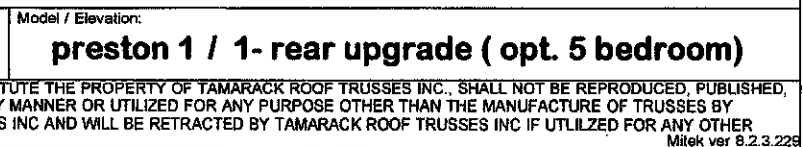
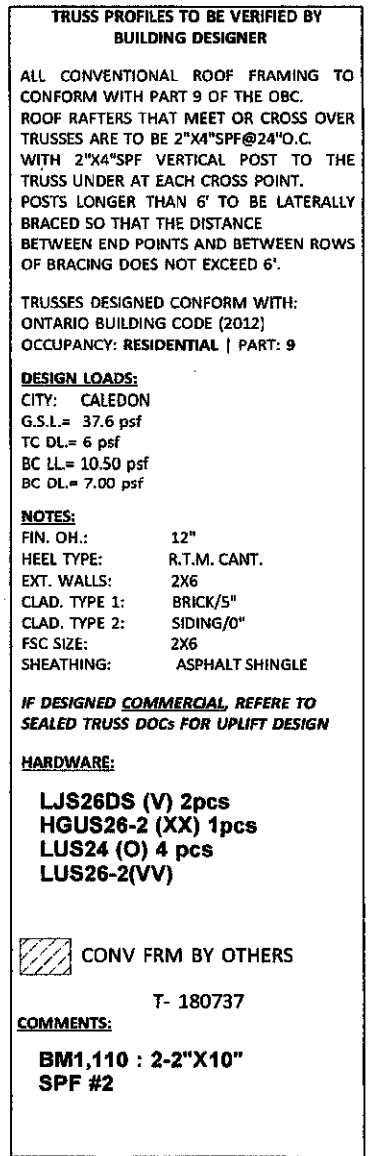


TOWN OF CALEDON
BUILDING SECTION
FILE NO _____





DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 1
 Lot #:
 Elevation: 1- rear upgrade (opt. 5 bedroom)

Job Track: 50120
 PlanLog: 200168
 Layout ID: 401597
 Ref #
 Page: 1 of 2
 Date: 02/26/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	1 2-ply	T1210 Hip Girder	9/12	29-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	277.77 176.00		
	2	T2 Hip	9/12	29-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	257.01 162.67		
	2	T3 Hip	9/12	29-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.02 162.67		
	2	T4 Hip	9/12	29-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	256.45 166.33		
	2	T5 Hip	9/12	29-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.41 182.00		
	2	T6 Hip	9/12	29-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.37 178.33		
	2	T7 Hip	9/12	29-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	275.24 176.00		
	3	T8 Hip	9/12	29-00-00	11-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	449.62 284.00		
	1	T9 Common Girder	9/12	10-02-00	5-04-00	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	50.43 32.83		
	1	T9G GABLE	9/12	10-02-00	5-04-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	46.57 31.50		
	6	T10 Common	9/12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	338.76 226.00		
	2	T10S Scissor	9/12 4/12	12-03-00	6-01-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	108.28 72.67		
	2	T11 Common	5/12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	63.43 38.00		

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 1
 Lot #:
 Elevation: 1- rear upgrade (opt. 5 bedroom)

Job Track: 50120
 PlanLog: 200168
 Layout ID: 401597
 Ref #
 Page: 2 of 2
 Date: 02/26/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T11Z Common Girder	5 /12	9-11-00	2-06-02	2 x 4 2 x 6		5-06 5-06	31.71 19.00		
	2	T111 Common	9 /12	15-09-00	7-05-02	2 x 4		1-06-04 1-06-04	132.68 87.00		
	1	G111 GABLE	9 /12	15-09-00	7-05-02	2 x 4		1-06-04 1-06-04	70.69 46.50		
	13	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	218.33 138.67		
	5	J3 Jack-Open	8 /12	2-06-13	2-11-01	2 x 4	1-03-08	9-10 2-06-02	44.21 30.00		
	2	J4 Jack-Open	4 /12	5-05-08	2-07-03	2 x 4	1-05-00	3-15 2-01-12	29.2 18.67		

TOTAL # TRUSS= 55 TOTAL BFT OF ALL TRUSSES= 2404.84 BFT. TOTAL WEIGHT OF ALL TRSSES 3749.95 LBS

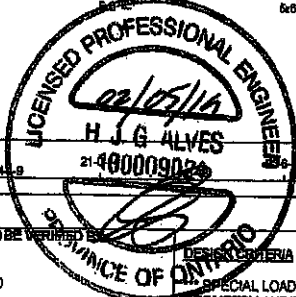
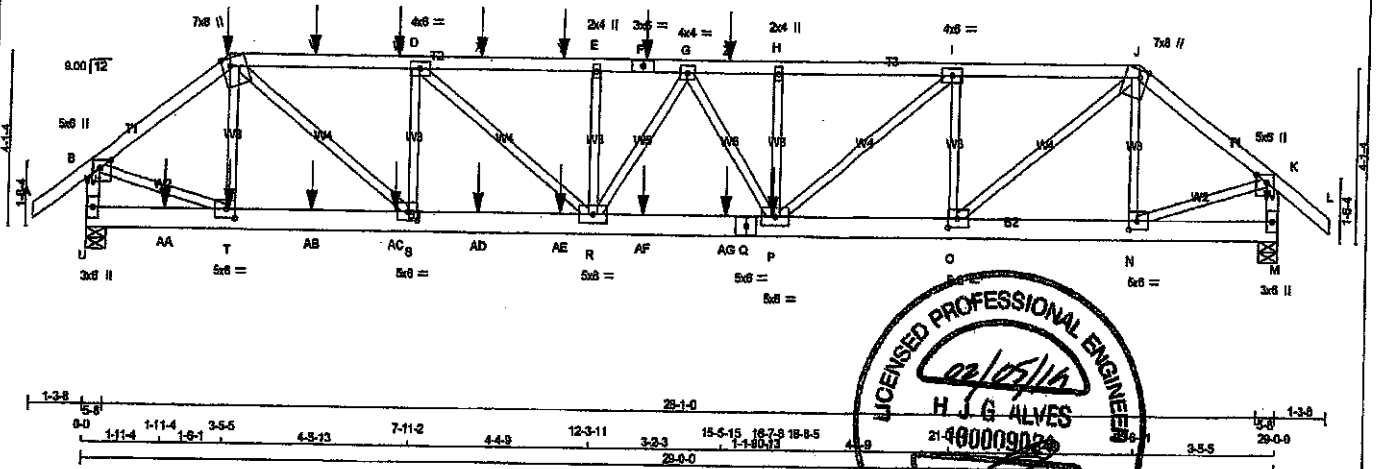
HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
6	Hardware	LJS26DS	
4	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 12

JOB NAME 200168-400364	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitok Industries, Inc. Tue Feb 5 20:42:03 2018 Page 1
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 1-3-8 0-0 3-5-5 4-5-13 7-11-2 4-4-9 12-3-11 2-2-5 14-6-0 15-5-15 18-8-5 4-4-9 21-0-14 25-8-11 3-5-5 29-0-0 30-3-8
 Scale = 1:50.0



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - F	2x4	DRY	No.2
F - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
U - B	2x4	DRY	No.2
M - K	2x4	DRY	No.2
U - Q	2x6	DRY	No.2
Q - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122'X3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-F	12	SIDE (61.0)
F-J	12	SIDE (61.0)
J-L	12	TOP
U-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122'X3") SPIRAL NAILS		
U-Q	12	SIDE (183.1)
Q-M	12	SIDE (183.1)
WEBS: (0.122'X3") SPIRAL NAILS		
2x3	6	SIDE (176.7)
H-P	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT	TMVW+p	MT20	5.0	6.0	Edge 3.00
B	TTWW+p	MT20	7.0	8.0	Edge 2.50
C	TMVW-i	MT20	4.0	6.0	
D	TMVW+w	MT20	2.0	4.0	
E	TS-i	MT20	3.0	6.0	
F	TMVW-i	MT20	4.0	4.0	
G	TMVW+w	MT20	2.0	4.0	
H	TMVW-i	MT20	4.0	6.0	
I	TTWW+m	MT20	7.0	8.0	Edge 2.50
K	TMVW+p	MT20	5.0	6.0	Edge 3.00
M	BMV1+p	MT20	3.0	6.0	
N	BMVW-i	MT20	5.0	6.0	2.50 2.50
O	BMVW-i	MT20	5.0	6.0	2.50 2.75
P	BMVWV-i	MT20	5.0	6.0	
Q	BS-i	MT20	5.0	6.0	
R	BMVWV-i	MT20	5.0	6.0	
S	BMVW-i	MT20	5.0	6.0	2.50 2.75
T	BMVW-i	MT20	5.0	6.0	2.50 2.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
FACTORED	MAXIMUM FACTORED	INPUT	REQD		
GROSS REACTION	GROSS REACTION	BRG	BRG		
JT VERT	DOWN	HORIZ	UPLIFT	IN-SX	IN-SX
U	3855	0	0	5-8	5-8
M	3425	0	0	5-8	5-8

UNFACTORED REACTIONS					
1ST LCASE	MAX/MIN	COMPONENT	REACTIONS		
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
U	2935	1684 / 0	538 / 0	0 / 0	712 / 0
M	2540	1464 / 0	483 / 0	0 / 0	612 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED CSI (LC)
FR-TO			
A-B	0 / 42	-102.1 -102.1 0.08 (1)	10.00
B-C	-4194 / 0	-102.1 -102.1 0.19 (1)	4.50
C-V	-1820 / 0	-102.1 -102.1 0.43 (1)	3.57
V-W	-1610 / 0	-102.1 -102.1 0.43 (1)	3.57
W-D	-1610 / 0	-102.1 -102.1 0.43 (1)	3.57
D-X	-7849 / 0	-102.1 -102.1 0.52 (1)	3.14
X-Y	-7849 / 0	-102.1 -102.1 0.52 (1)	3.14
Y-E	-7849 / 0	-102.1 -102.1 0.52 (1)	3.14
E-F	-7849 / 0	-102.1 -102.1 0.52 (1)	3.14
F-G	-7849 / 0	-102.1 -102.1 0.52 (1)	3.30
G-Z	-7982 / 0	-102.1 -102.1 0.28 (1)	3.30
Z-H	-7982 / 0	-102.1 -102.1 0.28 (1)	3.30
H-I	-7982 / 0	-102.1 -102.1 0.44 (1)	3.10
I-J	-5948 / 0	-102.1 -102.1 0.31 (1)	3.60
J-K	-3585 / 0	-102.1 -102.1 0.17 (1)	4.81
K-L	0 / 42	-102.1 -102.1 0.08 (1)	10.00
U-B	-3891 / 0	0.0 0.0 0.22 (1)	5.98
M-K	-3381 / 0	0.0 0.0 0.19 (1)	6.35

WEBS			
MEMB.	FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO			
T-C	-719 / 0	0.09 (1)	0.09 (1)
C-S	0 / 3879	0.48 (1)	0.48 (1)
S-D	-2079 / 0	0.28 (1)	0.28 (1)
D-R	0 / 1955	0.24 (1)	0.24 (1)
R-E	-535 / 0	0.07 (1)	0.07 (1)
P-H	-380 / 0	0.05 (1)	0.05 (1)
H-I	0 / 2779	0.34 (1)	0.34 (1)
O-I	-2308 / 0	0.29 (1)	0.29 (1)
C-J	0 / 3890	0.48 (1)	0.48 (1)
N-J	-697 / 0	0.09 (1)	0.09 (1)
B-T	0 / 3507	0.43 (1)	0.43 (1)
N-K	0 / 3006	0.37 (1)	0.37 (1)
R-G	-490 / 0	0.08 (1)	0.08 (1)
G-P	0 / 128	0.02 (1)	0.02 (1)

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	LC1	MAX-	MAX+	
C	3-5-5	-50	-56		
C	3-5-5	-130	-130		
C	3-5-5	-285	-285		

TOTAL WEIGHT = 2 X 139 = 278 lb

DESIGNER'S SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH LL = 28.0 PSF
 DL = 8.0 PSF
 BOT CH LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 62.5 PSF

SPACING = 240 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
 ADDITIONAL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2018, CBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.97")
 CALCULATED VERT. DEFL. (LL) = 1 / 999 (0.18")
 ALLOWABLE DEFL. (TL) = L/360 (0.97")
 CALCULATED VERT. DEFL. (TL) = 1 / 999 (0.31")

CSI: TC=0.52/1.00 (D-E-1), BC=0.50/1.00 (P-R-1),
 WB=0.48/1.00 (J-C-1), SI=0.20/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE GRIP (DRY)	SHEAR	SECTION			
(PSI)	(PLI)	(PLI)			
MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1667	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
 JSI METAL= 0.78 (C) (INPUT = 1.00)

DRWG NO. TAM T1902646
 STRUCTURAL
 COMPONENT ONLY 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T1Z	1	2	Preston 1	
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue Feb 5 20:42:03 2019 Page 2	
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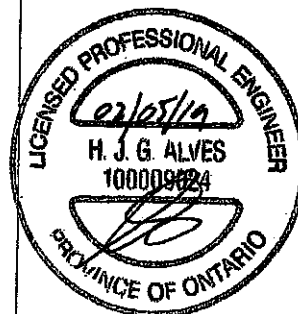
PLATES (table is in inches)

JT TYPE PLATES W LEN Y X
U BMV1+p MT20 3.0 6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	13-6-1	-123	-123	---	BACK	VERT	TOTAL	---	---
P	18-7-8	-1378	-1378	---	BACK	VERT	TOTAL	---	---
T	3-8-1	-55	-70	---	BACK	VERT	TOTAL	---	---
V	5-8-1	-123	-123	---	BACK	VERT	TOTAL	---	---
W	7-8-1	-123	-123	---	BACK	VERT	TOTAL	---	---
X	9-8-1	-123	-123	---	BACK	VERT	TOTAL	---	---
Y	11-8-1	-123	-123	---	BACK	VERT	TOTAL	---	---
Z	15-5-15	-123	-123	---	BACK	VERT	TOTAL	---	---
AA	1-11-4	-55	-70	---	BACK	VERT	TOTAL	---	---
AB	5-8-1	-55	-70	---	BACK	VERT	TOTAL	---	---
AC	7-8-1	-55	-70	---	BACK	VERT	TOTAL	---	---
AD	9-8-1	-55	-70	---	BACK	VERT	TOTAL	---	---
AE	11-8-1	-55	-70	---	BACK	VERT	TOTAL	---	---
AF	13-6-1	-55	-70	---	BACK	VERT	TOTAL	---	---
AG	15-5-15	-55	-70	---	BACK	VERT	TOTAL	---	---



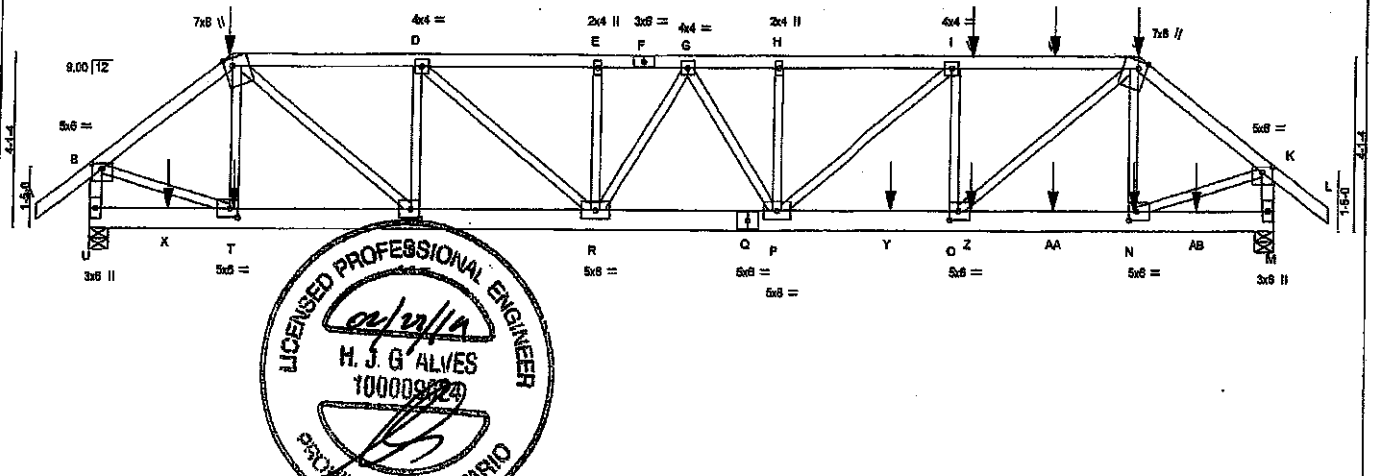
DWG NO. TAM T1902646
STRUCTURAL
COMPONENT ONLY 42

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401596	T1Z10	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed Feb 27 17:51:53 2019 Page 1
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LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. A - C 2x4 DRY No.2 SPF C - F 2x4 DRY No.2 SPF F - J 2x4 DRY No.2 SPF J - L 2x4 DRY No.2 SPF U - B 2x4 DRY No.2 SPF M - K 2x4 DRY No.2 SPF U - Q 2x6 DRY No.2 SPF Q - M 2x6 DRY No.2 SPF ALL WEBS 2x3 DRY No.2 SPF EXCEPT DRY: SEASONED LUMBER. DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS: CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF) TOP CHORDS : (0.122"x3") SPIRAL NAILS A-C 1 12 SIDE(81.0) C-F 1 12 SIDE(81.0) F-J 1 12 SIDE(81.0) J-L 1 12 SIDE(81.0) U-B 1 12 TOP M-K 1 12 TOP BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS U-Q 2 12 SIDE(183.1) Q-M 2 12 SIDE(183.1) WEBS : (0.122"x3") SPIRAL NAILS T-C 1 6 SIDE(205.3) 2x3 1 6 NAILS TO BE DRIVEN FROM ONE SIDE ONLY. GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS. TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY. SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP. PLATES (table in inches) JT TYPE PLATES W LEN Y X B TMWV-p MT20 5.0 6.0 1.50 3.00 C TTWW+m MT20 7.0 8.0 Edge 2.50 D G.I. D TMWV-t MT20 4.0 4.0 E TMWV-w MT20 2.0 4.0 F TS-t MT20 3.0 8.0 H TMWV-w MT20 2.0 4.0 J TMWV+m MT20 7.0 8.0 Edge 2.50 K TMWV-p MT20 5.0 6.0 1.50 3.00 M BMV1+p MT20 3.0 6.0 N BMWV-t MT20 5.0 8.0 2.50 2.25 O BMWV-t MT20 5.0 6.0 2.50 2.50 P BMWVW-t MT20 5.0 8.0 Q BS-t MT20 5.0 6.0 R BMWVW-t MT20 5.0 8.0 S BMWV-t MT20 5.0 6.0 2.50 2.50 T BMWV-t MT20 5.0 6.0 2.50 2.25 U BMV1+p MT20 3.0 6.0				UNIFORM SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS FACTORED MAXIMUM FACTORED INPUT REQD GROSS REACTION GROSS REACTION BRG BRG JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX U 4484 0 4484 0 0 5-8 5-8 M 4193 0 4193 0 0 5-8 5-8 UNFACTORED REACTIONS 1ST LCASE MAX/MIN COMPONENT REACTIONS JT COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL U 3301 1931 / 0 582 / 0 0 / 0 0 / 0 798 / 0 0 / 0 M 3107 1797 / 0 559 / 0 0 / 0 0 / 0 750 / 0 0 / 0 BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.24 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED. LOADING TOTAL LOAD CASES: (4) CHORDS MEMB. MAX. FACTORED FORCE (LBS) FACTORED VERT. LOAD LC1 MAX. (PLF) CSI (LC) UNBRAC MAX. (LBS) WEBS FR-TO FROM TO LENGTH FR-TO A-B 0 / 42 -102.1 -102.1 0.08 (1) 10.00 T-C 0 / 483 0.06 (2) B-C -4751 / 0 -102.1 -102.1 0.21 (1) 4.25 C-S 0 / 2822 0.32 (1) C-D -3824 / 0 -102.1 -102.1 0.31 (1) 3.81 S-D -1450 / 0 0.18 (1) D-E -8938 / 0 -102.1 -102.1 0.37 (1) 3.47 D-R 0 / 1463 0.18 (1) E-F -8938 / 0 -102.1 -102.1 0.22 (1) 3.59 R-E -330 / 0 0.04 (1) F-G -8938 / 0 -102.1 -102.1 0.22 (1) 3.59 P-H -294 / 0 0.04 (1) G-H -7534 / 0 -102.1 -102.1 0.23 (1) 3.45 P-I 0 / 998 0.12 (1) H-I -7534 / 0 -102.1 -102.1 0.48 (1) 3.24 O-I -1349 / 0 0.17 (1) I-V -8774 / 0 -102.1 -102.1 0.45 (1) 3.41 O-J 0 / 4233 0.52 (1) V-W -8774 / 0 -102.1 -102.1 0.45 (1) 3.41 N-J -942 / 0 0.12 (1) W-J -8774 / 0 -102.1 -102.1 0.45 (1) 3.41 B-T 0 / 3972 0.49 (1) J-K -4436 / 0 -102.1 -102.1 0.20 (1) 4.39 N-K 0 / 3708 0.48 (1) K-L 0 / 42 -102.1 -102.1 0.08 (1) 10.00 R-G -899 / 0 0.11 (1) L-B -4385 / 0 0.0 0.0 0.25 (1) 5.70 G-P 0 / 479 0.06 (1) M-K -4097 / 0 0.0 0.0 0.23 (1) 5.68 U-X 0 / 0 -38.5 -38.5 0.05 (2) 10.00 X-T 0 / 0 -38.5 -38.5 0.05 (2) 10.00 T-S 0 / 3905 -38.5 -38.5 0.28 (1) 10.00 S-R 0 / 5824 -38.5 -38.5 0.46 (1) 10.00 R-Q 0 / 7282 -38.5 -38.5 0.59 (1) 10.00 Q-P 0 / 7282 -38.5 -38.5 0.59 (1) 10.00 P-Y 0 / 6775 -38.5 -38.5 0.87 (1) 10.00 Y-O 0 / 6775 -38.5 -38.5 0.87 (1) 10.00 O-Z 0 / 3514 -38.5 -38.5 0.59 (1) 10.00 Z-AA 0 / 3514 -38.5 -38.5 0.59 (1) 10.00 AA-N 0 / 3514 -38.5 -38.5 0.59 (1) 10.00 N-AB 0 / 0 -38.5 -38.5 0.04 (2) 10.00 AB-M 0 / 0 -38.5 -38.5 0.04 (2) 10.00 FACTORED CONCENTRATED LOADS (LBS) JT LOC. LC1 MAX. MAX+ FACE DIR TYPE HEEL CONN. C 3-5-5 -50 -56 -- FRONT VERT DEAD -- -- C 3-5-5 -285 -285 -- FRONT VERT SNOW -- -- J 25-6-11 -50 -56 -- FRONT VERT DEAD -- -- J 25-6-11 -130 -130 -- FRONT VERT TOTAL -- -- J 25-6-11 -285 -285 -- FRONT VERT SNOW -- -- N 25-5-15 -55 -70 -- FRONT VERT TOTAL -- -- T 3-6-8 -1489 -1489 -- FRONT VERT TOTAL -- -- V 21-5-15 -123 -123 -- FRONT VERT TOTAL -- --				DESIGN CRITERIA *** SPECIAL LOADS ANALYSIS *** GEOMETRY AND/OR BASIC LOADS CHANGED BY USER. LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 6.0 PSF BOT CH. LL = 10.5 PSF DL = 7.0 PSF TOTAL LOAD = 52.5 PSF SPACING = 24.0 IN. G.C. LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12 *** NON STANDARD GIRDER *** ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES. THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (55 % OF 37.8 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.97") CALCULATED VERT. DEFL.(LL) = L/999 (0.18") ALLOWABLE DEFL.(TL) = L/360 (0.97") CALCULATED VERT. DEFL.(TL) = L/999 (0.29") CSI: TC=0.46/1.00 (H-I), BC=0.87/1.00 (O-P:1), WB=0.52/1.00 (J-O:1), SS=0.38/1.00 (O-P:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 610 354 1667 788 1997 1658 PLATE PLACEMENT TOL = 0.250 inches PLATE ROTATION TOL = 5.0 Deg. JSI GRIP= 0.90 (C) (INPUT = 0.90) JSI METAL= 0.70 (Q) (INPUT = 1.00)			
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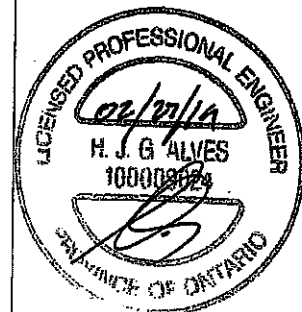
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DWG NO.
401596	T1Z10	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 M Tek Industries, Inc. Wed Feb 27 17:51:53 2019 Page 2		
				ID:IFxFHxFe38FX8N7VhzQQIYyQBwW-gj5fW3SRxj6J9BKp2ommmH9EN1JNNZqZlonr4zaUxQ		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	23-5-15	-123	-123	—	FRONT	VERT	TOTAL	—	—
X	1-11-4	-55	-70	—	FRONT	VERT	TOTAL	—	—
Y	18-8-8	-1489	-1489	—	FRONT	VERT	TOTAL	—	—
Z	21-5-15	-55	-70	—	FRONT	VERT	TOTAL	—	—
AA	23-5-15	-55	-70	—	FRONT	VERT	TOTAL	—	—
AB	27-0-12	-55	-70	—	FRONT	VERT	TOTAL	—	—

RECEIVED
JUN 13 2019

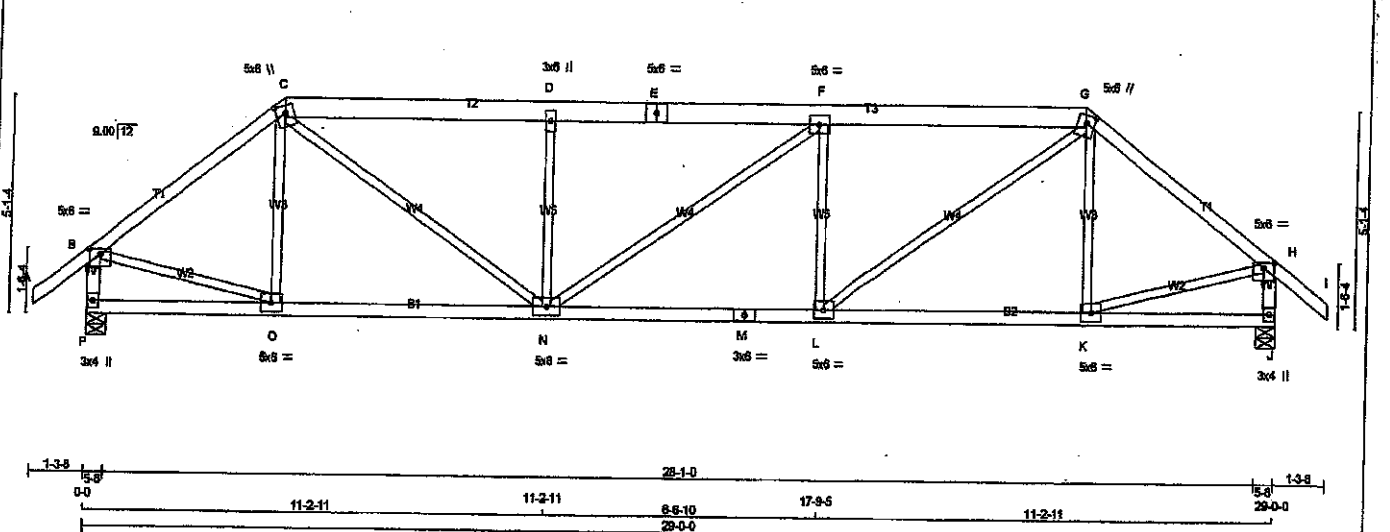
TOWN OF CALEDON
BUILDING SECTION
FILE NO. _____



DWG NO. TAM 17904519
STRUCTURAL
COMPONENT ONLY 3/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T2	2	1	Preston 1	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitak Industries, Inc. Tue Feb 5 20:42:04 2019 Page 1
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 29-0-0 30-3-8
 Scale = 1:50.0



TOTAL WEIGHT = 2 X 129 = 257 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x6	DRY	No.2	SPF	
E - G	2x6	DRY	No.2	SPF	
G - I	2x4	DRY	No.2	SPF	
P - B	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
P - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TTWW+m	MT20	5.0	6.0	2.25	2.50
D	TMVW-w	MT20	3.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMVW-t	MT20	5.0	6.0		
G	TTWW+m	MT20	5.0	6.0	2.25	2.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K, L, O						
K	BMVW-t	MT20	5.0	6.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	5.0	6.0		
P	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
P	2180	0	2180	0
J	2180	0	2180	0

UNFACTORED REACTIONS

1ST LOASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL -
P	1820	822 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1820	822 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.08 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 MAX (FLP) CSI (LC)	MEMB.	FORCE (LBS)
FR-TO			PR-TO	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	C-C	-201 / 98
B-C	-2157 / 0	-102.1 -102.1 0.54 (1)	K-G	-201 / 98
C-D	-3071 / 0	-102.1 -102.1 0.32 (1)	B-O	0 / 1786
D-E	-3071 / 0	-102.1 -102.1 0.32 (1)	K-H	0 / 1786
E-F	-3071 / 0	-102.1 -102.1 0.32 (1)	L-G	0 / 1681
F-G	-3071 / 0	-102.1 -102.1 0.32 (1)	C-N	0 / 1681
G-H	-2157 / 0	-102.1 -102.1 0.54 (1)	L-F	-732 / 0
H-I	0 / 42	-102.1 -102.1 0.14 (1)	N-D	-732 / 0
P-B	-2115 / 0	0.0 0.0 0.22 (1)	N-F	0 / 1
J-H	-2115 / 0	0.0 0.0 0.22 (1)		
P-O	0 / 0	-38.5 -38.5 0.22 (3)		
O-N	0 / 1734	-38.5 -38.5 0.47 (2)		
N-M	0 / 3071	-38.5 -38.5 0.67 (1)		
M-L	0 / 3071	-38.5 -38.5 0.67 (1)		
L-K	0 / 1734	-38.5 -38.5 0.47 (2)		
K-J	0 / 0	-38.5 -38.5 0.22 (3)		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBCC 2010, OBC 2012
 - CSA 086-08, CSA 086-14
 - TPC 2011, TPC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/886 (0.14")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/886 (0.23")

CSI: TC=0.54/1.00 (B-C:1), BC=0.87/1.00 (L-N:1), WB=0.40/1.00 (H-K:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

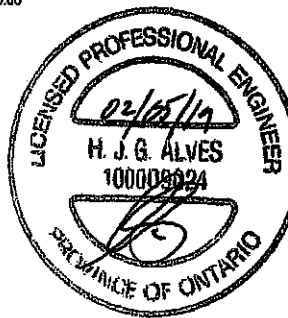
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1657 788 1997 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.80 (K) (INPUT = 0.90)
 JSI METAL= 0.88 (N) (INPUT = 1.00)

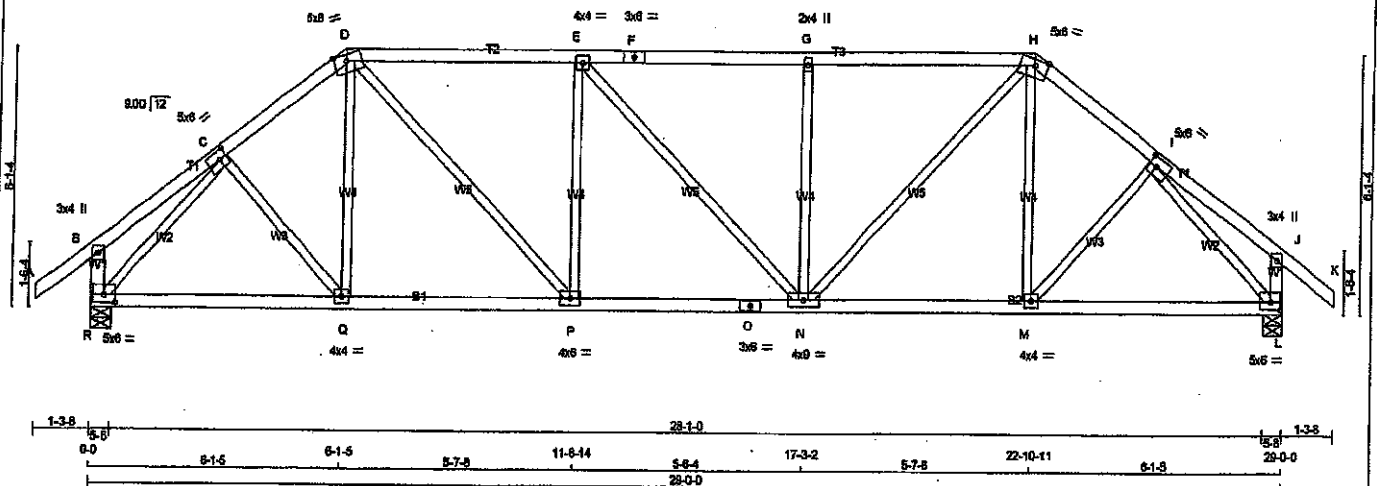


DWG NO. TAM 7190 2647
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T3	2	1	Preston 1	
Tamarack Roof Truss, Burlington		Version 6.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:05 2019 Page 1			
		ID:IFXFXH9e38FX8N7VhzQQIYyQBwVW-Nd0qht8zZJUT7QSSiNZavUzr44s8LQXGpXVbAUQzo56G			

1-3-8 0-0 3-1-15 3-1-15 2-11-7 5-1-5 5-7-8 11-8-14 17-3-2 5-7-8 22-10-11 2-11-7 25-10-1 3-1-15 29-0-0 30-3-8 1-3-8 1-3-8

Scale = 1:50.0



CHORDS	SIZE	DRY	NO.2	SPF
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

PLATES (table is in inches)	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	5.0	6.0	2.50	2.25
D	TMVW-m	MT20	5.0	6.0	Edges	3.50
E	TMVW-1	MT20	4.0	4.0		
F	TS-1	MT20	3.0	6.0		
G	TMVW-m	MT20	2.0	4.0		
H	TMVW-m	MT20	5.0	6.0	Edges	3.50
I	TMVW-1	MT20	5.0	6.0	2.50	2.25
J	TMV+p	MT20	3.0	4.0		
L	BMVW-1	MT20	5.0	6.0	2.25	3.00
M	BMVW-1	MT20	4.0	4.0		
N	BMVW-1	MT20	4.0	4.0		
O	BS-1	MT20	3.0	6.0		
P	BMVW-1	MT20	4.0	4.0		
Q	BMVW-1	MT20	4.0	4.0		
R	BMVW-1	MT20	5.0	6.0	2.25	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
		GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
R	2180	0	2180	0	0	5-8	5-8		
L	2180	0	2180	0	0	5-8	5-8		

UNFACTORED REACTIONS		1ST LOSE		MAX/MIN. COMPONENT REACTIONS		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SNOW	LIVE	PERM. LIVE
R	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	1820	922 / 0	305 / 0	0 / 0
L	1820	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0	1820	922 / 0	305 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.74 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-Q	0 / 212	0.05 (2)
B-C	0 / 18	-102.1 -102.1	0.12 (1)	10.00	Q-D	0 / 202	0.05 (3)
C-D	-2175 / 0	-102.1 -102.1	0.18 (1)	4.48	D-P	0 / 1140	0.28 (1)
D-E	-2517 / 0	-102.1 -102.1	0.59 (1)	3.74	P-E	-615 / 0	0.38 (1)
E-F	-2518 / 0	-102.1 -102.1	0.59 (1)	3.74	E-N	-2 / 0	0.00 (1)
F-G	-2518 / 0	-102.1 -102.1	0.59 (1)	3.74	N-G	-614 / 0	0.38 (1)
G-H	-2518 / 0	-102.1 -102.1	0.59 (1)	3.75	H-N	0 / 1197	0.28 (1)
H-I	-2175 / 0	-102.1 -102.1	0.18 (1)	4.48	M-H	0 / 202	0.05 (3)
I-J	0 / 18	-102.1 -102.1	0.12 (1)	10.00	M-I	0 / 212	0.05 (2)
J-K	0 / 42	-102.1 -102.1	0.14 (1)	10.00	R-C	-2413 / 0	0.82 (1)
R-B	-286 / 0	0.0	0.0	0.03 (1)	I-L	-2413 / 0	0.82 (1)
L-J	-286 / 0	0.0	0.0	0.03 (1)			
R-Q	0 / 1589	-38.5	-38.5	0.46 (2)			
Q-P	0 / 1723	-38.5	-38.5	0.49 (2)			
P-O	0 / 2517	-38.5	-38.5	0.51 (1)			
O-N	0 / 2517	-38.5	-38.5	0.51 (1)			
N-M	0 / 1723	-38.5	-38.5	0.49 (2)			
M-L	0 / 1589	-38.5	-38.5	0.46 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
CL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.6 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 084-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/898 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/899 (0.20")

CSI: TC=0.59/1.00 (D-E-1), BC=0.51/1.00 (N-P-1), WB=0.82/1.00 (I-L-1), SS=0.27/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

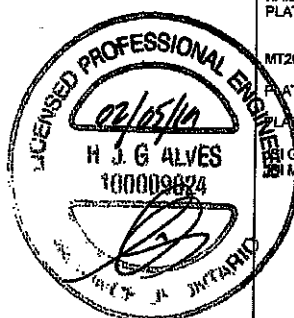
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES		PLATE GRIP(DRY) SHEAR SECTION	
(PSI)	(PL)	(PSI)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1667 788 1987 1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.87 (L) (INPUT = 0.90)
METAL = 0.78 (O) (INPUT = 1.00)

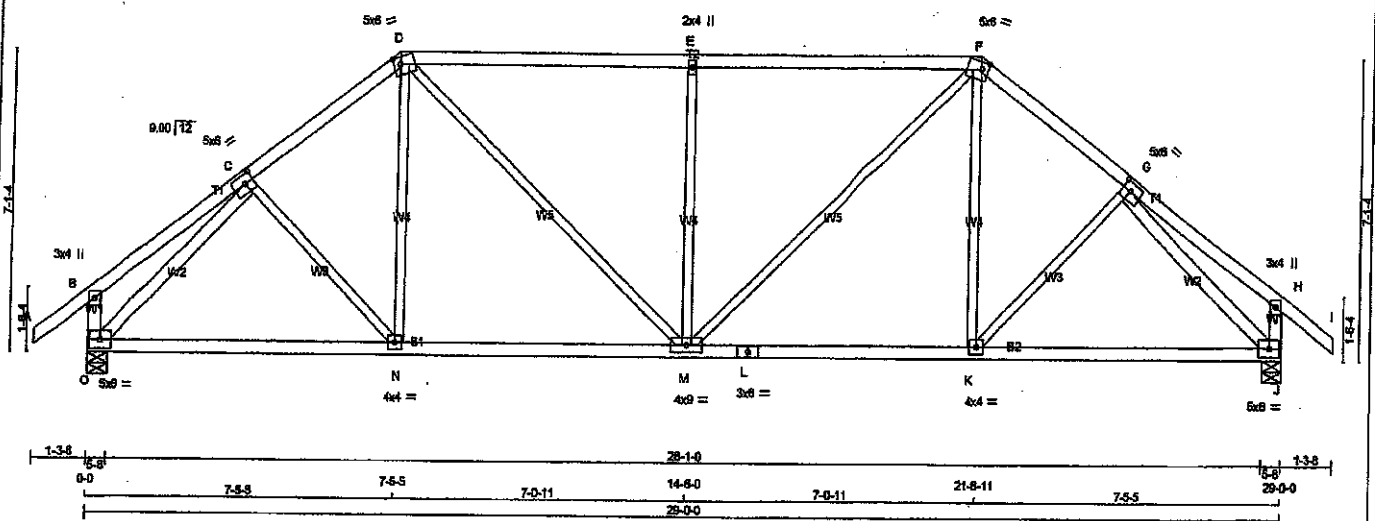


DWG NO. TAM T1902648
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: IFxHXe38FX8N7VhzQQIYyQBwW-Nd0qht8zZJL7QSSfINZevUzr1Ms7gQX5pXWbAUQzo56G

Scale = 1/50.0



LUMBER				
N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-1	MT20	5.0	6.0	2.50 2.50
D	TTVW-m	MT20	5.0	6.0	2.00 2.00
E	TMV+w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	6.0	2.00 2.00
G	TMVW-1	MT20	5.0	6.0	2.50 2.50
H	TMV+p	MT20	3.0	4.0	
J	BMVW-1	MT20	5.0	6.0	
K	BMVW-1	MT20	4.0	4.0	
L	BS-1	MT20	3.0	6.0	
M	BMVW-1	MT20	4.0	9.0	
N	BMVW-1	MT20	4.0	4.0	
O	BMVW-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
O	2180	0	2180	0	5-8	5-8	5-8
J	2180	0	2180	0	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM LIVE		WIND		DEAD		SOIL	
JT	COMBINED	JT	SNOW	JT	PERM LIVE	JT	WIND	JT	DEAD	JT	SOIL
O	1820	922 / 0	305 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0	394 / 0	0 / 0	0 / 0
J	1820	922 / 0	305 / 0	0 / 0	0 / 0	0 / 0	394 / 0	0 / 0	394 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.54 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-N	0 / 129	0.03 (3)
B-C	0 / 25	-102.1 -102.1	0.21 (1)	10.00	N-D	0 / 343	0.08 (2)
C-D	-2143 / 0	-102.1 -102.1	0.22 (1)	4.47	D-M	0 / 807	0.18 (1)
D-E	-2276 / 0	-102.1 -102.1	0.77 (1)	3.54	M-E	-884 / 0	0.77 (1)
E-F	-2276 / 0	-102.1 -102.1	0.77 (1)	3.54	M-F	0 / 807	0.18 (1)
F-G	-2143 / 0	-102.1 -102.1	0.22 (1)	4.47	K-F	0 / 343	0.08 (2)
G-H	0 / 25	-102.1 -102.1	0.21 (1)	10.00	K-G	0 / 129	0.03 (3)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	O-C	-2427 / 0	0.83 (1)
O-B	-288 / 0	0.0 0.0	0.03 (1)	7.81	G-J	-2427 / 0	0.83 (1)
J-H	-288 / 0	0.0 0.0	0.03 (1)	7.81			
O-N	0 / 1655	-38.5 -38.5	0.81 (2)	10.00			
N-M	0 / 1695	-38.5 -38.5	0.82 (2)	10.00			
M-L	0 / 1695	-38.5 -38.5	0.82 (2)	10.00			
L-K	0 / 1695	-38.5 -38.5	0.82 (2)	10.00			
K-J	0 / 1655	-38.5 -38.5	0.81 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CSI: TC=0.77/1.00 (E-F-1), EC=0.62/1.00 (M-N-2), WB=0.83/1.00 (O-C-1), SSI=0.35/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1657 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.80 (C) (INPUT = 1.00)



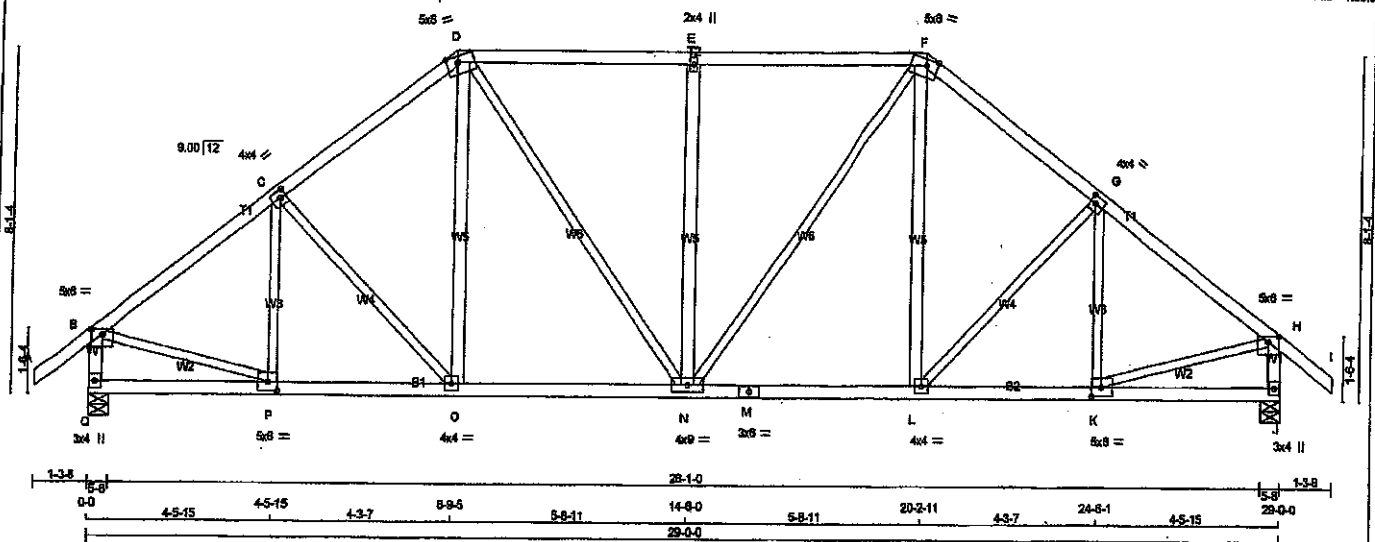
RECEIVED
JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION

DWG NO. TAM 1902649
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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 ID:IFxHXFe38FX3N7VhzQQYyQBwVW-rqaCuD9bK0c 2cRUXG980BNHZGVH90CzALK1tzo56F
 Scale = 1:50.0



TOTAL WEIGHT = 2 X 142 = 284 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - D	2x4	DRY	No.2
N - E	2x4	DRY	No.2
L - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTVW-m	MT20	5.0	8.0	1.75	3.25
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	8.0	1.75	3.25
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1-p	MT20	3.0	4.0		
K	BMVW-t	MT20	5.0	6.0	2.50	2.75
L	BMVW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMVW-t	MT20	4.0	9.0		
O	BMVW-t	MT20	4.0	4.0		
P	BMVW-t	MT20	5.0	6.0	2.50	2.75
Q	BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	2180	0	5-8	5-8
J	2180	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LOASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.29 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)		FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	P-C	-288 / 24	0.10 (1)	
B-C	-2168 / 0	-102.1 -102.1	0.31 (1)	C-O	-219 / 0	0.16 (1)	
C-D	-2045 / 0	-102.1 -102.1	0.30 (1)	O-D	0 / 376	0.06 (2)	
D-E	-1958 / 0	-102.1 -102.1	0.48 (1)	D-N	0 / 579	0.13 (1)	
E-F	-1958 / 0	-102.1 -102.1	0.48 (1)	N-E	-713 / 0	0.64 (1)	
F-G	-2045 / 0	-102.1 -102.1	0.30 (1)	N-F	0 / 579	0.13 (1)	
G-H	-2168 / 0	-102.1 -102.1	0.31 (1)	L-F	0 / 376	0.06 (2)	
H-I	0 / 42	-102.1 -102.1	0.14 (1)	L-G	-219 / 0	0.16 (1)	
Q-B	-2108 / 0	0.0 0.0	0.22 (1)	K-G	-288 / 24	0.10 (1)	
J-H	-2108 / 0	0.0 0.0	0.22 (1)	B-P	0 / 1820	0.41 (1)	
				K-H	0 / 1820	0.41 (1)	
Q-P	0 / 0	-38.5 -38.5	0.13 (3)				
P-O	0 / 1759	-38.5 -38.5	0.39 (1)				
O-N	0 / 1613	-38.5 -38.5	0.40 (2)				
N-M	0 / 1613	-38.5 -38.5	0.40 (2)				
M-L	0 / 1613	-38.5 -38.5	0.40 (2)				
L-K	0 / 1759	-38.5 -38.5	0.39 (1)				
K-J	0 / 0	-38.5 -38.5	0.13 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.6 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, OBC 2012
 - CSA 086-08, CSA 086-14
 - TPOC 2011, TPOC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (0.97")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.48/1.00 (D-E:1), BC=0.40/1.00 (N-O:2)
 WB=0.64/1.00 (E-N:1), BS=0.28/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

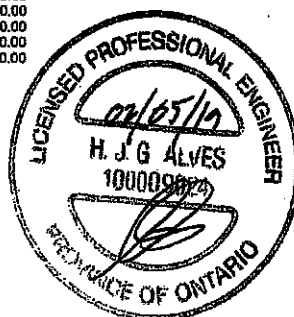
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.80)
 JSI METAL= 0.51 (M) (INPUT = 1.00)

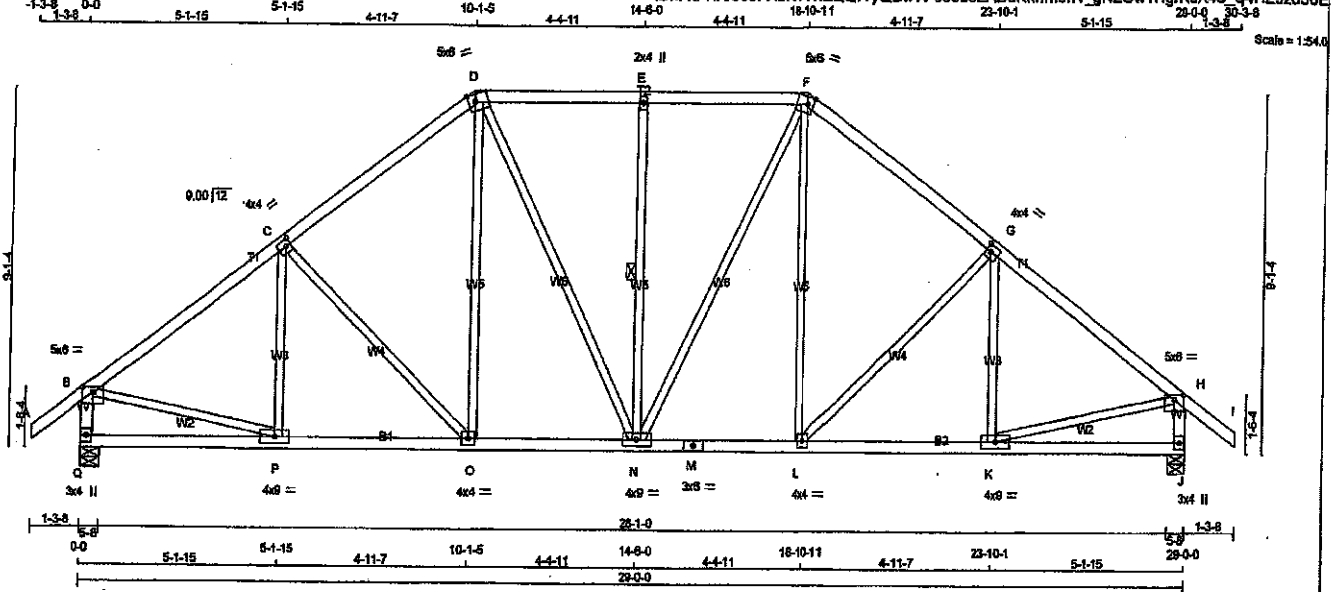


DRWG NO. TAM T5102650
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T6	2	1	Preston 1	
Tamarack Roof Truss, Burlington					

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TOTAL WEIGHT = 2 X 142 = 283 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - I	2x4	DRY	No.2	SPF	
I - M	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.00
C TMVW-w	MT20	4.0	4.0	2.00	1.50
D TMVW-m	MT20	5.0	6.0	Edge 2.00	
E TMVW-w	MT20	2.0	4.0		
F TMVW-w	MT20	5.0	6.0	Edge 2.00	
G TMVW-w	MT20	4.0	4.0	2.00	1.50
H TMVW-p	MT20	5.0	6.0	1.50	3.00
J BMV1-p	MT20	3.0	4.0		
K BMVW-w	MT20	4.0	9.0		
L BMVW-w	MT20	4.0	4.0		
M BS-1	MT20	3.0	6.0		
N BMVW-w	MT20	4.0	9.0		
O BMVW-w	MT20	4.0	4.0		
P BMVW-w	MT20	4.0	9.0		
Q BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
JT	2180	0	2180	0	0	5-8	5-8	5-8	5-8
J	2180	0	2180	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1820	822 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
J	1820	822 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO
A-B	0 / 42	-102.1 -102.1	0.14 (1) 10.00
B-C	-2188 / 0	-102.1 -102.1	0.42 (1) 4.21
C-D	-1954 / 0	-102.1 -102.1	0.39 (1) 4.44
D-E	-1709 / 0	-102.1 -102.1	0.28 (1) 4.82
E-F	-1709 / 0	-102.1 -102.1	0.28 (1) 4.82
F-G	-1954 / 0	-102.1 -102.1	0.39 (1) 4.44
G-H	-2188 / 0	-102.1 -102.1	0.42 (1) 4.21
H-I	0 / 42	-102.1 -102.1	0.14 (1) 10.00
Q-B	-2088 / 0	0.0 0.0	0.22 (1) 5.84
J-H	-2088 / 0	0.0 0.0	0.22 (1) 5.84
Q-P	0 / 0	-38.5 -38.5	0.19 (3) 10.00
P-O	0 / 1787	-38.5 -38.5	0.42 (2) 10.00
O-N	0 / 1535	-38.5 -38.5	0.34 (1) 10.00
N-M	0 / 1535	-38.5 -38.5	0.34 (1) 10.00
M-L	0 / 1535	-38.5 -38.5	0.34 (1) 10.00
L-K	0 / 1787	-38.5 -38.5	0.42 (2) 10.00
K-J	0 / 0	-38.5 -38.5	0.19 (3) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.87")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.87")
CALCULATED VERT. DEFL.(TL) = L/999 (0.12")

CSI: TC=0.42/1.00 (B-C:1), BC=0.42/1.00 (Q-P:2), WB=0.41/1.00 (B-P:1), SS=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

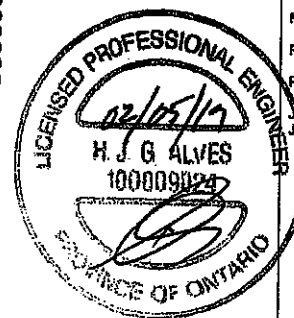
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLF)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP=0.89 (P) (INPUT = 0.90)

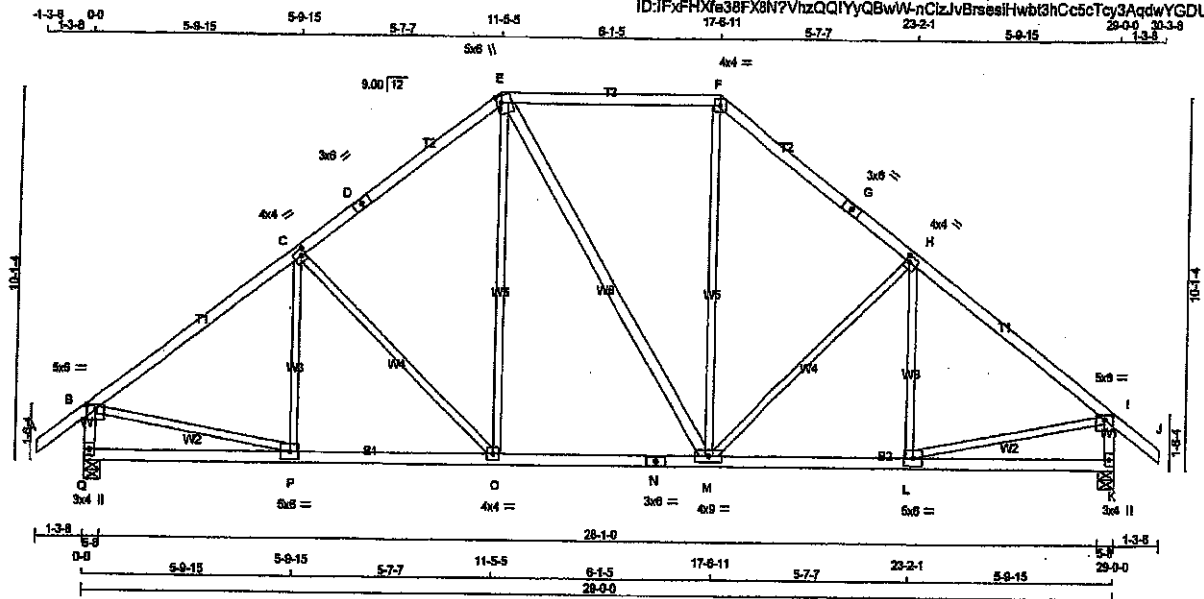
METAL=0.50 (M) (INPUT = 1.00)



DWG NO. TAM T1902651
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:IFxHXfa38FX8N7VhzQQIYyQBwW-nCizJvBrsesHwb3hCc5cTcy3AqdwYGDUqq5zo58D
Scale = 1:57.8



LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
G - J	2x4	DRY	No.2	SPF	
Q - B	2x4	DRY	No.2	SPF	
K - I	2x4	DRY	No.2	SPF	
Q - N	2x4	DRY	No.2	SPF	
N - K	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					
E - M	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	6.0	1.50	3.00
C TMWV-i	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTWV+m	MT20	5.0	6.0	2.25	1.50
F TTW-i	MT20	4.0	4.0		
G TS-i	MT20	3.0	6.0		
H TMWV-i	MT20	4.0	4.0	2.00	1.50
I TMWV-p	MT20	5.0	6.0	1.50	3.00
K BMV1+p	MT20	3.0	4.0		
L BMWV-i	MT20	5.0	6.0		
M BMWV-i	MT20	4.0	9.0		
N BS-i	MT20	3.0	6.0		
O BMWV-i	MT20	4.0	4.0		
P BMWV-i	MT20	5.0	6.0		
Q BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
Q	2180	0	5-8	5-8
K	2180	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0	0 / 0	394 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-123 / 150	
B-C	-2209 / 0	-102.1 -102.1	0.55 (1)	4.04	C-O	-492 / 0	
C-D	-1855 / 0	-102.1 -102.1	0.50 (1)	4.38	O-E	0 / 571	
D-E	-1855 / 0	-102.1 -102.1	0.50 (1)	4.38	E-M	0 / 1	
E-F	-1484 / 0	-102.1 -102.1	0.52 (1)	4.75	M-F	0 / 573	
F-G	-1856 / 0	-102.1 -102.1	0.50 (1)	4.38	M-H	-491 / 0	
G-H	-1856 / 0	-102.1 -102.1	0.50 (1)	4.38	L-H	-125 / 149	
H-I	-2209 / 0	-102.1 -102.1	0.55 (1)	4.04	B-P	0 / 1833	
I-J	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-I	0 / 1838	
Q-B	-2088 / 0	0.0	0.0	0.22 (1)	5.86		
K-I	-2087 / 0	0.0	0.0	0.22 (1)	5.86		
Q-P	0 / 0	-38.5	-38.5	0.24 (3)	10.00		
P-O	0 / 1801	-38.5	-38.5	0.46 (2)	10.00		
Q-N	0 / 1453	-38.5	-38.5	0.39 (2)	10.00		
N-M	0 / 1453	-38.5	-38.5	0.39 (2)	10.00		
M-L	0 / 1800	-38.5	-38.5	0.46 (2)	10.00		
L-K	0 / 0	-38.5	-38.5	0.23 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/860 (0.97")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/860 (0.97")
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")

CSI: TC=0.55/1.00 (B-C-1), BC=0.48/1.00 (O-P-2), WB=0.65/1.00 (C-O-1), SSI=0.24/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

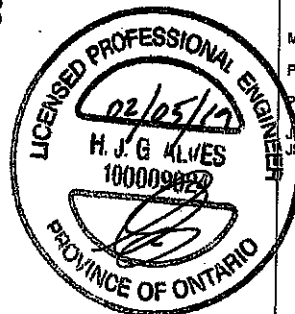
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 788 1987 1558

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

GRIP = 0.88 (F) (INPUT = 0.80)
METAL = 0.46 (N) (INPUT = 1.00)



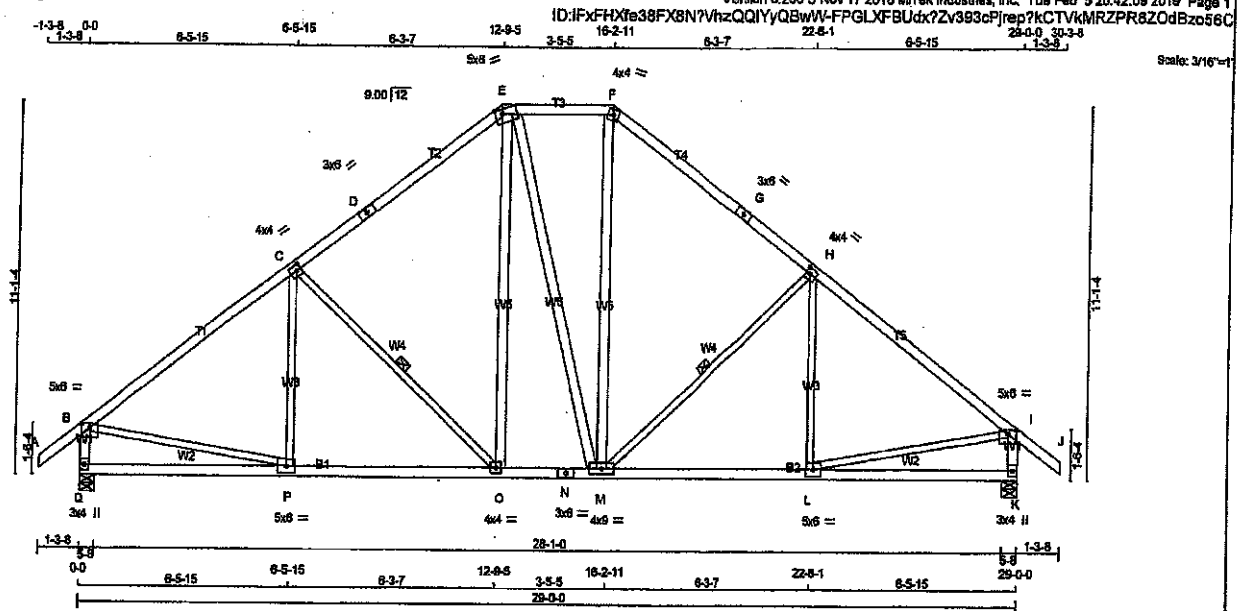
DRWG NO. TAM 71902652
STRUCTURAL
COMPONENT ONLY

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T8	3	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 3 X 150 = 450 LB
(M/F)

LUMBER	N.L.G.A. RULES	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - E	2x4	DRY	No.2
E - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

PLATE TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50 3.00
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	6.0	
E	TTVW-m	MT20	5.0	6.0	1.75 2.75
F	TTW-m	MT20	4.0	4.0	
G	TS-t	MT20	3.0	6.0	
H	TMVW-t	MT20	4.0	4.0	2.00 1.50
I	TMVW-p	MT20	5.0	6.0	1.50 3.00
K	BMV1-p	MT20	3.0	4.0	
L	BMVW-t	MT20	5.0	6.0	
M	BMVW-t	MT20	4.0	6.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW-t	MT20	4.0	4.0	
P	BMVW-t	MT20	5.0	6.0	
Q	BMV1-p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORIZ	DOWN	HORIZ
Q	2180	0	2180	0
K	2180	0	2180	0

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1620	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0	0 / 0
K	1620	922 / 0	305 / 0	0 / 0	0 / 0	384 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-O, H-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CBI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CBI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	P-C	-43 / 221	0.05 (3)	
B-C	-2211 / 0	-102.1 -102.1	0.71 (1)	C-O	-628 / 0	0.34 (1)	
C-D	-1738 / 0	-102.1 -102.1	0.63 (1)	Q-E	0 / 597	0.10 (1)	
D-E	-1738 / 0	-102.1 -102.1	0.63 (1)	E-M	0 / 4	0.00 (1)	
E-F	-1358 / 0	-102.1 -102.1	0.18 (1)	M-F	0 / 601	0.10 (1)	
F-G	-1738 / 0	-102.1 -102.1	0.63 (1)	M-H	-628 / 0	0.33 (1)	
G-H	-1738 / 0	-102.1 -102.1	0.63 (1)	L-H	-48 / 219	0.05 (3)	
H-I	-2210 / 0	-102.1 -102.1	0.71 (1)	B-P	0 / 1836	0.41 (1)	
I-J	0 / 42	-102.1 -102.1	0.14 (1)	L-I	0 / 1836	0.41 (1)	
Q-B	-2080 / 0	0.0 0.0	0.22 (1)				
K-I	-2080 / 0	0.0 0.0	0.22 (1)				
Q-P	0 / 0	-38.5 -38.5	0.32 (3)				
P-O	0 / 1806	-38.5 -38.5	0.55 (2)				
O-N	0 / 1357	-38.5 -38.5	0.33 (1)				
N-M	0 / 1357	-38.5 -38.5	0.33 (1)				
M-L	0 / 1806	-38.5 -38.5	0.54 (2)				
L-K	0 / 0	-38.5 -38.5	0.32 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 089-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.97)
CALCULATED VERT. DEFL.(LL) = L/999 (0.99)
ALLOWABLE DEFL.(TL) = L/360 (0.97)
CALCULATED VERT. DEFL.(TL) = L/999 (0.15)

CSI: TC=0.71/1.00 (B-C-1), BC=0.55/1.00 (O-P-2), WB=0.41/1.00 (B-P-1), SS=0.26/1.00 (H-I-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

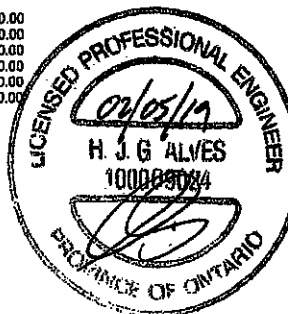
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1887 1666

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

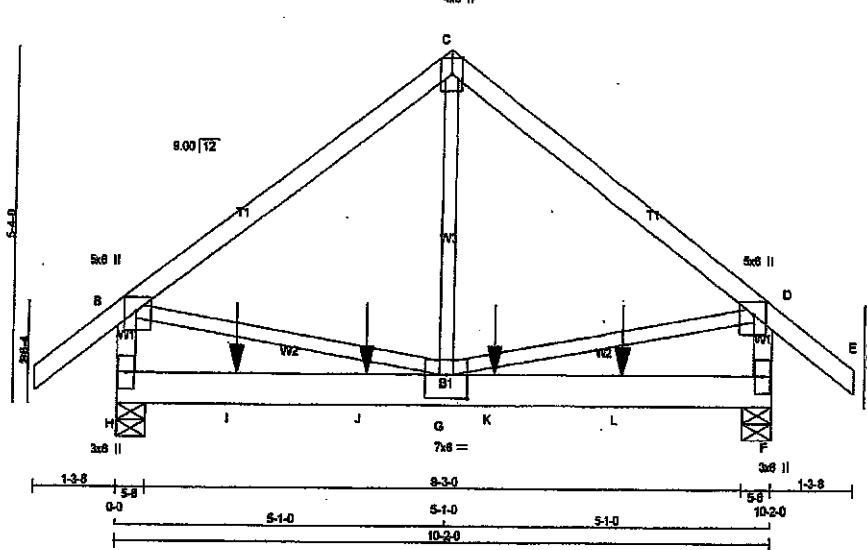
JSI GRIP= 0.87 (Q) (INPUT = 0.90)
JSI METAL= 0.48 (N) (INPUT = 1.00)



DRWG NO. TAM T1902653
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T9	1	1	Preston 1	
Tamarack Roof Truss, Burlington					

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 ID:IFxHX0e38FX8N?VhzQQYyQBwVW-jbqtkbC8OF7QWdKGA6E4B1YxMtoN5vaZgoJxAezo56B
 Scale: 3/8"=1'



TOTAL WEIGHT = 50 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - G	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x8	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	5.0	8.0	2.50 2.25
C	TTVW+p	MT20	4.0	8.0	Edge
D	TMVW+p	MT20	5.0	8.0	2.50 2.25
F	BMV1+p	MT20	3.0	8.0	
G	BMVWW+L	MT20	7.0	8.0	4.25 4.00
H	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
H	2129	0	2129	0	5-8	5-8	
F	2013	0	2013	0	5-8	5-8	

UNFACTORED REACTIONS

1ST LOASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	1549	987 / 0	214 / 0	0 / 0	0 / 0	347 / 0
H	1465	931 / 0	205 / 0	0 / 0	0 / 0	329 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.33 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.15 (1)	10.00	G-C	0 / 1472	0.38 (1)
B-C	-1659 / 0	-102.1 -102.1	0.82 (1)	4.33	B-G	0 / 1359	0.34 (1)
C-D	-1659 / 0	-102.1 -102.1	0.82 (1)	4.33	G-D	0 / 1359	0.34 (1)
D-E	0 / 42	-102.1 -102.1	0.15 (1)	10.00			
H-B	-1686 / 0	0.0 0.0	0.19 (1)	6.38			
R-D	-1686 / 0	0.0 0.0	0.19 (1)	6.38			
H-I	0 / 0	-38.5 -38.5	0.71 (1)	10.00			
I-J	0 / 0	-38.5 -38.5	0.71 (1)	10.00			
J-G	0 / 0	-38.5 -38.5	0.71 (1)	10.00			
G-K	0 / 0	-38.5 -38.5	0.71 (1)	10.00			
K-L	0 / 0	-38.5 -38.5	0.71 (1)	10.00			
L-F	0 / 0	-38.5 -38.5	0.71 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
I	1-10-1	-808	-808	---	FRONT	VERT	TOTAL	---	---
J	3-10-1	-808	-808	---	FRONT	VERT	TOTAL	---	---
K	5-10-1	-808	-808	---	FRONT	VERT	TOTAL	---	---
L	7-10-1	-808	-808	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-08, CSA 086-14
 - TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.34")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.06")
 ALLOWABLE DEFL. (TL) = L/360 (0.34")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.06")

CSI: TC=0.82/1.00 (C-D:1), BC=0.71/1.00 (G-H:1), WB=0.38/1.00 (C-G:1), SS=0.72/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

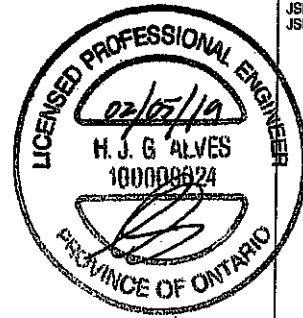
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1688

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

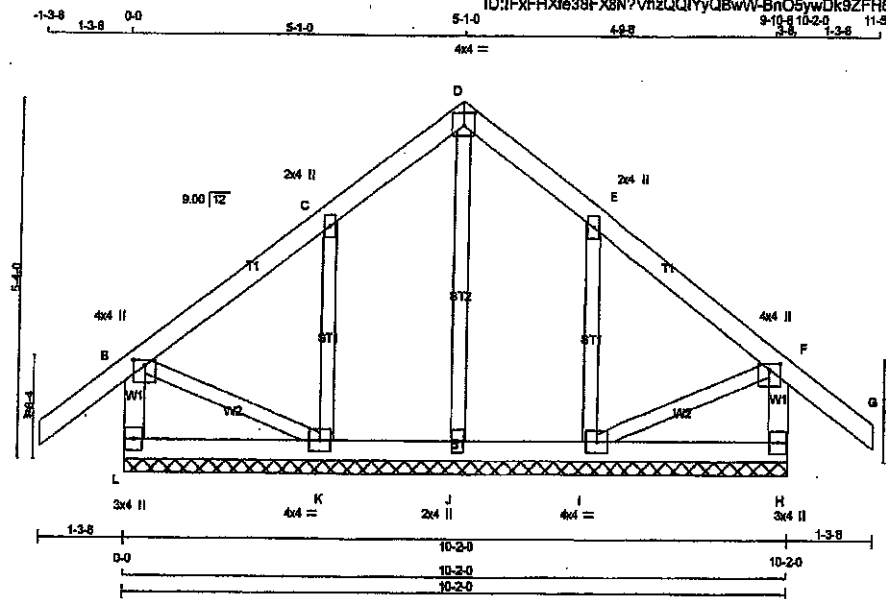
JSI GRIP= 0.84 (D) (INPUT = 0.90)
 JSI METAL= 0.35 (C) (INPUT = 1.00)



DWG NO. TAM 77902654
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T9G	QUANTITY 1	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:IFxHXfe38FX8N7VhzQQIYyQBwW-BnO5ywDk9ZF8NJSkplJE5DhHikqQLvS2V4zo56A
Scale = 1:31.6



TOTAL WEIGHT = 47 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				
GABLE STUDS SPACED AT	2'-0" OC.			

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C TMVW+w	MT20	2.0	4.0		
D TTVV+p	MT20	4.0	4.0	2.25	2.00
E TMVW+w	MT20	2.0	4.0		
F TMVW+p	MT20	4.0	4.0	1.00	2.00
H BMV1+p	MT20	3.0	4.0		
I BMVW1-i	MT20	4.0	4.0		
J BMV1+w	MT20	2.0	4.0		
K BMVW1-i	MT20	4.0	4.0		
L BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (FLP)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (FLP)	MAX. FACTORED CSI (LC)
FR-TO				FR-TO			
L-B	-283 / 0	0.0	0.0 0.03 (1)	J-D	-117 / 0	0.05 (1)	
A-B	0 / 42	-102.1	-102.1 0.14 (1)	K-C	-325 / 0	0.08 (1)	
B-C	-3 / 0	-102.1	-102.1 0.12 (1)	I-E	-325 / 0	0.08 (1)	
C-D	-35 / 0	-102.1	-102.1 0.12 (1)	B-K	0 / 17	0.00 (1)	
D-E	-35 / 0	-102.1	-102.1 0.12 (1)	I-F	0 / 17	0.00 (1)	
E-F	-3 / 0	-102.1	-102.1 0.12 (1)				
F-G	0 / 42	-102.1	-102.1 0.14 (1)				
H-F	-283 / 0	0.0	0.0 0.03 (1)				
L-K	0 / 0	-38.5	-38.5 0.07 (3)				
K-J	0 / 8	-38.5	-38.5 0.07 (3)				
J-I	0 / 8	-38.5	-38.5 0.07 (3)				
I-H	0 / 0	-38.5	-38.5 0.07 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.14/1.00 (F-G:1), BC=0.07/1.00 (I-L:3), WB=0.08/1.00 (E-L:1), SS=0.11/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

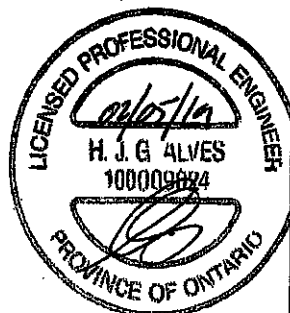
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (C) (INPUT = 0.80)
JSI METAL= 0.17 (C) (INPUT = 1.00)

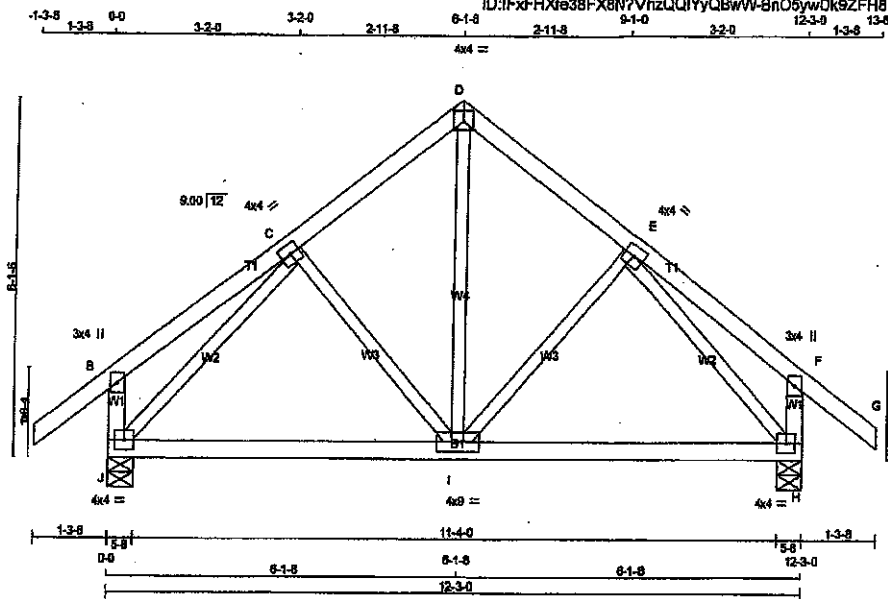
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JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 77902655
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10-Cond1	6	1	Preston 1	
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:11 2018 Page 1			
		ID:IFxHFXe38FX8N7VhzQQIYyQBwW-BnO5yWk9ZFH8NJSkplJJE5DeHergNxivS2V4zo56A			



TOTAL WEIGHT = 6 X 55 = 330 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	4.0		
D	TTW+p	MT20	4.0	4.0	2.25	2.00
E	TMVW+1	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW+1	MT20	4.0	4.0		
I	BMVW+1	MT20	4.0	4.0		
J	BMVW+1	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
J	1002	0	1002	0
H	1002	0	1002	0

UNFACTORED REACTIONS

1ST CASE	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
JT	740	438 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0
H	740	438 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. HORIZ. LOAD (LBS)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	I-D	0 / 481	0.11 (1)	
B-C	0 / 21	-102.1	-102.1	0.15 (1)	10.00	I-E	-127 / 29	0.04 (1)	
C-D	-627 / 0	-102.1	-102.1	0.11 (1)	6.25	C-I	-127 / 29	0.04 (1)	
D-E	-627 / 0	-102.1	-102.1	0.11 (1)	6.25	J-C	-364 / 0	0.28 (1)	
E-F	0 / 21	-102.1	-102.1	0.15 (1)	10.00	E-H	-364 / 0	0.28 (1)	
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
J-B	-262 / 0	0.0	0.0	0.03 (1)	7.81				
H-F	-262 / 0	0.0	0.0	0.03 (1)	7.81				
J-I	0 / 569	-38.5	-38.5	0.36 (2)	10.00				
I-H	0 / 569	-38.5	-38.5	0.36 (2)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.41")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (B-C-1), BC=0.38/1.00 (I-J-2),
WB=0.29/1.00 (C-I-1), SS=0.16/1.00 (J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

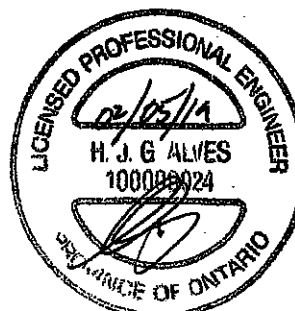
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (E) (INPUT = 0.90)
JSI METAL = 0.31 (C) (INPUT = 1.00)



DRWG NO. TAM 71402656
STRUCTURAL
COMPONENT ONLY

amarack Roof House, Burlington

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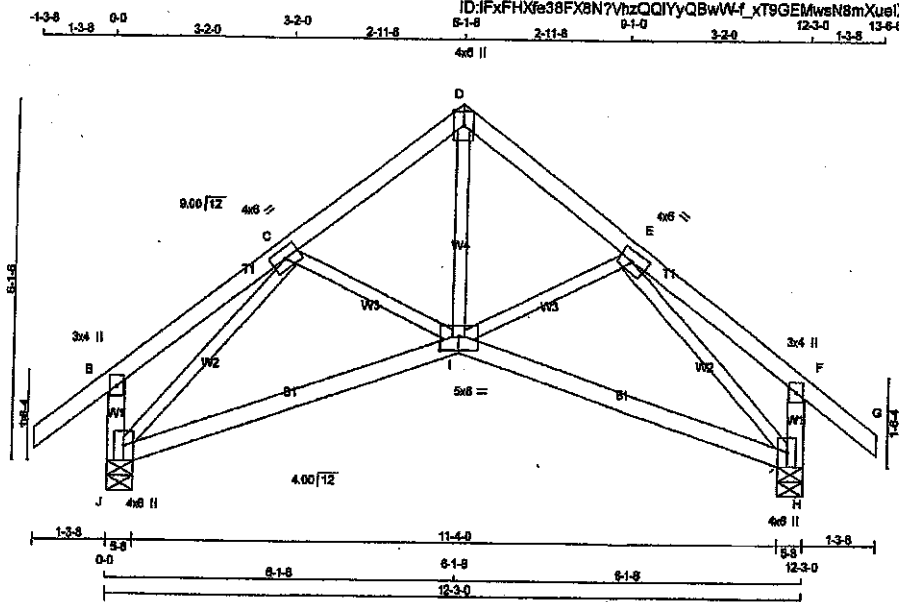
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DWG NO. TAM 17902657
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	T10S	2	1	Preston 1	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue Feb 5 20:42:12 2019 Page 1
ID:IFxHXfe38FX8N?VhzQQIYyQBwWkf_XT9GEMwsN8mXuelXGYGSdOrHYXZoar86o2EWz0569



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 2 X 54 = 108 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	IN-SX	IN-SX	
J	1002 0	1002 0	0	5-8	5-8	5-8	
H	1002 0	1002 0	0	5-8	5-8	5-8	

UNFACTORED REACTIONS							
JT	1ST LOSE	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
J	740	438 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0
H	740	438 / 0	129 / 0	0 / 0	0 / 0	178 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO	LENGTH	FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	I-D	0 / 767	0.17 (1)
B-C	0 / 19	-102.1 -102.1	0.13 (1)	10.00	I-E	-52 / 55	0.01 (3)
C-D	-868 / 0	-102.1 -102.1	0.11 (1)	6.25	C-I	-52 / 55	0.01 (3)
D-E	-868 / 0	-102.1 -102.1	0.11 (1)	6.25	J-C	-1124 / 0	0.40 (1)
E-F	0 / 19	-102.1 -102.1	0.13 (1)	10.00	E-H	-1124 / 0	0.40 (1)
F-G	0 / 42	-102.1 -102.1	0.14 (1)	10.00			
J-B	-268 / 0	0.0 0.0	0.03 (1)	7.81			
H-F	-268 / 0	0.0 0.0	0.03 (1)	7.81			
J-I	0 / 764	-38.5 -38.5	0.41 (2)	10.00			
I-H	0 / 764	-38.5 -38.5	0.41 (2)	10.00			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+	MT20	4.0	6.0	
D	TTV+p	MT20	4.0	6.0	Edge
E	TMVW+	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BVMW1+p	MT20	4.0	6.0	
I	EBVWVW+p	MT20	5.0	8.0	2.75 4.00
J	BVMW1+p	MT20	4.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF CBC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPC 2011, TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/848 (0.17")

CSI: TC=0.14/1.00 (A-B-1), BC=0.41/1.00 (I-J-2), WB=0.40/1.00 (C-J-1), SSI=0.12/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP(DRY)	SHEAR	SECTION	(PSI)	(PLI)
MT20	618	354	1687	788	1987

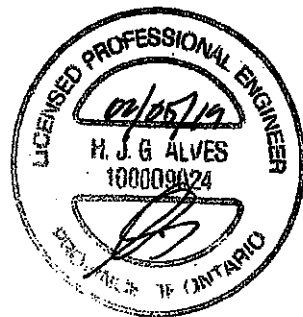
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (J) (INPUT = 0.90)
JSI METAL = 0.27 (C) (INPUT = 1.00)

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TOWN OF BUILDING SECTION
FILE NO.

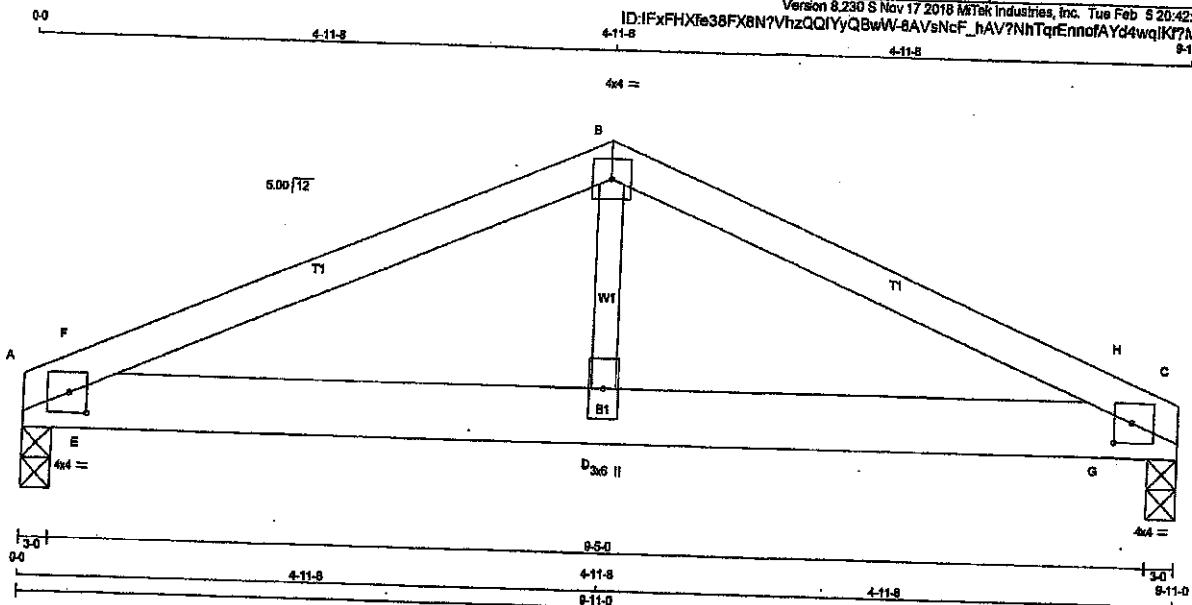


DRWG NO. TAM 17902658
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364 Tamarack Roof Truss, Burlington	TRUSS NAME T11	QUANTITY 2	PLY 1	JOB DESC. Preston 1	TRUSS DESC.	DRWG NO.
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Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:13 2019 Page 1
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4-11-8 4-11-8 9-11-0

Scale = 1:17.7



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF	
B - C	2x4	DRY	No.2	SPF	
A - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMB1-4	MT20	4.0	4.0	2.00	1.75
B	TTW-p	MT20	4.0	4.0		
C	TMB1-4	MT20	4.0	4.0	2.00	1.75
D	BMW-w	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX
A	697	0	697	0	0	3-0	3-0	3-0
C	697	0	697	0	0	3-0	3-0	3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	521	288 / 0	104 / 0	0 / 0	0 / 0	129 / 0	0 / 0
C	521	288 / 0	104 / 0	0 / 0	0 / 0	129 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
A-F	-897 / 0	-102.1	-102.1	0.17 (1)	6.25	D-B	0 / 385	0.09 (2)					
F-B	-898 / 0	-102.1	-102.1	0.24 (1)	5.99	E-F	-452 / 0	0.00 (1)					
B-H	-899 / 0	-102.1	-102.1	0.24 (1)	5.99	G-H	-452 / 0	0.00 (1)					
H-C	-897 / 0	-102.1	-102.1	0.17 (1)	6.25								
A-E	0 / 901	-38.5	-38.5	0.28 (1)	10.00								
E-D	0 / 901	-38.5	-38.5	0.28 (1)	10.00								
D-G	0 / 901	-38.5	-38.5	0.28 (1)	10.00								
G-C	0 / 901	-38.5	-38.5	0.28 (1)	10.00								

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.33")
CALCULATED VERT. DEFL. (TL) = $L/899$ (0.03")

CS1 TC=0.24/1.00 (B-F:1), BC=0.28/1.00 (A-E:1),
WB=0.09/1.00 (B-D:2), SS=0.27/1.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENG=1.10

COMPANION LIVE LOAD FACTOR = 1.00

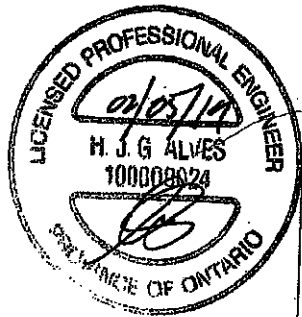
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 3.0 Deg.

JSI GRIP= 0.45 (A) (INPUT = 0.90)
JSI METAL= 0.40 (C) (INPUT = 1.00)

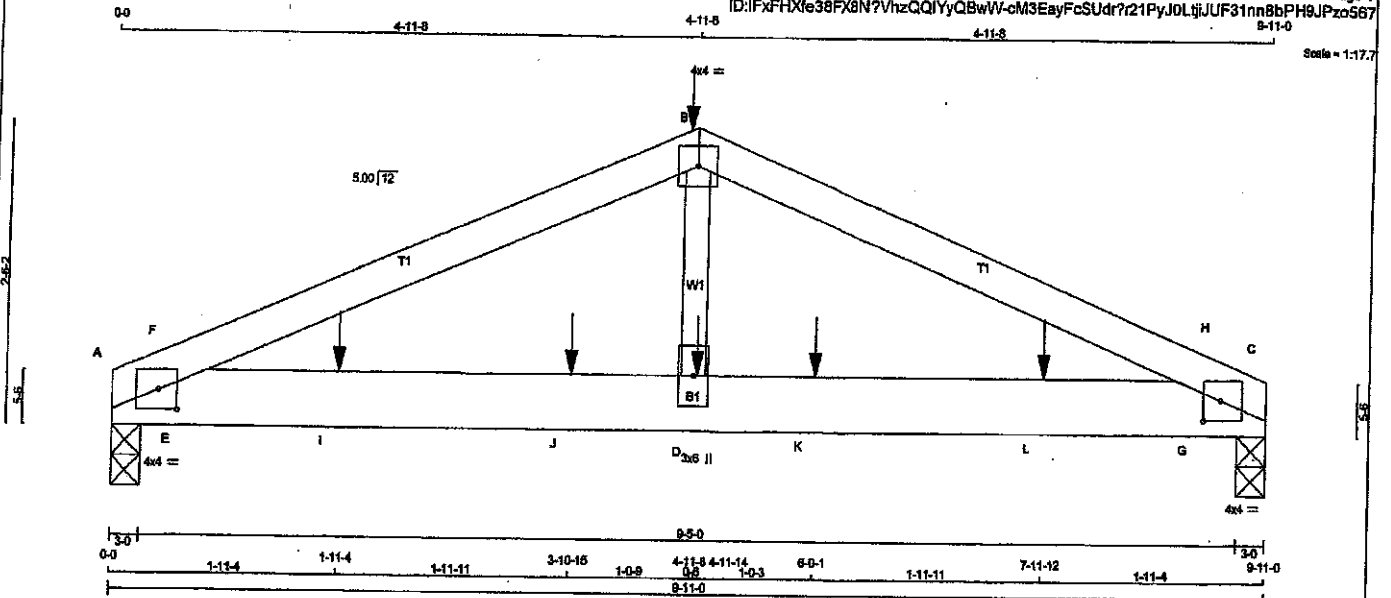


DRWG NO. TAM 17907659
STRUCTURAL
COMPONENT ONLY

JOB NAME 200168-400364	TRUSS NAME T11Z	QUANTITY 1	PLY 1	JOB DESC. Preston 1	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:14 2019 Page 1
ID:IFxHXfe38FX8N7VhzQQIYyQBwW-cM3EayFcSUdr7r21PyJDLtjUUF31nn8bPH9JPzo567

Scale = 1:17.7



TOTAL WEIGHT = 32 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE LUMBER DESCR. SPF
A - B 2x4 DRY No.2
B - C 2x4 DRY No.2
A - C 2x6 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
A TMB1-I MT20 4.0 4.0 2.00 1.75
B TTVV-p MT20 4.0 4.0
C TMB1-I MT20 4.0 4.0 2.00 1.75
D BMVW-w MT20 3.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	BRG IN-SX	REQD BRG IN-SX
A	857	0	857	0	0	3-0	3-0
C	857	0	857	0	0	3-0	3-0

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	631	379 / 0	108 / 0	0 / 0	0 / 0	144 / 0	0 / 0
C	631	379 / 0	108 / 0	0 / 0	0 / 0	144 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	FORCE (LBS)	FACTORED MAX. CSI (LC)
FR-TO							
A-F	-1294 / 0	-102.1	-102.1 0.24 (1)	5.34	D-B	0 / 383	0.09 (2)
F-B	-1370 / 0	-102.1	-102.1 0.31 (1)	5.16	E-F	-415 / 0	0.00 (1)
B-H	-1370 / 0	-102.1	-102.1 0.31 (1)	5.16	G-H	-415 / 0	0.00 (1)
H-C	-1294 / 0	-102.1	-102.1 0.24 (1)	5.34			
A-E	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
E-I	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
I-J	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
J-D	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
D-K	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
K-L	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
L-G	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			
G-C	0 / 1253	-38.5	-38.5 0.35 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	4-11-8	-25	-28	---	FRONT	VERT	DEAD	---	---
B	4-11-8	-7	-7	---	FRONT	VERT	TOTAL	---	---
B	4-11-8	-273	-273	---	FRONT	VERT	SNOW	---	---
D	4-11-14	-3	-4	---	FRONT	VERT	TOTAL	---	---
I	1-11-4	-3	-4	---	FRONT	VERT	TOTAL	---	---
J	3-10-15	-3	-4	---	FRONT	VERT	TOTAL	---	---
K	6-0-1	-3	-4	---	FRONT	VERT	TOTAL	---	---
L	7-11-12	-3	-4	---	FRONT	VERT	TOTAL	---	---

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")
ALLOWABLE DEFL.(TL) = L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.31/1.00 (B-H:1), BC=0.35/1.00 (D-G:1), WS=0.09/1.00 (B-D:2), SS=0.29/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

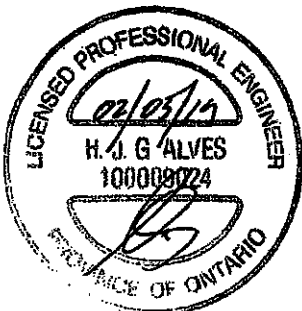
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1567 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

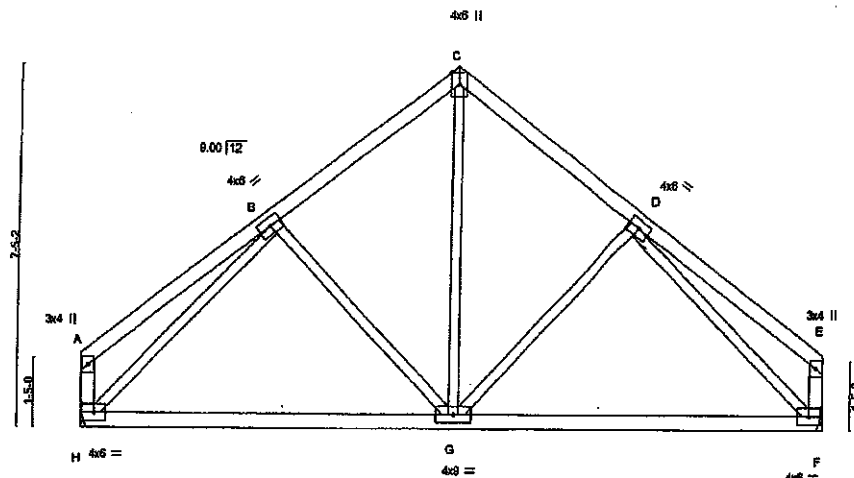
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.59 (A) (INPUT = 0.80)
JSI METAL= 0.54 (C) (INPUT = 1.00)

DWG NO. TAM 1790 2660
STRUCTURAL
COMPONENT ONLY



JOB NAME 401596	TRUSS NAME T111	QUANTITY 2	PLY 1	JOB DESC. Greenpark	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitel Industries, Inc. Wed Feb 27 17:51:54 2019 Page 1 ID:1FxFHXfe38FX8N7VhzQQIYyQBwW-4vHTs44CFnamSkN0XZ1zpfOfmRX6pL_oMxKNWz9Xp	



TOTAL WEIGHT = 2 X 88 = 133 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
H - A	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	

EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVW+H	MT20	4.0	6.0		
C	TMV+p	MT20	4.0	6.0	Edge	
D	TMVW+H	MT20	4.0	6.0		
E	TMV+p	MT20	3.0	4.0		
F	BMVW+H	MT20	4.0	6.0		
G	BMVW+H	MT20	4.0	6.0		
H	BMVW+H	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
H	1107 0	1107 0	MECHANICAL	MECHANICAL
F	1107 0	1107 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, F. MINIMUM BEARING LENGTH AT JOINT H = 1-8, JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	827	457 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0	0 / 0
F	827	457 / 0	165 / 0	0 / 0	0 / 0	205 / 0	0 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)
FR-TO				FR-TO			
A-B	0 / 28	-102.1	0.25 (1)	G-C	0 / 670	0.15 (1)	10.00
B-C	-848 / 0	-102.1	0.20 (1)	G-D	-203 / 21	0.11 (1)	6.25
C-D	-848 / 0	-102.1	0.20 (1)	B-G	-203 / 21	0.11 (1)	6.25
D-E	0 / 28	-102.1	0.25 (1)	H-B	-1158 / 0	0.62 (1)	10.00
E-A	-154 / 0	0.0	0.02 (1)	D-F	-1158 / 0	0.62 (1)	7.81
F-E	-154 / 0	0.0	0.02 (1)				7.81
H-G	0 / 795	-38.5	0.61 (2)				10.00
G-F	0 / 795	-38.5	0.61 (2)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.53")
CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.53")
CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.25/1.00 (A-B:1), BC=0.61/1.00 (G-H:2), WB=0.62/1.00 (B-H:1), SS=0.20/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.87 (D) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RESERVED
JUN 13 2019

TOWN OF CALEDON BUILDING SECTION
FILE NO

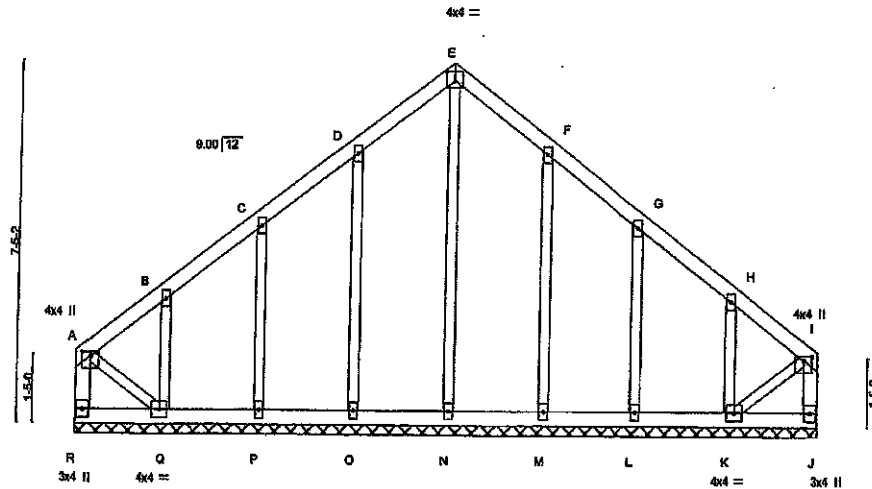


DWG NO. TAM 77904520
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Greenpark	DRWG NO.
401596	G111	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Feb 27 17:51:52 2019 Page 1
ID:IFxHXfe38FX8N7VhzQQIYyQBwVW-8W9JSA2qgaXs9a?m6XZDYk5mzuBe0ThL22DJeZg0X

Scale = 1:43.6



TOTAL WEIGHT = 71 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
R - A	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - I	2x4	DRY	No.2
R - J	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 0.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD			=	52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TO=0.08/1.00 (D-E-1), BO=0.03/1.00 (K-L-2), WB=0.13/1.00 (E-N-1), SSI=0.08/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 793 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

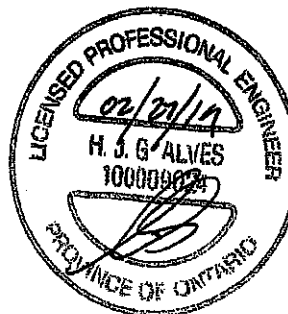
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (D) (INPUT = 0.90)
JSI METAL= 0.12 (F) (INPUT = 1.00)

PLATES (table in inches)

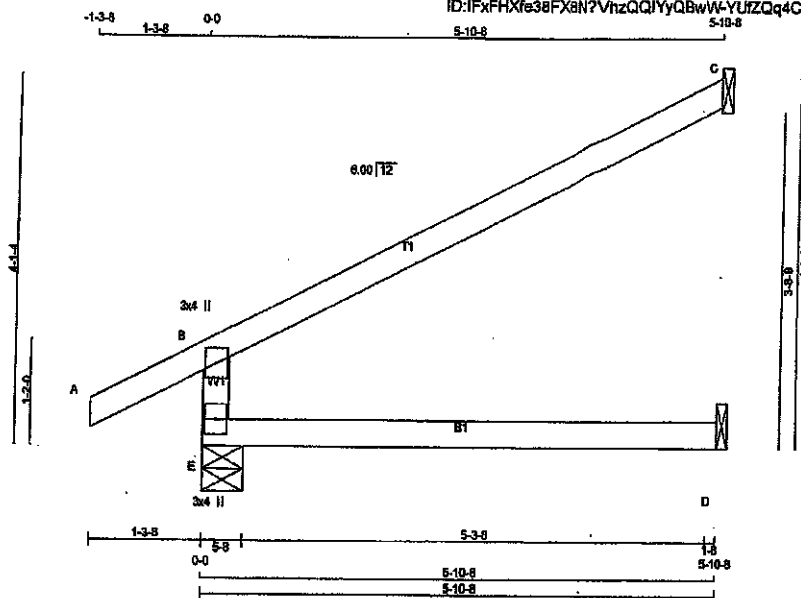
JT	TYPE	PLATES	W	LEN	Y	X
A	TMWV+p	MT20	4.0	4.0	1.00	2.00
B, C, D, F, G, H						
B	TMWV+w	MT20	2.0	4.0		
E	TTW+p	MT20	4.0	4.0	2.25	2.00
I	TMWV+p	MT20	4.0	4.0	1.00	2.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWV1-i	MT20	4.0	4.0		
L, M, N, O, P						
L	BMWV1+w	MT20	2.0	4.0		
Q	BMWV1-i	MT20	4.0	4.0		
R	BMV1+p	MT20	3.0	4.0		

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO	MAX. LC1 CS1 (LC)
FR-TO		FROM TO		FR-TO			
R-A	-125 / 0	0.0	0.0 0.01 (1)	N-E	-136 / 0	7.81	0.13 (1)
A-B	-37 / 0	-102.1	-102.1 0.05 (1)	O-D	-232 / 0	8.25	0.13 (1)
B-C	-41 / 0	-102.1	-102.1 0.05 (1)	P-C	-193 / 0	8.25	0.08 (1)
C-D	-31 / 0	-102.1	-102.1 0.08 (1)	Q-B	-211 / 0	8.25	0.04 (1)
D-E	-42 / 0	-102.1	-102.1 0.08 (1)	M-F	-232 / 0	8.25	0.13 (1)
E-F	-42 / 0	-102.1	-102.1 0.08 (1)	L-G	-193 / 0	8.25	0.08 (1)
F-G	-31 / 0	-102.1	-102.1 0.08 (1)	K-H	-211 / 0	8.25	0.04 (1)
G-H	-41 / 0	-102.1	-102.1 0.05 (1)	A-Q	0 / 46	8.25	0.01 (1)
H-I	-37 / 0	-102.1	-102.1 0.05 (1)	K-I	0 / 46	8.25	0.01 (1)
J-I	-125 / 0	0.0	0.0 0.01 (1)				
R-Q	0 / 0	-38.5	-38.5 0.03 (3)				
Q-P	0 / 32	-38.5	-38.5 0.03 (2)				
P-O	0 / 28	-38.5	-38.5 0.03 (2)				
O-N	0 / 24	-38.5	-38.5 0.03 (2)				
N-M	0 / 24	-38.5	-38.5 0.03 (2)				
M-L	0 / 28	-38.5	-38.5 0.03 (2)				
L-K	0 / 32	-38.5	-38.5 0.03 (2)				
K-J	0 / 0	-38.5	-38.5 0.03 (3)				



DRWG NO. TAM 19404521
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond1	22	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 22 X 17 = 369 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	646	0	646	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	85	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	292/0	72/0	0/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	0/0	28/0	0/0
D	85	0/0	51/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

FR-TO	CHORDS		FACTORED		WEBS	
	MAX. FACTORED MEMB. FORCE (LBS)	VERT. LOAD (PLF)	MAX. VERT. LOAD (LBS)	MAX. HORIZ. LOAD (LBS)	MAX. FACTORED MEMB. FORCE (LBS)	MAX. HORIZ. LOAD (LBS)
E-B	-513/0	0.0	0.0	0.22 (3)	7.81	
A-B	0/31	-102.1	-102.1	0.13 (1)	10.00	
B-C	-34/0	-102.1	-102.1	0.60 (1)	6.25	
E-D	0/0	-38.5	-38.5	0.22 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 62.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/380$ (0.20")
CALCULATED VERT. DEFL.(LL) = $L/889$ (0.04")
ALLOWABLE DEFL.(TL) = $L/380$ (0.20")
CALCULATED VERT. DEFL.(TL) = $L/942$ (0.07")

CSI: TC=0.80/1.00 (B-C:1), GC=0.22/1.00 (D-E:3),
WB=0.00/1.00 (A-B:0), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

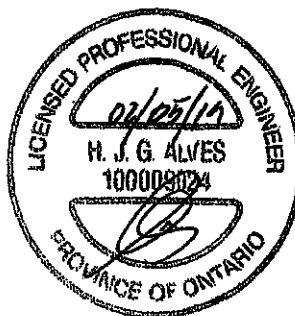
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1857
	788		1857

PLATE PLACEMENT TOL. = 0.250 inches

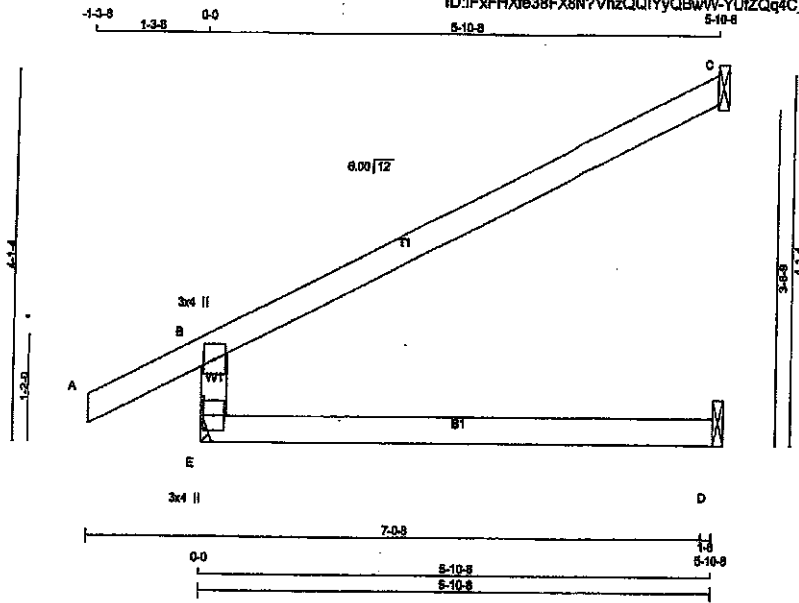
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



DRWG NO. TAM 71902661
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J1-Cond2	22	1	Preston 1	
Tamarack Roof Truss, Burlington		TRUSS DESC.			



TOTAL WEIGHT = 22 X 17 = 369 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
E - B	2x4 DRY	No.2	SPF
A - C	2x4 DRY	No.2	SPF
E - D	2x4 DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMV+p	MT20	3.0	4.0
E	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	546	0	546	0	MECHANICAL	1-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. MINIMUM BEARING LENGTH AT JOINT E = 1-8.

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	473	262/0	72/0	0/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	0/0	25/0	0/0
D	85	0/0	51/0	0/0	0/0	34/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-513/0	0.0	0.0 0.18 (3)	7.81			
A-B	0/31	-102.1	-102.1 0.13 (1)	10.00			
B-C	-34/0	-102.1	-102.1 0.46 (1)	6.25			
E-D	0/0	-38.5	-38.5 0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBG 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/888$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/942$ (0.07")

CSI: TO=0.46/1.00 (B-C:1), BO=0.22/1.00 (D-E:3), WB=0.00/1.00 (V/A:0), SSI=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY)	SHEAR	SECTION	(PSI)	(PL)
				MAX	MIN
MT20	618	354	1657	788	1987

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.21 (E) (INPUT = 0.90)
JSI METAL=0.14 (B) (INPUT = 1.00)

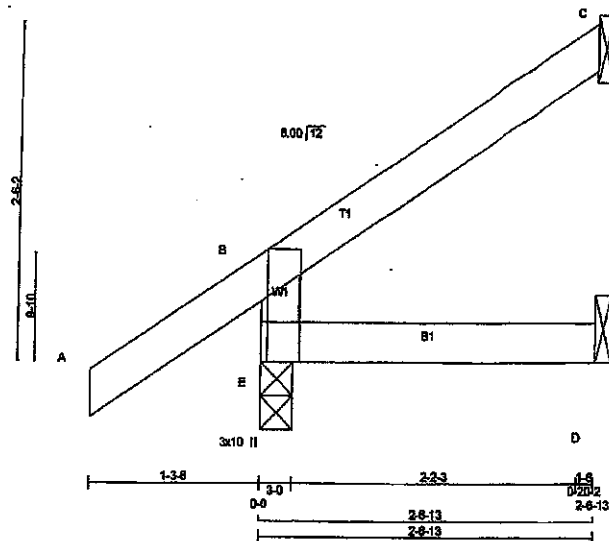
RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM 71902662
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J3	5	1	Preston 1	
Temerack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:41:58 2019 Page 1
ID:IFxHXfe38FX8N?VhzQQIYyQBwW-YUHZQq4C_k_XP81IXVEibAKRCI00Yx8a8sHnzo58M
Scale = 1:15.6



TOTAL WEIGHT = 5 X 9 = 44 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
E					
B					
E	TMBMV1+p	MT20	3.0	10.0	Edge 0.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
E	381	0	381	0	3-0	3-0
C	95	0	95	0	1-8	1-8
D	42	0	53	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED	MAX./MIN. COMPONENT REACTIONS				
		SNOW	LIVE	PERM.LIVE	WIND	DEAD
E	281	173 / 0	31 / 0	0 / 0	0 / 0	57 / 0
C	68	58 / 0	0 / 0	0 / 0	0 / 0	12 / 0
D	38	0 / 0	23 / 0	0 / 0	0 / 0	15 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD		MAX. UNBRACED LENGTH	FR-TO
		FROM	TO		
E-B	-304 / 0	0.0	0.0	0.04 (3)	7.81
A-B	0 / 39	-102.1	-102.1	0.14 (1)	10.00
B-C	-18 / 0	-102.1	-102.1	0.11 (1)	6.25
E-D	0 / 0	-38.5	-38.5	0.04 (3)	10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.04/1.00 (D-E:3),
WB=0.00/1.00 (n/a:0), SSI=0.11/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

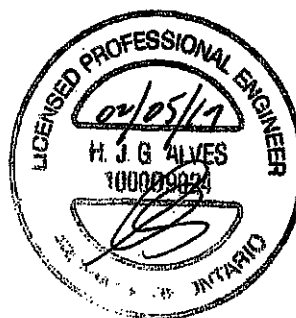
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(P.S.I.) (P.L.I.) (P.L.I.)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.08 (E) (INPUT = 0.90)
JSI METAL= 0.10 (E) (INPUT = 1.00)

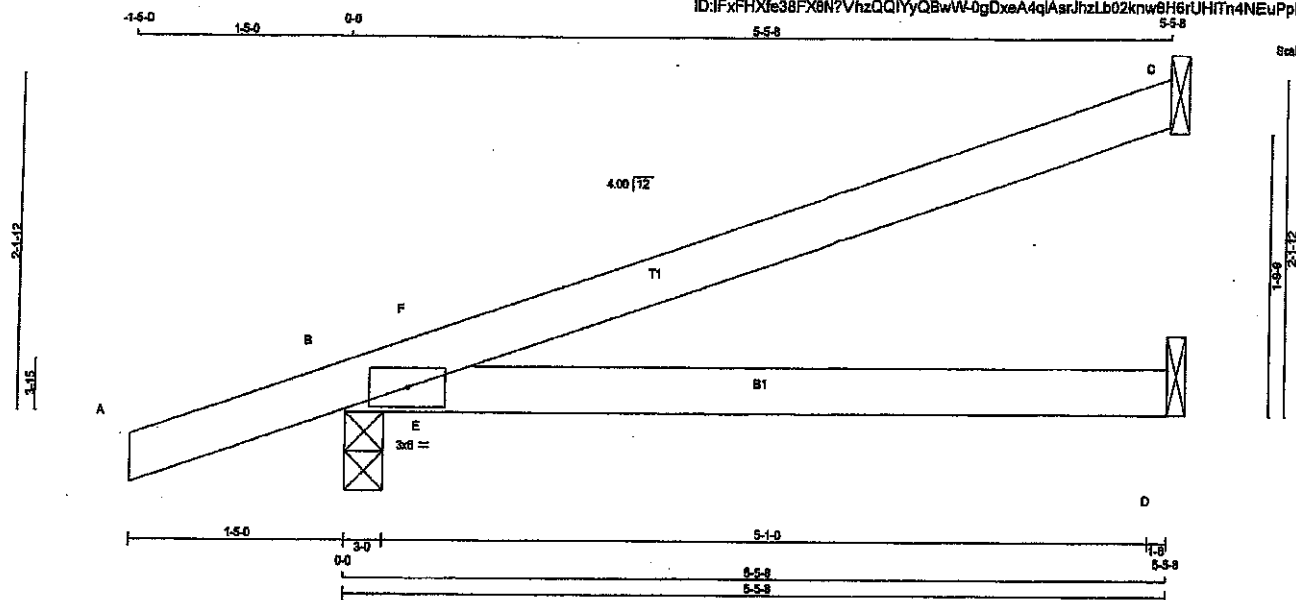


DRWG NO. TAM 1902663
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200168-400364	J4	2	1	Preston 1	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 20:42:00 2019 Page 1
ID:IFxHFXle38FX8N7VhzQQIYyQBwW-0gDxeA4qlAsrJhzLb02knw8H6rUHfTn4NEuPpDzo56L

Scale = 1:13.6



TOTAL WEIGHT = 2 X 15 = 29 lb

LUMBER
N.L.G.A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY
LUMBER No.2
DESCR. SPF
SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB14 MT20 3.0 6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER
BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT IN-SX	IN-SX
C	250	0	250	0	0	1-8
B	533	0	533	0	0	3-0
D	133	0	145	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	369	243 / 0	57 / 0	0 / 0	0 / 0	89 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	LC1 MAX	CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS MAX. FACTORED FORCE (LBS)	CS1 (LC)
A-B	0 / 22	-102.1	-102.1	0.15 (1)	10.00	E-F	-254 / 122	0.00 (1)
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25			
F-C	0 / 8	-102.1	-102.1	0.42 (1)	10.00			
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00			
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
CL = 8.0 PSF
BOT CH. LL = 10.5 PSF
CL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2015
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/380$ (0.18")
CALCULATED VERT. DEFL. (LL) = $L/842$ (0.08")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/489$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DRWG NO. TAM 17902664
STRUCTURAL
COMPONENT ONLY



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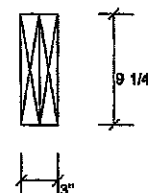
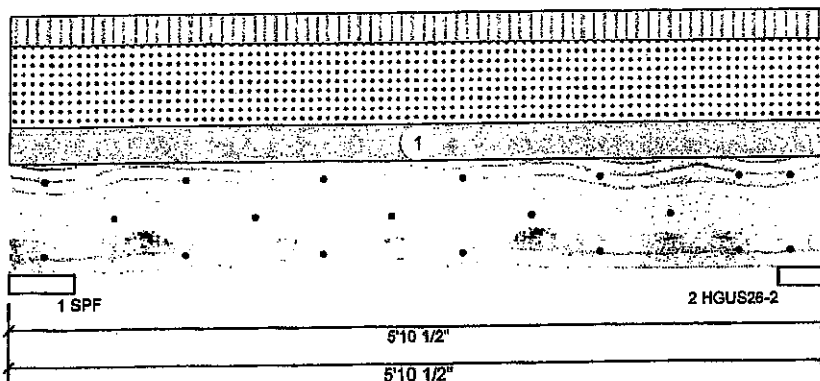
Client:
Project:
Address:

Date: 2/27/2019
Designer: JG
Job Name: 401585 / GREENPARK/LAMBERTS LANE PH2
Project #:

Page 1 of 2

B110 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Ply:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
General Load		Vibration:	Not Checked
Floor Live:	10.5 PSF		
Dead:	8 PSF		
Snow:	29 PSF		

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	284	361	783	0
2	272	336	750	0

Bearings and Factored Reactions

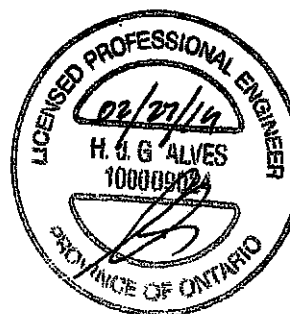
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	19%	439 / 1458	1897	L	1.25D+1.5S +L
2 - HGUS...	4.000"	25%	420 / 1397	1818	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2144 ft-lb	3'	6039 ft-lb	0.355 (38%)	1.25D+1.5S +L	L
Unbraced	2144 ft-lb	3'	5236 ft-lb	0.409 (41%)	1.25D+1.5S +L	L
Shear	1846 lb	4'10"	3884 lb	0.413 (41%)	1.25D+1.5S +L	L
LL Defl inch	0.019 (L/3338)	3'	0.174 (L/360)	0.110 (11%)	S+0.5L	L
TL Defl inch	0.026 (L/2419)	3'	0.174 (L/360)	0.150 (15%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17904522
STRUCTURAL
COMPONENT ONLY

42

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		9-0-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

JUN 13 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 395-1115



This design is valid until 12/11/2021





isDesign™

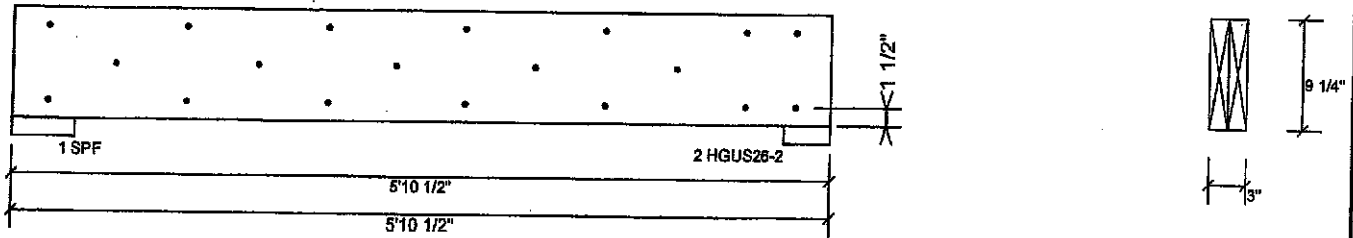
Client:
Project:
Address:

Date: 2/27/2019
Designer: JG
Job Name: 401596 / GREENPARK/LAMBERTS LANE PH.2
Project #:

Page 2 of 2

B110 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

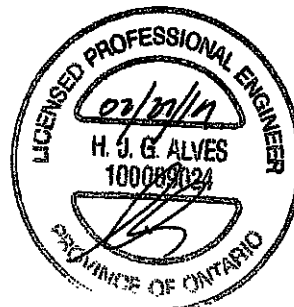
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	93.0 %
Load	316.1 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17904522
STRUCTURAL
COMPONENT ONLY

3/2

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021





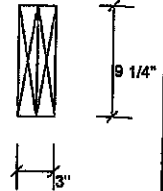
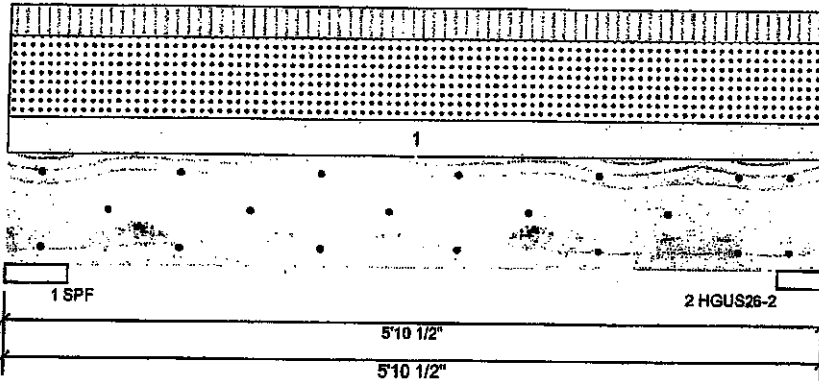
isDesign™

Client: Greenpark Homes
 Project: Preston 1
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200168

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Floor (Residential)
Piles:	2	Design Method:	LSD
Moisture Condition:	Dry	Building Code:	NBCC 2015 / OBC 2012
Deflection LL:	360	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal	Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	231	273	609	0
2	221	262	584	0

Bearings and Factored Reactions

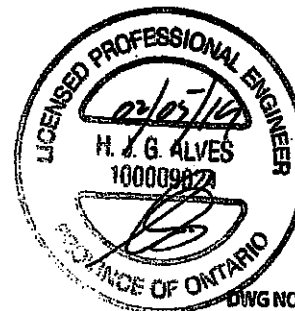
Bearing	Length	Cap.	React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	341 / 1145	1486 L	1.25D+1.5S +L
2 - HGUS...	4.000"	19%	327 / 1097	1424 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1679 ft-lb	3'	6039 ft-lb	0.278 (28%)	1.25D+1.5S	L
Unbraced	1679 ft-lb	3'	5236 ft-lb	0.321 (32%)	1.25D+1.5S	L
Shear	1290 lb	12"	3984 lb	0.324 (32%)	1.25D+1.5S	L
Perm Def in. (L/11308)	0.006	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch (L/4280)	0.015	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch (L/3094)	0.020	3'	0.174 (L/360)	0.120 (12%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 77902665
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7-0-0	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	

Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD, ON
 CANADA
 (905) 335-1115



This design is valid until 2021-12-11





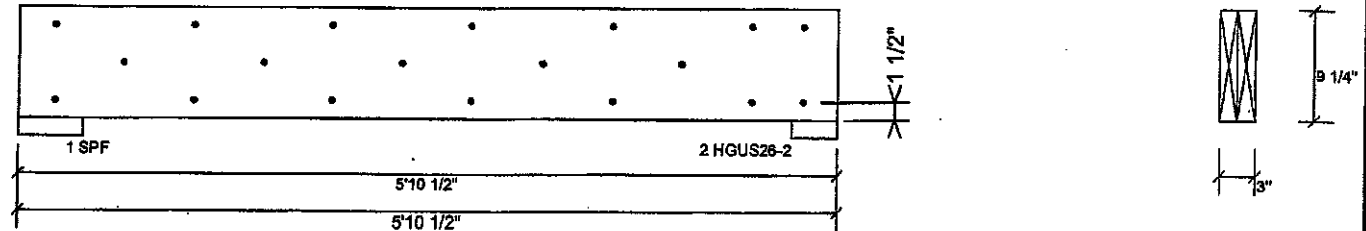
isDesign™

Client: Greenpark Homes
Project: Preston 1
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200168

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

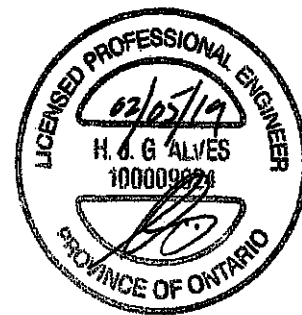
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	72.8 %
Load	247.6 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



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TOWN OF CALEDON
BUILDING SECTION
FILE NO. _____

DWG NO: TAM 77902665
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info

TAMARACK LUMBER
3255 NORTH SERVICE RD, ON
CANADA
(805) 335-1115



This design is valid until 2021-12-11



LUL/LUS/LJS/HUS/HHUS/HGUS



Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 18d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1 1/2" nails into the header and 10d commons into the joist, and reduce the resistance to 0.84 of the table value where 18d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options Information on p. 126.



Double-Shear Nailing Top View



Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View (available on some models)
U.S. Patent 5,603,580



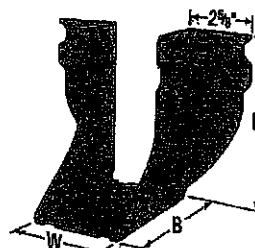
LUS28



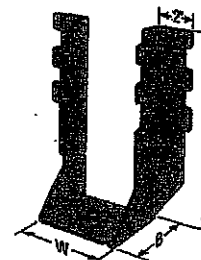
LU26L



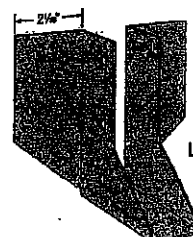
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2

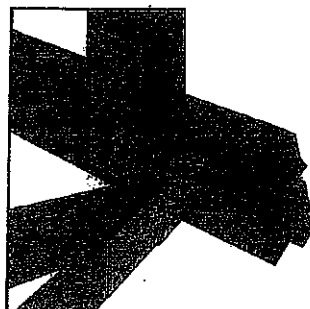


HHUS210-2



LJS26DS

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



Plated Truss Connectors

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options Information on pp. 125–127.

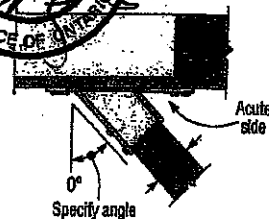
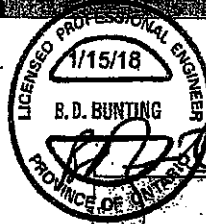
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.82 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 8"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga	Dimensions (in.)				Fastenings		Factored Resistance			
		W	H	B	d _h ²	Header	Joist	Down		Uplift	
								lb.	lb.	lb.	lb.
Single 2x Sizes											
LUS24	18	1 1/4	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
LU24L	22	1 1/4	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
LU24H	22	1 1/4	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
LUS26	18	1 1/4	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
LU26L	18	1 1/4	5 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	2705	4940	2065	3875
LUS26S	18	1 1/4	5	3 1/4	4 1/4	(10) 10d	(6) 10d	2055	4265	1480	4115
HUS26	12	1 1/4	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2885	8625	2885	5700
LU28L	20	1 1/4	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
LUS28	18	1 1/4	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	1420	2520	1290	1790
HUS28	16	1 1/4	7 1/4	3	6 1/4	(22) 18d	(8) 18d	3605	5365	2675	4345
HGUS28	12	1 1/4	7 1/4	5	6 1/4	(36) 18d	(12) 18d	3310	7675	3310	6900
LU210L	20	1 1/4	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2485	1020	1770
LU210	18	1 1/4	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d	1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_h is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.

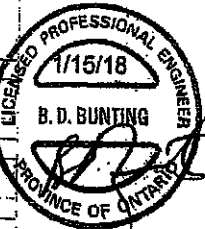
Face-Mount Hangers

SIMPSON
Strong-Tie

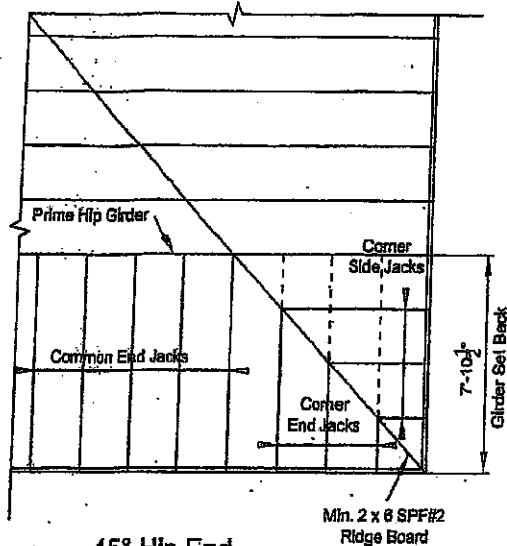
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Gn.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _p	Header	Joist			S-P-F	
								lb.	lb.	lb.	lb.
										KN	KN
Double 2x Sizes											
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d	835	2020	590	1435
LUS26-2	18	3 1/4	4 1/4	2	1 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HLUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2850	7335	2065	5205
HGUS26-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS28-2	18	3 1/4	7 1/4	2	1 1/4	(4) 16d	(4) 16d	1720	3325	1545	2575
HLUS28-2	14	3 1/4	7 1/4	3	3 1/4	(22) 16d	(8) 16d	3765	8840	2675	6345
HGUS28-2	12	3 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	6070	12980	4310	9215
LUS210-2	18	3 1/4	9 1/4	2	1 1/4	(8) 16d	(8) 16d	2580	4500	2320	3195
HLUS210-2	14	3 1/4	9 1/4	3	3 1/4	(30) 16d	(10) 16d	4670	9660	4235	7900
HGUS210-2	12	3 1/4	9 1/4	4	4 1/4	(48) 16d	(16) 16d	6840	14015	4855	10270
Triple 2x Sizes											
HGUS28-3	12	4 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	4385	8950	3110	6355
HGUS28-3	12	4 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	6070	12980	4310	9215
HLUS210-3	14	4 1/4	9 1/4	3	3 1/4	(30) 16d	(10) 16d	4670	9660	4235	6885
HGUS210-3	12	4 1/4	9 1/4	4	4 1/4	(48) 16d	(16) 16d	6840	14015	4855	10400
Quadruple 2x Sizes											
HGUS28-4	12	6 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	4385	8950	3110	6355
HGUS28-4	12	6 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	6070	12980	4310	9215
HLUS210-4	14	6 1/4	9 1/4	3	3 1/4	(30) 16d	(10) 16d	4670	10155	4235	7210
HGUS210-4	12	6 1/4	9 1/4	4	4 1/4	(48) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7840	14995	5425	10645
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645
4x Sizes											
LUS48	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
HLUS48	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2540	7335	2065	5205
HGUS48	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
LUS48	18	3 1/4	6 1/4	2	3 1/4	(8) 16d	(4) 16d	1720	3325	1545	2575
HLUS48	14	3 1/4	7 1/4	3	3 1/4	(22) 16d	(8) 16d	3765	8840	2675	6345
HGUS48	12	3 1/4	7 1/4	4	4 1/4	(30) 16d	(12) 16d	6070	12980	4310	9215
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(8) 16d	2580	4500	2320	3195
HGUS410	12	3 1/4	9 1/4	4	8 1/4	(48) 16d	(16) 16d	6840	14015	4855	10270
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d	7840	14995	5425	10645
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d	10130	16400	7195	11645



Plated Tuss Connectors



45° Hip End

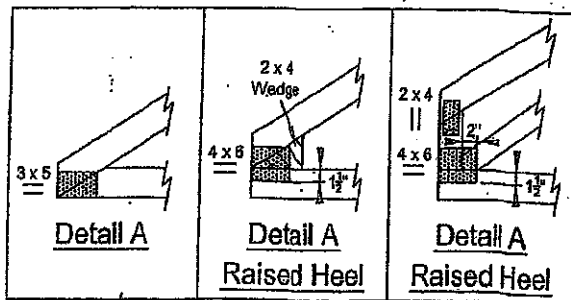
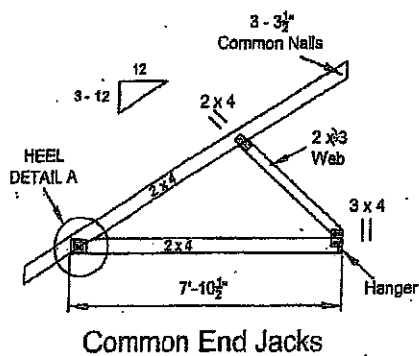
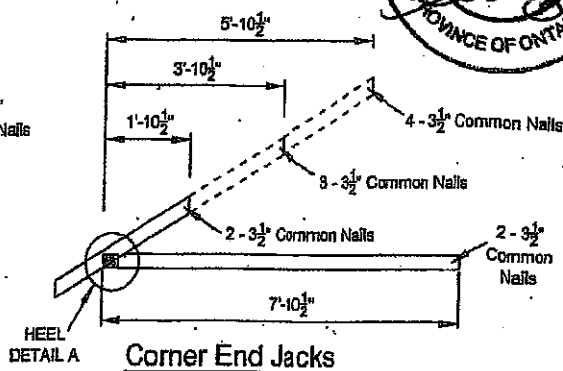
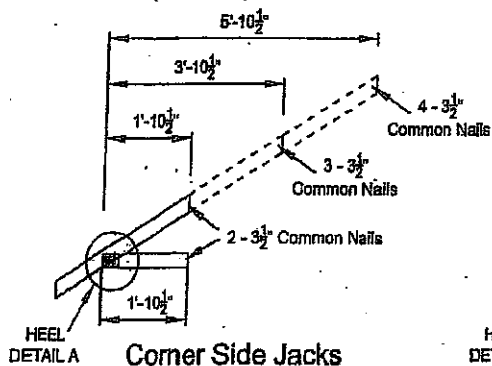
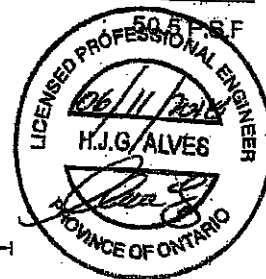
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

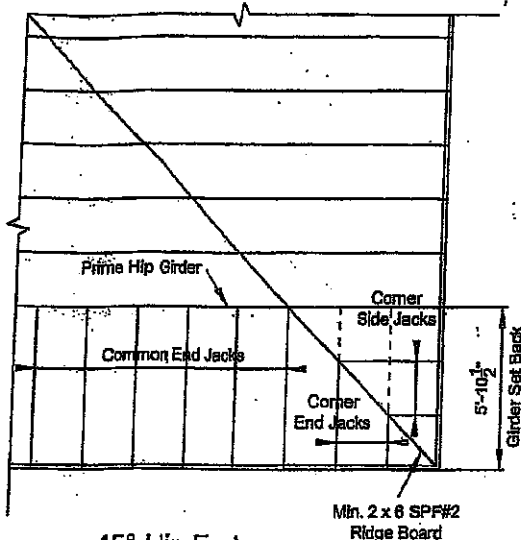
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



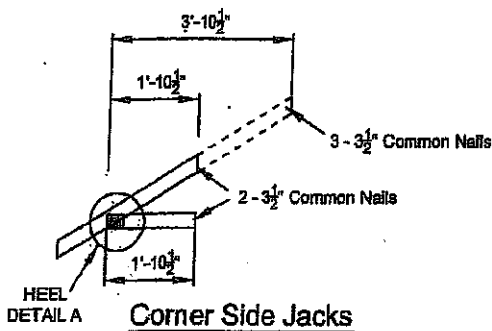
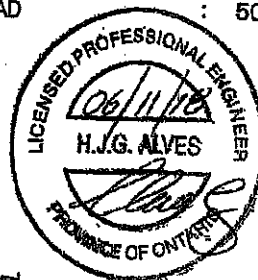
45° Hip End

LUMBER SPECIFICATION

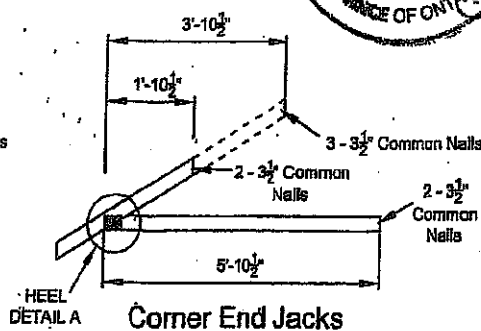
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

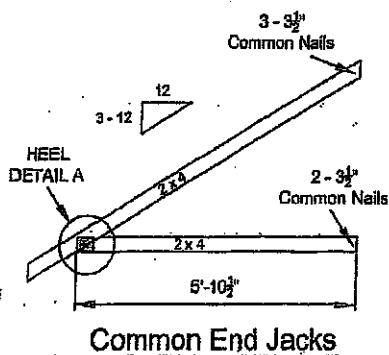
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F.



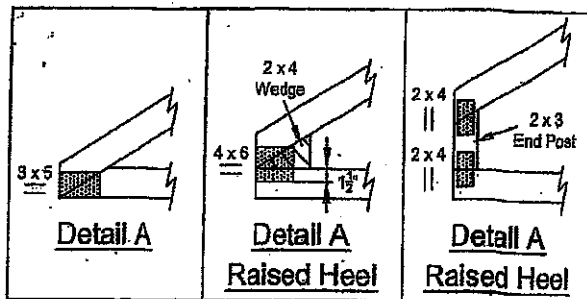
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

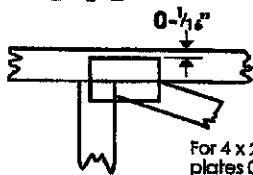
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T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



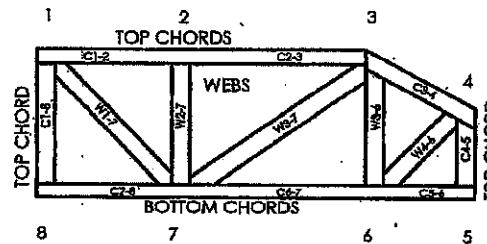
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MI-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or putins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

JUN 13 2019

TOWN OF GALEPON
BUILDING SECTION
FILE NO.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet M117473C.REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218
JUN 13 2019

Feb 09, 2018

TOWN OF CALEDON
BUILDING SECTION
FILE NO _____