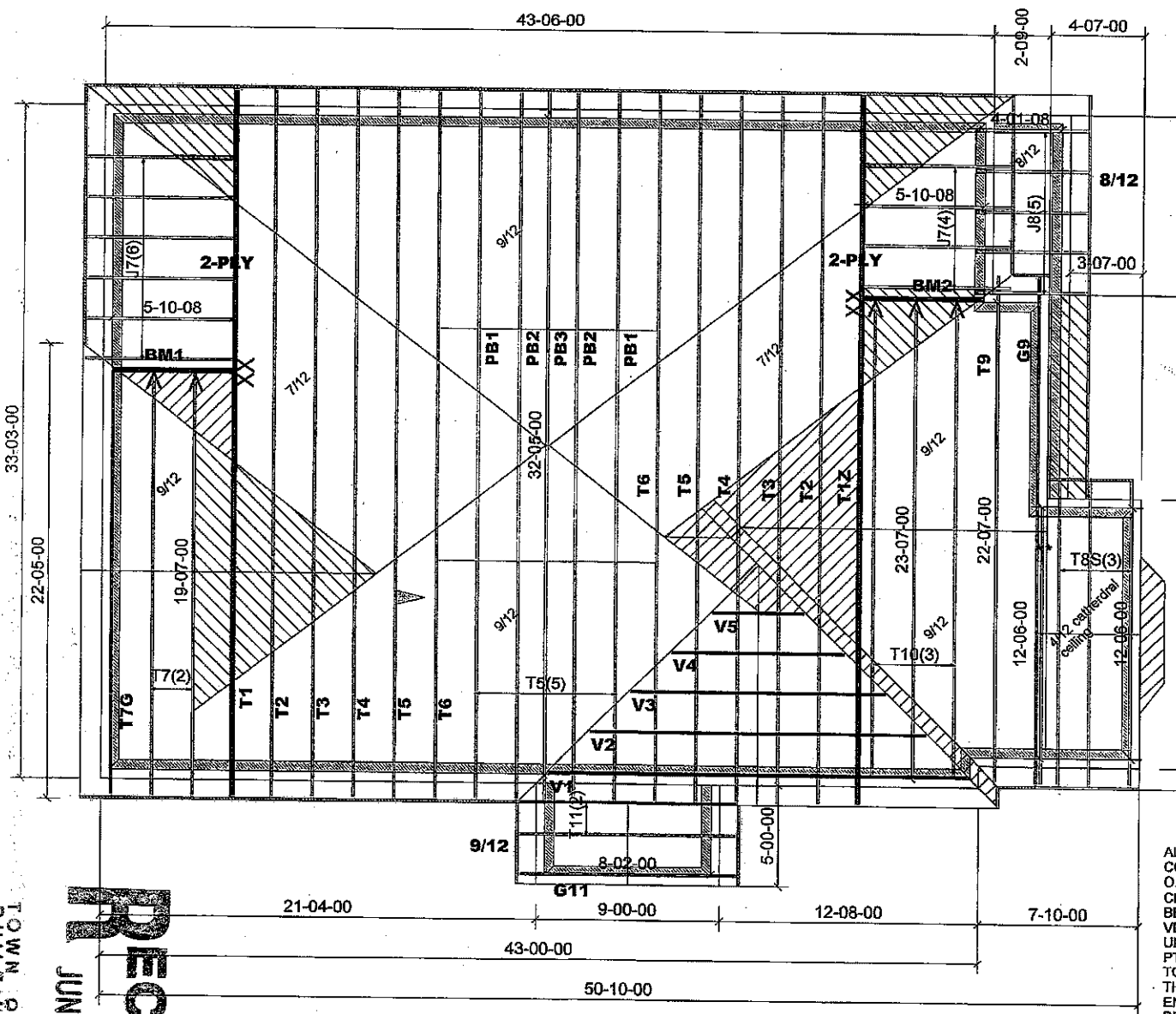


** NAIL 2-2X10 BESIDE G9 FOR CONTINUOUS BEARING SUPPORT

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

GSL 37.6 PSF

DESIGN CONFORMS WITH O.B.C. 2018 PART 9
DESIGN LOADS:

TC LIVE	29 PSF
TC DEAD	6 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

BM1,2= 2- 2x10

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012 ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

CERTIFIED MODEL
PRE-APPROVED
FOR PERMIT APPLICATION AS PER THE ONTARIO BUILDING CODE
TOWN OF CALEDON BUILDING DIVISION
REVIEWED BY J. Swell
DATE July 8/19
FILE # KINMOUNT 11B - ELEV 1/2

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING DIVISION
FILE NO.

APPLICANT COPY

Job Tracking	50033
Plan Log	200666
Layout ID	401827

Builder / Location: **GREEN PARK HOMES / CALEDON**
Project: **LAMBERT LANE PH.2**
Date: 2019-03-21 | Sales: Mario DiCano | Designer: Chris/JG

Model / Elevation: **KINMOUNT 11B / 1**

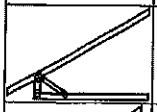
Mitek ver 8.2.3.229

ISSUED FOR BUILDING PERMIT
NO ITEM OF DEFICIENCY
ISSUED FOR CONSTRUCTION

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	GREEN PARK HOMES	PlanLog:	200659
Project:	Lamberts Lane Home Corp.	Layout ID:	401811
Location:	Caledon	Ref #	
Model:	KINMOUNT 11B	Page:	2 of 2
Lot #:		Date:	03/13/2019
Elevation:	1	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	G11 GABLE	9/12	8-02-00	4-07-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	38.15 26.00		
	1	V1 Valley	9/12	20-05-08	7-08-01	2 x 4			78.75 51.33		
	1	V2 Valley	9/12	16-05-08	6-02-01	2 x 4			57.19 35.67		
	1	V3 Valley	9/12	12-05-08	4-08-01	2 x 4			34 21.83		
	1	V4 Valley	9/12	8-05-08	3-02-01	2 x 4			22.51 14.17		
	1	V5 Valley	9/12	4-05-08	1-08-01	2 x 4			10.2 7.33		
	2	PB1 Piggyback	9/12	11-05-09	2-04-00	2 x 4			66.31 43.33		
	2	PB2 Piggyback	9/12	11-05-09	3-06-00	2 x 4			68.93 44.67		
	1	PB3 Piggyback	9/12	11-05-09	4-03-09	2 x 4			30.72 20.00		
	10	J7 Jack-Open	7/12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	186.14 116.67		
	5	J8 Jack-Open	8/12	4-01-08	3-11-09	2 x 4	1-03-08	4-07 3-01-07	64.24 43.33		

TOTAL # TRUSS= **58** TOTAL BFT OF ALL TRUSSES= **3020.49** BFT. TOTAL WEIGHT OF ALL TRSSES **4814.25** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
5	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **7**

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	GREEN PARK HOMES	PlanLog:	200659
Project:	Lamberts Lane Home Corp.	Layout ID:	401811
Location:	Caledon	Ref #	
Model:	KINMOUNT 11B	Page:	1 of 2
Lot #:		Date:	03/13/2019
Elevation:	1	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	9 /12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	1 2-ply	T1Z Hip Girder	9 /12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	2	T2 Hip	9 /12	32-05-00	5-10-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.98 186.00		
	2	T3 Hip	9 /12	32-05-00	7-00-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	286.62 182.33		
	2	T4 Hip	9 /12	32-05-00	8-02-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	303.91 191.33		
	7	T5 Hip	9 /12	32-05-00	9-04-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1116 698.83		
	2	T6 Hip	9 /12	32-05-00	10-06-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	335.48 208.00		
	2	T7 Common	9 /12	19-07-00	8-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	179.67 114.00		
	1	T7G GABLE	9 /12	19-07-00	8-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	96.04 62.50		
	3	T8S Scissor	9 /12 4 /12	12-06-00	6-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	170.08 113.00		
	1	T9 Common	9 /12	22-07-00	9-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	108.62 68.00		
	1	G9 GABLE	9 /12	22-07-00	9-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	114.49 72.33		
	3	T10 Common	9 /12	23-07-00	9-11-14	2 x 4		9-04 1-06-04	321.53 202.50		
	2	T11 Common	9 /12	8-02-00	4-07-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	74.13 48.00		

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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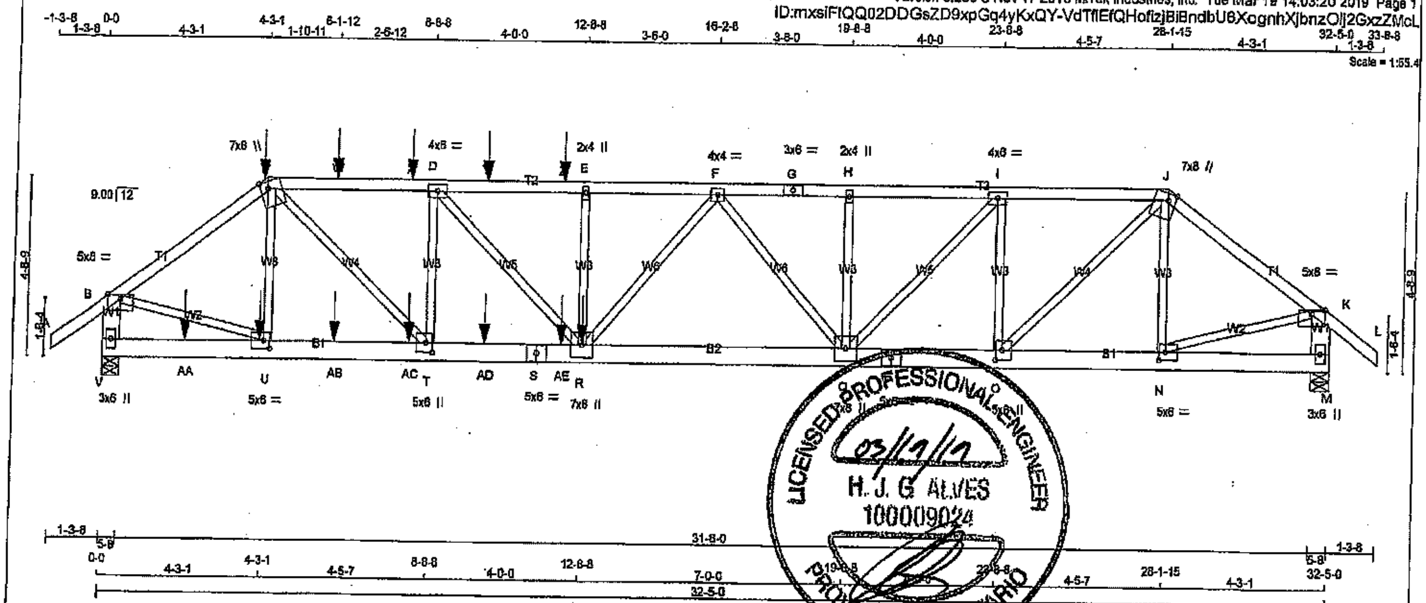


TOTAL WEIGHT = 2 X 158 = 316 lb

(M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
C- G	1	12	SIDE(61.0)
G- J	1	12	TOP
J- L	1	12	TOP
V- B	2	12	TOP
M- K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V- S	2	12	SIDE(183.1)
S- P	2	12	SIDE(183.1)
P- M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
U- C	1	6	SIDE(24.1)
E- R	1	6	SIDE(316.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	6.0	2.50	2.00
O	BMWW+t	MT20	5.0	6.0	3.00	2.00
P	BS-t	MT20	5.0	6.0		
Q	BMWWW+t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	4988	0	4988	0	0	5-8	5-8
M	3567	0	3567	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	3698	2132 / 0	672 / 0	0 / 0	0 / 0	894 / 0	0 / 0
M	2647	1522 / 0	486 / 0	0 / 0	0 / 0	639 / 0	0 / 0

(MAX./MIN. COMPONENT REACTIONS)

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B- C	-5631 / 0	-102.1	-102.1	0.34 (1)	3.85
C- W	-7634 / 0	-102.1	-102.1	0.58 (1)	3.08
W- X	-7634 / 0	-102.1	-102.1	0.58 (1)	3.08
X- D	-7634 / 0	-102.1	-102.1	0.58 (1)	3.08
D- Y	-9279 / 0	-102.1	-102.1	0.68 (1)	2.66
Y- Z	-9279 / 0	-102.1	-102.1	0.68 (1)	2.66
Z- E	-9279 / 0	-102.1	-102.1	0.68 (1)	2.66
E- F	-9279 / 0	-102.1	-102.1	0.41 (1)	2.98
F- G	-7444 / 0	-102.1	-102.1	0.29 (1)	3.43
G- H	-7444 / 0	-102.1	-102.1	0.29 (1)	3.43
H- I	-7444 / 0	-102.1	-102.1	0.37 (1)	3.35
I- J	-5683 / 0	-102.1	-102.1	0.29 (1)	3.68
J- K	-3905 / 0	-102.1	-102.1	0.26 (1)	4.57
K- L	0 / 42	-102.1	-102.1	0.08 (1)	10.00
V- B	-4893 / 0	0.0	0.0	0.18 (1)	6.56
M- K	-3503 / 0	0.0	0.0	0.13 (1)	7.48
V-AA	0 / 0	-38.5	-38.5	0.06 (3)	10.00
AA- U	0 / 0	-38.5	-38.5	0.06 (3)	10.00
U-AB	0 / 4477	-38.5	-38.5	0.33 (1)	10.00
AB-AC	0 / 4477	-38.5	-38.5	0.33 (1)	10.00
AC- T	0 / 4477	-38.5	-38.5	0.33 (1)	10.00
T-AD	0 / 7634	-38.5	-38.5	0.56 (1)	10.00
AD- S	0 / 7634	-38.5	-38.5	0.56 (1)	10.00
S-AE	0 / 7634	-38.6	-38.5	0.56 (1)	10.00
AE- R	0 / 7634	-38.5	-38.5	0.56 (1)	10.00
R- Q	0 / 8508	-38.5	-38.5	0.65 (1)	10.00
Q- P	0 / 5683	-38.5	-38.5	0.40 (1)	10.00
P- O	0 / 5683	-38.5	-38.5	0.40 (1)	10.00
O- N	0 / 3103	-38.5	-38.5	0.23 (1)	10.00
N- M	0 / 0	-38.5	-38.5	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U- C	-760 / 0	0.12 (1)
C- T	0 / 4398	0.54 (1)
T- D	-2753 / 0	0.44 (1)
D- R	0 / 2428	0.30 (1)
R- E	-552 / 0	0.09 (1)
R- F	0 / 1228	0.15 (1)
F- Q	-1695 / 0	0.45 (1)
Q- H	-348 / 0	0.06 (1)
Q- I	0 / 2597	0.32 (1)
O- I	-2386 / 0	0.38 (1)
O- J	0 / 3594	0.44 (1)
N- J	-578 / 0	0.09 (1)
B- U	0 / 4639	0.57 (1)
N- K	0 / 3217	0.40 (1)

DWG NO. TAM T1905690
STRUCTURAL
COMPONENT ONLY
1/2

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-3-1	-53	-59	—	FRONT	VERT	DEAD	—	—
C	4-3-1	-205	-205	—	FRONT	VERT	TOTAL	—	—
C	4-3-1	-303	-303	—	FRONT	VERT	SNOW	—	—
R	12-8-8	-1917	-1917	—	FRONT	VERT	TOTAL	—	—
U	4-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.68/1.00 (D-E:1) , BC=0.65/1.00 (Q-R:1) , WB=0.57/1.00 (B-U:1) , SSI=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.72 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:mxsIFIQQ02DDGsZD9xpGq4yKxQY-VdTfIEfQHofizjBIBndbU6XqgnhXjbnzOjj2GxzZMcL

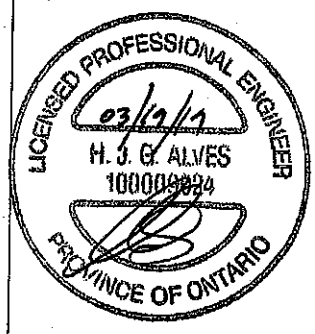
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW+t	MT20	7.0	8.0		
S	BS-t	MT20	5.0	6.0		
T	BMWW+t	MT20	5.0	6.0	3.00	2.00
U	BMWW-t	MT20	5.0	6.0	2.50	2.00
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	6-1-12	-198	-198	—	FRONT	VERT	TOTAL	—	—
X	8-1-12	-198	-198	—	FRONT	VERT	TOTAL	—	—
Y	10-1-12	-198	-198	—	FRONT	VERT	TOTAL	—	—
Z	12-1-12	-198	-198	—	FRONT	VERT	TOTAL	—	—
AA	2-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
AB	6-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
AC	8-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
AD	10-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—
AE	12-1-12	-75	-95	—	FRONT	VERT	TOTAL	—	—



DWG NO. TAM 77905890
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T1Z	1	2	GREEN PARK HOMES TRUSS DESC.	T1Z

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 19 14:03:22 2019 Page 1

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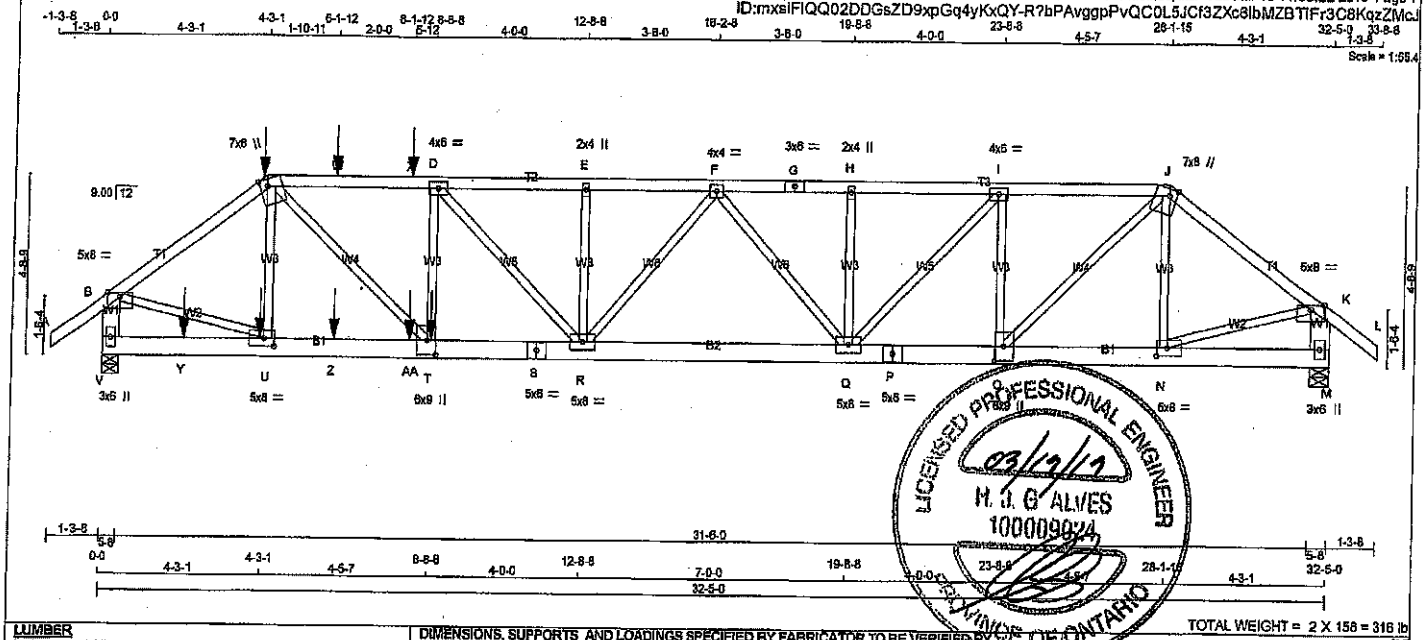

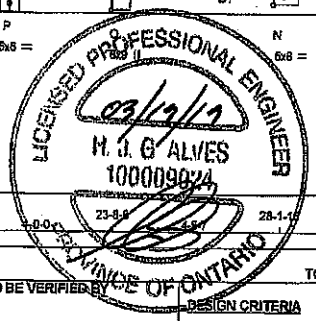

TOTAL WEIGHT = 2 X 156 = 316 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x8	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
C-G	1	12	SIDE(61.0)
G-J	1	12	TOP
J-L	1	12	TOP
V-B	2	12	TOP
M-K	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
V-S	2	12	SIDE(183.1)
S-P	2	12	TOP
P-M	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
D-T	1	3	SIDE(631.1)
I-O	1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMW+w	MT20	2.0	4.0		
I	TMWW-t	MT20	4.0	6.0		
J	TTWW+m	MT20	7.0	8.0	Edge	2.50
K	TMVW-p	MT20	5.0	8.0	Edge	
M	BMV1+p	MT20	3.0	6.0		
N	BMWW-t	MT20	5.0	8.0	2.50	3.25
O	BMWW+t	MT20	6.0	9.0	4.50	2.75
P	BS-t	MT20	5.0	6.0		
Q	BMWWW-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
V	5748	0	5748	0	0	5-8	5-8
M	3467	0	3467	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
V	4268	2442 / 0	790 / 0	0 / 0	0 / 0	1035 / 0	0 / 0
M	2576	1469 / 0	481 / 0	0 / 0	0 / 0	625 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	LC1 TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B-C	-6591 / 0	-102.1	-102.1	0.40 (1)	3.53
C-W	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69
W-X	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69
X-D	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69
D-E	-8837 / 0	-102.1	-102.1	0.55 (1)	2.89
E-F	-8837 / 0	-102.1	-102.1	0.39 (1)	3.07
F-G	-7142 / 0	-102.1	-102.1	0.28 (1)	3.50
G-H	-7142 / 0	-102.1	-102.1	0.28 (1)	3.50
H-I	-7142 / 0	-102.1	-102.1	0.36 (1)	3.43
I-J	-5484 / 0	-102.1	-102.1	0.28 (1)	3.95
J-K	-3779 / 0	-102.1	-102.1	0.26 (1)	4.63
K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00
V-B	-5666 / 0	0.0	0.0	0.21 (1)	6.16
M-K	-3402 / 0	0.0	0.0	0.12 (1)	7.55
V-Y	0 / 0	-38.5	-38.5	0.08 (2)	10.00
Y-U	0 / 0	-38.5	-38.5	0.08 (2)	10.00
U-Z	0 / 5241	-38.5	-38.5	0.44 (1)	10.00
Z-AA	0 / 5241	-38.5	-38.5	0.44 (1)	10.00
AA-T	0 / 5241	-38.5	-38.5	0.44 (1)	10.00
T-S	0 / 9079	-38.5	-38.5	0.68 (1)	10.00
S-R	0 / 9079	-38.5	-38.5	0.68 (1)	10.00
R-Q	0 / 8147	-38.5	-38.5	0.61 (1)	10.00
Q-P	0 / 5484	-38.5	-38.5	0.39 (1)	10.00
P-O	0 / 5484	-38.5	-38.5	0.39 (1)	10.00
O-N	0 / 3779	-38.5	-38.5	0.23 (1)	10.00
N-M	0 / 0	-38.5	-38.5	0.04 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
U-C	-872 / 0	0.14 (1)
C-T	0 / 5347	0.68 (1)
T-D	-510 / 0	0.06 (1)
D-R	-356 / 0	0.11 (1)
R-E	-301 / 0	0.05 (1)
R-F	0 / 1099	0.14 (1)
F-Q	-1500 / 0	0.42 (1)
Q-H	-343 / 0	0.05 (1)
Q-I	0 / 2445	0.30 (1)
O-I	-2275 / 0	0.36 (1)
O-J	0 / 3455	0.43 (1)
N-J	-557 / 0	0.09 (1)
B-U	0 / 5430	0.67 (1)
N-K	0 / 3113	0.39 (1)

FACTORED CONCENTRATED LOADS(LBS)

JT	LOC.	LC1	MAX+	SURFACE	DIR.	TYPE	HEEL	CONN.
C	4-3-1	-53	-53	—	FRONT	VERT	DEAD	—	—
C	4-3-1	-205	-205	—	BACK	VERT	TOTAL	—	—
C	4-3-1	-303	-303	—	FRONT	VERT	SNOW	—	—
T	8-6-8	-3121	-3121	—	BACK	VERT	TOTAL	—	—
U	4-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—
W	6-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—
X	8-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—
Y	2-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—
Z	6-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL =	29.0	PSF
		DL =	6.0	PSF
BOT	CH.	LL =	10.5	PSF
		DL =	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.21")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.35")

CSI: TC=0.71/1.00 (C-D:1) , BC=0.68/1.00 (R-T:1) , WB=0.67/1.00 (B-U:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (B) (INPUT = 0.90)
JSI METAL= 0.81 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

DWG NO. TAM T19056 R1 STRUCTURAL COMPONENT ONLY 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T1Z	1	2	TRUSS DESC.	T1Z

Tamarack Roof Truss, Burlington

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PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWW+t	MT20	6.0	9.0	4.50	2.75
U	BMWW-t	MT20	5.0	8.0	2.50	3.25
V	BMV1+p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	8-1-12	-75	-95	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100019624
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 7905691
STRUCTURAL
COMPONENT ONLY
2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T2	2	1	TRUSS DESC.	**T2**

Tamarack Roof Truss, Burlington

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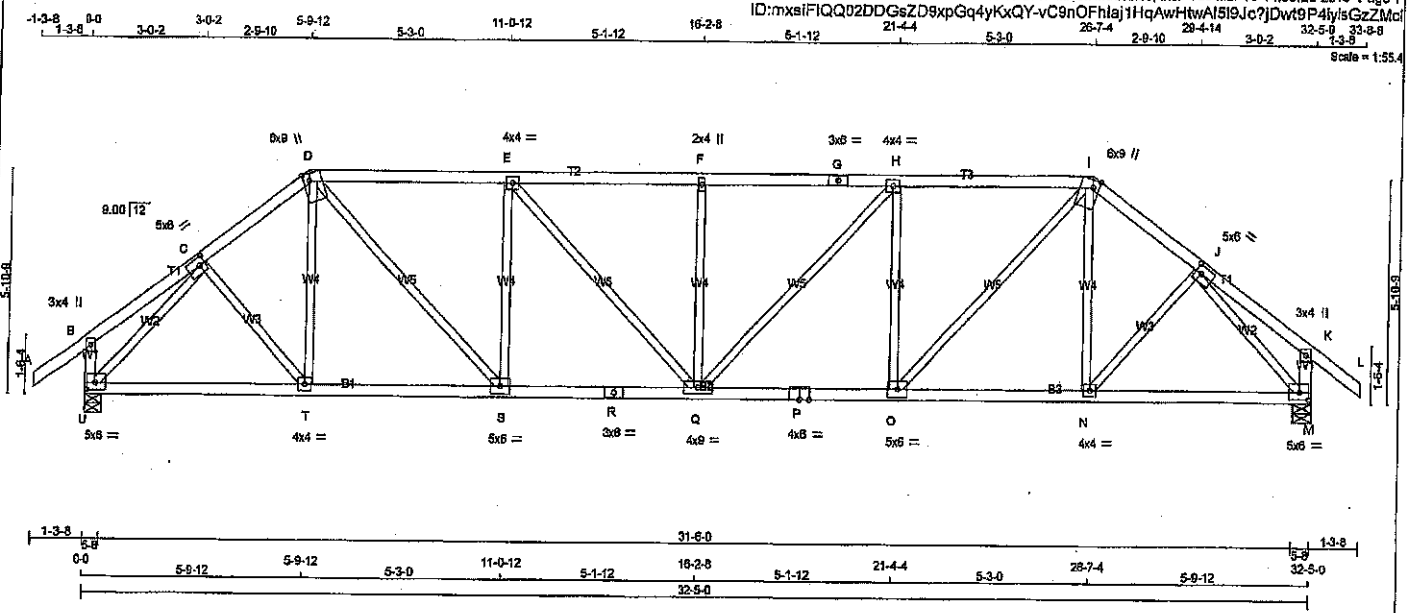


TOTAL WEIGHT = 2 X 142 = 285 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in Inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.25	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2420	0	2420	0	0	5-8	5-8
M	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
U	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
M	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	C-T	0 / 301	0.07 (1)	
B-C	0 / 15	-102.1	-102.1	0.10 (1)	10.00	T-D	0 / 153	0.04 (3)	
C-D	-2492 / 0	-102.1	-102.1	0.18 (1)	4.22	D-S	0 / 1542	0.35 (1)	
D-E	-3033 / 0	-102.1	-102.1	0.60 (1)	3.44	S-E	-938 / 0	0.50 (1)	
E-F	-3349 / 0	-102.1	-102.1	0.64 (1)	3.25	E-Q	0 / 466	0.10 (1)	
F-G	-3349 / 0	-102.1	-102.1	0.64 (1)	3.25	Q-F	-484 / 0	0.26 (1)	
G-H	-3349 / 0	-102.1	-102.1	0.64 (1)	3.25	Q-H	0 / 466	0.10 (1)	
H-I	-3033 / 0	-102.1	-102.1	0.60 (1)	3.44	C-H	-936 / 0	0.50 (1)	
I-J	-2492 / 0	-102.1	-102.1	0.18 (1)	4.22	O-I	0 / 1542	0.35 (1)	
J-K	0 / 15	-102.1	-102.1	0.10 (1)	10.00	N-I	0 / 153	0.04 (3)	
K-L	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-J	0 / 301	0.07 (1)	
U-B	-263 / 0	0.0	0.0	0.03 (1)	7.81	U-C	-2722 / 0	0.85 (1)	
M-K	-263 / 0	0.0	0.0	0.03 (1)	7.81	J-M	-2722 / 0	0.85 (1)	
U-T	0 / 1773	-38.5	-38.5	0.46 (2)	10.00				
T-S	0 / 1976	-38.5	-38.5	0.50 (2)	10.00				
S-R	0 / 3033	-38.5	-38.5	0.59 (1)	10.00				
R-Q	0 / 3033	-38.5	-38.5	0.59 (1)	10.00				
Q-P	0 / 3033	-38.5	-38.5	0.59 (1)	10.00				
P-O	0 / 3033	-38.5	-38.5	0.59 (1)	10.00				
O-N	0 / 1976	-38.5	-38.5	0.50 (2)	10.00				
N-M	0 / 1773	-38.5	-38.5	0.46 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.64/1.00 (E-F:1) , BC=0.59/1.00 (O-Q:1) , WB=0.85/1.00 (C-U:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

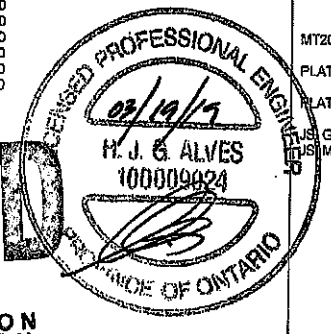
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1957	1673

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LS: GRIP= 0.90 (M) (INPUT = 0.90)
LS: METAL= 1.00 (R) (INPUT = 1.00)



DWG NO. TAM T190 5692
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T3	2	1	TRUSS DESC.	T3

Tamarack Roof Truss, Burlington

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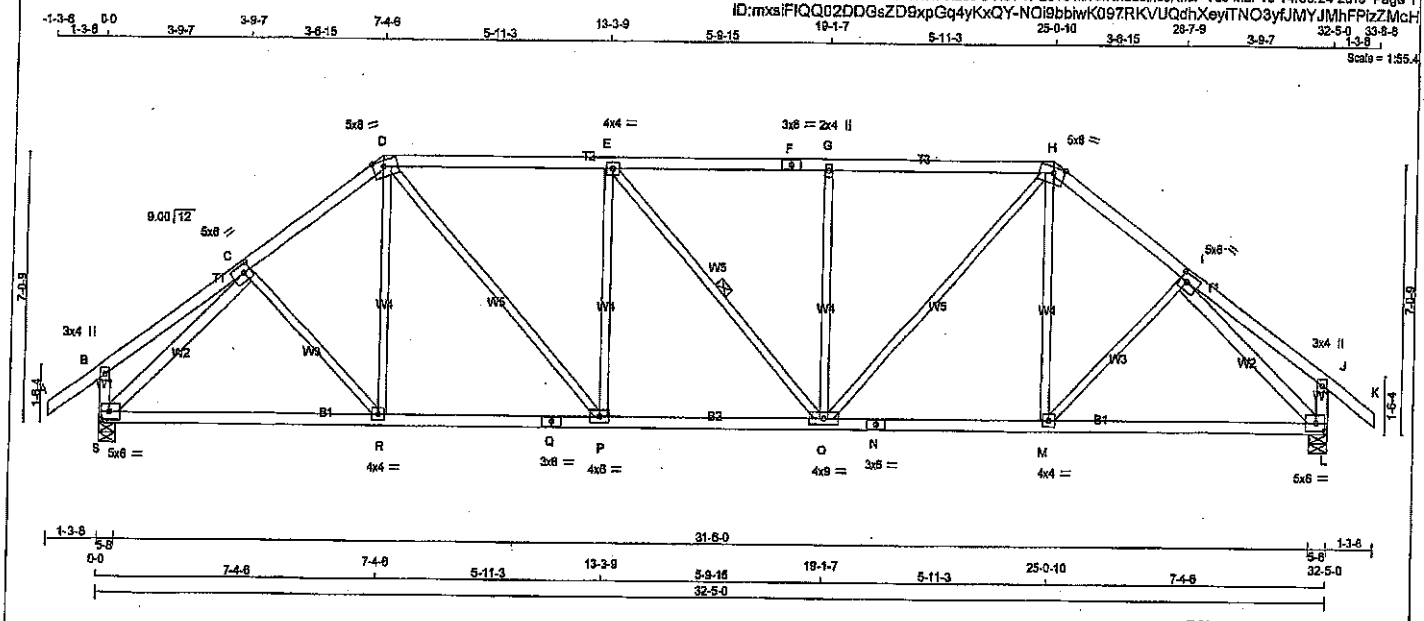

TOTAL WEIGHT = 2 X 147 = 293 lb (M)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.25
D	TTWW-m	MT20	6.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	8.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMWW1-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	4.0	9.0		
P	BMWW-t	MT20	4.0	8.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMWW1-t	MT20	5.0	8.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2420	0	2420	0	0	5-8	5-8
L	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE — MAX./MIN. COMPONENT REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
L	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B- C	0 / 24	-102.1	-102.1	0.20 (1)	10.00	
C- D	-2471 / 0	-102.1	-102.1	0.30 (1)	4.11	
D- E	-2709 / 0	-102.1	-102.1	0.70 (1)	3.50	
E- F	-2707 / 0	-102.1	-102.1	0.69 (1)	3.50	
F- G	-2707 / 0	-102.1	-102.1	0.69 (1)	3.50	
G- H	-2707 / 0	-102.1	-102.1	0.69 (1)	3.52	
H- I	-2471 / 0	-102.1	-102.1	0.30 (1)	4.11	
I- J	0 / 24	-102.1	-102.1	0.20 (1)	10.00	
J- K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
S- B	-287 / 0	0.0	0.0	0.03 (1)	7.81	
L- J	-287 / 0	0.0	0.0	0.03 (1)	7.81	
S- R	0 / 1872	-38.5	-38.5	0.60 (2)	10.00	
R- Q	0 / 1957	-38.5	-38.5	0.62 (2)	10.00	
Q- P	0 / 1957	-38.5	-38.5	0.62 (2)	10.00	
P- O	0 / 2709	-38.5	-38.5	0.55 (1)	10.00	
O- N	0 / 1957	-38.5	-38.5	0.62 (2)	10.00	
N- M	0 / 1957	-38.5	-38.5	0.62 (2)	10.00	
M- L	0 / 1872	-38.5	-38.5	0.60 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- R	0 / 157	0.04 (3)
R- D	0 / 274	0.06 (3)
D- P	0 / 1139	0.26 (1)
P- E	-649 / 0	0.55 (1)
E- O	-2 / 0	0.00 (1)
O- G	-648 / 0	0.55 (1)
O- H	0 / 1136	0.26 (1)
M- H	0 / 275	0.06 (3)
M- I	0 / 157	0.04 (3)
S- C	-2750 / 0	0.92 (1)
I- L	-2750 / 0	0.92 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.05")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.24")

CSI: TC=0.70/1.00 (D-E:1) , BC=0.62/1.00 (M-O:2) , WB=0.92/1.00 (I-L:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

WSI GRIP= 0.87 (I) (INPUT = 0.90)
WSI METAL= 0.62 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T1905893
STRUCTURAL
COMPONENT ONLY

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES before use. Design for stability, storage, construction and bracing are the responsibility of the fabricator. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive; Madison, WI 53719.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T4	2	1	TRUSS DESC.	T4

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 19 14:03:25 2019 Page 1
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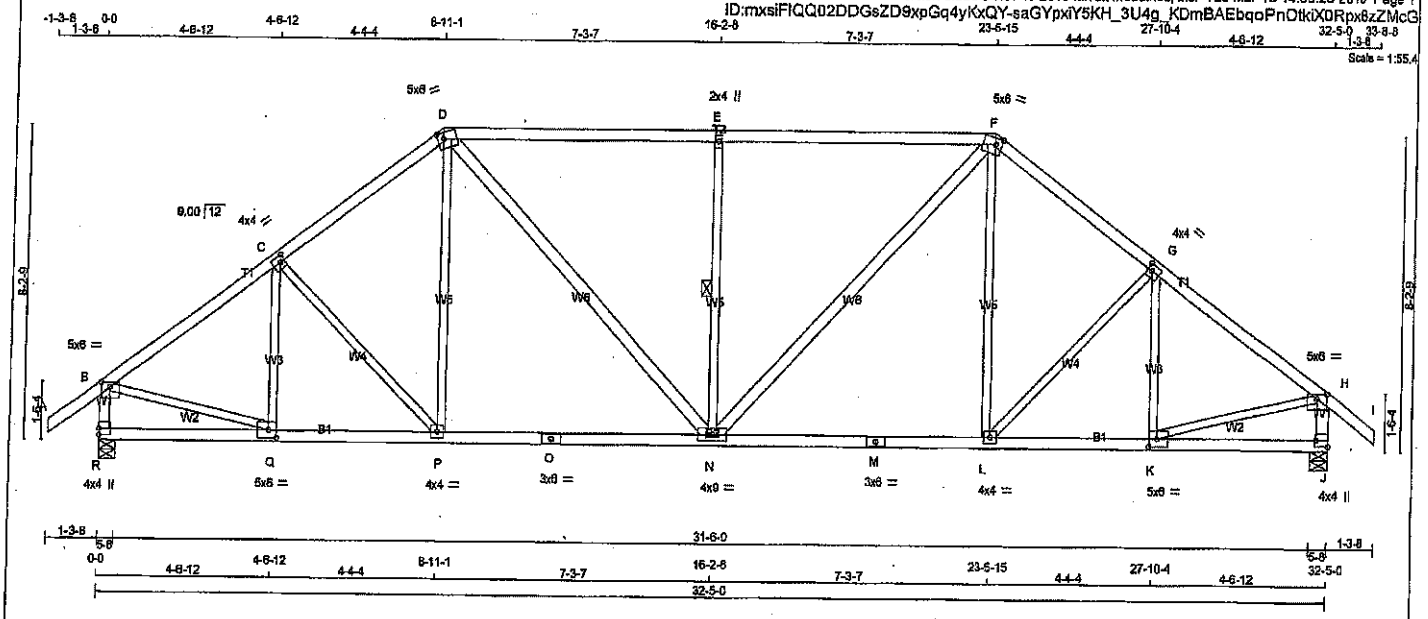

TOTAL WEIGHT = 2 X 152 = 304 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	2.00	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	2.00	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMWW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.50
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2420	0	2420	0	0	5-8	5-8
J	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
J	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2465 / 0	-102.1	-102.1	0.35 (1)	4.10	
C-D	-2393 / 0	-102.1	-102.1	0.33 (1)	4.16	
D-E	-2437 / 0	-102.1	-102.1	0.85 (1)	3.26	
E-F	-2437 / 0	-102.1	-102.1	0.85 (1)	3.26	
F-G	-2393 / 0	-102.1	-102.1	0.33 (1)	4.16	
G-H	-2465 / 0	-102.1	-102.1	0.35 (1)	4.10	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
R-B	-2342 / 0	0.0	0.0	0.24 (1)	5.58	
J-H	-2342 / 0	0.0	0.0	0.24 (1)	5.58	
R-Q	0 / 0	-38.5	-38.5	0.13 (3)	10.00	
Q-P	0 / 1997	-38.5	-38.5	0.48 (2)	10.00	
P-O	0 / 1881	-38.5	-38.5	0.58 (2)	10.00	
O-N	0 / 1881	-38.5	-38.5	0.58 (2)	10.00	
N-M	0 / 1881	-38.5	-38.5	0.58 (2)	10.00	
M-L	0 / 1881	-38.5	-38.5	0.58 (2)	10.00	
L-K	0 / 1997	-38.5	-38.5	0.48 (2)	10.00	
K-J	0 / 0	-38.5	-38.5	0.13 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-355 / 0	0.13 (1)
C-P	-173 / 0	0.13 (1)
P-D	0 / 392	0.09 (2)
D-N	0 / 821	0.13 (1)
N-E	-914 / 0	0.39 (1)
N-F	0 / 821	0.13 (1)
L-F	0 / 392	0.09 (2)
L-G	-173 / 0	0.13 (1)
K-G	-355 / 0	0.13 (1)
B-Q	0 / 2064	0.46 (1)
K-H	0 / 2064	0.46 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.06")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.06")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.85/1.00 (E-F:1) , BC=0.58/1.00 (L-N:2) , WB=0.46/1.00 (H-K:1) , SSI=0.36/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	786	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (H) (INPUT = 0.90)
SSI METAL= 0.78 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009823
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 7905894
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T5	7	1	TRUSS DESC.	**T5**

Tamarack Roof Truss, Burlington

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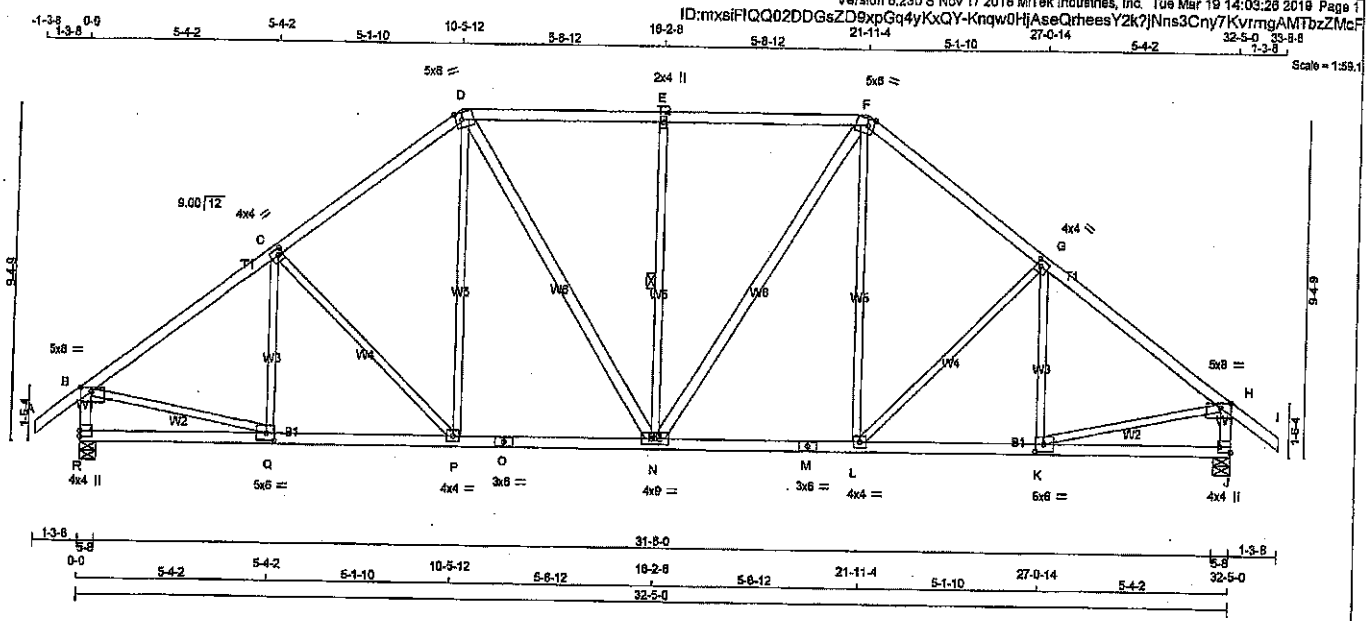

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.50
J	BMV1+p	MT20	4.0	4.0	2.00	Edge
K	BMWW-t	MT20	5.0	6.0	2.50	2.75
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	4.0	4.0		
Q	BMWW-t	MT20	5.0	6.0	2.50	2.75
R	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	2420	0	2420	0	0	5-8	5-8
J	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
J	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2509 / 0	-102.1	-102.1	0.48 (1)	3.92	
C-D	-2277 / 0	-102.1	-102.1	0.45 (1)	4.11	
D-E	-2088 / 0	-102.1	-102.1	0.49 (1)	4.16	
E-F	-2088 / 0	-102.1	-102.1	0.49 (1)	4.16	
F-G	-2277 / 0	-102.1	-102.1	0.45 (1)	4.11	
G-H	-2509 / 0	-102.1	-102.1	0.48 (1)	3.92	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
R-B	-2334 / 0	0.0	0.0	0.24 (1)	5.58	
J-H	-2334 / 0	0.0	0.0	0.24 (1)	5.58	
R-Q	0 / 0	-38.5	-38.5	0.19 (3)	10.00	
Q-P	0 / 2037	-38.5	-38.5	0.46 (2)	10.00	
P-O	0 / 1792	-38.5	-38.5	0.43 (2)	10.00	
O-N	0 / 1792	-38.5	-38.5	0.43 (2)	10.00	
N-M	0 / 1792	-38.5	-38.5	0.43 (2)	10.00	
M-L	0 / 1792	-38.5	-38.5	0.43 (2)	10.00	
L-K	0 / 2037	-38.5	-38.5	0.46 (2)	10.00	
K-J	0 / 0	-38.5	-38.5	0.19 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-233 / 82	0.11 (1)
C-P	-352 / 0	0.38 (1)
P-D	0 / 469	0.11 (2)
D-N	0 / 553	0.09 (1)
N-E	-713 / 0	0.41 (1)
N-F	0 / 553	0.09 (1)
L-F	0 / 469	0.11 (2)
L-G	-352 / 0	0.38 (1)
K-G	-233 / 82	0.11 (1)
B-Q	0 / 2087	0.47 (1)
K-H	0 / 2087	0.47 (1)

TOTAL WEIGHT = 7 X 159 = 1115 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 699 (0.16")

CSI: TC=0.49/1.00 (D-E:1) , BC=0.46/1.00 (E-L:2) , WB=0.47/1.00 (H-K:1) , SSI=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (Q) (INPUT = 0.90)
JSI METAL= 0.55 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100089073
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T1905695
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T6	2	1	GREEN PARK HOMES TRUSS DESC.	T6

Tamarack Roof Truss, Burlington

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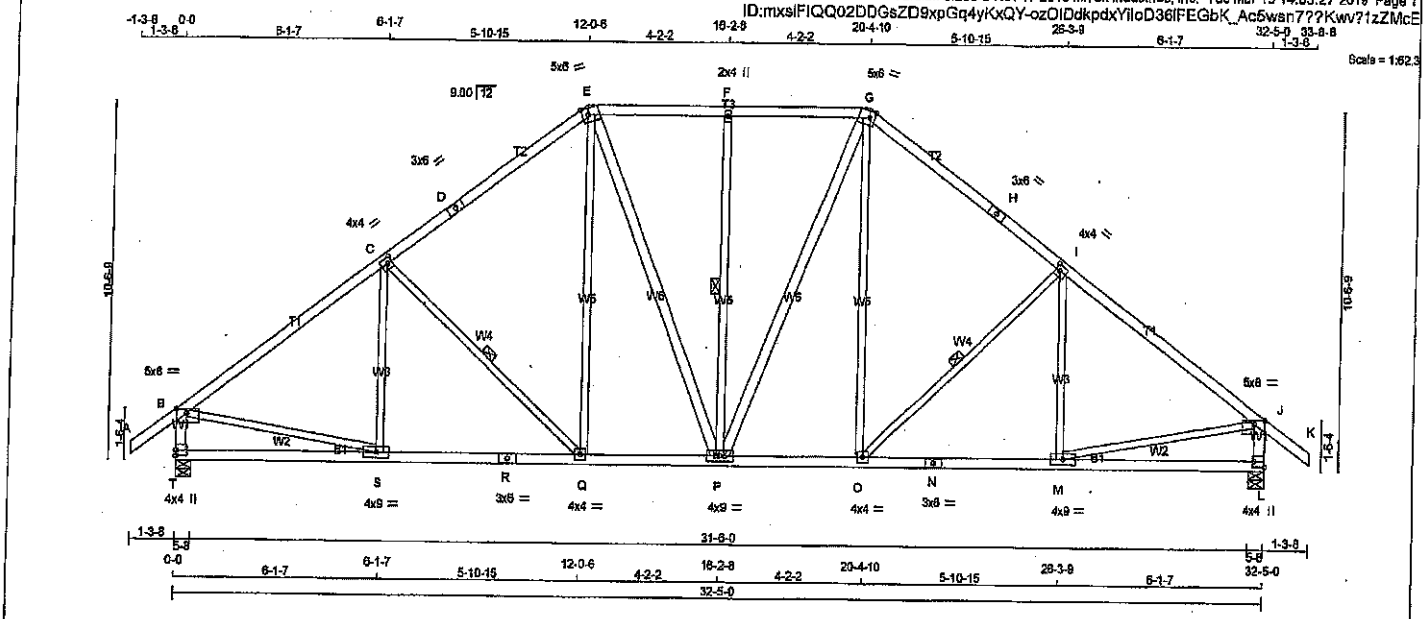

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	1.50	3.50
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	Edge	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1+p	MT20	4.0	4.0	2.00	Edge
M	BMWW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	9.0		
T	BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2420	0	2420	0	0	5-8	5-8
L	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
L	1800	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2528 / 0	-102.1	-102.1	0.66 (1)	3.70	
C-D	-2158 / 0	-102.1	-102.1	0.59 (1)	4.02	
D-E	-2158 / 0	-102.1	-102.1	0.59 (1)	4.02	
E-F	-1825 / 0	-102.1	-102.1	0.26 (1)	4.72	
F-G	-1825 / 0	-102.1	-102.1	0.26 (1)	4.72	
G-H	-2158 / 0	-102.1	-102.1	0.59 (1)	4.02	
H-I	-2158 / 0	-102.1	-102.1	0.59 (1)	4.02	
I-J	-2528 / 0	-102.1	-102.1	0.66 (1)	3.70	
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
T-B	-2324 / 0	0.0	0.0	0.24 (1)	5.60	
L-J	-2324 / 0	0.0	0.0	0.24 (1)	5.60	
T-S	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
S-R	0 / 2058	-38.5	-38.5	0.54 (2)	10.00	
R-Q	0 / 2058	-38.5	-38.5	0.54 (2)	10.00	
Q-P	0 / 1693	-38.5	-38.5	0.38 (1)	10.00	
P-O	0 / 1693	-38.5	-38.5	0.38 (1)	10.00	
O-N	0 / 2058	-38.5	-38.5	0.54 (2)	10.00	
N-M	0 / 2058	-38.5	-38.5	0.54 (2)	10.00	
M-L	0 / 0	-38.5	-38.5	0.27 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S-C	-136 / 161	0.08 (1)
C-Q	-513 / 0	0.24 (1)
Q-E	0 / 550	0.12 (2)
E-P	0 / 349	0.06 (1)
P-F	-509 / 0	0.40 (1)
P-G	0 / 349	0.06 (1)
O-G	0 / 550	0.12 (2)
O-I	-513 / 0	0.24 (1)
M-I	-136 / 161	0.08 (1)
B-S	0 / 2096	0.47 (1)
M-J	0 / 2096	0.47 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.68/1.00 (B-C:1) , BC=0.54/1.00 (M-O:2) , WB=0.47/1.00 (J-M:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR.= 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.89 (S) (INPUT = 0.90)
SI METAL= 0.74 (R) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/12/19
H.J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905696
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T7	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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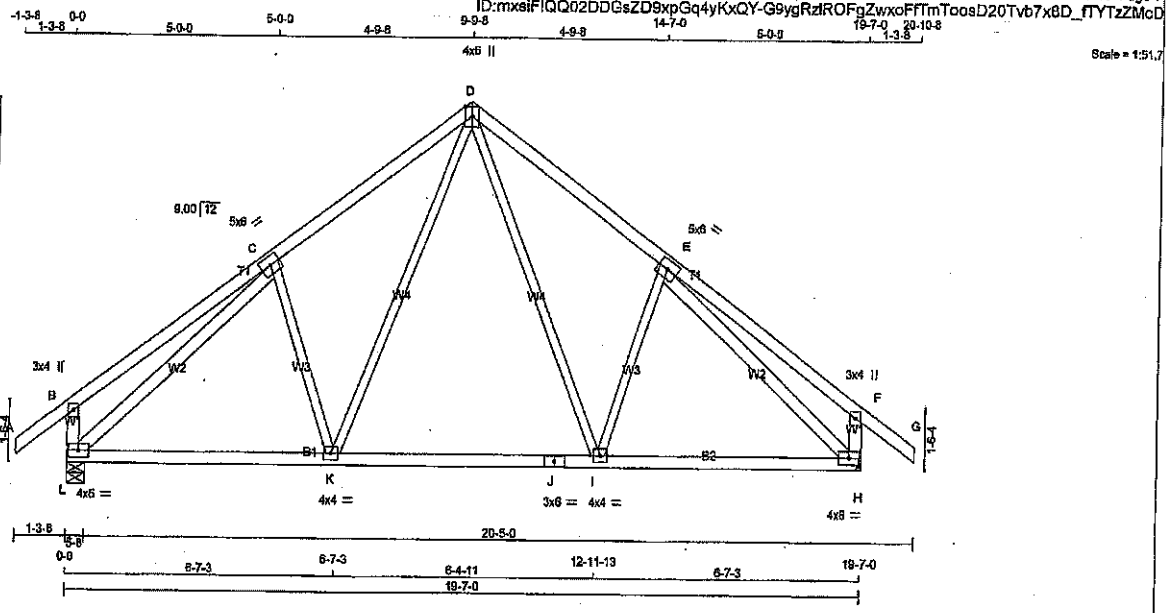

TOTAL WEIGHT = 2 X 94 = 188 lb (M)(F)

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

THIS TRUSS MUST BE INSPECTED BY THE BUILDING DESIGNER PRIOR TO INSTALLATION. INSTALLATION, BRACING AND ATTACHMENT OF THIS TRUSS MUST COMPLY WITH THE ONTARIO BUILDING CODE

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1518	0	1518	0	0	IN-SX	IN-SX
H	1518	0	1518	0	0	5-8 MECHANICAL	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1125	649 / 0	206 / 0	0 / 0	0 / 0	271 / 0	0 / 0
H	1125	649 / 0	206 / 0	0 / 0	0 / 0	271 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	1	0.14	(1)	10.00
B-C	0 / 36	-102.1	-102.1	1	0.40	(1)	10.00
C-D	-1277 / 0	-102.1	-102.1	1	0.33	(1)	5.32
D-E	-1277 / 0	-102.1	-102.1	1	0.33	(1)	5.32
E-F	0 / 36	-102.1	-102.1	1	0.40	(1)	10.00
F-G	0 / 42	-102.1	-102.1	1	0.14	(1)	10.00
L-B	-328 / 0	0.0	0.0	1	0.03	(1)	7.81
H-F	-328 / 0	0.0	0.0	1	0.03	(1)	7.81
L-K	0 / 1091	-38.5	-38.5	2	0.42	(2)	10.00
K-J	0 / 784	-38.5	-38.5	2	0.38	(2)	10.00
J-I	0 / 784	-38.5	-38.5	2	0.38	(2)	10.00
I-H	0 / 1091	-38.5	-38.5	2	0.42	(2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
D-I	0 / 589	0.13	(1)
I-E	-293 / 23	0.13	(1)
K-D	0 / 589	0.13	(1)
C-K	-293 / 23	0.13	(1)
L-C	-1540 / 0	0.95	(1)
E-H	-1540 / 0	0.95	(1)

LICENSED PROFESSIONAL ENGINEER
02/12/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.65")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.65")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.42/1.00 (H-I:2) , WB=0.65/1.00 (C-L:1) , SSI=0.19/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.88 (I) (INPUT = 0.90)
LS METAL= 0.37 (E) (INPUT = 1.00)

DWG NO. TAM T7905897
STRUCTURAL COMPONENT FAMILY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T7G	1	1	TRUSS DESC.	T7G

Tamarack Roof Truss, Burlington

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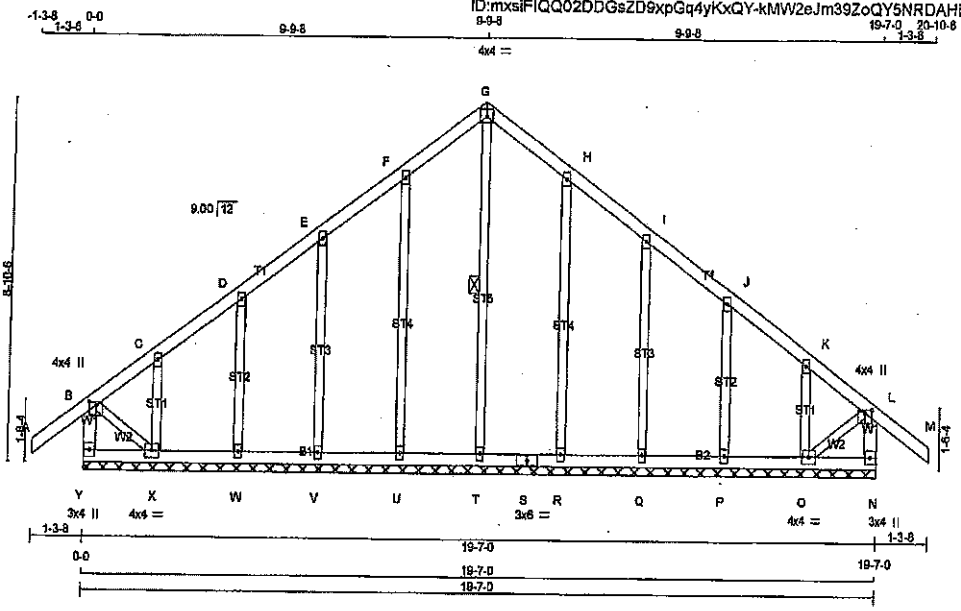

TOTAL WEIGHT = 98 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
Y - B	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - M	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
Y - S	2x4	DRY	No.2	SPF
S - N	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMW+w	MT20	2.0	4.0		
G	TTW-p	MT20	4.0	4.0	2.25	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMWW1-t	MT20	4.0	4.0		
P, Q, R, T, U, V, W						
P	BMW1+w	MT20	2.0	4.0		
S	BS-t	MT20	3.0	6.0		
X	BMWW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
Y-B	-320 / 0	0.0	0.0	0.03 (1)		7.81
A-B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B-C	-67 / 0	-102.1	-102.1	0.13 (1)		6.25
C-D	-25 / 0	-102.1	-102.1	0.05 (1)		6.25
D-E	-28 / 0	-102.1	-102.1	0.05 (1)		6.25
E-F	-19 / 0	-102.1	-102.1	0.06 (1)		6.25
F-G	-31 / 0	-102.1	-102.1	0.08 (1)		6.25
G-H	-31 / 0	-102.1	-102.1	0.05 (1)		6.25
H-I	-19 / 0	-102.1	-102.1	0.08 (1)		6.25
I-J	-28 / 0	-102.1	-102.1	0.05 (1)		6.25
J-K	-25 / 0	-102.1	-102.1	0.05 (1)		6.25
K-L	-67 / 0	-102.1	-102.1	0.13 (1)		6.25
L-M	0 / 42	-102.1	-102.1	0.14 (1)		10.00
N-L	-320 / 0	0.0	0.0	0.03 (1)		7.81
Y-X	0 / 0	-38.5	-38.5	0.03 (3)		10.00
X-W	0 / 26	-38.5	-38.5	0.03 (2)		10.00
W-V	0 / 21	-38.5	-38.5	0.02 (2)		10.00
V-U	0 / 18	-38.5	-38.5	0.02 (2)		10.00
U-T	0 / 15	-38.5	-38.5	0.02 (3)		10.00
T-S	0 / 15	-38.5	-38.5	0.02 (3)		10.00
S-R	0 / 15	-38.5	-38.5	0.02 (3)		10.00
R-Q	0 / 18	-38.5	-38.5	0.02 (2)		10.00
Q-P	0 / 21	-38.5	-38.5	0.02 (2)		10.00
P-O	0 / 26	-38.5	-38.5	0.03 (2)		10.00
O-N	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-G	-149 / 0	0.07 (1)
U-F	-233 / 0	0.23 (1)
V-E	-193 / 0	0.10 (1)
W-D	-217 / 0	0.08 (1)
X-C	-124 / 0	0.02 (1)
R-H	-233 / 0	0.23 (1)
Q-I	-193 / 0	0.10 (1)
P-J	-217 / 0	0.08 (1)
O-K	-124 / 0	0.02 (1)
B-X	0 / 36	0.01 (1)
O-L	0 / 36	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (L-M:1) , BC=0.03/1.00 (W-X:2) , WB=0.23/1.00 (F-U:1) , SS!=0.09/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.50 (O) (INPUT = 0.90) SI METAL= 0.12 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100089824
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 7905698
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T8S	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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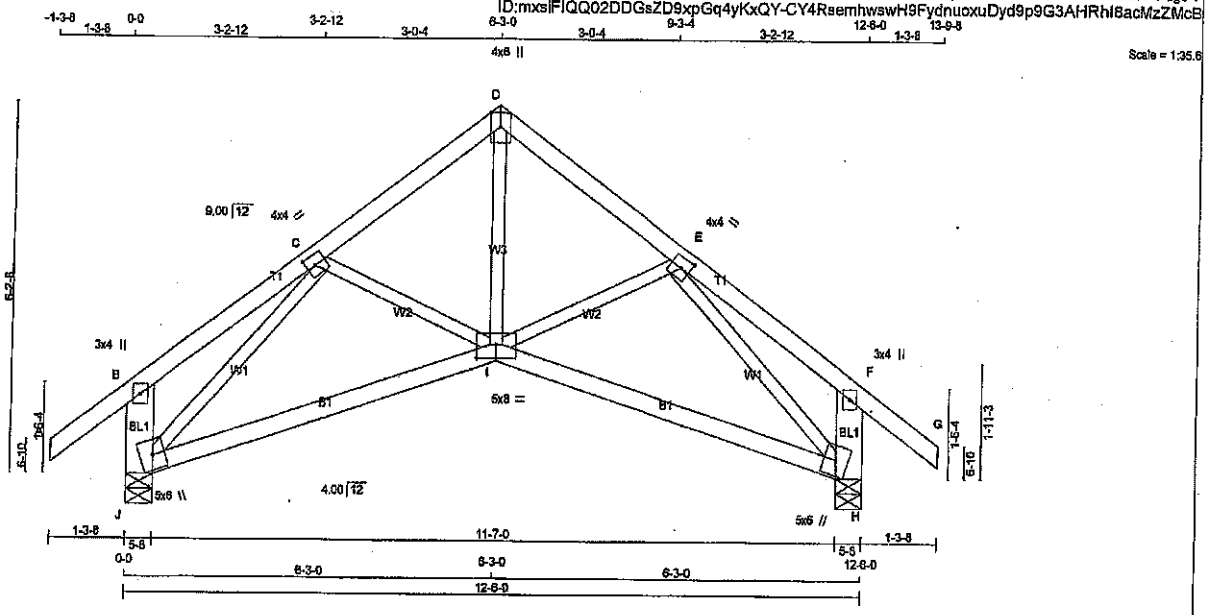

TOTAL WEIGHT = 3 X 57 = 170 lb (MiTek)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2 - 2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMK+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.75
F	TMK+p	MT20	3.0	4.0		
H	BWKM1*+m	MT20	5.0	6.0		
I	BBWWW-p	MT20	5.0	6.0	2.75	4.00
J	BWKM1*+m	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
J	1020	0	1020	0	0	5-8	5-8
H	1020	0	1020	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	753	443 / 0	131 / 0	0 / 0	0 / 0	179 / 0	0 / 0
H	753	443 / 0	131 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 47	-102.1	-102.1	0.17 (1)		10.00
B-C	-18 / 0	-102.1	-102.1	0.10 (1)		6.25
C-D	-896 / 0	-102.1	-102.1	0.11 (1)		6.25
D-E	-896 / 0	-102.1	-102.1	0.11 (1)		6.25
E-F	-18 / 0	-102.1	-102.1	0.10 (1)		6.25
F-G	0 / 47	-102.1	-102.1	0.17 (1)		10.00
J-B	-299 / 0	0.0	0.0	0.02 (1)		7.81
H-F	-299 / 0	0.0	0.0	0.02 (1)		7.81
J-I	0 / 793	-38.5	-38.5	0.43 (2)		10.00
I-H	0 / 793	-38.5	-38.5	0.43 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I-D	0 / 794	0.18 (1)
I-E	-57 / 55	0.01 (3)
C-I	-57 / 55	0.01 (3)
J-C	-1118 / 0	0.38 (1)
E-H	-1118 / 0	0.38 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.11")
ALLOWABLE DEFL.(TL)= L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/ 799 (0.19")

CSI: TC=0.17/1.00 (A-B:1) , BC=0.43/1.00 (I-J:2) , WB=0.38/1.00 (C-J:1) , SSI=0.12/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	850	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.66 (E) (INPUT = 0.90)
JSI METAL= 0.36 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DWG NO. TAM T905699
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T9	1	1	TRUSS DESC.	**T9**

Tamarack Roof Truss, Burlington

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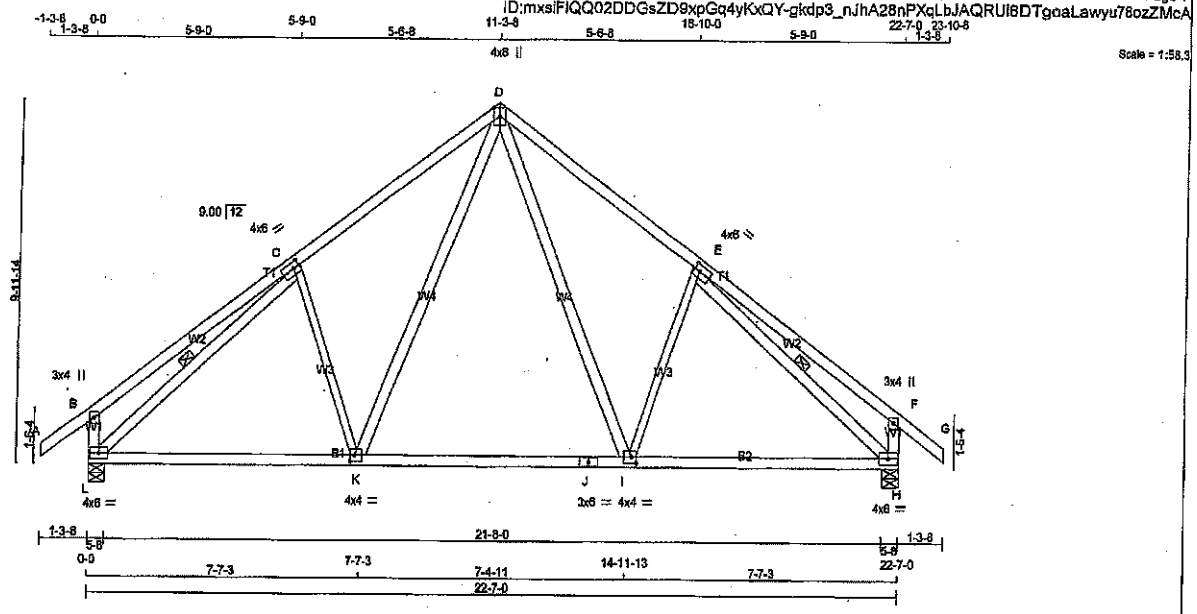

TOTAL WEIGHT = 113 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
C - K	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0	2.00	2.00
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0	2.00	1.50
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0	2.00	1.50
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
L	1728	0	1728	0	0	5-8	5-8
H	1728	0	1728	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1283	736 / 0	237 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1283	736 / 0	237 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 42	-102.1	-102.1	0.54 (1)	10.00
C- D	-1514 / 0	-102.1	-102.1	0.47 (1)	4.80
D- E	-1514 / 0	-102.1	-102.1	0.47 (1)	4.80
E- F	0 / 42	-102.1	-102.1	0.54 (1)	10.00
F- G	0 / 42	-102.1	-102.1	0.14 (1)	10.00
L- B	-356 / 0	0.0	0.0	0.04 (1)	7.81
H- F	-356 / 0	0.0	0.0	0.04 (1)	7.81
L- K	0 / 1303	-38.5	-38.5	0.55 (2)	10.00
K- J	0 / 921	-38.5	-38.5	0.50 (2)	10.00
J- I	0 / 921	-38.5	-38.5	0.50 (2)	10.00
I- H	0 / 1303	-38.5	-38.5	0.55 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 709	0.11 (1)
I- E	-366 / 14	0.21 (1)
K- D	0 / 709	0.11 (1)
C- K	-366 / 14	0.21 (1)
L- C	-1809 / 0	0.52 (1)
E- H	-1809 / 0	0.52 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.55/1.00 (H-I:2) , WB=0.52/1.00 (E-H:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	785	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.50 (J) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100089024
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T7405700
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO:
401811	G9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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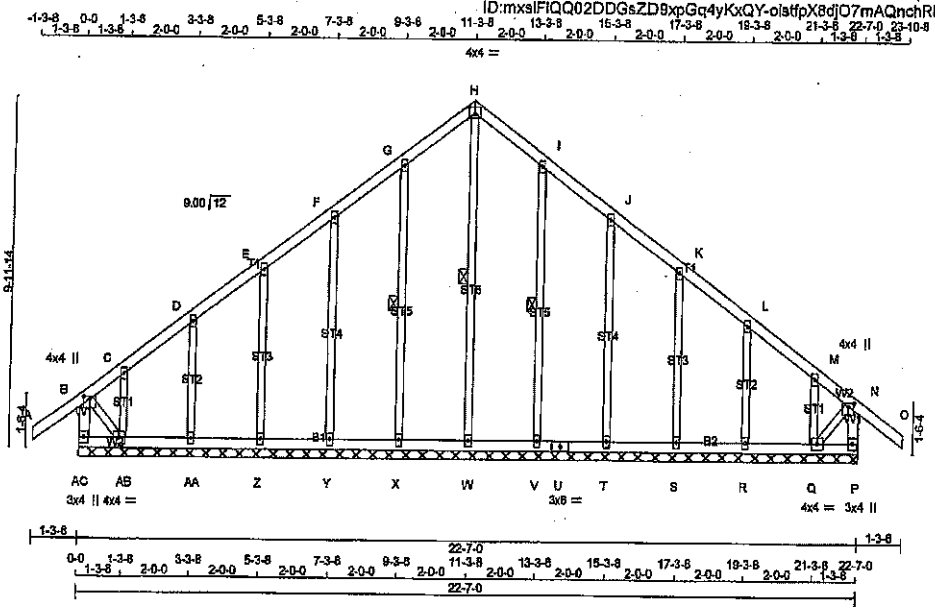

TOTAL WEIGHT = 114 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
AC- B	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AC- U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G, I, J, K, L, M						
C	TMW+w	MT20	2.0	4.0		
H	TTW-p	MT20	4.0	4.0	2.25	2.00
N	TMVW+p	MT20	4.0	4.0	1.00	2.00
P	BMV1+p	MT20	3.0	4.0		
Q	BMWW1-I	MT20	4.0	4.0		
R, S, T, V, W, X, Y, Z, AA						
R	BMW1+w	MT20	2.0	4.0		
U	BS-t	MT20	3.0	6.0		
AB	BMWW1-I	MT20	4.0	4.0		
AC	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF H-W, G-X, I-V.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-81 / 0	-102.1	-102.1	0.13 (1)	6.25
C- D	-28 / 0	-102.1	-102.1	0.05 (1)	6.25
D- E	-30 / 0	-102.1	-102.1	0.05 (1)	6.25
E- F	-24 / 0	-102.1	-102.1	0.05 (1)	6.25
F- G	-18 / 0	-102.1	-102.1	0.06 (1)	6.25
G- H	-29 / 0	-102.1	-102.1	0.06 (1)	6.25
H- I	-29 / 0	-102.1	-102.1	0.06 (1)	6.25
I- J	-18 / 0	-102.1	-102.1	0.06 (1)	6.25
J- K	-24 / 0	-102.1	-102.1	0.05 (1)	6.25
K- L	-30 / 0	-102.1	-102.1	0.05 (1)	6.25
L- M	-26 / 0	-102.1	-102.1	0.05 (1)	6.25
M- N	-81 / 0	-102.1	-102.1	0.13 (1)	6.25
N- O	0 / 42	-102.1	-102.1	0.14 (1)	10.00
AC- B	-325 / 0	0.0	0.0	0.03 (1)	7.81
P- N	-325 / 0	0.0	0.0	0.03 (1)	7.81
AC-AB	0 / 0	-38.5	-38.5	0.02 (3)	10.00
AB-AA	0 / 28	-38.5	-38.5	0.03 (2)	10.00
AA- Z	0 / 22	-38.5	-38.5	0.03 (2)	10.00
Z- Y	0 / 19	-38.5	-38.5	0.02 (2)	10.00
Y- X	0 / 16	-38.5	-38.5	0.02 (2)	10.00
X- W	0 / 14	-38.5	-38.5	0.02 (3)	10.00
W- V	0 / 14	-38.5	-38.5	0.02 (3)	10.00
V- U	0 / 16	-38.5	-38.5	0.02 (2)	10.00
U- T	0 / 16	-38.5	-38.5	0.02 (2)	10.00
T- S	0 / 19	-38.5	-38.5	0.02 (2)	10.00
S- R	0 / 22	-38.5	-38.5	0.03 (2)	10.00
R- Q	0 / 28	-38.5	-38.5	0.03 (2)	10.00
Q- P	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
W- H	-152 / 0	0.10 (1)
X- G	-232 / 0	0.11 (1)
Y- F	-197 / 0	0.17 (1)
Z- E	-199 / 0	0.09 (1)
AA- D	-217 / 0	0.05 (1)
AB- C	-77 / 0	0.01 (1)
V- I	-232 / 0	0.11 (1)
T- J	-197 / 0	0.17 (1)
S- K	-199 / 0	0.09 (1)
R- L	-217 / 0	0.05 (1)
Q- M	-77 / 0	0.01 (1)
B-AB	0 / 42	0.01 (1)
Q- N	0 / 42	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (AA-AB:2) , WB=0.17/1.00 (J-T:1) , SSI=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.47 (H) (INPUT = 0.90)
SSI METAL= 0.12 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100019929
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T1905701
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T10	3	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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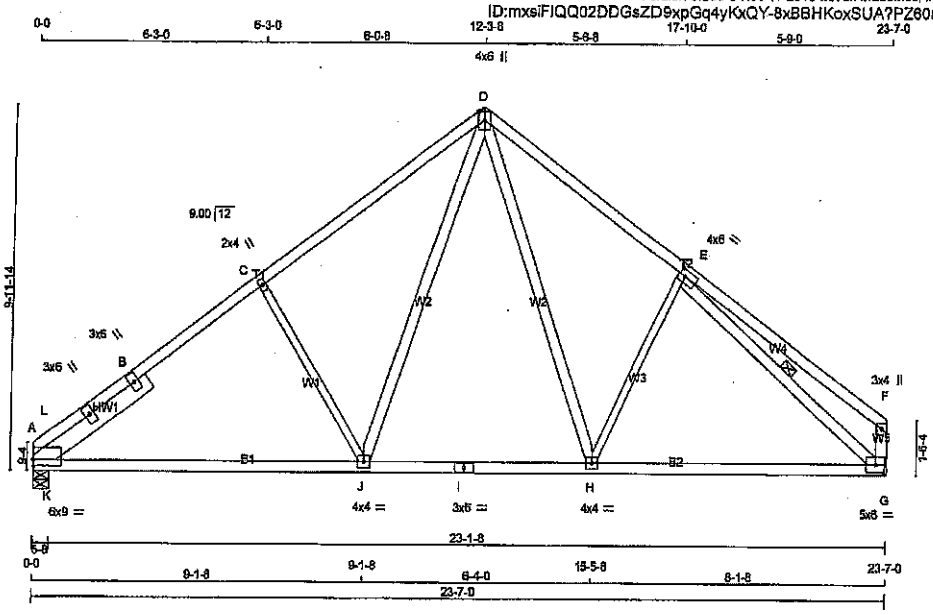

TOTAL WEIGHT = 3 X 109 = 328 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
A - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
REINFORCING MEMBERS				
HW1	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF
C - J	2x3	DRY	No.2	SPF
H - E	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBMR1-t	MT20	6.0	9.0	2.00	0.25
A	RT+t	MT20	3.0	6.0		
A	RT+t	MT20	3.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
G	BMWV1-t	MT20	5.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT	REQRD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG	BRG
A	1658	0	1658	0	0	IN-SX 5-8	IN-SX 5-8
G	1658	0	1658	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-13.

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	1239	685 / 0	248 / 0	0 / 0	0 / 0	307 / 0	0 / 0
G	1239	685 / 0	248 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-L	-2387 / 0	-102.1	-102.1	0.10 (1)		4.38
L-B	-1695 / 0	-102.1	-102.1	0.47 (1)		4.80
B-C	-1698 / 0	-102.1	-102.1	0.47 (1)		4.80
C-D	-1612 / 0	-102.1	-102.1	0.52 (1)		4.82
D-E	-1560 / 0	-102.1	-102.1	0.47 (1)		4.74
E-F	0 / 42	-102.1	-102.1	0.54 (1)		10.00
G-F	-216 / 0	0.0	0.0	0.02 (1)		7.81
A-K	0 / 1242	-38.5	-38.5	0.54 (2)		10.00
K-J	0 / 1538	-38.5	-38.5	0.58 (2)		10.00
J-I	0 / 1010	-38.5	-38.5	0.52 (2)		10.00
I-H	0 / 1010	-38.5	-38.5	0.52 (2)		10.00
H-G	0 / 1362	-38.5	-38.5	0.57 (2)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-J	-552 / 0	0.33 (1)
J-D	0 / 799	0.13 (1)
D-H	0 / 656	0.11 (1)
H-E	-347 / 16	0.22 (1)
E-G	-1891 / 0	0.54 (1)
K-L	0 / 929	0.00 (1)
K-B	-439 / 173	0.05 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 932 (0.30")

CSI: TC=0.54/1.00 (E-F:1) , BC=0.57/1.00 (G-H:2) , WB=0.54/1.00 (E-G:1) , SSI=0.50/1.00 (A-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	918	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (J) (INPUT = 0.90)
JSI METAL= 0.62 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100000094
PROVINCE OF ONTARIO

DWG No. T1905702
STRUCTURAL COMPONENT ONLY

THIS STRUCTURAL COMPONENT IS DESIGNED TO BE INCORPORATED INTO A BUILDING DESIGNED BY A PROFESSIONAL ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER TO ENSURE THAT THE TEMPORARY AND PERMANENT BRACING OF THE ENTIRE STRUCTURE IS PROVIDED AS REQUIRED BY GOOD ENGINEERING PRACTICE AND THE BUILDING CODE.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	T11	2	1	TRUSS DESC.	**T11**

Tamarack Roof Truss, Burlington

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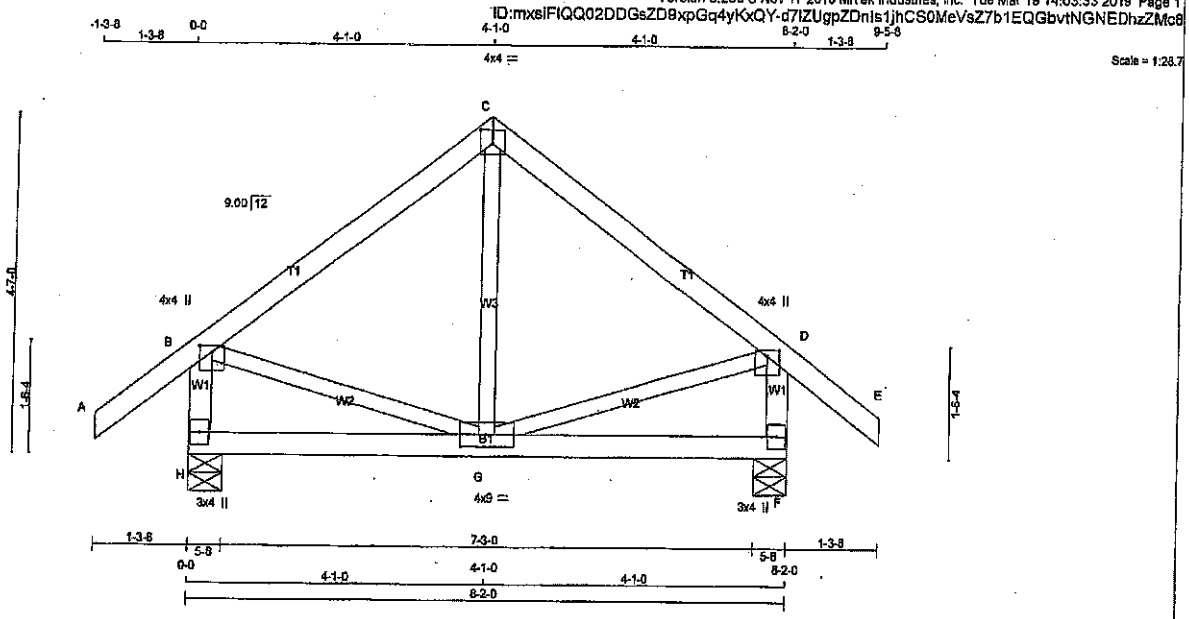

TOTAL WEIGHT = 2 X 37 = 74 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	715	0	715	0	0	5-8	5-8
F	715	0	715	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	526	317 / 0	86 / 0	0 / 0	0 / 0	123 / 0	0 / 0
F	526	317 / 0	86 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-366 / 0	-102.1	-102.1	0.22 (1)	6.25	
C-D	-366 / 0	-102.1	-102.1	0.22 (1)	6.25	
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
H-B	-654 / 0	0.0	0.0	0.07 (1)	7.81	
F-D	-654 / 0	0.0	0.0	0.07 (1)	7.81	
H-G	0 / 0	-38.5	-38.5	0.15 (3)	10.00	
G-F	0 / 0	-38.5	-38.5	0.15 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 161	0.04 (3)
B-G	0 / 305	0.07 (1)
G-D	0 / 305	0.07 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.22/1.00 (C-D:1) , BC=0.15/1.00 (G-H:3) , WB=0.07/1.00 (D-G:1) , SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	786	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009827
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DWG NO. TAM 7905703
STRUCTURAL
CONSIDERATION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	G11	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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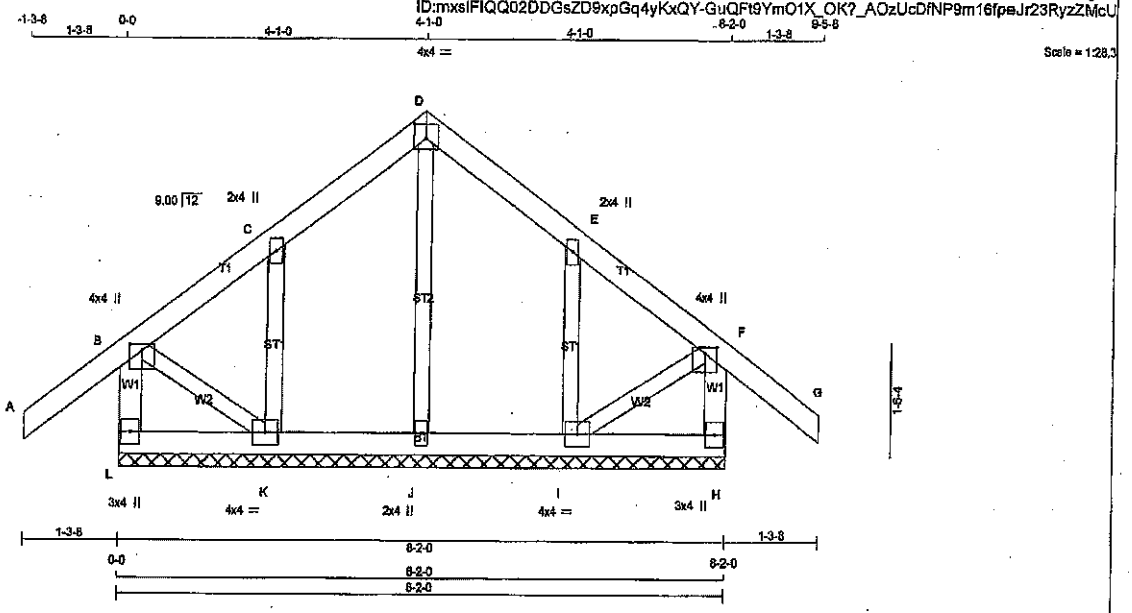


TOTAL WEIGHT = 38 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMVW+p	MT20	4.0	4.0	1.00	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMWW1-t	MT20	4.0	4.0		
J	BMW1+w	MT20	2.0	4.0		
K	BMWW1-t	MT20	4.0	4.0		
L	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
L- B	-245 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B- C	-11 / 0	-102.1	-102.1	0.07 (1)		6.25
C- D	-31 / 0	-102.1	-102.1	0.07 (1)		6.25
D- E	-31 / 0	-102.1	-102.1	0.07 (1)		6.25
E- F	-11 / 0	-102.1	-102.1	0.07 (1)		6.25
F- G	0 / 42	-102.1	-102.1	0.14 (1)		10.00
H- F	-245 / 0	0.0	0.0	0.03 (1)		7.81
L- K	0 / 0	-38.5	-38.5	0.03 (3)		10.00
K- J	0 / 13	-38.5	-38.5	0.03 (3)		10.00
J- I	0 / 13	-38.5	-38.5	0.03 (3)		10.00
I- H	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-144 / 0	0.04 (1)
K- C	-253 / 0	0.05 (1)
I- E	-253 / 0	0.05 (1)
B- K	0 / 25	0.01 (1)
I- F	0 / 25	0.01 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1) , BC=0.03/1.00 (H-J:3) , WB=0.05/1.00 (E-I:1) , SSI=0.06/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

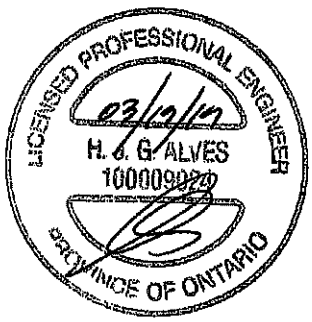
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (F) (INPUT = 0.90)
JSI METAL= 0.13 (E) (INPUT = 1.00)



DWG NO. TAM 7905704
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	PB1	2	1	TRUSS DESC.	**PB1**

Tamarack Roof Truss, Burlington

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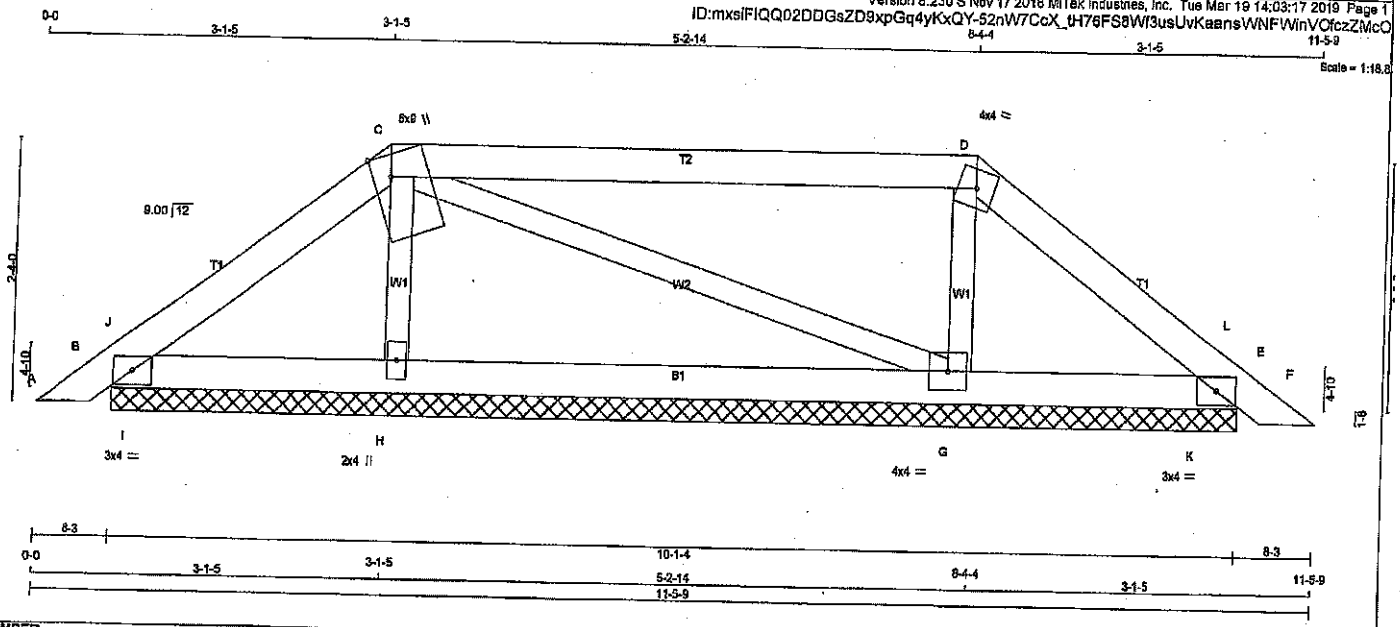


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	6.0	9.0	Edge	2.00
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-t	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	270	0	270	0	0	10-1-4	10-1-4
E	256	0	256	0	0	10-1-4	10-1-4
H	494	0	494	0	0	10-1-4	10-1-4
G	521	0	521	0	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	190	142 / 0	11 / 0	0 / 0	0 / 0	37 / 0	0 / 0
E	181	134 / 0	11 / 0	0 / 0	0 / 0	35 / 0	0 / 0
H	378	182 / 0	95 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	398	198 / 0	95 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI	MAX. UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	
B-J	-63 / 0	-102.1	-102.1	0.03 (1)	6.25	
J-C	-111 / 0	-102.1	-102.1	0.07 (1)	6.25	
C-D	-49 / 0	-102.1	-102.1	0.48 (1)	6.25	
D-L	-89 / 0	-102.1	-102.1	0.07 (1)	6.25	
L-E	-39 / 1	-102.1	-102.1	0.03 (1)	6.25	
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00	
B-I	0 / 84	-38.5	-38.5	0.08 (1)	10.00	
I-H	0 / 84	-38.5	-38.5	0.13 (2)	10.00	
H-G	0 / 67	-38.5	-38.5	0.13 (2)	10.00	
G-K	0 / 66	-38.5	-38.5	0.13 (2)	10.00	
K-E	0 / 66	-38.5	-38.5	0.08 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-298 / 0	0.05 (1)
C-G	-20 / 0	0.01 (1)
G-D	-319 / 0	0.05 (1)
I-J	-194 / 0	0.00 (1)
K-L	-196 / 0	0.00 (1)

TOTAL WEIGHT = 2 X 33 = 66 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-D:1) , BC=0.13/1.00 (H-I:2) , WB=0.05/1.00 (D-G:1) , SSI=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

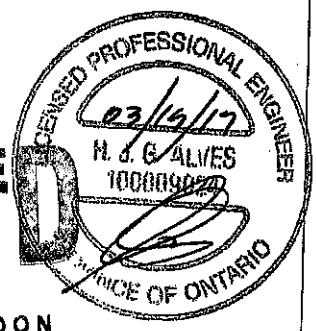
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)



DWG NO. TAM T1905705
STRUCTURAL
CLARK TRUSS CO LTD

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	PB2	2	1	TRUSS DESC.	**PB2**

Tamarack Roof Truss, Burlington

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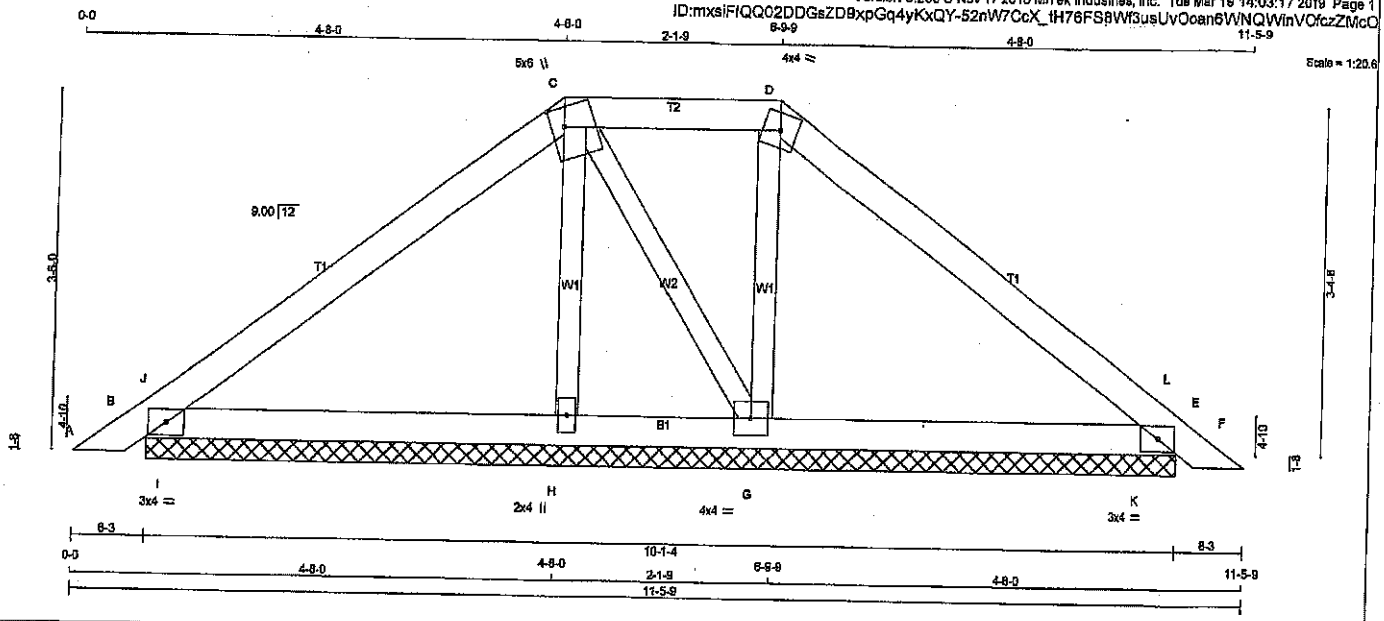


TOTAL WEIGHT = 2 X 34 = 69 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMWW1-I	MT20	4.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	439	0	439	0	0	10-1-4	10-1-4
E	418	0	418	0	0	10-1-4	10-1-4
H	285	0	285	0	0	10-1-4	10-1-4
G	399	0	399	0	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	320	202 / 0	46 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	306	189 / 0	46 / 0	0 / 0	0 / 0	70 / 0	0 / 0
H	221	98 / 0	62 / 0	0 / 0	0 / 0	61 / 0	0 / 0
G	298	165 / 0	59 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B-J	-122 / 0	-102.1	-102.1	0.07 (1)	6.25
J-C	-197 / 0	-102.1	-102.1	0.21 (1)	6.25
C-D	-116 / 0	-102.1	-102.1	0.08 (1)	6.25
D-L	-162 / 0	-102.1	-102.1	0.21 (1)	6.25
L-E	-118 / 0	-102.1	-102.1	0.07 (1)	6.25
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B-I	0 / 151	-38.5	-38.5	0.17 (1)	10.00
I-H	0 / 151	-38.5	-38.5	0.18 (1)	10.00
H-G	0 / 147	-38.5	-38.5	0.14 (1)	10.00
G-K	0 / 123	-38.5	-38.5	0.18 (1)	10.00
K-E	0 / 123	-38.5	-38.5	0.17 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-125 / 0	0.02 (1)
C-G	-55 / 0	0.01 (1)
G-D	-192 / 0	0.04 (1)
I-J	-373 / 71	0.00 (1)
K-L	-377 / 70	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1) , BC=0.18/1.00 (H-I:1) , WB=0.04/1.00 (D-G:1) , SSI=0.32/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

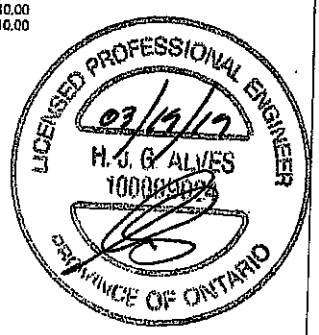
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	768	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM T7905706
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	PB3	1	1	GREEN PARK HOMES TRUSS DESC.	PB3

Tamarack Roof Truss, Burlington

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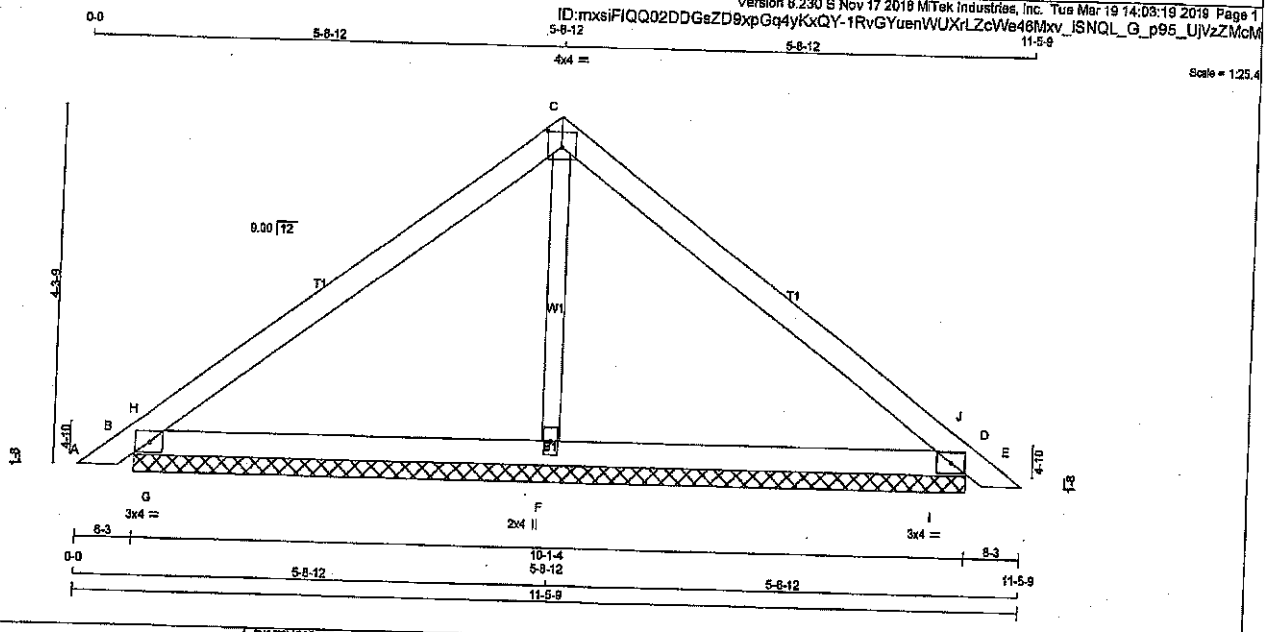

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	528	0	528	0	0	10-1-4	10-1-4
D	528	0	528	0	0	10-1-4	10-1-4
F	485	0	485	0	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	384	246 / 0	53 / 0	0 / 0	0 / 0	86 / 0	0 / 0
D	384	246 / 0	53 / 0	0 / 0	0 / 0	86 / 0	0 / 0
F	377	164 / 0	107 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- H	-128 / 25	-102.1	-102.1	0.17 (1)	6.25
H- C	-270 / 0	-102.1	-102.1	0.33 (1)	6.25
C- J	-270 / 0	-102.1	-102.1	0.33 (1)	6.25
J- D	-128 / 25	-102.1	-102.1	0.17 (1)	6.25
D- E	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- G	0 / 204	-38.5	-38.5	0.29 (1)	10.00
G- F	0 / 204	-38.5	-38.5	0.33 (1)	10.00
F- I	0 / 204	-38.5	-38.5	0.33 (1)	10.00
I- D	0 / 204	-38.5	-38.5	0.29 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- C	-132 / 10	0.03 (1)
G- H	-683 / 45	0.00 (1)
I- J	-683 / 45	0.00 (1)

TOTAL WEIGHT = 31 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	28.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1) , BC=0.33/1.00 (F-G:1) , WB=0.03/1.00 (C-F:1) , SSI=0.55/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/19/19
J. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. YAM 71905707
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	V1	1	1	GREEN PARK HOMES: TRUSS DESC.	

Tamarack Roof Truss, Burlington

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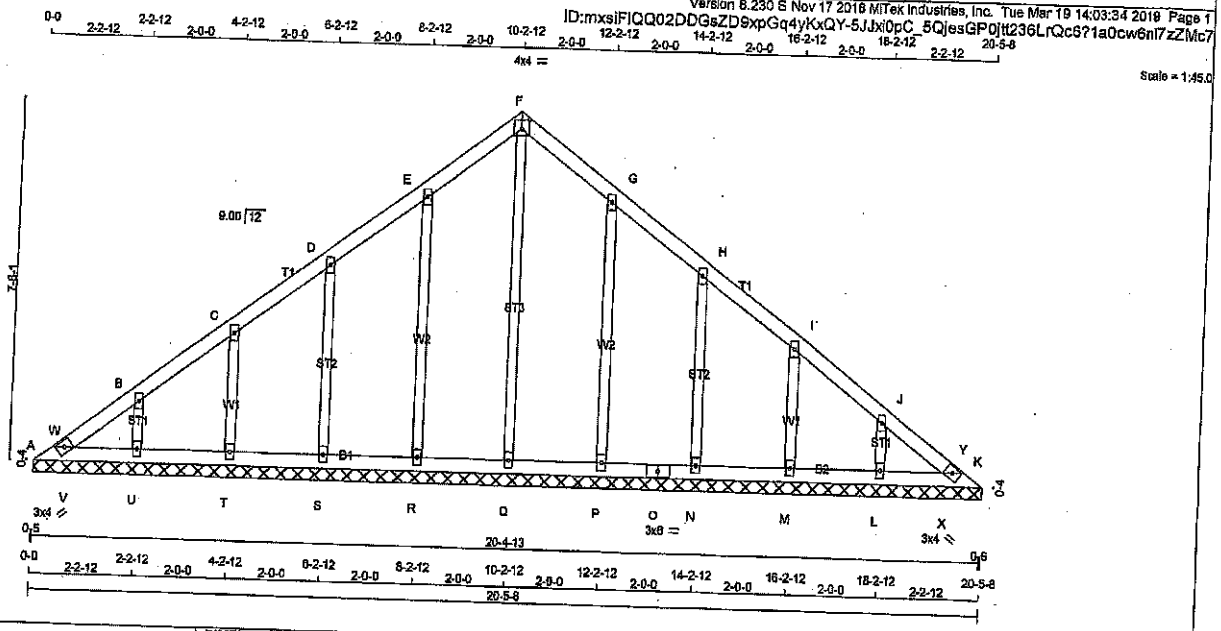

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
A - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, D, E, G, H, I, J						
B	TMW+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
K	TBM1-h	MT20	3.0	4.0		
L, M, N, P, Q, R, S, T, U						
L	BMW1+w	MT20	2.0	4.0		
O	BS-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	141	0	141	0	0	20-5-8 (6-11-12)	5-8
K	141	0	141	0	0	20-5-8 (6-11-12)	5-8
Q	238	0	238	0	0	20-5-8 (6-11-12)	5-8
S	276	0	276	0	0	20-5-8 (6-11-12)	5-8
U	329	0	329	0	0	20-5-8 (6-11-12)	5-8
N	276	0	276	0	0	20-5-8 (6-11-12)	5-8
L	329	0	329	0	0	20-5-8 (6-11-12)	5-8
R	308	0	308	0	0	20-5-8 (6-11-12)	5-8
T	261	0	261	0	0	20-5-8 (6-11-12)	5-8
P	308	0	308	0	0	20-5-8 (6-11-12)	5-8
M	261	0	261	0	0	20-5-8 (6-11-12)	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	104	62 / 0	17 / 0	0 / 0	0 / 0	24 / 0	0 / 0
K	104	62 / 0	17 / 0	0 / 0	0 / 0	24 / 0	0 / 0
Q	182	88 / 0	46 / 0	0 / 0	0 / 0	48 / 0	0 / 0
S	207	113 / 0	42 / 0	0 / 0	0 / 0	52 / 0	0 / 0
U	246	135 / 0	50 / 0	0 / 0	0 / 0	61 / 0	0 / 0
N	207	113 / 0	42 / 0	0 / 0	0 / 0	52 / 0	0 / 0
L	246	135 / 0	50 / 0	0 / 0	0 / 0	61 / 0	0 / 0
R	229	132 / 0	42 / 0	0 / 0	0 / 0	55 / 0	0 / 0
T	198	107 / 0	40 / 0	0 / 0	0 / 0	49 / 0	0 / 0
P	229	132 / 0	42 / 0	0 / 0	0 / 0	55 / 0	0 / 0
M	198	107 / 0	40 / 0	0 / 0	0 / 0	49 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K, Q, S, U, N, L, R, T, P, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-W	-54 / 0	-102.1	-102.1	0.03 (1)	6.25
W-B	-17 / 8	-102.1	-102.1	0.08 (1)	6.25
B-C	-29 / 0	-102.1	-102.1	0.06 (1)	6.25
C-D	-19 / 4	-102.1	-102.1	0.04 (1)	6.25
D-E	-10 / 5	-102.1	-102.1	0.06 (1)	6.25
E-F	-22 / 3	-102.1	-102.1	0.08 (1)	6.25
F-G	-22 / 3	-102.1	-102.1	0.08 (1)	6.25
G-H	-10 / 5	-102.1	-102.1	0.06 (1)	6.25
H-I	-19 / 4	-102.1	-102.1	0.04 (1)	6.25
I-J	-29 / 0	-102.1	-102.1	0.06 (1)	6.25
J-Y	-17 / 8	-102.1	-102.1	0.08 (1)	6.25
Y-K	-54 / 0	-102.1	-102.1	0.03 (1)	6.25
A-V	-2 / 27	-38.5	-38.5	0.05 (1)	10.00
V-U	0 / 35	-38.5	-38.5	0.05 (1)	10.00
U-T	-2 / 21	-38.5	-38.5	0.04 (1)	10.00
T-S	-3 / 15	-38.5	-38.5	0.02 (2)	10.00
S-R	-3 / 11	-38.5	-38.5	0.02 (2)	10.00
R-Q	-4 / 7	-38.5	-38.5	0.02 (3)	10.00
Q-P	-4 / 7	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-F	-160 / 0	0.17 (1)
S-D	-196 / 0	0.06 (1)
U-B	-211 / 0	0.03 (1)
N-H	-196 / 0	0.06 (1)
L-J	-211 / 0	0.03 (1)
R-E	-232 / 0	0.14 (1)
T-C	-196 / 0	0.04 (1)
P-G	-232 / 0	0.14 (1)
M-I	-196 / 0	0.04 (1)
V-W	-40 / 17	0.00 (1)
X-Y	-40 / 17	0.00 (1)

TOTAL WEIGHT = 79 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (E-F:1) , BC=0.06/1.00 (L-X:1) , WB=0.17/1.00 (F-Q:1) , SSI=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (F) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

RECEIVED
DWG NO. T1905726
JUN 25 2019
COMMUNITY PLANNING
TOWN OF CALEDON
BUILDING SECTION
FILE NO

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	V1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-O	-3 / 11	-38.5	-38.5	0.02 (2)	10.00			
O-N	-3 / 11	-38.5	-38.5	0.02 (2)	10.00			
N-M	-3 / 15	-38.5	-38.5	0.02 (2)	10.00			
M-L	-2 / 21	-38.5	-38.5	0.04 (1)	10.00			
L-X	0 / 35	-38.5	-38.5	0.05 (1)	10.00			
X-K	-2 / 27	-38.5	-38.5	0.05 (1)	10.00			

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100099074
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 79057 08
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. - GREEN PARK HOMES	DRWG NO.
401811	V2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 19 15:10:35 2019 Page 1
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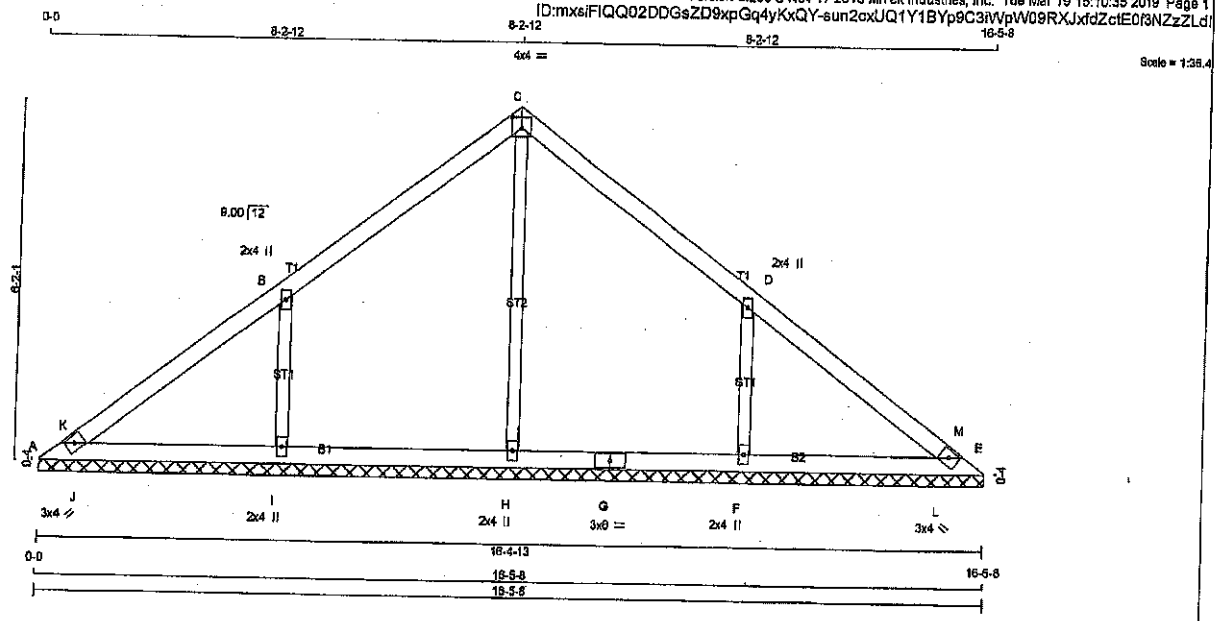


TOTAL WEIGHT = 50 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, H, I						
F	BMW1+w	MT20	2.0	4.0		
G	BS-t	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	176	0	176	0	0	16-4-13 (6-5-10)	4-13
E	178	0	178	0	0	16-4-13 (6-5-10)	4-13
H	572	0	572	0	0	16-4-13 (6-5-10)	4-13
I	689	0	689	0	0	16-4-13 (6-5-10)	4-13
F	689	0	689	0	0	16-4-13 (6-5-10)	4-13

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	130	80 / 0	21 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	130	80 / 0	21 / 0	0 / 0	0 / 0	30 / 0	0 / 0
H	438	209 / 0	112 / 0	0 / 0	0 / 0	118 / 0	0 / 0
I	512	282 / 0	96 / 0	0 / 0	0 / 0	124 / 0	0 / 0
F	512	282 / 0	96 / 0	0 / 0	0 / 0	124 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-K	0 / 56	-102.1	-102.1	0.06 (2)	10.00
K-B	0 / 124	-102.1	-102.1	0.29 (1)	10.00
B-C	0 / 80	-102.1	-102.1	0.28 (1)	10.00
C-D	0 / 80	-102.1	-102.1	0.28 (1)	10.00
D-M	0 / 124	-102.1	-102.1	0.29 (1)	10.00
M-E	0 / 56	-102.1	-102.1	0.06 (2)	10.00
A-J	-89 / 0	-38.5	-38.5	0.10 (1)	6.25
J-I	-70 / 0	-38.5	-38.5	0.13 (2)	6.25
I-H	-82 / 0	-38.5	-38.5	0.13 (2)	6.25
H-G	-82 / 0	-38.5	-38.5	0.13 (2)	6.25
G-F	-82 / 0	-38.5	-38.5	0.13 (2)	6.25
F-L	-70 / 0	-38.5	-38.5	0.13 (2)	6.25
L-E	-89 / 0	-38.5	-38.5	0.10 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-444 / 0	0.26 (1)
I-B	-466 / 0	0.09 (1)
F-D	-466 / 0	0.09 (1)
J-K	-85 / 47	0.00 (1)
L-M	-85 / 47	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.29/1.00 (B-K:1) , BC=0.13/1.00 (H-I:2) , WB=0.26/1.00 (C-H:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

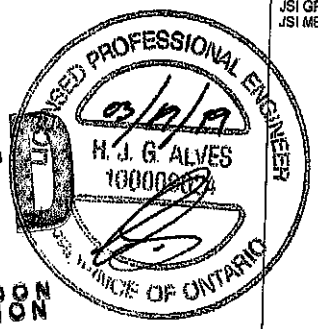
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.90)
JSI METAL= 0.26 (D) (INPUT = 1.00)



DWG NO. TAM T1905709
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	V3	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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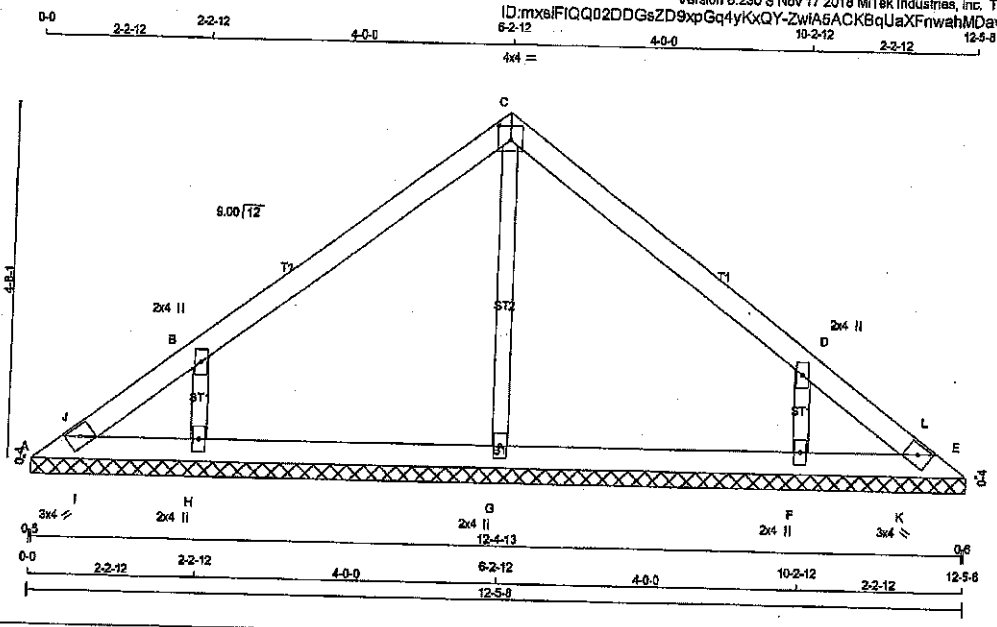


TOTAL WEIGHT = 38 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TMW+w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMW+w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	110	0	110	0	0	12-5-8 (12-4-13)5-8	
E	110	0	110	0	0	12-5-8 (12-4-13)5-8	
G	409	0	409	0	0	12-5-8 (12-4-13)5-8	
H	557	0	557	0	0	12-5-8 (12-4-13)5-8	
F	557	0	557	0	0	12-5-8 (12-4-13)5-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	81	49 / 0	13 / 0	0 / 0	0 / 0	19 / 0	0 / 0
E	81	49 / 0	13 / 0	0 / 0	0 / 0	19 / 0	0 / 0
G	317	139 / 0	90 / 0	0 / 0	0 / 0	88 / 0	0 / 0
H	412	241 / 0	72 / 0	0 / 0	0 / 0	98 / 0	0 / 0
F	412	241 / 0	72 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

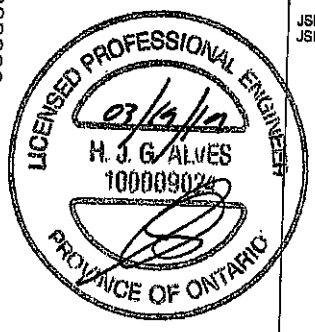
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-J	-120 / 0	-102.1	-102.1	0.03 (1)	6.25	
J-B	-32 / 0	-102.1	-102.1	0.23 (1)	6.25	
B-C	-102 / 0	-102.1	-102.1	0.23 (1)	6.25	
C-D	-102 / 0	-102.1	-102.1	0.23 (1)	6.25	
D-L	-32 / 0	-102.1	-102.1	0.23 (1)	6.25	
L-E	-120 / 0	-102.1	-102.1	0.03 (1)	6.25	
A-I	0 / 95	-38.5	-38.5	0.02 (1)	10.00	
I-H	0 / 92	-38.5	-38.5	0.08 (2)	10.00	
H-G	0 / 62	-38.5	-38.5	0.11 (2)	10.00	
G-F	0 / 62	-38.5	-38.5	0.11 (2)	10.00	
F-K	0 / 92	-38.5	-38.5	0.08 (2)	10.00	
K-E	0 / 95	-38.5	-38.5	0.02 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	-243 / 0	0.06 (1)
H-B	-437 / 0	0.06 (1)
F-D	-437 / 0	0.06 (1)
I-J	0 / 45	0.00 (1)
K-L	0 / 45	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (B-C:1) , BC=0.11/1.00 (G-H:2) , WB=0.08/1.00 (C-G:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	816	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

DWG NO. TAM T1905710
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	V4	1	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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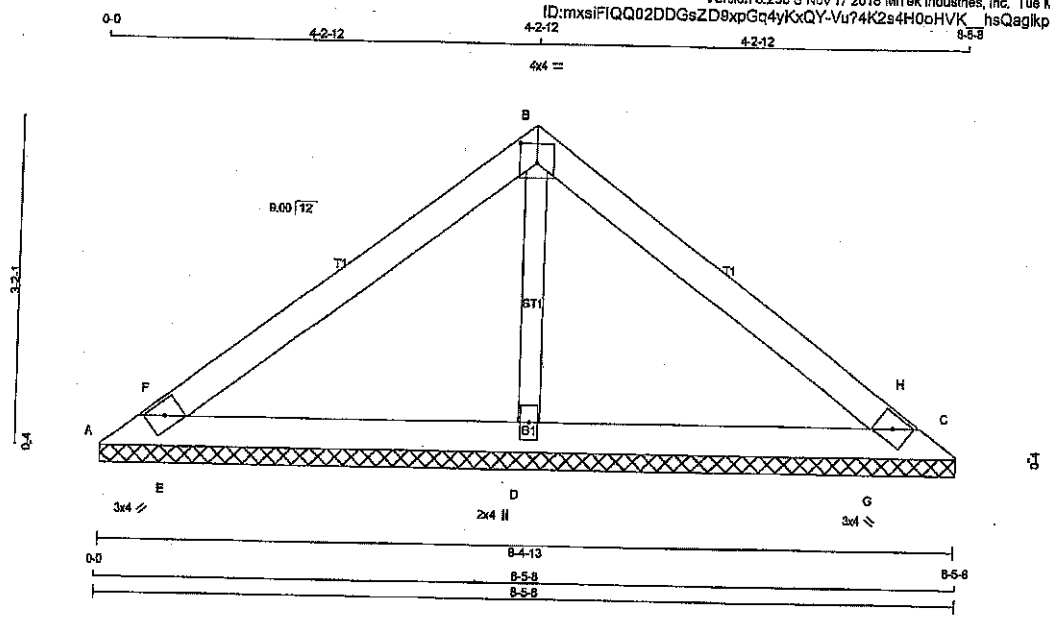

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 23 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	70	0	70	0	0	8-4-13	8-4-13
C	70	0	70	0	0	8-4-13	8-4-13
D	1042	0	1042	0	0	8-4-13	8-4-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	52	29 / 0	10 / 0	0 / 0	0 / 0	13 / 0	0 / 0
C	52	29 / 0	10 / 0	0 / 0	0 / 0	13 / 0	0 / 0
D	778	430 / 0	156 / 0	0 / 0	0 / 0	193 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- F	0 / 347	-102.1	-102.1	0.11 (1)	10.00	
F- B	0 / 347	-102.1	-102.1	0.24 (1)	10.00	
B- H	0 / 347	-102.1	-102.1	0.24 (1)	10.00	
H- C	0 / 347	-102.1	-102.1	0.11 (1)	10.00	
A- E	-322 / 0	-38.5	-38.5	0.17 (1)	6.25	
E- D	-283 / 0	-38.5	-38.5	0.20 (1)	6.25	
D- G	-283 / 0	-38.5	-38.5	0.20 (1)	6.25	
G- C	-322 / 0	-38.5	-38.5	0.17 (1)	6.25	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- B	-755 / 0	0.14 (1)
E- F	-222 / 19	0.00 (1)
G- H	-222 / 19	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.6	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.6	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (B-F:1) , BC=0.20/1.00 (D-E:1) , WB=0.14/1.00 (B-D:1) , SSI=0.14/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H.J. G. ALVES
100090277
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

BMG ICO. TAM T1905711
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	V5	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Tue Mar 19 14:03:38 2019 Page 1
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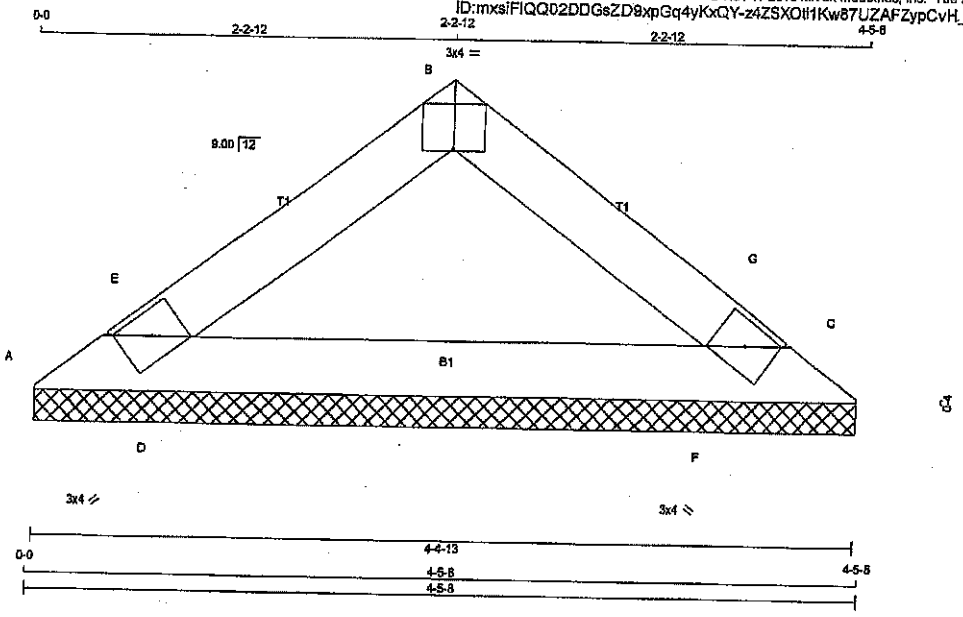

Scale = 1:11.2

TOTAL WEIGHT = 10 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TT-p	MT20	3.0	4.0	Edge	2.00
C	TBM1-h	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	309	0	309	0	0	4-4-13	4-4-13
C	309	0	309	0	0	4-4-13	4-4-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	231	126 / 0	46 / 0	0 / 0	0 / 0	57 / 0	0 / 0
C	231	126 / 0	46 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-E	-291 / 0	-102.1	-102.1	0.15 (1)		6.25
E-B	-203 / 0	-102.1	-102.1	0.15 (1)		6.25
B-G	-203 / 0	-102.1	-102.1	0.15 (1)		6.25
G-C	-291 / 0	-102.1	-102.1	0.15 (1)		6.25
A-D	0 / 192	-38.5	-38.5	0.08 (1)		10.00
D-F	0 / 197	-38.5	-38.5	0.13 (1)		10.00
F-C	0 / 192	-38.5	-38.5	0.08 (1)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-E	0 / 104	0.00 (1)
F-G	0 / 104	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-G:1) , BC=0.13/1.00 (D-F:1) , WB=0.00/1.00 (D-E:1) , SSI=0.13/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (C) (INPUT = 0.90)
JSI METAL= 0.09 (A) (INPUT = 1.00)

THIS STRUCTURAL TRUSS DESIGN IS BASED ON THE PARAMETERS SHOWN, AND IS FOR AN INDIVIDUAL BUILDING COMPONENT TO BE INCORPORATED INTO THE OVERALL BUILDING DESIGN BY THE BUILDING DESIGNER. IT IS NOT SUITABLE FOR USE WHERE THE PARAMETERS SHOWN ARE EXCEEDED. THE BUILDING DESIGNER IS RESPONSIBLE FOR TEMPORARY AND PERMANENT BRACING OF THE OVERALL STRUCTURE. FOR GENERAL GUIDANCE REGARDING BRACING, SEE BCSI AVAILABLE FROM THE TRUSS PLATE INSTITUTE.

LICENSED PROFESSIONAL ENGINEER
02/19/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

BWS NO. TAM T1905712
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401811	J7	10	1	TRUSS DESC.	J7

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 19 14:03:14 2019 Page 1
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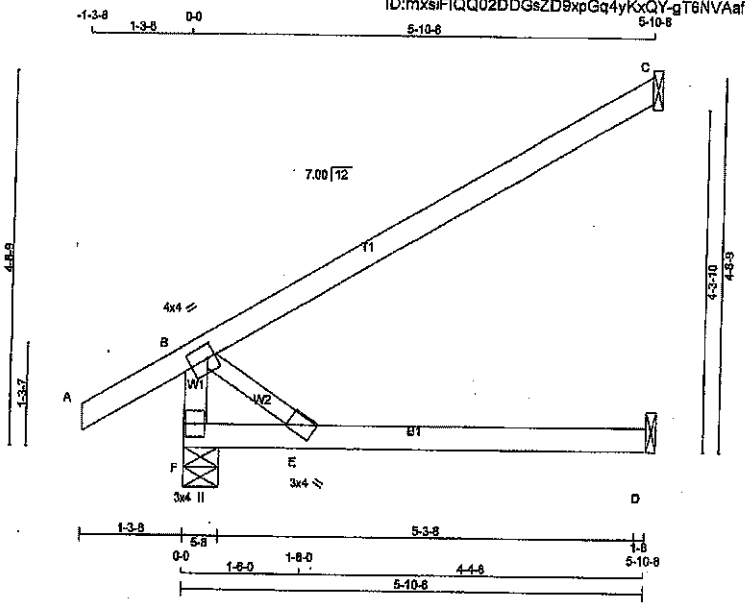


TOTAL WEIGHT = 10 X 19 = 186 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	552	0	552	0	0	5-8	5-8
C	300	0	300	0	0	1-8	1-8
D	113	0	144	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	404	250 / 0	82 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	208	171 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-439 / 0	0.0	0.0	0.04 (1)		7.81
A- B	0 / 35	-102.1	-102.1	0.14 (1)		10.00
B- C	0 / 0	-102.1	-102.1	0.60 (1)		10.00
F- E	0 / 0	-38.5	-38.5	0.24 (3)		10.00
E- D	0 / 0	-38.5	-38.5	0.31 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.31/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.20/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

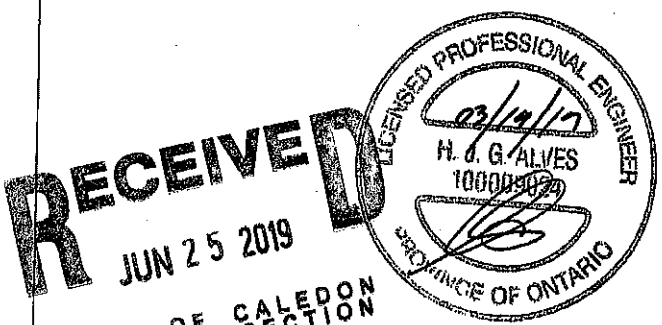
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 17905713
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	J8	5	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Mar 19 14:03:16 2019 Page 1
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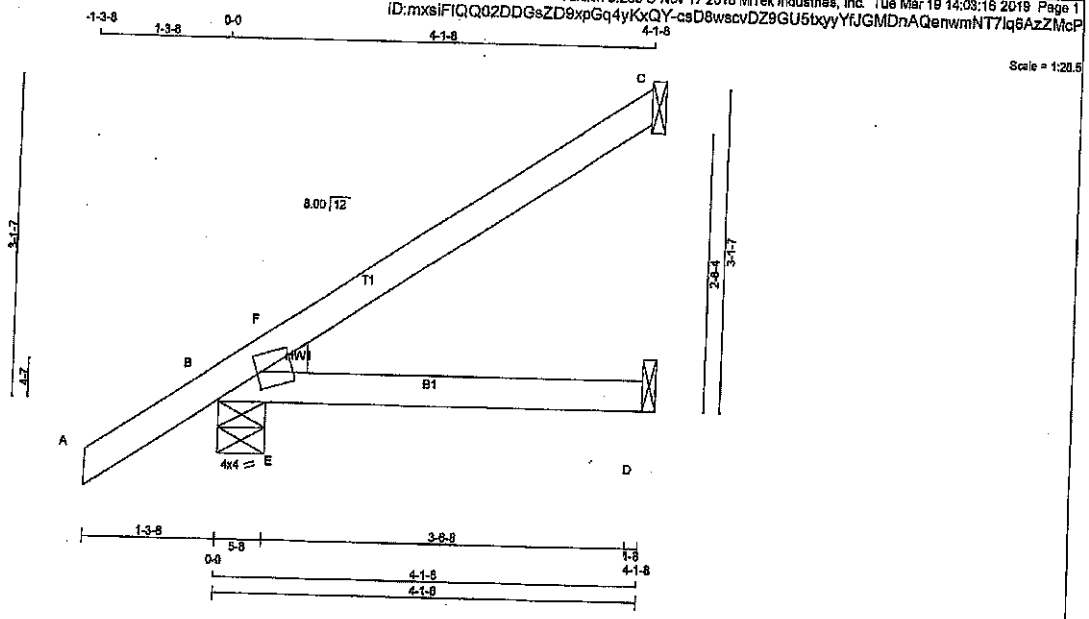


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	4.0	2.00	0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 5 X 13 = 64 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	184	0	184	0	0	1-8	1-8	
B	430	0	430	0	0	5-8	5-8	2x4 L
D	106	0	113	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	129	98 / 0	7 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	313	199 / 0	43 / 0	0 / 0	0 / 0	70 / 0	0 / 0
D	88	22 / 0	37 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 37	-102.1	-102.1	0.14 (1)	10.00	
B-F	-47 / 0	-102.1	-102.1	0.07 (3)	6.25	
F-C	0 / 6	-102.1	-102.1	0.23 (1)	10.00	
B-E	0 / 0	-38.5	-38.5	0.18 (1)	10.00	
E-D	0 / 0	-38.5	-38.5	0.20 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-171 / 62	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.23/1.00 (C-F:1) , BC=0.20/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM T7905714
STRUCTURAL
COMPONENT ONLY

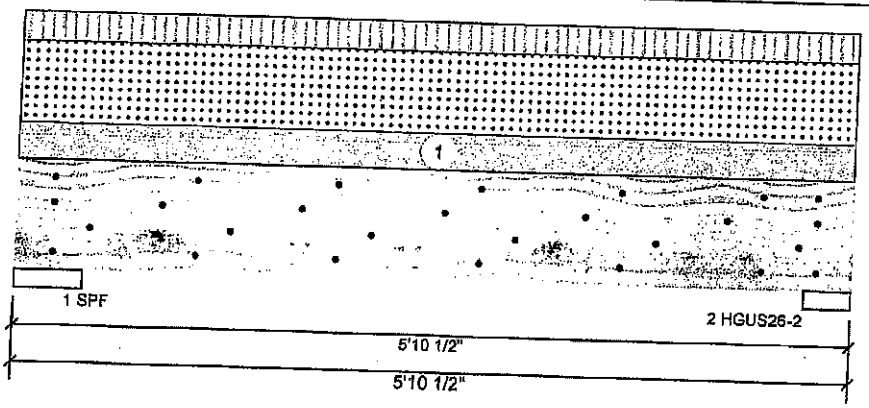
isDesign™

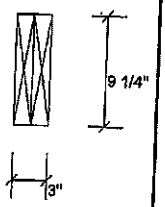
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	360
Importance:	Normal

Application:	Roof (Residential)
Slope:	0/12
Design Method:	LSD
Building Code:	NBCC 2015
Load Sharing:	No
Deck:	Not Checked
Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	340	421	939	0
2	326	403	900	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	22%	526 / 1748	2274	L	1.25D+1.5S +L
2 - HGUS...	4.000"	30%	504 / 1675	2180	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2571 ft-lb	3'	6039 ft-lb	0.426 (43%)	1.25D+1.5S +L	L
Unbraced	2571 ft-lb	3'	5236 ft-lb	0.491 (49%)	1.25D+1.5S +L	L
Shear	1974 lb	1'2"	3984 lb	0.496 (50%)	1.25D+1.5S +L	L
LL Defl inch	0.022 (L/2784)	3'	0.174 (L/360)	0.130 (13%)	S+0.5L	L
TL Defl inch	0.031 (L/2018)	3'	0.174 (L/360)	0.180 (18%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/12/19
H. J. GALVES
100009922
PROVINCE OF ONTARIO

BWS NO. TAM 1905728
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		10-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

Manufacturer Info

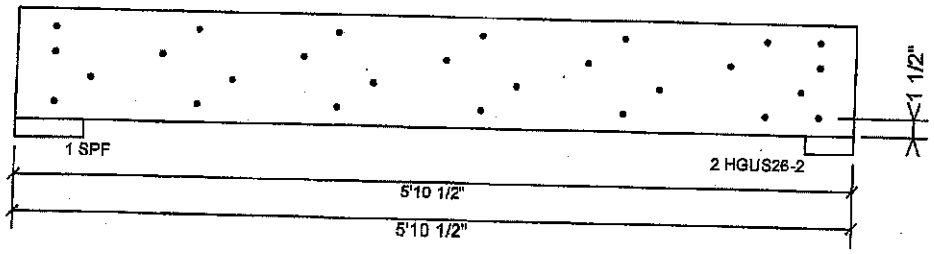
Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

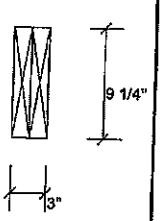
TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	83.6 %
Load	379.1 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
H.J. G ALVES
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

PROVIDED TAM 7905728
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 336-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

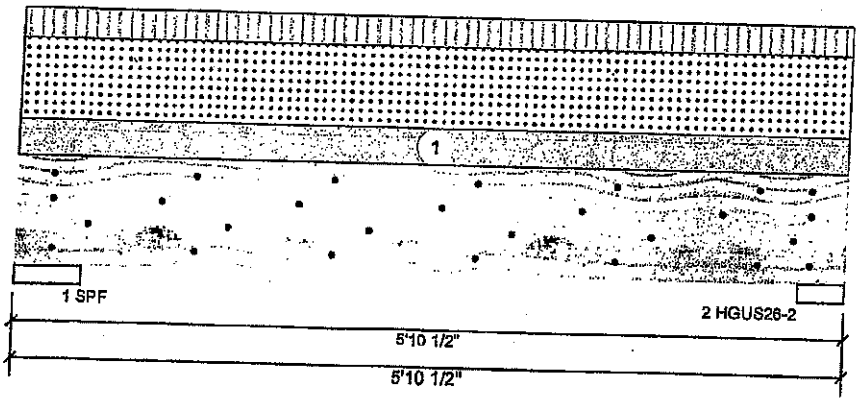
isDesign™

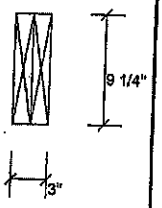
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	360
Importance:	Normal

Application:	Roof (Residential)
Slope:	0/12
Design Method:	LSD
Building Code:	NBCC 2015
Load Sharing:	No
Deck:	Not Checked
Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	403	499	1113	0
2	386	478	1067	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	27%	824 / 2072	2896	L	1.25D+1.5S +L
2 - HGUS...	4.000"	35%	598 / 1986	2584	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3047 ft-lb	3'	6038 ft-lb	0.505 (50%)	1.25D+1.5S +L	L
Unbraced	3047 ft-lb	3'	5236 ft-lb	0.582 (58%)	1.25D+1.5S +L	L
Shear	2340 lb	4'10"	3984 lb	0.587 (59%)	1.25D+1.5S +L	L
LL Defl inch	0.027 (L/2348)	3'	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.037 (L/1702)	3'	0.174 (L/360)	0.210 (21%)	D+S+0.5L	L

LICENSED PROFESSIONAL ENGINEER
03/19/19
H.J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 1905729
STRUCTURAL
COMPONENT ONLY
1/2

Design Notes

1 Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 8".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		12-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

This design is valid until 12/11/2021

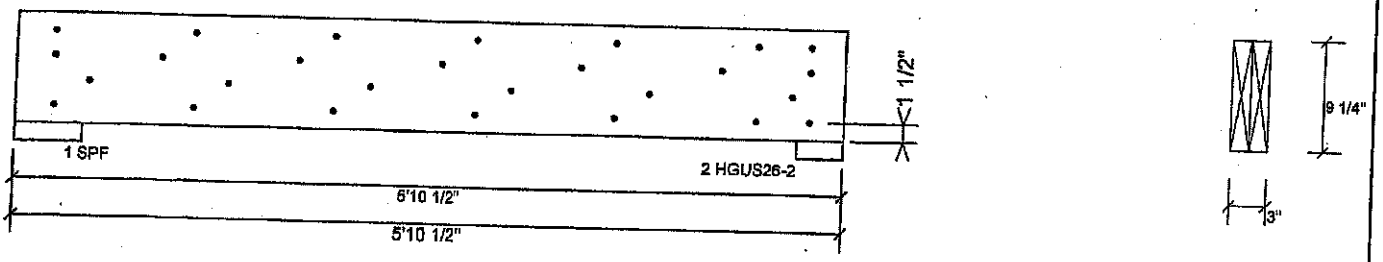
Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 385-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	99.1 %
Load	449.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
3/19/19
H/J. G. ALVES
100089024
PROVINCE OF ONTARIO

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

BUILDING NO. TAM 1905724
STRUCTURAL
ASSESSMENT ONLY 2/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

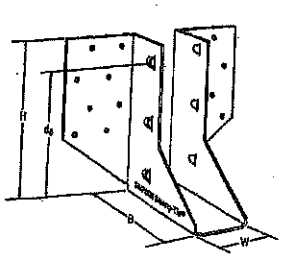
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

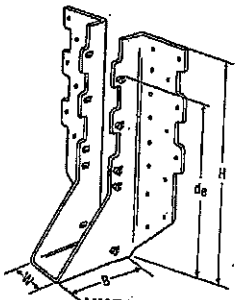
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

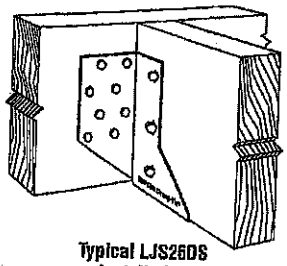
- See current catalogue for options



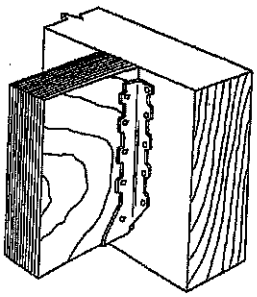
LJS26DS



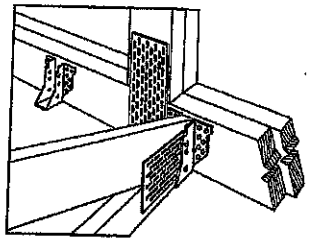
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



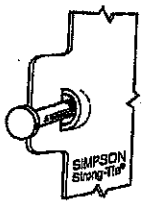
Typical HUS Installation



Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

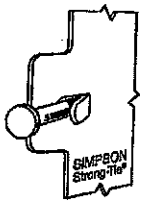
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅝	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7⁹⁄₃₂	3	6¾₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9⁹⁄₃₂	3	7³¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹⁹⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

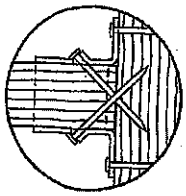


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

RECEIVED
JUN 25 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
01/13/2015
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

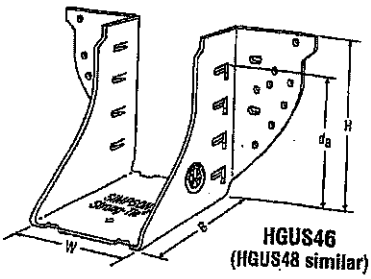
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

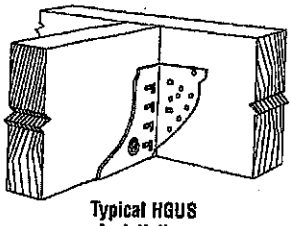
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



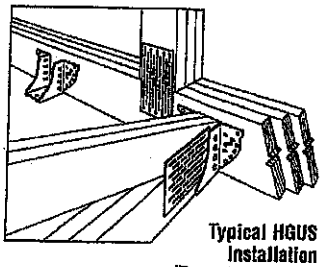
HGUS46
(HGUS48 similar)

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HGUS Installation



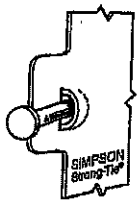
Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

OPTIONS:

- See current catalogue for options

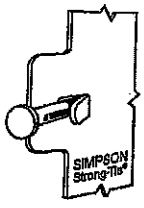
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS48	12	3⅝	7⅛	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS410	12	3⅝	9	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS412	12	3⅝	10⅞	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS414	12	3⅝	12⅞	4	11⅛	66-16d	22-16d	10130	16400	7195	11645
HGUS5.50/8	12	5½	6⅝	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS5.50/10	12	5½	8⅝	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS5.50/12	12	5½	10½	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS5.50/14	12	5½	12½	4	11⅛	66-16d	22-16d	10130	16400	7195	11645
HGUS7.25/8	12	7¼	7⅞	4	6⅞	36-16d	12-16d	6070	12980	4310	9215
HGUS7.25/10	12	7¼	8⅝	4	8¼	46-16d	16-16d	6840	15760	4855	11190
HGUS7.25/12	12	7¼	10⅝	4	10¼	56-16d	20-16d	7640	16110	5425	11435
HGUS7.25/14	12	7¼	12⅝	4	11¼	66-16d	22-16d	10130	18200	7195	12920

1. d_B is the distance from the seat of the hanger to the highest joist nail.

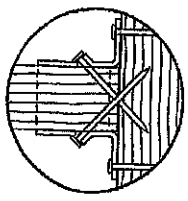


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

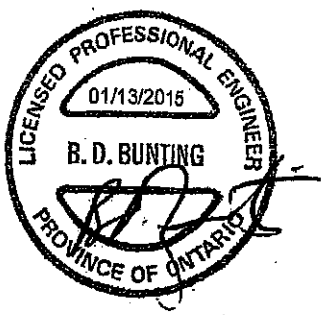
U.S. Patent 6,603,680



Double Shear Nailing Side View. Do not bend tab back.



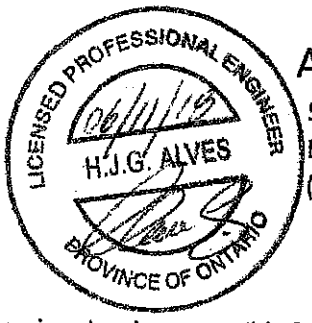
Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

800-999-5099
www.strongtie.com



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

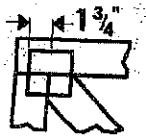
6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

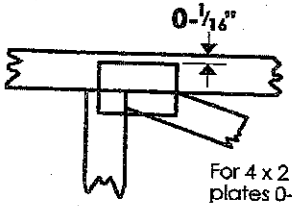
8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

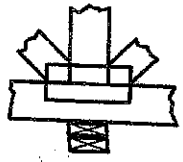
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

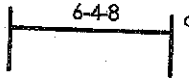
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

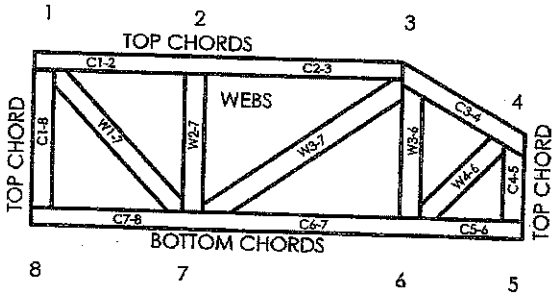
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

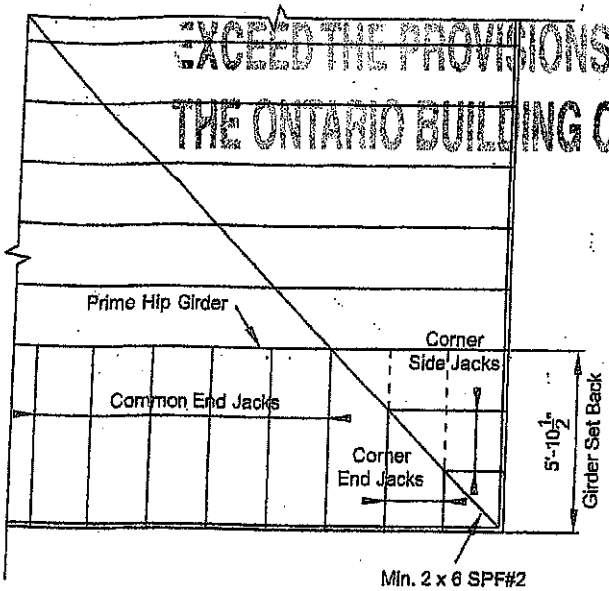
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

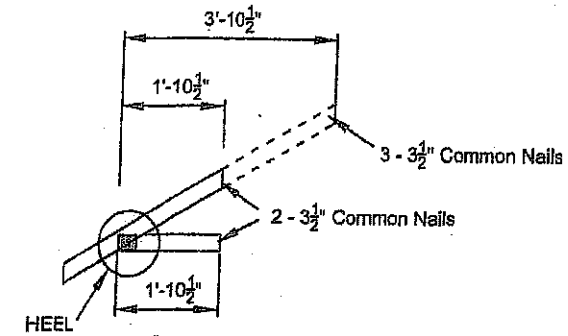
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

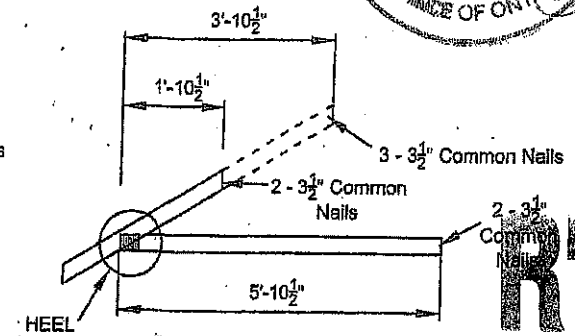


45° Hip End

LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO



Corner Side Jacks



Corner End Jacks

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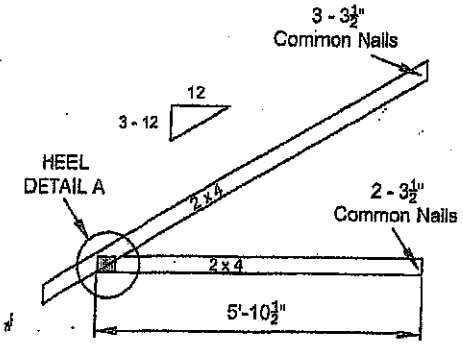
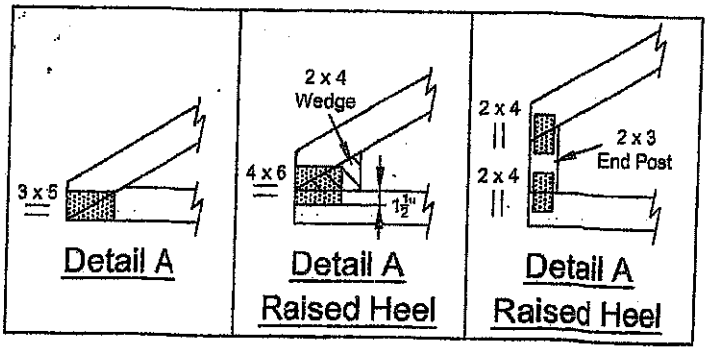
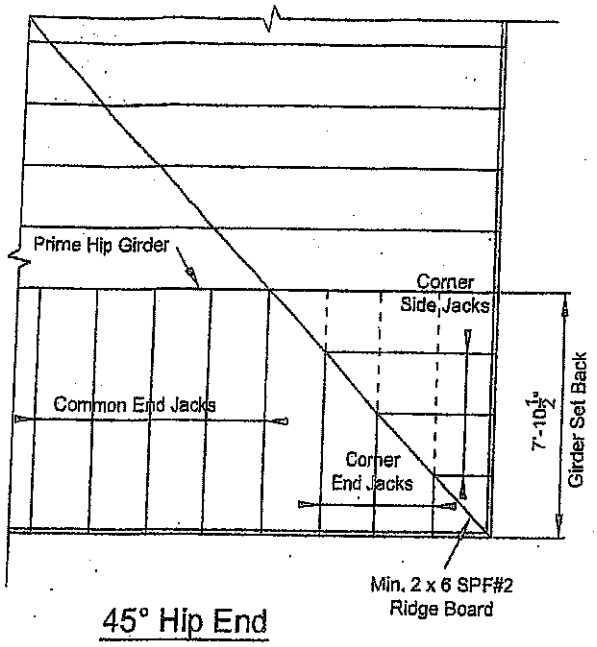


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1900216



45° Hip End

LUMBER SPECIFICATION

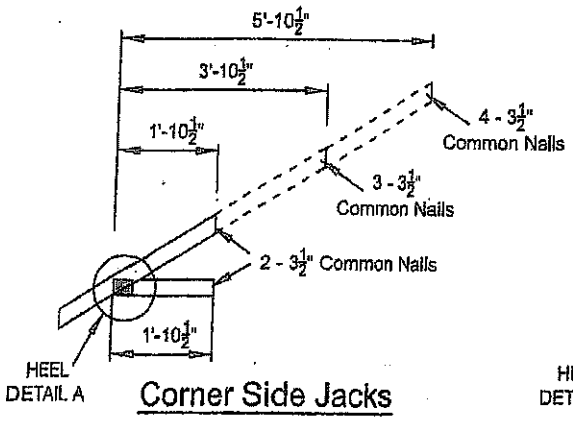
TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

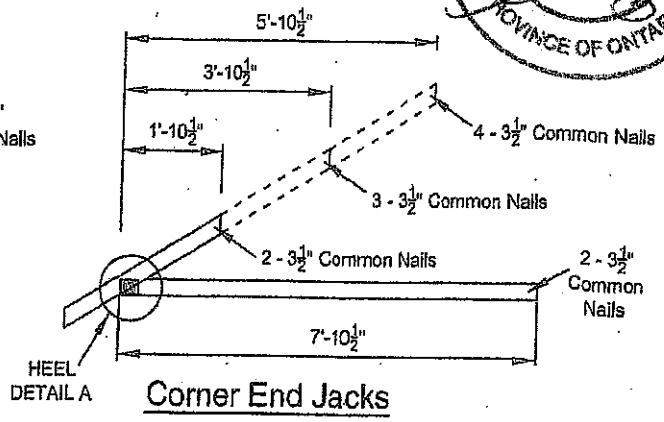
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

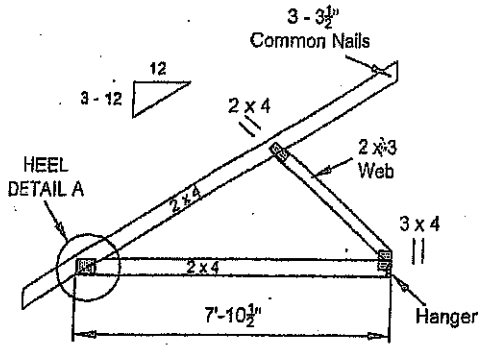
LICENSED PROFESSIONAL ENGINEER
26/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



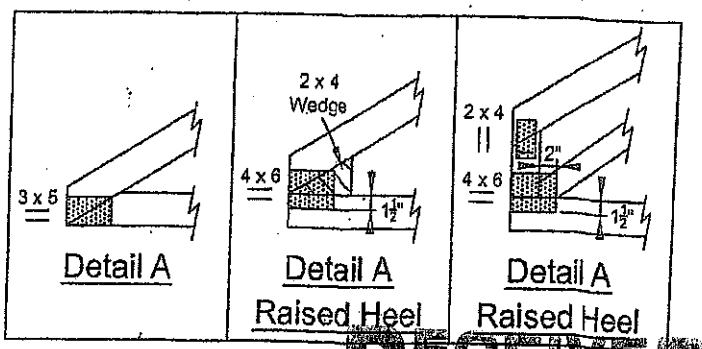
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A

Detail A Raised Heel

Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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