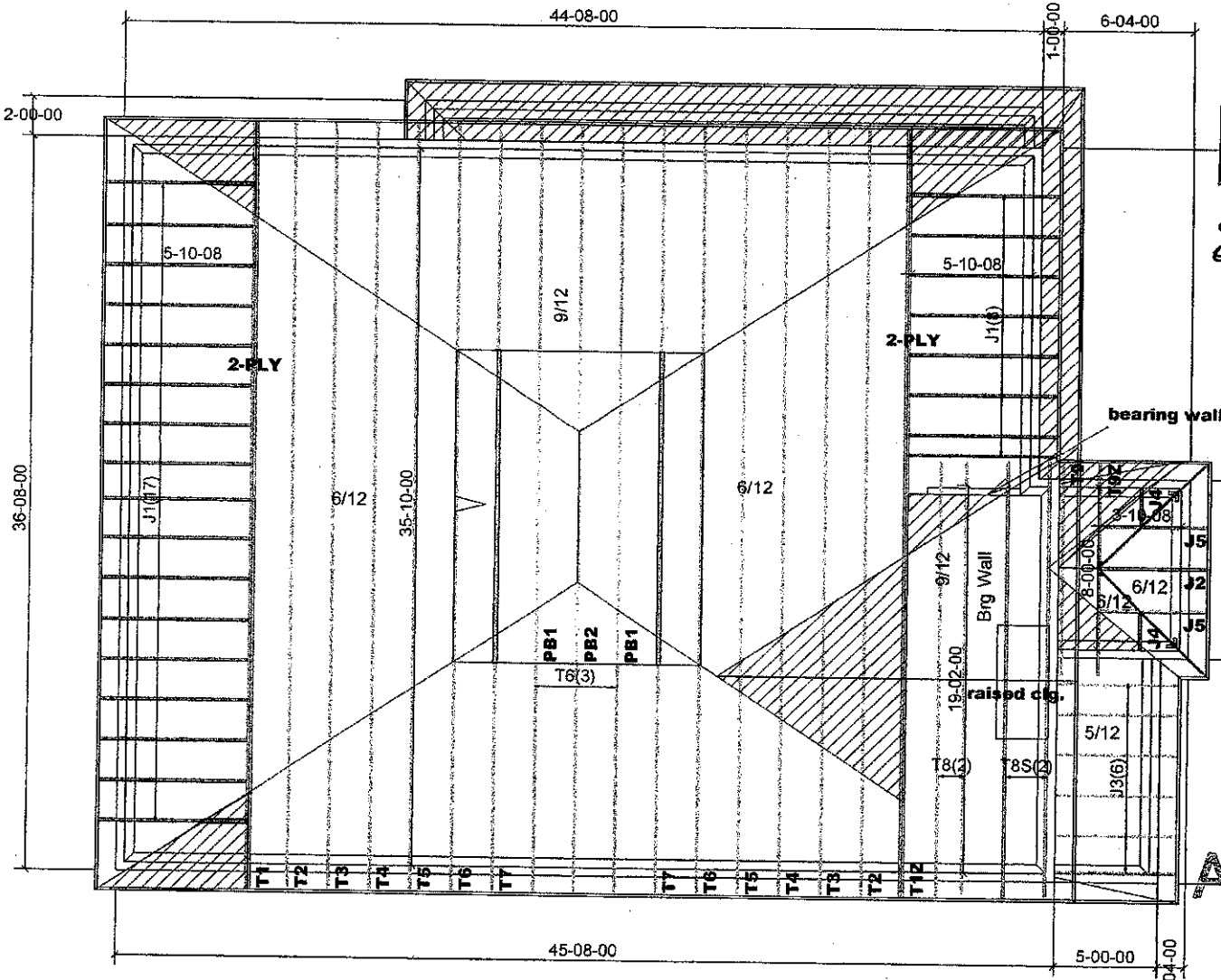


CERTIFIED MODEL

PRE-APPROVED

FOR PERMIT APPLICATION AS PER THE ONTARIO BUILDING CODE
TOWN OF CALEDON BUILDING DIVISION

REVIEWED BY J.SNELL
DATE JULY 18/19
FILE # WOODCROFT 3 - ELEV. 1,2,3

OPT. 5-BED
OPT. DECK ASPHALT SHINGLES
OPT. W.O.B. 12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

DENOTES CONV. FRAMING

GSL 37.6 PSF

DESIGN CONFORMS WITH O.B.C. 2012 PART
DESIGN LOADS:

TC LIVE	29 PSF
TC DEAD	6 PSF
BC LIVE	10.5 PSF
BC DEAD	7 PSF

T - 180737

APPLICANT COPY

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

RECEIVED
JUN 14 2019
TOWN OF CALEDON
BUILDING DIVISION

TAMARACK LUMBER INC.

Job Track:	50033
Plan Log:	200625
Layout ID:	401703

Builder / Location: **GREENPARK HOMES / CALEDON**

Model / Elevation: **WOODCROFT 3 / 1 (4 OR 5 BEDROOM)**

Project: **LAMBERT LANE PH.2**

Date: 5/29/2019 Sales: Mario DiCano Designer: AG/JG

Mitek ver 8.3.0.234

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREENPARK HOMES	PlanLog:	200625
Project:	LAMBERT LANE PH.2	Layout ID:	401703
Location:	CALEDON	Ref #	
Model:	WOODCROFT 3	Page:	1 of 2
Lot #:		Date:	03/06/2019
Elevation:	1 (4 OR 5 BEDROOM)	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

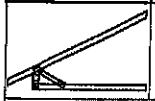
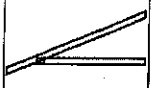
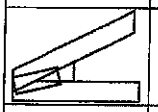
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	T1 Hip Girder	9 /12	35-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	354.14 216.67		
	1 2-ply	T1Z Hip Girder	9 /12	35-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	354.14 216.67		
	2	T2 Hip	9 /12	35-10-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	299.62 188.33		
	2	T3 Hip	9 /12	35-10-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	309.43 194.67		
	2	T4 Hip	9 /12	35-10-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	326.47 209.00		
	2	T5 Hip	9 /12	35-10-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	322.29 202.00		
	5	T6 Hip	9 /12	35-10-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	864.46 548.33		
	2	T7 Hip	9 /12	35-10-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	336.84 212.67		
	2	T8 Common	9 /12	19-02-00	8-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	179.57 114.00		
	2	T8S Roof Special	9 /12	19-02-00	8-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	219.57 138.33		
	1	T9 Common	6 /12	8-00-00	2-11-11	2 x 4	1-03-08 1-03-08	5-08 5-08	25.78 17.00		
	1	T9Z Common Girder	6 /12	8-00-00	2-11-11	2 x 4	1-03-08 1-03-08	5-08 5-08	25.78 17.00		
	2	PB1 Piggyback	9 /12	15-07-05	2-00-00	2 x 4			93.72 61.33		
	1	PB2 Piggyback	9 /12	15-07-05	2-11-14	2 x 4			48.71 31.50		

DELIVERY SHIPLIST

TAMARACK LUMBER INC.
ALPA LUMBER GROUP

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREENPARK HOMES	PlanLog:	200625
Project:	LAMBERT LANE PH.2	Layout ID:	401703
Location:	CALEDON	Ref #	
Model:	WOODCROFT 3	Page:	2 of 2
Lot #:		Date:	03/06/2019
Elevation:	1 (4 OR 5 BEDROOM)	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	25	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	451.94 291.67		
	1	J2 Jack-Open	6 /12	3-10-08	2-10-15	2 x 4	1-03-08	5-08 2-04-12	11.84 7.33		
	6	J3 Monopitch Supported Gable	5 /12	5-04-00	3-00-15	2 x 4	1-03-08	4-01 2-03-00	86.43 56.00		
	2	J4 Jack-Open	6 /12	1-10-08	1-04-03	2 x 4		5-08 1-04-12	10.49 8.00		
	2	J5 Jack-Open Girder	6 /12	1-09-07	1-10-06	2 x 4	1-03-08 2-01-01	5-08 1-04-03	18.35 12.00		

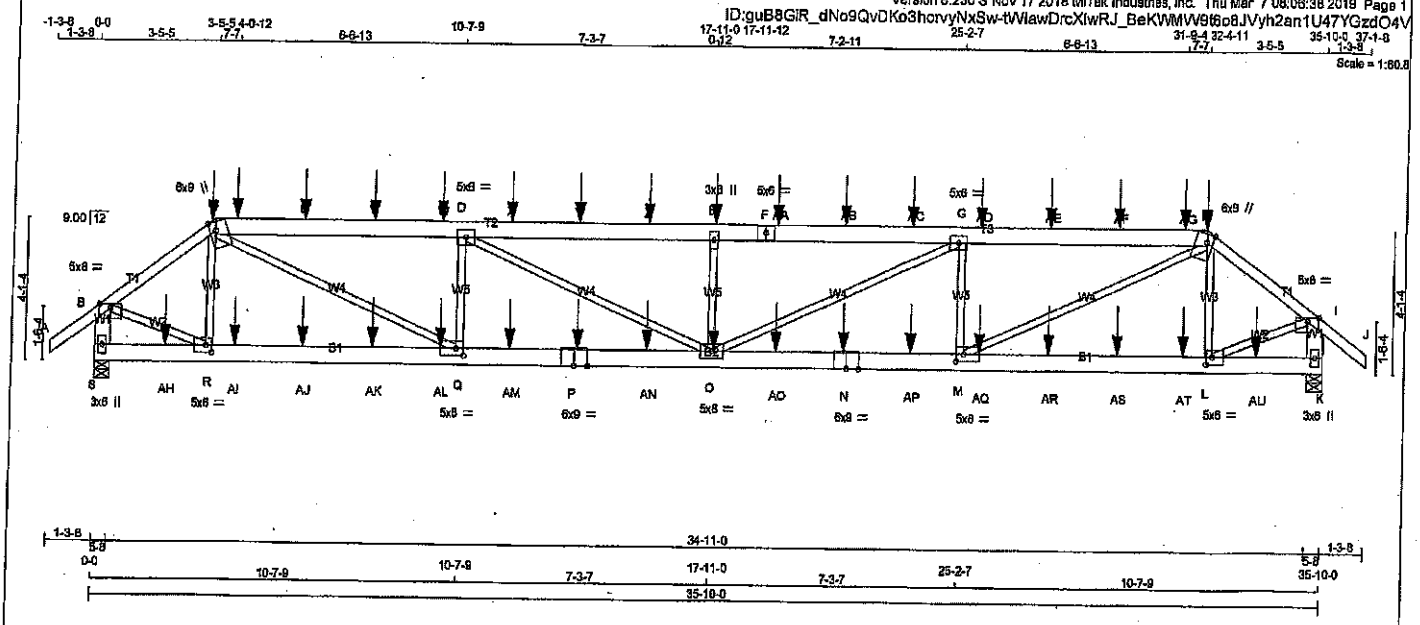
TOTAL # TRUSS= **64** TOTAL BFT OF ALL TRUSSES= **2742.5** BFT. TOTAL WEIGHT OF ALL TRSSES **4339.56** LBS

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401703	T1	1	2	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Thu Mar 7 08:08:38 2019 Page 1
ID:guB8GIR_dNo9QvDKo3horvyNxSw-tWlawDrcXlwRJ_BeKWMW9t6o8JVyh2an1U47YGzdO4V



TOTAL WEIGHT = 2 X 177 = 354 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	SIDE(61.0)
H-J	1	12	SIDE(61.0)
C-F	2	12	SIDE(61.0)
F-H	2	12	SIDE(61.0)
S-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S-P	2	12	SIDE(0.0)
P-N	2	12	SIDE(183.1)
N-K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	6.0	8.0	Edge	2.00
D	TMWW-t	MT20	6.0	6.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMVW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	5087	0	5087	0	0	5-8	5-8
K	5087	0	5087	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	3788	2140 / 0	722 / 0	0 / 0	0 / 0	924 / 0	0 / 0
K	3788	2140 / 0	722 / 0	0 / 0	0 / 0	924 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.91 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	(PLF)	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.08 (1)		10.00
B-C	-5545 / 0	-102.1	-102.1	0.24 (1)		3.95
C-T	-10687 / 0	-102.1	-102.1	0.63 (1)		3.26
T-U	-10687 / 0	-102.1	-102.1	0.63 (1)		3.26
U-V	-10687 / 0	-102.1	-102.1	0.63 (1)		3.26
V-W	-10687 / 0	-102.1	-102.1	0.63 (1)		3.26
W-D	-10687 / 0	-102.1	-102.1	0.63 (1)		3.26
D-X	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
X-Y	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
Y-Z	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
Z-E	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
E-F	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
F-AA	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
AA-AB	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
AB-AC	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
AC-G	-12651 / 0	-102.1	-102.1	0.74 (1)		2.91
G-AD	-10686 / 0	-102.1	-102.1	0.63 (1)		3.26
AD-AE	-10686 / 0	-102.1	-102.1	0.63 (1)		3.26
AE-AF	-10686 / 0	-102.1	-102.1	0.63 (1)		3.26
AF-AG	-10686 / 0	-102.1	-102.1	0.63 (1)		3.26
AG-H	-10686 / 0	-102.1	-102.1	0.63 (1)		3.26
H-I	-5542 / 0	-102.1	-102.1	0.24 (1)		3.96
I-J	0 / 42	-102.1	-102.1	0.08 (1)		10.00
S-B	-5102 / 0	0.0	0.0	0.19 (1)		6.45
K-I	-5100 / 0	0.0	0.0	0.18 (1)		6.45
S-AH	0 / 0	-38.5	-38.5	0.15 (2)		10.00
AH-R	0 / 0	-38.5	-38.5	0.15 (2)		10.00
R-AI	0 / 4477	-38.5	-38.5	0.42 (1)		10.00
AI-AJ	0 / 4477	-38.5	-38.5	0.42 (1)		10.00
AJ-AK	0 / 4477	-38.5	-38.5	0.42 (1)		10.00
AK-AL	0 / 4477	-38.5	-38.5	0.42 (1)		10.00
AL-Q	0 / 4477	-38.5	-38.5	0.42 (1)		10.00
Q-AM	0 / 10686	-38.5	-38.5	0.83 (1)		10.00
AM-P	0 / 10686	-38.5	-38.5	0.83 (1)		10.00
P-AN	0 / 10686	-38.5	-38.5	0.83 (1)		10.00
AN-O	0 / 10686	-38.5	-38.5	0.83 (1)		10.00
O-AO	0 / 10685	-38.5	-38.5	0.83 (1)		10.00
AO-N	0 / 10685	-38.5	-38.5	0.83 (1)		10.00
N-AP	0 / 10685	-38.5	-38.5	0.83 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-C	-944 / 0	0.12 (1)
L-H	-946 / 0	0.12 (1)
B-R	0 / 4689	0.56 (1)
L-I	0 / 4687	0.56 (1)
M-H	0 / 6973	0.86 (1)
C-Q	0 / 6972	0.86 (1)
M-G	-2648 / 0	0.32 (1)
Q-D	-2648 / 0	0.32 (1)
O-G	0 / 2199	0.27 (1)
D-O	0 / 2197	0.27 (1)
O-E	-1405 / 0	0.17 (1)

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G. ALVES
100088024
PROVINCE OF ONTARIO

DWG NO. TAM TM05135
STRUCTURAL
COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.36")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 714 (0.50")

CSI: TC=0.74/1.00 (E-G:1) , BC=0.83/1.00 (O-Q:1) , WB=0.86/1.00 (H-M:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

THE TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (Q) (INPUT = 0.90)
JSI METAL= 0.88 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

ID:guB8GiR_dNo9QvDKo3horvyNxSw-fWlawDrcXlwRJ_BeKWMW9t6o8JVyh2an1U47YGzdO4V

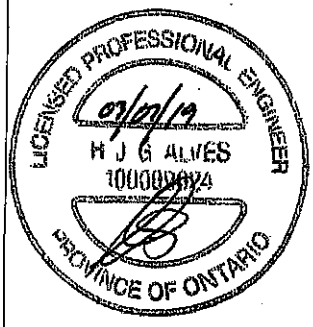
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	WEBS MAX. MEMB. UNBRAC LENGTH FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
AP- M	0 / 10685	-38.5	-38.5	0.83 (1)	10.00		
M-AQ	0 / 4475	-38.5	-38.5	0.42 (1)	10.00		
AQ-AR	0 / 4475	-38.5	-38.5	0.42 (1)	10.00		
AR-AS	0 / 4475	-38.5	-38.5	0.42 (1)	10.00		
AS-AT	0 / 4475	-38.5	-38.5	0.42 (1)	10.00		
AT- L	0 / 4475	-38.5	-38.5	0.42 (1)	10.00		
L-AU	0 / 0	-38.5	-38.5	0.15 (2)	10.00		
AU- K	0 / 0	-38.5	-38.5	0.15 (2)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-43	-48	—	FRONT	VERT	DEAD	—	—
C	3-5-5	-248	-248	—	FRONT	VERT	SNOW	—	—
E	17-11-12	-198	-198	—	BACK	VERT	TOTAL	—	—
H	32-4-11	-43	-48	—	FRONT	VERT	DEAD	—	—
H	32-4-11	-248	-248	—	FRONT	VERT	SNOW	—	—
N	21-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
O	17-11-12	-75	-95	—	BACK	VERT	TOTAL	—	—
P	14-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
T	4-0-12	-217	-217	—	BACK	VERT	TOTAL	—	—
U	6-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
V	8-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
W	10-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
X	12-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
Y	14-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
Z	16-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
AA	19-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AB	21-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AC	23-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AD	25-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AE	27-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AF	29-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AG	31-9-4	-217	-217	—	BACK	VERT	TOTAL	—	—
AH	2-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AI	4-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AJ	6-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AK	8-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AL	10-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AM	12-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AN	16-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AO	19-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AP	23-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AQ	25-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AR	27-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AS	29-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AT	31-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AU	33-10-12	-75	-95	—	BACK	VERT	TOTAL	—	—



DWG NO. TAM 7190 5735
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Thu Mar 7 08:08:40 2019 Page 1
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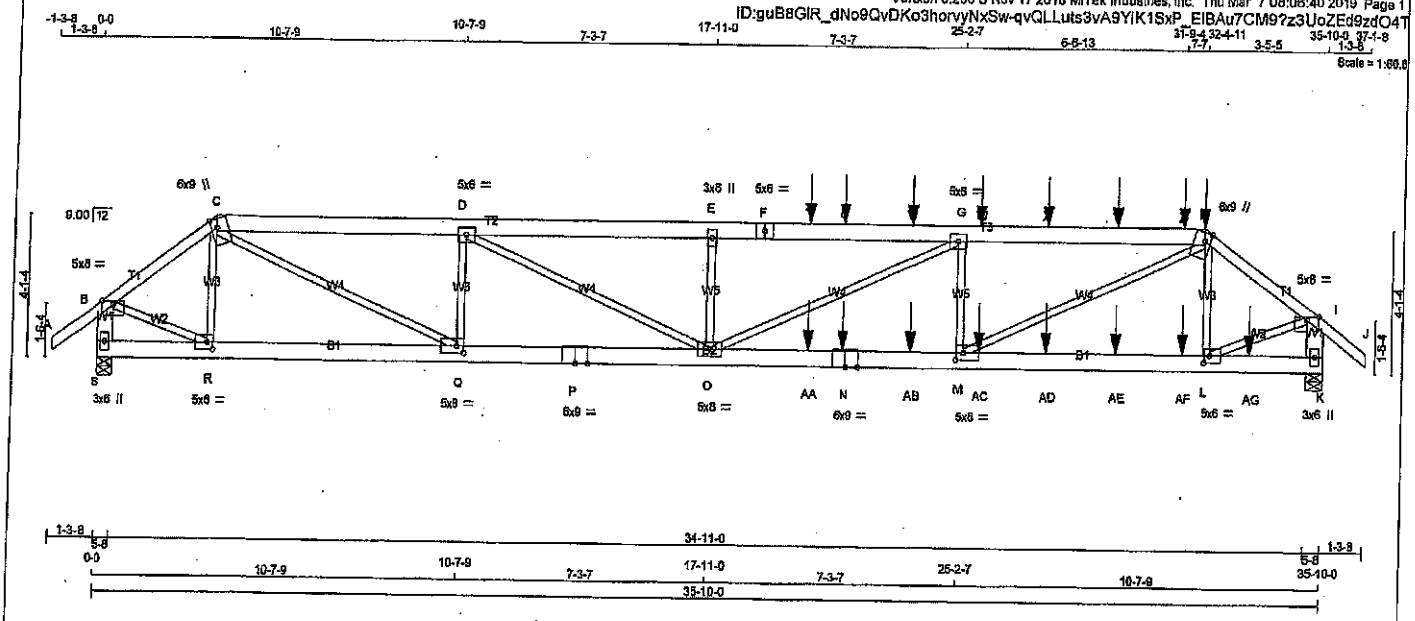

TOTAL WEIGHT = 2 X 177 = 354 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	TOP
H- J	1	12	SIDE(61.0)
C- F	2	12	TOP
F- H	2	12	SIDE(61.0)
S- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
S- P	2	12	TOP
P- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	8.0	9.0	Edge	2.00
D	TMWW-t	MT20	5.0	8.0		
E	TMW+w	MT20	3.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	6.0		
H	TTWW+m	MT20	6.0	9.0	Edge	2.00
I	TMW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.00
M	BMWW-t	MT20	5.0	8.0	2.50	2.50
N	BS-t	MT20	6.0	9.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BS-t	MT20	6.0	9.0		
Q	BMWW-t	MT20	5.0	8.0	2.50	2.50
R	BMWW-t	MT20	5.0	6.0	2.50	2.00
S	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	3222	0	3222	0	0	5-8	5-8
K	4389	0	4389	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	2398	1356 / 0	457 / 0	0 / 0	0 / 0	585 / 0	0 / 0
K	3265	1649 / 0	621 / 0	0 / 0	0 / 0	796 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.52 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B- C	-3365 / 0	-102.1	-102.1	0.16 (1)	4.94
C- D	-6924 / 0	-102.1	-102.1	0.29 (1)	4.34
D- E	-9114 / 0	-102.1	-102.1	0.37 (1)	3.60
E- F	-9114 / 0	-102.1	-102.1	0.59 (1)	3.52
F- T	-9114 / 0	-102.1	-102.1	0.59 (1)	3.52
T- U	-9114 / 0	-102.1	-102.1	0.59 (1)	3.52
U- V	-9114 / 0	-102.1	-102.1	0.59 (1)	3.52
V- G	-9114 / 0	-102.1	-102.1	0.59 (1)	3.52
G- W	-8688 / 0	-102.1	-102.1	0.57 (1)	3.61
W- X	-8688 / 0	-102.1	-102.1	0.57 (1)	3.61
X- Y	-8688 / 0	-102.1	-102.1	0.57 (1)	3.61
Y- Z	-8688 / 0	-102.1	-102.1	0.57 (1)	3.61
Z- H	-8688 / 0	-102.1	-102.1	0.57 (1)	3.61
H- I	-4725 / 0	-102.1	-102.1	0.21 (1)	4.27
I- J	0 / 42	-102.1	-102.1	0.08 (1)	10.00
S- B	-3221 / 0	0.0	0.0	0.12 (1)	7.72
K- I	-4394 / 0	0.0	0.0	0.16 (1)	6.65
S- R	0 / 0	-38.5	-38.5	0.09 (2)	10.00
R- Q	0 / 2718	-38.5	-38.5	0.25 (1)	10.00
Q- P	0 / 6924	-38.5	-38.5	0.52 (1)	10.00
P- O	0 / 6924	-38.5	-38.5	0.52 (1)	10.00
O-AA	0 / 8687	-38.5	-38.5	0.71 (1)	10.00
AA- N	0 / 8687	-38.5	-38.5	0.71 (1)	10.00
N-AB	0 / 8687	-38.5	-38.5	0.71 (1)	10.00
AB- M	0 / 8687	-38.5	-38.5	0.71 (1)	10.00
M-AC	0 / 3813	-38.5	-38.5	0.37 (1)	10.00
AC-AD	0 / 3813	-38.5	-38.5	0.37 (1)	10.00
AD-AE	0 / 3813	-38.5	-38.5	0.37 (1)	10.00
AE-AF	0 / 3813	-38.5	-38.5	0.37 (1)	10.00
AF- L	0 / 3813	-38.5	-38.5	0.37 (1)	10.00
L-AG	0 / 0	-38.5	-38.5	0.14 (2)	10.00
AG- K	0 / 0	-38.5	-38.5	0.14 (2)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R- C	-605 / 0	0.07 (1)
L- H	-746 / 22	0.09 (1)
B- R	0 / 2846	0.35 (1)
L- I	0 / 3998	0.49 (1)
M- H	0 / 5473	0.68 (1)
C- Q	0 / 4723	0.58 (1)
M- G	-1937 / 0	0.23 (1)
Q- D	-1904 / 0	0.23 (1)
O- G	0 / 477	0.06 (1)
D- O	0 / 2446	0.30 (1)
O- E	-91 / 0	0.01 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	32-4-11	-43	-48	—	FRONT	VERT	DEAD	—	—
H	32-4-11	-248	-248	—	FRONT	VERT	SNOW	—	—
N	21-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 6.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = U 999 (0.27")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = U 956 (0.45")

CSI: TC=0.89/1.00 (E-G:1) , BC=0.71/1.00 (M-O:1) , WB=0.68/1.00 (H-M:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.75 (O) (INPUT = 0.90)
JSI METAL= 0.75 (P) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/07/19
H J G ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM TM05136
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
T	20-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
U	21-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
V	23-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
W	25-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
X	27-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
Y	29-9-4	-198	-198	--	FRONT	VERT	TOTAL	--	--
Z	31-9-4	-217	-217	--	FRONT	VERT	TOTAL	--	--
AA	20-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AB	23-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AC	25-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AD	27-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AE	29-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AF	31-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--
AG	33-9-4	-75	-95	--	FRONT	VERT	TOTAL	--	--

DWG NO. TAM 7405736
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401703	T2	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Mar 7 08:08:24 2019 Page 1
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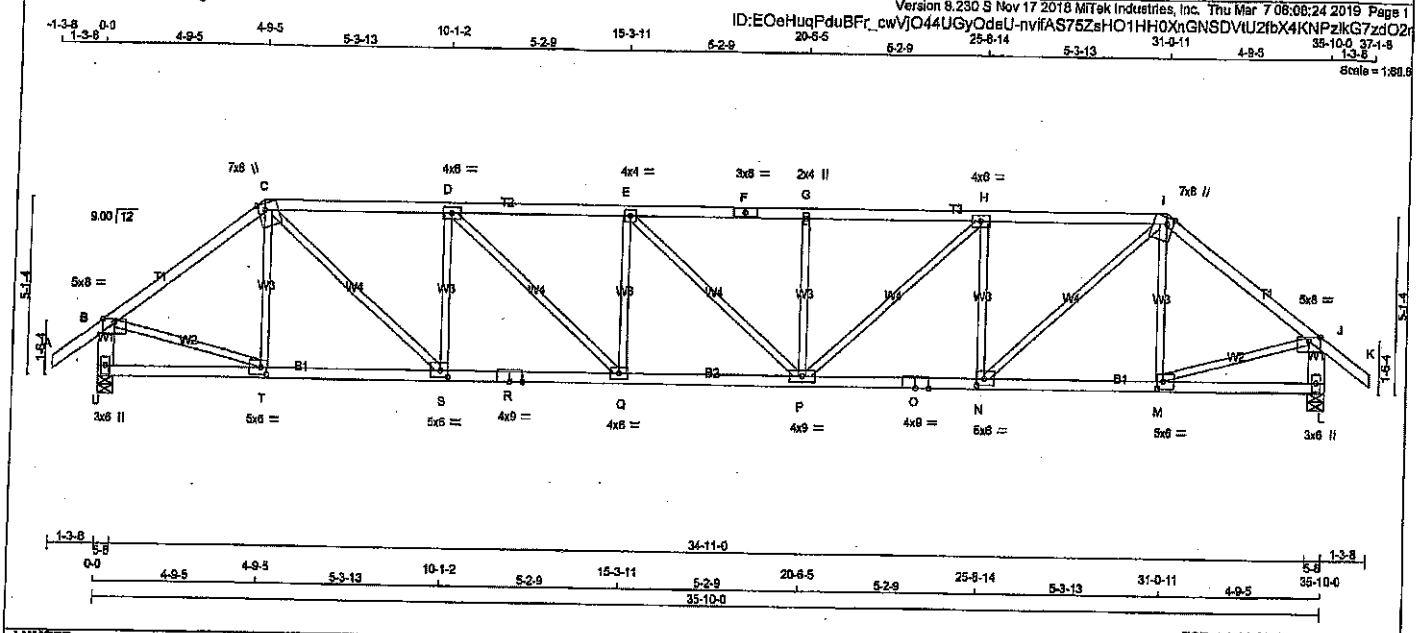

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BMWW-t	MT20	5.0	6.0	2.25	2.50
O	BS-t	MT20	4.0	9.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	6.0		
R	BS-t	MT20	4.0	9.0		
S	BMWW-t	MT20	5.0	6.0	2.25	2.50
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
U	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2762 / 0	-102.1	-102.1	0.62 (1)	3.60	
C-D	-3868 / 0	-102.1	-102.1	0.73 (1)	2.98	
D-E	-4647 / 0	-102.1	-102.1	0.86 (1)	2.61	
E-F	-4646 / 0	-102.1	-102.1	0.72 (1)	2.79	
F-G	-4646 / 0	-102.1	-102.1	0.72 (1)	2.79	
G-H	-4646 / 0	-102.1	-102.1	0.86 (1)	2.61	
H-I	-3869 / 0	-102.1	-102.1	0.73 (1)	2.98	
I-J	-2762 / 0	-102.1	-102.1	0.62 (1)	3.60	
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
U-B	-2566 / 0	0.0	0.0	0.18 (1)	6.45	
L-J	-2566 / 0	0.0	0.0	0.18 (1)	6.45	
U-T	0 / 0	-38.5	-38.5	0.18 (3)	10.00	
T-S	0 / 2199	-38.5	-38.5	0.49 (1)	10.00	
S-R	0 / 3868	-38.5	-38.5	0.72 (1)	10.00	
R-Q	0 / 3868	-38.5	-38.5	0.72 (1)	10.00	
Q-P	0 / 4647	-38.5	-38.5	0.84 (1)	10.00	
P-O	0 / 3869	-38.5	-38.5	0.71 (1)	10.00	
O-N	0 / 3869	-38.5	-38.5	0.71 (1)	10.00	
N-M	0 / 3868	-38.5	-38.5	0.49 (1)	10.00	
M-L	0 / 3868	-38.5	-38.5	0.18 (3)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-319 / 29	0.12 (1)
C-S	0 / 2246	0.51 (1)
S-D	-1310 / 0	0.50 (1)
D-Q	0 / 1060	0.24 (1)
Q-E	-517 / 0	0.20 (1)
E-P	-2 / 0	0.00 (1)
P-G	-516 / 0	0.20 (1)
P-H	0 / 1057	0.24 (1)
N-H	-1310 / 0	0.50 (1)
N-I	0 / 2246	0.51 (1)
M-I	-319 / 29	0.12 (1)
B-T	0 / 2270	0.51 (1)
M-J	0 / 2270	0.51 (1)

TOTAL WEIGHT = 2 X 150 = 300 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 672 (0.49")

CSI: TC=0.86/1.00 (G-H:1) , BC=0.84/1.00 (P-Q:1) , WB=0.51/1.00 (B-T:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.69 (N) (INPUT = 0.90)
JSI METAL= 0.90 (O) (INPUT = 1.00)

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/10/19
M. J. G. ALVES
100099024
PROVINCE OF ONTARIO

DWG NO. TAM T2@ 5/37
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401703	T3	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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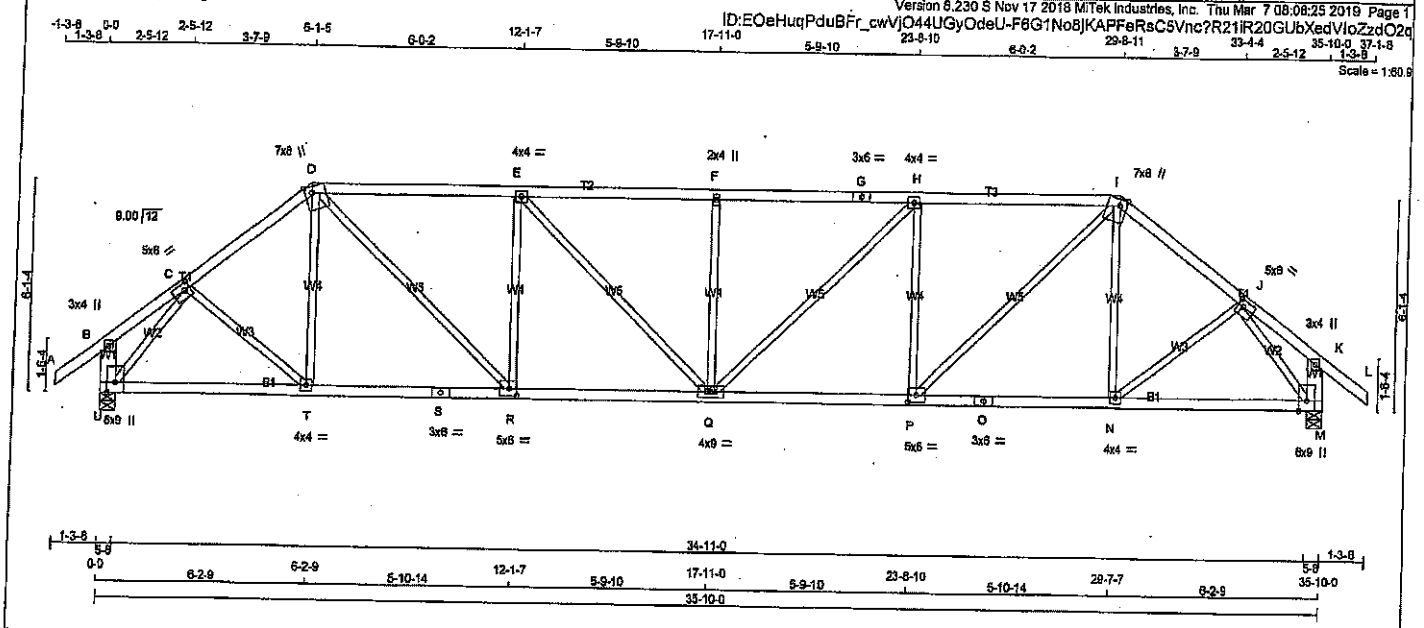

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMWW-t	MT20	5.0	6.0	2.25	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMWWW-t	MT20	4.0	9.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2660	0	2660	0	0	5-8	5-8
U	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1		0.14 (1)	10.00	T- D	-13 / 160	0.04 (3)
B-C	0 / 26	-102.1	-102.1		0.14 (1)	10.00	N- I	-13 / 160	0.04 (3)
C-D	-2812 / 0	-102.1	-102.1		0.31 (1)	3.89	C- T	0 / 458	0.10 (1)
D-E	-3547 / 0	-102.1	-102.1		0.89 (1)	2.91	D- R	0 / 1836	0.41 (1)
E-F	-3940 / 0	-102.1	-102.1		0.96 (1)	2.70	R- E	-1068 / 0	0.62 (1)
F-G	-3940 / 0	-102.1	-102.1		0.96 (1)	2.70	E- Q	0 / 553	0.12 (1)
G-H	-3940 / 0	-102.1	-102.1		0.96 (1)	2.70	Q- F	-551 / 0	0.32 (1)
H-I	-3547 / 0	-102.1	-102.1		0.89 (1)	2.91	Q- H	0 / 553	0.12 (1)
I-J	-2812 / 0	-102.1	-102.1		0.31 (1)	3.89	P- H	-1068 / 0	0.62 (1)
J-K	0 / 26	-102.1	-102.1		0.14 (1)	10.00	P- I	0 / 1836	0.41 (1)
K-L	0 / 42	-102.1	-102.1		0.14 (1)	10.00	N- J	0 / 458	0.10 (1)
U-B	-219 / 0	0.0	0.0		0.01 (1)	7.81	U- C	-2999 / 0	0.70 (1)
M-K	-219 / 0	0.0	0.0		0.01 (1)	7.81	J- M	-2999 / 0	0.70 (1)
U-T	0 / 1668	-38.5	-38.5		0.51 (2)	10.00			
T-S	0 / 2231	-38.5	-38.5		0.57 (2)	10.00			
S-R	0 / 2231	-38.5	-38.5		0.57 (2)	10.00			
R-Q	0 / 3547	-38.5	-38.5		0.70 (1)	10.00			
Q-P	0 / 3547	-38.5	-38.5		0.70 (1)	10.00			
P-O	0 / 2231	-38.5	-38.5		0.57 (2)	10.00			
O-N	0 / 2231	-38.5	-38.5		0.57 (2)	10.00			
N-M	0 / 1668	-38.5	-38.5		0.51 (2)	10.00			

TOTAL WEIGHT = 2 X 155 = 309 lb [M]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.37")

CSI: TC=0.96/1.00 (E-F:1) , BC=0.70/1.00 (Q-R:1) , WB=0.70/1.00 (J-M:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.76 (J) (INPUT = 1.00)

DWG NO. TAM T7905138
STRUCTURAL COMPONENT ONLY

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401703	T4	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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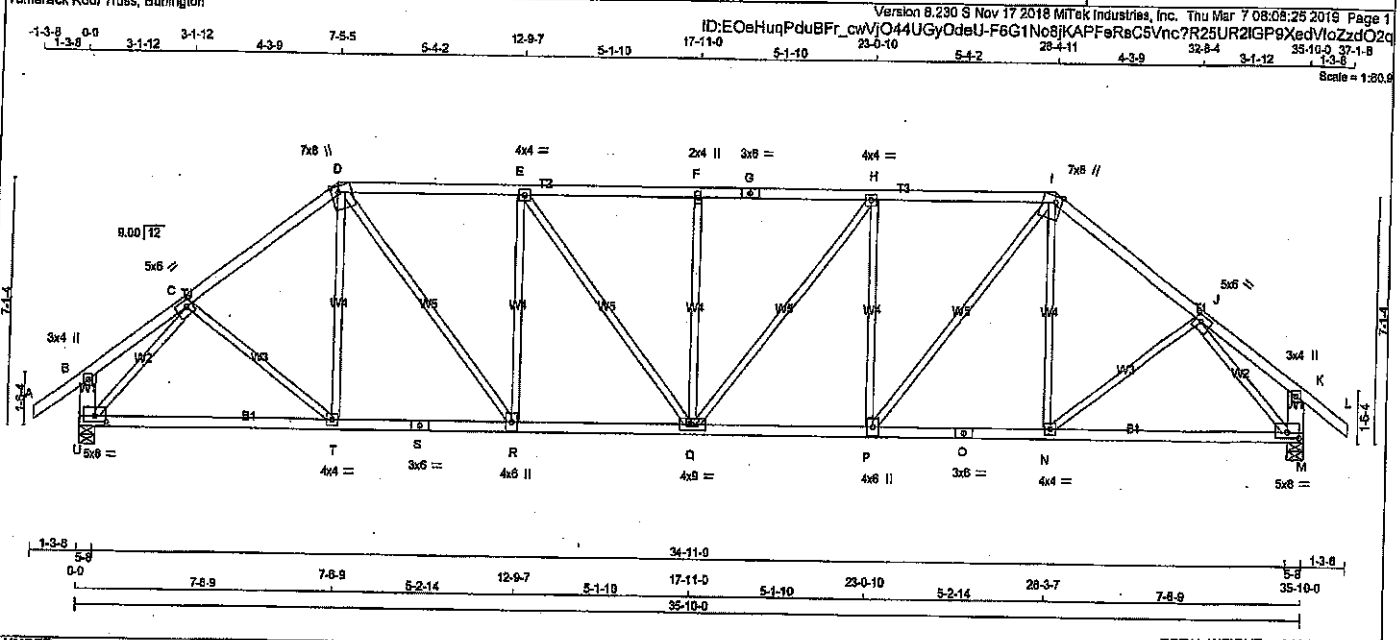


TOTAL WEIGHT = 2 X 163 = 326 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMWW-t	MT20	5.0	6.0	2.25	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	8.0	2.00	4.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	8.0	2.00	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2660	0	2660	0	0	5-8	5-8
U	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42		-102.1	-102.1	0.14 (1)		10.00
B- C	0 / 32		-102.1	-102.1	0.23 (1)		10.00
C- D	-2797 / 0		-102.1	-102.1	0.40 (1)		3.83
D- E	-3085 / 0		-102.1	-102.1	0.62 (1)		3.39
E- F	-3352 / 0		-102.1	-102.1	0.65 (1)		3.23
F- G	-3352 / 0		-102.1	-102.1	0.65 (1)		3.23
G- H	-3352 / 0		-102.1	-102.1	0.65 (1)		3.23
H- I	-3085 / 0		-102.1	-102.1	0.62 (1)		3.39
I- J	-2797 / 0		-102.1	-102.1	0.40 (1)		3.83
J- K	0 / 32		-102.1	-102.1	0.23 (1)		10.00
K- L	0 / 42		-102.1	-102.1	0.14 (1)		10.00
U- B	-242 / 0		0.0	0.0	0.02 (1)		7.81
M- K	-242 / 0		0.0	0.0	0.02 (1)		7.81
U- T	0 / 2009		-38.5	-38.5	0.62 (2)		10.00
T- S	0 / 2218		-38.5	-38.5	0.65 (2)		10.00
S- R	0 / 2218		-38.5	-38.5	0.65 (2)		10.00
R- Q	0 / 3085		-38.5	-38.5	0.61 (1)		10.00
Q- P	0 / 3085		-38.5	-38.5	0.61 (1)		10.00
P- O	0 / 2218		-38.5	-38.5	0.65 (2)		10.00
O- N	0 / 2218		-38.5	-38.5	0.65 (2)		10.00
N- M	0 / 2009		-38.5	-38.5	0.62 (2)		10.00

WEBS MEMB.	MAX. FACTORED FORCE (LBS)	FR-TO	MAX CSI (LC)
T- D	0 / 246		0.08 (3)
N- I	0 / 246		0.06 (3)
C- T	0 / 277		0.08 (2)
D- R	0 / 1413		0.32 (1)
R- E	-950 / 0		0.83 (1)
E- Q	0 / 440		0.10 (1)
Q- F	-487 / 0		0.43 (1)
Q- H	0 / 440		0.10 (1)
P- H	-950 / 0		0.83 (1)
P- I	0 / 1413		0.32 (1)
N- J	0 / 277		0.08 (2)
U- C	-3053 / 0		0.99 (1)
J- M	-3053 / 0		0.99 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.65/1.00 (F-H:1) , BC=0.65/1.00 (R-T:2) , WB=0.99/1.00 (J-M:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

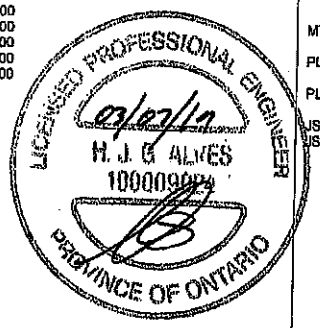
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.90 (M) (INPUT = 0.90)
USI METAL= 0.76 (J) (INPUT = 1.00)



DWG NO. TAM 1905139
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401706	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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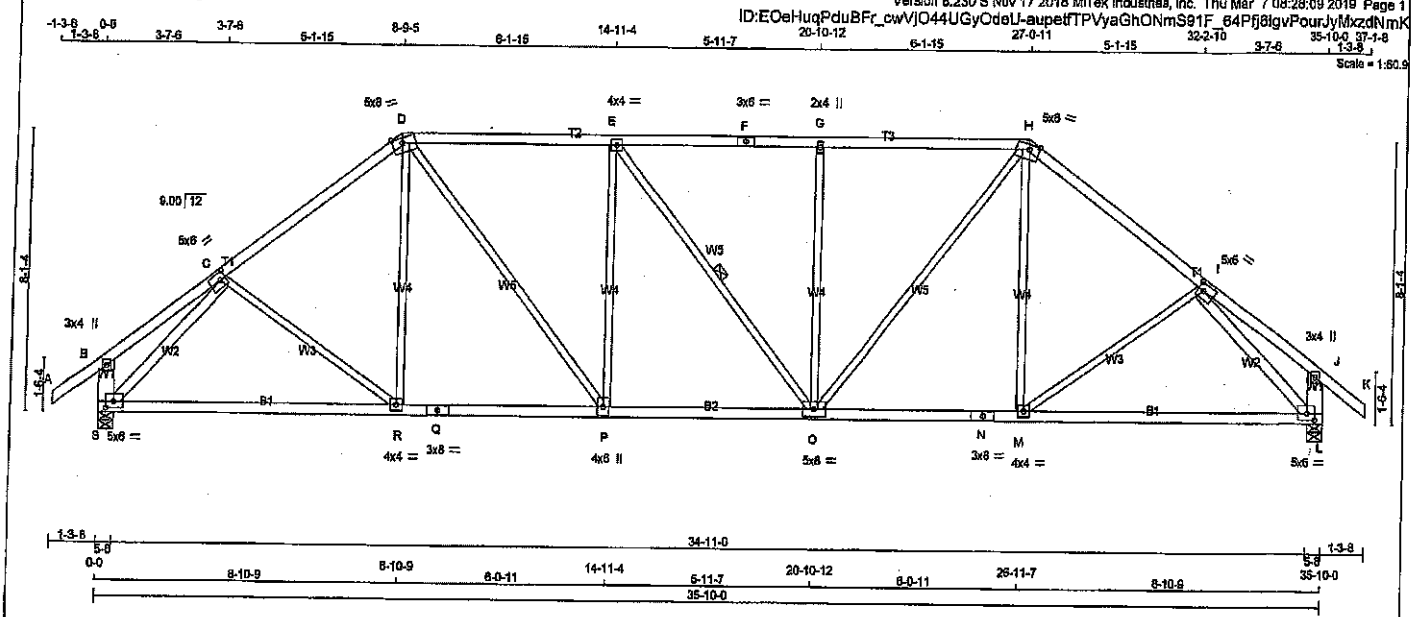

TOTAL WEIGHT = 2 X 164 = 328 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	8.0	2.50	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	8.0		
Q	BS-t	MT20	3.0	8.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
S	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)		10.00
B-C	0 / 43	-102.1	-102.1	0.35 (1)		10.00
C-D	-2752 / 0	-102.1	-102.1	0.59 (1)		3.70
D-E	-2864 / 0	-102.1	-102.1	0.78 (1)		3.33
E-F	-2863 / 0	-102.1	-102.1	0.77 (1)		3.33
F-G	-2863 / 0	-102.1	-102.1	0.77 (1)		3.33
G-H	-2863 / 0	-102.1	-102.1	0.77 (1)		3.34
H-I	-2752 / 0	-102.1	-102.1	0.59 (1)		3.70
I-J	0 / 43	-102.1	-102.1	0.35 (1)		10.00
J-K	0 / 42	-102.1	-102.1	0.14 (1)		10.00
S-B	-249 / 0	0.0	0.0	0.02 (1)		7.81
L-J	-249 / 0	0.0	0.0	0.02 (1)		7.81
S-R	0 / 2083	-38.5	-38.5	0.76 (2)		10.00
R-Q	0 / 2179	-38.5	-38.5	0.78 (2)		10.00
Q-P	0 / 2179	-38.5	-38.5	0.78 (2)		10.00
P-O	0 / 2664	-38.5	-38.5	0.60 (1)		10.00
O-N	0 / 2179	-38.5	-38.5	0.78 (2)		10.00
N-M	0 / 2179	-38.5	-38.5	0.78 (2)		10.00
M-L	0 / 2083	-38.5	-38.5	0.76 (2)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
R-D	0 / 352	0.08 (3)
M-H	0 / 353	0.08 (3)
P-E	-671 / 0	0.85 (1)
O-G	-670 / 0	0.85 (1)
C-R	0 / 179	0.04 (3)
D-P	0 / 1109	0.25 (1)
E-O	-2 / 0	0.00 (1)
O-H	0 / 1108	0.25 (1)
M-I	0 / 179	0.04 (3)
S-C	-3082 / 0	0.91 (1)
I-L	-3082 / 0	0.91 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.44")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.78/1.00 (M-O:2) , WB=0.91/1.00 (I-L:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.99 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/07/19
H. J. G. ALVES
100000
PROVINCE OF ONTARIO

DWG NO. TAM T1905140
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T6	5	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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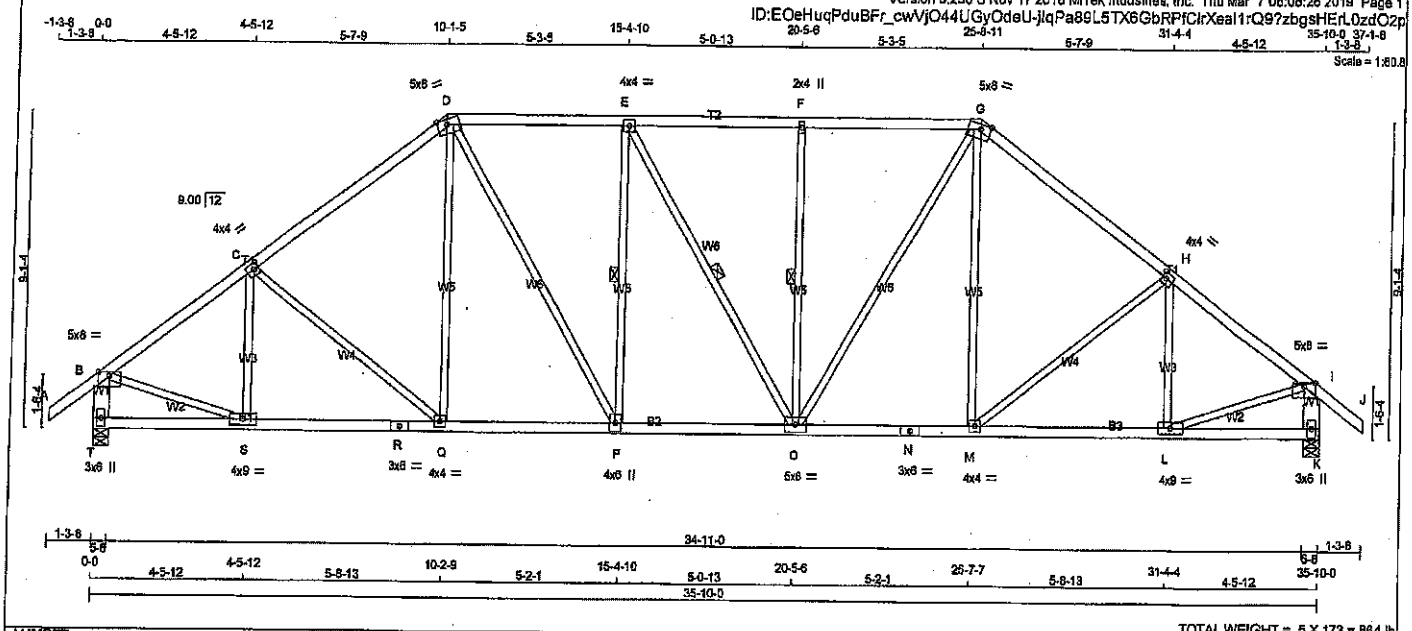


TOTAL WEIGHT = 5 X 173 = 864 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	9.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2660	0	2660	0	0	5-8	5-8
K	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
K	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, F-O, E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-2778 / 0	-102.1	-102.1	0.46 (1)	3.75
C-D	-2648 / 0	-102.1	-102.1	0.53 (1)	3.79
D-E	-2547 / 0	-102.1	-102.1	0.40 (1)	3.96
E-F	-2546 / 0	-102.1	-102.1	0.38 (1)	3.89
F-G	-2546 / 0	-102.1	-102.1	0.40 (1)	3.96
G-H	-2648 / 0	-102.1	-102.1	0.54 (1)	3.79
H-I	-2778 / 0	-102.1	-102.1	0.46 (1)	3.75
I-J	0 / 42	-102.1	-102.1	0.14 (1)	10.00
T-B	-2592 / 0	0.0	0.0	0.18 (1)	6.45
K-I	-2592 / 0	0.0	0.0	0.18 (1)	6.45
T-S	0 / 0	-38.5	-38.5	0.17 (3)	10.00
S-R	0 / 2257	-38.5	-38.5	0.50 (1)	10.00
R-Q	0 / 2257	-38.5	-38.5	0.50 (1)	10.00
Q-P	0 / 2092	-38.5	-38.5	0.47 (1)	10.00
P-O	0 / 2548	-38.5	-38.5	0.51 (1)	10.00
O-N	0 / 2092	-38.5	-38.5	0.47 (1)	10.00
N-M	0 / 2092	-38.5	-38.5	0.47 (1)	10.00
M-L	0 / 2257	-38.5	-38.5	0.50 (1)	10.00
L-K	0 / 0	-38.5	-38.5	0.17 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q-D	0 / 385	0.09 (2)
M-G	0 / 387	0.09 (2)
S-C	-390 / 5	0.14 (1)
P-E	-570 / 0	0.31 (1)
O-F	-569 / 0	0.31 (1)
L-H	-390 / 5	0.14 (1)
C-Q	-216 / 0	0.23 (1)
D-P	0 / 891	0.20 (1)
E-O	-3 / 0	0.00 (1)
O-G	0 / 887	0.20 (1)
M-H	-215 / 0	0.23 (1)
B-S	0 / 2335	0.53 (1)
L-I	0 / 2335	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.51/1.00 (O-P:1) , WB=0.53/1.00 (B-S:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

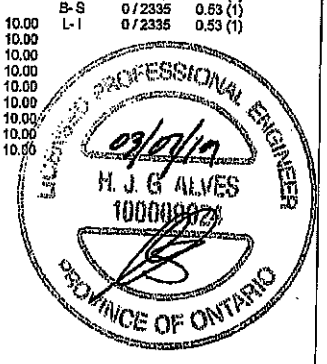
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (B) (INPUT = 0.90)
JSI METAL= 0.68 (R) (INPUT = 1.00)



DWG NO. TAM 7405741
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401703	T7	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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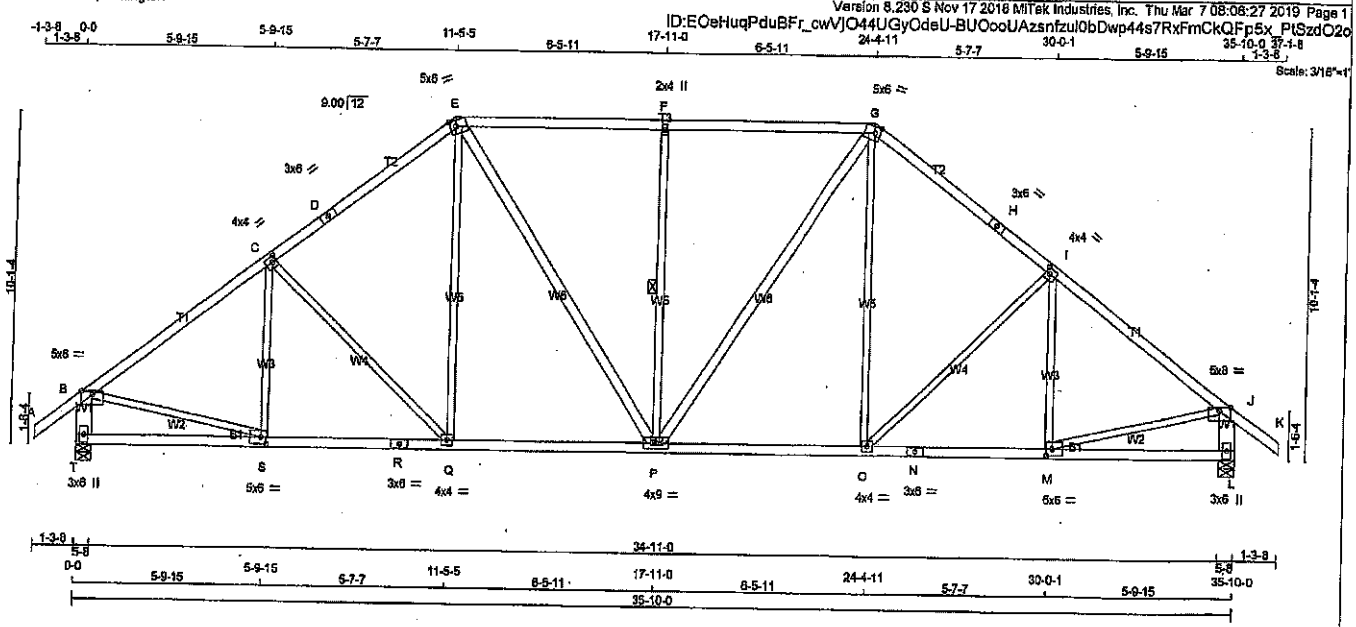


TOTAL WEIGHT = 2 X 176 = 352 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	Edge	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.56 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-2836 / 0	-102.1	-102.1	0.63 (1)	3.56	
C-D	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
D-E	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
E-F	-2366 / 0	-102.1	-102.1	0.65 (1)	3.70	
F-G	-2366 / 0	-102.1	-102.1	0.65 (1)	3.70	
G-H	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
H-I	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
I-J	-2836 / 0	-102.1	-102.1	0.63 (1)	3.56	
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
T-B	-2567 / 0	0.0	0.0	0.17 (1)	6.48	
L-J	-2567 / 0	0.0	0.0	0.17 (1)	6.48	
T-S	0 / 0	-38.5	-38.5	0.23 (3)	10.00	
S-R	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
R-Q	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
Q-P	0 / 2016	-38.5	-38.5	0.52 (2)	10.00	
P-O	0 / 2016	-38.5	-38.5	0.52 (2)	10.00	
O-N	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
N-M	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
M-L	0 / 0	-38.5	-38.5	0.23 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FORCE (LBS)	MAX CSI (LC)
S-C	-229 / 100	0.12 (1)
C-Q	-407 / 0	0.54 (1)
Q-E	0 / 529	0.12 (2)
E-P	0 / 636	0.10 (1)
P-F	-606 / 0	0.56 (1)
P-G	0 / 636	0.10 (1)
O-G	0 / 529	0.12 (2)
O-I	-407 / 0	0.54 (1)
M-I	-229 / 100	0.12 (1)
B-S	0 / 2350	0.53 (1)
M-J	0 / 2350	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.16")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.16")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.53/1.00 (Q-S:2) , WB=0.56/1.00 (F-P:1) , SSI=0.32/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

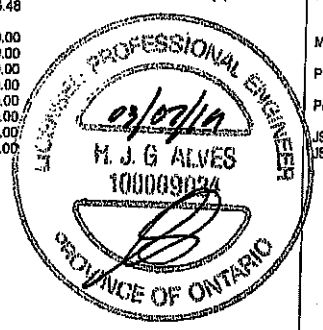
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)



DWG NO. TAM 1905742
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401706	T8	2	1	GREENPARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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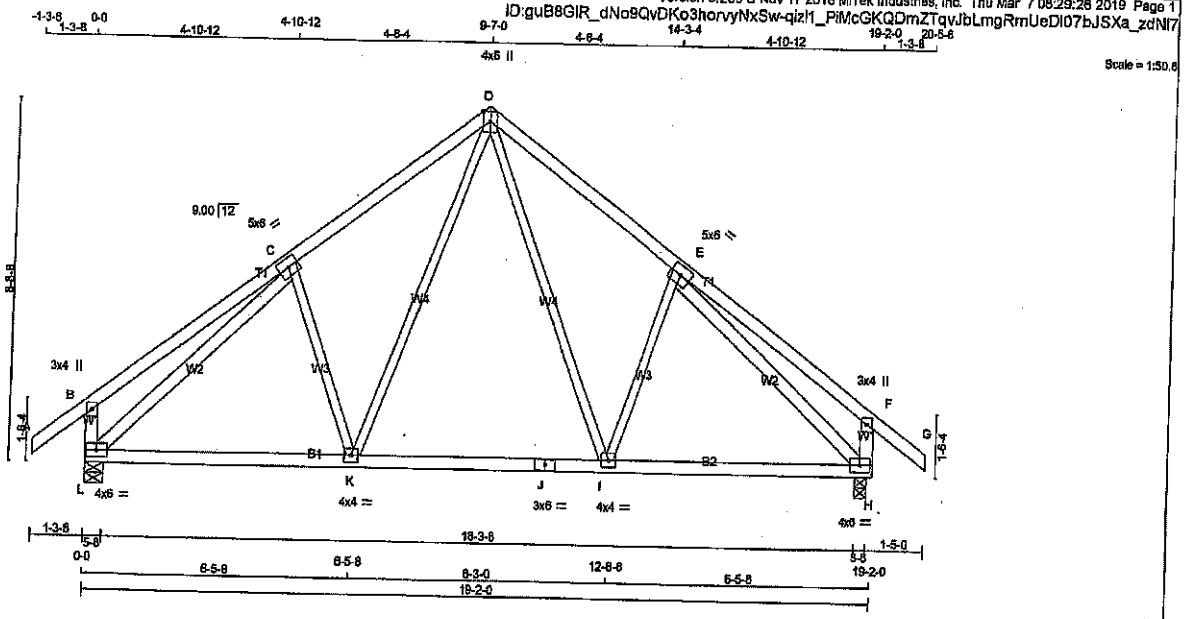

TOTAL WEIGHT = 2 X 82 = 165 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1488	0	1488	0	0	5-8	5-8
H	1488	0	1488	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0
H	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.41 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	0 / 35	-102.1	-102.1	0.38 (1)	10.00	
C-D	-1235 / 0	-102.1	-102.1	0.32 (1)	5.41	
D-E	-1235 / 0	-102.1	-102.1	0.32 (1)	5.41	
E-F	0 / 35	-102.1	-102.1	0.38 (1)	10.00	
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
L-B	-324 / 0	0.0	0.0	0.03 (1)	7.81	
H-F	-324 / 0	0.0	0.0	0.03 (1)	7.81	
L-K	0 / 1059	-38.5	-38.5	0.41 (2)	10.00	
K-J	0 / 766	-38.5	-38.5	0.37 (2)	10.00	
J-I	0 / 768	-38.5	-38.5	0.37 (2)	10.00	
I-H	0 / 1059	-38.5	-38.5	0.41 (2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-I	0 / 564	0.13 (1)
I-E	-282 / 23	0.12 (1)
K-D	0 / 564	0.13 (1)
C-K	-282 / 23	0.12 (1)
L-C	-1500 / 0	0.88 (1)
E-H	-1500 / 0	0.88 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.38/1.00 (B-C:1) , BC=0.41/1.00 (K-L:2) , WB=0.88/1.00 (E-H:1) , SSI=0.19/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT = 0.90)
JSI METAL= 0.39 (E) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G. ALVES
100090084
PROVINCE OF ONTARIO

DWG NO. TAM T1905143
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401706	T8S1	2	1	TRUSS DESC.	

Tamarack Roof Trues, Burlington

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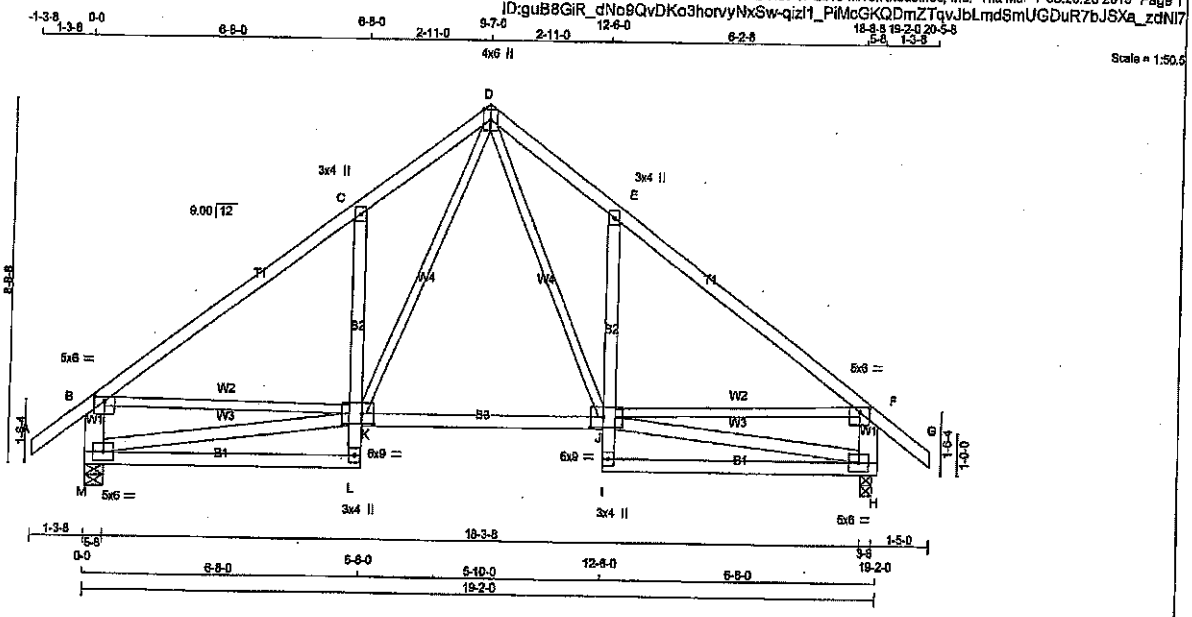


TOTAL WEIGHT = 2 X 107 = 214 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - L	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
I - E	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	1.25	3.00
C	TMV+p	MT20	3.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	TMVW-p	MT20	5.0	6.0	1.25	3.00
H	BMVW1-t	MT20	5.0	6.0		
I	BMV+p	MT20	3.0	4.0		
J	BVMWWW-I	MT20	6.0	9.0	3.00	3.50
K	BVMWWW-I	MT20	6.0	9.0	3.00	3.50
L	BMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	1488	0	1488	0	0	5-8	5-8
H	1488	0	1488	0	0	3-8	3-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0
H	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.68 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	-1487 / 0	-102.1	-102.1	0.57 (1)	4.68	
C-D	-1582 / 0	-102.1	-102.1	0.42 (1)	4.75	
D-E	-1582 / 0	-102.1	-102.1	0.42 (1)	4.75	
E-F	-1487 / 0	-102.1	-102.1	0.57 (1)	4.68	
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
M-B	-1360 / 0	0.0	0.0	0.09 (1)	7.81	
H-F	-1360 / 0	0.0	0.0	0.09 (1)	7.81	
M-L	0 / 16	-38.5	-38.5	0.38 (3)	10.00	
L-K	0 / 160	0.0	0.0	0.04 (2)	10.00	
K-C	-659 / 0	0.0	0.0	0.16 (1)	7.81	
K-J	0 / 850	-38.5	-38.5	0.43 (2)	10.00	
I-J	0 / 160	0.0	0.0	0.04 (2)	10.00	
J-E	-659 / 0	0.0	0.0	0.16 (1)	7.81	
I-H	0 / 16	-38.5	-38.5	0.38 (3)	10.00	

WEBS FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D-J	0 / 935	0.21 (1)
K-D	0 / 935	0.21 (1)
M-K	-16 / 0	0.01 (1)
J-H	-16 / 0	0.01 (1)
B-K	0 / 1218	0.27 (1)
J-F	0 / 1218	0.27 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD .

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.57/1.00 (B-C:1) , BC=0.43/1.00 (J-K:2) , WB=0.27/1.00 (B-K:1) , SSI=0.24/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

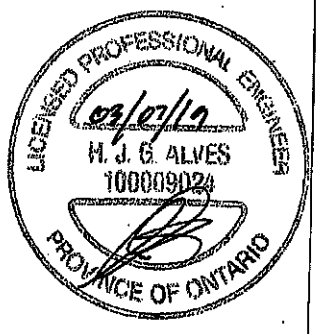
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (J) (INPUT = 0.90)
JSI METAL= 0.35 (F) (INPUT = 1.00)



DWG NO. TAM 19051/44
STRUCTURAL COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T9	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Thu Mar 7 06:08:29 2019 Page 1
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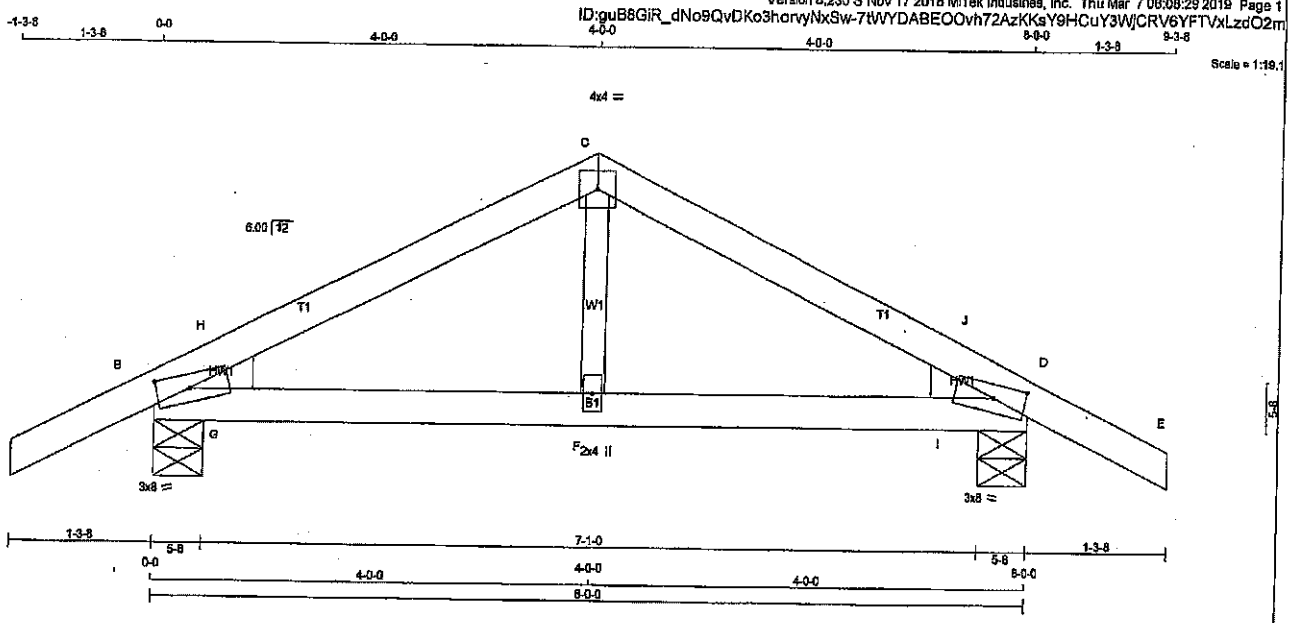

TOTAL WEIGHT = 26 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	3.50
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	8.0	1.50	3.50
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	701	0	701	0	0	5-8	5-8	2x4 L
D	701	0	701	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	515	311 / 0	84 / 0	0 / 0	0 / 0	120 / 0	0 / 0
D	515	311 / 0	84 / 0	0 / 0	0 / 0	120 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 25	-102.1	-102.1	0.15 (1)	10.00	
B-H	-683 / 0	-102.1	-102.1	0.05 (1)	6.25	
H-C	-647 / 0	-102.1	-102.1	0.20 (1)	6.25	
C-J	-647 / 0	-102.1	-102.1	0.20 (1)	6.25	
J-D	-683 / 0	-102.1	-102.1	0.05 (1)	6.25	
D-E	0 / 25	-102.1	-102.1	0.15 (1)	10.00	
B-G	0 / 575	-38.5	-38.5	0.25 (1)	10.00	
G-F	0 / 575	-38.5	-38.5	0.27 (1)	10.00	
F-I	0 / 575	-38.5	-38.5	0.27 (1)	10.00	
I-D	0 / 575	-38.5	-38.5	0.25 (1)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 289	0.07 (2)
G-H	-132 / 57	0.00 (1)
I-J	-132 / 57	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.20/1.00 (C-J:1) , BC=0.27/1.00 (F-G:1) , WB=0.07/1.00 (C-F:2) , SSI=0.14/1.00 (C-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.30 (C) (INPUT = 0.90)
JSI METAL= 0.13 (D) (INPUT = 1.00)

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G. ALVES
100009[illegible]
PROVINCE OF ONTARIO

DWG NO. TAM T1905145
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	T9Z	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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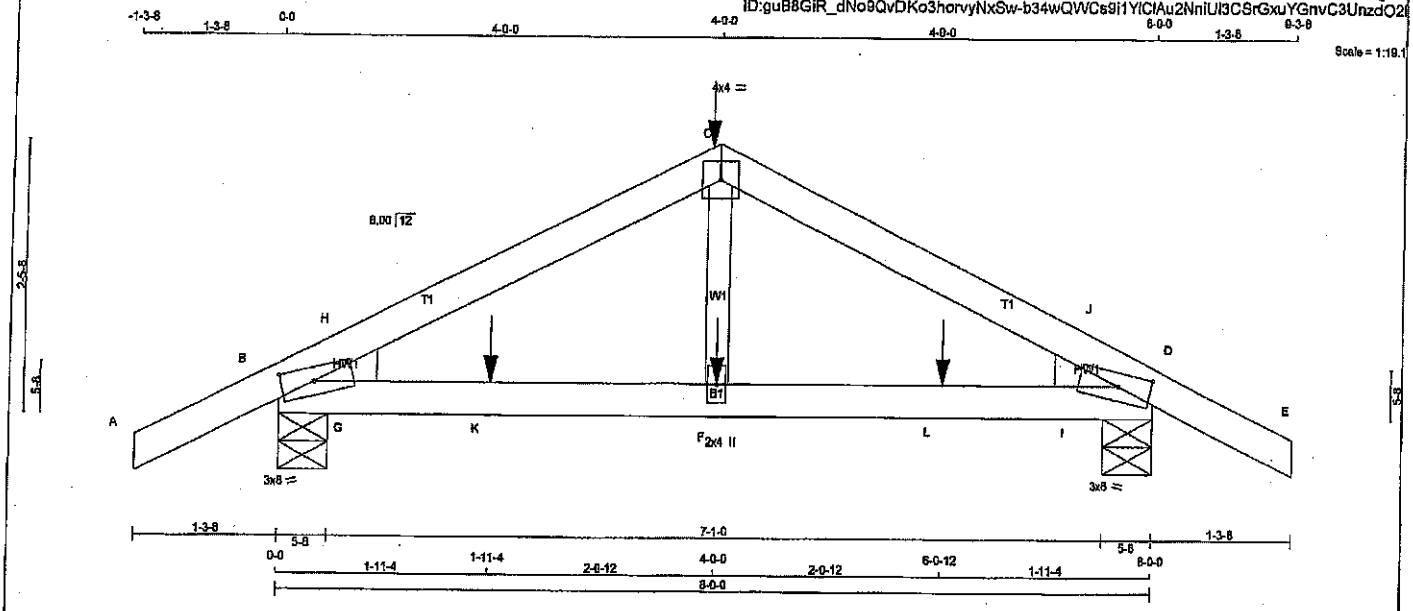

TOTAL WEIGHT = 26 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	6.0	1.50	3.50
C	TTW-p	MT20	4.0	4.0		
D	TMBH1-m	MT20	3.0	6.0	1.50	3.50
F	BMW+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	939	0	939	0	0	5-8	5-8	2x4 L
D	939	0	939	0	0	5-8	5-8	2x4 R

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	693	411 / 0	117 / 0	0 / 0	0 / 0	165 / 0	0 / 0
D	693	411 / 0	117 / 0	0 / 0	0 / 0	165 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.73 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 25	-102.1	-102.1	0.15 (1)		10.00
B-H	-1185 / 0	-102.1	-102.1	0.08 (1)		5.76
H-C	-1113 / 0	-102.1	-102.1	0.20 (1)		5.73
C-J	-1113 / 0	-102.1	-102.1	0.20 (1)		5.73
J-D	-1185 / 0	-102.1	-102.1	0.08 (1)		5.76
D-E	0 / 25	-102.1	-102.1	0.15 (1)		10.00
B-G	0 / 991	-38.5	-38.5	0.28 (1)		10.00
G-K	0 / 991	-38.5	-38.5	0.31 (1)		10.00
K-F	0 / 991	-38.5	-38.5	0.31 (1)		10.00
F-L	0 / 991	-38.5	-38.5	0.31 (1)		10.00
L-I	0 / 991	-38.5	-38.5	0.31 (1)		10.00
I-D	0 / 991	-38.5	-38.5	0.28 (1)		10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
F-C	0 / 319	0.08 (2)
G-H	-60 / 92	0.00 (1)
I-J	-60 / 92	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-0-0	-377	-377	—	BACK	VERT	TOTAL	—	—
F	4-0-0	-60	-60	—	BACK	VERT	TOTAL	—	—
K	1-11-4	-19	-23	—	BACK	VERT	TOTAL	—	—
L	6-0-12	-19	-23	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.20/1.00 (C-J:1) , BC=0.31/1.00 (F-G:1) , WB=0.08/1.00 (C-F:2) , SSI=0.14/1.00 (C-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.46 (D) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G ALVES
100090024
PROVINCE OF ONTARIO

DWG NO. TAM T7405746
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	PB1	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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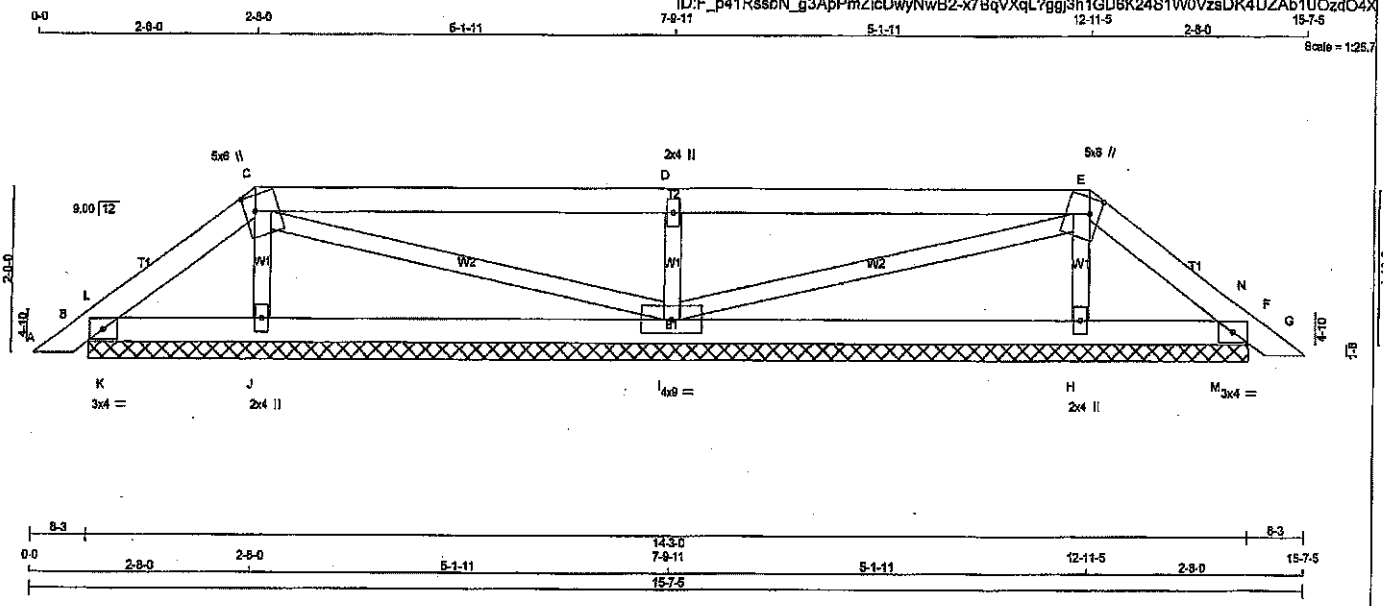

TOTAL WEIGHT = 2 X 47 = 94 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-t	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	209	0	209	0	0	14-3-0	14-3-0
F	209	0	209	0	0	14-3-0	14-3-0
J	418	0	418	0	0	14-3-0	14-3-0
I	672	0	672	0	0	14-3-0	14-3-0
H	418	0	418	0	0	14-3-0	14-3-0

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	145	113 / 0	5 / 0	0 / 0	0 / 0	27 / 0	0 / 0
F	145	113 / 0	5 / 0	0 / 0	0 / 0	27 / 0	0 / 0
J	323	146 / 0	88 / 0	0 / 0	0 / 0	89 / 0	0 / 0
I	644	379 / 0	112 / 0	0 / 0	0 / 0	153 / 0	0 / 0
H	323	146 / 0	88 / 0	0 / 0	0 / 0	89 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- L	-28 / 12	-102.1	-102.1	0.02 (1)	6.25
L- C	-65 / 0	-102.1	-102.1	0.04 (1)	6.25
C- D	-8 / 0	-102.1	-102.1	0.46 (1)	10.00
D- E	-8 / 0	-102.1	-102.1	0.46 (1)	10.00
E- N	-65 / 0	-102.1	-102.1	0.04 (1)	6.25
N- F	-28 / 12	-102.1	-102.1	0.02 (1)	6.25
F- G	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- K	0 / 47	-38.5	-38.5	0.08 (1)	10.00
K- J	0 / 47	-38.5	-38.5	0.12 (2)	10.00
J- I	0 / 32	-38.5	-38.5	0.17 (3)	10.00
I- H	0 / 32	-38.5	-38.5	0.17 (3)	10.00
H- M	0 / 47	-38.5	-38.5	0.12 (2)	10.00
M- F	0 / 47	-38.5	-38.5	0.08 (1)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-233 / 0	0.03 (1)
C- I	-24 / 0	0.01 (1)
I- D	-656 / 0	0.10 (1)
I- E	-24 / 0	0.01 (1)
H- E	-233 / 0	0.03 (1)
K- L	-155 / 0	0.00 (1)
M- N	-155 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.46/1.00 (C-D:1) , BC=0.17/1.00 (H-I:3) , WB=0.10/1.00 (D-I:1) , SSI=0.26/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.14 (D) (INPUT = 1.00)

DWG NO. TAM 17005147
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	PB2	1	1	TRUSS DESC.	

Tamarack Roof Trues, Burlington

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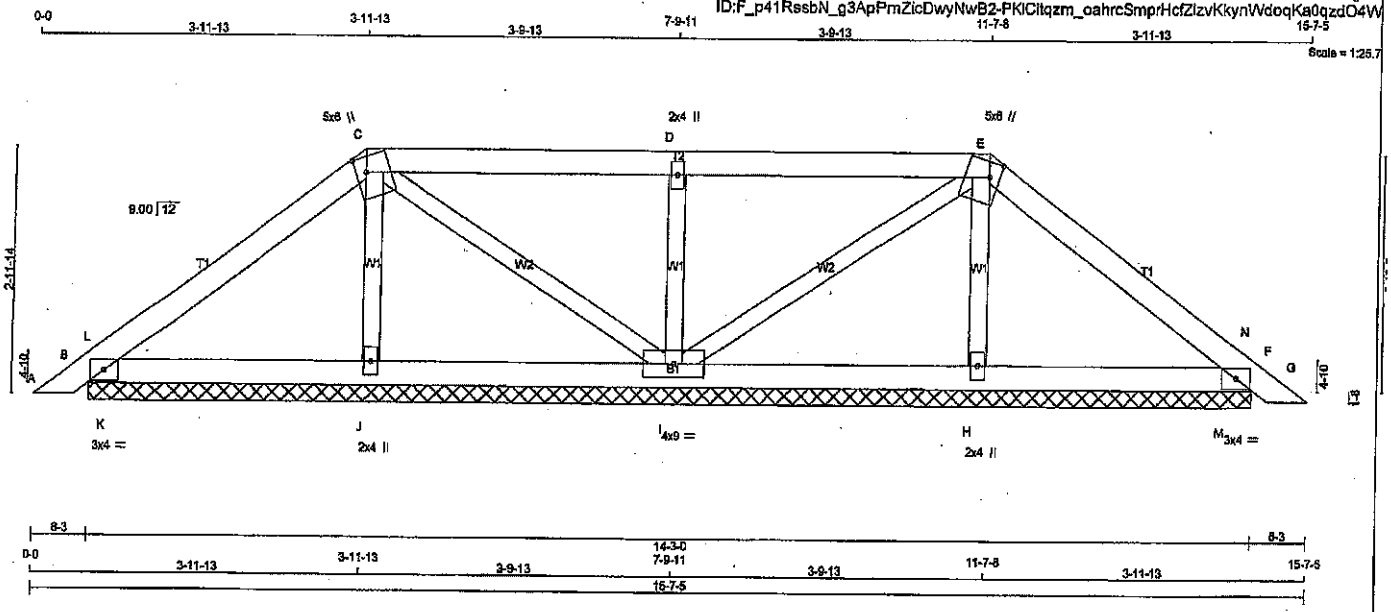


TOTAL WEIGHT = 49 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
B - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	TTWW+m	MT20	5.0	6.0	2.25	1.50
D	TMW+w	MT20	2.0	4.0		
E	TTWW+m	MT20	5.0	6.0	2.25	1.50
F	TMB1-I	MT20	3.0	4.0		
H	BMW1+w	MT20	2.0	4.0		
I	BMWWW1-I	MT20	4.0	9.0		
J	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	343	0	343	0	0	14-3-0	14-3-0
F	343	0	343	0	0	14-3-0	14-3-0
J	376	0	376	0	0	14-3-0	14-3-0
I	683	0	683	0	0	14-3-0	14-3-0
H	376	0	376	0	0	14-3-0	14-3-0

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	248	162 / 0	32 / 0	0 / 0	0 / 0	55 / 0	0 / 0
F	248	162 / 0	32 / 0	0 / 0	0 / 0	55 / 0	0 / 0
J	292	133 / 0	78 / 0	0 / 0	0 / 0	80 / 0	0 / 0
I	501	306 / 0	79 / 0	0 / 0	0 / 0	116 / 0	0 / 0
H	292	133 / 0	78 / 0	0 / 0	0 / 0	80 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, F, J, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- L	-63 / 0	-102.1	-102.1	0.04 (1)	6.25
L- C	-116 / 0	-102.1	-102.1	0.14 (1)	6.25
C- D	-24 / 0	-102.1	-102.1	0.25 (1)	6.25
D- E	-24 / 0	-102.1	-102.1	0.25 (1)	6.25
E- N	-116 / 0	-102.1	-102.1	0.14 (1)	6.25
N- F	-63 / 0	-102.1	-102.1	0.04 (1)	6.25
F- G	0 / 17	-102.1	-102.1	0.03 (1)	10.00
B- K	0 / 88	-38.5	-38.5	0.12 (1)	10.00
K- J	0 / 88	-38.5	-38.5	0.13 (1)	10.00
J- I	0 / 80	-38.5	-38.5	0.11 (2)	10.00
I- H	0 / 80	-38.5	-38.5	0.11 (2)	10.00
H- M	0 / 88	-38.5	-38.5	0.13 (1)	10.00
M- F	0 / 88	-38.5	-38.5	0.12 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- C	-187 / 0	0.03 (1)
C- I	-66 / 0	0.02 (1)
I- D	-466 / 0	0.08 (1)
I- E	-66 / 0	0.02 (1)
H- E	-187 / 0	0.03 (1)
K- L	-279 / 30	0.00 (1)
M- N	-279 / 30	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (C-D:1) , BC=0.13/1.00 (J-K:1) , WB=0.08/1.00 (D-I:1) , SSI=0.24/1.00 (F-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

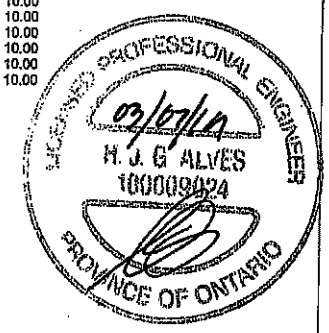
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.10 (D) (INPUT = 1.00)



DWG NO. TAM 1905148
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	J1	25	1	TRUSS DESC.	

Tamarack Roof Trues, Burlington

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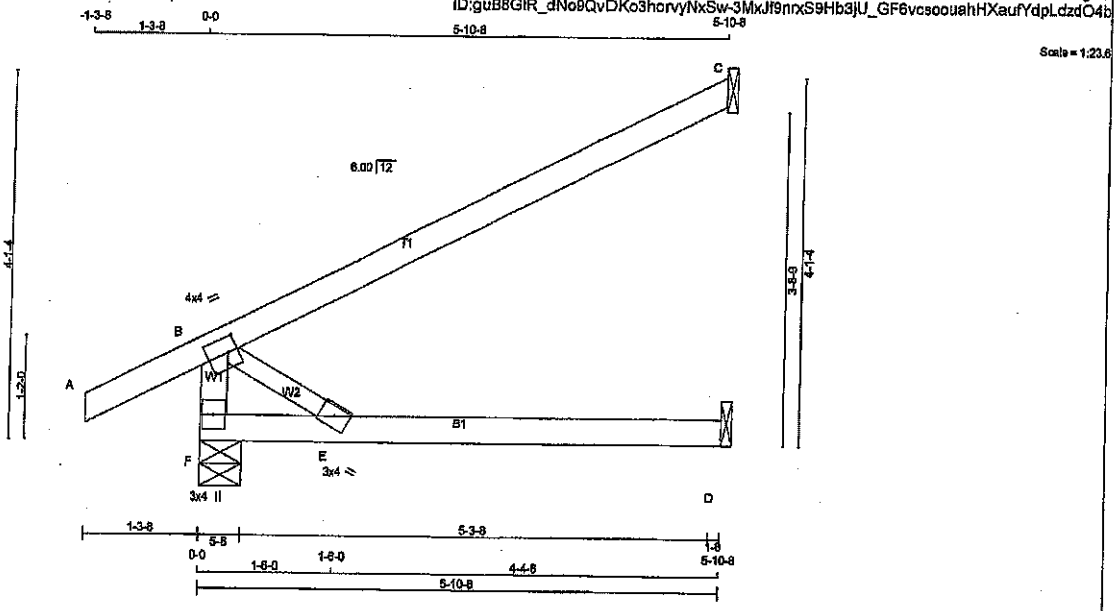

TOTAL WEIGHT = 25 X 18 = 452 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TM/W-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	552	0	552	0	0	5-8	5-8
C	300	0	300	0	0	1-8	1-8
D	113	0	144	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	404	249 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	206	171 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-438 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
B- C	0 / 0	-102.1	-102.1	0.60 (1)	10.00	
F- E	0 / 0	-38.5	-38.5	0.24 (3)	10.00	
E- D	0 / 0	-38.5	-38.5	0.31 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.31/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.80)
JSI METAL= 0.07 (B) (INPUT = 1.00)

THIS SHOP DRAWING MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

RECEIVED
JUN 10 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM 77905154
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	J2	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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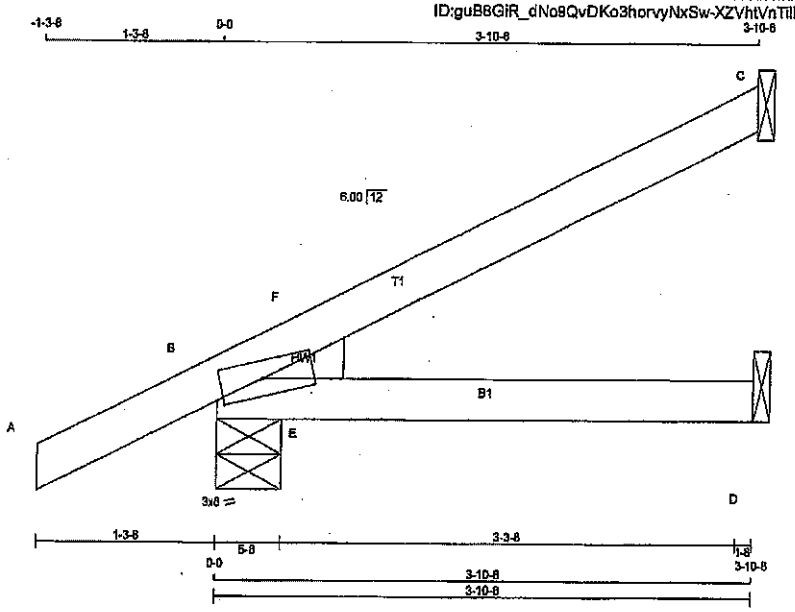

TOTAL WEIGHT = 12 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	8.0	1.50	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	174	0	174	0	0	1-8	1-8	
B	411	0	411	0	0	5-8	5-8	2x4 L
D	99	0	105	0	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	122	92 / 0	7 / 0	0 / 0	0 / 0	23 / 0	0 / 0
B	299	191 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0
D	82	21 / 0	34 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 25	-102.1	-102.1	0.13 (1)	10.00		E-F	-137 / 57	0.00 (1)
B-F	-41 / 0	-102.1	-102.1	0.07 (3)	6.25				
F-C	0 / 7	-102.1	-102.1	0.20 (1)	10.00				
B-E	0 / 0	-38.5	-38.5	0.16 (1)	10.00				
E-D	0 / 0	-38.5	-38.5	0.17 (1)	10.00				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.20/1.00 (C-F:1) , BC=0.17/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.15/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1557	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.14 (B) (INPUT = 0.80)
JSI METAL= 0.03 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/07/19
H. J. G ALVES
100090024
PROVINCE OF ONTARIO

DWG No. TAM 1905155
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	J3	6	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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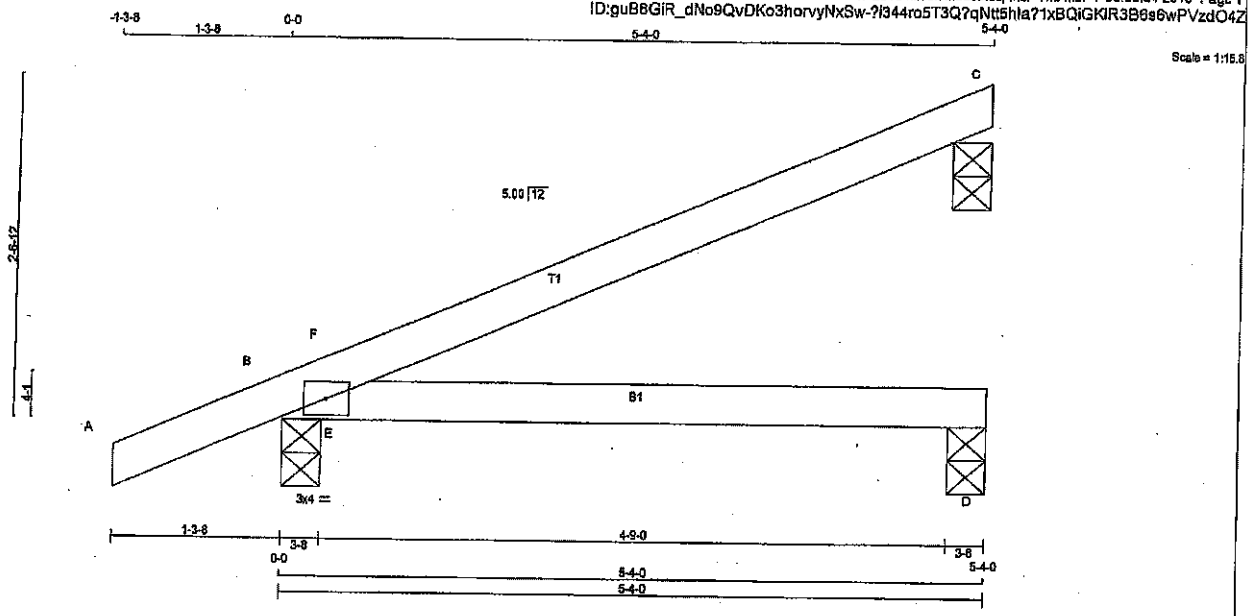


TOTAL WEIGHT = 6 X 14 = 86 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	513	0	513	0	0	3-8	3-8
D	130	0	142	0	0	3-8	3-8
C	245	0	245	0	0	3-8	3-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	375	233 / 0	56 / 0	0 / 0	0 / 0	86 / 0	0 / 0
D	109	24 / 0	48 / 0	0 / 0	0 / 0	37 / 0	0 / 0
C	171	131 / 0	8 / 0	0 / 0	0 / 0	32 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX. CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A- B	0 / 25	-102.1	-102.1	0.13 (1)	10.00		E- F	-302 / 137	0.00 (1)
B- F	-70 / 12	-102.1	-102.1	0.11 (3)	6.25				
F- C	-1 / 6	-102.1	-102.1	0.40 (1)	10.00				
B- E	0 / 0	-38.5	-38.5	0.24 (1)	10.00				
E- D	0 / 0	-38.5	-38.5	0.30 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.18")
CALCULATED VERT. DEFL.(LL) = L/ 911 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 527 (0.12")

CSI: TC=0.40/1.00 (C-F:1) , BC=0.30/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.29/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

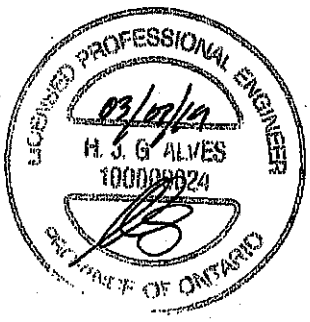
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.33 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. IAM 71905752
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	J4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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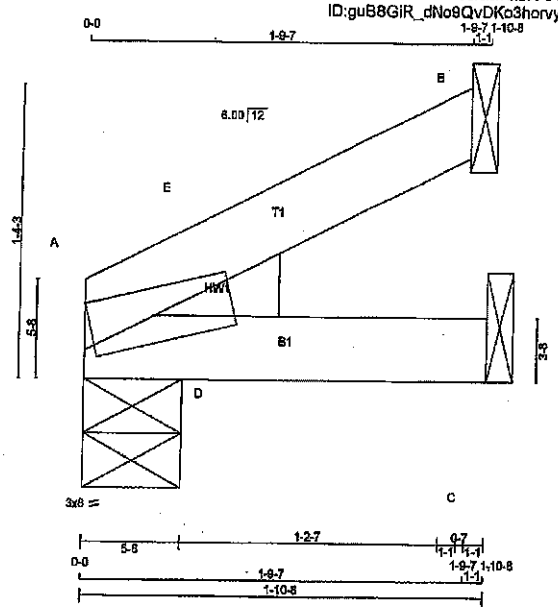


TOTAL WEIGHT = 2 X 5 = 10 lb (M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMBH1-m	MT20	3.0	8.0		Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
B	77	0	77	0	0	1-8	1-8	
C	50	0	51	0	0	1-8	1-8	
A	128	0	128	0	0	5-8	5-8	2x4 L

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) B , C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	55	39 / 0	5 / 0	0 / 0	0 / 0	11 / 0	0 / 0
C	40	12 / 0	15 / 0	0 / 0	0 / 0	13 / 0	0 / 0
A	96	52 / 0	19 / 0	0 / 0	0 / 0	24 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, A

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- E	-20 / 0	-102.1	-102.1	0.03 (1)		6.25
E- B	0 / 9	-102.1	-102.1	0.04 (1)		10.00
A- D	0 / 0	-38.5	-38.5	0.04 (1)		10.00
D- C	0 / 0	-38.5	-38.5	0.04 (1)		10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- E	-37 / 21	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.00")

CSI: TC=0.04/1.00 (B-E:1) , BC=0.04/1.00 (A-D:1) , WB=0.00/1.00 (D-E:1) , SSI=0.05/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

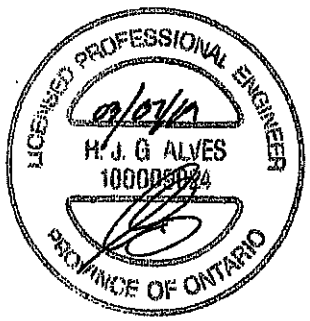
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.02 (A) (INPUT = 0.90)
JSI METAL= 0.00 (A) (INPUT = 1.00)



DWG NO. TAM T1905157
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREENPARK HOMES	DRWG NO.
401703	J5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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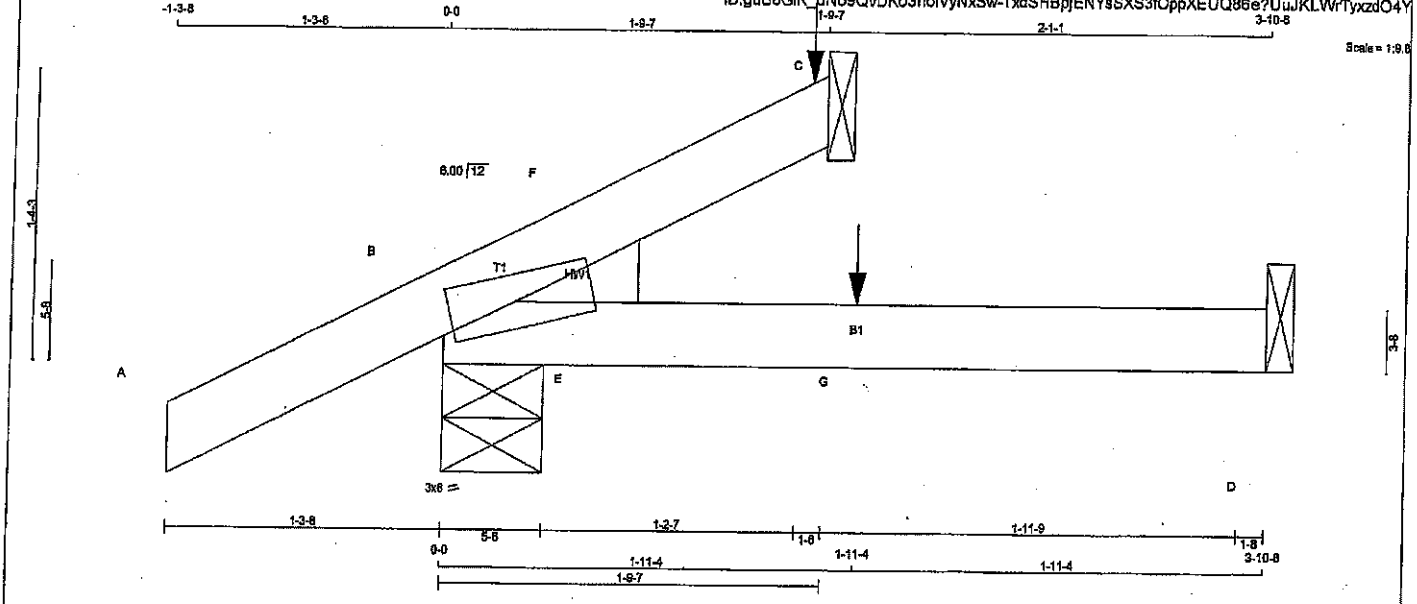


TOTAL WEIGHT = 2 X 9 = 18 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	3.0	6.0	1.50	3.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX	HEEL WEDGE
C	129	0	129	0	-9	1-8	1-8	
B	336	0	336	0	0	5-8	5-8	2x4 L
D	58	0	72	0	0	1-8	1-8	

SEE MiTEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	100	45 / -21	28 / 0	0 / 0	0 / 0	27 / 0	0 / 0
B	242	162 / 0	28 / 0	0 / 0	0 / 0	52 / 0	0 / 0
D	52	2 / -4	30 / 0	0 / 0	0 / 0	20 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	CSI	MAX (LC)	MAX. UNBRAC LENGTH
A-B	0 / 25	-102.1	-102.1	0.15	(1)	10.00	
B-F	-117 / 0	-102.1	-102.1	0.14	(1)	6.25	
F-C	-9 / 24	-102.1	-102.1	0.09	(3)	10.00	
B-E	0 / 0	-38.5	-38.5	0.05	(2)	10.00	
E-G	0 / 0	-38.5	-38.5	0.08	(2)	10.00	
G-D	0 / 0	-38.5	-38.5	0.08	(2)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
E-F	0 / 168	0.00 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	1-9-7	-34	-34	—	FRONT	VERT	TOTAL	—	—
G	1-11-4	-19	-19	—	FRONT	VERT	TOTAL	—	—

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.15/1.00 (A-B;1) , BC=0.09/1.00 (D-E;2) , WB=0.00/1.00 (E-F;1) , SSI=0.16/1.00 (B-F;1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

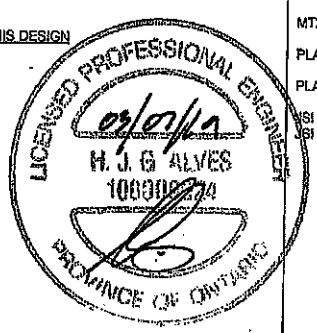
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1656

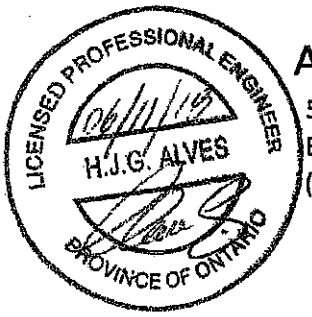
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.08 (B) (INPUT = 0.90)
SSI METAL= 0.02 (B) (INPUT = 1.00)



DWG NO. TAM T19051SB
STRUCTURAL COMPONENT ONLY



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

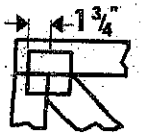
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

T-1800218

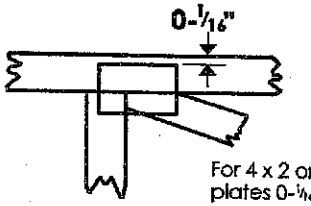
Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

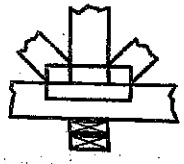
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

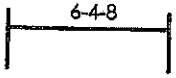


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

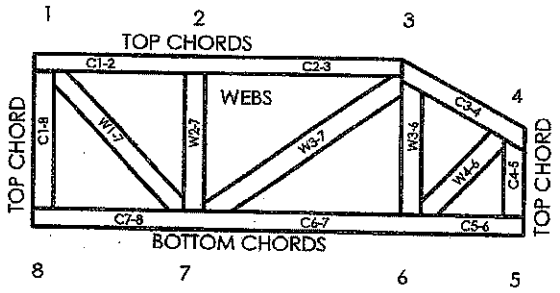
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

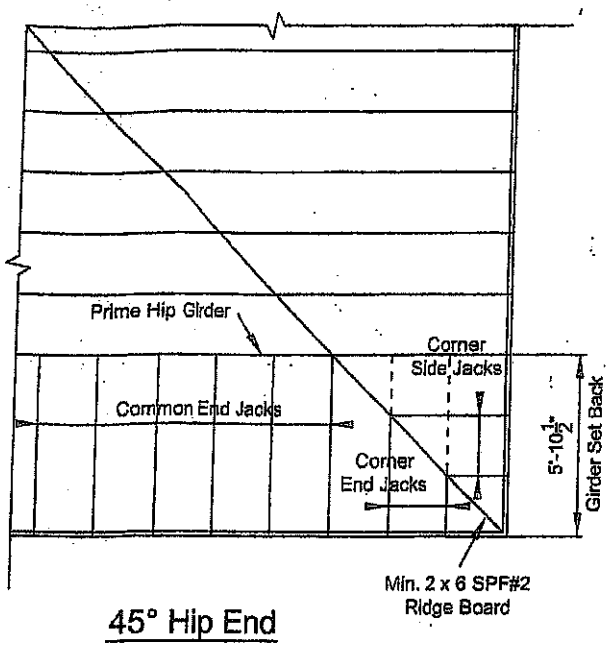
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

LUMBER SPECIFICATION

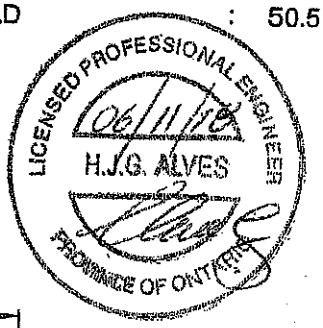
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

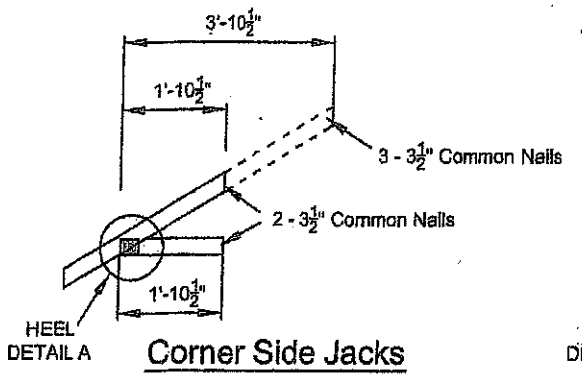
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

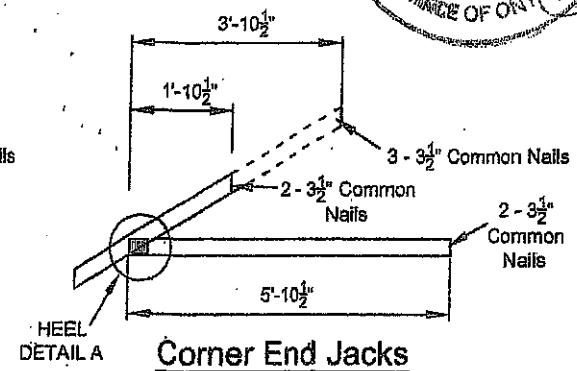


45° Hip End

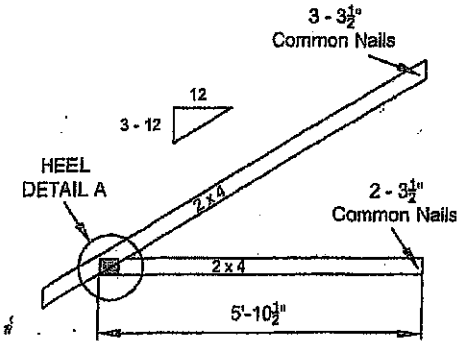




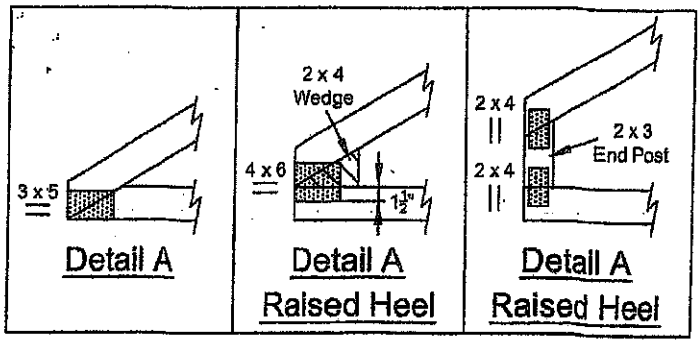
Corner Side Jacks



Corner End Jacks

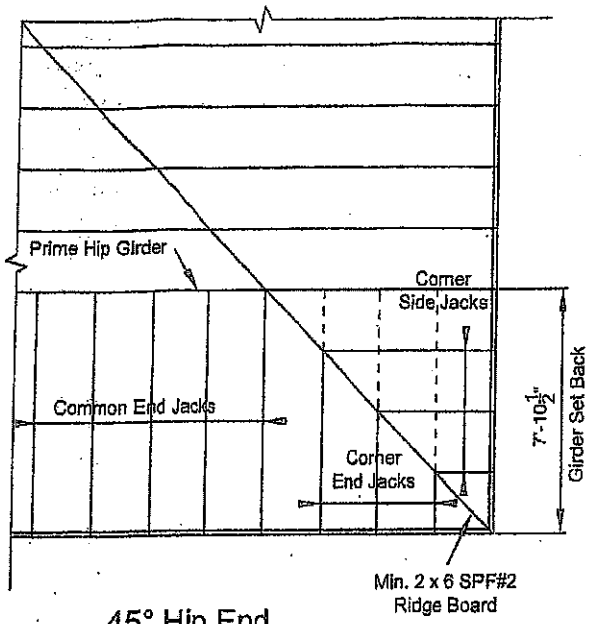


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

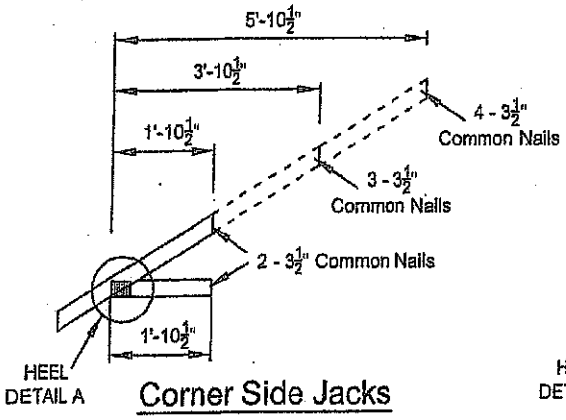
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

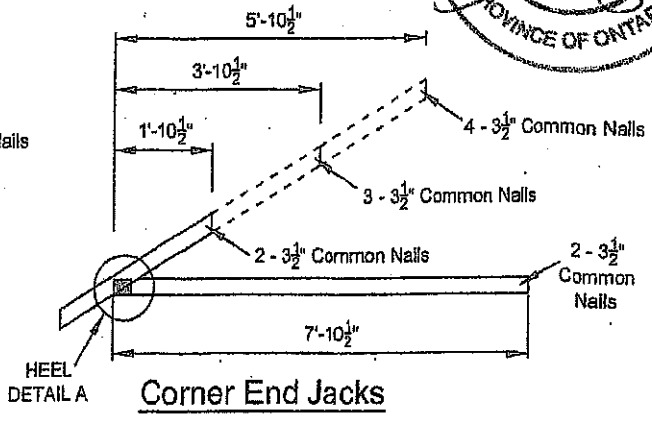
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

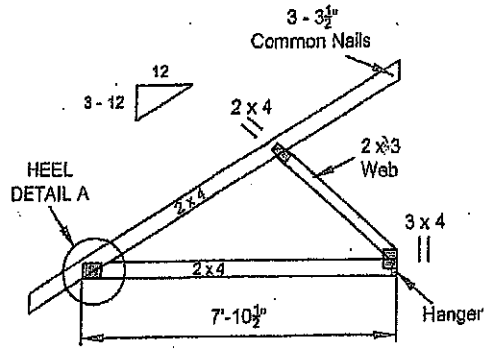
LICENSED PROFESSIONAL ENGINEER
26/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



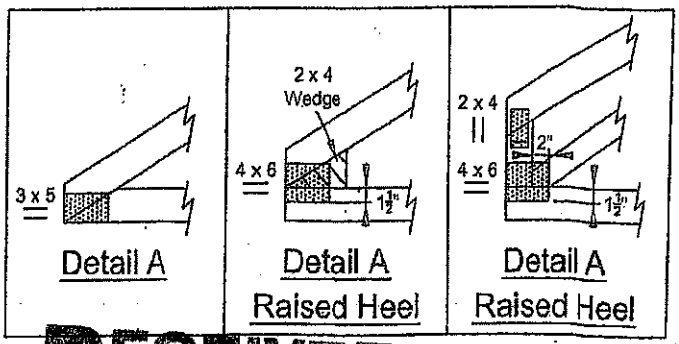
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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JUN 10 2019
TOWN OF CALEDON
PLANNING SECTION
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