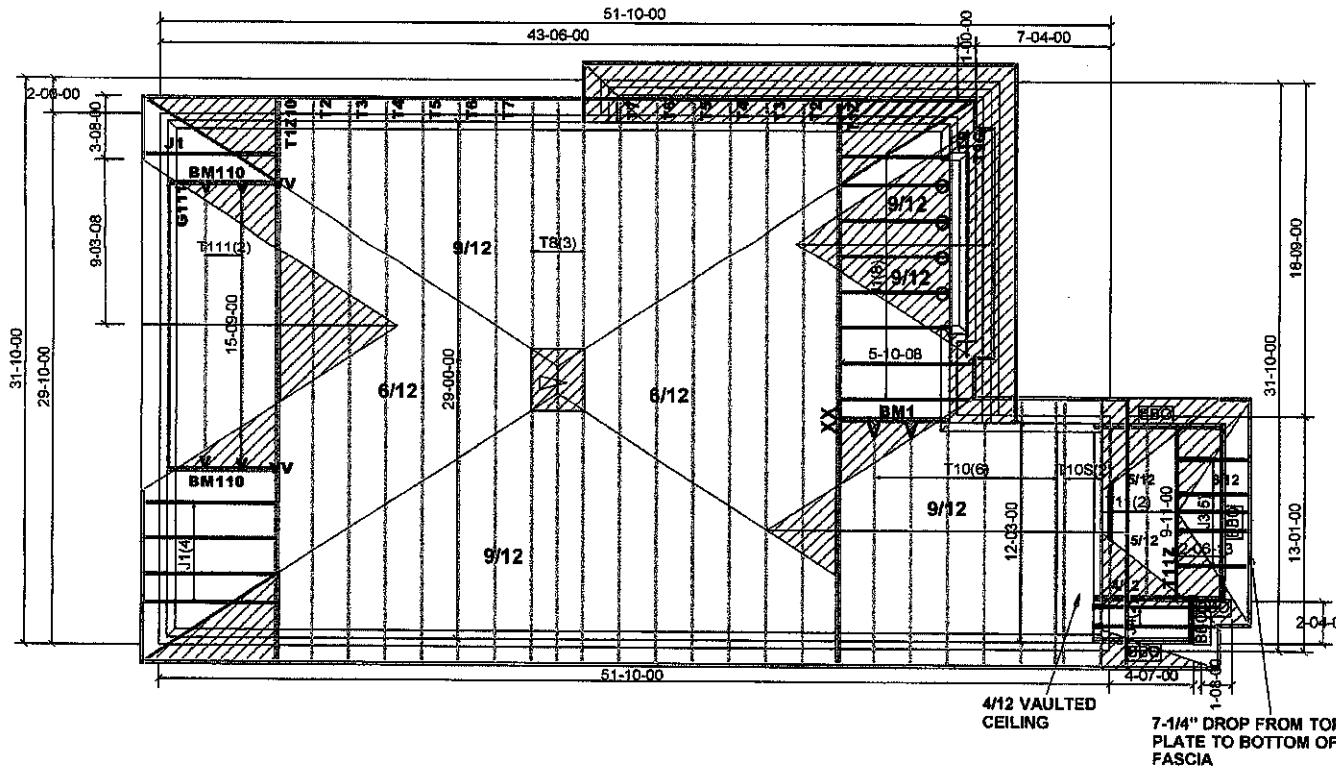


THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



TRUSS PROFILES TO BE VERIFIED BY
BUILDING DESIGNER

ALL CONVENTIONAL ROOF FRAMING TO
CONFORM WITH PART 9 OF THE OBC.
ROOF RAFTERS THAT MEET OR CROSS OVER
TRUSSES ARE TO BE 2"x4"SPF@24"O.C.
WITH 2"x4"SPF VERTICAL POST TO THE
TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally
BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS
OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: RESIDENTIAL | PART: 9

DESIGN LOADS:
CITY: CALEDON
G.S.L.= 37.6 psf
TC DL= 6 psf
BC LL= 10.50 psf
BC DL= 7.00 psf

NOTES:
FIN. OH: 12"
HEEL TYPE: R.T.M. CANT.
EXT. WALLS: 2X6
CLAD. TYPE 1: BRICK/5"
CLAD. TYPE 2: SIDING/0"
FSC SIZE: 2X6
SHEATHING: ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFER TO
SEALED TRUSS DOCS FOR UPLIFT DESIGN

HARDWARE:

LJS26DS (V) 2pcs
HGUS26-2 (XX) 1pcs
LUS24 (O) 4 pcs
LUS26-2(VV)

CONV FRM BY OTHERS

T- 180737

COMMENTS:

BM1,110 : 2-2"x10"
SPF #2



Job Track: **50120**
Plan Log: **200168**
Layout ID: **401596**

Builder / Location:
Greenpark / Caledon
Project: **Lamberts Lane Home Corp.**
Date: **2/26/2019** Sales: **Mario DiCano** Designer: **Brian/JG**

Model / Elevation:
preston 1 / 1-rear upgrade (4 bedroom)
THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED,
OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY
TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER
PURPOSE
Mitek ver 8.2.3.229



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 1
 Lot #:
 Elevation: 1-rear upgrade (4 bedroom)

Job Track: 50120
 PlanLog: 200168
 Layout ID: 401596
 Ref #
 Page: 1 of 2
 Date: 02/26/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	9 /12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	1 2-ply	T1210 Hip Girder	9 /12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	2	T2 Hip	9 /12	29-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	257.01 182.67		
	2	T3 Hip	9 /12	29-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.02 162.67		
	2	T4 Hip	9 /12	29-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	256.45 166.33		
	2	T5 Hip	9 /12	29-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.41 182.00		
	2	T6 Hip	9 /12	29-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.37 178.33		
	2	T7 Hip	9 /12	29-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	275.24 176.00		
	3	T8 Hip	9 /12	29-00-00	11-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	449.62 284.00		
	1	T9 Common Girder	9 /12	10-02-00	5-04-00	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	50.43 32.83		
	1	T9G GABLE	9 /12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	46.57 31.50		
	6	T10 Common	9 /12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	338.76 226.00		
	2	T10S Scissor	9 /12 4 /12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	108.28 72.67		
	2	T11 Common	5 /12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	63.43 38.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 1
 Lot #:
 Elevation: 1-rear upgrade (4 bedroom)

Job Track: 50120
 PlanLog: 200168
 Layout ID: 401596
 Ref #
 Page: 2 of 2
 Date: 02/26/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11Z Common Girder	5 /12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	31.71 19.00		
	2	T111 Common	9 /12	15-09-00	7-05-02	2 x 4		1-06-04 1-06-04	132.68 87.00		
	1	G111 GABLE	9 /12	15-09-00	7-05-02	2 x 4		1-06-04 1-06-04	70.69 46.50		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	5	J3 Jack-Open	8 /12	2-06-13	2-11-01	2 x 4	1-03-08	9-10 2-06-02	44.21 30.00		
	2	J4 Jack-Open	4 /12	5-05-08	2-07-03	2 x 4	1-05-00	3-15 2-01-12	29.2 18.67		

TOTAL # TRUSS= 55

TOTAL BFT OF ALL TRUSSES= 2404.84 BFT.

TOTAL WEIGHT OF ALL TRSSES 3749.95 LBS

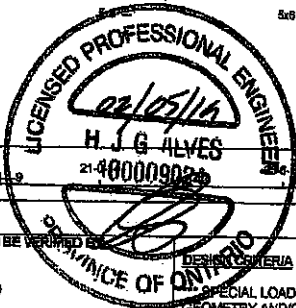
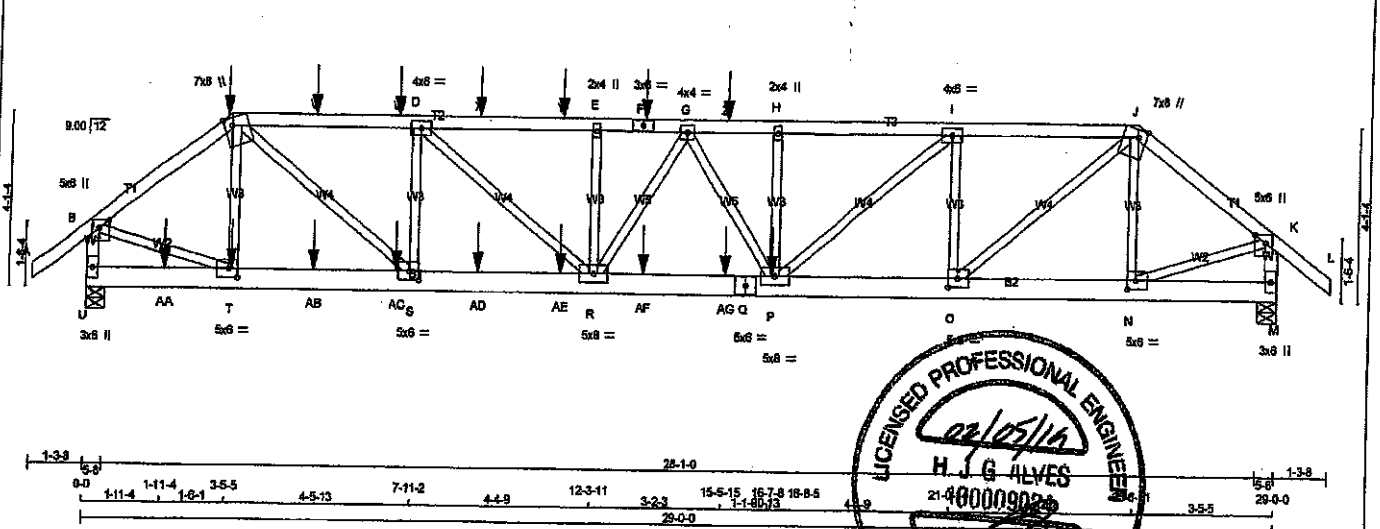
HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
4	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 12

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T1Z	1	2	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2016 Mitek Industries, Inc. Tue Feb 5 20:42:03 2019 Page 1
 ID:IFxHxH38FX8N7VhzQQYyQBwW-QFv4GB7J26EPB8vG8cRPYB2RUyJ0X3C63QYzo56
 1-3-8 0-0 3-5-5 3-5-5 4-5-13 7-11-2 4-4-9 12-3-11 14-4-0 15-5-15 16-8-5 21-0-14 4-5-13 25-0-11 3-5-5 29-0-0 30-3-8
 Scale = 1/50.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - F	2x4	DRY	No.2	SPF	
F - J	2x4	DRY	No.2	SPF	
J - L	2x4	DRY	No.2	SPF	
U - B	2x4	DRY	No.2	SPF	
M - K	2x4	DRY	No.2	SPF	
U - Q	2x6	DRY	No.2	SPF	
Q - M	2x6	DRY	No.2	SPF	
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-J	12	SIDE(61.0)
J-L	12	TOP
U-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
U-Q	12	SIDE(183.1)
Q-M	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
2x3	6	SIDE(176.7)
H-P	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	3.00	
C	TMVW+m	MT20	7.0	8.0	Edge	2.50	
D	TMVW-w	MT20	4.0	8.0			
E	TMVW+w	MT20	2.0	4.0			
F	TS-1	MT20	3.0	6.0			
G	TMVW-w	MT20	4.0	4.0			
H	TMVW-w	MT20	2.0	4.0			
I	TMVW-w	MT20	4.0	6.0			
J	TMVW+m	MT20	7.0	8.0	Edge	2.50	
K	TMVW+m	MT20	5.0	8.0	Edge	3.00	
L	BMV1+p	MT20	3.0	6.0			
M	BMVW-1	MT20	5.0	6.0	2.50	2.50	
N	BMVW-1	MT20	5.0	6.0	2.50	2.75	
O	BMVW-1	MT20	5.0	8.0			
P	BS-1	MT20	5.0	8.0			
Q	BMVW-1	MT20	5.0	8.0			
R	BMVW-1	MT20	5.0	8.0			
S	BMVW-1	MT20	5.0	6.0	2.50	2.75	
T	BMVW-1	MT20	5.0	6.0	2.50	2.50	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 3955	DOWN 3955	0	5-8
U	HORIZ 3425	HORIZ 3425	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM LIVE WIND DEAD SOIL
U	2835	1684 / 0 538 / 0 0 / 0 0 / 0 712 / 0 0 / 0
M	2540	1464 / 0 463 / 0 0 / 0 0 / 0 612 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	T-C	-718 / 0	0.08 (1)		
B-C	-4194 / 0	-102.1	-102.1	0.19 (1)	4.50	C-S	0 / 2879	0.46 (1)		
C-V	-6160 / 0	-102.1	-102.1	0.43 (1)	3.57	S-D	-2079 / 0	0.28 (1)		
V-W	-6160 / 0	-102.1	-102.1	0.43 (1)	3.57	D-R	0 / 1955	0.24 (1)		
W-D	-6160 / 0	-102.1	-102.1	0.43 (1)	3.57	R-E	-535 / 0	0.07 (1)		
D-X	-7649 / 0	-102.1	-102.1	0.52 (1)	3.14	P-H	-360 / 0	0.05 (1)		
X-Y	-7649 / 0	-102.1	-102.1	0.52 (1)	3.14	P-I	0 / 2779	0.34 (1)		
Y-E	-7649 / 0	-102.1	-102.1	0.30 (1)	3.34	O-I	-2308 / 0	0.28 (1)		
E-F	-7649 / 0	-102.1	-102.1	0.30 (1)	3.34	O-J	0 / 3390	0.48 (1)		
F-G	-7649 / 0	-102.1	-102.1	0.28 (1)	3.30	N-J	-607 / 0	0.09 (1)		
G-Z	-7982 / 0	-102.1	-102.1	0.28 (1)	3.30	B-T	0 / 3507	0.43 (1)		
Z-H	-7982 / 0	-102.1	-102.1	0.28 (1)	3.30	N-K	0 / 3008	0.37 (1)		
H-I	-7982 / 0	-102.1	-102.1	0.44 (1)	3.19	R-G	-490 / 0	0.08 (1)		
I-J	-5846 / 0	-102.1	-102.1	0.17 (1)	3.80	G-P	0 / 128	0.02 (1)		
J-K	-3595 / 0	-102.1	-102.1	0.17 (1)	4.81					
K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00					
U-B	-3891 / 0	0.0	0.0	0.22 (1)	5.98					
M-K	-3381 / 0	0.0	0.0	0.19 (1)	6.35					
U-AA	0 / 0	-38.5	-38.5	0.05 (3)	10.00					
AA-T	0 / 0	-38.5	-38.5	0.05 (3)	10.00					
T-AB	0 / 3327	-38.5	-38.5	0.28 (1)	10.00					
AB-AC	0 / 3327	-38.5	-38.5	0.28 (1)	10.00					
AC-S	0 / 3327	-38.5	-38.5	0.26 (1)	10.00					
S-AD	0 / 6161	-38.5	-38.5	0.46 (1)	10.00					
AD-AE	0 / 6161	-38.5	-38.5	0.46 (1)	10.00					
AE-R	0 / 6161	-38.5	-38.5	0.46 (1)	10.00					
R-AF	0 / 7897	-38.5	-38.5	0.60 (1)	10.00					
AF-AG	0 / 7897	-38.5	-38.5	0.60 (1)	10.00					
AG-Q	0 / 7897	-38.5	-38.5	0.60 (1)	10.00					
Q-P	0 / 7897	-38.5	-38.5	0.60 (1)	10.00					
P-O	0 / 5948	-38.5	-38.5	0.44 (1)	10.00					
O-N	0 / 2849	-38.5	-38.5	0.22 (1)	10.00					
N-M	0 / 0	-38.5	-38.5	0.04 (2)	10.00					

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-50	-56	-	-	FRONT	VERT	DEAD	-	-
C	3-5-5	-130	-130	-	-	BACK	VERT	TOTAL	-	-
C	3-5-5	-285	-285	-	-	FRONT	VERT	SNOW	-	-

SPECIAL LOADS ANALYSIS ---
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	6.0	PSF
SPECIFIED LOADS:	BOT CH.	LL	PSF
	DL	10.5	PSF
	DL	7.0	PSF
	TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC2010, CBC2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/998 (0.19")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.31")

CSF TC=0.52/1.00 (D-E-1), BC=0.80/1.00 (P-R-1), WB=0.48/1.00 (J-Q-1), SS=0.20/1.00 (G-D-1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES	PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1667
	768	1997	1658

PLATE PLACEMENT TOL = 0.250 inches

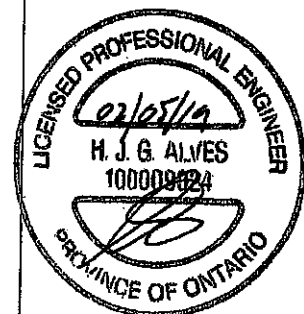
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL= 0.78 (C) (INPUT = 1.00)

JOB NAME 200168-400364	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. Preston 1	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MTek Industries, Inc. Tue Feb 5 20:42:03 2019 Page 2 ID:IFxHXfe38FX8N?VhzQQIYyQBwW-QFv4GB7i25EPB8wG8cRPYII2RUyJ0X3C63QYzo56		

PLATES (table in inches)
 JT TYPE PLATES W LEN Y X
 U BMV1+p MT20 3.0 6.0
 Edge - INDICATES REFERENCE CORNER OF PLATE
 TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	13-8-1	-123	-123	—	BACK	VERT	TOTAL	—	—
P	18-7-6	-1379	-1379	—	BACK	VERT	TOTAL	—	—
T	3-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
V	5-8-1	-123	-123	—	BACK	VERT	TOTAL	—	—
W	7-8-1	-123	-123	—	BACK	VERT	TOTAL	—	—
X	9-8-1	-123	-123	—	BACK	VERT	TOTAL	—	—
Y	11-8-1	-123	-123	—	BACK	VERT	TOTAL	—	—
Z	15-5-15	-123	-123	—	BACK	VERT	TOTAL	—	—
AA	1-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
AB	5-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
AC	7-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
AD	9-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
AE	11-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
AF	13-8-1	-55	-70	—	BACK	VERT	TOTAL	—	—
AG	15-5-15	-55	-70	—	BACK	VERT	TOTAL	—	—



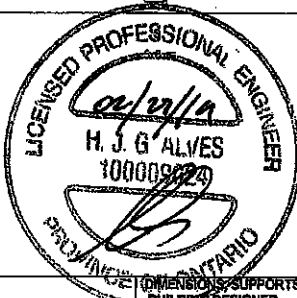
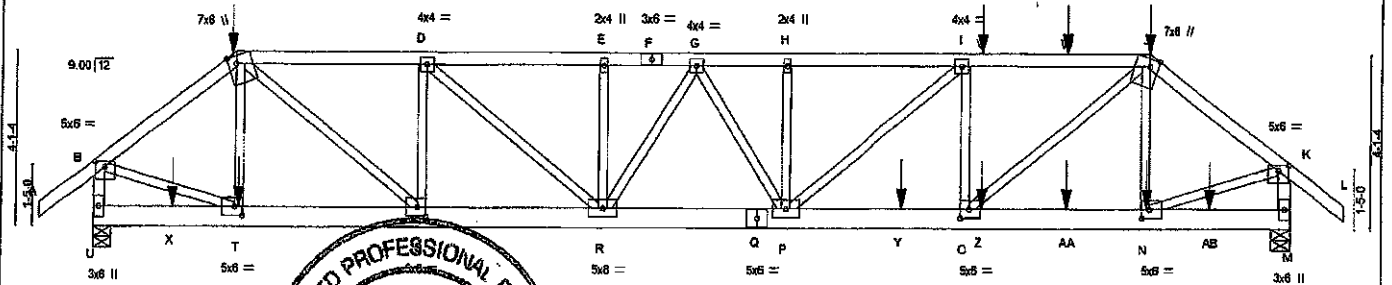
DWG NO. TAM **T1702646**
 STRUCTURAL
 COMPONENT ONLY **2/2**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401596	T1Z10	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MITek Industries, Inc. Wed Feb 27 17:51:53 2019 Page 1
ID:IFxHXfe38FX8N7VhzQQIYyQBwW-cj5fV3SRxf9J9BKp2omH9EN1JNNZqZionr4zgUXq

Scale = 1:50.0



TOTAL WEIGHT = 2 X 139 = 278 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - M	2x4	DRY	No.2
M - Q	2x6	DRY	No.2
Q - N	2x6	DRY	No.2
N - AB	2x6	DRY	No.2
AB	2x6	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-F	12	SIDE(61.0)
F-J	12	SIDE(61.0)
J-L	12	SIDE(61.0)
L-K	12	TOP
K-M	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
U-Q	12	SIDE(183.1)
Q-M	12	SIDE(183.1)
WEBS : (0.122'X3') SPIRAL NAILS		
T-C	6	SIDE(205.3)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	6.0	1.50	3.00
C TTWV+m	MT20	7.0	8.0	Edge	2.50
D G.I.					
D TMWV-I	MT20	4.0	4.0		
E TMWV-w	MT20	2.0	4.0		
F TS-I	MT20	3.0	6.0		
H TMWV-w	MT20	2.0	4.0		
I TTWV+m	MT20	7.0	8.0	Edge	2.50
K TMWV-p	MT20	5.0	6.0	1.50	3.00
M BMV-I-p	MT20	3.0	6.0		
N BMWV-I	MT20	5.0	6.0	2.50	2.25
O BMWV-I	MT20	5.0	6.0	2.50	2.50
P BMWV-I	MT20	5.0	6.0		
Q BS-I	MT20	5.0	6.0		
R BMWV-I	MT20	5.0	6.0		
S BMWV-I	MT20	5.0	6.0	2.50	2.50
T BMWV-I	MT20	5.0	6.0	2.50	2.25
U BMV-I-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQ'D BRG
U	4484	4484	5-8	5-8
M	4193	4193	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN SNOW	MAX/MIN LIVE	MAX/MIN PERM. LIVE	MAX/MIN WIND	MAX/MIN DEAD	MAX/MIN SOIL
U	3301	1931 / 0	582 / 0	0 / 0	0 / 0	788 / 0	0 / 0
M	3107	1797 / 0	569 / 0	0 / 0	0 / 0	750 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO	A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	T-C	0 / 483	0.06 (2)
	B-C	-4751 / 0	-102.1	-102.1	0.21 (1)	4.25	C-S	0 / 2822	0.32 (1)
	C-D	-5824 / 0	-102.1	-102.1	0.31 (1)	3.81	S-D	-1450 / 0	0.18 (1)
	D-E	-6938 / 0	-102.1	-102.1	0.37 (1)	3.47	D-R	0 / 1463	0.18 (1)
	E-F	-6938 / 0	-102.1	-102.1	0.22 (1)	3.59	R-H	-330 / 0	0.04 (1)
	F-G	-6938 / 0	-102.1	-102.1	0.22 (1)	3.59	P-H	-294 / 0	0.04 (1)
	G-H	-7534 / 0	-102.1	-102.1	0.23 (1)	3.45	P-I	0 / 998	0.12 (1)
	H-I	-7534 / 0	-102.1	-102.1	0.46 (1)	3.24	O-I	-1349 / 0	0.17 (1)
	I-V	-8774 / 0	-102.1	-102.1	0.45 (1)	3.41	O-J	0 / 4233	0.52 (1)
	V-W	-8774 / 0	-102.1	-102.1	0.45 (1)	3.41	N-J	-942 / 0	0.12 (1)
	W-J	-8774 / 0	-102.1	-102.1	0.45 (1)	3.41	B-T	0 / 3972	0.49 (1)
	J-K	-4438 / 0	-102.1	-102.1	0.20 (1)	4.39	N-K	0 / 3709	0.48 (1)
	K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00	R-G	-899 / 0	0.11 (1)
	U-B	-4385 / 0	0.0	0.0	0.25 (1)	5.70	G-P	0 / 479	0.06 (1)
	M-K	-4097 / 0	0.0	0.0	0.23 (1)	5.86			
	U-X	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
	X-T	0 / 0	-38.5	-38.5	0.05 (2)	10.00			
	T-S	0 / 3805	-38.5	-38.5	0.28 (1)	10.00			
	S-R	0 / 5824	-38.5	-38.5	0.45 (1)	10.00			
	R-Q	0 / 7292	-38.5	-38.5	0.59 (1)	10.00			
	Q-P	0 / 7292	-38.5	-38.5	0.59 (1)	10.00			
	P-Y	0 / 6775	-38.5	-38.5	0.87 (1)	10.00			
	Y-O	0 / 6775	-38.5	-38.5	0.87 (1)	10.00			
	O-Z	0 / 3514	-38.5	-38.5	0.39 (1)	10.00			
	Z-AA	0 / 3514	-38.5	-38.5	0.39 (1)	10.00			
	AA-N	0 / 3514	-38.5	-38.5	0.39 (1)	10.00			
	N-AB	0 / 0	-38.5	-38.5	0.04 (2)	10.00			
	AB-M	0 / 0	-38.5	-38.5	0.04 (2)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-6	-50	-56	FRONT	VERT	DEAD	---	---
C	3-5-6	-285	-285	FRONT	VERT	SNOW	---	---
J	25-6-11	-50	-56	FRONT	VERT	DEAD	---	---
J	25-6-11	-130	-130	FRONT	VERT	TOTAL	---	---
J	25-6-11	-285	-285	FRONT	VERT	SNOW	---	---
N	25-6-15	-55	-70	FRONT	VERT	TOTAL	---	---
T	3-6-8	-1489	-1489	FRONT	VERT	TOTAL	---	---
V	21-5-15	-123	-123	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL)= L/999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL)= L/990 (0.29")

CSI: TC=0.46/1.00 (H-I), BC=0.87/1.00 (O-P), WB=0.52/1.00 (J-O), SS=0.38/1.00 (O-P)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

PLATE GRIP (DRY)	PLATE GRIP (WET)	PLATE GRIP (SHEAR)	PLATE GRIP (TENS)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (O) (INPUT = 0.90)
JSI METAL= 0.70 (Q) (INPUT = 1.00)

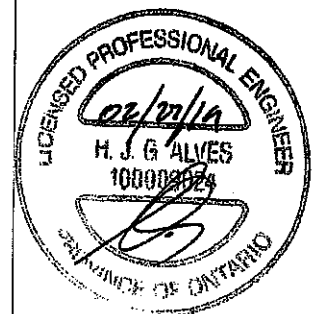
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401596	T1Z10	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MTek Industries, Inc. Wed Feb 27 17:51:53 2019 Page 2 ID:IFxHXfe38FX8N7VhzQQIYyQBwW-cj5M3SRxfj9J9BKo2ommH9EN1JNNZqZlonr4zg0Xg		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	23-5-15	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	1-11-4	-55	-70	---	FRONT	VERT	TOTAL	---	---
Y	19-6-8	-1489	-1489	---	FRONT	VERT	TOTAL	---	---
Z	21-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AA	23-5-15	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	27-0-12	-55	-70	---	FRONT	VERT	TOTAL	---	---

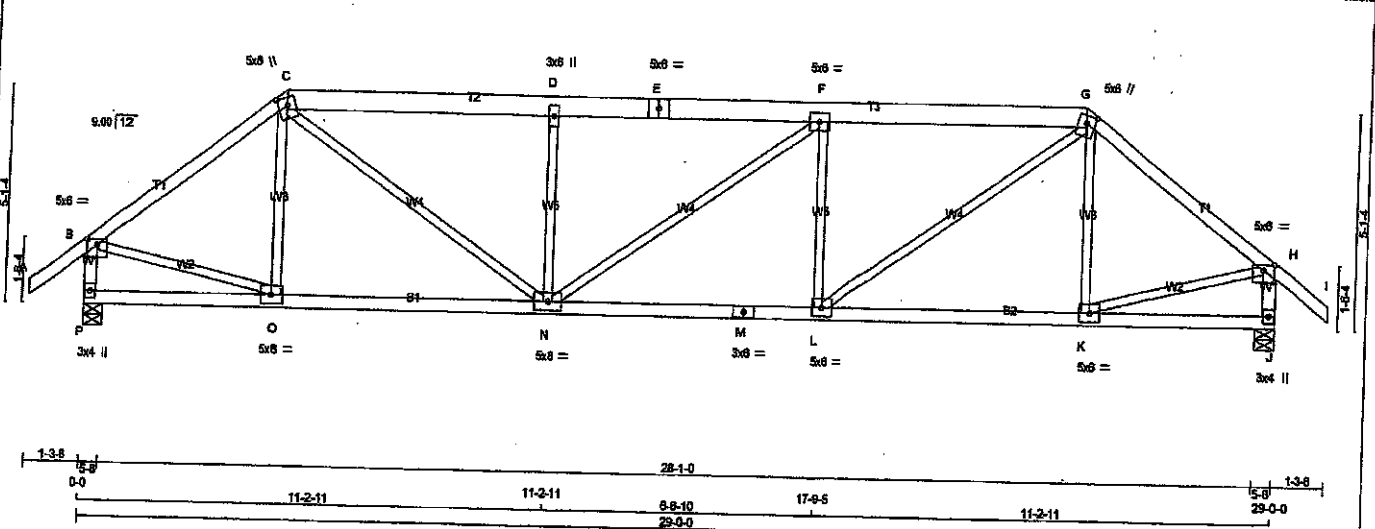
RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO



DRWG NO. TAM 77904519
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME 200168-400364	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. Preston 1	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington						

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:04 2019 Page 1
 ID: IFxHXfe38FX8N7VhzQQIYyQBwW-uRSSUX8LoPMGoiH5qr7gxmiw8SmjhAVWglady_2056H
 1-3-8 0-0 11-2-11 11-2-11 8-8-10 17-9-5 11-2-11 28-0-0 30-3-8
 Scale = 1:50.0



TOTAL WEIGHT = 2 X 129 = 257 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x6	DRY	No.2
E - G	2x6	DRY	No.2
G - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - M	2x4	DRY	No.2
M - L	2x4	DRY	No.2
L - K	2x4	DRY	No.2
K - J	2x4	DRY	No.2
J - P	2x4	DRY	No.2
P - Q	2x4	DRY	No.2
Q - R	2x4	DRY	No.2
R - S	2x4	DRY	No.2
S - T	2x4	DRY	No.2
T - U	2x4	DRY	No.2
U - V	2x4	DRY	No.2
V - W	2x4	DRY	No.2
W - X	2x4	DRY	No.2
X - Y	2x4	DRY	No.2
Y - Z	2x4	DRY	No.2
Z - A	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.50	3.00
C	TTWV+m	MT20	5.0	6.0	2.25	2.50
D	TMWV-w	MT20	3.0	6.0		
E	TS-	MT20	5.0	6.0		
F	TMWV-l	MT20	5.0	6.0		
G	TTWV+m	MT20	5.0	6.0	2.25	2.50
H	TMWV-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K, L, O						
K	BMWVW4	MT20	5.0	6.0		
M	BS-1	MT20	3.0	6.0		
N	BMWVW4	MT20	5.0	6.0		
P	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	VERT	HORZ	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
P	2180	0	2180	0	5-8
J	2180	0	2180	0	5-8

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
P	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0
J	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.06 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO					FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	O-C	-201 / 98
B-C	-2157 / 0	-102.1	-102.1	0.54 (1)	4.06	K-G	-201 / 98
C-D	-3071 / 0	-102.1	-102.1	0.32 (1)	4.52	B-O	0 / 1786
D-E	-3071 / 0	-102.1	-102.1	0.32 (1)	4.52	K-H	0 / 1786
E-F	-3071 / 0	-102.1	-102.1	0.32 (1)	4.52	L-G	0 / 1881
F-G	-3071 / 0	-102.1	-102.1	0.32 (1)	4.52	C-N	0 / 1881
G-H	-2157 / 0	-102.1	-102.1	0.54 (1)	4.06	L-F	-732 / 0
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	N-D	-732 / 0
P-B	-2115 / 0	0.0	0.0	0.22 (1)	5.83	N-F	0 / 1
J-H	-2115 / 0	0.0	0.0	0.22 (1)	5.83		
P-O	0 / 0	-38.5	-38.5	0.22 (3)	10.00		
O-N	0 / 1734	-38.5	-38.5	0.47 (2)	10.00		
N-M	0 / 3071	-38.5	-38.5	0.67 (1)	10.00		
M-L	0 / 3071	-38.5	-38.5	0.67 (1)	10.00		
L-K	0 / 1734	-38.5	-38.5	0.47 (2)	10.00		
K-J	0 / 0	-38.5	-38.5	0.22 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (LL) = $1/869$ (0.14")
ALLOWABLE DEFL. (TL) = $L/360$ (0.97")
CALCULATED VERT. DEFL. (TL) = $1/869$ (0.23")

CSI: TC=0.54/1.00 (B-C-1), BC=0.67/1.00 (L-N-1), WB=0.40/1.00 (H-K-1), SS=0.23/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

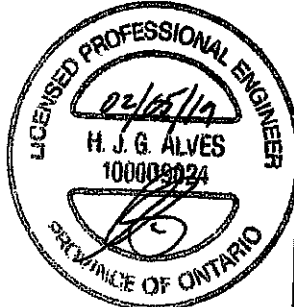
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

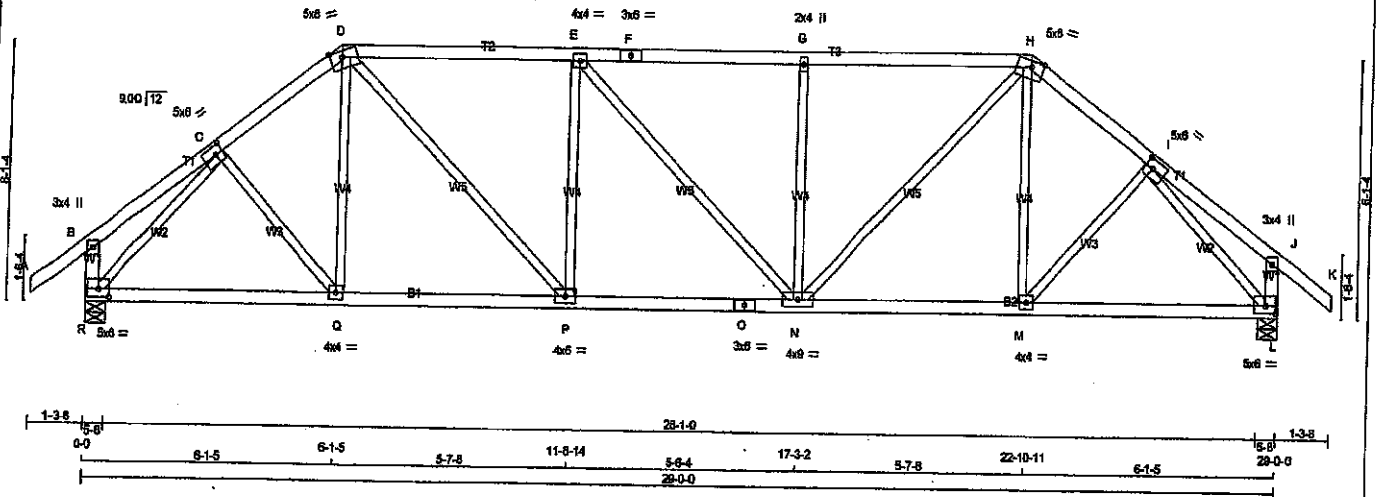
JSI GRIP= 0.80 (Q) (INPUT = 0.80)
JSI METAL= 0.86 (M) (INPUT = 1.00)



DWG NO. TAM 7140 2647
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.250 S Nov 17 2018 Mitak Industries, Inc. Tue Feb 5 20:42:05 2019 Page 1
 ID:IFxHXe38FX8N?VhzQQIYyQBwW-Nd0qhtszZU7QSeINZevUzr44s8LQXGpXWbAUQzo56G
 1-3-8 0-0 3-1-15 3-1-15 2-11-7 6-1-5 5-7-8 11-8-14 5-8-4 17-3-2 5-7-8 22-10-11 2-11-7 25-10-1 3-1-15 1-3-8 28-0-0 30-3-8
 Scale = 1/8" = 1'-0"



TOTAL WEIGHT = 2 X 127 = 254 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
R - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-1	MT20	5.0	6.0	2.50	2.25
D TTWW-m	MT20	5.0	8.0	Edge	3.50
E TMVW-1	MT20	4.0	4.0		
F TS-1	MT20	3.0	6.0		
G TMVW-w	MT20	2.0	4.0		
H TTWW-m	MT20	5.0	8.0	Edge	3.50
I TMVW-1	MT20	5.0	8.0	2.50	2.25
J TMV+p	MT20	3.0	4.0		
L BMVW-1	MT20	5.0	6.0	2.25	3.00
M BMVW-1	MT20	4.0	4.0		
N BMVW-1	MT20	4.0	9.0		
O BS-1	MT20	3.0	6.0		
P BMVW-1	MT20	4.0	6.0		
Q BMVW-1	MT20	4.0	4.0		
R BMVW-1	MT20	5.0	6.0	2.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
R	2180	0	2180	0	5-8	5-8
L	2180	0	2180	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	SNOW	LIVE	PERM	LIVE			
R	1820	922/0	365/0	0/0	0/0	394/0	0/0
L	1820	922/0	365/0	0/0	0/0	394/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO				
FR-TO					FR-TO		
A-B	0/42	-102.1	-102.1	0.14 (1)	C-Q	0/212	0.05 (2)
B-C	0/18	-102.1	-102.1	0.12 (1)	Q-D	0/202	0.05 (3)
C-D	-2175/0	-102.1	-102.1	0.18 (1)	D-P	0/1140	0.26 (1)
D-E	-2517/0	-102.1	-102.1	0.59 (1)	P-E	-815/0	0.36 (1)
E-F	-2516/0	-102.1	-102.1	0.59 (1)	E-N	-2/0	0.00 (1)
F-G	-2516/0	-102.1	-102.1	0.59 (1)	N-G	-814/0	0.36 (1)
G-H	-2515/0	-102.1	-102.1	0.59 (1)	H-N	0/1137	0.26 (1)
H-I	-2175/0	-102.1	-102.1	0.18 (1)	M-H	0/202	0.05 (3)
I-J	0/18	-102.1	-102.1	0.12 (1)	M-I	0/212	0.05 (2)
J-K	0/42	-102.1	-102.1	0.14 (1)	R-C	-2413/0	0.82 (1)
R-B	-268/0	0.0	0.0	0.03 (1)	I-L	-2413/0	0.82 (1)
L-J	-268/0	0.0	0.0	0.03 (1)			
R-Q	0/1589	-38.5	-38.5	0.48 (2)			
Q-P	0/1723	-38.5	-38.5	0.49 (2)			
P-O	0/2517	-38.5	-38.5	0.51 (1)			
O-N	0/2517	-38.5	-38.5	0.51 (1)			
N-M	0/1723	-38.5	-38.5	0.49 (2)			
M-L	0/1589	-38.5	-38.5	0.48 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 CL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 CL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA C88-09, CSA C88-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/860 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/959 (0.11")
 ALLOWABLE DEFL.(TL) = L/860 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/959 (0.20")

CSI: TC=0.59/1.00 (D-E-1), BC=0.51/1.00 (N-P-1), WB=0.82/1.00 (L-1), SS=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

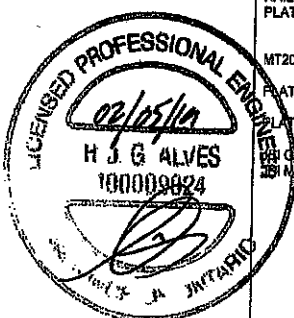
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP=0.87 (L) (INPUT = 0.90)
 METAL=0.78 (C) (INPUT = 1.00)



DWG NO. TAM T1902648
 STRUCTURAL
 COMPONENT ONLY

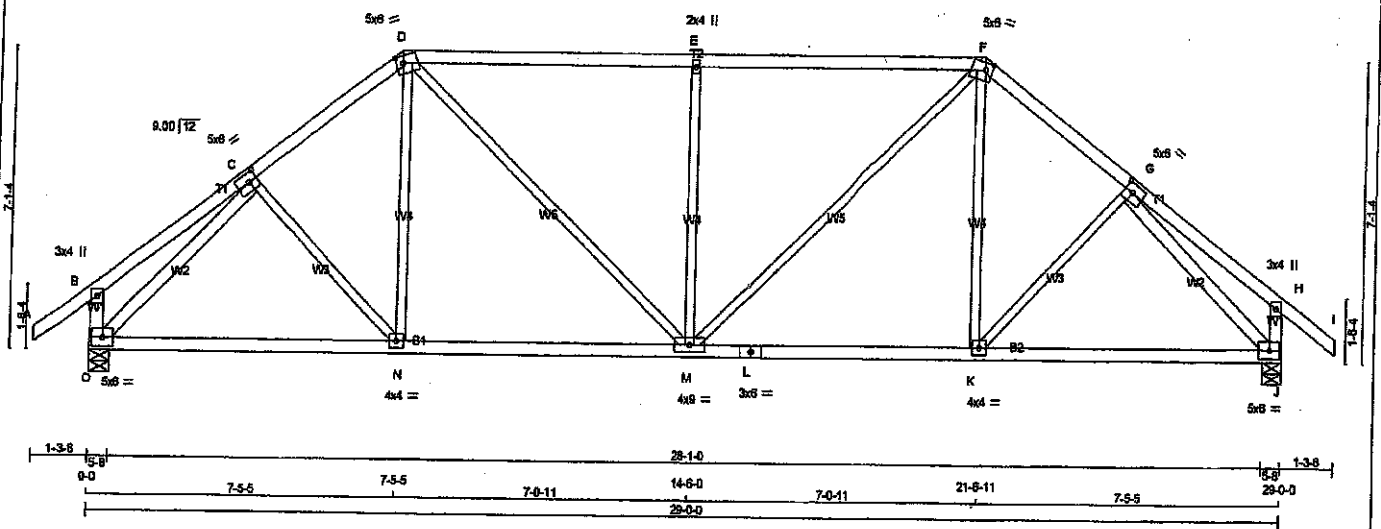
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T4	2	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 M/Tek Industries, Inc. Tue Feb 5 20:42:05 2019 Page 1

ID:IFxFHxFe38FX8N7VhzQQIYyQBwW-Nd0qht8zJlU7QsSfNZsvUzr1Ms7gQX5pXWbAUQzo56g

1-3-8 0-0 3-9-15 3-9-15 3-7-7 7-5-6 7-9-11 14-8-0 7-9-11 21-6-11 3-7-7 25-2-1 3-9-15 29-0-0 30-3-8

Scale: 1/8" = 1'-0"



LUMBER				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
O - H	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)						
JT	PLATE	PLATES	W	LEN	Y	X
B	TMW+p	M120	3.0	4.0		
C	TMWV-t	M120	5.0	8.0	2.50	2.50
D	TTWV-m	M120	5.0	8.0	2.00	2.00
E	TMW+w	M120	2.0	4.0		
F	TTWV-m	M120	5.0	6.0	2.00	2.00
G	TMWV-t	M120	5.0	6.0	2.50	2.50
H	TMV+p	M120	3.0	4.0		
J	BMWV-1	M120	5.0	6.0		
K	BMWV-t	M120	4.0	4.0		
L	BS-t	M120	3.0	6.0		
M	BMWVWV-t	M120	4.0	9.0		
N	BMWV-1	M120	4.0	4.0		
O	BMWV-1-t	M120	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2180	0	2180	0	0	5-8	5-8
J	2180	0	2180	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST LCASE		MAX. MIN. COMPONENT REACTIONS					
JO	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
T	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O. J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBB				
MEMB.	MAX. FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. CSH (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	FACTORED MAX CSH (LC)	
FR-TO		FROM TO			FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	C-N	0/129	0.03 (3)
B-C	0/25	-102.1	-102.1	0.21 (1)	10.00	N-D	0/343	0.08 (2)
C-D	-2143/0	-102.1	-102.1	0.22 (1)	4.47	D-M	0/807	0.18 (1)
D-E	-2276/0	-102.1	-102.1	0.77 (1)	3.54	M-E	-284/0	0.77 (1)
E-F	-2276/0	-102.1	-102.1	0.77 (1)	3.54	M-F	0/807	0.18 (1)
F-G	-2143/0	-102.1	-102.1	0.22 (1)	4.47	K-F	0/343	0.08 (2)
G-H	0/25	-102.1	-102.1	0.21 (1)	10.00	K-G	0/129	0.03 (3)
H-I	0/42	-102.1	-102.1	0.14 (1)	10.00	Q-C	-2427/0	0.83 (1)
O-B	-288/0	0.0	0.0	0.03 (1)	7.81	G-J	-2427/0	0.83 (1)
J-H	-288/0	0.0	0.0	0.03 (1)	7.81			
C-N	0/1655	-38.5	-38.5	0.81 (2)	10.00			
N-M	0/1895	-38.5	-38.5	0.62 (2)	10.00			
M-L	0/1895	-38.5	-38.5	0.62 (2)	10.00			
L-K	0/1895	-38.5	-38.5	0.62 (2)	10.00			
K-J	0/1655	-38.5	-38.5	0.81 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		=	52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A
SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.77/1.00 (E-F:1), BC=0.62/1.00 (M-N:2),
WB=0.83/1.00 (C-Q:1), SS=0.35/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

DATE	SHR (DRT)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1867	786	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

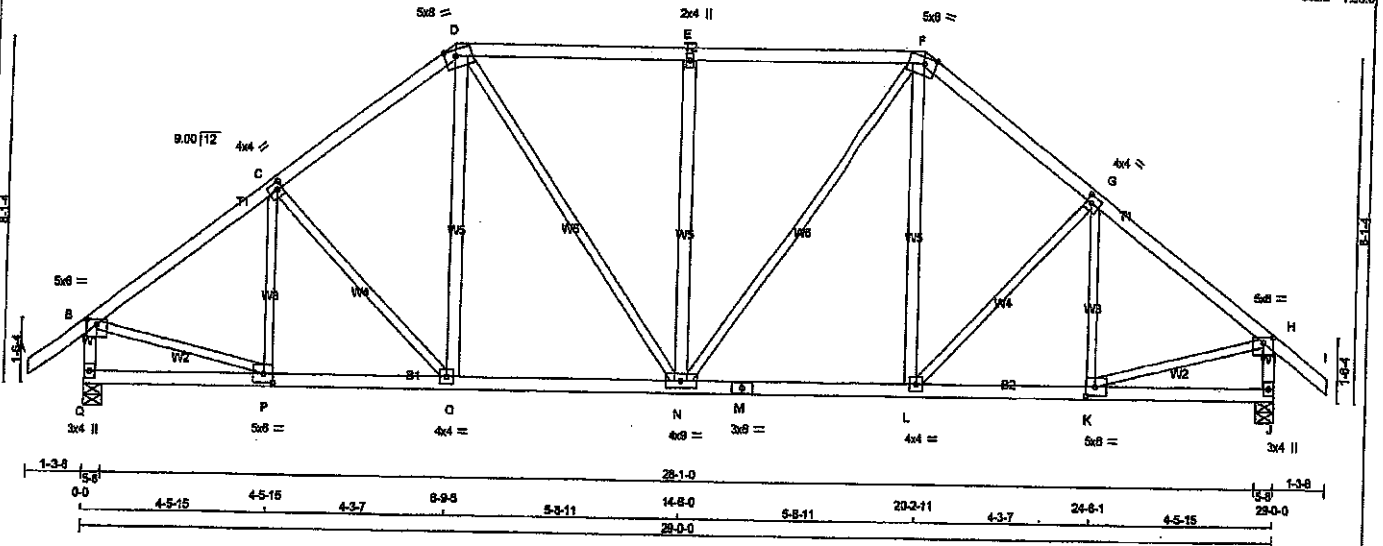
PLATE ROTATION TOL = 5.0 Deg.

US! GRIP= 0.89 (D) (INPUT = 0.90)
US! METAL= 0.60 (C) (INPUT = 1.00)

DWG NO. TAM T1902649
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. Preston 1	DRWG NO.
Famerack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2018 MITek Industries, Inc. Tue Feb 5 20:42:08 2019 Page 1
 ID:1FxFHXfe38FX8N7VhzQQIYyQBwW-rqaCuD9bK0c_2cRUxG980BNHZGVH90Cz1ALK1tzo56F
 1-3-8 0-0 4-5-15 4-5-15 4-3-7 8-9-5 5-5-11 14-8-0 5-5-11 20-2-11 4-3-7 24-6-1 4-5-15 29-0-0 1-3-8
 Scale = 1:50.0



TOTAL WEIGHT = 2 X 142 = 284 lb

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
O - D	2x4	DRY	No.2
N - E	2x4	DRY	No.2
L - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWV-m	MT20	5.0	8.0	1.75	3.25
E	TMVW-t	MT20	2.0	4.0		
F	TTWV-m	MT20	5.0	8.0	1.75	3.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
		GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	UPLIFT	IN-SX
Q	2180	0	2180	0	5-8
J	2180	0	2180	0	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
Q	1820	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0
J	1820	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.28 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	P-C	-288 / 24
B-C	-2188 / 0	C-O	-219 / 0
C-D	-2045 / 0	O-D	0 / 376
D-E	-1956 / 0	D-N	0 / 579
E-F	-1956 / 0	N-E	-713 / 0
F-G	-2045 / 0	N-F	0 / 579
G-H	-2188 / 0	L-F	0 / 376
H-I	0 / 42	L-G	-219 / 0
Q-B	-2108 / 0	K-G	-288 / 24
J-H	-2108 / 0	B-P	0 / 1820
		K-H	0 / 1820
Q-P	0 / 0		
P-O	0 / 1799		
O-N	0 / 1613		
N-M	0 / 1613		
M-L	0 / 1613		
L-K	0 / 1799		
K-J	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
 CALCULATED VERT. DEFL.(LL) = L/998 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (0.87")
 CALCULATED VERT. DEFL.(TL) = L/998 (0.14")

CSI: TC=0.48/1.00 (D-E-1), BC=0.40/1.00 (N-O-2),
 WB=0.84/1.00 (E-N-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

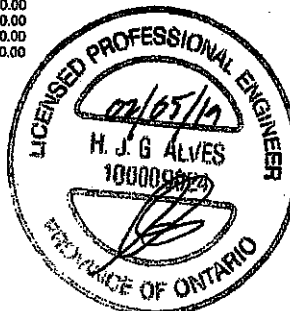
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION
 (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

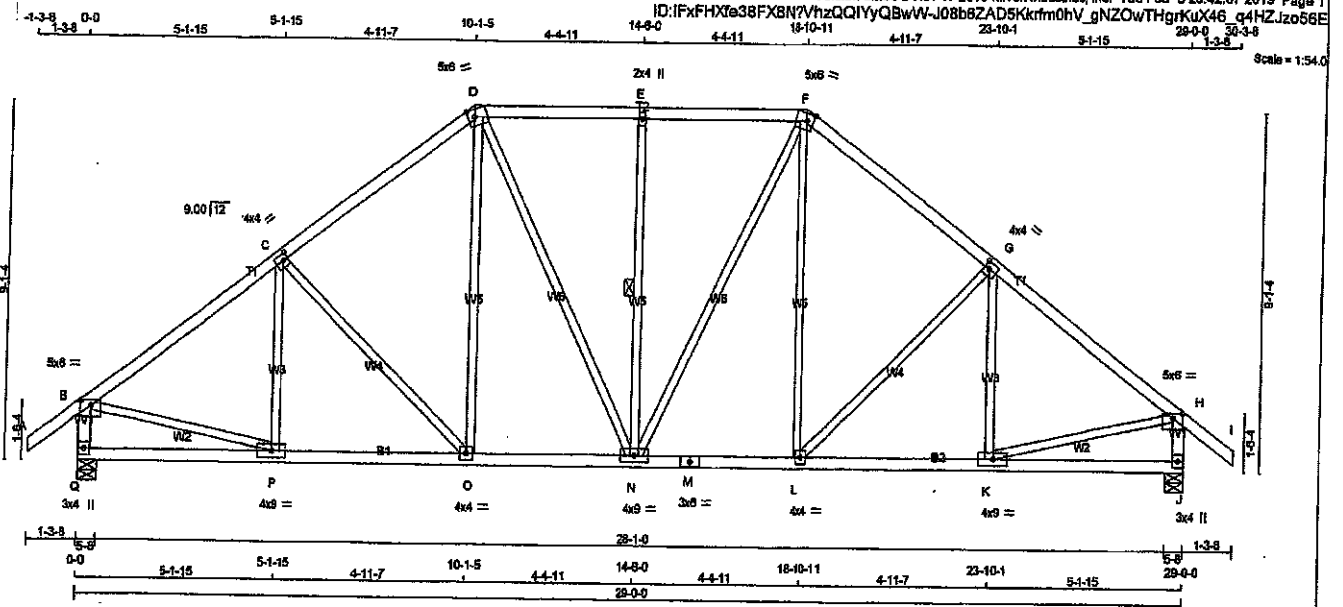
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.80)
 JSI METAL= 0.51 (M) (INPUT = 1.00)



DWG NO. TAM T5102650
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T6	2	1	Preston 1	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 2 X 142 = 283 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMWW-p	MT20	5.0	6.0	1.50 3.00
C TMWW-t	MT20	4.0	4.0	2.00 1.50
D TTWW-m	MT20	5.0	6.0	Edge 2.00
E TMWW-w	MT20	2.0	4.0	
F TTWW-m	MT20	5.0	6.0	Edge 2.00
G TMWW-t	MT20	4.0	4.0	2.00 1.50
H TMWW-p	MT20	5.0	6.0	1.50 3.00
J BMV1+p	MT20	3.0	4.0	
K BMWW-t	MT20	4.0	9.0	
L BMWW-t	MT20	4.0	4.0	
M BS-4	MT20	3.0	6.0	
N BMWW-t	MT20	4.0	9.0	
O BMWW-t	MT20	4.0	4.0	
P BMWW-t	MT20	4.0	9.0	
Q BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
Q	2180	0	2180	0	0	5-8	5-8	5-8	5-8
J	2180	0	2180	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERMIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH FR-TO (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LC)	MAX. UNBRACED LENGTH FR-TO (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00	P-C	-194 / 98	0.08 (1)
B-C	-2198 / 0	-102.1	-102.1 0.42 (1)	4.21	C-O	-364 / 0	0.38 (1)
C-D	-1954 / 0	-102.1	-102.1 0.39 (1)	4.44	O-D	0 / 444	0.10 (2)
D-E	-1709 / 0	-102.1	-102.1 0.28 (1)	4.82	D-N	0 / 389	0.09 (1)
E-F	-1709 / 0	-102.1	-102.1 0.28 (1)	4.82	N-E	-538 / 0	0.29 (1)
F-G	-1954 / 0	-102.1	-102.1 0.39 (1)	4.44	N-F	0 / 389	0.08 (1)
G-H	-2198 / 0	-102.1	-102.1 0.42 (1)	4.21	L-F	0 / 444	0.10 (2)
H-I	0 / 42	-102.1	-102.1 0.14 (1)	10.00	L-G	-364 / 0	0.38 (1)
Q-B	-2098 / 0	0.0	0.0 0.22 (1)	5.84	K-G	-194 / 98	0.08 (1)
J-H	-2098 / 0	0.0	0.0 0.22 (1)	5.84	B-P	0 / 1834	0.41 (1)
					K-H	0 / 1834	0.41 (1)
Q-P	0 / 0	-38.5	-38.5 0.19 (3)	10.00			
P-O	0 / 1787	-38.5	-38.5 0.42 (2)	10.00			
O-N	0 / 1535	-38.5	-38.5 0.34 (1)	10.00			
N-M	0 / 1535	-38.5	-38.5 0.34 (1)	10.00			
M-L	0 / 1535	-38.5	-38.5 0.34 (1)	10.00			
L-K	0 / 1787	-38.5	-38.5 0.42 (2)	10.00			
K-J	0 / 0	-38.5	-38.5 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/899 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/899 (0.12")

CSI: TC=0.42/1.00 (B-C:1), BC=0.42/1.00 (C-P:2), VB=0.41/1.00 (B-P:1), SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

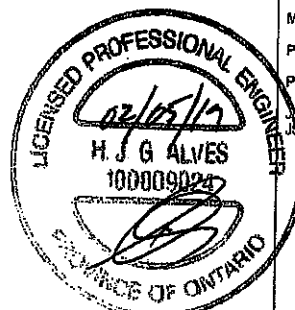
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1687 788 1987 1886

PLATE PLACEMENT TOL = 0.250 inches

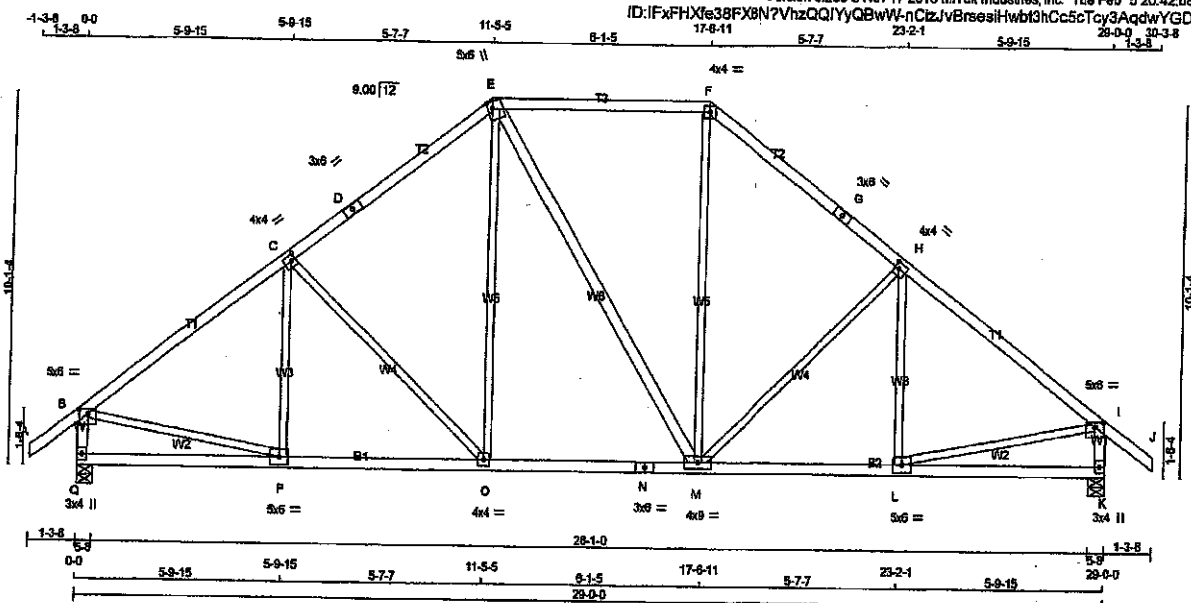
PLATE ROTATION TOL = 5.0 Deg.

SL GRIP=0.89 (P) (INPUT=0.90)
METAL=0.50 (M) (INPUT=1.00)



DWG NO. TAM 71902651
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	17	2	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 138 = 275 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	8.0		
E	TTW-t	MT20	5.0	6.0	2.25	1.50
F	TTW-t	MT20	4.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMVW-t	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV1-p	MT20	3.0	4.0		
L	BMVW-t	MT20	5.0	6.0		
M	BMVW-t	MT20	4.0	9.0		
N	BS-t	MT20	3.0	6.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0		
Q	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	DOWN	GROSS REACTION	DOWN	BRG	IN-SX	BRG	IN-SX
Q	2180	0	2180	0	0	5-8	5-8	5-8	5-8
K	2180	0	2180	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1920	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
K	1920	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	CS1 (LC)	MEMB.	FORCE (LBS)	MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 42	-102.1	-102.1	10.00	P-C	-123 / 150	0.07 (1)
B-C	-2208 / 0	-102.1	-102.1	0.55 (1)	C-O	-492 / 0	0.65 (1)
C-D	-1855 / 0	-102.1	-102.1	0.50 (1)	O-E	0 / 571	0.13 (2)
D-E	-1855 / 0	-102.1	-102.1	0.50 (1)	E-M	0 / 1	0.00 (2)
E-F	-1454 / 0	-102.1	-102.1	0.52 (1)	M-F	0 / 573	0.13 (2)
F-G	-1856 / 0	-102.1	-102.1	0.50 (1)	M-H	-491 / 0	0.65 (1)
G-H	-1856 / 0	-102.1	-102.1	0.50 (1)	L-H	-125 / 149	0.07 (1)
H-I	-2208 / 0	-102.1	-102.1	0.55 (1)	B-P	0 / 1838	0.41 (1)
I-J	0 / 42	-102.1	-102.1	0.14 (1)	L-I	0 / 1838	0.41 (1)
Q-B	-2088 / 0	0.0	0.0	0.22 (1)			
K-I	-2087 / 0	0.0	0.0	0.22 (1)			
Q-P	0 / 0	-38.5	-38.5	0.24 (3)			
P-O	0 / 1801	-38.5	-38.5	0.46 (2)			
O-N	0 / 1453	-38.5	-38.5	0.39 (2)			
N-M	0 / 1453	-38.5	-38.5	0.39 (2)			
M-L	0 / 1800	-38.5	-38.5	0.46 (2)			
L-K	0 / 0	-38.5	-38.5	0.23 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.8 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OSC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CS1: TC=0.55/1.00 (B-C-1), BC=0.48/1.00 (O-P-2), WB=0.65/1.00 (C-O-1), SS1=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

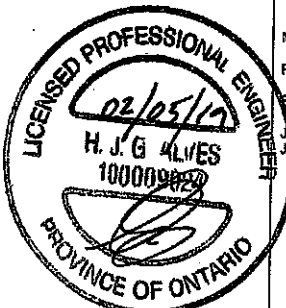
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

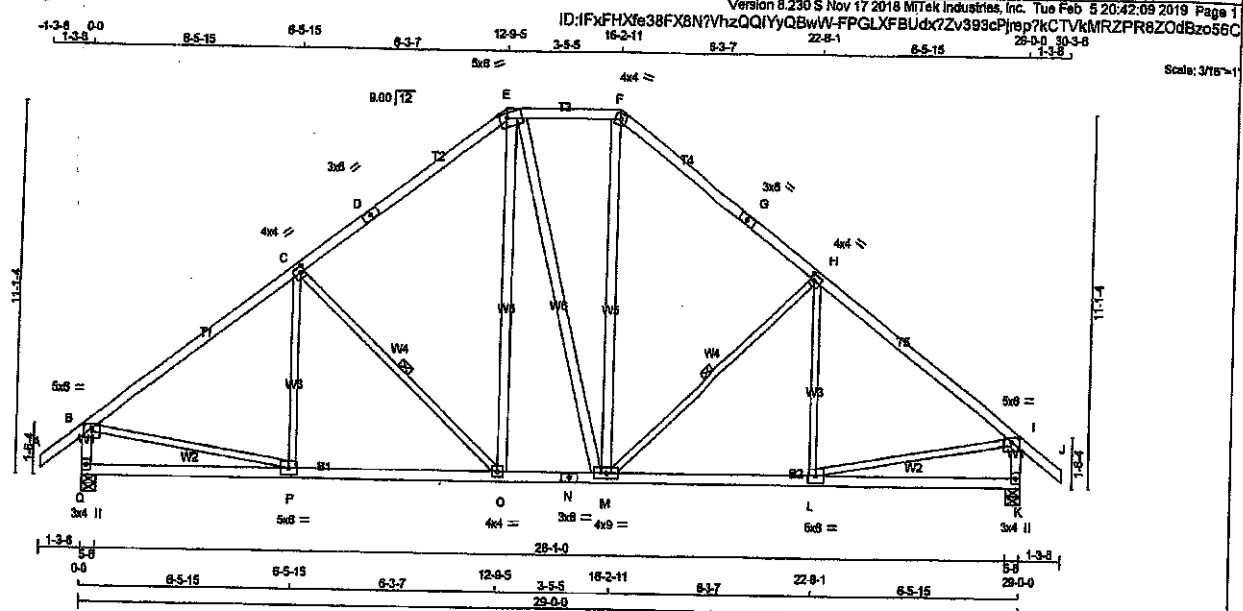
JT GRIP= 0.88 (F) (INPUT = 0.90)
JL METAL= 0.46 (N) (INPUT = 1.00)



DWG NO. TAM 71902652
STRUCTURAL
COMPONENT ONLY

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T8	3	1	Preston 1	
Tamarack Roof Truss, Sudington					



TOTAL WEIGHT = 3 X 150 = 450 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
J - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
O - E	2x4	DRY	No.2	SPF	
E - M	2x4	DRY	No.2	SPF	
M - F	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-l	MT20	4.0	4.0	2.00	1.50
D	TS-l	MT20	3.0	6.0		
E	TTW-m	MT20	6.0	8.0	1.75	2.75
F	TTW-m	MT20	4.0	4.0		
G	TS-l	MT20	3.0	6.0		
H	TMVW-l	MT20	4.0	4.0	2.00	1.50
I	TMVW-p	MT20	5.0	6.0	1.50	3.00
K	BMV-l	MT20	3.0	4.0		
L	BMV-l	MT20	5.0	6.0		
M	BMVW-l	MT20	4.0	9.0		
N	BS-l	MT20	3.0	6.0		
O	BMVW-l	MT20	4.0	4.0		
P	BMVW-l	MT20	5.0	8.0		
Q	BMV-l	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	2180	0	5-8	5-8
K	2180	0	5-8	5-8

UNFACTORED REACTIONS

1ST LOASE	MAX. MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE		
Q	1820	922/0	305/0	0/0	394/0
K	1820	922/0	305/0	0/0	394/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

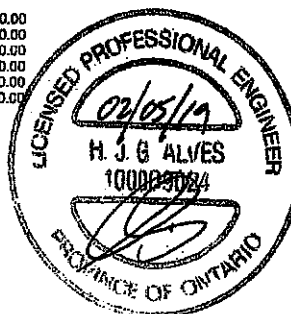
1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	P-C	-43/221	0.05 (3)
B-C	-2211/0	-102.1	-102.1	0.71 (1)	3.81	C-O	-628/0	0.34 (1)
C-D	-1738/0	-102.1	-102.1	0.63 (1)	4.30	O-E	0/597	0.10 (1)
D-E	-1738/0	-102.1	-102.1	0.63 (1)	4.30	E-M	0/4	0.00 (1)
E-F	-1359/0	-102.1	-102.1	0.18 (1)	5.40	M-F	0/601	0.10 (1)
F-G	-1738/0	-102.1	-102.1	0.63 (1)	4.30	M-H	-826/0	0.33 (1)
G-H	-1738/0	-102.1	-102.1	0.63 (1)	4.30	H-I	-46/218	0.05 (3)
H-I	-2210/0	-102.1	-102.1	0.71 (1)	3.81	I-P	0/1836	0.41 (1)
I-J	0/42	-102.1	-102.1	0.14 (1)	10.00	P-L	0/1836	0.41 (1)
Q-B	-2080/0	0.0	0.0	0.22 (1)	5.87			
K-I	-2080/0	0.0	0.0	0.22 (1)	5.87			
Q-P	0/0	-38.5	-38.5	0.32 (3)	10.00			
P-O	0/1806	-38.5	-38.5	0.55 (2)	10.00			
O-N	0/1357	-38.5	-38.5	0.33 (1)	10.00			
N-M	0/1357	-38.5	-38.5	0.33 (1)	10.00			
M-L	0/1806	-38.5	-38.5	0.54 (2)	10.00			
L-K	0/0	-38.5	-38.5	0.32 (3)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
CALCULATED VERT. DEFL. (LL) = L/999 (0.09")
ALLOWABLE DEFL. (TL) = L/360 (0.97")
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.71/1.00 (B-C:1), BC=0.55/1.00 (C-P:2), WB=0.41/1.00 (B-P:1), SB=0.28/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 798 1687 1686

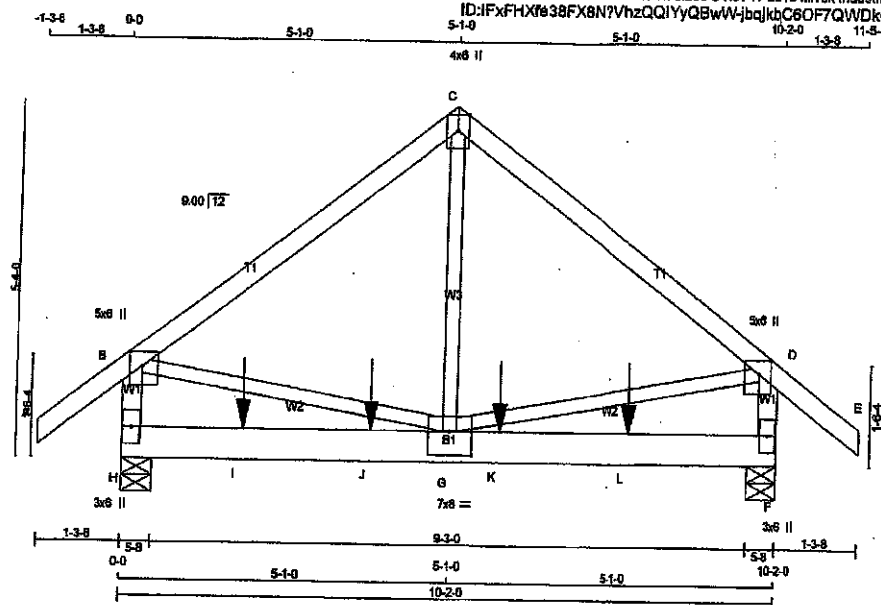
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)

DWG NO. TAM T1902653
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T9	1	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 50 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
H - B	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
H - F	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	6.0	2.50	2.25
C TTW+p	MT20	4.0	6.0	Edge	
D TMVW+p	MT20	5.0	6.0	2.50	2.25
F BMV1+p	MT20	3.0	6.0		
G BMVWW-t	MT20	7.0	6.0	4.25	4.00
H BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	GROSS REACTION	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
H	2129	0	2129	0	0	0	5-8	5-8	5-8
F	2013	0	2013	0	0	0	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1549	987 / 0	214 / 0	0 / 0	0 / 0	0 / 0	347 / 0	0 / 0
F	1465	931 / 0	205 / 0	0 / 0	0 / 0	0 / 0	329 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				FACTORED				WEBS			
MAX. FACTORED		VERT. LOAD		LC1 MAX		MAX.		MEMB.		MAX. FACTORED	
MEMB.	(LBS)	(PLF)	CS1 (LC)	MAX.	UNBRAC	MEMB.	FORCE	MAX	CS1 (LC)		
FR-TO		FROM	TO	LENGTH	FR-TO						
A-B	0 / 42	-102.1	-102.1	0.15 (1)	10.00	G-C	0 / 1472	0.36 (1)			
B-C	-1659 / 0	-102.1	-102.1	0.62 (1)	4.33	B-G	0 / 1359	0.34 (1)			
C-D	-1659 / 0	-102.1	-102.1	0.62 (1)	4.33	G-D	0 / 1359	0.34 (1)			
D-E	0 / 42	-102.1	-102.1	0.15 (1)	10.00						
H-B	-1686 / 0	0.0	0.0	0.19 (1)	6.36						
F-D	-1686 / 0	0.0	0.0	0.19 (1)	6.36						
H-I	0 / 0	-38.5	-38.5	0.71 (1)	10.00						
I-J	0 / 0	-38.5	-38.5	0.71 (1)	10.00						
J-G	0 / 0	-38.5	-38.5	0.71 (1)	10.00						
G-K	0 / 0	-38.5	-38.5	0.71 (1)	10.00						
K-L	0 / 0	-38.5	-38.5	0.71 (1)	10.00						
L-F	0 / 0	-38.5	-38.5	0.71 (1)	10.00						

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-10-1	-808	-808	—	FRONT	VERT	TOTAL	—	—
J	3-10-1	-808	-808	—	FRONT	VERT	TOTAL	—	—
K	5-10-1	-808	-808	—	FRONT	VERT	TOTAL	—	—
L	7-10-1	-808	-808	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(LL) = $1/999$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.34")
CALCULATED VERT. DEFL.(TL) = $1/999$ (0.08")

CS1 TO=0.62/1.00 (C-D-1), BC=0.71/1.00 (G-H-1), WB=0.36/1.00 (C-G-1), SS=0.72/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

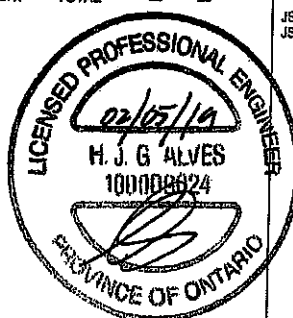
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

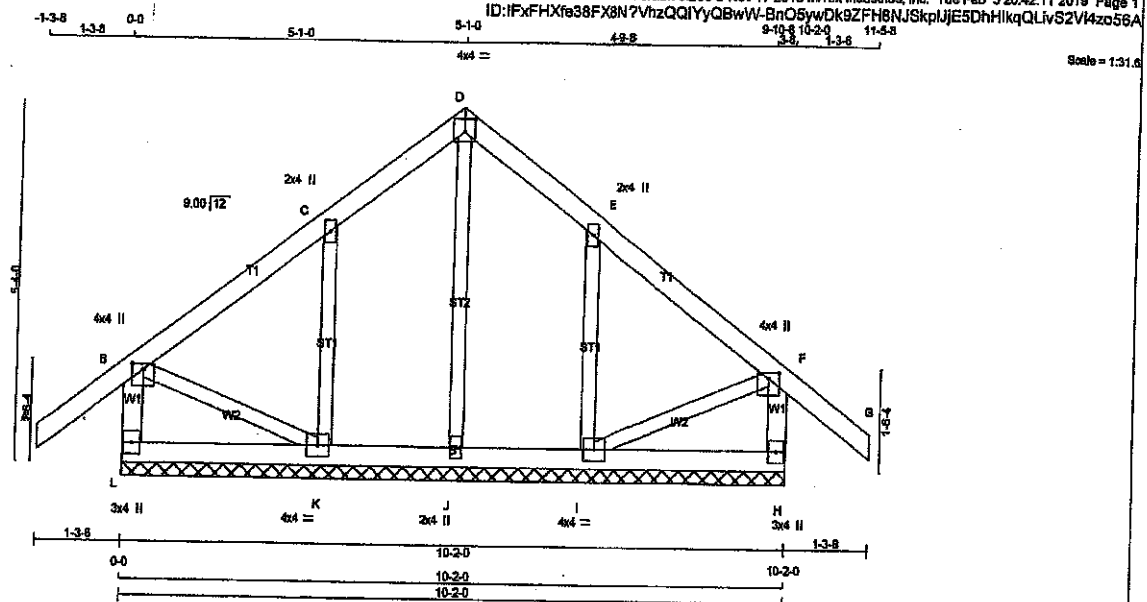
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (D) (INPUT = 0.80)
JSI METAL= 0.35 (C) (INPUT = 1.00)



DWG NO. TAM 71902654
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T9G	1	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER	SIZE	LUMBER
N.L.G.A. RULES		
CHORDS	SIZE	LUMBER
L - B	2x4	DRY No.2
A - D	2x4	DRY No.2
D - G	2x4	DRY No.2
H - F	2x4	DRY No.2
L - H	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
ALL GABLE WEBS	2x3	DRY No.2
DRY: SEASONED LUMBER		
GABLE STUDS SPACED AT	2'-0" O.C.	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED (LC1) (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED (LC1) (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-283 / 0	0.0	0.0 0.03 (1)	J-D	-117 / 0	0.05 (1)	
A-B	0 / 42	-102.1	-102.1 0.14 (1)	K-C	-325 / 0	0.08 (1)	
B-C	-3 / 0	-102.1	-102.1 0.12 (1)	I-E	-325 / 0	0.08 (1)	
C-D	-35 / 0	-102.1	-102.1 0.12 (1)	B-K	0 / 17	0.00 (1)	
D-E	-35 / 0	-102.1	-102.1 0.12 (1)	I-F	0 / 17	0.00 (1)	
E-F	-3 / 0	-102.1	-102.1 0.12 (1)				
F-G	0 / 42	-102.1	-102.1 0.14 (1)				
H-F	-283 / 0	0.0	0.0 0.03 (1)				
L-K	0 / 0	-38.5	-38.5 0.07 (3)				
K-J	0 / 8	-38.5	-38.5 0.07 (3)				
J-I	0 / 8	-38.5	-38.5 0.07 (3)				
I-H	0 / 0	-38.5	-38.5 0.07 (3)				

TOTAL WEIGHT = 47 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.14/1.00 (F-G:1), BC=0.07/1.00 (I-L:3), WB=0.08/1.00 (E-L:1), SS=0.11/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1067 788 1987 1658

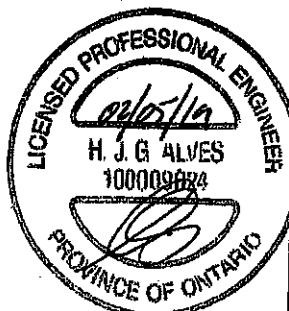
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.90)
JSI METAL= 0.17 (C) (INPUT = 1.00)

RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

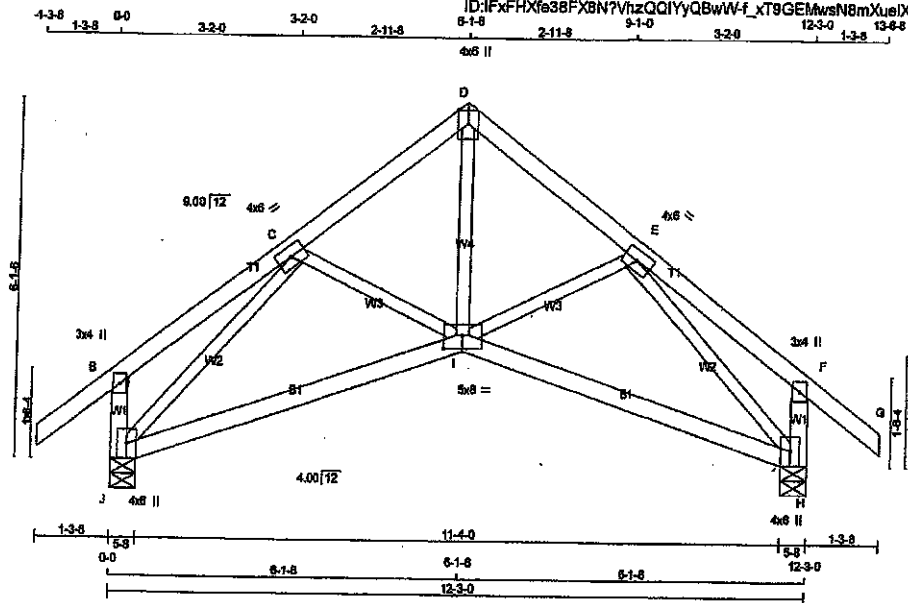


DRWG NO. TAM 71902655
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10S	2	1	Preston 1	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitak Industries, Inc. Tue Feb 5 20:42:12 2019 Page 1
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Scale = 1/32

TOTAL WEIGHT = 2 X 54 = 108 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
J - I	2x4	DRY	No.2	SPF		
I - H	2x4	DRY	No.2	SPF		
ALL WEBS	2x8	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+p	MT20	4.0	6.0		
D TMV+p	MT20	4.0	6.0	Edge	
E TMVW+p	MT20	4.0	6.0		
F TMV+p	MT20	3.0	4.0		
H BVWV1+p	MT20	4.0	6.0		
I BVWVW+p	MT20	5.0	8.0	2.75	4.00
J BVWV1+p	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
J	1002	0	0	5-8
H	1002	0	0	5-8

UNFACTORED REACTIONS	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
J	740	436 / 0 128 / 0 0 / 0 0 / 0 178 / 0 0 / 0
H	740	436 / 0 128 / 0 0 / 0 0 / 0 178 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (PLF) CSI (LC) UNBRAC	MEMB.	FORCE (LBS) MAX (LC)
FR-TO		FROM TO LENGTH FR-TO		
A-B	0 / 42	-102.1 -102.1 0.14 (1) 10.00	I-D	0 / 787
B-C	0 / 19	-102.1 -102.1 0.13 (1) 10.00	I-E	-52 / 55
C-D	-866 / 0	-102.1 -102.1 0.11 (3) 6.25	C-I	-52 / 55
D-E	-866 / 0	-102.1 -102.1 0.11 (1) 6.25	J-C	-1124 / 0
E-F	0 / 19	-102.1 -102.1 0.13 (1) 10.00	E-H	-1124 / 0
F-G	0 / 42	-102.1 -102.1 0.14 (1) 10.00		
J-B	-268 / 0	0.0 0.0 0.03 (1) 7.81		
H-F	-268 / 0	0.0 0.0 0.03 (1) 7.81		
J-I	0 / 784	-38.5 -38.5 0.41 (2) 10.00		
I-H	0 / 784	-38.5 -38.5 0.41 (2) 10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 088-14
- TPIG 2011, TPIG 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/989 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/846 (0.17")

CSI: TC=0.14/1.00 (A-B-1), BC=0.41/1.00 (I-J-2), WB=0.40/1.00 (C-J-1), SSI=0.12/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

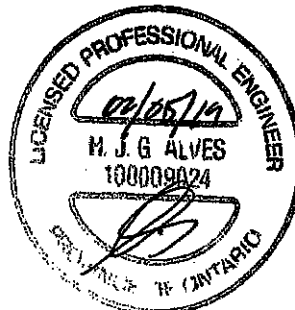
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1666

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (J) (INPUT = 0.90)
JSI METAL= 0.27 (C) (INPUT = 1.00)



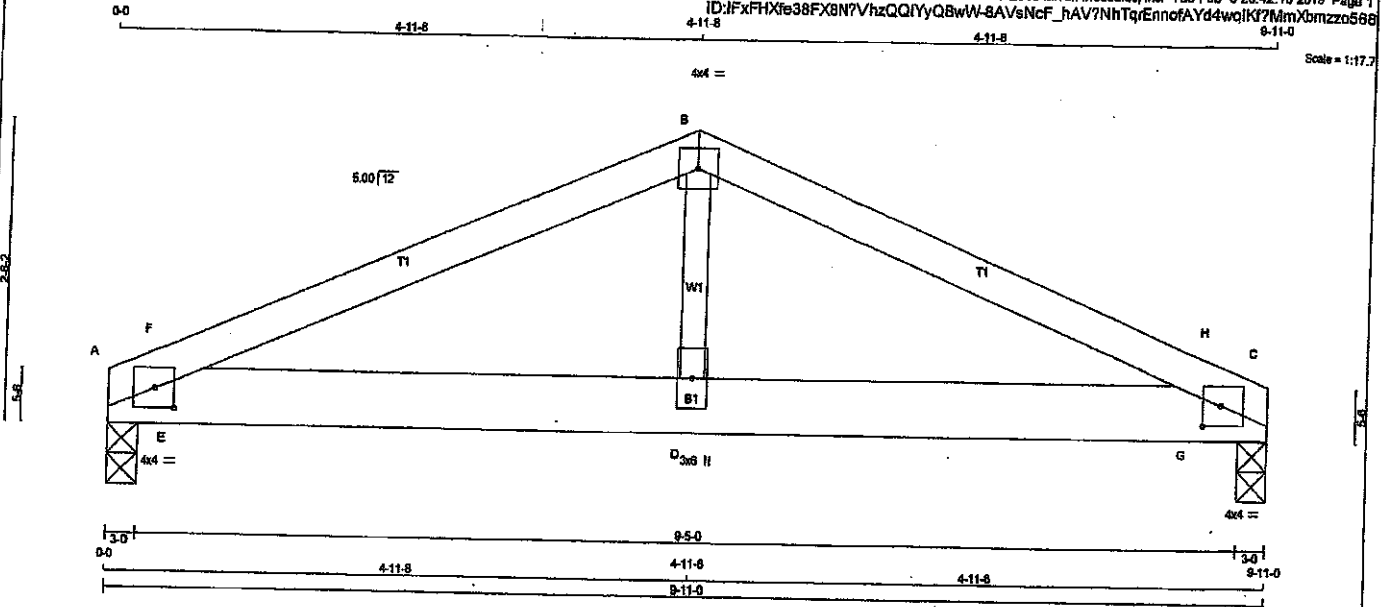
DRWG NO. TAM 7902658
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T11	QUANTITY 2	PLY 1	JOB DESC. Preston 1	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue Feb 5 20:42:13 2019 Page 1

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LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - B 2x4 DRY No.2
B - C 2x4 DRY No.2
A - C 2x6 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.
SPF
SPF
SPF

BEARINGS
DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	REQD
A	697	0	697	0	0	3-0	3-0	3-0
C	697	0	697	0	0	3-0	3-0	3-0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
A	521	288 / 0	104 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0
C	521	288 / 0	104 / 0	0 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX CSI (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO	A-F	-897 / 0	-102.1	-102.1	0.17 (1)	6.25	D-B	0 / 385	0.09 (2)
	F-B	-899 / 0	-102.1	-102.1	0.24 (1)	5.99	E-F	-452 / 0	0.00 (1)
	B-H	-899 / 0	-102.1	-102.1	0.24 (1)	5.99	G-H	-452 / 0	0.00 (1)
	H-C	-897 / 0	-102.1	-102.1	0.17 (1)	6.25			
	A-E	0 / 901	-38.5	-38.5	0.28 (1)	10.00			
	E-D	0 / 901	-38.5	-38.5	0.28 (1)	10.00			
	D-G	0 / 901	-38.5	-38.5	0.28 (1)	10.00			
	G-C	0 / 901	-38.5	-38.5	0.28 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/D

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.02")
ALLOWABLE DEFL.(TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.03")

CSI: TC=0.24/1.00 (B-F-1), BC=0.28/1.00 (A-E-1),
WB=0.09/1.00 (B-D-2), SSI=0.27/1.00 (A-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

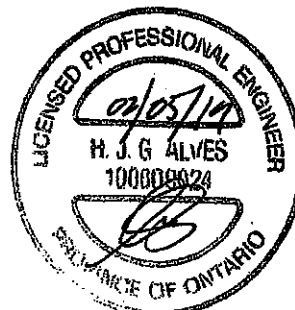
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1697 1698

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

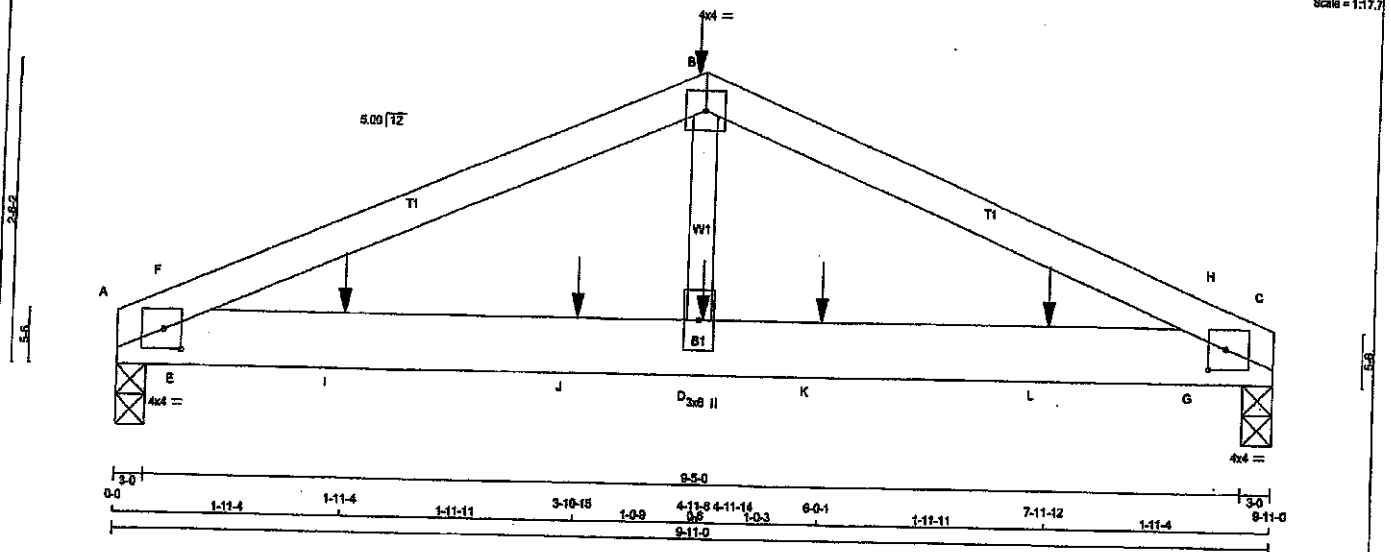
JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)



DRWG NO. TAM 17902659
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T11Z	1	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2016 Mitek Industries, Inc. Tue Feb 5 20:42:14 2019 Page 1
 ID:IFxHXfe38FX8N7VhzQQIYyQBwV-cM3EayFcSUdr7r21PyJdLqJUF31nn8bPH9JPz0567
 0-0 4-11-8 4-11-8 4-11-8 9-11-0
 Scale = 1:17



<u>LUMBER</u>				
N. L. G. A. RULES				
CHORDS	SIZE		LUMBER	DESCR
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)						
JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-I	MT20	4.0	4.0	2.00	1.75
B	TTW-p	MT20	4.0	4.0		
C	TMB1-I	MT20	4.0	4.0	2.00	1.75
D	BMW+w	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	857	0	857	0	3-0	3-0
C	857	0	857	0	3-0	3-0

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JA	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
	631	379 / 0	108 / 0	0 / 0	0 / 0	144 / 0	0 / 0
C	631	379 / 0	108 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A. C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY
APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED			MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED		
		VERT. LOAD	LC1	MAX CSI (LC)			MAX UNBRAC	MAX CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO				
A-F	-1294 / 0	-102.1	-102.1	0.24 (1)	5.34	D-B	0 / 383	0.09 (2)	
F-B	-1370 / 0	-102.1	-102.1	0.31 (1)	5.16	E-F	-415 / 0	0.00 (1)	
B-H	-1370 / 0	-102.1	-102.1	0.31 (1)	5.16	G-H	-415 / 0	0.00 (1)	
H-C	-1294 / 0	-102.1	-102.1	0.24 (1)	5.34				
A-E	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
E-I	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
I-J	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
J-D	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
D-K	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
K-L	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
L-G	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				
G-C	0 / 1253	-38.5	-38.5	0.35 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LÖC.	LCI	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN
A	4-11-8	-25	-28	—	FRONT	VERT	DEAD	—	—
B	4-11-8	-7	-7	—	FRONT	VERT	TOTAL	—	—
B	4-11-8	-273	-273	—	FRONT	VERT	SNOW	—	—
D	4-11-14	-3	-4	—	FRONT	VERT	TOTAL	—	—
I	1-11-4	-3	-4	—	FRONT	VERT	TOTAL	—	—
J	3-10-15	-3	-4	—	FRONT	VERT	TOTAL	—	—
K	8-0-1	-3	-4	—	FRONT	VERT	TOTAL	—	—
L	7-11-12	-3	-4	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED
BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP	CH.	LL	=	29.0	PSF
		DL	=	8.0	PSF
BOT	CH.	LL	=	10.5	PSF
		DL	=	7.0	PSF
TOTAL LOAD				=	52.5 PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADD'T'L USER-DEFINED LOADS APPLIED TO
ALL LOAD CASES.

**THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015**

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.33")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = 1/360 (0.33")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CS: TC=0.31/1.00 (B-H:1), BC=0.35/1.00 (D-G:1),
WB=0.00/1.00 (B-D:2), SS(=0.29/1.00 (C-G:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR ≈ 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

TAIL VALUES

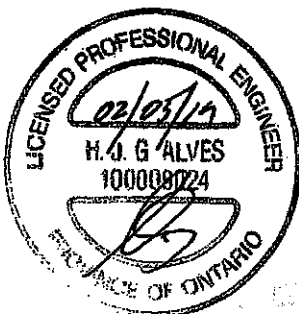
	GRIP(DRY)		SHEAR		SECTION	
	(PSI)	(PLI)	(PLI)	(PLI)	(PLI)	(PLI)
T20	618	354	1667	788	1987	1656

LATE PLACEMENT TOL = 0.250 inches

LATE ROTATION TOL = 5.0 Deg.

GRIP= 0.59 (A) (INPUT = 0.90)
METAL= 0.54 (C) (INPUT = 1.00)

DWG NO. TAM 1790 2660
STRUCTURAL
COMPONENT ONLY



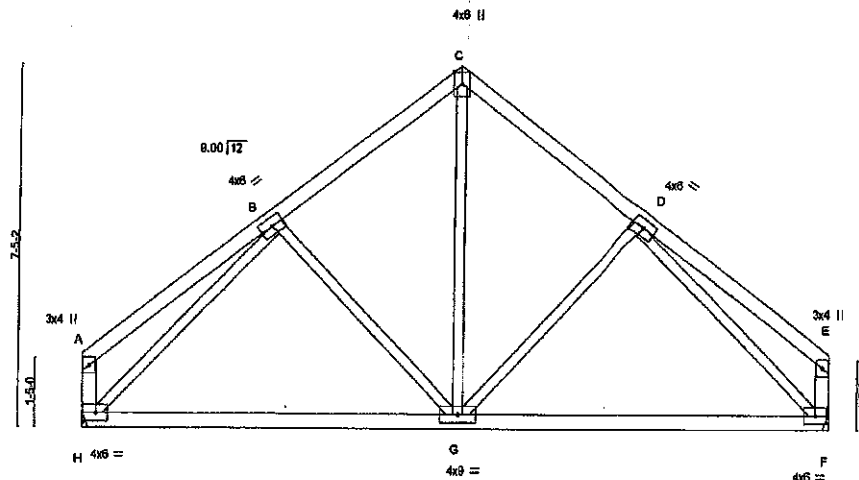
JUN 13 2019

BOOK OF CALEDON
BUILDING SECTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401596	T111	2	1	Greenpark	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 MTak Industries, Inc. Wed Feb 27 17:51:54 2019 Page 1	

ID:IFxHXie38FX8N7VhzQQIYyQBwW-4vHTts44CFnmSkNtXZ1lpOFmRX6pL_oMXKNVzgIXp

Scale = 1:43.8



TOTAL WEIGHT = 2 X 66 = 133 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0	
B	TMVW+1	MT20	4.0	8.0	
C	TTW+p	MT20	4.0	8.0	Edge
D	TMVW+1	MT20	4.0	8.0	
E	TMV+p	MT20	3.0	4.0	
F	BMVW+1	MT20	4.0	8.0	
G	BMVW+1	MT20	4.0	9.0	
H	BMVW+1	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	1107	0	1107	0	0	MECHANICAL	MECHANICAL
F	1107	0	1107	0	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	827	457 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0
F	827	457 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO						FR-TO			
A-B	0 / 28	-102.1	-102.1	0.25 (1)	10.00	G-C	0 / 870	0.15 (1)	10.00
B-C	-846 / 0	-102.1	-102.1	0.20 (1)	8.25	G-D	-203 / 21	0.11 (1)	8.25
C-D	-846 / 0	-102.1	-102.1	0.20 (1)	8.25	B-G	-203 / 21	0.11 (1)	8.25
D-E	0 / 28	-102.1	-102.1	0.25 (1)	10.00	H-B	-1158 / 0	0.62 (1)	10.00
H-A	-154 / 0	0.0	0.0	0.02 (1)	7.81	D-F	-1158 / 0	0.62 (1)	7.81
F-E	-154 / 0	0.0	0.0	0.02 (1)	7.81				
H-G	0 / 795	-38.5	-38.5	0.81 (2)	10.00				
G-F	0 / 795	-38.5	-38.5	0.81 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/969 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/969 (0.19")

CSI: TC=0.25/1.00 (A-B:1), BC=0.61/1.00 (G-H:2), WB=0.62/1.00 (B-H:1), SS=0.20/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

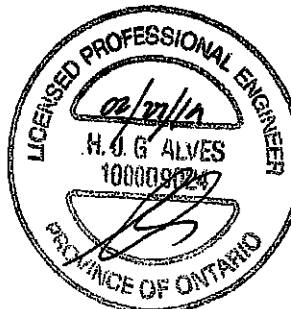
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

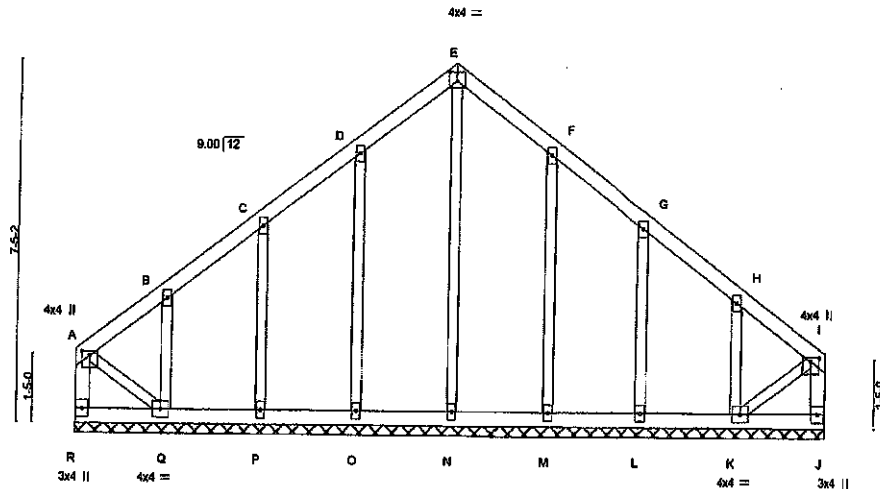


DWG NO. TAM 77904520
STRUCTURAL
COMPONENT ONLY

JOB NAME 401596	TRUSS NAME G111	QUANTITY 1	PLY 1	JOB DESC. Greenpark	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2018 MiTek Industries, Inc. Wed Feb 27 17:51:52 2019 Page 1
ID:IFxHFXe38FX8N?VhzQQYyQBwVW-8W9JSA2qgeXsX9a?m6XZDYk5mzuBe0ThL22DJeZgtX

Scale = 1:43.8



TOTAL WEIGHT = 71 lb

LUMBER

N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
R - A	2x4 DRY	No.2	SPF
A - E	2x4 DRY	No.2	SPF
E - I	2x4 DRY	No.2	SPF
J - I	2x4 DRY	No.2	SPF
R - J	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
ALL GABLE WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A TMVW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, F, G, H					
B TMVW+w	MT20	2.0	4.0		
E TTW-p	MT20	4.0	4.0	2.25	2.00
I TMVW+p	MT20	4.0	4.0	1.00	2.00
J BMV1+p	MT20	3.0	4.0		
K BMVW1-i	MT20	4.0	4.0		
L, M, N, O, P					
L BMW1+w	MT20	2.0	4.0		
Q BMVW1-i	MT20	4.0	4.0		
R BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO	CSI (LC)	FR-TO			
R-A	-125 / 0	0.0	0.01 (1)	N-E	-136 / 0	0.13 (1)	
A-B	-37 / 0	-102.1	-102.1 0.05 (1)	O-D	-232 / 0	0.13 (1)	
B-C	-41 / 0	-102.1	-102.1 0.05 (1)	P-C	-163 / 0	0.08 (1)	
C-D	-31 / 0	-102.1	-102.1 0.08 (1)	Q-B	-211 / 0	0.04 (1)	
D-E	-42 / 0	-102.1	-102.1 0.08 (1)	M-F	-232 / 0	0.13 (1)	
E-F	-42 / 0	-102.1	-102.1 0.08 (1)	L-G	-193 / 0	0.08 (1)	
F-G	-31 / 0	-102.1	-102.1 0.08 (1)	K-H	-211 / 0	0.04 (1)	
G-H	-41 / 0	-102.1	-102.1 0.05 (1)	A-Q	0 / 48	0.01 (1)	
H-I	-37 / 0	-102.1	-102.1 0.05 (1)	K-I	0 / 48	0.01 (1)	
J-I	-125 / 0	0.0	0.01 (1)				
R-Q	0 / 0	-38.5	-38.5 0.03 (3)				
Q-P	0 / 32	-38.5	-38.5 0.03 (2)				
P-O	0 / 28	-38.5	-38.5 0.03 (2)				
O-N	0 / 24	-38.5	-38.5 0.03 (2)				
N-M	0 / 24	-38.5	-38.5 0.03 (2)				
M-L	0 / 28	-38.5	-38.5 0.03 (2)				
L-K	0 / 32	-38.5	-38.5 0.03 (2)				
K-J	0 / 0	-38.5	-38.5 0.03 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.08/1.00 (D-E-1), BC=0.03/1.00 (K-L-2), WB=0.13/1.00 (E-N-1), SS=0.08/1.00 (E-F-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

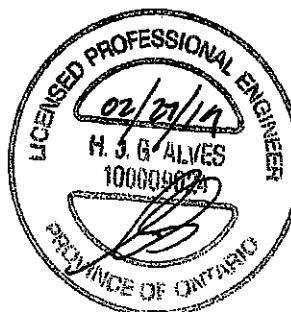
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (FLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

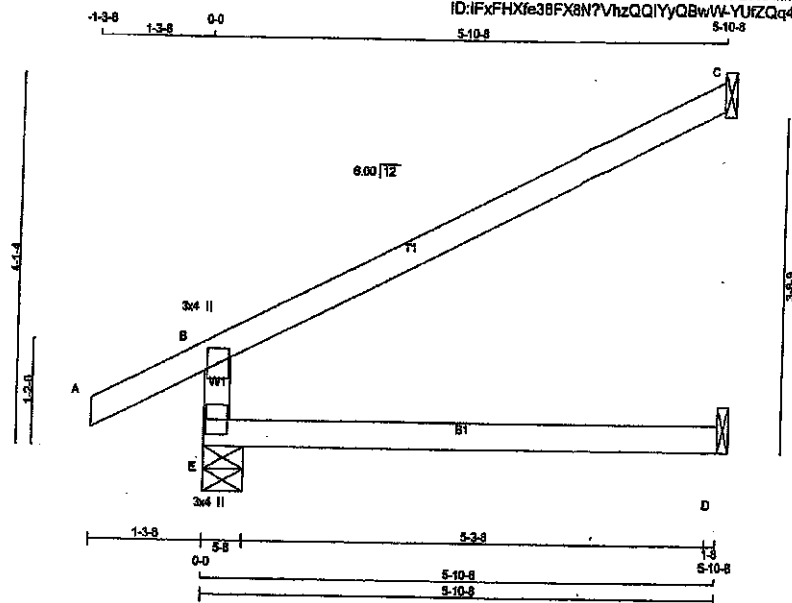
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)



DWG NO. TAM 17904521
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond1	22	1	Preston 1	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:41:59 2019 Page 1					
ID: IFxHXfe38FX8N7VhzQQIYyQBwWV-YUJZQq4C_ik_XP81DXVeib3VR9b00Yx8a8eHnzo56M					
Scale = 1:23.6					



LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY

A - C 2x4 DRY

E - D 2x4 DRY

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	VERT	DOWN	IN-SX	IN-SX
E	848	0	5-8	5-8
C	225	0	1-8	1-8
D	93	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	COMBINED	SNOW	LIVE			
E	473	292 / 0	72 / 0	0 / 0	109 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	28 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO									
E-B	-513 / 0	0.0	0.0	0.22 (3)	7.81				
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00				
B-C	-34 / 0	-102.1	-102.1	0.60 (1)	6.25				
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00				

TOTAL WEIGHT = 22 X 17 = 369 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/998 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/942 (0.07")

CSI: TC=0.80/1.00 (B-C:1), BC=0.22/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), 8SI=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

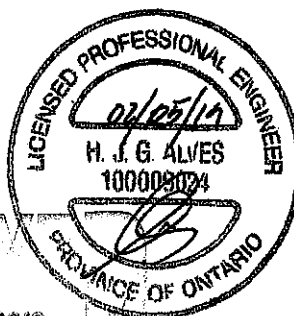
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (E) (INPUT = 1.00)



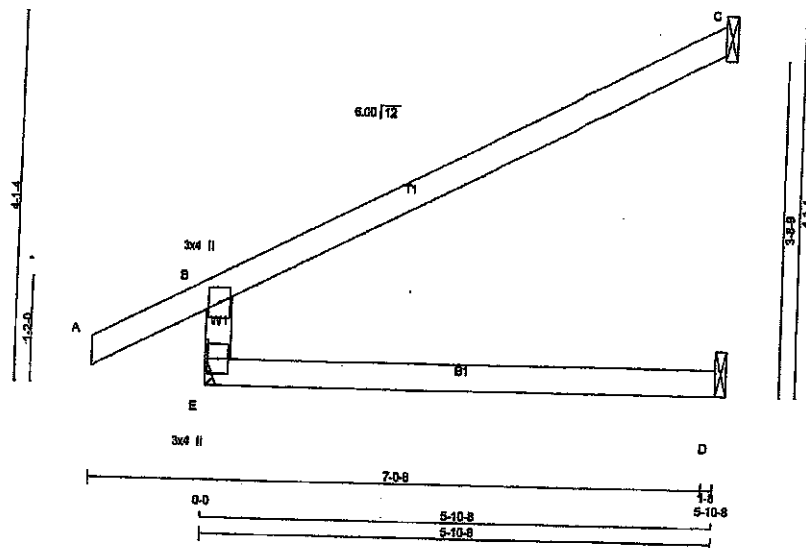
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.

DWG NO. TAM 7902661
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond2	22	1	Preston 1	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:41:59 2019 Page 1
 ID: FxHXFe38FX3N7VhzQQIYyQBwVW-YUJZQq4C_k_XP61DXvEb5hR9b00Yx8a8sHnzo56M
 Scale = 1/23.6



LUMBER	N. L. O. A. RULES	CHORDS	SIZE	LUMBER
E - B	2x4	DRY	No.2	
A - C	2x4	DRY	No.2	
E - D	2x4	DRY	No.2	

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	646	0	646	0	1-8	1-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	473	292 / 0	72 / 0	0 / 0	0 / 0	108 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	28 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)		MAX. CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FROM	TO					
FR-TO						FR-TO		
E-B	-513 / 0	0.0	0.0	0.18 (3)	7.81			
A-B	0 / 31	-102.1	-102.1	0.13 (1)	10.00			
B-C	-34 / 0	-102.1	-102.1	0.46 (1)	6.25			
E-D	0 / 0	-38.5	-38.5	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 8.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2010, CBC 2012
 - CSA 088-08, CSA 088-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/380$ (0.20")
 CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
 ALLOWABLE DEFL. (TL) = $L/380$ (0.20")
 CALCULATED VERT. DEFL. (TL) = $L/942$ (0.07")

CSI: TC=0.45/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP (DRY) SHEAR SECTION (PSD) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1887 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
 JSI METAL= 0.14 (B) (INPUT = 1.00)

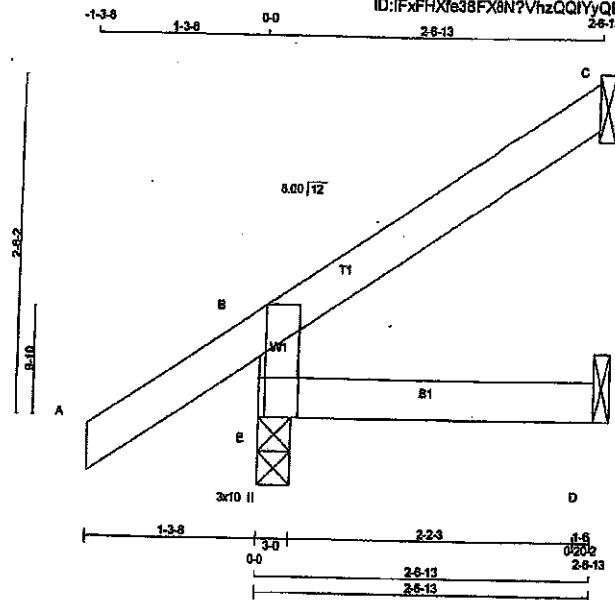


DRWG NO. TAM 71902662
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J3	5	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:41:59 2019 Page 1
ID:IFxHFXe38FX8N?VhzQQYyQBwW-YUfZQq4C_k_IXP81(XVEibAKRCI00Yx8a8sHnzo56M)

Scale = 1:15.6



TOTAL WEIGHT = 5 X 9 = 44 lb

LUMBER

N.L.G.A. RULES

CHORDS SIZE

E - B 2x4 DRY

A - C 2x4 DRY

E - D 2x4 DRY

LUMBER

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B

E

E TMBMV1+p MT20 3.0 10.0 Edge 0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	381	0	381	0	3-0	3-0
C	98	0	98	0	1-8	1-8
D	42	0	53	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	281	173 / 0	31 / 0	0 / 0	0 / 0	57 / 0	0 / 0
C	88	55 / 0	0 / 0	0 / 0	0 / 0	12 / 0	0 / 0
D	38	0 / 0	23 / 0	0 / 0	0 / 0	15 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	LC2 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO				FR-TO			
E-B	-304 / 0	0.0	0.0	0.04 (3)	7.81				
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00				
B-C	-18 / 0	-102.1	-102.1	0.11 (1)	6.25				
E-D	0 / 0	-38.5	-38.5	0.04 (3)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.04/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	818	354	1667
	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

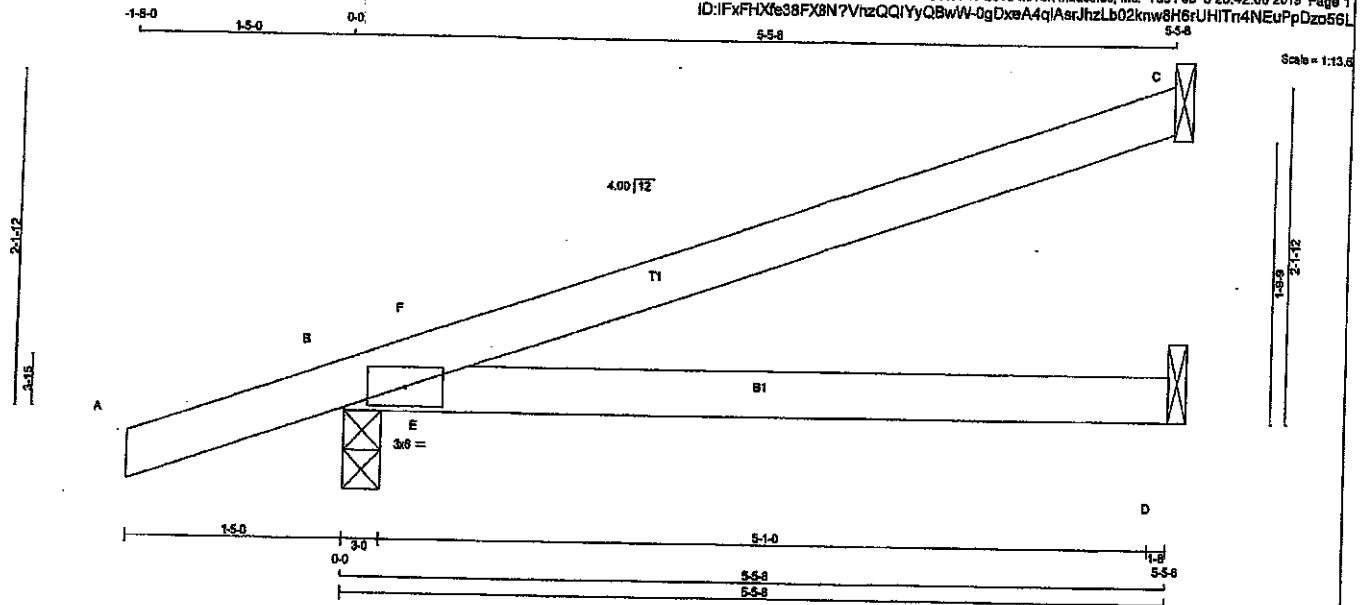


DRWG NO. TAM 17902663
STRUCTURAL
COMPONENT ONLY

JOB NAME: 200168-400364 TRUSS NAME: J4 QUANTITY: 2 PLY: 1 JOB DESC: Preston 1 TRUSS DESC: DRWG NO.

Tamarack Roof Truss, Burlington

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:00 2019 Page 1
ID:1FxFHXe38FX8N?VrzQQYyQBwW-DgDxeA4q|AsrJhzLb02knw8H6rUHTr4NEuPpDzo56L



TOTAL WEIGHT = 2 X 15 = 29 lb

LUMBER

N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY

LUMBER
No.2
No.2

DESCR.
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	250	0	250	0	1-8	1-8
B	533	0	533	0	3-0	3-0
D	133	0	145	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	1ST LCASE MAX/MIN COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE				
C	175	133/0	9/0	0/0	0/0	33/0	0/0
B	389	243/0	57/0	0/0	0/0	88/0	0/0
D	111	25/0	49/0	0/0	0/0	38/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)		FACTORED VERT. LOAD (PLF)		MAX. UNBRACED LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)
	FROM	TO	FROM	TO		
A-B	0/22	-102.1	-102.1	0.15 (1)	10.00	E-F -254/122 0.00 (1)
B-F	-51/0	-102.1	-102.1	0.13 (3)	6.25	
F-C	0/6	-102.1	-102.1	0.42 (1)	10.00	
B-E	0/0	-38.5	-38.5	0.26 (1)	10.00	
E-D	0/0	-38.5	-38.5	0.31 (1)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/842 (0.06")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/489 (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1687 788 1907 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)



DRWG NO. TAM 17902664
STRUCTURAL
COMPONENT ONLY



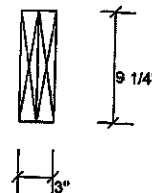
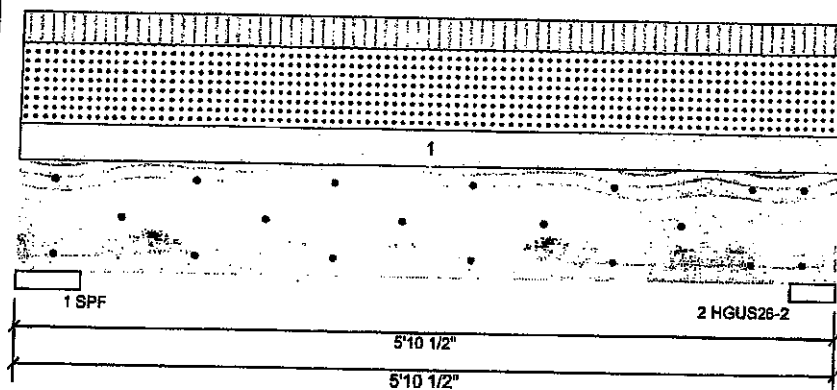
isDesign™

Client: Greenpark Homes
 Project: Preston 1
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200168

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
 Piles: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Floor (Residential)
 Design Method: LSD
 Building Code: NBCC 2015 / OBC 2012
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	231	273	609	0
2	221	262	584	0

Bearings and Factored Reactions

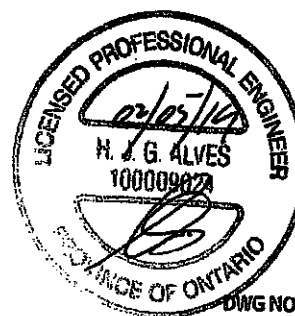
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500'	15%	341 / 1145	1486 L
2 - HGUS...	4.000'	19%	327 / 1097	1424 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1679 ft-lb	3'	6039 ft-lb	0.278 (28%)	1.25D+1.5S	L
Unbraced	1679 ft-lb	3'	5236 ft-lb	0.321 (32%)	1.25D+1.5S	L
Shear	1290 lb	12"	3984 lb	0.324 (32%)	1.25D+1.5S	L
Perm Defl in. (L/11306)	0.006	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/4280)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3094)	3'	0.174 (L/360)	0.120 (12%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17902665
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7'-0"	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	

Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD, ON
 CANADA
 (905) 335-1115



This design is valid until 2021-12-11





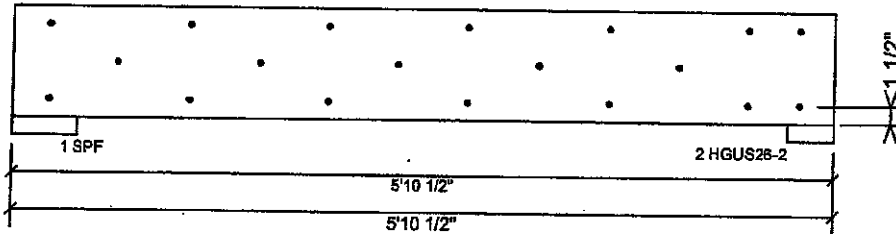
isDesign™

Client: Greenpark Homes
Project: Preston 1
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200188

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

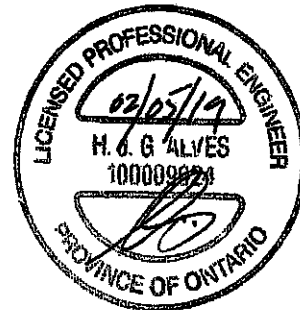
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	72.8 %
Load	247.6 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	0
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO: T&M 71902665
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	TAMARACK LUMBER 3255 NORTH SERVICE RD, ON CANADA (905) 335-1115
	 ALFA LUMBER GROUP

This design is valid until 2021-12-11





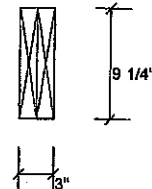
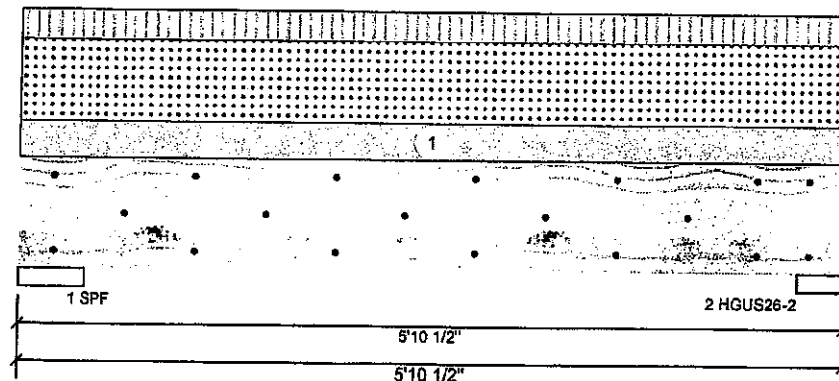
isDesign™

Client:
Project:
Address:Date: 2/27/2019
Designer: JG
Job Name: 401996 / GREENPARK/LAMBERTS LANE PH2
Project #:

Page 1 of 2

B110 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	10.5 PSF		
Dead:	6 PSF		
Snow:	29 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	284	351	783	0
2	272	336	750	0

Bearings and Factored Reactions

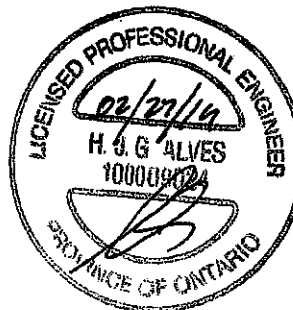
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	19%	439 / 1458	1897 L
2 - HGUS...	4.000"	25%	420 / 1397	1818 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2144 ft-lb	3'	6039 ft-lb	0.355 (36%)	1.25D+1.5S	L
Unbraced	2144 ft-lb	3'	5236 ft-lb	0.409 (41%)	1.25D+1.5S	L
Shear	1846 lb	4'10"	3984 lb	0.413 (41%)	1.25D+1.5S	L
LL Defl inch	0.019 (L/3338)	3'	0.174 (L/360)	0.110 (11%)	S+0.5L	L
TL Defl inch	0.026 (L/2419)	3'	0.174 (L/360)	0.150 (15%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17904522
STRUCTURAL
COMPONENT ONLY

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		9-0-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(800) 335-1115





isDesign™

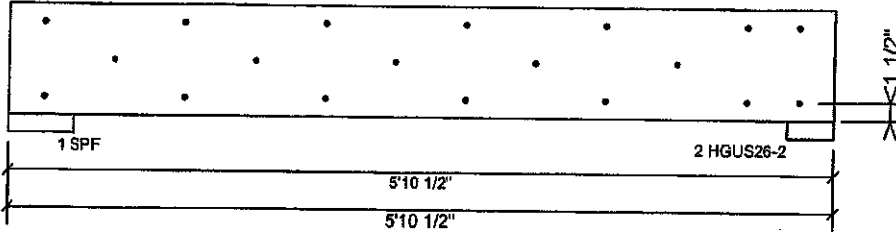
Client:
Project:
Address:

Date: 2/27/2019
Designer: JG
Job Name: 401696 / GREENPARK/LAMBERTS LANE PH2
Project #:

Page 2 of 2

B110 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

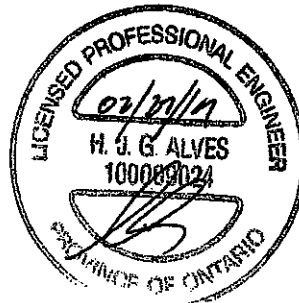
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	93.0 %
Load	316.1 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17904522
STRUCTURAL
COMPONENT ONLY

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



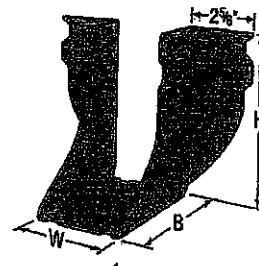
✓ LUS28



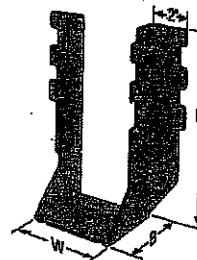
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

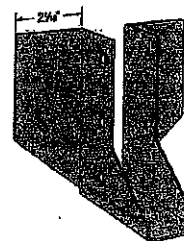
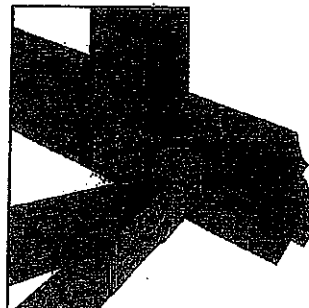


Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,560

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options Information on pp. 125-127.

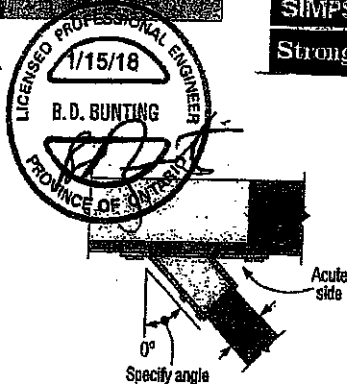
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Std.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _h	Header	Joist	S-P-F		D-Form	
								lb.	lb.	lb.	lb.
								lb.	lb.	lb.	lb.
Single 2x 6zies											
LUS24	18	1 1/2	3 1/2	1 1/2	2 1/4	(4) 10d	(2) 10d	710	1825	845	1155
LU24L	22	1 1/2	3	1 1/2	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
LU28L	22	1 1/2	5	1 1/2	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1805	845	1140
LUS28	18	1 1/2	4 1/4	1 1/2	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1830
LU28L	18	1 1/2	5 1/4	3	3 1/4	(14) 10d	(6) 10d	2705	4940	2085	3875
LJS26DS	18	1 1/2	5	3 1/2	4 1/4	(16) 10d	(6) 10d	2055	4285	1450	4115
HGUS26	12	1 1/2	5 1/2	5	4 1/4	(20) 10d	(8) 10d	2685	6625	2685	5700
LU28L	20	1 1/2	6 1/4	1 1/2	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1950
LUS28	18	1 1/2	6 1/4	1 1/2	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
HUS28	16	1 1/2	7 1/4	3	5 1/4	(22) 10d	(8) 10d	3605	5365	2675	4345
HGUS28	12	1 1/2	7 1/4	5	6 1/4	(30) 10d	(12) 10d	3310	7675	3310	6900
LU210L	20	1 1/2	8	1 1/2	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2485	1020	1770
LUS210	18	1 1/2	7 1/4	1 1/2	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_h is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 10d = 0.162" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

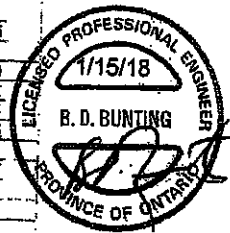
Face-Mount Hangers

SIMPSON
Strong-Tie

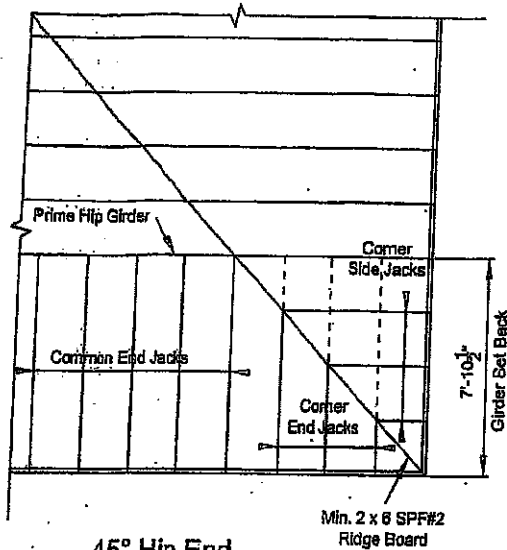
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _p	Header	Joist			S-P-F	
								lb.	lb.	kip (k = 1.0)	kip (k = 1.0)
Double 2x Sizes											
LUS2-2	18	3 1/8	3 1/8	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
LUS2-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS2-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2085	5205
HGUS2-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS2-2	18	3 1/8	7 1/8	2	4	(8) 16d	(4) 16d	1720	3325	1545	2827
HHUS2-2	14	3 1/8	7 1/8	3	4 1/8	(22) 16d	(8) 16d	3785	8940	2675	6345
HGUS2-2	12	3 1/8	7 1/8	4	6 1/8	(30) 16d	(12) 16d	6070	12980	4310	9215
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS2-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS2-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS2-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d	4670	9670	4235	6865
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
Quadruple 2x Sizes											
HGUS2-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
HGUS2-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
HHUS210-4	14	6 1/8	8 1/8	3	7 1/8	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11845
4x Sizes											
LUS48	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d	1720	2595	1545	1920
HHUS48	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2850	7335	2085	5205
HGUS48	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 1/8	6 1/8	2	3 1/8	(8) 16d	(4) 16d	1720	3325	1545	2827
HHUS48	14	3 1/8	7 1/8	3	4 1/8	(22) 16d	(8) 16d	3785	8940	2675	6345
HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d	2580	4500	2320	3195
HGUS410	12	3 1/8	9	4	8 1/8	(46) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS414	12	3 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d	10130	16400	7195	11845



Plated Truss Connectors



45° Hip End

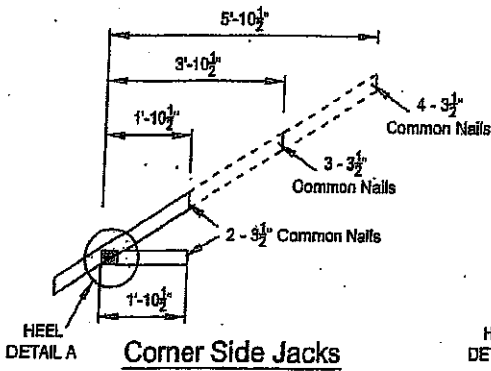
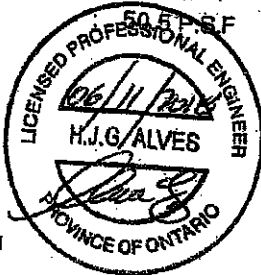
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

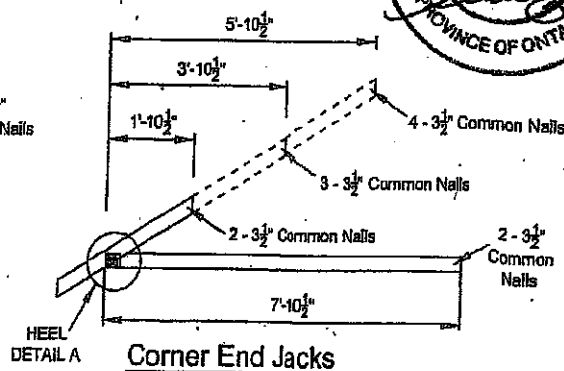
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

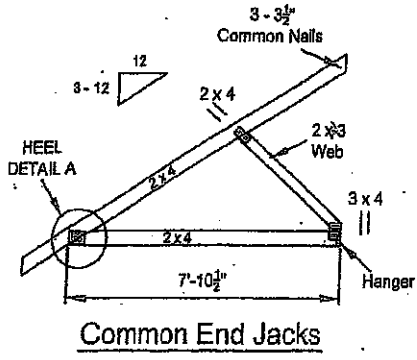
TOTAL LOAD



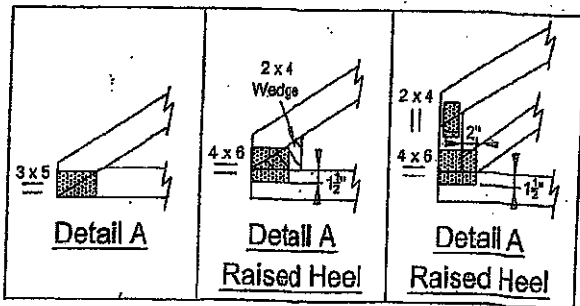
Corner Side Jacks



Corner End Jacks



Common End Jacks

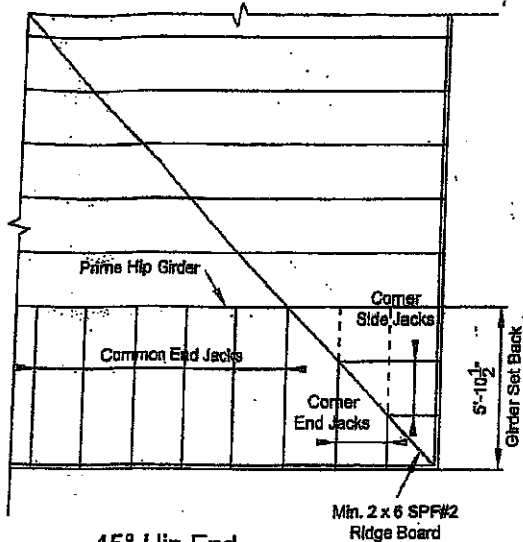


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

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TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.



45° Hip End

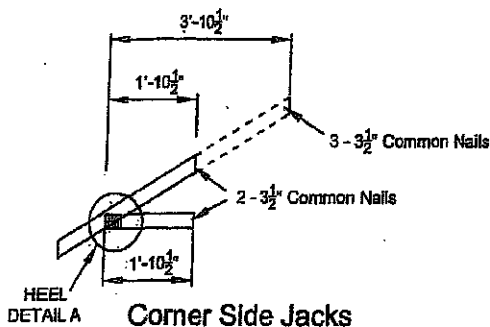
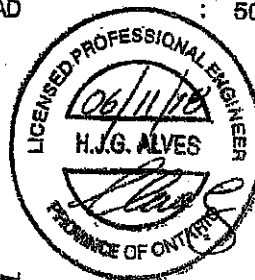
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

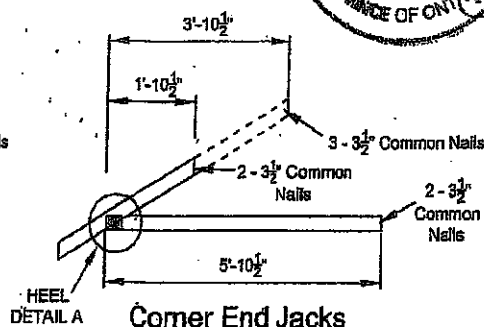
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

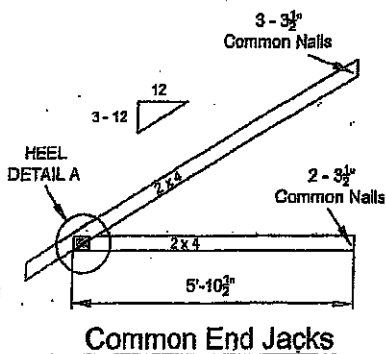
TOTAL LOAD : 50.5 P.S.F



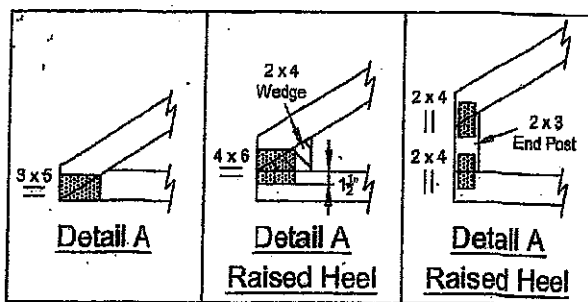
Corner Side Jacks



Corner End Jacks



Common End Jacks



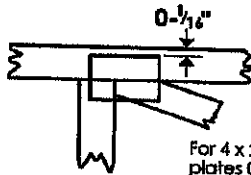
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16\"/>

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



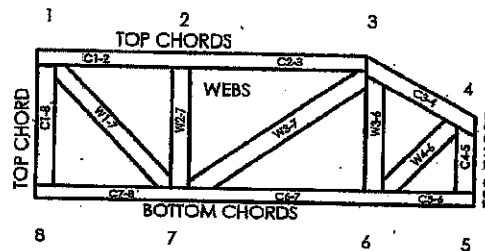
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIL-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

TOWN OF CALEPON
BUILDING SECTION
FILE NO.

JUN 13 2019



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218

Feb 09, 2018