

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

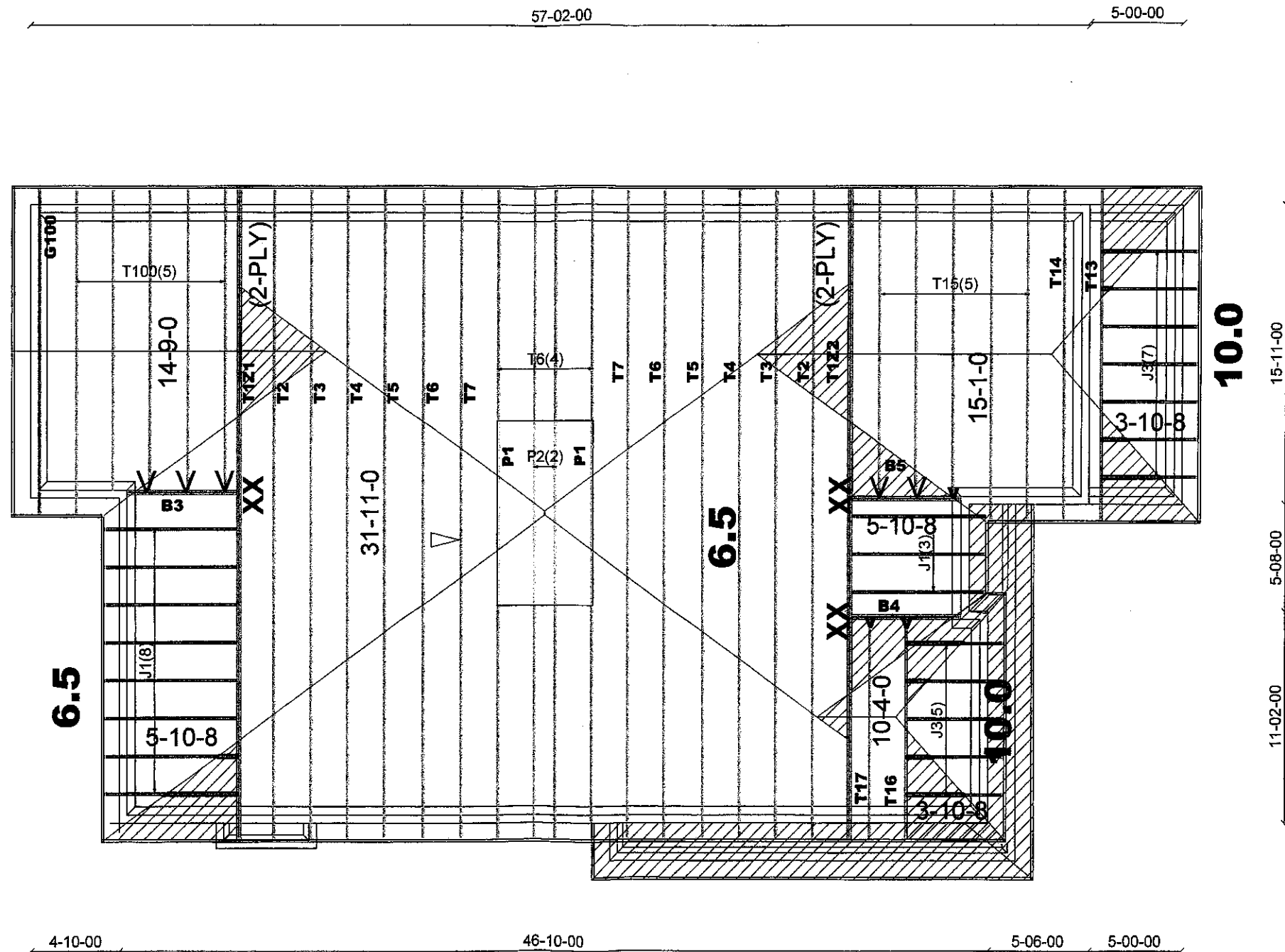
ALL B- 2-2X10 (FLUSH)

DENOTES:
CONVENTIONAL
FRAMING



THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

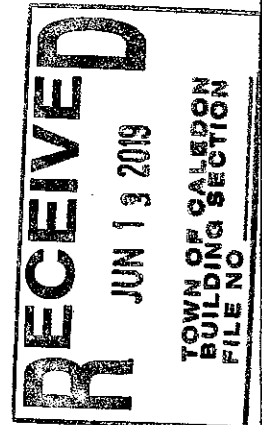
9/12 PITCHES (TYP.)
UNLESS NOTED



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC. LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally Braced so that the distance
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.8 \text{ kPa}$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF



M12255



Job Track: 50120
Plan Log: 200598
Layout ID: 401608

Builder / Location:
GREEN PARK HOMES / Caledon
Project: LAMBERTS LANE HOME CORP.
Date: 5/9/2019 Sales: Mario DiCano Designer: Sunny Jarek

Model / Elevation:
KINMOUNT 6A/2-REAR UPGRADE

Mitek ver 8.2.3.229

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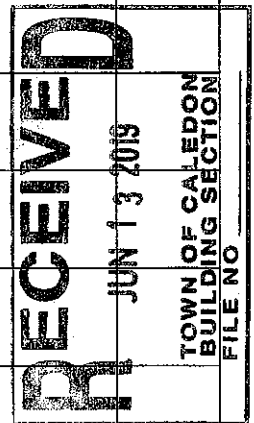
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #:
 Elevation: 2-REAR UPGRADE

Job Track: 50120
 PlanLog: 200598
 Layout ID: 401608
 Ref #
 Page: 1 of 2
 Date: 05/14/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

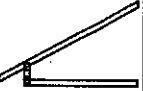
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1Z1 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	1 2-ply	T1Z2 Hip Girder	9/12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	2	T2 Hip	9/12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	267.53 170.33		
	2	T3 Hip	9/12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9/12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9/12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9/12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9/12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.92 208.00		
	1	T13 Hip Girder	9/12	15-01-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	78.15 50.17		
	1	T14 Hip	9/12	15-01-00	6-06-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	70.61 47.33		
	5	T15 Common	9/12	15-01-00	7-02-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	337.18 224.17		
	1	T16 Hip Girder	9/12	10-04-00	4-10-07	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	47.63 32.00		
	1	T17 Common	9/12	10-04-00	5-04-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	44.99 29.50		
	5	T100 Common	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	330.72 215.83		



DELIVERY SHIPLIST		
 TAMARACK LUMBER INC. ALPA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERTS LANE HOME CORP. Location: Caledon Model: KINMOUNT 6A Lot #: Elevation: 2-REAR UPGRADE	Job Track: 50120 PlanLog: 200598 Layout ID: 401608 Ref # Page: 2 of 2 Date: 05/14/2019 Designer: Jane Gong Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G100 GABLE	9/12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.06 45.17		
	2	P1 Piggyback	9/12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9/12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	11	J1 Jack-Open	6.5/12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	187.42 117.33		
	12	J3 Jack-Open	10/12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	182.71 116.00		

TOTAL # TRUSS= 62 TOTAL BFT OF ALL TRUSSES= 2903.83 BFT. TOTAL WEIGHT OF ALL TRSSES 4592.03 LBS

HARDWARE

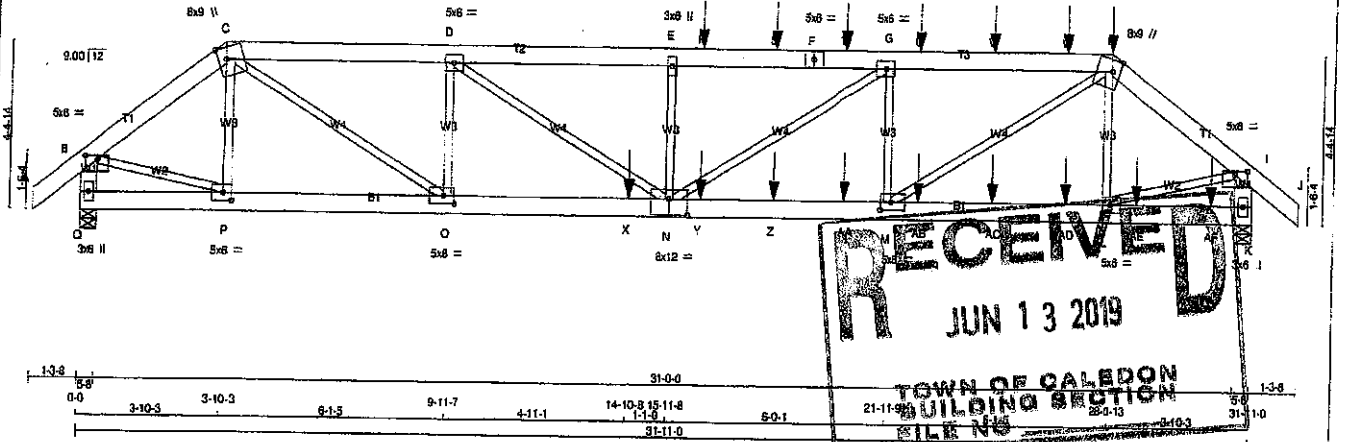
QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
8	Hardware	LJS26DS	

TOTAL NUMBER OF
ITEMS= 11



JOB NAME 401607	TRUSS NAME T1Z1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 11:01:02 2019 Page 1
 ID: D7uIPnW9?DzGk1HTFaFreyNCH-K29D18FYs3Uwv_BgCzQ2GqW4PidtXvSDigLP2zGckF
 1-3-6 0-0 3-10-3 3-10-3 5-1-5 9-11-7 5-0-1 15-11-8 5-0-1 21-11-9 5-1-5 25-0-13 3-10-3 1-3-6
 Scale = 1:50.3



LUMBER			
N. L. & A. RULES	CHORDS	SIZE	DRY
A - C	2x6	DRY	No.2
F - H	2x6	DRY	No.2
H - J	2x6	DRY	No.2
K - B	2x6	DRY	No.2
K - I	2x6	DRY	No.2
Q - N	2x6	DRY	1650F 1.5E
N - K	2x6	DRY	1650F 1.5E
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
Q-B	2	12
K-I	2	12
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
Q-N	2	12
N-K	2	12
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	8.0	1.25	4.00
C TMWV-m	MT20	8.0	9.0	3.75	2.50
D TMWV-w	MT20	5.0	6.0		
E TMWV-w	MT20	3.0	6.0		
F TS-1	MT20	5.0	6.0		
G TMWV-w	MT20	5.0	6.0		
H TMWV-m	MT20	8.0	9.0	3.75	2.50
I TMWV-p	MT20	5.0	8.0	1.25	4.00
K BMV1-p	MT20	3.0	6.0		
L BMWV-w	MT20	5.0	6.0	2.50	2.50
M BMWV-w	MT20	5.0	6.0	2.50	3.50
N BSWVW-w	MT20	8.0	12.0	5.00	8.00
O BMWV-w	MT20	5.0	6.0	2.50	3.50
P BMWV-w	MT20	5.0	6.0	2.50	2.50
Q BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
Q 3943	0	3943	0
K 4534	0	4534	0

UNFACTORED REACTIONS			
1ST CASE	MAX/MIN	COMPONENT REACTIONS	
JT COMBINED	SNOW	LIVE	PERM. LIVE
Q 2923	1883/0	533/0	0/0
K 3370	1917/0	655/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.53 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSF (LC)
FR-TO			
A-B	0/44	-102.1	-102.1
B-C	-4372/0	-102.1	-102.1
C-D	-8021/0	-102.1	-102.1
D-E	-10251/0	-102.1	-102.1
E-R	-10251/0	-102.1	-102.1
R-S	-10251/0	-102.1	-102.1
S-F	-10251/0	-102.1	-102.1
F-T	-10251/0	-102.1	-102.1
T-G	-10251/0	-102.1	-102.1
G-U	-8340/0	-102.1	-102.1
U-V	-8340/0	-102.1	-102.1
V-W	-8340/0	-102.1	-102.1
W-H	-8340/0	-102.1	-102.1
H-I	-5110/0	-102.1	-102.1
I-J	0/44	-102.1	-102.1
Q-B	-3988/0	0.0	0.0
K-I	-4464/0	0.0	0.0
Q-P	0/0	-38.5	-38.5
P-O	0/3464	-38.5	-38.5
O-X	0/8021	-38.5	-38.5
X-N	0/8021	-38.5	-38.5
N-Y	0/8340	-38.5	-38.5
Y-Z	0/8340	-38.5	-38.5
Z-AA	0/8340	-38.5	-38.5
AA-M	0/8340	-38.5	-38.5
MA-B	0/4057	-38.5	-38.5
AB-AC	0/4057	-38.5	-38.5
AC-AD	0/4057	-38.5	-38.5
AD-L	0/4057	-38.5	-38.5
LA-E	0/0	-38.5	-38.5
EA-F	0/0	-38.5	-38.5
AF-K	0/0	-38.5	-38.5

FACTORED CONCENTRATED LOADS (LBS)			
JT	LOC.	LC1	MAX.
H	28-0-13	-49	-55
H	28-0-13	-282	-282
R	16-9-12	-123	-123
S	18-9-12	-123	-123
T	20-9-12	-123	-123
U	22-9-12	-123	-123

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.26")
 ALLOWABLE DEFL.(TL) = L/360 (1.06")
 CALCULATED VERT. DEFL.(TL) = L/893 (0.43")

CSI: TO=0.41/1.00 (E-G-1), BC=0.82/1.00 (N-O-1),
 WB=0.67/1.00 (C-O-1), SS=0.60/1.00 (N-O-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PL) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

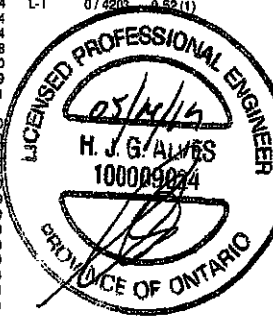
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (I) (INPUT = 0.90)
 JSI METAL=0.58 (N) (INPUT = 1.00)

Structural component only

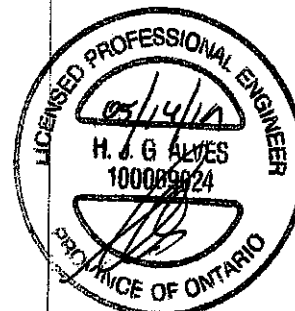
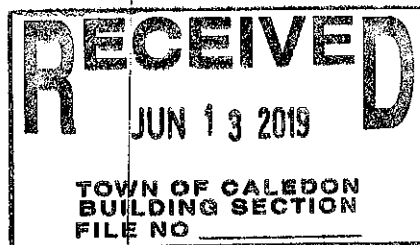
DWG# T-1911498 CONTINUED ON PAGE 2



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401607	T1Z1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

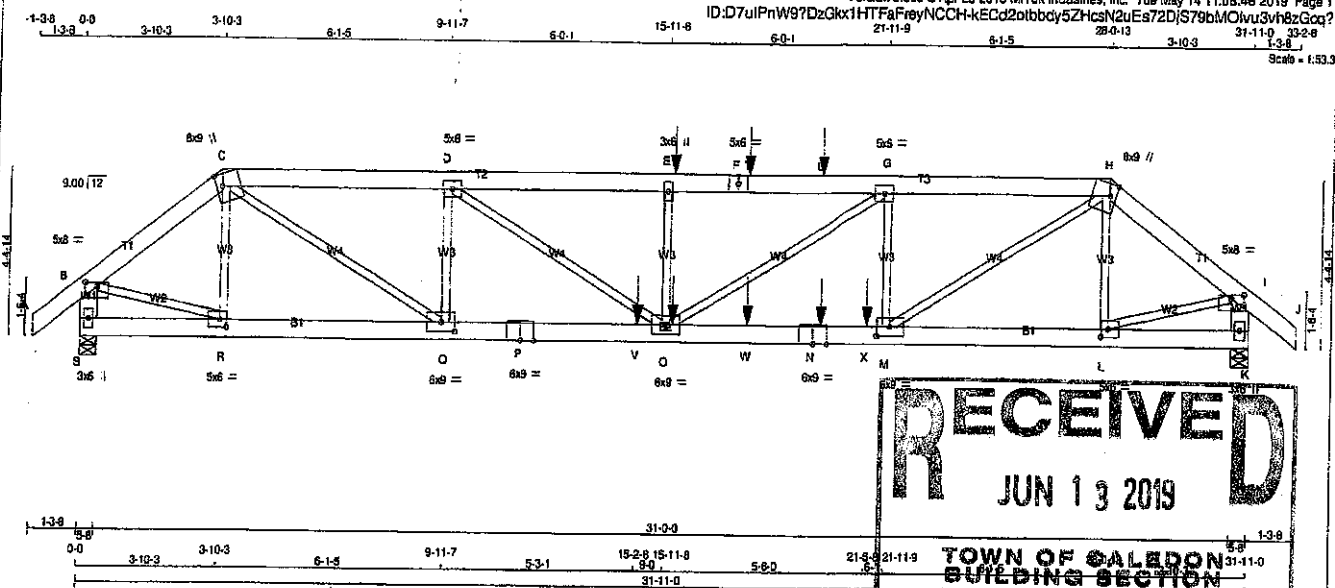
Version 8.300 5 Apr 2019 MiTek Industries, Inc. Tue May 14 11:01:02 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	14-10-8	-2186	-2186	---	FRONT	VERT	TOTAL	---	---
Y	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
Z	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AA	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
 DWG# T-1911498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403059	T1Z2	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington						



N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESIGN
A - C	2x6	No.2	SP
C - F	2x6	No.2	SP
F - H	2x6	No.2	SP
H - J	2x6	No.2	SP
S - B	2x6	No.2	SP
K - I	2x6	No.2	SP
S - P	2x6	1850F 1.5E	SP
P - N	2x6	1850F 1.5E	SP
N - K	2x6	1850F 1.5E	SP

DRY: SEASONED LUMBER.

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS			
A-C	2	12	TOP
C-F	2	12	SIDE(183.
F-H	2	12	SIDE(0.0)
H-J	2	12	TOP
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS			
S-P	2	12	TOP
P-N	2	12	SIDE(183.
N-K	2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS			

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	1.25	4.00
C	TTWW-v-m	MT20	8.0	9.0	3.75	2.00
D	TMWV-l	MT20	5.0	6.0		
E	TMW-v-w	MT20	3.0	6.0		
F	TS-l	MT20	5.0	6.0		
G	TMWV-l	MT20	5.0	6.0		
H	TTWW-v-m	MT20	8.0	9.0	3.75	2.00
I	TMWV-p	MT20	8.0	8.0	1.25	4.00
K	BMV-l-p	MT20	3.0	6.0		
L	BMWV-l	MT20	5.0	6.0	2.50	2.25
M	BMV-l	MT20	6.0	9.0	3.00	4.25
N	BS-l	MT20	5.0	6.0		
O	BMWV-w-l	MT20	6.0	9.0		
P	BS-l	MT20	8.0	8.0		
Q	BMWV-l	MT20	6.0	9.0	3.00	4.25
R	BMV-l	MT20	5.0	6.0	2.50	2.25
S	TMWV-p	MT20	3.0	6.0		

BEARINGS

UNFACTORED REACTIONS

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S.S.K

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.36 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

CHORDS				WEBBS			
MAX. FACTORED		FACTORED		MAX. FACTORED			
MEMB.	(FORCE)	VERT. LOAD	LC1 MAX	MAX. UNBRAC	MEMB.	(FORCE)	MAX
	LB/CS	(PLF)	CS1 (LC)				CS1 (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 44	-102.1	-102.1	0.04 (1)	R-C	-729 / 0	0.10 (1)
B-C	-8484 / 0	-102.1	-102.1	0.10 (1)	C-Q	0 / 5610	0.69 (1)
C-D	-8404 / 0	-102.1	-102.1	0.27 (1)	Q-D	-2829 / 0	0.35 (1)
E	-11425 / 0	-102.1	-102.1	0.35 (1)	D-O	0 / 3815	0.45 (1)
E-F	-11425 / 0	-102.1	-102.1	0.43 (1)	C-E	-804 / 0	0.11 (1)
F	-11425 / 0	-102.1	-102.1	0.43 (1)	C-G	0 / 1897	0.23 (1)
T-U	-11425 / 0	-102.1	-102.1	0.43 (1)	M-G	-1828 / 0	0.24 (1)
U-G	-11425 / 0	-102.1	-102.1	0.43 (1)	M-H	0 / 6853	0.82 (1)
G-H	-9840 / 0	-102.1	-102.1	0.31 (1)	L-H	-566 / 0	0.24 (1)
H-I	-5349 / 0	-102.1	-102.1	0.11 (1)	B-R	0 / 3821	0.47 (1)
I-J	0 / 44	-102.1	-102.1	0.04 (1)	L-I	0 / 3821	0.47 (1)
S-B	-4089 / 0	0.0	0.0	0.15 (1)			
K-I	-4657 / 0	0.0	0.0	0.17 (1)			
S-R	0 / 0	-38.5	-38.5	0.04 (2)			
R-Q	0 / 3683	-38.5	-38.5	0.20 (1)			
P-Q	0 / 8405	-38.5	-38.5	0.53 (1)			
P-V	0 / 8405	-38.5	-38.5	0.33 (1)			
V-O	0 / 8405	-38.5	-38.5	0.53 (1)			
O-W	0 / 9840	-38.5	-38.5	0.57 (1)			
W-N	0 / 9840	-38.5	-38.5	0.57 (1)			
N-X	0 / 9840	-38.5	-38.5	0.57 (1)			
X-M	0 / 9840	-38.5	-38.5	0.57 (1)			
M-L	0 / 4241	-38.5	-38.5	0.22 (1)			
L-K	0 / 0	-38.5	-38.5	0.04 (2)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LG1	MAX-	MAX+	FACE	DIR.	HEAT
E	16-12	-123	-123	---	BACK	VERT	TOTAL
N	20-12	-55	-70	---	BACK	VERT	TOTAL
O	16-12	-55	-70	---	BACK	VERT	TOTAL
T	18-12	-123	-123	---	BACK	VERT	TOTAL
U	20-12	-123	-123	---	BACK	VERT	TOTAL
V	15-2-9	-1834	-1834	---	BACK	VERT	TOTAL
W	18-1-12	-55	-70	---	BACK	VERT	TOTAL
X	21-5-8	-1883	-1583	---	BACK	VERT	TOTAL

DESIGN CRITERIA

SPACING = 24.0 | N. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE
OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL(LL) = $L/360$ (1.06")
CALCULATED VERT. DEFL(LL) = $L/999$ (0.25")
ALLOWABLE DEFL(TL) = $L/360$ (1.06")
CALCULATED VERT. DEFL(TL) = $L/937$ (0.41")

CSI: TC=0.43/1.00 (E-G:1), BC=0.57/1.00 (M-O:1)
WB=0.82/1.00 (H-M:1), SS=0.59/1.00 (O-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
	PLATE GRIP (DRY)		SHEAR		SECTION
	(PSI)		(PLI)		(PLI)
	MAX	MIN	MAX	MIN	MAX
MT20	618	354	1667	788	1987

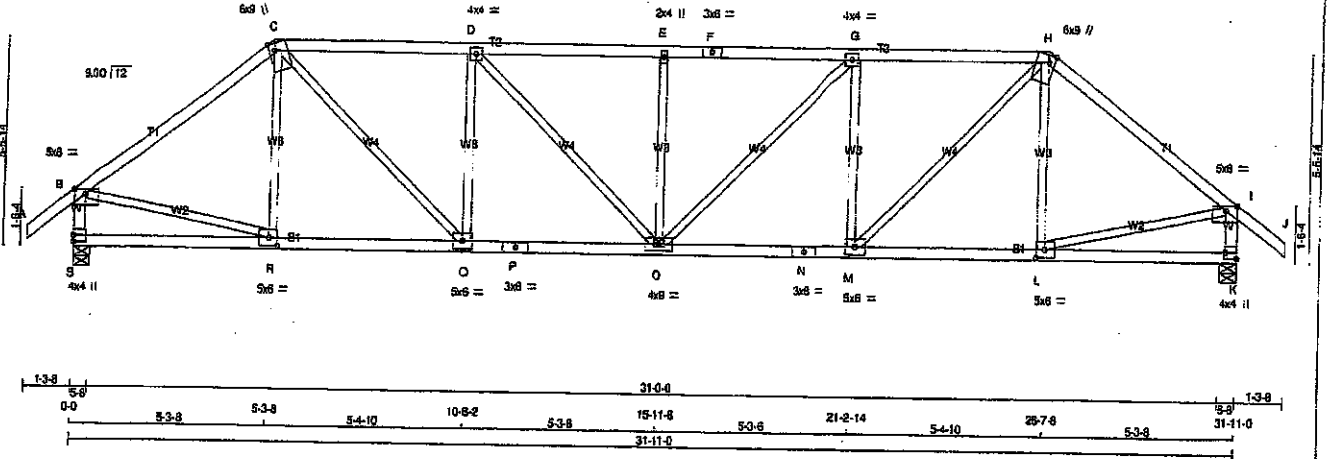
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.
JSI GRF = 0.88 (Q) (INPUT = 0.80)
JSI METAL = 0.87 (P) (INPUT = 1.00)

Structural component only
DWG# T-1911500

JOB NAME: 403045 TRUSS NAME: T2 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

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 ID: D7uIPmW9?DzGkx1HTFaFreyNCCH-PmloJd?CIVUMW4tUhdPCLgT1deT4MR?zGdD
 21-2-14 31-11-0 33-2-8 1-3-8 5-3-8 5-4-10 10-8-2 5-3-8 15-11-8 5-3-8 21-2-14 5-4-10 25-7-8 5-3-8 31-11-0
 Scale = 1/8" = 1'-0"



LUMBER
N.L.G.A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - F	2x4	DRY No.2
F - H	2x4	DRY No.2
H - J	2x4	DRY No.2
S - B	2x4	DRY No.2
K - I	2x4	DRY No.2
S - P	2x4	DRY No.2
P - N	2x4	DRY No.2
N - K	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	5.0	8.0	1.50	3.50
C TTWW+m	MT20	6.0	9.0	Edge	2.00
D TMW-w	MT20	4.0	4.0		
E TMW-w	MT20	2.0	4.0		
F TS-l	MT20	3.0	6.0		
G TMW-w	MT20	4.0	4.0		
H TTWW+m	MT20	6.0	9.0	Edge	2.00
I TMW-p	MT20	5.0	8.0	1.50	3.50
K BMV-l	MT20	4.0	4.0	2.00	Edge
L BMW-w	MT20	5.0	6.0	2.50	2.75
M BMW-w	MT20	5.0	6.0		
N BS-l	MT20	3.0	6.0		
O BMW-w	MT20	4.0	9.0		
P BS-l	MT20	3.0	6.0		
Q BMW-w	MT20	5.0	6.0		
R BMW-w	MT20	5.0	6.0	2.50	2.75
S BMV-l	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	DOWN	IN-SX	IN-SX
JT	2385	0	0	0
S	2385	0	0	0
K	2385	0	0	0

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
S	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
K	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (I)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 (1)	10.00	R-C	-185 / 98	0.09 (1)
B-C	-2428 / 0	-102.1	-102.1 (1)	0.73 (1)	C-Q	0 / 1667	0.38 (1)
C-D	-3138 / 0	-102.1	-102.1 (1)	0.65 (1)	Q-D	-961 / 0	0.43 (1)
D-E	-3494 / 0	-102.1	-102.1 (1)	0.69 (1)	D-O	0 / 500	0.11 (1)
E-F	-3494 / 0	-102.1	-102.1 (1)	0.69 (1)	O-E	-497 / 0	0.22 (1)
F-G	-3494 / 0	-102.1	-102.1 (1)	0.69 (1)	O-G	0 / 500	0.11 (1)
G-H	-3138 / 0	-102.1	-102.1 (1)	0.65 (1)	M-G	-961 / 0	0.43 (1)
H-I	-2428 / 0	-102.1	-102.1 (1)	0.73 (1)	M-H	0 / 1667	0.38 (1)
I-J	0 / 42	-102.1	-102.1 (1)	10.00	L-H	-195 / 98	0.09 (1)
S-B	-2301 / 0	0.0	0.0 (2)	10.00	B-R	0 / 1987	0.45 (1)
K-I	-2301 / 0	0.0	0.0 (2)	10.00	L-I	0 / 1987	0.45 (1)
S-R	0 / 0	-38.5	-38.5 (1)	0.21 (3)			
R-O	0 / 1936	-38.5	-38.5 (1)	0.47 (2)			
Q-P	0 / 3138	-38.5	-38.5 (1)	0.80 (1)			
P-O	0 / 3138	-38.5	-38.5 (1)	0.80 (1)			
O-N	0 / 3138	-38.5	-38.5 (1)	0.80 (1)			
N-M	0 / 3138	-38.5	-38.5 (1)	0.80 (1)			
M-L	0 / 1936	-38.5	-38.5 (1)	0.47 (2)			
L-K	0 / 0	-38.5	-38.5 (1)	0.21 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-05, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/989 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/999 (0.29")

CSI: TC=0.73/1.00 (H=1), BC=0.60/1.00 (O-Q=1), WS=0.45/1.00 (H=1), SS=0.28/1.00 (C-D=1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

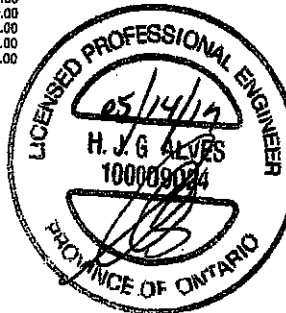
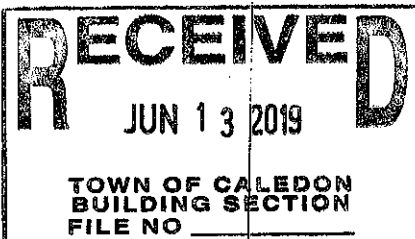
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1887 1666

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (L) (INPUT = 0.90)
JSI METAL = 0.98 (P) (INPUT = 1.00)

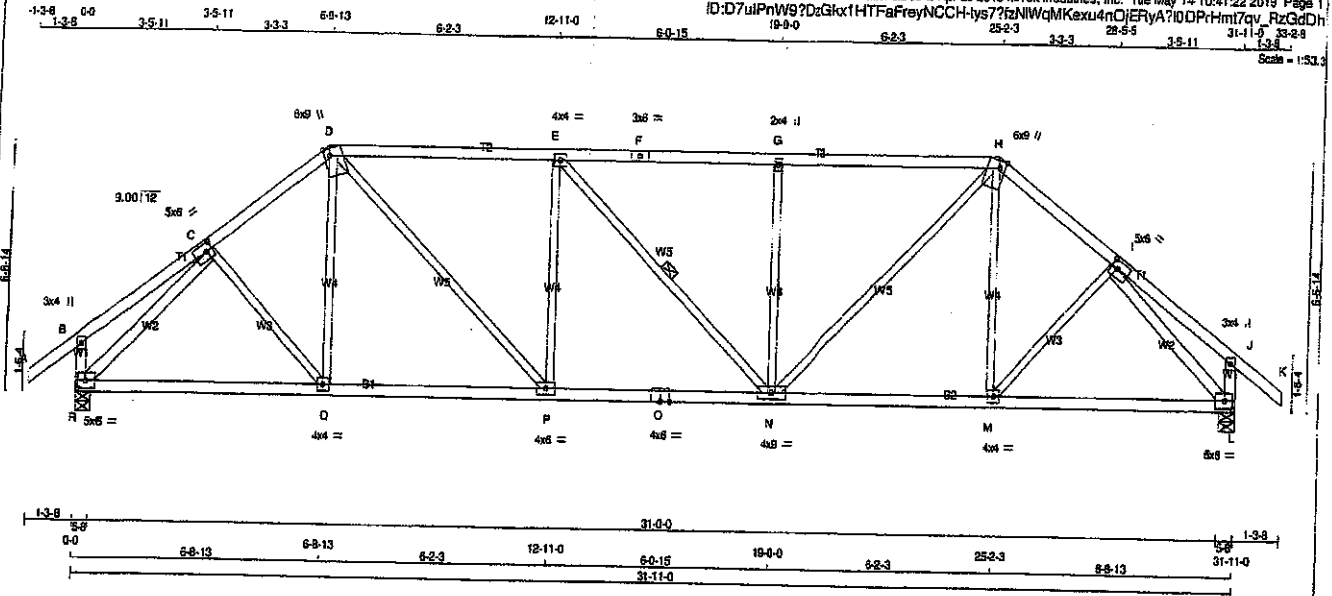


Structural component only
DWG# T-1911484

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
403045	T3	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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ID:D7uPnW9?DzGkx1HTFaFreyNCCCHys7?hNwqMKexu4nOjEryA?10DP:Hmt7qv_RzGdDh



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE	LUMBER		DESCR
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
R - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
DRY: SEASONED LUMBER				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMV+w	MT20	2.0	4.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0	2.50	2.75
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-t	MT20	4.0	9.0		
O	BS-t	MT20	4.0	6.0		
P	BMVW-t	MT20	4.0	6.0		
Q	BMVW-t	MT20	4.0	4.0		
R	BMVW-t	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	2385	0	5-8	5-8
L	2385	0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN COMPONENT REACTIONS
	COMBINED	SNOW
JT	1773	1007 / 0
R	1773	1007 / 0
L	1773	1007 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX.	MEMB.	MAX. FACTORED	FORCE	MAX
MEMB.	FORCE	(LBS)	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE	MAX	CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-Q	0 / 201	0.05 (2)		
B-C	0 / 21	-102.1	-102.1	0.16 (1)	10.00	Q-D	0 / 237	0.06 (3)		
C-D	-2441 / 0	-102.1	-102.1	0.25 (1)	4.19	D-P	0 / 1265	0.28 (1)		
D-E	-2822 / 0	-102.1	-102.1	0.79 (1)	3.34	P-E	-677 / 0	0.48 (1)		
E-F	-2820 / 0	-102.1	-102.1	0.78 (1)	3.34	E-N	-2 / 0	0.00 (1)		
F-G	-2820 / 0	-102.1	-102.1	0.78 (1)	3.34	N-G	-678 / 0	0.48 (1)		
G-H	-2820 / 0	-102.1	-102.1	0.78 (1)	3.36	H-H	0 / 1282	0.28 (1)		
H-I	-2441 / 0	-102.1	-102.1	0.25 (1)	4.19	M-H	0 / 237	0.06 (3)		
I-J	0 / 21	-102.1	-102.1	0.16 (1)	10.00	M-I	0 / 201	0.05 (2)		
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	R-C	-2698 / 0	0.77 (1)		
R-B	-276 / 0	0.0	0.0	0.03 (1)	7.81	I-L	-2698 / 0	0.77 (1)		
L-J	-276 / 0	0.0	0.0	0.03 (1)	7.81					
R-Q	0 / 1809	-38.5	-38.5	0.55 (2)	10.00					
Q-P	0 / 1933	-38.5	-38.5	0.57 (2)	10.00					
P-O	0 / 2822	-38.5	-38.5	0.59 (1)	10.00					
O-N	0 / 2822	-38.5	-38.5	0.59 (1)	10.00					
N-M	0 / 1809	-38.5	-38.5	0.57 (2)	10.00					
M-L	0 / 1933	-38.5	-38.5	0.55 (2)	10.00					

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-08, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/359 (0.14")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/359 (0.24")

CSI: TC=0.79/1.00 (D-E:1), BC=0.59/1.00 (N-P:1), WB=0.77/1.00 (H-I), SS=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE WHEELS OFF

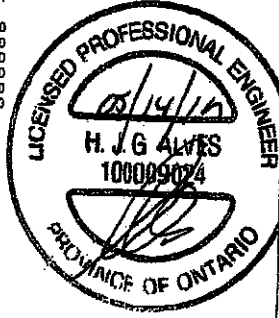
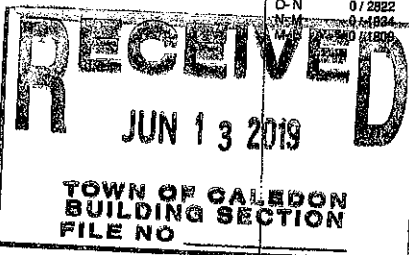
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

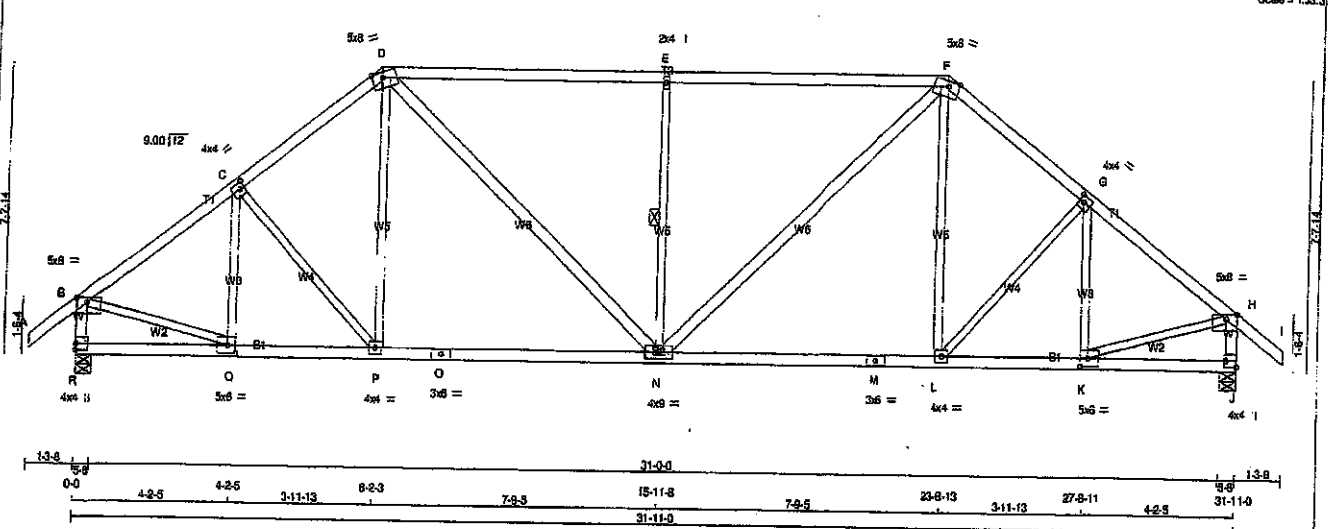
SI GRIP= 0.89 (P) (INPUT = 0.90)
SI METAL= 0.73 (O) (INPUT = 1.00)



Structural component only
DWG# T-1911485

JOB NAME: 403045 TRUSS NAME: T4 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: DRWG NO. 1

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Scale = 1:53.3



CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
J - K	2x4	DRY No.2	SPF
K - L	2x4	DRY No.2	SPF
L - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
D - N	2x4	DRY No.2	SPF
N - F	2x4	DRY No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE PLATES				
B TMW-p	MT20	5.0	8.0	1.50 3.50
C TMW-t	MT20	4.0	4.0	2.00 1.50
D TTW-w	MT20	5.0	8.0	Edge 3.50
E TMW-w	MT20	2.0	4.0	Edge 3.50
F TTW-w	MT20	5.0	8.0	Edge 3.50
G TMW-t	MT20	4.0	4.0	2.00 1.50
H TMW-p	MT20	5.0	8.0	1.50 3.50
J BMV-t-p	MT20	4.0	4.0	2.00 Edge
K BMW-t	MT20	5.0	6.0	2.50 2.50
L BMW-t	MT20	4.0	4.0	
M BS-t	MT20	3.0	6.0	
N BMW-t	MT20	4.0	4.0	
O BS-t	MT20	3.0	6.0	
P BMW-t	MT20	4.0	4.0	
Q BMW-t	MT20	5.0	6.0	2.50 2.50
R BMV-t-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	2385 0	2385 0	5-8	5-8
J	2385 0	2385 0	5-8	5-8

UNFACTORED REACTIONS	1ST CASE	MAX/MIN. COMPONENT REACTIONS
	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	1773 1007/0	335/0 0/0 0/0 431/0 0/0
J	1773 1007/0	335/0 0/0 0/0 431/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1 MAX	MEMB.	FORCE
	(LBS)	(PLF)	CSI (LC)		(LBS)
FR-TO		FROM TO	UNBRAC	FR-TO	
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00	C-C	-410/0 0.13 (1)
B-C	-2391/0	-102.1 -102.1 0.29 (1)	4.21	C-P	-89/0 0.05 (1)
C-D	-2377/0	-102.1 -102.1 0.28 (1)	4.22	P-D	0/353 0.08 (2)
D-E	-2581/0	-102.1 -102.1 1.00 (1)	2.73	D-N	0/939 0.15 (1)
E-F	-2581/0	-102.1 -102.1 1.00 (1)	2.73	N-E	-977/0 0.38 (1)
F-G	-2377/0	-102.1 -102.1 0.28 (1)	4.22	N-F	0/939 0.15 (1)
G-H	-2391/0	-102.1 -102.1 0.29 (1)	4.21	L-F	0/353 0.08 (2)
H-I	0/42	-102.1 -102.1 0.14 (1)	10.00	L-G	-89/0 0.05 (1)
I-B	-2311/0	0.0 0.0 0.24 (1)	5.82	K-G	-410/0 0.13 (1)
J-H	-2311/0	0.0 0.0 0.24 (1)	5.82	B-Q	0/2012 0.45 (1)
R-Q	0/0	-38.5 -38.5 0.12 (3)	10.00	K-H	0/2012 0.45 (1)
Q-P	0/1935	-38.5 -38.5 0.50 (2)	10.00		
P-O	0/1879	-38.5 -38.5 0.82 (2)	10.00		
O-N	0/1879	-38.5 -38.5 0.82 (2)	10.00		
N-M	0/1879	-38.5 -38.5 0.82 (2)	10.00		
M-L	0/1879	-38.5 -38.5 0.82 (2)	10.00		
L-K	0/1935	-38.5 -38.5 0.50 (2)	10.00		
K-J	0/0	-38.5 -38.5 0.12 (3)	10.00		

TOTAL WEIGHT = 2 X 147 = 294 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = U/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = U/999 (0.23")

CSI: TC=1.00/1.00 (E-F:1), BC=0.62/1.00 (N-P:2), WB=0.45/1.00 (B-C:1), SS=0.39/1.00 (E-F:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

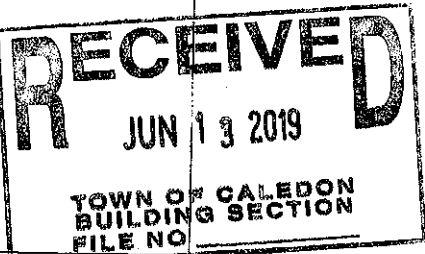
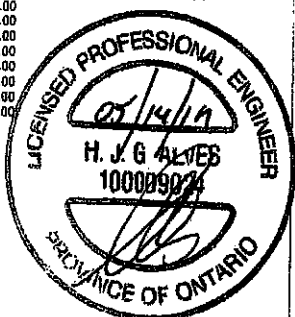
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1867 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (K) (INPUT = 0.90)

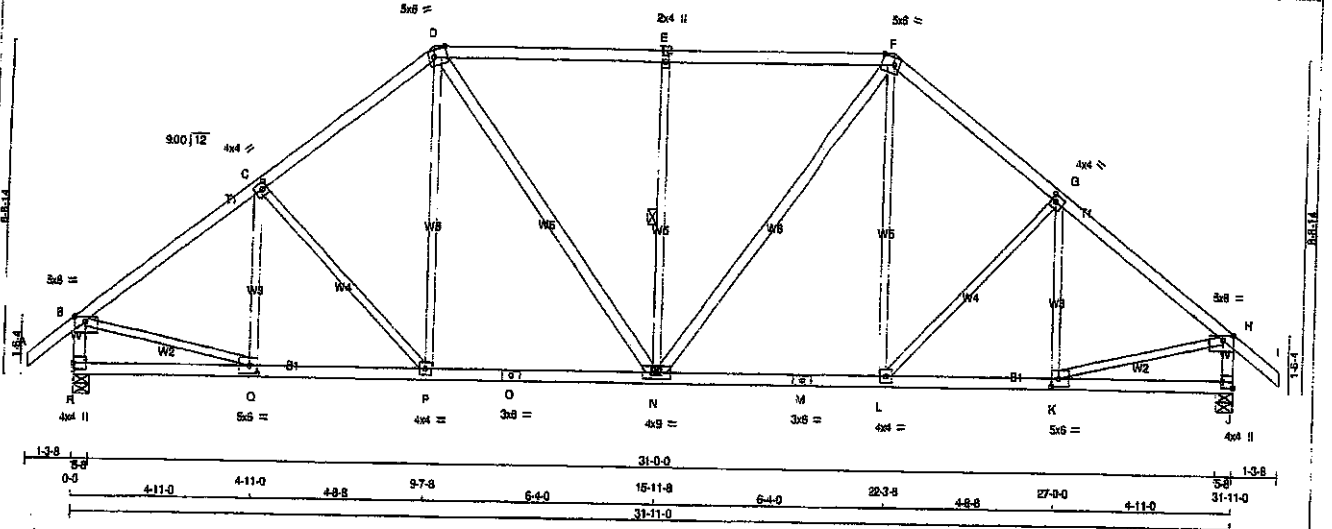
JSI METAL= 0.65 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911486

JOB NAME: 403045 TRUSS NAME: T5 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: T5 DRWG NO. 1

Tamarack Roof Truss, Burlington Version 8.300 S Apr 23 2019 MITek Industries, Inc. Tue May 14 10:41:24 2019 Page 1
ID: D7uIPnW9?DzGkx1HTFaFreyNCCt-pL_IQL7dH744ax4HBCQB,Is1ZFyGtpa3LRJ02,rcGdDr
Scale = 1/8"=1'-0"



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - S	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-i	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	8.0	Edge	4.25
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	Edge	4.25
G	TMVW-i	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMVW-i	MT20	4.0	4.0	2.00	Edge
K	BMVW-i	MT20	5.0	8.0	2.50	2.50
L	BMVW-i	MT20	4.0	4.0		
M	BS-i	MT20	3.0	6.0		
N	BMVWVW-i	MT20	4.0	9.0		
O	BS-i	MT20	3.0	6.0		
P	BMVW-i	MT20	4.0	4.0		
Q	BMVW-i	MT20	5.0	8.0	2.50	2.50
R	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
R	2385	0	5-8	5-8
J	2385	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
R	1773	1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0
J	1773	1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.89 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSF (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSF (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-C	-265 / 38	0.11 (1)
B-C	-2445 / 0	-102.1 -102.1	0.40 (1)	4.05	C-P	-265 / 0	0.23 (1)
C-D	-3286 / 0	-102.1 -102.1	0.38 (1)	4.18	P-D	0 / 425	0.10 (2)
D-E	-2193 / 0	-102.1 -102.1	0.61 (1)	3.89	D-N	0 / 652	0.10 (1)
E-F	-2193 / 0	-102.1 -102.1	0.61 (1)	3.89	N-E	-791 / 0	0.39 (1)
F-G	-2285 / 0	-102.1 -102.1	0.38 (1)	4.18	N-F	0 / 652	0.10 (1)
G-H	-2445 / 0	-102.1 -102.1	0.40 (1)	4.05	L-F	0 / 425	0.10 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-G	-265 / 0	0.23 (1)
R-B	-2304 / 0	0.0	0.0	5.82	K-G	-265 / 38	0.11 (1)
J-H	-2304 / 0	0.0	0.0	5.82	B-Q	0 / 2041	0.46 (1)
					K-H	0 / 2041	0.46 (1)
R-Q	0 / 0	-38.5 -38.5	0.15 (3)	10.00			
C-P	0 / 1983	-38.5 -38.5	0.45 (1)	10.00			
P-O	0 / 1802	-38.5 -38.5	0.49 (2)	10.00			
O-N	0 / 1802	-38.5 -38.5	0.49 (2)	10.00			
N-M	0 / 1802	-38.5 -38.5	0.49 (2)	10.00			
M-L	0 / 1802	-38.5 -38.5	0.49 (2)	10.00			
L-K	0 / 1983	-38.5 -38.5	0.45 (1)	10.00			
K-J	0 / 0	-38.5 -38.5	0.15 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")

CALCULATED VERT. DEFL.(LL) = L/889 (0.10")

ALLOWABLE DEFL.(TL) = L/360 (1.08")

CALCULATED VERT. DEFL.(TL) = L/989 (0.17")

CSF: TC=0.61/1.00 (D-E), SC=0.49/1.00 (N-P), WB=0.46/1.00 (H-K), SG=0.31/1.00 (E-F, I)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.30

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1657 798 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.86 (C) (INPUT = 0.90)

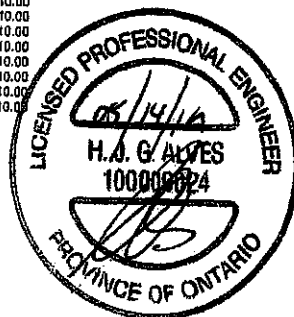
JSI METAL=0.68 (C) (INPUT = 1.00)

RECEIVED

JUN 13 2019

TOWN OF CALEDON BUILDING SECTION

FILE NO.



Structural component only

DWG# T-1911487

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TOTAL WEIGHT = 6 X 181 = 986 LB

[REVIEW]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	29.0	PSF	
	DL	=	6.0	PSF
BOT CH. LL	=	10.5	PSF	
	DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR SEISMICITY: #

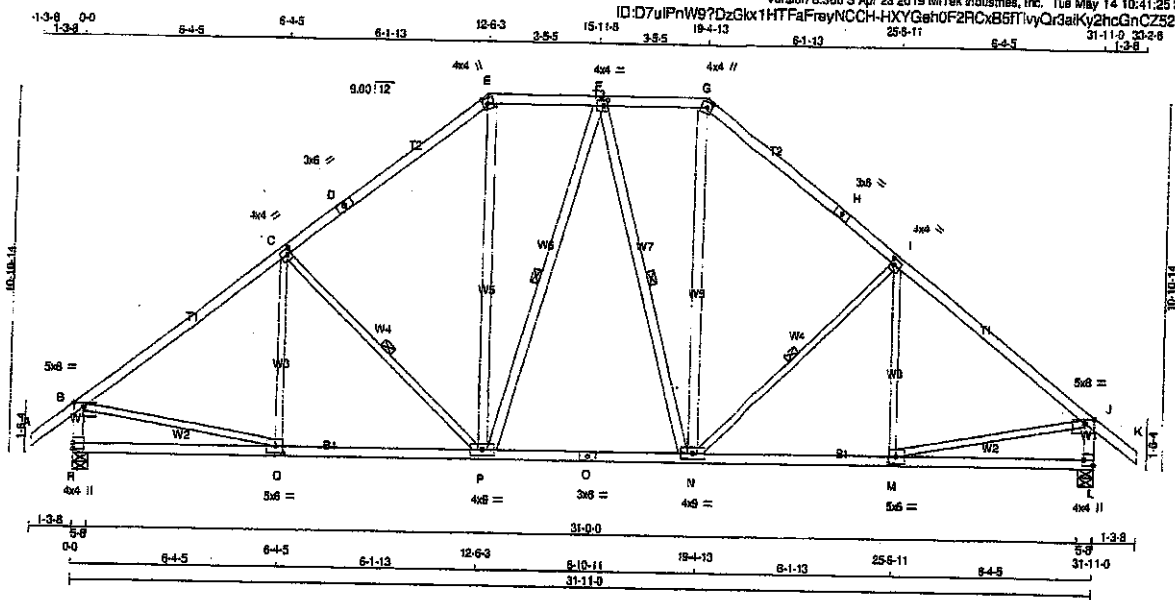
9

R-Q	0/0	-38.5	-38.5	0.22 (3)	10.00
P-P	0/2011	-38.5	-38.5	0.48 (2)	10.00
O-O	0/2011	-38.5	-38.5	0.48 (2)	10.00
N-N	0/1715	-38.5	-38.5	0.38 (1)	10.00
M-M	0/1715	-38.5	-38.5	0.38 (1)	10.00
L-L	0/2011	-38.5	-38.5	0.48 (2)	10.00
K-K	0/2011	-38.5	-38.5	0.48 (2)	10.00
J-J	0/0	-38.5	-38.5	0.00 (0)	10.00

W E B S		
MAX. FACTORED		
MEMB.	FORCE	MAX
THRU-TO	(LBS)	C/SI (LC)
C-C	-185 / 191	0.09 (1)
C-O	-422 / 0	0.52 (1)
C-D	0 / 498	0.11 (2)
D-N	0 / 440	0.07 (1)
N-E	-603 / 0	0.39 (1)
N-F	0 / 440	0.07 (1)
M-F	0 / 498	0.11 (2)
M-G	-422 / 0	0.52 (1)
K-G	-185 / 119	0.09 (1)
B-Q	0 / 2055	0.46 (1)
K-H	0 / 2055	0.46 (1)

Structural component only
DWG# T-1911488

JOB NAME 403045	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1/80.7

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - E	2x4	DRY No.2
E - G	2x4	DRY No.2
G - H	2x4	DRY No.2
H - K	2x4	DRY No.2
K - J	2x4	DRY No.2
J - I	2x4	DRY No.2
I - O	2x4	DRY No.2
O - L	2x4	DRY No.2
ALL WEBS EXCEPT	2x3	DRY No.2
P - E	2x4	DRY No.2
P - F	2x4	DRY No.2
F - N	2x4	DRY No.2
N - G	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.50	3.50
C TMVW-i	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTW+m	MT20	4.0	4.0	2.00	1.75
F TMVW-i	MT20	4.0	4.0	2.00	1.75
G TTW+m	MT20	4.0	4.0	2.00	1.75
H TS-t	MT20	3.0	8.0		
I TMVW-i	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	8.0	1.50	3.50
L BMV1-p	MT20	4.0	4.0	2.00	Edge
M BMVW-i	MT20	5.0	6.0	2.50	2.75
N BMVW-i	MT20	4.0	9.0		
O BS-t	MT20	3.0	6.0		
P BMVW-i	MT20	4.0	9.0		
Q BMVW-i	MT20	5.0	6.0	2.50	2.75
R BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT				
R	2385	0	5-8	5-8
L	2385	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS						
R	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0	
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, M-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB. FROM TO	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB. FROM TO
FR-TO							
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-C	-111 / 174
B-C	-2481 / 0	-102.1	-102.1	0.72 (1)	3.65	C-P	-559 / 0
C-D	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	P-E	0 / 832
D-E	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	P-F	-212 / 0
E-F	-1627 / 0	-102.1	-102.1	0.76 (1)	5.04	F-N	-216 / 0
F-G	-1627 / 0	-102.1	-102.1	0.76 (1)	5.04	N-G	0 / 837
G-H	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	N-I	-559 / 0
H-I	-2070 / 0	-102.1	-102.1	0.84 (1)	4.02	M-I	-111 / 174
I-J	-2481 / 0	-102.1	-102.1	0.72 (1)	3.65	B-Q	0 / 2057
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	M-J	0 / 2057
R-S	-2284 / 0	0.0	0.0	0.24 (1)	5.84		
L-J	-2284 / 0	0.0	0.0	0.24 (1)	5.84		
R-Q	0 / 0	-38.5	-38.5	0.28 (3)	10.00		
Q-P	0 / 2022	-38.5	-38.5	0.54 (2)	10.00		
P-O	0 / 1691	-38.5	-38.5	0.46 (2)	10.00		
O-N	0 / 1691	-38.5	-38.5	0.46 (2)	10.00		
N-M	0 / 2022	-38.5	-38.5	0.54 (2)	10.00		
M-L	0 / 0	-38.5	-38.5	0.28 (3)	10.00		

TOTAL WEIGHT = 2 X 167 = 335 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 52.5	PSF

SPACING = 24.0 IN/CG

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/959 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/959 (0.18")

CSI: TC=0.72/1.00 (B-C:1), BC=0.54/1.00 (P-Q:2), WB=0.46/1.00 (B-Q:1), SS=0.25/1.00 (B-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

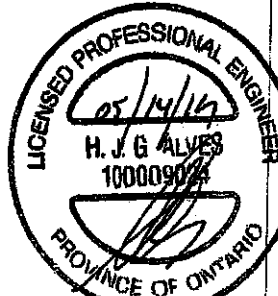
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788
	1987	1856

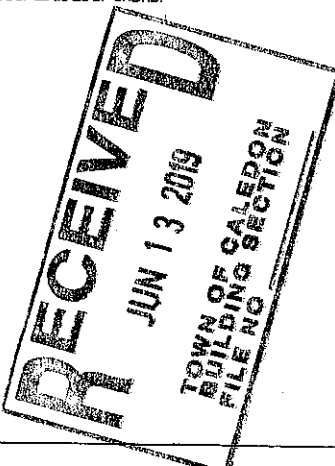
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

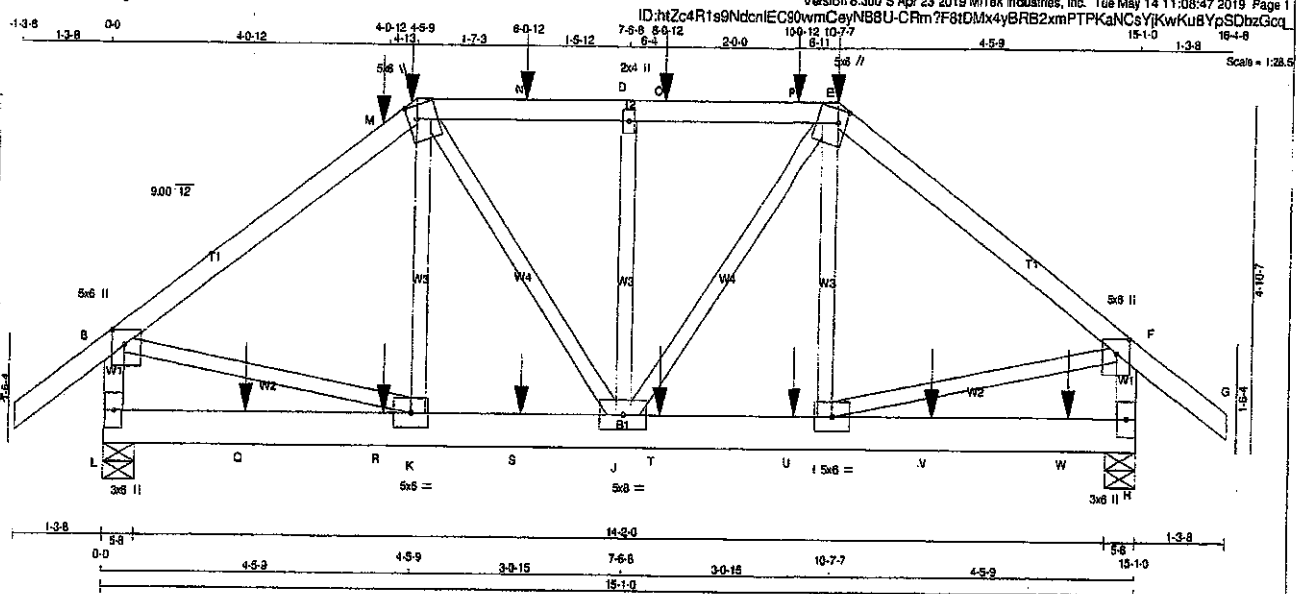
GRIP = 0.88 (C) (INPUT = 0.90)
METAL = 0.89 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911489



JOB NAME 403059	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 78 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	2.50	2.25
C TTWW+m	MT20	5.0	6.0	2.25	1.50
D TMVW+w	MT20	2.0	4.0		
E TTWW+m	MT20	5.0	6.0	2.25	1.50
F TMVW+p	MT20	5.0	6.0	2.50	2.25
H BMVW+p	MT20	3.0	6.0		
I BMVW+t	MT20	5.0	6.0		
J BMVW+w	MT20	5.0	6.0		
K BMVW+t	MT20	5.0	6.0		
L BMVW+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	1772	0	0	5-8
H	1759	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERMA LIVE	WIND	DEAD	SOIL
L	1307	776 / 0	223 / 0	0 / 0	0 / 0	309 / 0	0 / 0
H	1302	758 / 0	232 / 0	0 / 0	0 / 0	312 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED		
	FORCE	VERT. LOAD	MAX		FORCE	MAX	
	(LBS)	(PLF)	CSI (LC)		(LBS)	CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.15 (1)	10.00	K-C	-59 / 195	0.05 (3)
B-M	-1598 / 0	-102.1 -102.1	0.51 (1)	4.57	C-J	0 / 374	0.09 (1)
M-C	-1598 / 0	-102.1 -102.1	0.51 (1)	4.57	J-D	-551 / 0	0.19 (1)
C-N	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	J-E	0 / 404	0.10 (1)
N-D	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	E-I	-40 / 201	0.05 (3)
D-O	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	B-K	0 / 133	0.33 (1)
O-P	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99	I-F	0 / 132	0.33 (1)
P-E	-1511 / 0	-102.1 -102.1	0.28 (1)	4.99			
E-F	-1607 / 0	-102.1 -102.1	0.47 (1)	4.63			
F-G	0 / 42	-102.1 -102.1	0.15 (1)	10.00			
L-B	-1677 / 0	0.0	0.0	0.19 (1)	6.38		
H-F	-1648 / 0	0.0	0.0	0.19 (1)	6.41		
L-Q	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
Q-R	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
R-K	0 / 0	-38.5	-38.5	0.11 (3)	10.00		
K-S	0 / 1300	-38.5	-38.5	0.22 (1)	10.00		
S-J	0 / 1300	-38.5	-38.5	0.22 (1)	10.00		
J-T	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
T-U	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
U-I	0 / 1282	-38.5	-38.5	0.22 (1)	10.00		
I-V	0 / 0	-38.5	-38.5	0.10 (3)	10.00		
V-W	0 / 0	-38.5	-38.5	0.10 (3)	10.00		
W-H	0 / 0	-38.5	-38.5	0.10 (3)	10.00		

05/14/17

H. J. G. ALVES

100009874

LICENSED PROFESSIONAL

PROMOTED BY

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC.	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	4-6-8	-33	-36		FRONT	VERT	DEAD		
C	4-6-8	-188	-188		FRONT	VERT	SNOW		
E	10-7-7	-33	-36		FRONT	VERT	DEAD		
E	10-7-7	-188	-188		FRONT	VERT	SNOW		
N	4-0-12	-126	-126		BACK	VERT	TOTAL		
N	4-0-12	-96	-96		BACK	VERT	TOTAL		
O	8-0-12	-96	-96		BACK	VERT	TOTAL		
P	10-0-12	-118	-118		BACK	VERT	TOTAL		
Q	2-0-12	-36	-46		BACK	VERT	TOTAL		
R	4-0-12	-36	-46		BACK	VERT	TOTAL		
S	6-0-12	-36	-46		BACK	VERT	TOTAL		
T	8-0-12	-36	-46		BACK	VERT	TOTAL		
U	10-0-12	-36	-46		BACK	VERT	TOTAL		
V	12-0-12	-36	-46		BACK	VERT	TOTAL		
W	14-0-12	-36	-46		BACK	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 085-09, CSA 085-14
- TPIC 2011, TPIC 2014

65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/959 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = L/959 (0.05")

CSI: TC=0.51/1.00 (B-C:1), BC=0.22/1.00 (I-J:1), WB=0.33/1.00 (B-K:1), SB=0.27/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 789 1987 1656

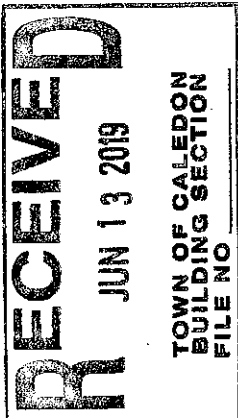
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.32 (I) (INPUT = 1.00)

Structural component only

DWG# T-1911501 CONTINUED ON PAGE 2



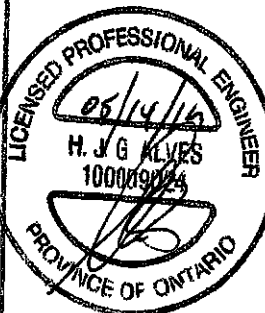
Tamarack Roof Truss, Burlington



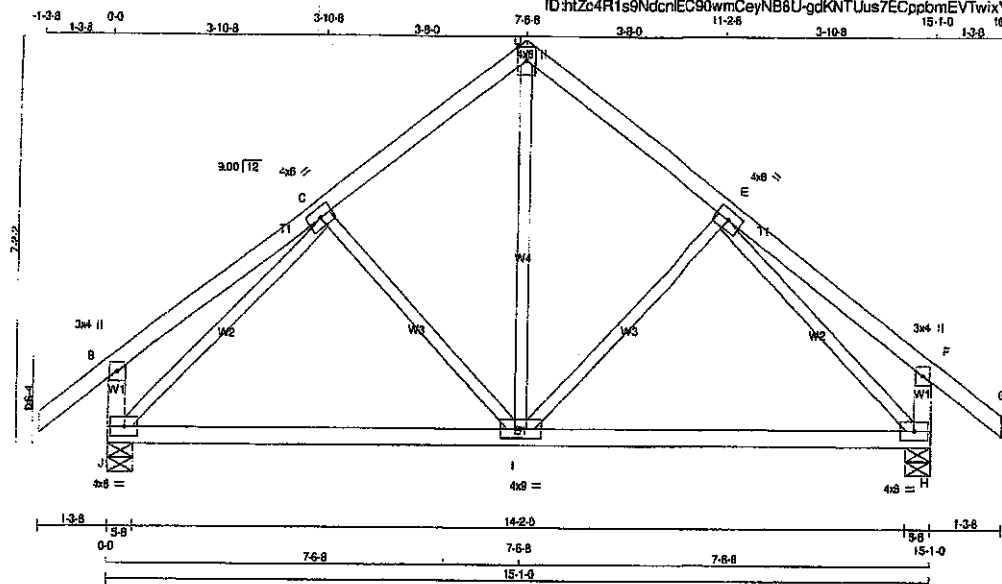
Structural component only
DWG# T-1911502

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMVW-t	MT20	4.0	4.0	2.00 1.75
G	TMV+p	MT20	3.0	4.0	
I	BMVW-t	MT20	4.0	4.0	
J	BMVWVW-t	MT20	4.0	12.0	
K	BMVW-t	MT20	4.0	4.0	

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 403059	TRUSS NAME T15	QUANTITY 5	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:08:48 2019 Page 1 ID:htZc4R1s9NdcnIEC90wmCeyNB8U-gdKNTUus7ECppdmEVTwixY7cKgpI3KK2NCY0l1zGgp2	



Scale = 1:35.0

TOTAL WEIGHT = 5 X 67 = 337 b (M/F)

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4 DRY	No.2	SPF
D - G	2x4 DRY	No.2	SPF
J - B	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
J - H	2x4 DRY	No.2	SPF
ALL WEBS	2x3 DRY	No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	6.0	Edge
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
I	BMWW1-t	MT20	4.0	6.0	
J	BMWW1-t	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
J	1201	0	1201	0	0
H	1201	0	1201	0	0

UNFACTORED REACTIONS		1ST CASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	889	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
H	889	518 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	MAX. UNBRACED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD	CSI (LC)	FROM TO	LENGTH	MEMB.	FORCE (LBS)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 634
B-C	0 / 26	-102.1	-102.1	0.23 (1)	10.00	I-E	-188 / 23
C-D	-804 / 0	-102.1	-102.1	0.18 (1)	6.25	C-I	-188 / 23
D-E	-804 / 0	-102.1	-102.1	0.18 (1)	6.25	J-C	-1100 / 0
E-F	0 / 26	-102.1	-102.1	0.23 (1)	10.00	E-H	-1100 / 0
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
J-B	-288 / 0	0.0	0.0	0.03 (1)	7.81		
H-F	-288 / 0	0.0	0.0	0.03 (1)	7.81		
J-I	0 / 752	-38.5	-38.5	0.56 (2)	10.00		
I-H	0 / 752	-38.5	-38.5	0.56 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = 1 / 999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.50")
CALCULATED VERT. DEFL.(TL) = 1 / 999 (0.16")

CSI: TC=0.23/1.00 (E-F:1), BC=0.59/1.00 (I-J:2), WB=0.54/1.00 (E-H:1), SS=0.19/1.00 (H-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

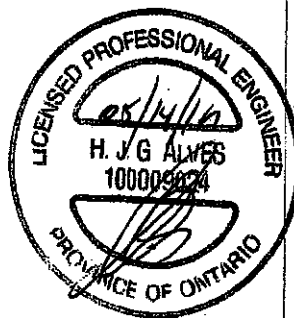
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLJ) (PLJ)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

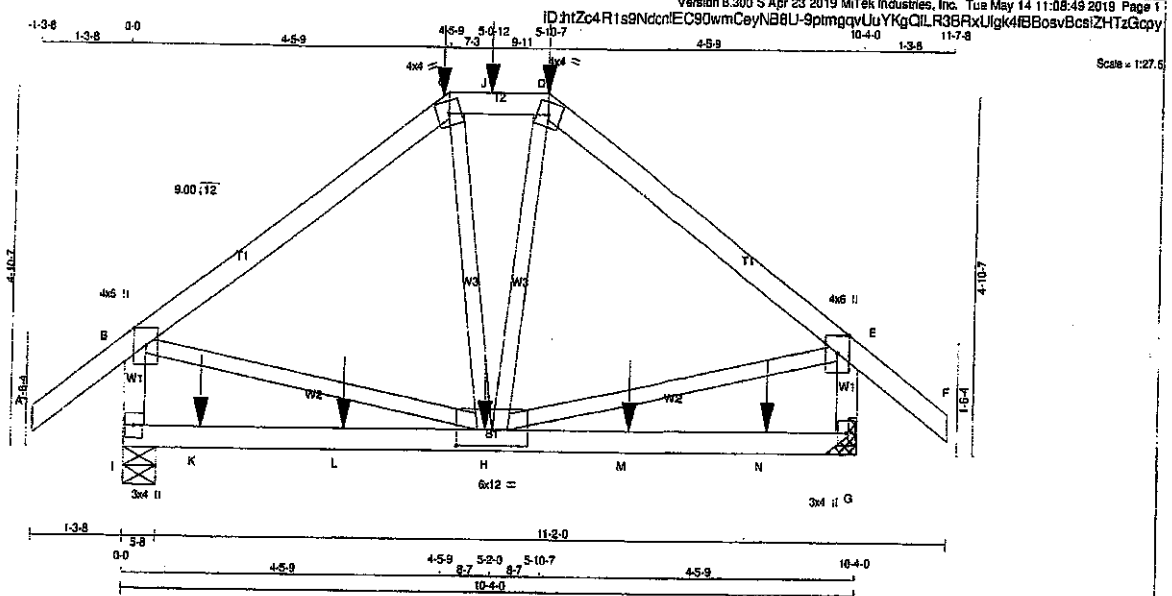
PLATE ROTATION TOL = 5.0 Deg.

ISI GRIP= 0.65 (C) (INPUT = 0.90)
ISI METAL= 0.27 (E) (INPUT = 1.00)



Structural component only
DWG# T-1911503

JOB NAME 403059	TRUSS NAME T16	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



Scale = 1/27.6

TOTAL WEIGHT = 48 lb

LUMBER

N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2		SPF
C - D	2x4	DRY	No.2		SPF
D - F	2x4	DRY	No.2		SPF
F - B	2x4	DRY	No.2		SPF
G - E	2x4	DRY	No.2		SPF
I - G	2x4	DRY	No.2		SPF
ALL WEBS		2x3	DRY	No.2	SPF
EXCEPT:					

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	6.0	Edge
C	TTW-m	MT20	4.0	4.0	
D	TTW-m	MT20	4.0	4.0	
E	TMW+p	MT20	4.0	6.0	Edge
G	BMV+p	MT20	3.0	4.0	
H	BMVWW+m	MT20	6.0	12.0	2.75 8.00
I	BMV+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
I	1245	0	0	5-8
G	1239	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 5-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	918	543/0	159/0	0/0	0/0	218/0	0/0
G	915	541/0	157/0	0/0	0/0	218/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.68 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	WEBS MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0/42	-102.1 -102.1	0.15 (1)	10.00	B-H	0/799	0.20 (1)
B-C	-975/0	-102.1 -102.1	0.42 (1)	5.88	H-E	0/796	0.20 (1)
C-D	-777/0	-102.1 -102.1	0.11 (1)	6.25	C-H	-7/158	0.04 (3)
J-D	-777/0	-102.1 -102.1	0.11 (1)	6.25	H-D	0/161	0.04 (3)
D-E	-971/0	-102.1 -102.1	0.42 (1)	5.88			
E-F	0/42	-102.1 -102.1	0.15 (1)	10.00			
F-B	-1135/0	0.0 0.0	0.13 (1)	7.42			
G-E	-1133/0	0.0 0.0	0.13 (1)	7.43			
I-K	0/0	-38.5 -38.5	0.35 (3)	10.00			
K-L	0/0	-38.5 -38.5	0.35 (3)	10.00			
L-H	0/0	-38.5 -38.5	0.35 (3)	10.00			
H-M	0/0	-38.5 -38.5	0.35 (3)	10.00			
M-N	0/0	-38.5 -38.5	0.35 (3)	10.00			
N-G	0/0	-38.5 -38.5	0.35 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	4-5-9	-33	-36	-	FRONT	VERT.	DEAD	-	-
C	4-5-9	-189	-189	-	FRONT	VERT.	SNOW	-	-
D	5-10-7	-33	-36	-	FRONT	VERT.	DEAD	-	-
D	5-10-7	-189	-189	-	FRONT	VERT.	SNOW	-	-
H	5-0-12	-38	-48	-	FRONT	VERT.	TOTAL	-	-
J	5-0-12	-128	-128	-	FRONT	VERT.	TOTAL	-	-
K	1-0-12	-38	-48	-	FRONT	VERT.	TOTAL	-	-
L	3-0-12	-38	-48	-	FRONT	VERT.	TOTAL	-	-
M	7-0-12	-38	-48	-	FRONT	VERT.	TOTAL	-	-
N	9-0-12	-38	-48	-	FRONT	VERT.	TOTAL	-	-

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.037")
ALLOWABLE DEFL.(TL) = L/360 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.037")

CSI: TC=0.42/1.00 (B-C:1), BC=0.35/1.00 (G-H:3), WB=0.20/1.00 (B-H:1), SG=0.19/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MT20	618 354 1687 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

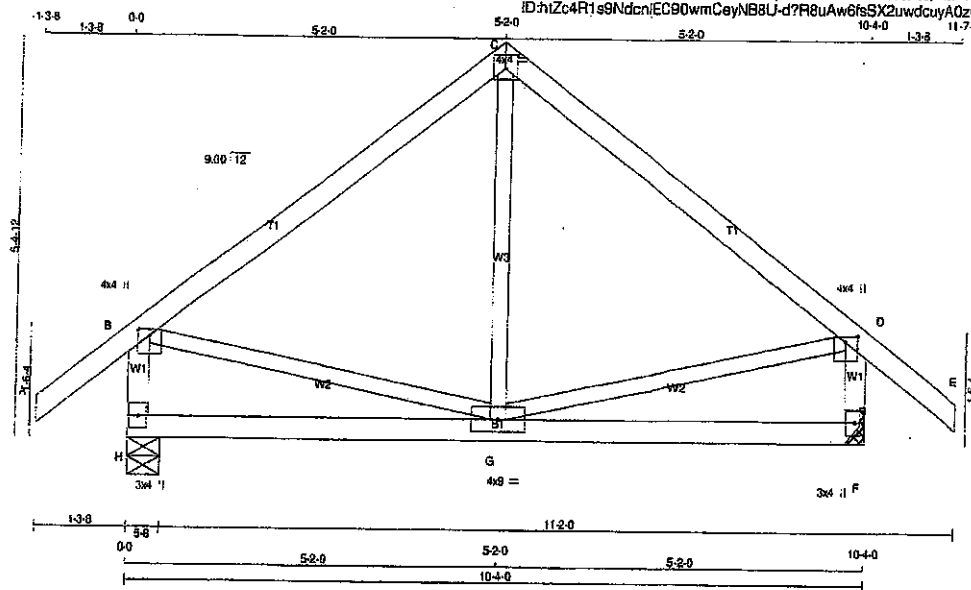
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (E) (INPUT = 0.90)
JSI METAL = 0.53 (E) (INPUT = 1.00)

Structural component only
DWG# T-1911504



JOB NAME 403059	TRUSS NAME T17	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	Version 8.300 5 Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:08:50 2019 Page 1 ID:h1Zc4R1s9NdcnEC90wmCeyNB8U-d?R8uAw6fsX2uwdcuyA0zCwv3ZHXLPqW16pwzGcpx



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 45 lb

LUMBER

N. L. G. A. RULES	
CHORDS	SIZE
A - C	2x4
C - E	2x4
H - B	2x4
F - D	2x4
H - F	2x4

ALL WEBS 2x3 DRY No.2
EXCEPT
DRY, SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMVW+p	MT20	4.0	9.0		
H	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
H	867	0	867	0	0
F	867	0	867	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	839	380 / 0	109 / 0	0 / 0	0 / 0	151 / 0	0 / 0
F	839	380 / 0	109 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 223
B-C	-493 / 0	-102.1	-102.1	0.35 (1)	8.25	B-G	0 / 405
C-D	-493 / 0	-102.1	-102.1	0.35 (1)	8.25	G-D	0 / 405
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
H-B	-791 / 0	0.0	0.0	0.08 (1)	7.81		
F-D	-791 / 0	0.0	0.0	0.08 (1)	7.81		
H-G	0 / 0	-38.5	-38.5	0.23 (3)	10.00		
G-F	0 / 0	-38.5	-38.5	0.23 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24" IN LC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.34")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/380 (0.34")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.35/1.00 (C-D:1), BC=0.23/1.00 (F-G:3), WB=0.08/1.00 (B-G:1), SSB=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

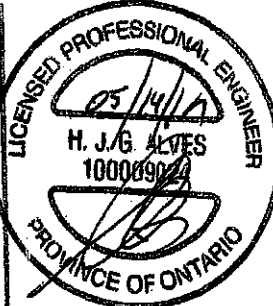
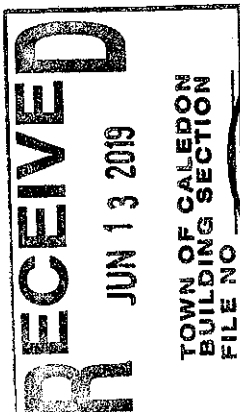
NAIL VALUES

PLATE	GRP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
MT20	618	354	1667
	788	1987	1558

PLATE PLACEMENT TOL. = 0.250 inches

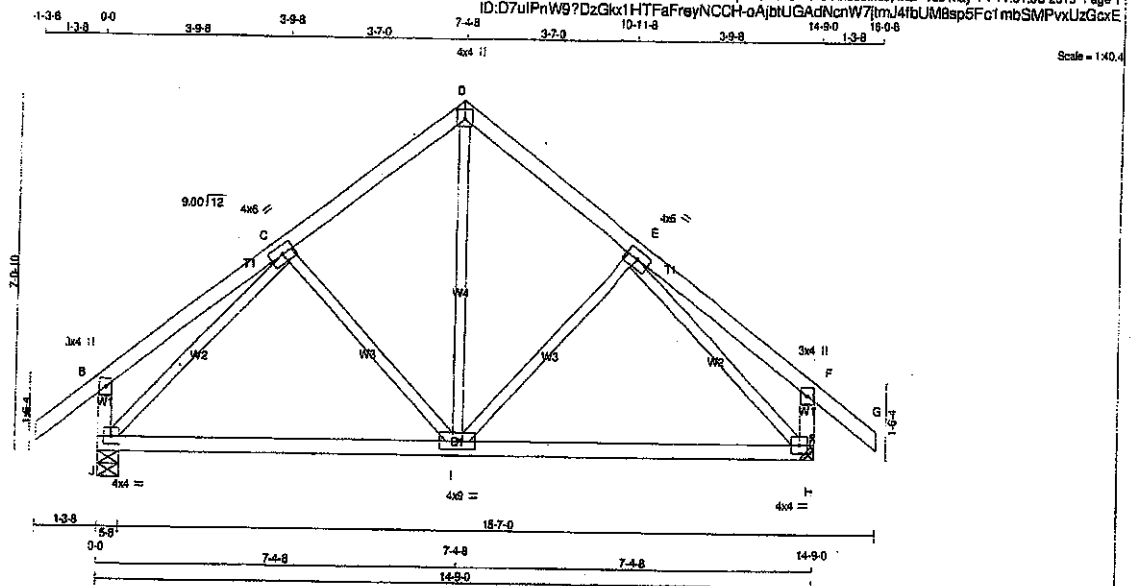
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.75 (B) (INPUT = 0.90)
JSI METAL = 0.19 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
401607	T100	5	1	GREEN PARK HOMES		
Tamarack Roof Truss, Burlington						



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

DRY: SEASONED LUMBER

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW-1	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	4.0		
I	BMVW-1	MT20	4.0	4.0		
J	BMVW-1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	IN-SX
J	1178	0	0	5-8
H	1178	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3'-8".

UNFACTORED REACTIONS

JT	1ST LOASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
J	872	508 / 0	155 / 0
H	872	508 / 0	155 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED LCH (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO									
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 618	0.14 (1)	
B-C	0 / 26	-102.1	-102.1	0.22 (1)	10.00	I-E	-181 / 24	0.09 (1)	
C-D	-783 / 0	-102.1	-102.1	0.17 (1)	6.25	C-I	-181 / 24	0.09 (1)	
D-E	-783 / 0	-102.1	-102.1	0.17 (1)	6.25	J-C	-1072 / 0	0.50 (1)	
E-F	0 / 26	-102.1	-102.1	0.22 (1)	10.00	E-H	-1072 / 0	0.50 (1)	
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
J-B	-285 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-285 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 / 730	-38.5	-38.5	0.54 (2)	10.00				
I-H	0 / 730	-38.5	-38.5	0.54 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/989 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/989 (0.15")

CSI: TC=0.22/1.00 (B-C:1), BC=0.54/1.00 (I-J:2), WB=0.50/1.00 (C-J:1), SS=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

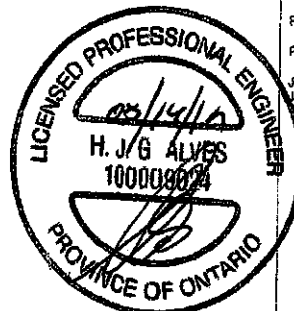
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1687

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.89 (D) (INPUT = 0.90)
ISI METAL = 0.27 (J) (INPUT = 1.00)



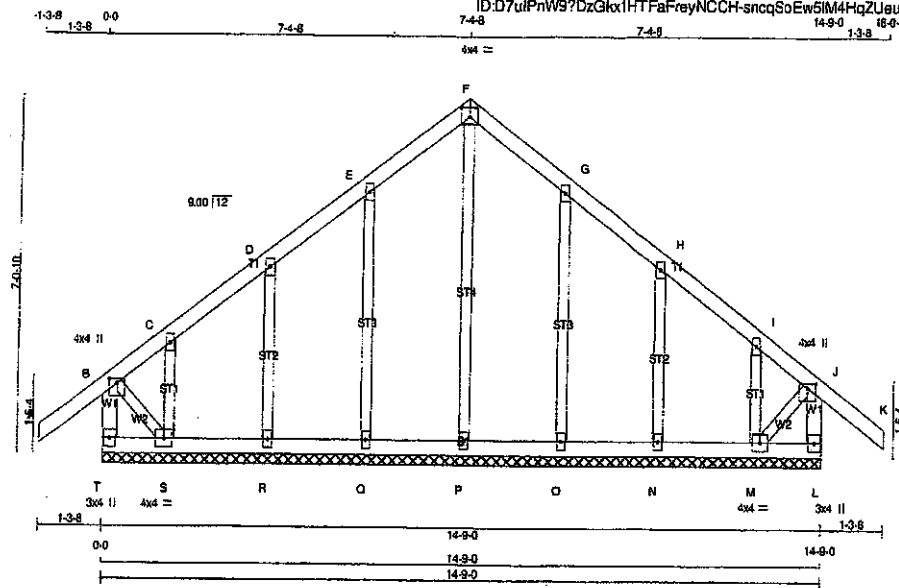
Structural component only
DWG# T-1911516

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401607	G100	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 69 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - L	2x4	DRY	No.2	SPF	

ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					

GABLE STUDS SPACED AT 2'-0" OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED LCI (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	FACTORED LCI (LBS)
FR-TO				FR-TO			
T-B	-310 / 0	0.0	0.03 (1)	P-F	-157 / 0	0.13 (1)	
A-B	0 / 42	-102.1	-102.1 0.14 (1)	Q-E	-229 / 0	0.11 (1)	
B-C	-68 / 0	-102.1	-102.1 0.13 (1)	R-D	-210 / 0	0.05 (1)	
C-D	-18 / 0	-102.1	-102.1 0.05 (1)	S-C	-88 / 0	0.01 (1)	
D-E	-18 / 0	-102.1	-102.1 0.06 (1)	O-G	-228 / 0	0.11 (1)	
E-F	-25 / 0	-102.1	-102.1 0.06 (1)	N-H	-218 / 0	0.05 (1)	
F-G	-25 / 0	-102.1	-102.1 0.06 (1)	M-I	-85 / 0	0.01 (1)	
G-H	-18 / 0	-102.1	-102.1 0.06 (1)	B-S	0 / 30	0.01 (1)	
H-I	-18 / 0	-102.1	-102.1 0.05 (1)	M-J	0 / 30	0.01 (1)	
I-J	-68 / 0	-102.1	-102.1 0.13 (1)				
J-K	0 / 42	-102.1	-102.1 0.14 (1)				
L-K	-310 / 0	0.0	0.03 (1)				
T-S	0 / 0	-38.5	-38.5 0.02 (3)				
S-R	0 / 19	-38.5	-38.5 0.03 (2)				
R-Q	0 / 14	-38.5	-38.5 0.02 (3)				
Q-P	0 / 10	-38.5	-38.5 0.02 (3)				
P-O	0 / 10	-38.5	-38.5 0.02 (3)				
O-N	0 / 14	-38.5	-38.5 0.02 (3)				
N-M	0 / 19	-38.5	-38.5 0.03 (2)				
M-L	0 / 0	-38.5	-38.5 0.02 (3)				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMVW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMVW+p	MT20	3.0	4.0		
M BMVW1-i	MT20	4.0	4.0		
N, O, P, Q, R					
N BMVW+w	MT20	2.0	4.0		
S BMVW1-i	MT20	4.0	4.0		
T BMVW+p	MT20	3.0	4.0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (M-N:2),
WB=0.13/1.00 (F-P:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT YOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.30)
JSI METAL= 0.12 (E) (INPUT = 1.00)



Structural component only
DWG# T-1911497

Tamarack Roof Truss, Buxington

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0-0 2-10-11 2-10-11 4-0-0 5-10-11 2-10-11 5-5

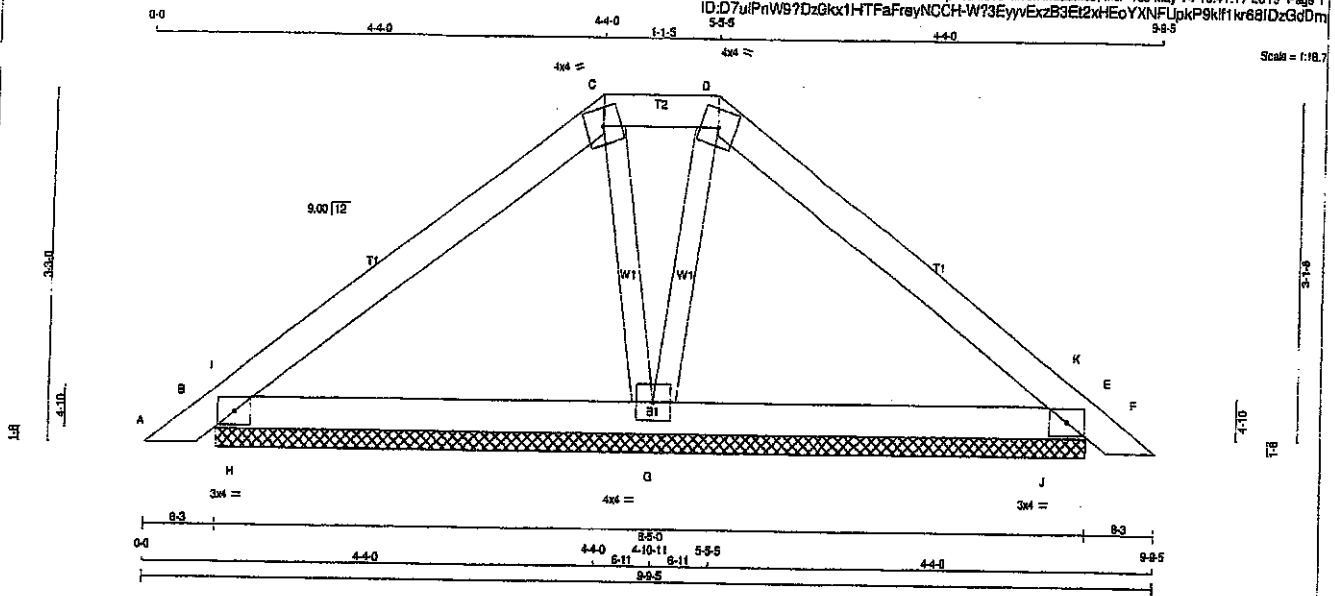
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	TRUSS DESC.	DRWG NO.
403045	P2	2	1	GREEN PARK HOMES		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 2 X 27 = 54 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.	SPF
SPF	
SPF	
SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	401	0	401	0	8-5-0	8-5-0
E	390	0	390	0	8-5-0	8-5-0
G	514	0	514	0	8-5-0	8-5-0

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
B	293	182 / 0	43 / 0	0 / 0	0 / 0	67 / 0	0 / 0
E	285	179 / 0	42 / 0	0 / 0	0 / 0	65 / 0	0 / 0
G	390	197 / 0	92 / 0	0 / 0	0 / 0	102 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
		FROM	TO				
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	C-G	-127 / 0
B-I	-117 / 0	-102.1	-102.1	0.06 (1)	8.25	G-D	-129 / 0
I-C	-185 / 0	-102.1	-102.1	0.18 (1)	8.25	H-I	-304 / 76
C-D	-98 / 0	-102.1	-102.1	0.02 (1)	8.25	J-K	-327 / 56
D-K	-157 / 0	-102.1	-102.1	0.17 (1)	8.25		
K-E	-103 / 0	-102.1	-102.1	0.08 (1)	8.25		
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00		
B-H	0 / 126	-38.5	-38.5	0.14 (1)	10.00		
H-G	0 / 126	-38.5	-38.5	0.20 (2)	10.00		
G-I	0 / 119	-38.5	-38.5	0.19 (2)	10.00		
I-J	0 / 119	-38.5	-38.5	0.15 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.18/1.00 (C-I), BC=0.20/1.00 (G-H:2), WB=0.02/1.00 (D-G:1), SI=0.28/1.00 (E-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

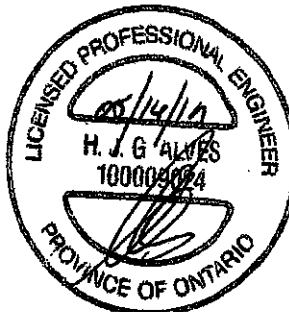
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

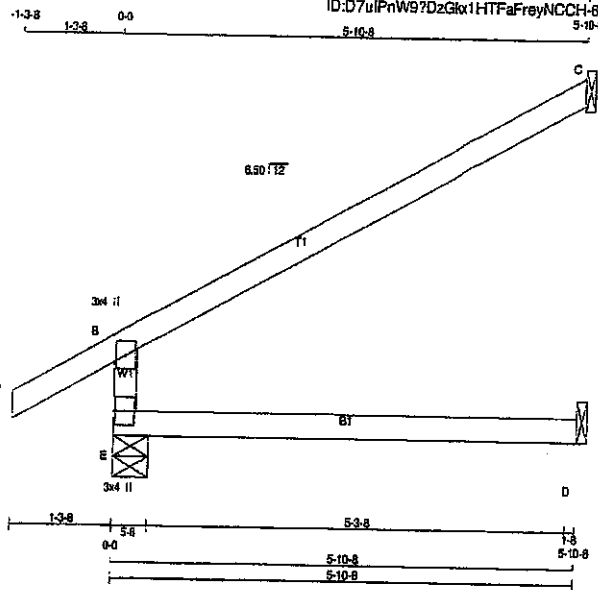
JSI GRIP= 0.30 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO



Structural component only
DWG# T-1911480

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	U1	17	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Scale = 1/24"

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER

PLATES (table is in inches)	JT	TYPE	PLATES	-W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0			
E	BMV1+p	MT20	3.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	RECORD BRG
	VERT	HORZ	DOWN	HORZ		
E	846	0	846	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	94	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN. COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
E	473	292/0	72/0	0/0	0/0	189/0	0/0
C	154	128/0	0/0	0/0	0/0	26/0	0/0
D	85	0/0	51/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM TO		FR-TO			
E-B	-514/0	0.0	0.0	0.21 (3)	7.81		
A-B	0/33	-102.1	-102.1	0.13 (1)	10.00		
B-C	-36/0	-102.1	-102.1	0.80 (1)	8.25		
E-D	0/0	-38.5	-38.5	0.21 (3)	10.00		

TOTAL WEIGHT = 17 X 17 = 289 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 18.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OSC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/969$ (0.05")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/930$ (0.08")

CSI: TC=0.80/1.80 (B-C:1), BC=0.21/1.00 (D-E:3), WB=0.00/1.00 (W-E:0), SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

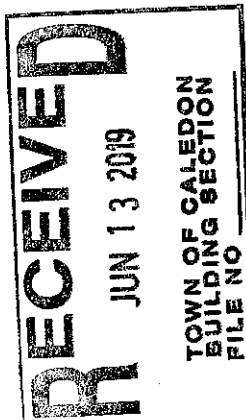
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	818	354	1667
	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.22 (E) (INPUT = 0.80)
JSI METAL=0.15 (B) (INPUT = 1.00)

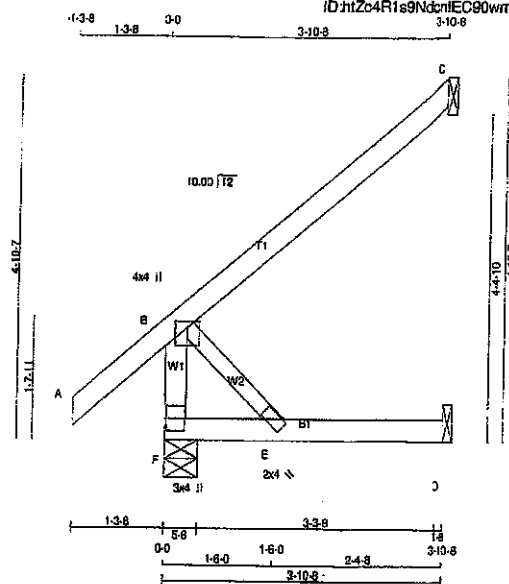


Structural component only
DWG# T-1911476

Tamarack Roof Truss, Burlington

Version 8.300 S Apr 23 2019 MTEK Industries, Inc. Tue May 14 11:08:45 2019 Page 1
ID:htZc4R1s9NdcnIEC90wmCeyNB8U-G2eFrSszcJqEY71foLN?JvV4c2lds5?chEKM9izGcc0

Scale = 1:27.3



TOTAL WEIGHT = $12 \times 15 = 183 \text{ lb}$

11月30日

CONSUMER N. L. G. A. RULES

CHORDS	SIZE	
F - B	2x4	DRY
A - C	2x4	DRY
F - D	2x4	DRY

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER

LUMBER

No.2
No.2
No.2

DESCR

SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REGRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
F	414	0	414	0	0	5-8	5-8
C	198	0	198	0	0	1-8	1-8
D	75	0	95	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C. D.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	301	193/0	41/0	0/0	0/0	67/0	0/0
C	136	113/0	0/0	0/0	0/0	23/0	0/0
D	68	0/0	41/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED

LOADING

TOTAL LOAD CASES: (5)

CHORDS				WEBBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO		FROM TO		FR-TO			
F-B	-339 / 0	0.0	0.0	0.04 (1)	B-E	0 / 0	0.00 (1)
A-B	0 / 45	-102.1	-102.1	0.14 (1)			
B-C	0 / 0	-102.1	-102.1	0.26 (1)			
F-E	0 / 0	-38.5	-38.5	0.13 (3)			
E-D	0 / 0	-38.5	-38.5	0.14 (3)			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/D

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR
SMALL BUILDING REQUIREMENTS OF PART 9,
NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPK 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(L) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(L) = $L/999$ (0.01")
 ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
 CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.25/1.00 (B-C:1), BC=0.14/1.00 (D-E:3),
WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Structural component only
DWG# T-1911499





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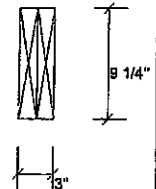
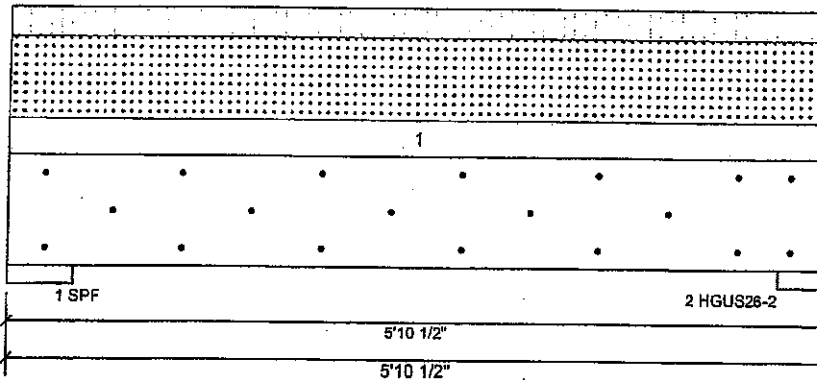
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 6 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	264	327	741	0
2	253	313	710	0

Bearings and Factored Reactions

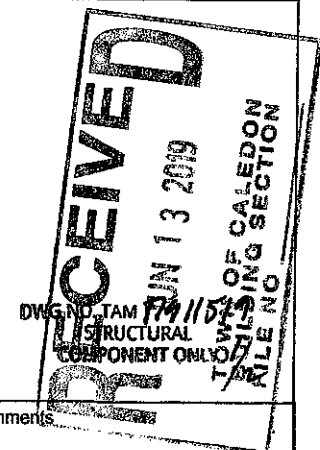
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	408 / 1376	1784 L	1.25D+1.5S +L
2 - HGUS...	4.000"	23%	391 / 1318	1710 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2016 ft-lb	3'	8039 ft-lb	0.334 (33%)	1.25D+1.5S +L	L
Unbraced	2016 ft-lb	3'	5236 ft-lb	0.385 (39%)	1.25D+1.5S +L	L
Shear	1549 lb	12"	3984 lb	0.389 (39%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3535)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.024 (L/2573)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral steadiness ratio based on single ply width.



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-4-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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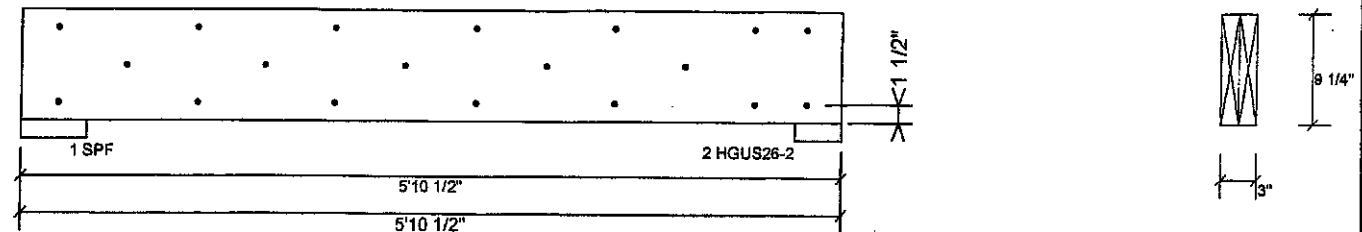
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 7 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	87.4 %
Load	297.3 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.2SD+1.5S+L
Duration Factor	1.00



DWG NO. TAM 71911579
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(800) 335-1115



This design is valid until 12/11/2021





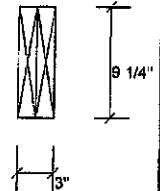
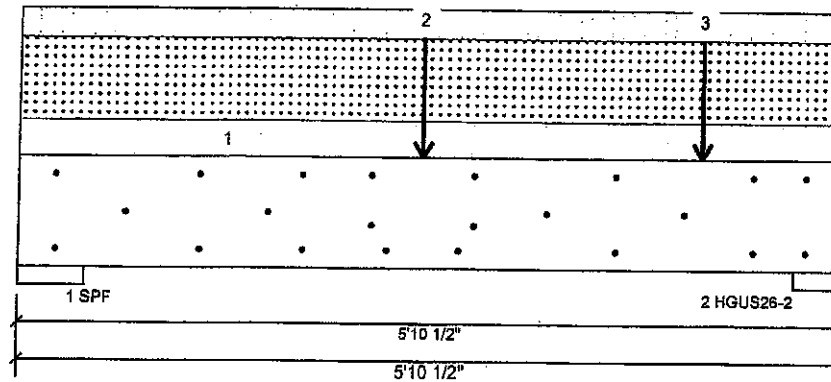
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 12 of 13

B4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	181	212	514	0
2	229	307	754	0

Bearings and Factored Reactions

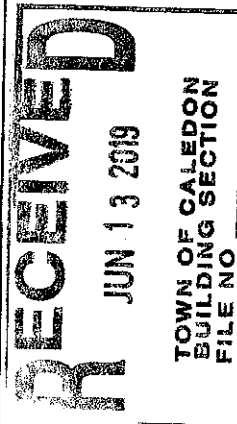
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	285 / 931	1196	L	1.25D+1.5S +L
2 - HGUS...	4.000"	24%	384 / 1360	1744	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2390 ft-lb	2'10 1/2"	6039 ft-lb	0.396 (40%)	1.25D+1.5S +L	L
Unbraced	2390 ft-lb	2'10 1/2"	5236 ft-lb	0.457 (46%)	1.25D+1.5S +L	L
Shear	1659 lb	4'10"	3984 lb	0.416 (42%)	1.25D+1.5S +L	L
LL Defl inch	0.019 (L/3279)	3' 3/8"	0.174 (L/360)	0.110 (11%)	S+0.5L	L
TL Defl inch	0.026 (L/2421)	3' 5/16"	0.174 (L/360)	0.150 (15%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all piles.
- 6 Top braced at bearings.
- 7 Bottom braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

DWG NO. TAM 1791522
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	28.5 PSF	0 PSF	
2	Point	2-10-8		Far Face	216 lb	157 lb	541 lb	0 lb	T16
3	Point	4-10-8		Top	151 lb	109 lb	380 lb	0 lb	T17

Manufacturer InfoTamarack Roof Trusses
3268 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115

This design is valid until 12/11/2021





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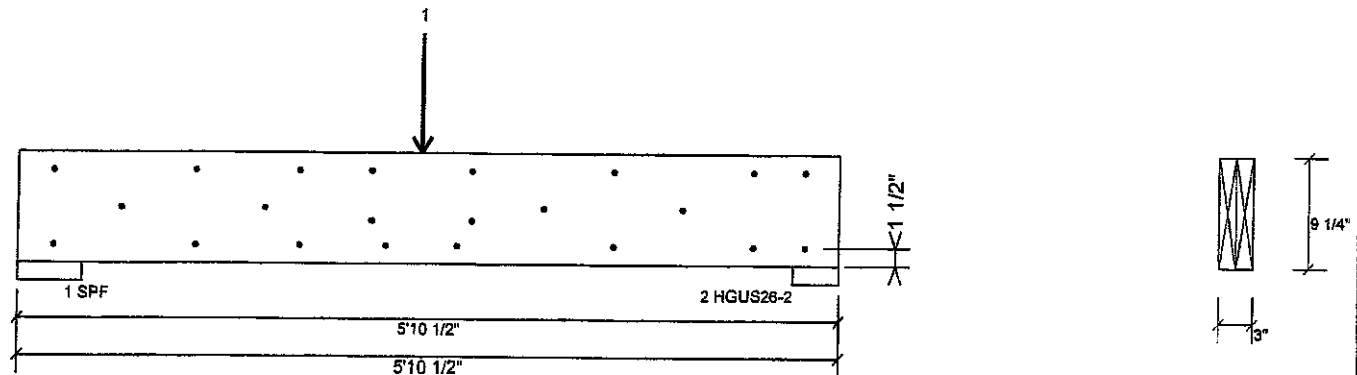
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 13 of 13

B4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

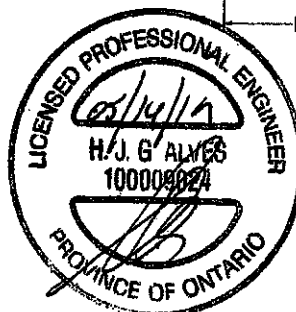
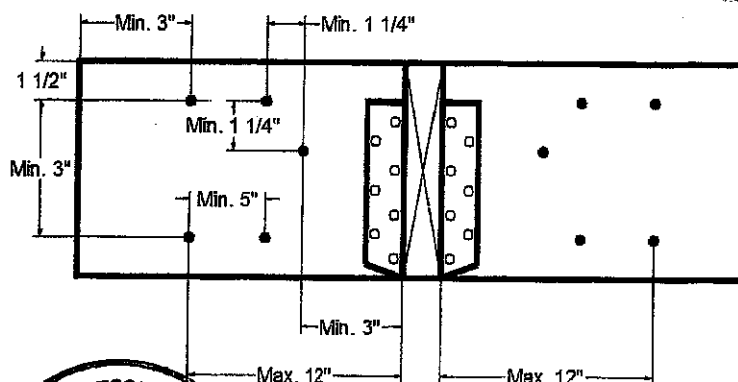
Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 2-10-8 with a minimum of (6) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	91.1 %
Load	619.3lb.
Total Yield Limit	680.1 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. 1AM *TMH/522*
STRUCTURAL
COMPONENT ONLY *7/2*

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N9G2
(805) 335-1115



This design is valid until 12/11/2021





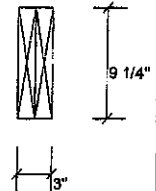
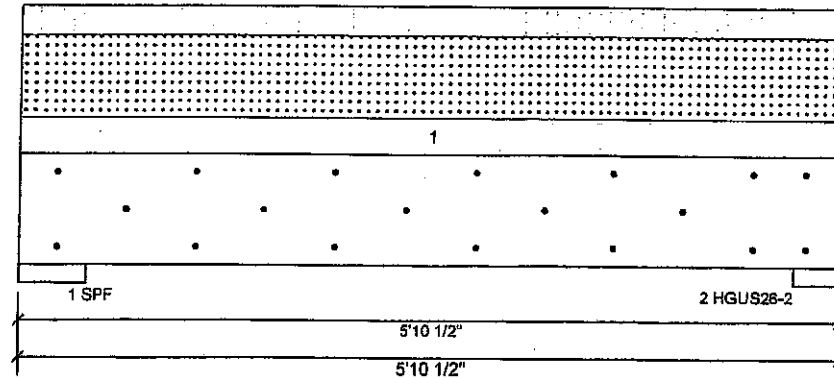
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 10 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED 1b (Uplift)

Brg	Live	Dead	Snow	Wind
1	269	333	756	0
2	258	319	724	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	416 / 1403	1819 L	1.25D+1.5S +L
2 - HGUS...	4.000"	24%	399 / 1345	1744 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2056 ft-lb	3'	6039 ft-lb	0.341 (34%)	1.25D+1.5S +L	L
Unbraced	2056 ft-lb	3'	5236 ft-lb	0.393 (39%)	1.25D+1.5S +L	L
Shear	1579 lb	12"	3984 lb	0.398 (40%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3466)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.025 (L/2523)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

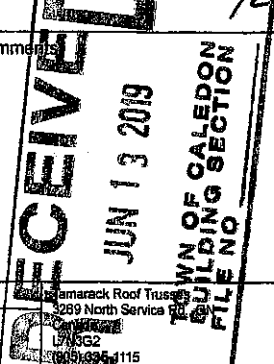
- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 194521

STRUCTURAL

COMMENT ONLY 1/2



ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-6-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

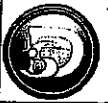
Manufacturer Info

amarack Roof Trusses
3289 North Service Rd.
L7R 3G2
(905) 995-1115

AMARACK
LUMBER INC
ALMA LUMBER GROUP

This design is valid until 12/11/2021





isDesign™

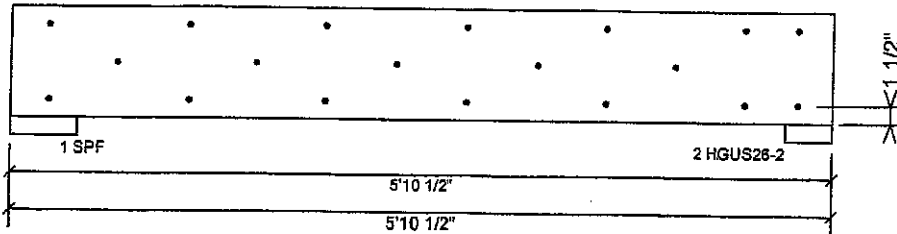
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 11 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.2 %
Load	303.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.2SD+1.5S+L
Duration Factor	1.00



DWG NO. TAM 1791/521
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (800) 335-1115
	TAMARACK LUMBER INC ALFA LUMBER GROUP

This design is valid until 12/11/2021

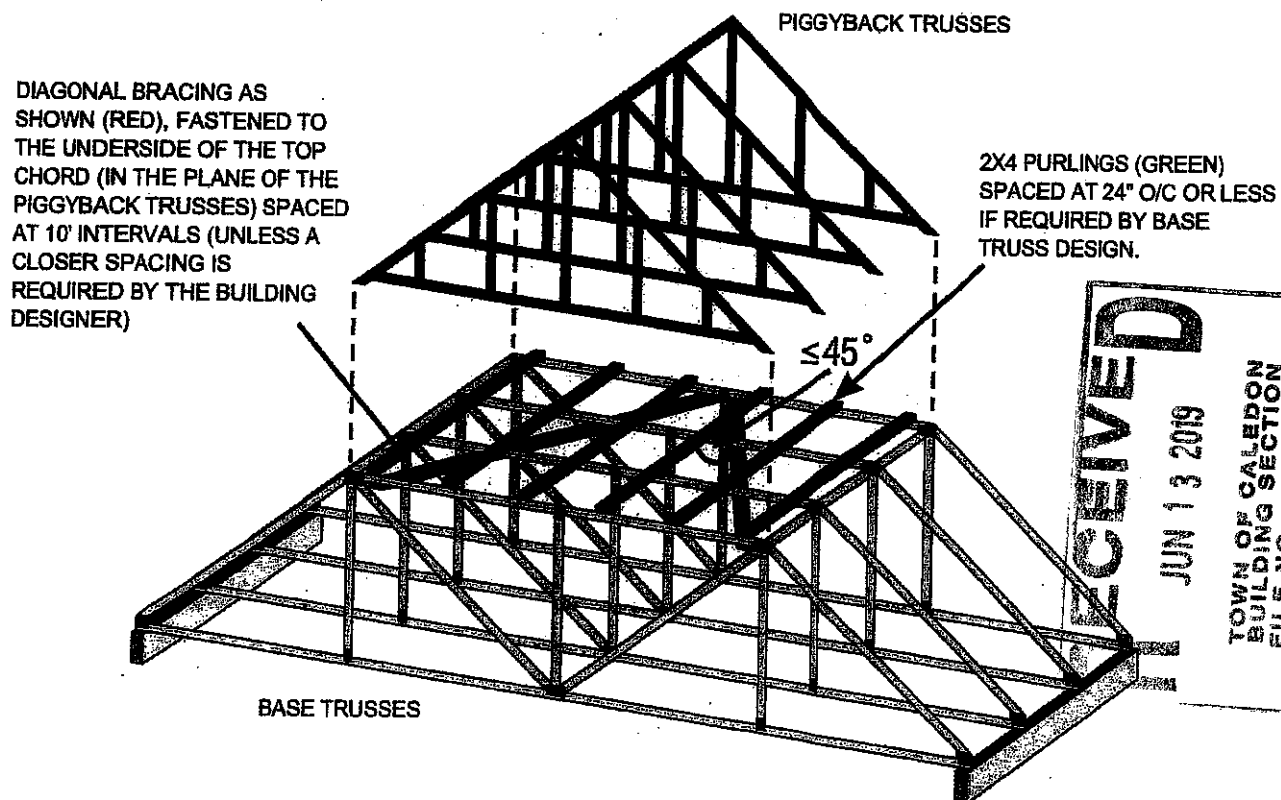


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

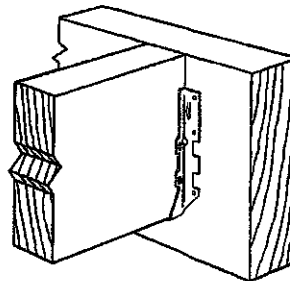
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

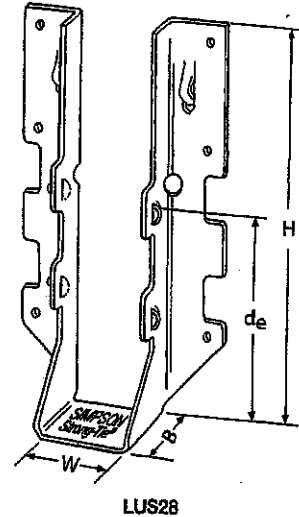
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



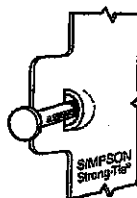
Typical LUS Installation



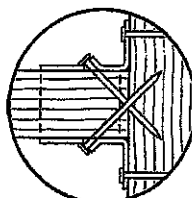
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
								(K _c =1.15)	(K _c =1.00)	(K _c =1.15)	(K _c =1.00)
LUS24	18	1 1/8	3 1/2	1 1/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/2	3 1/2	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/4	1 1/4	3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/2	4 3/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/4	4 3/4	2	3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/4	1 1/4	3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/2	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/4	6 1/4	2	3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/2	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/4	8 3/4	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



DESIGN

These products are engineered for use in accordance with the Canadian Building Code (CSA O86-14) and the International Building Code (IBC). The information is provided for informational purposes only. It is not intended to be used as a substitute for professional engineering or architectural services. The user is responsible for ensuring that the products are used in accordance with the applicable code requirements and the manufacturer's instructions. © 2017 Simpson Strong-Tie Company Inc.

SEE LIST 1-17 150-1719

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

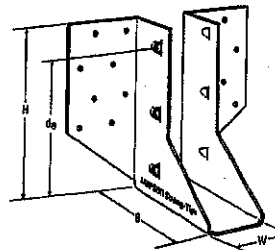
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

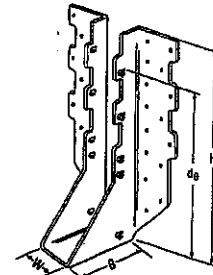
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

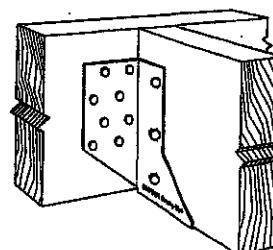
- See current catalogue for options



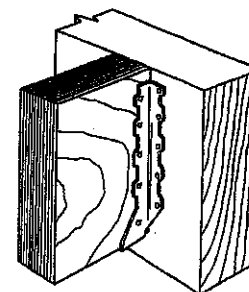
LJS26DS



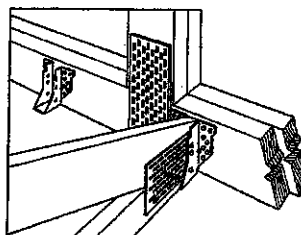
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



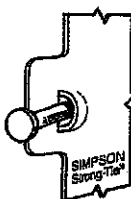
Typical HUS
Installation



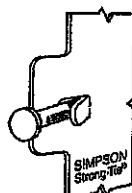
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	18	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/4	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

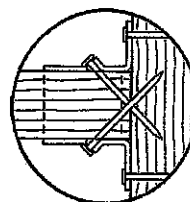
1. d₀ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,560



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



UNITED STATES PATENT AND TRADEMARK OFFICE
REGISTERED DESIGN

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SPS-CHUS7-12/17 Exp. 6/19

(800) 999-5099
strongtie.com

HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

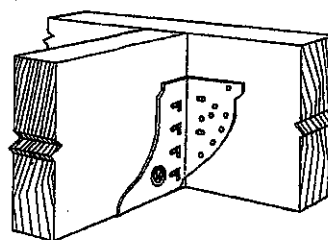
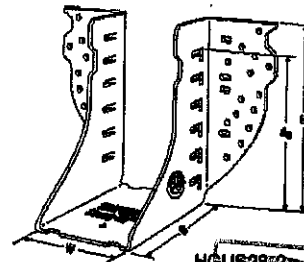
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

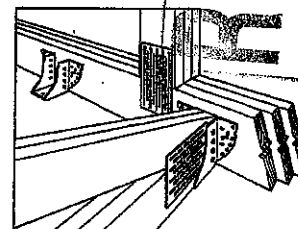
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



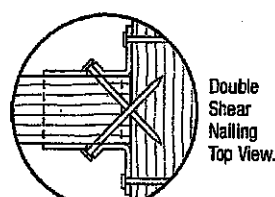
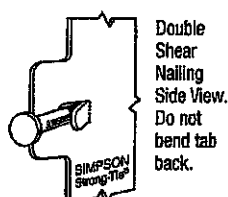
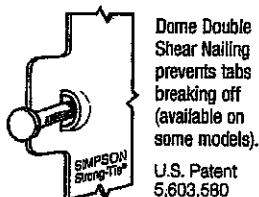
Typical HGUS
Installation



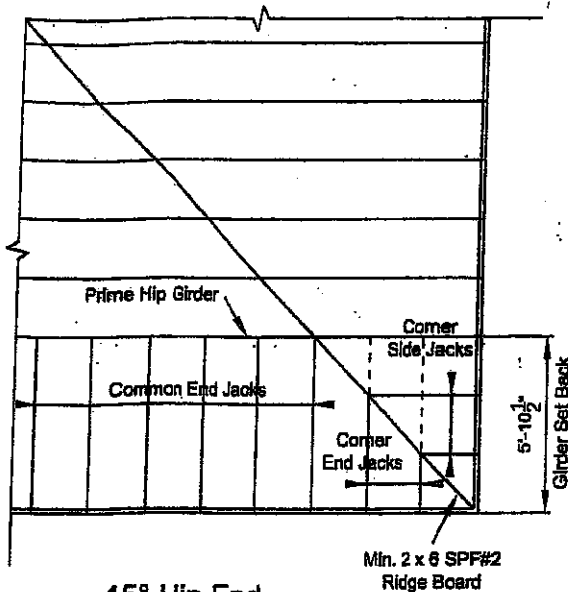
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/2	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11645

1. d_o is the distance from the seat of the hanger to the highest joist nail.



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strongtie.com



45° Hip End

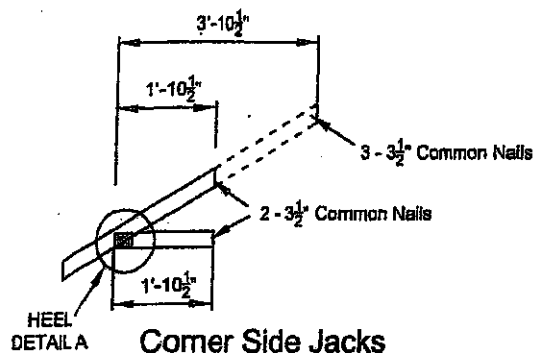
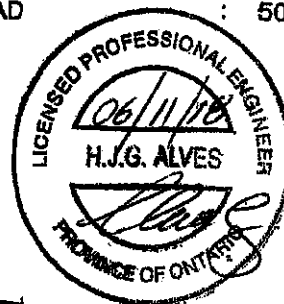
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

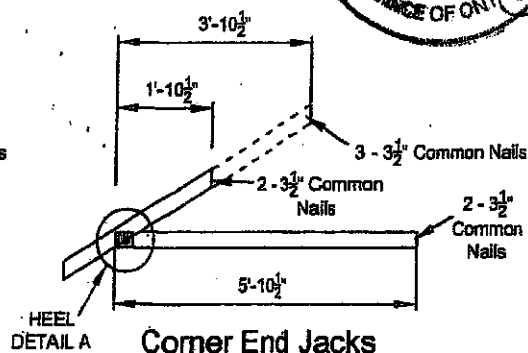
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

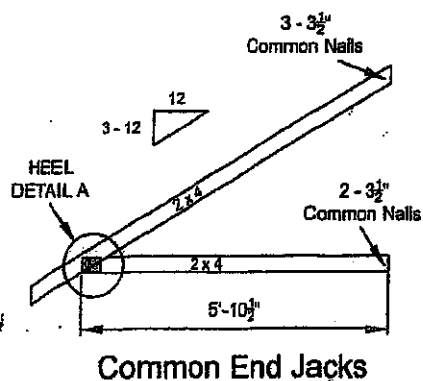
TOTAL LOAD : 50.5 P.S.F



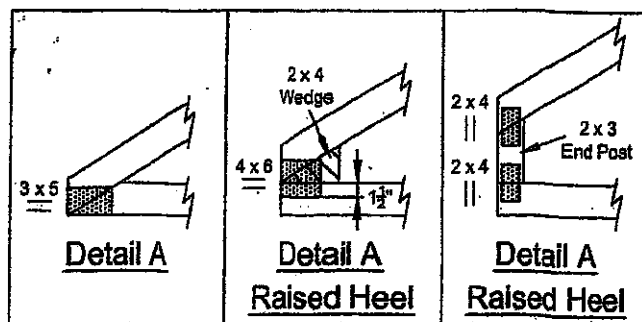
Corner Side Jacks



Corner End Jacks

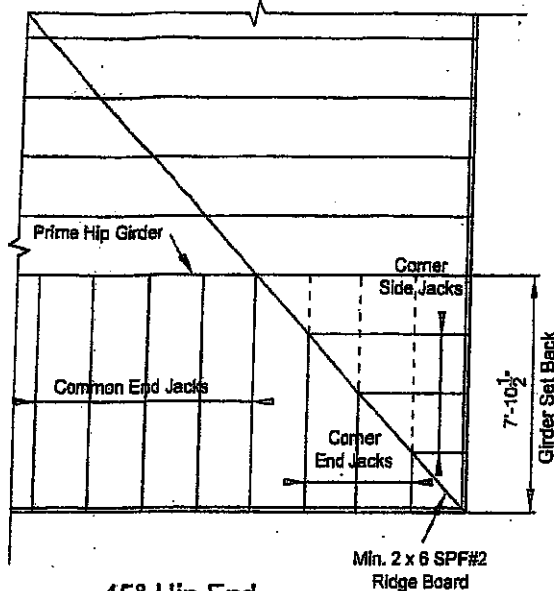


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

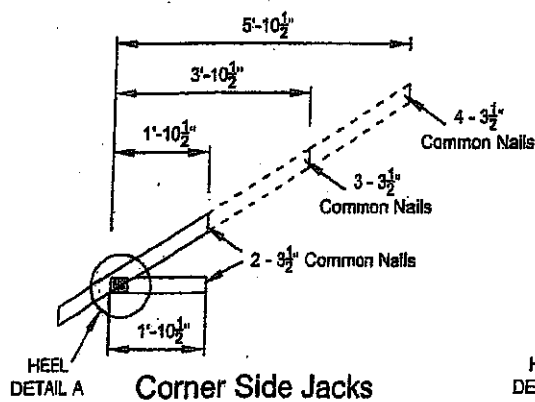
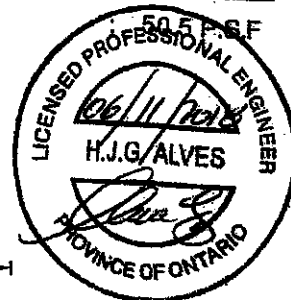
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

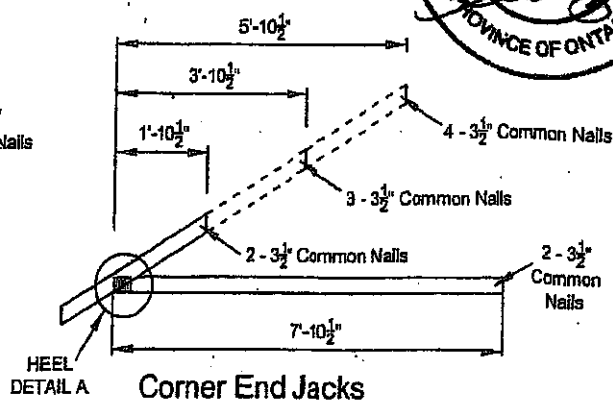
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

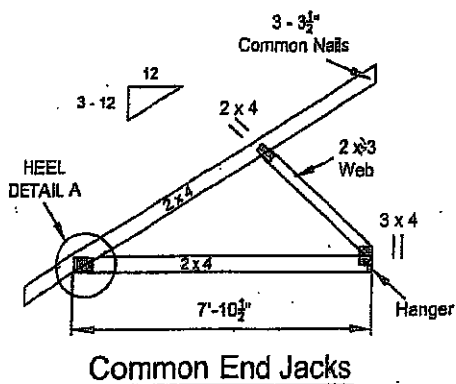
TOTAL LOAD



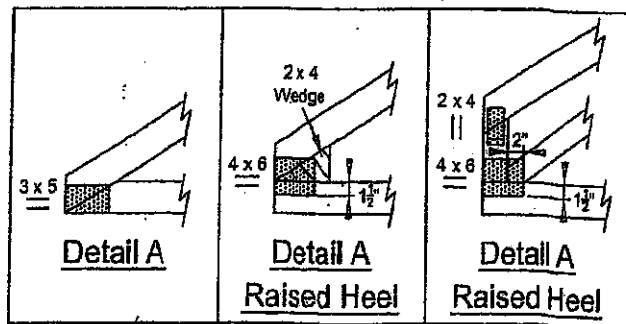
Corner Side Jacks



Corner End Jacks



Common End Jacks

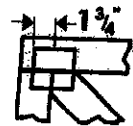


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

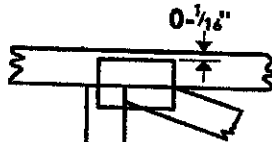
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

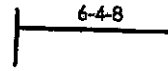


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

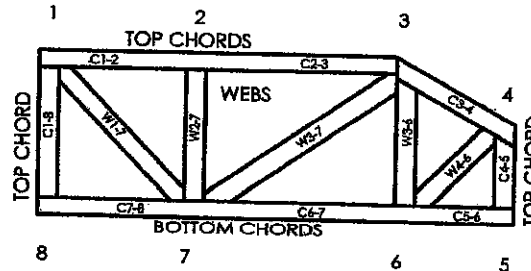
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, may be required. See BCSI.
2. Truss bracing must be designed by a professional engineer. Wide truss spacing, individual lateral bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

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(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300213

Feb 09, 2010

