 Ref #
 Page: 1 of 3
 Date: 04/30/2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	6 /12	27-00-00	4-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	213.21 136.67		
	1	T2 Hip	6 /12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	109.31 68.83		
	1	T3 Hip	6 /12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	110.98 70.67		
	1	T4 Hip	6 /12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	112.6 70.67		
	11	T5 Common	6 /12	27-00-00	7-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1251.35 788.33		
	1 2-ply	T6 Hip Girder	9 /12	27-00-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	271.8 174.00		
	2	T7 Hip	9 /12	27-00-00	7-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	249.14 157.67		
	3	T8 Hip	9 /12	27-00-00	8-11-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	385.47 248.00		
	1	T9 Hip Girder	9 /12	21-09-00	7-05-02	2 x 4 2 x 6	1-03-08	1-06-04 3-04-00	118.06 74.33		
	1	T10 Half Hip Girder	9 /12	12-05-08	4-05-02	2 x 4 2 x 6		1-06-04 4-05-02	58.03 37.33		
	1 2-ply	T14 Flat Girder	0 /12	5-10-08	1-06-00	2 x 6		1-06-00 1-06-00	54.19 33.67		
	1 2-ply	T14Z Flat Girder	0 /12	5-10-08	1-06-00	2 x 4 2 x 6		1-06-00 1-06-00	47.73 29.67		
	1	T21 Hip Girder	9 /12	10-02-00	4-05-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	47.93 31.83		
	3	T22A Common	9 /12	9-09-00	5-04-00	2 x 4	1-03-08	1-06-04 1-10-00	124.71 82.50		

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TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

DELIVERY SHIPLIST		
 TAMARACK LUMBER INC. ALPA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: Greenpark Project: Lamberts Lane Home Corp. Location: CALEDON Model: Brentwood 3A Lot #: Elevation: 2	Job Track: 50120 PlanLog: 200142 Layout ID: 400341 Ref # Page: 3 of 3 Date: 04/30/2019 Designer: Brian Faneca Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J23 Jack-Open	12 /12	2-05-08	4-01-15	2 x 4	1-03-08	11-06 3-04-06	88.15 60.00		

TOTAL # TRUSS= 86

TOTAL BFT OF ALL TRUSSES= 2441.33

BFT.

TOTAL WEIGHT OF ALL TRSSES 3812.76 LBS

HARDWARE

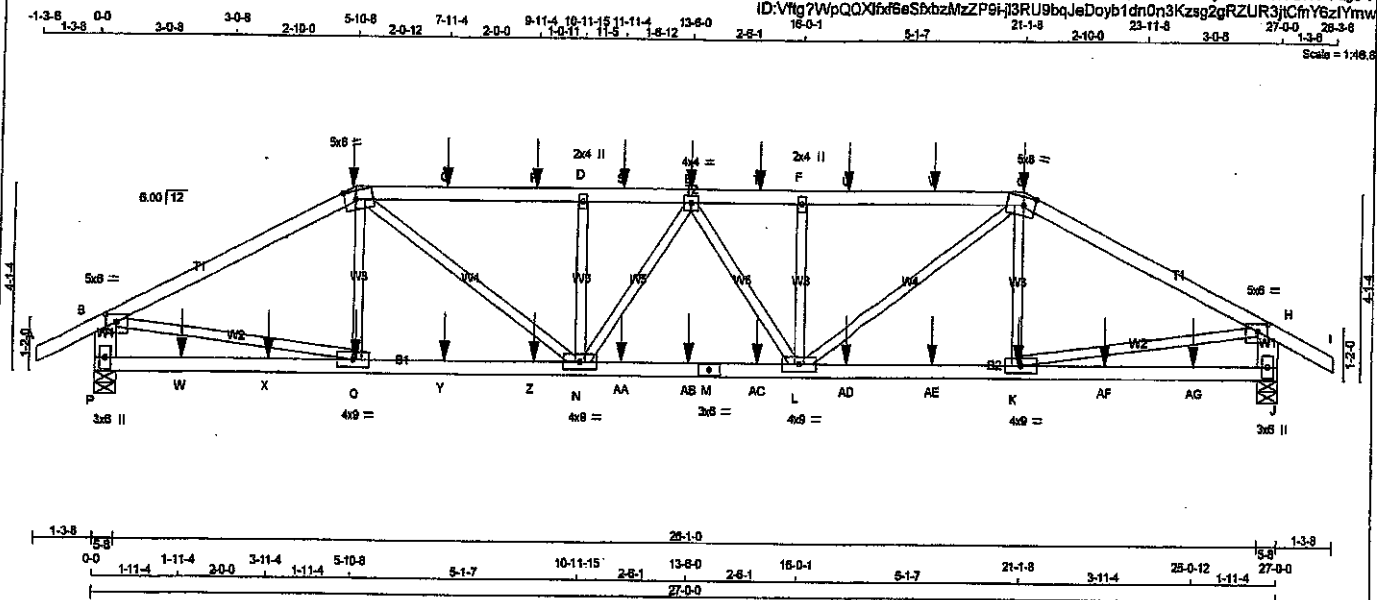
QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
4	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 6

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TOWN OF CALEDON
FILED 06 04 2019

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	T1	1	2	Brentwood 3A	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 107 = 213 lb

LUMBER	SIZE	DRY	No.2	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE(61.0)
C - G	12	SIDE(61.0)
G - I	12	SIDE(61.0)
P - B	12	TOP
J - H	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
P - M	12	SIDE(61.0)
M - J	12	SIDE(61.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-m	MT20	5.0	8.0	2.00 2.75
C	TTWV-m	MT20	5.0	8.0	2.25 3.25
D	TMWV-w	MT20	2.0	4.0	
E	TMWV-t	MT20	4.0	4.0	
F	TMWV-w	MT20	2.0	4.0	
G	TTWV-m	MT20	5.0	8.0	2.25 3.25
H	TMWV-p	MT20	5.0	8.0	2.00 2.75
J	BMV1+p	MT20	3.0	6.0	
K	BMWV-t	MT20	4.0	9.0	
L	BMWV-w	MT20	4.0	9.0	
M	BS-t	MT20	3.0	6.0	
N	BMWV-w	MT20	4.0	9.0	
O	BMWV-t	MT20	4.0	9.0	
P	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
P	3235	0	5-8	5-8
J	3235	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM	LIVE
P	2420	1328 / 0	489 / 0	0 / 0	603 / 0
J	2420	1328 / 0	489 / 0	0 / 0	603 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MEMB.	FORCE
(LBS)	(PLF)	CS1 (LC)	(LBS)	CS1 (LC)
FR-TO			FR-TO	
A-B	0 / 31	-102.1 -102.1 0.07 (1)	O-C	-177 / 254 0.03 (3)
B-C	-4607 / 0	-102.1 -102.1 0.53 (1)	C-N	0 / 2066 0.26 (1)
C-Q	-5772 / 0	-102.1 -102.1 0.47 (1)	N-D	-844 / 0 0.11 (1)
Q-R	-5772 / 0	-102.1 -102.1 0.47 (1)	L-F	-844 / 0 0.11 (1)
R-D	-5772 / 0	-102.1 -102.1 0.47 (1)	L-G	0 / 2066 0.26 (1)
D-S	-5772 / 0	-102.1 -102.1 0.39 (1)	K-G	-177 / 254 0.03 (3)
S-E	-5772 / 0	-102.1 -102.1 0.39 (1)	B-O	0 / 4160 0.51 (1)
E-T	-5772 / 0	-102.1 -102.1 0.39 (1)	K-H	0 / 4160 0.51 (1)
T-F	-5772 / 0	-102.1 -102.1 0.39 (1)	N-E	-78 / 0 0.01 (1)
F-U	-5772 / 0	-102.1 -102.1 0.47 (1)	E-L	-78 / 0 0.01 (1)
U-V	-5772 / 0	-102.1 -102.1 0.47 (1)		
V-G	-5772 / 0	-102.1 -102.1 0.47 (1)		
G-H	-4607 / 0	-102.1 -102.1 0.53 (1)		
H-I	0 / 31	-102.1 -102.1 0.07 (1)		
P-B	-3085 / 0	0.0 0.0 1.11 (1)		
J-H	-3085 / 0	0.0 0.0 1.11 (1)		

P-W	0 / 0	-38.5 -38.5 0.20 (3)	10.00
W-X	0 / 0	-38.5 -38.5 0.20 (3)	10.00
X-O	0 / 0	-38.5 -38.5 0.20 (3)	10.00
O-Y	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
Y-Z	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
Z-N	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
N-AA	0 / 5815	-38.5 -38.5 0.82 (1)	10.00
AA-AB	0 / 5815	-38.5 -38.5 0.82 (1)	10.00
AB-M	0 / 5815	-38.5 -38.5 0.82 (1)	10.00
M-AC	0 / 5815	-38.5 -38.5 0.82 (1)	10.00
AC-L	0 / 5815	-38.5 -38.5 0.82 (1)	10.00
L-AD	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
AD-AE	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
AE-K	0 / 4113	-38.5 -38.5 0.51 (2)	10.00
K-AG	0 / 0	-38.5 -38.5 0.20 (3)	10.00
AG-J	0 / 0	-38.5 -38.5 0.20 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	FACE	DIR	TYPE	HEEL	CONN.
C	5-10-8	-409	-409	FRONT	VERT	TOTAL	-	-
E	13-6-0	-123	-123	FRONT	VERT	TOTAL	-	-
G	21-1-8	-409	-409	FRONT	VERT	TOTAL	-	-

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/899 (0.15")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/999 (0.25")

CS1: TO=0.58/1.00 (B-C:1), BC=0.62/1.00 (L-N:1), WB=0.51/1.00 (B-O:1), BS=0.21/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP = 0.80 (H) (INPUT = 0.80)
SI METAL = 0.99 (M) (INPUT = 1.00)

DRWG NO. TAM 7410420
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

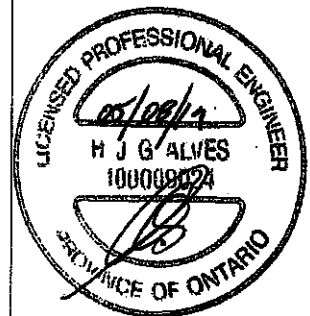
JOB NAME 200142-400307 Tamarack Roof Truss, Burlington	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. Brentwood 3A TRUSS DESC.	DRWG NO.
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Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Wed May 8 14:06:59 2019 Page 2
ID:Vftg?WpQOQXf6eSfxbzWzZP9LH3RU9bqJeDoyb1dn0n3Kzsq2gRZUR3HChY6zYmW

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	21-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
O	5-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Q	7-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
R	9-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
S	11-11-4	-123	-123	—	FRONT	VERT	TOTAL	—	—
T	15-0-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
U	17-0-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
V	19-0-12	-123	-123	—	FRONT	VERT	TOTAL	—	—
W	1-11-4	-55	-74	—	FRONT	VERT	TOTAL	—	—
X	3-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	7-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Z	9-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	11-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	13-0-0	-55	-70	—	FRONT	VERT	TOTAL	—	—
AC	15-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AD	17-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AE	19-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AF	23-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—
AG	25-0-12	-55	-74	—	FRONT	VERT	TOTAL	—	—

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BUILDING SECTION

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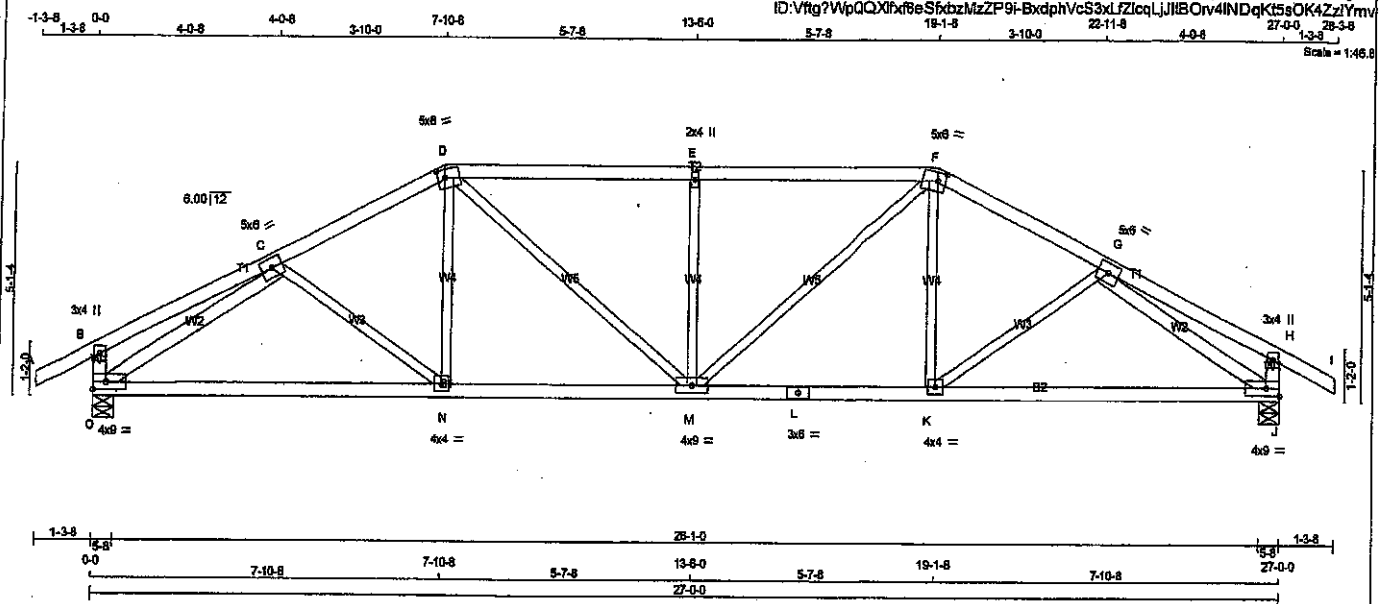
DWG NO. TAM 17710920
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	T2	1	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 14:07:00 2019 Page 1

ID:Vftg?WpQQXfR6eS6bzMzZP9i-BxdphVcS3xLzIcqLjIIBOrv4INDqKt5sOK4ZziYmv

Scale = 1:48.8



TOTAL WEIGHT = 109 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF

ALL WEBS	EXCEPT	SIZE	LUMBER	DESCR.
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVWV-1	MT20	5.0	6.0		
D	TTVWV-m	MT20	5.0	6.0	2.25	2.00
E	TMVWV-w	MT20	2.0	4.0		
F	TTVWV-m	MT20	5.0	6.0	2.25	2.00
G	TMVWV-1	MT20	5.0	6.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVWV-1	MT20	4.0	9.0		Edge
K	BMVWV-1	MT20	4.0	4.0		
L	BS-1	MT20	3.0	6.0		
M	BMVWV-1	MT20	4.0	9.0		
N	BMVWV-1	MT20	4.0	4.0		
O	BMVWV-1	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
O	2037	0	2037	0	0
J	2037	0	2037	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX.	WEBS	MAX. FACTORED	FORCE	MAX
MEMB.		(LBS)			(PLF)	(LC)	MEMB.		(LBS)	(LC)
FR-TO			FROM	TO			FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00		C-N	-3 / 138	0.03 (3)	
B-C	0 / 19	-102.1	-102.1	0.23 (1)	10.00		N-D	0 / 375	0.08 (2)	
C-D	-2533 / 0	-102.1	-102.1	0.25 (1)	4.13		D-M	0 / 647	0.15 (1)	
D-E	-2746 / 0	-102.1	-102.1	0.52 (1)	3.68		M-E	-702 / 0	0.27 (1)	
E-F	-2746 / 0	-102.1	-102.1	0.25 (1)	4.13		K-F	0 / 375	0.08 (2)	
F-G	-2533 / 0	-102.1	-102.1	0.23 (1)	10.00		K-G	-3 / 138	0.03 (3)	
G-H	0 / 19	-102.1	-102.1	0.13 (1)	10.00		O-C	-2768 / 0	0.77 (1)	
H-I	0 / 31	-102.1	-102.1	0.13 (1)	10.00		G-J	-2768 / 0	0.77 (1)	
O-B	-299 / 0	0.0	0.0	0.03 (1)	7.81					
J-H	-299 / 0	0.0	0.0	0.03 (1)	7.81					
O-N	0 / 2250	-38.5	-38.5	0.70 (2)	10.00					
N-M	0 / 2254	-38.5	-38.5	0.71 (2)	10.00					
M-L	0 / 2254	-38.5	-38.5	0.71 (2)	10.00					
L-K	0 / 2250	-38.5	-38.5	0.71 (2)	10.00					
K-J	0 / 2250	-38.5	-38.5	0.70 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CSI: TC=0.52/1.00 (D-E-1), BC=0.71/1.00 (K-M-2), WB=0.77/1.00 (G-J-1), SS=0.28/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 819 354 1867 788 1987 1858

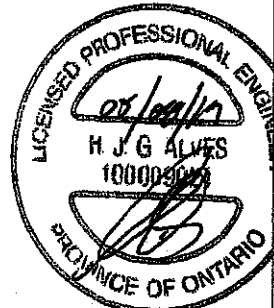
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JG GRIP=0.88 (G) (INPUT = 0.80)
JG METAL=0.73 (L) (INPUT = 1.00)

DRWG NO. TAM 1410921
STRUCTURAL
COMPONENT ONLY

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JUN 11 2019
TOWN OF BURLINGTON
BUILDING DEPARTMENT
FILE NO. 200142-400307

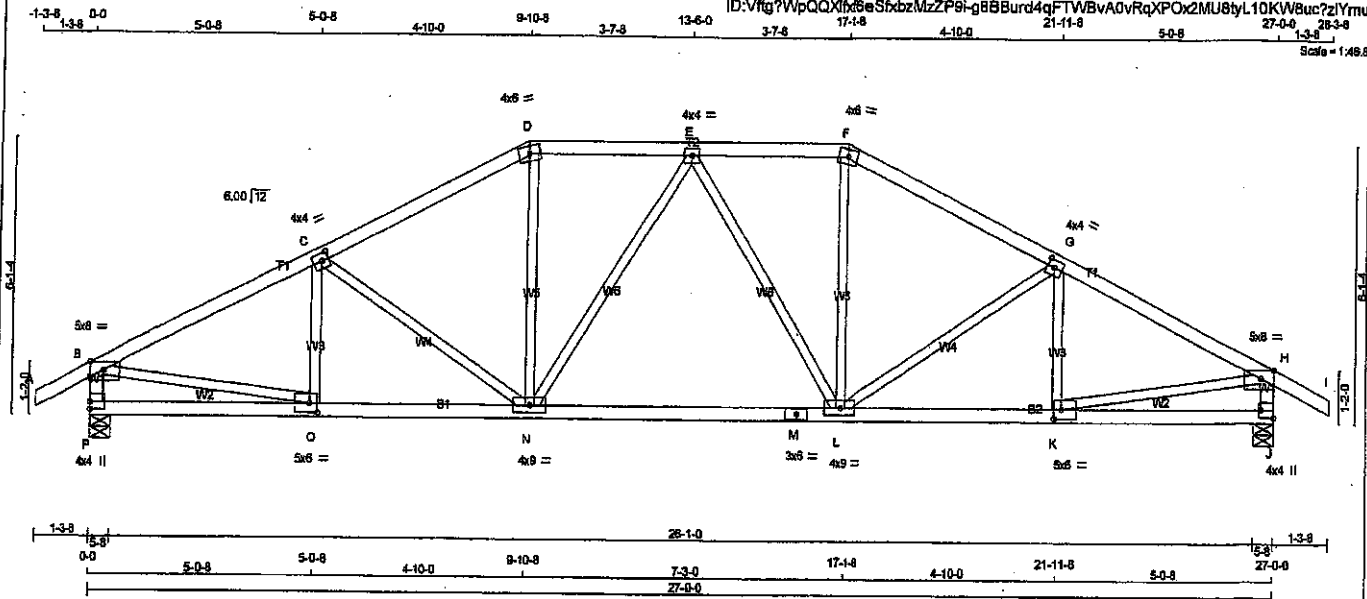


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	T3	1	1	Brentwood 3A	

Tamersck Roof Truss, Burlington

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ID:VfRg?WpQQXIMeSfcbzMzZP9h-g88Burd4qFTWBvA0vRqXPOx2MU8tL10KW8uc?zIYmu



TOTAL WEIGHT = 111 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	Edge	3.50
C TMVW-i	MT20	4.0	4.0	2.00	1.75
D TMVW-m	MT20	4.0	6.0		
E TMVW-k	MT20	4.0	4.0		
F TMVW-m	MT20	4.0	6.0		
G TMVW-k	MT20	4.0	4.0	2.00	1.75
H TMVW-p	MT20	5.0	8.0	Edge	3.50
J BMV1-p	MT20	4.0	4.0	2.00	Edge
K BMVW-k	MT20	5.0	6.0	2.50	2.00
L BMVWV-i	MT20	4.0	9.0		
M BS-i	MT20	3.0	6.0		
N BMVWV-i	MT20	4.0	9.0		
O BMVW-k	MT20	5.0	6.0	2.50	2.00
P BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
P	2037	0	2037	0	5-8	5-8
J	2037	0	2037	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
P	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0 / 31	-102.1 -102.1	0.13 (1)	10.00	C-C	-221 / 64	0.05 (1)
B-C	-2863 / 0	-102.1 -102.1	0.41 (1)	3.88	C-N	-410 / 0	0.26 (1)
C-D	-2337 / 0	-102.1 -102.1	0.38 (1)	4.14	N-D	0 / 698	0.16 (1)
D-E	-2078 / 0	-102.1 -102.1	0.20 (1)	4.53	N-E	-256 / 0	0.22 (1)
E-F	-2078 / 0	-102.1 -102.1	0.20 (1)	4.53	E-L	-256 / 0	0.22 (1)
F-G	-2337 / 0	-102.1 -102.1	0.38 (1)	4.14	L-F	0 / 698	0.16 (1)
G-H	-2863 / 0	-102.1 -102.1	0.41 (1)	3.88	L-G	-410 / 0	0.26 (1)
H-I	0 / 31	-102.1 -102.1	0.13 (1)	10.00	K-G	-221 / 64	0.05 (1)
P-B	-1951 / 0	0.0 0.0	0.20 (1)	6.03	B-O	0 / 2438	0.55 (1)
J-H	-1951 / 0	0.0 0.0	0.20 (1)	6.03	K-H	0 / 2438	0.55 (1)
P-O	0 / 0	-38.5 -38.5	0.16 (3)	10.00			
O-N	0 / 2404	-38.5 -38.5	0.56 (1)	10.00			
N-M	0 / 2211	-38.5 -38.5	0.53 (2)	10.00			
M-L	0 / 2211	-38.5 -38.5	0.53 (2)	10.00			
L-K	0 / 2404	-38.5 -38.5	0.56 (1)	10.00			
K-J	0 / 0	-38.5 -38.5	0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/969 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/969 (0.23")
CSI: TC=0.41/1.00 (G-H-1), BC=0.56/1.00 (N-O-1)
WB=0.55/1.00 (B-O-1), SS=0.22/1.00 (G-H-1)

DO NOT LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1686

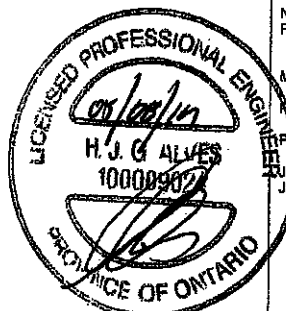
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP=0.79 (K) (INPUT = 0.90)
METAL=0.85 (M) (INPUT = 1.00)

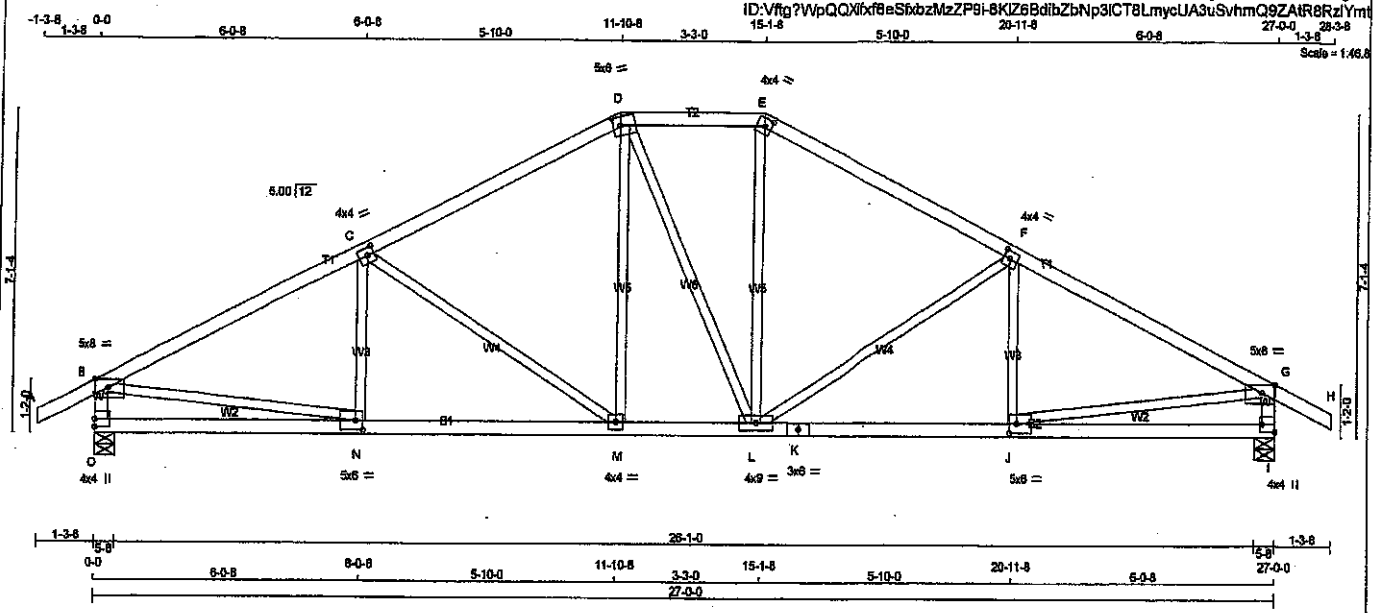
DRWG NO. TAM 77910922
STRUCTURAL
COMPONENT ONLY

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JUN 11 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 200142-400307	TRUSS NAME T4	QUANTITY 1	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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TOTAL WEIGHT = 113 lb
[MJP]

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
O - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
O - K	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	5.0	8.0	Edge 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTWV-m	MT20	5.0	6.0	2.25 2.00
E	TTWV-h	MT20	4.0	4.0	2.00 1.75
F	TMVW-h	MT20	4.0	4.0	2.00 1.75
G	TMVW-p	MT20	5.0	8.0	Edge 3.50
I	BMV1-p	MT20	4.0	4.0	2.00 Edge
J	BMVW-h	MT20	5.0	8.0	2.50 2.00
K	BS-t	MT20	3.0	6.0	
L	BMVW-t	MT20	4.0	9.0	
M	BMVW-t	MT20	4.0	4.0	
N	BMVW-h	MT20	5.0	6.0	2.50 2.00
O	BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	O	VERT	HORZ	GROSS REACTION	GROSS REACTION	BRG	IN-SX	BRG	IN-SX
O		2037	0	2037	0	0	5-8	5-8	5-8
I		2037	0	2037	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX. MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	O	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL	SOIL
O		1514	863 / 0	284 / 0	0 / 0	0 / 0	387 / 0	0 / 0	387 / 0	0 / 0	0 / 0
I		1514	863 / 0	284 / 0	0 / 0	0 / 0	387 / 0	0 / 0	387 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0 / 31	-102.1	-102.1 0.13 (1)	N-C	-84 / 185	0.04 (3)	10.00
B-C	-2894 / 0	-102.1	-102.1 0.61 (1)	C-M	-682 / 0	0.67 (1)	3.63
C-D	-2111 / 0	-102.1	-102.1 0.53 (1)	M-D	0 / 537	0.12 (1)	4.11
D-E	-1868 / 0	-102.1	-102.1 0.18 (1)	D-L	0 / 4	0.00 (1)	4.76
E-F	-2113 / 0	-102.1	-102.1 0.53 (1)	L-E	0 / 541	0.12 (1)	4.11
F-G	-2693 / 0	-102.1	-102.1 0.61 (1)	L-F	-689 / 0	0.66 (1)	3.63
G-H	0 / 31	-102.1	-102.1 0.13 (1)	J-F	-86 / 183	0.04 (3)	10.00
O-B	-1942 / 0	0.0	0.0 0.20 (1)	B-N	0 / 2462	0.55 (1)	8.04
I-G	-1942 / 0	0.0	0.0 0.20 (1)	J-G	0 / 2461	0.55 (1)	6.04
O-N	0 / 0	-38.5	-38.5 0.27 (3)				10.00
N-M	0 / 2438	-38.5	-38.5 0.58 (2)				10.00
M-L	0 / 1866	-38.5	-38.5 0.40 (1)				10.00
L-K	0 / 2437	-38.5	-38.5 0.57 (2)				10.00
K-J	0 / 2437	-38.5	-38.5 0.57 (2)				10.00
J-I	0 / 0	-38.5	-38.5 0.27 (3)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.90")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.12")
ALLOWABLE DEFL. (TL) = $L/360$ (0.90")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.20")

CS1: TC=0.61/1.00 (B-C-1), BC=0.58/1.00 (M-N-2), WB=0.87/1.00 (C-M-1), SS=0.29/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

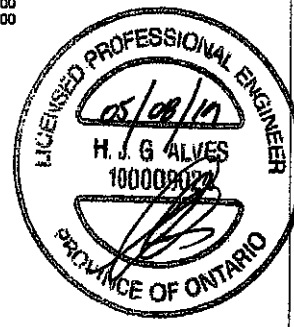
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
	MAX	MIN	MAX MIN
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.84 (E) (INPUT = 0.90)
SI METAL= 0.71 (K) (INPUT = 1.00)



DWG NO. TAM **T1910923**
STRUCTURAL
COMPONENT ONLY

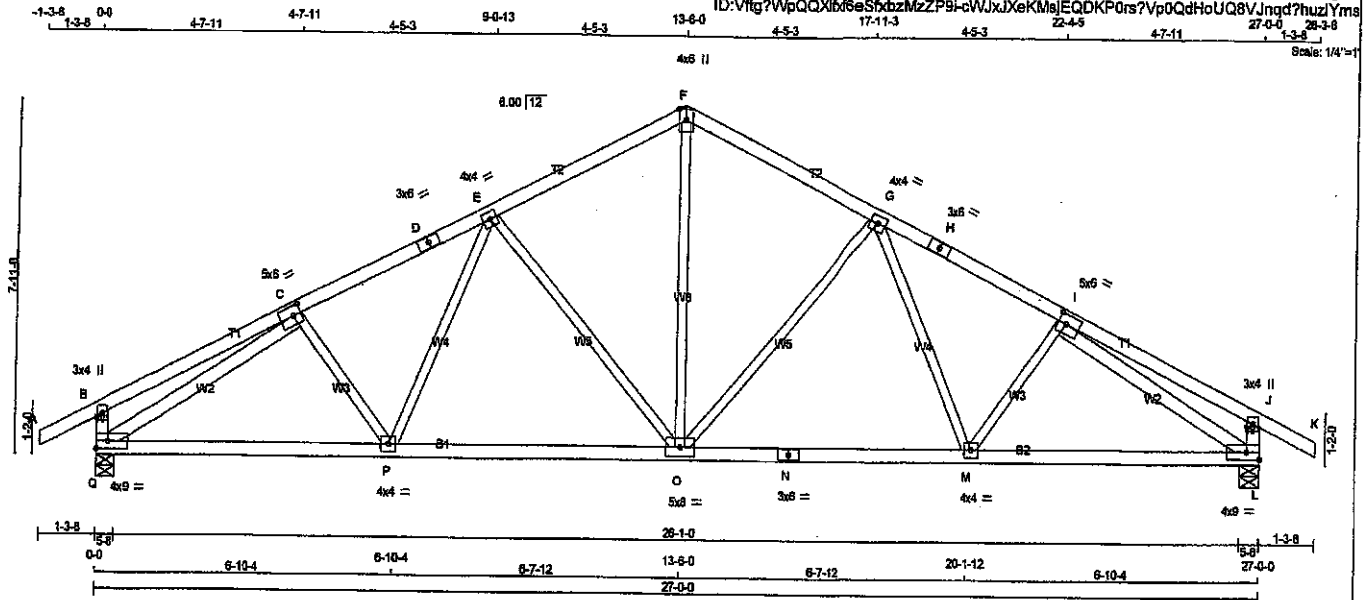
RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	T5	11	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

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ID: Vftg?WpQQXim6eShbzMzP9i-cWJxJXeKMajEQDKP0rs?Vp0QdHoUQ8VJnqd?huzlYms

Scale: 1/4"=1'



TOTAL WEIGHT = 11 X 114 = 1251 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.50	2.25
D	TS-1	MT20	3.0	6.0		
E	TMVW-1	MT20	4.0	4.0		
F	TTW+p	MT20	4.0	6.0	Edge	
G	TMVW-1	MT20	4.0	4.0		
H	TS-1	MT20	3.0	6.0		
I	TMVW-1	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW-1	MT20	4.0	9.0	Edge	
M	BMVW-1	MT20	4.0	4.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-1	MT20	5.0	8.0		
P	BMVW-1	MT20	4.0	4.0		
Q	BMVW-1	MT20	4.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRO	BRO	IN-SX	IN-SX
Q	2037	0	2037	0	0	5-8	5-8		
L	2037	0	2037	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
Q	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	Q	1514	863 / 0	284 / 0	0 / 0	0 / 0
L	1514	863 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0	L	1514	863 / 0	284 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	PLF	CSI (LC)	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO						FR-TO			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	O-F	0 / 1325	0.30 (1)	
B-C	0 / 17	-102.1	-102.1	0.25 (1)	10.00	O-G	-706 / 0	0.86 (1)	
C-D	-2581 / 0	-102.1	-102.1	0.30 (1)	4.08	G-M	0 / 380	0.09 (2)	
D-E	-2581 / 0	-102.1	-102.1	0.30 (1)	4.08	M-I	-49 / 112	0.03 (3)	
E-F	-1921 / 0	-102.1	-102.1	0.28 (1)	4.80	E-O	-708 / 0	0.66 (1)	
F-G	-1921 / 0	-102.1	-102.1	0.28 (1)	4.80	P-E	0 / 380	0.09 (2)	
G-H	-2581 / 0	-102.1	-102.1	0.30 (1)	4.08	C-P	-49 / 112	0.03 (3)	
H-I	-2581 / 0	-102.1	-102.1	0.30 (1)	4.08	Q-C	-2819 / 0	1.00 (1)	
I-J	0 / 17	-102.1	-102.1	0.25 (1)	10.00	I-L	-2819 / 0	1.00 (1)	
J-K	0 / 31	-102.1	-102.1	0.13 (1)	10.00				
Q-B	-331 / 0	0.0	0.0	0.03 (1)	7.81				
L-J	-331 / 0	0.0	0.0	0.03 (1)	7.81				
Q-P	0 / 2320	-38.5	-38.5	0.82 (2)	10.00				
P-O	0 / 2158	-38.5	-38.5	0.80 (2)	10.00				
O-N	0 / 2158	-38.5	-38.5	0.80 (2)	10.00				
N-M	0 / 2158	-38.5	-38.5	0.80 (2)	10.00				
M-L	0 / 2320	-38.5	-38.5	0.82 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/989 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/989 (0.18")

CSI: TO=0.30/1.00 (G-I-1), BC=0.62/1.00 (L-M-2), WB=1.00/1.00 (J-L-1), SSI=0.19/1.00 (P-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

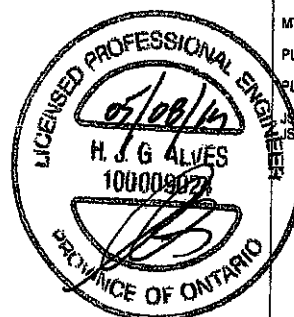
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

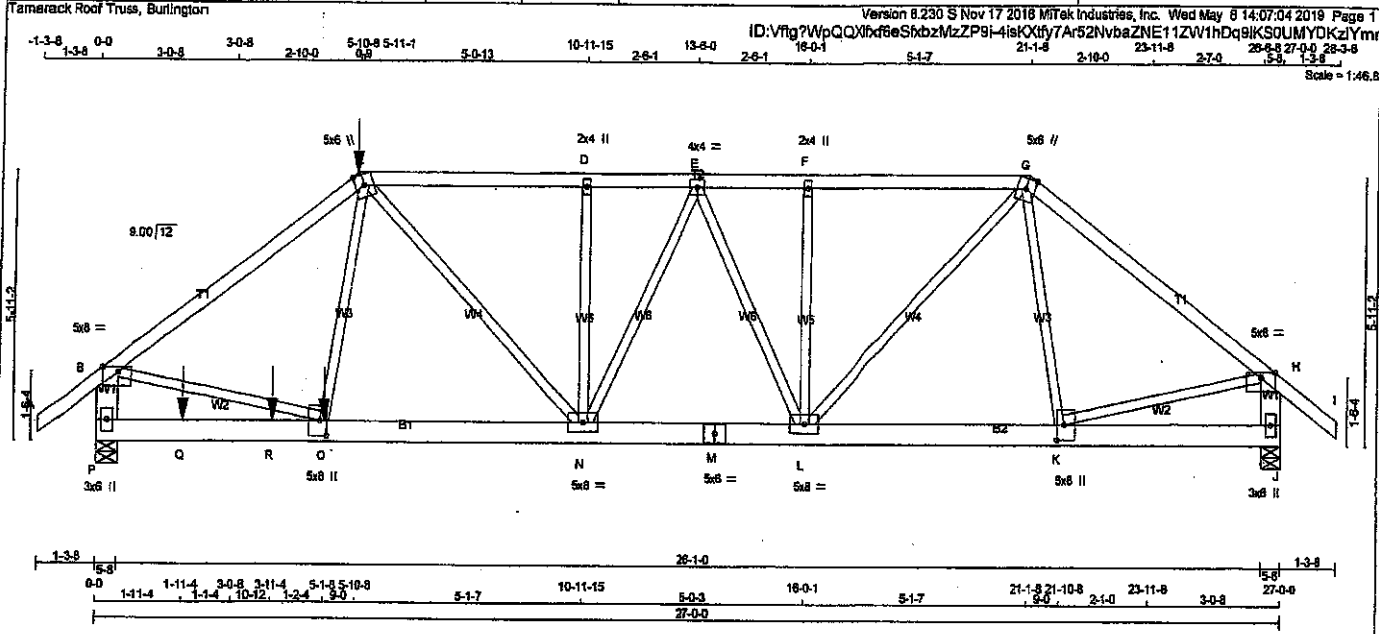
JT GRIP= 0.86 (C) (INPUT = 0.90)
JUS METAL= 0.77 (N) (INPUT = 1.00)

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 12910924
STRUCTURAL
COMPONENT ONLY

JOB NAME 200142-400307	TRUSS NAME T6	QUANTITY 1	PLY 2	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
P - B	2x6	DRY	No.2
J - H	2x6	DRY	No.2
P - M	2x6	DRY	No.2
M - J	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A-C	1	12	SIDE(81.0)
C-G	1	12	SIDE(81.0)
G-I	1	12	TOP
P-B	2	12	TOP
J-H	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
P-M	2	12	SIDE(183.1)
M-J	2	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS			
O-C	1	6	SIDE(441.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWW+m	MT20	5.0	8.0	Edge
D	TMWV-w	MT20	2.0	4.0	
E	TMWV-t	MT20	4.0	4.0	
F	TMWV-w	MT20	2.0	4.0	
G	TTWW+m	MT20	5.0	8.0	Edge
H	TMWV-p	MT20	5.0	8.0	Edge
J	BMV1+p	MT20	3.0	6.0	
K	BMWVW-t	MT20	5.0	8.0	4.00 2.00
L	BMWVW-t	MT20	5.0	8.0	
M	BS-t	MT20	5.0	8.0	
N	BMWVW-t	MT20	5.0	8.0	
O	BMWVW-t	MT20	5.0	8.0	4.00 2.00
P	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT REQD BRG
P	4521 0	4521 0	5-8
J	2625 0	2625 0	5-8

UNFACTORED REACTIONS			
JT	1ST LOOSE COMBINED	MAX/MIN. LIVE PERM. LIVE	DEAD WIND SOIL
P	3355 1925 / 0	619 / 0	0 / 0 811 / 0
J	1945 1124 / 0	354 / 0	0 / 0 468 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. MAX. UNBRACED LENGTH FR-TO
A-B	0 / 42	-102.1 -102.1	0.08 (1) 10.00
B-C	-5068 / 0	-102.1 -102.1	0.84 (1) 3.81
C-D	-3970 / 0	-102.1 -102.1	0.25 (1) 4.53
D-E	-3970 / 0	-102.1 -102.1	0.22 (1) 4.54
E-F	-3435 / 0	-102.1 -102.1	0.21 (1) 4.83
F-G	-3435 / 0	-102.1 -102.1	0.23 (1) 4.83
G-H	-2719 / 0	-102.1 -102.1	0.44 (1) 5.05
H-I	0 / 42	-102.1 -102.1	0.08 (1) 10.00
P-B	-4362 / 0	0.0 0.0	0.15 (1) 6.87
J-H	-2548 / 0	0.0 0.0	0.09 (1) 7.81
P-Q	0 / 0	-38.5 -38.5	0.10 (2) 10.00
Q-R	0 / 0	-38.5 -38.5	0.10 (2) 10.00
R-O	0 / 0	-38.5 -38.5	0.10 (2) 10.00
O-N	0 / 3794	-38.5 -38.5	0.29 (1) 10.00
N-M	0 / 3716	-38.5 -38.5	0.27 (1) 10.00
M-L	0 / 3716	-38.5 -38.5	0.27 (1) 10.00
L-K	0 / 2208	-38.5 -38.5	0.18 (1) 10.00
K-J	0 / 0	-38.5 -38.5	0.05 (3) 10.00

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1 MAX-	FACE DIR.
C	5-11-1	-522	BACK VERT
O	5-1-8	-2396	BACK VERT
Q	1-11-4	-75	BACK VERT
R	3-11-4	-75	BACK VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/969 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")
CSI: TC=0.64/1.00 (B-C:1), BC=0.29/1.00 (N-O:1),
WB=0.51/1.00 (B-C:1), SS=0.13/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	618	354	1687	788	1987

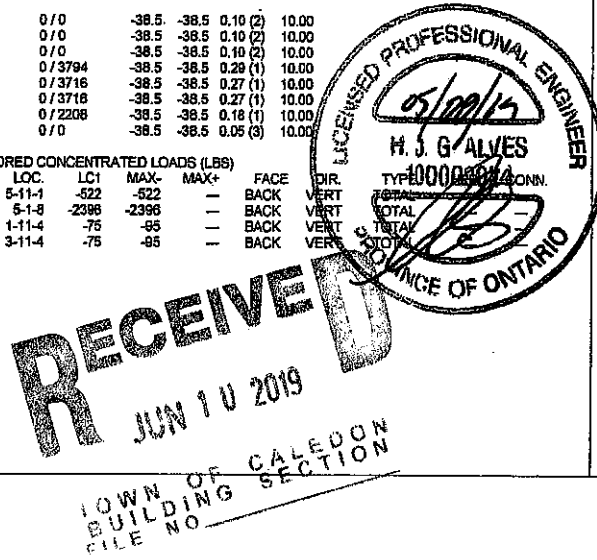
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.90 (K) (INPUT = 0.90)

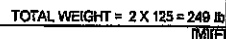
JSI METAL= 0.68 (C) (INPUT = 1.00)

DWG NO. TAM 17910925
STRUCTURAL
COMPONENT ONLY



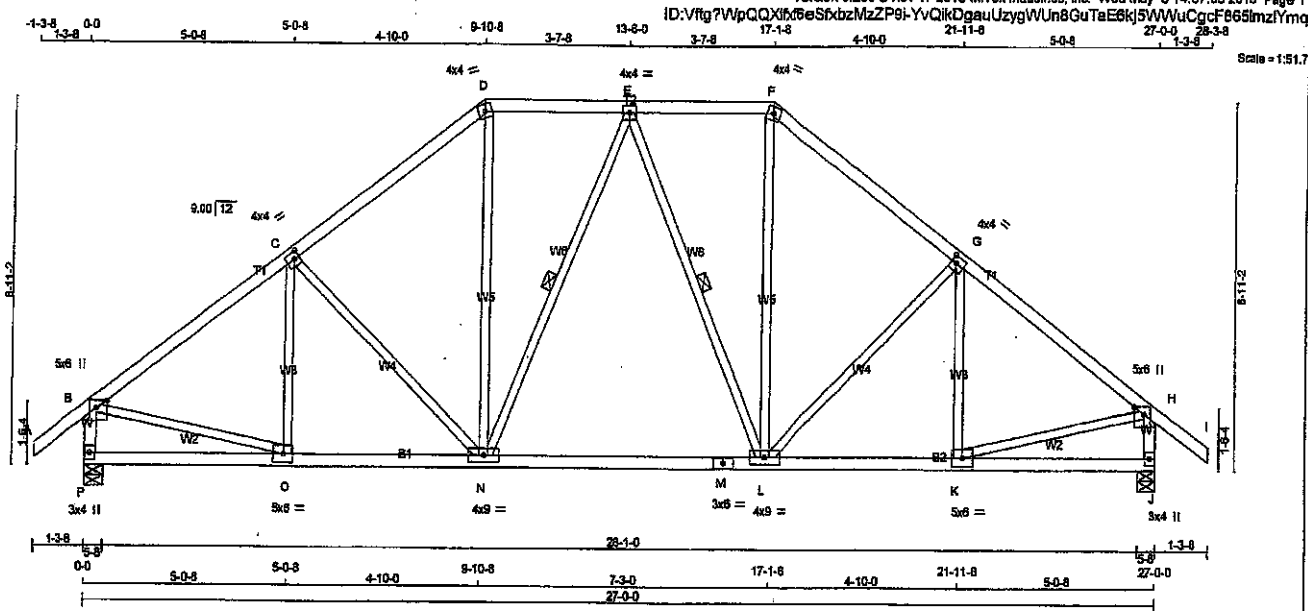
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 14:07:04 2018 Page 1
ID:Vrtg?WpQQXfYf6eSfxbz MzZP9J4lsKXffv7Ar52NvbaZNE11ZYgh8r9d2SQUIMYDKzYr



DWG NO. TAM *7/11/0926*
STRUCTURAL
COMMITMENT ONLY

JOB NAME 200142-400307	TRUSS NAME T8	QUANTITY 3	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 3 X 128 = 386 lb

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
P - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

SPF

DRY: SEASONED LUMBER.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	5.0	Edge	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW+p	MT20	5.0	5.0	Edge	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	5.0		
L	BMVW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	9.0		
O	BMVW-t	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	REQD
P	2039	0	2039	0	0	5-8	5-8
J	2039	0	2039	0	0	5-8	5-8

JT	COMBINED	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
P	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO		FROM-TO	CSI (L/C)		FR-TO		CSI (L/C)	
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	O-C	-194 / 76	0.08 (1)
B-C	-2013 / 0	-102.1	-102.1	0.39 (1)	4.40	C-N	-351 / 0	0.33 (1)
C-D	-1778 / 0	-102.1	-102.1	0.39 (1)	4.64	N-D	0 / 710	0.16 (1)
D-E	-1399 / 0	-102.1	-102.1	0.17 (1)	5.33	L-F	0 / 710	0.16 (1)
E-F	-1399 / 0	-102.1	-102.1	0.17 (1)	5.33	L-G	-351 / 0	0.33 (1)
F-G	-1778 / 0	-102.1	-102.1	0.39 (1)	4.64	K-G	-194 / 76	0.08 (1)
G-H	-2013 / 0	-102.1	-102.1	0.39 (1)	4.40	B-O	0 / 1884	0.36 (1)
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	K-H	0 / 1884	0.36 (1)
P-B	-1956 / 0	0.0	0.0	0.20 (1)	6.02	N-E	-235 / 0	0.14 (1)
J-H	-1956 / 0	0.0	0.0	0.20 (1)	6.02	E-L	-235 / 0	0.14 (1)
P-O	0 / 0	-38.5	-38.5	0.16 (3)	10.00			
O-N	0 / 1838	-38.5	-38.5	0.45 (2)	10.00			
N-M	0 / 1488	-38.5	-38.5	0.43 (2)	10.00			
M-L	0 / 1488	-38.5	-38.5	0.43 (2)	10.00			
L-K	0 / 1638	-38.5	-38.5	0.45 (2)	10.00			
K-J	0 / 0	-38.5	-38.5	0.16 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/380 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.39/1.00 (G-H-1), BC=0.45/1.00 (K-L-2), WB=0.38/1.00 (H-K-1), SS=0.20/1.00 (G-H-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 798 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

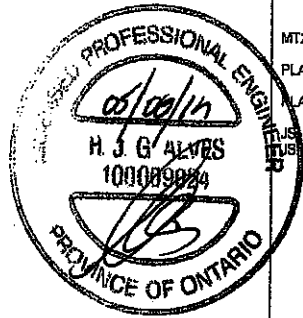
PLATE ROTATION TOL. = 5.0 Deg.

US GRIP=0.90 (B) (INPUT=0.90)
US METAL=0.45 (M) (INPUT=1.00)

DRWG NO. TAM 87910987
STRUCTURAL
COMPONENT ONLY

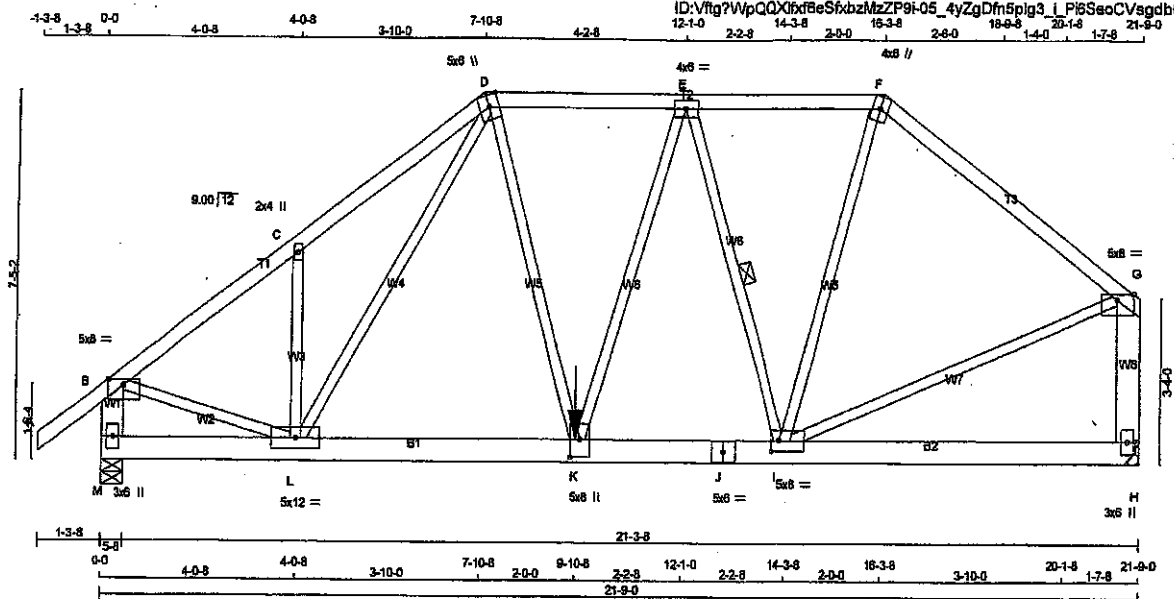
RECEIVED
JUN 10 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

JOB NAME: 200142-400307 TRUSS NAME: T9 QUANTITY: 1 PLY: 1 JOB DESC: Brentwood 3A DRWG NO. T9
Tamarack Roof Truss, Burlington



TOTAL WEIGHT = 118 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
M - B	2x6	DRY	No.2
H - G	2x6	DRY	No.2
M - J	2x6	DRY	No.2
J - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TMVW-m	MT20	2.0	4.0		
D	TMVW-m	MT20	5.0	6.0	Edge	
E	TMVW-t	MT20	4.0	6.0		
F	TMVW-m	MT20	4.0	6.0		
G	TMVW-p	MT20	5.0	6.0	Edge	
H	BMV1-p	MT20	3.0	6.0		
I	BMVW-t	MT20	5.0	8.0	2.50	2.00
J	BS-1	MT20	5.0	6.0		
K	BMVW-t	MT20	5.0	8.0	4.25	2.50
L	BMVW-t	MT20	5.0	12.0		
M	BMV1-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
M	2779	0	2779	0
H	2482	0	2482	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

	1ST CASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM LIVE
M	2082	1188 / 0	378 / 0	0 / 0
H	1835	1031 / 0	354 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED	MAX	MEMB.	MAX. FACTORED	FACTORED	MAX
	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CSI (LC)		FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.15 (1)	I-G	0 / 1942	0.48 (1)	
B-C	-2918 / 0	-102.1 -102.1	0.43 (1)	D-K	0 / 1442	0.36 (1)	
C-D	-2958 / 0	-102.1 -102.1	0.44 (1)	K-E	0 / 834	0.21 (1)	
D-E	-2715 / 0	-102.1 -102.1	0.43 (1)	E-I	-1377 / 0	0.52 (1)	
E-F	-2071 / 0	-102.1 -102.1	0.38 (1)	I-F	0 / 848	0.23 (1)	
F-G	-2263 / 0	-102.1 -102.1	0.85 (1)	L-C	-483 / 0	0.14 (1)	
M-B	-2712 / 0	0.0 0.0	0.20 (1)	L-D	0 / 89	0.22 (3)	
H-G	-2343 / 0	0.0 0.0	0.29 (1)	B-L	0 / 242	0.80 (1)	
M-L	0 / 0	-38.5 -38.5	0.09 (3)				
L-K	0 / 2319	-38.5 -38.5	0.41 (1)				
K-J	0 / 2472	-38.5 -38.5	0.48 (1)				
J-I	0 / 2472	-38.5 -38.5	0.46 (1)				
I-H	0 / 0	-38.5 -38.5	0.19 (3)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC	MAX	MAX+	FACE	DR
				FRONT	VERT
		2082	2042		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.1	PSF
DL	=	6.0	PSF	
BOT CH.	LL	=	10.5	PSF
DL	=	7.0	PSF	
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/899 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/899 (0.13")

CSI: TC=0.85/1.00 (F-G-1), BC=0.48/1.00 (I-K-1), WB=0.60/1.00 (B-L-1), SS=0.22/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX	MIN	MAX
618	354	1697

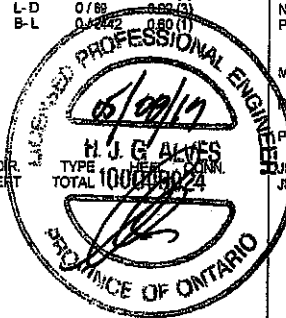
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

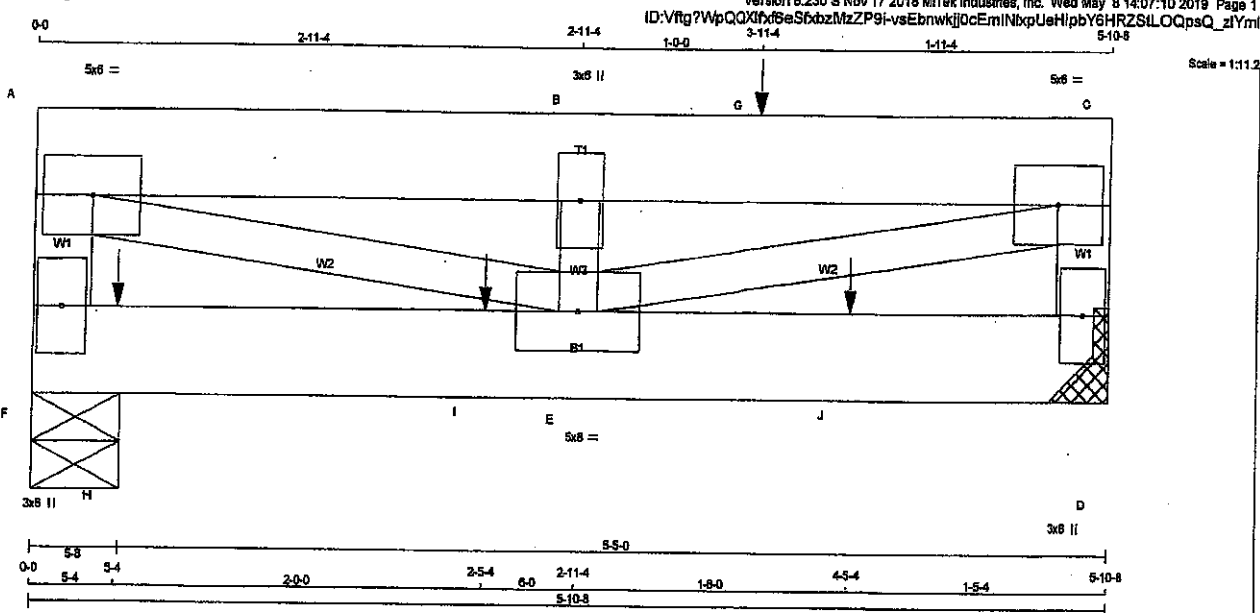
GRIP=0.88 (I) (INPUT=0.90)
METAL=0.52 (B) (INPUT=1.00)

DRWG NO. TAM 17910928
STRUCTURAL
CONFORM ONLY

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 200142-400307	TRUSS NAME T14	QUANTITY 1	PLY 2	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4 DRY	No.2	SPF		
A - C	2x6 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x6 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F - A 1	12	TOP
C - D 1	12	TOP
A - C 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F - D 2	11	SIDE (163.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW4	MT20	5.0	6.0		
B	TMVW4	MT20	3.0	6.0		
C	TMVW4	MT20	5.0	6.0		
D	BMV1+p	MT20	3.0	6.0		
E	BMVW4	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	1982	0	0	5-8
D	2081	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4'-0".

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	1473	840 / 0	275 / 0	0 / 0	358 / 0	0 / 0
D	1548	873 / 0	297 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.88 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LC1)	MAX. VERT. LOAD (LC2)	MAX. VERT. LOAD (LC3)	WEBS	MAX. FACTORED MEMB. FORCE (LBS)	MAX. VERT. LOAD (LC1)	MAX. VERT. LOAD (LC2)	MAX. VERT. LOAD (LC3)
FR-TO						FR-TO				
F-A	-1210 / 0	0.0	0.0	0.07 (1)	7.81	A-E	0 / 3085	0.38 (1)	0.08 (1)	0.08 (1)
A-B	-2908 / 0	-102.1	-102.1	0.07 (1)	6.25	E-B	-1088 / 0	0.08 (1)	0.08 (1)	0.08 (1)
B-G	-2908 / 0	-102.1	-102.1	0.38 (1)	5.86	E-C	0 / 3085	0.38 (1)	0.08 (1)	0.08 (1)
G-C	-2908 / 0	-102.1	-102.1	0.38 (1)	5.86					
D-C	-1648 / 0	0.0	0.0	0.09 (1)	7.81					

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	3-11.4	-1285	-1285	---	TOP	VERT	TOTAL	---	---
H	5-4	-658	-658	---	BACK	VERT	TOTAL	---	---
I	2-5.4	-647	-647	---	BACK	VERT	TOTAL	---	---
J	4-5.4	-647	-647	---	BACK	VERT	TOTAL	---	---

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	3-11.4	-1285	-1285	---	TOP	VERT	TOTAL	---	---
H	5-4	-658	-658	---	BACK	VERT	TOTAL	---	---
I	2-5.4	-647	-647	---	BACK	VERT	TOTAL	---	---
J	4-5.4	-647	-647	---	BACK	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/900 (0.02")
CALCULATED VERT. DEFL. (LL) = 1/999 (0.02")
ALLOWABLE DEFL. (TL) = L/900 (0.02")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.04")

CSL TC=0.38/1.00 (B-C-1), BC=0.21/1.00 (D-E-1), WS=0.38/1.00 (A-E-1), SSI=0.32/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354	1667

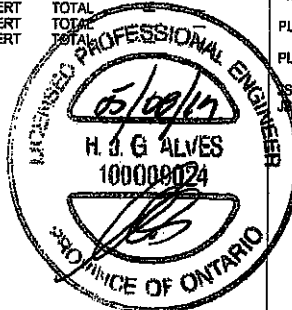
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.79 (E) (INPUT = 0.90)
METAL = 0.36 (A) (INPUT = 1.00)

RECEIVED
JUN 10 2019

TOWN OF CALADON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 77910933
STRUCTURAL
CONSULTANT ONLY

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 14:07:11 2019 Page 1
ID:VfvgWpQQX5f6eSfbzMzZP9lN3oz?GkLUKk5ORxxUX?bvLlRWXkSud3ZQzQzYmk



TOTAL WEIGHT = $2 \times 27 = 54$ lb

ALL WEBS 2x3 DRY No.2 SPF
 DRY: SEASONED LUMBER.

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A 1	12	TOP
C-D 1	12	TOP
A-C 2	12	TOP

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0	2.50	2.75
B	TMVW+w	MT20	3.0	6.0		
C	TMVW-t	MT20	5.0	6.0	2.50	2.75
D	BMV1+p	MT20	3.0	6.0		
E	BMVWW-t	MT20	6.0	9.0		
F	BMV1+p	MT20	3.0	6.0		

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX
F	2020	0	2020	0	0	5-8
D	2434	0	2434	0	0	MECHANICAL

UNFACTORED REACTIONS							
	1ST CASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1507	840 / 0	285 / 0	0 / 0	0 / 0	373 / 0	0 / 0
D	1812	1023 / 0	348 / 0	0 / 0	0 / 0	443 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY
APPLIED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBB			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX. CSI (L)	LC1 MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (L)
FR-TO		FROM TO		LENGTH	FR-TO		
F-A	-1966/0	0.0	0.0	1.11 (1)	7.81	A-E	0/3947
A-G	-3720/0	-102.1	-102.1	0.41 (1)	5.29	E-B	-921/0
G-B	-3720/0	-102.1	-102.1	0.41 (1)	5.29	E-C	0/3947
B-C	-3720/0	-102.1	-102.1	0.12 (1)	5.73		
D-C	-1555/0	0.0	0.0	0.09 (1)	7.81		
F-E	0/0	-38.5	-38.5	0.02 (2)	10.00		
E-H	0/0	-38.5	-38.5	0.60 (1)	10.00		
H-D	0/0	-38.5	-38.5	0.60 (1)	10.00		

FACTORED CONCENTRATED LOADS (LES)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-1204	-1204	—	TOP	VERT	TOTAL	—	—
H	3-11-4	-2424	-2424	—	BACK	VERT	TOTAL	—	—

SPECIFIED LOADS:			
TOP CH.	LL =	29.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD =		52.5	PSF

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 2.00/12 MINIMUM

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.06")

CSI: TC=0.41/1.00 (A-B:1), BC=0.60/1.00 (D-E:1),
WB=0.49/1.00 (C-E:1), SSF=0.55/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

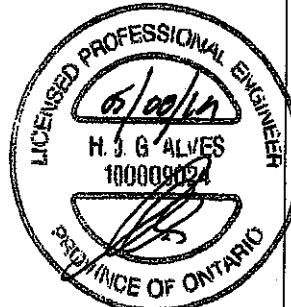
NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP= 0.83 (A) (INPUT = 0.90)
ISI METAL= 0.46 (C) (INPUT = 1.00)

DWG NO. TAM 17910934
STRUCTURAL
EXHIBIT ONLY

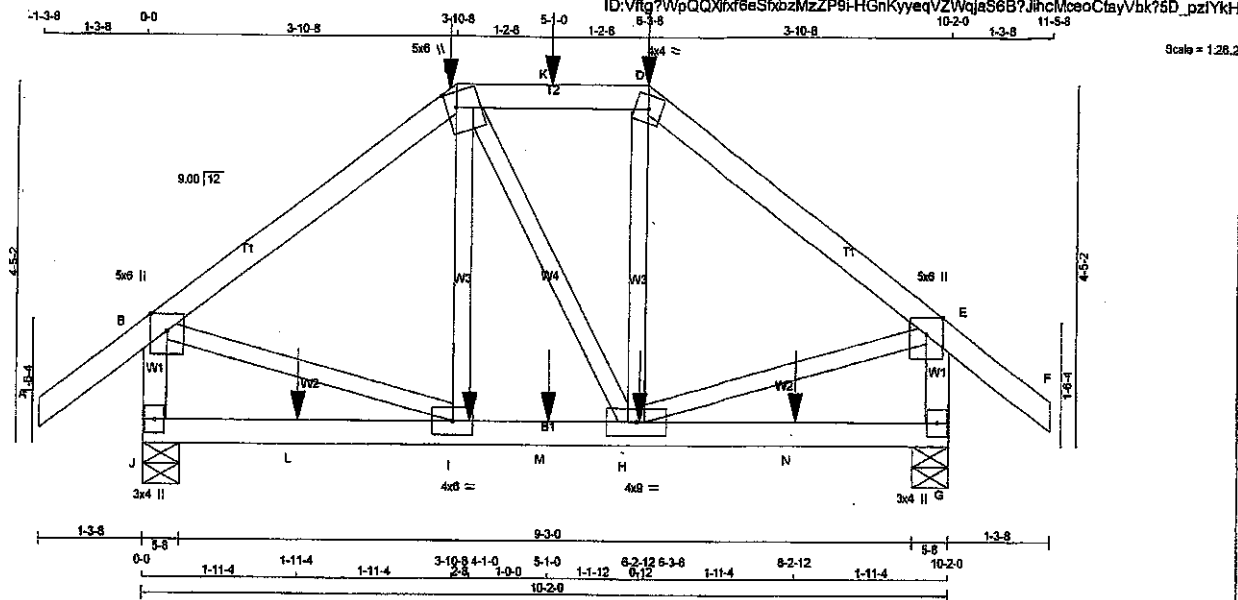


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Brentwood 3A	DRWG NO.
200142-400341	T21	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/28.2

TOTAL WEIGHT = 48 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
G - E	2x4	DRY	No.2	SPF	
J - G	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMVW-t	MT20	4.0	9.0		
I	BMVW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	1318	0	1318	0
G	1318	0	1318	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS				
			SNOW	LIVE	PERM.LIVE	WIND	DEAD
J	989	584/0	156/0	0/0	0/0	230/0	0/0
G	989	584/0	156/0	0/0	0/0	230/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.65 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	WEBS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX
MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MAX	MEMB.	FORCE	VERT. LOAD	LC1	MAX	MAX	MAX
	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)		(LBS)	(LBS)	(LBS)	(LBS)	(LBS)	(LBS)
FR-TO							FR-TO						
A-B	0/42	-102.1	-102.1	0.15 (1)	10.00		I-C	-53/153	0.04 (3)				
B-C	-1088/0	-102.1	-102.1	0.32 (1)	5.85		C-H	0/7	0.00 (3)				
C-K	-853/0	-102.1	-102.1	0.21 (1)	6.25		H-D	-49/158	0.04 (3)				
K-D	-853/0	-102.1	-102.1	0.21 (1)	6.25		B-I	0/890	0.22 (1)				
D-E	-1071/0	-102.1	-102.1	0.32 (1)	5.85		H-E	0/891	0.22 (1)				
E-F	0/42	-102.1	-102.1	0.15 (1)	10.00								
J-B	-1238/0	0.0	0.0	0.14 (1)	7.18								
G-E	-1238/0	0.0	0.0	0.14 (1)	7.18								
J-L	0/0	-38.5	-38.5	0.17 (3)	10.00								
L-I	0/0	-38.5	-38.5	0.17 (3)	10.00								
I-M	0/851	-38.5	-38.5	0.25 (2)	10.00								
M-H	0/851	-38.5	-38.5	0.25 (2)	10.00								
H-N	0/0	-38.5	-38.5	0.17 (3)	10.00								
N-G	0/0	-38.5	-38.5	0.17 (3)	10.00								

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-324	-324		FRONT	VERT	TOTAL		
D	6-3-8	-325	-325		FRONT	VERT	TOTAL		
H	6-2-12	-38	-38		FRONT	VERT	TOTAL		
I	4-1-0	-38	-38		FRONT	VERT	TOTAL		
K	5-1-0	-46	-46		FRONT	VERT	TOTAL		
L	1-11-4	-38	-38		FRONT	VERT	TOTAL		
M	5-1-0	-38	-38		FRONT	VERT	TOTAL		
N	6-2-12	-38	-38		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.32/1.00 (D-E:1), BC=0.25/1.00 (H-I:2), WB=0.22/1.00 (E-H:1), SI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

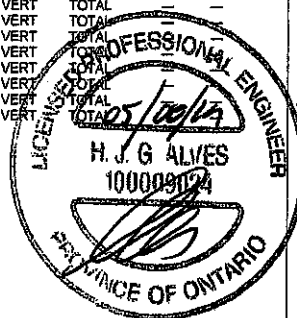
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (I) (INPUT = 0.90)
JSI METAL= 0.45 (E) (INPUT = 1.00)

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JUN 10 2019

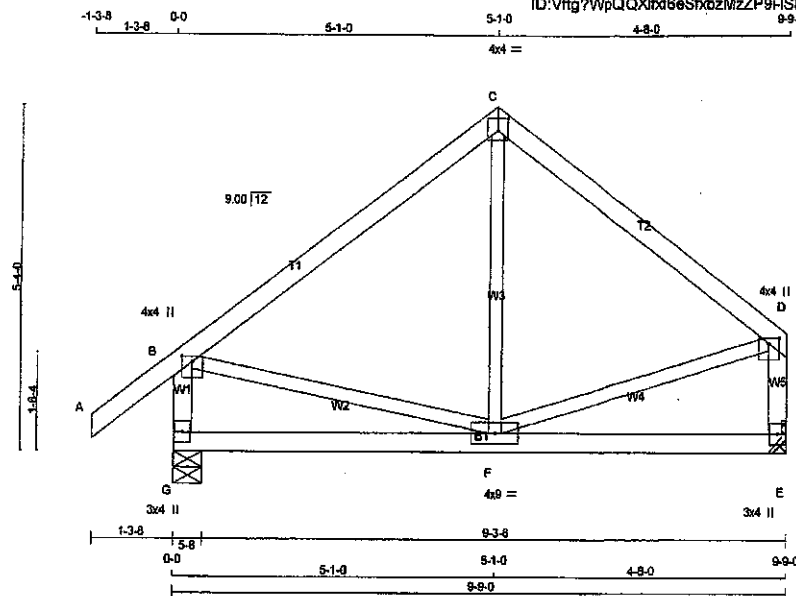
TOWN OF CALLEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 17910935
STRUCTURAL
CHECK ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400341	T22A	3	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2016 MITek Industries, Inc. Wed May 8 14:09:48 2019 Page 1
ID: Vftg?WpQQXf8eShpZMzZP9HSL9fSGsehlJ0lJrxDqum3CYU RulyfqnWGzIYkG



TOTAL WEIGHT = 3 X 42 = 125 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
G - B	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TTW-p	MT20	4.0	4.0	2.25	2.00
D TMVW+p	MT20	4.0	4.0	1.00	2.00
E BMV1+p	MT20	3.0	4.0		
F BMVWV-1	MT20	4.0	9.0		
G BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED		MAXIMUM FACTORED		INPUT	REQD	
JT	VERT	HORZ	GROSS REACTION	GROSS REACTION	BRG	BRG	
			DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	826	0	826	0	0	5-8	5-8
E	686	0	686	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 3-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	609	363 / 0	102 / 0	0 / 0	0 / 0	143 / 0	0 / 0
E	512	283 / 0	102 / 0	0 / 0	0 / 0	127 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX	MAX	UNBRAC	MEMB.	FORCE (LBS)	MAX	CSI (LC)
FR-TO	0 / 42	-102.1	-102.1	0.14 (1)	10.00	F-C	0 / 197	0.05 (3)	
B-C	-444 / 0	-102.1	-102.1	0.34 (1)	8.25	B-F	0 / 365	0.08 (1)	
C-D	-444 / 0	-102.1	-102.1	0.29 (1)	8.25	F-D	0 / 373	0.08 (1)	
G-B	-750 / 0	0.0	0.0	0.08 (1)	7.81				
E-D	-619 / 0	0.0	0.0	0.07 (1)	7.81				
G-F	0 / 0	-38.5	-38.5	0.21 (3)	10.00				
F-E	0 / 0	-38.5	-38.5	0.21 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.32")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.32")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.34/1.00 (B-C:1), BC=0.21/1.00 (F-G:3), WB=0.08/1.00 (D-F:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 783 1987 1856

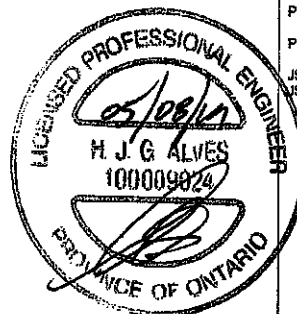
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (B) (INPUT = 0.90)
JSI METAL= 0.16 (S) (INPUT = 1.00)

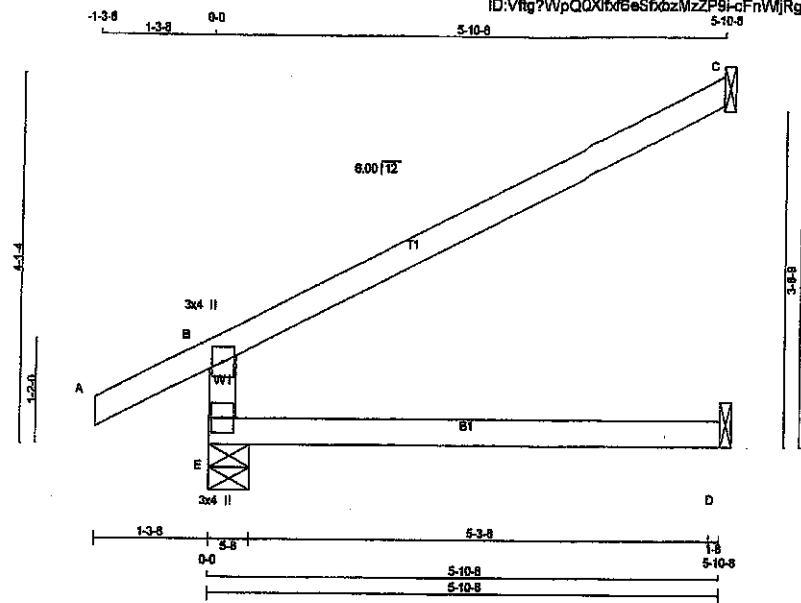
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TOWN OF SALEM
BUILDING DEPARTMENT
FILE NO.



DRWG NO. TAM 77910936
STRUCTURAL
QUALITY CHECK ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J1	9	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 14:06:46 2019 Page 1					
ID: Vfg?WpQQXfW6eSf6zZp9i-cFwWjRgNeauecX8Xn20IEpRAR5oxhtph7bbNzfYn7					
Scale = 1:25.0					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2			
A - C	2x4	DRY	No.2			
E - D	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	UP	IN-SX
E	646	0	0	5-8
C	225	0	0	1-8
D	93	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW
		LIVE
E	473	292/0
C	154	128/0
D	65	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MEMB.	MAX. FACTORED
MEMB.	FORCE	(PLF)	CS1	(LC)	UNBRAC	LENGTH	FORCE
FR-TO	(LBS)	FROM	TO				(LBS)
E-B	-514/0	0.0	0.0	0.22 (3)	7.81		
A-B	0/31	-102.1	-102.1	0.13 (1)	10.00		
B-C	-34/0	-102.1	-102.1	0.60 (1)	6.25		
E-D	0/0	-38.5	-38.5	0.22 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.04")
ALLOWABLE DEFL.(TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/942$ (0.07")

CS1 TC=0.60/1.00 (B-C-1), BC=0.22/1.00 (D-E-3),
WB=0.00/1.00 (n/a), SS=0.20/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)		
	MAX	MIN	MAX	MIN
MT20	618	354	1687	788

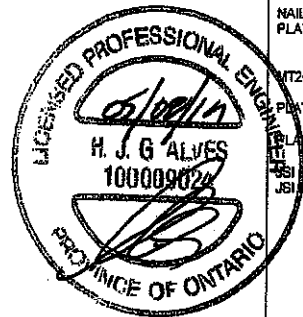
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CS1 BRP=0.21 (E) (INPUT = 0.90)
CS1 METAL=0.14 (B) (INPUT = 1.00)

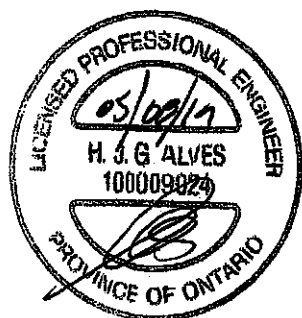
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JUN 10 2019

TOWN OF SALEM
BUILDING SECTION
FILE NO.

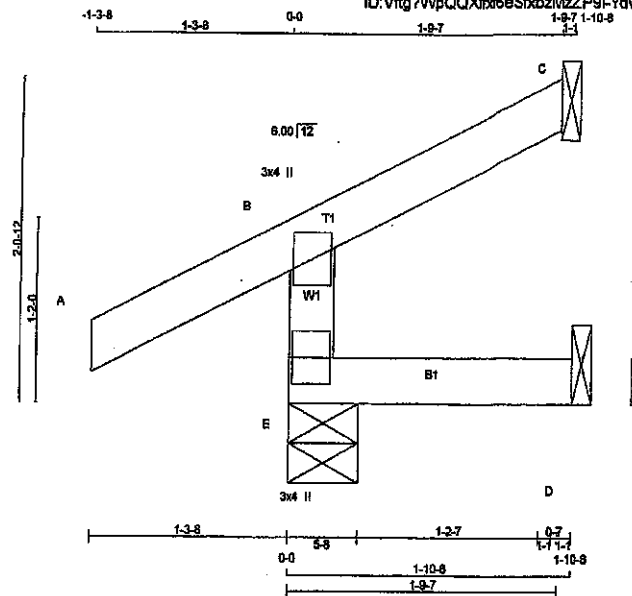


DWG NO. TAM 12910937
STRUCTURAL
COMPLEMENT ONLY

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Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 14:08:48 2019 Page 1
ID: Vftg7WpQQXf6eSfbzMMZP9I-YdvHAPSwuFqM7vhWeC5UOfvu_Eq?PaN6L_UigFzIYn5



Size = 1:13.5

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR
E - B	2x4	DRY No.2	SPF
A - C	2x4	DRY No.2	SPF
E - D	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
E	318	0	318	0	0	5-8
C	51	0	51	0	-28	1-8
D	24	0	41	0	-4	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	228	180 / 0	20 / 0	0 / 0	0 / 0	46 / 0	0 / 0
C	38	27 / -21	2 / 0	0 / 0	0 / 0	7 / 0	0 / 0
D	24	0 / -9	18 / 0	0 / 0	0 / 0	11 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBBS			
MAX. FACTORED		FACTORED		FACTORED		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-270 / 0	0.0	0.0	0.04 (5)	7.81		
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.09		
B-C	-18 / 0	-102.1	-102.1	0.10 (1)	6.25		
E-D	0 / 0	-38.5	-38.5	0.04 (5)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/960 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/960 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.13/1.00 (A-B:1), BC=0.04/1.00 (D-E:5),
WB=0.00/1.00 (n/a:0), SS=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1667	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.11 (E) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

BPMS NO. TAM 7510990
STRUCTURAL
CONCRETE ONLY

RECEIVED
JUN 10 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

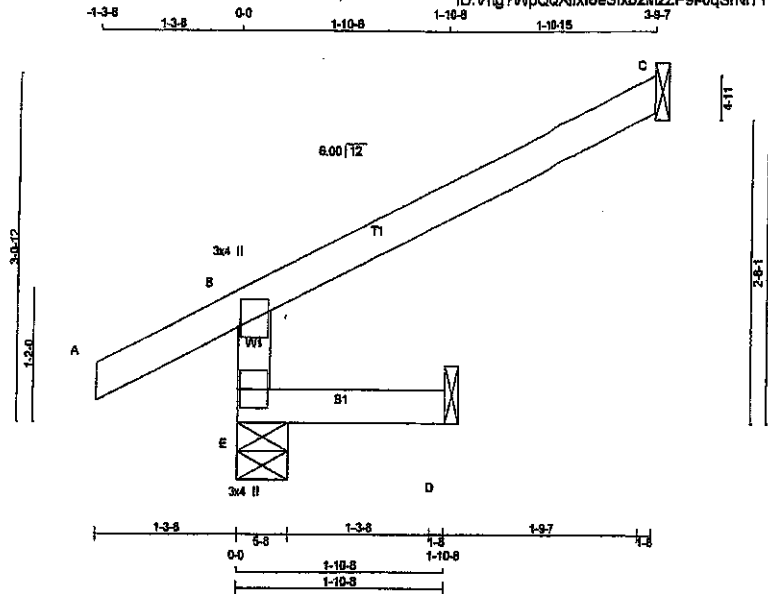


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J5	2	1	Brentwood 3A	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:Vftg?WpQQXfxf6eSf6bzMzZP9t-0qSfNfTYZyDf3GfCvcjwR2we9Q81dFzEfChzIYn4

Scale = 1:18.7



TOTAL WEIGHT = 2 X 10 = 19 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY No.2

A - C 2x4 DRY No.2

E - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

LUMBER

DESCR.

SPF

SPF

SPF

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TMV+p MT20 3.0 4.0

E BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
E	420	0	420	0	5-8	5-8
C	145	0	145	0	1-8	1-8
D	32	0	41	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM/LIVE	WIND	DEAD	SOIL
E	297	218 / 0	22 / 0	0 / 0	0 / 0	0 / 0	58 / 0	0 / 0	0 / 0
C	100	83 / 0	0 / 0	0 / 0	0 / 0	0 / 0	17 / 0	0 / 0	0 / 0
D	29	0 / 0	18 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
E-B	-380 / 0	0.0 0.0 0.01 (3)	7.81	
A-B	0 / 31	-102.1 -102.1 0.13 (1)	10.00	
B-C	-22 / 0	-102.1 -102.1 0.25 (1)	8.25	
E-D	0 / 0	-38.5 -38.5 0.03 (3)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/969$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/969$ (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.03/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(ORY) SHEAR SECTION (PS) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 788 1967 1656

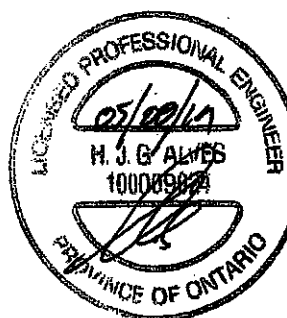
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.16 (E) (INPUT = 0.90)
JSI METAL=0.11 (B) (INPUT = 1.00)

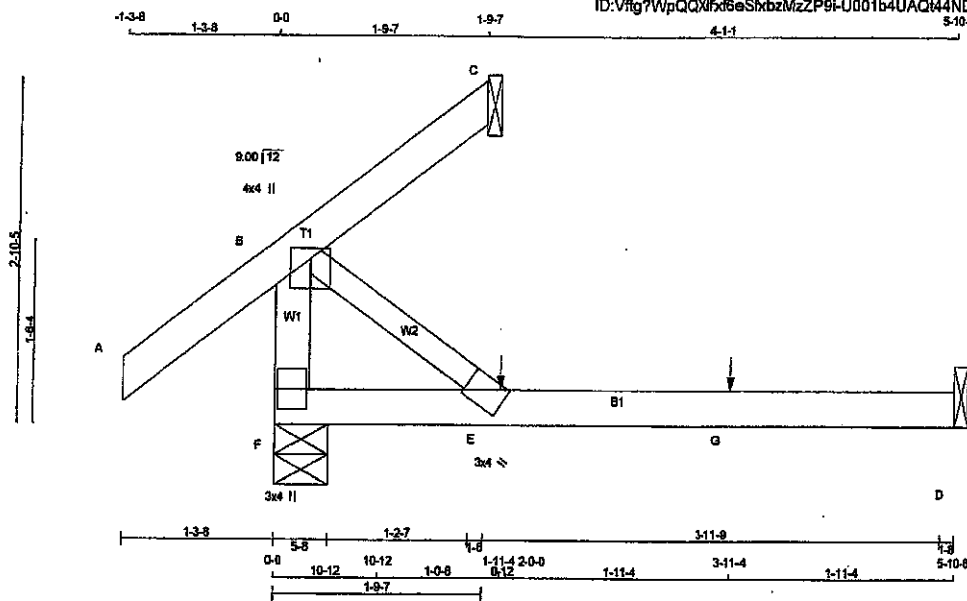
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 11910941
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J7	1	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 14:08:50 2019 Page 1					
ID: Vfg7WpQQXf6eShbzMzZP9tU001b4UAC44NDvmd7yT4_EA2R9tUKolzpk8zIYn3					
Scale = 1:17.7					



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMW+p	MT20	4.0	4.0	1.00 2.00
E BMW+w	MT20	3.0	4.0	2.00 1.25
F BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	400	0	0	5-8
C	37	0	0	1-8
D	113	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
F	299	163/0	62/0
C	25	21/0	0/0
D	103	0/0	62/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO							
F-B	-288/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0
A-B	0/42	-102.1	-102.1	0.15 (1)	10.00		
B-C	-33/0	-102.1	-102.1	0.15 (1)	6.25		
F-E	0/0	-38.5	-38.5	0.27 (2)	10.00		
E-G	0/0	-38.5	-38.5	0.32 (2)	10.00		
G-D	0/0	-38.5	-38.5	0.32 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11.4	1	1	1	FRONT	VERT	TOTAL	1	1
G	3-11.4	1	1	1	FRONT	VERT	TOTAL	1	1

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/925 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/555 (0.13")

CSI: TC=0.15/1.00 (A-B-1), BC=0.32/1.00 (D-E-2),
WB=0.00/1.00 (B-E-1), SS=0.12/1.00 (E-F-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 619 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

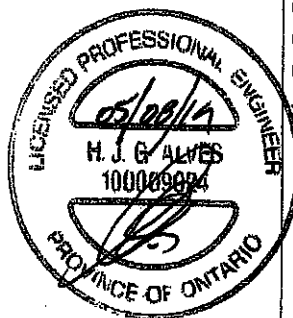
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL = 0.06 (B) (INPUT = 1.00)

ENGR. NO. TAM 17910942
STRUCTURAL
CITY OF BURLINGTON ONLY

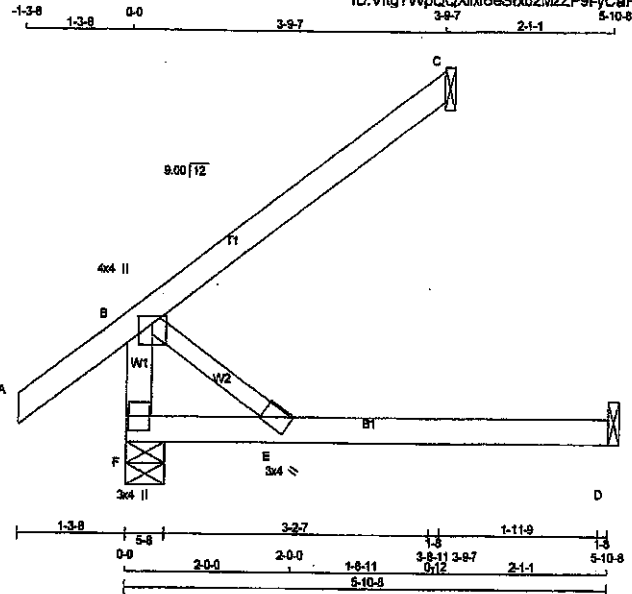
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JUN 10 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO



JOB NAME 200142-400307	TRUSS NAME J8	QUANTITY 1	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 14:08:51 2019 Page 1
ID: Vltg?WpQQXdxrBzSxbzLzZP9l-yCaPoQVoBACX_NQ5JKeB?IWOPSnix7Y1yIMGazYn2



Scale = 1/24.9

TOTAL WEIGHT = 17 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
F	447	0	447	0
C	193	0	193	0
D	113	0	144	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	332	190/0	62/0	0/0	0/0	80/0	0/0
C	133	110/0	0/0	0/0	0/0	23/0	0/0
D	103	0/0	62/0	0/0	0/0	41/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	UNBRACED LENGTH	MEMB. FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO							
F-B	-334/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00		
B-C	0/0	-102.1	-102.1	0.25 (1)	10.00		
F-E	0/0	-38.5	-38.5	0.25 (3)	10.00		
E-D	0/0	-38.5	-38.5	0.30 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.9 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/925 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/555 (0.13")

CSI: TC=0.25/1.00 (B-C:1), BC=0.30/1.00 (D-E:3),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.80)
JSI METAL = 0.07 (B) (INPUT = 1.00)

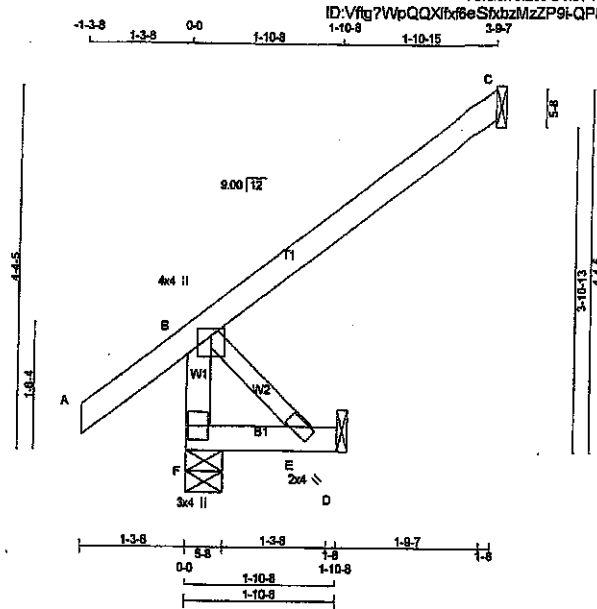
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DATE OF TAM 7/11/0943
STRUCTURAL
ONLY

JOB NAME 200142-400307	TRUSS NAME J10	QUANTITY 1	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1/25.0

TOTAL WEIGHT = 12 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMVW+p	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	IN-SX
F	370	0	370	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	36	0	48	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	262	190/0	20/0	0/0	0/0	52/0	0/0
C	133	110/0	0/0	0/0	0/0	23/0	0/0
D	33	0/0	20/0	0/0	0/0	13/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	UNBRAC LENGTH FR-TO
F-B	-334/0	0.0	0.0 0.03 (1)	F-E	0/0	0.00 (1)	7.81
A-B	0/42	-102.1	-102.1 0.14 (1)	E-D	0/0		10.00
B-C	0/0	-102.1	-102.1 0.25 (1)				10.00
F-E	0/0	-38.5	-38.5 0.04 (3)				10.00
E-D	0/0	-38.5	-38.5 0.03 (3)				10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.25/1.00 (B-C:1), BC=0.04/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES:
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

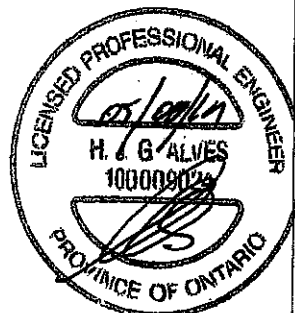
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

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JUN 10 2019

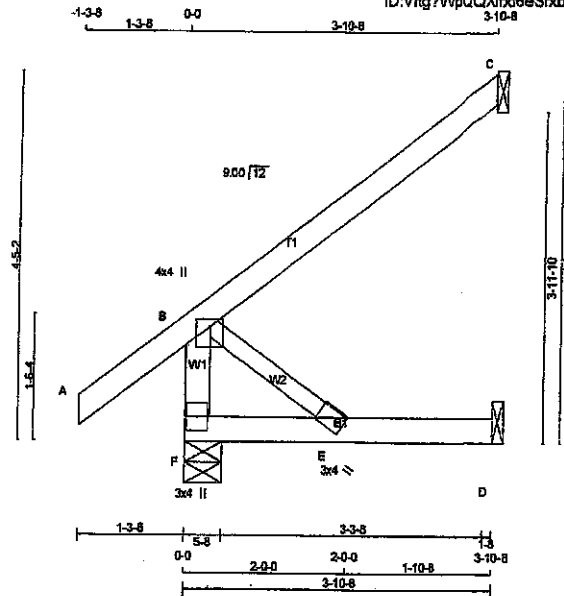
TOWN OF CALROON
BUILDING SECTION
FILE NO



DATE: JUN 10 2019
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J11	5	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Wed May 8 14:08:53 2019 Page 1
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Scale = 1/25.6

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	MAXIMUM FACTORED	INPUT	REQD					
	GROSS REACTION	GROSS REACTION	BRG	BRG					
JT	VERT	DOWN	UP	IN-SX	IN-SX				
F	413	0	413	0	0	5-8	5-8		
C	188	0	188	0	0	1-8	1-8		
D	75	0	95	0	0	1-8	1-8		

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00	
E	BMVW+p	MT20	3.0	4.0			
F	BMV1+p	MT20	3.0	4.0			

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS									
1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL			
JT	COMBINED								
F	300	193 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0		
C	138	113 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0		
D	68	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0		

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)		FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
F-B	-339 / 0	0.0	0.0	0.04 (1)	7.81	0 / 0	0.00 (1)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
B-C	0 / 0	-102.1	-102.1	0.26 (1)	10.00		
F-E	0 / 0	-38.5	-38.5	0.14 (3)	10.00		
E-D	0 / 0	-38.5	-38.5	0.14 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 5 X 15 = 75 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL)= L/988 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL)= L/899 (0.02")

CS1: TC=0.28/1.00 (B-C:1), EC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1658

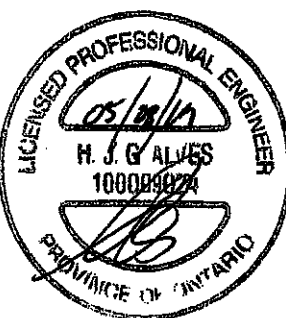
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

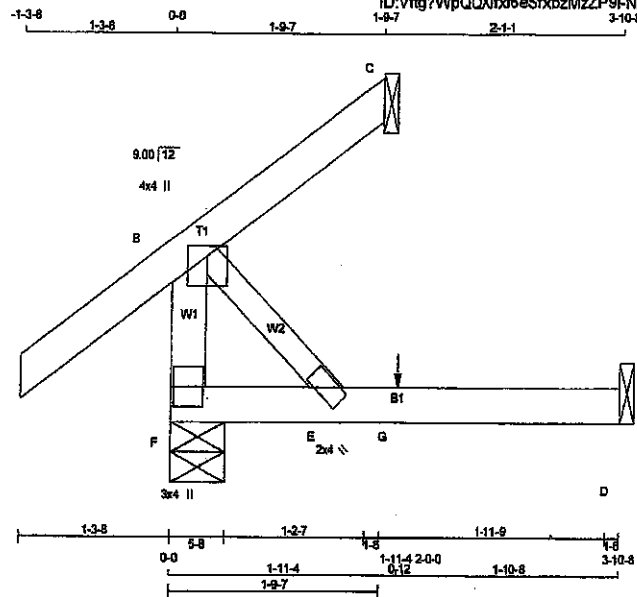
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TOWN OF CALEDON
BUILDING SECTION
FILE NO



DESIGNATED TAM 1710746
STRUCTURAL
(1) (2) (3) (4) (5) (6) (7) (8) (9) (10) (11) (12)

JOB NAME 200142-400307	TRUSS NAME J12	QUANTITY 1	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1:17.7

TOTAL WEIGHT = 12 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	381	0	0	5-8
C	37	0	45	1-8
D	75	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	284	183 / 0	41 / 0	0 / 0	0 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	88	0 / 0	41 / 0	0 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LC1
FR-TO					FR-TO		
F-B	-288 / 0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 42	-102.1	-102.1	0.15 (1)			
B-C	-33 / 0	-102.1	-102.1	0.15 (1)			
F-E	0 / 0	-38.5	-38.5	0.12 (2)			
E-G	0 / 0	-38.5	-38.5	0.14 (2)			
G-D	0 / 0	-38.5	-38.5	0.14 (2)			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	—	—	FRONT	VERT	TOTAL	—	—

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.13")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.13")
CALCULATED VERT. DEFL.(TL) = L/989 (0.02")

CSI: TC=0.15/1.00 (A-B-1), BC=0.14/1.00 (D-E-2), WB=0.00/1.00 (B-E-1), SS=0.10/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687

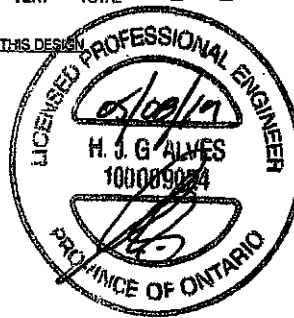
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

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TOWN OF CALEDON
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FILE NO

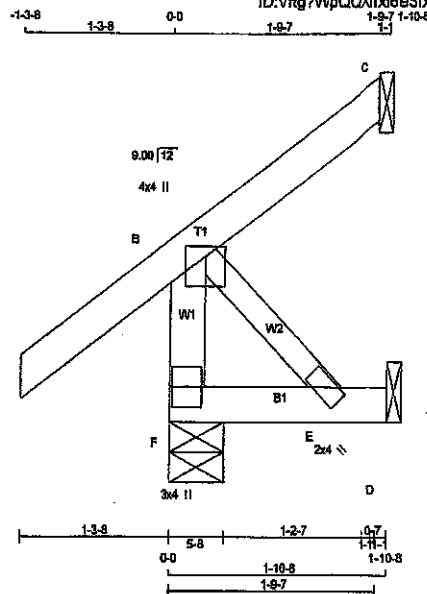


7/19/0947
STRUCTURAL
FOR INFO ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J13	1	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

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ID:Vftg?WpQQXf6eSfcbzMzZP9iNnGYQsXhU5bWrg8g?SCudw8vNRXpls_jwx0tvlYn?



Scale = 1:17.7

TOTAL WEIGHT = 9 lb

LUMBER	SIZE	DRY	LUMBER	DESCR.
N.L.G.A. RULES				
CHORDS	2x4	DRY	No.2	SPF
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	323	0	323	0	5-8	5-8
C	37	0	37	0	1-8	1-8
D	36	0	48	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS				WIND	DEAD	SOIL
		SNOW	LIVE	PERM.LIVE				
F	229	183/0	20/0	0/0	0/0	47/0	0/0	0/0
C	25	21/-31	0/0	0/0	0/0	4/0	0/0	0/0
D	33	0/0	20/0	0/0	0/0	13/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)			MAX. UNBRACED LENGTH FR-TO	MEMB.	FACTORED MAX. FORCE (LBS)		
		FROM	TO	CSI (LC)			FROM	TO	CSI (LC)
F-B	-286/0	0.0	0.0	0.03 (1)	7.81	B-E	0/0	0.00 (1)	
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00				
B-C	-33/0	-102.1	-102.1	0.13 (1)	6.25				
F-E	0/0	-38.5	-38.5	0.03 (3)	10.00				
E-D	0/0	-38.5	-38.5	0.03 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/889 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOOLIVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618 354 1687 768 1987 1656	

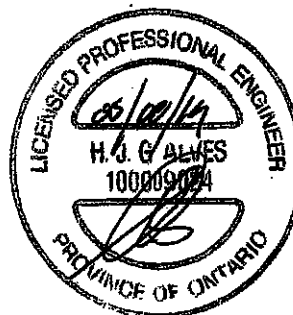
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

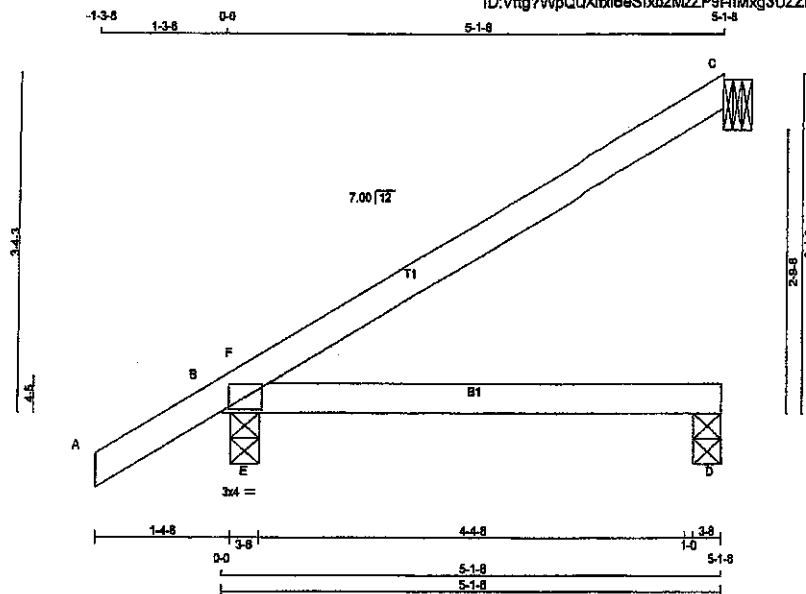
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TOWN OF CALEDON
BUILDING SECTION
FILE NO.



ENGINEER TAM 7/1/2019
STRUCTURAL
OFFICE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J21	3	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 14:08:57 2019 Page 1					
ID: Vftg?WpQQXltdf6eS5bzMzZP9i-nMxg3UZZn0z4lIfGblbFYmN2lqQ0fcQPuAgTEzYmy					



Scale = 1/2" = 1'-0"

TOTAL WEIGHT = 3 X 15 = 44 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT	235	0	235	0
C	125	0	138	0
D	500	0	500	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
C	164	128 / 0	7 / 0	0 / 0	0 / 0	31 / 0
D	105	23 / 0	47 / 0	0 / 0	0 / 0	36 / 0
B	365	228 / 0	54 / 0	0 / 0	0 / 0	83 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CS1 (LC)	MEMB. FORCE (LBS)
FR-TO		FROM TO	UNBRACED LENGTH	FR-TO
A-B	-0 / 34	-102.1 -102.1	0.13 (1)	10.00
B-F	-105 / 72	-102.1 -102.1	0.08 (3)	6.25
F-C	-5 / 7	-102.1 -102.1	0.37 (1)	10.00
B-E	0 / 0	-38.5 -38.5	0.22 (1)	10.00
E-D	0 / 0	-38.5 -38.5	0.28 (1)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:	
TOP CH. LL =	29.1 PSF
DL =	8.0 PSF
BOT CH. LL =	10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	52.5 PSF

SPACING = 24.0 IN. CC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA D88-09, CSA 098-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/280 (0.19")
CALCULATED VERT. DEFL. (LL) = L/899 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/590 (0.10")

CS1: TO=0.37/1.00 (C-F:1), BC=0.28/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.36/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
618	354	1667	788
1987	1856		

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS1 GRIP= 0.37 (B) (INPUT = 0.80)
JS1 METAL = 0.09 (B) (INPUT = 1.00)

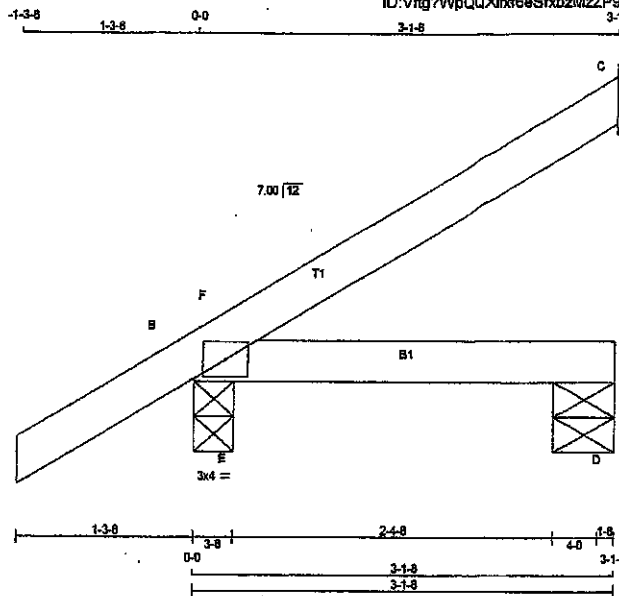
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TOWN OF CALEDON
BUILDING DEPARTMENT



WIND LOAD 97410951
STRUCTURAL
CALCULATIONS

JOB NAME 200142-400307	TRUSS NAME J22	QUANTITY 6	PLY 1	JOB DESC. Brentwood 3A	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 14:08:57 2019 Page 1			
		ID: Vfg?WpQQXfxf6eShbzMzZP9i-nMxg3UZZn0z4lIfFgbIFymQht30fcQPuAgTEzYmy			



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y
B	TMB14	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	VERT	GROSS	REACTION	GROSS	REACTION	BRG	IN-SX	BRG	IN-SX
C	142	0	142	0	0	1-8	1-8	1-8	1-8
B	359	0	359	0	0	3-8	3-8	3-8	3-8
D	78	0	85	0	0	5-8	5-8	5-8	5-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS				DEAD	SOIL
		SNOW	LIVE	PERM/LIVE	WIND		
C	99	76/0	5/0	0/0	0/0	19/0	0/0
B	280	170/0	33/0	0/0	0/0	57/0	0/0
D	65	15/0	28/0	0/0	0/0	22/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD LC1	MAX	MEMB.	FORCE (LBS)	MAX
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0/34	-102.1	-102.1	0.13 (1)	10.00	E-F	-159/82
B-F	-44/0	-102.1	-102.1	0.04 (3)	8.25		
F-C	0/5	-102.1	-102.1	0.13 (1)	10.00		
B-E	0/0	-38.5	-38.5	0.09 (1)	10.00		
E-D	0/0	-38.5	-38.5	0.11 (1)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

TOTAL WEIGHT = 6 X 10 = 58 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.10")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.10")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.13/1.00 (A-B:1), BC=0.11/1.00 (D-E:1).
WB=0.00/1.00 (E-F:1), SS=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

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JUN 10 2019

TOWN OF SALEDON
BUILDING SECTION
FILE NO. _____



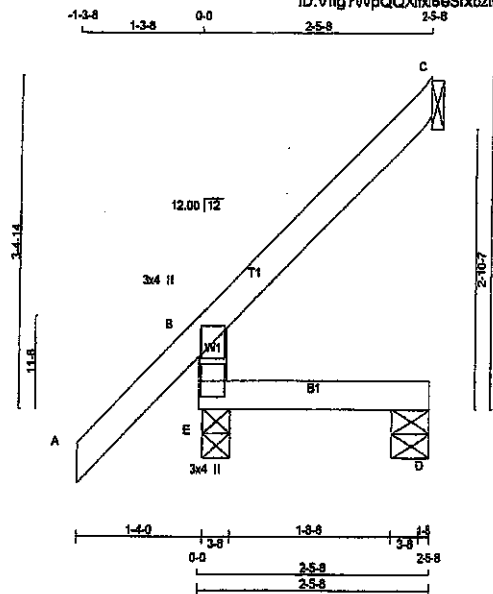
DATE: 11/9/2019
DRAWN BY: T1910952
CHECKED BY: _____
APPROVED BY: _____

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200142-400307	J23	9	1	Brentwood 3A	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 14:08:58 2018 Page 1

ID:Vfig?WpQQXm6eSfxbzMzZP9L-FZV2GqBYK5xKSSREIGqomJbHGEL6saeYVe7gzlYmx

Scale = 1:21.8



TOTAL WEIGHT = 9 X 10 = 90 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	IN-SX	IN-SX
JT	353	0	0	0
E	94	0	0	0
C	94	0	0	0
D	40	0	0	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	255	170/0	30/0	0/0	0/0	55/0	0/0
C	65	54/0	0/0	0/0	0/0	11/0	0/0
D	37	0/0	22/0	0/0	0/0	15/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX	LC2 MAX	MAX. UNBRACED LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
E-B	-299/0	0.0	0.0	0.03 (3)	7.81			
A-B	0/50	-102.1	-102.1	0.14 (1)	10.00			
B-C	-22/0	-102.1	-102.1	0.10 (1)	6.25			
E-D	0/0	-38.5	-38.5	0.04 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 085-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.00")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.04/1.00 (D-E:3),
WB=0.00/1.00 (A-E:0), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.15 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)

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TECH-NOTES

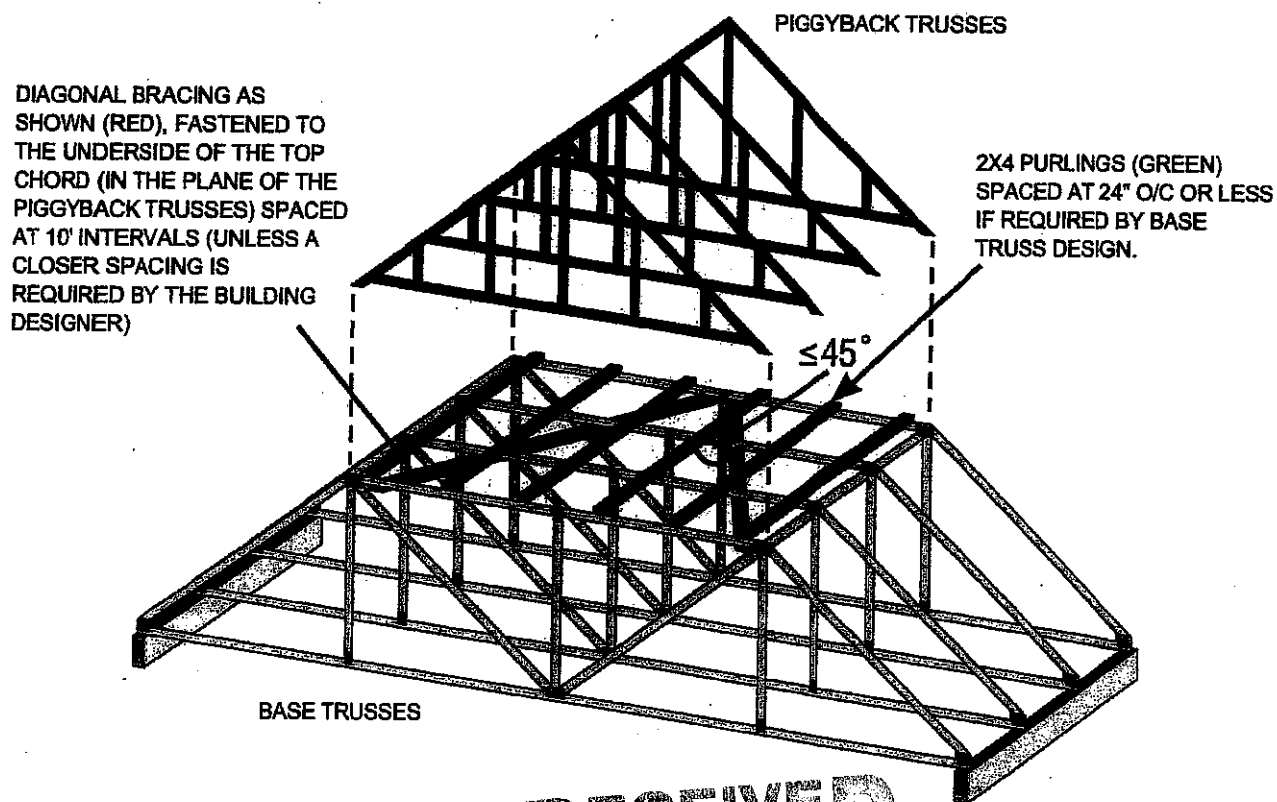
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

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SKETCH FROM BCSI-CANADA 2013

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Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

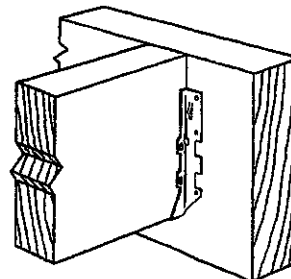
Finish: G90 galvanized

Design:

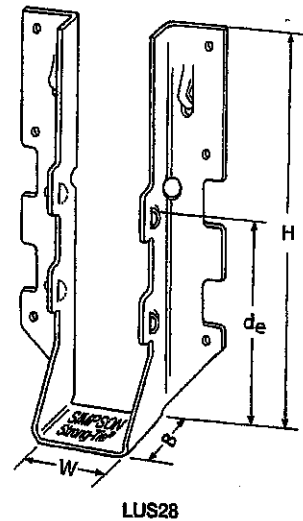
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



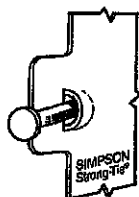
Typical LUS
Installation



LUS28

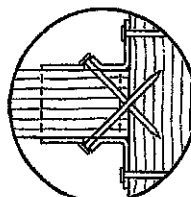
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _e =1.15)	(K _e =1.00)	(K _e =1.15)	(K _e =1.00)
LUS24	18	1½	3½	1½	1½	(4) 10d	(2) 10d	710	1630	645	1165
LUS24-2	18	3½	3½	2	1½	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1½	4½	1½	3½	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3½	4½	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4½	4½	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1½	6½	1½	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3½	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4½	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1½	7½	1½	3¾	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3½	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4½	8¾	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent
5,603,580



Double Shear Nailing
Top View.

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DESIGN

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(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

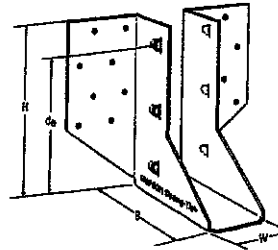
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

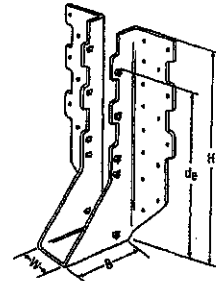
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

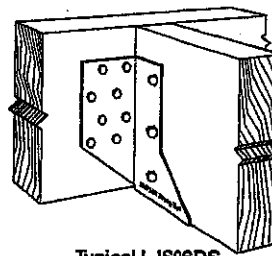
- See current catalogue for options



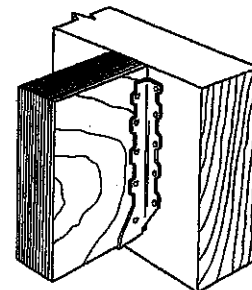
LJS26DS



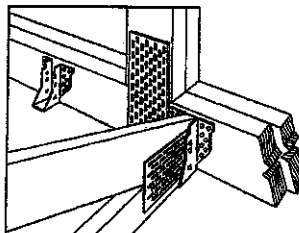
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



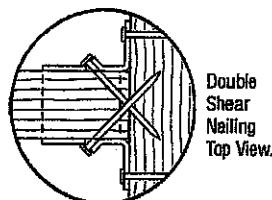
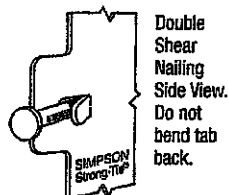
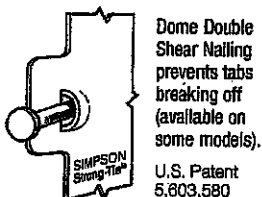
Typical HUS
Installation



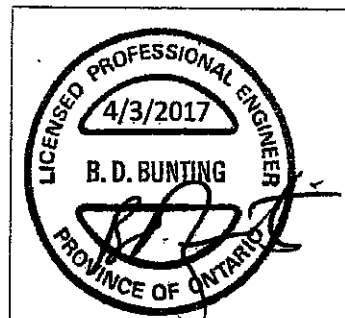
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 3/4	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/16	5 1/2	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/16	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2875	4345
HUS210	16	1 1/16	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.



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HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

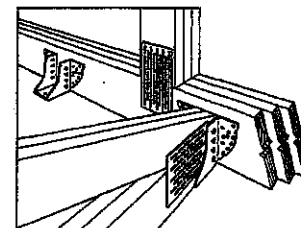
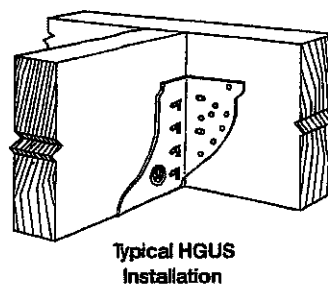
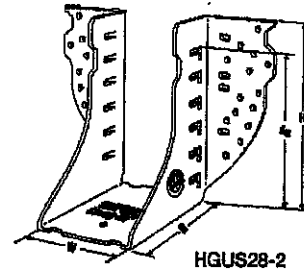
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

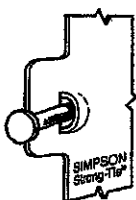
- See current catalogue for options



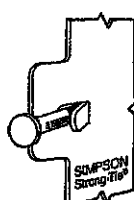
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

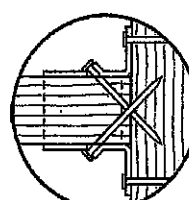
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



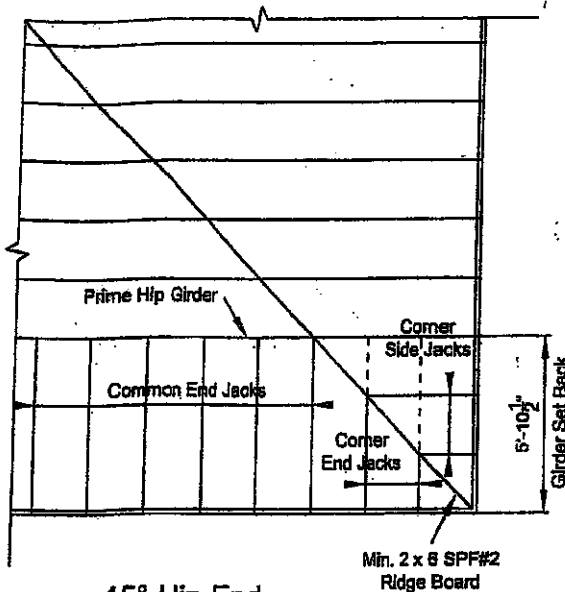
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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45° Hip End

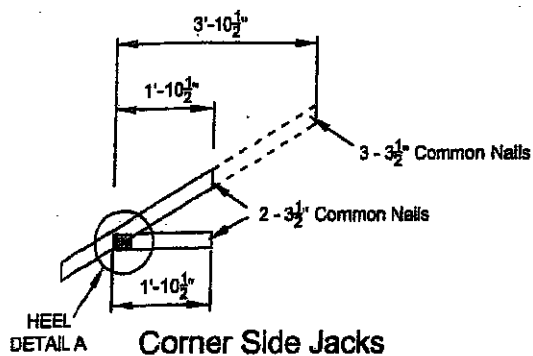
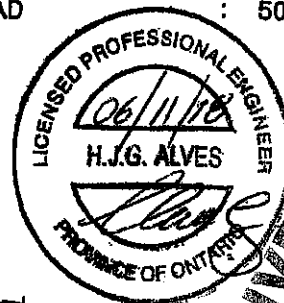
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

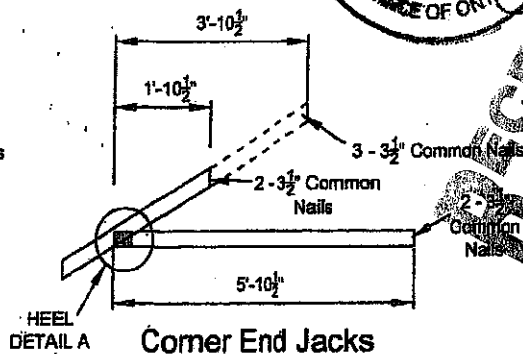
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

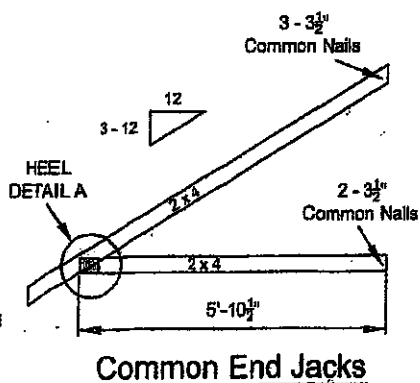
TOTAL LOAD : 50.5 P.S.F



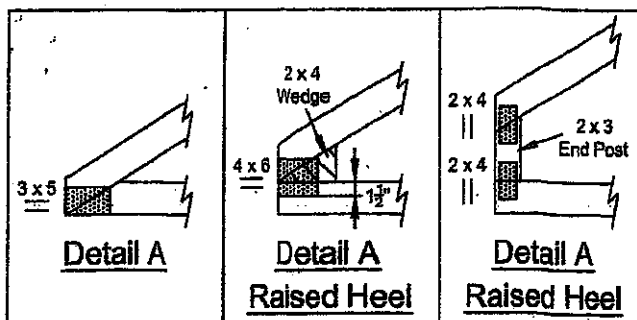
Corner Side Jacks



Corner End Jacks



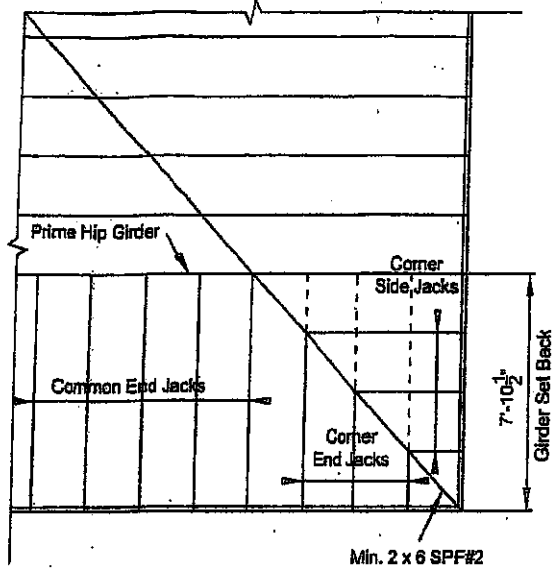
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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45° Hip End

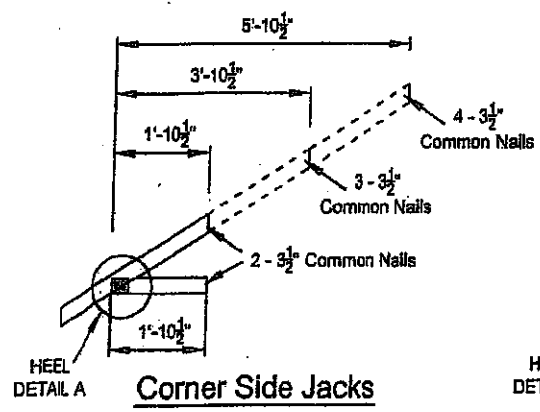
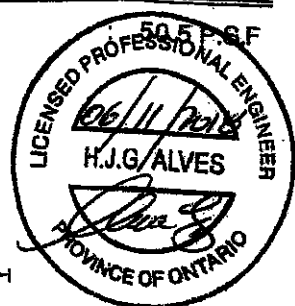
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

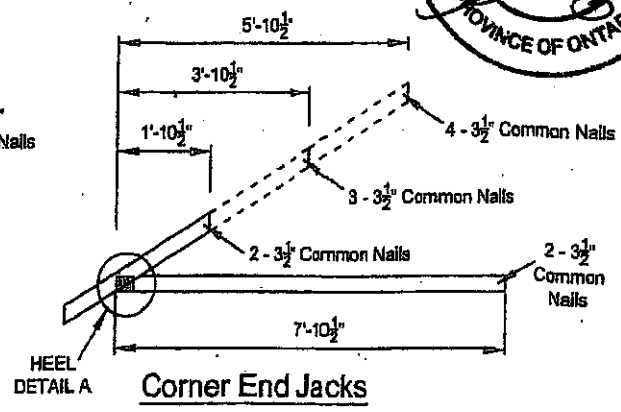
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

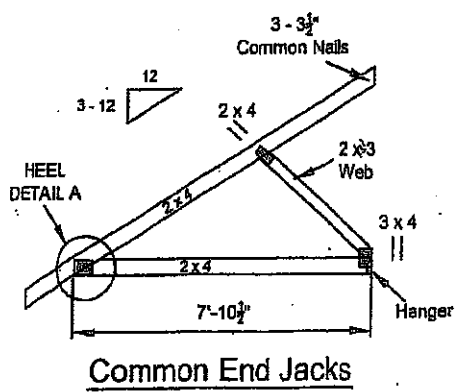
TOTAL LOAD



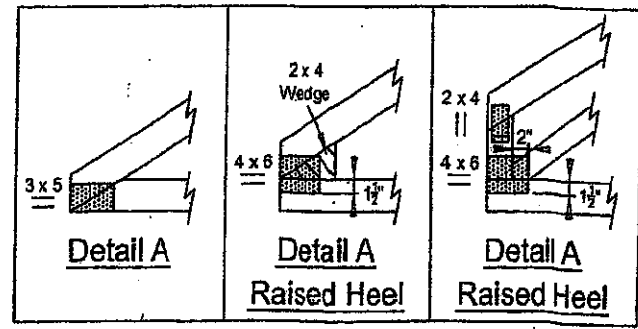
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

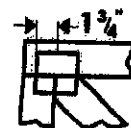
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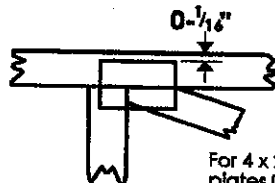
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

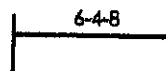


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

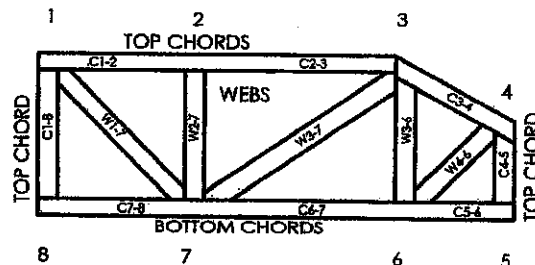
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIT-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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TOWN OF CALEPON
BUILDING SECTION
FILE NO.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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TOWN OF CALEDON
BUILDING SECTION
FILE NO.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE