

12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:		
GS LOAD	1.8	KPA
TC DEAD	6	PSF
BC LIVE	10.5	PSF
BC DEAD	7	PSF

ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4" SPF @24"o.c. WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT. POSTS LONGER THAN 6' TO BE Laterally Braced SO THAT THE DISTANCE BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

HARDWARE
HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V)

BM1:2-2X10

— DENOTES
CONVENTIONAL
FRAMING



T-

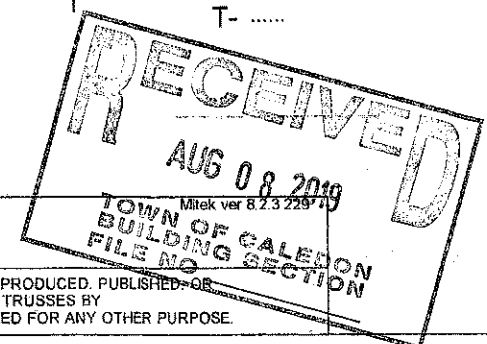
TAMARACK
LUMBER INC.
ALFA LUMBER GROUP

Job Track:	50033
Plan Log:	201018
Layout ID:	402975

Builder / Location:		
GREEN PARK HOMES / CALEDON		
Project		
LAMBERT LANE PH.2		
Date: 5/8/2019	Sales: Mario DiCiano	Designer: J

Model / Elevation:
BARTON 3/2- REAR UPGRADE LOT 5

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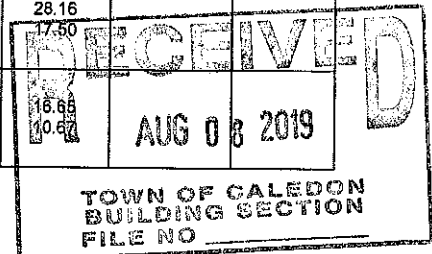
DELIVERY SHIPLIST

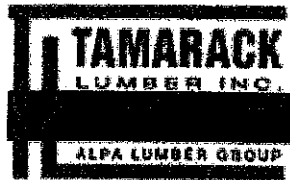
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 3
 Lot #:
 Elevation: 2- REAR UPGRADE

Job Track: 50033
 PlanLog: 201018
 Layout ID: 402975
 Ref #
 Page: 1 of 2
 Date: 05/06/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	4	T9 Common	10 / 12	11-02-00	6-03-08	2 x 4	1-03-08 1-03-08	1-07-11 1-07-11	198.87 132.00		
	1	G9 GABLE	10 / 12	11-02-00	6-03-08	2 x 4	1-05-00 1-03-08	1-07-11 1-07-11	54.46 37.00		
	1 2-ply	T21 Hip Girder	6 / 12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-02-00 1-02-00	252.71 158.00		
	1	T22 Hip	6 / 12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	109.31 68.83		
	1	T23 Hip	6 / 12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	110.98 70.67		
	1	T24 Hip	6 / 12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	112.6 70.67		
	11	T25 Common	6 / 12	27-00-00	7-11-00	2 x 4	1-03-08 1-03-08	1-02-00 1-02-00	1199.12 740.67		
	1 2-ply	T28 Hip Girder	10 / 12	27-00-00	4-10-07	2 x 4 2 x 6	1-03-08 1-03-08	1-07-11 1-07-11	266.28 170.33		
	2	T29 Hip	10 / 12	27-00-00	6-06-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	250.96 158.67		
	2	T30 Hip	10 / 12	27-00-00	8-02-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	280.71 165.33		
	2	T31 Hip	10 / 12	27-00-00	9-10-07	2 x 4	1-03-08 1-05-00	1-07-11 1-07-11	269.72 171.33		
	1	V20 Valley	6 / 12	15-07-01	3-10-12	2 x 4			44.6 29.87		
	1	V21 Valley	6 / 12	11-07-01	2-10-12	2 x 4			28.16 17.50		
	1	V22 Valley	6 / 12	7-07-01	1-10-12	2 x 4			16.65 10.67		





DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON
 Lot #:
 Elevation: 2- REAR UPGRADE

Job Track: 50033
 PlanLog: 201018
 Layout ID: 402975
 Ref #
 Page: 2 of 2
 Date: 05/06/2019
 Designer:
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	9	J1 Jack-Open	6 /12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	151.15 96.00		
	2	J2 Jack-Open Girder	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 4-01-01	1-02-00 2-00-12	23.16 14.67		
	2	J3 Jack-Open	6 /12	3-09-07	3-00-12	2 x 4	1-03-08 2-01-01	1-02-00 3-00-12	28.26 17.33		
	2	J4 Jack-Open	6 /12	1-09-07	2-00-12	2 x 4	1-03-08 1-01	1-02-00 2-00-12	14.04 9.33		
	2	J5 Jack-Open	6 /12	1-10-08	3-00-12	2 x 4	1-03-08 1-10-15	1-02-00 2-01-04	19.14 12.00		
	6	J7 Jack-Open	10 /12	3-10-08	4-10-07	2 x 4	1-03-08	1-07-11 4-10-07	91.36 58.00		
	1	J21 Jack-Open Girder	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 2-01-01	1-07-11 3-01-09	12.02 8.33		
	1	J22 Jack-Open	10 /12	1-09-07	3-01-09	2 x 4	1-03-08 1-01	1-07-11 3-01-09	9.74 7.00		
	7	J23 Jack-Open	4 /12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 66.33		

TOTAL # TRUSS= 64

TOTAL BFT OF ALL TRUSSES= 2289.33

BFT.

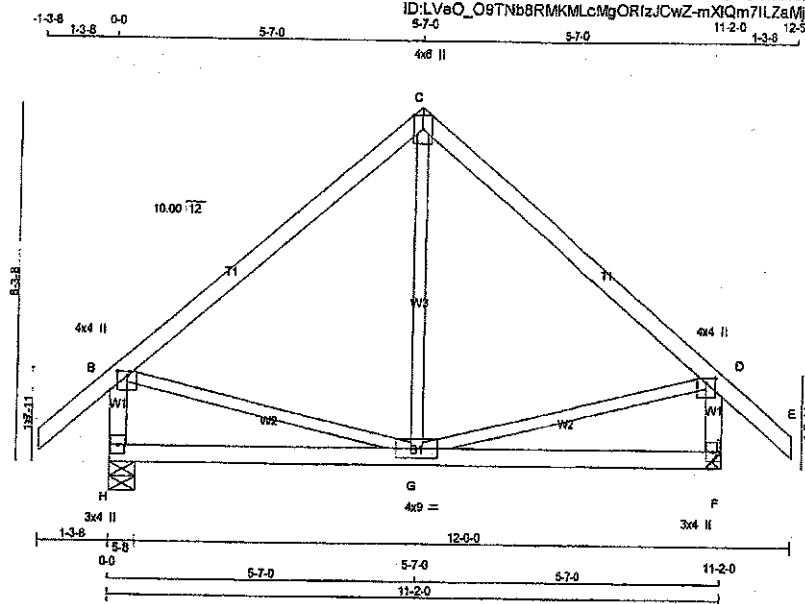
TOTAL WEIGHT OF ALL TRSSES 3625.15 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
1	Hardware	LJS26DS	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 2

JOB NAME 402973	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:38:48 2019 Page 1 ID:LVaO_Q9TNb8RMKMLcMgORizJCwZ-mXlQm7ILZaMj5WBAm20SmYpf7679Lw_NxAFKwozfdab 5-7-0 11-2-0 12-5-8 Scale = 1:37.5		



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p MT20	4.0	4.0	1.00	2.00
C	TTW+p MT20	4.0	6.0	Edge	
D	TMVW+p MT20	4.0	4.0	1.00	2.00
F	BMV1+p MT20	3.0	4.0		
G	BMVW+p MT20	4.0	9.0		
H	BMV1+p MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	UPLIFT	IN-SX	BRG	IN-SX
H	927	0	927	0	0	0	5-8	5-8	
F	927	0	927	0	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	684	405 / 0	117 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0	0 / 0
F	684	405 / 0	117 / 0	0 / 0	0 / 0	0 / 0	182 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	FORCE (LBS)	MAX. CS (LC)	UNBRACED LENGTH
FR-TO		FROM TO		FR-TO			
A-B	0 / 45	-102.1 -102.1	0.14 (1)	G-C	0 / 248	0.05 (3)	10.00
B-C	-510 / 0	-102.1 -102.1	0.41 (1)	B-G	0 / 403	0.09 (1)	6.25
C-D	-510 / 0	-102.1 -102.1	0.41 (1)	G-D	0 / 403	0.09 (1)	6.25
D-E	0 / 45	-102.1 -102.1	0.14 (1)				10.00
H-B	-845 / 0	0.0 0.0	0.09 (1)				7.81
F-D	-845 / 0	0.0 0.0	0.09 (1)				7.81
H-G	0 / 0	-38.5 -38.5	0.27 (3)				10.00
G-F	0 / 0	-38.5 -38.5	0.27 (3)				10.00

TOTAL WEIGHT = 2 X 50 = 99 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.37")
CALCULATED VERT. DEFL. (LL) = 1/989 (0.03")
ALLOWABLE DEFL. (TL) = L/360 (0.37")
CALCULATED VERT. DEFL. (TL) = 1/989 (0.03")

CSI: TC=0.41/1.00 (B-C:1), BC=0.27/1.00 (G-H:3), WB=0.09/1.00 (D-G:1), SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

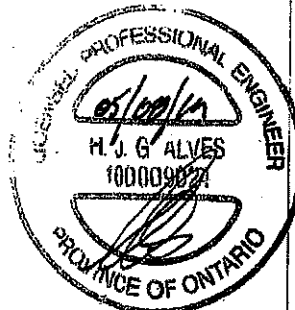
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1887
	788	1887	1856

PLATE PLACEMENT TOL. = 0.250 inches

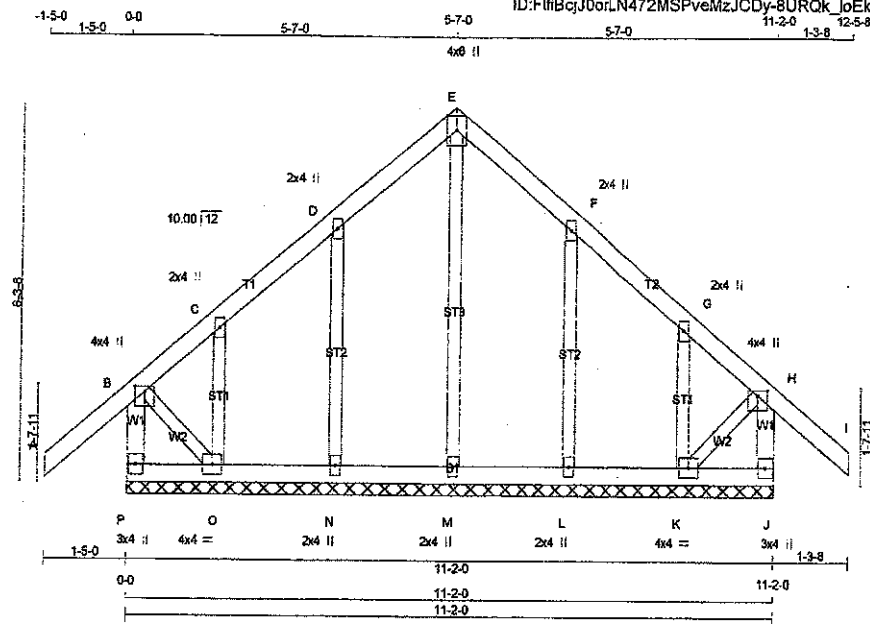
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.76 (D) (INPUT = 0.90)
JSI METAL= 0.21 (D) (INPUT = 1.00)



DWG NO. TAM 191/0807
STRUCTURAL
COMPONENT PART 7

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	ORWG NO.
402975	G9	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:55:57 2019 Page 1					
ID: FfBcjJ0orLN472MSPveMzJCDy-8URQk_JoEk?j2fMxYf_lzXoZlvGoyJfrYm0GYozldKW					
1-5-0 1-5-0 0-0 5-7-0 5-7-0 5-7-0 11-2-0 12-5-8 1-3-8					
4x6 II					
Scale = 1:37.1					



TOTAL WEIGHT = 54 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
ALL GABLE WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, F, G					
C TMVW+p	MT20	2.0	4.0		
E TMVW+p	MT20	4.0	4.0	1.00	2.00
H TMVW+p	MT20	3.0	4.0		
J BMVW1+p	MT20	4.0	4.0		
L, M, N					
L BMVW1+p	MT20	2.0	4.0		
O BMVW1+p	MT20	4.0	4.0		
P BMVW1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
P-B	-331 / 0	0.0	0.0 0.04 (1)	M-E	-154 / 0	0.09 (1)	
A-B	0 / 49	-102.1 -102.1	0.17 (1)	N-D	-250 / 0	0.08 (1)	
B-C	-69 / 0	-102.1 -102.1	0.16 (1)	O-C	-80 / 0	0.01 (1)	
C-D	-3 / 0	-102.1 -102.1	0.07 (1)	L-F	-247 / 0	0.08 (1)	
D-E	-25 / 0	-102.1 -102.1	0.07 (1)	K-G	-96 / 0	0.02 (1)	
E-F	-25 / 0	-102.1 -102.1	0.07 (1)	B-O	0 / 21	0.00 (1)	
F-G	-4 / 0	-102.1 -102.1	0.07 (1)	K-H	0 / 22	0.00 (1)	
G-H	-60 / 0	-102.1 -102.1	0.13 (1)				
H-I	0 / 45	-102.1 -102.1	0.14 (1)				
J-H	-305 / 0	0.0	0.0 0.03 (1)				
P-O	0 / 0	-38.5 -38.5	0.02 (3)				
O-N	0 / 13	-38.5 -38.5	0.02 (3)				
N-M	0 / 8	-38.5 -38.5	0.02 (3)				
M-L	0 / 8	-38.5 -38.5	0.02 (3)				
L-K	0 / 13	-38.5 -38.5	0.02 (3)				
K-J	0 / 0	-38.5 -38.5	0.02 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	29.1 PSF
DL	=	6.0 PSF
BOT CH. LL	=	10.5 PSF
DL	=	7.0 PSF
TOTAL LOAD	=	52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.5 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (A-B:1), BC=0.02/1.00 (N-O:3), WB=0.09/1.00 (E-M:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

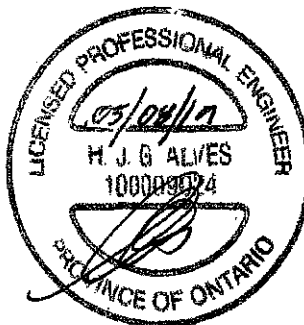
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788	1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

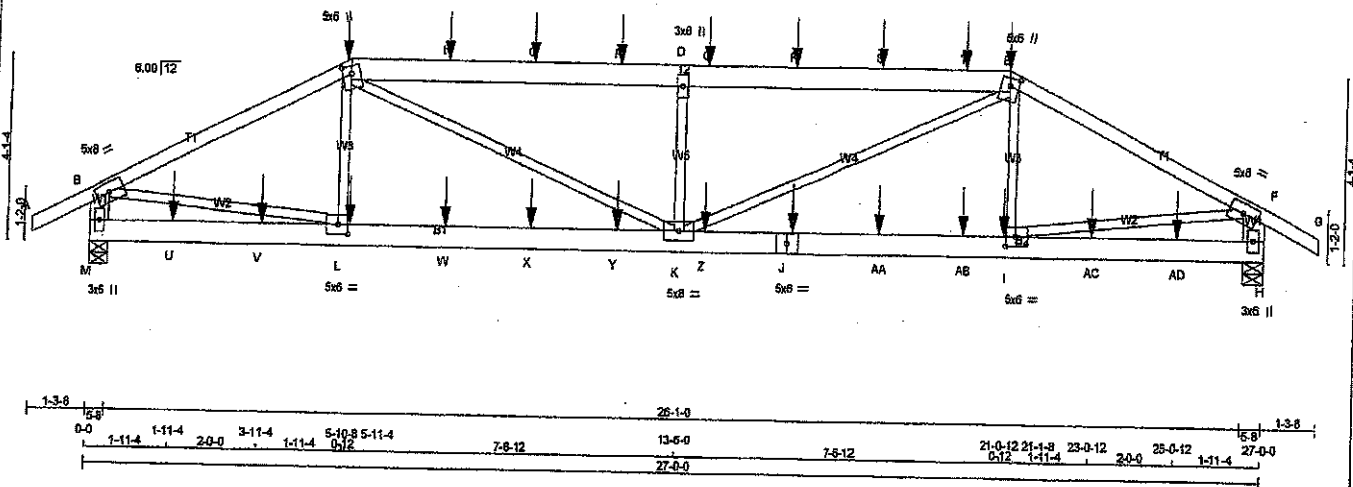
JSI GRIP= 0.25 (B) (INPUT = 0.90)
JSI METAL = 0.13 (F) (INPUT = 1.00)



DWG NO. TAM 11910920
STRUCTURAL
COMPONENT ONLY

JOB NAME 402939	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:48:13 2019 Page 1 ID: yH3luQzwKBaXQAh?hsf8URzKCTp-qVVnxrdSag6XyilP_FslsBCy48h4xqRDdElHyczlqdW	

1-3-8 0-0 5-10-8 5-10-8 7-7-8 13-4-0 7-7-8 21-1-8 5-10-8 27-0-0 29-3-8
Scale = 1/4" = 1'-0"



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
M - B	2x6	DRY No.2	SPF
H - F	2x8	DRY No.2	SPF
M - J	2x6	DRY No.2	SPF
J - H	2x6	DRY No.2	SPF
ALL WEBS EXCEPT	2x3	DRY No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
E-G	12	SIDE(61.0)
C-E	12	SIDE(61.0)
M-B	12	TOP
H-F	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
M-J	12	SIDE(183.1)
J-H	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS		
L-C	8	SIDE(1.4)
I-E	8	SIDE(1.4)
2x3	8	SIDE(1.4)

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMWV4	MT20	5.0	8.0
C	TMWV4m	MT20	5.0	6.0
D	TMWV4w	MT20	3.0	6.0
E	TMWV4w	MT20	5.0	6.0
F	TMWV4	MT20	5.0	8.0
H	BMV4+p	MT20	3.0	8.0
I	BMWV4	MT20	5.0	8.0
J	BS-1	MT20	5.0	2.50 2.75
K	BMWV4	MT20	3.0	8.0
L	BMWV4	MT20	5.0	6.0
M	BMV4+p	MT20	3.0	6.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

	FACTORED		MAXIMUM FACTORED		INPUT	REQD
	GROSS REACTION	VERT	GROSS REACTION	DOWN		
JT	VERT	3270	0	3270	0	5-8
M	3270	0	3270	0	5-8	5-8
H	3325	0	3325	0	5-8	5-8

UNFACTORED REACTIONS						
	1ST CASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD
	COMBINED	SNOW	LIVE	PERM. LIVE		
JT	2443	1351/0	487/0	0/0	0/0	605/0
M	2485	1373/0	487/0	0/0	0/0	615/0
H						

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	L1 MAX. CSI (LC)	UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	0/31	-102.1	-102.1	0.07 (1)	10.00	L-C	-80/247	0.04 (3)	
B-C	-4802/0	-102.1	-102.1	0.54 (1)	3.92	C-K	0/2416	0.30 (1)	
C-N	-6483/0	-102.1	-102.1	0.48 (1)	4.20	K-D	-1559/0	0.19 (1)	
N-O	-6483/0	-102.1	-102.1	0.48 (1)	4.20	D-E	0/2317	0.28 (1)	
O-P	-6483/0	-102.1	-102.1	0.48 (1)	4.20	E-F	-71/381	0.05 (3)	
P-D	-6483/0	-102.1	-102.1	0.48 (1)	4.20	F-G	0/4343	0.54 (1)	
D-Q	-6483/0	-102.1	-102.1	0.48 (1)	4.20	I-F	0/4433	0.55 (1)	
Q-R	-6483/0	-102.1	-102.1	0.48 (1)	4.20				
R-S	-6483/0	-102.1	-102.1	0.48 (1)	4.20				
S-T	-6483/0	-102.1	-102.1	0.48 (1)	4.20				
T-E	-6483/0	-102.1	-102.1	0.48 (1)	4.20				
E-F	-4902/0	-102.1	-102.1	0.55 (1)	3.88				
F-G	0/31	-102.1	-102.1	0.07 (1)	10.00				
M-B	-3144/0	0.0	0.0	0.11 (1)	7.78				
H-F	-3201/0	0.0	0.0	0.11 (1)	7.74				
M-U	0/0	-38.5	-38.5	0.13 (3)	10.00				
U-V	0/0	-38.5	-38.5	0.13 (3)	10.00				
V-L	0/0	-38.5	-38.5	0.13 (3)	10.00				
L-W	0/4307	-38.5	-38.5	0.38 (1)	10.00				
W-X	0/4307	-38.5	-38.5	0.38 (1)	10.00				
X-Y	0/4307	-38.5	-38.5	0.38 (1)	10.00				
Y-K	0/4307	-38.5	-38.5	0.38 (1)	10.00				
K-Z	0/4396	-38.5	-38.5	0.39 (1)	10.00				
Z-J	0/4396	-38.5	-38.5	0.39 (1)	10.00				
J-AA	0/4396	-38.5	-38.5	0.39 (1)	10.00				
AA-AB	0/4396	-38.5	-38.5	0.39 (1)	10.00				
AB-I	0/4396	-38.5	-38.5	0.39 (1)	10.00				
I-AC	0/0	-38.5	-38.5	0.13 (3)	10.00				
AC-AD	0/0	-38.5	-38.5	0.13 (3)	10.00				
AD-H	0/0	-38.5	-38.5	0.13 (3)	10.00				

FACTORED CONCENTRATED LOADS (LBS)					
JT	LOC.	L1	MAX.	FACE	DIR.
C	5-10-8	-473	-473	FRONT	VERT
E	21-1-8	-473	-473	FRONT	VERT
I	21-0-12	-55	-70	FRONT	VERT
J	18-1-4	-55	-70	FRONT	VERT
L	5-11-4	-55	-70	FRONT	VERT
N	8-1-4	-123	-123	FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 240 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
ALLOWABLE DEFL.(TL) = L/380 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.22")

CS: TC=0.55/1.00 (E-F:1), BC=0.39/1.00 (I-K:1),
WB=0.55/1.00 (F-I:1), SS=0.26/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

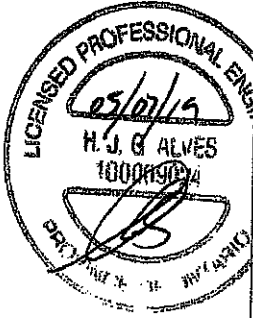
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
618 354 1687 798 1687 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP=0.88 (L) (INPUT = 0.90)
SI METAL=0.55 (L) (INPUT = 1.00)

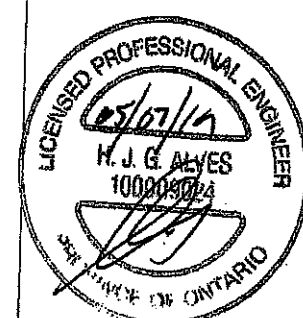


ENGR. NO. TAM 1910779
1/2

JOB NAME 402939	TRUSS NAME T21	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:46:13 2019 Page 2
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FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
O	10-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
P	12-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
Q	14-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
R	16-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
S	18-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
T	20-1-4	-123	-123	--	FRONT	VERT	TOTAL	--	--
U	1-11-4	-55	-74	--	FRONT	VERT	TOTAL	--	--
V	3-11-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
W	8-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
X	10-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
Y	12-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
Z	14-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AA	16-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AB	20-1-4	-55	-70	--	FRONT	VERT	TOTAL	--	--
AC	23-0-12	-55	-70	--	FRONT	VERT	TOTAL	--	--
AD	25-0-12	-55	-74	--	FRONT	VERT	TOTAL	--	--



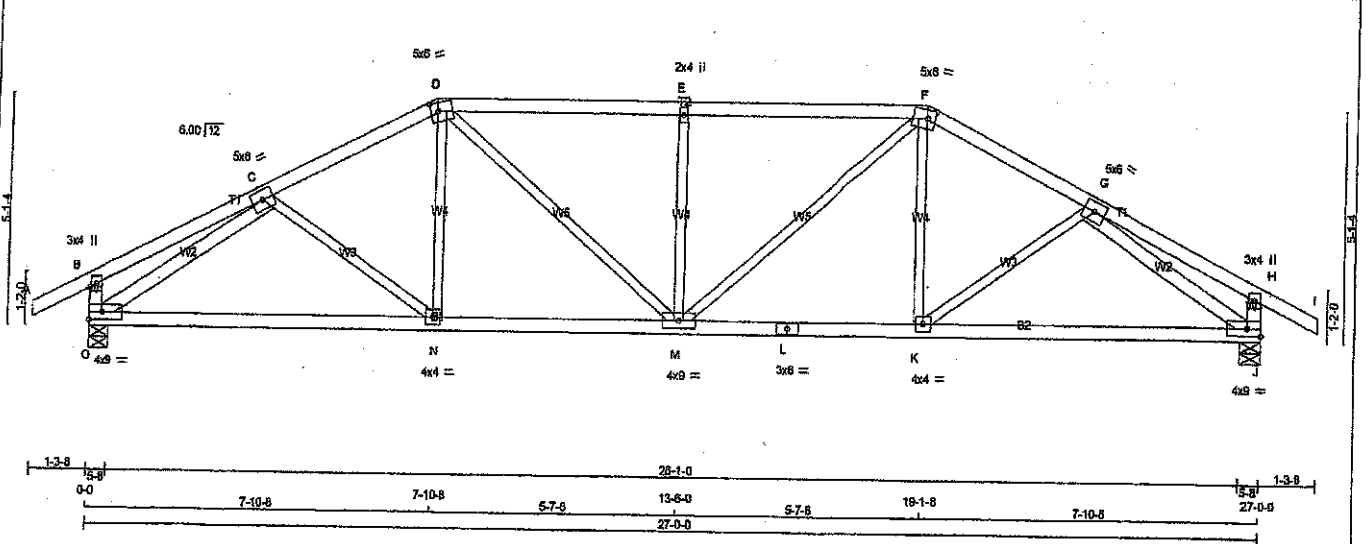
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 TOWN OF CALEDON
 BUILDING SECTION
 FILE NO

REVISED BY TAM P910779
 STRUCTURAL
 CHECKED ONLY

2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402939	T22	1	1	GREEN PARK HOMES	
Tamarack Roof Trusses, Burlington					

Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:48:14 2018 Page 1
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 Scale = 1/48



TOTAL WEIGHT = 109 LB

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW-t	MT20	5.0	6.0	
D	TTVW-m	MT20	5.0	6.0	2.25 2.00
E	TMVW-w	MT20	2.0	4.0	
F	TTVW-m	MT20	5.0	6.0	2.25 2.00
G	TMVW-t	MT20	5.0	6.0	
H	TMV+p	MT20	3.0	4.0	
J	BMVW-t	MT20	4.0	9.0	Edge
K	BMVW-t	MT20	4.0	4.0	
L	BS4	MT20	3.0	6.0	
M	BMVW-t	MT20	4.0	9.0	
N	BMVW-t	MT20	4.0	4.0	
O	BMVW-t	MT20	4.0	9.0	Edge

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	
O	2037 0	2037 0	5-8	5-8	
J	2037 0	2037 0	5-8	5-8	

UNFACTORED REACTIONS					
JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS			
O	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
O	1514	883 / 0	284 / 0	0 / 0	0 / 0
J	1514	883 / 0	284 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.68 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)		
FR-TO		FROM TO	LENGTH	FR-TO				
A-B	0 / 31	-102.1 -102.1	0.13 (1)	10.00	C-N	-3 / 138	0.03 (3)	
B-C	0 / 19	-102.1 -102.1	0.23 (1)	10.00	N-D	0 / 375	0.08 (2)	
C-D	-2533 / 0	-102.1 -102.1	0.25 (1)	4.13	D-M	0 / 647	0.15 (1)	
D-E	-2746 / 0	-102.1 -102.1	0.52 (1)	3.68	M-E	-702 / 0	0.27 (1)	
E-F	-2746 / 0	-102.1 -102.1	0.52 (1)	3.68	M-F	0 / 647	0.15 (1)	
F-G	-2533 / 0	-102.1 -102.1	0.25 (1)	4.13	K-F	0 / 375	0.08 (2)	
G-H	0 / 19	-102.1 -102.1	0.23 (1)	10.00	K-G	-3 / 138	0.03 (3)	
H-I	0 / 31	-102.1 -102.1	0.13 (1)	10.00	O-C	-2768 / 0	0.77 (1)	
O-B	-299 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2768 / 0	0.77 (1)
J-H	-299 / 0	0.0	0.0	0.03 (1)	7.81			
O-N	0 / 2250	-38.5	-38.5	0.70 (2)	10.00			
N-M	0 / 2254	-38.5	-38.5	0.71 (2)	10.00			
M-L	0 / 2254	-38.5	-38.5	0.71 (2)	10.00			
L-K	0 / 2254	-38.5	-38.5	0.71 (2)	10.00			
K-J	0 / 2250	-38.5	-38.5	0.70 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018 ; OBC 2012
 - CSA S88-09, CSA S88-14
 - TPC 2011, TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.80")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
 ALLOWABLE DEFL. (TL) = L/360 (0.80")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.30")

CS1: TC=0.52/1.00 (D-E-1), BC=0.71/1.00 (K-M-2), WB=0.77/1.00 (G-J-1), SH=0.28/1.00 (D-E-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

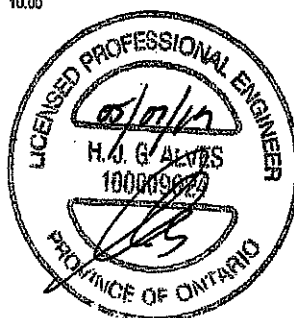
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN
MT20	818	354	1657 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (C) (INPUT = 0.90)
 JSI METAL= 0.73 (L) (INPUT = 1.00)



DESIGNED BY: J910780
 STRUCTURAL
 CONSULTING INC.

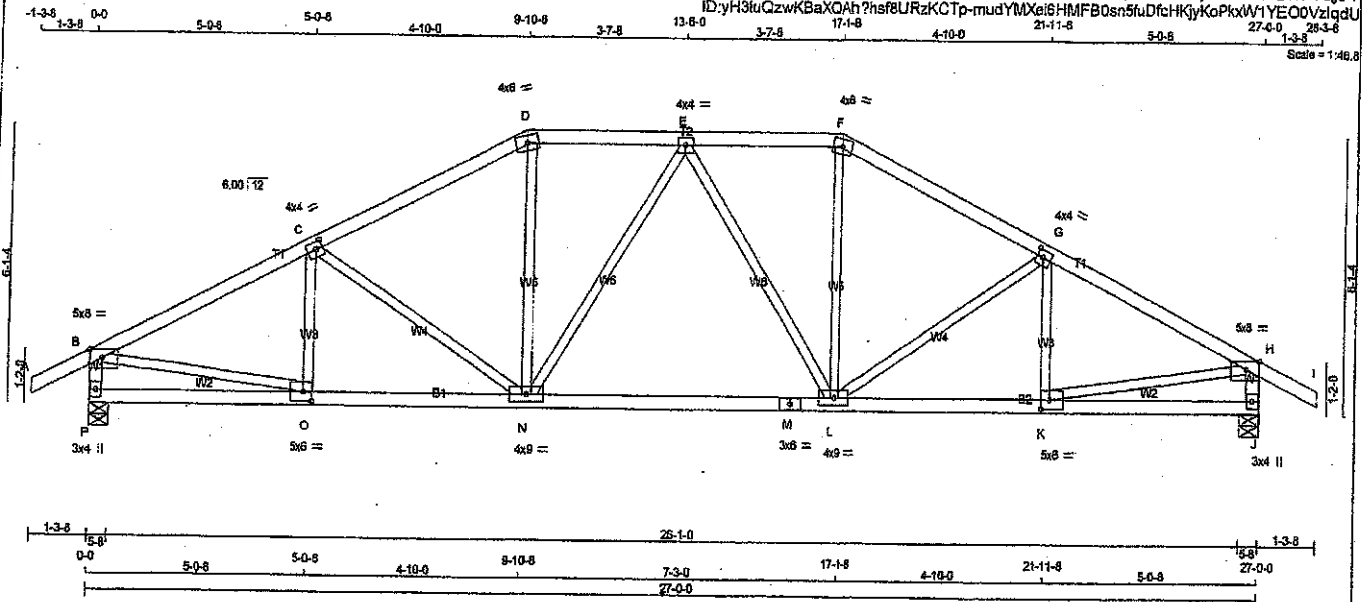
JOB NAME 402939	TRUSS NAME T23	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MTek Industries, Inc. Tue May 7 17:48:15 2019 Page 1

ID: yH3tuGzwKBaXQAh?nsf8URzKCTp-mudYMXeISHMFB0sn5fDfcHKjyKoPkwW1YEOOVzldU

Scale = 1/40.0



TOTAL WEIGHT = 111 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
P - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMW-p	MT20	5.0	8.0	Edge 3.50
C	TMW-t	MT20	4.0	4.0	2.00 1.75
D	TTW-m	MT20	4.0	6.0	
E	TMW-t	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	6.0	
G	TMW-t	MT20	4.0	4.0	2.00 1.75
H	TMW-p	MT20	5.0	8.0	Edge 3.50
I	BMV-t-p	MT20	3.0	4.0	
J	BMV-t	MT20	5.0	6.0	2.50 2.25
K	BMV-t	MT20	4.0	9.0	
L	BMV-t	MT20	3.0	6.0	
M	BS-t	MT20	4.0	9.0	
N	BMV-t	MT20	5.0	6.0	2.50 2.25
O	BMV-t	MT20	5.0	6.0	
P	BMV-t-p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	2037	2037	0	5-8
JT HORZ	0	0	0	5-8
P VERT	2037	2037	0	5-8
P HORZ	0	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS						
JT COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	
P	1514	883 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0
J	1514	883 / 0	284 / 0	0 / 0	0 / 0	367 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)
FR-TO				FR-TO			
A-B	0 / 31	-102.1	-102.1 0.13 (1)	10.00	C-C	-221 / 64	0.05 (1)
B-C	-2663 / 0	-102.1	-102.1 0.41 (1)	3.88	C-N	-410 / 0	0.26 (1)
C-D	-2337 / 0	-102.1	-102.1 0.38 (1)	4.14	N-D	0 / 688	0.16 (1)
D-E	-2078 / 0	-102.1	-102.1 0.20 (1)	4.53	E-E	-258 / 0	0.22 (1)
E-F	-2078 / 0	-102.1	-102.1 0.20 (1)	4.53	E-L	-258 / 0	0.22 (1)
F-G	-2337 / 0	-102.1	-102.1 0.38 (1)	4.14	L-F	0 / 688	0.16 (1)
G-H	-2663 / 0	-102.1	-102.1 0.41 (1)	3.88	L-G	-410 / 0	0.26 (1)
H-I	0 / 31	-102.1	-102.1 0.13 (1)	10.00	K-G	-221 / 64	0.05 (1)
P-B	-1951 / 0	0.0	0.0 0.20 (1)	6.03	B-O	0 / 2438	0.55 (1)
J-H	-1951 / 0	0.0	0.0 0.20 (1)	6.03	K-H	0 / 2438	0.55 (1)
P-O	0 / 0	-38.5	-38.5 0.18 (3)	10.00			
C-N	0 / 2404	-38.5	-38.5 0.56 (1)	10.00			
N-M	0 / 2211	-38.5	-38.5 0.53 (2)	10.00			
M-L	0 / 2211	-38.5	-38.5 0.53 (2)	10.00			
L-K	0 / 2404	-38.5	-38.5 0.56 (1)	10.00			
K-J	0 / 0	-38.5	-38.5 0.18 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GIC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/989 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/899 (0.23")

CSI: TC=0.41/1.00 (G-H), BC=0.56/1.00 (K-L), WB=0.55/1.00 (H-K), SS=0.22/1.00 (G-H)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

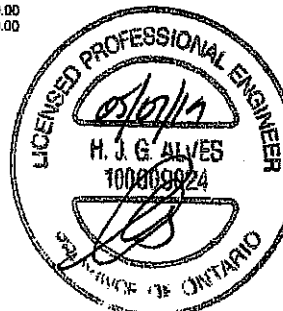
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

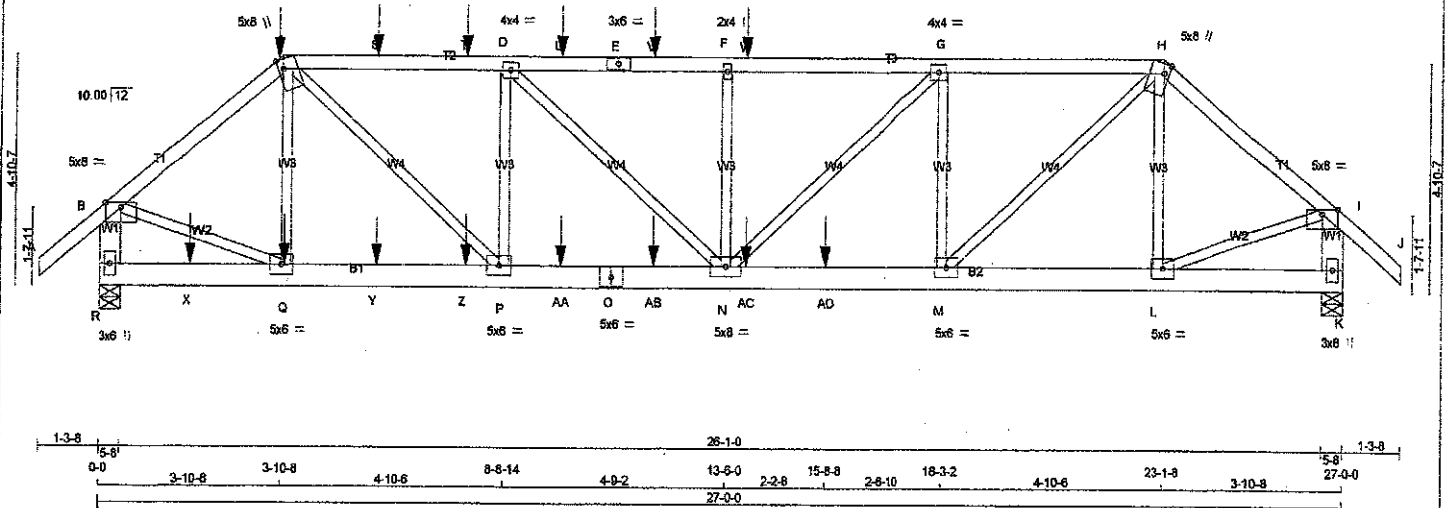
JSI GRIP = 0.85 (K) (INPUT = 0.90)
JSI METAL = 0.85 (M) (INPUT = 1.00)



ENR 100781
STRUCTURAL
ONLY

[illegible]

[illegible]



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
R - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
R - O	2x6	DRY	No.2	SPF
O - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
 EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(61.0)
C-E 1	12	SIDE(61.0)
E-H 1	12	SIDE(0.0)
H-J 1	12	TOP
R-B 2	12	TOP
K-I 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-O 2	12	SIDE(163.1)
O-K 2	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	5.0	8.0	Edge	
C TTWW-m	MT20	5.0	8.0	Edge 1.25	
D TMWW-t	MT20	4.0	4.0		
E TS-t	MT20	3.0	8.0		
F TMWV-w	MT20	2.0	4.0		
G TMWW-t	MT20	4.0	4.0		
H TTWW-m	MT20	5.0	8.0	Edge 1.25	
I TMWV-w	MT20	5.0	8.0	Edge	
K BMV1-p	MT20	3.0	8.0		
L M, P, Q					
L BMWW-t	MT20	5.0	8.0		
N BMWW-t	MT20	5.0	8.0		
O BS-t	MT20	5.0	8.0		
R BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	3087	0	5-8	5-8
R	3087	0	5-8	5-8
K	2783	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R	2274 1311/0 412/0 0/0 0/0 550/0 0/0
K	2047 1188/0 368/0 0/0 0/0 491/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.04 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (PLF)	MAX. FACTORED (CS) (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (CS) (LC)
FR-TO					FR-TO		
A-B	0/45	-102.1	-102.1	0.08 (1)	Q-C	-406/44	0.07 (1)
B-C	-3032/0	-102.1	-102.1	0.19 (1)	C-P	0/2393	0.30 (1)
C-S	-4075/0	-102.1	-102.1	0.37 (1)	P-D	-1438/0	0.24 (1)
S-T	-4075/0	-102.1	-102.1	0.37 (1)	D-N	0/919	0.11 (1)
T-D	-4075/0	-102.1	-102.1	0.37 (1)	N-F	-616/0	0.10 (1)
D-U	-4743/0	-102.1	-102.1	0.40 (1)	N-G	0/1001	0.12 (1)
U-E	-4743/0	-102.1	-102.1	0.40 (1)	M-G	-1228/0	0.21 (1)
E-V	-4743/0	-102.1	-102.1	0.40 (1)	M-H	0/2681	0.33 (1)
V-F	-4743/0	-102.1	-102.1	0.40 (1)	L-H	-509/0	0.09 (1)
F-W	-4743/0	-102.1	-102.1	0.29 (1)	B-Q	0/2430	0.30 (1)
W-G	-4743/0	-102.1	-102.1	0.29 (1)	L-I	0/2148	0.27 (1)
G-H	-4015/0	-102.1	-102.1	0.26 (1)			
H-I	-2680/0	-102.1	-102.1	0.18 (1)			
I-J	0/45	-102.1	-102.1	0.08 (1)			
R-B	-3002/0	0.0	0.0	0.11 (1)			
K-I	-2682/0	0.0	0.0	0.10 (1)			
R-X	0/0	-38.5	-38.5	0.05 (2)			
X-Q	0/0	-38.5	-38.5	0.05 (2)			
Q-Y	0/2314	-38.5	-38.5	0.19 (1)			
Y-Z	0/2314	-38.5	-38.5	0.19 (1)			
Z-P	0/2314	-38.5	-38.5	0.19 (1)			
P-AA	0/4075	-38.5	-38.5	0.36 (1)			
AA-O	0/4075	-38.5	-38.5	0.36 (1)			
O-AB	0/4075	-38.5	-38.5	0.36 (1)			
AB-N	0/4075	-38.5	-38.5	0.36 (1)			
N-AC	0/4015	-38.5	-38.5	0.53 (1)			
AC-AD	0/4015	-38.5	-38.5	0.53 (1)			
AD-M	0/4015	-38.5	-38.5	0.53 (1)			
M-L	0/2041	-38.5	-38.5	0.23 (1)			
L-K	0/0	-38.5	-38.5	0.03 (3)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
C	3-10-8	-274	-274	---	BACK	VERT	TOTAL	---
Q	3-11-4	-36	-46	---	BACK	VERT	TOTAL	---
S	5-11-4	-96	-96	---	BACK	VERT	TOTAL	---
T	7-11-4	-96	-96	---	BACK	VERT	TOTAL	---
U	9-11-4	-96	-96	---	BACK	VERT	TOTAL	---
V	11-11-4	-96	-96	---	BACK	VERT	TOTAL	---

TOTAL WEIGHT = 2 X 133 = 266 lb

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF BCBC 2018, CBC 2012
 - CSA 086-09, CSA 086-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.90")
 CALCULATED VERT. DEFL.(LL)= L/999 (0.10")
 ALLOWABLE DEFL.(TL)= L/360 (0.90")
 CALCULATED VERT. DEFL.(TL)= L/999 (0.17")

CSI: TC=0.40/1.00 (D-F-1), BC=0.53/1.00 (M-N-1), WS=0.33/1.00 (H-M-1), SS=0.18/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

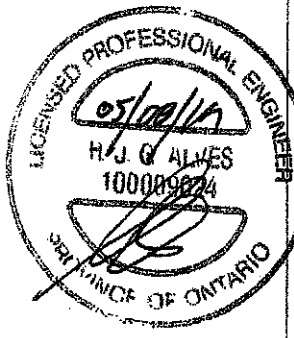
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SI GRIP= 0.89 (C) (INPUT = 0.90)
 SI METAL= 0.65 (O) (INPUT = 1.00)

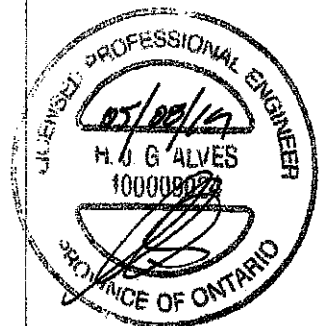


DRWG NO. TAM 11910924
 STRUCTURAL COMPONENT ONLY 1/2

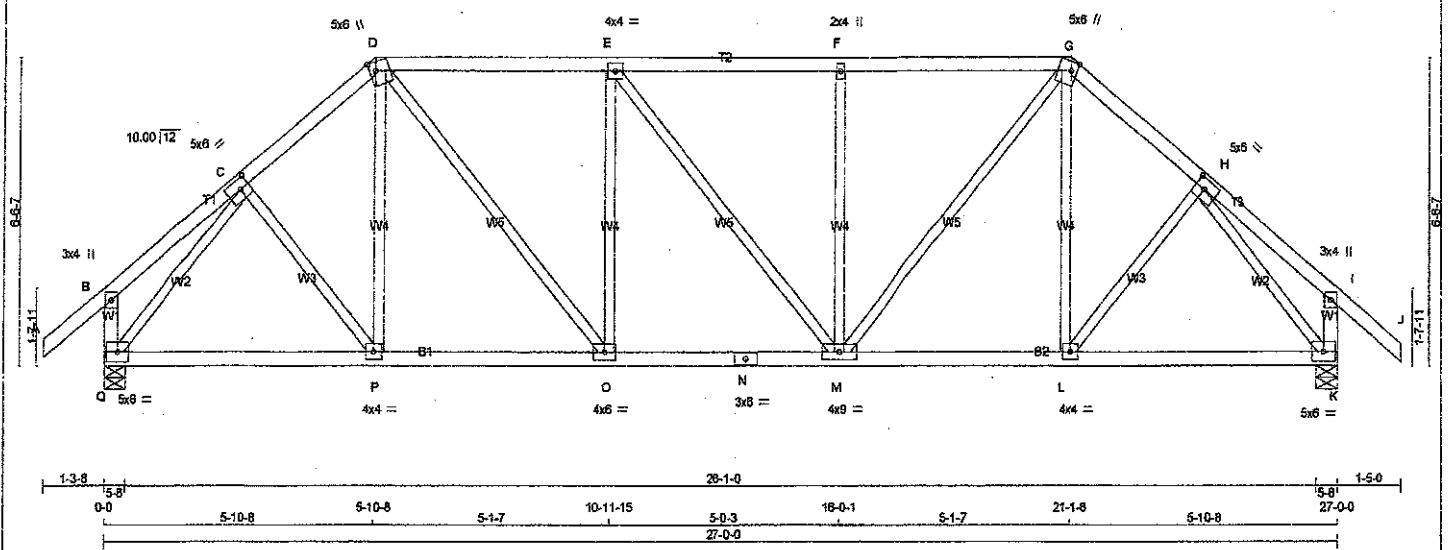
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	T28	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:56:01 2019 Page 2 ID: FfBcjJ0ortL N472MSPveMzJCDy-0FgxaLoJfzV9XGfinV2h8N5BgVWvou2vQTO ThZzIdKS		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
W	13-11-4	-86	-86	---	BACK	VERT	TOTAL	---	---
X	1-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
Y	5-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
Z	7-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
AA	9-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
AB	11-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
AC	13-11-4	-36	-46	---	BACK	VERT	TOTAL	---	---
AD	15-8-8	-745	-745	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 1910929
STRUCTURAL
COMPONENT ONLY 72



8 TOTAL WEIGHT = 2 X 125 = 251 LB [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER			

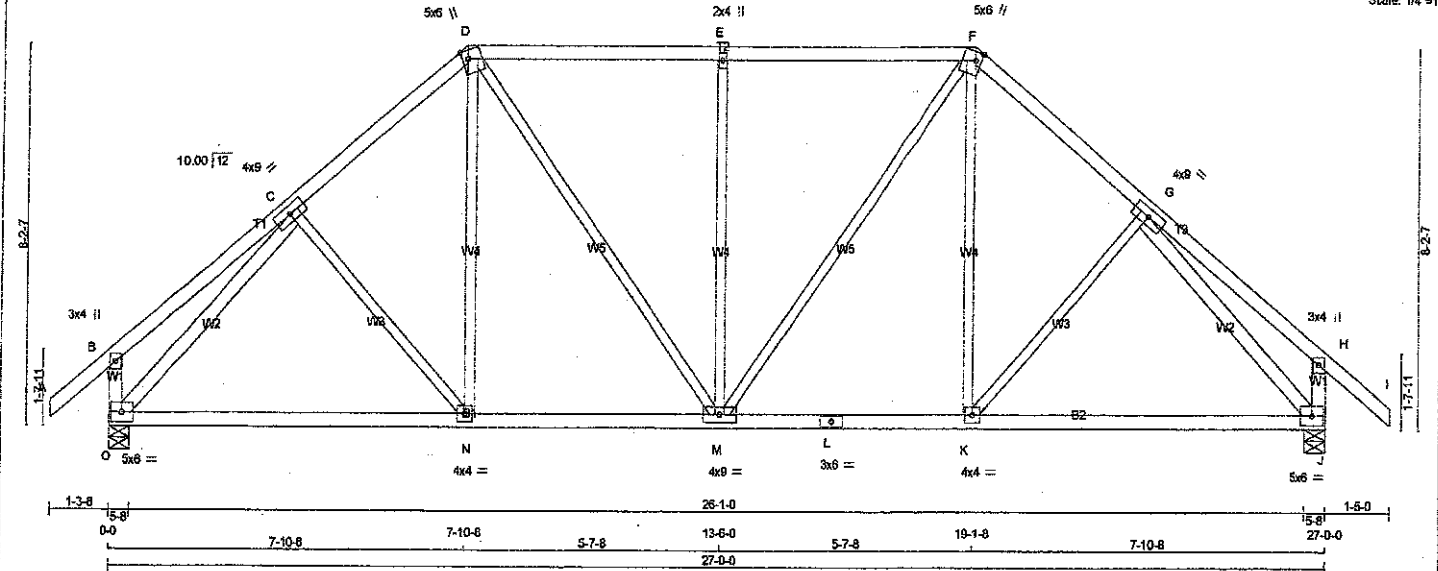
PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMVW+t	MT20	5.0	6.0	2.50 2.50
D	TTWV+m	MT20	5.0	6.0	2.25 1.50
E	TMVW+t	MT20	4.0	4.0	
F	TMV+w	MT20	2.0	4.0	
G	TTWV+m	MT20	5.0	6.0	2.25 1.50
H	TMVW+t	MT20	5.0	6.0	2.50 2.50
I	TMV+p	MT20	3.0	4.0	
K	BMVW1-t	MT20	5.0	6.0	
L	BMVW1-t	MT20	4.0	4.0	
M	BMVWVW-t	MT20	4.0	9.0	
N	BS-t	MT20	3.0	6.0	
O	BMVW-t	MT20	4.0	6.0	
P	BMVW-t	MT20	4.0	4.0	
Q	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
			FACTORED			MAXIMUM FACTORED			
			GROSS REACTION			GROSS REACTION			
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG	REGRD
Q	2040	0	2040	0	0	5-8	5-8		
K	2053	0	2053	0	0	5-8	5-8		
UNFACTORED REACTIONS									
			1ST LCASE			MAX./MIN. COMPONENT REACTIONS			
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL		
Q	1516	865 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0		
K	1525	872 / 0	284 / 0	0 / 0	0 / 0	369 / 0	0 / 0		
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K									
BRACING									
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.									
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.									
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.									
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
			MAX. FACTORED			MAX. FACTORED			
			FORCE			FORCE			
			(LBS)			(LBS)			
			VERT. LOAD			UNBRAC			
			(PLF)			LENGTH			
			LC1 (LC)			FR-TO			
			MAX.			MAX.			
			CS1 (LC)			CS1 (LC)			
			FR-TO			FR-TO			
A-B	0 / 45		-102.1	-102.1	0.14 (1)	10.00	C-P	0 / 186	0.04 (2)
B-C	0 / 18		-102.1	-102.1	0.12 (1)	10.00	P-D	0 / 193	0.05 (3)
C-D	-1877 / 0		-102.1	-102.1	0.16 (1)	4.76	D-O	0 / 857	0.22 (1)
D-E	-2032 / 0		-102.1	-102.1	0.44 (1)	4.27	O-E	-557 / 0	0.39 (1)
E-F	-2031 / 0		-102.1	-102.1	0.44 (1)	4.27	E-M	-3 / 0	0.00 (1)
F-G	-2030 / 0		-102.1	-102.1	0.44 (1)	4.28	M-F	-556 / 0	0.39 (1)
G-H	-1877 / 0		-102.1	-102.1	0.16 (1)	4.78	M-G	0 / 954	0.21 (1)
H-I	0 / 18		-102.1	-102.1	0.12 (1)	10.00	L-G	0 / 193	0.05 (3)
I-J	0 / 49		-102.1	-102.1	0.17 (1)	10.00	L-H	0 / 187	0.04 (2)
Q-B	-261 / 0		0.0	0.0	0.03 (1)	7.81	Q-C	-2137 / 0	0.76 (1)
K-I	-274 / 0		0.0	0.0	0.03 (1)	7.81	H-K	-2137 / 0	0.76 (1)
Q-P	0 / 1314		-38.5	-38.5	0.40 (2)	10.00			
P-O	0 / 1424		-38.5	-38.5	0.42 (2)	10.00			
O-N	0 / 2032		-38.5	-38.5	0.41 (1)	10.00			
N-M	0 / 2032		-38.5	-38.5	0.41 (1)	10.00			
M-L	0 / 1425		-38.5	-38.5	0.42 (2)	10.00			
L-K	0 / 1314		-38.5	-38.5	0.40 (2)	10.00			

DESIGN CRITERIA			
SPECIFIED LOADS:			
TOP CH.	LL	=	29.1 PSF
	DL	=	6.0 PSF
BOT CH.	LL	=	10.5 PSF
	DL	=	7.0 PSF
TOTAL LOAD		= 52.5 PSF	
SPACING = 24.0 IN. C/C			
LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM			
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015			
THIS DESIGN COMPLIES WITH:			
- PART 9 OF BCBC 2018, CBC 2012			
- CSA 088-09, CSA 086-14			
- TPIC 2011, TPIC 2014			
(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD			
ALLOWABLE DEFL.(LL)= L/360 (0.90")			
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")			
ALLOWABLE DEFL.(TL)= L/360 (0.90")			
CALCULATED VERT. DEFL.(TL) = L/999 (0.14")			
CSI: TC=0.44/1.00 (D-E-1), BC=0.42/1.00 (L-M-2), WB=0.78/1.00 (H-K-1), SSI=0.24/1.00 (D-E-1)			
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10			
COMP=1.10 SHEAR=1.10 TENS=1.10			
COMPANION LIVE LOAD FACTOR = 1.00			
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.			
NAIL VALUES			
PLATE GRIP(DRY)	SHEAR	SECTION	
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1667 788	1987 1656
PLATE PLACEMENT TOL = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
GRIP=	0.89 (G)	(INPUT = 0.90)	
METAL=	0.72 (N)	(INPUT = 1.00)	
DWG NO. TAM 1910925			
STRUCTURAL COMPONENT ONLY			

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	T30	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 08:56:03 2019 Page 1
 ID: FifiBqJ0ortLN472MSPveMzJCdy-zeoh?1pZpaltmap5uw59DoAWKKALMp2jwITalRzldKQ



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
O - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
O - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
O - C	2x4	DRY	No.2
G - J	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW+t	MT20	4.0	8.0		
D	TTWW+m	MT20	5.0	8.0	2.25	1.50
E	TMW+w	MT20	2.0	4.0		
F	TTWW+m	MT20	5.0	8.0	2.25	1.50
G	TMWW-t	MT20	4.0	8.0		
H	TMV+p	MT20	3.0	4.0		
J	BMVW1-t	MT20	5.0	8.0		
K	BMVW-t	MT20	4.0	4.0		
L	BS-t	MT20	3.0	8.0		
M	BMVWW-t	MT20	4.0	8.0		
N	BMVW-t	MT20	4.0	4.0		
O	BMVW1-t	MT20	5.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2040	0	2040	0
J	2053	0	2053	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX	MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE	PERM. LIVE
O	1518	885 / 0	284 / 0	0 / 0
J	1525	872 / 0	284 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00	C-N	-28 / 94
B-C	0 / 29	-102.1	-102.1	0.25 (1)	10.00	N-D	0 / 393
C-D	-1815 / 0	-102.1	-102.1	0.24 (1)	4.76	D-M	0 / 515
D-E	-1872 / 0	-102.1	-102.1	0.45 (1)	4.61	M-E	-700 / 0
E-F	-1872 / 0	-102.1	-102.1	0.45 (1)	4.61	M-F	0 / 515
F-G	-1815 / 0	-102.1	-102.1	0.24 (1)	4.76	K-F	0 / 393
G-H	0 / 29	-102.1	-102.1	0.25 (1)	10.00	K-G	-28 / 94
H-I	0 / 49	-102.1	-102.1	0.17 (1)	10.00	O-C	-2133 / 0
O-B	-295 / 0	0.0	0.0	0.03 (1)	7.81	G-J	-2133 / 0
J-H	-308 / 0	0.0	0.0	0.03 (1)	7.81		
O-N	0 / 1387	-38.5	-38.5	0.59 (2)	10.00		
N-M	0 / 1373	-38.5	-38.5	0.59 (2)	10.00		
M-L	0 / 1373	-38.5	-38.5	0.59 (2)	10.00		
L-K	0 / 1373	-38.5	-38.5	0.59 (2)	10.00		
K-J	0 / 1387	-38.5	-38.5	0.59 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
 CALCULATED VERT. DEFL. (LL) = L/999 (0.18")
 ALLOWABLE DEFL. (TL) = L/360 (0.90")
 CALCULATED VERT. DEFL. (TL) = L/999 (0.27")

CSI: TC=0.45/1.00 (D-E-1), BC=0.59/1.00 (K-M-2), WB=0.93/1.00 (G-J-1), SSI=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

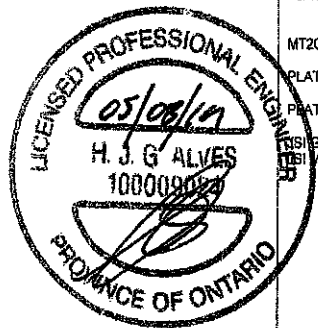
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

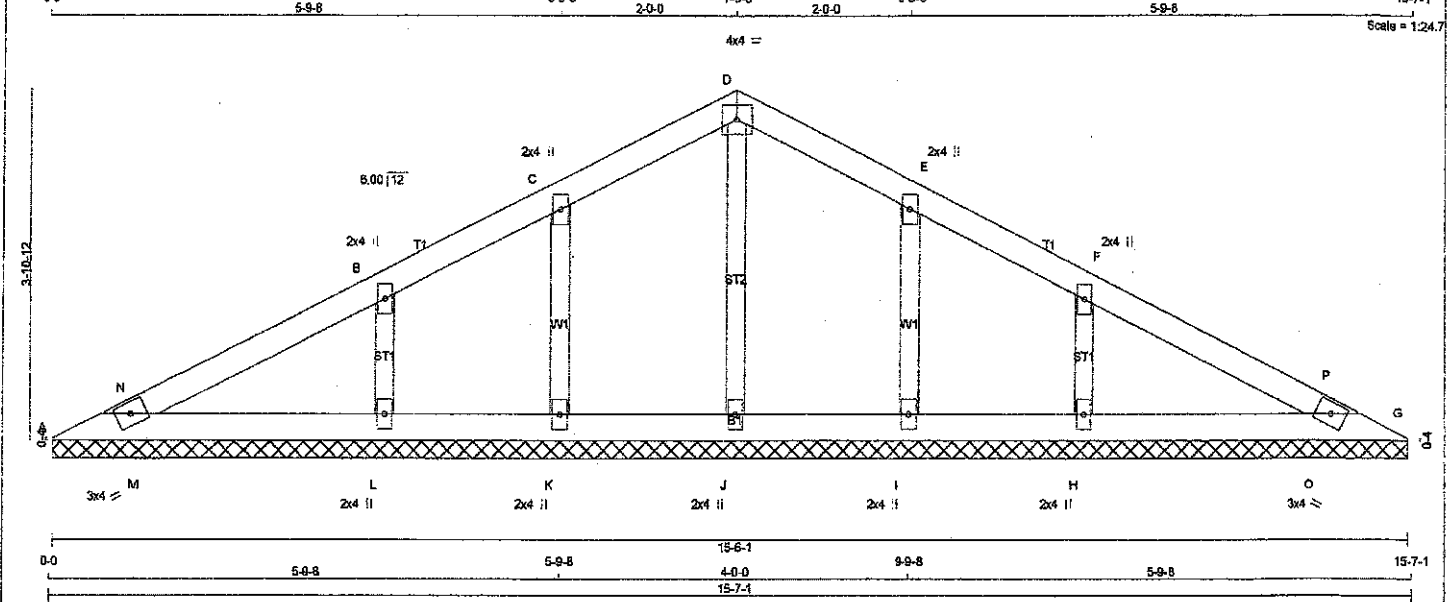
PLATE ROTATION TOL. = 5.0 Deg.

SSI GRIP= 0.88 (C) (INPUT = 0.90)
 SSI METAL= 0.46 (L) (INPUT = 1.00)



DRWG NO. TAM 11910.926
 STRUCTURAL
 COMPONENT ONLY

DWG NO. TAM 719/0927
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, E, F						
S	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0		
G	TBM1-h	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
A	167	167	15-6-1	15-6-1
G	167	167	15-6-1	15-6-1
J	417	417	15-6-1	15-6-1
L	499	499	15-6-1	15-6-1
H	499	499	15-6-1	15-6-1
K	216	216	15-6-1	15-6-1
I	216	216	15-6-1	15-6-1

UNFACTORED REACTIONS

JT	1ST CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	124	70/0	24/0	0/0	0/0	30/0	0/0	0/0
G	124	70/0	24/0	0/0	0/0	30/0	0/0	0/0
J	314	184/0	70/0	0/0	0/0	80/0	0/0	0/0
L	374	204/0	77/0	0/0	0/0	93/0	0/0	0/0
H	374	204/0	77/0	0/0	0/0	93/0	0/0	0/0
K	159	94/0	28/0	0/0	0/0	36/0	0/0	0/0
I	159	94/0	28/0	0/0	0/0	36/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, L, H, K, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (LC)	MEMB.
FR-TO		FROM TO		FR-TO			
A-N	0/110	-102.1 -102.1	0.08 (1)	J-D	-314/0	0.07 (1)	
N-B	0/153	-102.1 -102.1	0.15 (1)	L-B	-314/0	0.05 (1)	
B-C	0/131	-102.1 -102.1	0.15 (1)	K-F	-314/0	0.05 (1)	
C-D	0/143	-102.1 -102.1	0.08 (1)	K-C	-189/0	0.03 (1)	
D-E	0/143	-102.1 -102.1	0.08 (1)	L-E	-189/0	0.03 (1)	
E-F	0/131	-102.1 -102.1	0.15 (1)	M-N	-81/37	0.00 (1)	
F-P	0/153	-102.1 -102.1	0.15 (1)	O-P	-81/37	0.00 (1)	
P-G	0/110	-102.1 -102.1	0.08 (1)				

A-M	-129/0	-38.5	-38.5	0.11 (1)	6.25
M-L	-116/0	-38.5	-38.5	0.11 (1)	6.25
L-K	-129/0	-38.5	-38.5	0.10 (1)	6.25
K-J	-134/0	-38.5	-38.5	0.03 (2)	6.25
J-I	-134/0	-38.5	-38.5	0.03 (2)	6.25
I-H	-129/0	-38.5	-38.5	0.10 (1)	6.25
H-O	-116/0	-38.5	-38.5	0.11 (1)	6.25
O-G	-129/0	-38.5	-38.5	0.11 (1)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (B-N:1), BC=0.11/1.00 (L-M:1), WB=0.07/1.00 (D-J:1), CSI=0.13/1.00 (B-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

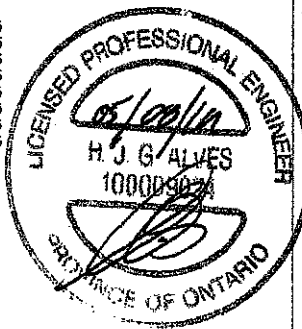
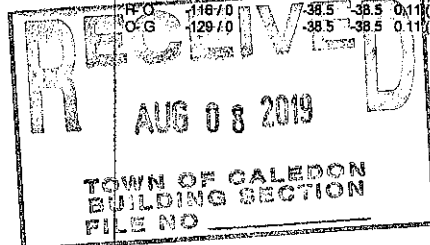
NAIL VALUES

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

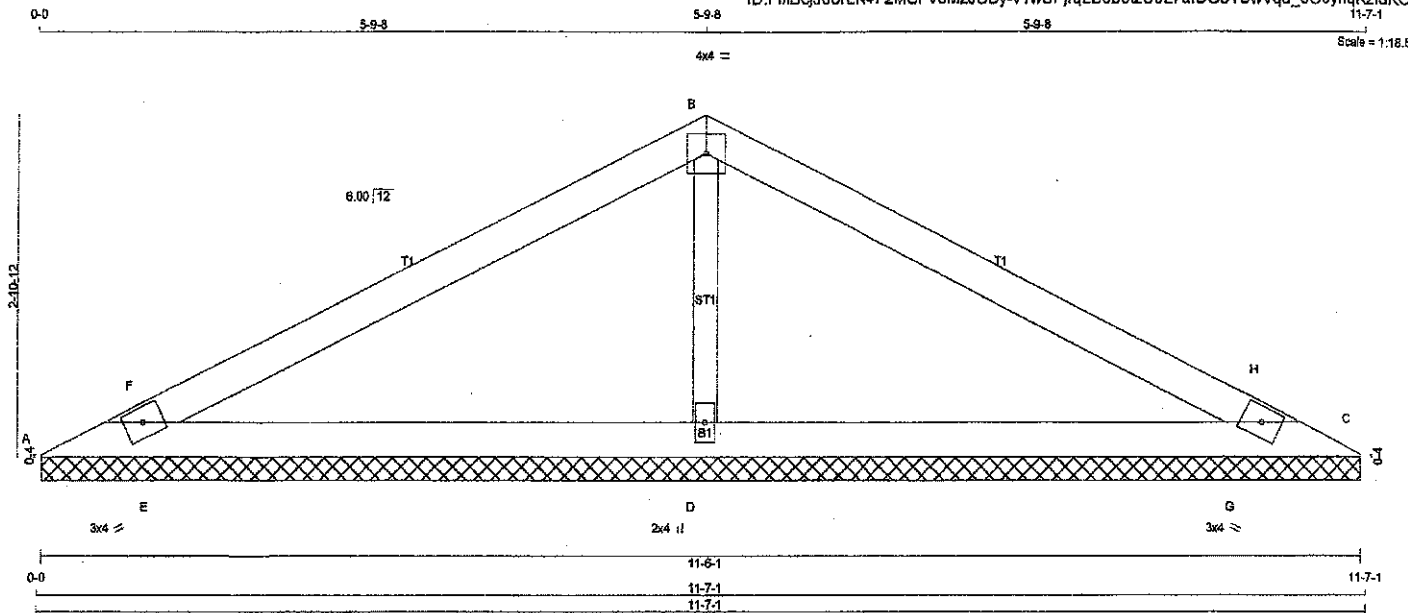
JSI GRIP = 0.19 (F) (INPUT = 0.90)
JSI METAL = 0.13 (F) (INPUT = 1.00)



DWG NO. TAM 1910928
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	V21	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 08:58:05 2019 Page 1
ID: F#BcJ0orLN472MSPveMzJCDy-v1wSPJqLB0b0zU0L7dIDGsY8wVqu_000yhqKzldKO



TOTAL WEIGHT = 28 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0		
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	UPLIFT
A	36	0	36	0
C	36	0	36	0
D	1545	0	1545	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
A	29	11/0	9/0	0/0	0/0	8/0	0/0
C	29	11/0	9/0	0/0	0/0	8/0	0/0
D	1151	646/0	223/0	0/0	0/0	282/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	LC1	MAX	CS1 (LC)	UNBRAC LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	CS1 (LC)
FR-TO								FR-TO			
A-F	0/792	-102.1	-102.1	0.23 (1)	10.00	D-B	-1174/0	0.20 (1)			
F-B	0/801	-102.1	-102.1	0.47 (1)	10.00	E-F	-278/32	0.00 (1)			
B-H	0/801	-102.1	-102.1	0.47 (1)	10.00	G-H	-279/32	0.00 (1)			
H-C	0/792	-102.1	-102.1	0.23 (1)	10.00						
A-E	-757/0	-38.5	-38.5	0.28 (1)	6.25						
E-D	-722/0	-38.5	-38.5	0.35 (1)	6.25						
D-G	-722/0	-38.5	-38.5	0.35 (1)	6.25						
G-C	-757/0	-38.5	-38.5	0.28 (1)	6.25						

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.47/1.00 (B-F:1), BC=0.35/1.00 (D-E:1), WB=0.20/1.00 (B-D:1), SS=0.18/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

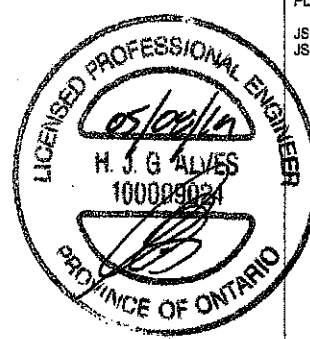
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

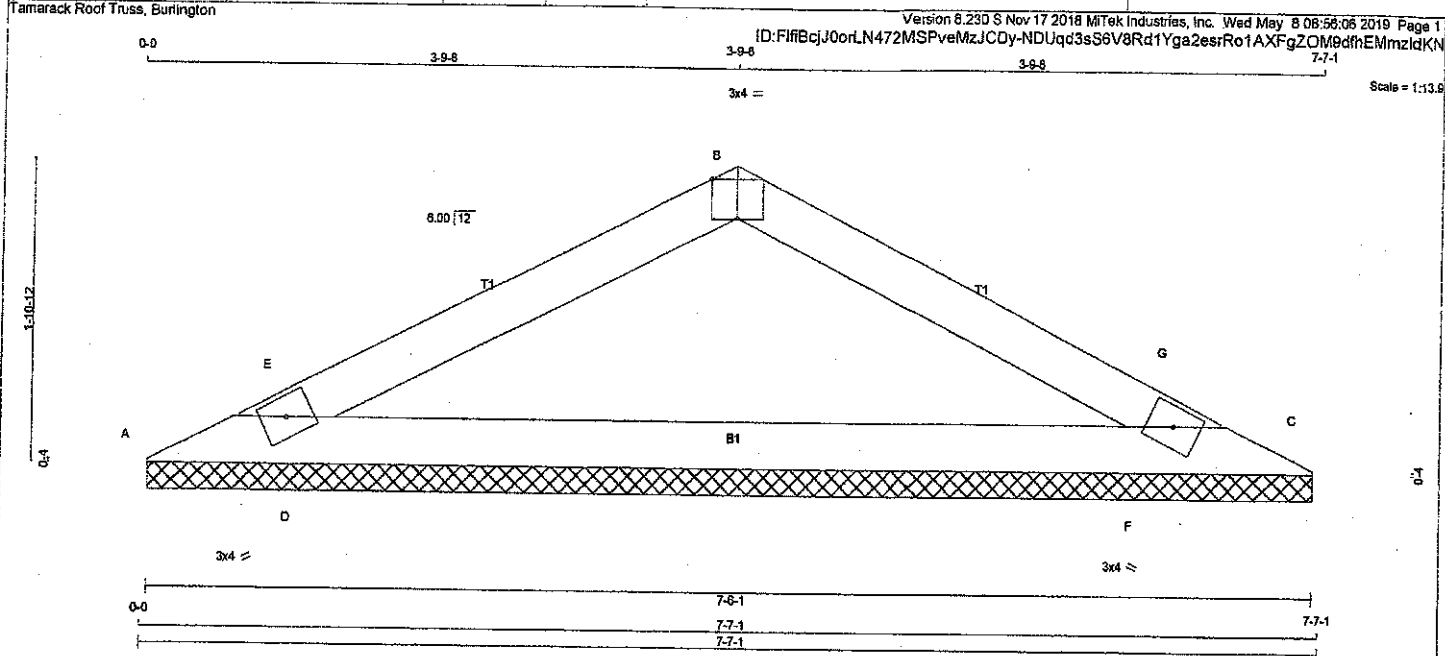
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.90)
JSI METAL= 0.35 (B) (INPUT = 1.00)



DWG NO. TAM 191092A
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402975	V22	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:58:06 2019 Page 1					
ID: F1fBcjJ0orL N472MSPveMzJCdy-NDUqd3sS6V8Rd1Yga2esrRo1AXFgZOMedfEMmzldKN					
7-7-1					
Scale = 1:13.6					



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA			
N L G A RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.	SPF	SPF	FACTORED	MAXIMUM FACTORED	INPUT	REQD
A - B	2x4	DRY	No.2	SPF	SPF	SPF	SPF	GROSS REACTION	GROSS REACTION	BRG	BRG
B - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF	DOWN	HORIZ	UPLIFT	IN-SX
A - C	2x4	DRY	No.2	SPF	SPF	SPF	SPF	528	0	528	0
DRY: SEASONED LUMBER.								528	0	528	0

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TBM1-h	MT20	3.0	4.0	
B	TT-p	MT20	3.0	4.0	Edge 2.00
C	TBM1-h	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX/MIN	COMPONENT REACTIONS	SNOW	LIVE	PERM.LIVE	WIND
A	394	218 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0
C	394	218 / 0	79 / 0	0 / 0	0 / 0	98 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FACTORED	FORCE	MAX	MEMB.	FACTORED	FORCE	MAX
FR-TO	VERT. LOAD	LC1	MAX	FR-TO	VERT. LOAD	LC1	MAX
A-E	-822 / 0	-102.1	-102.1	D-E	0 / 210	0.00	(1)
E-B	-504 / 0	-102.1	-102.1	F-G	0 / 210	0.00	(1)
B-G	-504 / 0	-102.1	-102.1				
G-C	-822 / 0	-102.1	-102.1				
A-D	0 / 490	-38.5	-38.5				
D-F	0 / 494	-38.5	-38.5				
F-C	0 / 490	-38.5	-38.5				

CSI: TC=0.41/1.00 (B-G:1), BC=0.38/1.00 (D-F:1),
WB=0.00/1.00 (D-E:1), SSI=0.25/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

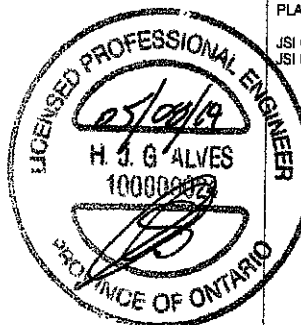
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1856

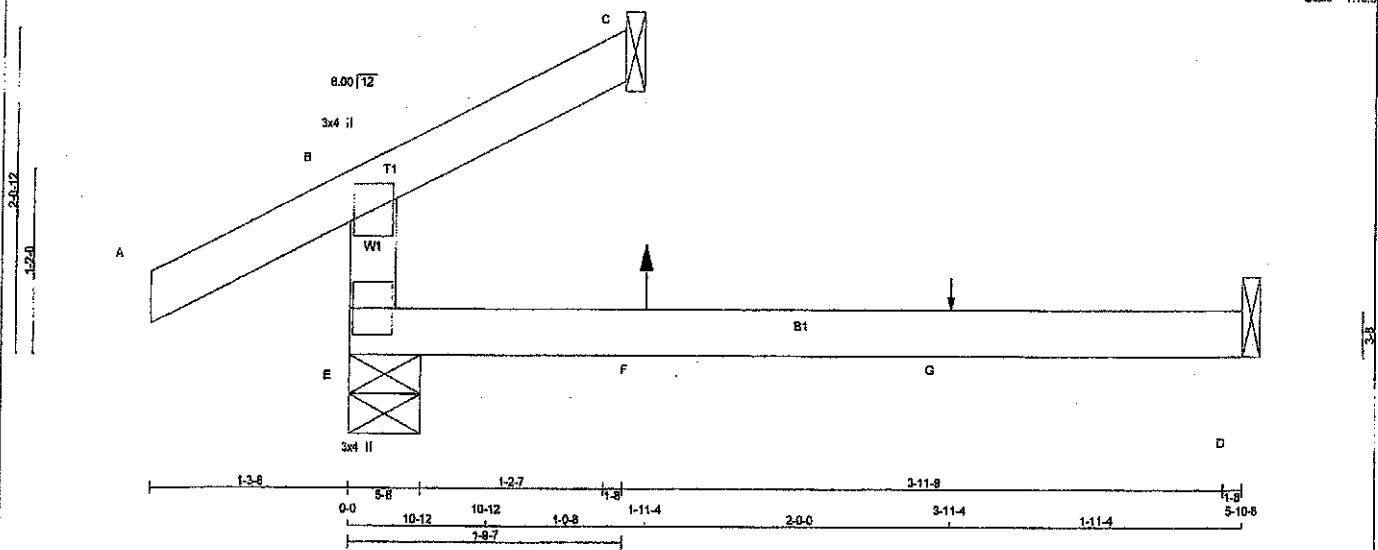
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.35 (C) (INPUT = 0.90)
JSI METAL= 0.21 (C) (INPUT = 1.00)



DWG NO. TAM 1910930
STRUCTURAL
COMPONENT ONLY



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
E	BMV1+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD SRG
	VERT	HORZ	DOWN	HORZ		
E	352	0	352	0	5-8	5-8
C	94	0	97	0	1-8	1-8
D	94	0	123	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
E	259	158/0	42/0	0/0	0/0	61/0	0/0
C	76	24/0	26/0	0/0	0/0	24/0	0/0
D	86	0/-3	53/0	0/0	0/0	35/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (7)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	FORCE (LBS)	VERT. LOAD (PLF)	LC1	MAX	MEMB.	FORCE (LBS)	MAX	CS1 (LC)
FR-TO								
E-B	-229/26	0.0	0.0	0.16 (3)	7.81			
A-B	0/31	-102.1	-102.1	0.15 (1)	10.00			
B-C	0/27	-102.1	-102.1	0.16 (3)	10.00			
E-F	0/0	-38.5	-38.5	0.23 (3)	10.00			
F-G	0/0	-38.5	-38.5	0.23 (3)	10.00			
G-D	0/0	-38.5	-38.5	0.23 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
F	1-11.4	8	1	13	FRONT	VERT	TOTAL	-	-
G	3-11.4	1	1	-	FRONT	VERT	TOTAL	-	-

DESIGN CRITERIA

SPECIFIED LOADS:
 TOP CH. LL = 20.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF NBC 2015, OBC 2012
 - CSA 088-09, CSA 088-14
 - TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(LL) = L/998 (0.05")
 ALLOWABLE DEFL.(TL) = L/360 (0.20")
 CALCULATED VERT. DEFL.(TL) = L/839 (0.08")

CS1 TC=0.18/1.00 (B-E3), BC=0.23/1.00 (D-E3),
 WB=0.80/1.00 (V&H), SSI=0.14/1.00 (D-E3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

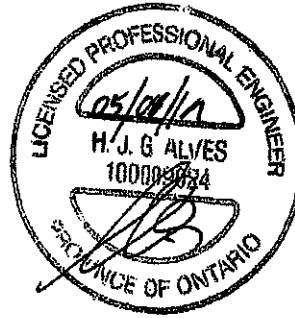
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN	MAX MIN	MAX MIN
MT20	818	354	1987	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.10 (E) (INPUT = 0.50)
 JSI METAL= 0.06 (B) (INPUT = 1.00)



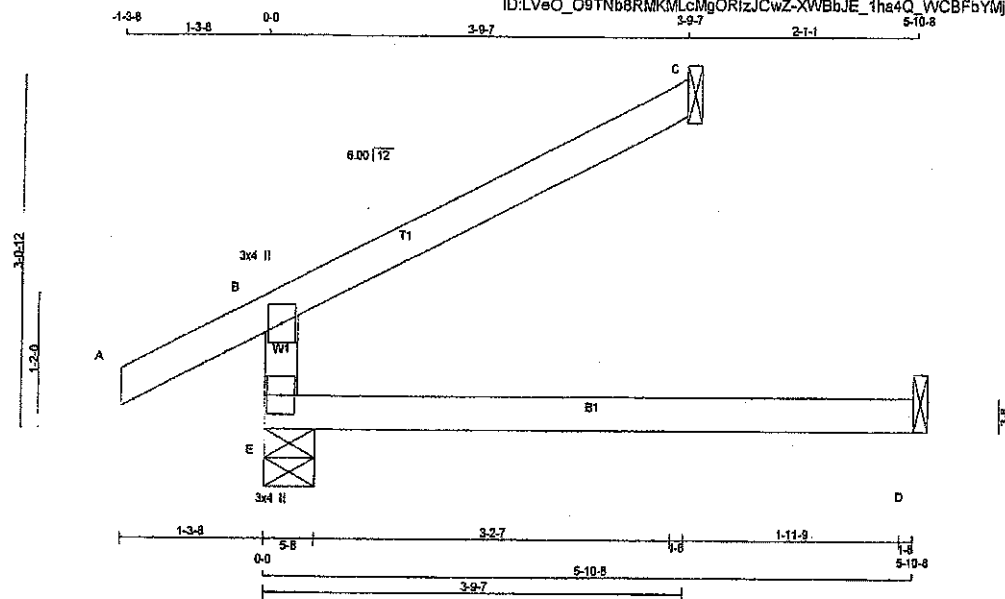
DWG NO. TAM 11910799
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 08:38:22 2019 Page 1
ID:LVeO_O9TNb8RMKMLcMgORizJCwZ-XWBbJE_1hs4Q_WCBFbYmJ3kFFe87GAyYy7yy0Rzldb?

Scale = 1:18.5



TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
E - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
E	513	0	513	0	5-8	5-8
C	145	0	145	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MITK STANDARD DETAIL B37621H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
		COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	382	216 / 0	72 / 0	0 / 0	0 / 0	93 / 0
C	100	83 / 0	0 / 0	0 / 0	0 / 0	17 / 0
D	85	0 / 0	51 / 0	0 / 0	0 / 0	34 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRAC. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC. CSI (LC)	
FR-TO		FROM TO		FR-TO			
E-B	-380 / 0	0.0 0.0	0.21 (3)				7.81
A-B	0 / 31	-102.1 -102.1	0.13 (1)				10.00
B-C	-22 / 0	-102.1 -102.1	0.25 (1)				6.25
E-D	0 / 0	-38.5 -38.5	0.21 (3)				10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/942 (0.07")

CS: TC=0.25/1.00 (B-C:1), BC=0.21/1.00 (D-E:3)
WB=0.00/1.00 (A-B:0), SS=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

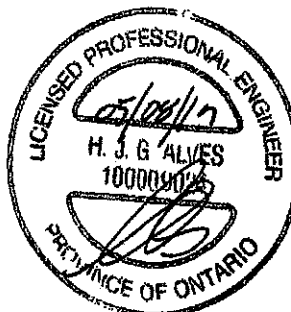
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL. = 0.250 inches

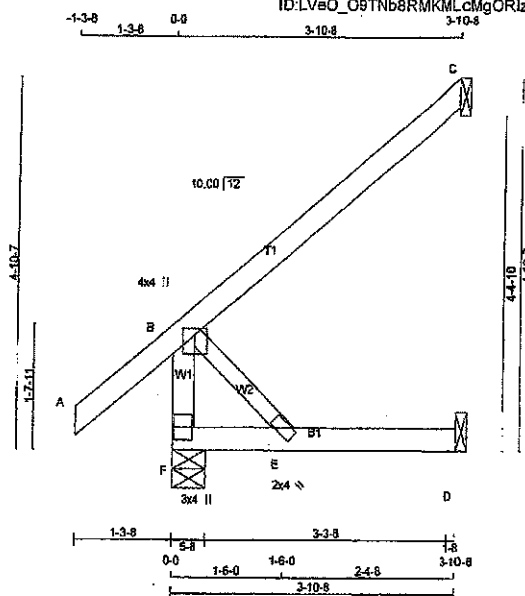
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.16 (E) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



DRWG NO. TAM T1910799
STRUCTURAL
COMPONENT ONLY

JOB NAME 402973	TRUSS NAME J7	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 4 X 15 = 61 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
F - D	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
DRY: SEASONED LUMBER.						

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
F	414	0	0	5-8
C	198	0	0	1-8
D	75	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	301	193 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	136	113 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	68	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)
F-B	-339 / 0	0.0	0.0	0.04 (1)
A-B	0 / 45	-102.1	-102.1	0.14 (1)
B-C	0 / 0	-102.1	-102.1	0.28 (1)
F-E	0 / 0	-38.5	-38.5	0.13 (3)
E-D	0 / 0	-38.5	-38.5	0.14 (3)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBCC 2015
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.28/1.00 (B-C:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

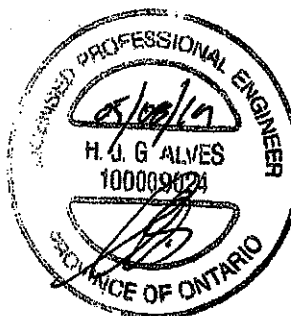
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1667
	768	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI CRIP= 0.28 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

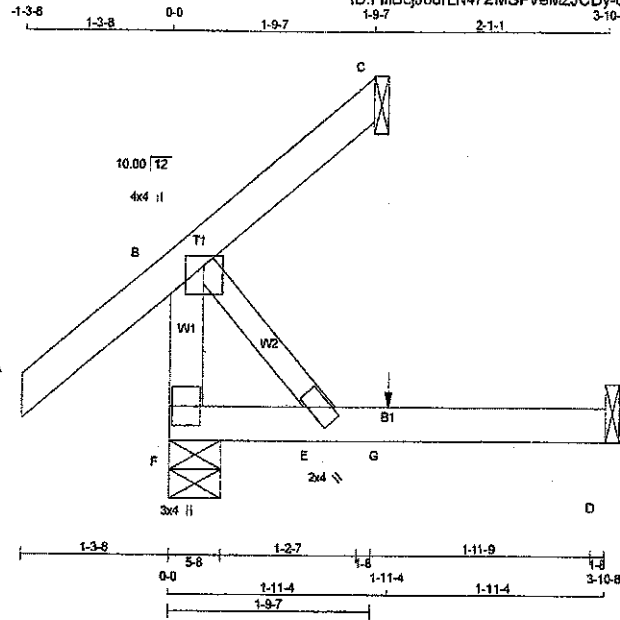


DRWG NO. TAM 1910803
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	J21	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:55:58 2019 Page 1
ID: Ff6BcjJ0orLN472MSPveMzJCdy-cg?oxJmR?27agox86MV_WITkDJaGhnN_nQlp4EzldKV



Scale = 1/10

TOTAL WEIGHT = 12 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4 DRY	No.2	SPF		
A - C	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	DOWN	HORIZ	UPLIFT
JT	VERT	HORIZ	DOWN	HORIZ
F	362	0	362	0
C	36	0	36	0
D	75	0	95	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED							
F	285	184/0	41/0	0/0	0/0	0/0	61/0	0/0
C	25	21/-31	0/0	0/0	0/0	0/0	4/0	0/0
D	68	0/0	41/0	0/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MAX. FACTORED VERT. LOAD (LC4)	MAX. FACTORED VERT. LOAD (LC5)	MAX. FACTORED VERT. LOAD (LC6)	MAX. FACTORED VERT. LOAD (LC7)	MAX. FACTORED VERT. LOAD (LC8)	MAX. FACTORED VERT. LOAD (LC9)	MAX. FACTORED VERT. LOAD (LC10)
FR-TO													
F-B	-288/0	0.0	0.0	0.03 (1)	7.81	0/0	0.00 (1)						
A-B	0/45	-102.1	-102.1	0.15 (1)	10.00								
B-C	-35/0	-102.1	-102.1	0.15 (1)	6.25								
F-E	0/0	-38.5	-38.5	0.12 (2)	10.00								
E-G	0/0	-38.5	-38.5	0.14 (3)	10.00								
G-D	0/0	-38.5	-38.5	0.14 (3)	10.00								

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11.4	1	1	1	FRONT	VERT	TOTAL		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.16")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.16")
CALCULATED VERT. DEFL. (TL) = L/999 (0.02")

CSI: TC=0.15/1.00 (A-B:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

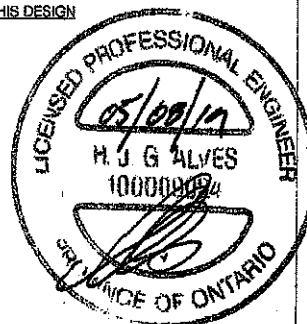
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	788	1987	1655

PLATE PLACEMENT TOL = 0.250 inches

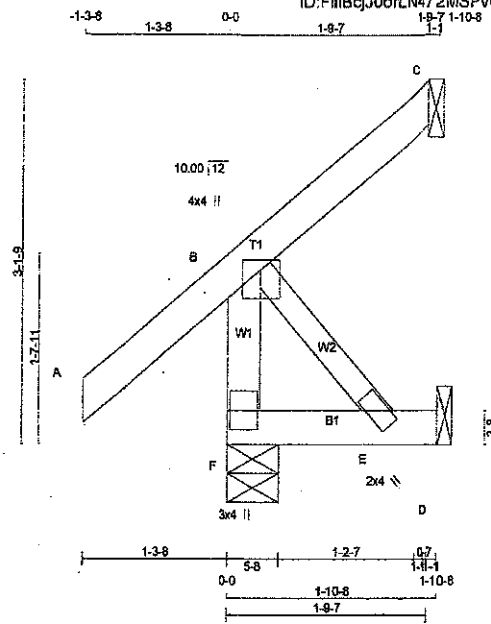
PLATE ROTATION TOL = 5.0 Deg.

SI GRIP=0.22 (B) (INPUT = 0.50)
SI METAL=0.06 (B) (INPUT = 1.00)



DRG NO. YAM 71910921
STRUCTURAL
COMPONENT ONLY

ID: FmBcJ0orL N472MSPveMzJCdy-4tYB9fm3mLFRlyWKf40D2y0vBjy8QEEd804VNgzldKU
1-9-7 1-10-8



Scale = 1/16"

TOTAL WEIGHT = 10 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY No.2			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMVW+p	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	GROSS REACTION	DOWN	HORZ	UP	BRG	IN-SX	BRG
F	324	0	324	0	0	5-8	5-8		
C	36	0	36	0	0	1-8	1-8		
D	36	0	48	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS							
JT	COMBINED	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD
F	230	164	10	20	0	0	47
C	25	21	1	0	0	0	4
D	33	0	0	20	0	0	13

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (5)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO				FR-TO			
F-B	-288	0.0	0.0	B-E	0	0.0	
A-B	0	-102	0.14				
B-C	-35	-102	0.13				
F-E	0	-38.5	0.03				
E-D	0	-38.5	0.03				

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/999 (0.00")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

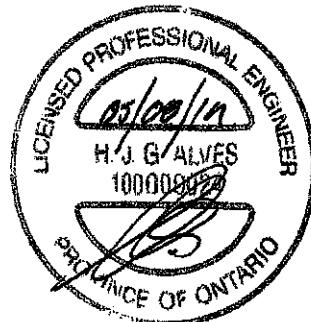
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.60)
JSI METAL= 0.06 (B) (INPUT = 1.00)



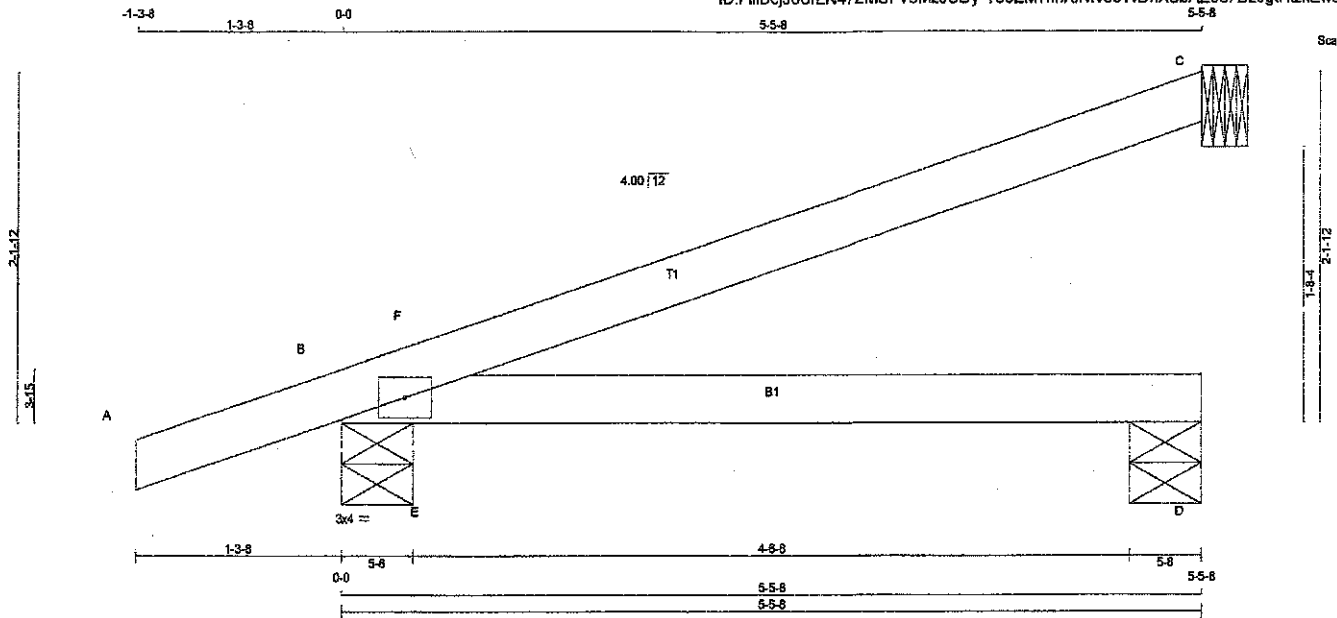
DRWG NO. TAM 1916 922
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402975	J23	7	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 08:56:00 2019 Page 1
ID: FfBcJ0orLN472MSPveMzJCDy-Y36ZM7nhXNiv65WDnXSbAZ0c7Dz9gtHEKEw87zIdKT

Scale = 1:13.6



TOTAL WEIGHT = 7 X 14 = 101 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
C	250	0	250	0	5-8	5-8
B	520	0	520	0	5-8	5-8
D	133	0	145	0	5-8	5-8

SEE MITEK STANDARD DETAIL 837821H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	361	236 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX CS1 (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
FR-TO		FROM	TO			FR-TO			
A-B	0 / 20	-102.1	-102.1	0.13 (1)	10.00	E-F	-254 / 122	0.00 (1)	
B-F	-51 / 0	-102.1	-102.1	0.13 (3)	6.25				
F-C	0 / 6	-102.1	-102.1	0.42 (1)	10.00				
B-E	0 / 0	-38.5	-38.5	0.26 (1)	10.00				
E-D	0 / 0	-38.5	-38.5	0.31 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1	PSF
	DL = 6.0	PSF
BOT CH.	LL = 10.5	PSF
	DL = 7.0	PSF
TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/842$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/489$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.26/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

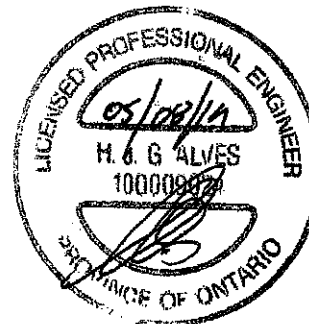
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PL)	(PL)
	MAX	MIN	MAX
MT20	618	354	1667
	788	1987	1858

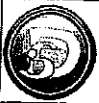
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DRWG NO. TAM 7910023
STRUCTURAL
COMPONENT ONLY



isDesign™

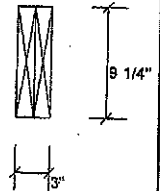
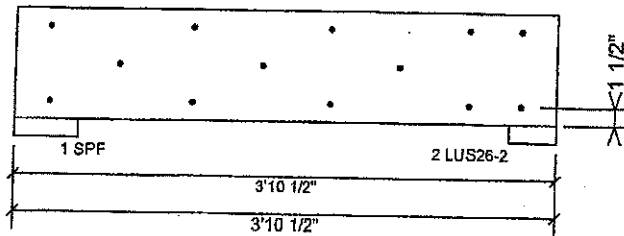
Client:
Project:
Address:

Date: 8/8/2019
Designer:
Job Name: 201018
Project #:

Page 2 of 2

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

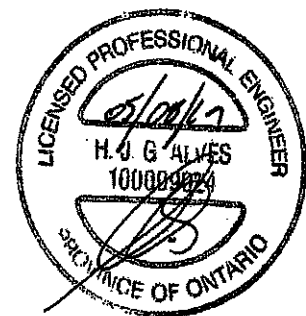
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	68.7 %
Load	233.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 17910811
STRUCTURAL
COMPONENT ONLY 4/2

Manufacturer Info	Tamarack Roof Trusses 3288 North Service Rd., ON Canada L7N3G2 (905) 335-1115
	TAMARACK LUMBER INC ALMA LUMBER GROUP

This design is valid until 12/11/2021

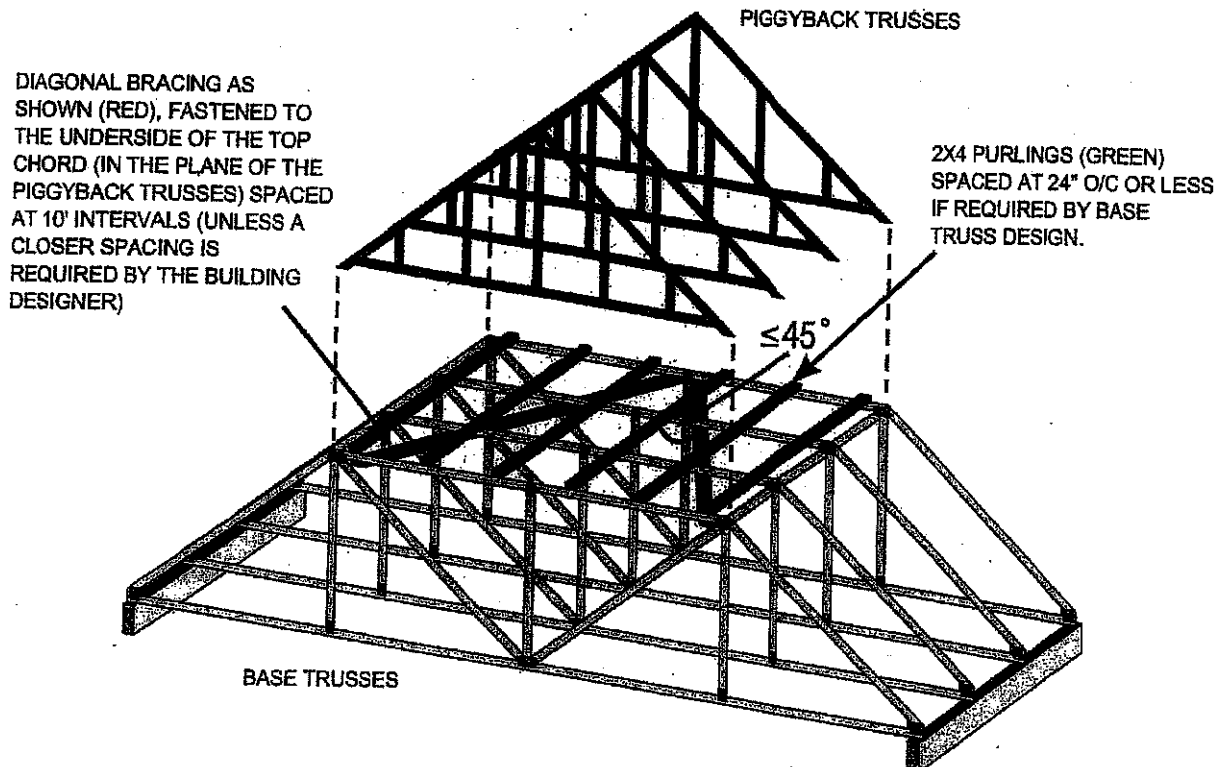


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:

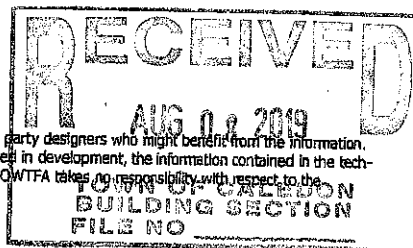


NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.



LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

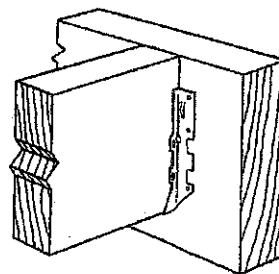
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

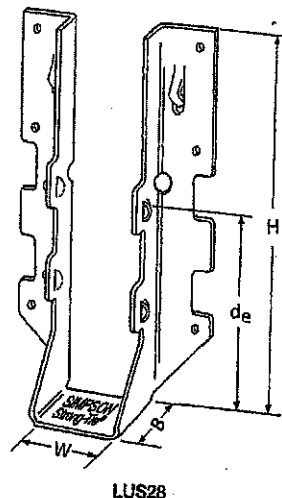
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

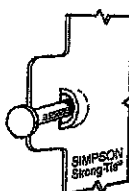


Typical LUS
Installation

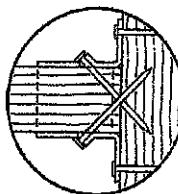


Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LUS24	18	1 1/8	3 1/4	1 3/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/4	2	1 1/8	(4) 16d	(2) 16d	835	2020	580	1435
LUS26	18	1 1/8	4 1/4	1 3/4	3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/4	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/4	2	3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/4	2	3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Double Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing
Top View.



(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

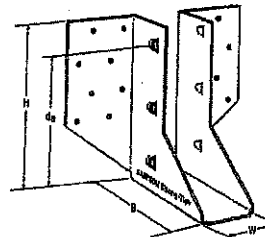
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

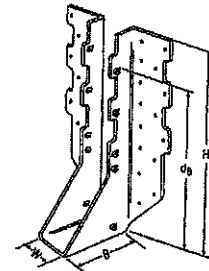
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

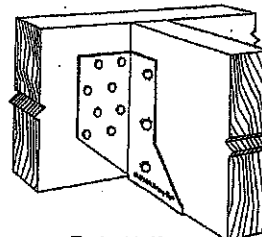
- See current catalogue for options



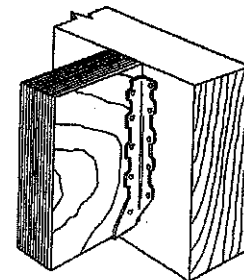
LJS26DS



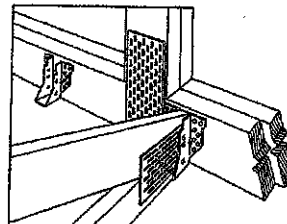
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



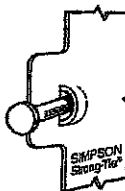
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

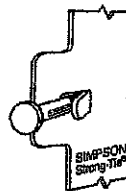
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
LJS26DS	18	1 1/8	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/2	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 3/8	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

1. d_s is the distance from the seat of the hanger to the highest joist nail.

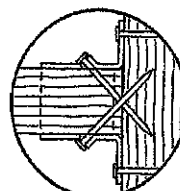


Double Shear Nailing prevents tabs breaking off (available on some models).

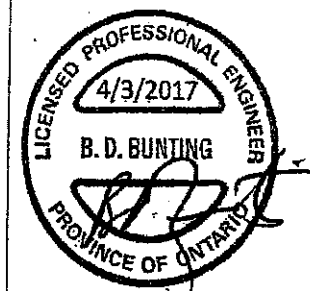
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



(800) 999-5099
strongtie.com

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

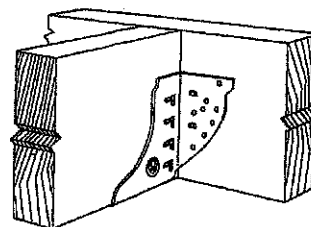
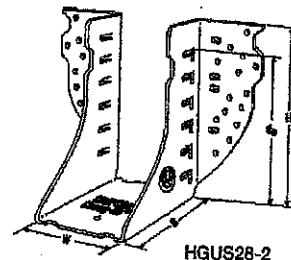
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

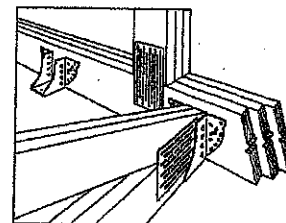
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

- See current catalogue for options



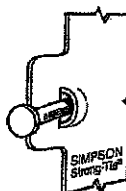
Typical HGUS
Installation



Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

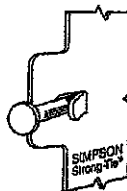
Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
HGUS28	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS28-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(18) 16d		6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(18) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(18) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7840	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645

1. d_s is the distance from the seat of the hanger to the highest joist nail.

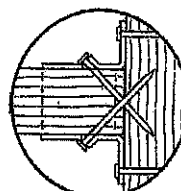


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



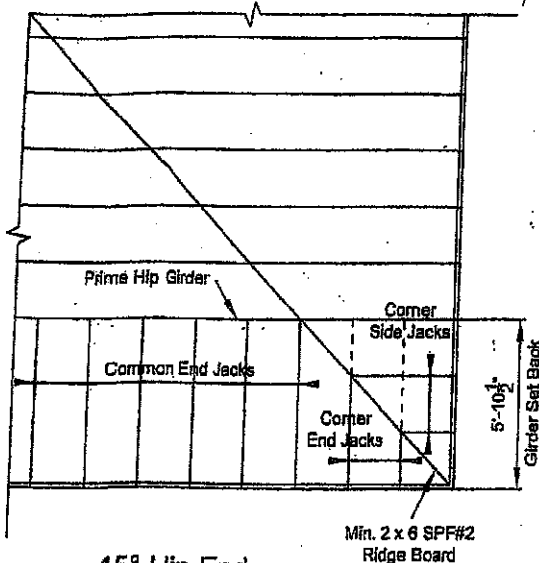
Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



(800) 999-5099
strongtie.com



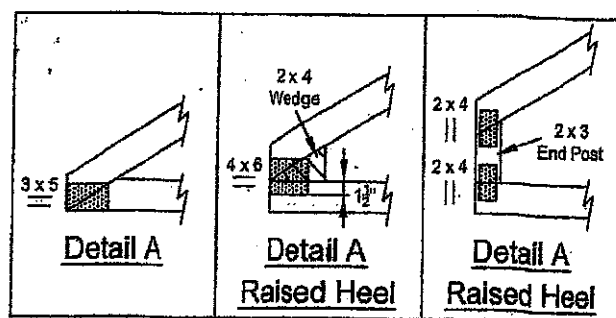
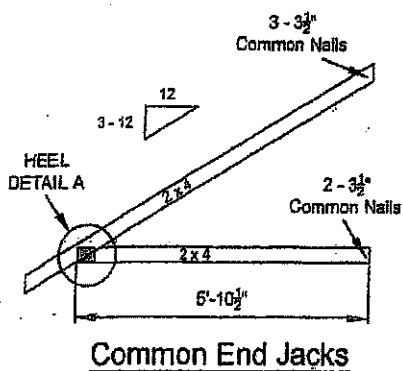
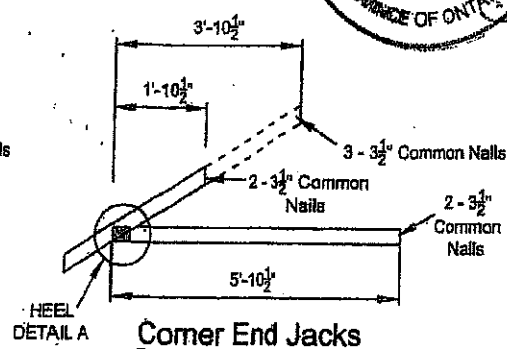
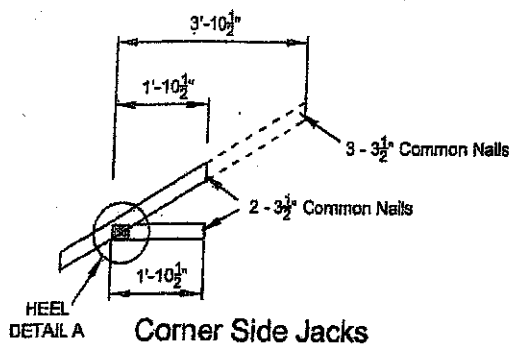
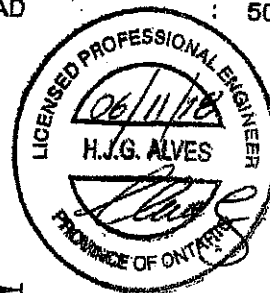
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

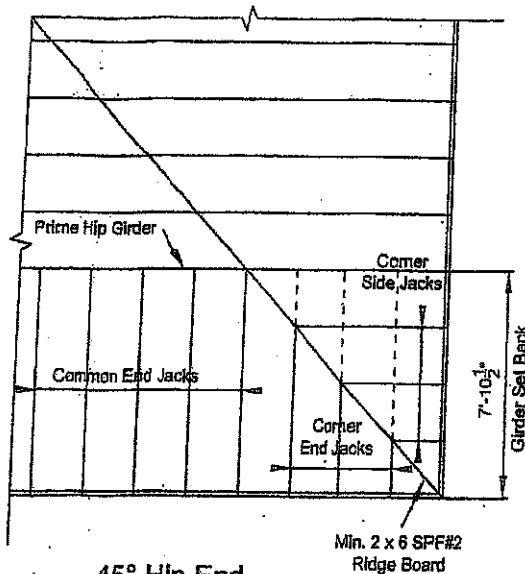
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F



NOTE: DESIGN CONFORMS TO PART 6, O.B.C. 2012 (L.S.D. DESIGN)

T-1800218



45° Hip End

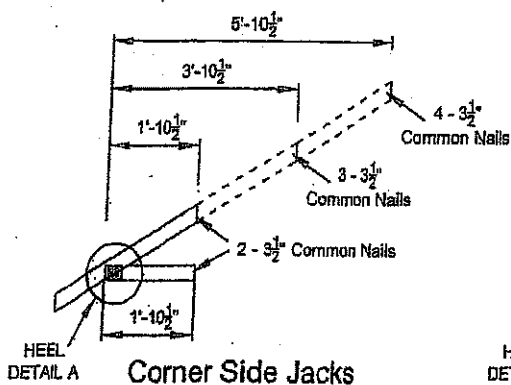
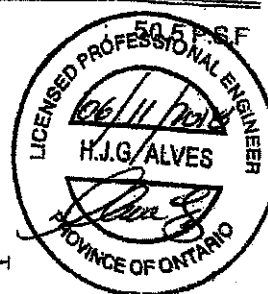
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

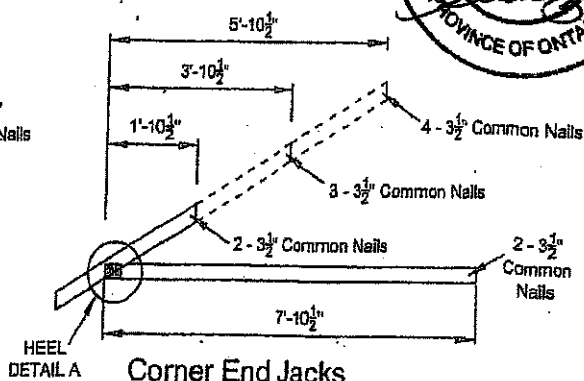
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

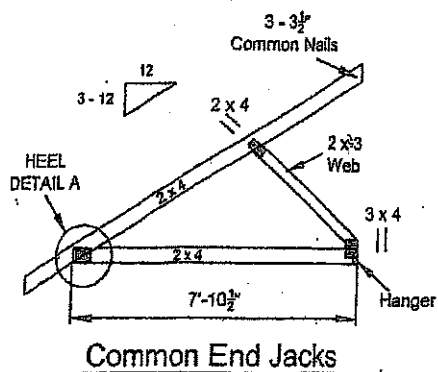
TOTAL LOAD



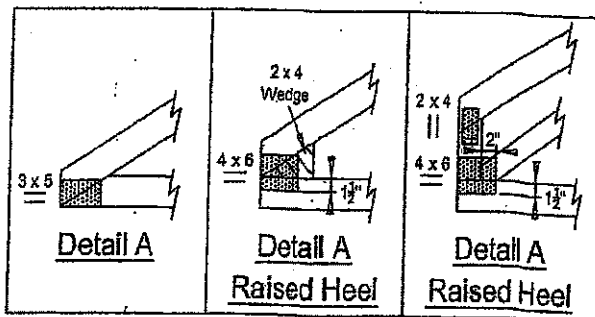
Corner Side Jacks



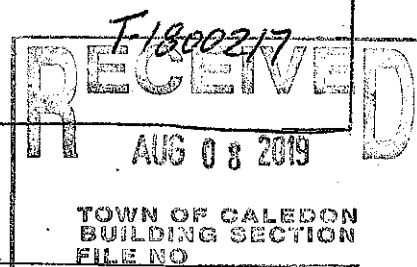
Corner End Jacks



Common End Jacks

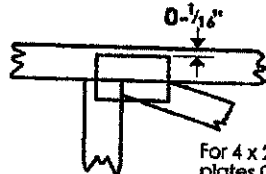
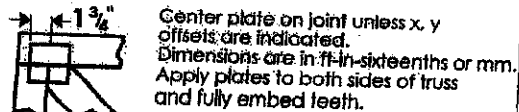


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)



Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0- $\frac{1}{4}$ \"/>



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

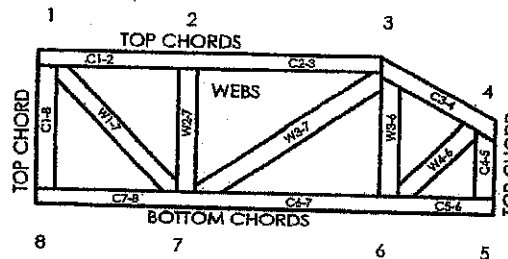


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: Mii-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate Institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300218

Feb 09, 2018