

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

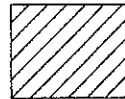
HARDWARE:

HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

ALL B- 2-2X10 (FLUSH)

DENOTES:
CONVENTIONAL
FRAMING

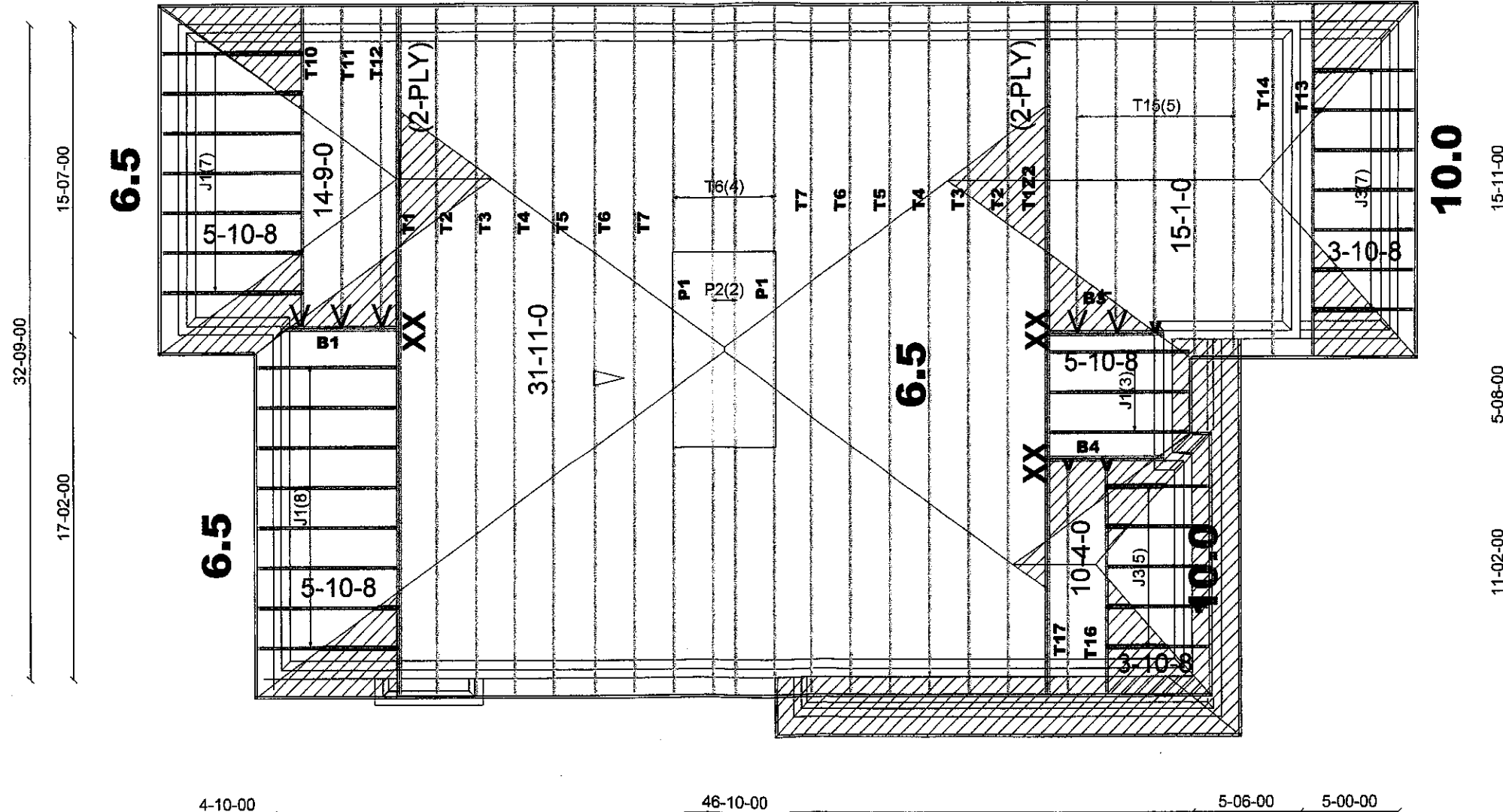


THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

9/12 PITCHES (TYP.)
UNLESS NOTED

57-02-00

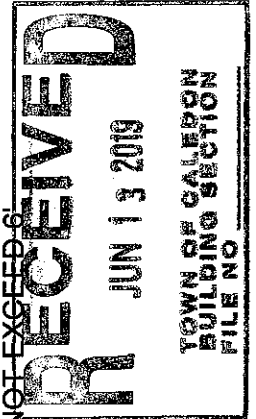
5-00-00



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.8 \text{ kPa}$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



M12254



Job Track: **50120**
Plan Log: **200598**
Layout ID: **403059**

Builder / Location:
GREEN PARK HOMES / Caledon
Project: **LAMBERTS LANE HOME CORP.**
Date: 5/9/2019 Sales: **Mario DiCano** Designer: **Sunny Jane**

Model / Elevation:
KINMOUNT 6A/2

Milek ver 8.2.3.229

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TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



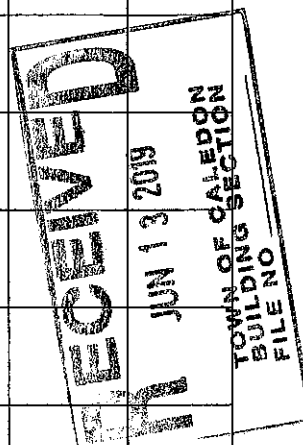
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200598
 Layout ID: 403059
 Ref #
 Page: 1 of 2
 Date: 05/09/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	1 2-ply	T122 Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	267.53 170.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.92 208.00		
	1	T10 Hip Girder	9/12	14-09-00	4-04-14	2 x 4 2 x 6	1-03-08	1-06-04 1-06-04	72.72 46.33		
	1	T11 Hip	9/12	14-09-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	64.66 42.50		
	1	T12 Hip	9/12	14-09-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.72 46.67		
	1	T13 Hip Girder	9/12	15-01-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	78.15 50.17		
	1	T14 Hip	9/12	15-01-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	70.61 47.33		
	5	T15 Common	9/12	15-01-00	7-02-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	337.18 224.17		





DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 2

Job Track: 50120
 PlanLog: 200598
 Layout ID: 403059
 Ref #
 Page: 2 of 2
 Date: 05/09/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T16 Hip Girder	9/12	10-04-00	4-10-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	47.63 32.00		
	1	T17 Common	9/12	10-04-00	5-04-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	44.99 29.50		
	2	P1 Piggyback	9/12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9/12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	18	J1 Jack-Open	6.5/12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	308.69 192.00		
	12	J3 Jack-Open	10/12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	182.71 116.00		

TOTAL # TRUSS= 66

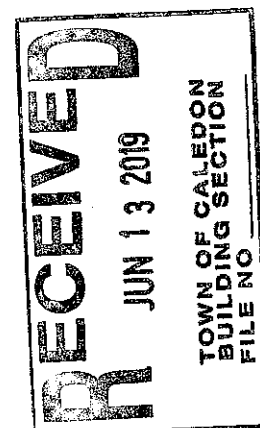
TOTAL BFT OF ALL TRUSSES= 2841

BFT. TOTAL WEIGHT OF ALL TRSSES 4474.64 LBS

HARDWARE

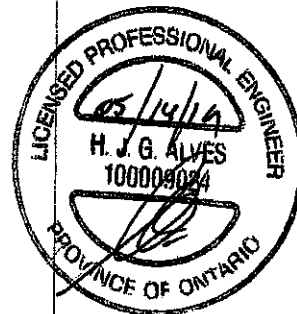
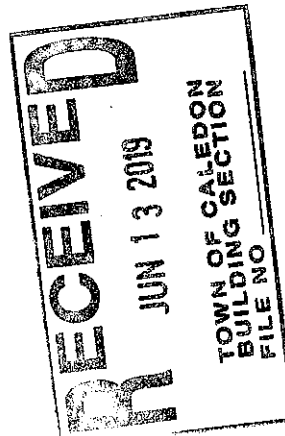
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
8	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 11



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:19 2019 Page 2	
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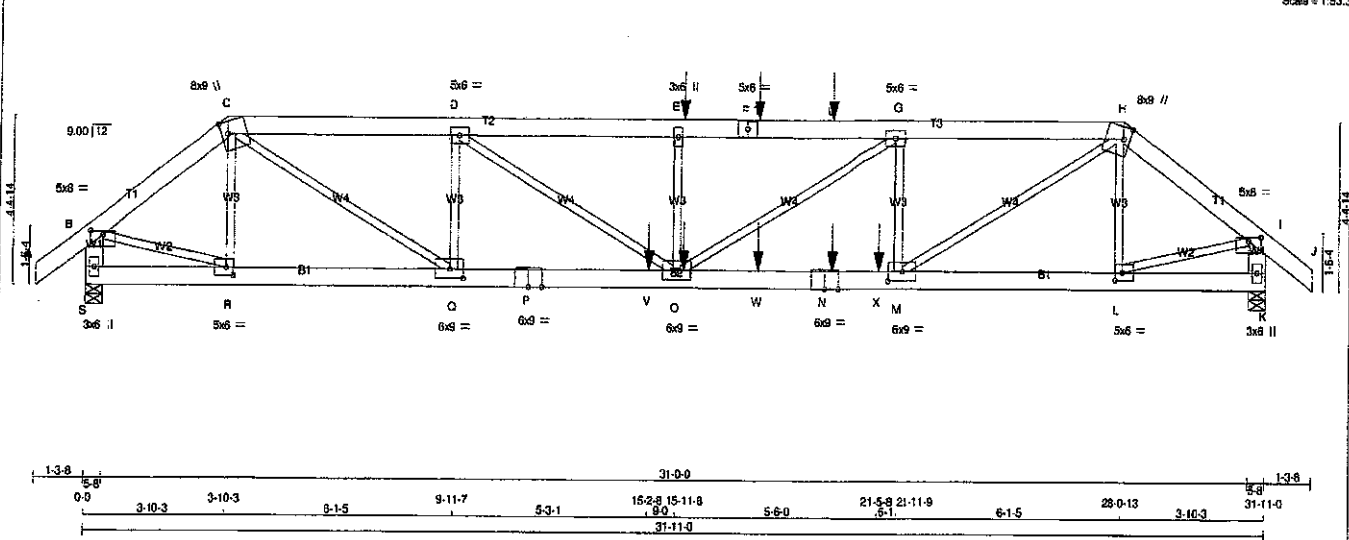
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	20-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	22-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Y	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Z	14-10-8	-2332	-2332	---	FRONT	VERT	TOTAL	---	---
AA	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AG	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AH	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
DWG# T-1911482

JOB NAME 403059	TRUSS NAME T1Z2	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:08:46 2019 Page 1
 ID: D7uIPnW9?DzGk1HTFaFreyNCCCHkECd2otlbbdy5ZHcsN2uEs72DjS79bMOlvu3vh8zGcoq?
 1-3-8 0-0 3-10-3 3-10-3 6-1-5 9-11-7 6-0-1 15-11-8 6-0-1 21-11-9 6-1-5 28-0-13 3-10-3 1-3-8
 Scale = 1:50.3



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF	
C - F	2x6	DRY	No.2	SPF	
F - H	2x6	DRY	No.2	SPF	
H - J	2x6	DRY	No.2	SPF	
J - K	2x6	DRY	No.2	SPF	
K - L	2x6	DRY	No.2	SPF	
L - M	2x6	DRY	No.2	SPF	
M - N	2x6	DRY	No.2	SPF	
N - K	2x6	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2 12	TOP
C-F	2 12	SIDE (183.1)
F-H	2 12	SIDE (0.0)
H-J	2 12	TOP
J-K	2 12	TOP
K-L	2 12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S-P	2 12	TOP
P-N	2 12	SIDE (183.1)
N-K	2 12	SIDE (0.0)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-p	MT20	5.0	8.0	1.25 4.00
C TTVW+m	MT20	8.0	9.0	3.75 2.00
D TMVW-l	MT20	5.0	6.0	
E TMVW-w	MT20	3.0	6.0	
F TS-l	MT20	5.0	6.0	
G TMVW-l	MT20	5.0	6.0	
H TTVW+m	MT20	8.0	9.0	3.75 2.00
I TMVW-p	MT20	5.0	8.0	1.25 4.00
K BMV1+p	MT20	3.0	6.0	
L BMVW-l	MT20	5.0	6.0	2.50 2.25
M BMVW-l	MT20	6.0	9.0	3.00 4.25
N BS-l	MT20	6.0	9.0	
O BMVW-l	MT20	6.0	9.0	
P BS-l	MT20	6.0	9.0	
Q BMVW-l	MT20	6.0	9.0	3.00 4.25
R BMVW-l	MT20	5.0	8.0	2.50 2.25
S BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG	
JT VERT	4134	0	4134	0
S VERT	4134	0	4134	0
K VERT	4688	0	4688	0

UNFACTORED REACTIONS	MAX/MIN COMPONENT REACTIONS
1ST LCASE	
JT COMBINED	3080
S COMBINED	1780 / 0
K COMBINED	2033 / 0
JT LIVE	547 / 0
S LIVE	613 / 0
K LIVE	613 / 0
JT PERM. LIVE	0 / 0
S PERM. LIVE	0 / 0
K PERM. LIVE	0 / 0
JT WIND	733 / 0
S WIND	829 / 0
K WIND	829 / 0
JT DEAD	0 / 0
S DEAD	0 / 0
K DEAD	0 / 0
JT SOIL	0 / 0
S SOIL	0 / 0
K SOIL	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 44	-102.1	-102.1 0.04 (1)	R-C	-729 / 0
B-C	-4645 / 0	-102.1	-102.1 0.10 (1)	C-Q	0 / 5510
C-D	-4904 / 0	-102.1	-102.1 0.27 (1)	Q-D	-2629 / 0
D-E	-11425 / 0	-102.1	-102.1 0.35 (1)	D-Q	0 / 3615
E-F	-11425 / 0	-102.1	-102.1 0.43 (1)	Q-E	-804 / 0
F-T	-11425 / 0	-102.1	-102.1 0.43 (1)	T-Q	0 / 1897
T-U	-11425 / 0	-102.1	-102.1 0.43 (1)	Q-H	-1825 / 0
U-G	-11425 / 0	-102.1	-102.1 0.43 (1)	H-H	0 / 6653
G-H	-9840 / 0	-102.1	-102.1 0.31 (1)	H-L	-868 / 0
H-I	-5349 / 0	-102.1	-102.1 0.11 (1)	L-I	0 / 3621
I-J	0 / 44	-102.1	-102.1 0.04 (1)		
S-B	-4089 / 0	0.0	0.0 0.15 (1)		
K-I	-4857 / 0	0.0	0.0 0.17 (1)		
S-R	0 / 0	-38.5	-38.5 0.04 (2)		
R-Q	0 / 3683	-38.5	-38.5 0.20 (1)		
Q-P	0 / 8405	-38.5	-38.5 0.53 (1)		
P-V	0 / 8405	-38.5	-38.5 0.53 (1)		
V-O	0 / 8405	-38.5	-38.5 0.53 (1)		
O-W	0 / 9840	-38.5	-38.5 0.57 (1)		
W-N	0 / 9840	-38.5	-38.5 0.57 (1)		
N-X	0 / 9840	-38.5	-38.5 0.57 (1)		
X-M	0 / 9840	-38.5	-38.5 0.57 (1)		
M-L	0 / 4241	-38.5	-38.5 0.22 (1)		
L-K	0 / 0	-38.5	-38.5 0.04 (2)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	D/R	TOTAL
E	18-1-12	-123	-123	-	BACK	VERT	TOTAL
N	20-1-12	-55	-70	-	BACK	VERT	TOTAL
O	16-1-12	-55	-70	-	BACK	VERT	TOTAL
T	18-1-12	-123	-123	-	BACK	VERT	TOTAL
U	20-1-12	-123	-123	-	BACK	VERT	TOTAL
V	15-2-8	-1834	-1834	-	BACK	VERT	TOTAL
W	18-1-12	-55	-70	-	BACK	VERT	TOTAL
X	21-5-8	-1683	-1683	-	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 BOT CH. LL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/998 (0.25")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/937 (0.41")

CSI: TC=0.43/1.00 (E-G:1), EC=0.57/1.00 (M-O:1), WB=0.82/1.00 (H-M:1), SS=0.59/1.00 (O-Q:1)

DCL LUMBER=1.00 NAIL=1.00 LBS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

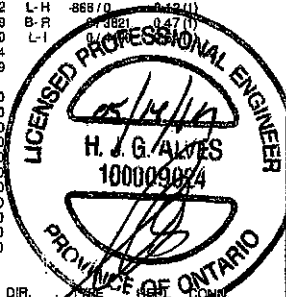
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1488

PLATE PLACEMENT TO MIN. 0.75 INCHES

PLATE ROTATION TO MIN. 0.25 DEG.

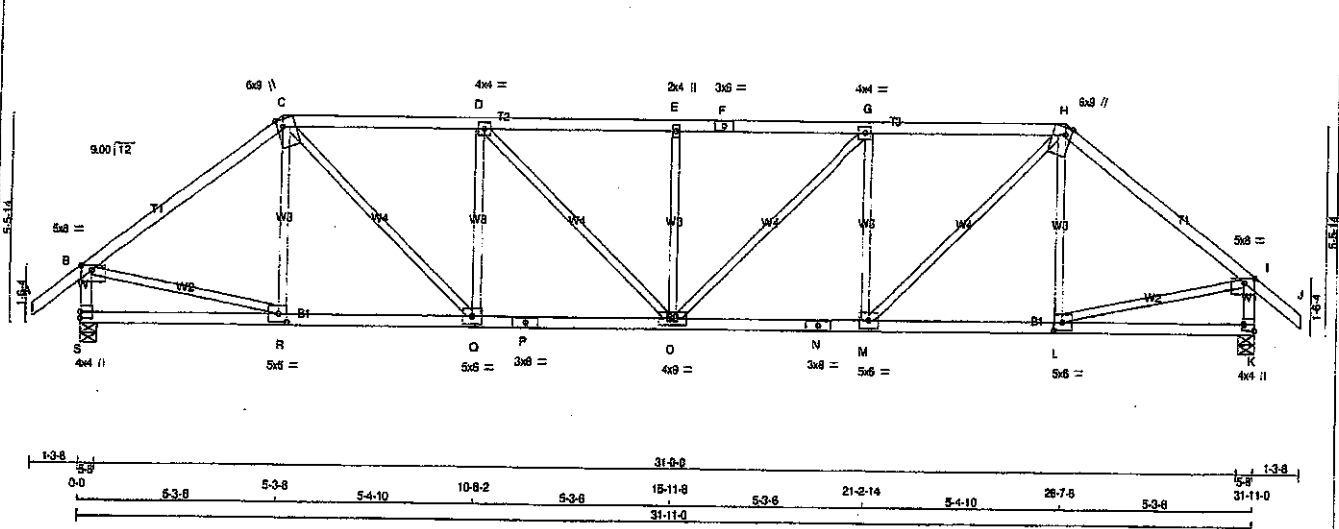
JSI GRIP = 0.88 (CONCRETE = 0.90)
 JSI METAL = 0.87 (STEEL = 1.00)

RECEIVED JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 DWG# T-1911500



JOB NAME 403045	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:21 2019 Page 1
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 1-3-8 0-0 5-3-8 5-3-8 5-4-10 10-8-2 5-3-8 15-11-8 5-3-8 21-2-14 5-4-10 26-7-8 5-3-8 31-11-0 33-2-8
 Scale = 1/32.3



LUMBER

N. L. Q. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
S - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TMWW-t	MT20	4.0	4.0		
E	TMW-w	MT20	2.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMWW-t	MT20	4.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
L	BMWW-t	MT20	5.0	8.0	2.50	2.75
M	BMWW-t	MT20	5.0	8.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BS-t	MT20	3.0	8.0		
Q	BMWW-t	MT20	5.0	8.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
VT	VERT	DOWN	HORIZ	UPLIFT
S	2385	0	2385	0
K	2385	0	2385	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS
VT	COMBINED	SNOW
S	1773	1007 / 0
K	1773	1007 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	(0.14 (1))	10.00	R-C	-195 / 98	
B-C	-2429 / 0	-102.1 -102.1	(0.73 (1))	3.67	Q-D	0 / 1687	
C-D	-3138 / 0	-102.1 -102.1	(0.85 (1))	3.34	Q-E	-861 / 0	
D-E	-3494 / 0	-102.1 -102.1	(0.89 (1))	3.14	D-O	0 / 500	
E-F	-3494 / 0	-102.1 -102.1	(0.89 (1))	3.14	O-E	-497 / 0	
F-G	-3494 / 0	-102.1 -102.1	(0.89 (1))	3.14	C-G	0 / 500	
G-H	-3138 / 0	-102.1 -102.1	(0.85 (1))	3.34	M-G	-861 / 0	
H-I	-2429 / 0	-102.1 -102.1	(0.73 (1))	3.67	M-H	0 / 1687	
I-J	0 / 42	-102.1 -102.1	(0.14 (1))	10.00	L-H	-195 / 98	
S-B	-2301 / 0	0.0 0.0	0.24 (1)	5.62	B-R	0 / 1987	
K-I	-2301 / 0	0.0 0.0	0.24 (1)	5.62	L-I	0 / 1987	
S-R	0 / 0	-38.5 -38.5	0.21 (3)	10.00			
R-Q	0 / 1936	-38.5 -38.5	0.47 (2)	10.00			
Q-P	0 / 3138	-38.5 -38.5	0.80 (1)	10.00			
P-O	0 / 3138	-38.5 -38.5	0.80 (1)	10.00			
O-N	0 / 3138	-38.5 -38.5	0.80 (1)	10.00			
N-M	0 / 3138	-38.5 -38.5	0.80 (1)	10.00			
M-L	0 / 1936	-38.5 -38.5	0.47 (2)	10.00			
L-K	0 / 1936	-38.5 -38.5	0.21 (3)	10.00			

A circular seal for a Professional Engineer. The outer ring contains the text "PROFESSIONAL" at the top and "ENGINEER" at the bottom. The center of the seal is blank.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.80/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (1.06")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.18")

ALLOWABLE DEFL. (TL) = $L/360$ (1.06")

CALCULATED VERT. DEFL. (TL) = $L/999$ (0.29")

CS1: TC=0.73/1.00 (H-E), BC=0.80/1.00 (O-C-1), WB=0.45/1.00 (L-I), SS=0.28/1.00 (O-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TEN=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687
	788	1587	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (L) (INPUT = 0.90)

JSI METAL=0.95 (F) (INPUT = 1.00)

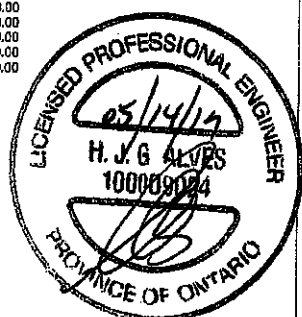
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JUN 13 2019

TOWN OF CALEDON

BUILDING SECTION

FILE NO.



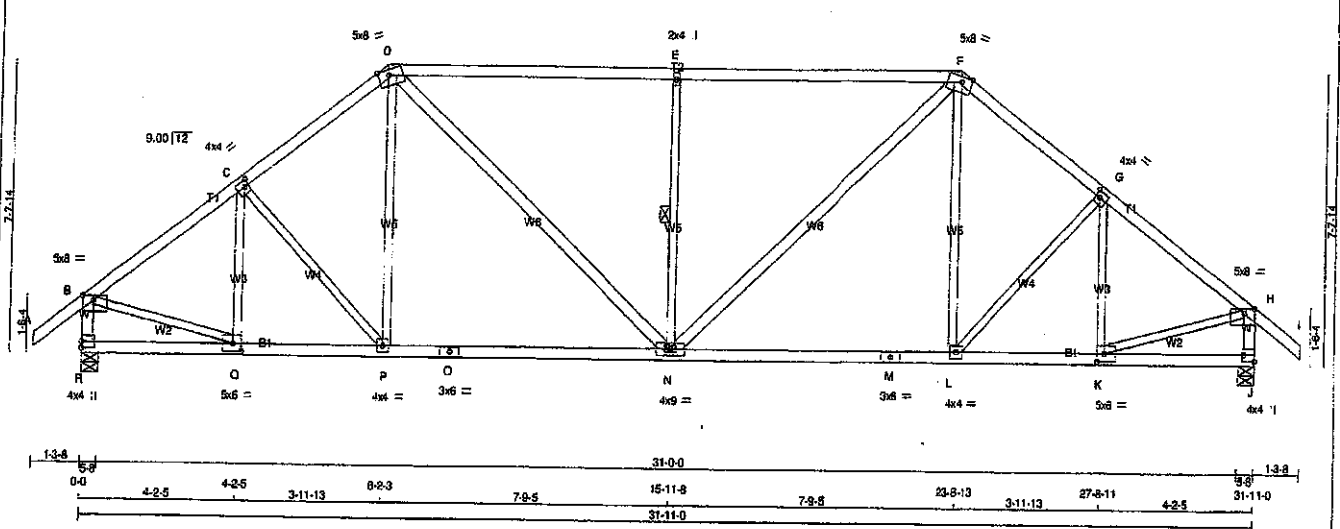
Structural component only

DWG# T-1911484

Structural component only
DWG# T-1911485

JOB NAME 403045	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 9.300 3 Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:23 2019 Page 1
 ID:07uIPnW9?DzGKx1HTFaFrayNCCH-L9QVD?_7WpyDyoV4dUvymVIT9Ms8MRw6nZSWzGdDg
 1-3-8 0-0 4-2-5 4-2-5 3-11-13 8-2-3 7-9-5 15-11-8 7-9-5 23-8-13 3-11-13 27-8-11 4-2-5 31-11-0 1-3-8
 Scale = 1:50.0



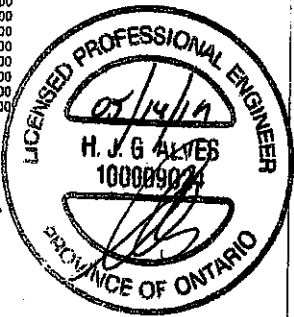
LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 147 = 294 lb									
N. L. G. A. RULES										BEARINGS										DESIGN CRITERIA									
CHORDS										FACTORED GROSS REACTION										SPECIFIED LOADS:									
SIZE										MAXIMUM FACTORED GROSS REACTION										TOP CH. LL = 29.0 PSF									
LUMBER										INPUT BRG										DL = 6.0 PSF									
DESCR.										REQD BRG										BOT CH. LL = 10.5 PSF									
A - D										JT										DL = 7.0 PSF									
D - F										R										TOTAL LOAD = 52.5 PSF									
F - I										J																			
R - B																													
J - H																													
R - O																													
O - M																													
M - J																													
										UNFACTORED REACTIONS																			
										1ST LCASE MAX/MIN COMPONENT REACTIONS																			
ALL WEBS EXCEPT										JT COMBINED										LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12									
D - N										R										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR									
N - F										J																			
DRY: SEASONED LUMBER.										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J																			

PLATES (table is in inches)				BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J			
JT TYPE	PLATES	W	LEN	Y	X	1ST CASE	MAX. MIN. COMPONENT REACTIONS
B TMWV-p	MT20	5.0	8.0	1.50	3.50	JT	COMBINED
C TMWV-t	MT20	4.0	4.0	2.00	1.50	R	1773
D TTWV-m	MT20	5.0	8.0	Edge	3.50	J	1773
E TMW-w	MT20	2.0	4.0				
F TTWV-m	MT20	5.0	8.0	Edge	3.50		
G TMWV-t	MT20	4.0	4.0	2.00	1.50		
H TMWV-p	MT20	5.0	8.0	1.50	3.50		
J BMV1-p	MT20	4.0	4.0	2.00	Edge		
K BMWV-t	MT20	5.0	6.0	2.50	2.50		
L BMWV-t	MT20	4.0	4.0				
M BS-t	MT20	3.0	6.0				
N BMWV-t	MT20	4.0	4.0				
O BS-t	MT20	3.0	6.0				
P BMWV-t	MT20	4.0	4.0				
Q BMWV-t	MT20	5.0	6.0	2.50	2.50		
R BMV1-p	MT20	4.0	4.0				

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED (PL)	MAX. FACTORED (PL)	MEMB.	FORCE (LBS)	MAX. FACTORED (PL)
FR-TO					FR-TO		
A-B	0/42	-102.1	-102.1	0.14 (1)	Q-C	-410/0	0.13 (1)
S-C	-2391/0	-102.1	-102.1	0.29 (1)	C-P	-89/0	0.05 (1)
C-D	-2377/0	-102.1	-102.1	0.28 (1)	P-D	0/353	0.08 (2)
D-E	-2581/0	-102.1	-102.1	1.00 (1)	D-N	0/939	0.15 (1)
E-F	-2581/0	-102.1	-102.1	1.00 (1)	N-E	-977/0	0.36 (1)
F-G	-2377/0	-102.1	-102.1	0.28 (1)	N-F	0/939	0.15 (1)
G-H	-2391/0	-102.1	-102.1	0.29 (1)	L-F	0/353	0.08 (2)
H-I	0/42	-102.1	-102.1	0.14 (1)	L-G	-89/0	0.05 (1)
R-B	-2311/0	0.0	0.0	0.24 (1)	K-G	-410/0	0.13 (1)
J-H	-2311/0	0.0	0.0	0.24 (1)	B-Q	0/2012	0.45 (1)
R-Q	0/0	-38.5	-38.5	0.12 (3)	K-H	0/2012	0.45 (1)
Q-P	0/1935	-38.5	-38.5	0.50 (2)			
P-O	0/1879	-38.5	-38.5	0.62 (2)			
O-N	0/1879	-38.5	-38.5	0.62 (2)			
N-M	0/1879	-38.5	-38.5	0.62 (2)			
M-L	0/1879	-38.5	-38.5	0.62 (2)			
L-K	0/1879	-38.5	-38.5	0.50 (2)			
K-J	0/0	-38.5	-38.5	0.12 (3)			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH. LL = 29.0 PSF			
DL = 6.0 PSF			
BOT CH. LL = 10.5 PSF			
DL = 7.0 PSF			
TOTAL LOAD = 52.5 PSF			
SPACING = 24.0 IN. CC			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF CBC 2018, OBC 2012			
- CSA 086-09, CSA 086-14			
- TPIC 2011, TPIC 2014			
(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL) = L/360 (1.06")			
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")			
ALLOWABLE DEFL.(TL) = L/360 (1.06")			
CALCULATED VERT. DEFL.(TL) = L/999 (0.23")			
CSI: TC=1.00/1.00 (E-F:1) , SC=0.62/1.00 (N-P:2) , WB=0.45/1.00 (B-Q:1) , SS=0.39/1.00 (E-F:1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
COMP=1.10 SHEAR=1.10 TENS=1.10			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .			
NAIL VALUES			
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)			
MAX MIN MAX MIN MAX MIN			
MT20 618 354 1687 788 1987 1688			
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP= 0.89 (K) (INPUT = 0.90)			
JSI METAL= 0.65 (C) (INPUT = 1.00)			

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 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO. _____



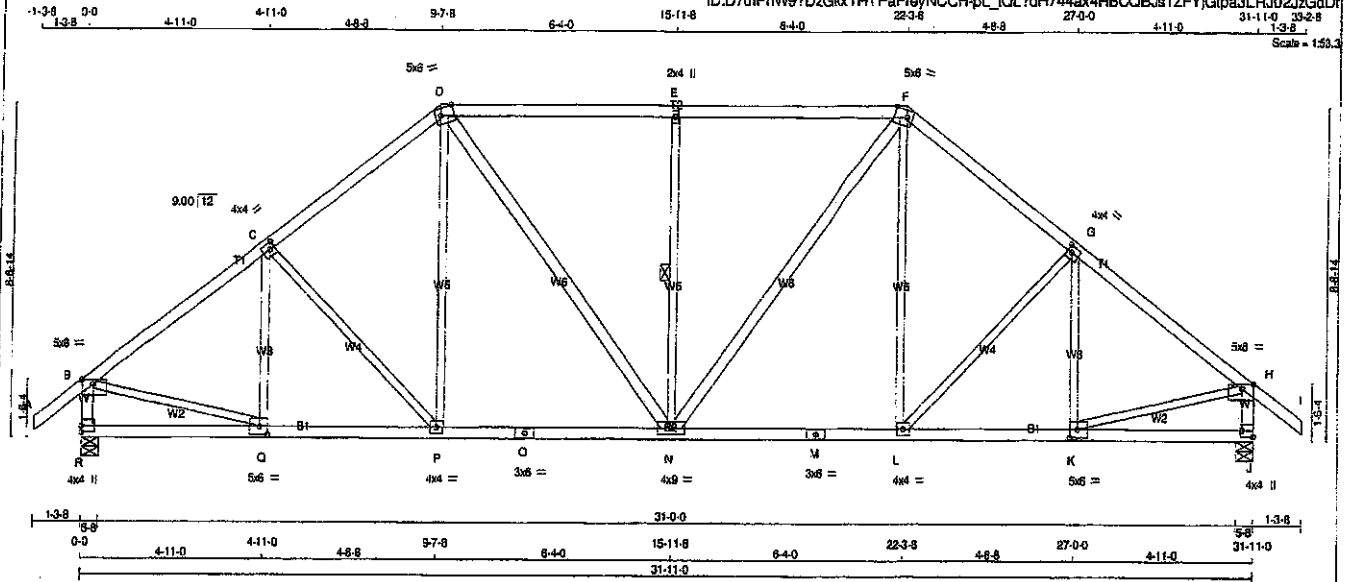
Structural component only
 DWG# T-1911486

JOB NAME 403045	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:D7uIPnW9?DzGkx1HTFaFreyNCCH-pL_1QL?dH744ax4HBCOB.Js1ZFjGtpa3LRJ02JzGdDr



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	4.25
E	TMWW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	4.25
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	8.0		
N	BMWWWW-t	MT20	4.0	8.0		
O	BS-t	MT20	3.0	8.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG
VERT		DOWN	UP
HT	2385	0	0
R	2385	0	0
J	2385	0	0

UNFACTORED REACTIONS			
JT	1ST LOASE	MAX/MIN. COMPONENT REACTIONS	
COMBINED			
R	1773	1007 / 0	335 / 0
J	1773	1007 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

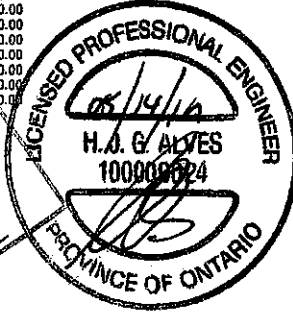
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)
B-C	-2445 / 0	-102.1	-102.1 0.40 (1)
C-D	-2286 / 0	-102.1	-102.1 0.38 (1)
D-E	-2193 / 0	-102.1	-102.1 0.81 (1)
E-F	-2193 / 0	-102.1	-102.1 0.81 (1)
F-G	-2286 / 0	-102.1	-102.1 0.38 (1)
G-H	-2445 / 0	-102.1	-102.1 0.40 (1)
H-I	0 / 42	-102.1	-102.1 0.14 (1)
I-J	-2304 / 0	0.0	0.0 0.24 (1)
J-K	-2304 / 0	0.0	0.0 0.24 (1)
K-L	0 / 0	-38.5	-38.5 0.15 (3)
L-M	0 / 1993	-38.5	-38.5 0.45 (1)
M-N	0 / 1802	-38.5	-38.5 0.49 (2)
N-O	0 / 1802	-38.5	-38.5 0.48 (2)
O-P	0 / 1802	-38.5	-38.5 0.49 (2)
P-Q	0 / 1802	-38.5	-38.5 0.49 (2)
Q-R	0 / 1993	-38.5	-38.5 0.45 (1)
R-S	0 / 0	-38.5	-38.5 0.15 (3)



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN LC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OSC 2012
- CSA 088-09, CSA 088-14
- TFC 2011, TFC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.61/1.00 (D-E:1), BC=0.49/1.00 (N-P:2), WB=0.48/1.00 (H-K:1), SS=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1667 1656

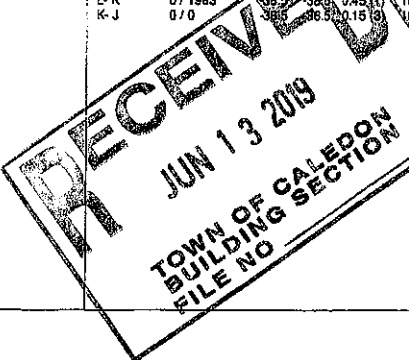
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)

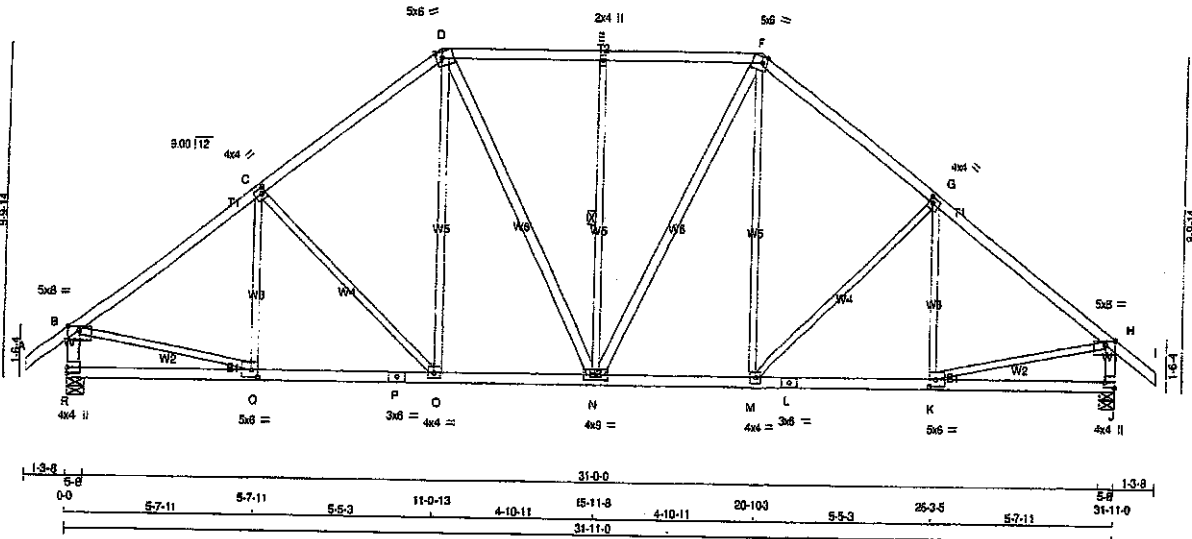
JSI METAL= 0.68 (C) (INPUT = 1.00)

Structural component only
DWG# T-1911487



JOB NAME 403045	TRUSS NAME T6	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 5 Apr 23 2019 Mitek Industries, Inc. Tue May 14 16:41:25 2019 Page 1
 ID: D7uIPnW9?DzGk1HTFaFreyNCCH+HXyGeh0F2RCxB5TtyQr3aI6y3XcFxCZ52ZamzGdDe
 1-3-8 0-0 5-7-11 5-7-11 5-5-3 11-0-13 4-10-11 15-11-8 4-10-11 20-103 5-5-3 25-3-5 5-7-11 31-11-0 1-3-8
 Scale = 1:50.0



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - P	2x4	DRY	No.2
P - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	5.0	Edge	2.00
E	TMVW-m	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
R	2385	0	2385	0	0	5-8	5-8	5-8	5-8
J	2385	0	2385	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/AVG COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	1773	1007/0	335/0	0/0	0/0
J	1773	1007/0	335/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.09 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	CSH (LC)	MEMB.	FORCE (LBS)	VERT. LOAD	CSH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0/42	-102.1 -102.1	0.14 (1)	Q-C	-185/119	0.09 (1)	
B-C	-2474/0	-102.1 -102.1	0.54 (1)	C-D	-422/0	0.52 (1)	
C-D	-2181/0	-102.1 -102.1	0.48 (1)	D-E	0/498	0.11 (2)	
D-E	-1916/0	-102.1 -102.1	0.35 (1)	E-F	0/440	0.07 (1)	
E-F	-1916/0	-102.1 -102.1	0.35 (1)	F-G	-803/0	0.39 (1)	
F-G	-2181/0	-102.1 -102.1	0.48 (1)	G-H	0/440	0.07 (1)	
G-H	-2474/0	-102.1 -102.1	0.54 (1)	H-I	0/498	0.11 (2)	
H-I	0/42	-102.1 -102.1	0.14 (1)	I-J	-422/0	0.52 (1)	
I-J	-2296/0	0.0 0.0	0.24 (1)	J-K	-185/119	0.09 (1)	
J-K	-2296/0	0.0 0.0	0.24 (1)	K-L	0/2055	0.46 (1)	
K-L	0/0	-38.5 -38.5	0.22 (3)	L-M	0/2055	0.46 (1)	
L-M	0/2011	-38.5 -38.5	0.48 (2)	M-N	0/2011	-38.5 -38.5	0.48 (2)
M-N	0/2011	-38.5 -38.5	0.48 (2)	N-O	0/1715	-38.5 -38.5	0.39 (1)
N-O	0/1715	-38.5 -38.5	0.39 (1)	O-P	0/2011	-38.5 -38.5	0.48 (2)
O-P	0/2011	-38.5 -38.5	0.48 (2)	P-Q	0/2011	-38.5 -38.5	0.48 (2)
P-Q	0/2011	-38.5 -38.5	0.48 (2)	Q-R	0/0	-38.5 -38.5	0.22 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2018, OBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSL: TC=0.54/1.00 (B-C:1), BC=0.48/1.00 (O-Q:2), WB=0.52/1.00 (G-O:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

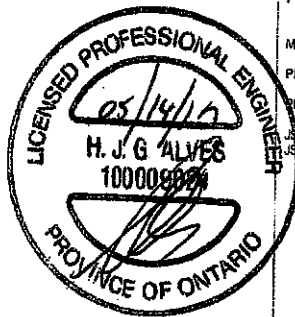
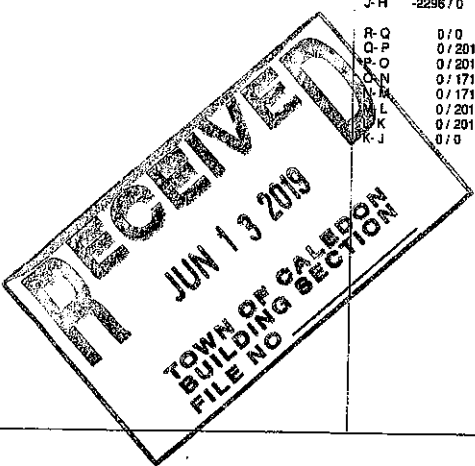
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLJ) (PLJ)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

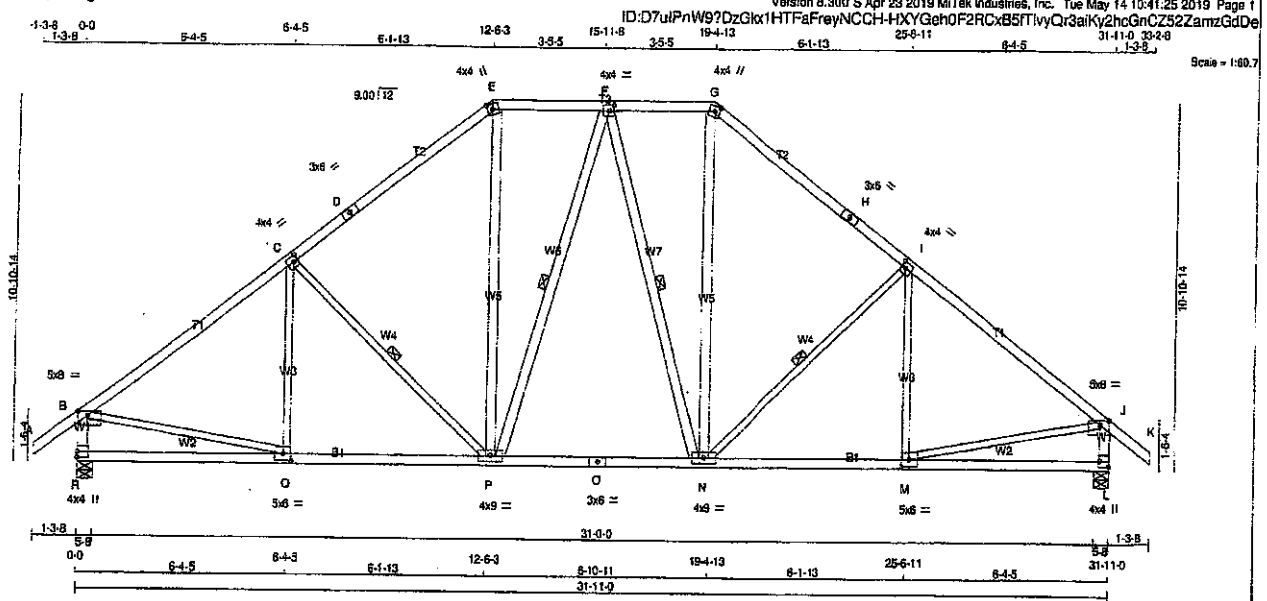
PLATE PLACEMENT TOL. = 0.250 inches
 PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.85 (B) (INPUT = 0.90)
 METAL = 0.60 (P) (INPUT = 1.00)



Structural component only
 DWG# T-1911488

JOB NAME 403045	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
P - E	2x4	DRY	No.2
P - F	2x4	DRY	No.2
F - N	2x4	DRY	No.2
N - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TS-l	MT20	3.0	8.0		
E	TTW+m	MT20	4.0	4.0	2.00	1.75
F	TMVW-l	MT20	4.0	4.0	2.00	1.75
G	TTW+m	MT20	4.0	4.0	2.00	1.75
H	TS-l	MT20	3.0	8.0		
I	TMVW-l	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV-l-p	MT20	4.0	4.0	2.00	Edge
M	BMVW-l	MT20	5.0	8.0	2.50	2.75
N	BMVW-l	MT20	4.0	4.0	9.0	
O	BS-l	MT20	3.0	6.0		
P	BMVW-l	MT20	4.0	4.0	9.0	
Q	BMVW-l	MT20	5.0	8.0	2.50	2.75
R	BMV-l-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
R	2385	0	2385	0	5-8
L	2385	0	2385	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PL)	MAX. CSI (LC)	MEMB.	MAX. FORCE (LBS)	MAX. CSI (LC)	UNBRACED LENGTH
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-111 / 174
B-C	-2481 / 0	-102.1	-102.1	0.72 (1)	3.65	C-P	-559 / 0
C-D	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	P-E	0 / 832
D-E	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	F-F	-212 / 0
E-F	-1827 / 0	-102.1	-102.1	0.16 (1)	5.04	F-N	-216 / 0
F-G	-1827 / 0	-102.1	-102.1	0.16 (1)	5.04	N-G	0 / 837
G-H	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	N-I	-559 / 0
H-I	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	M-I	-111 / 174
I-J	-2481 / 0	-102.1	-102.1	0.72 (1)	3.65	S-Q	0 / 2057
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	M-J	0 / 2057
R-B	-2284 / 0	0.0	0.0	0.24 (1)	5.84		
L-J	-2284 / 0	0.0	0.0	0.24 (1)	5.84		
R-Q	0 / 0	-38.5	-38.5	0.28 (3)	10.00		
O-P	0 / 2022	-38.5	-38.5	0.54 (2)	10.00		
P-O	0 / 1891	-38.5	-38.5	0.46 (2)	10.00		
O-N	0 / 1891	-38.5	-38.5	0.46 (2)	10.00		
N-M	0 / 2022	-38.5	-38.5	0.54 (2)	10.00		
M-L	0 / 0	-38.5	-38.5	0.28 (3)	10.00		

TOTAL WEIGHT = 2 X 167 = 335 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 089-09, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.18")

CSI: TC=0.72/1.00 (B-C:1), BC=0.54/1.00 (P-Q:2), WB=0.48/1.00 (B-Q:1), SS=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

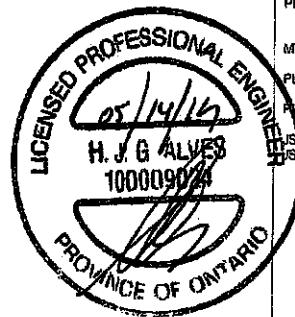
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1867

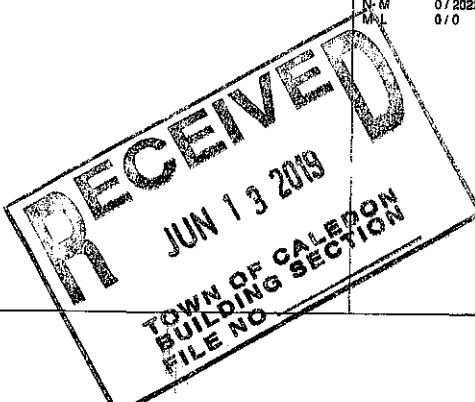
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP=0.88 (D) (INPUT = 0.90)
METAL=0.69 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911489



Version 8.300 S Apr 23 2019 MiTek Industries, Inc. Tue May 14 10:41:28 2019 Page 1
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Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

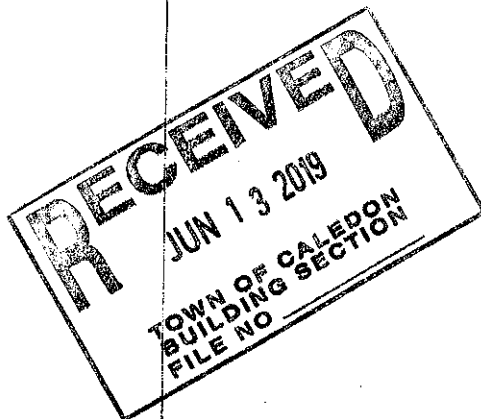
LOADING
TOTAL LOAD CASES: (4)

FACTORED CONCENTRATED LOADS (LBS)				FACE	DIR.	TYPE	HEEL	CONF.
LOC.	LC1	MAX.	MAX.					
3-10-3	-49	-55	---	FRONT	VERT	DEAD	---	---
3-10-3	-282	-282	---	FRONT	VERT	SNOW	---	---
10-10-13	-49	-55	---	FRONT	VERT	DEAD	---	---
10-10-13	-129	-129	---	FRONT	VERT	TOTAL	---	---
10-10-13	-282	-282	---	FRONT	VERT	SNOW	---	---
5-0-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
7-0-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
9-0-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
1-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
3-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
5-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
7-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
9-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---

DWG# T-1911492 CONTINUED ON PAGE 2

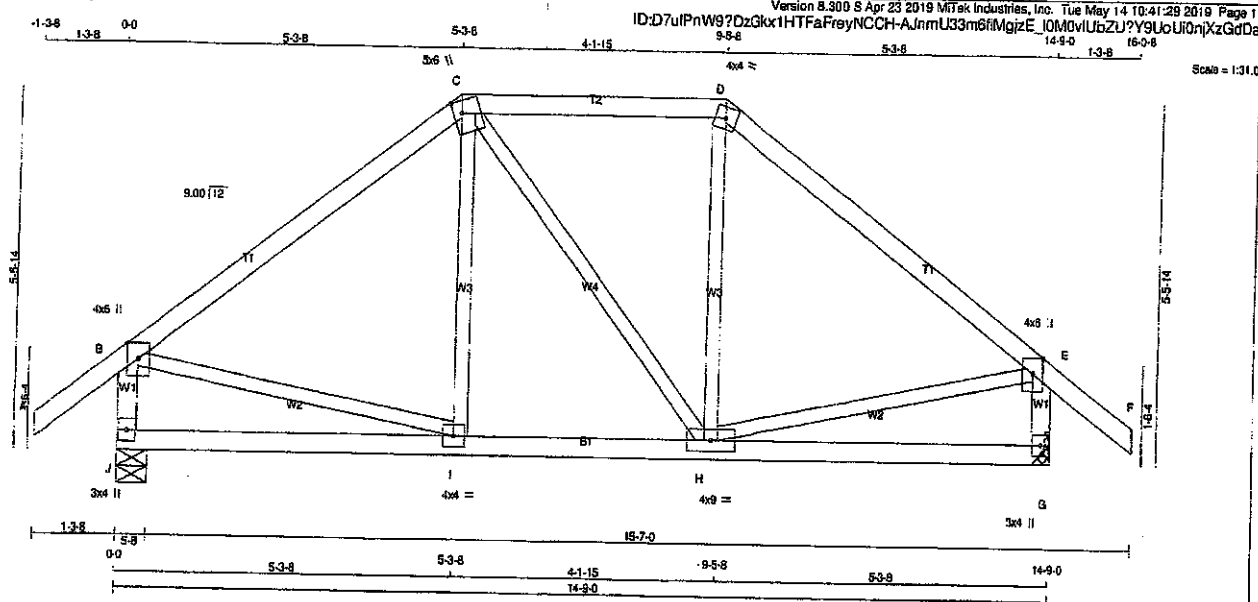
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T10	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.300 9 Apr 23 2019 Mittek Industries, Inc. Tue May 14 10:41:28 2019 Page 2	
ID:D7uIPnW9?DzGkx1HTFaFevNCCH-i6DOGI28LLaV2ZO2Q2V7TICIA6IpeIF2HD85z3dDb					

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	11-9-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
U	13-9-12	-55	-70	--	FRONT	VERT	TOTAL	--	--



Structural component only
DWG# T-1911492

JOB NAME 403045	TRUSS NAME T11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 65 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
J - B	2x4	DRY	No.2
G - E	2x4	DRY	No.2
J - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMW+p	MT20	4.0	6.0	Edge
C	TTW+m	MT20	5.0	6.0	2.25 1.50
D	TTW+m	MT20	4.0	4.0	
E	TMW+p	MT20	4.0	5.0	Edge
G	BMW+p	MT20	3.0	4.0	
H	BMW+m	MT20	4.0	5.0	
I	BMW+p	MT20	4.0	4.0	
J	BMW+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
J	1178	0	1178	0	0	5-8	5-8
G	1178	0	1178	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 3'-8".

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
J	872	508/0	155/0	0/0	0/0	208/0
G	872	508/0	155/0	0/0	0/0	208/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.05 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	
FR-TO		FROM TO	CSI (LC)	UNBRAC LENGTH	FR-TO		
A-B	0/42	-102.1 -102.1	0.14 (1)	10.00	F-C	0/174	0.04 (3)
B-C	-872/0	-102.1 -102.1	0.38 (1)	6.05	C-H	0/4	0.00 (3)
C-D	-899/0	-102.1 -102.1	0.38 (1)	6.25	H-D	0/177	0.04 (3)
D-E	-873/0	-102.1 -102.1	0.38 (1)	6.05	B-I	0/713	0.16 (1)
E-F	0/42	-102.1 -102.1	0.14 (1)	10.00	H-E	0/714	0.16 (1)
J-B	-1080/0	0.0 0.0	0.11 (1)	7.57			
G-E	-1089/0	0.0 0.0	0.11 (1)	7.57			
J-I	0/0	-38.5 -38.5	0.18 (3)	10.00			
I-H	0/897	-38.5 -38.5	0.26 (2)	10.00			
H-G	0/0	-38.5 -38.5	0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.38/1.00 (D-E:1), BC=0.28/1.00 (H-I:2), WB=0.16/1.00 (E-H:1), SS=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

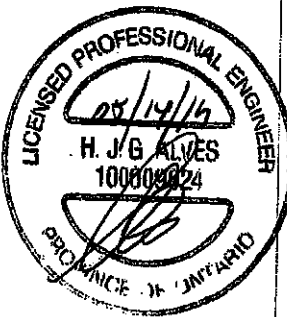
NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRP=0.82 (I) (INPUT=0.90)
SI METAL=0.49 (E) (INPUT=1.00)

RECEIVED
JUN 13 2019
TOWN OF CALLETON
BUILDING SECTION
FILE NO.

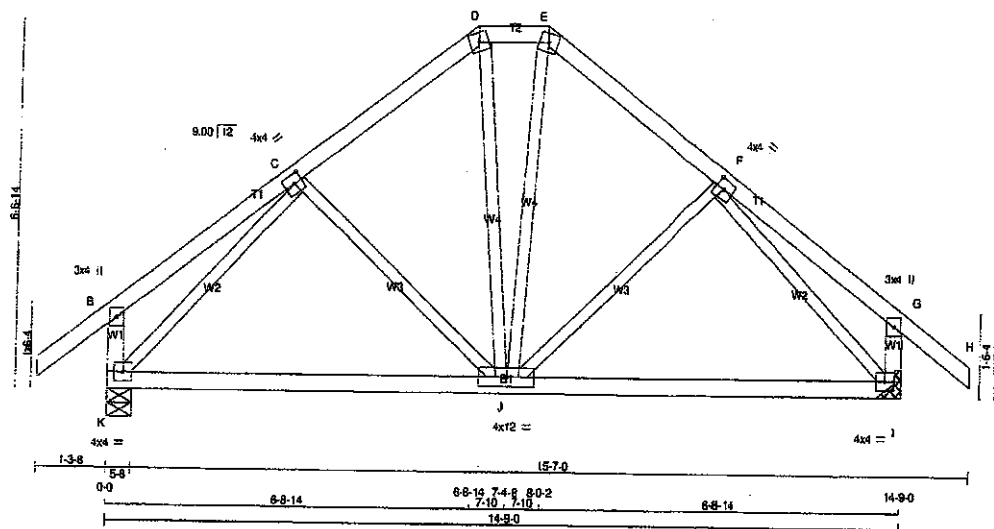


Structural component only
DWG# T-1911493

JOB NAME 403045	TRUSS NAME T12	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: D7ulPnW97DzGkx1HTFaFreyNCCH-eVL9hO3OtzaDIsYQVTXbY7HITzmoHXZxjMmKGzzGdDZ
11-3-5 14-9-0 15-0-8
Scale = 1:35



TOTAL WEIGHT = 70 lb (MIP)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
K - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
K - I	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-t	MT20	4.0	4.0	2.00	1.75
D	TTW-m	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	4.0	4.0	2.00	1.75
G	TMV+p	MT20	3.0	4.0		
I	BMVW1-t	MT20	4.0	4.0		
J	BMVW1-t	MT20	4.0	12.0		
K	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
K	1178	0	1178	0	0
I	1178	0	1178	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 5-8.

UNFACTORED REACTIONS

1ST CASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
K	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0
I	872	508 / 0	155 / 0	0 / 0	0 / 0	208 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
A-B	0 / 42	C-J	-137 / 33
B-C	0 / 23	D-E	-137 / 33
C-D	-800 / 0	E-F	-1074 / 0
D-E	-847 / 0	F-G	-1074 / 0
E-F	-800 / 0	G-H	0 / 275
F-G	0 / 23	H-I	0 / 275
G-H	0 / 42	I-J	0 / 275
H-I	-273 / 0	J-K	0 / 275
I-J	-273 / 0	K-L	0 / 275



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.18/1.00 (B-C:1), BC=0.54/1.00 (J-K:2), WE=0.43/1.00 (C-K:1), SS=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(FS) (PL) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

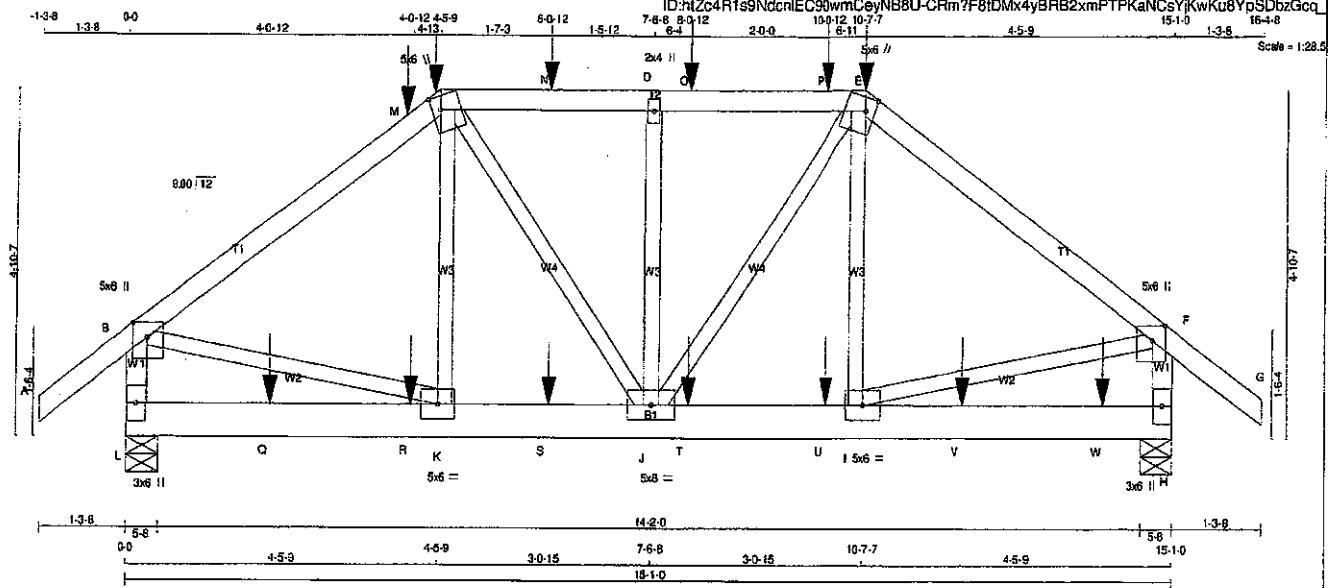
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (K) (INPUT = 0.90)
JSI METAL= 0.39 (F) (INPUT = 1.00)

Structural component only
DWG# T-1911494

JOB NAME 403059	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:08:47 2019 Page 1	



TOTAL WEIGHT = 78 lb

LUMBER			
N.L.G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50 2.25
C	TTWW+m	MT20	5.0	6.0	2.25 1.50
D	TMVW+p	MT20	2.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.25 1.50
F	TMVW+p	MT20	5.0	6.0	2.50 2.25
H	BMV1+p	MT20	3.0	6.0	
I	BMVW+1	MT20	5.0	6.0	
J	BMVW+1	MT20	5.0	6.0	
K	BMVW+1	MT20	5.0	6.0	
L	BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	REQD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT
JT	1772	0	1772	0	0
H	1759	0	1759	0	0

UNFACTORED REACTIONS					
1ST LCASE	MAX/MIN	COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
L	1307	776 / 0	223 / 0	0 / 0	0 / 0
H	1302	758 / 0	232 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORZ. LOAD (LBS)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-102.1 -102.1	0.15 (1)	10.00	K-C	-50 / 195	0.05 (3)
B-M	-1588 / 0	-102.1 -102.1	0.51 (1)	4.57	C-J	0 / 374	0.09 (1)
M-C	-1598 / 0	-102.1 -102.1	0.51 (1)	4.57	J-D	-551 / 0	0.19 (1)
C-N	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	J-E	0 / 404	0.10 (1)
N-D	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	I-E	-40 / 201	0.05 (3)
D-O	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	B-K	0 / 1388	0.33 (1)
O-P	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	I-F	0 / 1321	0.33 (1)
P-E	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99			
E-F	-1607 / 0	-102.1 -102.1	0.47 (1)	4.63			
F-G	0 / 42	-102.1 -102.1	0.15 (1)	10.00			
L-B	-1877 / 0	0.0	0.0	0.19 (1)	8.38		
H-F	-1648 / 0	0.0	0.0	0.19 (1)	6.41		
L-O	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
Q-R	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
R-K	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
K-S	0 / 1300	-38.5	-38.5	0.22 (1)	10.00		
S-J	0 / 1300	-38.5	-38.5	0.22 (1)	10.00		
J-T	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
T-U	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
U-I	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
I-V	0 / 0	-38.5	-38.5	0.10 (3)	10.00		
V-W	0 / 0	-38.5	-38.5	0.10 (3)	10.00		
W-H	0 / 0	-38.5	-38.5	0.10 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)							
JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE
C	4-5-9	-33	-36		FRONT	VERT	DEAD
C	4-5-9	-188	-188		FRONT	VERT	SNOW
E	10-7-7	-33	-36		FRONT	VERT	DEAD
E	10-7-7	-188	-188		FRONT	VERT	SNOW
M	4-0-12	-128	-128		BACK	VERT	TOTAL
N	6-0-12	-96	-96		BACK	VERT	TOTAL
O	8-0-12	-96	-96		BACK	VERT	TOTAL
P	10-0-12	-118	-118		BACK	VERT	TOTAL
Q	2-0-12	-36	-46		BACK	VERT	TOTAL
R	4-0-12	-36	-46		BACK	VERT	TOTAL
S	6-0-12	-36	-46		BACK	VERT	TOTAL
T	8-0-12	-36	-46		BACK	VERT	TOTAL
U	10-0-12	-36	-46		BACK	VERT	TOTAL
V	12-0-12	-36	-46		BACK	VERT	TOTAL
W	14-0-12	-36	-46		BACK	VERT	TOTAL

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/599 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TC=0.51/1.00 (B-C:1), BC=0.22/1.00 (I-J:1),
WB=0.33/1.00 (B-K:1), SS=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

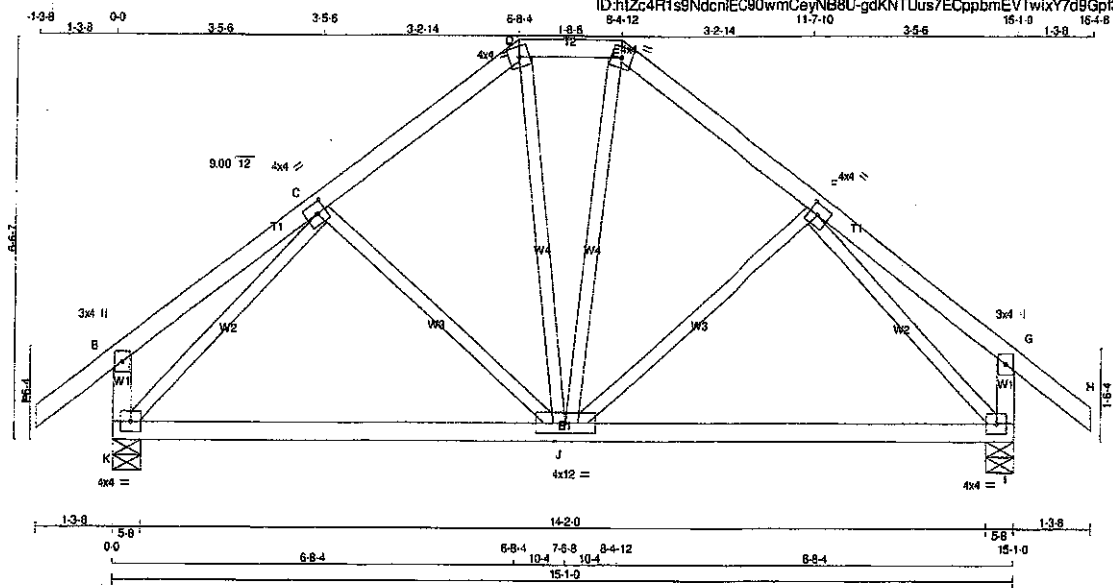
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.32 (I) (INPUT = 1.00)

Structural component only

DWG# T-1911501 CONTINUED ON PAGE 2

JOB NAME 403059	TRUSS NAME T14	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	CRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 71 lb (M/F)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
K - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
K - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	X
B	TMAV-p	MT20	3.0	4.0	
C	TMAVW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMAVW-t	MT20	4.0	4.0	2.00 1.75
G	TMAV-p	MT20	3.0	4.0	
I	BMVWV-t	MT20	4.0	4.0	
J	BMVWVW-t	MT20	4.0	12.0	
K	BMVWV-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
K	VERT	DOWN	DOWN	IN-SX	IN-SX
I	VERT	DOWN	DOWN	IN-SX	IN-SX

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
K	889	518/0	158/0	0/0	0/0	213/0	0/0	
I	889	518/0	158/0	0/0	0/0	213/0	0/0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0/42	C-J	-128/36
B-C	0/23	J-F	-128/36
C-D	-828/0	K-C	-1103/0
D-E	-879/0	F-I	-1103/0
E-F	-828/0	D-J	0/275
F-G	0/23	J-E	0/275
G-H	0/42		
K-B	-272/0		
I-G	-272/0		
K-J	0/739		
	0/739		

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.18/1.00 (B-C:1), BC=0.56/1.00 (F-J:2), WB=0.43/1.00 (C-K:1), SSI=0.19/1.00 (J-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

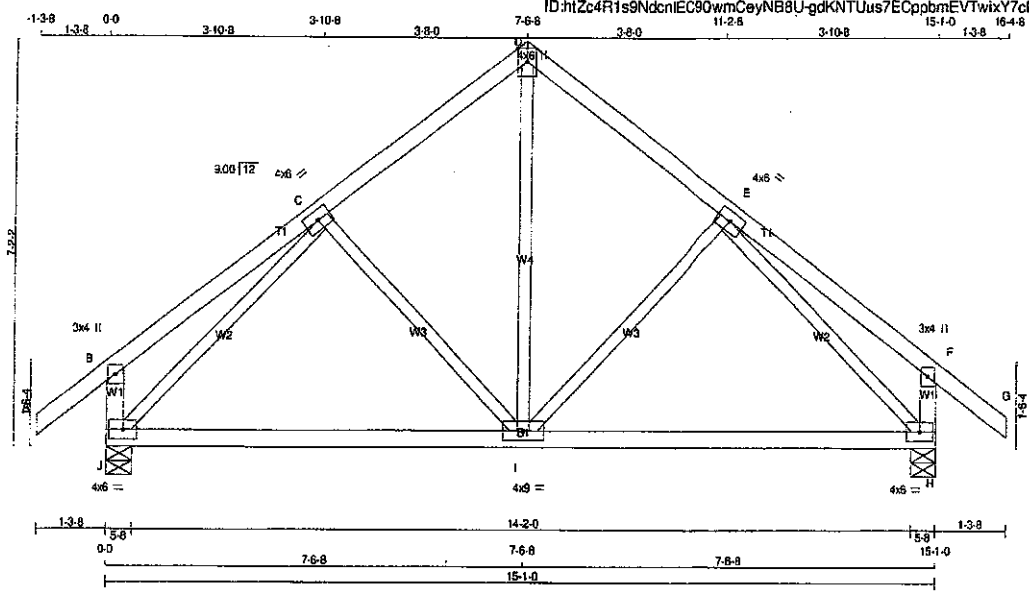
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.40 (F) (INPUT = 1.00)

Structural component only
DWG# T-1911502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403059	T15	5	1	GREEN PARK HOMES	

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LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
C	TMWW-i	MT20	4.0	6.0
D	TTW+p	MT20	4.0	6.0
E	TMWW-i	MT20	4.0	6.0
F	TMV+p	MT20	3.0	4.0
H	BMVW-i	MT20	4.0	6.0
I	BMWWW-i	MT20	4.0	9.0
J	BMVW-i	MT20	4.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED			INPUT	REQD
		GROSS REACTION		GROSS REACTION			BRG	BRG
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1201	0		1201	0	0	5-8	5-8
H	1201	0		1201	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE		MAX/MIN COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
J	889	518/0	158/0	0/0	0/0
H	889	518/0	158/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO			
A-B	0/42	-102.1	-102.1
B-C	0/26	-102.1	-102.1
C-D	-804/0	-102.1	-102.1
D-E	-804/0	-102.1	-102.1
E-F	0/26	-102.1	-102.1
F-G	0/42	-102.1	-102.1
J-B	-288/0	0.0	0.0
H-F	-288/0	0.0	0.0
J-I	0/752	-38.5	-38.5
I-H	0/752	-38.5	-38.5

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/989 (0.16")

CSI: TC=0.23/1.00 (E-F-1), BC=0.56/1.00 (I-J-2), WB=0.54/1.00 (E-H-1), SS=0.19/1.00 (I-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

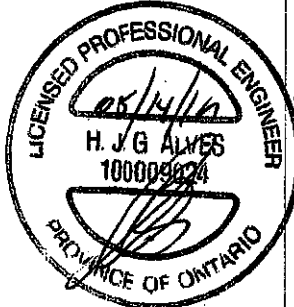
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.65 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO



Structural component only
DWG# T-1911503

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TOTAL WEIGHT = 48 lb

DRY; SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

BEARINGS

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 5'-8".

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

I-K	0/0	-38.5	-38.5	0.35 (3)	10.00
K-L	0/0	-38.5	-38.5	0.35 (3)	10.00
L-H	0/0	-38.5	-38.5	0.35 (3)	10.00
H-M	0/0	-38.5	-38.5	0.35 (3)	10.00
M-N	0/0	-38.5	-38.5	0.35 (3)	10.00
N-G	0/0	-38.5	-38.5	0.35 (3)	10.00

FACTORED CONCENTRATED LOADS (LBS)									
JOINT	LOC.	LC1	MAX	FACE	DIR.	TYPE	HEEL	CONN.	
UT	4-5-9	-33	-35	---	FRONT	VERT	DEAD	---	
CT	4-5-9	-189	-189	---	FRONT	VERT	SNOW	---	
CD	5-10-7	-33	-35	---	FRONT	VERT	DEAD	---	
CH	5-10-7	-189	-189	---	FRONT	VERT	SNOW	---	
J	5-0-12	-36	-46	---	FRONT	VERT	TOTAL	---	
K	5-0-12	-126	-126	---	FRONT	VERT	TOTAL	---	
L	1-0-12	-36	-46	---	FRONT	VERT	TOTAL	---	
M	3-0-12	-36	-46	---	FRONT	VERT	TOTAL	---	
N	7-0-12	-36	-46	---	FRONT	VERT	TOTAL	---	
	9-0-12	-36	-46	---	FRONT	VERT	TOTAL	---	

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY
USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL
LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.42/1.00 (B-C:1), BC=0.35/1.00 (G-H:3),
WB=0.20/1.00 (B-H:1), SS=0.19/1.00 (G-H:3)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	GRIP (DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1667	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

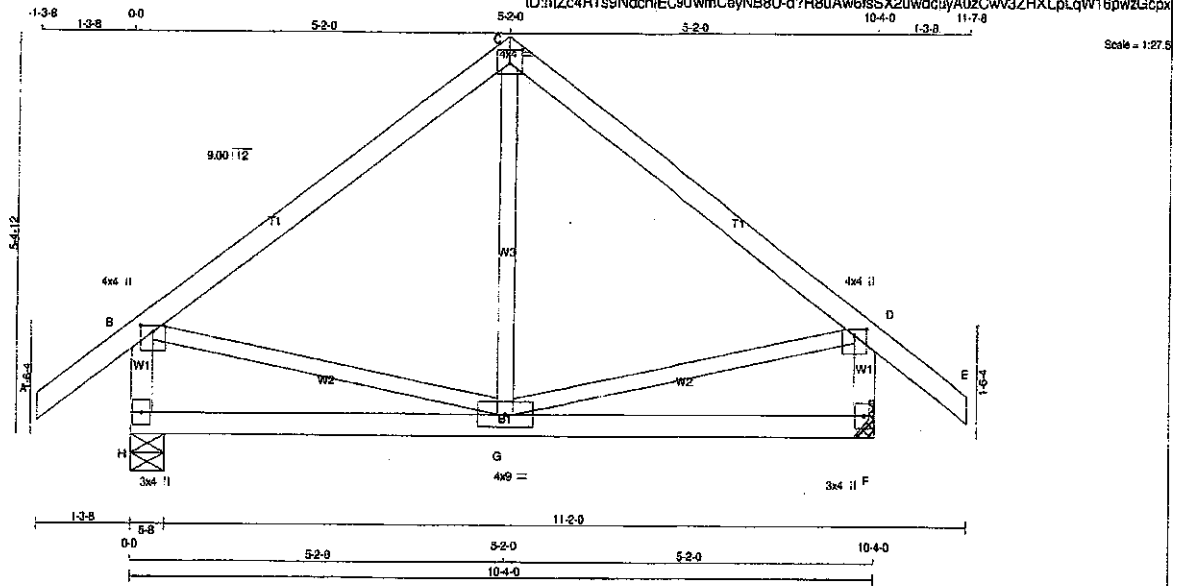
PLATE ROTATION TOL. = 5.0 Deg.
JSI GRIP= 0.85 (E) (INPUT = 0.90)
JSI METAL= 0.53 (E) (INPUT = 1.00)

Structural component only
DWG# T-1911504

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403059	T17	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:hZc4R1s9NdonIEC90wmCayNB8U-d?R8uAw6fsSX2uwdcuyA0zCww3ZHXLpLqW16pwzGcpx



TOTAL WEIGHT = 45 lb
(M/F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	No.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
C	TTW+p	MT20	4.0	4.0	2.25 2.00
D	TMVW+p	MT20	4.0	4.0	1.00 2.00
F	BMV1+p	MT20	3.0	4.0	
G	BMVWW+	MT20	4.0	9.0	
H	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ	UPLIFT
H	867	0	867	0
F	867	0	867	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	COMBINED	639	380 / 0	109 / 0	0 / 0	0 / 0	151 / 0	0 / 0
F	COMBINED	639	380 / 0	109 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLP) CSI (LC)	MEMB.	FORCE (LBS) CSI (LC)
FR-TO		FROM TO	FR-TO	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	G-C	0 / 223 0.05 (3)
B-C	-493 / 0	-102.1 -102.1 0.35 (1)	B-G	0 / 405 0.09 (1)
C-D	-493 / 0	-102.1 -102.1 0.35 (1)	G-D	0 / 405 0.09 (1)
D-E	0 / 42	-102.1 -102.1 0.14 (1)		
H-B	-791 / 0	0.0 0.0 0.08 (1)		
F-D	-791 / 0	0.0 0.0 0.08 (1)		
H-G	0 / 0	-38.5 -38.5 0.23 (3)		
G-F	0 / 0	-38.5 -38.5 0.23 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.35/1.00 (C-D:1), BC=0.23/1.00 (F-G:3), WB=0.09/1.00 (B-G:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

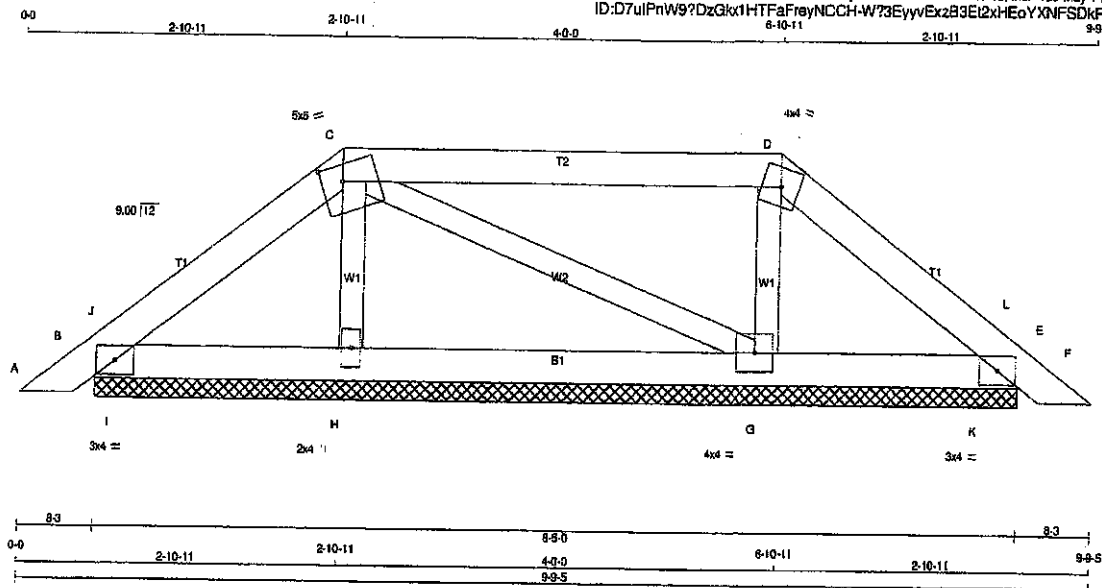
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.75 (B) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	P1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:17 2019 Page 1					
ID: D7ulPnW9?DzGk1HTFaFreyNCCH-W?3EyyvEx2B3E12xHEoYXNFSDkRvK1kr68IDzGdOm					
6-10-11 9-9-5					



Scale = 1/17.8

LUMBER

N. L. & A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
B	257	0	0	8-5-0
E	243	0	0	8-5-0
H	386	0	0	8-5-0
G	417	0	0	8-5-0

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	SNOW	SNOW	SNOW	SNOW	SNOW
B	183	130/0	16/0	0/0	0/0	37/0	0/0
E	173	122/0	16/0	0/0	0/0	38/0	0/0
H	295	144/0	73/0	0/0	0/0	78/0	0/0
G	316	161/0	73/0	0/0	0/0	82/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
FR-TO		FORCE (LBS)	(PLF)	FROM	TO	CS1 (LC)	UNBRAC	FR-TO		FORCE (LBS)	(PLF)	FROM	TO
A-B		0/17	-102.1	-102.1	0.03 (1)	10.00	H-C		-232/0	0.03 (1)			
B-J		-75/0	-102.1	-102.1	0.02 (1)	6.25	C-G		-21/0	0.01 (1)			
J-C		-98/0	-102.1	-102.1	0.06 (1)	6.25	G-D		-255/0	0.04 (1)			
C-D		-42/0	-102.1	-102.1	0.28 (1)	6.25	I-J		-148/0	0.00 (1)			
D-E		-75/0	-102.1	-102.1	0.06 (1)	6.25	K-L		-148/0	0.00 (1)			
E-F		-51/0	-102.1	-102.1	0.02 (1)	6.25							
F-G		0/17	-102.1	-102.1	0.03 (1)	10.00							
B-H		0/75	-38.5	-38.5	0.07 (1)	10.00							
H-I		0/75	-38.5	-38.5	0.08 (2)	10.00							
I-J		0/61	-38.5	-38.5	0.08 (2)	10.00							
J-K		0/57	-38.5	-38.5	0.08 (2)	10.00							
K-L		0/57	-38.5	-38.5	0.06 (1)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
OL = 6.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-03, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.28/1.00 (C-D:1), BC=0.08/1.00 (H-I:2), WB=0.04/1.00 (D-G:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.19 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

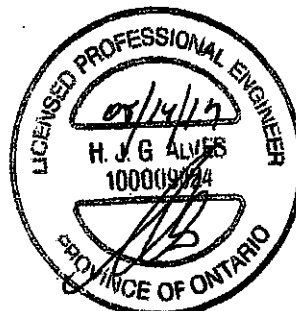
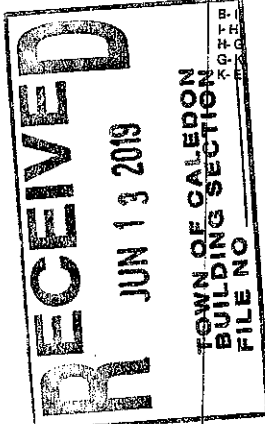
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	518	354	1887
	518	354	1887

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

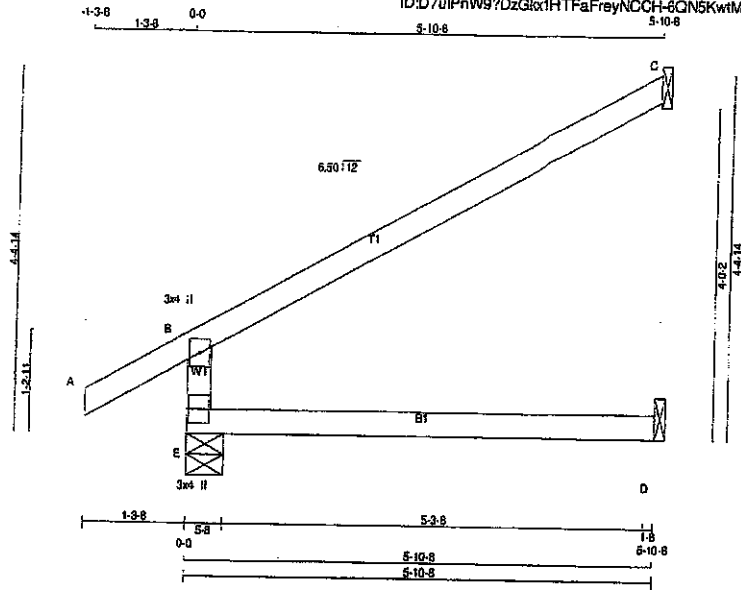
JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.05 (H) (INPUT = 1.00)



Structural component only
DWG# T-1911479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
403045	J1	17	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						

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 ID:D7uIPnW9?DzGt1HTFaFrayNCCH-6QN5KwtMe2pVNPJMc8FrvldsxWNBXOHb1tuUhzGdDp
 Scale = 1:24.6



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
E	646	0	646	0
C	225	0	225	0
D	94	0	119	0

SEE MTEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292/0	72/0	0/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	0/0	28/0	0/0
D	85	0/0	51/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO				
E-B	-514/0	0.0	0.0	0.21 (3)
A-B	0/33	-102.1	-102.1	0.13 (1)
B-C	-36/0	-102.1	-102.1	0.80 (1)
E-D	0/0	-38.5	-38.5	0.21 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/930 (0.08")

CSI: TC=0.80/1.00 (B-C:1), BC=0.21/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.22 (E) (INPUT = 0.90)
 JSI METAL=0.15 (B) (INPUT = 1.00)

RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

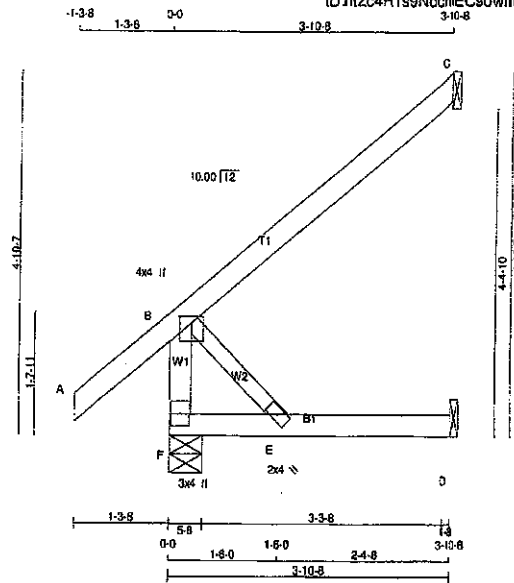


Structural component only
 DWG# T-1911476

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403059	J3	12	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:08:45 2019 Page 1
ID:htZc4R1s9NdcnEC90wmCeyNB8U-G2eFrSszqJqEy711qLN?JvV4c2tds5?chEKM9izGcq0

Scale = 1:27.3



TOTAL WEIGHT = 12 X 15 = 183 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (tablets in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ
F	414	0	414	0
C	198	0	198	0
D	75	0	95	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
F	301	193/0	41/0	0/0	0/0	0/0	67/0	0/0
C	136	113/0	0/0	0/0	0/0	0/0	23/0	0/0
D	68	0/0	41/0	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LBS)	MAX. LC2 (LBS)	MAX. UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LBS)
FR-TO							FR-TO		
F-B		-339/0	0.0	0.0	0.04 (1)	7.81	B-E	0/0	0.00 (1)
A-B		0/45	-102.1	-102.1	0.14 (1)	10.00			
B-C		0/0	-102.1	-102.1	0.26 (1)	10.00			
F-E		0/0	-38.5	-38.5	0.13 (3)	10.00			
E-D		0/0	-38.5	-38.5	0.14 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD = 52.5 PSF			

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.26/1.00 (B-C:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.80

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	1067	788	1987

PLATE PLACEMENT TOL = 1/8" inches

PLATE ROTATION TOL = 5.0 degrees

JSI CRIP = 0.26 (INPUT = 0.26)
SIP TOL = 0.04 (INPUT = 0.04)



RECEIVED
JUN 13 2019
TOWN OF CANTON
BUILDING SECTION
FILE NO.
Structural component only
DWG# T-1911499



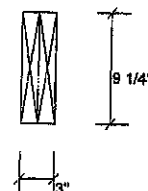
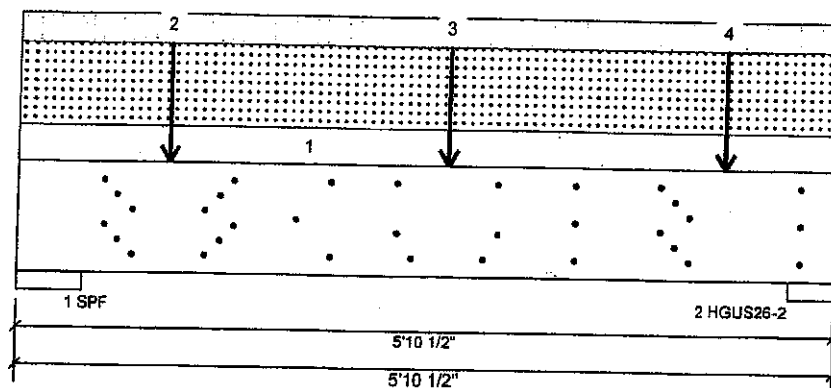
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 1 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Roof (Residential)
 Slope: 0/12
 Design Method: LSD
 Building Code: NBCC 2015
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	376	503	1210	0
2	314	415	1001	0

Bearings and Factored Reactions

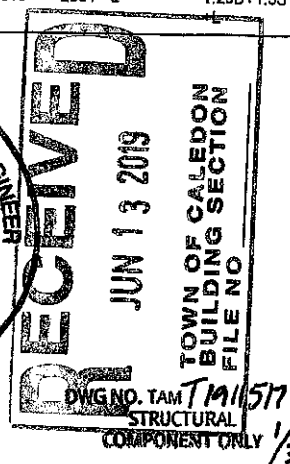
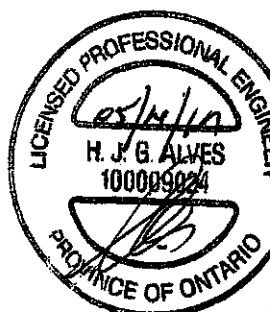
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	28%	629 / 2190	2819 L	1.25D+1.5S +L
2 - HGUS...	4.000"	32%	519 / 1815	2334 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2982 ft-lb	3'1"	8039 ft-lb	0.494 (49%)	1.25D+1.5S +L	L
Unbraced	2982 ft-lb	3'1"	5236 ft-lb	0.570 (57%)	1.25D+1.5S +L	L
Shear	2763 lb	12"	3984 lb	0.694 (69%)	1.25D+1.5S +L	L
LL Defl inch	0.025 (L/2459)	2'11 9/16"	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.035 (L/1809)	2'11 9/16"	0.174 (L/360)	0.200 (20%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	1-1-0		Far Face	350 lb	256 lb	848 lb	0 lb	
3	Point	3-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	
4	Point	5-1-0		Far Face	208 lb	155 lb	508 lb	0 lb	

Manufacturer Info

Tamarack Roof Trusses
 3289 North Service Rd., ON
 Canada
 L7N3G2
 (905) 333-1115



This design is valid until 12/11/2021





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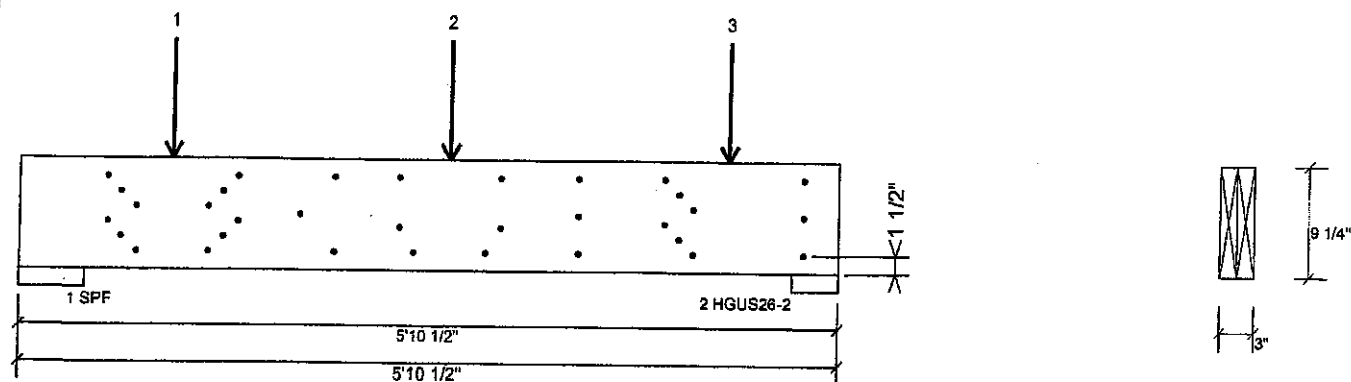
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 2 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 1-1-0 with a minimum of (12) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

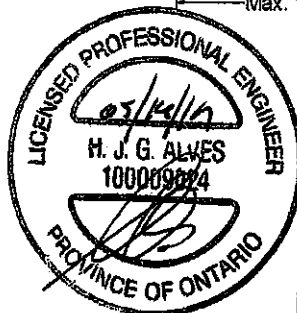
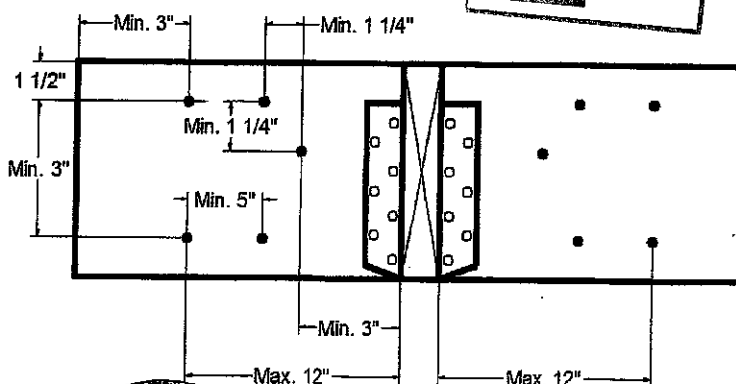
Capacity	72.3 %
Load	992.8lb.
Total Yield Limit	1380.2 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 3-1-0 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	88.5 %
Load	588.5lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 1911517
STRUCTURAL
COMPONENT ONLY 2/3

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(800) 338-1115



This design is valid until 12/11/2021



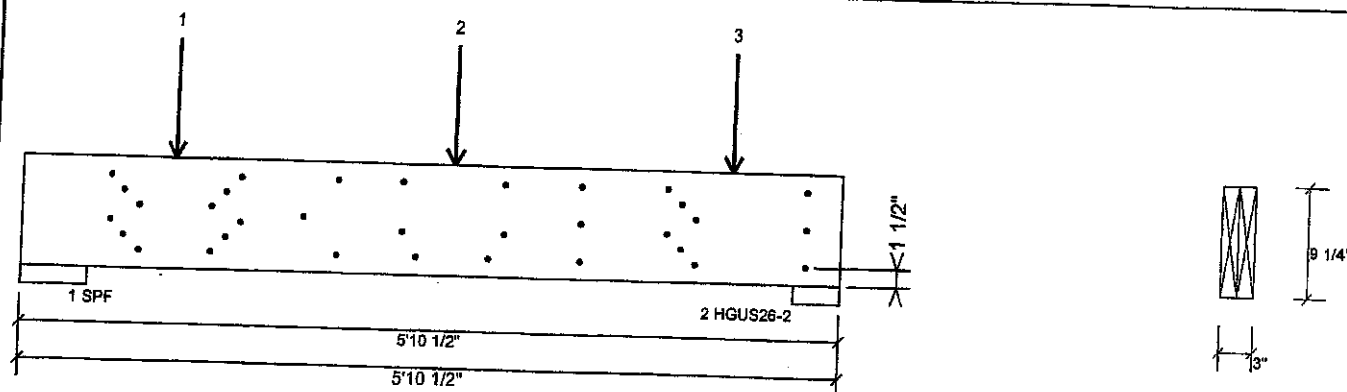
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 3 of 13

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

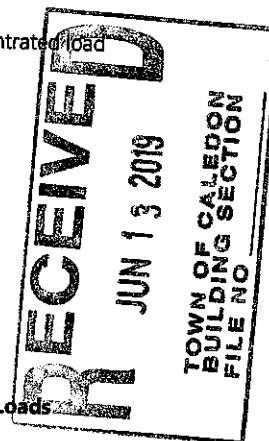
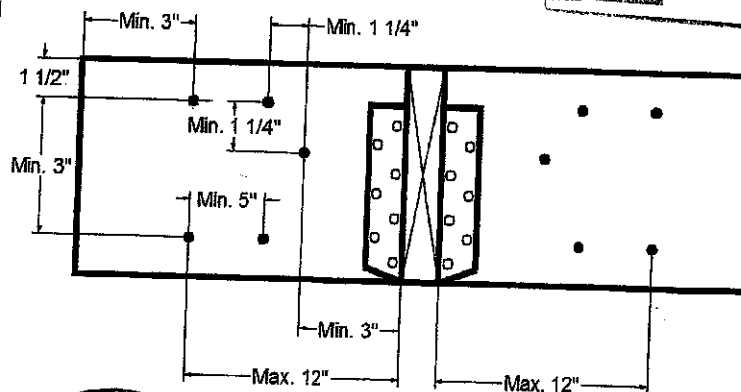
Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 5-1-0 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	88.5 %
Load	588.5 lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

DWG NO. TAM 11911517
STRUCTURAL
COMPONENT ONLY 3/3

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





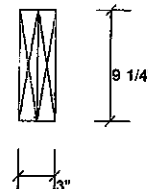
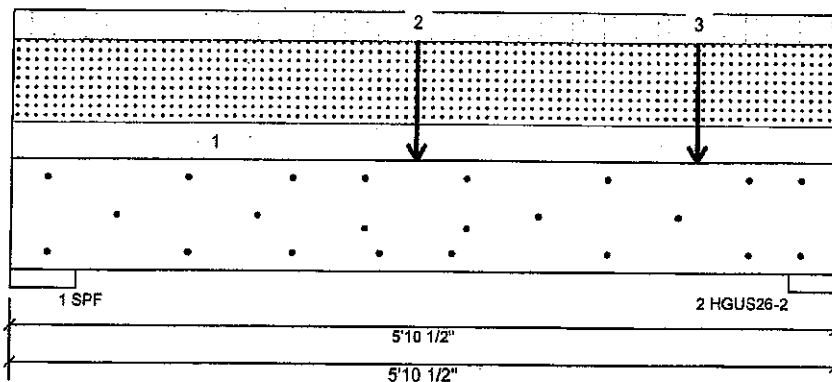
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 12 of 13

B4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	161	212	514	0
2	229	307	754	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	265 / 931	1196 L
2 - HGUS...	4.000"	24%	384 / 1360	1744 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	2'10 1/2"	8039 ft-lb	0.396 (40%)	1.25D+1.5S +L	L
Unbraced	2390 ft-lb	2'10 1/2"	5236 ft-lb	0.457 (46%)	1.25D+1.5S +L	L
Shear	1659 lb	4'10"	3984 lb	0.416 (42%)	1.25D+1.5S +L	L
LL Defl inch	0.019 (L/3279)	3' 3/8"	0.174 (L/360)	0.110 (11%)	S+0.5L	L
TL Defl inch	0.026 (L/2421)	3' 5/16"	0.174 (L/360)	0.150 (15%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17411522
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	2-10-8		Far Face	216 lb	157 lb	541 lb	0 lb	T16
3	Point	4-10-8		Top	151 lb	109 lb	380 lb	0 lb	T17

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





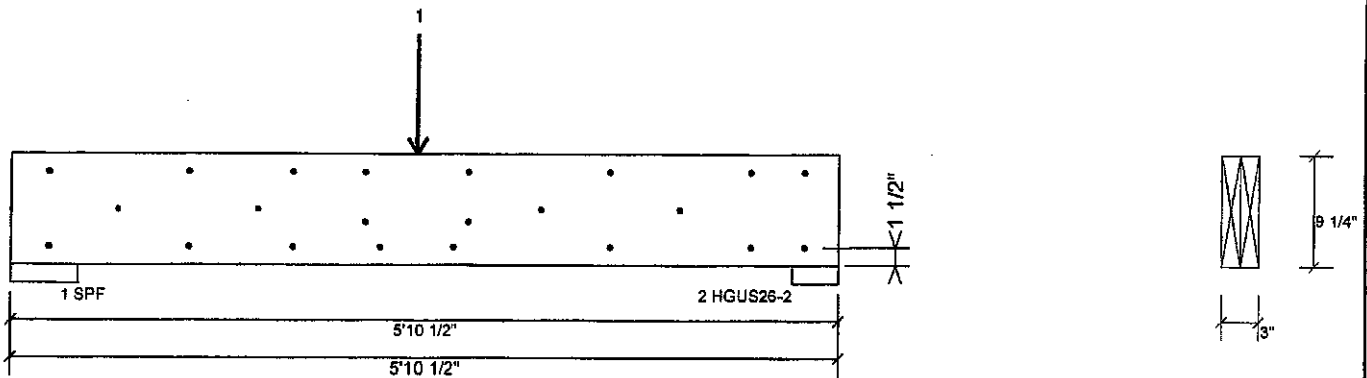
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 13 of 13

B4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

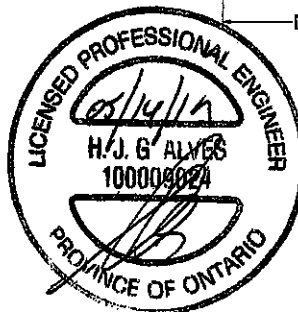
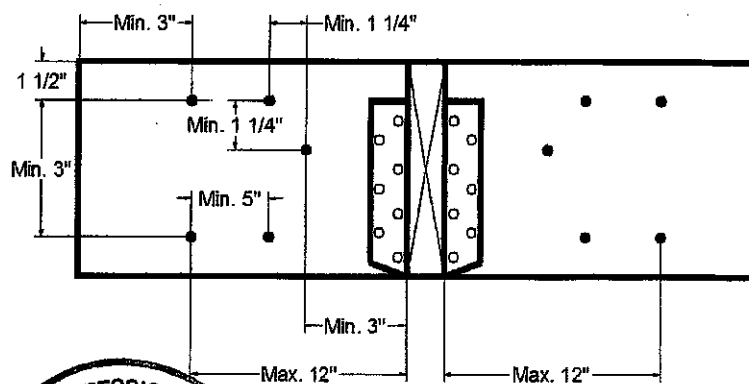
Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 2-10-8 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	91.1 %
Load	619.3lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

OWG NO. 1AM *TRAIL 522*
STRUCTURAL
COMPONENT ONLY *7/2*

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





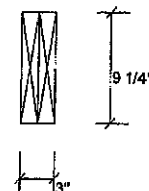
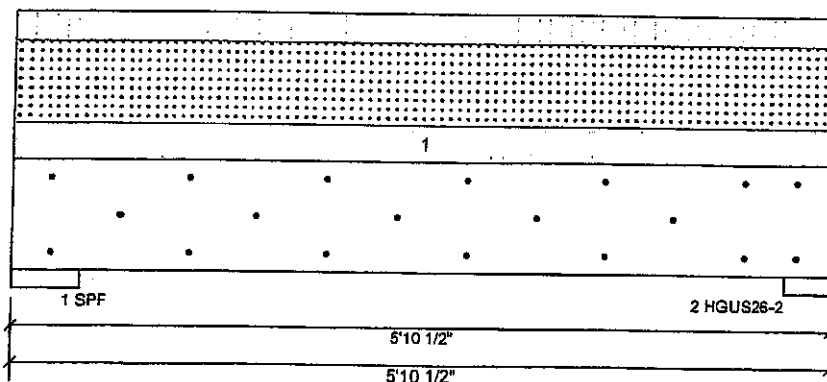
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 10 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	289	333	756	0
2	258	319	724	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	416 / 1403	1819 L
2 - HGUS...	4.000"	24%	399 / 1345	1744 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2058 ft-lb	3'	6039 ft-lb	0.341 (34%)	1.25D+1.5S	L
Unbraced	2058 ft-lb	3'	5236 ft-lb	0.393 (39%)	1.25D+1.5S	L
Shear	1579 lb	1'2"	3984 lb	0.396 (40%)	1.25D+1.5S	L
LL Defl inch	0.018 (L/3466)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.025 (L/2523)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 194521
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-6-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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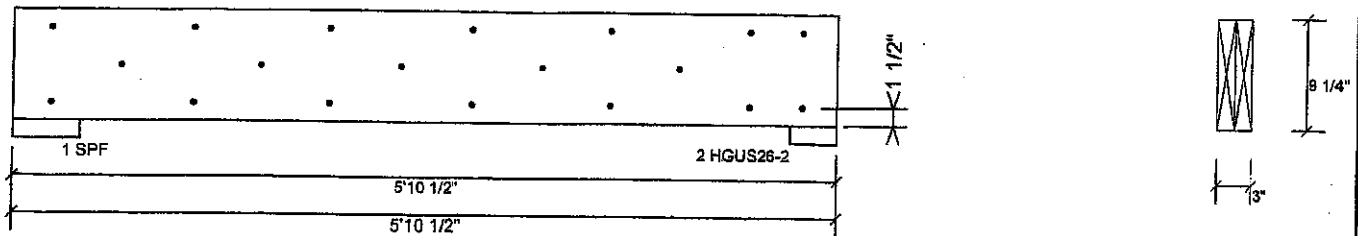
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 11 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.2 %
Load	303.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17911521
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021

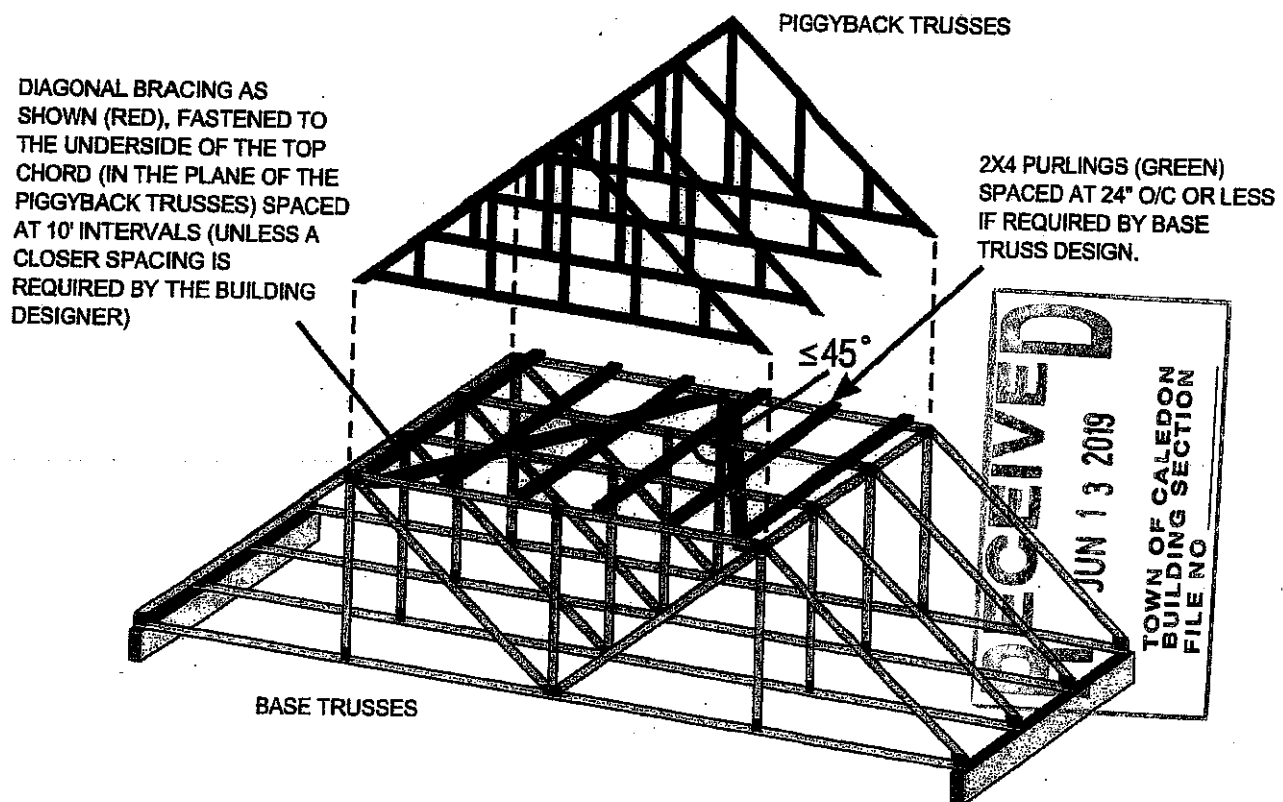


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

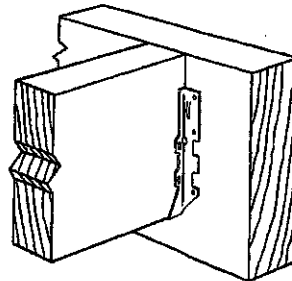
Finish: G90 galvanized

Design:

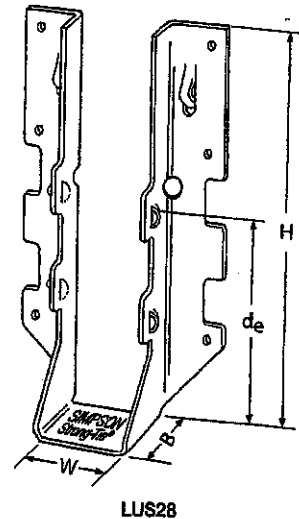
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



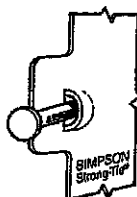
Typical LUS Installation



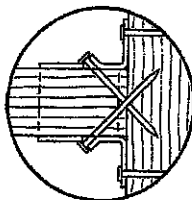
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3 1/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

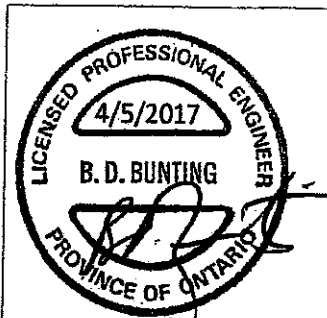
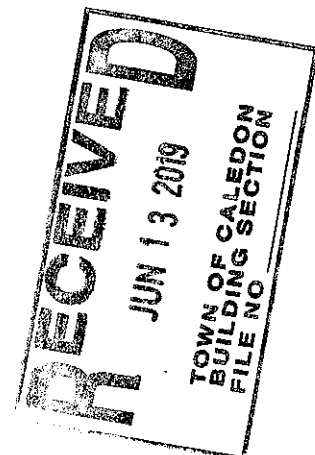
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Top View.



DESIGN

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Simpson Strong-Tie Company

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HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

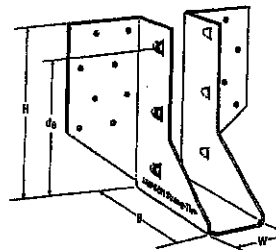
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

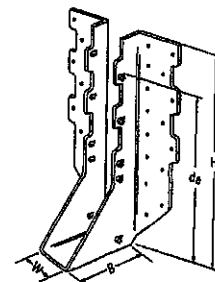
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

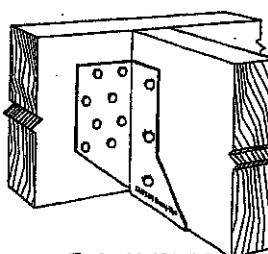
- See current catalogue for options



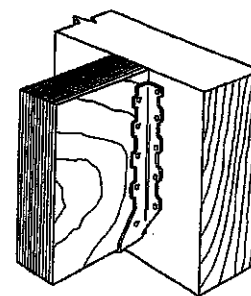
LJS26DS



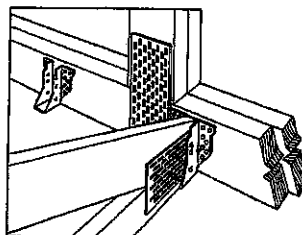
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



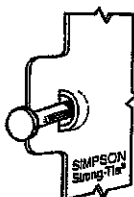
Typical HUS
Installation



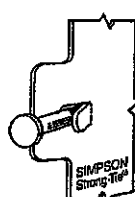
Typical HUS installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/8	(16) 16d	(8) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/16	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

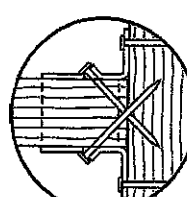
1. d_s is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



DESIGN

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HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

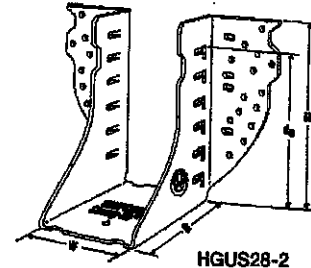
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

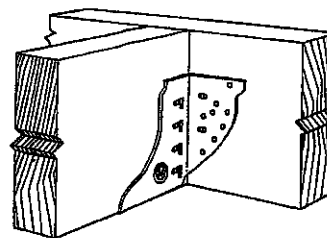
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

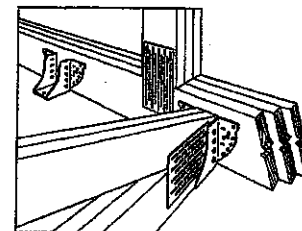
- See current catalogue for options



HGUS28-2



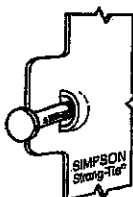
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

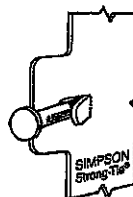
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 7/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/4	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 3/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/4	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 3/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 3/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 3/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_g is the distance from the seat of the hanger to the highest joist nail.

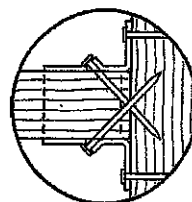


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

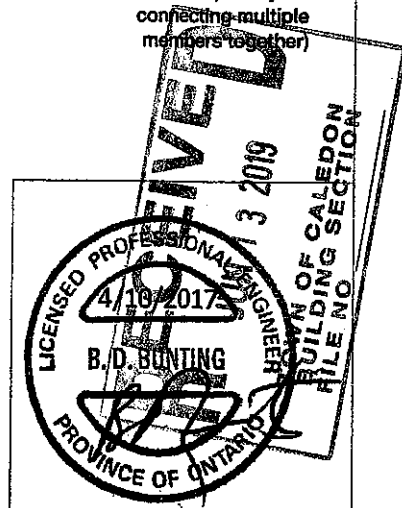
U.S. Patent
5,603,580



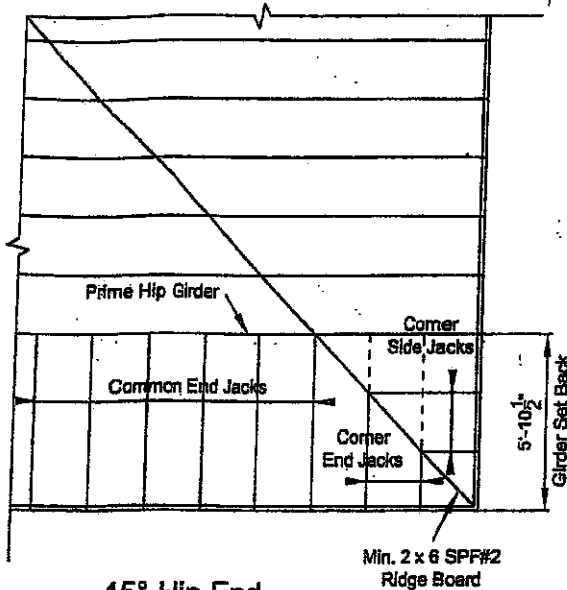
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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45° Hip End

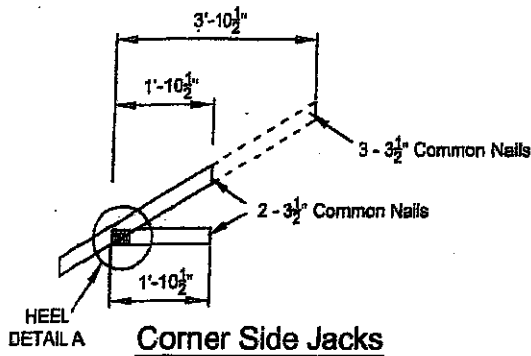
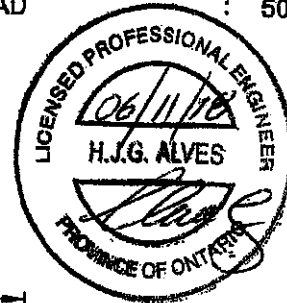
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

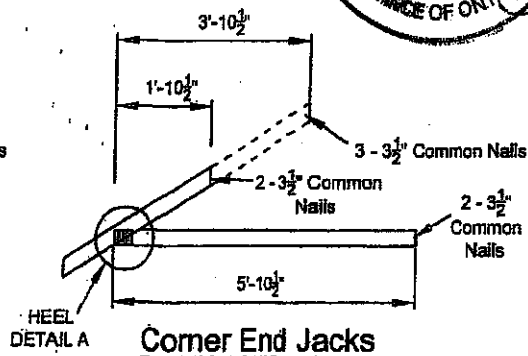
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

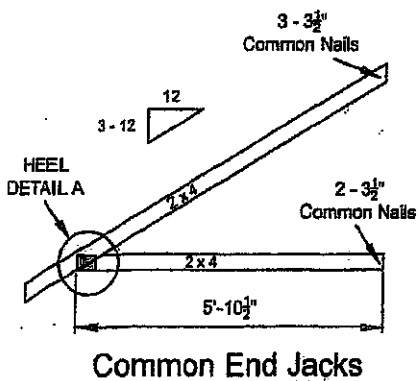
TOTAL LOAD : 50.5 P.S.F.



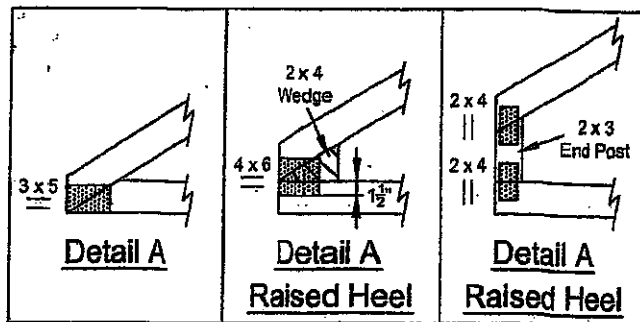
Corner Side Jacks



Corner End Jacks

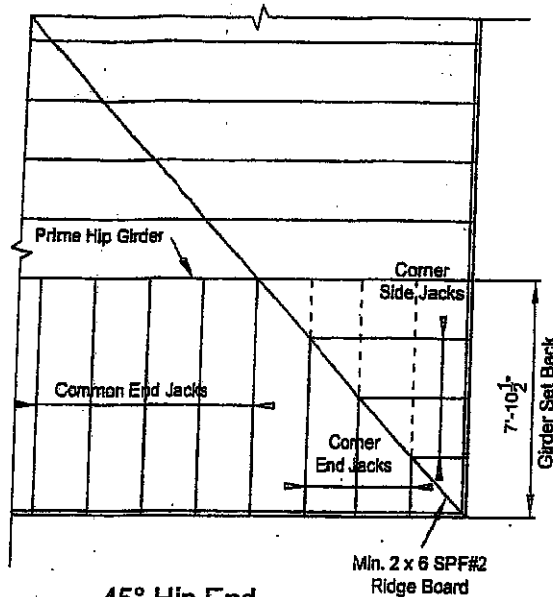


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

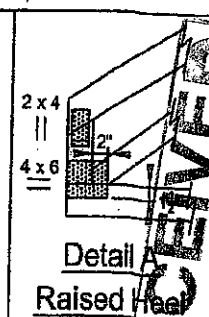
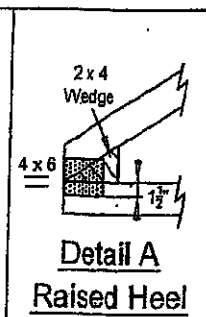
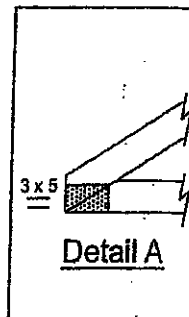
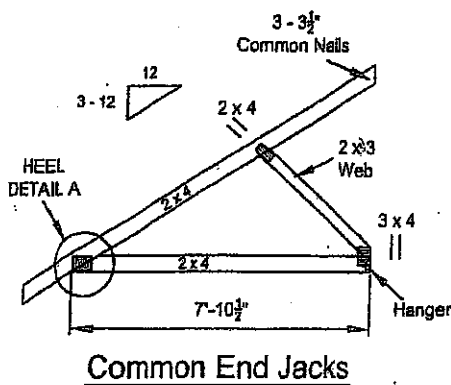
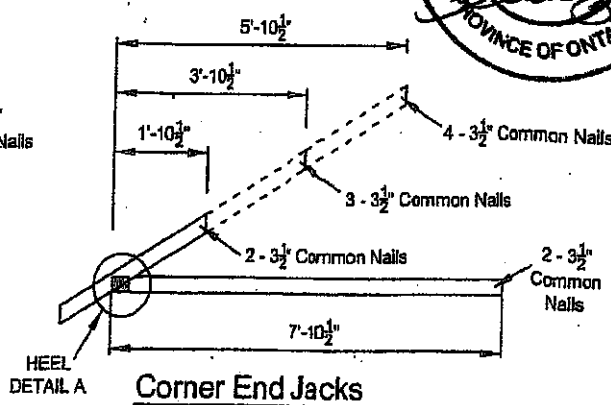
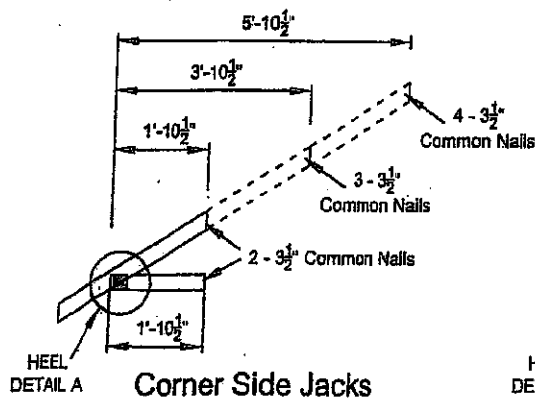
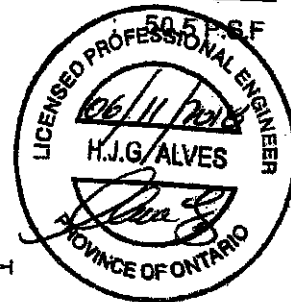
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

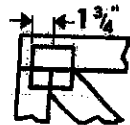
T-18002AT7

JUN 13 2019

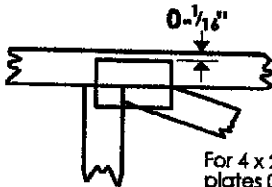
TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

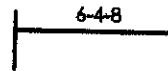


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

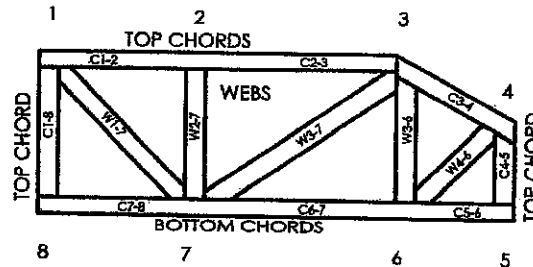
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufacturer's responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018