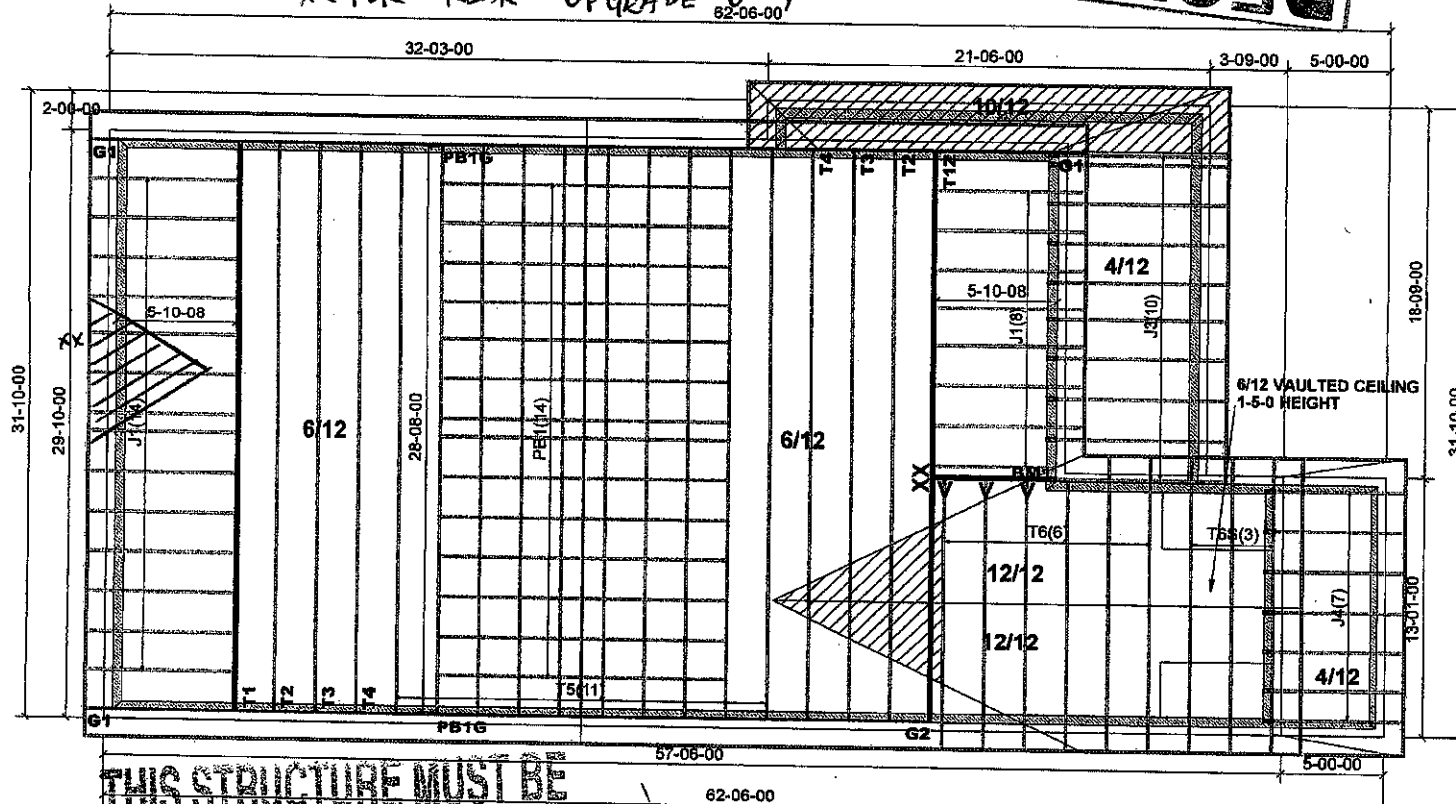


RECEIVED
 JUN 13 2019
 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO

XX FOR REAR UPGRADE ONLY
62-06-00



THIS STRUCTURE MUST BE
 CONSTRUCTED TO MEET OR
 EXCEED THE PROVISIONS OF
 THE ONTARIO BUILDING CODE

NO CHANGE TO TRS

JG 2/26/2019

**TRUSS PROFILES TO BE VERIFIED BY
BUILDING DESIGNER**

ALL CONVENTIONAL ROOF FRAMING TO
 CONFORM WITH PART 9 OF THE OBC.
 ROOF RAFTERS THAT MEET OR CROSS OVER
 TRUSSES ARE TO BE 2"x4"SPF@24"O.C.
 WITH 2"x4"SPF VERTICAL POST TO THE
 TRUSS UNDER AT EACH CROSS POINT.
 POSTS LONGER THAN 6' TO BE Laterally
 BRACED SO THAT THE DISTANCE
 BETWEEN END POINTS AND BETWEEN ROWS
 OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
 ONTARIO BUILDING CODE (2012)
 OCCUPANCY: RESIDENTIAL | PART: 9

DESIGN LOADS:

CITY: CALEDON
 G.S.L = 37.6 psf
 TC DL = 6 psf
 BC LL = 10.50 psf
 BC DL = 7.00 psf

NOTES:

FIN. OH.: 12"
 HEEL TYPE: R.T.M. CANT.
 EXT. WALLS: 2X6
 CLAD. TYPE 1: BRICK/5"
 CLAD. TYPE 2: SIDING/0"
 FSC SIZE: 2X6
 SHEATHING: ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFERE TO
 SEALED TRUSS DOCS FOR UPLIFT DESIGN

HARDWARE:

LJS26DS (V) 3pcs
 HGUS26-2 (XX) 1pcs

CONV FRM BY OTHERS

T- 180737

COMMENTS:

BM1: 2-2"x10" SPF #2

	Job Track: 50120	Builder / Location: Greenpark / Caledon	Model / Elevation: Preston 2 / Elv. 1 - REAR UPGRADE (4 BED ROOM)
	Plan Log: 200170	Project: Lamberts Lane Home Corp.	OR (5 BED ROOM)
	Layout ID: 400367	Date: 2019-02-01 Designer: Brian	THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



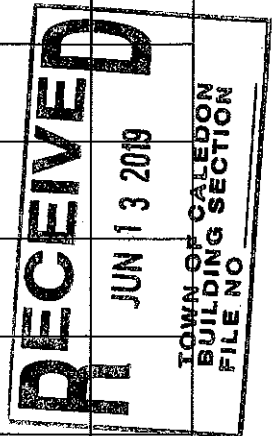
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 2
 Lot #:
 Elevation: 1

Job Track: 50120
 PlanLog: 200170
 Layout ID: 400367
 Ref #
 Page: 1 of 2
 Date: 02-01-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Flat Girder	0 / 12	28-08-00	4-01-04	2 x 6		4-01-04 4-01-04	296.21 182.67		
	1 2-ply	T12 Flat Girder	0 / 12	28-08-00	4-01-04	2 x 6		4-01-04 4-01-04	296.21 182.67		
	2	T2 Flat	0 / 12	28-08-00	5-01-04	2 x 6		5-01-04 5-01-04	297.43 184.33		
	2	T3 Flat	0 / 12	28-08-00	6-01-04	2 x 6		6-01-04 6-01-04	310.55 190.00		
	2	T4 Flat	0 / 12	28-08-00	7-01-04	2 x 6		7-01-04 7-01-04	324.15 199.67		
	11	T5 Flat	0 / 12	28-08-00	8-01-04	2 x 6		8-01-04 8-01-04	2334.75 1400.87		
	6	T6 Common	12 / 12	12-03-00	8-00-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	387.79 252.00		
	3	T6S Roof Special	12 / 12 6 / 12	12-03-00	8-00-00	2 x 4	1-03-08 1-03-08	1-10-08 1-10-08	190.86 122.00		
	14	PB1 Common	6 / 12	14-03-00	4-06-12	2 x 4		1-00-00 1-00-00	732.87 504.00		
	2	PB1G GABLE	6 / 12	14-03-00	4-06-12	2 x 4		1-00-00 1-00-00	94.6 62.00		
	3	G1 GABLE	6 / 12	23-00-00	8-01-04	2 x 4	1-03-08	1-02-00 8-01-04	348.37 224.50		
	1	G2 GABLE	6 / 12	17-01-08	8-01-04	2 x 4		4-01-04 8-01-04	100.58 64.83		
	22	J1 Jack-Open	6 / 12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	369.48 234.67		
	10	J3 Jack-Open	4 / 12	7-04-08	3-02-06	2 x 4	1-03-08	3-15 2-09-07	217.51 140.00		



DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: Greenpark
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: Preston 2
 Lot #:
 Elevation: 1

Job Track: 50120
 PlanLog: 200170
 Layout ID: 400367
 Ref #
 Page: 2 of 2
 Date: 02-01-2019
 Designer: Brian Faneca
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	7	J4 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= 89

TOTAL BFT OF ALL TRUSSES= 4009.34

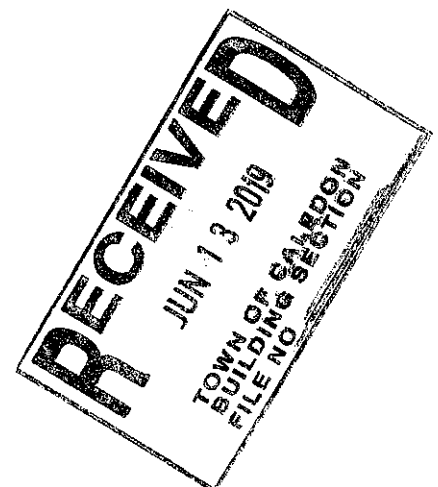
BFT.

TOTAL WEIGHT OF ALL TRSSES 6402.5 LBS

HARDWARE

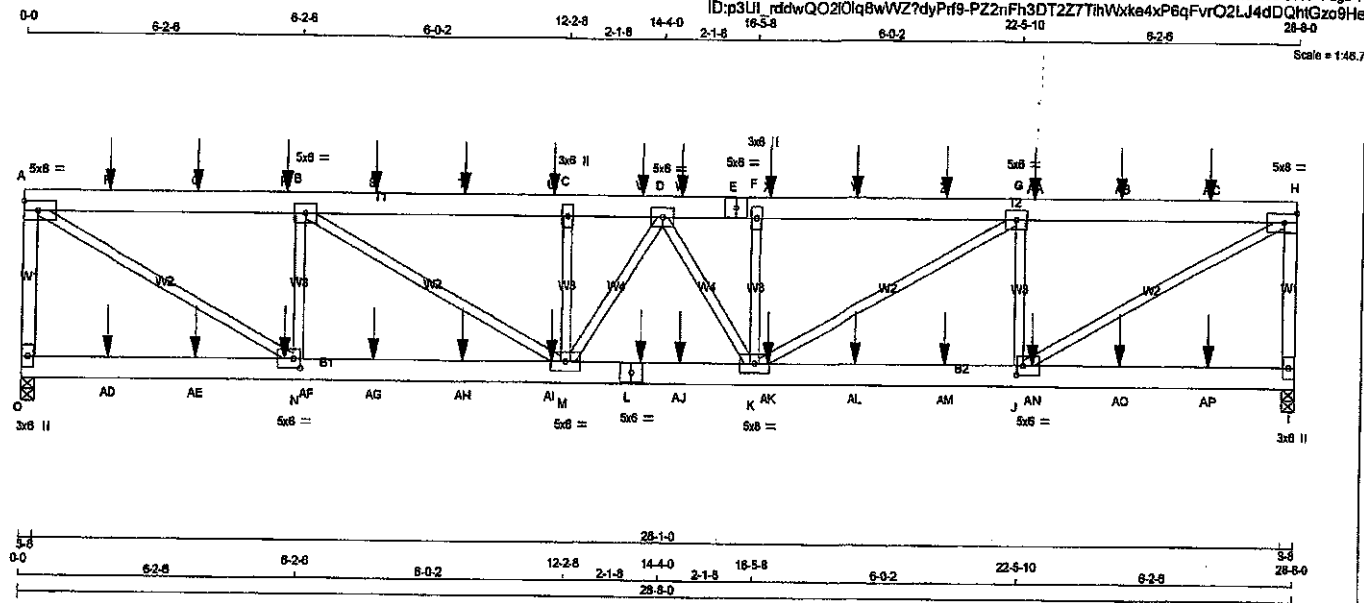
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 4



JOB NAME 200170-400367	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. Preston 2	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:58:53 2019 Page 1
ID: p3U1_rddwQO2lOq8wVWZ?dyPrf9-PZ2nFh3DT2Z7TthVxke4xP6qFvrO2LJ4dDQhtGzo9He
28-8-0
Scale = 1/48.7



LUMBER
N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY No.2	SPF
A - E	2x6	DRY No.2	SPF
E - H	2x6	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS: (0.122'x3") SPIRAL NAILS		
O-A	1 12	TOP
A-E	1 12	TOP
E-H	2 12	SIDE(183.1)
BOTTOM CHORDS: (0.122'x3") SPIRAL NAILS		
O-L	2 12	SIDE(183.1)
L-I	2 12	SIDE(183.1)
WEBS: (0.122'x3") SPIRAL NAILS		
2x3	1 6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	8.0	Edge
B, D, G					
B	TMVW-t	MT20	5.0	8.0	
C	TMVW-w	MT20	3.0	6.0	
E	TS-t	MT20	5.0	8.0	
F	TMVW-w	MT20	3.0	6.0	
H	TMVW-t	MT20	5.0	8.0	Edge
I	BMV1+p	MT20	3.0	8.0	
J	BMVW-t	MT20	5.0	6.0	2.50 1.75
K	BMVW-t	MT20	5.0	8.0	
L	BS-t	MT20	5.0	6.0	
M	BMVW-t	MT20	5.0	8.0	
N	BMVW-t	MT20	5.0	6.0	2.50 1.75
O	BMV1+p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	RECORD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
O	3260	0	3260	0	3-8	3-8
I	3260	0	3260	0	3-8	3-8

UNFACTORED REACTIONS							
JT	1ST LCASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	2448	1322 / 0	511 / 0	0 / 0	0 / 0	614 / 0	0 / 0
I	2448	1322 / 0	511 / 0	0 / 0	0 / 0	613 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
		FR-TO	FROM-TO					
O-A	-3108 / 0	0.0	0.0	0.36 (1)	6.57	K-F	-704 / 0	0.08 (1)
A-P	-4684 / 0	-102.1	-102.1	0.28 (1)	5.10	M-C	-704 / 0	0.08 (1)
P-Q	-4684 / 0	-102.1	-102.1	0.28 (1)	5.10	B-M	0 / 2200	0.27 (1)
Q-R	-4684 / 0	-102.1	-102.1	0.28 (1)	5.10	A-N	0 / 5411	0.67 (1)
R-S	-4684 / 0	-102.1	-102.1	0.28 (1)	5.10	N-B	-2293 / 0	0.27 (1)
S-T	-6545 / 0	-102.1	-102.1	0.30 (1)	4.41	J-H	0 / 5409	0.67 (1)
T-U	-6545 / 0	-102.1	-102.1	0.30 (1)	4.41	K-G	0 / 2198	0.27 (1)
U-C	-6545 / 0	-102.1	-102.1	0.30 (1)	4.41	J-G	-2292 / 0	0.27 (1)
C-V	-6545 / 0	-102.1	-102.1	0.30 (1)	4.41	M-D	-119 / 0	0.02 (1)
V-D	-6545 / 0	-102.1	-102.1	0.14 (1)	4.58	D-K	-125 / 0	0.02 (1)
D-W	-6542 / 0	-102.1	-102.1	0.14 (1)	4.58			
W-E	-6542 / 0	-102.1	-102.1	0.14 (1)	4.58			
E-F	-6542 / 0	-102.1	-102.1	0.14 (1)	4.58			
F-X	-6542 / 0	-102.1	-102.1	0.30 (1)	4.41			
X-Y	-6542 / 0	-102.1	-102.1	0.30 (1)	4.41			
Y-Z	-6542 / 0	-102.1	-102.1	0.30 (1)	4.41			
Z-G	-6542 / 0	-102.1	-102.1	0.30 (1)	4.41			
G-AA	-4682 / 0	-102.1	-102.1	0.28 (1)	5.10			
AA-AB	-4682 / 0	-102.1	-102.1	0.28 (1)	5.10			
AB-AC	-4682 / 0	-102.1	-102.1	0.28 (1)	5.10			
AC-H	-4682 / 0	-102.1	-102.1	0.28 (1)	5.10			
H-I	-3105 / 0	0.0	0.0	0.38 (1)	6.57			
O-AD	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
AD-AE	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
AE-AF	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
AF-N	0 / 0	-38.5	-38.5	0.11 (3)	10.00			
N-AG	0 / 4684	-38.5	-38.5	0.38 (1)	10.00			
AG-AH	0 / 4684	-38.5	-38.5	0.38 (1)	10.00			
AH-AI	0 / 4684	-38.5	-38.5	0.38 (1)	10.00			
AI-M	0 / 4684	-38.5	-38.5	0.38 (1)	10.00			
M-L	0 / 6605	-38.5	-38.5	0.48 (1)	10.00			
L-AJ	0 / 6605	-38.5	-38.5	0.48 (1)	10.00			
AJ-K	0 / 6605	-38.5	-38.5	0.48 (1)	10.00			
K-AK	0 / 4682	-38.5	-38.5	0.38 (1)	10.00			
AK-AL	0 / 4682	-38.5	-38.5	0.38 (1)	10.00			
AL-AM	0 / 4682	-38.5	-38.5	0.38 (1)	10.00			
AM-J	0 / 4682	-38.5	-38.5	0.38 (1)	10.00			
J-AN	0 / 0	-38.5	-38.5	0.11 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/899 (0.28")

CSI: TC=0.38/1.00 (A-O-1), BC=0.48/1.00 (K-M-1), WB=0.67/1.00 (A-N-1), SS=0.21/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

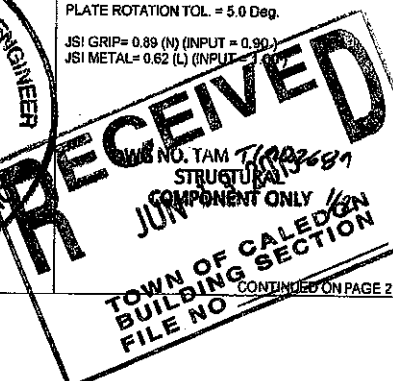
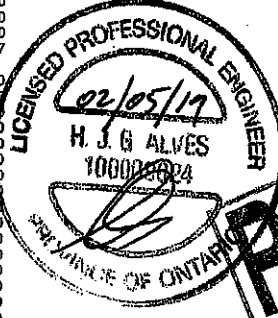
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1056

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.62 (L) (INPUT = 0.60)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T1	1	2	Preston 2	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:56:53 2019 Page 2
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LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM	TO	LENGTH	FR-TO		
AN-AO	0/0	-38.5	-38.5	0.11 (3)	10.00		
AO-AP	0/0	-38.5	-38.5	0.11 (3)	10.00		
AP-I	0/0	-38.5	-38.5	0.11 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	22-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
J	22-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
L	13-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
P	1-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
Q	3-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
R	5-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
S	7-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
T	9-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
U	11-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
V	13-10-12	-123	-123	--	BACK	VERT	TOTAL	--	--
W	14-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
X	16-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
Y	18-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
Z	20-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
AB	24-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
AC	26-9-4	-123	-123	--	BACK	VERT	TOTAL	--	--
AD	1-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AE	3-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AF	5-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AG	7-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AH	9-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AI	11-10-12	-55	-70	--	BACK	VERT	TOTAL	--	--
AJ	14-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AK	16-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AL	18-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AM	20-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AO	24-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--
AP	26-9-4	-55	-70	--	BACK	VERT	TOTAL	--	--

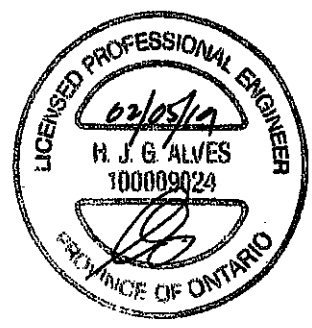
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JUN 13 2019

TOWN OF CALEDON

BUILDING SECTION

FILE NO

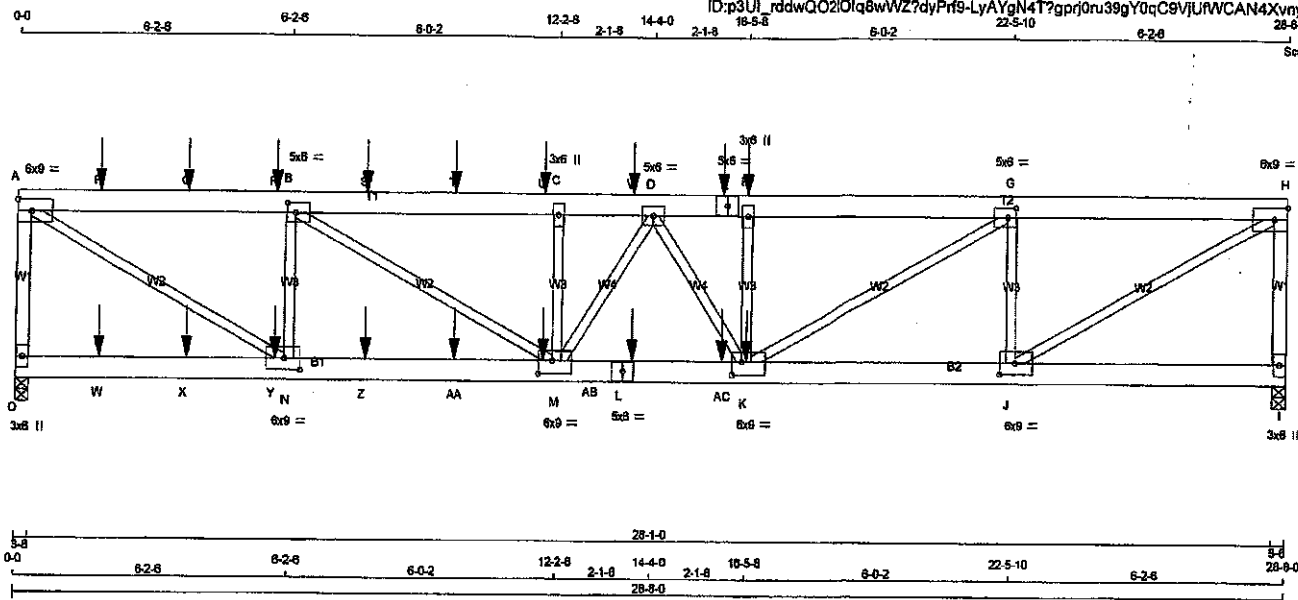


DWG NO. TAM T1902689

STRUCTURAL

COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T1Z	1	2	Preston 2	
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:56:55 2019 Page 1			
		ID: p3UI_rddwQO2OIq8wWZ7dyPrf9-LyAYgN4T7gpr0r039gY0qC9VJURWCAN4Xvny8zo9Hc			
		22-5-10 28-6-0			
		Scale = 1:40.7			



TOTAL WEIGHT = 2 X 148 = 286 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
O - A	2x4	DRY No.2	SPF
A - E	2x6	DRY No.2	SPF
E - H	2x6	DRY No.2	SPF
I - H	2x4	DRY No.2	SPF
O - L	2x6	DRY No.2	SPF
L - I	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3") SPIRAL NAILS		
O - A	12	TOP
A - E	12	TOP
E - H	12	SIDE(183.1)
2	12	SIDE(122.0)
BOTTOM CHORDS : (0.122'X3") SPIRAL NAILS		
O - L	12	SIDE(183.1)
L - I	12	SIDE(183.1)
WEBS : (0.122'X3") SPIRAL NAILS		
K - F	4	SIDE(389.8)
2x3	4	
M - C	4	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMWV-t	MT20	6.0	9.0	Edge
B	TMWV-t	MT20	5.0	6.0	2.25
C	TMWV-t	MT20	3.0	6.0	
D	TMWV-t	MT20	5.0	6.0	
E	TS-t	MT20	5.0	6.0	
F	TMWV-t	MT20	3.0	6.0	
G	TMWV-t	MT20	5.0	6.0	2.50 2.25
H	TMWV-t	MT20	6.0	9.0	Edge
I	BMV1-p	MT20	3.0	6.0	
J	BMWV-t	MT20	6.0	9.0	3.00 4.25
K	BMWV-t	MT20	6.0	9.0	3.50 2.75
L	BS-t	MT20	5.0	6.0	
M	BMWV-t	MT20	6.0	9.0	3.50 3.75
N	BMWV-t	MT20	6.0	9.0	3.00 4.25
O	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3954	3954	0	3-8
I VERT	3747	3747	0	3-8

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERMLIVE	WIND	DEAD	SOIL
O	2942	1869 / 0	565 / 0	0 / 0	707 / 0	0 / 0
I	2777	1815 / 0	509 / 0	0 / 0	653 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 3.75 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS	MAX. FACTORED MEMB. FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)
O-A	-3797 / 0	0.0	0.0	0.44 (1)	8.05	K-F	-467 / 0	0.06 (1)	
A-P	-5836 / 0	-102.1	-102.1	0.28 (1)	4.84	M-C	-571 / 0	0.08 (1)	
P-Q	-5836 / 0	-102.1	-102.1	0.28 (1)	4.84	B-M	0 / 3522	0.44 (1)	
Q-R	-5836 / 0	-102.1	-102.1	0.28 (1)	4.84	A-N	0 / 6771	0.84 (1)	
R-B	-5836 / 0	-102.1	-102.1	0.28 (1)	4.84	N-B	-2983 / 0	0.38 (1)	
B-S	-8848 / 0	-102.1	-102.1	0.37 (1)	3.82	J-H	0 / 6671	0.83 (1)	
S-T	-8848 / 0	-102.1	-102.1	0.37 (1)	3.82	K-G	0 / 4514	0.58 (1)	
T-U	-8848 / 0	-102.1	-102.1	0.37 (1)	3.82	J-G	-3082 / 0	0.37 (1)	
U-C	-8848 / 0	-102.1	-102.1	0.37 (1)	3.82	M-D	-927 / 0	0.14 (1)	
C-V	-8848 / 0	-102.1	-102.1	0.18 (1)	4.00	D-K	-36 / 567	0.07 (1)	
V-D	-8848 / 0	-102.1	-102.1	0.18 (1)	4.00				
D-E	-9810 / 0	-102.1	-102.1	0.21 (1)	3.83				
E-F	-9810 / 0	-102.1	-102.1	0.21 (1)	3.83				
F-G	-9810 / 0	-102.1	-102.1	0.31 (1)	3.75				
G-H	-5750 / 0	-102.1	-102.1	0.20 (1)	4.77				
H-I	-3643 / 0	0.0	0.0	0.42 (1)	6.15				

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-10-12	-123	-123		BACK	VERT	TOTAL		
F	16-5-8	-52	-78	232	BACK	VERT	TOTAL		
K	16-5-8	-2196	-2196		BACK	VERT	TOTAL		
L	13-10-12	-55	-70		BACK	VERT	TOTAL		
P	1-10-12	-123	-123		BACK	VERT	TOTAL		
Q	3-10-12	-123	-123		BACK	VERT	TOTAL		
R	5-10-12	-123	-123		BACK	VERT	TOTAL		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	15-10-12	-123	-123		BACK	VERT	TOTAL		
F	16-5-8	-52	-78	232	BACK	VERT	TOTAL		
K	16-5-8	-2196	-2196		BACK	VERT	TOTAL		
L	13-10-12	-55	-70		BACK	VERT	TOTAL		
P	1-10-12	-123	-123		BACK	VERT	TOTAL		
Q	3-10-12	-123	-123		BACK	VERT	TOTAL		
R	5-10-12	-123	-123		BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.96")
CALCULATED VERT. DEFL. (LL) = L/999 (0.22")
ALLOWABLE DEFL. (TL) = L/360 (0.96")
CALCULATED VERT. DEFL. (TL) = L/949 (0.38")

CSL TC=0.44/1.00 (A-O:1), BC=0.69/1.00 (K-M:1),
WB=0.84/1.00 (A-N:1), SS=0.21/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (J) (INPUT=0.90)
JSI METAL=0.90 (L) (INPUT=1.00)

DWG NO. TAM 17902690
STRUCTURAL
COMPONENT ONLY 1/2

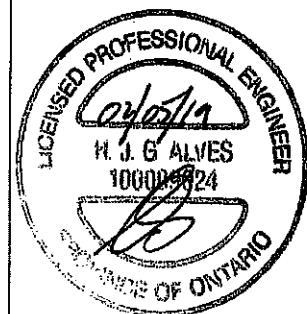
CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	Preston 2	DRWG NO.
200170-400367	T1Z	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

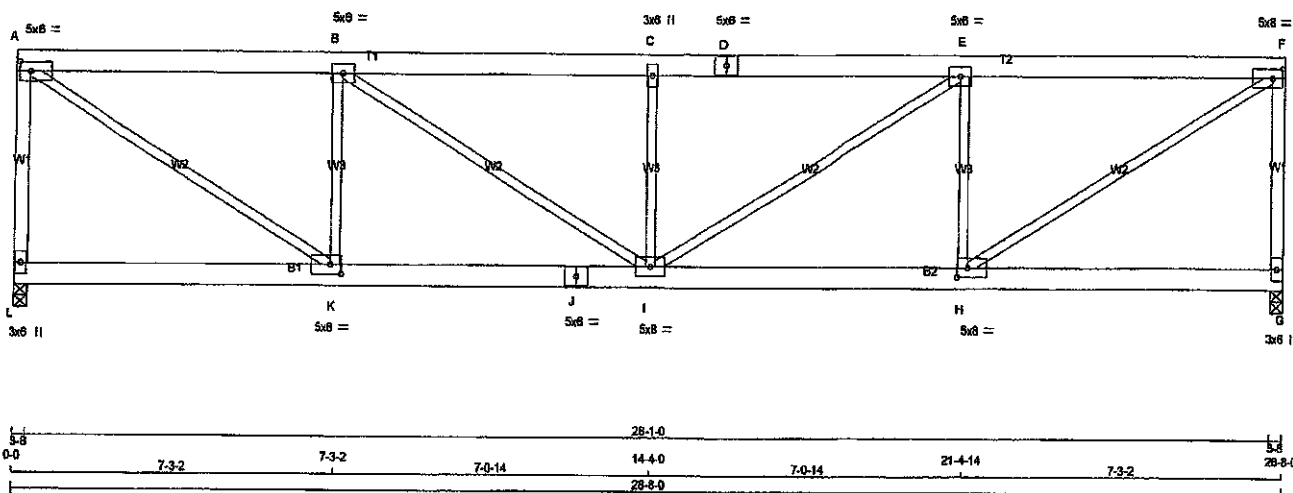
Version 6.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:59:55 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
S	7-10-12	-123	-123	---	BACK	VERT	TOTAL	---	---
T	9-10-12	-123	-123	---	BACK	VERT	TOTAL	---	---
U	11-10-12	-123	-123	---	BACK	VERT	TOTAL	---	---
V	13-10-12	-123	-123	---	BACK	VERT	TOTAL	---	---
W	1-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
X	3-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
Y	5-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
Z	7-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AA	9-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AB	11-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AC	15-10-12	-55	-70	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 71902690
STRUCTURAL
COMPONENT ONLY 3/4

JOB NAME 200170-400367	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. Preston 2	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:56:56 2018 Page 1	
ID: p3U1_rddwQO2OIq8wVZ7dyPrf9-p8kwuj55mzxhKAP4csCnZ1kF26ulF6XJBaLUbz09Hb				28-9-0	
Scale = 1:48.7					



TOTAL WEIGHT = 2 X 149 = 297 lb

LUMBERS

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
L - A	2x4	DRY	No.2
L - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - G	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

DESCR.

DESCR.	SPF
SPF	SPF
SPF	SPF
SPF	SPF
SPF	SPF
SPF	SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
L	2015	0	2015	0
G	2015	0	2015	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN.	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
L	1506	832 / 0	301 / 0
G	1506	832 / 0	301 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.32 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	CS1 (LC)	FR-TO	(LBS)
L-A	-1898 / 0	0.0	0.0	0.70 (1)	6.10
A-B	-2496 / 0	-102.1	-102.1	0.41 (1)	4.60
B-C	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32
C-D	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32
D-E	-3193 / 0	-102.1	-102.1	0.44 (1)	4.32
E-F	-2496 / 0	-102.1	-102.1	0.41 (1)	4.60
G-F	-1898 / 0	0.0	0.0	0.70 (1)	6.10
L-K	0 / 0	-38.5	-38.5	0.17 (3)	10.00
K-J	0 / 2496	-38.5	-38.5	0.41 (2)	10.00
J-I	0 / 2496	-38.5	-38.5	0.41 (2)	10.00
I-H	0 / 2496	-38.5	-38.5	0.41 (2)	10.00
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.96")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.15")
ALLOWABLE DEFL. (TL) = $L/360$ (0.96")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.25")

CS1: TC=0.70/1.00 (A-L1), BC=0.41/1.00 (I-K2), WB=0.67/1.00 (F-H1), SS=0.27/1.00 (A-B1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1887

PLATE PLACEMENT TOL. = 0.250 inches

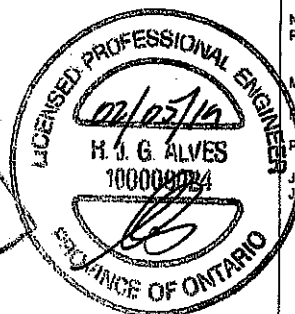
PLATE ROTATION TOL. = 5.0 Deg.

J3 GRIP= 0.89 (F) (INPUT = 0.80)
J1 METAL= 0.49 (J) (INPUT = 1.00)

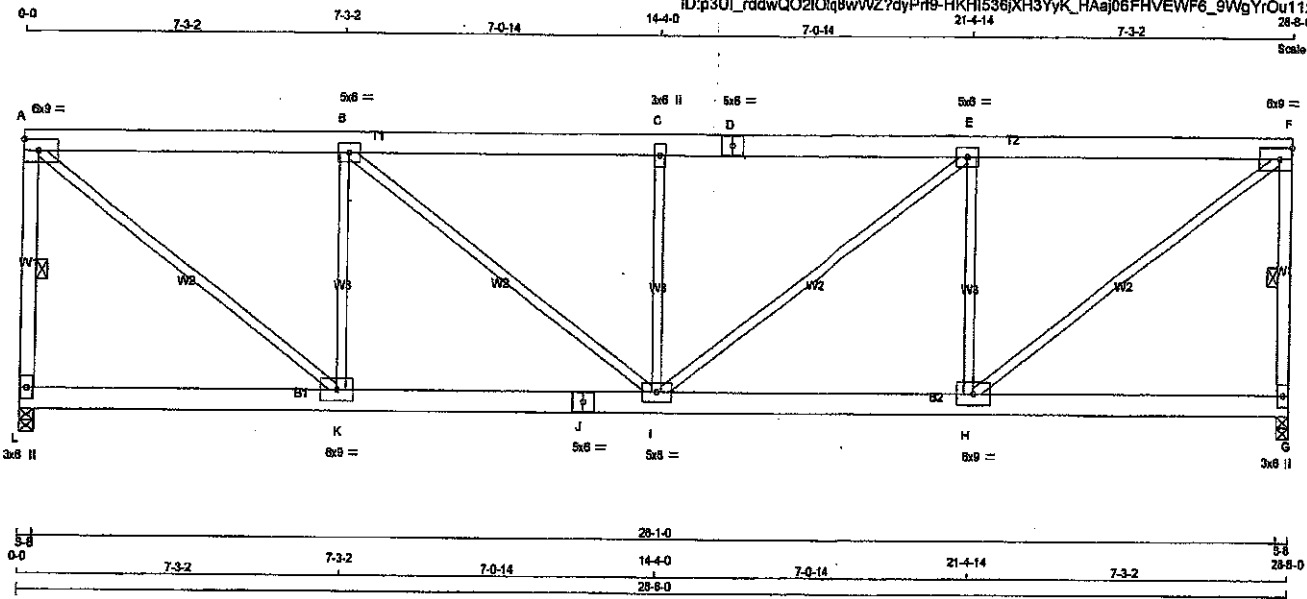
DWG NO. TAM 7902691
STRUCTURAL
COMPONENT ONLY

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



JOB NAME 200170-400367	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. Preston 2	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:56:57 2019 Page 1 id:p3UL_rddwQO2IOlg8wVWZ?dyPrf9-HKH1536jXH3YyK_HAaj06FHVEWf6_9WgYrOu11zo9Hb 28-8-0 Scale = 1/48.0		



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4 DRY	No.2	SPF
A - D	2x6 DRY	No.2	SPF
D - F	2x6 DRY	No.2	SPF
G - F	2x4 DRY	No.2	SPF
L - J	2x6 DRY	No.2	SPF
J - G	2x6 DRY	No.2	SPF
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN Y X
A	TM/VW-1	MT20	6.0	9.0
B	TM/VW-1	MT20	5.0	6.0
C	TM/VW-1	MT20	3.0	6.0
D	TS-1	MT20	5.0	6.0
E	TM/VW-1	MT20	5.0	6.0
F	TM/VW-1	MT20	6.0	9.0
G	BM/V1+P	MT20	3.0	6.0
H	BM/VW-1	MT20	6.0	9.0
I	BM/VW-1	MT20	5.0	6.0
J	BS-1	MT20	5.0	6.0
K	BM/VW-1	MT20	6.0	9.0
L	BM/V1+P	MT20	3.0	6.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	BRG
L	2015	0	2015	0	0	3-8	3-8		
G	2015	0	2015	0	0	3-8	3-8		

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS		SNOW		LIVE		PERM. LIVE		WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL								
L	1508	832/0	301/0	0/0	0/0	373/0	0/0								
G	1508	832/0	301/0	0/0	0/0	373/0	0/0								

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. (PLF)	MAX. (LC)	MEMB.	FORCE (LBS)	MAX. (LC)	MAX. (LC)
FR-TO		FROM	TO	FR-TO			
L-A	-1900/0	0.0	0.0 0.31 (1)	H-F	0/2608	0.59 (1)	
A-B	-2059/0	-102.1	-102.1 0.40 (1)	A-K	0/2608	0.59 (1)	
B-C	-2630/0	-102.1	-102.1 0.42 (1)	H-E	-1289/0	0.66 (1)	
C-D	-2630/0	-102.1	-102.1 0.42 (1)	K-B	-1289/0	0.66 (1)	
D-E	-2630/0	-102.1	-102.1 0.42 (1)	I-E	0/731	0.16 (1)	
E-F	-2059/0	-102.1	-102.1 0.40 (1)	B-I	0/731	0.16 (1)	
G-F	-1900/0	0.0	0.0 0.31 (1)	I-C	-665/0	0.34 (1)	
L-K	0/0	-38.5	-38.5 0.16 (3)				
K-J	0/2059	-38.5	-38.5 0.37 (2)				
J-I	0/2059	-38.5	-38.5 0.37 (2)				
I-H	0/2059	-38.5	-38.5 0.37 (2)				
H-G	0/0	-38.5	-38.5 0.16 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.96")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (0.96")
CALCULATED VERT. DEFL. (TL) = L/999 (0.20")

CSK: TC=0.42/1.00 (B-C:1), BC=0.37/1.00 (H-C:2), WB=0.66/1.00 (E-H:1), SSI=0.27/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

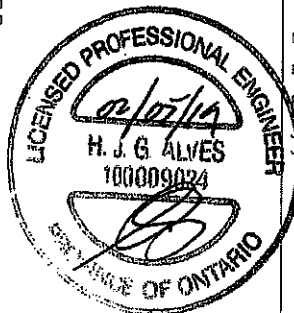
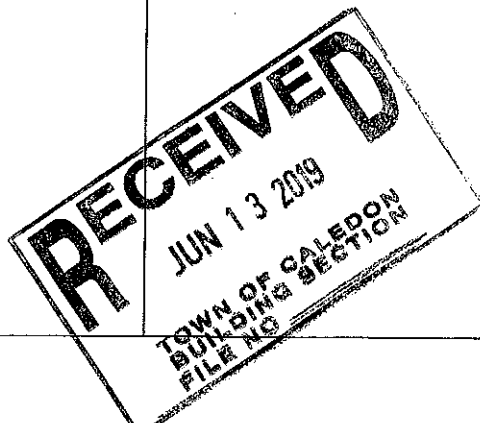
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

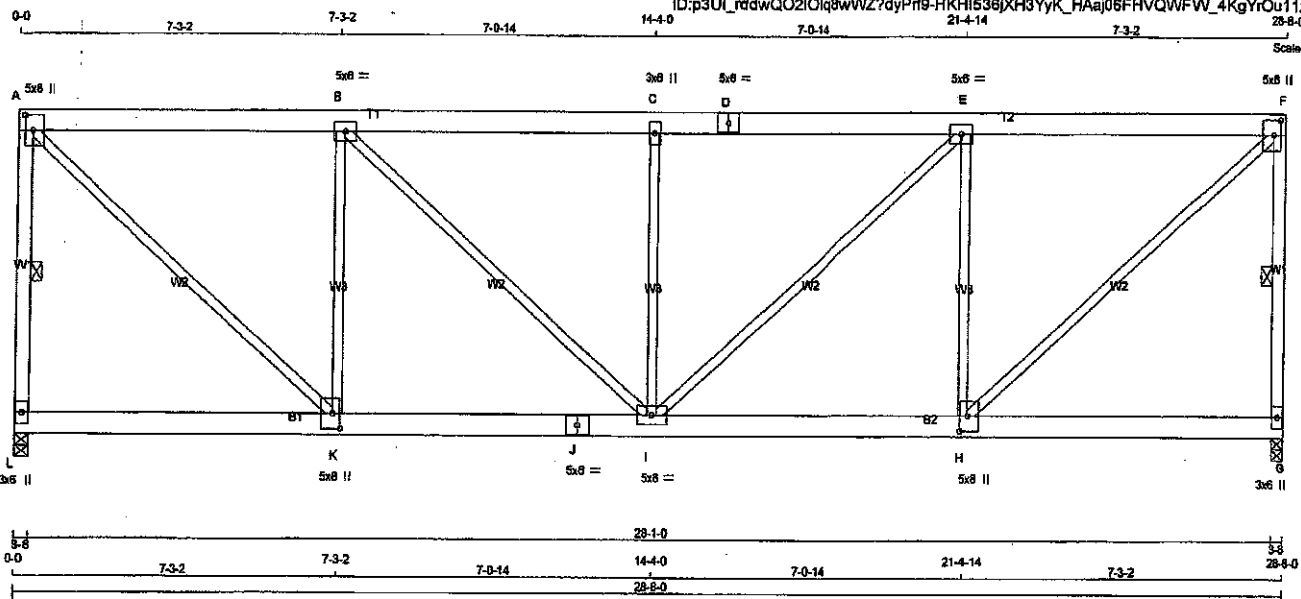
PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.77 (K) (INPUT = 0.99)
SI METAL= 0.41 (J) (INPUT = 1.00)

DWG NO. TAM T1902692
STRUCTURAL
COMPONENT ONLY



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T4	2	1	Preston 2	
Tamarack Roof Truss, Burlington		Version 8.230 8 Nov 17 2018 MITek Industries, Inc. Tue Feb 5 15:58:57 2019 Page 1			
		ID:p3U1_rddwQ02iOlg8wWZ?dyPrf9-HKH1536jXH3YyK_HAAj06FHVQWFW_4KgYrOu11zo8Ma			
		28-8-0			
		Scale = 1:48.0			



TOTAL WEIGHT = 2 X 162 = 324 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
L - A	2x4	DRY	No.2	SPF
A - D	2x6	DRY	No.2	SPF
D - F	2x6	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
J - G	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVV+p	MT20	5.0	8.0	4.00	2.00
B	TMVV-w	MT20	5.0	6.0		
C	TMVV-w	MT20	3.0	6.0		
D	TS-t	MT20	5.0	6.0		
E	TMVV-w	MT20	5.0	6.0		
F	TMVV+p	MT20	5.0	8.0	4.00	2.00
G	BMV1+p	MT20	3.0	6.0		
H	BMVV-w	MT20	5.0	8.0	4.00	2.00
I	BMVV-w	MT20	5.0	8.0		
J	BS-t	MT20	5.0	6.0		
K	BMVV-w	MT20	5.0	8.0	4.00	2.00
L	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
L	2015	0	0	3-8
G	2015	0	0	3-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
L	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0
G	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-L, F-G.

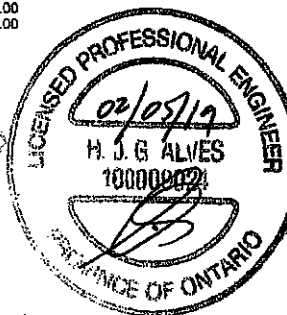
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 MAX (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO
L-A	-1901 / 0	0.0	0.0	0.40 (1)	4.87	H-F	0 / 2374
A-B	-1751 / 0	-102.1	-102.1	0.39 (1)	5.51	A-K	0 / 2374
B-C	-2238 / 0	-102.1	-102.1	0.41 (1)	5.00	H-E	-1289 / 0
C-D	-2238 / 0	-102.1	-102.1	0.41 (1)	5.00	K-B	-1289 / 0
D-E	-2238 / 0	-102.1	-102.1	0.41 (1)	5.00	I-E	0 / 884
E-F	-1751 / 0	-102.1	-102.1	0.39 (1)	5.51	B-I	0 / 884
G-F	-1901 / 0	0.0	0.0	0.40 (1)	4.87	I-C	-884 / 0
L-K	0 / 0	-38.5	-38.5	0.18 (3)	10.00		
K-J	0 / 1751	38.5	38.5	0.34 (2)	10.00		
J-I	0 / 1751	38.5	38.5	0.34 (2)	10.00		
I-H	0 / 1751	38.5	38.5	0.34 (2)	10.00		
H-G	0 / 0	-38.5	-38.5	0.18 (3)	10.00		

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DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 088-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.98")
CALCULATED VERT. DEFL.(LL) = L/999 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (0.98")
CALCULATED VERT. DEFL.(TL) = L/999 (0.17")

CSI: TC=0.41/1.00 (B-C-1), BC=0.34/1.00 (I-K-2), WB=0.99/1.00 (E-H-1), SG=0.27/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

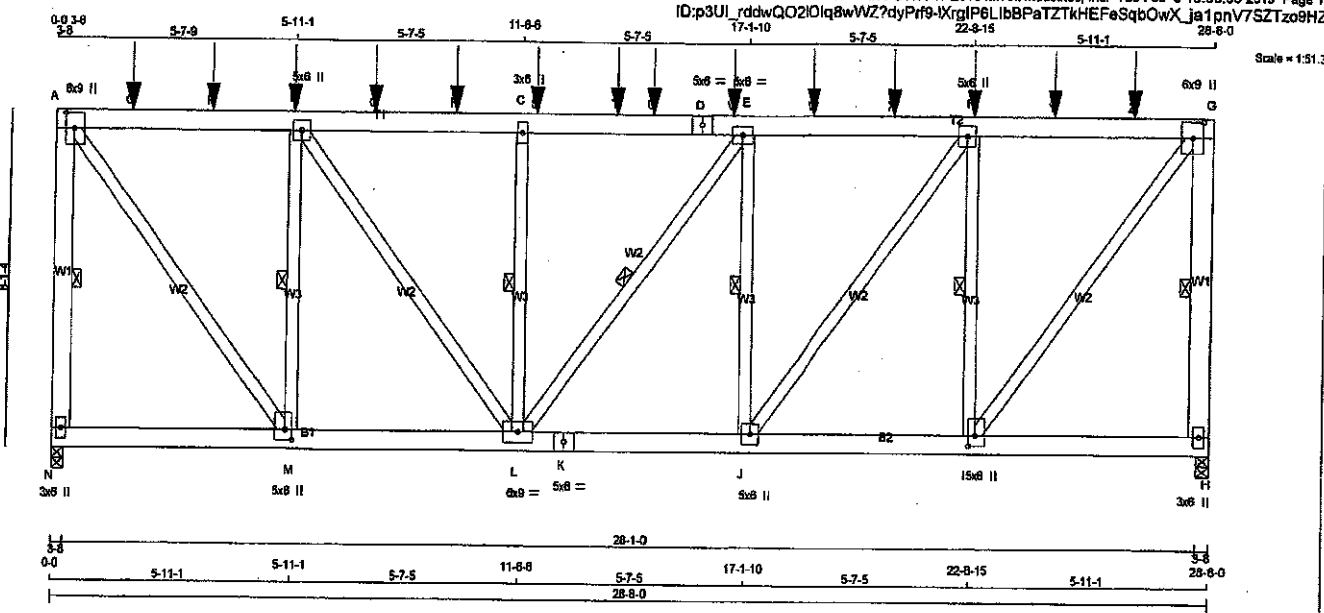
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.83 (K) (INPUT = 1.00)

DWG NO. TAM 71902673
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T5	11	1	Preston 2	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Feb 5 15:56:58 2019 Page 1
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TOTAL WEIGHT = 11 X 212 = 2335 lb

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	FACTORED	MAXIMUM FACTORED	INPUT
N - A	2x6	DRY	No.2	SPF	GROSS REACTION	GROSS REACTION	BRG
A - D	2x6	DRY	No.2	SPF	VERT	DOWN	IN-SX
D - G	2x6	DRY	No.2	SPF	H	H	3-8
H - G	2x6	DRY	No.2	SPF			
N - K	2x6	DRY	No.2	SPF			
K - H	2x6	DRY	No.2	SPF			

ALL WEBS 2x4 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)				UNFACTORED REACTIONS			
JT	TYPE	PLATES	W LEN Y X	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND
A	TMVW+p	MT20	6.0 9.0 4.50 2.75	2934	1473 / 0	694 / 0	0 / 0
B	TMVW+t	MT20	5.0 6.0	2934	1473 / 0	694 / 0	0 / 0
C	TMVW+w	MT20	3.0 6.0				
D	TS-t	MT20	5.0 6.0				
E	TMVW-t	MT20	5.0 6.0				
F	TMVW+t	MT20	5.0 6.0				
G	TMVW+p	MT20	6.0 9.0 4.50 2.75				
H	BMV1+p	MT20	3.0 6.0				
I	BMVW+t	MT20	5.0 6.0				
J	BMVW+t	MT20	5.0 6.0				
K	BS-t	MT20	5.0 6.0				
L	BMVW+t	MT20	5.0 6.0				
M	BMVW+t	MT20	5.0 6.0				
N	BMV1+p	MT20	3.0 6.0				

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H
INSTALL TWO USP SBP4 BEARING PLATES AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF A-N, G-H, F-I, B-M, E-J, C-L, E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

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BUILDING SECTION
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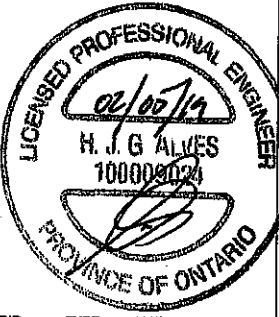
I-G	-3707 / 0	-102.1 -102.1 0.52 (1)	3.82
U-D	-3707 / 0	-102.1 -102.1 0.52 (1)	3.82
D-V	-3707 / 0	-102.1 -102.1 0.52 (1)	3.82
V-E	-3707 / 0	-102.1 -102.1 0.52 (1)	3.82
E-W	-3717 / 0	-102.1 -102.1 0.59 (1)	3.80
W-X	-3717 / 0	-102.1 -102.1 0.59 (1)	3.80
X-F	-3717 / 0	-102.1 -102.1 0.59 (1)	3.80
F-Y	-2566 / 0	-102.1 -102.1 0.54 (1)	4.51
Y-Z	-2566 / 0	-102.1 -102.1 0.54 (1)	4.51
Z-G	-2566 / 0	-102.1 -102.1 0.54 (1)	4.51
G-H	-3766 / 0	0.0 0.0 0.65 (1)	4.42
N-M	0 / 0	-38.5 -38.5 0.11 (3)	10.00
M-L	0 / 2569	-38.5 -38.5 0.38 (1)	10.00
L-K	0 / 3717	-38.5 -38.5 0.52 (1)	10.00
K-J	0 / 3717	-38.5 -38.5 0.52 (1)	10.00
J-I	0 / 2566	-38.5 -38.5 0.37 (1)	10.00
I-H	0 / 0	-38.5 -38.5 0.11 (3)	10.00

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MT20	618 354 1667 788	1987 1666
PLATE PLACEMENT TOL. = 0.250 inches		
PLATE ROTATION TOL. = 5.0 Deg.		
SEI GRIP= 0.89 (M)	(INPUT = 0.90)	
SEI METAL= 0.93 (M)	(INPUT = 1.00)	

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	5-10-12	-264	-264	-	TOP	VERT	TOTAL	-	-
F	22-9-4	-264	-264	-	TOP	VERT	TOTAL	-	-
O	1-10-12	-264	-264	-	TOP	VERT	TOTAL	-	-
P	3-10-12	-264	-264	-	TOP	VERT	TOTAL	-	-

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DWG NO. TAM 11902694
STRUCTURAL
COMPONENT ONLY 1/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T5	11	1	Preston 2	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

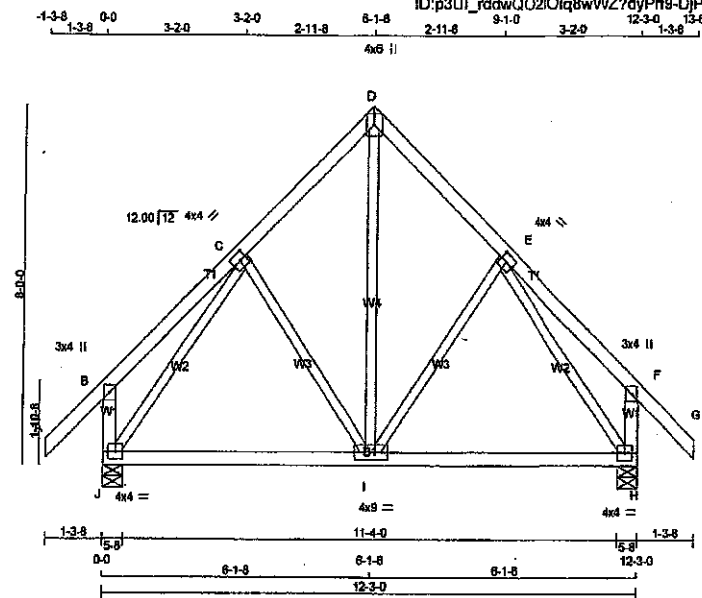
Version 8.230 S Nov 17 2018 Mitak Industries, Inc. Tue Feb 5 15:55:58 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Q	7-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
R	9-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
S	11-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
T	13-10-12	-264	-264	—	TOP	VERT	TOTAL	—	—
U	14-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
V	18-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
W	19-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
X	20-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
Y	24-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—
Z	26-9-4	-264	-264	—	TOP	VERT	TOTAL	—	—



DWG NO. TAM 17902694
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T6-Cond1	6	1	Preston 2	
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:56:59 2019 Page 1			
		ID:p3UI_rddwQO2iOiq8wVWZ?dyP19-DJP3VVI7z3uJGCd8il?iUBgMu_KwJS7Fz79t75wzo9HY			
		Scale = 1/47.5			



TOTAL WEIGHT = 6 X 65 = 396 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
J - B	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW+t	MT20	4.0	4.0	2.00	1.75
D TTW+p	MT20	4.0	6.0		
E TMVW+t	MT20	4.0	4.0	2.00	1.75
F TMV+p	MT20	3.0	4.0		
H BMVW1-t	MT20	4.0	4.0		
I BMVWV1-t	MT20	4.0	9.0		
J BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED		MAXIMUM FACTORED		INPUT	REQRD
	GROSS REACTION	DOWN	GROSS REACTION	UP		
JT	VERT	HORZ	DOWN	HORZ	BRG	BRG
J	1004	0	1004	0	5-8	5-8
H	1004	0	1004	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
JT	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0
H	741	437 / 0	129 / 0	0 / 0	0 / 0	176 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

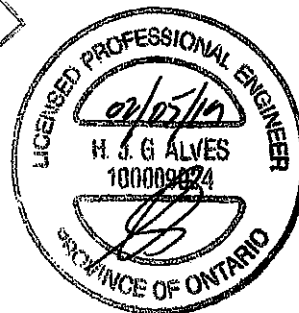
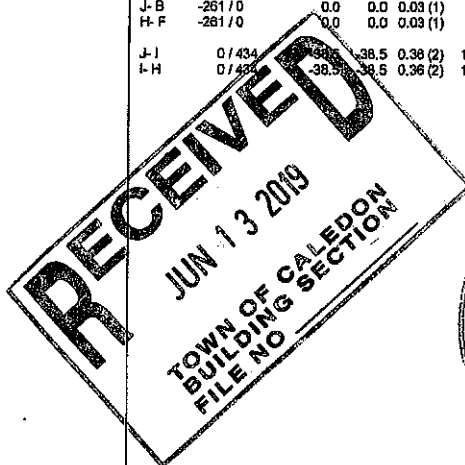
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 50	-102.1 -102.1	0.14 (1)	I-D	0 / 486	0.11 (1)	
B-C	0 / 25	-102.1 -102.1	0.15 (1)	I-E	-117 / 24	0.06 (1)	
C-D	-544 / 0	-102.1 -102.1	0.12 (1)	C-I	-117 / 24	0.06 (1)	
D-E	-544 / 0	-102.1 -102.1	0.12 (1)	J-C	-784 / 0	0.38 (1)	
E-F	0 / 25	-102.1 -102.1	0.15 (1)	E-H	-784 / 0	0.38 (1)	
F-G	0 / 50	-102.1 -102.1	0.14 (1)				
J-B	-261 / 0	0.0	0.03 (1)				
H-F	-261 / 0	0.0	0.03 (1)				
J-I	0 / 434	38.5	0.36 (2)				
I-H	0 / 434	38.5	0.36 (2)				



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA C88-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.15/1.00 (B-C-1), BC=0.36/1.00 (I-J-2), WB=0.36/1.00 (C-J-1), SSI=0.16/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1907 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (E) (INPUT = 0.80)
JSI METAL = 0.30 (E) (INPUT = 1.00)

DRWG NO. TAM 11902695
STRUCTURAL
COMPONENT ONLY

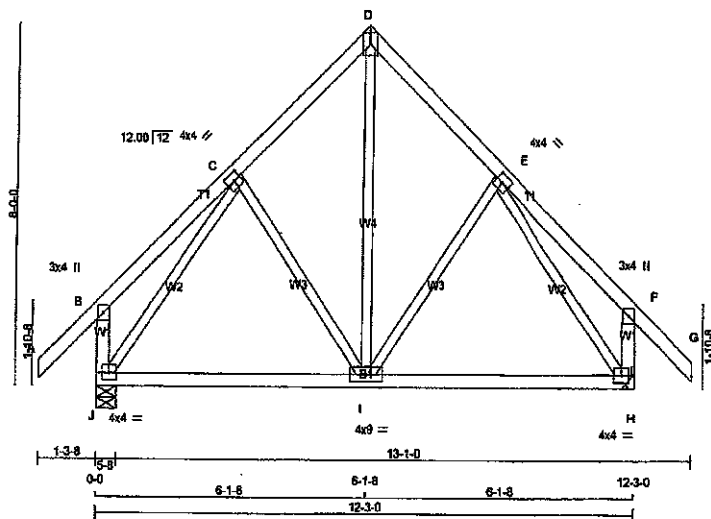
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	T6-Cond2	6	1	Preston 2	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

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1-3-8 0-0 3-2-0 3-2-0 2-11-8 6-1-8 2-11-8 8-1-0 3-2-0 12-3-0 13-6-8

Scale = 1/4" = 1'-0"



TOTAL WEIGHT = 6 X 65 = 390 lb

[M/F]

LUMBER

N. L. G. A. RULES

CHORDS SIZE LUMBER

A - D 2x4 DRY No.2

D - G 2x4 DRY No.2

H - F 2x4 DRY No.2

J - H 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

SPF

SPF

SPF

SPF

SPF

SPF

SPF

SPF

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SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION MAXIMUM FACTORED INPUT REQD

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

J 1004 0 1004 0 0 5-3 5-3

H 1004 0 1004 0 0 MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM

BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

1ST LCASE MAX/MIN. COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOIL

J 741 437 / 0 129 / 0 0 / 0 0 / 0 176 / 0 0 / 0

H 741 437 / 0 129 / 0 0 / 0 0 / 0 176 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY

APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE VERT. LOAD LC1 MAX. MAX. WEBS

FR-TO 0 / 50 -102.1 -102.1 0.14 (1) 10.00 I-D 0 / 488 0.11 (1)

A-B 0 / 25 -102.1 -102.1 0.15 (1) 10.00 I-E -117 / 24 0.06 (1)

B-C -544 / 0 -102.1 -102.1 0.12 (1) 6.25 C-I -117 / 24 0.06 (1)

C-D -544 / 0 -102.1 -102.1 0.12 (1) 6.25 J-C -784 / 0 0.38 (1)

D-E 0 / 25 -102.1 -102.1 0.15 (1) 10.00 E-H -784 / 0 0.38 (1)

E-F 0 / 50 -102.1 -102.1 0.14 (1) 10.00

F-G -261 / 0 0.0 0.0 0.03 (1) 7.81

G-H -261 / 0 0.0 0.0 0.03 (1) 7.81

H-I 0 / 434 0.0 0.0 0.03 (2) 10.00

I-H 0 / 434 0.0 0.0 0.03 (2) 10.00

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

RECEIVED
JUN 13 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

LICENSED PROFESSIONAL ENGINEER
02/05/14
H. J. G. ALVES
100009824
PROFESSIONAL ENGINEER
ON THE ENGINE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL

OR SMALL BUILDING REQUIREMENTS OF

PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012

- CSA 088-08, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F.

RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED

ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.41")

CALCULATED VERT. DEFL. (LL) = L/999 (0.04")

ALLOWABLE DEFL. (TL) = L/360 (0.41")

CALCULATED VERT. DEFL. (TL) = L/889 (0.07")

CSI: TC=0.15/1.00 (B-C-1), BC=0.36/1.00 (I-J-2),

WB=0.38/1.00 (C-J-1), SS=0.16/1.00 (I-J-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT

RESPONSIBLE FOR QUALITY CONTROL IN

THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.88 (E) (INPUT = 0.90)

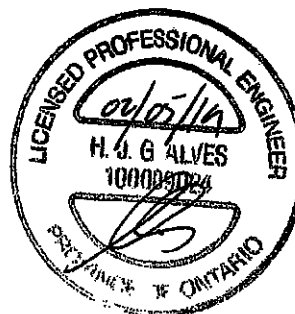
ISI METAL= 0.30 (E) (INPUT = 1.00)

DWG NO. TAM 11902696
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

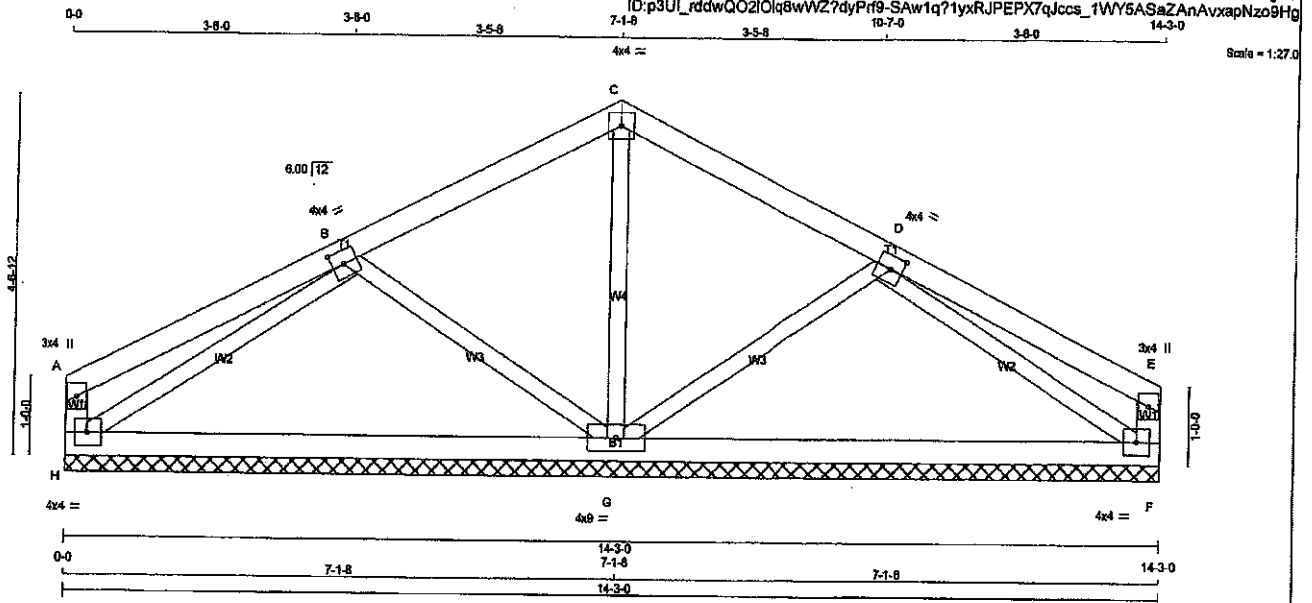


DWG NO. TAM 11902697
STRUCTURAL
COMPONENT ONLY



JOB NAME 200170-400367	TRUSS NAME PB1	QUANTITY 14	PLY 1	JOB DESC. Preston 2	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 5 Nov 17 2016 Mitek Industries, Inc. Tue Feb 5 15:56:51 2019 Page 1
ID: p3UI_rddwQO2IOlq8wWZ7dyPrf9-SAw1q71yxRJPEPX7qJccs_1WY5ASaZAnAvxapNzo9Hg



TOTAL WEIGHT = 14 X 52 = 733 LB

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
H - A	2x4	DRY No.2	SPF
F - E	2x4	DRY No.2	SPF
H - F	2x4	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (Table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMVWV-t	MT20	4.0	4.0	2.00	1.75
C	TTW-p	MT20	4.0	4.0		
D	TMVWV-t	MT20	4.0	4.0	2.00	1.75
E	TMV+p	MT20	3.0	4.0		
F	BMVWV-t	MT20	4.0	4.0		
G	BMVWV-t	MT20	4.0	4.0		
H	BMVWV-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
G	1272	0	1272	0
H	366	0	366	0
F	366	0	366	0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
G	949	528 / 0	187 / 0	0 / 0	234 / 0	0 / 0
H	274	150 / 0	58 / 0	0 / 0	88 / 0	0 / 0
F	274	150 / 0	58 / 0	0 / 0	88 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED VERT. LOAD LC1 (LC)	MAX. UNBRAC.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD LC1 (LC)
FR-TO		FROM TO		LENGTH	FR-TO		
A-B	0 / 20	-102.1	-102.1	0.22 (1)	10.00	G-C	-442 / 0
B-C	0 / 189	-102.1	-102.1	0.24 (1)	10.00	G-D	-425 / 0
C-D	0 / 189	-102.1	-102.1	0.24 (1)	10.00	B-G	-425 / 0
D-E	0 / 20	-102.1	-102.1	0.22 (1)	10.00	H-B	-219 / 0
H-A	-138 / 0	0.0	0.0	0.01 (1)	7.81	D-F	-219 / 0
F-E	-138 / 0	0.0	0.0	0.01 (1)	7.81		
H-G	0 / 180	-38.5	-38.5	0.45 (3)	10.00		
G-F	0 / 180	-38.5	-38.5	0.45 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	8.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (C-D:1), BC=0.45/1.00 (F-G:3), WB=0.14/1.00 (D-G:1), SSI=0.18/1.00 (F-G:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

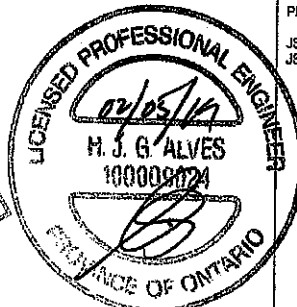
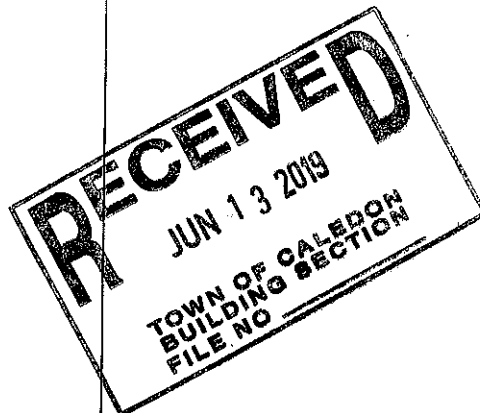
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

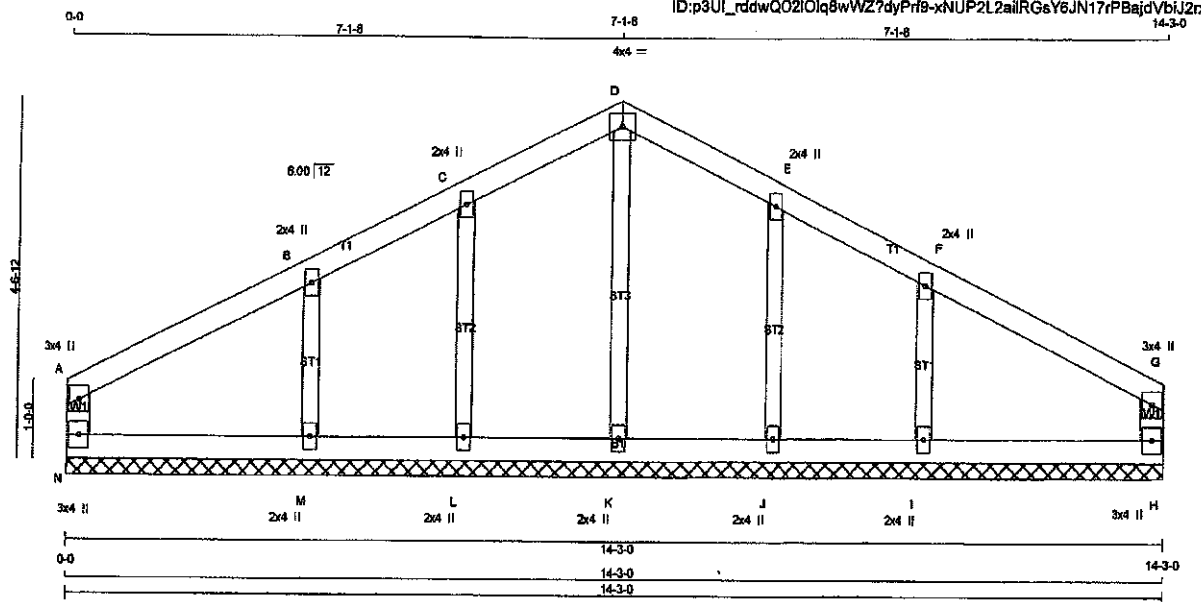
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.41 (B) (INPUT = 0.80)
JSI METAL = 0.13 (C) (INPUT = 1.00)



DRWG NO. TAM 17902700
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	PB1G	2	1	Preston 2	
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Feb 5 15:56:52 2019 Page 1	
				ID:p3UI_rddwQO2IOlq8wVZ7dyPrf9-xNUP2L2ailRGsY6JN17rPBajdvbiJ2nxOZg7Lqzo9Hf	
				Scale = 1:27.0	



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
N - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
A, TMW+p	MT20	3.0	4.0		
B, C, E, F					
B, TMW+w	MT20	2.0	4.0		
D, TMW+p	MT20	4.0	4.0		
G, TMW+p	MT20	3.0	4.0		
H, BMV1+p	MT20	3.0	4.0		
I, J, K, L, M					
I, BMV1+w	MT20	2.0	4.0		
N, BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	CS (LC)
FR-TO				FR-TO			
N-A	-213 / 0	0.0	0.08 (1)	K-D	-122 / 11	0.04 (1)	
A-B	-130 / 0	-102.1	0.08 (1)	L-C	-176 / 0	0.04 (1)	
B-C	-131 / 0	-102.1	0.08 (1)	M-B	-278 / 0	0.04 (1)	
C-D	-114 / 0	-102.1	0.08 (1)	J-E	-178 / 0	0.04 (1)	
D-E	-114 / 0	-102.1	0.08 (1)	I-F	-278 / 0	0.04 (1)	
E-F	-131 / 0	-102.1	0.08 (1)				
F-G	-130 / 0	-102.1	0.08 (1)				
H-G	-213 / 0	0.0	0.08 (1)				
N-M	0 / 117	-38.5	-38.5	0.06 (2)	10.00		
M-L	0 / 109	-38.5	-38.5	0.06 (2)	10.00		
L-K	0 / 105	-38.5	-38.5	0.04 (2)	10.00		
K-J	0 / 105	-38.5	-38.5	0.04 (2)	10.00		
J-I	0 / 109	-38.5	-38.5	0.06 (2)	10.00		
I-H	0 / 117	-38.5	-38.5	0.06 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.09/1.00 (F-G-1), BC=0.08/1.00 (H-I-2), WB=0.04/1.00 (F-I-1), SSI=0.11/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

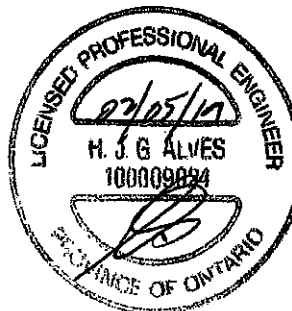
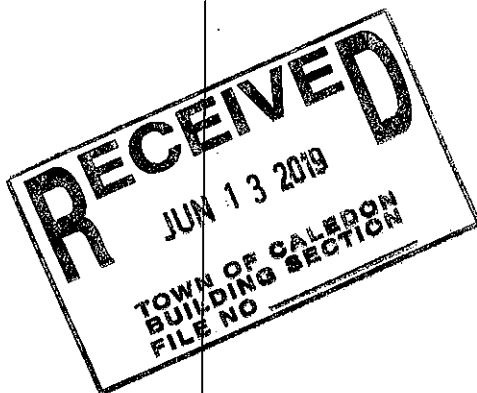
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)

JSI METAL= 0.12 (G) (INPUT = 1.00)

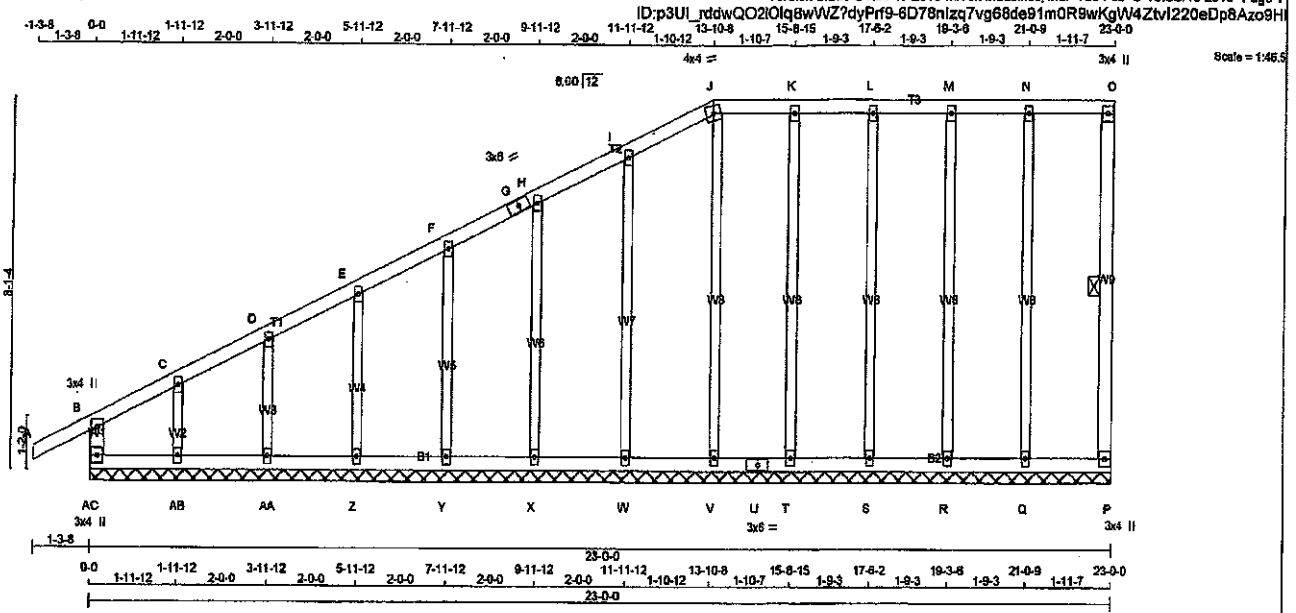


DWG NO. TAM 17902701

STRUCTURAL

COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	G1	3	1	Preston 2	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue Feb 5 15:56:46 2019 Page 1	



TOTAL WEIGHT = 3 X 118 = 348 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
AC - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - O	2x4	DRY	No.2
P - O	2x4	DRY	No.2
AC - U	2x4	DRY	No.2
U - P	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2 SPF			
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

PLATES (table in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C, D, E, F, H, I, K, L, M, N				
C TMW+w	MT20	2.0	4.0	
G TS-J	MT20	3.0	8.0	
J TTW-m	MT20	4.0	4.0	
O TMV+p	MT20	3.0	4.0	
P BMV1+p	MT20	3.0	4.0	
Q, R, S, T, V, W, X, Y, Z, AA, AB				
Q BMW1+w	MT20	2.0	4.0	
U BS-t	MT20	3.0	6.0	
AC BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF O-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO		FROM TO		FR-TO			
AC-B	-250 / 0	0.0 0.0 0.02 (1)	7.81	V-J	-183 / 0	0.24 (1)	
A-B	0 / 31	-102.1 -102.1 0.13 (1)	10.00	Q-N	-201 / 0	0.26 (1)	
B-C	-30 / 0	-102.1 -102.1 0.04 (1)	6.25	T-K	-185 / 0	0.23 (1)	
C-D	-22 / 0	-102.1 -102.1 0.05 (1)	6.25	R-M	-179 / 0	0.23 (1)	
D-E	-17 / 0	-102.1 -102.1 0.05 (1)	6.25	S-L	-179 / 0	0.23 (1)	
E-F	-13 / 0	-102.1 -102.1 0.05 (1)	6.25	AB-C	-186 / 0	0.03 (1)	
F-G	-10 / 0	-102.1 -102.1 0.05 (1)	6.25	AA-D	-203 / 0	0.04 (1)	
G-H	-10 / 0	-102.1 -102.1 0.05 (1)	6.25	Z-E	-202 / 0	0.05 (1)	
H-I	-8 / 0	-102.1 -102.1 0.05 (1)	10.00	Y-F	-203 / 0	0.08 (1)	
I-J	-6 / 0	-102.1 -102.1 0.05 (1)	10.00	X-H	-203 / 0	0.12 (1)	
J-K	-2 / 0	-102.1 -102.1 0.04 (1)	10.00	W-I	-202 / 0	0.18 (1)	
K-L	-2 / 0	-102.1 -102.1 0.04 (1)	10.00				
L-M	-2 / 0	-102.1 -102.1 0.04 (1)	10.00				
M-N	-2 / 0	-102.1 -102.1 0.04 (1)	10.00				
N-O	-2 / 0	-102.1 -102.1 0.04 (1)	10.00				
P-O	-80 / 0	0.0 0.0 0.02 (1)	6.25				
AC-AB	0 / 27	-38.5 -38.5 0.03 (3)	10.00				
AB-AA	0 / 20	-38.5 -38.5 0.03 (3)	10.00				
AA-Z	0 / 15	-38.5 -38.5 0.02 (2)	10.00				
Z-Y	0 / 12	-38.5 -38.5 0.02 (3)	10.00				
Y-X	0 / 9	-38.5 -38.5 0.02 (3)	10.00				
X-W	0 / 7	-38.5 -38.5 0.02 (3)	10.00				
W-V	0 / 5	-38.5 -38.5 0.02 (3)	10.00				
V-U	0 / 2	-38.5 -38.5 0.02 (3)	10.00				
U-T	0 / 2	-38.5 -38.5 0.02 (3)	10.00				
T-S	0 / 2	-38.5 -38.5 0.02 (3)	10.00				
S-R	0 / 2	-38.5 -38.5 0.02 (3)	10.00				
R-Q	0 / 2	-38.5 -38.5 0.02 (3)	10.00				
Q-P	0 / 2	-38.5 -38.5 0.02 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 8.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B-1), BC=0.03/1.00 (AB-AC-3), WB=0.25/1.00 (N-Q-1), SSI=0.10/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

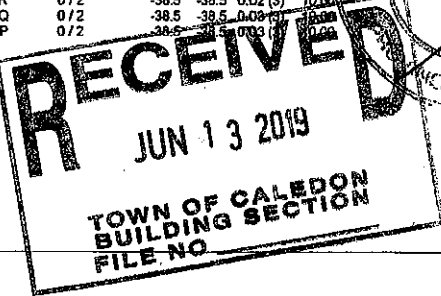
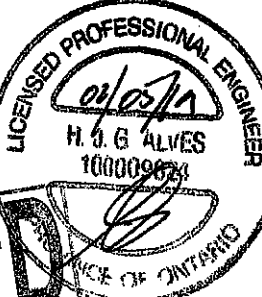
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (J) (INPUT = 0.90)

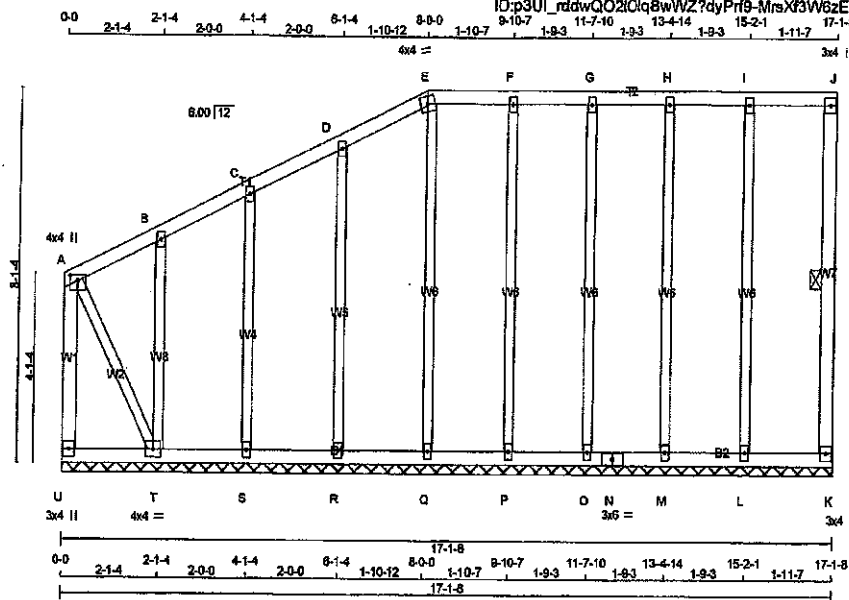
JSI METAL= 0.08 (H) (INPUT = 1.00)



DRWG NO. TAM 1900269

STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	G2	1	1	Preston 2	
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 5 Nov 17 2018 M/Tek Industries, Inc. Tue Feb 5 15:58:54 2019 Page 1	



Scale = 1/4" = 1'-0"

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
U - A	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
U - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.
GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMW+p	MT20	4.0	4.0	1.50	2.00
B, C, D, F, G, H, I					
B - TMW+p	MT20	2.0	4.0		
E - TTW+m	MT20	4.0	4.0		
J - TMW+p	MT20	3.0	4.0		
K - BMV1+p	MT20	3.0	4.0		
L, M, O, P, Q, R, S					
L - BMW1+w	MT20	2.0	4.0		
N - BS-1	MT20	3.0	6.0		
T - BMW1-t	MT20	4.0	4.0		
U - BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH FR-TO (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH FR-TO (LC)	
FR-TO				FR-TO			
U-A	-108/0	0.0	0.0 0.03 (1)	Q-E	-157/0	0.20 (1)	
A-B	-2/0	-102.1	-102.1 0.06 (1)	A-T	0/20	0.00 (1)	
B-C	-9/0	-102.1	-102.1 0.06 (1)	T-B	-230/0	0.09 (1)	
C-D	-3/0	-102.1	-102.1 0.05 (1)	S-C	-195/0	0.12 (1)	
D-E	-11/0	-102.1	-102.1 0.05 (1)	R-D	-225/0	0.20 (1)	
E-F	0/0	-102.1	-102.1 0.05 (1)	L-I	-210/0	0.27 (1)	
F-G	0/0	-102.1	-102.1 0.05 (1)	P-F	-207/0	0.28 (1)	
G-H	0/0	-102.1	-102.1 0.04 (1)	M-H	-177/0	0.22 (1)	
H-I	0/0	-102.1	-102.1 0.04 (1)	O-G	-176/0	0.22 (1)	
I-J	0/0	-102.1	-102.1 0.05 (1)				
K-J	-83/0	0.0	0.0 0.02 (1)				
U-T	0/0	-38.5	-38.5 0.03 (3)				
T-S	0/6	-38.5	-38.5 0.03 (3)				
S-R	0/4	-38.5	-38.5 0.02 (3)				
R-Q	0/2	-38.5	-38.5 0.02 (3)				
Q-P	0/0	-38.5	-38.5 0.02 (3)				
P-O	0/0	-38.5	-38.5 0.02 (3)				
O-N	0/0	-38.5	-38.5 0.02 (3)				
N-M	0/0	-38.5	-38.5 0.02 (3)				
M-L	0/0	-38.5	-38.5 0.03 (3)				
L-K	0/0	-38.5	-38.5 0.03 (3)				

TOTAL WEIGHT = 101 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.06/1.00 (B-C-1), BC=0.03/1.00 (T-U-3), WB=0.27/1.00 (L-1), SB=0.09/1.00 (I-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

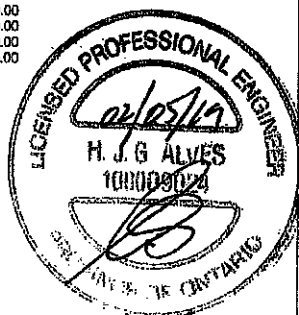
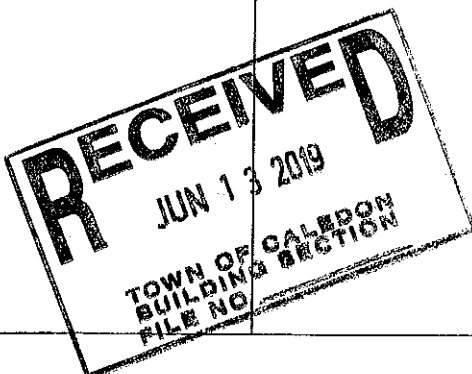
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 786 1907 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.60 (E) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



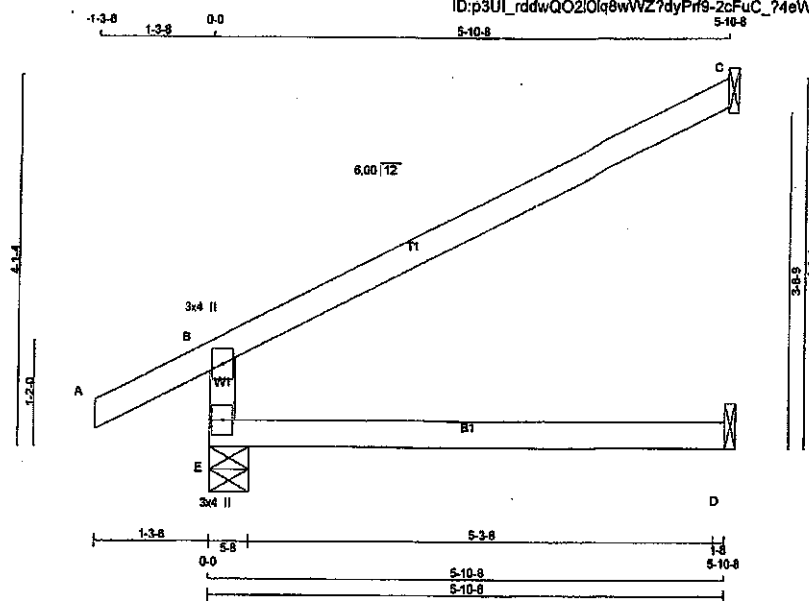
DWG NO. TAM 17902699
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200170-400367	J1	22	1	Preston 2	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 5 Nov 17 2018 Mitek Industries, Inc. Tue Feb 5 15:56:48 2019 Page 1

ID:p3UI_rddwQO2IOlq8wVWZ7dyPrf9-2cFuC_74eVWwqNxoY8B2vELPviuCPNEXLTyiwC2zo9Hj

Scale = 1/23.6



TOTAL WEIGHT = 22 X 17 = 368 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

E - B 2x4 DRY

A - C 2x4 DRY

E - D 2x4 DRY

DRY: SEASONED LUMBER.

LUMBER

No.2

No.2

No.2

DESCR.

SPF

SPF

SPF

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
E	646	646	0	0
C	225	225	0	0
D	93	119	0	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	COMBINED	292/0	72/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	28/0	0/0
D	85	0/0	51/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	WEBS MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO			
E-B	-513/0	0.0	0.0	0.22 (3)	7.81	
A-B	0/31	-102.1	-102.1	0.13 (1)	10.00	
B-C	-34/0	-102.1	-102.1	0.60 (1)	6.25	
E-D	0/0	-38.5	-38.5	0.22 (3)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.20")
CALCULATED VERT. DEFL. (TL) = $L/942$ (0.07")

CS1: TC=0.60/1.00 (B-C-1), BC=0.22/1.00 (D-E-3),
WB=0.00/1.00 (n/a-0), SS=0.28/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

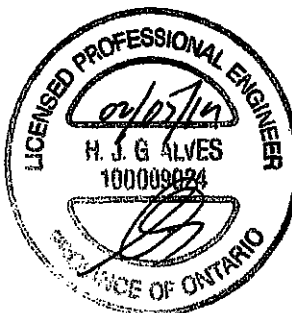
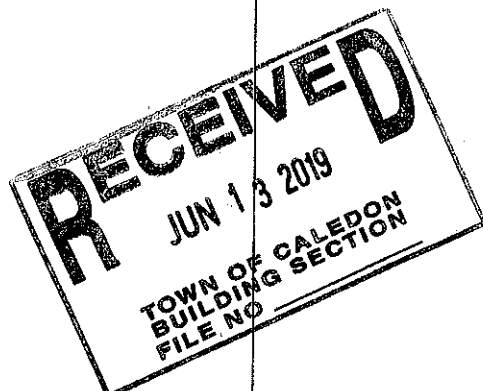
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX	MIN	MAX
MT20	818	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.21 (E) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)



DRWG NO. TAM 11902702
STRUCTURAL
COMPONENT ONLY

JOB NAME 200170-400367	TRUSS NAME J4	QUANTITY 7	PLY 1	JOB DESC. Preston 2	DRWG NO. 5-5-8
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Tamarack Roof Truss, Burlington
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Feb 15 15:56:50 2019 Page 1

ID: p3UI_rddwQO2IOlq8wVZ7dyPrf9- Mfdg0KA7AYcFyxGc5NKmUI2IKr81exFBCHxzo9Hh
 5-5-8

Scale = 1:13.6

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY

B - D 2x4 DRY

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
C	250	0	250	0	1-8	1-8
B	520	0	520	0	3-8	3-8
D	133	0	145	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	380	238 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		PR-TO			
A-B	0 / 20	-102.1 -102.1	0.13 (1)	E-F	-254 / 122	0.00 (1)	
B-F	-51 / 0	-102.1 -102.1	0.13 (3)				
F-C	0 / 6	-102.1 -102.1	0.42 (1)				
B-E	0 / 0	-38.5 -38.5	0.28 (1)				
E-D	0 / 0	-38.5 -38.5	0.31 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = L/842 (0.08")

ALLOWABLE DEFL.(TL) = L/360 (0.19")

CALCULATED VERT. DEFL.(TL) = L/489 (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	816 354	1687 1897

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)

JSI METAL= 0.09 (B) (INPUT = 1.00)

RECEIVED

JUN 13 2019

TOWN OF CALEDON

BUILDING SECTION

FILE NO

LICENSED PROFESSIONAL ENGINEER

02/05/19

H. J. G. ALVES

100009824

PROFESSIONAL ENGINEER

PROVINCE OF ONTARIO

DWG NO. TAM 17902704

STRUCTURAL

COMPONENT ONLY



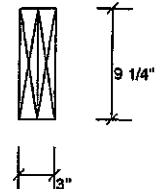
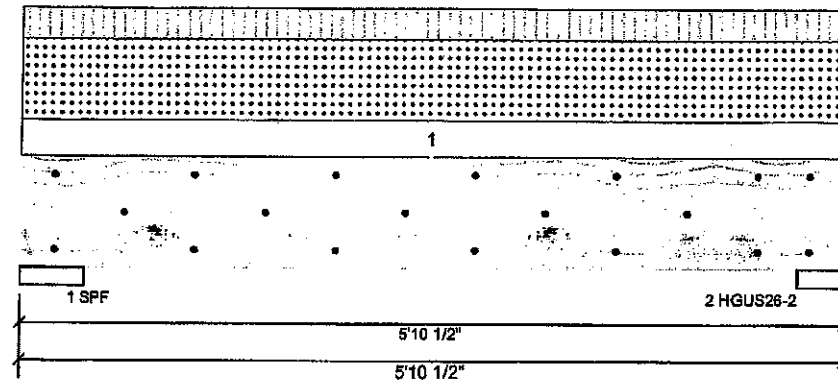
isDesign™

Client: Greenpark Homes
 Project: Preston 2
 Address: Caledon

Date: 2019-02-05
 Designer: Brian
 Job Name: Lamberts Lane Homes Corp.
 Project #: 200170

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
 Plies: 2
 Moisture Condition: Dry
 Deflection LL: 360
 Deflection TL: 360
 Importance: Normal

Application: Floor (Residential)
 Design Method: LSD
 Building Code: NBCC 2015 / OBC 2012
 Load Sharing: No
 Deck: Not Checked
 Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	231	273	609	0
2	221	262	584	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	15%	341 / 1145	1486 L
2 - HGUS...	4.000"	19%	327 / 1097	1424 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1679 ft-lb	3'	6039 ft-lb	0.278 (28%)	1.25D+1.5S	L
Unbraced	1679 ft-lb	3'	5236 ft-lb	0.321 (32%)	1.25D+1.5S	L
Shear	1290 lb	1'2"	3984 lb	0.324 (32%)	1.25D+1.5S	L
Perm Defl in.	0.006 (L/11306)	3'	0.174 (L/360)	0.030 (3%)	D	Uniform
LL Defl inch	0.015 (L/4260)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3084)	3'	0.174 (L/360)	0.120 (12%)	D+S+0.5L	L

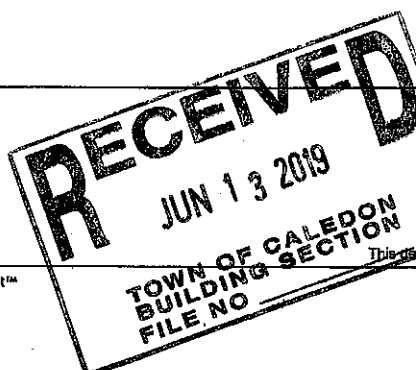
Design Notes

- 1 Fasten all plies using 3 rows of 10d Box nails (.126x3") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17902705
 STRUCTURAL
 COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		7-0-0	Near Face	13 PSF	11 PSF	29 PSF	0 PSF	



Manufacturer Info

TAMARACK LUMBER
 3255 NORTH SERVICE RD., ON
 CANADA
 (905) 335-1115





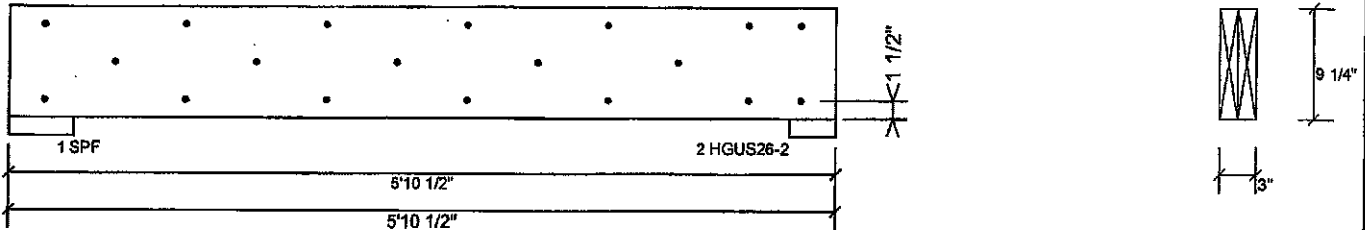
isDesign™

Client: Greenpark Homes
Project: Preston 2
Address: Caledon

Date: 2019-02-05
Designer: Brian
Job Name: Lamberts Lane Homes Corp.
Project #: 200170

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

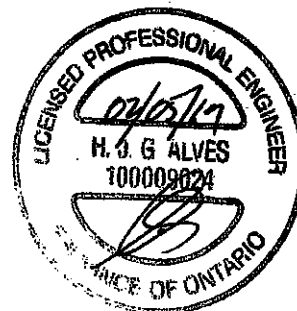


Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.6 %
Load	247.6 PLF
Yield Limit per Foot	383.4 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



DWG NO. TAM 17902705
STRUCTURAL
COMPONENT ONLY
2/2

Manufacturer Info	TAMARACK LUMBER 3256 NORTH SERVICE RD., ON CANADA (905) 335-1115
	TAMARACK LUMBER INC. ALMA LUMBER GROUP

This design is valid until 2021-12-11



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



Double-Shear
Nailing
Top View



Double-Shear
Nailing
Side View;
Do not
bend tab



Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580



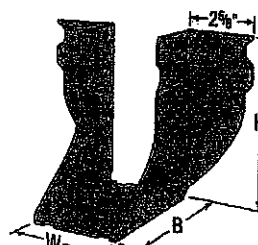
✓ LUS26



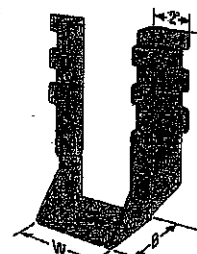
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



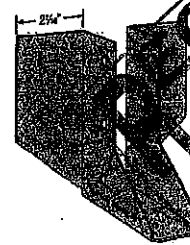
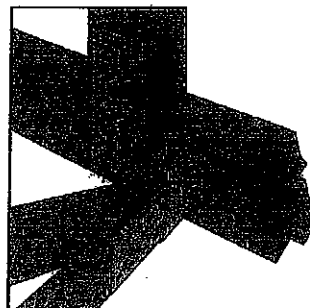
✓ HGUS28-2



✓ HHUS210-2

Plated Truss Connectors

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



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LUL/LUS/LJS/HUS/HHUS/HGUS**SIMPSON**
Strong-Tie**HHUS/HGUS**

See Hanger Options information on pp. 125-127.

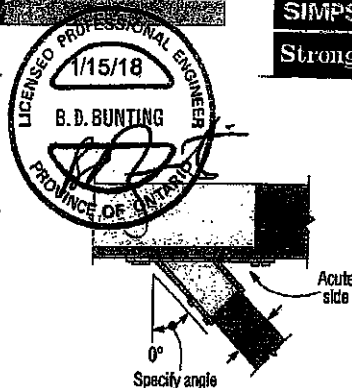
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.82 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.87 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

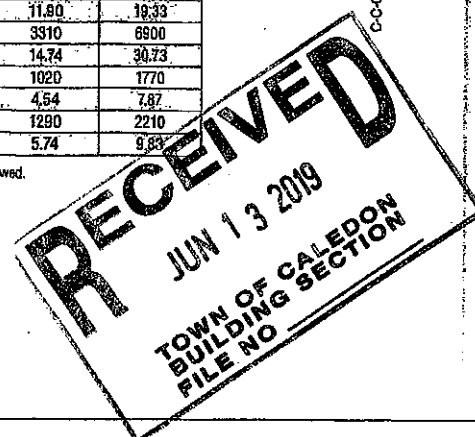
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _h	Header	Joist		S-P-F			
									Uplift	Normal	Uplift	Normal
									(K _g = 1.4)	(K _g = 1.0)	(K _g = 1.4)	(K _g = 1.0)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Single 2x Sizes												
LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d		710	1625	645	1155
LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"		360	1020	320	725
LU28L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"		720	1805	645	1140
LUS26	18	1 1/8	4 1/4	1 1/4	3 3/4	(4) 10d	(4) 10d		1420	2170	1290	1630
HUS26	16	1 1/8	6 1/4	3	3 1/4	(14) 16d	(6) 16d		2705	4940	2065	3875
LUS26DS	18	1 1/8	5	3 1/4	4 1/4	(16) 16d	(6) 16d		2055	4265	1460	4115
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(28) 16d	(8) 16d		2885	6625	2685	5700
LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"		1140	2185	1020	1560
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d		1420	2520	1290	1790
HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d		3605	5365	2675	4345
HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d		3310	7675	3310	6900
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"		1140	2495	1020	1770
LUS210	18	1 1/8	7 1/4	1 1/4	3 3/4	(8) 10d	(4) 10d		1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_h is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27-28 for other nail sizes and information.

C-C-CON2018 ©2017 SIMPSON STRONG-TIE COMPANY INC.



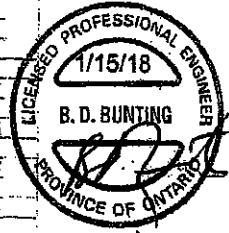
Face-Mount Hangers

SIMPSON
Strong-Tie

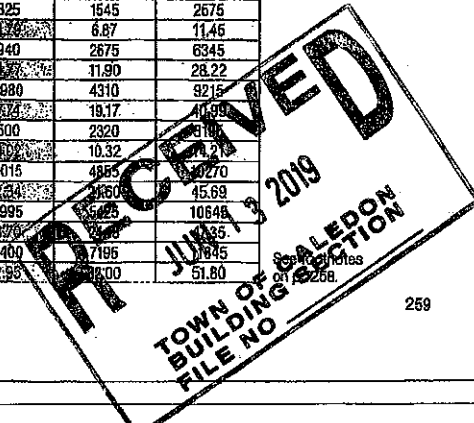
These products are available with additional corrosion protection. For more information, see p. 24.

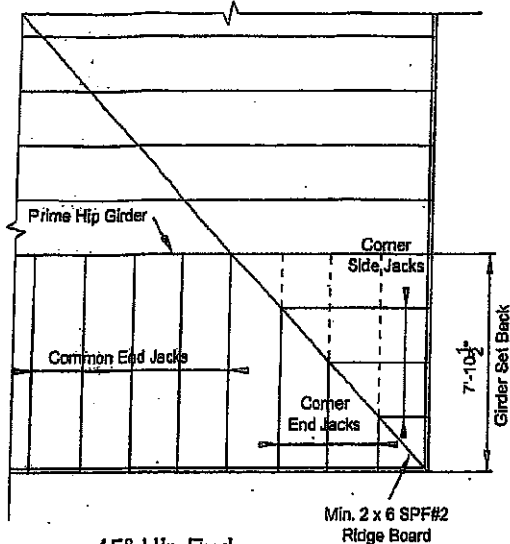
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _p	Header	Joist		S-P-F		Uplift (K ₂ = 1.15) lb.	Normal (K ₂ = 1.00) lb.
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Double 2x Sizes												
■ LUS24-2	18	3 1/8	6 1/8	2	1 1/4	(4) 16d	(2) 16d		835	2020	590	1435
■ LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
■ HHUS26-2	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d		2850	7335	2065	5205
■ HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
■ LUS28-2	18	3 1/8	7	2	4	(8) 16d	(4) 16d		1720	3325	1545	2675
■ HHUS28-2	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d		3785	8940	2675	6345
■ HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(30) 16d	(12) 16d		6070	12980	4310	9215
■ LUS210-2	18	3 1/8	9	2	6	(8) 16d	(8) 16d		2580	4500	2320	3195
■ HHUS210-2	14	3 1/8	9 1/8	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
■ HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		8840	14015	4855	10270
Triple 2x Sizes												
■ HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
■ HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
■ HHUS210-3	14	4 1/8	9	3	7 1/8	(30) 16d	(10) 16d		4670	9670	4235	8865
■ HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		8840	14845	4855	10400
Quadruple 2x Sizes												
■ HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
■ HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
■ HHUS210-4	14	6 1/8	9	3	7 1/8	(30) 16d	(10) 16d		4670	10155	4235	7210
■ HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(48) 16d	(16) 16d		8840	14845	4855	10400
■ HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10845
■ HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(68) 16d	(22) 16d		10130	16400	7195	11845
4x Sizes												
■ LUS46	18	3 1/8	4 1/8	2	3 1/8	(4) 16d	(4) 16d		1720	2595	1545	1920
■ HHUS46	14	3 1/8	5 1/8	3	3 1/8	(14) 16d	(8) 16d		2540	7335	2065	5205
■ HGUS46	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3110	6355
■ LUS48	18	3 1/8	6 1/8	2	3 1/8	(8) 16d	(4) 16d		1720	3325	1545	2675
■ HHUS48	14	3 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d		3785	8940	2675	6345
■ HGUS48	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
■ LUS410	18	3 1/8	8 1/8	2	5 1/8	(8) 16d	(6) 16d		2580	4500	2320	3195
■ HGUS410	12	3 1/8	9	4	8 1/8	(48) 16d	(16) 16d		8840	14015	4855	10270
■ HGUS412	12	3 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10845
■ HGUS414	12	3 1/8	12 1/8	4	11 1/8	(68) 16d	(22) 16d		10130	16400	7195	11845



Plated Truss Connectors





45° Hip End

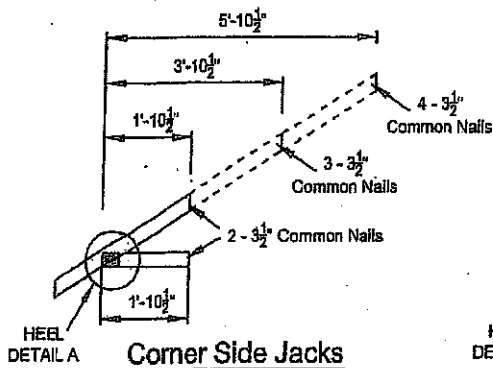
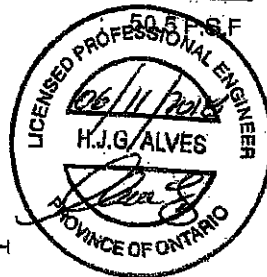
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

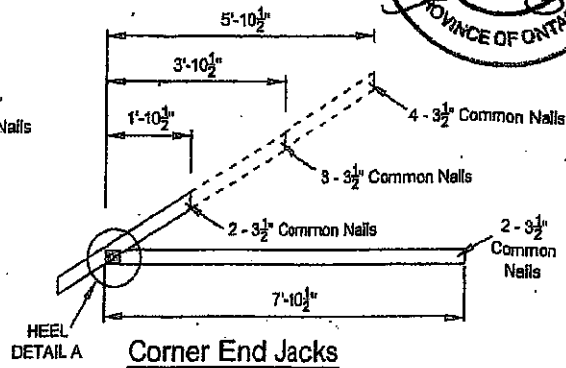
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

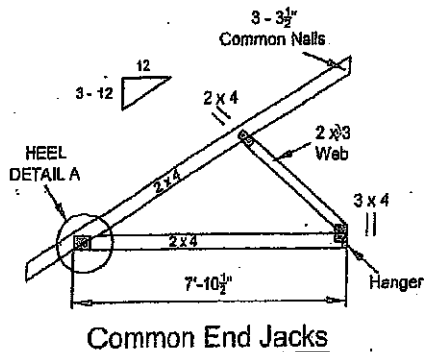
TOTAL LOAD



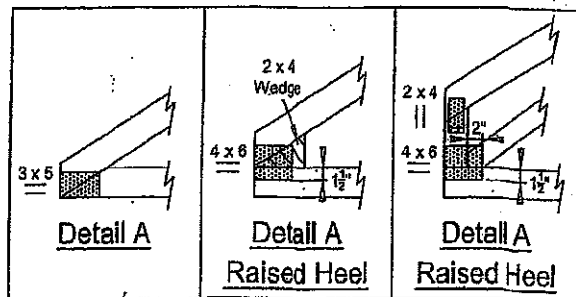
Corner Side Jacks



Corner End Jacks

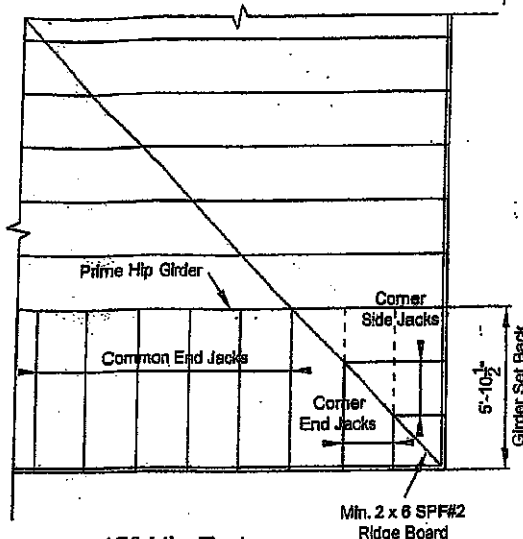


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



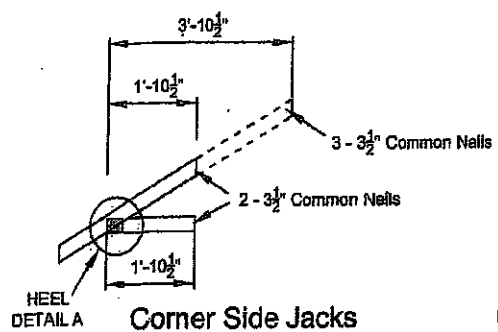
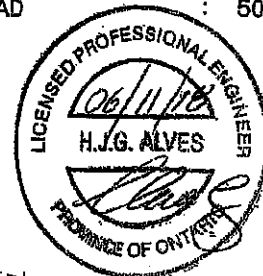
45° Hip End

LUMBER SPECIFICATION

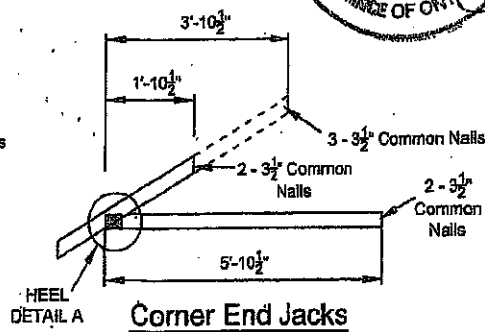
TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

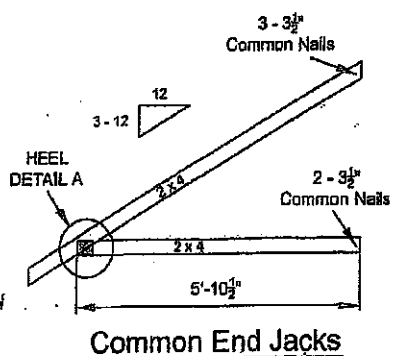
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F



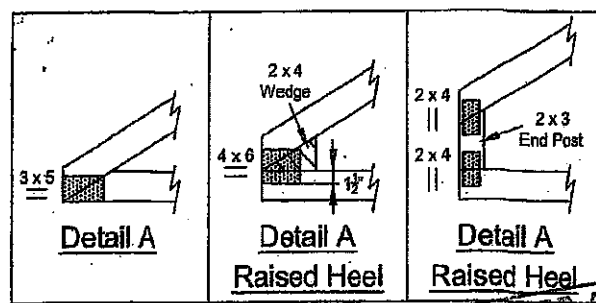
Corner Side Jacks



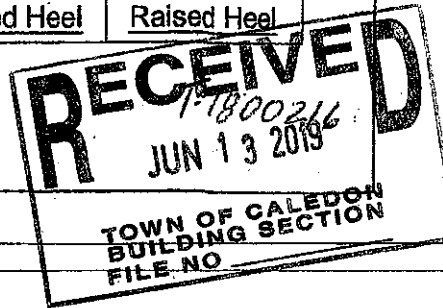
Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

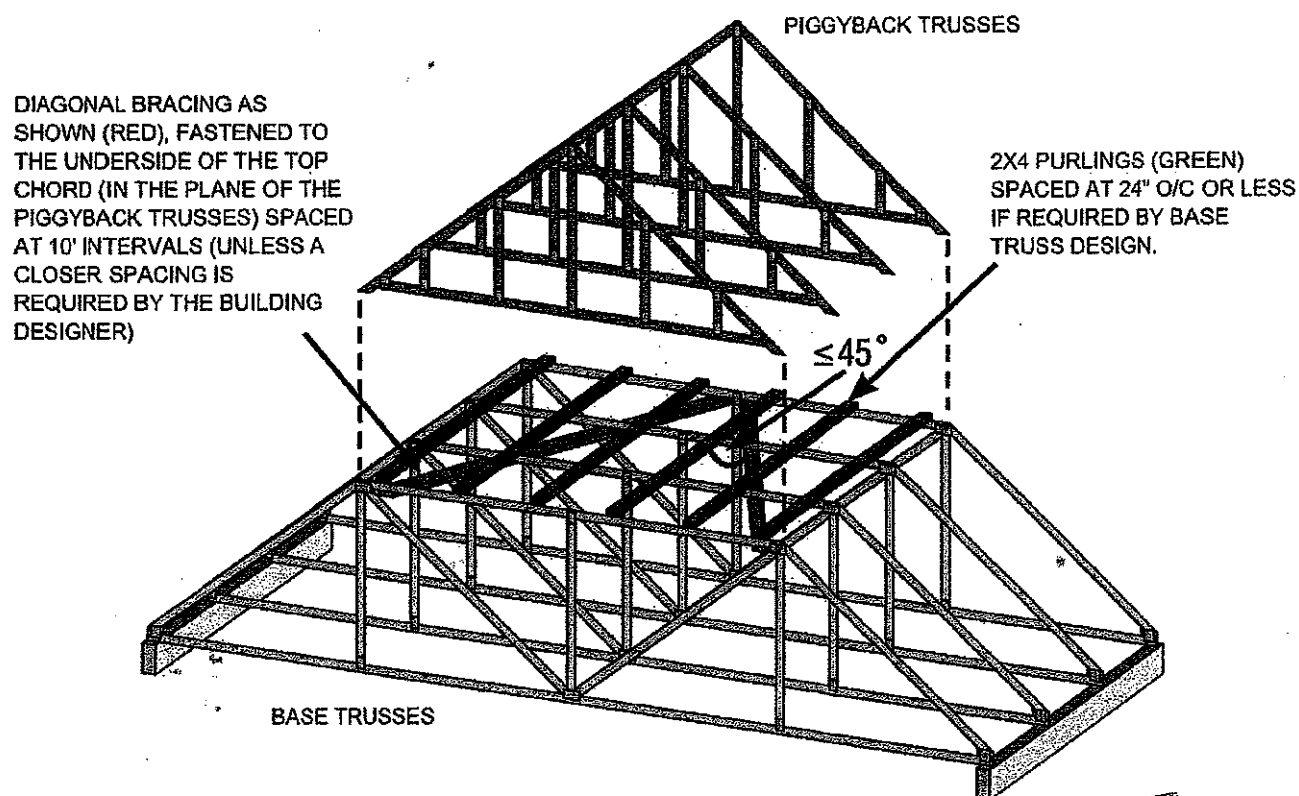


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

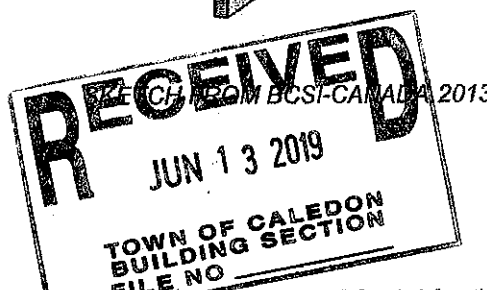
Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

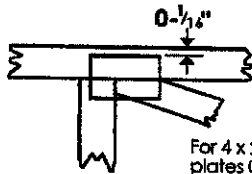


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in 1/16-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

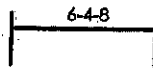


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

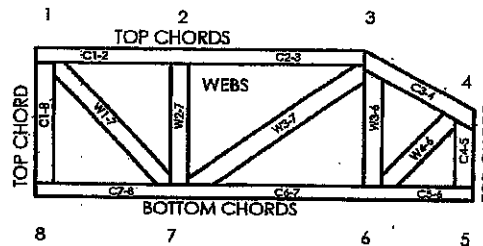
Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate-Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in 1/16-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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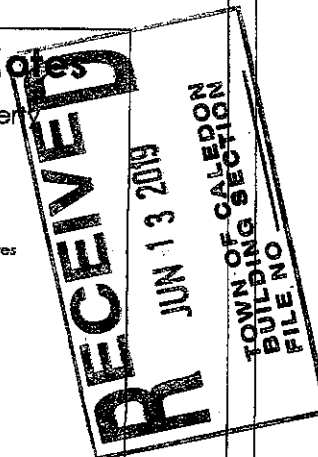
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: Mil-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Proper Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
- Design assumes trusses will be suitably protected from the environment in accord with TPIC.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with TPIC Quality Criteria.





Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet M117473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

