

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

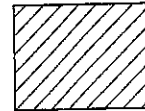
HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

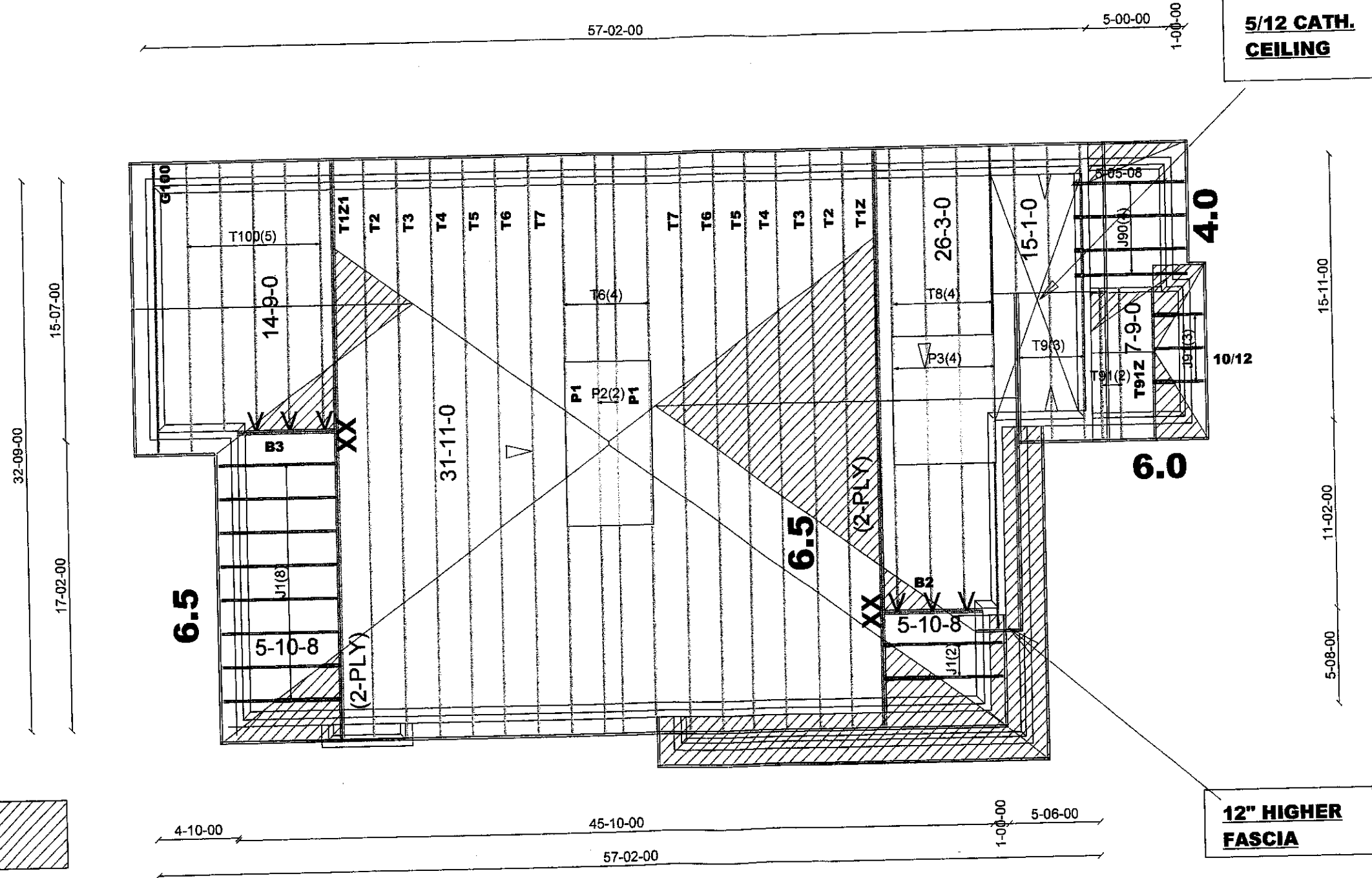
ALL B- 2-2X10 (FLUSH)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

DENOTES:
CONVENTIONAL
FRAMING



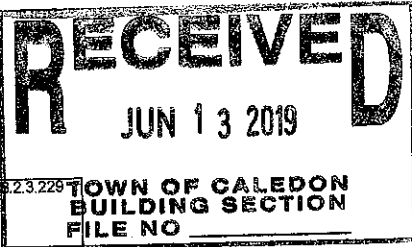
9/12 PITCHES (TYP.)
UNLESS NOTED



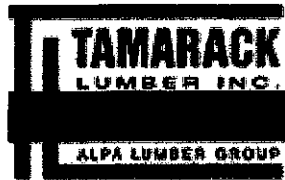
ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2\"X4\"SPF@24\"o.c.
WITH A 2\"X4\"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD Ss= 1.8kPa
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



 ALPHA LUMBER GROUP	Job Track: 50120	Builder / Location: GREEN PARK HOMES / Caledon		Model / Elevation: KINMOUNT 6A/1-REAR UPGRADE	
	Plan Log: 200598	Project: LAMBERTS LANE HOME CORP.			
	Layout ID: 401607	Date: 5/8/2019	Sales: Mario DiCano	Designer: S.V.	Checker: JG
	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.				



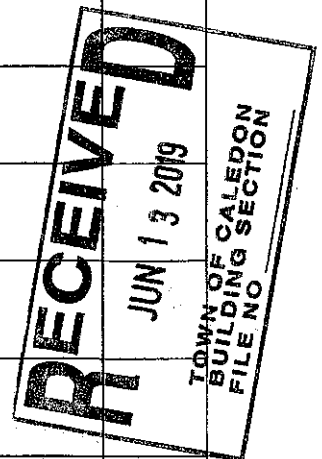
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 1-REAR UPGRADE

Job Track: 50120
 PlanLog: 200598
 Layout ID: 401607
 Ref #
 Page: 1 of 2
 Date: 05/08/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	1 2-ply	T121 Hip Girder	9/12	31-11-00	4-04-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	317.54 198.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	267.53 170.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.93 208.00		
	4	T8 Piggyback Base	9/12	26-03-00	9-00-00	2 x 4	1-03-08	1-06-04 2-06-04	502.55 320.67		
	3	T9 Scissor	9/12 5/12	15-01-00	7-02-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	193.33 124.00		
	2	T91 Common	6/12	7-09-00	2-10-15	2 x 4	1-03-08 1-03-08	6-12 6-12	50.42 31.33		
	1	T912 Common Girder	6/12	7-09-00	2-10-15	2 x 4	1-03-08 1-03-08	6-12 6-12	25.21 15.67		
	5	T100 Common	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	330.72 215.83		
	1	G100 GABLE	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.06 45.17		



		DELIVERY SHIPLIST	
	Lumber Yard:	TAMARACK LUMBER	Job Track: 50120
	Builder:	GREEN PARK HOMES	PlanLog: 200598
	Project:	LAMBERTS LANE HOME CORP.	Layout ID: 401607
	Location:	Caledon	Ref #
	Model:	KINMOUNT 6A	Page: 2 of 2
	Lot #:		Date: 05/08/2019
Elevation:	1-REAR UPGRADE	Designer: Jane Gong	Sales Rep: Mario DiCano

Roof Trusses

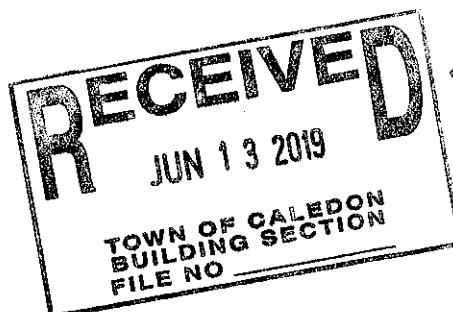
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	2	P1 Piggyback	9 /12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9 /12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	4	P3 Piggyback	9 /12	7-07-11	2-10-06	2 x 4			72.19 45.33		
	10	J1 Jack-Open	6.5 /12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	170.4 106.67		
	4	J90 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	57.79 37.33		
	3	J91 Jack-Open	10 /12	1-09-02	2-11-01	2 x 4	1-03-08	1-00-07 2-06-00	22.51 16.00		

TOTAL # TRUSS= 61 TOTAL BFT OF ALL TRUSSES= 2972.33 BFT. TOTAL WEIGHT OF ALL TRSSES 4693.76 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
8	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

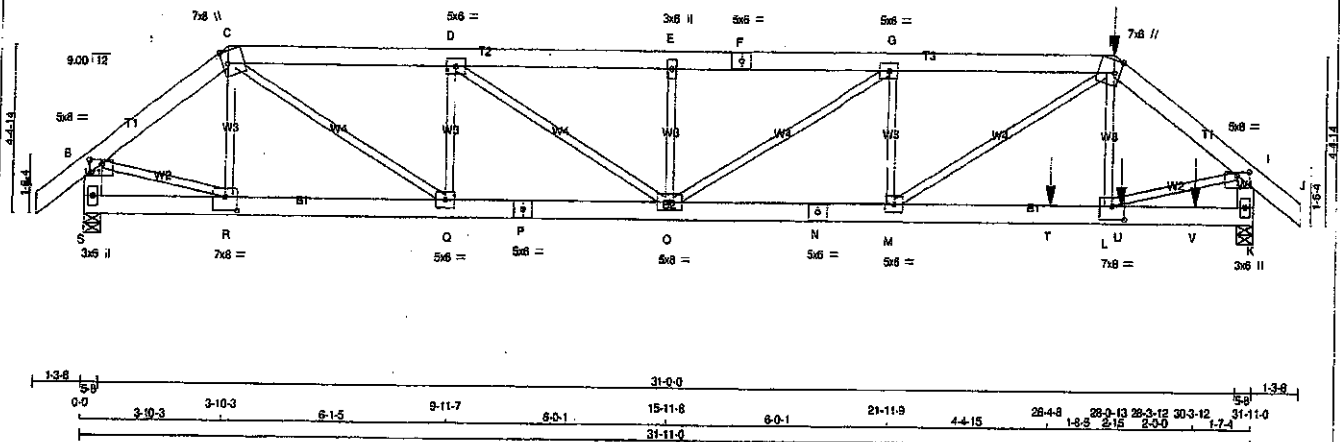
TOTAL NUMBER OF ITEMS= 8



JOB NAME 403045	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MiTek Industries, Inc. Tue May 14 10:41:26 2019 Page 1
ID: D7uIPnW9?DzGlx1HTFaFreyNCCX-xa/Nb_y7EuZe6KnVyMMF80L_zxGbyuTQpLoyZGdD

1-3-8 0-0 3-10-3 3-10-3 6-1-5 9-11-7 6-0-1 15-11-8 6-0-1 21-11-9 6-1-5 28-0-13 3-11-0 33-2-8 1-3-8
Scale = 1:50.0



TOTAL WEIGHT = 2 X 170 = 340 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x8 DRY	No.2	SPF		
C - F	2x8 DRY	No.2	SPF		
F - H	2x8 DRY	No.2	SPF		
H - J	2x8 DRY	No.2	SPF		
J - L	2x8 DRY	No.2	SPF		
L - P	2x8 DRY	No.2	SPF		
P - R	2x8 DRY	No.2	SPF		
R - S	2x8 DRY	No.2	SPF		
S - T	2x8 DRY	No.2	SPF		
T - U	2x8 DRY	No.2	SPF		
U - V	2x8 DRY	No.2	SPF		
V - W	2x8 DRY	No.2	SPF		
W - X	2x8 DRY	No.2	SPF		
X - Y	2x8 DRY	No.2	SPF		
Y - Z	2x8 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A - C	2	12
C - F	2	12
F - H	2	12
H - J	2	12
J - L	2	12
L - P	2	12
P - R	2	12
R - S	2	12
S - T	2	12
T - U	2	12
U - V	2	12
V - W	2	12
W - X	2	12
X - Y	2	12
Y - Z	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
S - P	2	12
P - N	2	12
N - K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.25	4.00
C TTWW-m	MT20	7.0	8.0	4.00	1.75
D TMVW-i	MT20	5.0	8.0		
E TMVW-w	MT20	3.0	6.0		
F TS-i	MT20	5.0	6.0		
G TMVW-i	MT20	5.0	6.0		
H TTWW-m	MT20	7.0	8.0	4.00	1.75
I TMVW-p	MT20	5.0	8.0	1.25	4.00
J BMVW-i	MT20	3.0	6.0		
K BMVW-p	MT20	7.0	8.0	4.25	4.00
L BMVW-i	MT20	5.0	6.0		
M BMVW-p	MT20	5.0	6.0		
N BS-i	MT20	5.0	6.0		
O BMVW-w	MT20	5.0	6.0		
P BS-i	MT20	5.0	6.0		
Q BMVW-i	MT20	5.0	6.0		
R BMVW-p	MT20	7.0	8.0	4.25	4.00
S BMVW-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG	BRG	IN-SX	IN-SX
S	3010	0	3010	0	0	5-8	5-8		
K	5542	0	5542	0	0	5-8	5-8		

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST CASE COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
S	2236	1277 / 0	417 / 0	0 / 0	0 / 0	542 / 0	0 / 0
K	4112	2362 / 0	755 / 0	0 / 0	0 / 0	995 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS	MEMB.	FORCE (LBS)	FACTORED
FR-TO	FR-TO	FR-TO	FR-TO
A-B	0 / 44	-102.1	-102.1
B-C	-3254 / 0	-102.1	-102.1
C-D	-5555 / 0	-102.1	-102.1
D-E	-7045 / 0	-102.1	-102.1
E-F	-7045 / 0	-102.1	-102.1
F-G	-7045 / 0	-102.1	-102.1
G-H	-7615 / 0	-102.1	-102.1
H-I	-8846 / 0	-102.1	-102.1
I-J	0 / 44	-102.1	-102.1
J-K	-2987 / 0	0.0	0.0
K-L	-5884 / 0	0.0	0.0
S-R	0 / 0	-38.5	-38.5
R-Q	0 / 2582	-38.5	-38.5
Q-P	0 / 5555	-38.5	-38.5
P-O	0 / 5555	-38.5	-38.5
O-N	0 / 7616	-38.5	-38.5
N-M	0 / 7616	-38.5	-38.5
M-T	0 / 5506	-38.5	-38.5
T-L	0 / 5506	-38.5	-38.5
L-U	0 / 0	-38.5	-38.5
U-V	0 / 0	-38.5	-38.5
V-K	0 / 0	-38.5	-38.5

FACTORED CONCENTRATED LOADS (LBS)	
JT	LOC.
H	28-0-13
H	28-0-13
H	28-0-13
T	28-4-8
U	28-3-12
V	30-3-12

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-06, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.25/1.00 (G-H-T), BC=0.83/1.00 (L-M-T), WB=0.70/1.00 (L-T), SS=0.87/1.00 (L-M-T)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

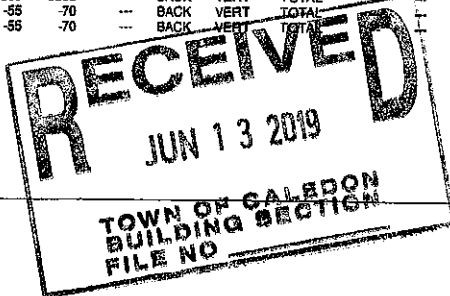
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

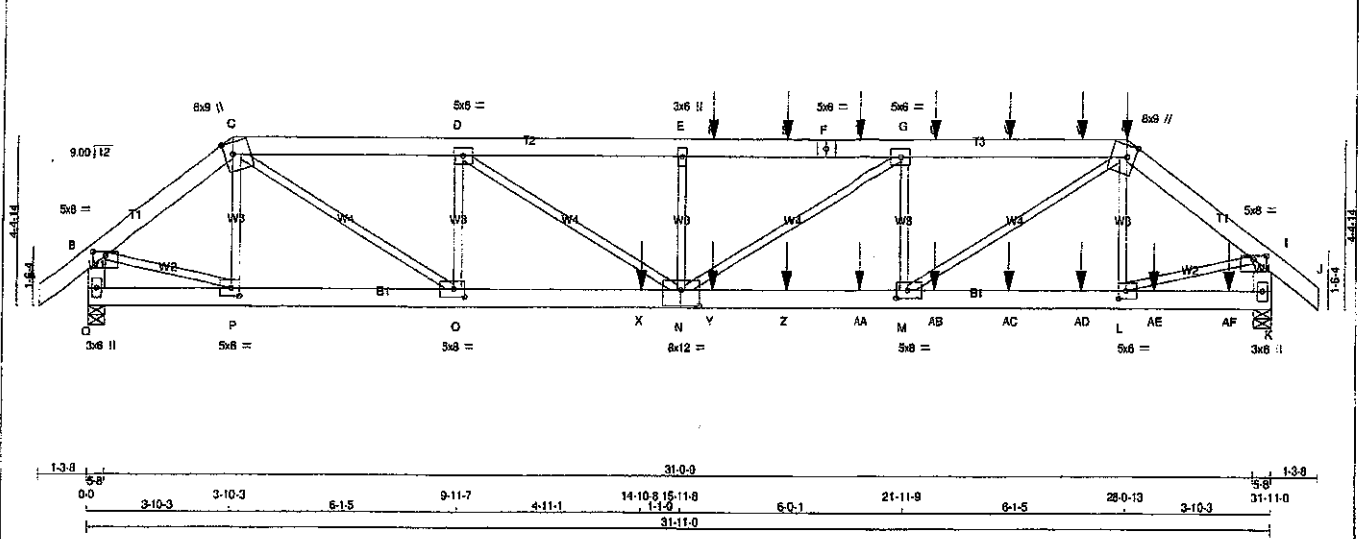
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (L) (INPUT = 0.90)
JSI METAL=0.79 (N) (INPUT = 1.00)

Structural component only
DWG# T-1911483



JOB NAME 401607	TRUSS NAME T1Z1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 11:01:02 2019 Page 1			
1-3-8 0-0 3-10-3 3-10-3 6-1-5 9-11-7 6-0-1 15-11-8 6-0-1 21-11-9 28-0-13 3-10-3 31-11-0 1-3-8		ID: D7uIPnW9?DzGkx1HTFaFreyNCCCH-Kz9DI8FYs3Uwv_BgCcZQ2GqW4PdtXvSDigLP2zGcxK			
Scale = 1:53.3					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
C - F	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
Q - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x6	DRY	1650F 1.5E
N - K	2x6	DRY	1650F 1.5E
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
Q-B	2	12
K-I	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Q-N	2	12
N-K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	1MWV-p	MT20	5.0	8.0	1.25 4.00
C	1TWV-m	MT20	8.0	9.0	3.75 2.50
D	1MWV-t	MT20	5.0	6.0	
E	1MWV-w	MT20	3.0	6.0	
F	TS-t	MT20	5.0	6.0	
G	1MWV-t	MT20	5.0	6.0	
H	1TWV-m	MT20	8.0	9.0	3.75 2.50
I	1MWV-p	MT20	5.0	8.0	1.25 4.00
K	1MWV-t	MT20	3.0	6.0	
L	1MWV-w	MT20	5.0	6.0	2.50 2.50
M	1MWV-t	MT20	5.0	8.0	2.50 3.50
N	1MWV-w	MT20	8.0	12.0	5.00 8.00
O	1MWV-t	MT20	5.0	8.0	2.50 3.50
P	1MWV-w	MT20	5.0	6.0	2.50 2.50
Q	1MWV-p	MT20	3.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQ'D	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG	BRG
Q	3943	0	3943	0	0	5-8	5-8	5-8	5-8
K	4534	0	4534	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	2928	1688 / 0	533 / 0	0 / 0	0 / 0	704 / 0	0 / 0
K	3370	1917 / 0	635 / 0	0 / 0	0 / 0	819 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	FROM TO	FR-TO	FROM TO
A-B	0 / 44	P-C	-788 / 0
B-C	-4372 / 0	C-D	0 / 5415
C-D	-8021 / 0	D-E	-2108 / 0
D-E	-10251 / 0	E-F	0 / 2663
E-F	-10251 / 0	F-G	-722 / 0
F-G	-10251 / 0	G-H	0 / 2287
G-H	-10251 / 0	H-I	-2364 / 0
H-I	-10251 / 0	I-J	0 / 5060
I-J	-10251 / 0	J-K	-824 / 14
J-K	-10251 / 0	K-L	0 / 3586
K-L	-10251 / 0	L-M	0 / 4203
L-M	-10251 / 0	M-N	-824 / 14
M-N	-10251 / 0	N-O	0 / 3586
N-O	-10251 / 0	O-P	0 / 4203
O-P	-10251 / 0	P-Q	-824 / 14
P-Q	-10251 / 0	Q-R	0 / 3586
Q-R	-10251 / 0	R-S	0 / 4203
R-S	-10251 / 0	S-T	-824 / 14
S-T	-10251 / 0	T-U	0 / 3586
T-U	-10251 / 0	U-V	0 / 4203
U-V	-10251 / 0	V-W	-824 / 14
V-W	-10251 / 0	W-X	0 / 3586
W-X	-10251 / 0	X-Y	0 / 4203
X-Y	-10251 / 0	Y-Z	-824 / 14
Y-Z	-10251 / 0	Z-AA	0 / 3586
Z-AA	-10251 / 0	AA-AB	0 / 4203
AA-AB	-10251 / 0	AB-AC	-824 / 14
AB-AC	-10251 / 0	AC-AD	0 / 3586
AC-AD	-10251 / 0	AD-AE	0 / 4203
AD-AE	-10251 / 0	AE-AF	-824 / 14
AE-AF	-10251 / 0	AF-K	0 / 3586

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
H	28-0-13	-48	-55	FRONT	VERT	DEAD	---	---
H	28-0-13	-282	-282	FRONT	VERT	SNOW	---	---
R	18-9-12	-123	-123	FRONT	VERT	TOTAL	---	---
T	18-9-12	-123	-123	FRONT	VERT	TOTAL	---	---
T	20-9-12	-123	-123	FRONT	VERT	TOTAL	---	---
U	22-9-12	-123	-123	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	32.5	PSF	

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.06/12

*** NON STANDARD GIRDER ***

ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/989 (0.28")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/993 (0.43")

CSI: TC=0.41/1.00 (E-G:1), BC=0.82/1.00 (N-O:1), WB=0.67/1.00 (C-O:1), SS=0.60/1.00 (N-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1857 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

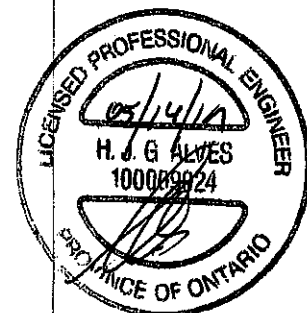
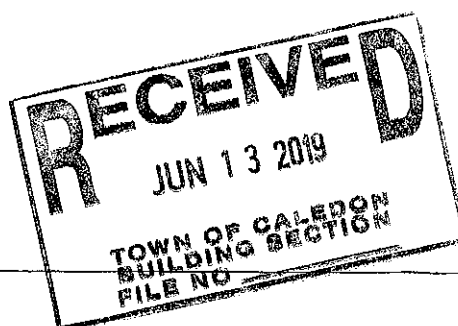
JSI GRIP= 0.88 (L) (INPUT = 0.90)
JSI METAL= 0.58 (N) (INPUT = 1.00)

Structural component only

DWG# T-1911498 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401607	T1Z1	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 11:01:02 2019 Page 2 ID:D7uIPnWg?DzGkx1HTFaFreyNCCH-Kz9DIBFYs3Uwv 8pCoZQ2Gqw4PidtXvSDigLP2zGcxP		

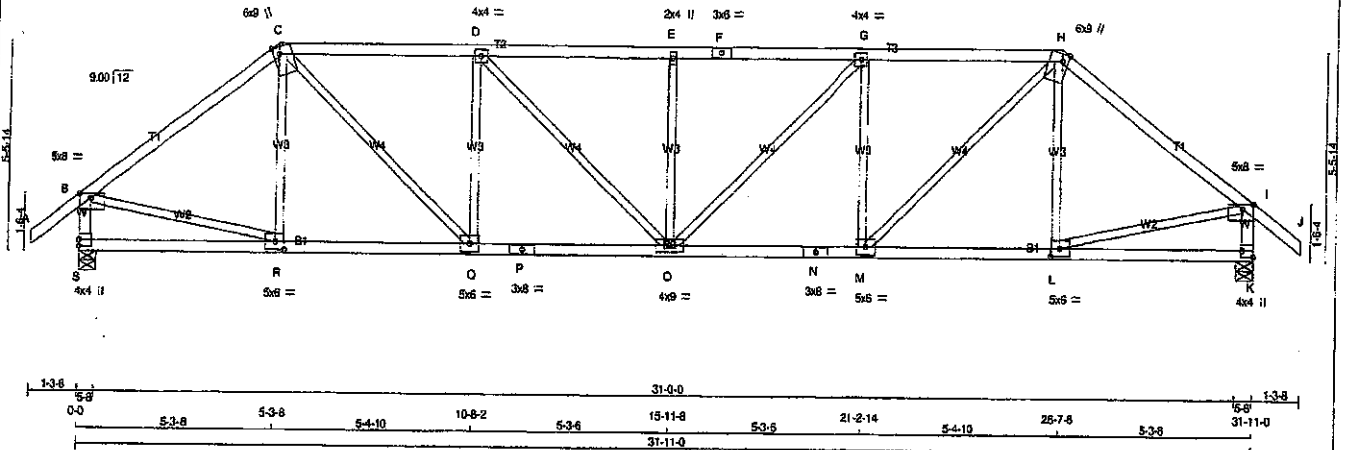
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	14-10-8	-2188	-2188	---	FRONT	VERT	TOTAL	---	---
Y	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
Z	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AA	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
DWG# T-1911498

JOB NAME 403045	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:21 2019 Page 1
 ID:D7uIPnW9?DzGkx1HTFaFreyNCC-H-PmItoJzI?CIVUMW4tUHDPOCLgT1deT4MR7zGdD
 1-3-8 0-0 5-3-8 5-3-8 5-4-10 10-8-2 5-3-6 15-11-8 5-3-6 21-2-14 5-4-10 25-7-8 5-3-8 31-11-0 33-2-8
 Scale = 1:33.3



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
S - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
S - P	2x4	DRY	No.2
P - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW-p	MT20	5.0	8.0	1.50 3.50
C TTWW-m	MT20	5.0	9.0	Edge 2.00
D TMVW-t	MT20	4.0	4.0	
E TMVW-w	MT20	2.0	4.0	
F TS-t	MT20	3.0	8.0	
G TMVW-H	MT20	4.0	4.0	
H TTWW-m	MT20	6.0	9.0	Edge 2.00
I TMVW-p	MT20	5.0	8.0	1.50 3.50
K BMV1-p	MT20	4.0	4.0	2.00 Edge
L BMVW-t	MT20	5.0	6.0	2.50 2.75
M BMVW-t	MT20	5.0	6.0	
N BS-t	MT20	3.0	8.0	
O BMVWW-t	MT20	4.0	9.0	
P BS-t	MT20	3.0	8.0	
Q BMVW-t	MT20	5.0	6.0	
R BMVW-t	MT20	5.0	6.0	2.50 2.75
S BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
S	2385 0	2385 0	0 0	5-8	5-8
K	2385 0	2385 0	0 0	5-8	5-8

UNFACTORED REACTIONS		1ST LOOSE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
S	1773	1007 / 0	335 / 0
K	1773	1007 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING		TOTAL LOAD CASES: (4)	
CHORDS		WEBS	
MEMB.	FORCE (LBS)	MAX. FACTORED	MAX. FACTORED
FR-TO	FROM TO	VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (LBS)
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00
B-C	-2429 / 0	-102.1 -102.1 0.73 (1)	3.87
C-D	-3138 / 0	-102.1 -102.1 0.85 (1)	3.34
D-E	-3494 / 0	-102.1 -102.1 0.89 (1)	3.14
E-F	-3494 / 0	-102.1 -102.1 0.89 (1)	3.14
F-G	-3494 / 0	-102.1 -102.1 0.89 (1)	3.14
G-H	-3138 / 0	-102.1 -102.1 0.85 (1)	3.34
H-I	-2429 / 0	-102.1 -102.1 0.73 (1)	3.87
I-J	0 / 42	-102.1 -102.1 0.14 (1)	10.00
S-B	-2301 / 0	0.0 0.0 0.24 (1)	5.62
K-I	-2301 / 0	0.0 0.0 0.24 (1)	5.62
S-R	0 / 0	-38.5 -38.5 0.21 (3)	10.00
R-Q	0 / 1936	-38.5 -38.5 0.47 (2)	10.00
Q-P	0 / 3138	-38.5 -38.5 0.80 (1)	10.00
P-O	0 / 3138	-38.5 -38.5 0.80 (1)	10.00
O-N	0 / 3138	-38.5 -38.5 0.80 (1)	10.00
N-M	0 / 3138	-38.5 -38.5 0.80 (1)	10.00
M-L	0 / 1936	-38.5 -38.5 0.47 (2)	10.00
L-K	0 / 0	-38.5 -38.5 0.21 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
 ALLOWABLE DEFL. (TL) = L/360 (1.06")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.29")

CSR: TC=0.79/1.00 (H-T), BC=0.60/1.00 (O-Q), WS=0.45/1.00 (L-L), SSI=0.26/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

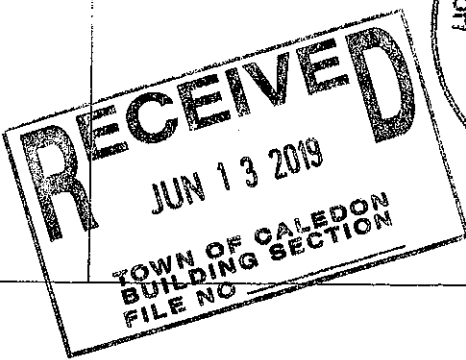
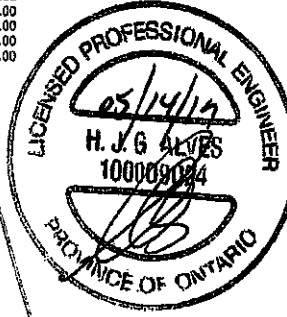
NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 798 1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.88 (L) (INPUT = 0.99)

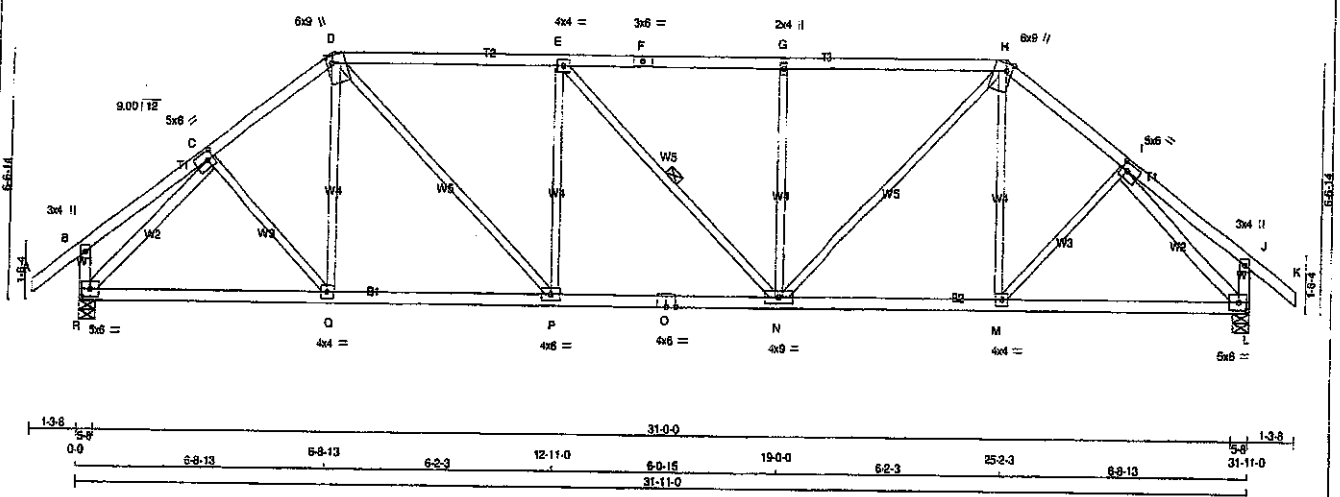
JSI METAL=0.96 (P) (INPUT = 1.00)



Structural component only
 DWG# T-1911484

JOB NAME 403045	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:22 2019 Page 1
 ID:D7ulPnW87DzGkx1HTFaFr9yNCCH-lys77zNWqMKexu4nOjERYA7iODPrHmt7qv_RzGdOh
 1-3-8 0-0 3-5-11 3-5-11 6-9-13 6-9-13 12-11-0 19-0-0 6-9-13 25-2-3 3-5-11 3-5-11 3-5-11
 Scale = 1/50.3



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
K - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
R - C	2x4	DRY	No.2
I - L	2x4	DRY	No.2

DRY: SEASONED LUMBER

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW-p	MT20	3.0	4.0
C	TMVW-t	MT20	5.0	6.0 Edge 2.00
D	TTWW+m	MT20	6.0	9.0 Edge 2.00
E	TMVW-t	MT20	4.0	4.0
F	TS-1	MT20	3.0	8.0
G	TMVW-w	MT20	2.0	4.0
H	TTWW+m	MT20	6.0	9.0 Edge 2.00
I	TMVW-t	MT20	5.0	6.0 Edge 2.00
J	TMVW-p	MT20	3.0	4.0
L	BMVW-t	MT20	5.0	6.0 2.50 2.75
M	BMVW-t	MT20	4.0	4.0
N	BMVW-t	MT20	4.0	9.0
O	BS-1	MT20	4.0	8.0
P	BMVW-t	MT20	4.0	8.0
Q	BMVW-t	MT20	4.0	4.0
R	BMVW-t	MT20	5.0	6.0 2.50 2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	UP	BRG
R	2385 0	2385 0	0	5-8	5-8
L	2385 0	2385 0	0	5-8	5-8

UNFACTORED REACTIONS		1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
R	1773	1007 / 0	335 / 0
L	1773	1007 / 0	335 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 3.34 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
 TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	MAX. (PL)	MEMB.	FORCE (LBS)
FR-TO				FR-TO	
A-B	0 / 42	-102.1	-102.1 0.14 (1)	C-Q	0 / 201
B-C	0 / 21	-102.1	-102.1 0.16 (1)	Q-D	0 / 237
C-D	-2441 / 0	-102.1	-102.1 0.25 (1)	D-P	0 / 1265
D-E	-2822 / 0	-102.1	-102.1 0.79 (1)	P-E	-677 / 0
E-F	-2820 / 0	-102.1	-102.1 0.78 (1)	E-N	-2 / 0
F-G	-2820 / 0	-102.1	-102.1 0.78 (1)	N-G	-678 / 0
G-H	-2820 / 0	-102.1	-102.1 0.78 (1)	H-N	0 / 1262
H-I	-2441 / 0	-102.1	-102.1 0.25 (1)	M-H	0 / 237
I-J	0 / 21	-102.1	-102.1 0.18 (1)	M-I	0 / 201
J-K	0 / 42	-102.1	-102.1 0.14 (1)	R-C	-2898 / 0
R-B	-276 / 0	0.0	0.0 0.03 (1)	I-L	-2698 / 0
L-J	-276 / 0	0.0	0.0 0.03 (1)		
R-Q	0 / 1809	-38.5	-38.5 0.55 (2)		
Q-P	0 / 1933	-38.5	-38.5 0.57 (2)		
P-O	0 / 2822	-38.5	-38.5 0.59 (1)		
O-N	0 / 2822	-38.5	-38.5 0.59 (1)		
N-M	0 / 1834	-38.5	-38.5 0.57 (2)		
M-L	0 / 1809	-38.5	-38.5 0.55 (2)		

TOTAL WEIGHT = 2 X 142 = 283 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/IC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 089-09, CSA 089-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.14")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.24")

CSI: TC=0.78/1.00 (D-E:1), BC=0.59/1.00 (N-P:1), WB=0.77/1.00 (L-1), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

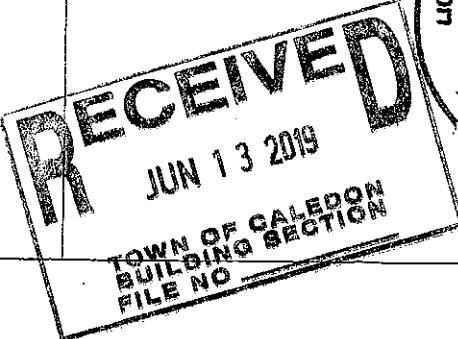
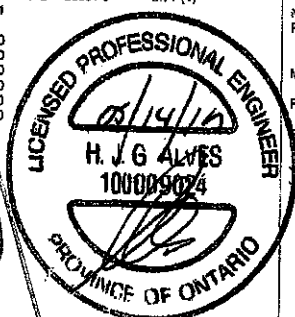
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

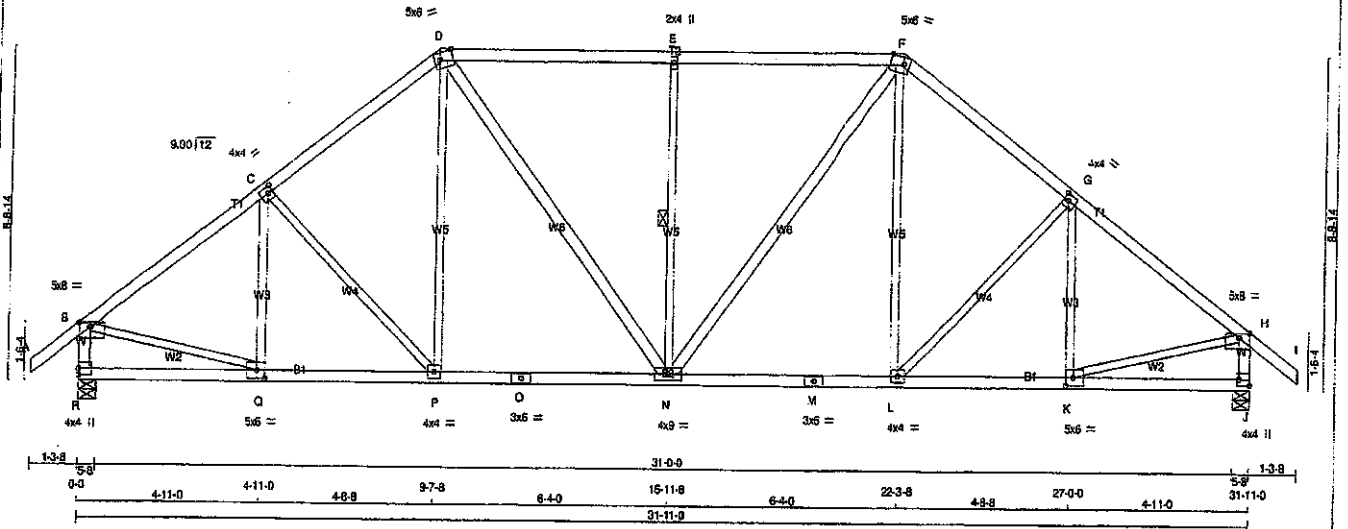
SI GRIP = 0.89 (P) (INPUT = 0.90)
 SI METAL = 0.73 (O) (INPUT = 1.00)



Structural component only
 DWG# T-1911485

JOB NAME 403045	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MTok Industries, Inc. Tue May 14 10:41:24 2019 Page 1
 ID:D7uIPnW9?DzGKc1HTFaFryNCCH-pL_IQL?dH744ax4HBCQBJs1ZFVjGtpa3LRJ02JzGdDf
 1-3-8 0-0 4-11-0 4-11-0 4-8-8 9-7-8 6-4-0 15-11-8 6-4-0 22-3-8 4-8-8 27-0-0 31-11-0 33-2-8
 1-3-8 0-0 4-11-0 4-11-0 4-8-8 9-7-8 6-4-0 15-11-8 6-4-0 22-3-8 4-8-8 27-0-0 31-11-0 33-2-8
 Scale = 1:53.3



CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT D - N 2x4 DRY No.2

N - F 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMWW-p MT20	5.0	8.0	1.50	3.50
C TMWW-t MT20	4.0	4.0	2.00	1.50
D TTWW-m MT20	5.0	8.0	Edge	4.25
E TMWW-w MT20	2.0	4.0		
F TTWW-m MT20	5.0	8.0	Edge	4.25
G TMWW-t MT20	4.0	4.0	2.00	1.50
H TMWW-p MT20	5.0	8.0	1.50	3.50
J BMVW-p MT20	4.0	4.0	2.00	Edge
K BMVW-t MT20	5.0	8.0	2.50	2.50
L BMVW-t MT20	4.0	4.0		
M BS-t MT20	3.0	6.0		
N BMVWW-t MT20	4.0	9.0		
O BS-t MT20	3.0	6.0		
P BMVWW-t MT20	4.0	4.0		
Q BMVWW-t MT20	5.0	6.0	2.50	2.50
R BMV1-p MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT VERT	2385 0	2385 0	0 0	5-8
R	2385 0	2385 0	0 0	5-8
J	2385 0	2385 0	0 0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
R	1773 1007/0 335/0 0/0 0/0 431/0 0/0
J	1773 1007/0 335/0 0/0 0/0 431/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
A-B	0/42	-102.1 -102.1 0.14 (1)	Q-C	-285/38 0.11 (1)
B-C	-2445/0	-102.1 -102.1 0.40 (1)	C-P	-265/0 0.23 (1)
C-D	-2285/0	-102.1 -102.1 0.38 (1)	P-D	0/425 0.10 (2)
D-E	-2183/0	-102.1 -102.1 0.61 (1)	D-N	0/852 0.10 (1)
E-F	-2183/0	-102.1 -102.1 0.61 (1)	N-E	-791/0 0.38 (1)
F-G	-2286/0	-102.1 -102.1 0.38 (1)	N-F	0/852 0.10 (1)
G-H	-2445/0	-102.1 -102.1 0.40 (1)	L-F	0/425 0.10 (2)
H-I	0/42	-102.1 -102.1 0.14 (1)	L-G	-265/0 0.23 (1)
R-B	-2304/0	0.0 0.0 0.24 (1)	K-G	-285/38 0.11 (1)
J-H	-2304/0	0.0 0.0 0.24 (1)	B-Q	0/2041 0.46 (1)
R-Q	0/0	-38.5 -38.5 0.15 (3)	K-H	0/2041 0.46 (1)
Q-P	0/1983	-38.5 -38.5 0.45 (1)		
P-O	0/1802	-38.5 -38.5 0.49 (2)		
O-N	0/1802	-38.5 -38.5 0.49 (2)		
N-M	0/1802	-38.5 -38.5 0.49 (2)		
M-L	0/1802	-38.5 -38.5 0.49 (2)		
L-K	0/1802	-38.5 -38.5 0.45 (1)		
K-J	0/0	-38.5 -38.5 0.15 (3)		

TOTAL WEIGHT = 2 X 154 = 307 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/CI

LOADING IN FLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2018, CBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.81/1.00 (D-E:1), BC=0.49/1.00 (N-P:2), WB=0.48/1.00 (H-K:1), SS=0.31/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

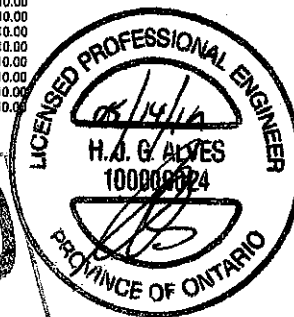
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1987 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

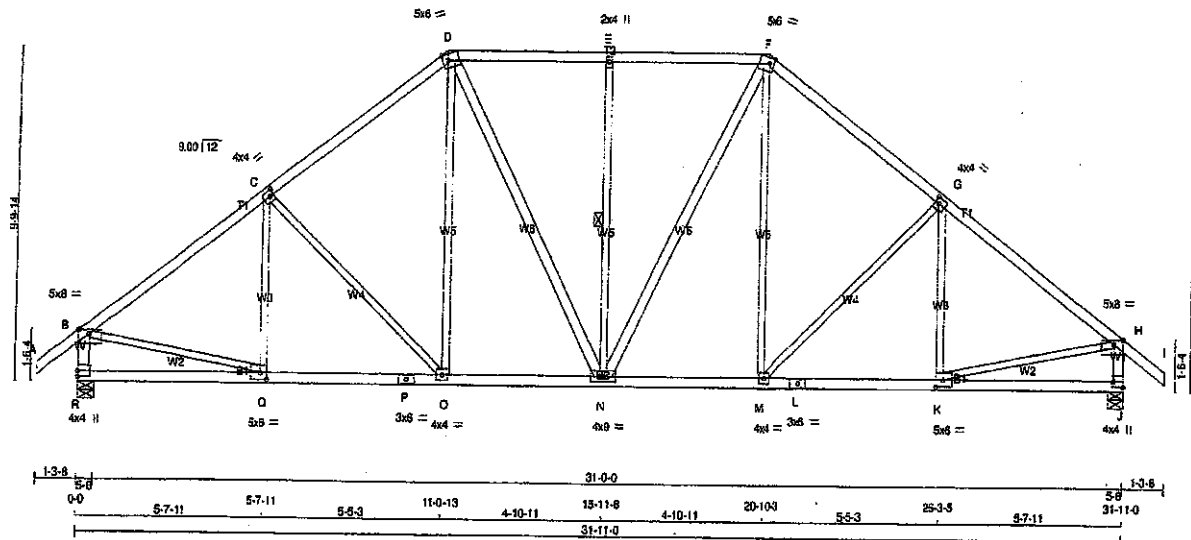
JSI GRIP=0.88 (C) (INPUT = 0.90)
 JSI METAL=0.89 (C) (INPUT = 1.00)



Structural component only
 DWG# T-1911487

JOB NAME: 403045 TRUSS NAME: T6 QUANTITY: 6 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 MTek Industries, Inc. Tue May 14 10:41:25 2019 Page 1
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ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A - D	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	6.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	6.0	2.50	2.50
L	BS-t	MT20	3.0	6.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	6.0		
Q	BMVW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT		REQD	
	VERT	HORZ	DOWN	UP	IN-SX	IN-SX	IN-SX	IN-SX
R	2385	0	2385	0	5-8	5-8	5-8	5-8
J	2385	0	2385	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
J	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (L) (PLF)	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO			FR-TO		
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	Q-C	-185 / 119	0.09 (1)
B-C	-2474 / 0	-102.1 -102.1	0.54 (1)	3.88	Q-D	-422 / 0	0.52 (1)
C-D	-2181 / 0	-102.1 -102.1	0.49 (1)	4.13	C-O	0 / 498	0.11 (2)
D-E	-1916 / 0	-102.1 -102.1	0.35 (1)	4.50	D-N	0 / 440	0.07 (1)
E-F	-1916 / 0	-102.1 -102.1	0.35 (1)	4.50	N-E	-803 / 0	0.39 (1)
F-G	-2181 / 0	-102.1 -102.1	0.49 (1)	4.13	N-F	0 / 440	0.07 (1)
G-H	-2474 / 0	-102.1 -102.1	0.54 (1)	3.88	M-F	0 / 498	0.11 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	M-G	-422 / 0	0.52 (1)
I-J	-2286 / 0	0.0 0.0	0.24 (1)	5.63	K-G	-185 / 119	0.09 (1)
J-K	-2286 / 0	0.0 0.0	0.24 (1)	5.63	B-Q	0 / 2055	0.46 (1)
					Q-K	0 / 2055	0.46 (1)
R-Q	0 / 0	-38.5 -38.5	0.22 (3)	10.00			
Q-P	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
P-O	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
O-N	0 / 1715	-38.5 -38.5	0.38 (1)	10.00			
N-M	0 / 1715	-38.5 -38.5	0.38 (1)	10.00			
M-L	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
L-K	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.54/1.00 (B-C:1), BC=0.48/1.00 (C-D:2), WB=0.52/1.00 (C-O:1), SS=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

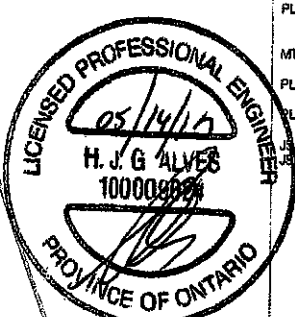
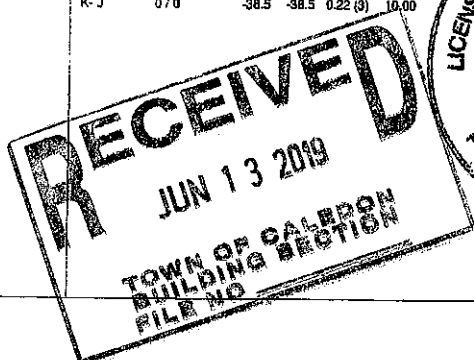
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JS GRIP= 0.85 (B) (INPUT = 0.90)
METAL= 0.80 (P) (INPUT = 1.00)



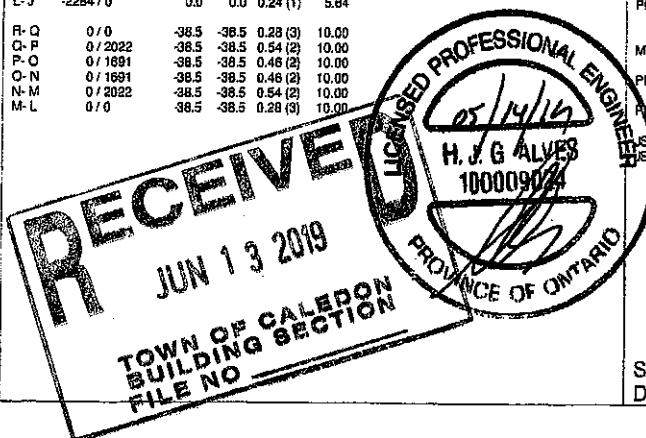
Structural component only
DWG# T-1911488

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T7	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

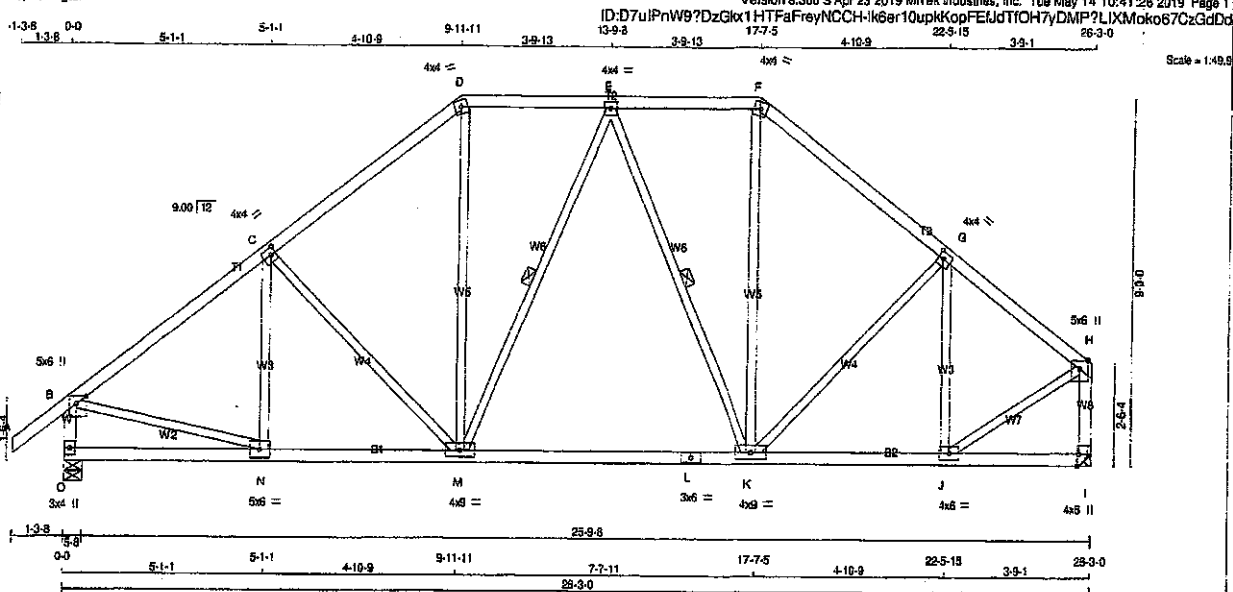
LUMBER		LUMBER		DESCRIPTION
M. L. G. A. RULES				
CHARDS	SIZE			
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEISS EXCEPT	2x3	DRY	No.2	SPF
P - E	2x4	DRY	No.2	SPF
P - F	2x4	DRY	No.2	SPF
F - N	2x4	DRY	No.2	SPF
N - G	2x4	DRY	No.2	SPF

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.50	3.50
C	TMWV-p	MT20	4.0	4.0	2.00	1.90
D	TS-1	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	4.0	2.00	1.75
F	TMWV-1	MT20	4.0	4.0	2.00	1.75
G	TTW-m	MT20	4.0	4.0	2.00	1.75
H	TS-1	MT20	3.0	6.0		
I	TMWV-1	MT20	4.0	4.0	2.00	1.50
J	TMWV-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWV-1	MT20	5.0	6.0	2.50	2.75
N	BMWVW-1	MT20	4.0	9.0		
O	GS-1	MT20	3.0	6.0		
P	BMWVW-1	MT20	4.0	9.0		
Q	BMWV-1	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY



JOB NAME 403045	TRUSS NAME T8	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 4 X 126 = 503 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - H	2x4	DRY	No.2	SPF	
O - B	2x4	DRY	No.2	SPF	
I - H	2x4	DRY	No.2	SPF	
O - L	2x4	DRY	No.2	SPF	
L - I	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWW-p	MT20	5.0	6.0	Edge	3.00
C TMWW-I	MT20	4.0	4.0	2.00	1.50
D TTW-m	MT20	4.0	4.0		
E TMWW-I	MT20	4.0	4.0		
F TTW-m	MT20	4.0	4.0		
G TMWW-I	MT20	4.0	4.0	2.00	1.50
H TMWW-p	MT20	5.0	6.0	Edge	
I BMV-I	MT20	4.0	8.0	Edge	0.50
J BMWW-I	MT20	4.0	9.0		
K BMWW-I	MT20	3.0	6.0		
L BS-I	MT20	3.0	6.0		
M BMWW-I	MT20	4.0	9.0		
N BMWW-I	MT20	5.0	6.0		
O BMV-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED	MAXIMUM FACTORED	INPUT	RECORD	
	GROSS REACTION	GROSS REACTION	BRG	BRG	
JT	VERT	DOWN	HORIZ	UPLIFT	IN-SX
O	1986	0	1986	0	5-8
I	1845	0	1845	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
O	1476	842 / 0	276 / 0	0 / 0	0 / 0	359 / 0	0 / 0
I	1379	762 / 0	276 / 0	0 / 0	0 / 0	341 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M, E-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS						WEBS					
MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MEMB.	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX
	(LBS)	(PLF)		(LC)			(LBS)	(PLF)		(LC)	
FR-TO						FR-TO					
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-C	-178 / 83	0.08 (1)			
B-C	-1948 / 0	-102.1	-102.1	0.39 (1)	4.46	C-M	-369 / 0	0.35 (1)			
C-D	-1897 / 0	-102.1	-102.1	0.37 (1)	4.73	M-D	0 / 654	0.15 (1)			
D-E	-1333 / 0	-102.1	-102.1	0.20 (1)	5.41	M-E	-152 / 12	0.10 (1)			
E-F	-1253 / 0	-102.1	-102.1	0.19 (1)	5.54	E-K	-354 / 0	0.23 (1)			
F-G	-1590 / 0	-102.1	-102.1	0.32 (1)	4.91	K-F	0 / 592	0.13 (1)			
G-H	-1514 / 0	-102.1	-102.1	0.28 (1)	5.05	K-G	0 / 732	0.01 (3)			
O-B	-1502 / 0	0.0	0.0	0.20 (1)	6.09	J-G	-569 / 0	0.24 (1)			
I-H	-1755 / 0	0.0	0.0	0.24 (1)	6.26	B-N	0 / 1628	0.37 (1)			
						J-H	0 / 1439	0.32 (1)			
O-N	0 / 0	-38.5	-38.5	0.16 (3)	10.00						
N-M	0 / 1585	-38.5	-38.5	0.47 (2)	10.00						
M-L	0 / 1392	-38.5	-38.5	0.45 (2)	10.00						
L-K	0 / 1392	-38.5	-38.5	0.45 (2)	10.00						
K-J	0 / 1242	-38.5	-38.5	0.43 (2)	10.00						
J-I	0 / 0	-38.5	-38.5	0.09 (3)	10.00						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.86")
CALCULATED VERT. DEFL. (LL) = L/998 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.86")
CALCULATED VERT. DEFL. (TL) = L/999 (0.17")

CSI: TC=0.39/1.00 (B-C:1), BC=0.47/1.00 (M-N:2), WS=0.37/1.00 (B-N:1), SS=0.20/1.00 (B-C:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEAD=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

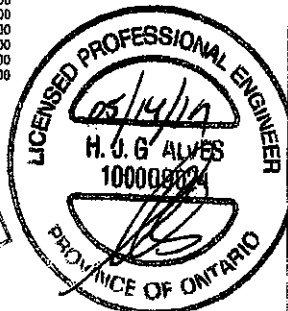
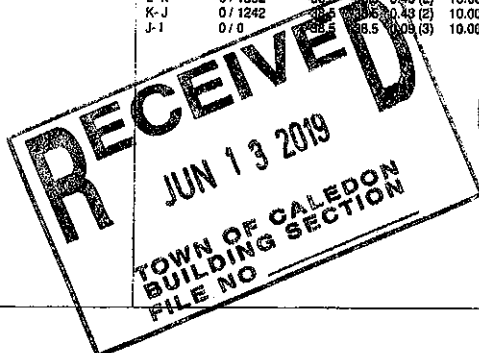
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(FBI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

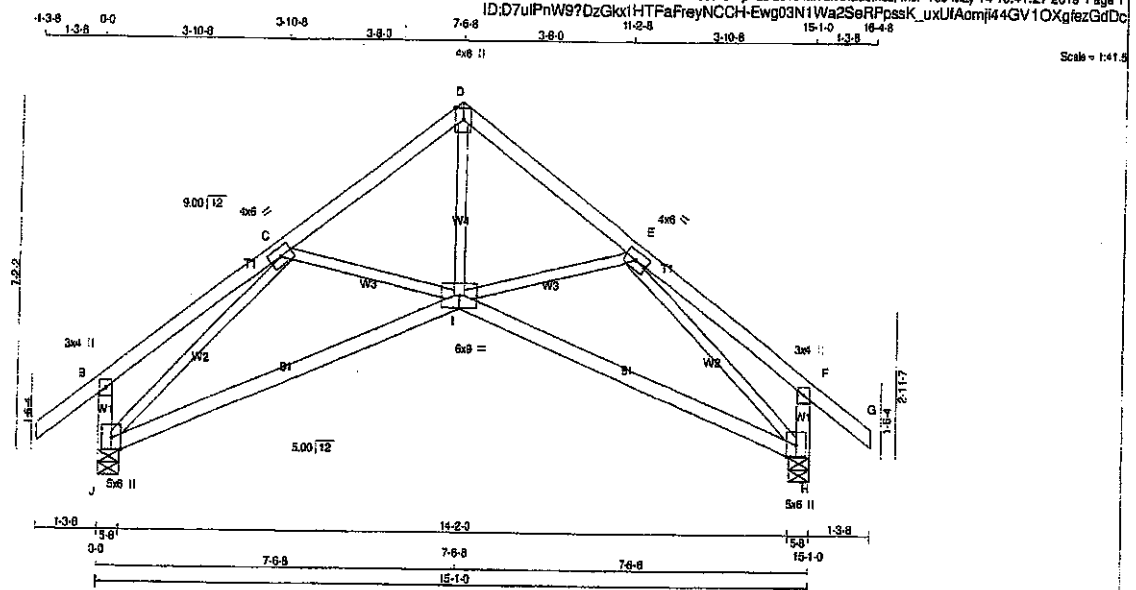
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (B) (INPUT = 0.90)
JSI METAL = 0.58 (H) (INPUT = 1.00)



Structural component only
DWG# T-1911490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T9	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMW-w	MT20	4.0	6.0		
D	TTW-p	MT20	4.0	6.0	Edge	
E	TMW-w	MT20	4.0	6.0		
F	TMV-p	MT20	3.0	4.0		
H	BVMW-p	MT20	5.0	6.0	Edge	
I	BBWW-p	MT20	6.0	9.0		
J	BVMW-p	MT20	5.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
J	1201	0	0	5-8
H	1201	0	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
J	COMBINED	SNOW	LIVE	PERM. LIVE	
J	889	518 / 0	158 / 0	0 / 0	213 / 0
H	889	518 / 0	158 / 0	0 / 0	213 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 1239
B-C	0 / 24	-102.1	-102.1	0.21 (1)	10.00	I-E	-68 / 79
C-D	-1310 / 0	-102.1	-102.1	0.18 (1)	5.47	C-I	-68 / 79
D-E	-1310 / 0	-102.1	-102.1	0.18 (1)	5.47	J-C	-1637 / 0
E-F	0 / 24	-102.1	-102.1	0.21 (1)	10.00	E-H	-1637 / 0
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
J-B	-294 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-294 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 1185	-38.5	-38.5	0.63 (2)	10.00		
I-H	0 / 1185	-38.5	-38.5	0.63 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/742 (0.24")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/444 (0.41")

CSI: TC=0.21/1.00 (B-C:1), BC=0.63/1.00 (I-J:2), WB=0.85/1.00 (E-H:1), SS=0.15/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1956

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.70 (J) (INPUT = 0.90)
JSI METAL = 0.39 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911491

Tamarack Roof Truss, Burlington

 Version 8.300 \$ Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:30 2019 Page 1
 ID: D7uIPnW9?DzGlx1HTFaFreyNCCH-eVL9hO3OlzqDisYQYTXbY7H?Zr6HcQxjIMmKGz3GdDZ

Dimensions: 1-3-8, 1-3-8, 0-0, 3-10-8, 3-10-8, 7-9-0, 3-10-8, 1-3-8, 9-9-8

Scale = 1/16"



Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT	RECORD			
GROSS REACTION		GROSS REACTION		BRG	BRG	HEEL		
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	WEDGE
B	683	0	683	0	0	3-8	3-8	2x4 L
D	683	0	683	0	0	3-8	3-8	2x4 R

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	502	304 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0
D	502	304 / 0	81 / 0	0 / 0	0 / 0	117 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MAX. FACTORED		FACTORED		MAX. FACTORED		MAX. FACTORED		
MEMB.	FORCE (LBS)	VERT. LOAD (K)	MAX. CSI (LC)	MAX. UNBRAC	MEMB.	FORCE (LBS)	MAX. CSI (LC)	
FR-TO	FROM	TO	LENGTH	FR-TO	FROM	TO	LENGTH	
A-B	0/21	-102.1	-102.1	0.13 (1)	10.00	F-C	0/235	0.05 (2)
B-H	-649/0	-102.1	-102.1	0.07 (1)	6.25	G-H	-80/97	0.00 (1)
H-C	-580/0	-102.1	-102.1	0.15 (1)	6.25	I-J	-80/97	0.00 (1)
C-J	-580/0	-102.1	-102.1	0.15 (1)	6.25			
J-D	-849/0	-102.1	-102.1	0.07 (1)	6.25			
D-E	0/21	-102.1	-102.1	0.13 (1)	10.00			
B-G	0/521	-38.5	-38.5	0.16 (1)	10.00			
G-F	0/521	-38.5	-38.5	0.20 (1)	10.00			
F-I	0/521	-38.5	-38.5	0.20 (1)	10.00			
I-D	0/521	-38.5	-38.5	0.18 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD				= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (C-H:1), BC=0.20/1.00 (F-I:1)
WB=0.06/1.00 (C-F:2), SSI=0.13/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

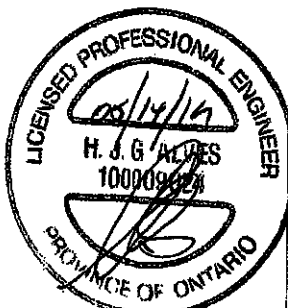
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PL)		(PL)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	818	354	1867	788	1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

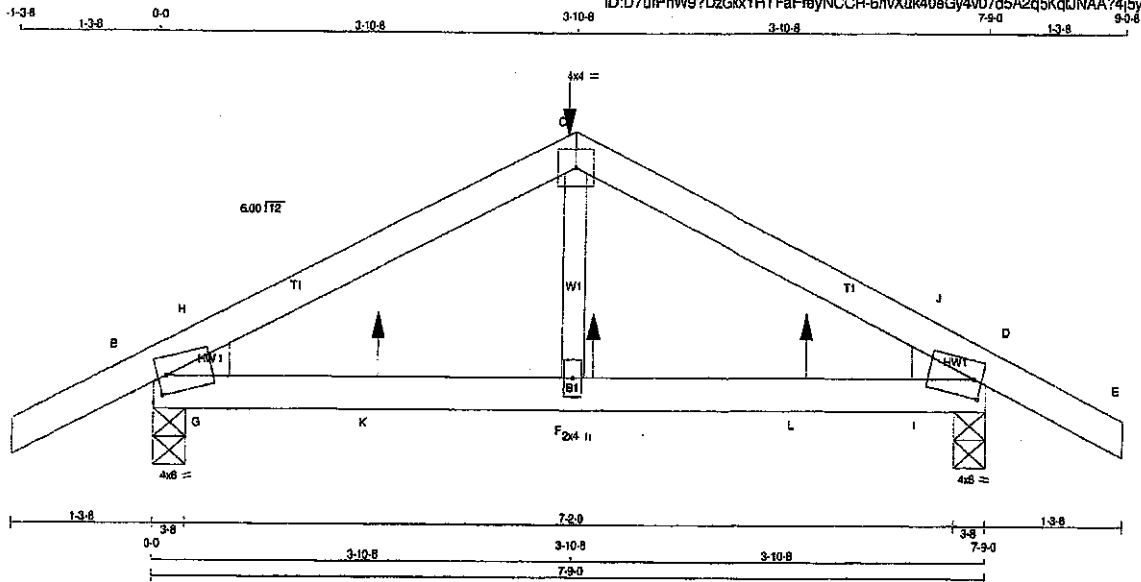
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (B) (INPUT = 0.90)
JSI METAL = 0.15 (D) (INPUT = 1.00)



Structural component only
DWG# T-1911495

JOB NAME 403045	TRUSS NAME T912	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 5 Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:31 2019 Page 1			
		ID: D7uIPnW9?DzGlx1HTFaFbyNCC8-6hvXuk40eGy4w07d5A2q5KqJNAA74i5y0VtoPzGdDY			



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 25 b (lb)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER				

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMBH1-m	MT20	4.0	6.0	2.00 0.75
C TTW-p	MT20	4.0	4.0	
D TMBH1-m	MT20	4.0	6.0	2.00 0.75
F BMW-w	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD		HEEL	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	UPLIFT	IN-SX	IN-SX	IN-SX	WEDGE	
B	792	0	792	0	0	0	3-8	3-8	3-8	2x4 L	
D	790	0	790	0	0	0	3-8	3-8	3-8	2x4 R	

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	577	363 / 0	82 / 0	0 / 0	0 / 0	132 / 0	0 / 0
D	576	363 / 0	82 / 0	0 / 0	0 / 0	132 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 21	-102.1	-102.1 0.14 (1)	10.00	F-C	0 / 201	0.05 (7)
B-H	-890 / 0	-102.1	-102.1 0.13 (1)	6.25	G-H	-65 / 108	0.00 (1)
H-C	-802 / 0	-102.1	-102.1 0.17 (5)	6.25	I-J	-65 / 108	0.00 (1)
C-J	-802 / 0	-102.1	-102.1 0.17 (5)	6.25			
J-D	-890 / 0	-102.1	-102.1 0.13 (1)	6.25			
D-E	0 / 21	-102.1	-102.1 0.14 (1)	10.00			
B-G	0 / 708	-38.5	-38.5 0.18 (5)	10.00			
G-K	0 / 708	-38.5	-38.5 0.21 (1)	10.00			
K-F	0 / 708	-38.5	-38.5 0.21 (1)	10.00			
F-L	0 / 708	-38.5	-38.5 0.21 (1)	10.00			
L-I	0 / 708	-38.5	-38.5 0.21 (1)	10.00			
I-D	0 / 708	-38.5	-38.5 0.18 (5)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-38	-41	—	FRONT	VERT	DEAD	—	—
C	3-10-8	-1	-2	117	FRONT	VERT	TOTAL	—	—
C	3-10-8	-210	-210	—	FRONT	VERT	SNOW	—	—
F	4-0-12	11	1	18	FRONT	VERT	TOTAL	—	—
K	2-0-12	11	1	18	FRONT	VERT	TOTAL	—	—
L	6-0-12	11	1	18	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.28")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.28")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.171/1.00 (C-H5), BC=0.21/1.00 (F-G:1), WB=0.05/1.00 (C-F:7), SS=0.15/1.00 (C-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

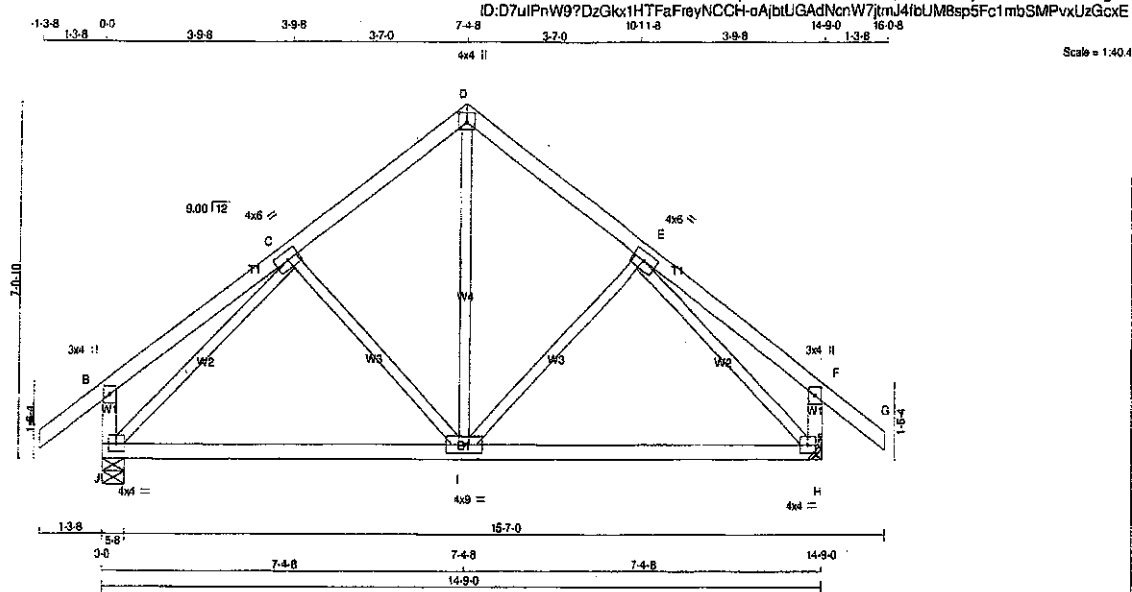
PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP= 0.70 (C) (INPUT = 0.90)
ISI METAL= 0.21 (B) (INPUT = 1.00)

Structural component only
DWG# T-1911496



JOB NAME 401607	TRUSS NAME T100	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N L G A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS		2x3	DRY	No.2
EXCEPT				SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-1	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	4.0	2.25 2.00
E	TMWW-1	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW-1	MT20	4.0	4.0	
I	BMVW-1	MT20	4.0	9.0	
J	BMVW-1	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED		MAXIMUM FACTORED		INPUT	RECORD
	GROSS REACTION	GROSS REACTION	GROSS REACTION	GROSS REACTION		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
J	1178	0	1178	0	5-8	5-8
H	1178	0	1178	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST CASE - MAX/MIN COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	872	508/0	155/0	0/0	0/0	208/0	0/0
H	872	508/0	155/0	0/0	0/0	208/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)
FR-TO				FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	I-D	0/616
B-C	0/26	-102.1	-102.1	0.22 (1)	10.00	I-E	-181/24
C-D	-783/0	-102.1	-102.1	0.17 (1)	6.25	C-I	-181/24
D-E	-783/0	-102.1	-102.1	0.17 (1)	6.25	J-C	-1072/0
E-F	0/26	-102.1	-102.1	0.22 (1)	10.00	E-H	-1072/0
F-G	0/42	-102.1	-102.1	0.14 (1)	10.00		
J-B	-285/0	0.0	0.0	0.03 (1)	7.81		
H-F	-285/0	0.0	0.0	0.03 (1)	7.81		
J-I	0/730	-38.5	-38.5	0.54 (2)	10.00		
I-H	0/730	-38.5	-38.5	0.54 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	PSF
DL	23.0	PSF
DL	6.0	PSF
BOT CH.	LL	PSF
DL	10.5	PSF
DL	7.0	PSF
TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCG 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 23.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSH: TO=0.22/1.00 (B-C:1), BC=0.54/1.00 (H-J:2), WB=0.50/1.00 (C-J:1), SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

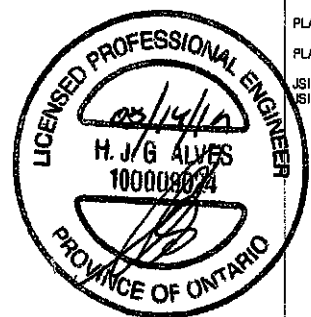
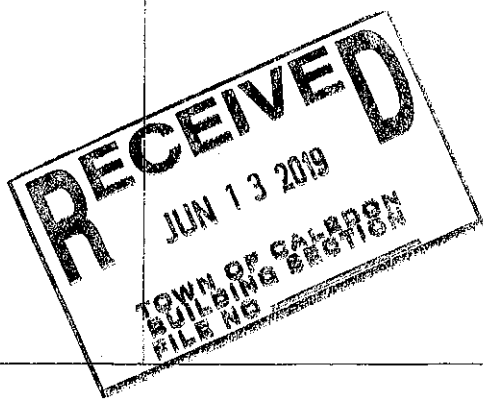
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	818 354	1667 788

PLATE PLACEMENT TOL. = 0.250 inches

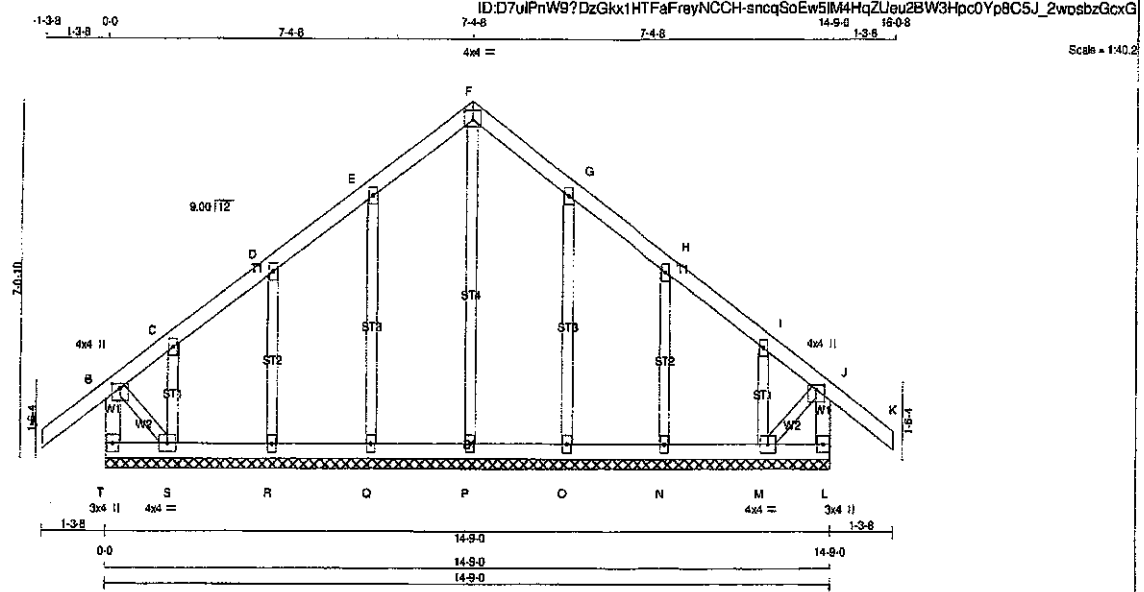
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.89 (D) (INPUT = 0.90)
JSI METAL=0.27 (J) (INPUT = 1.00)



Structural component only
DWG# T-1911516

JOB NAME 401607	TRUSS NAME G100	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 69 lb

LUMBER

CHORDS	SIZE	DRY	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT 2'-0" OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I	TMVW+w	MT20	2.0	4.0		
F	TTW+p	MT20	4.0	4.0	2.25	2.00
J	TMVW+p	MT20	4.0	4.0	1.00	2.00
L	BMV1+p	MT20	3.0	4.0		
M	BMVW1-i	MT20	4.0	4.0		
N, O, P, Q, R	TMVW+w	MT20	2.0	4.0		
S	BMVW1-i	MT20	4.0	4.0		
T	BMV1+p	MT20	3.0	4.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. FACTORED (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	LC1 MAX. FACTORED (LBS)	
FR-TO		FROM TO	LENGTH	FR-TO			
T-B	-310 / 0	0.0 0.0 0.03 (1)	7.81	P-F	-157 / 0	0.13 (1)	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	Q-E	-228 / 0	0.11 (1)	
B-C	-68 / 0	-102.1 -102.1 0.13 (1)	8.25	R-D	-210 / 0	0.05 (1)	
C-D	-16 / 0	-102.1 -102.1 0.05 (1)	8.25	S-C	-85 / 0	0.01 (1)	
D-E	-16 / 0	-102.1 -102.1 0.06 (1)	8.25	O-G	-229 / 0	0.11 (1)	
E-F	-25 / 0	-102.1 -102.1 0.06 (1)	8.25	N-H	-210 / 0	0.05 (1)	
F-G	-25 / 0	-102.1 -102.1 0.08 (1)	8.25	M-I	-85 / 0	0.01 (1)	
G-H	-16 / 0	-102.1 -102.1 0.08 (1)	8.25	D-S	0 / 30	0.01 (1)	
H-I	-16 / 0	-102.1 -102.1 0.05 (1)	8.25	M-J	0 / 30	0.01 (1)	
I-J	-68 / 0	-102.1 -102.1 0.13 (1)	8.25				
J-K	0 / 42	-102.1 -102.1 0.14 (1)	10.00				
L-J	-310 / 0	0.0 0.0 0.03 (1)	7.81				
T-S	0 / 0	-38.5 -38.5 0.02 (3)	10.00				
S-R	0 / 19	-38.5 -38.5 0.03 (2)	10.00				
R-Q	0 / 14	-38.5 -38.5 0.02 (3)	10.00				
Q-P	0 / 10	-38.5 -38.5 0.02 (3)	10.00				
P-O	0 / 10	-38.5 -38.5 0.02 (3)	10.00				
O-N	0 / 14	-38.5 -38.5 0.02 (3)	10.00				
N-M	0 / 19	-38.5 -38.5 0.03 (2)	10.00				
M-L	0 / 0	-38.5 -38.5 0.02 (3)	10.00				

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (M-N:2), WB=0.13/1.00 (P-P:1), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

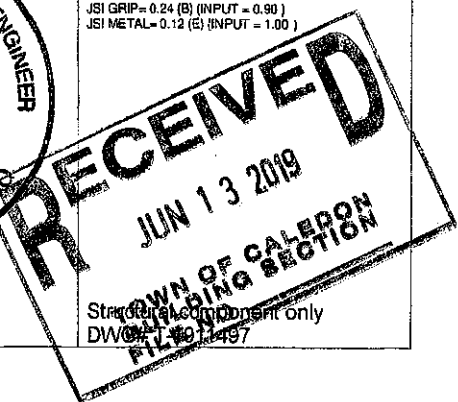
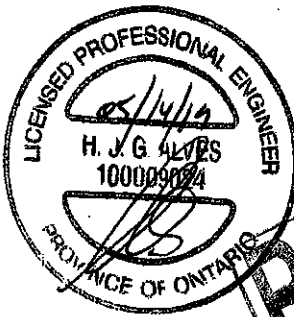
PLATE	GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)		
MAX	MIN	MAX	MIN	
MT20	618	354	1667	788
		1987	1656	

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.24 (B) (INPUT = 0.90)

JSI METAL = 0.12 (E) (INPUT = 1.00)



Tamarack Roof Truss, Burlington

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DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGN CRITERIA

TOTAL WEIGHT = 2 X 28 = 56 lb

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH

ALL TYPICAL BRIDGES AND PERMANENT CONNECTIONS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (C-D:1), BC=0.08/1.00 (H-I:2),
WB=0.04/1.00 (D-G:1), SSI=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1887	788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg

JSI GRIP= 0.20 (B) (INPUT = 0.90)
JSI METAL= 0.05 (H) (INPUT = 1.00)

11

RECEIVED

FILE

11/13/2019

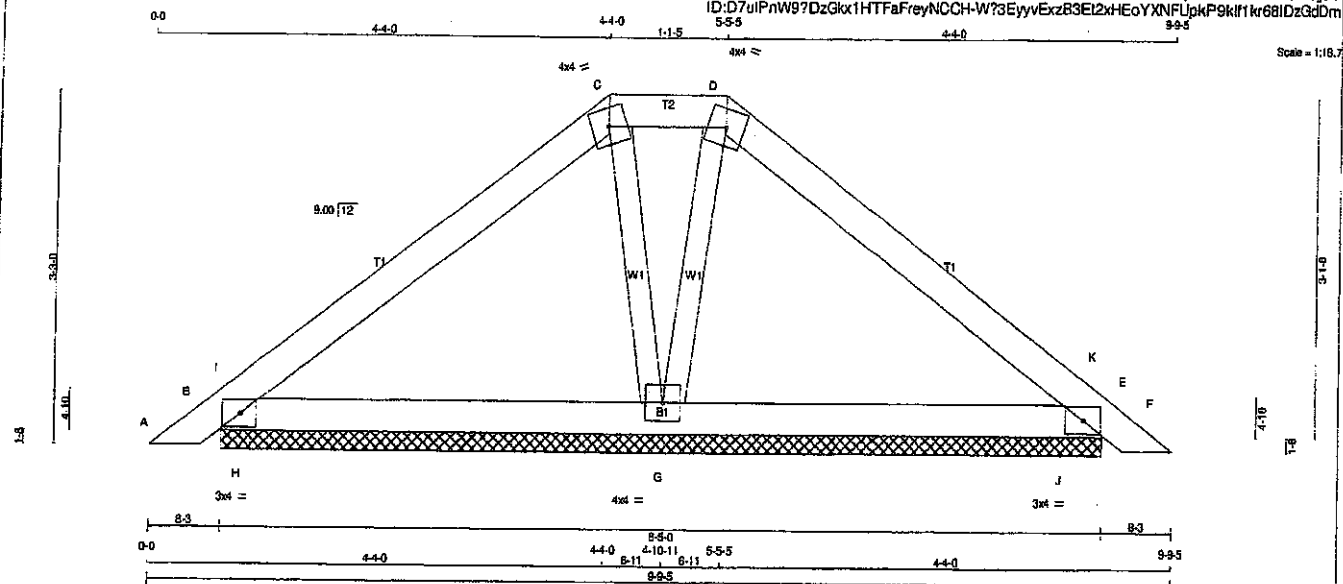
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TOWN OF U...
RING SECT...

Structural component only
DAIC# T-10-11479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403045	P2	2	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

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TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
D	TTW-m	MT20	4.0	4.0		
C	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	401	0	401	0	8-5-0	8-5-0
E	390	0	390	0	8-5-0	8-5-0
G	514	0	514	0	8-5-0	8-5-0

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM. LIVE	WIND	DEAD
B	253	182/0	43/0	0/0	0/0	57/0
E	265	179/0	42/0	0/0	0/0	55/0
G	390	197/0	92/0	0/0	0/0	102/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
A-B	0/17	-102.1	-102.1	0.02 (1)
B-E	-117/0	-102.1	-102.1	0.05 (1)
I-C	-165/0	-102.1	-102.1	0.18 (1)
C-D	-98/0	-102.1	-102.1	0.02 (1)
D-K	-157/0	-102.1	-102.1	0.17 (1)
K-E	-103/0	-102.1	-102.1	0.06 (1)
E-F	0/17	-102.1	-102.1	0.03 (1)
B-H	0/126	-38.5	-38.5	0.14 (1)
H-G	0/126	-38.5	-38.5	0.20 (2)
G-J	0/119	-38.5	-38.5	0.19 (2)
J-E	0/119	-38.5	-38.5	0.15 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
OL = 8.0 PSF
BOT CH. LL = 10.5 PSF
OL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

OSI: TC=0.18/1.00 (C-I:1), BC=0.20/1.00 (G-H:2), WB=0.02/1.00 (D-G:1), SS=0.28/1.00 (E-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1887
	788	1987	1856

PLATE PLACEMENT TOL = 0.250 inches

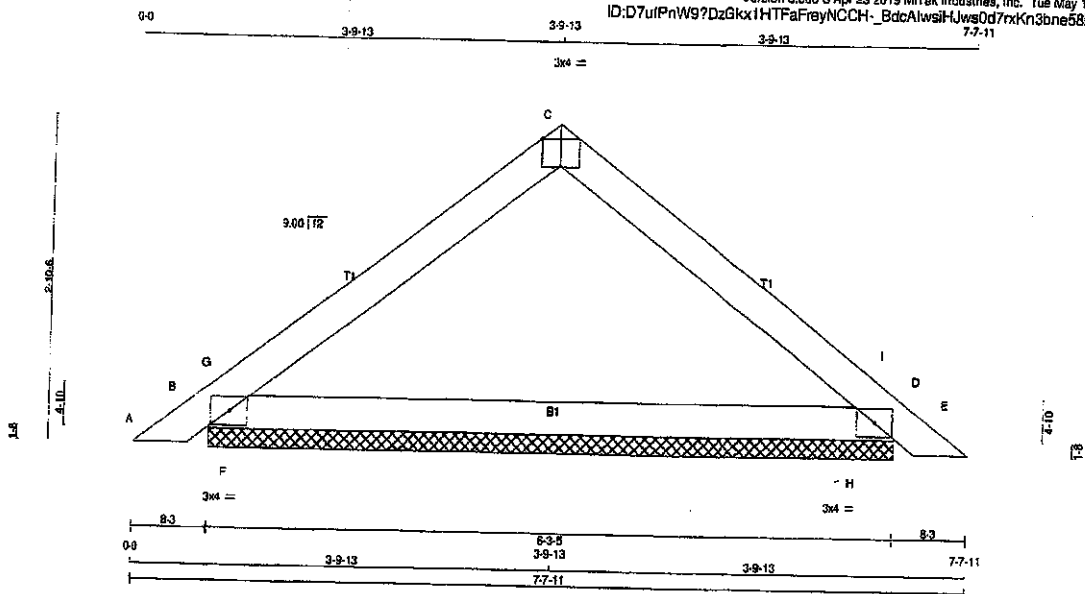
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.30 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911480

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	P3	4	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:18 2019 Page 1					
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7-7-11					
Scale = 1:12.0					



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

B - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
B	502 0	502 0	6-3-5	6-3-5
D	502 0	502 0	6-3-5	6-3-5

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	371	217 / 0	66 / 0	0 / 0	0 / 0	69 / 0	0 / 0	0 / 0
D	371	217 / 0	66 / 0	0 / 0	0 / 0	69 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 17	-102.1 -102.1	0.03 (1)	10.00	F-G	0 / 437	0.00 (1)
B-G	-537 / 0	-102.1 -102.1	0.16 (3)	6.25	H-I	0 / 437	0.00 (1)
G-C	-275 / 0	-102.1 -102.1	0.21 (1)	6.25			
C-I	-275 / 0	-102.1 -102.1	0.21 (1)	6.25			
I-D	-537 / 0	-102.1 -102.1	0.16 (3)	6.25			
D-E	0 / 17	-102.1 -102.1	0.03 (1)	10.00			
B-F	0 / 233	-38.5 -38.5	0.13 (3)	10.00			
F-H	0 / 233	-38.5 -38.5	0.23 (2)	10.00			
H-D	0 / 233	-38.5 -38.5	0.13 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

OL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-I), BC=0.23/1.00 (F-H), WB=0.00/1.00 (F-G), SB=0.25/1.00 (B-F)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

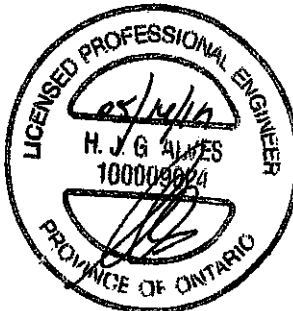
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	516	354
	1667	768
	1987	1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.32 (D) (INPUT = 0.50)
JSI METAL = 0.10 (B) (INPUT = 1.00)



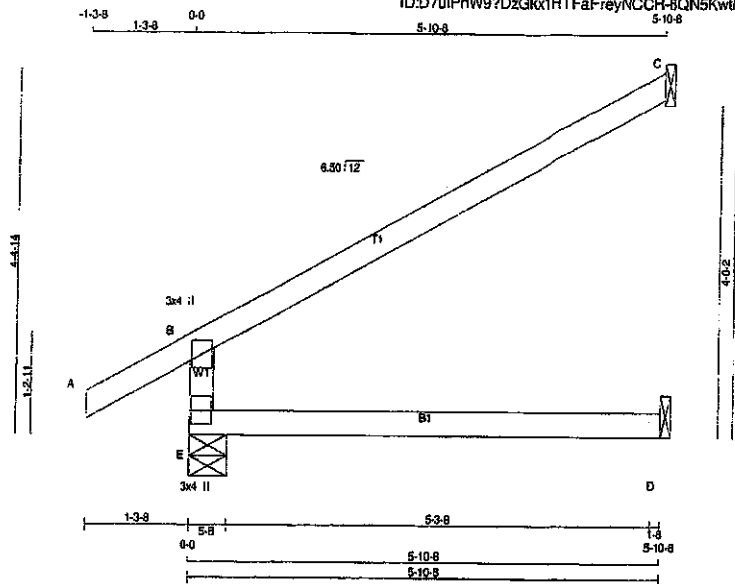
Structural component only
DWG# T-1911481

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403045	J1	17	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:14 2019 Page 1
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Scale = 1:24.6



TOTAL WEIGHT = 17 X 17 = 290 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	648	0	648	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	94	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1			MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
			FROM	TO	TO					
FR-TO										
E-B	-514 / 0	0.0	0.0	0.21 (3)	7.81					
A-B	0 / 33	-102.1	-102.1	0.13 (1)	10.00					
B-C	-38 / 0	-102.1	-102.1	0.50 (1)	8.25					
E-D	0 / 0	-38.5	-38.5	0.21 (3)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/930$ (0.08")

CS1: TC=0.60/1.00 (B-C:1), BC=0.21/1.00 (D-E:3),
WB=0.00/1.00 (N/A:0), SS1=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

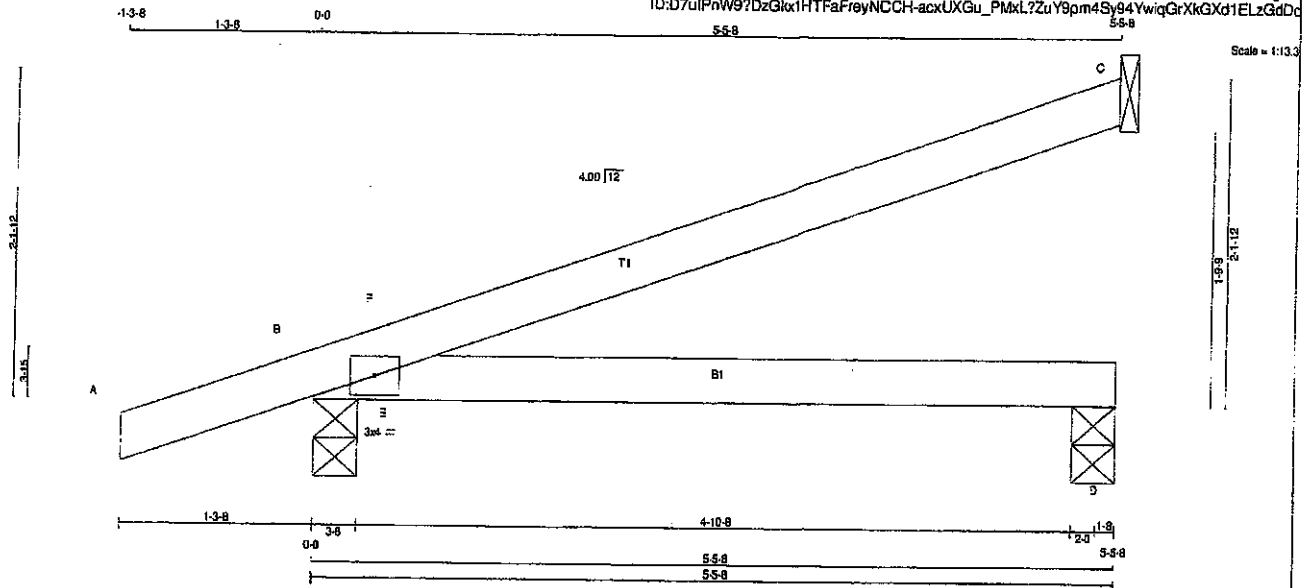
JSI GRIP= 0.22 (E) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911476

JOB NAME 403045	TRUSS NAME J90	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:15 2019 Page 1
ID:D7uIPnW9?DzGlox1HTFaFreyNCCH-acxUXGu_PmXL7ZuY8pm4Sy94YwiqGrXkGXdlELzGdDc



Scale = 1/16" = 1'-0"

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER
A - C 2x4 DRY No.2
B - D 2x4 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB14 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
	GROSS REACTION	VERT	GROSS REACTION	VERT	BRG	IN-SX	BRG	IN-SX
C	250	0	250	0	1-8	1-8	1-8	1-8
B	520	0	520	0	3-8	3-8	3-8	3-8
D	133	0	145	0	3-8	3-8	3-8	3-8

SEE MITEK STANDARD DETAIL B9771H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

JT	1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0	
B	390	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0	
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	CS1 (LC) MAX (LBS)	MEMB.	FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	0 / 20	-102.1 -102.1	0.13 (1)	E-F	-254 / 122
B-F	-51 / 0	-102.1 -102.1	0.13 (3)		0.00 (1)
F-C	0 / 6	-102.1 -102.1	0.42 (1)		
B-E	0 / 0	-38.5 -38.5	0.26 (1)		
E-D	0 / 0	-38.5 -38.5	0.31 (1)		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/942 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/469 (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

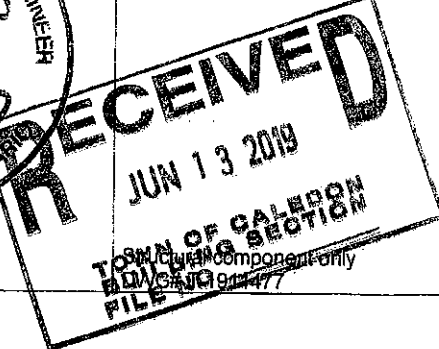
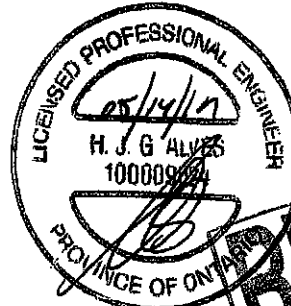
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

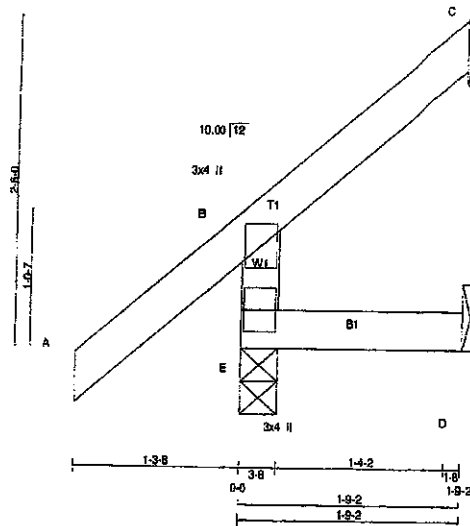
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
403045	J91	3	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:16 2019 Page 1
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 Scale = 1:15.5



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	321	0	321	0	3-8	3-8
C	49	0	49	0	1-8	1-8
D	19	0	38	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
 PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LOOSE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
E	228	183/0	19/0	0/0	0/0	48/0	0/0
C	34	27/-20	1/0	0/0	0/0	8/0	0/0
D	20	0/-12	17/0	0/0	0/0	10/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FORCE (LBS)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO	CSI (LC)		FR-TO		
E-B	-272/0	0.0 0.0	0.05 (5)	7.81			
A-B	0/45	-102.1 -102.1	0.14 (1)	10.00			
B-C	-28/0	-102.1 -102.1	0.10 (1)	8.25			
E-D	0/0	-38.5 -38.5	0.05 (5)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")
 ALLOWABLE DEFL.(TL) = L/360 (0.19")
 CALCULATED VERT. DEFL.(TL) = 1/999 (0.00")

CS: TC=0.14/1.00 (A-B:1), BC=0.05/1.00 (D-E:5),
 WB=0.00/1.00 (A-B:0), SS=0.08/1.00 (A-B:1)

OOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

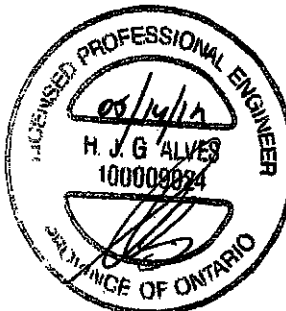
PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 785

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.13 (B) (INPUT = 0.90)

JSI METAL = 0.10 (B) (INPUT = 1.00)



Structural component only
 DWG# T-1911478



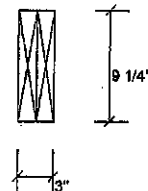
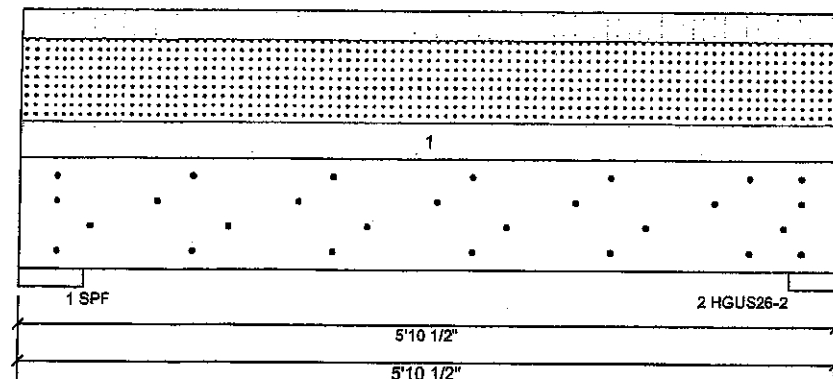
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 4 of 13

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	445	551	1250	0
2	426	528	1198	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	30%	689 / 2320	3009 L	1.25D+1.5S +L
2 - HGUS...	4.000"	39%	660 / 2223	2883 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3401 ft-lb	3'	6039 ft-lb	0.563 (56%)	1.25D+1.5S +L	L
Unbraced	3401 ft-lb	3'	5236 ft-lb	0.650 (65%)	1.25D+1.5S +L	L
Shear	2612 lb	1'2"	3984 lb	0.656 (66%)	1.25D+1.5S +L	L
LL Defl inch	0.030 (L/2096)	3'	0.174 (L/360)	0.170 (17%)	S+0.5L	L
TL Defl inch	0.041 (L/1525)	3'	0.174 (L/360)	0.240 (24%)	D+S+0.5L	L

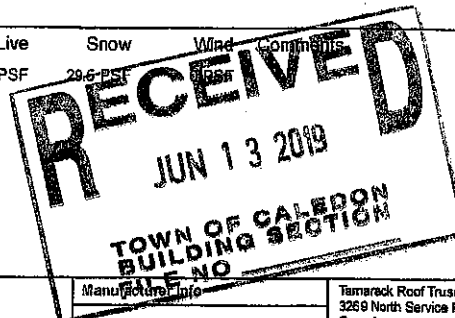
Design Notes

- 1 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1911518
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		14-1-8	Near Face	13 PSF	10.5 PSF	29.6 PSF		



Tamarack Roof Trusses
3256 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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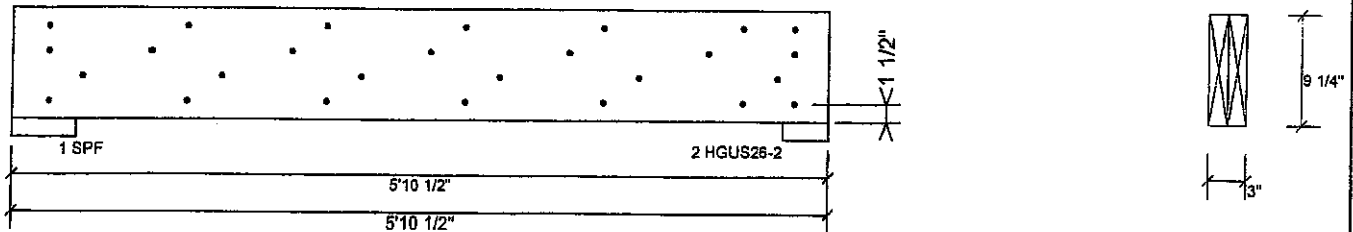
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 5 of 13

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

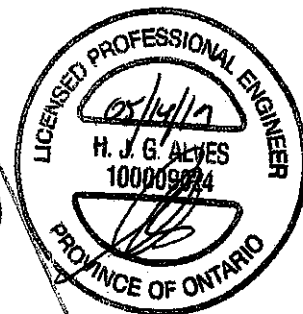
Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	98.1 %
Load	501.4 PLF
Yield Limit per Foot	511.3 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 1791/518
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3259 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021





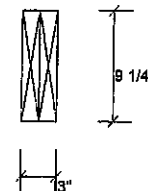
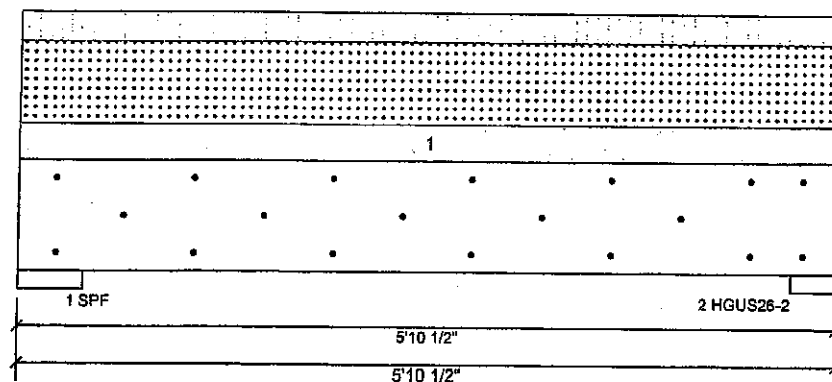
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 6 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	264	327	741	0
2	253	313	710	0

Bearings and Factored Reactions

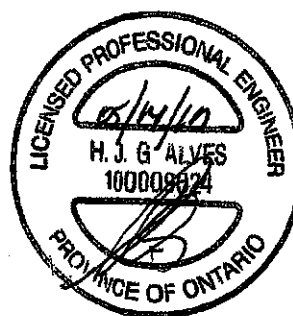
Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	408 / 1376	1784	L	1.25D+1.5S +L
2 - HGUS...	4.000"	23%	391 / 1318	1710	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2016 ft-lb	3'	6039 ft-lb	0.334 (33%)	1.25D+1.5S +L	L
Unbraced	2016 ft-lb	3'	5236 ft-lb	0.385 (39%)	1.25D+1.5S +L	L
Shear	1549 lb	12"	3984 lb	0.389 (39%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3535)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.024 (L/2573)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.

DWG NO. TAM 1911519
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-4-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer InfoTamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

This design is valid until 12/11/2021





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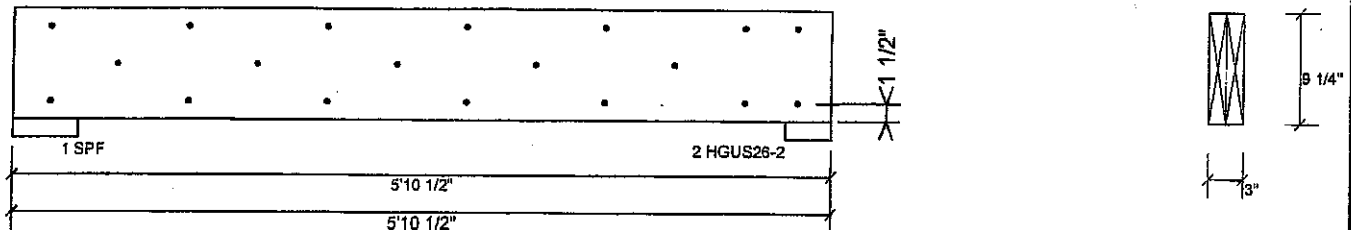
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200568
Project #:

Page 7 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

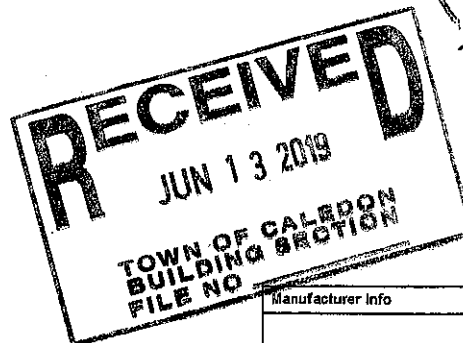
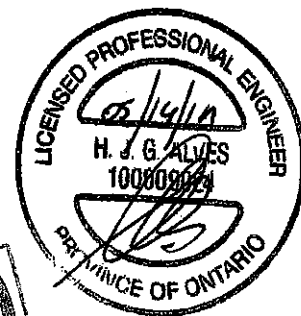
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	87.4 %
Load	297.3 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 11911579
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021

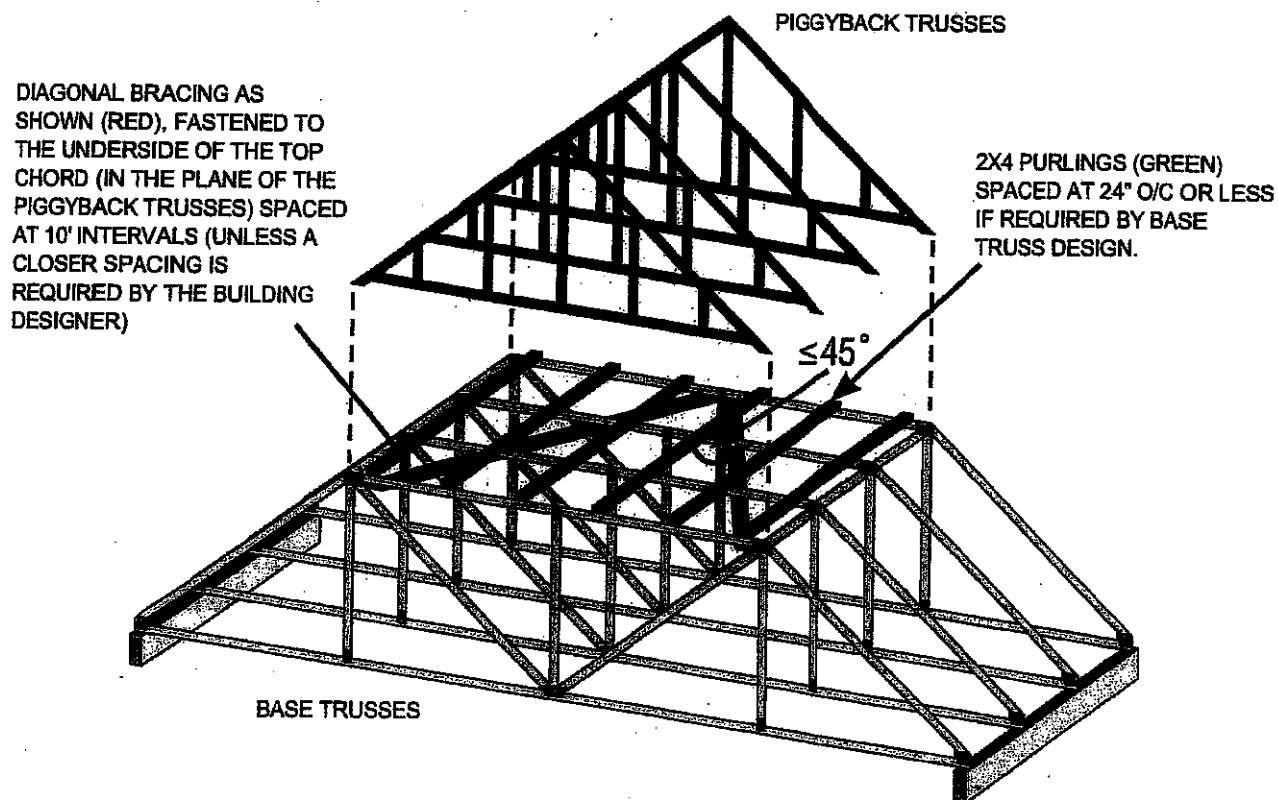


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

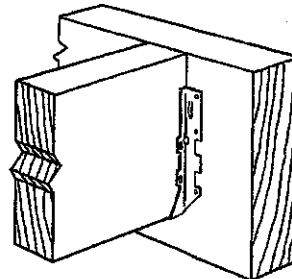
Finish: G90 galvanized

Design:

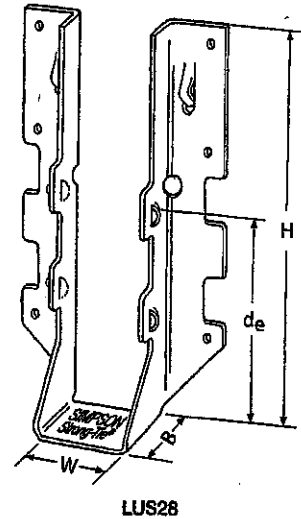
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

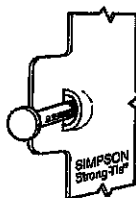


Typical LUS Installation



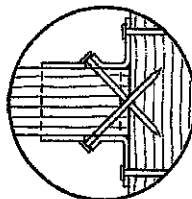
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 13/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 7/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 3/4	3 3/4	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 3/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Top View.



DESIGN

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Simpson Strong-Tie Company, Inc.

15000 Strong-Tie Drive, Houston, TX 77058

(800) 999-5099
strongtie.com

HUS/LJS -- Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

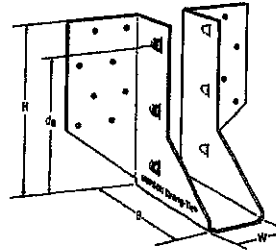
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

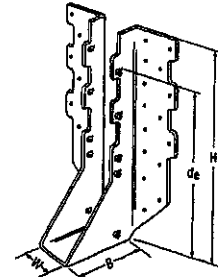
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

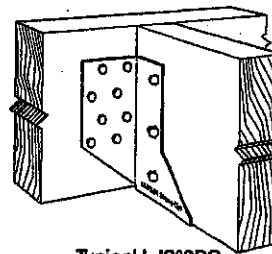
- See current catalogue for options



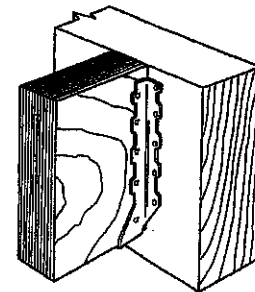
LJS26DS



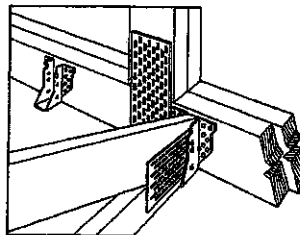
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



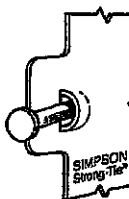
Typical HUS
Installation



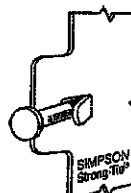
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 1/2	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

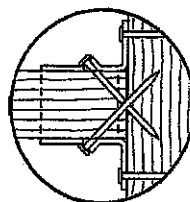
1. d_e is the distance from the seat of the hanger to the highest joist nail.



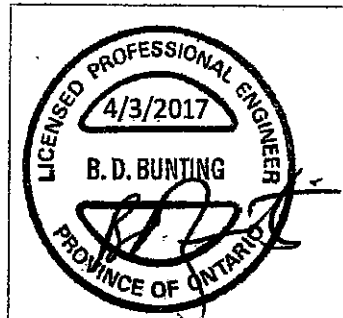
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



DESIGN

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HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

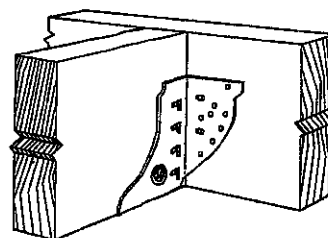
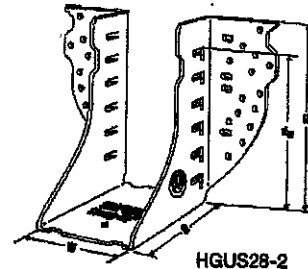
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

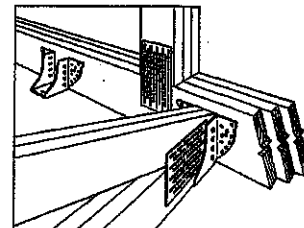
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



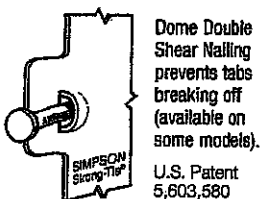
Typical HGUS
Installation



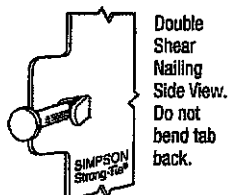
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

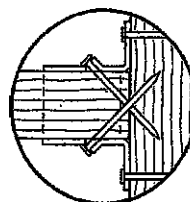
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



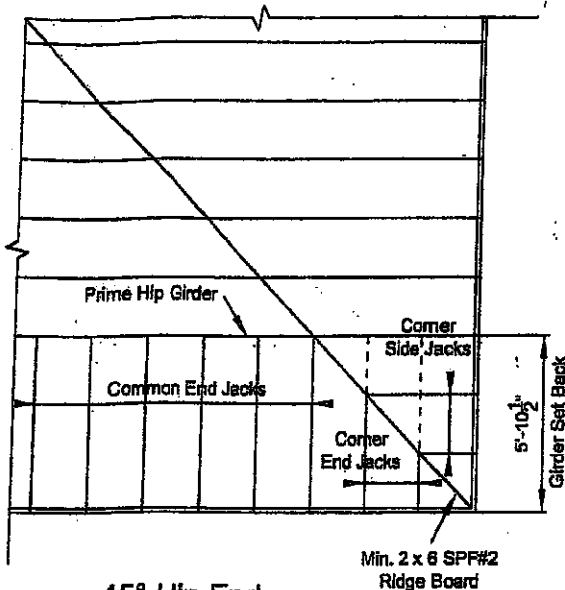
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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45° Hip End

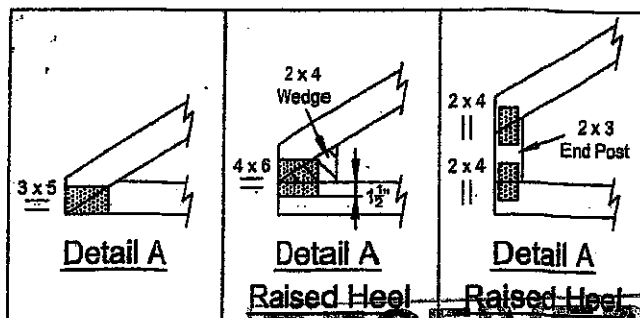
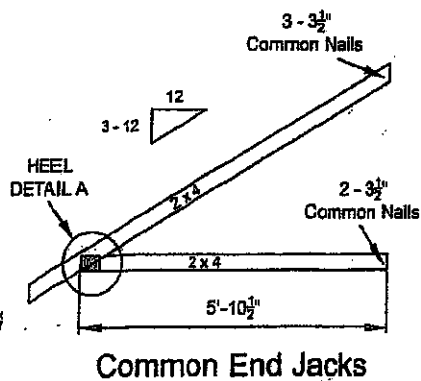
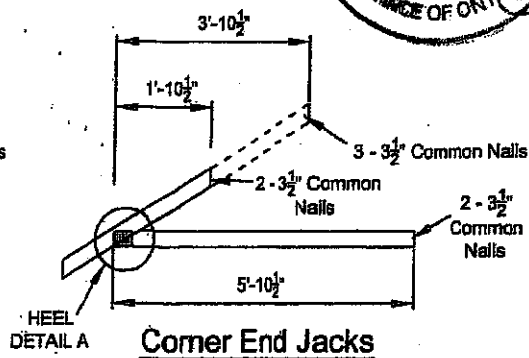
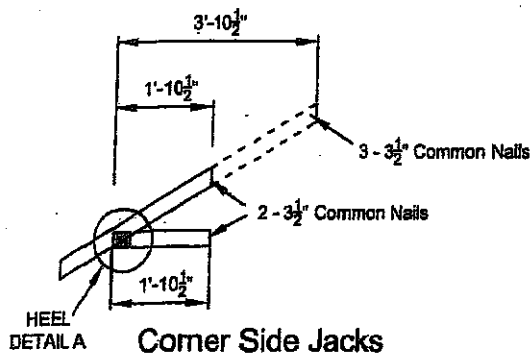
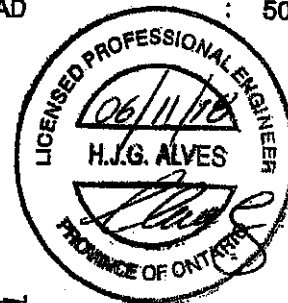
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

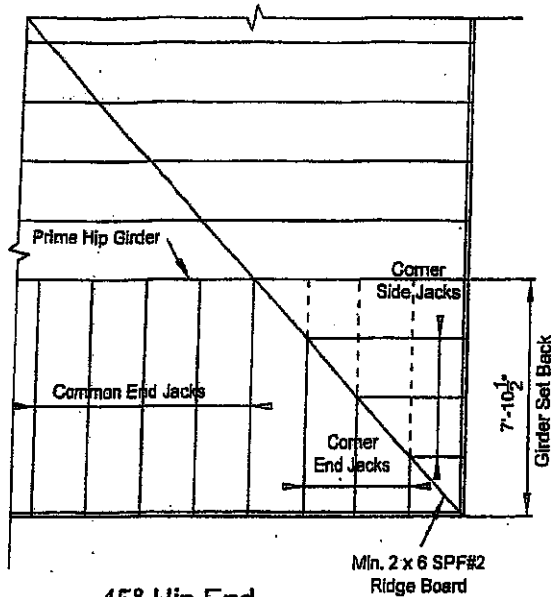
TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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 FILE NO



45° Hip End

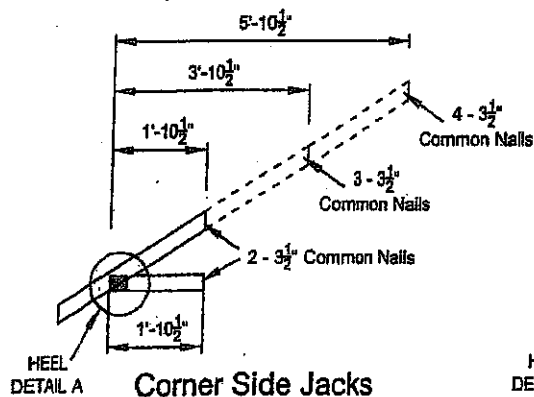
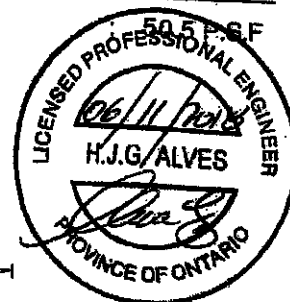
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

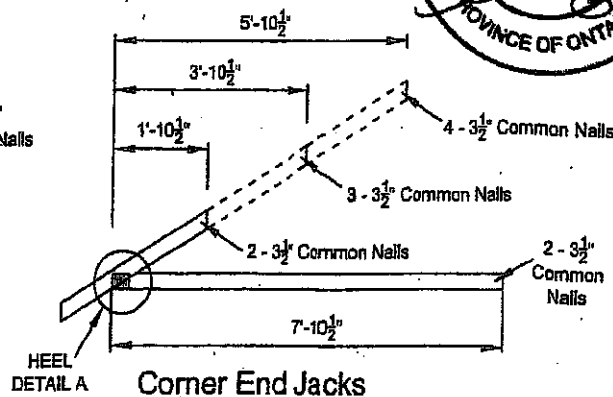
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

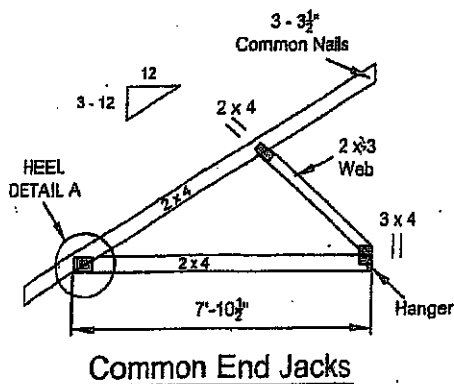
TOTAL LOAD



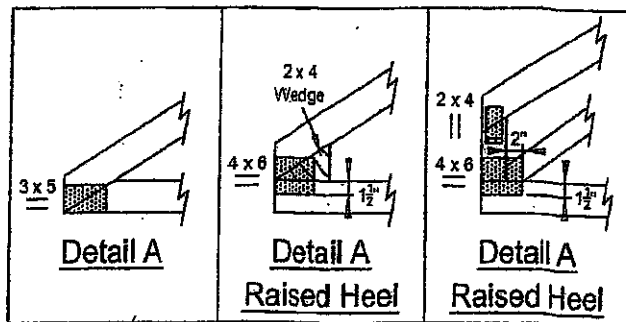
Corner Side Jacks



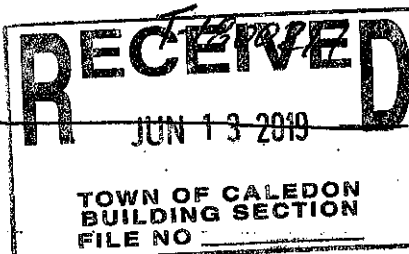
Corner End Jacks



Common End Jacks

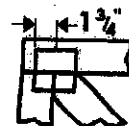


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

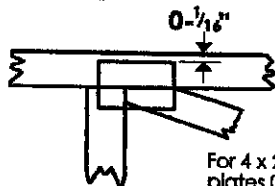


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



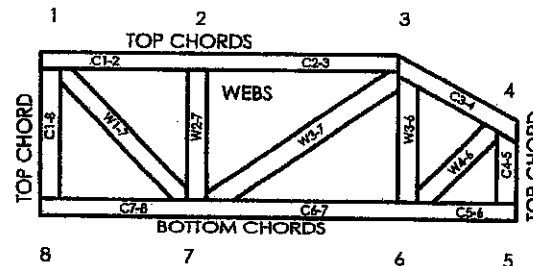
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See B.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

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 FILE NO



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

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