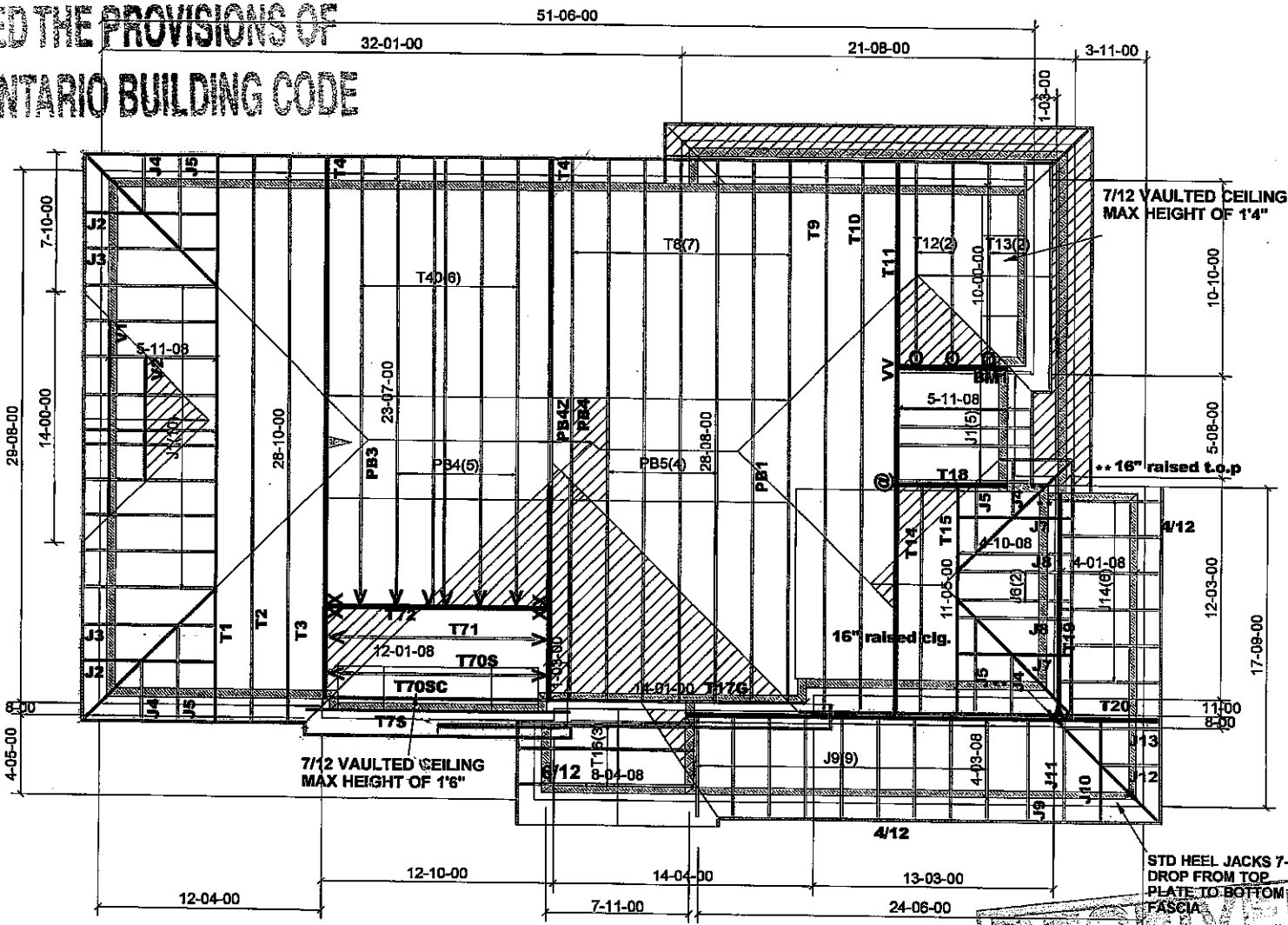
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE



TRUSS PROFILES TO BE VERIFIED BY BUILDING DESIGNER

ALL CONVENTIONAL ROOF FRAMING TO CONFORM WITH PART 9 OF THE OBC.
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"X4"SPF@24"O.C. WITH 2"X4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE LATERALLY BRACED SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED CONFORM WITH:
ONTARIO BUILDING CODE (2012)
OCCUPANCY: **RESIDENTIAL | PART: 9**

DESIGN LOADS:
CITY: CALEDON
G.S.L.= 37.6 psf
TC DL.= 6 psf
BC LL.= 10.50 psf
BC DL.= 7.00 psf

NOTES:

FIN. OH.:	12"
HEEL TYPE:	R.T.M. CANT.
EXT. WALLS:	2X6
CLAD. TYPE 1:	BRICK/5"
CLAD. TYPE 2:	SIDING/0"
FSC SIZE:	2X6
SHEATHING:	ASPHALT SHINGLE

IF DESIGNED COMMERCIAL, REFERE TO SEALED TRUSS DOCs FOR UPLIFT DESIGN

HARDWARE:
LJS26DS (V) 6pcs
HGUS26-2 (XX) 3pcs
HUC26-2 (@) 1pcs
LUS24 (O) 7pcs

1'4" RAISED WITH CEILING/TOP PLATE
CONV FRM BY OTHERS

T- 180737

COMMENTS:
BM1: 2-2"X10" SPF #2
9/12 PITCH UNLESS NOTED OTHERWISE

APPLICANT COPY

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

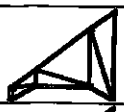
TAMARACK	Job Track: **50033**	Builder / Location: **GREEN PARK HOMES / CALEDON**	Model / Elevation: **PRESTON 11 / 1**
	Plan Log: **200664**	Project: **LAMBERT LANE PH.2**	
	Layout ID: **401820**	Date: **2019-03-20** Designer: **Brian/JG**	

Mitek ver 8.2.3.229

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	Greenpark	PlanLog:	200172
Project:	Lamberts Lane Home Corp.	Layout ID:	400371
Location:	Caledon	Ref #	
Model:	Preston 11	Page:	1 of 4
Lot #:		Date:	03/12/2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	T1 Hip Girder	9 /12	28-10-00	5-11-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	162.93 101.00		
	1	T2 Hip	9 /12	28-10-00	7-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	126.61 79.17		
	1	T3 Hip	9 /12	28-10-00	8-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	140.45 88.50		
	2 2-ply	T4 Hip Girder	9 /12	28-10-00	10-05-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	700.14 428.67		
	6	T40 Piggyback Base	9 /12	23-07-00	10-05-14	2 x 4	1-03-08	1-06-04 5-05-08	754.34 467.00		
	1	T7S Roof Special	9 /12 7 /12	12-00-00	6-00-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	55.34 37.00		
	1	T70S Jack-Closed	9 /12 7 /12	12-01-08	10-06-04	2 x 4		1-05-02 10-06-04	66.36 42.67		
	1	T70SC Monopitch	9 /12 7 /12	12-01-08	10-06-04	2 x 4		1-05-02 10-06-04	66.36 42.67		
	1	T71 Jack-Closed	9 /12	12-01-08	10-06-04	2 x 4		1-05-02 10-06-04	59.19 38.67		
	1 2-ply	T72 Jack-Closed Girder	9 /12	12-01-08	10-06-04	2 x 4 2 x 6		1-05-02 10-06-04	153.15 96.33		
	7	T8 Hip	9 /12	28-08-00	10-05-14	2 x 4	1-03-08	1-06-04 2-03-08	961.33 592.67		
	1	T9 Hip	9 /12	27-11-00	8-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	142.42 89.83		
	1	T10 Hip	9 /12	27-11-00	7-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	141.37 88.67		
	1 2-ply	T11 Hip Girder	9 /12	27-11-00	5-11-14	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	301.42 188.67		

DELIVERY SHIPLIST

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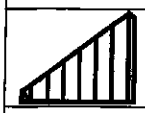
Location:	Caledon	Ref #	
Model:	Preston 11	Page:	2 of 4
Lot #:		Date:	03/12/2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

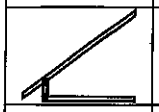
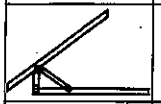
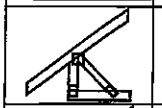
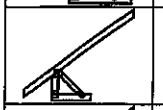
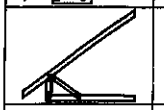
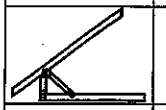
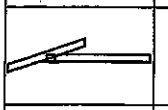
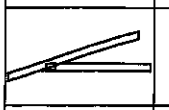
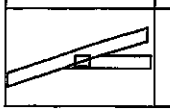
Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	2	T12 Common	9 /12	10-00-00	5-03-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	87.54 57.67		
	2	T13 Roof Special	9 /12 7 /12	10-00-00	5-03-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	96.56 64.67		
	1	T14 Common	9 /12	11-05-00	5-09-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	48.96 31.67		
	1	T15 Hip Girder	9 /12	11-05-00	5-02-02	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	54.5 35.17		
	3	T16 Common	6 /12	8-04-08	3-00-13	2 x 4	1-03-08	6-12 6-12	75.6 50.50		
	1	T17G GABLE	9 /12	14-01-00	11-04-12	2 x 4		1-06-04 10-08-08	90.12 57.67		
	1 2-ply	T18 Roof Special Girder	0 /12	5-11-08	1-04-00	2 x 4		1-04-00 1-04-00	40.56 25.67		
	1 2-ply	T19 Flat Girder	0 /12	12-04-08	1-08-07	2 x 4		1-08-07 1-08-07	84.52 57.00		
	1 2-ply	T20D Half Hip Girder	4 /12	24-09-12	2-02-00	2 x 6	1-03-08	3-15 1-09-02	231.5 136.00		
	1	V1 Valley	9 /12	10-09-09	4-00-09	2 x 4			29.22 18.67		
	1	V2 Valley	9 /12	6-09-09	2-06-09	2 x 4			17.74 11.00		
	1	PB1 Piggyback	9 /12	5-09-05	1-06-00	2 x 4			14.05 9.67		
	1	PB3 Piggyback	9 /12	4-11-00	1-06-00	2 x 4			12.09 8.33		
	6	PB4 Piggyback	9 /12	4-11-00	1-10-02	2 x 4			66.37 48.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	Greenpark	PlanLog:	200172
Project:	Lamberts Lane Home Corp.	Layout ID:	400371
Location:	Caledon	Ref #	
Model:	Preston 11	Page:	3 of 4
Lot #:		Date:	03/12/2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	PB4Z Piggyback	9 /12	4-11-00	1-10-02	2 x 4			22.12 16.00		
	4	PB5 Piggyback	9 /12	5-09-05	2-02-00	2 x 4			53.09 34.67		
	15	J1 Jack-Open	9 /12	5-11-08	5-11-14	2 x 4	1-03-08	1-06-04 5-11-14	279.2 180.00		
	2	J2 Jack-Open Girder	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 4-02-01	1-06-04 2-10-05	28.62 20.33		
	2	J3 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 2-02-01	1-06-04 4-04-05	34.33 23.00		
	4	J4 Jack-Open	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	37.25 28.00		
	4	J5 Jack-Open	9 /12	1-10-08	4-04-05	2 x 4	1-03-08 1-10-15	1-06-04 2-11-02	48.89 33.33		
	2	J6 Jack-Open	9 /12	4-10-08	5-02-02	2 x 4	1-03-08	1-06-04 5-02-02	34.96 23.00		
	2	J7 Jack-Open Girder	9 /12	1-09-07	2-10-05	2 x 4	1-03-08 3-01-01	1-06-04 2-10-05	25.47 18.00		
	2	J8 Jack-Open	9 /12	3-09-07	4-04-05	2 x 4	1-03-08 1-01-01	1-06-04 4-04-05	31.4 20.67		
	10	J9 Jack-Open	4 /12	4-03-08	2-02-00	2 x 4	1-03-08	3-15 1-09-02	117.16 73.33		
	1	J10 Jack-Open Girder	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 2-06-01	3-15 11-01	8.7 6.00		
	1	J11 Jack-Open	4 /12	3-09-07	2-00-00	2 x 4	1-03-08 6-01	3-15 1-07-01	11.11 7.33		
	1	J12 Jack-Open	4 /12	1-09-07	1-04-00	2 x 4	1-03-08 1-01	3-15 11-01	5.95 4.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50120
Builder:	Greenpark	PlanLog:	200172
Project:	Lamberts Lane Home Corp.	Layout ID:	400371
Location:	Caledon	Ref #	
Model:	Preston 11	Page:	4 of 4
Lot #:		Date:	03/12/2019
Elevation:	1	Designer:	Brian Faneca
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	J13 Jack-Open	4 /12	1-10-08	2-00-00	2 x 4	1-03-08 1-10-15	3-15 11-07	8.35 5.33		
	6	J14 Jack-Open	4 /12	4-01-08	2-01-06	2 x 4	1-03-08	3-15 1-08-07	67.95 44.00		

TOTAL # TRUSS= **115** TOTAL BFT OF ALL TRUSSES= **3528.2** BFT. TOTAL WEIGHT OF ALL TRSSES **5595.3** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	
3	Hardware	HGUS26-2	
1		HUC26-2	
3	Hardware	LUS24	
2	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= **18**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:57 2019 Page 1
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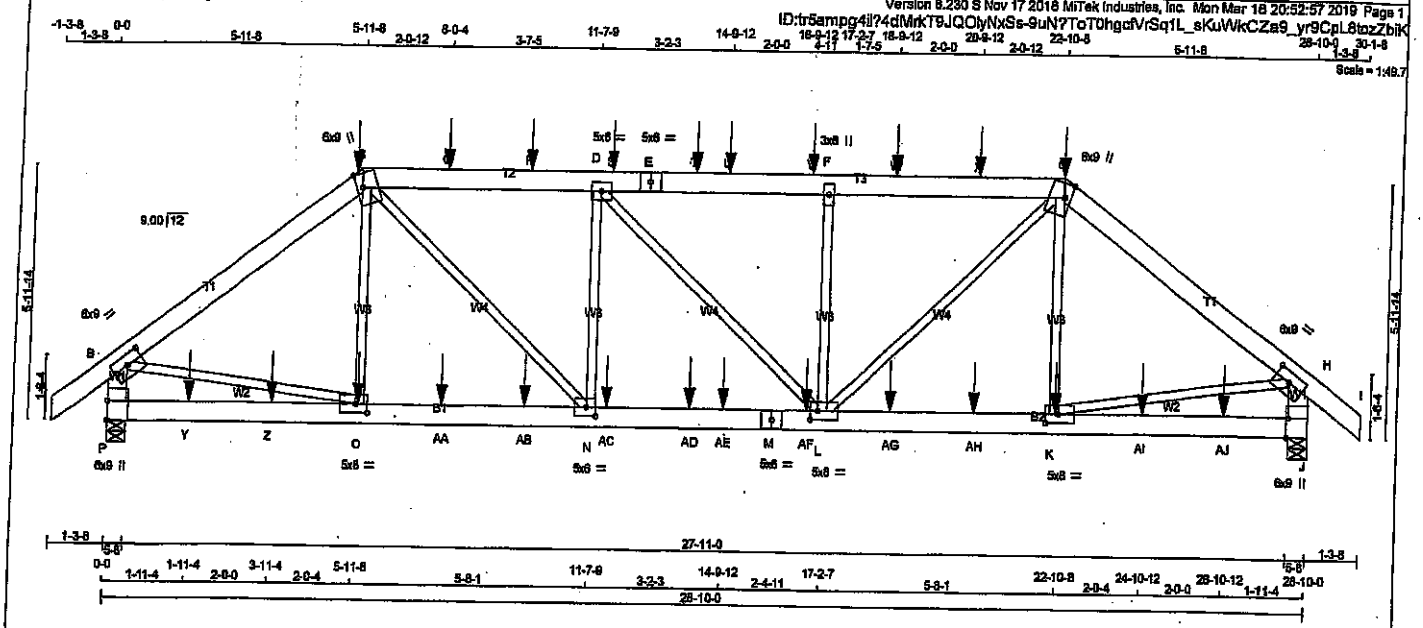


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
G - I	2x6	DRY	No.2	SPF
P - B	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
M - J	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-t	MT20	6.0	9.0	2.75	4.50
C	TTWV+m	MT20	6.0	9.0	4.00	1.75
D	TMWV-t	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMV+w	MT20	3.0	6.0		
G	TTWV+m	MT20	6.0	9.0	4.00	1.75
H	TMWV-t	MT20	6.0	9.0	2.75	4.50
J	BMV1+t	MT20	6.0	9.0	Edge	0.50
K	BMWW-t	MT20	5.0	8.0	2.50	3.50
L	BMWWW-t	MT20	5.0	8.0	2.50	2.25
M	BS-t	MT20	5.0	6.0		
N	BMWV-t	MT20	5.0	6.0	2.50	2.75
O	BMWV-t	MT20	5.0	8.0	2.50	3.50
P	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
P	3425	0	3425	0	0	5-8	5-8
J	3425	0	3425	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	2524	1466 / 0	366 / 0	0 / 0	0 / 0	672 / 0	0 / 0
J	2524	1466 / 0	366 / 0	0 / 0	0 / 0	672 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

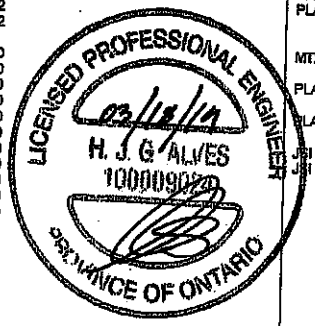
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 44	-102.1	-102.1	0.08 (1)	10.00	
B-C	-3830 / 0	-102.1	-102.1	0.48 (1)	3.92	
C-Q	-4393 / 0	-102.1	-102.1	0.51 (1)	3.61	
Q-R	-4393 / 0	-102.1	-102.1	0.51 (1)	3.61	
R-D	-4393 / 0	-102.1	-102.1	0.51 (1)	3.61	
D-S	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
S-E	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
E-T	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
T-U	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
U-V	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
V-F	-4383 / 0	-102.1	-102.1	0.51 (1)	3.62	
F-W	-4383 / 0	-102.1	-102.1	0.49 (1)	3.65	
W-X	-4383 / 0	-102.1	-102.1	0.49 (1)	3.65	
X-G	-4383 / 0	-102.1	-102.1	0.49 (1)	3.65	
G-H	-3831 / 0	-102.1	-102.1	0.48 (1)	3.92	
H-I	0 / 44	-102.1	-102.1	0.08 (1)	10.00	
P-B	-3264 / 0	0.0	0.0	0.23 (1)	5.62	
J-H	-3265 / 0	0.0	0.0	0.23 (1)	5.62	
P-Y	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
Y-Z	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
Z-O	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
O-AA	0 / 3055	-38.5	-38.5	0.55 (1)	10.00	
AA-AB	0 / 3055	-38.5	-38.5	0.55 (1)	10.00	
AB-N	0 / 3055	-38.5	-38.5	0.55 (1)	10.00	
N-AC	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
AC-AD	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
AD-AE	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
AE-M	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
M-AF	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
AF-L	0 / 4393	-38.5	-38.5	0.69 (1)	10.00	
L-AG	0 / 3056	-38.5	-38.5	0.55 (1)	10.00	
AG-AH	0 / 3056	-38.5	-38.5	0.55 (1)	10.00	
AH-K	0 / 3056	-38.5	-38.5	0.55 (1)	10.00	
K-AI	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
AI-AJ	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
AJ-J	0 / 0	-38.5	-38.5	0.27 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
O-C	-99 / 297	0.09 (3)
C-N	0 / 1864	0.46 (1)
N-D	-1021 / 0	0.51 (1)
D-L	-14 / 0	0.02 (1)
L-F	-1017 / 0	0.51 (1)
L-G	0 / 1849	0.46 (1)
K-G	-92 / 268	0.09 (3)
B-O	0 / 3099	0.77 (1)
K-H	0 / 3100	0.77 (1)



TOTAL WEIGHT = 182 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF SBCC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.14")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.25")

CSI: TC=0.51/1.00 (C-D:1) , BC=0.69/1.00 (L-N:1) , WB=0.77/1.00 (H-K:1) , SSI=0.36/1.00 (D-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

(PSI GRIP= 0.90 (K) (INPUT = 0.90)
(PSI METAL= 0.83 (M) (INPUT = 1.00)

DWG NO. TAM T1905520
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-11-8	-463	-463	—	BACK	VERT	TOTAL	—	—
G	22-10-8	-463	-463	—	BACK	VERT	TOTAL	—	—
K	22-8-12	-26	-26	—	BACK	VERT	TOTAL	—	—
O	8-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—
Q	8-0-4	-126	-126	—	BACK	VERT	TOTAL	—	—
R	10-0-4	-126	-126	—	BACK	VERT	TOTAL	—	—
S	12-0-4	-126	-126	—	BACK	VERT	TOTAL	—	—
T	14-0-4	-126	-126	—	BACK	VERT	TOTAL	—	—
U	14-9-12	-126	-126	—	BACK	VERT	TOTAL	—	—
V	16-9-12	-126	-126	—	BACK	VERT	TOTAL	—	—
W	18-9-12	-126	-126	—	BACK	VERT	TOTAL	—	—
X	20-9-12	-126	-126	—	BACK	VERT	TOTAL	—	—
Y	1-11-4	-76	-97	—	BACK	VERT	TOTAL	—	—
Z	3-11-4	-76	-97	—	BACK	VERT	TOTAL	—	—
AA	8-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—
AB	10-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—
AC	12-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—
AD	14-0-4	-26	-26	—	BACK	VERT	TOTAL	—	—
AE	14-9-12	-26	-26	—	BACK	VERT	TOTAL	—	—
AF	18-9-12	-26	-26	—	BACK	VERT	TOTAL	—	—
AG	18-9-12	-26	-26	—	BACK	VERT	TOTAL	—	—
AH	20-9-12	-26	-26	—	BACK	VERT	TOTAL	—	—
AI	24-10-12	-76	-97	—	BACK	VERT	TOTAL	—	—
AJ	26-10-12	-76	-97	—	BACK	VERT	TOTAL	—	—

DWG NO. TAM 1905520
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:58 2019 Page 1

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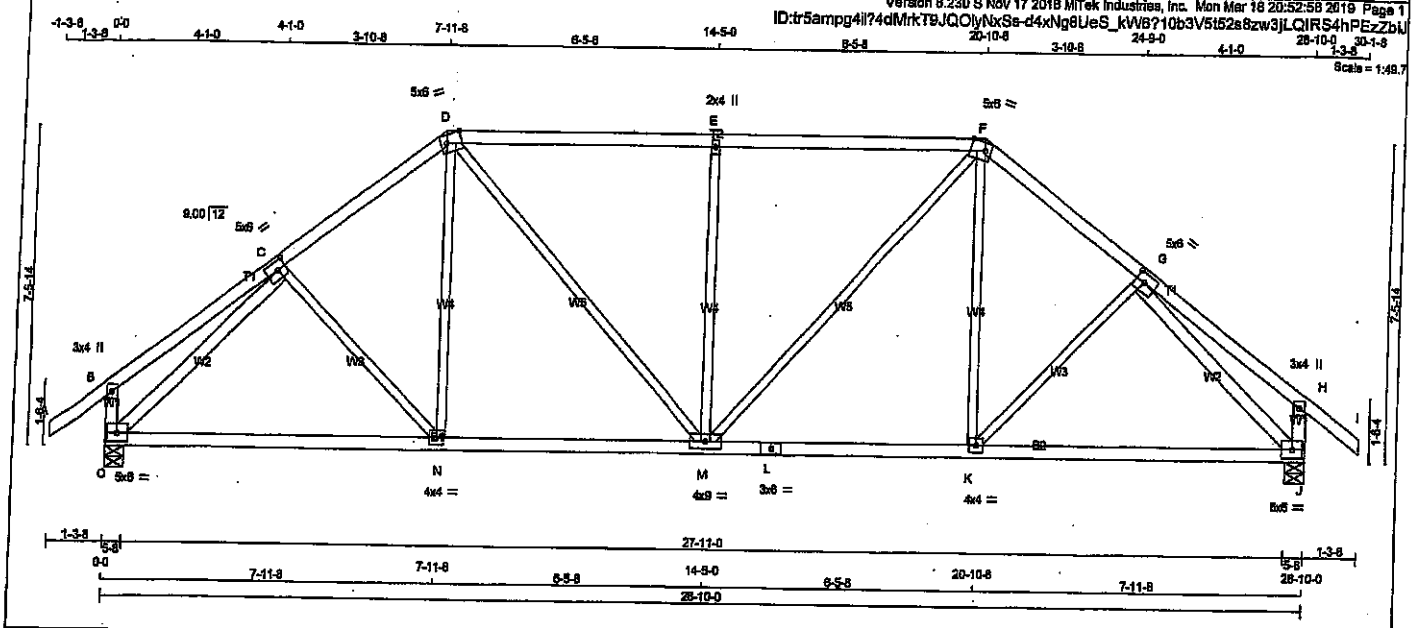


TOTAL WEIGHT = 130 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
O - C	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.50
D	TTWW-m	MT20	5.0	6.0	Edge	4.25
E	TMV+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	4.25
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMWWW-t	MT20	4.0	9.0		
N	BMWW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
O	2168	0	2168	0	0	5-8	5-8
J	2168	0	2168	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1611	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0
J	1611	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.93 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

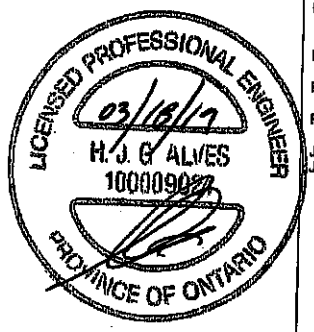
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	0 / 27	-102.1	-102.1	0.25 (1)	10.00	
C-D	-2103 / 0	-102.1	-102.1	0.25 (1)	4.47	
D-E	-2110 / 0	-102.1	-102.1	0.63 (1)	3.93	
E-F	-2110 / 0	-102.1	-102.1	0.63 (1)	3.93	
F-G	-2103 / 0	-102.1	-102.1	0.25 (1)	4.47	
G-H	0 / 27	-102.1	-102.1	0.25 (1)	10.00	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
O-B	-297 / 0	0.0	0.0	0.03 (1)	7.81	
J-H	-297 / 0	0.0	0.0	0.03 (1)	7.81	
O-N	0 / 1659	-38.5	-38.5	0.64 (2)	10.00	
N-M	0 / 1662	-38.5	-38.5	0.65 (2)	10.00	
M-L	0 / 1662	-38.5	-38.5	0.65 (2)	10.00	
L-K	0 / 1662	-38.5	-38.5	0.65 (2)	10.00	
K-J	0 / 1659	-38.5	-38.5	0.64 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-N	-3 / 112	0.03 (3)
N-D	0 / 387	0.09 (2)
D-M	0 / 671	0.15 (1)
M-E	-807 / 0	0.82 (1)
M-F	0 / 671	0.15 (1)
K-F	0 / 387	0.09 (2)
K-G	-3 / 112	0.03 (3)
O-C	-2408 / 0	0.94 (1)
G-J	-2408 / 0	0.94 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.17")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.65/1.00 (M-N:2) , WB=0.94/1.00 (C-O:1) , SSI=0.32/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
JSI METAL= 0.59 (G) (INPUT = 1.00)

DWG NO. TAM T1905521
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:59 2019 Page 1

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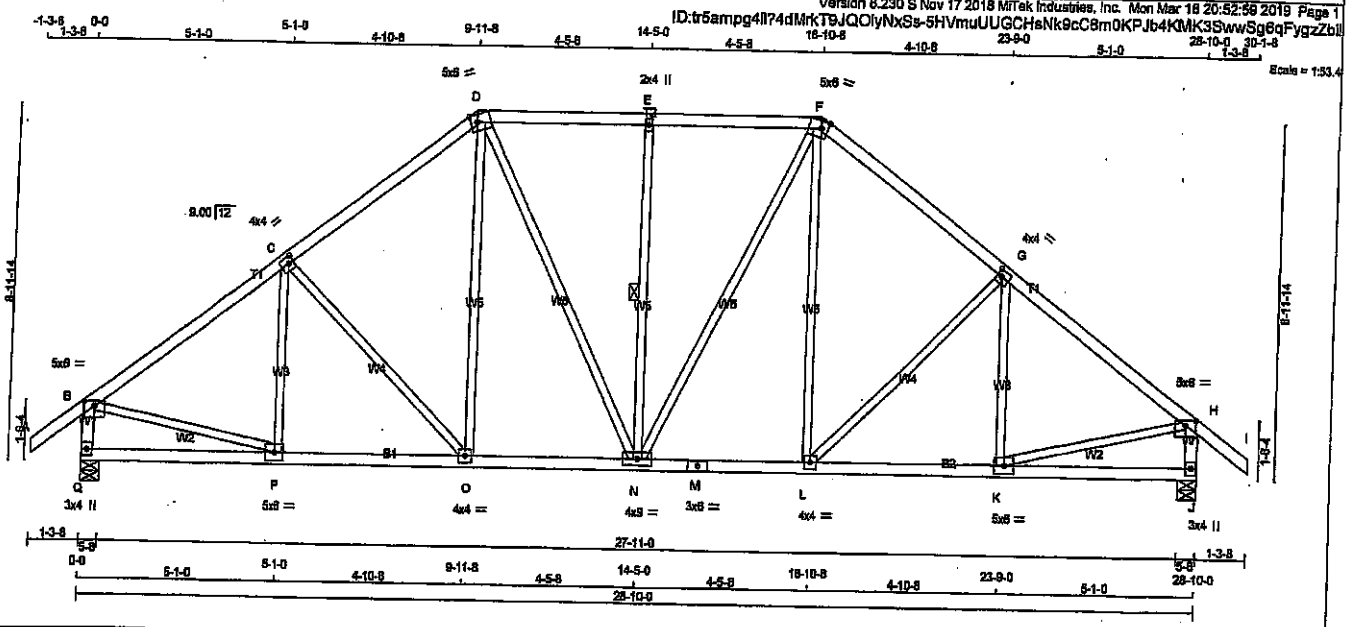

TOTAL WEIGHT = 140 lb (MiTek)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	2.00
E	TMW+w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.00
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BS-t	MT20	3.0	6.0		
N	BMWWW-t	MT20	4.0	9.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
Q	2168	0	2168	0	0	IN-SX	IN-SX
J	2168	0	2168	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1611	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0
J	1611	917 / 0	303 / 0	0 / 0	0 / 0	391 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2161 / 0	-102.1	-102.1	0.41 (1)	4.24	
C-D	-1948 / 0	-102.1	-102.1	0.38 (1)	4.46	
D-E	-1713 / 0	-102.1	-102.1	0.28 (1)	4.60	
E-F	-1713 / 0	-102.1	-102.1	0.28 (1)	4.60	
F-G	-1948 / 0	-102.1	-102.1	0.38 (1)	4.46	
G-H	-2161 / 0	-102.1	-102.1	0.41 (1)	4.24	
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
Q-B	-2087 / 0	0.0	0.0	0.22 (1)	5.86	
J-H	-2087 / 0	0.0	0.0	0.22 (1)	5.86	
Q-P	0 / 0	-38.5	-38.5	0.18 (3)	10.00	
P-O	0 / 1773	-38.5	-38.5	0.41 (2)	10.00	
O-N	0 / 1531	-38.5	-38.5	0.33 (1)	10.00	
N-M	0 / 1531	-38.5	-38.5	0.33 (1)	10.00	
M-L	0 / 1531	-38.5	-38.5	0.33 (1)	10.00	
L-K	0 / 1773	-38.5	-38.5	0.41 (2)	10.00	
K-J	0 / 0	-38.5	-38.5	0.18 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-201 / 91	0.09 (1)
C-O	-350 / 0	0.33 (1)
O-D	0 / 436	0.10 (2)
D-N	0 / 399	0.09 (1)
N-E	-547 / 0	0.29 (1)
N-F	0 / 399	0.09 (1)
L-F	0 / 436	0.10 (2)
L-G	-350 / 0	0.33 (1)
K-G	-201 / 91	0.09 (1)
B-P	0 / 1821	0.41 (1)
K-H	0 / 1821	0.41 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9; NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.41/1.00 (K-L:2) , WB=0.41/1.00 (H-K:1) , SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

(S) GRIP= 0.90 (P) (INPUT = 0.90)
(S) METAL= 0.50 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100090928
PROVINCE OF ONTARIO

DWG NO. TAM 71905522
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T4	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:01 2019 Page 1
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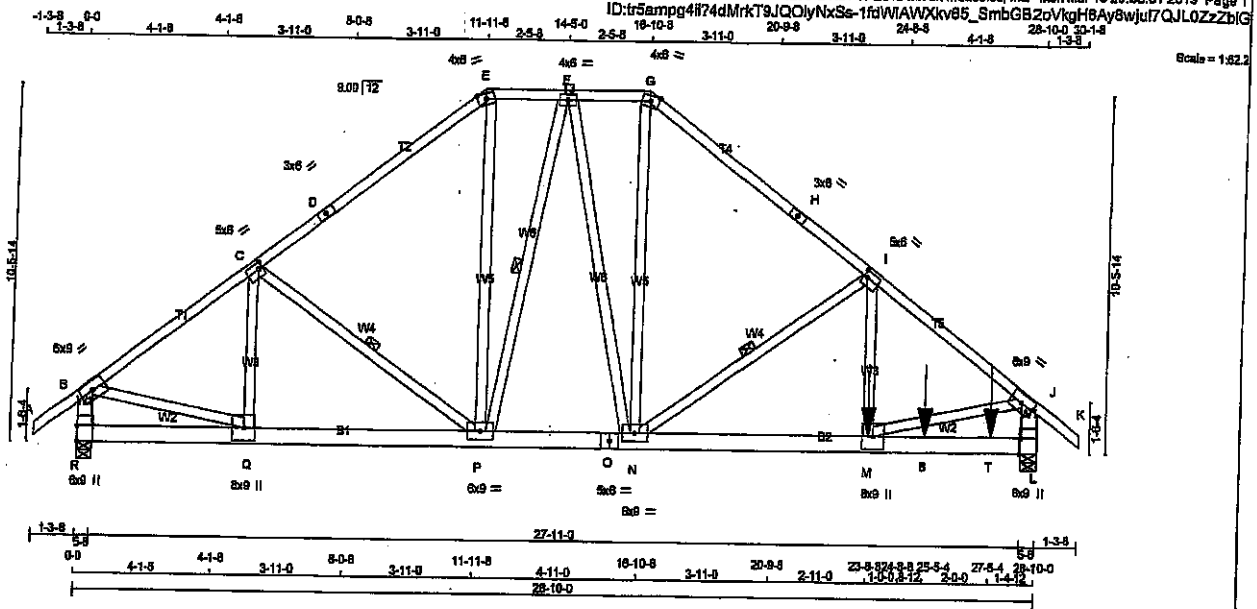

TOTAL WEIGHT = 4 X 184 = 736 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
D-A	1	12	TOP
D-E	1	12	TOP
E-G	1	12	TOP
G-H	1	12	TOP
H-K	1	12	TOP
R-B	2	12	TOP
L-J	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
R-O	2	12	TOP
O-L	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
I-M	1	2	SIDE(1273.2)
2x4	1	2	
P-C	1	6	
P-E	1	6	
N-G	1	8	
N-I	1	6	
Q-B	1	6	
M-J	1	6	
P-F	1	6	
F-N	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	6.0	9.0		
C	TMWW-t	MT20	5.0	6.0	2.25	1.50
D	TS-t	MT20	3.0	6.0		
E	TTW-m	MT20	4.0	6.0	Edge	
F	TMWW-t	MT20	4.0	6.0		
G	TTW-m	MT20	4.0	6.0	Edge	
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	5.0	6.0	2.25	1.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
R	3295	0	3295	0	0	5-8	5-8
L	6250	0	6250	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	2448	1397 / 0	458 / 0	0 / 0	0 / 0	594 / 0	0 / 0
L	6125	3510 / 0	1134 / 0	0 / 0	0 / 0	1481 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.63 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, I-N, F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB:	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
FR-TO						
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	
B-C	-3692 / 0	-102.1	-102.1	0.49 (1)	4.40	
C-D	-3485 / 0	-102.1	-102.1	0.59 (1)	4.52	
D-E	-3485 / 0	-102.1	-102.1	0.59 (1)	4.52	
E-F	-2775 / 0	-102.1	-102.1	0.08 (1)	5.43	
F-G	-3276 / 0	-102.1	-102.1	0.09 (1)	5.07	
G-H	-4103 / 0	-102.1	-102.1	0.63 (1)	4.23	
H-I	-4103 / 0	-102.1	-102.1	0.63 (1)	4.23	
I-J	-9064 / 0	-102.1	-102.1	0.96 (1)	2.63	
J-K	0 / 42	-102.1	-102.1	0.08 (1)	10.00	
R-B	-3208 / 0	0.0	0.0	0.12 (1)	7.73	
L-J	-7365 / 0	0.0	0.0	0.27 (1)	5.62	
R-Q	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
Q-P	0 / 2998	-38.5	-38.5	0.25 (1)	10.00	
P-O	0 / 3050	-38.5	-38.5	0.25 (1)	10.00	
O-N	0 / 3050	-38.5	-38.5	0.26 (1)	10.00	
N-M	0 / 7293	-38.5	-38.5	0.62 (1)	10.00	
M-S	0 / 0	-38.5	-38.5	0.44 (1)	10.00	
S-T	0 / 0	-38.5	-38.5	0.44 (1)	10.00	
T-L	0 / 0	-38.5	-38.5	0.44 (1)	10.00	

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-C	-419 / 37	0.08 (1)
C-P	-302 / 0	0.05 (1)
P-E	0 / 1647	0.15 (1)
N-G	0 / 2012	0.18 (1)
N-I	-4981 / 0	0.89 (1)
M-I	0 / 5136	0.45 (1)
B-Q	0 / 3087	0.27 (1)
M-J	0 / 7462	0.66 (1)
P-F	-1231 / 0	0.35 (1)
F-N	0 / 1016	0.09 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	23-8-8	-5582	-5582	—	BACK	VERT	TOTAL
S	25-5-4	-814	-814	—	BACK	VERT	TOTAL
T	27-5-4	-814	-814	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.17")

CSI: TC=0.96/1.00 (I-J:1) , BC=0.62/1.00 (M-N:1) , WB=0.89/1.00 (I-N:1) , SSI=0.31/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.90 (J) (INPUT = 0.90)
JS METAL= 0.62 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

DWG NO. TAM 7905523
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T4	2	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
J	TMVW-t	MT20	6.0	9.0		
L	BMV1+t	MT20	6.0	9.0	Edge	0.50
M	BMWW+t	MT20	6.0	9.0		
N	BMWWW-t	MT20	6.0	9.0		
O	BS-t	MT20	5.0	6.0		
P	BMWWW-t	MT20	6.0	9.0		
Q	BMWW+t	MT20	6.0	9.0		
R	BMV1+t	MT20	6.0	9.0	5.50	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009027
PROVINCE OF ONTARIO

DWG NO. TAM 1905523
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T40	6	1	Preston 11 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:21 2019 Page 1
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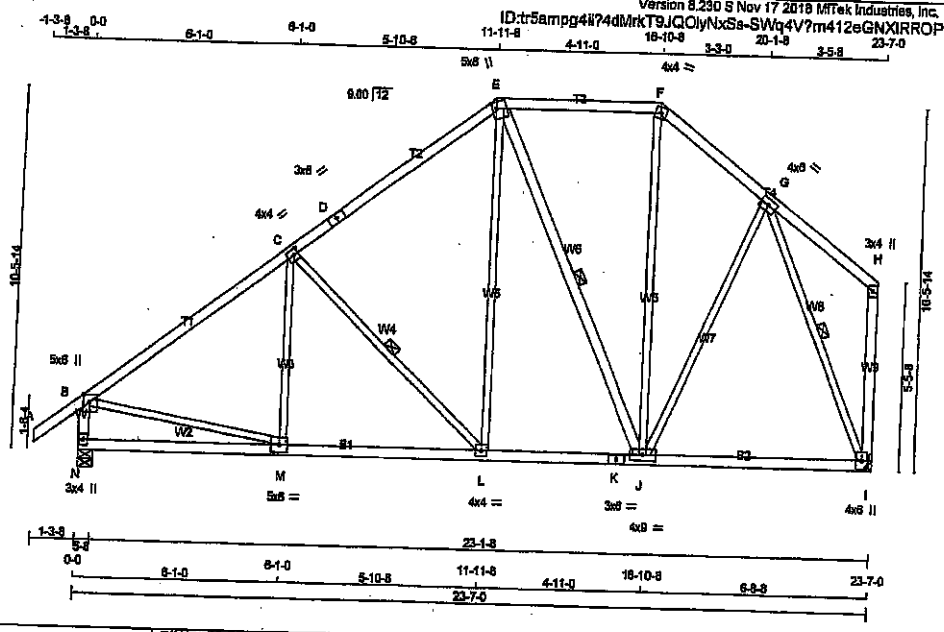

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - J	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	2.50	2.25
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	6.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW1+p	MT20	4.0	6.0		
J	BMWWW-t	MT20	4.0	9.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	4.0		
M	BMWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 6 X 126 = 754 lb [MiTek]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
N	1799	0	1799	0	0	5-8	5-8
I	1659	0	1656	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	1336	765 / 0	248 / 0	0 / 0	0 / 0	323 / 0	0 / 0
I	1239	685 / 0	248 / 0	0 / 0	0 / 0	307 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.45 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-J, G-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-1710 / 0	-102.1	-102.1	0.56 (1)	4.45
C-D	-1248 / 0	-102.1	-102.1	0.51 (1)	5.10
D-E	-1248 / 0	-102.1	-102.1	0.51 (1)	5.10
E-F	-802 / 0	-102.1	-102.1	0.32 (1)	6.25
F-G	-1026 / 0	-102.1	-102.1	0.15 (1)	6.03
G-H	0 / 23	-102.1	-102.1	0.18 (1)	10.00
N-B	-1706 / 0	0.0	0.0	0.18 (1)	6.36
I-H	-136 / 0	0.0	0.0	0.08 (1)	7.81
N-M	0 / 0	-38.5	-38.5	0.28 (3)	10.00
M-L	0 / 1403	-38.5	-38.5	0.45 (2)	10.00
L-K	0 / 967	-38.5	-38.5	0.41 (2)	10.00
K-J	0 / 967	-38.5	-38.5	0.41 (2)	10.00
J-I	0 / 626	-38.5	-38.5	0.36 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-C	-5 / 223	0.05 (3)
C-L	-614 / 0	0.29 (1)
L-E	0 / 596	0.13 (1)
E-J	-365 / 0	0.27 (1)
J-F	0 / 256	0.06 (2)
J-G	0 / 428	0.10 (1)
B-M	0 / 1430	0.32 (1)
G-I	-1547 / 0	0.72 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100099277
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.09")
ALLOWABLE DEFL.(TL)= L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.56/1.00 (B-C:1) , BC=0.45/1.00 (L-M:2) , WB=0.72/1.00 (G-I:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	518	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.60)
METAL= 0.52 (K) (INPUT = 1.00)

DWG NO. TAM T1905538
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T7S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:02 2019 Page 1
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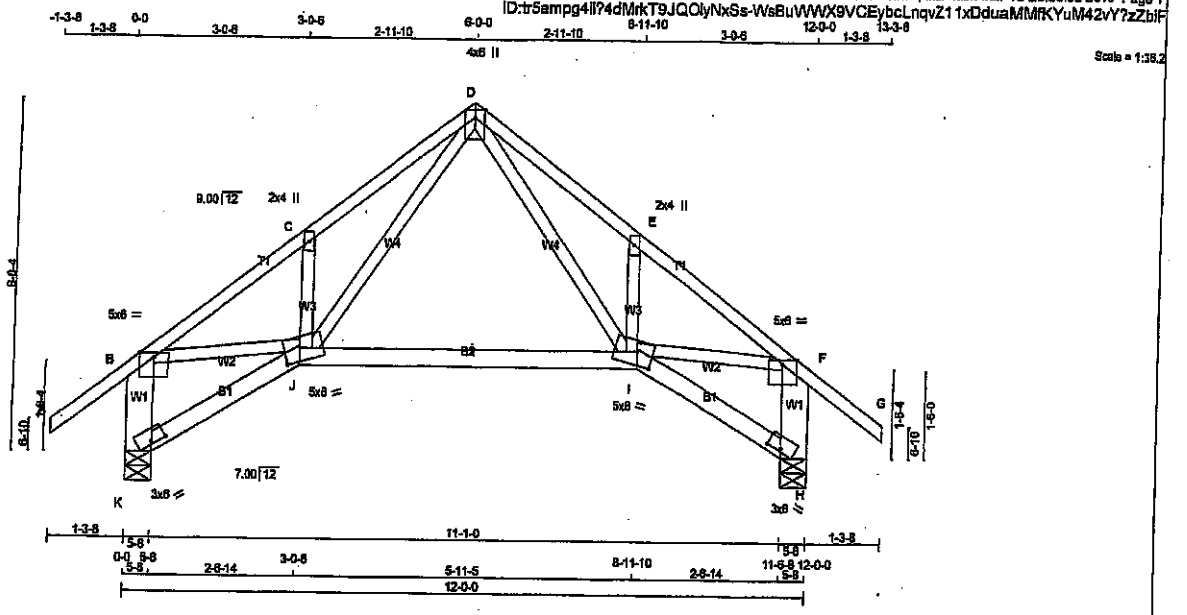


TOTAL WEIGHT = 50 lb [MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x3	DRY	No.2	SPF
D - G	2x3	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMW-p	MT20	5.0	6.0	Edge	
H	BVM1-I	MT20	3.0	6.0	0.50	3.00
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BVM1-I	MT20	3.0	6.0	0.50	3.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	982	0	982	0	0	5-8	5-8
H	982	0	982	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	725	427 / 0	126 / 0	0 / 0	0 / 0	172 / 0	0 / 0
H	725	427 / 0	126 / 0	0 / 0	0 / 0	172 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.77 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 41	-102.1	-102.1	0.26 (1)	10.00	
B-C	-1228 / 0	-102.1	-102.1	0.25 (1)	4.84	
C-D	-1277 / 0	-102.1	-102.1	0.25 (1)	4.77	
D-E	-1277 / 0	-102.1	-102.1	0.25 (1)	4.77	
E-F	-1228 / 0	-102.1	-102.1	0.25 (1)	4.84	
F-G	0 / 41	-102.1	-102.1	0.26 (1)	10.00	
K-B	-924 / 0	0.0	0.0	0.07 (1)	7.81	
H-F	-924 / 0	0.0	0.0	0.07 (1)	7.81	
K-J	0 / 0	-38.5	-38.5	0.08 (3)	10.00	
J-I	0 / 558	-38.5	-38.5	0.37 (2)	10.00	
I-H	0 / 0	-38.5	-38.5	0.08 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 760	0.18 (1)
I-E	-378 / 0	0.06 (1)
J-D	0 / 760	0.18 (1)
J-C	-378 / 0	0.06 (1)
B-J	0 / 1002	0.23 (1)
I-F	0 / 1002	0.23 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 827 (0.17")

CSI: TC=0.26/1.00 (A-B:1) , BC=0.37/1.00 (I-J:2) , WB=0.23/1.00 (B-J:1) , SSI=0.17/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

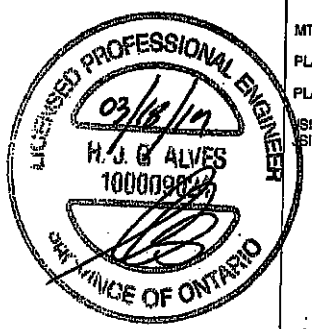
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.74 (B) (INPUT = 0.90)
SI METAL= 0.31 (B) (INPUT = 1.00)



DWG NO. TAM 9200524
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T70S	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:53:23 2019 Page 1
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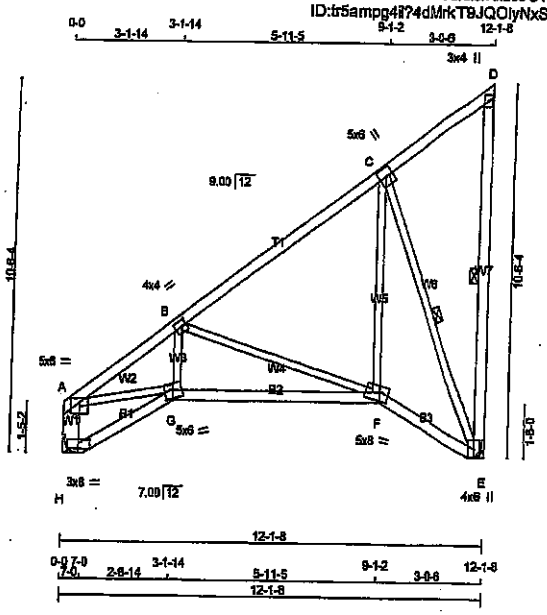


TOTAL WEIGHT = 66 lb

M F

LUMBER
N. L. G. A. RULES

	SIZE		LUMBER	DESCR.
CHORDS H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMWW-t	MT20	5.0	6.0		
D	TMV-p	MT20	3.0	4.0		
E	BVMW1-p	MT20	4.0	6.0	Edge	
F	BBWW-m	MT20	5.0	8.0	2.75	3.00
G	BBWW-m	MT20	5.0	6.0	2.75	3.00
H	BVM1-p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	852	0	852	0	0	MECHANICAL	
E	852	0	852	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 3-8, JOINT E = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0
E	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

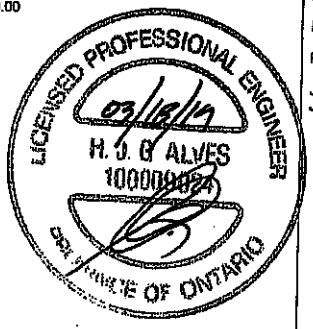
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MAX. MEMB.	FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	FORCE (LBS)	MAX CSI (LC)
FR-TO								
H- A	-792 / 0	0.0	0.0	0.08 (1)	7.81	A- G	0 / 1126	0.25 (1)
A- B	-1356 / 0	-102.1	-102.1	0.25 (1)	5.30	G- B	0 / 348	0.08 (2)
B- C	-428 / 0	-102.1	-102.1	0.26 (1)	6.25	B- F	-801 / 0	0.53 (1)
C- D	-45 / 0	-102.1	-102.1	0.25 (1)	6.25	F- C	0 / 842	0.14 (1)
E- D	-84 / 0	0.0	0.0	0.05 (1)	6.25	C- E	-981 / 0	0.48 (1)
H- G	0 / 0	-38.5	-38.5	0.09 (3)	10.00			
G- F	0 / 1100	-38.5	-38.5	0.44 (2)	10.00			
F- E	0 / 410	-38.5	-38.5	0.13 (2)	10.00			



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 871 (0.17")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.44/1.00 (F-G:2) , WB=0.53/1.00 (B-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.29 (A) (INPUT = 1.00)

DWG NO. TAM T1905539
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T70SC	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:53:23 2019 Page 1
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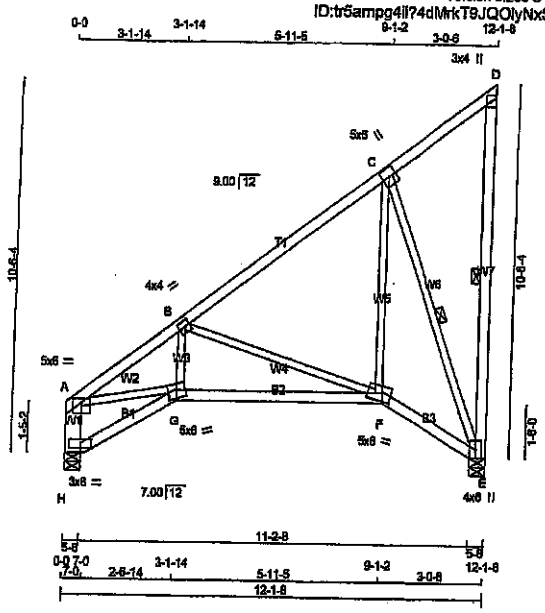

TOTAL WEIGHT = 66 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	6.0	1.25	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMWW+t	MT20	5.0	6.0		
D	TMV+p	MT20	3.0	4.0		
E	BVMW1+p	MT20	4.0	6.0	Edge	
F	BBWW-m	MT20	5.0	8.0	2.75	3.00
G	BBWW-m	MT20	5.0	6.0	2.75	3.00
H	BVM1-p	MT20	3.0	8.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	852	0	852	0	0	5-8	5-8
E	852	0	852	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0
E	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.30 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX. CSI (LC)	MAX. UNBRAC LENGTH
H-A	-792 / 0	0.0	0.0	0.06 (1)	7.81
A-B	-1356 / 0	-102.1	-102.1	0.25 (1)	5.30
B-C	-428 / 0	-102.1	-102.1	0.26 (1)	6.25
C-D	-45 / 0	-102.1	-102.1	0.25 (1)	6.25
E-D	-84 / 0	0.0	0.0	0.05 (1)	6.25
H-G	0 / 0	-38.5	-38.5	0.09 (3)	10.00
G-F	0 / 1100	-38.5	-38.5	0.44 (2)	10.00
F-E	0 / 410	-38.5	-38.5	0.13 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-G	0 / 1126	0.25 (1)
G-B	0 / 348	0.08 (2)
B-F	-801 / 0	0.53 (1)
F-C	0 / 642	0.14 (1)
C-E	-981 / 0	0.48 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 871 (0.17")

CSI: TC=0.28/1.00 (B-C:1) , BC=0.44/1.00 (F-G:2) , WB=0.53/1.00 (B-F:1) , SSI=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.70 (C) (INPUT = 0.90)
USI METAL= 0.29 (A) (INPUT = 1.00)

DWG NO. TAM T7005540
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T71	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:24 2019 Page 1
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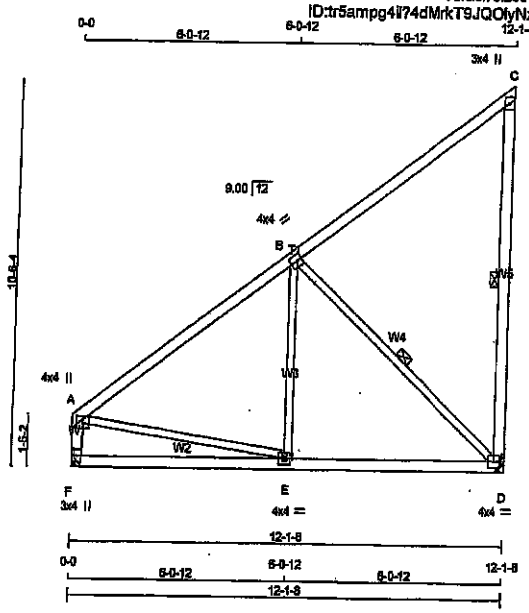


Scale = 1:57.7

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TMVW-t	MT20	4.0	4.0	2.00	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	4.0		
E	BMVW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	852	0	852	0	0	IN-SX	IN-SX
D	852	0	852	0	0	MECHANICAL	
						MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT D = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0
D	637	352 / 0	127 / 0	0 / 0	0 / 0	158 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D, B-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

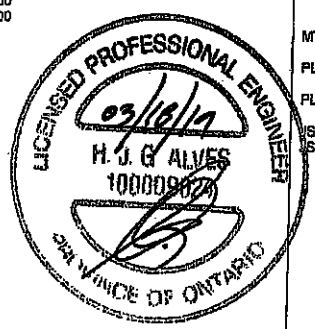
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- A	-763 / 0	0.0	0.0	0.08 (1)	7.81
A- B	-666 / 0	-102.1	-102.1	0.50 (1)	6.25
B- C	-46 / 0	-102.1	-102.1	0.49 (1)	6.25
D- C	-233 / 0	0.0	0.0	0.13 (1)	6.25
F- E	0 / 0	-36.5	-36.5	0.31 (3)	10.00
E- D	0 / 569	-36.5	-36.5	0.37 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- E	0 / 579	0.13 (1)
E- B	0 / 323	0.08 (3)
B- D	-778 / 0	0.35 (1)



TOTAL WEIGHT = 59 lb
[M|F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014 .

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.50/1.00 (A-B:1) , BC=0.37/1.00 (D-E:2) , WB=0.35/1.00 (B-D:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.85 (A) (INPUT = 0.90)
SI METAL= 0.22 (A) (INPUT = 1.00)

DWG NO. TAM 7905541
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T72	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:28 2019 Page 1
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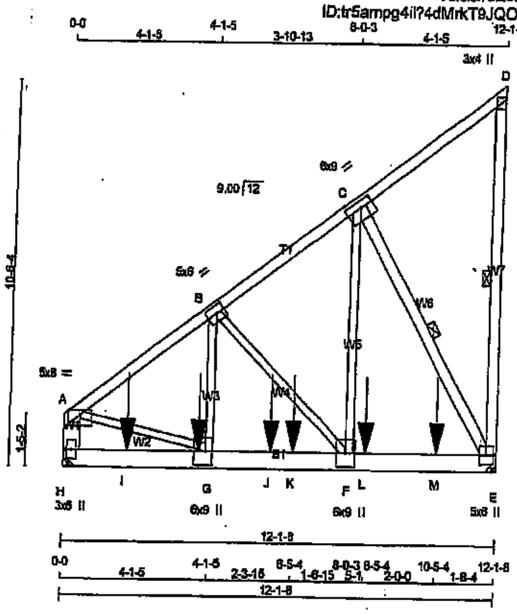

STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED PROVISIONS OF ONTARIO BUILDING CODE

TOTAL WEIGHT = 2 X 77 = 153 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H- A	2	12	TOP
A- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H- E	2	12	SIDE(183.1)
WEBS : (0.122"X3") SPIRAL NAILS			
B- G	1	6	SIDE(239.4)
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	5.0	6.0	2.50	2.50
C	TMWW-t	MT20	6.0	9.0	2.25	4.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	5.0	6.0		
F	BMWW+t	MT20	6.0	9.0		
G	BMWW+t	MT20	6.0	9.0	4.50	2.25
H	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	5621	0	5621	0	0	MECHANICAL	
E	5801	0	5801	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, E. MINIMUM BEARING LENGTH AT JOINT H = 4-0, JOINT E = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
H	4182	2368 / 0	795 / 0	0 / 0	0 / 0	1019 / 0	0 / 0
E	4316	2444 / 0	820 / 0	0 / 0	0 / 0	1052 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.81 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
H- A	-4694 / 0	0.0	0.0	0.18 (1)	6.56
A- B	-5884 / 0	-102.1	-102.1	0.31 (1)	3.81
B- C	-3499 / 0	-102.1	-102.1	0.20 (1)	4.83
C- D	-27 / 0	-102.1	-102.1	0.14 (1)	6.25
E- D	-164 / 0	0.0	0.0	0.04 (1)	6.25
H- I	0 / 0	-38.5	-38.5	0.46 (1)	10.00
I- G	0 / 0	-38.5	-38.5	0.46 (1)	10.00
G- J	0 / 4716	-38.5	-38.5	0.87 (1)	10.00
J- K	0 / 4716	-38.5	-38.5	0.87 (1)	10.00
K- F	0 / 4716	-38.5	-38.5	0.87 (1)	10.00
F- L	0 / 2813	-38.5	-38.5	0.74 (1)	10.00
L- M	0 / 2813	-38.5	-38.5	0.74 (1)	10.00
M- E	0 / 2813	-38.5	-38.5	0.74 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- G	0 / 4858	0.60 (1)
G- B	0 / 3028	0.37 (1)
B- F	-2760 / 0	0.75 (1)
F- C	0 / 6433	0.80 (1)
C- E	-5570 / 0	0.69 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE
G	3-9-12	-1619	-1619	—	BACK	VERT	TOTAL
I	1-9-12	-1619	-1619	—	BACK	VERT	TOTAL
J	5-9-12	-1619	-1619	—	BACK	VERT	TOTAL
K	6-5-4	-1619	-1619	—	BACK	VERT	TOTAL
L	8-5-4	-1619	-1619	—	BACK	VERT	TOTAL
M	10-5-4	-1619	-1619	—	BACK	VERT	TOTAL

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.40")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.40")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.31/1.00 (A-B:1) , BC=0.87/1.00 (F-G:1) , WB=0.69/1.00 (C-E:1) , SSI=0.85/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

CSI GRIP= 0.88 (C) (INPUT = 0.90)
CSI METAL= 0.71 (G) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM 7905542
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T8	7	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:03 2019 Page 1
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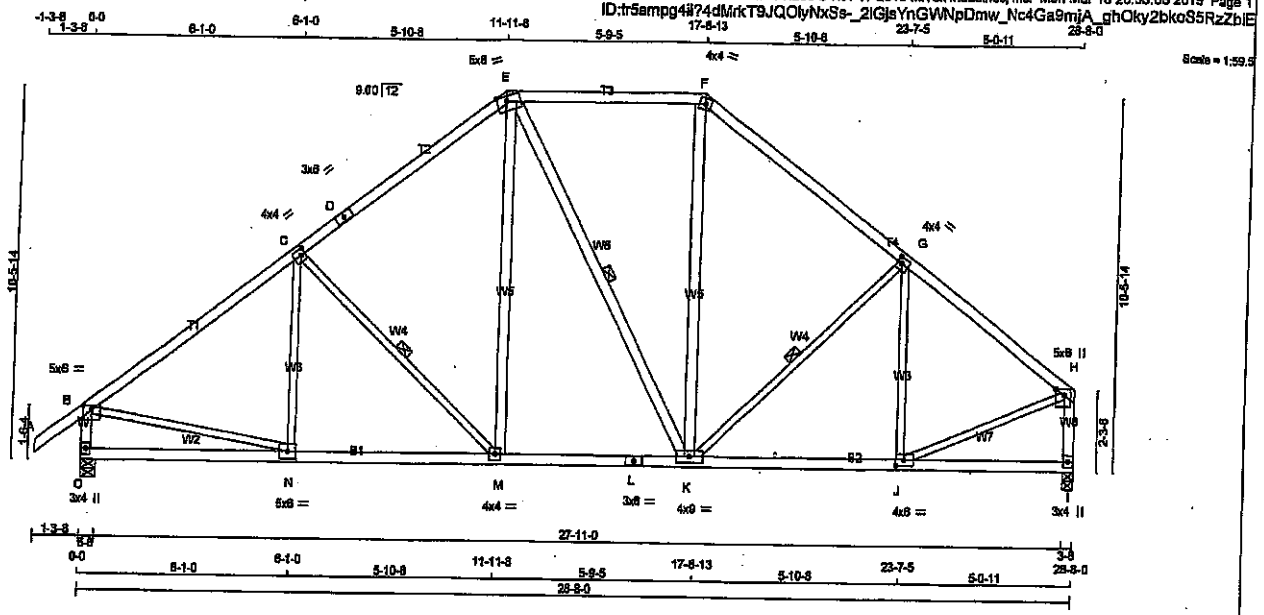

TOTAL WEIGHT = 7 X 144 = 1008 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
O - B	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
E - K	2x4	DRY	No.2	SPF
K - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	8.0	1.75	3.25
F	TTW-m	MT20	4.0	4.0		
G	TMWW-t	MT20	4.0	4.0	2.00	1.50
H	TMW+p	MT20	5.0	6.0	Edge	3.00
I	BMV1+p	MT20	3.0	4.0		
J	BMWW-t	MT20	4.0	6.0	2.00	2.75
K	BMWWW-t	MT20	4.0	9.0		
L	BS-t	MT20	3.0	6.0		
M	BMWW-t	MT20	4.0	4.0		
N	BMWW-t	MT20	5.0	6.0		
O	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	MAXIMUM FACTORED GROSS REACTION HORZ	MAXIMUM FACTORED GROSS REACTION UPLIFT	INPUT BRG	REQRD BRG
O	2156	0	2156	0	0	5-8	5-8
I	2015	0	2015	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
O	1603	912 / 0	301 / 0	0 / 0	0 / 0	389 / 0	0 / 0
I	1506	832 / 0	301 / 0	0 / 0	0 / 0	373 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, I

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-M, E-K, G-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2180 / 0	-102.1	-102.1	0.61 (1)	3.99	
C-D	-1775 / 0	-102.1	-102.1	0.55 (1)	4.40	
D-E	-1775 / 0	-102.1	-102.1	0.55 (1)	4.40	
E-F	-1352 / 0	-102.1	-102.1	0.46 (1)	4.89	
F-G	-1722 / 0	-102.1	-102.1	0.49 (1)	4.54	
G-H	-1883 / 0	-102.1	-102.1	0.48 (1)	4.41	
O-B	-2061 / 0	0.0	0.0	0.22 (1)	5.69	
I-H	-1939 / 0	0.0	0.0	0.24 (1)	6.04	
O-N	0 / 0	-38.5	-38.5	0.27 (3)	10.00	
N-M	0 / 1779	-38.5	-38.5	0.49 (2)	10.00	
M-L	0 / 1391	-38.5	-38.5	0.38 (2)	10.00	
L-K	0 / 1391	-38.5	-38.5	0.38 (2)	10.00	
K-J	0 / 1542	-38.5	-38.5	0.41 (2)	10.00	
J-I	0 / 0	-38.5	-38.5	0.21 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	-86 / 180	0.05 (1)
C-M	-549 / 0	0.26 (1)
M-E	0 / 594	0.10 (2)
E-K	-80 / 0	0.06 (1)
K-F	0 / 502	0.05 (2)
K-G	-272 / 0	0.13 (1)
J-G	-384 / 31	0.21 (1)
B-N	0 / 1813	0.41 (1)
J-H	0 / 1654	0.37 (1)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H J G ALVES
100090224
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.96")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.96")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.61/1.00 (B-C:1) , BC=0.49/1.00 (M-N:2) , WB=0.41/1.00 (B-N:1) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LS: GRIP= 0.89 (J) (INPUT = 0.90)
SH: METAL= 0.45 (B) (INPUT = 1.00)

DWG NO. TAM M05525
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T9	1	1	Preston 11 TRUSS DESC.	

Tamarack Roof Trues, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:05 2019 Page 1
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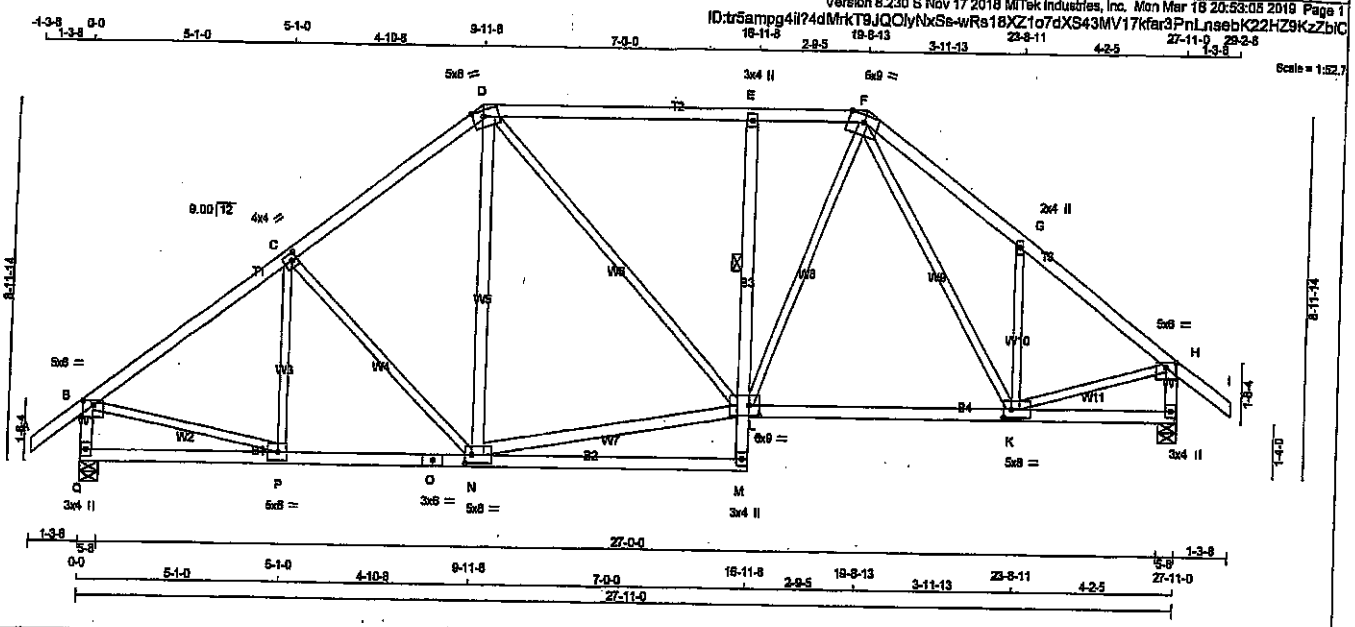


LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
Q - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
Q - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - E	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
N - D	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	6.0	1.50	3.00
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	1.75	3.25
E	TMV+p	MT20	3.0	4.0		
F	TTWW-m	MT20	6.0	9.0	Edge	
G	TMW+w	MT20	2.0	4.0		
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
J	BMV1+p	MT20	3.0	4.0		
K	BMWWW-t	MT20	5.0	8.0	2.50	2.25
L	BVMWWW-I	MT20	6.0	6.0	3.00	3.25
M	BMV+p	MT20	3.0	4.0		
N	BMWWW-t	MT20	5.0	6.0	2.50	2.00
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0		
Q	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
Q	2103	0	2103	0	0	5-8	5-8
J	2103	0	2103	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1563	891 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
J	1563	891 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-L.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

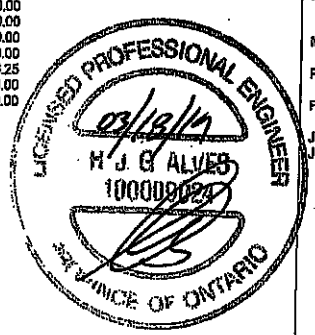
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM (PLF)	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-2093 / 0	-102.1	-102.1	0.40 (1)	4.32
C-D	-1871 / 0	-102.1	-102.1	0.38 (1)	4.53
D-E	-1837 / 0	-102.1	-102.1	0.63 (1)	4.17
E-F	-1841 / 0	-102.1	-102.1	0.49 (1)	4.85
F-G	-2134 / 0	-102.1	-102.1	0.27 (1)	4.43
G-H	-2100 / 0	-102.1	-102.1	0.26 (1)	4.46
H-I	0 / 42	-102.1	-102.1	0.14 (1)	10.00
Q-B	-2018 / 0	0.0	0.0	0.21 (1)	5.94
J-H	-2058 / 0	0.0	0.0	0.22 (1)	5.90
Q-P	0 / 0	-38.5	-38.5	0.17 (3)	10.00
P-O	0 / 1703	-38.5	-38.5	0.52 (2)	10.00
O-N	0 / 1703	-38.5	-38.5	0.52 (2)	10.00
N-M	0 / 19	-38.5	-38.5	0.30 (3)	10.00
M-L	0 / 136	0.0	0.0	0.05 (1)	10.00
L-E	-705 / 0	0.0	0.0	0.07 (1)	6.25
L-K	0 / 1625	-38.5	-38.5	0.49 (2)	10.00
K-J	0 / 0	-38.5	-38.5	0.30 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
P-C	-214 / 62	0.09 (1)
C-N	-340 / 0	0.32 (1)
N-D	0 / 296	0.05 (3)
N-L	0 / 1474	0.24 (1)
D-L	0 / 544	0.12 (1)
L-F	0 / 867	0.20 (1)
K-G	-500 / 0	0.16 (1)
B-P	0 / 1749	0.39 (1)
K-H	0 / 1768	0.40 (1)
F-K	0 / 335	0.08 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 869 (0.21")

CSI: TC=0.63/1.00 (D-E:1) , BC=0.52/1.00 (N-P:2) , WB=0.40/1.00 (H-K:1) , SSI=0.32/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (N) (INPUT = 0.90)
JSI METAL= 0.64 (O) (INPUT = 1.00)

DWG NO. TAM P105536
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T10	1	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:06 2019 Page 1
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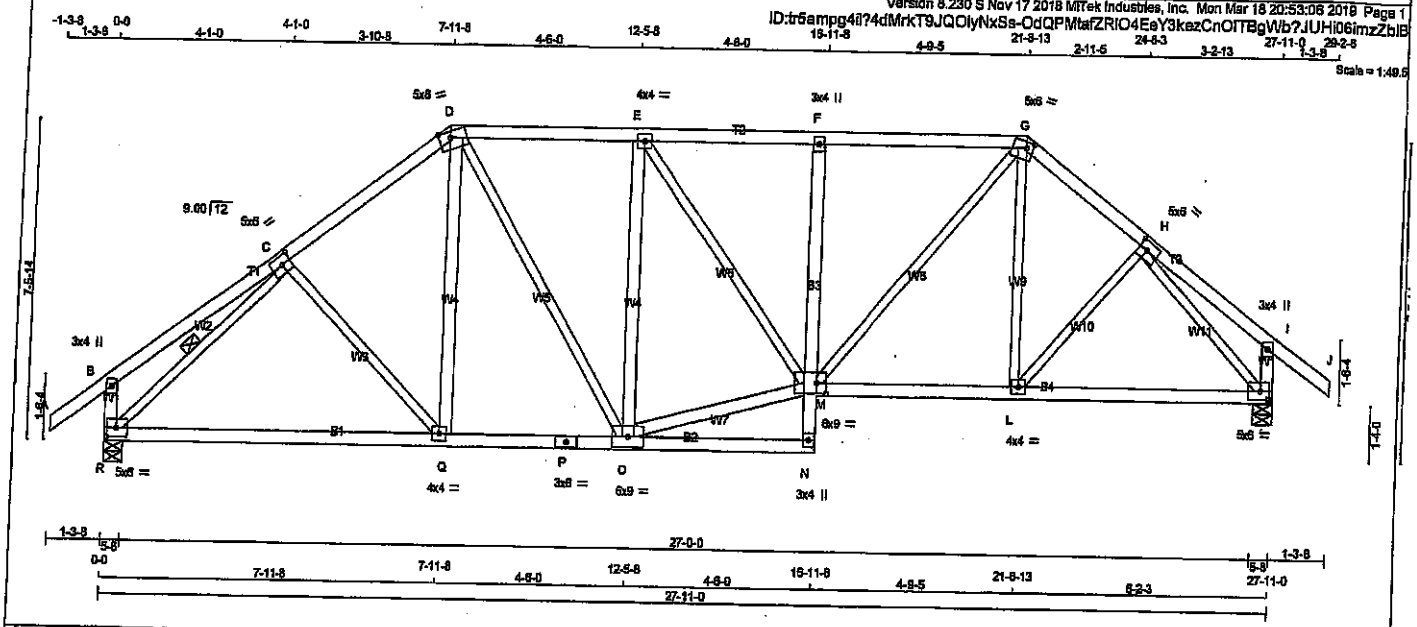

TOTAL WEIGHT = 141 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
R - P	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
Q - D	2x4	DRY	No.2	SPF
O - E	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B, F, I						
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.75
D	TTWW-m	MT20	5.0	8.0	1.75	3.25
E	TMWW-t	MT20	4.0	4.0		
G	TTWW-m	MT20	5.0	6.0	1.75	2.00
H	TMWW-t	MT20	5.0	6.0	2.50	2.50
K	BMVW1-t	MT20	5.0	6.0	2.50	2.50
L	BMWW-t	MT20	4.0	4.0		
M	BVMWWW-I	MT20	6.0	9.0	3.00	2.75
N	BMV+p	MT20	3.0	4.0		
O	BMWWW-t	MT20	6.0	9.0		
P	BS-t	MT20	3.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BMVW1-t	MT20	5.0	6.0	2.50	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
R	2103	0	2103	0	0	5-8	5-8
K	2103	0	2103	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
R	1563	891 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0
K	1563	891 / 0	293 / 0	0 / 0	0 / 0	379 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.21 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-R.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B-C	0 / 27	-102.1	-102.1	0.25 (1)		10.00
C-D	-2009 / 0	-102.1	-102.1	0.24 (1)		4.67
D-E	-1908 / 0	-102.1	-102.1	0.27 (1)		4.62
E-F	-2268 / 0	-102.1	-102.1	0.29 (1)		4.28
F-G	-2274 / 0	-102.1	-102.1	0.35 (1)		4.21
G-H	-2074 / 0	-102.1	-102.1	0.15 (1)		4.60
H-I	0 / 18	-102.1	-102.1	0.13 (1)		10.00
I-J	0 / 42	-102.1	-102.1	0.14 (1)		10.00
R-B	-298 / 0	0.0	0.0	0.03 (1)		7.81
K-I	-270 / 0	0.0	0.0	0.03 (1)		7.81
R-Q	0 / 1595	-38.5	-38.5	0.61 (2)		10.00
Q-P	0 / 1589	-38.5	-38.5	0.61 (2)		10.00
P-O	0 / 1589	-38.5	-38.5	0.61 (2)		10.00
O-N	0 / 27	-38.5	-38.5	0.14 (3)		10.00
N-M	0 / 98	0.0	0.0	0.08 (1)		10.00
M-F	-514 / 0	0.0	0.0	0.23 (1)		7.81
M-L	0 / 1643	-38.5	-38.5	0.49 (2)		10.00
L-K	0 / 1532	-38.5	-38.5	0.47 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C-Q	-18 / 105	0.03 (3)
Q-D	0 / 367	0.06 (2)
D-O	0 / 607	0.14 (1)
O-E	-955 / 0	0.69 (1)
O-M	0 / 1974	0.32 (1)
E-M	0 / 591	0.13 (1)
M-G	0 / 980	0.22 (1)
L-G	0 / 224	0.05 (3)
R-C	-2314 / 0	0.55 (1)
L-H	0 / 183	0.04 (2)
H-K	-2315 / 0	0.82 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.31")

CSI: TC=0.35/1.00 (F-G:1) , BC=0.61/1.00 (O-Q:2) , WB=0.82/1.00 (H-K:1) , SSI=0.23/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (R) (INPUT = 0.80)
JSI METAL= 0.57 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM T1905627
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T11	1	2	Preston 11 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:53:08 2019 Page 1
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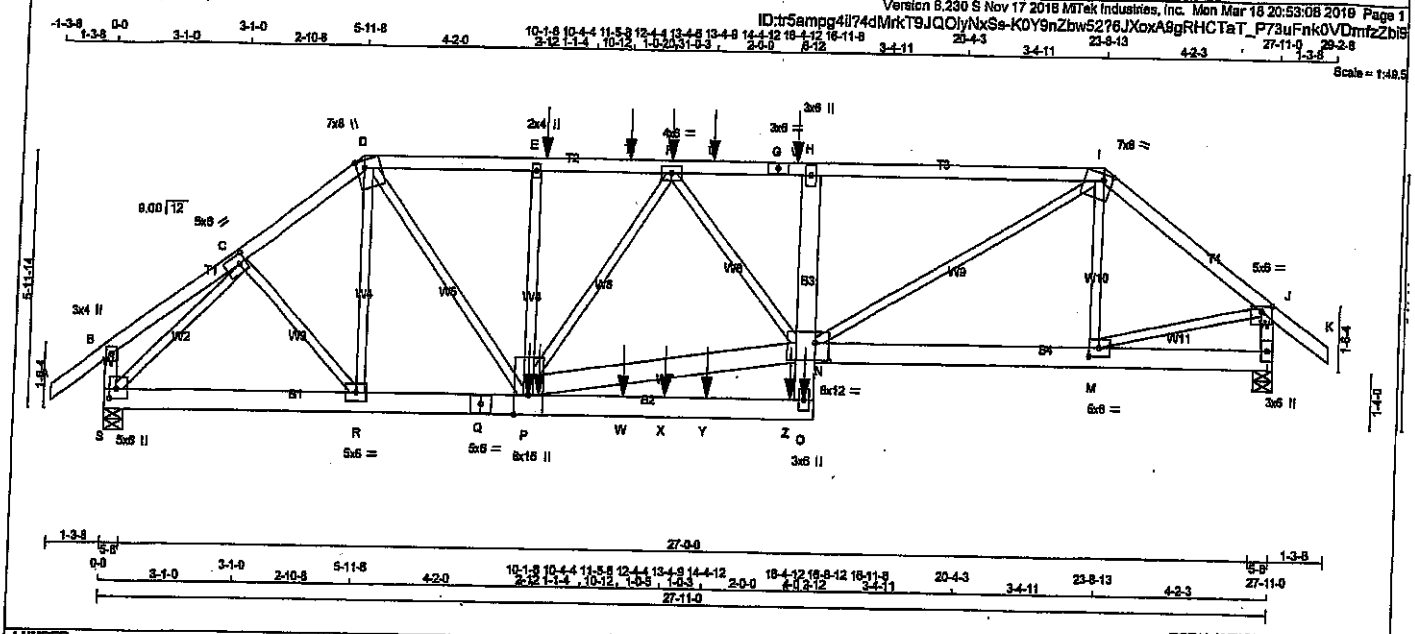

TOTAL WEIGHT = 2 X 154 = 308 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	1650F 1.5E	SPF
D - G	2x4	DRY	1650F 1.5E	SPF
G - I	2x4	DRY	1650F 1.5E	SPF
I - K	2x4	DRY	1650F 1.5E	SPF
S - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
S - Q	2x6	DRY	No.2	SPF
Q - O	2x6	DRY	No.2	SPF
O - H	2x6	DRY	No.2	SPF
N - L	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-D	1	12	TOP
D-G	1	12	SIDE(61.0)
G-I	1	12	SIDE(61.0)
I-K	1	12	TOP
S-B	1	12	TOP
L-J	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-Q	2	12	TOP
Q-O	2	12	SIDE(183.1)
O-H	2	12	SIDE(385.1)
N-L	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
E-P	1	6	SIDE(244.5)
2x3	1	6	
2x8	2	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	7.0	6.0	Edge	2.50
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TS-t	MT20	3.0	6.0		
H	TMV+p	MT20	3.0	6.0		
I	TTWW-m	MT20	7.0	6.0	1.75	2.50
J	TMW-p	MT20	5.0	6.0	1.50	3.00
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BVMWWW-t	MT20	8.0	12.0	5.00	4.00

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	4221	0	4221	0	0	5-8	5-8
S	4311	0	4311	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3117	1806 / 0	509 / 0	0 / 0	0 / 0	803 / 0	0 / 0
S	3176	1867 / 0	505 / 0	0 / 0	0 / 0	805 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, S

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.49 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.06 (1)	10.00
B-C	-2 / 11	-102.1	-102.1	0.05 (1)	10.00
C-D	-5051 / 0	-102.1	-102.1	0.10 (1)	4.80
D-E	-6278 / 0	-102.1	-102.1	0.17 (1)	4.33
E-T	-6278 / 0	-102.1	-102.1	0.16 (1)	4.33
T-F	-6278 / 0	-102.1	-102.1	0.16 (1)	4.33
F-U	-8567 / 0	-102.1	-102.1	0.37 (1)	3.56
U-G	-8567 / 0	-102.1	-102.1	0.37 (1)	3.56
G-V	-8567 / 0	-102.1	-102.1	0.37 (1)	3.56
V-H	-8567 / 0	-102.1	-102.1	0.37 (1)	3.56
H-I	-8699 / 0	-102.1	-102.1	0.64 (1)	3.49
I-J	-4734 / 0	-102.1	-102.1	0.20 (1)	4.82
J-K	0 / 42	-102.1	-102.1	0.06 (1)	10.00
S-B	-274 / 0	0.0	0.0	0.02 (1)	7.81
L-J	-4184 / 0	0.0	0.0	0.24 (1)	5.80
S-R	0 / 3500	-38.5	-38.5	0.28 (1)	10.00
R-Q	0 / 4014	-38.5	-38.5	0.30 (1)	10.00
Q-P	0 / 4014	-38.5	-38.5	0.30 (1)	10.00
P-W	0 / 425	-38.5	-38.5	0.14 (2)	10.00
W-X	0 / 425	-38.5	-38.5	0.14 (2)	10.00
X-Y	0 / 425	-38.5	-38.5	0.14 (2)	10.00
Y-Z	0 / 425	-38.5	-38.5	0.14 (2)	10.00
Z-O	0 / 425	-38.5	-38.5	0.14 (2)	10.00
O-N	0 / 2130	0.0	0.0	0.35 (1)	10.00
N-H	-815 / 0	0.0	0.0	0.22 (1)	7.81
N-M	0 / 3762	-38.5	-38.5	0.32 (1)	10.00
M-L	0 / 0	-38.5	-38.5	0.09 (3)	10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-D	-338 / 0	0.09 (1)
P-E	-561 / 0	0.16 (1)
N-I	0 / 5779	0.72 (1)
M-I	-690 / 0	0.11 (1)
M-J	0 / 3904	0.48 (1)
D-P	0 / 3790	0.47 (1)
P-N	0 / 7136	0.46 (1)
S-C	-5268 / 0	0.85 (1)
C-R	0 / 762	0.09 (1)
P-F	-2267 / 0	0.74 (1)
F-N	0 / 7856	0.77 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	10-4-4	-126	-126	—	BACK	VERT	TOTAL	—	—
F	13-4-9	-126	-126	—	BACK	VERT	TOTAL	—	—
O	16-8-12	-1944	-1944	—	BACK	VERT	TOTAL	—	—
P	10-1-8	-1613	-1613	—	BACK	VERT	TOTAL	—	—
P	10-4-4	-26	-26	—	BACK	VERT	TOTAL	—	—
T	12-4-4	-126	-126	—	BACK	VERT	TOTAL	—	—
U	14-4-12	-126	-126	—	BACK	VERT	TOTAL	—	—
V	16-4-12	-126	-126	—	BACK	VERT	TOTAL	—	—
W	12-4-4	-26	-26	—	BACK	VERT	TOTAL	—	—
X	13-4-9	-26	-26	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OSC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.93")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.16")
ALLOWABLE DEFL.(TL)= L/360 (0.93")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.28")

CSI: TC=0.64/1.00 (H-I:1) , BC=0.36/1.00 (N-O:1) , WB=0.85/1.00 (C-S:1) , SSI=0.18/1.00 (F-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.80 (I) (INPUT = 0.80)
JSI METAL= 0.66 (C) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM 1140552B
STRUCTURAL
COMPONENT ONLY

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T11	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:tr5ampg4il?4dMrkT9JQOlyNxSs-K0Y9nZbw5276JXoxA9gRHCTaT_P73uFnk0VDmfzZbig

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
O	BMV+p	MT20	3.0	6.0		
P	BMWWWW*+rMT20		8.0	16.0	Edge	
Q	BS-t	MT20	5.0	6.0		
R	BMWW-t	MT20	5.0	6.0		
S	BMVW1+p	MT20	5.0	6.0	2.75	2.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
Y	14-4-12	-26	-26	—	BACK	VERT	TOTAL	—	—
Z	16-4-12	-32	-32	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/18/19
H G ALVES
100009820
PROVINCE OF ONTARIO

DWG NO. TAM P905528
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T12	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:53:09 2019 Page 1
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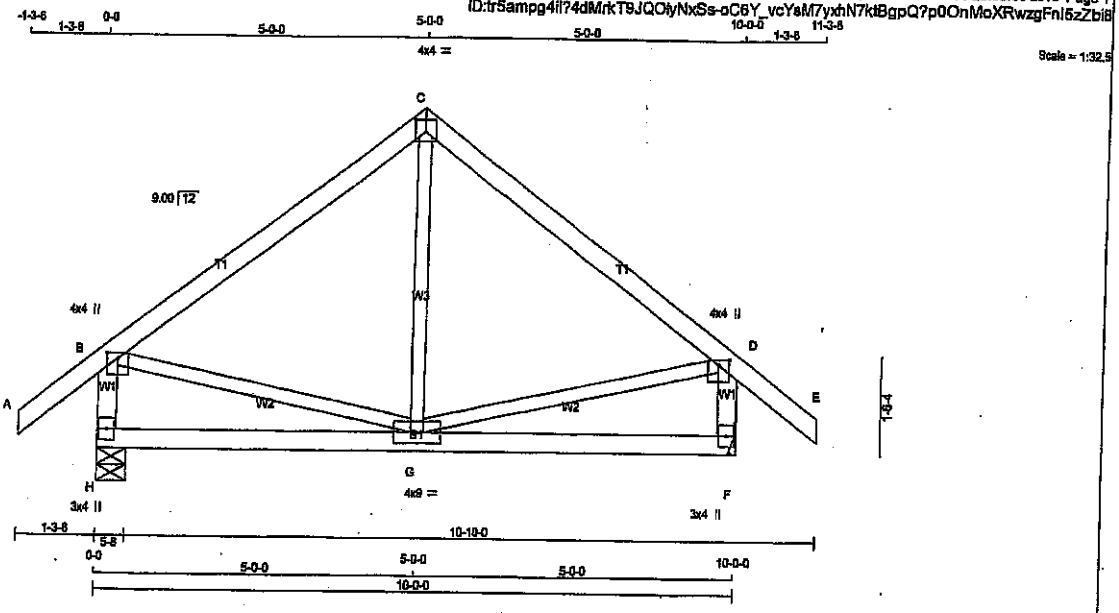


TOTAL WEIGHT = 2 X 44 = 88 lb (MII)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	844	0	844	0	0	5-8	5-8
F	844	0	844	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	622	370 / 0	105 / 0	0 / 0	0 / 0	147 / 0	0 / 0
F	622	370 / 0	105 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-473 / 0	-102.1	-102.1	0.33 (1)	6.25	
C-D	-473 / 0	-102.1	-102.1	0.33 (1)	6.25	
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
H-B	-770 / 0	0.0	0.0	0.08 (1)	7.81	
F-D	-770 / 0	0.0	0.0	0.08 (1)	7.81	
H-G	0 / 0	-38.5	-38.5	0.22 (3)	10.00	
G-F	0 / 0	-38.5	-38.5	0.22 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G-C	0 / 213	0.05 (3)
B-G	0 / 389	0.09 (1)
G-D	0 / 389	0.09 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.33/1.00 (C-D:1) , BC=0.22/1.00 (G-H:3) , WB=0.09/1.00 (B-G:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

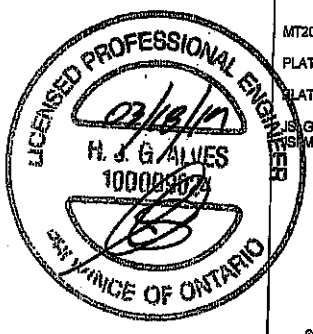
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.72 (D) (INPUT = 0.90)
TS METAL= 0.19 (D) (INPUT = 1.00)



DWG NO. TAM 7905529
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T13	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:53:11 2019 Page 1
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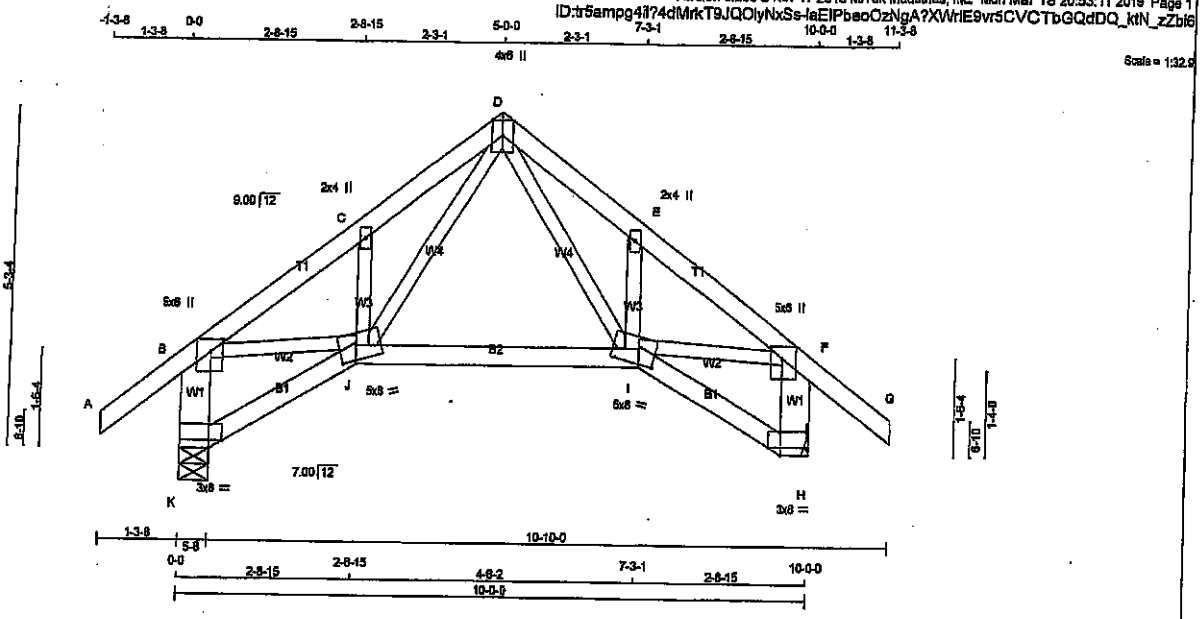

TOTAL WEIGHT = 2 X 48 = 97 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	2.25	2.25
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	5.0	6.0	2.25	2.25
H	BVM1-p	MT20	3.0	8.0	Edge	2.50
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BVM1-p	MT20	3.0	8.0	Edge	2.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
K	844	0	844	0	0	IN-SX	IN-SX
H	844	0	844	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	622	370 / 0	105 / 0	0 / 0	0 / 0	147 / 0	0 / 0
H	622	370 / 0	105 / 0	0 / 0	0 / 0	147 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-938 / 0	-102.1	-102.1	0.10 (1)	6.25
C-D	-966 / 0	-102.1	-102.1	0.06 (1)	6.25
D-E	-966 / 0	-102.1	-102.1	0.06 (1)	6.25
E-F	-938 / 0	-102.1	-102.1	0.10 (1)	6.25
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00
K-B	-791 / 0	0.0	0.0	0.06 (1)	7.81
H-F	-791 / 0	0.0	0.0	0.06 (1)	7.81
K-J	0 / 0	-38.5	-38.5	0.07 (3)	10.00
J-I	0 / 461	-38.5	-38.5	0.23 (2)	10.00
I-H	0 / 0	-38.5	-38.5	0.07 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 566	0.13 (1)
I-E	-298 / 0	0.05 (1)
J-D	0 / 566	0.13 (1)
J-C	-298 / 0	0.05 (1)
B-J	0 / 761	0.17 (1)
I-F	0 / 761	0.17 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.33")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.33")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.14/1.00 (F-G:1) , BC=0.23/1.00 (I-J:2) , WB=0.17/1.00 (B-J:1) , SSI=0.10/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JS GRIP= 0.75 (F) (INPUT = 0.90)
JS METAL= 0.39 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100099274
PROVINCE OF ONTARIO

DWG NO. TAM 7905530
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T14	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:12 2019 Page 1
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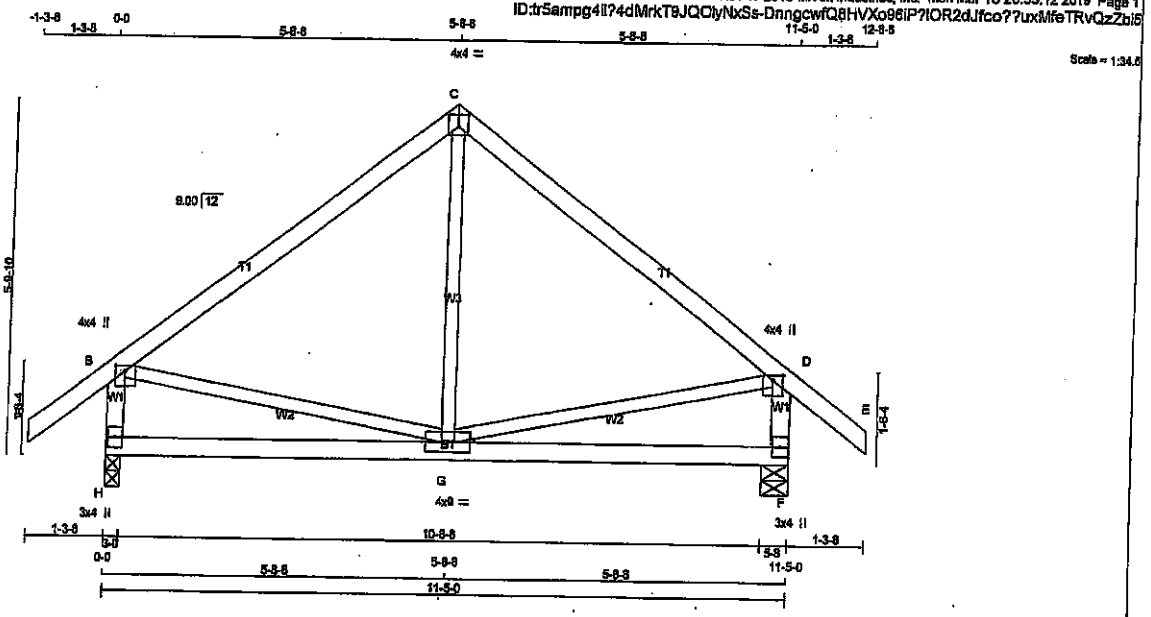

TOTAL WEIGHT = 49 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	4.0	9.0		
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	943	0	943	0	0	3-0	3-0
F	943	0	943	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	696	412 / 0	120 / 0	0 / 0	0 / 0	165 / 0	0 / 0
F	696	412 / 0	120 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14	(1)		10.00
B-C	-557 / 0	-102.1	-102.1	0.43	(1)		6.25
C-D	-557 / 0	-102.1	-102.1	0.43	(1)		6.25
D-E	0 / 42	-102.1	-102.1	0.14	(1)		10.00
H-B	-860 / 0	0.0	0.0	0.09	(1)		7.81
F-D	-860 / 0	0.0	0.0	0.09	(1)		7.81
H-G	0 / 0	-38.5	-38.5	0.29	(3)		10.00
G-F	0 / 0	-38.5	-38.5	0.29	(3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
G-C	0 / 254	0.06	(3)
B-G	0 / 455	0.10	(1)
G-D	0 / 455	0.10	(1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.43/1.00 (C-D:1) , BC=0.29/1.00 (G-H:3) , WB=0.10/1.00 (B-G:1) , SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (B) (INPUT = 0.90)
JSI METAL= 0.22 (B) (INPUT = 1.00)

DWG NO. TAM T1405531
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T15	1	1	Preston 11 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:13 2019 Page 1
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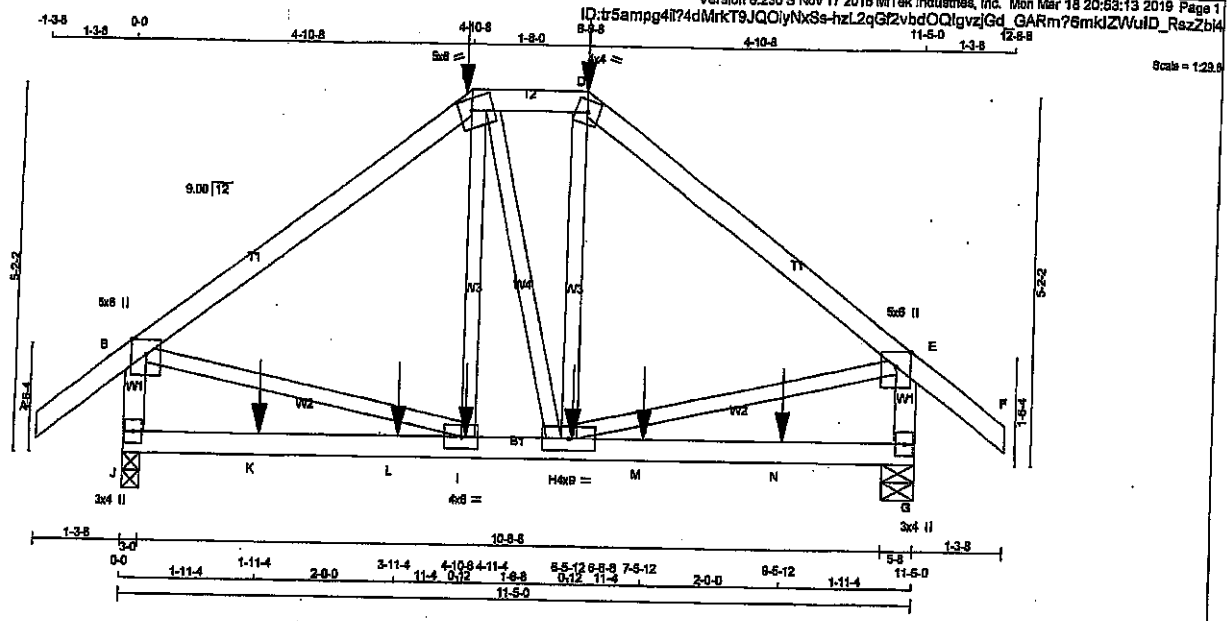

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW-m	MT20	5.0	6.0	2.00	2.25
D	TTW-m	MT20	4.0	4.0		
E	TMVW+p	MT20	5.0	6.0	Edge	
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	4.0	9.0		
I	BMWW-t	MT20	4.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	1558	0	1558	0	0	3-0	3-0
G	1557	0	1557	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	1156	662 / 0	211 / 0	0 / 0	0 / 0	283 / 0	0 / 0
G	1155	662 / 0	210 / 0	0 / 0	0 / 0	283 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.15 (1)	10.00	I-C	0 / 218	0.09 (3)
B-C	-1329 / 0	-102.1	-102.1	0.54 (1)	4.87	C-H	0 / 18	0.00 (3)
C-D	-1064 / 0	-102.1	-102.1	0.07 (1)	8.02	H-D	0 / 234	0.09 (3)
D-E	-1331 / 0	-102.1	-102.1	0.54 (1)	4.88	B-I	0 / 1061	0.27 (1)
E-F	0 / 42	-102.1	-102.1	0.15 (1)	10.00	H-E	0 / 218	0.27 (1)
J-B	-1444 / 0	0.0	0.0	0.17 (1)	6.76			
G-E	-1440 / 0	0.0	0.0	0.16 (1)	6.77			
J-K	0 / 0	-38.5	-38.5	0.29 (3)	10.00			
K-L	0 / 0	-38.5	-38.5	0.29 (3)	10.00			
L-I	0 / 0	-38.5	-38.5	0.29 (3)	10.00			
I-H	0 / 1061	-38.5	-38.5	0.44 (2)	10.00			
H-M	0 / 0	-38.5	-38.5	0.30 (3)	10.00			
M-N	0 / 0	-38.5	-38.5	0.30 (3)	10.00			
N-G	0 / 0	-38.5	-38.5	0.30 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	REEL	DOWN
C	4-10-8	-448	-448	—	BACK	VERT	TOTAL	—	—
D	8-8-8	-448	-448	—	BACK	VERT	TOTAL	—	—
H	6-5-12	-55	-70	—	BACK	VERT	TOTAL	—	—
I	4-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
K	1-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
L	3-11-4	-55	-70	—	BACK	VERT	TOTAL	—	—
M	7-5-12	-55	-70	—	BACK	VERT	TOTAL	—	—
N	9-5-12	-55	-70	—	BACK	VERT	TOTAL	—	—

TOTAL WEIGHT = 54 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.38")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.38")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.54/1.00 (D-E:1) , BC=0.44/1.00 (H-I:2) , WB=0.27/1.00 (E-H:1) , SSI=0.22/1.00 (G-H:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (I) (INPUT = 0.90)
JSI METAL= 0.55 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. ALVES
100009884
PROVINCE OF ONTARIO

DWG NO. TAM T1705532
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T16	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:14 2019 Page 1
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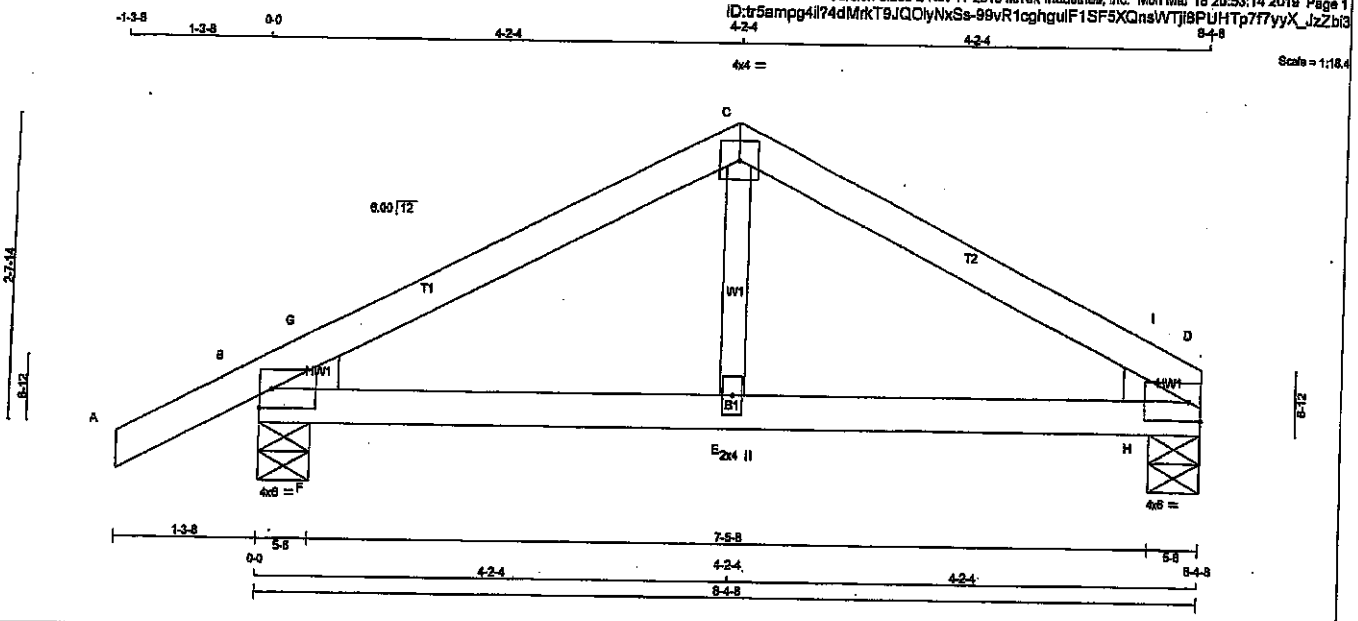

TOTAL WEIGHT = 3 X 25 = 76 lb

[M|F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-I	MT20	4.0	6.0		Edge
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-I	MT20	4.0	6.0		Edge
E	BMW+w	MT20	2.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	727	0	727	0	0	5-8	5-8	2x4 L
D	589	0	589	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	535	322 / 0	88 / 0	0 / 0	0 / 0	125 / 0	0 / 0
D	440	243 / 0	88 / 0	0 / 0	0 / 0	109 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 21	-102.1	-102.1	0.13 (1)	10.00
B- G	-698 / 0	-102.1	-102.1	0.08 (1)	6.25
G- C	-646 / 0	-102.1	-102.1	0.18 (1)	6.25
C- I	-646 / 0	-102.1	-102.1	0.18 (1)	6.25
I- D	-698 / 0	-102.1	-102.1	0.08 (1)	6.25
B- F	0 / 589	-38.5	-38.5	0.19 (1)	10.00
F- E	0 / 589	-38.5	-38.5	0.23 (1)	10.00
E- H	0 / 589	-38.5	-38.5	0.23 (1)	10.00
H- D	0 / 589	-38.5	-38.5	0.19 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- C	0 / 258	0.08 (2)
F- G	-101 / 107	0.00 (1)
H- I	-101 / 107	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.26")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.26")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.18/1.00 (C-I:1) , BC=0.23/1.00 (E-F:1) , WB=0.08/1.00 (C-E:2) , SSI=0.15/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687 ; 788		1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.86 (B) (INPUT = 0.90)
SI METAL= 0.16 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T7705533
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T17G	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:15 2019 Page 1
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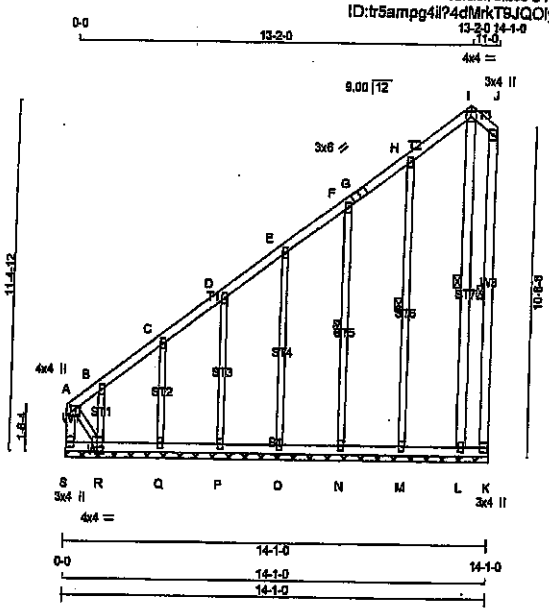

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
S - A	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
EXCEPT ST1	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW+p	MT20	4.0	4.0	1.00	2.00
B, C, D, E, F, H						
B	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
I	TTW-p	MT20	4.0	4.0	2.25	2.00
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, O, P, Q						
L	BMW1+w	MT20	2.0	4.0		
R	BMWW1-t	MT20	4.0	4.0		
S	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K, I-L, H-M, F-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF) CSI (LC)	MAX. UNBRAC LENGTH
FR-TO					
S-A	-72 / 0	0.0	0.0	0.01 (1)	7.81
A-B	-14 / 0	-102.1	-102.1	0.04 (1)	6.25
B-C	-17 / 0	-102.1	-102.1	0.05 (1)	6.25
C-D	-14 / 0	-102.1	-102.1	0.03 (1)	6.25
D-E	-9 / 0	-102.1	-102.1	0.05 (1)	10.00
E-F	-6 / 0	-102.1	-102.1	0.05 (1)	10.00
F-G	0 / 0	-102.1	-102.1	0.05 (1)	10.00
G-H	0 / 0	-102.1	-102.1	0.05 (1)	10.00
H-I	-12 / 0	-102.1	-102.1	0.05 (1)	6.25
I-J	0 / 0	-102.1	-102.1	0.01 (1)	10.00
K-J	-47 / 0	0.0	0.0	0.03 (1)	6.25
S-R	0 / 0	-38.5	-38.5	0.02 (3)	10.00
R-Q	0 / 16	-38.5	-38.5	0.03 (2)	10.00
Q-P	0 / 11	-38.5	-38.5	0.02 (3)	10.00
P-O	0 / 7	-38.5	-38.5	0.02 (3)	10.00
O-N	0 / 4	-38.5	-38.5	0.02 (3)	10.00
N-M	0 / 2	-38.5	-38.5	0.02 (3)	10.00
M-L	0 / 0	-38.5	-38.5	0.02 (3)	10.00
L-K	0 / 0	-38.5	-38.5	0.02 (3)	10.00

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		
L-I	-134 / 0	0.08 (1)
M-H	-230 / 0	0.15 (1)
N-F	-189 / 0	0.09 (1)
O-E	-203 / 0	0.16 (1)
P-D	-209 / 0	0.09 (1)
Q-C	-206 / 0	0.05 (1)
R-B	-171 / 0	0.03 (1)
A-R	0 / 33	0.01 (1)

TOTAL WEIGHT = 90 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (H-I:1) , BC=0.03/1.00 (Q-R:2) , WB=0.16/1.00 (E-O:1) , SSI=0.08/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (H) (INPUT = 0.90)
JSI METAL= 0.12 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100095024
PROVINCE OF ONTARIO

DWG NO. TAM T7905534
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T18	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:17 2019 Page 1
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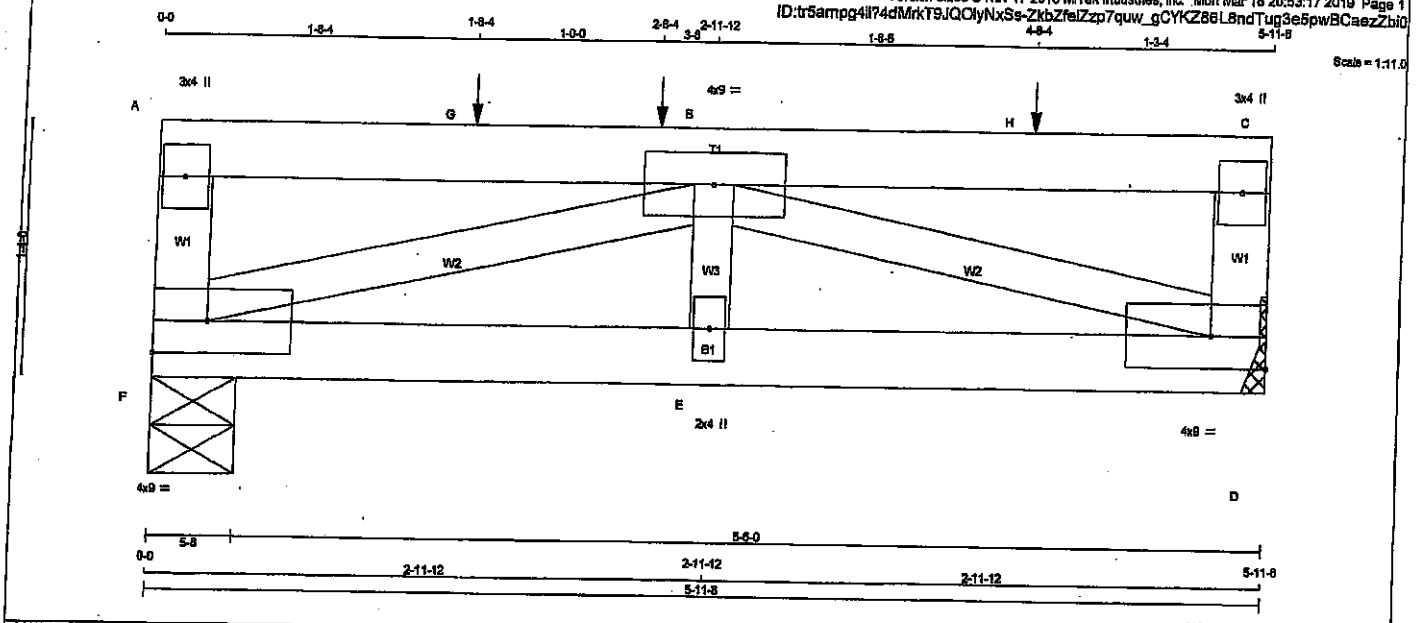

TOTAL WEIGHT = 2 X 20 = 41 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - A	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
F-A	1	12	TOP
A-C	1	3	SIDE(427.3)
C-D	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
F-D	1	12	SIDE(14.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWWW-t	MT20	4.0	9.0		
C	TMV+p	MT20	3.0	4.0		
D	BMVW1-t	MT20	4.0	9.0		Edge
E	BMW+w	MT20	2.0	4.0		
F	BMVW1-t	MT20	4.0	9.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1608	0	1608	0	0	5-8	5-8
D	1965	0	1965	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	1197	654 / 0	208 / 0	0 / 0	0 / 0	338 / 0	0 / 0
D	1467	794 / 0	268 / 0	0 / 0	0 / 0	405 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD FROM	TO	LC1 (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F-A	-204 / 0	0.0	0.0	0.01 (1)		7.81
A-G	0 / 0	-102.1	-102.1	0.16 (1)		10.00
G-B	0 / 0	-102.1	-102.1	0.16 (1)		10.00
B-H	0 / 0	-102.1	-102.1	0.47 (1)		10.00
H-C	0 / 0	-102.1	-102.1	0.47 (1)		10.00
D-C	-561 / 0	0.0	0.0	0.03 (1)		7.81
F-E	0 / 3623	-63.5	-63.5	0.43 (1)		10.00
E-D	0 / 3623	-63.5	-63.5	0.43 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-3636 / 0	0.39 (1)
B-D	-3636 / 0	0.39 (1)
E-B	0 / 215	0.03 (3)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	FEEL	CONN.
B	2-8-4	-1456	-1456	—	FRONT	VERT	TOTAL		
G	1-8-4	-289	-289	—	TOP	VERT	TOTAL		
H	4-8-4	-841	-841	—	TOP	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.43/1.00 (E-F:1) , WB=0.39/1.00 (B-F:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.83 (F) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
03/18/19
H.J. G. ALVES
100098[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM Tm05535
STRUCTURAL
CONFORMANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T19	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:18 2019 Page 1
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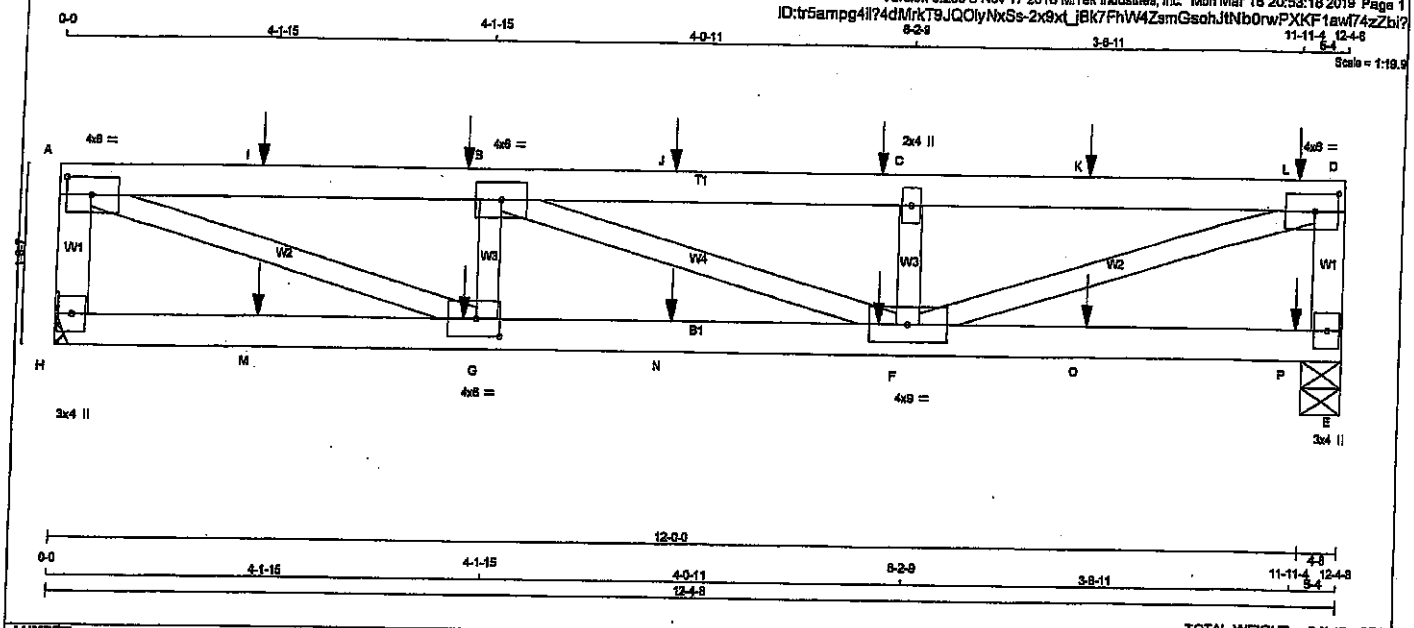

TOTAL WEIGHT = 2 X 42 = 85 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
H - A	2x4	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
H - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
H-A	1	12	TOP
A-D	1	12	SIDE(61.0)
D-E	1	7	SIDE(49.2)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
H-E	1	12	SIDE(61.0)
WEBS : (0.122"X3") SPIRAL NAILS			
B-G	1	6	SIDE(38.9)
C-F	1	6	SIDE(38.9)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	4.0	6.0	2.00	2.75
B	TMWW-t	MT20	4.0	6.0		
C	TMW+w	MT20	2.0	4.0		
D	TMWW-t	MT20	4.0	6.0	2.00	2.75
E	BMV1+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G	BMWW-t	MT20	4.0	6.0	2.00	2.75
H	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
H	1259	0	1259	0	0	MECHANICAL	
E	1417	0	1417	0	0	4-8	4-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	941	520 / 0	188 / 0	0 / 0	0 / 0	233 / 0	0 / 0
E	1059	585 / 0	212 / 0	0 / 0	0 / 0	262 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC. LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-A	-1155 / 0	0.0	0.0	0.07 (1)	7.81	A-G	0 / 2913	0.36 (1)	
A-I	-2759 / 0	-102.1	-102.1	0.21 (1)	5.28	G-B	-640 / 0	0.05 (1)	
I-B	-2759 / 0	-102.1	-102.1	0.21 (1)	5.28	B-F	0 / 32	0.00 (1)	
B-J	-2789 / 0	-102.1	-102.1	0.16 (1)	5.32	F-C	-640 / 0	0.05 (1)	
J-C	-2789 / 0	-102.1	-102.1	0.16 (1)	5.32	F-D	0 / 2913	0.36 (1)	
C-K	-2789 / 0	-102.1	-102.1	0.21 (1)	5.25				
K-L	-2789 / 0	-102.1	-102.1	0.21 (1)	5.25				
L-D	-2789 / 0	-102.1	-102.1	0.21 (1)	5.25				
E-D	-1255 / 0	0.0	0.0	0.07 (1)	7.81				
H-M	0 / 0	-38.5	-38.5	0.10 (2)	10.00				
M-G	0 / 0	-38.5	-38.5	0.10 (2)	10.00				
G-N	0 / 2759	-38.5	-38.5	0.32 (1)	10.00				
N-F	0 / 2759	-38.5	-38.5	0.32 (1)	10.00				
F-O	0 / 0	-38.5	-38.5	0.10 (2)	10.00				
O-P	0 / 0	-38.5	-38.5	0.10 (2)	10.00				
P-E	0 / 0	-38.5	-38.5	0.10 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	TOP
B	3-11-4	-86	-86	—	FRONT	VERT	TOTAL	—	—
C	7-11-4	-86	-86	—	FRONT	VERT	TOTAL	—	—
F	7-11-4	-64	-64	—	FRONT	VERT	TOTAL	—	—
G	3-11-4	-64	-64	—	FRONT	VERT	TOTAL	—	—
I	1-11-4	-86	-86	—	FRONT	VERT	TOTAL	—	—
J	5-11-4	-86	-86	—	FRONT	VERT	TOTAL	—	—
K	9-11-4	-86	-86	—	FRONT	VERT	TOTAL	—	—
L	11-11-4	-114	-114	—	FRONT	VERT	TOTAL	—	—
M	1-11-4	-64	-64	—	FRONT	VERT	TOTAL	—	—
N	5-11-4	-64	-64	—	FRONT	VERT	TOTAL	—	—
O	9-11-4	-64	-64	—	FRONT	VERT	TOTAL	—	—
P	11-11-4	-75	-75	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.41")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.41")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.10")

CSI: TC=0.21/1.00 (C-D:1) , BC=0.32/1.00 (F-G:1) , WB=0.36/1.00 (D-F:1) , SSI=0.14/1.00 (C-D:1)

DOL.LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (F) (INPUT = 0.90)
JSI METAL= 0.35 (D) (INPUT = 1.00)

DWG NO. TAM 1905536
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	T200	1	2	Preston 11 TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Tue Mar 19 09:15:58 2019 Page 1
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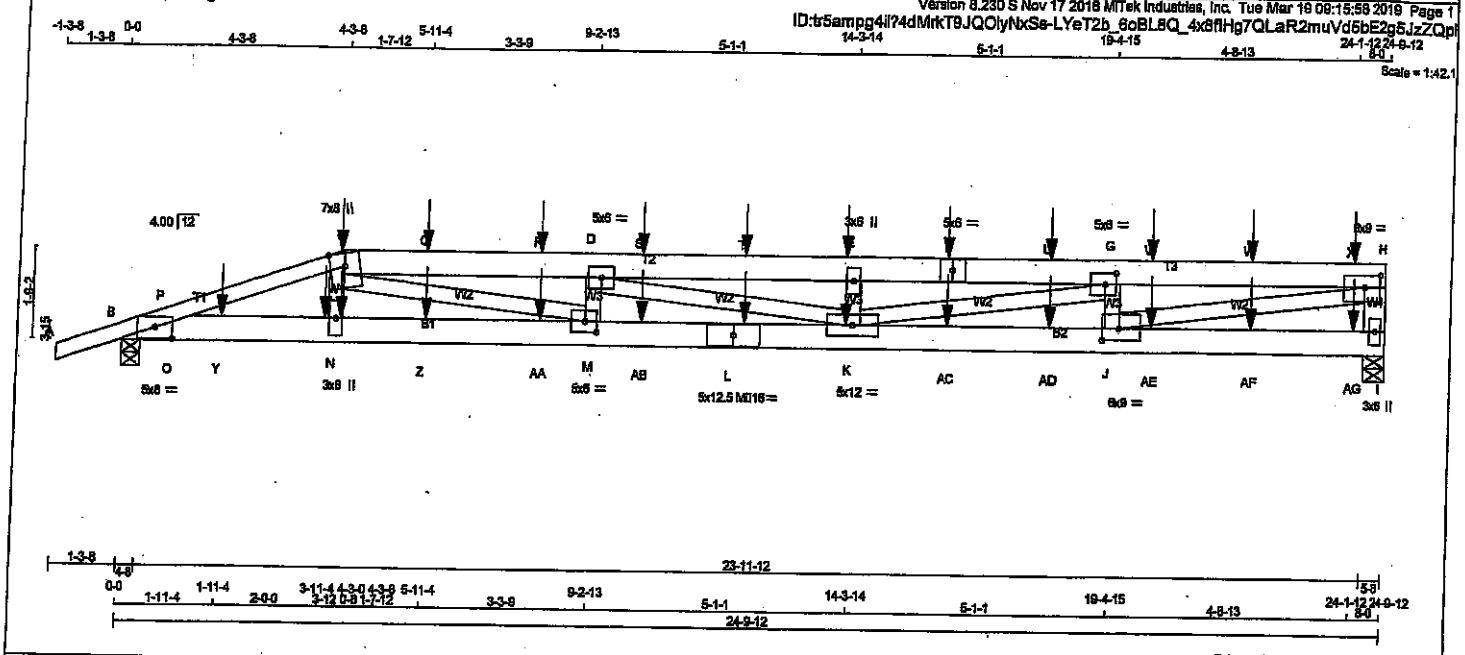

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	1650F 1.5E	SPF
C - F	2x6	DRY	1650F 1.5E	SPF
F - H	2x6	DRY	1650F 1.5E	SPF
I - H	2x6	DRY	No.2	SPF
B - L	2x6	DRY	2100F 1.8E	SPF
L - I	2x6	DRY	2100F 1.8E	SPF
ALL WEBS	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	11	SIDE(61.0)
C-F	2	11	SIDE(244.1)
F-H	2	12	SIDE(183.1)
H-I	2	4	SIDE(48.0)
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-L	2	11	SIDE(263.7)
L-I	2	12	SIDE(197.6)
WEBS : (0.122"X3") SPIRAL NAILS			
N-C	2	2	SIDE(272.2)
K-E	1	4	SIDE(41.9)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	5.0	8.0	2.75	4.00
C	TTWW+m	MT20	7.0	8.0	Edge	
D	TMWW-t	MT20	5.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TMW-t	MT20	6.0	9.0	2.75	4.00
I	BMV1+p	MT20	3.0	6.0		
J	BMWW-t	MT20	6.0	9.0	2.75	4.00
K	BMWWW-t	MT20	5.0	12.0		
L	BS-t	MI16	5.0	12.5		
M	BMWW-t	MT20	5.0	6.0	2.50	2.75
N	BMW+w	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
I	3023	0	3023	0	0	5-8	5-8
B	3909	0	3909	0	0	4-8	4-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	2257	1254 / 0	447 / 0	0 / 0	0 / 0	556 / 0	0 / 0
B	2903	1663 / 0	540 / 0	0 / 0	0 / 0	699 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 22	-102.1	-102.1	0.06 (1)	10.00
B-P	-11356 / 0	-102.1	-102.1	0.20 (1)	3.30
P-C	-11293 / 0	-102.1	-102.1	0.30 (1)	3.23
C-Q	-14487 / 0	-102.1	-102.1	0.29 (1)	3.59
Q-R	-14487 / 0	-102.1	-102.1	0.29 (1)	3.59
R-D	-14487 / 0	-102.1	-102.1	0.29 (1)	3.59
D-S	-14097 / 0	-102.1	-102.1	0.32 (1)	3.60
S-T	-14097 / 0	-102.1	-102.1	0.32 (1)	3.60
T-E	-14097 / 0	-102.1	-102.1	0.32 (1)	3.60
E-F	-14097 / 0	-102.1	-102.1	0.30 (1)	3.63
F-U	-14097 / 0	-102.1	-102.1	0.30 (1)	3.63
U-G	-14097 / 0	-102.1	-102.1	0.30 (1)	3.63
G-V	-9835 / 0	-102.1	-102.1	0.22 (1)	4.33
V-W	-9835 / 0	-102.1	-102.1	0.22 (1)	4.33
W-X	-9835 / 0	-102.1	-102.1	0.22 (1)	4.33
X-H	-9835 / 0	-102.1	-102.1	0.22 (1)	4.33
I-H	-2752 / 0	0.0	0.0	0.10 (1)	7.81
B-O	0 / 10736	-38.5	-38.5	0.41 (1)	10.00
O-Y	0 / 10756	-38.5	-38.5	0.41 (1)	10.00
Y-N	0 / 10756	-38.5	-38.5	0.41 (1)	10.00
N-Z	0 / 10574	-38.5	-38.5	0.39 (1)	10.00
Z-AA	0 / 10574	-38.5	-38.5	0.39 (1)	10.00
AA-M	0 / 10574	-38.5	-38.5	0.39 (1)	10.00
M-AB	0 / 14487	-38.5	-38.5	0.52 (1)	10.00
AB-L	0 / 14487	-38.5	-38.5	0.52 (1)	10.00
L-K	0 / 14487	-38.5	-38.5	0.52 (1)	10.00
K-AC	0 / 9635	-38.5	-38.5	0.38 (1)	10.00
AC-AD	0 / 9635	-38.5	-38.5	0.38 (1)	10.00
AD-J	0 / 9635	-38.5	-38.5	0.38 (1)	10.00
J-AE	0 / 0	-38.5	-38.5	0.09 (1)	10.00
AE-AF	0 / 0	-38.5	-38.5	0.09 (1)	10.00
AF-AG	0 / 0	-38.5	-38.5	0.09 (1)	10.00
AG-I	0 / 0	-38.5	-38.5	0.09 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
N-C	0 / 1882	0.15 (1)
J-H	0 / 9910	0.88 (1)
C-M	0 / 4054	0.36 (1)
J-G	-1967 / 0	0.11 (1)
M-D	-713 / 0	0.04 (1)
K-G	0 / 4605	0.41 (1)
D-K	-403 / 0	0.07 (1)
K-E	-715 / 0	0.04 (1)
O-P	-11 / 86	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-3-8	-282	-282	—	BACK	VERT	TOTAL	—	—
E	14-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
F	16-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—

TOTAL WEIGHT = 2 X 116 = 232 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.83")
CALCULATED VERT. DEFL.(LL) = L/ 637 (0.47")
ALLOWABLE DEFL.(TL)= L/360 (0.83")
CALCULATED VERT. DEFL.(TL) = L/ 385 (0.77")

CSI: TC=0.32/1.00 (D-E:1) , BC=0.52/1.00 (K-M:1) , WB=0.88/1.00 (H-J:1) , SSI=0.22/1.00 (B-O:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656
MI16	438	276	2341	1245	4283	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.90 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/19/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T190518
STRUCTURAL COMPONENT ONLY
1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	T200	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:tr5ampg4jl?4dMrkT9JQOlyNxSs-LYeT2b_6oBL8Q_4x8flHg7QLaR2muVd5bE2g5JzZQpl

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
K	14-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
L	12-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
N	3-11-4	-57	-57	—	BACK	VERT	TOTAL	—	—
N	4-3-0	-1221	-1221	—	FRONT	VERT	TOTAL	—	—
Q	5-11-4	-93	-93	—	BACK	VERT	TOTAL	—	—
R	8-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
S	10-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
T	12-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
U	18-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
V	20-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
W	22-2-4	-93	-93	—	BACK	VERT	TOTAL	—	—
X	24-1-12	-110	-110	—	BACK	VERT	TOTAL	—	—
Y	1-11-4	-112	-112	—	BACK	VERT	TOTAL	—	—
Z	5-11-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AA	8-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AB	10-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AC	16-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AD	18-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AE	20-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AF	22-2-4	-68	-68	—	BACK	VERT	TOTAL	—	—
AG	24-1-12	-74	-74	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/14/19
H. G. ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM T905619
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	V1	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:27 2019 Page 1
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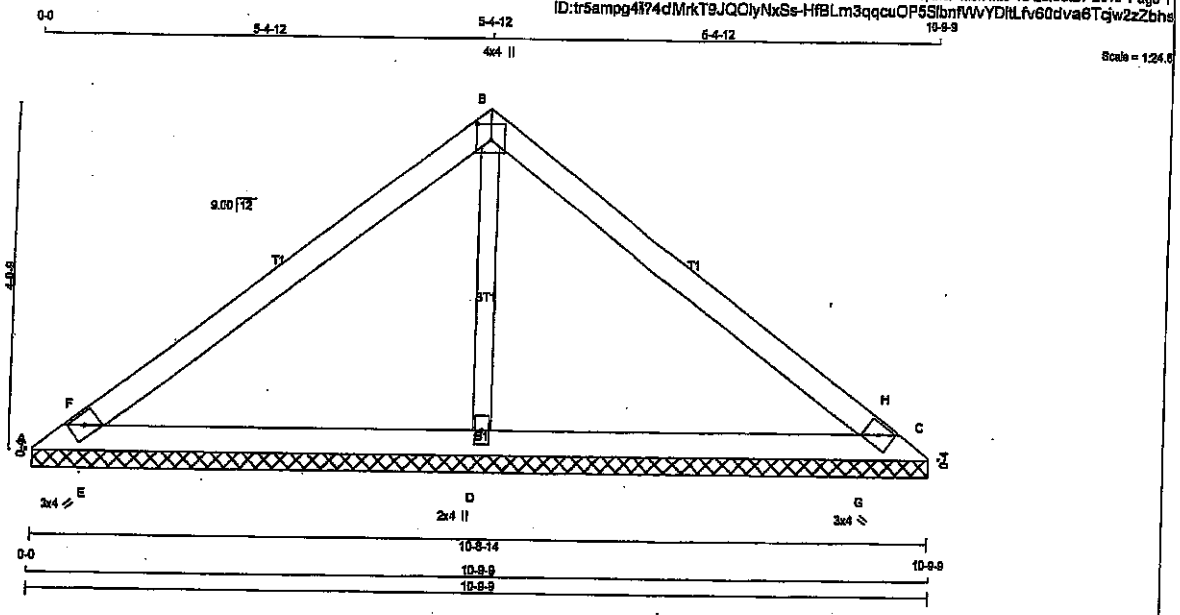


TOTAL WEIGHT = 29 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW+p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	5	0	5	0	0	10-8-14	10-8-14
C	5	0	5	0	0	10-8-14	10-8-14
D	1500	0	1500	0	0	10-8-14	10-8-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	5	0 / -1	3 / 0	0 / 0	0 / 0	2 / 0	0 / 0
C	5	0 / -1	3 / 0	0 / 0	0 / 0	2 / 0	0 / 0
D	1119	625 / 0	219 / 0	0 / 0	0 / 0	275 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

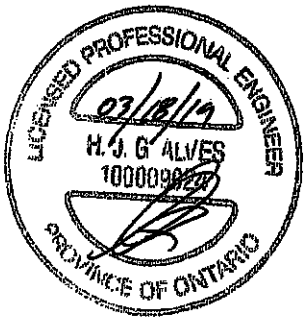
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI	LC	MAX. UNBRAC LENGTH
A-F	0 / 606	-102.1	-102.1	0.24	(1)	10.00
F-B	0 / 584	-102.1	-102.1	0.40	(1)	10.00
B-H	0 / 584	-102.1	-102.1	0.40	(1)	10.00
H-C	0 / 606	-102.1	-102.1	0.24	(1)	10.00
A-E	-537 / 0	-38.5	-38.5	0.25	(1)	6.25
E-D	-478 / 0	-38.5	-38.5	0.31	(1)	6.25
D-G	-478 / 0	-38.5	-38.5	0.31	(1)	6.25
G-C	-537 / 0	-38.5	-38.5	0.25	(1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	LC
D-B	-1147 / 0	0.28	(1)
E-F	-331 / 31	0.00	(1)
G-H	-331 / 31	0.00	(1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 **IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.40/1.00 (B-H:1) , BC=0.31/1.00 (D-G:1) , WB=0.28/1.00 (B-D:1) , SSI=0.20/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI)		SHEAR (PLI)		SECTION (PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	818	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.82 (B) (INPUT = 0.90)
JSI METAL= 0.28 (B) (INPUT = 1.00)

ENGINEERING BY T1905548
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	V2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:53:28 2019 Page 1
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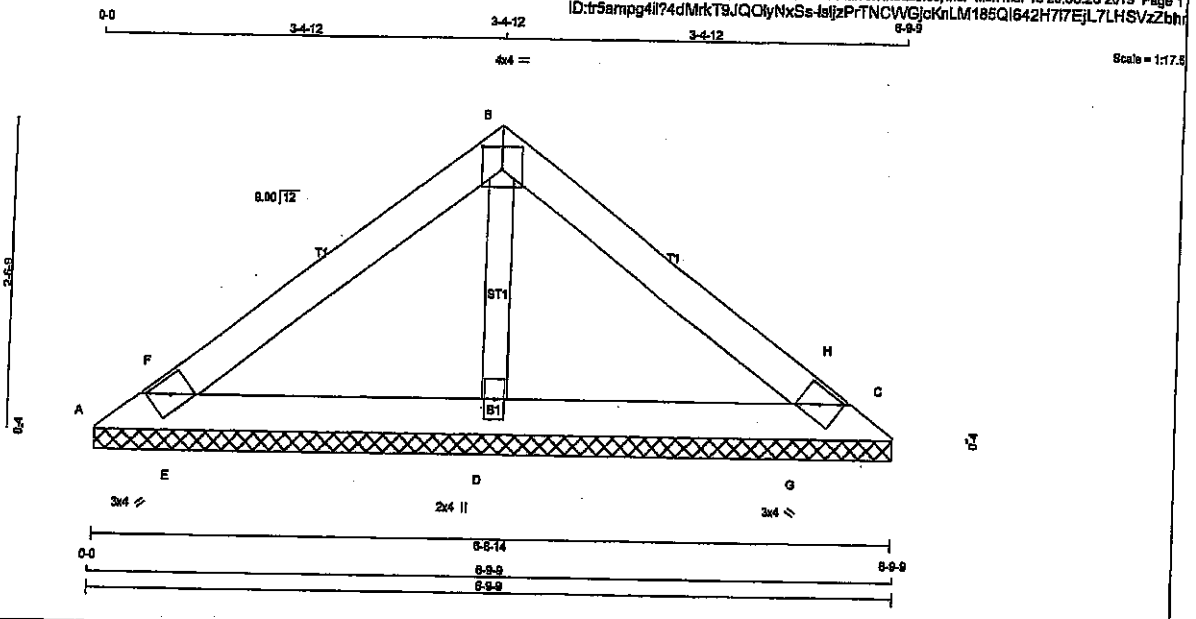

TOTAL WEIGHT = 18 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	94	0	94	0	0	6-8-14	6-8-14
C	94	0	94	0	0	6-8-14	6-8-14
D	760	0	760	0	0	6-8-14	6-8-14

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	69	40 / 0	13 / 0	0 / 0	0 / 0	17 / 0	0 / 0
C	69	40 / 0	13 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	569	312 / 0	116 / 0	0 / 0	0 / 0	142 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A-F	0 / 205	-102.1	-102.1	0.05 (1)	10.00
F-B	0 / 214	-102.1	-102.1	0.15 (1)	10.00
B-H	0 / 214	-102.1	-102.1	0.15 (1)	10.00
H-C	0 / 205	-102.1	-102.1	0.05 (1)	10.00
A-E	-200 / 0	-38.5	-38.5	0.12 (1)	6.25
E-D	-174 / 0	-38.5	-38.5	0.13 (1)	6.25
D-G	-174 / 0	-38.5	-38.5	0.13 (1)	6.25
G-C	-200 / 0	-38.5	-38.5	0.12 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-B	-520 / 0	0.08 (1)
E-F	-158 / 13	0.00 (1)
G-H	-158 / 13	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (B-H:1) , BC=0.13/1.00 (D-E:1) , WB=0.08/1.00 (B-D:1) , SSI=0.10/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1687	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.47 (B) (INPUT = 0.90)
JSI METAL= 0.15 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. GALVES
100[illegible]
PROVINCE OF ONTARIO

BRG NO. TAM 7905549
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	PB1	1	1	TRUSS DESC.	

Tamarack Roof Trues, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:50 2019 Page 1
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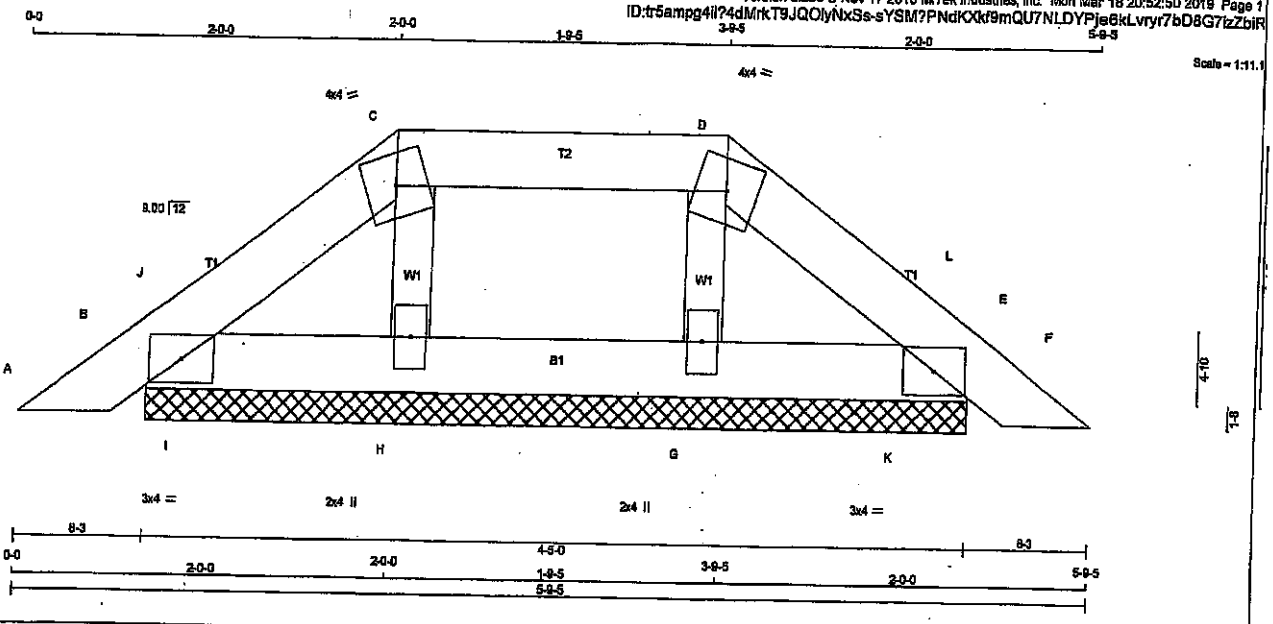

TOTAL WEIGHT = 14 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	176	0	176	0	0	4-5-0	4-5-0
E	176	0	176	0	0	4-5-0	4-5-0
H	195	0	195	0	0	4-5-0	4-5-0
G	195	0	195	0	0	4-5-0	4-5-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	126	67 / 0	13 / 0	0 / 0	0 / 0	26 / 0	0 / 0
E	126	67 / 0	13 / 0	0 / 0	0 / 0	26 / 0	0 / 0
H	148	76 / 0	34 / 0	0 / 0	0 / 0	38 / 0	0 / 0
G	148	76 / 0	34 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B-J	-51 / 0	-102.1	-102.1	0.00 (3)	6.25
J-C	-43 / 0	-102.1	-102.1	0.02 (1)	6.25
C-D	-23 / 0	-102.1	-102.1	0.05 (1)	6.25
D-L	-43 / 0	-102.1	-102.1	0.02 (1)	6.25
L-E	-51 / 0	-102.1	-102.1	0.00 (3)	6.25
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B-I	0 / 34	-38.5	-38.5	0.03 (1)	10.00
I-H	0 / 34	-38.5	-38.5	0.03 (1)	10.00
H-G	0 / 23	-38.5	-38.5	0.02 (2)	10.00
G-K	0 / 34	-38.5	-38.5	0.03 (1)	10.00
K-E	0 / 34	-38.5	-38.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H-C	-120 / 0	0.02 (1)
G-D	-120 / 0	0.02 (1)
I-J	-54 / 7	0.00 (1)
K-L	-54 / 7	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (C-D:1) , BC=0.03/1.00 (B-I:1) , WB=0.02/1.00 (C-H:1) , SSI=0.07/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (B) (INPUT = 0.90)
JSI METAL= 0.03 (B) (INPUT = 1.00)

DWG NO. TAM T1905543
STRUCTURAL
CHECK OF KEY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	PB3	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:51 2019 Page 1

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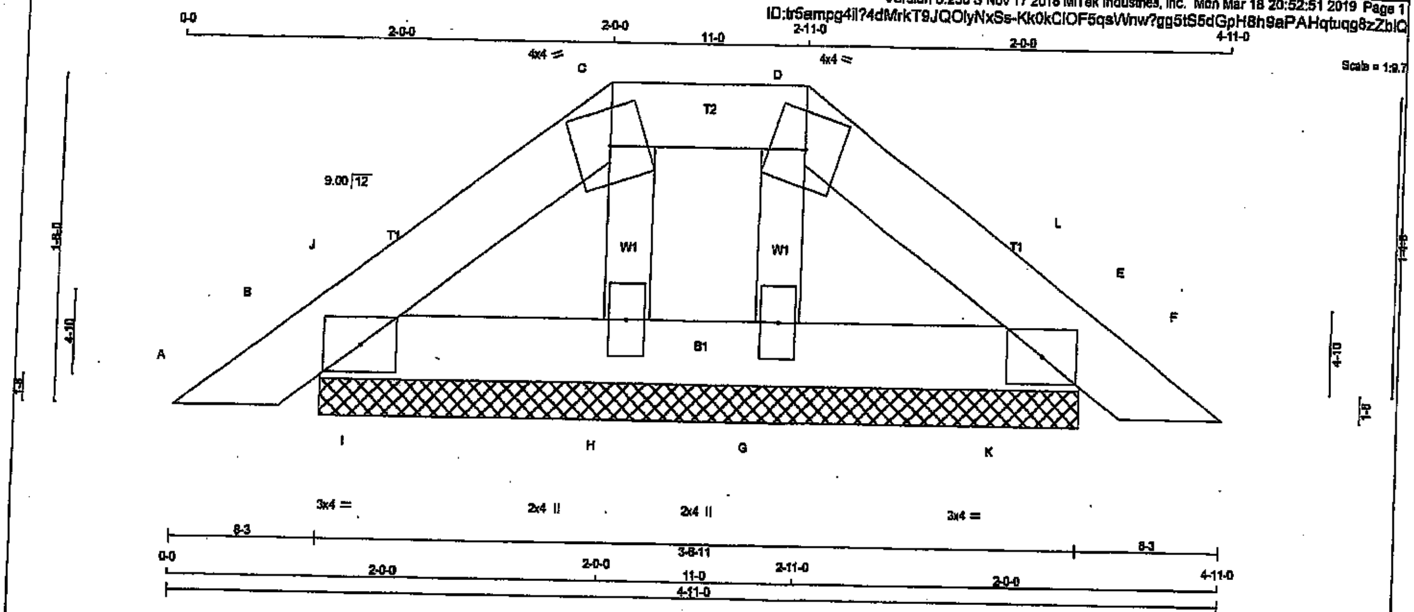

TOTAL WEIGHT = 12 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
B - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB1-I	MT20	3.0	4.0		
G	BMW1+w	MT20	2.0	4.0		
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	172	0	172	0	0	3-6-11	3-6-11
E	172	0	172	0	0	3-6-11	3-6-11
H	138	0	138	0	0	3-6-11	3-6-11
G	138	0	138	0	0	3-6-11	3-6-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	124	83 / 0	14 / 0	0 / 0	0 / 0	27 / 0	0 / 0
E	124	83 / 0	14 / 0	0 / 0	0 / 0	27 / 0	0 / 0
H	104	54 / 0	23 / 0	0 / 0	0 / 0	27 / 0	0 / 0
G	104	54 / 0	23 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- J	-46 / 0	-102.1	-102.1	0.01 (3)	6.25
J- C	-35 / 0	-102.1	-102.1	0.02 (1)	6.25
C- D	-21 / 0	-102.1	-102.1	0.01 (1)	6.25
D- L	-35 / 0	-102.1	-102.1	0.02 (1)	6.25
L- E	-46 / 0	-102.1	-102.1	0.01 (3)	6.25
E- F	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- I	0 / 29	-38.5	-38.5	0.03 (1)	10.00
I- H	0 / 29	-38.5	-38.5	0.03 (1)	10.00
H- G	0 / 21	-38.5	-38.5	0.02 (1)	10.00
G- K	0 / 29	-38.5	-38.5	0.03 (1)	10.00
K- E	0 / 29	-38.5	-38.5	0.03 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
H- C	-80 / 0	0.01 (1)
G- D	-80 / 0	0.01 (1)
I- J	-52 / 11	0.00 (1)
K- L	-52 / 11	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (E-F:1) , BC=0.03/1.00 (G-K:1) , WB=0.01/1.00 (D-G:1) , SSI=0.05/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.12 (E) (INPUT = 0.90)
JSI METAL= 0.03 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

BWG KU. 1AM 7705544
STRUCTURAL
[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	PB4	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:52 2019 Page 1
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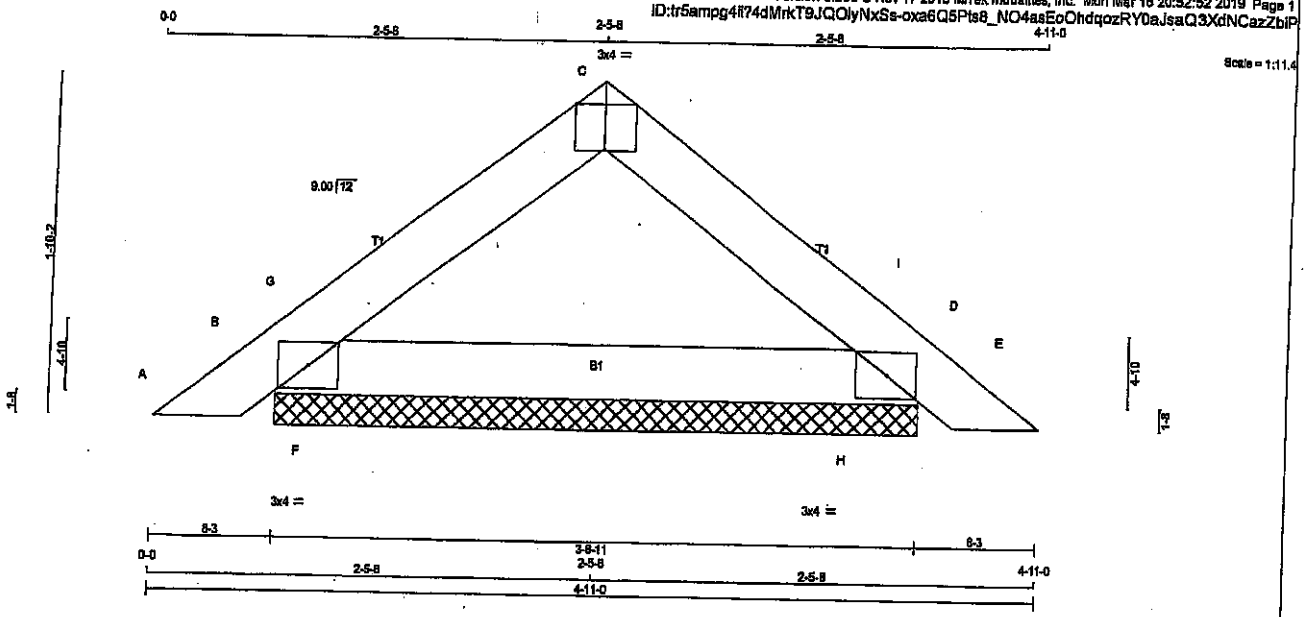


Scale = 1:11.4

TOTAL WEIGHT = 6 X 11 = 66 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
B	311	0	311	0	0	3-6-11	3-6-11
D	311	0	311	0	0	3-6-11	3-6-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	228	136 / 0	37 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	228	136 / 0	37 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.03 (1)		10.00
B-G	-262 / 0	-102.1	-102.1	0.05 (3)		6.25
G-C	-151 / 0	-102.1	-102.1	0.06 (1)		6.25
C-I	-151 / 0	-102.1	-102.1	0.06 (1)		6.25
I-D	-262 / 0	-102.1	-102.1	0.05 (3)		6.25
D-E	0 / 17	-102.1	-102.1	0.03 (1)		10.00
B-F	0 / 130	-38.5	-38.5	0.04 (3)		10.00
F-H	0 / 130	-38.5	-38.5	0.06 (2)		10.00
H-D	0 / 130	-38.5	-38.5	0.04 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-G	0 / 148	0.00 (1)
H-I	0 / 148	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-G:1) , BC=0.08/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.08/1.00 (D-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

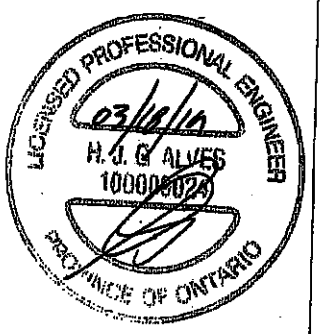
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. TAM 7905545
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	PB4Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:54 2019 Page 1
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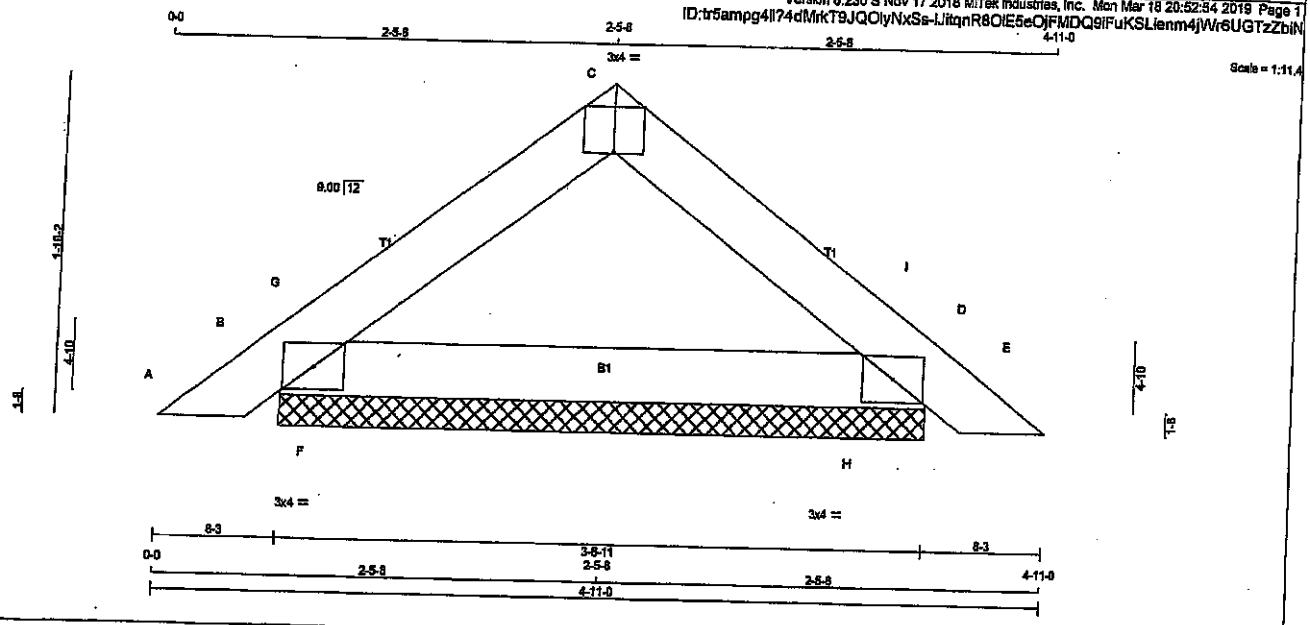

TOTAL WEIGHT = 2 X 11 = 22 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4.	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
B-D	1	12	TOP

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	311	0	311	0	0	3-8-11	3-8-11
D	311	0	311	0	0	3-8-11	3-8-11

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	228	138 / 0	37 / 0	0 / 0	0 / 0	53 / 0	0 / 0
D	228	138 / 0	37 / 0	0 / 0	0 / 0	53 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX (PLF)	CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 17	-102.1	-102.1	0.01 (1)		10.00
B-G	-261 / 0	-102.1	-102.1	0.03 (3)		6.25
G-C	-150 / 0	-102.1	-102.1	0.03 (1)		6.25
C-I	-150 / 0	-102.1	-102.1	0.03 (1)		6.25
I-D	-261 / 0	-102.1	-102.1	0.03 (3)		6.25
D-E	0 / 17	-102.1	-102.1	0.01 (1)		10.00
B-F	0 / 130	-38.5	-38.5	0.02 (3)		10.00
F-H	0 / 130	-38.5	-38.5	0.04 (2)		10.00
H-D	0 / 130	-38.5	-38.5	0.02 (3)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-G	0 / 147	0.00 (1)
H-I	0 / 147	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.03/1.00 (C-I:1) , BC=0.04/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.04/1.00 (D-I:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (D) (INPUT = 0.90)
JSI METAL= 0.03 (D) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009029
PROVINCE OF ONTARIO

DWG NO. TAM T7905546
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	PB5	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:55 2019 Page 1
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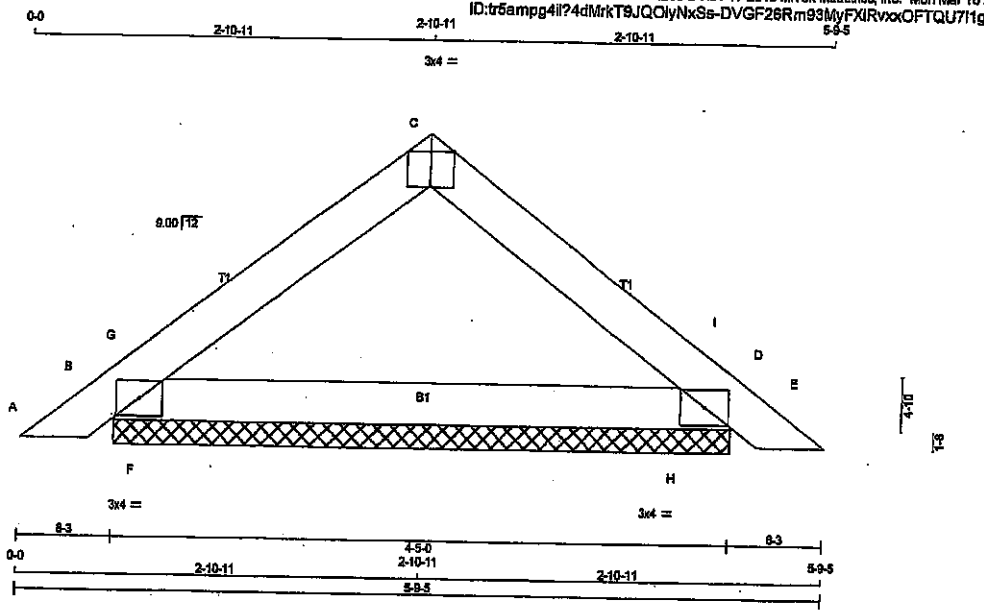


TOTAL WEIGHT = 4 X 13 = 53 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	371	0	371	0	0	4-5-0	4-5-0
D	371	0	371	0	0	4-5-0	4-5-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	274	163 / 0	46 / 0	0 / 0	0 / 0	65 / 0	0 / 0
D	274	163 / 0	46 / 0	0 / 0	0 / 0	65 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- G	-342 / 0	-102.1	-102.1	0.08 (3)	6.25
G- C	-190 / 0	-102.1	-102.1	0.10 (1)	6.25
C- I	-190 / 0	-102.1	-102.1	0.10 (1)	6.25
I- D	-342 / 0	-102.1	-102.1	0.08 (3)	6.25
D- E	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- F	0 / 163	-38.5	-38.5	0.06 (3)	10.00
F- H	0 / 163	-38.5	-38.5	0.12 (2)	10.00
H- D	0 / 163	-38.5	-38.5	0.08 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- G	0 / 223	0.00 (1)
H- I	0 / 223	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.10/1.00 (C-G:1) , BC=0.12/1.00 (F-H:2) , WB=0.00/1.00 (F-G:1) , SSI=0.12/1.00 (D-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

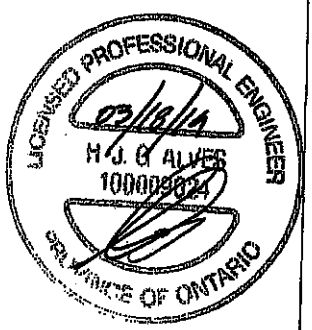
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.07 (D) (INPUT = 1.00)



BMS NO: TAM 1905547
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J1	15	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:33 2019 Page 1
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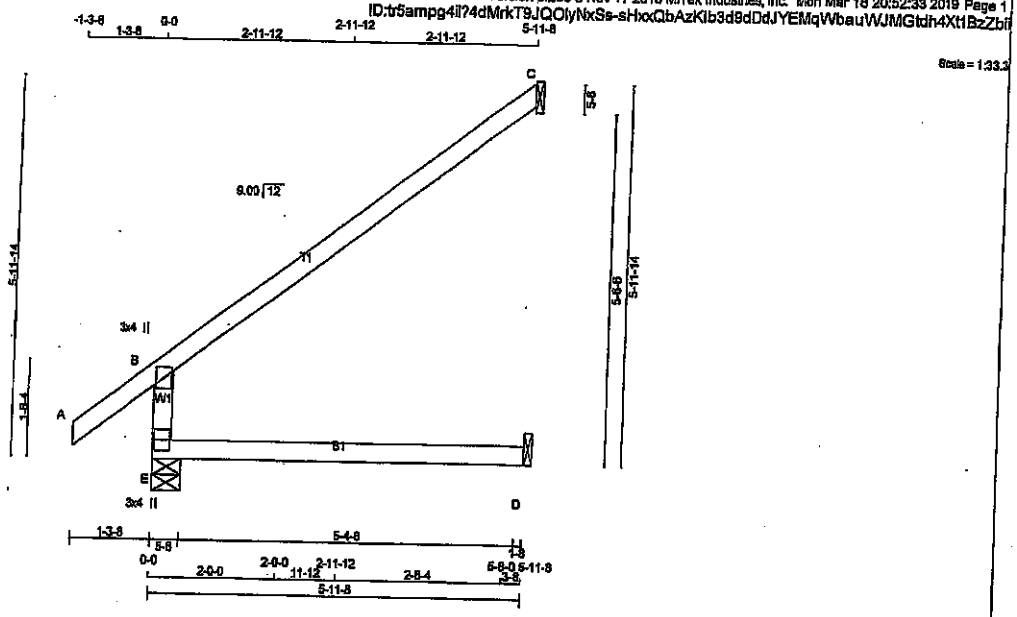


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 15 X 19 = 279 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	581	0	581	0	0	5-8	5-8
C	228	0	228	0	0	1-8	1-8
D	44	0	49	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	406	295 / 0	0 / 0	0 / 0	0 / 0	109 / 0	0 / 0
C	157	130 / 0	0 / 0	0 / 0	0 / 0	27 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-521 / 0	0.0	0.0	0.11	(4)	7.81			
A-B	0 / 42	-102.1	-102.1	0.14	(1)	10.00			
B-C	-46 / 0	-102.1	-102.1	0.62	(1)	6.25			
E-D	0 / 0	-17.5	-17.5	0.13	(4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.0 PSF
TOTAL LOAD = 42.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.62/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

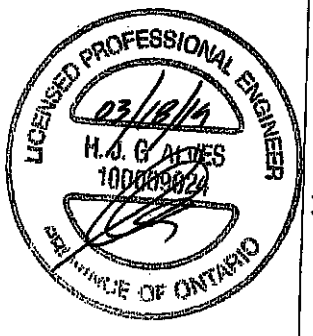
NAIL VALUES

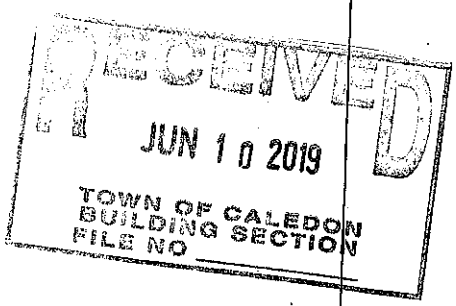
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1657	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL= 0.18 (B) (INPUT = 1.00)





BRS P.O. TAM 7905550
STRUCTURAL
CHECKSHEET ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:34 2019 Page 1
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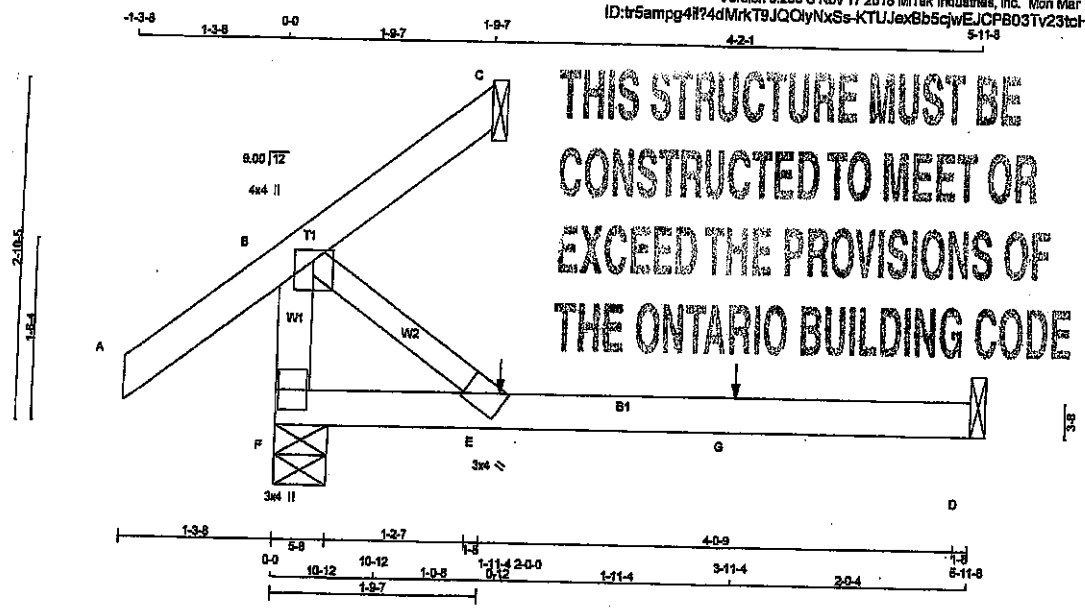

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0	2.00	1.25
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 14 = 29 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	401	0	401	0	0	5-8	5-8
C	37	0	37	0	0	1-8	1-8
D	115	0	146	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	301	163 / 0	63 / 0	0 / 0	0 / 0	75 / 0	0 / 0
C	26	21 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	104	0 / 0	63 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-286 / 0	0.0	0.0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 42	-102.1	-102.1	0.15 (1)	10.00				
B-C	-33 / 0	-102.1	-102.1	0.15 (1)	6.25				
F-E	0 / 0	-38.5	-38.5	0.28 (2)	10.00				
E-G	0 / 0	-38.5	-38.5	0.33 (2)	10.00				
G-D	0 / 0	-38.5	-38.5	0.33 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	1	1	—	FRONT	VERT	TOTAL	—	—
G	3-11-4	1	1	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100000823
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 887 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 532 (0.13")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.33/1.00 (D-E:2) , WB=0.00/1.00 (B-E:1) , SSI=0.13/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

DWG NO. TAM 7705551
STRUCTURAL
CONCURRENCE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:36 2019 Page 1
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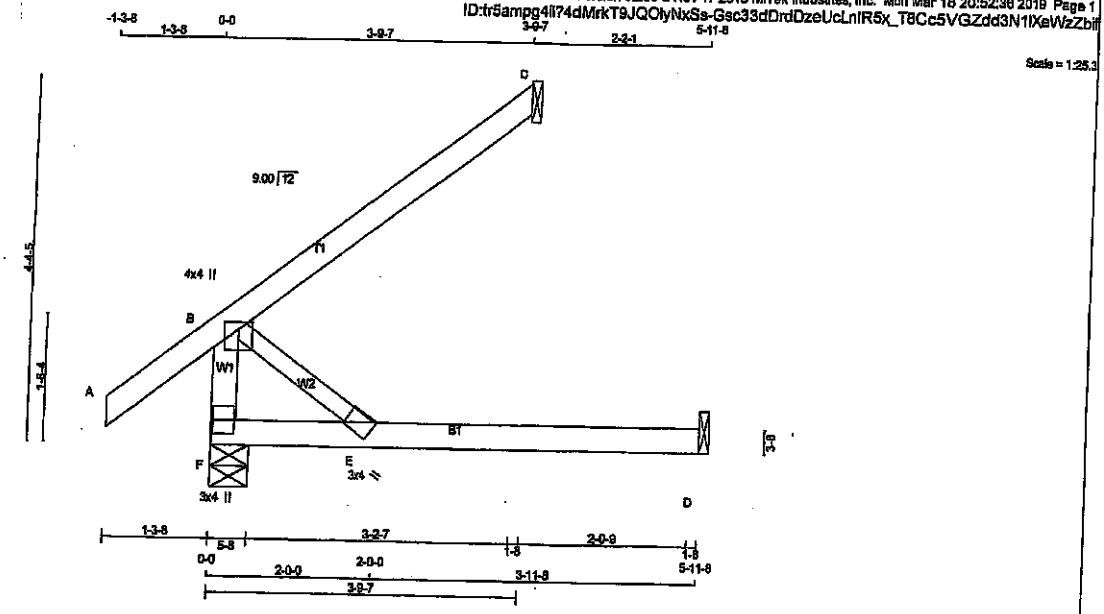

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	449	0	449	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	115	0	146	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	334	190 / 0	63 / 0	0 / 0	0 / 0	81 / 0	0 / 0
C	133	110 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	104	0 / 0	63 / 0	0 / 0	0 / 0	42 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-334 / 0	0.0	0.0	0.03	(1)	7.81	B- E	0 / 0	0.00 (1)
A- B	0 / 42	-102.1	-102.1	0.14	(1)	10.00			
B- C	0 / 0	-102.1	-102.1	0.25	(1)	10.00			
F- E	0 / 0	-38.5	-38.5	0.26	(3)	10.00			
E- D	0 / 0	-38.5	-38.5	0.31	(3)	10.00			

TOTAL WEIGHT = 2 X 17 = 34 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 667 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 532 (0.13")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.31/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

ENG. I.D. TAM 7905552
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J4	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:37 2019 Page 1
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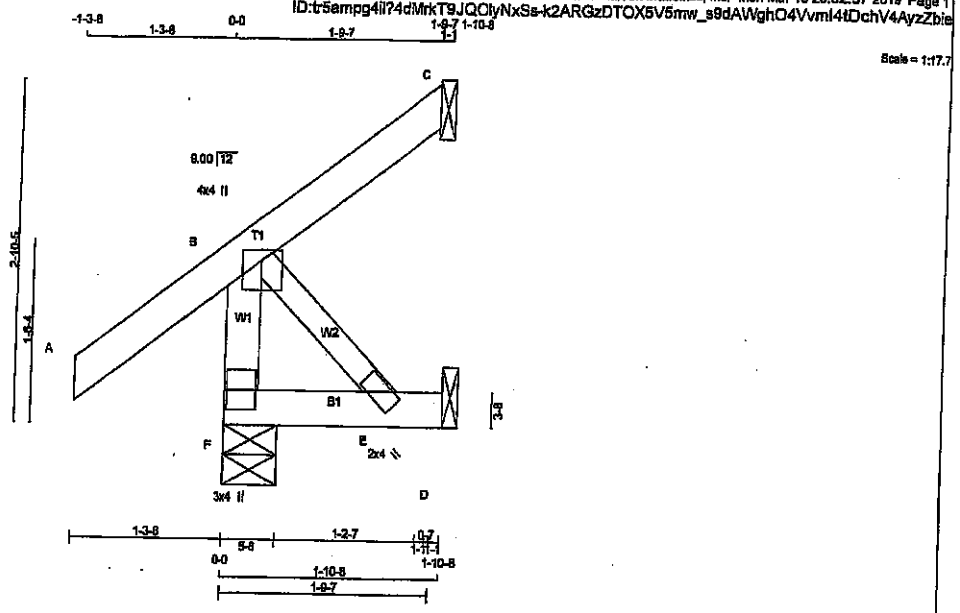


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	322	0	322	0	0	5-8	5-8
C	37	0	37	0	-45	1-8	1-8
D	36	0	46	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL 837821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	229	163 / 0	20 / 0	0 / 0	0 / 0	47 / 0	0 / 0
C	25	21 / -31	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	33	0 / 0	20 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

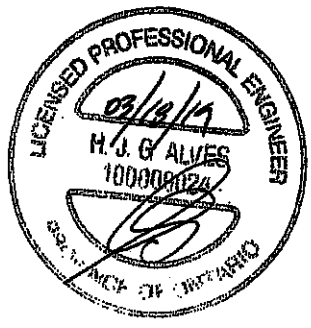
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-288 / 0	0.0	0.0	0.03 (1)		7.81
A- B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B- C	-33 / 0	-102.1	-102.1	0.13 (1)		6.25
F- E	0 / 0	-38.5	-38.5	0.03 (3)		10.00
E- D	0 / 0	-38.5	-38.5	0.03 (3)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 4 X 9 = 37 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.03/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1657	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

REGISTRATION 7905553
STRUCTURAL
CERTIFICATE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J5	4	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:39 2019 Page 1
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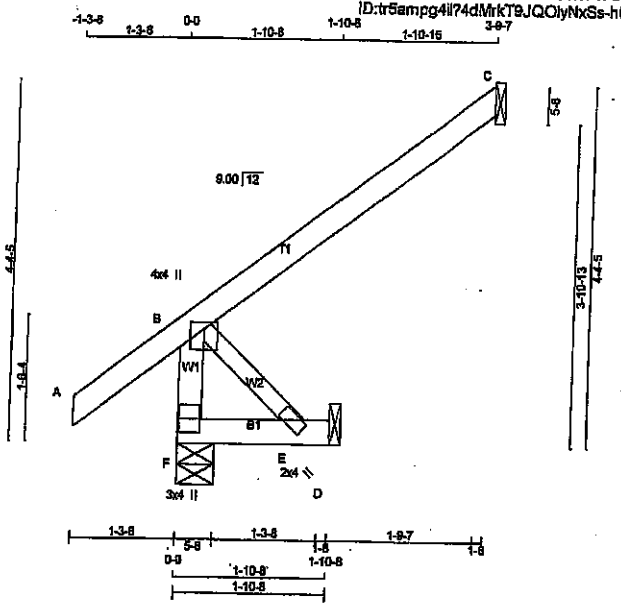

TOTAL WEIGHT = 4 X 12 = 49 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	370	0	370	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	36	0	46	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	262	190 / 0	20 / 0	0 / 0	0 / 0	62 / 0	0 / 0
C	133	110 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	33	0 / 0	20 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-B	-334 / 0	0.0	0.0	0	0.03 (1)	7.81	B-E	0 / 0	0.00 (1)
A-B	0 / 42	-102.1	-102.1	1	0.14 (1)	10.00			
B-C	0 / 0	-102.1	-102.1	1	0.25 (1)	10.00			
F-E	0 / 0	-38.5	-38.5	0	0.04 (3)	10.00			
E-D	0 / 0	-38.5	-38.5	0	0.03 (3)	10.00			

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA O86-09, CSA O86-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.04/1.00 (E-F:3) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 1.00)

BCIN NO. TAM TM05554
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J6	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:39 2019 Page 1
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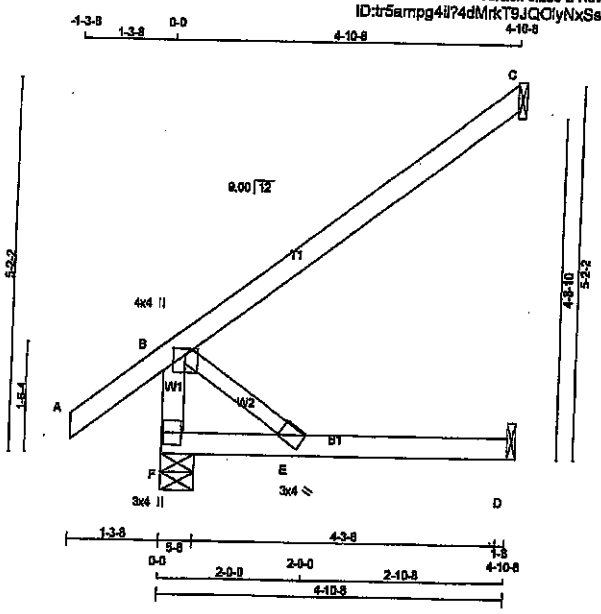

TOTAL WEIGHT = 2 X 17 = 35 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	484	0	484	0	0	5-8	5-8
C	249	0	249	0	0	1-8	1-8
D	84	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	353	222 / 0	51 / 0	0 / 0	0 / 0	80 / 0	0 / 0
C	171	142 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F- B	-390 / 0	0.0	0.0	0.04 (1)	7.81	B- E	0 / 0		0.00 (1)
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00				
B- C	0 / 0	-102.1	-102.1	0.41 (1)	10.00				
F- E	0 / 0	-38.5	-38.5	0.20 (3)	10.00				
E- D	0 / 0	-38.5	-38.5	0.21 (3)	10.00				

DESIGN CRITERIA [M]

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 971 (0.06")

CSI: TC=0.41/1.00 (B-C:1) , BC=0.21/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.16/1.00 (B-C:1)

DOL LUMBER=1.00,NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.80)
JSI METAL= 0.08 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. G. GALVES
100000024
PROVINCE OF ONTARIO

DWG NO. TAM T1905565
STRUCTURAL
CHECK DRAWING ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J7	2	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 19 20:52:40 2018 Page 1
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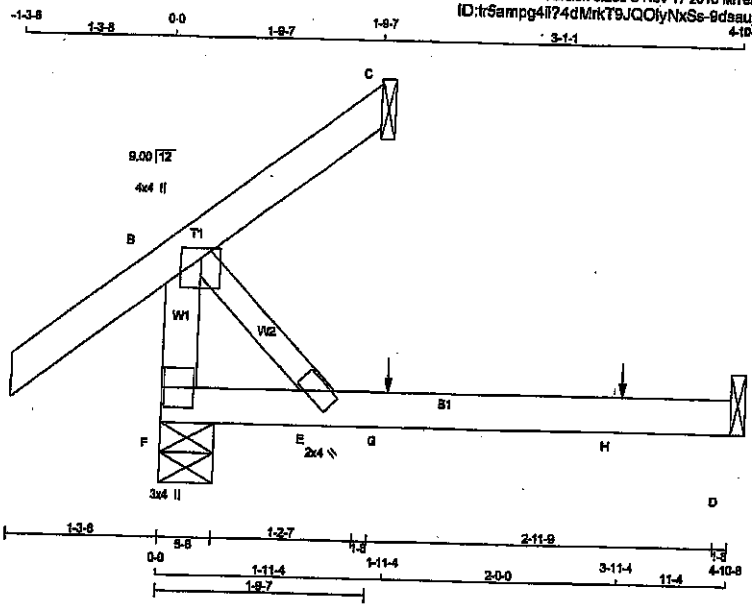

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

TOTAL WEIGHT = 2 X 13 = 25 lb [M]

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	380	0	380	0	0	5-8	5-8
C	37	0	37	0	0	1-8	1-8
D	94	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D .

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	282	163 / 0	51 / 0	0 / 0	0 / 0	68 / 0	0 / 0
C	25	21 / 0	0 / 0	0 / 0	0 / 0	4 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)
F-B	-266 / 0	0.0	0.0	0.03 (1)	
A-B	0 / 42	-102.1	-102.1	0.15 (1)	
B-C	-33 / 0	-102.1	-102.1	0.15 (1)	
F-E	0 / 0	-38.5	-38.5	0.17 (2)	
E-G	0 / 0	-38.5	-38.5	0.22 (2)	
G-H	0 / 0	-38.5	-38.5	0.22 (2)	
H-D	0 / 0	-38.5	-38.5	0.22 (2)	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
B-E	0 / 0	0.00 (1)	7.81

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	1	1	—	BACK	VERT	TOTAL	—	—
H	3-11-4	1	1	—	BACK	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/18/17
H. J. G. ALVES
100008822
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 971 (0.06")

CSI: TC=0.15/1.00 (A-B:1) , BC=0.22/1.00 (D-E:2) , WB=0.00/1.00 (B-E:1) , SSI=0.10/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1697	788	1987	1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)

BRACE NO. 1AM T1905566
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J8	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:42 2019 Page 1
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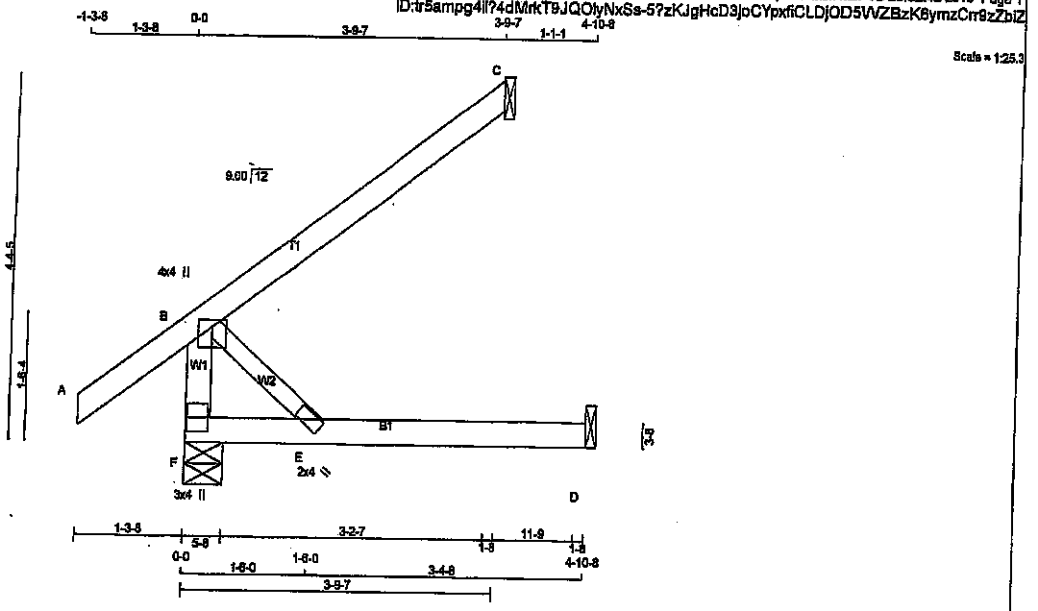


TOTAL WEIGHT = 2 X 16 = 31 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	428	0	428	0	0	5-8	5-8
C	193	0	193	0	0	1-8	1-8
D	94	0	119	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	315	190 / 0	51 / 0	0 / 0	0 / 0	73 / 0	0 / 0
C	133	110 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

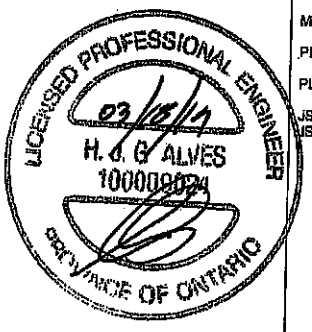
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
F-B	-334 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	0 / 0	-102.1	-102.1	0.25 (1)	10.00
F-E	0 / 0	-38.5	-38.5	0.18 (3)	10.00
E-D	0 / 0	-38.5	-38.5	0.21 (3)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-E	0 / 0	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 971 (0.06")

CSI: TC=0.25/1.00 (B-C:1) , BC=0.21/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

DWG NO. TAM 1705557
STRUCTURAL
GOOD FOR ONE BUILDING ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J9	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:43 2019 Page 1
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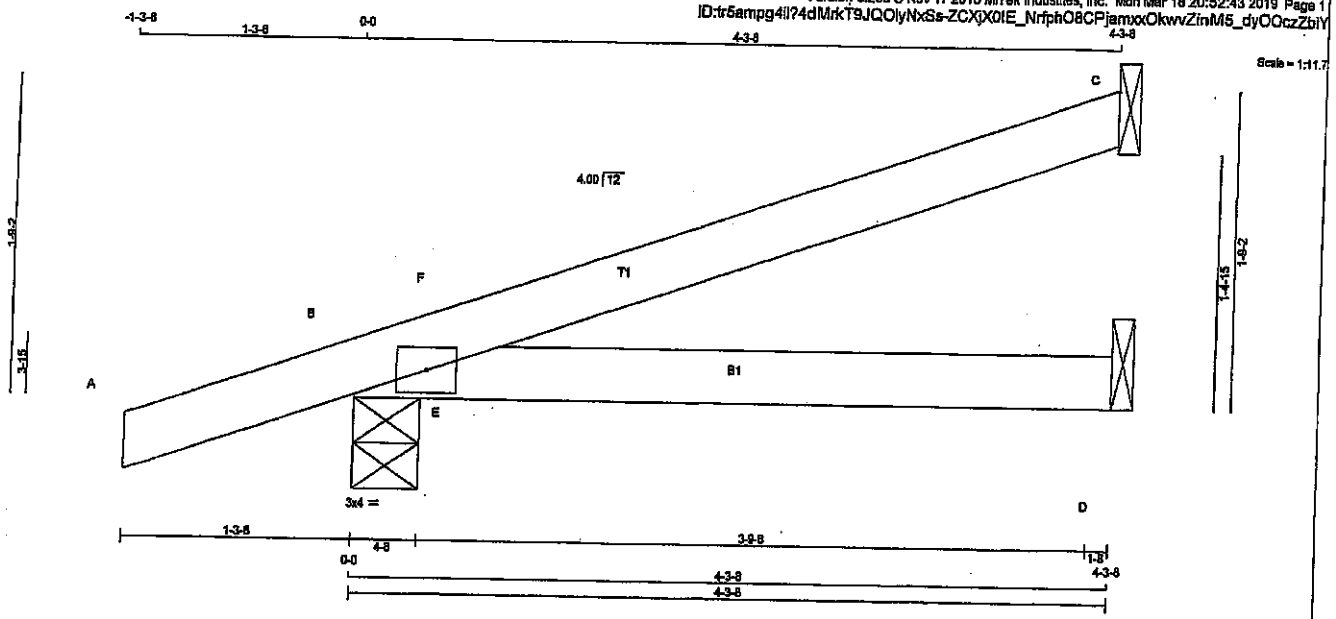


TOTAL WEIGHT = 10 X 12 = 117 lb

[M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	196	0	196	0	0	1-8	1-8
B	436	0	436	0	0	4-8	4-8
D	106	0	115	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	137	104 / 0	7 / 0	0 / 0	0 / 0	26 / 0	0 / 0
B	319	202 / 0	45 / 0	0 / 0	0 / 0	72 / 0	0 / 0
D	88	21 / 0	38 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B-F	-34 / 0	-102.1	-102.1	0.09 (3)	6.25	
F-C	0 / 5	-102.1	-102.1	0.26 (1)	10.00	
B-E	0 / 0	-36.5	-36.5	0.18 (1)	10.00	
E-D	0 / 0	-36.5	-36.5	0.20 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-166 / 79	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	28.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 983 (0.05")

CSI: TC=0.26/1.00 (C-F:1) , BC=0.20/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

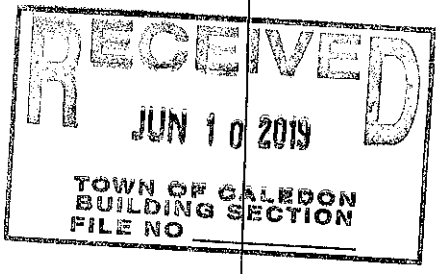
NAIL VALUES

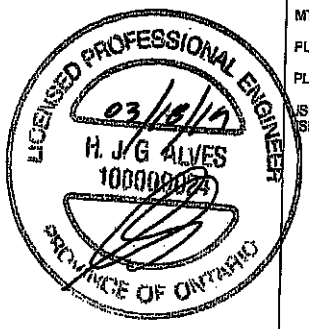
PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1957	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

(SI GRIP= 0.25 (B) (INPUT = 0.90)
(SI METAL= 0.07 (B) (INPUT = 1.00)





PERMIT NO. TAM 1905556
STRUCTURAL
CALEDON ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J10	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:45 2019 Page 1
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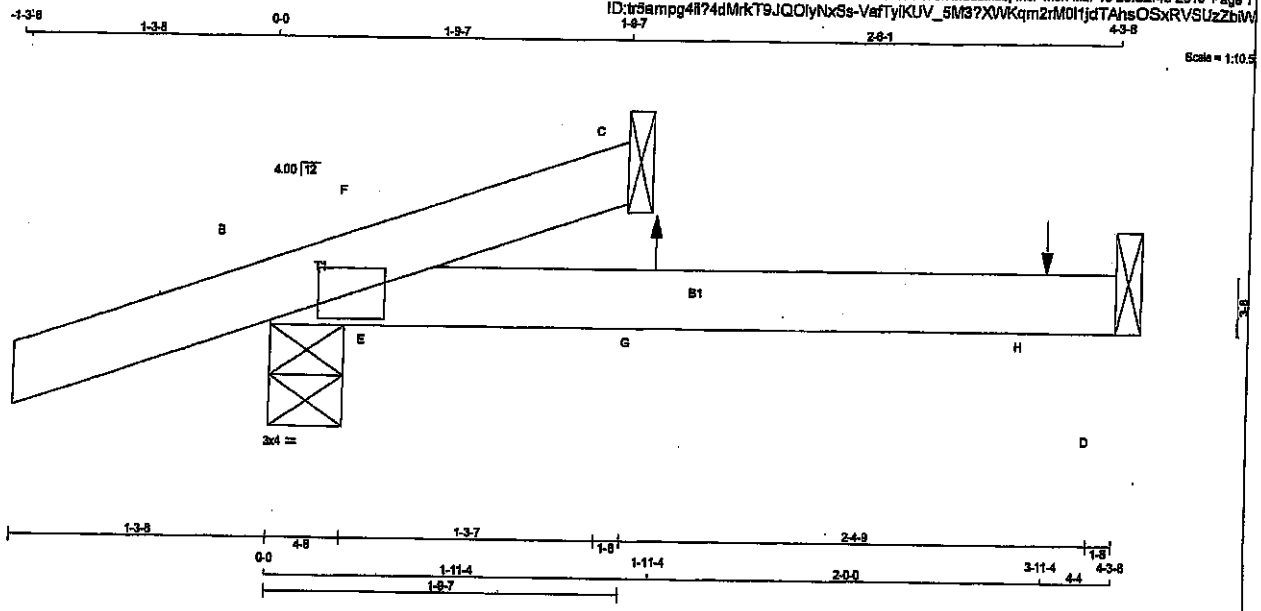

TOTAL WEIGHT = 9 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	103	0	103	0	0	1-8	1-8
B	329	0	329	0	0	4-8	4-8
D	151	0	151	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	82	29 / 0	28 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	238	157 / 0	28 / 0	0 / 0	0 / 0	52 / 0	0 / 0
D	118	48 / -5	36 / 0	0 / 0	0 / 0	34 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (7)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.14 (1)	10.00	
B- F	-83 / 0	-102.1	-102.1	0.14 (1)	6.25	
F- C	0 / 21	-102.1	-102.1	0.12 (3)	10.00	
B- E	0 / 0	-38.5	-38.5	0.08 (2)	10.00	
E- G	0 / 0	-38.5	-38.5	0.11 (2)	10.00	
G- H	0 / 0	-38.5	-38.5	0.11 (2)	10.00	
H- D	0 / 0	-38.5	-38.5	0.11 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	0 / 217	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	8	1	25	FRONT	VERT	TOTAL	—	—
H	3-11-4	-107	-107	—	FRONT	VERT	TOTAL	—	—

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. I. G. ALVES
100090024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.11/1.00 (D-E:2) , WB=0.00/1.00 (E-F:1) , SSI=0.21/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

DWG NO. TAM 7905559

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
200172-400371	J11	1	1	Preston 11	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:45 2019 Page 1
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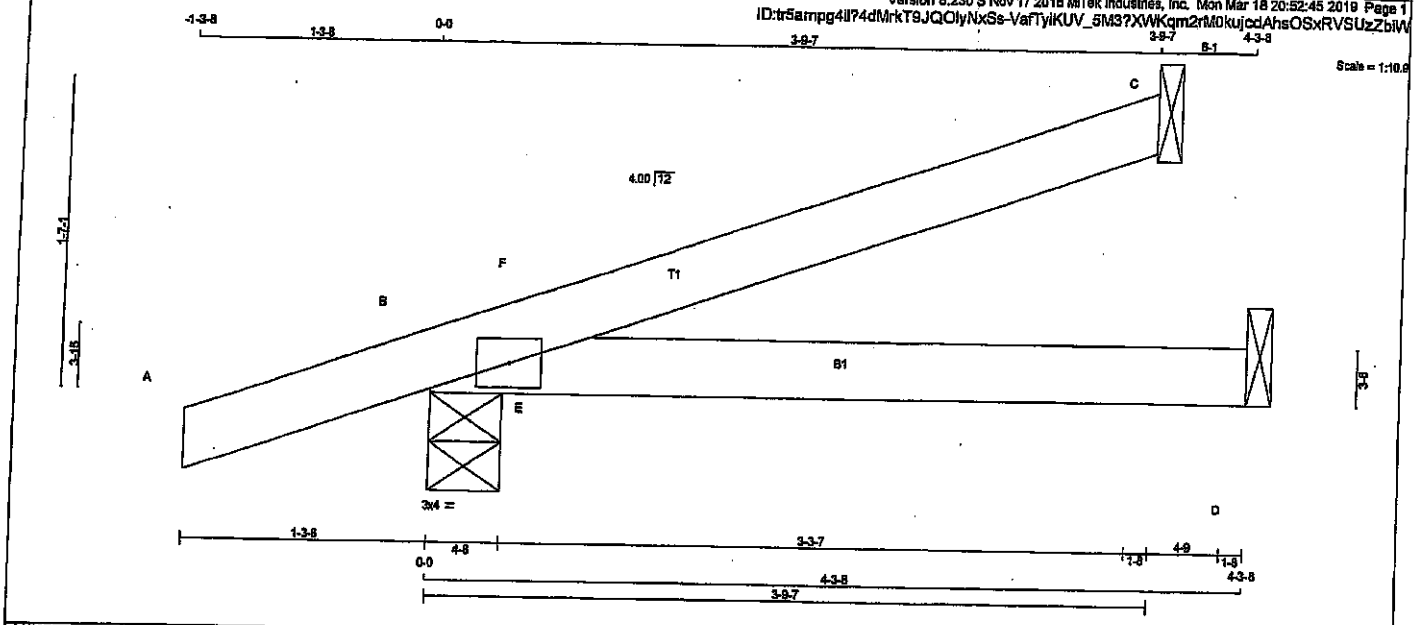


TOTAL WEIGHT = 11 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	179	0	179	0	0	1-8	1-8
B	414	0	414	0	0	4-8	4-8
D	95	0	106	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX /MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	126	92 / 0	9 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	302	190 / 0	44 / 0	0 / 0	0 / 0	69 / 0	0 / 0
D	81	15 / 0	37 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

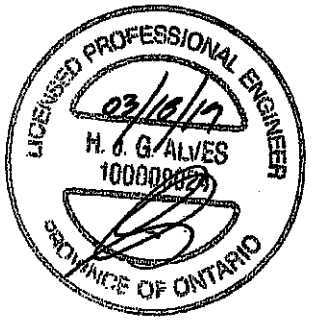
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD FROM	TO	LC1 (PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	
B- F	-39 / 0	-102.1	-102.1	0.10 (3)	5.25	
F- C	0 / 7	-102.1	-102.1	0.21 (1)	10.00	
B- E	0 / 0	-38.5	-38.5	0.12 (1)	10.00	
E- D	0 / 0	-38.5	-38.5	0.18 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-91 / 100	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.21/1.00 (C-F:1) , BC=0.18/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (C-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1655

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

BCIN 31-TAM T1905560
STRUCTURAL

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J12	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:46 2019 Page 1
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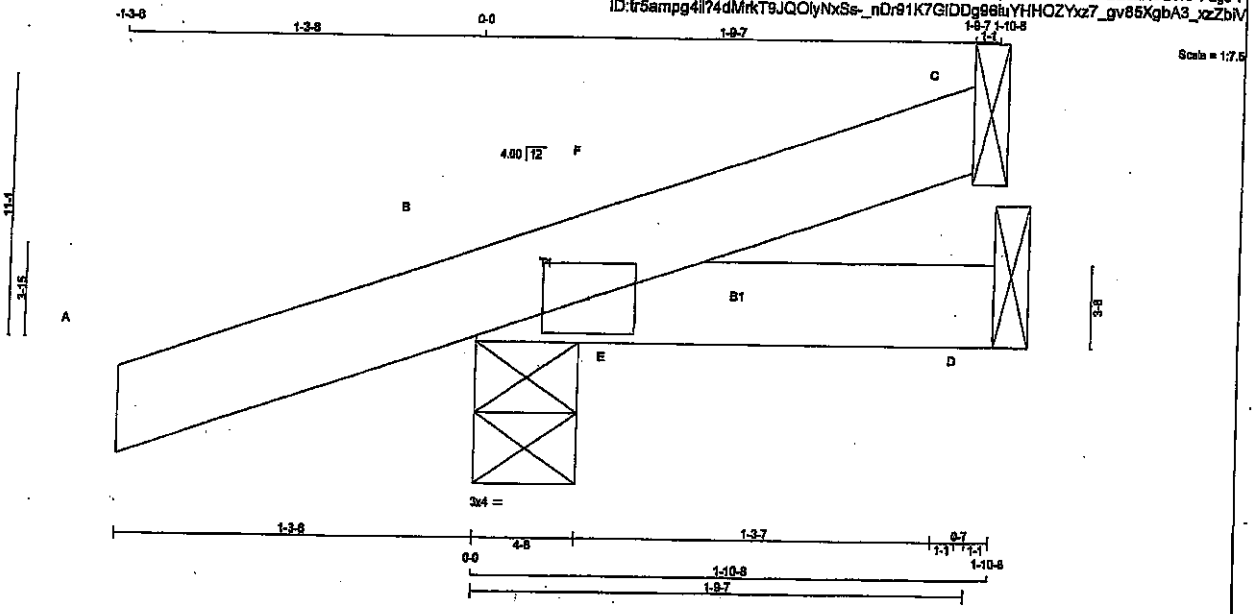


TOTAL WEIGHT = 6 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	59	0	59	0	-10 ✔	1-8 ✔	1-8
B	314	0	314	0	0	4-8	4-8
D	18	0	34	0	-19 ✔	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT
PROVIDE ANCHORAGE AT BEARING JOINT D FOR 150 LBS FACTORED UPLIFT

OK w/ TOE NAILING

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	43	29 / -11	5 / 0	0 / 0	0 / 0	9 / 0	0 / 0
B	224	158 / 0	19 / 0	0 / 0	0 / 0	46 / 0	0 / 0
D	18	0 / -17	15 / 0	0 / 0	0 / 0	9 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

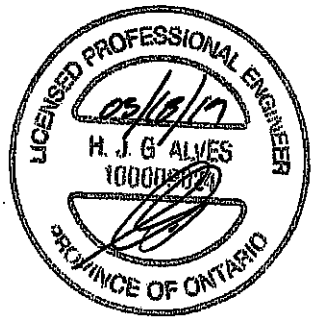
CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13	(1)	10.00
B- F	-53 / 0	-102.1	-102.1	0.12	(1)	6.25
F- C	-5 / 3	-102.1	-102.1	0.03	(5)	10.00
B- E	0 / 0	-38.5	-38.5	0.05	(5)	10.00
E- D	0 / 0	-38.5	-38.5	0.05	(5)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	0 / 94	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NECC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.13/1.00 (A-B:1) , BC=0.05/1.00 (B-E:5) , WB=0.00/1.00 (E-F:1) , SSI=0.13/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

[illegible] TAM 7905521
STRUCTURAL
[illegible] ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J13	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Mon Mar 18 20:52:48 2019 Page 1
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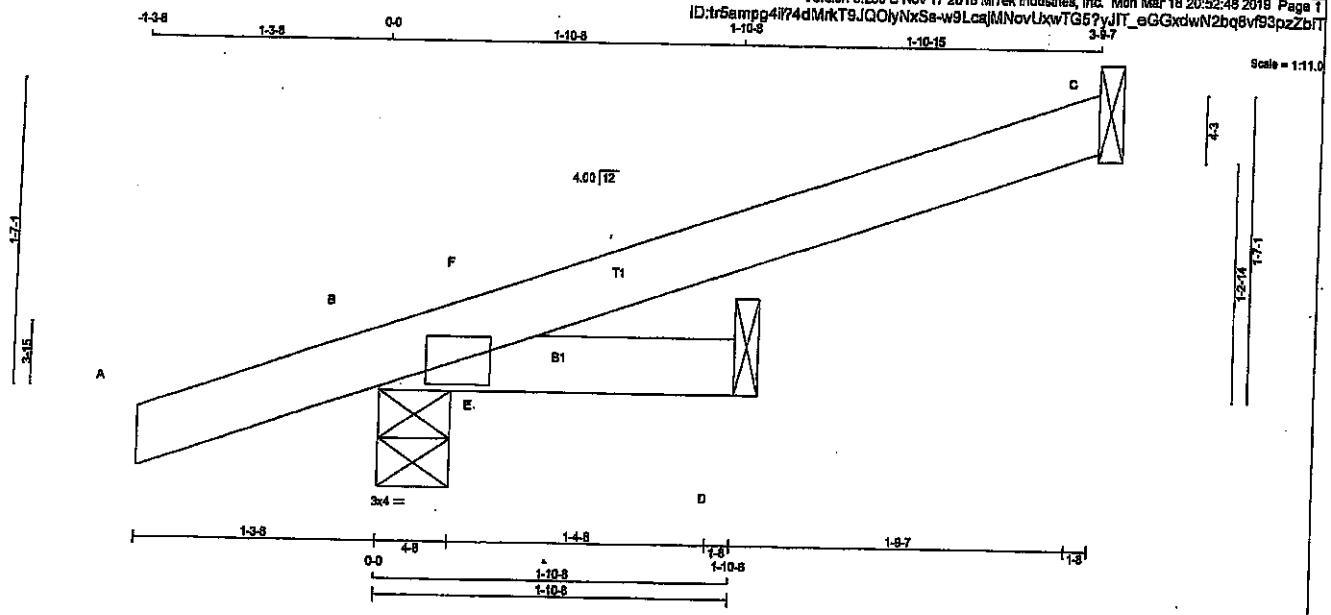


LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	145	0	145	0	0	1-8	1-8
B	317	0	317	0	0	4-8	4-8
D	133	0	133	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	100	82 / 0	1 / 0	0 / 0	0 / 0	18 / 0	0 / 0
B	226	159 / 0	21 / 0	0 / 0	0 / 0	47 / 0	0 / 0
D	99	57 / 0	18 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

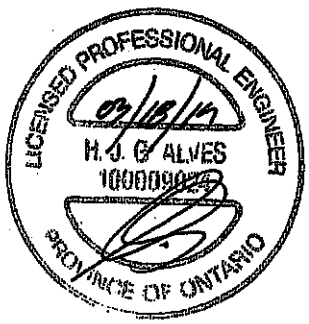
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00
B- F	0 / 31	-102.1	-102.1	0.08 (1)	10.00
F- C	-5 / 0	-102.1	-102.1	0.14 (1)	10.00
B- E	0 / 0	-38.5	-38.5	0.19 (1)	10.00
E- D	0 / 0	-38.5	-38.5	0.19 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-258 / 0	0.00 (1)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN



TOTAL WEIGHT = 8 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCSC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.14/1.00 (C-F:1) , BC=0.19/1.00 (B-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.19/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.05 (B) (INPUT = 1.00)

7905562

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. Preston 11	DRWG NO.
200172-400371	J14	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Mon Mar 18 20:52:49 2019 Page 1
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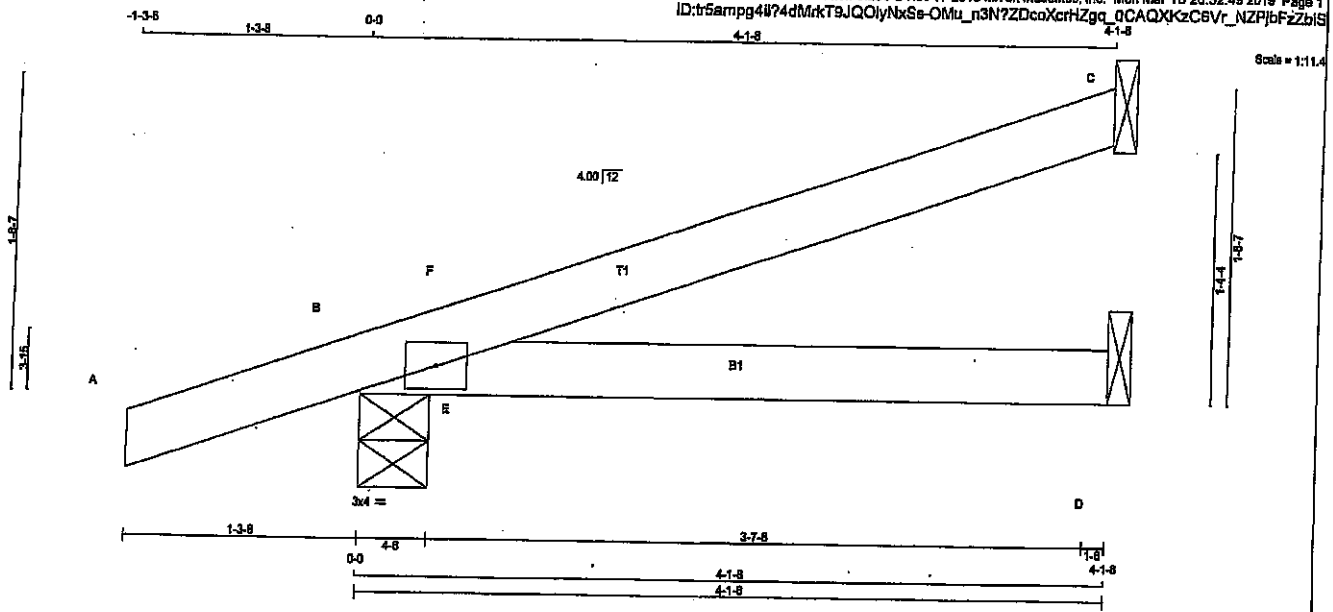


TOTAL WEIGHT = 6 X 11 = 66 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	188	0	188	0	0	1-8	1-8
B	427	0	427	0	0	4-8	4-8
D	102	0	110	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	132	100 / 0	.7 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	310	197 / 0	43 / 0	0 / 0	0 / 0	70 / 0	0 / 0
D	85	20 / 0	33 / 0	0 / 0	0 / 0	28 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

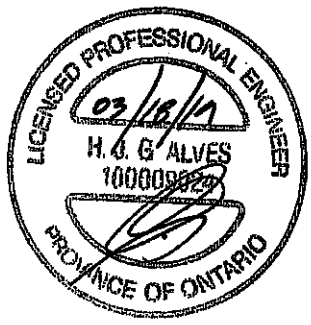
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	CSI (LC)	MAX UNBRAC LENGTH
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00
B-F	-32 / 0	-102.1	-102.1	0.08 (3)	8.25
F-C	0 / 5	-102.1	-102.1	0.24 (1)	10.00
B-E	0 / 0	-38.5	-38.5	0.17 (1)	10.00
E-D	0 / 0	-38.5	-38.5	0.18 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-F	-155 / 74	0.00 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.24/1.00 (C-F:1) , BC=0.18/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.17/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	769	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.25 (B) (INPUT = 0.93)
JSI METAL= 0.07 (B) (INPUT = 1.00)

Building No. 7905563

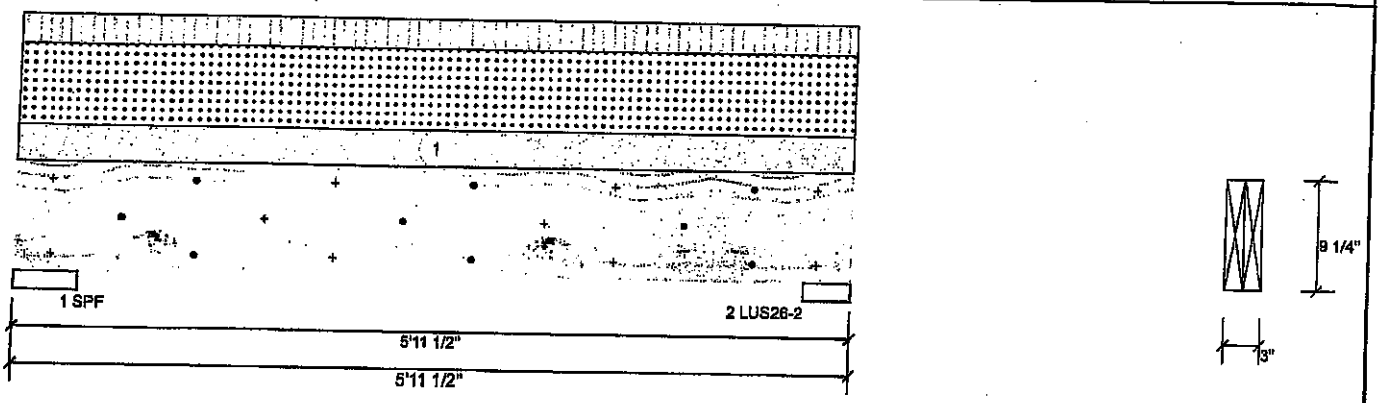
isDesign™

Client:
Project:
Address:

Date: 3/18/2019
Designer:
Job Name: 200172
Project #:

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	192	237	529	0
2	184	228	508	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	13%	297 / 986	1282	L	1.25D+1.5S +L
2 - LUS26-2	4.000"	17%	284 / 945	1229	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1475 ft-lb	3' 1/2"	6039 ft-lb	0.244 (24%)	1.25D+1.5S +L	L
Unbraced	1475 ft-lb	3' 1/2"	5210 ft-lb	0.283 (28%)	1.25D+1.5S +L	L
Shear	1115 lb	1'2"	3984 lb	0.280 (28%)	1.25D+1.5S +L	L
LL Defl inch	0.013 (L/4774)	3' 1/2"	0.176 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.018 (L/3460)	3' 1/2"	0.176 (L/360)	0.100 (10%)	D+S+0.5L	L

Design Notes

1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Nail from opposite sides as indicated by + and • symbols.
3 Refer to last page of calculations for fasteners required for specified loads.
4 Girders are designed to be supported on the bottom edge only.
5 Top braced at bearings.
6 Bottom braced at bearings.
7 Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DWG NO. TAM 71905564
STRUCTURAL COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-0-0	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

This design is valid until 12/11/2021

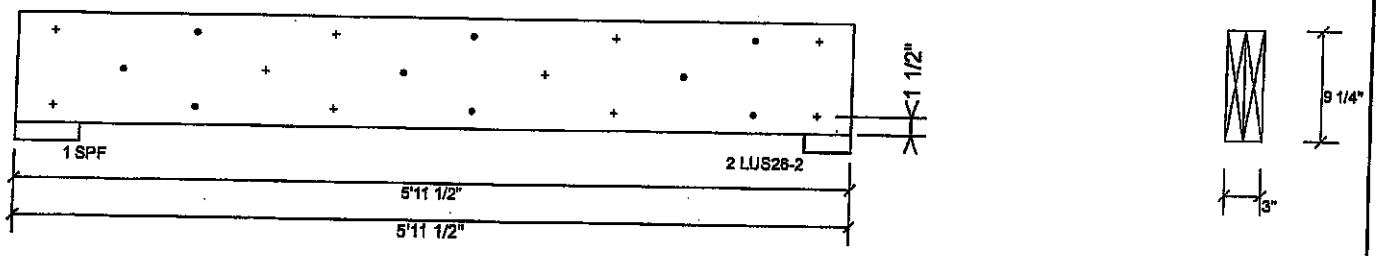
Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	62.0 %
Load	210.8 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

LICENSED PROFESSIONAL ENGINEER
03/18/19
H. J. G ALVES
100089029
PROVINCE OF ONTARIO

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM T1905564
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

Face-Mount Hangers LUCZ/LU/U/HU/HUC

SIMPSON Strong-Tie

Standard Joist Hangers

LUCZ concealed flange hanger available for 2x6, 2x8, 2x10 and 2x12 lumber. Ideal for end of ledger/header or post conditions, the LUCZ also provides cleaner lines for exposed conditions such as overhead decks.

See Hanger tables on pp. 134–140. See Hanger Options on p. 126 for hanger modifications, which may result in reduced resistances.

LU — Value engineered for strength and economy. Precision-formed — engineered for installation ease and design value.

U — The standard U hanger provides flexibility of joist to header installation. Versatile fastener selection with tested factored resistances.

HU/HUC — Most models have triangle and round holes. To achieve maximum resistances, fill both round and triangle holes with common nails. These heavy-duty connectors are designed for schools and other structures requiring additional strength, longevity and safety factors.

Material: See tables on pp. 134–140

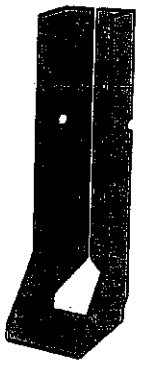
Finish: Galvanized. Some products available in ZMAX® coating.

Installation:

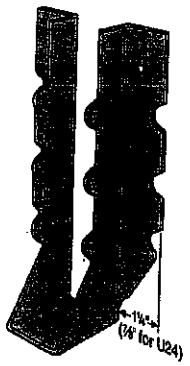
- Use all specified fasteners; see General Notes.
- HU — Can be installed filling round holes only, or filling round and triangle holes for maximum values.
- Joists sloped up to ¼:12 achieve tabulated values.
- For installations to masonry or concrete see p. 333–334.
- HU hangers can be welded to a steel member. Refer to technical bulletin T-C-HUHUC-W at **strongtie.com**.

Options:

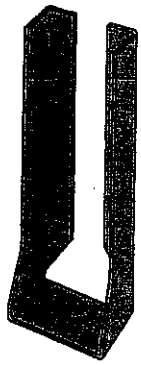
- HU hangers available with the header flanges turned in for 2⁵⁄₁₆" width and larger, with no reduction in resistances — order HUC hanger.
- HU only — rough beam sizes available by special order.
- See p. 140 for stocked U hanger rough sizes tables. Rough sizes are not available in 8x.
- Also see LUS and HUS series.



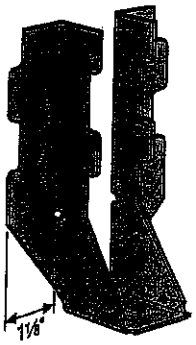
LUC210Z
(LUC26Z similar)



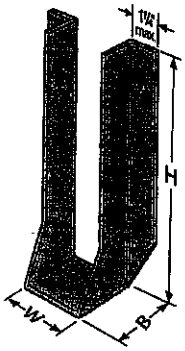
U210



HUC412
Concealed flanges

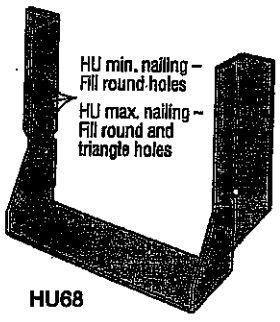


LU28L
(except LU roughs)

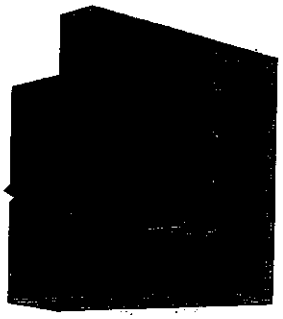


HU214
Projection seat on most models for maximum bearing and section economy.

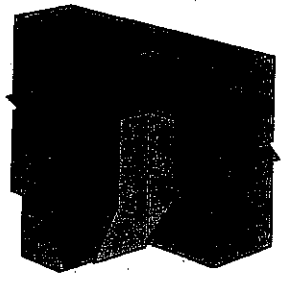
Model configurations may differ from those shown. Some HU models do not have triangle holes. Contact Simpson Strong-Tie.



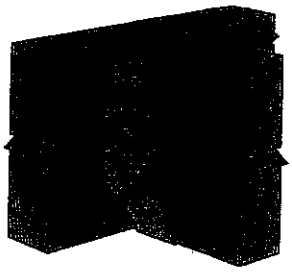
HU68



Typical LUCZ Installation



Typical HU Installation



Typical LU28L Installation

Face-Mount Hangers — Solid Sawn Lumber

SIMPSON Strong-Tie

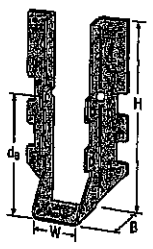
These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Joist Size	Model No.	Ga.	Dimensions (in.) W	H	B	d_s[4]	Fasteners Min./Max.	Header	Joist	Factored Resistance D.Fir-L Uplift (K_D = 1.15) lb. / kN	D.Fir-L Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN	Installed Cost Index
Sawn Lumber Sizes														
2x4	LU24L	22	1⅜	3⅛	1⅜	2 1⁵⁄₁₆	—	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22	Lowest
2x4	LUS24	18	1⅜	3⅛	1¾	2¼	—	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14	+11%
2x4	U24	16	1⅜	3⅛	1½	1 13⁄₁₆	—	(4) 10d	(2) 10d x 1½"	450 / 2.00	1340 / 5.96	355 / 1.58	1030 / 4.58	+90%
2x4	HU26	14	1⅜	3 1⁵⁄₁₆	2¼	1⅛	—	(4) 16d	(2) 10d x 1½"	490 / 2.18	1525 / 6.78	450 / 2.00	1080 / 4.80	+490%
DBL 2x4	LUS24-2	18	3⅛	3⅛	2	1 7⁄₃₂	—	(4) 16d	(2) 16d	835 / 3.71	2020 / 8.98	590 / 2.62	1435 / 6.38	Lowest
DBL 2x4	U24-2	16	3⅛	3	2	1 15⁄₁₆	—	(4) 10d	(2) 10d	480 / 2.14	1340 / 5.96	445 / 1.98	1030 / 4.58	+59%
DBL 2x4	HU24-2/ HUC24-2	14	3⅛	3 1⁵⁄₁₆	2½	2 1⁵⁄₁₆	—	(4) 16d	(2) 10d	525 / 2.34	1710 / 7.61	490 / 2.18	1585 / 7.05	+244%
2x6	LU26L	22	1⅜	5	1⅜	4 15⁄₃₂	—	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07	Lowest
SS 2x6	LUS26	18	1⅜	4¾	1¾	3 15⁄₃₂	—	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25	+10%
2x6	U26	16	1⅜	4¾	2	3 1⁵⁄₁₆	—	(6) 10d	(4) 10d x 1½"	895 / 3.98	2005 / 8.92	780 / 3.47	1860 / 8.27	+70%
SS 2x6	LUC26Z	18	1⅜	4¾	1¾	4	—	(6) 10d	(4) 10d x 1½"	830 / 3.69	1605 / 7.14	710 / 3.16	1140 / 5.07	+74%
2x6	HUS26	16	1⅝	5⅜	3	3 1⁵⁄₁₆	—	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.97	2065 / 9.20	3875 / 17.24	+320%
DBL 2x6	LU26-2L	20	3⅛	5	1⅜	4 15⁄₃₂	—	(6) 10d	(4) 10d x 1½"	760 / 3.38	1605 / 7.14	680 / 3.02	1140 / 5.07	Lowest
SS DBL 2x6	LUS26-2	18	3⅛	4⅞	2	4 1⁄₃₂	—	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	1920 / 8.54	+24%
DBL 2x6	U26-2	16	3⅛	5	2	3 1⁵⁄₁₆	—	(8) 10d	(4) 10d	960 / 4.27	2675 / 11.90	890 / 3.96	2475 / 11.01	+124%
DBL 2x6	HU26-2/ HUC26-2	14	3⅛	5⅜	2½	5	Min.	(8) 16d	(4) 10d	1055 / 4.69	3420 / 15.21	980 / 4.36	2845 / 12.66	+358%
							Max.	(12) 16d	(6) 10d	1580 / 7.03	4415 / 19.64	1470 / 6.54	3135 / 13.95	+372%
TPL 2x6	LUS26-3	18	4⅝	4⅛	2	3 1⁄₃₂	—	(4) 16d	(4) 16d	1720 / 7.65	2595 / 11.54	1545 / 6.87	2340 / 10.41	Lowest
TPL 2x6	U26-3	16	4⅝	4¼	2	3 1⁵⁄₁₆	—	(8) 10d	(4) 10d	960 / 4.27	2675 / 11.90	890 / 3.96	2475 / 11.01	+87%
TPL 2x6	HU26-3/ HUC26-3	14	4⅝	4⅝	2½	5	Min.	(8) 16d	(4) 10d	1055 / 4.69	3420 / 15.21	980 / 4.36	2845 / 12.66	+193%
							Max.	(12) 16d	(6) 10d	1580 / 7.03	4415 / 19.64	1470 / 6.54	3135 / 13.95	+198%

1. 10d common nails may be used instead of the specified 16d nails at 0.83 of the tabulated value.
2. Factored uplift resistances have been increased 15% for earthquake or wind loading with no further increase allowed. Reduce by 15% for standard term loading such as in cantilever construction.
3. Min. nailing quantity and factored resistances — fill all round holes; max. nailing quantity and factored resistances — fill all round and triangle holes.
4. D.Fir-L factored resistances can be used for most LVL. Verify with manufacturer prior to selecting hanger.
5. See p. 28 for hangers with reduced capacity due to installation with different nails.
6. d_s is the distance from the bearing seat to the top joist nail.
7. **Nails:** 16d = 0.162" dia. x 3½" long, 10d = 0.148" dia. x 3" long, 10d x 1½" = 0.148" dia. x 1½" long. See pp. 27–28 for other nail sizes and information.

*Hangers do not have an installed Cost Index.



TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

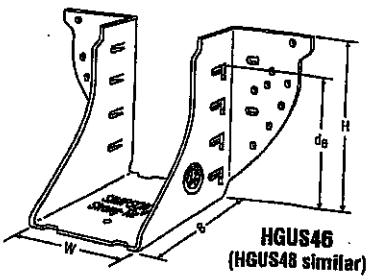
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

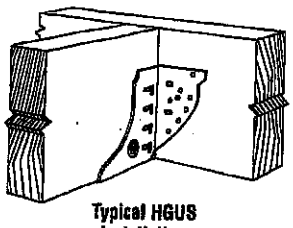
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

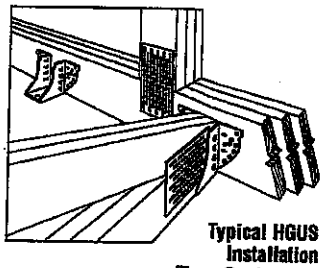
- See current catalogue for options



HGUS46
(HGUS48 similar)



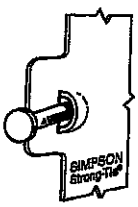
Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

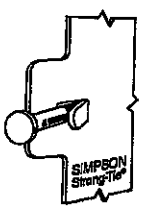
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_B[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS48	12	3⅝	7⅛	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS410	12	3⅝	9	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS412	12	3⅝	10⅞	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS414	12	3⅝	12⅞	4	11⅞	66-16d	22-16d	10130	16400	7195	11645
HGUS5.50/8	12	5½	6⅝	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS5.50/10	12	5½	8⅝	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS5.50/12	12	5½	10½	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS5.50/14	12	5½	12½	4	11⅞	66-16d	22-16d	10130	16400	7195	11645
HGUS7.25/8	12	7¼	7⅞	4	6⅝	36-16d	12-16d	6070	12980	4310	9215
HGUS7.25/10	12	7¼	8⅝	4	8¼	46-16d	16-16d	6840	15760	4855	11190
HGUS7.25/12	12	7¼	10⅝	4	10¼	56-16d	20-16d	7640	16110	5425	11435
HGUS7.25/14	12	7¼	12⅝	4	11¼	66-16d	22-16d	10130	18200	7195	12920

1. d_B is the distance from the seat of the hanger to the highest joist nail.

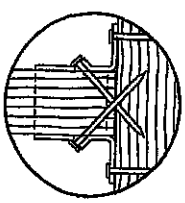


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

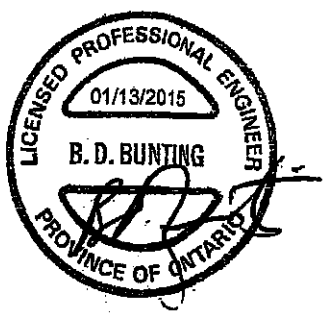
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECDSHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

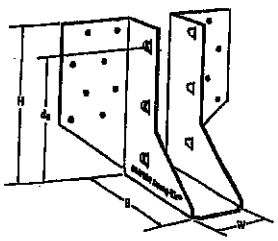
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

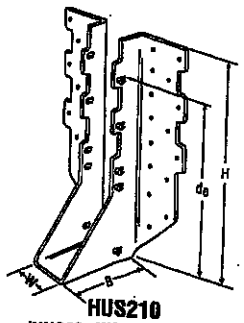
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

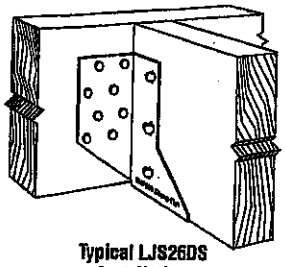
- See current catalogue for options



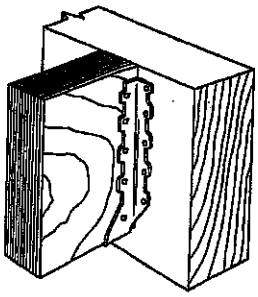
LJS26DS



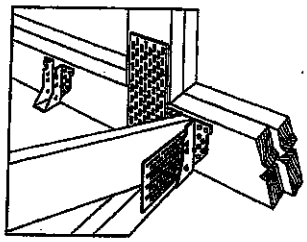
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



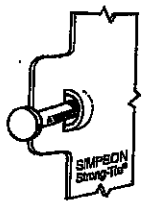
Typical HUS Installation



Typical HUS installation (Truss Designer to provide fastener quantity for connecting multiple members together)

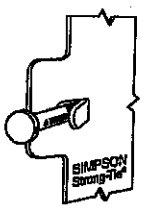
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1⁹⁄₁₆	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3¹⁵⁄₁₆	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7³⁄₃₂	3	6³⁄₃₂	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9³⁄₃₂	3	7⁸¹⁄₃₂	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1¹³⁄₁₆	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. d_e is the distance from the seat of the hanger to the highest joist nail.

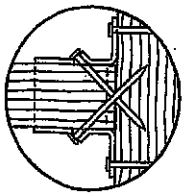


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

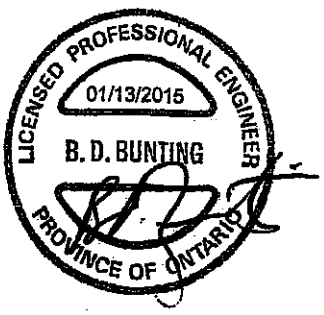
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

LUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

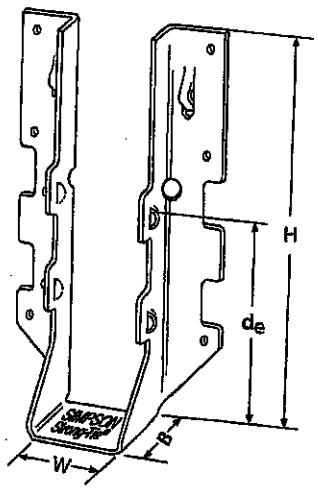
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

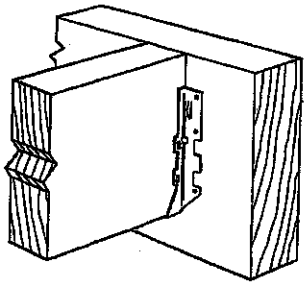
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3½" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads..
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

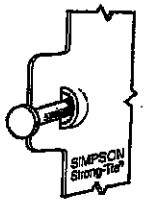


LUS28


Typical LUS Installation

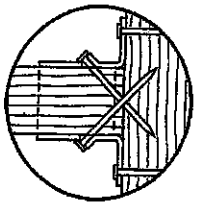
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LUS24	18	1⁹⁄₁₆	3⅛	1¾	1 ¹⁵⁄₁₆	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3⅛	3⅛	2	1 ¹³⁄₁₆	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1⁹⁄₁₆	4¾	1¾	3⅝	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4⅝	4 ³⁄₁₆	2	3¼	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1⁹⁄₁₆	6⅝	1¾	3¾	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4⅝	6¼	2	3¼	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1⁹⁄₁₆	7 ¹³⁄₁₆	1¾	3⅞	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4⅝	8 ³⁄₁₆	2	5¼	(8) 16d	(6) 16d	2580	3345	2320	2375

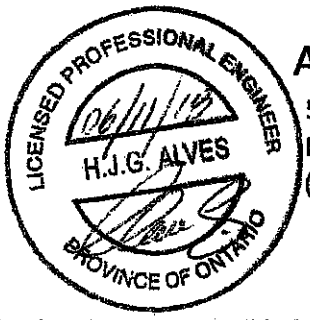
1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580


Double Shear Nailing Top View.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

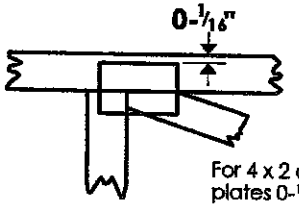
8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218 Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

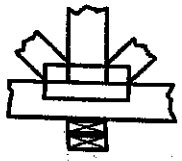
4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

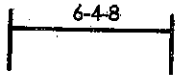
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

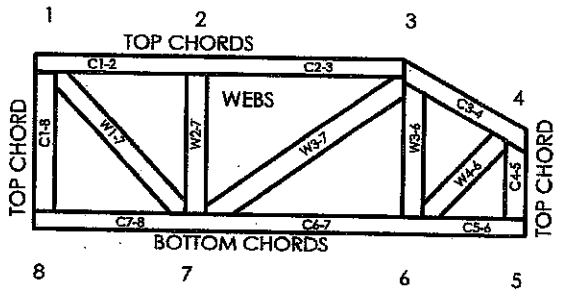
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

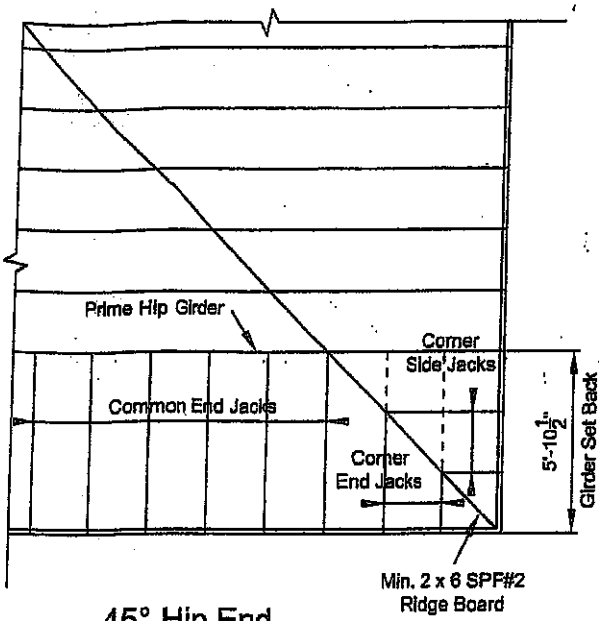
MiTek®

POWER TO PERFORM.™

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



45° Hip End

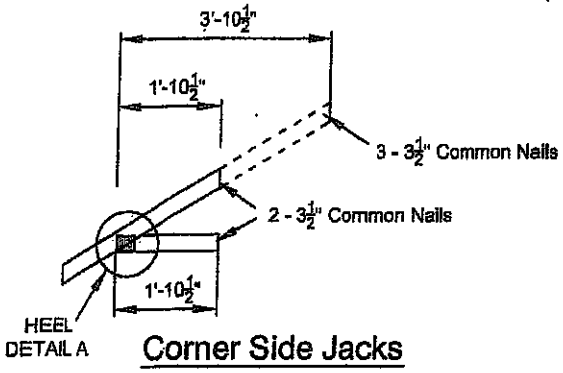
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

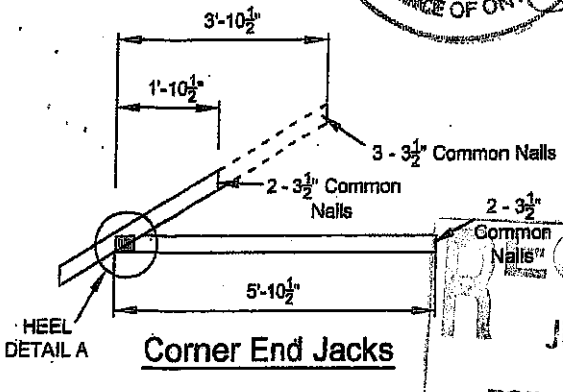
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

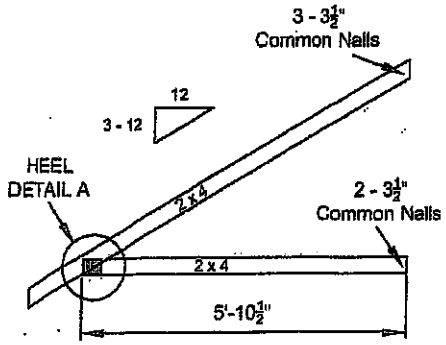
LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO



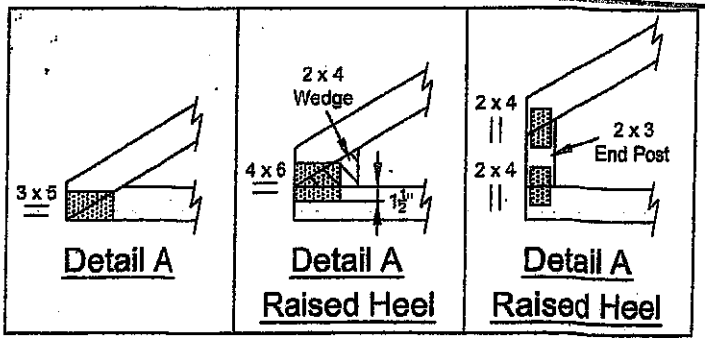
Corner Side Jacks



Corner End Jacks

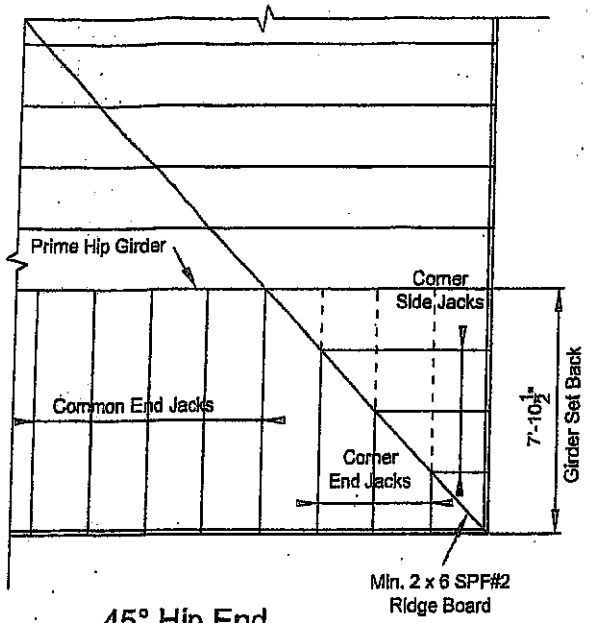


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

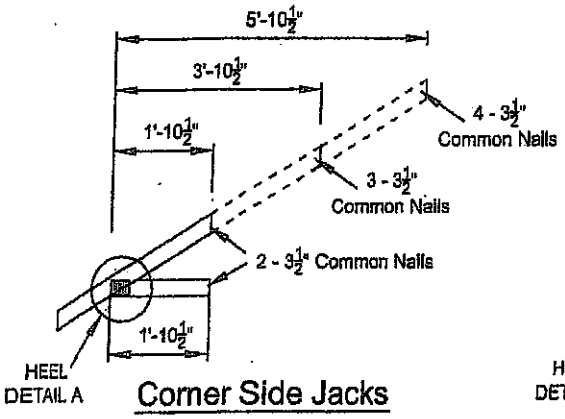
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

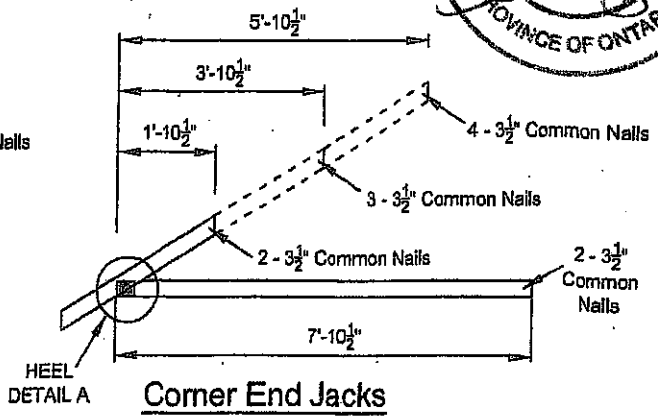
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

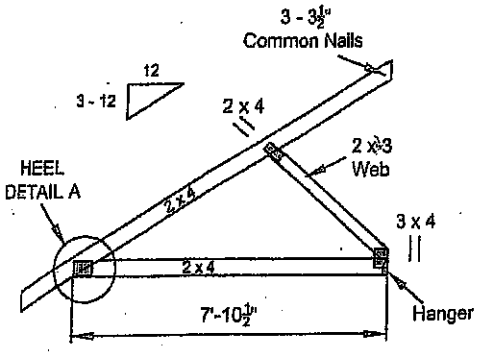
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



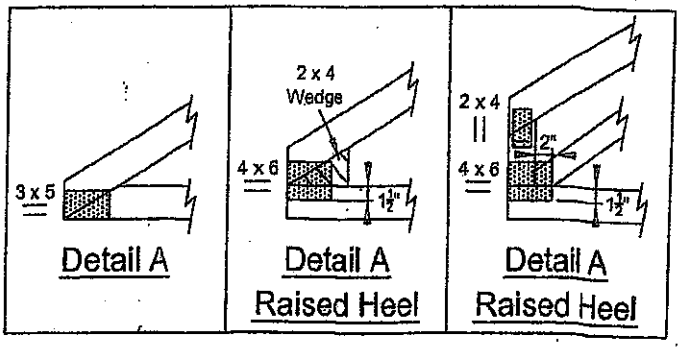
Corner Side Jacks



Corner End Jacks



Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217