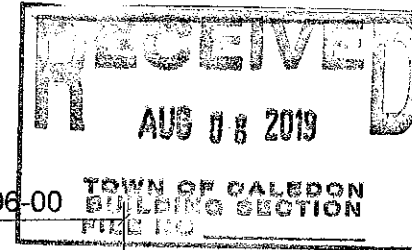


9/12 ROOF PITCH
UNLESS OTHERWISE
SPECIFIED

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
SNOW LOAD 1.8 KPA
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART 9 OF THE OBC. LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO BE
2"X4" SPF @24"o.c. WITH A
2"X4" SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS AND
BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

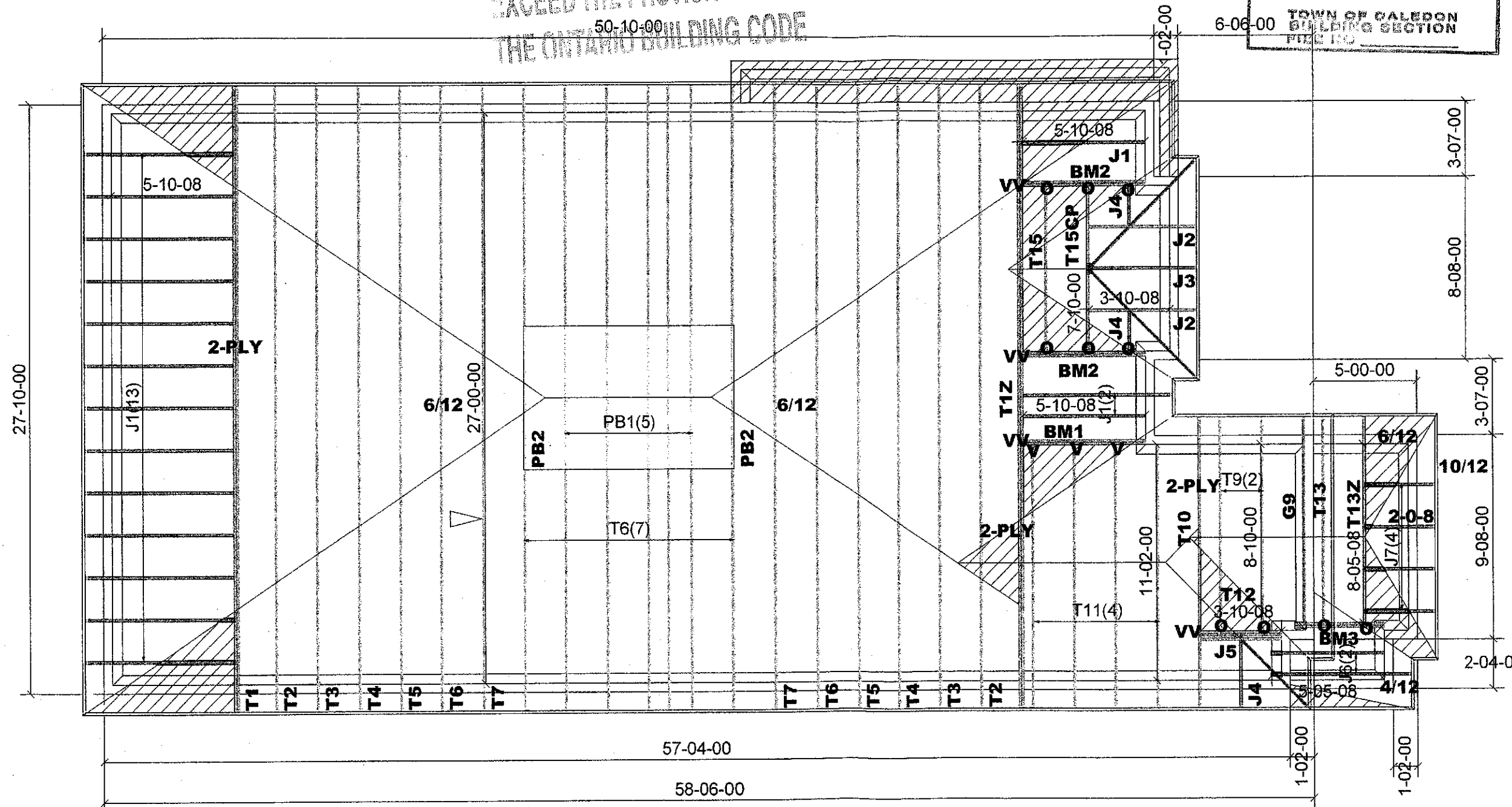
TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

HARDWARE
HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V) **BM1,2,3 :2-2X10**
LUS24(O)

DENOTES
CONVENTIONAL
FRAMING



T- ...



M12241

 ALFA LUMBER GROUP	Job Track: 50033	Builder / Location: GREEN PARK HOMES / CALEDON		Model / Elevation: BARTON 4/1 Lot 34	Milek ver 8.2.3.229
	Plan Log: 201021	Project: LAMBERT LANE PH.2		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC.. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.	
	Layout ID: 402979	Date: 5/7/2019	Sales: Mario DiCano	Designer: JG	



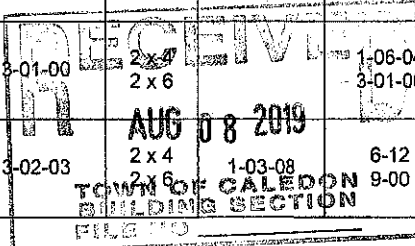
DELIVERY SHIPLIST

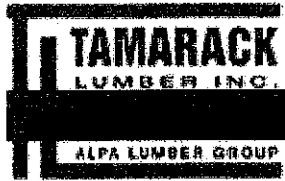
Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 4
 Lot #: 34
 Elevation: 1

Job Track: 50033
 PlanLog: 201021
 Layout ID: 402979
 Ref #
 Page: 1 of 3
 Date: 05/07/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	253.7 162.67		
	1 2-ply	T12 Hip Girder	9/12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	253.7 162.67		
	2	T2 Hip	9/12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	221.33 142.00		
	2	T3 Hip	9/12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	242.71 157.33		
	2	T4 Hip	9/12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	245.08 159.00		
	2	T5 Hip	9/12	27-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	259.05 166.67		
	9	T6 Hip	9/12	27-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1166.51 738.00		
	2	T7 Hip	9/12	27-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	264.11 169.33		
	2	T9 Common	9/12	8-10-00	4-10-00	2 x 4	1-03-08	1-06-04 1-06-04	75.32 51.00		
	1	G9 GABLE	9/12	8-10-00	4-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	40.94 27.83		
	1	T10 Hip Girder	9/12	11-02-00	4-05-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	61.14 40.00		
	4	T11 Common	9/12	11-02-00	5-08-08	2 x 4	1-03-08	1-06-04 1-06-04	184.83 124.00		
	1 2-ply	T12 Jack-Closed Girder	9/12	3-10-08	3-01-00	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 3-01-00	43.25 29.00		
	1	T13 Common	6/12	8-05-08	3-02-03	2 x 4 2 x 6	1-03-08 1-03-08	6-12 9-00	30.87 20.33		





DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 4
 Lot #: 34
 Elevation: 1

Job Track: 50033
 PlanLog: 201021
 Layout ID: 402979
 Ref #
 Page: 2 of 3
 Date: 05/07/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T13Z Common Girder	6/12	8-05-08	3-02-03	2 x 4 2 x 6	1-03-08	6-12 9-00	30.87 20.33		
	1	T15 Common	9/12	7-10-00	4-05-08	2 x 4		1-06-04 1-06-04	32.16 22.00		
	1	T15CP Hip Girder	9/12	7-10-00	4-05-02	2 x 4		1-06-04 1-06-04	32.12 22.00		
	5	PB1 Piggyback	9/12	6-09-05	2-06-08	2 x 4			79.19 53.33		
	2	PB2 Piggyback	9/12	6-09-05	2-00-00	2 x 4			35.06 25.33		
	16	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J2 Jack-Open Girder	9/12	1-09-07	2-10-05	2 x 4	1-03-08 2-01-01	1-06-04 2-10-05	26.68 18.67		
	1	J3 Jack-Open	9/12	3-10-08	4-05-02	2 x 4	1-03-08	1-06-04 4-05-02	14.68 9.67		
	3	J4 Jack-Open	9/12	1-10-08	2-10-05	2 x 4		1-06-04 2-11-02	22.59 17.00		
	1	J5 Jack-Open Girder	9/12	3-10-08	2-10-05	2 x 4		1-06-04 4-05-02	10.02 7.00		
	2	J6 GABLE	4/12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	28.9 18.67		
	4	J7 Jack-Open	10/12	2-00-08	3-01-14	2 x 4	1-03-08	1-00-07 2-08-14	32.99 24.00		

TOTAL # TRUSS= 73

TOTAL BFT OF ALL TRUSSES= 2558.5

BFT.

TOTAL WEIGHT OF ALL TRSSES 3956.5 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	LJS26DS	
10	Hardware	LUS24	

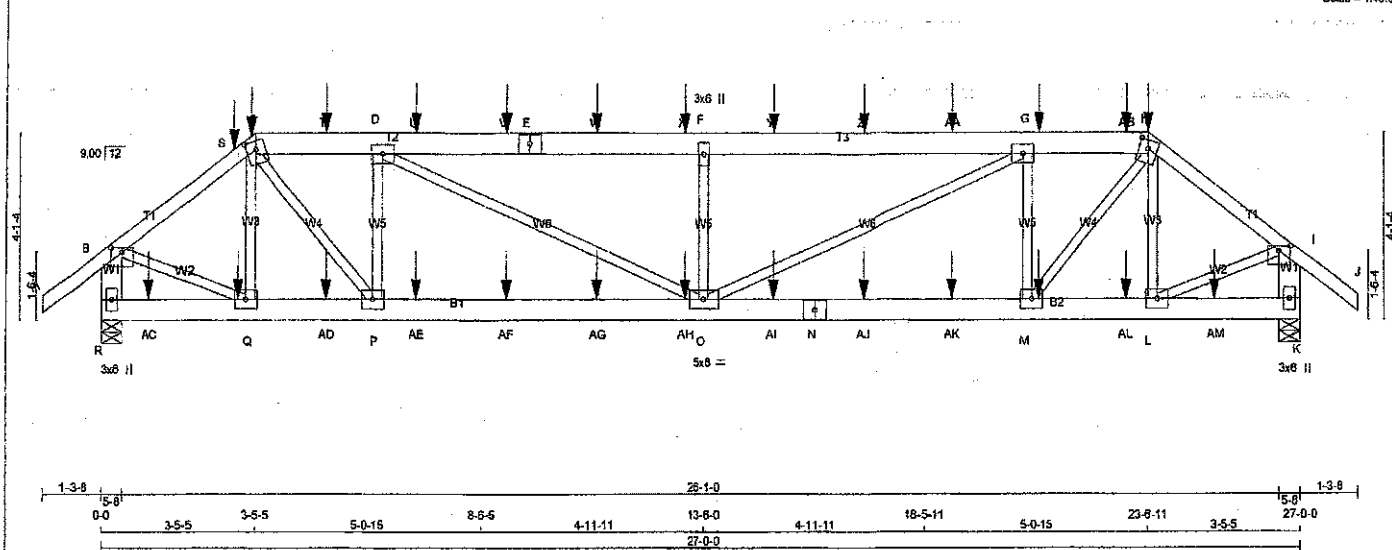
DELIVERY SHIPLIST		
 TAMARACK LUMBER INC. ALPA LUMBER GROUP	Lumber Yard: TAMARACK LUMBER	Job Track: 50033
	Builder: GREEN PARK HOMES	PlanLog: 201021
	Project: LAMBERT LANE PH.2	Layout ID: 402979
	Location: CALEDON	Ref #:
	Model: BARTON 4	Page: 3 of 3
	Lot #: 34	Date: 05/07/2019
	Elevation: 1	Designer: Jane Gong
		Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
4	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 17

Tamarack Roof Truss, Burlington Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:07:58 2019 Page 1
 1-3-8 0-0 3-0-12 3-0-12 3-5-5 5-0-15 8-6-5 4-11-11 13-6-0 4-11-11 18-5-11 4-7-1 23-0-12 23-6-11 3-5-5 27-0-0 1-3-8
 Scale = 1/4" = 1'-0"



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY No.2	SPF
C - E	2x6	DRY No.2	SPF
E - H	2x6	DRY No.2	SPF
H - J	2x4	DRY No.2	SPF
R - B	2x6	DRY No.2	SPF
K - I	2x6	DRY No.2	SPF
N - K	2x6	DRY No.2	SPF
N - N	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE(61.0)
H-J	12	SIDE(61.0)
C-E	12	SIDE(61.0)
E-H	12	SIDE(183.1)
R-B	12	TOP
K-I	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
R-N	12	SIDE(183.1)
N-K	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

<u>PLATES (table is in inches)</u>						
JT	TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	6.0	1.25	3.00
C	TTWW-m	MT20	5.0	6.0	2.25	2.50
D	TMWV-l	MT20	5.0	6.0		
E	TS-t	MT20	5.0	6.0		
F	TMWV-w	MT20	3.0	6.0		
G	TMWV-l	MT20	5.0	8.0		
H	TTWW-m	MT20	5.0	8.0	2.25	2.50
I	TMWV-p	MT20	5.0	8.0	1.25	3.00
K	BMV1+p	MT20	3.0	6.0		
L	M.P.Q					
N	BMWV-l	MT20	5.0	6.0		
O	BS-t	MT20	5.0	6.0		
P	BMWVW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT VERT	3433	0	5-8
R	3433	0	5-8
K	3358	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE
R	2554	1447 / 0
K	2497	1417 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 4.46 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.0 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

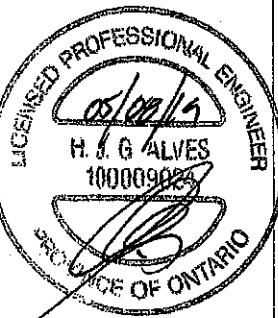
CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	Q-C	-777 / 0	0.10 (1)
B-S	-3441 / 0	-102.1	-102.1	0.09 (1)	4.87	C-P	0 / 2841	0.35 (1)
S-C	-3441 / 0	-102.1	-102.1	0.19 (1)	4.87	P-D	-1855 / 0	0.22 (1)
C-T	-4524 / 0	-102.1	-102.1	0.22 (1)	5.19	D-O	0 / 1907	0.24 (1)
T-D	-4524 / 0	-102.1	-102.1	0.22 (1)	5.19	O-F	-1243 / 0	0.15 (1)
D-U	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	G-G	0 / 1927	0.24 (1)
U-V	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	M-G	-1859 / 0	0.23 (1)
V-E	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	N-H	0 / 2874	0.38 (1)
E-W	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	L-H	-774 / 0	0.10 (1)
W-X	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	B-Q	0 / 2940	0.36 (1)
X-F	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48	L-I	0 / 2800	0.36 (1)
F-Y	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48			
Y-Z	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48			
Z-AA	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48			
AA-G	-8227 / 0	-102.1	-102.1	0.34 (1)	4.48			
G-AB	-4505 / 0	-102.1	-102.1	0.21 (1)	5.21			
AB-H	-4505 / 0	-102.1	-102.1	0.21 (1)	5.21			
H-I	-3429 / 0	-102.1	-102.1	0.18 (1)	4.91			
I-J	0 / 42	-102.1	-102.1	0.06 (1)	10.00			
R-B	-3334 / 0	0.0	0.0	0.12 (1)	7.61			
K-I	-3276 / 0	0.0	0.0	0.12 (1)	7.67			

CEASED PROFESSIONAL ENGINEER

05/02/13

W. J. B. JONES

MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS1 (LC)
R-A	0 / 0	-38.5	-38.5
AC-Q	0 / 0	-38.5	-38.5
Q-AD	0 / 2813	-38.5	-38.5
AD-P	0 / 2813	-38.5	-38.5
P-AE	0 / 4522	-38.5	-38.5
AE-AF	0 / 4522	-38.5	-38.5
AF-AG	0 / 4522	-38.5	-38.5
AG-AH	0 / 4522	-38.5	-38.5
AH-O	0 / 4522	-38.5	-38.5
O-AI	0 / 4504	-38.5	-38.5
AI-N	0 / 4504	-38.5	-38.5
N-AJ	0 / 4504	-38.5	-38.5
AJ-AK	0 / 4504	-38.5	-38.5
AK-M	0 / 4504	-38.5	-38.5
M-AL	0 / 2771	-38.5	-38.5
AL-L	0 / 2771	-38.5	-38.5
L-AM	0 / 0	-38.5	-38.5
AM-K	0 / 0	-38.5	-38.5



DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 29.1 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
 ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, CBC 2012
 - CSA C88-09, CSA C88-14
 - TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.11")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.19")

CSI: TC=0.34/1.00 (F-G-1), SC=0.39/1.00 (M-O-1), WB=0.38/1.00 (B-Q-1), SS=0.22/1.00 (D-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1867 768 1987 1558

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (L) (INPUT = 0.90)
 JSI METAL= 0.49 (N) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402979	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		TRUSS DESC.			

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 15:07:58 2018 Page 2
ID: JIMS6RLJVLhc2snmluv9GyJDZV-ShfhQlt csDdQvFsezx67av0Yel6IKFIFzVVtXziXt

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-44	-49	---	FRONT	VERT	DEAD	---	---
C	3-5-5	-251	-251	---	FRONT	VERT	SNOW	---	---
G	21-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
H	23-6-11	-44	-49	---	FRONT	VERT	DEAD	---	---
H	23-6-11	-251	-251	---	FRONT	VERT	SNOW	---	---
M	21-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
Q	3-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
S	3-0-12	-155	-155	---	BACK	VERT	TOTAL	---	---
T	5-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
U	7-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
V	9-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
W	11-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
X	13-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
Y	15-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
Z	17-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
AA	19-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
AB	23-0-12	-149	-149	---	BACK	VERT	TOTAL	---	---
AC	1-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AD	5-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AE	7-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AF	9-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AG	11-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AH	13-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AI	15-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AJ	17-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AK	19-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AL	23-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AM	25-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 17910977
STRUCTURAL
CHECKED ONLY 7/2

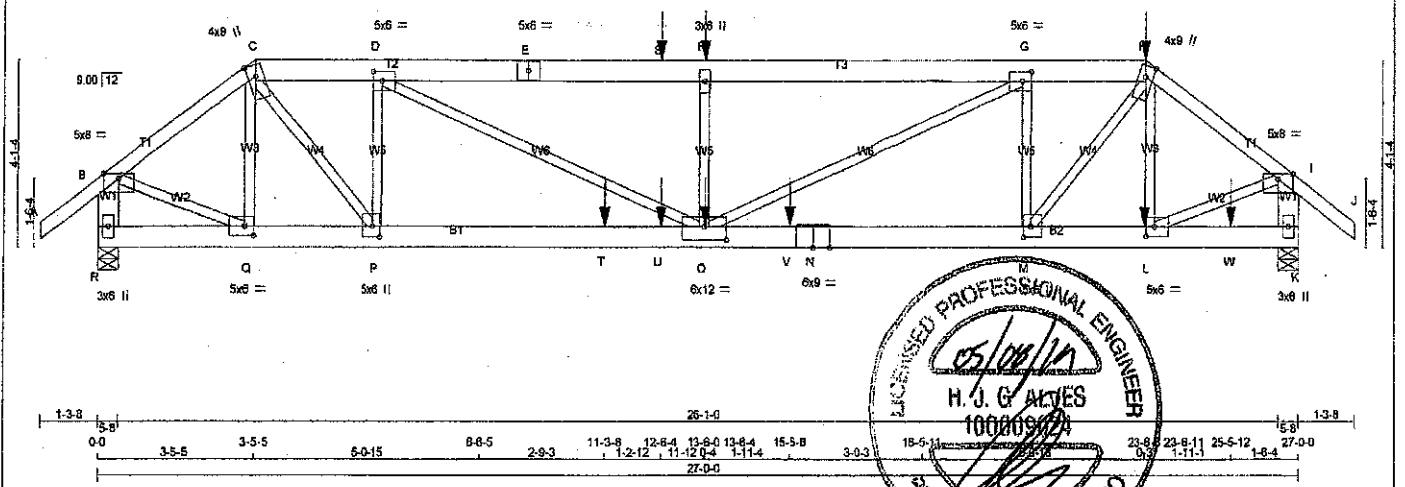
JOB NAME 402979	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO. 1
TRUSS DESC.				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:07:59 2019 Page 1	

Temerack Roof Truss, Burlington

ID: JIMS6RLJfVhC2nmfuv9GyJDZW-Z1D3deucNALU23g2CgSLgoRBs2VL1kRRUdFS?zzIXtk

1-3-8 0-0 3-5-5 3-5-5 5-0-15 8-6-5 3-11-15 12-8-4 13-6-0 13-6-4 4-11-7 18-5-11 5-0-15 23-8-11 3-5-5 27-0-0 28-3-8

Scale = 1/4" = 1'-0"



LUMBER	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
J - K	2x6	DRY	No.2	SPF
K - L	2x6	DRY	No.2	SPF
L - N	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A-C	1	12	TOP
H-J	1	12	SIDE(61.0)
C-E	2	12	TOP
E-H	2	12	SIDE(183.1)
R-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
R-N	2	12	SIDE(0.0)
N-K	2	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS			
L-H	1	6	SIDE(130.1)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
B TMVW-p	MT20	5.0	8.0	Edge
C TTWW-m	MT20	4.0	9.0	Edge
D TMVW-i	MT20	5.0	6.0	2.50 2.50
E TS-i	MT20	5.0	6.0	
F TMVW-w	MT20	3.0	6.0	
G TMVW-i	MT20	5.0	6.0	2.50 2.50
H TTWW-m	MT20	4.0	9.0	Edge
I TMVW-p	MT20	5.0	6.0	Edge
K BMV1+p	MT20	3.0	6.0	
L BMVW-i	MT20	5.0	6.0	2.50 2.25
M BMVW-H	MT20	5.0	6.0	2.75 2.00
N BS-i	MT20	6.0	9.0	
O BMVW-w	MT20	6.0	12.0	3.50 6.00
P BMVW-H	MT20	5.0	6.0	2.75 2.00
Q BMVW-i	MT20	5.0	6.0	2.50 2.25
R BMV1+p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UPLIFT	IN-SX
R 3913	0	0	5-8
K 4968	0	0	5-8

UNFACTORED REACTIONS	1ST LOASE	MAX./MIN.	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE	PERM.LIVE
R 2885	1685 / 0	516 / 0	0 / 0
K 3983	2167 / 0	626 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO						FR-TO			
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	Q-C	-1323 / 0	0.16 (1)	0.56 (1)
B-C	-4055 / 0	-102.1	-102.1	0.18 (1)	4.57	C-P	0 / 4517	0.56 (1)	0.56 (1)
C-D	-8011 / 0	-102.1	-102.1	0.19 (1)	4.67	P-D	-2682 / 0	0.32 (1)	0.32 (1)
D-E	-9854 / 0	-102.1	-102.1	0.37 (1)	3.66	D-O	0 / 4298	0.53 (1)	0.53 (1)
E-S	-9854 / 0	-102.1	-102.1	0.37 (1)	3.66	O-F	-946 / 0	0.11 (1)	0.11 (1)
S-F	-9854 / 0	-102.1	-102.1	0.37 (1)	3.66	C-G	0 / 3821	0.47 (1)	0.47 (1)
F-G	-9854 / 0	-102.1	-102.1	0.35 (1)	3.69	M-H	-2407 / 0	0.29 (1)	0.29 (1)
G-H	-6438 / 0	-102.1	-102.1	0.19 (1)	4.55	M-H	0 / 3693	0.46 (1)	0.46 (1)
H-I	-5242 / 0	-102.1	-102.1	0.23 (1)	4.06	L-H	-261 / 0	0.03 (1)	0.03 (1)
I-J	0 / 42	-102.1	-102.1	0.08 (1)	10.00	B-Q	0 / 3430	0.42 (1)	0.42 (1)
R-B	-3817 / 0	0.0	0.0	0.14 (1)	7.23	L-I	0 / 4434	0.55 (1)	0.55 (1)
K-I	-4841 / 0	0.0	0.0	0.18 (1)	6.59				
R-Q	0 / 0	-38.5	-38.5	0.04 (1)	10.00				
Q-P	0 / 3292	-38.5	-38.5	0.52 (1)	10.00				
P-T	0 / 8010	-38.5	-38.5	0.97 (1)	10.00				
T-U	0 / 8010	-38.5	-38.5	0.97 (1)	10.00				
U-O	0 / 8010	-38.5	-38.5	0.97 (1)	10.00				
O-V	0 / 6437	-38.5	-38.5	0.97 (1)	10.00				
V-N	0 / 6437	-38.5	-38.5	0.97 (1)	10.00				
N-M	0 / 6437	-38.5	-38.5	0.97 (1)	10.00				
M-L	0 / 4215	-38.5	-38.5	0.42 (1)	10.00				
L-W	0 / 0	-38.5	-38.5	0.06 (2)	10.00				
W-K	0 / 0	-38.5	-38.5	0.06 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX.	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
JT	12-8-4	123	123	123	123	FRONT	VERT.	TOTAL	---	---
H	23-6-11	251	251	251	251	FRONT	VERT.	DEAD	---	---
L	23-6-11	251	251	251	251	FRONT	VERT.	SNOW	---	---
O	23-6-11	251	251	251	251	FRONT	VERT.	TOTAL	---	---
D	12-8-4	123	123	123	123	FRONT	VERT.	TOTAL	---	---
S	12-8-4	123	123	123	123	FRONT	VERT.	TOTAL	---	---
T	11-3-8	1698	1698	1698	1698	FRONT	VERT.	TOTAL	---	---
U	12-8-4	123	123	123	123	FRONT	VERT.	TOTAL	---	---
V	12-8-4	123	123	123	123	FRONT	VERT.	TOTAL	---	---
W	25-5-12	55	55	55	55	FRONT	VERT.	TOTAL	---	---

PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MT20 618 354 1657 788 1987 1658
PLATE PLACEMENT TOL = 0.250 inches
PLATE ROTATION TOL = 5.0 Deg.
JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.95 (N) (INPUT = 1.00)

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/999 (0.22")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/899 (0.36")

CSI: TC=0.37/1.00 (D-F:1), BC=0.97/1.00 (M-O:1).
WB=0.58/1.00 (C-P:1), SS=0.48/1.00 (C-P:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

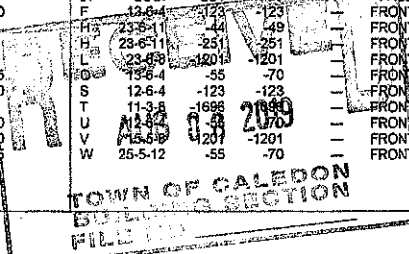
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

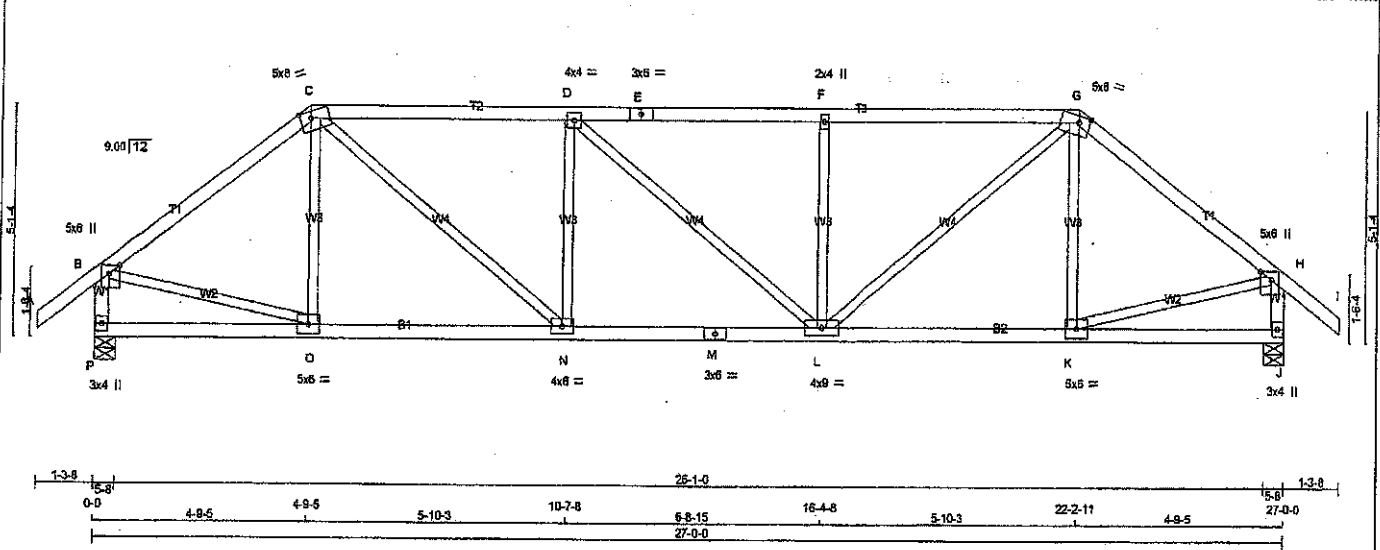
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (C) (INPUT = 0.90)
JSI METAL = 0.95 (N) (INPUT = 1.00)



JOB NAME 402938	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 6.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:40 2018 Page 1 ID:VdmZNlanRCr_yT1tp7OGfzKFiu-qvGDSTsx8wblJ9tSCJb9JQmVjWdM8zNCb3_fuzloj	



TOTAL WEIGHT = 2 X 111 = 221 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - E	2x4	DRY No.2	SPF
E - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - B	2x4	DRY No.2	SPF
B - H	2x4	DRY No.2	SPF
H - M	2x4	DRY No.2	SPF
M - J	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	IN-SX	IN-SX
P 2039 0	2039 0	5-8	5-8
J 2039 0	2039 0	5-8	5-8

UNFACTORED REACTIONS						
1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P 1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0
J 1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0

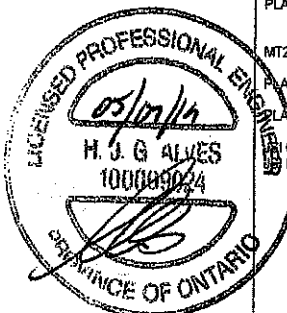
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)
FR-TO		FROM TO	LENGTH
A-B	0 / 42	-102.1 -102.1	0.14 (1) 10.00
B-C	-1985 / 0	-102.1 -102.1	0.52 (1) 4.22
C-D	-2836 / 0	-102.1 -102.1	0.55 (1) 3.59
D-E	-2828 / 0	-102.1 -102.1	0.65 (1) 3.59
E-F	-2828 / 0	-102.1 -102.1	0.65 (1) 3.59
F-G	-2828 / 0	-102.1 -102.1	0.65 (1) 3.59
G-H	-1985 / 0	-102.1 -102.1	0.52 (1) 4.22
H-I	0 / 42	-102.1 -102.1	0.14 (1) 10.00
P-B	-1967 / 0	0.0 0.0	0.21 (1) 6.00
J-H	-1967 / 0	0.0 0.0	0.21 (1) 6.00
P-O	0 / 0	-38.5 -38.5	0.20 (3) 10.00
O-N	0 / 1582	-38.5 -38.5	0.41 (2) 10.00
N-M	0 / 2830	-38.5 -38.5	0.55 (1) 10.00
M-L	0 / 2830	-38.5 -38.5	0.55 (1) 10.00
L-K	0 / 1582	-38.5 -38.5	0.41 (2) 10.00
K-J	0 / 0	-38.5 -38.5	0.20 (3) 10.00



DESIGN CRITERIA
SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $\frac{1}{360}$ (0.90")
CALCULATED VERT. DEFL.(LL) = $\frac{1}{899}$ (0.12")
ALLOWABLE DEFL.(TL) = $\frac{1}{360}$ (0.90")
CALCULATED VERT. DEFL.(TL) = $\frac{1}{899}$ (0.20")

CS1: TC=0.88/1.00 (C-D-1), BC=0.55/1.00 (L-N-1),
WB=0.37/1.00 (H-K-1), SS=0.28/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)	MAX MIN
MT20	618 354 1667 793 1987 1659		

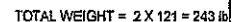
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

CS1 GRIP= 0.88 (G) (INPUT = 0.90)
SS METAL= 0.91 (M) (INPUT = 1.00)

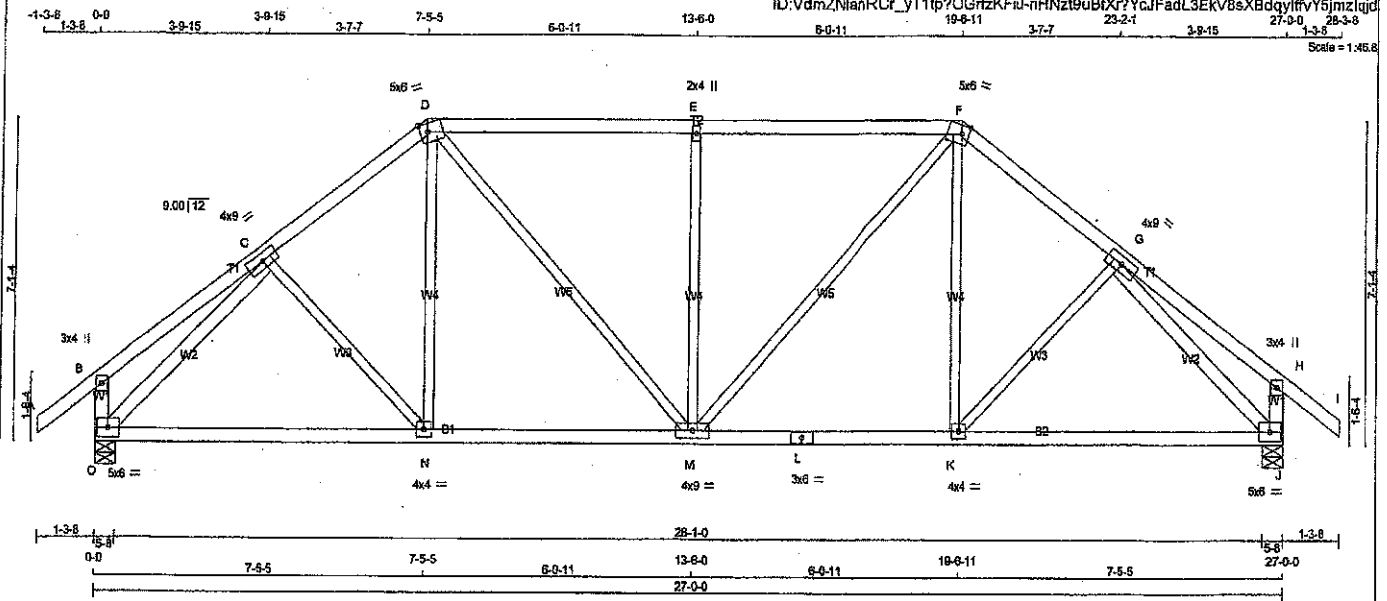
DWG NO. TAM 11910761
STRUCTURAL
COMPONENT ONLY

Scale = 1:48.6



DWG NO. TAM 11910762
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	T4	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:41:42 2019 Page 1					
ID:VdmZNianRCr_yT1tp?OGrtzKFiu-nHNzt9uBfX?YcJFadL3EkV8sXBdgyIffV5jmzlgjd					
Scale = 1:48.8					



TOTAL WEIGHT = 2 X 123 = 245 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4 DRY	No.2	SPF		
D - F	2x4 DRY	No.2	SPF		
F - I	2x4 DRY	No.2	SPF		
O - B	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
O - L	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					
O - C	2x4 DRY	No.2	SPF		
G - J	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	9.0		
D	TTVW-m	MT20	5.0	6.0	Edge 2.00	
E	TMVW-w	MT20	2.0	4.0		
F	TTVW-m	MT20	5.0	6.0	Edge 2.00	
G	TMVW+1	MT20	4.0	9.0		
H	TMV+p	MT20	3.0	4.0		
I	BMVW+1	MT20	5.0	6.0		
K	BMVW+1	MT20	4.0	4.0		
L	BS-t	MT20	3.0	6.0		
M	BMVW+1	MT20	4.0	9.0		
N	BMVW+1	MT20	4.0	4.0		
O	BMVW+1	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
O	2039	0	2039	0
J	2039	0	2039	0

UNFACTORED REACTIONS	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
JT	1515	864 / 0	284 / 0
O	1515	864 / 0	284 / 0
J	1515	864 / 0	284 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE	VERT. LOAD	LC1	MAX	MAX. UNBRACED	FR-TO	WEBS	MEMB.	MAX. FACTORED	FORCE	MAX
			(LBS)		(PLF)	CSI (LC)	LENGTH	FR-TO				(LBS)	CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14	(1)	10.00	C-N	0 / 113	0.03	(3)			
B-C	0 / 25	-102.1	-102.1	0.22	(1)	10.00	N-D	0 / 351	0.08	(2)			
C-D	-1948 / 0	-102.1	-102.1	0.21	(1)	4.85	D-M	0 / 626	0.14	(1)			
D-E	-1955 / 0	-102.1	-102.1	0.54	(1)	4.20	M-E	-766 / 0	0.66	(1)			
E-F	-1955 / 0	-102.1	-102.1	0.54	(1)	4.20	F-F	0 / 626	0.14	(1)			
F-G	-1948 / 0	-102.1	-102.1	0.21	(1)	4.85	K-F	0 / 351	0.08	(2)			
G-H	0 / 25	-102.1	-102.1	0.22	(1)	10.00	K-G	0 / 113	0.03	(3)			
H-I	0 / 42	-102.1	-102.1	0.14	(1)	10.00	O-C	-2234 / 0	0.76	(1)			
O-B	-288 / 0	0.0	0.0	0.03	(1)	7.81	G-J	-2234 / 0	0.76	(1)			
J-H	-288 / 0	0.0	0.0	0.03	(1)	7.81							
O-N	0 / 1524	-38.5	-38.5	0.57	(2)	10.00							
N-M	0 / 1539	-38.5	-38.5	0.58	(2)	10.00							
M-L	0 / 1539	-38.5	-38.5	0.58	(2)	10.00							
L-K	0 / 1539	-38.5	-38.5	0.58	(2)	10.00							
K-J	0 / 1524	-38.5	-38.5	0.57	(2)	10.00							

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = 1 / 999 (0.13")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = 1 / 999 (0.22")

CSI: TC=0.54/1.00 (D-E:1), BC=0.58/1.00 (K-M:2), WB=0.78/1.00 (G-J:1), SS=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1587 788 1987 1856

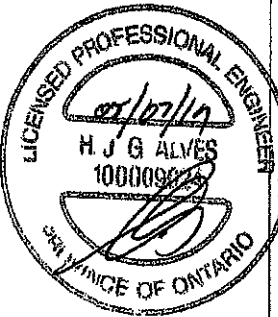
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.89 (C) (INPUT = 0.80)
SI METAL = 0.58 (L) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

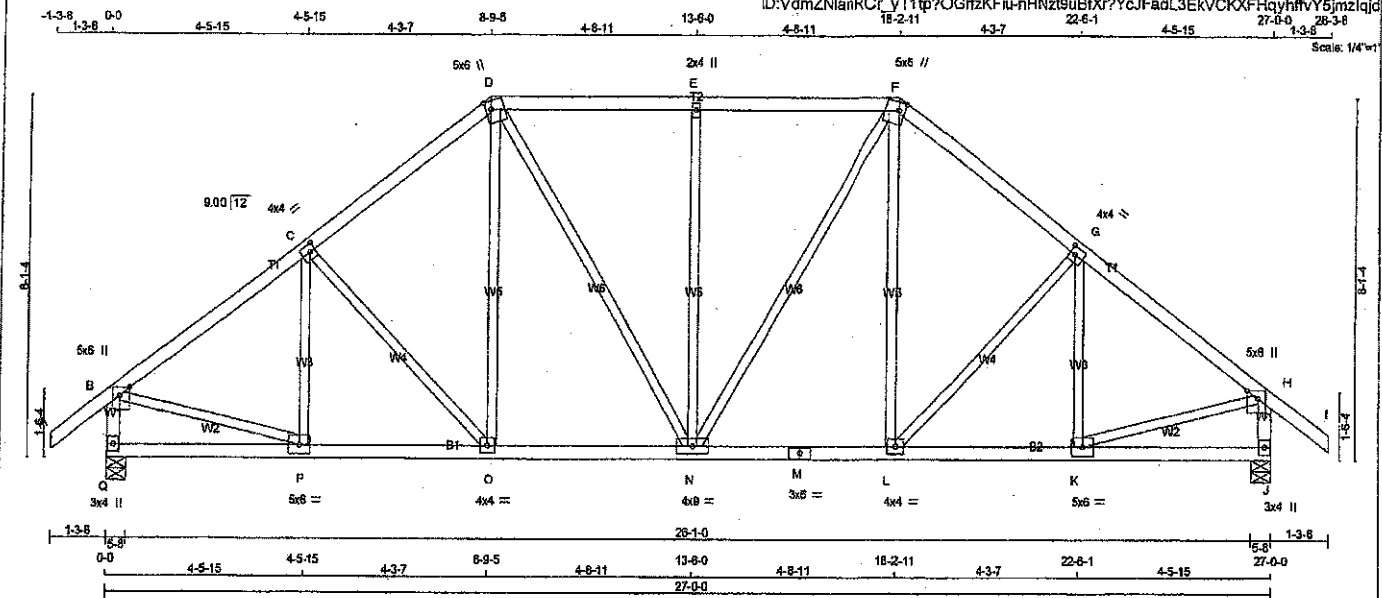
RECEIVED
AUG 08 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



DRWG NO. TAM T4/0763
STRUCTURAL
CONSULTANT ONLY

JOB NAME 402938	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC GREEN PARK HOMES	DRWG NO.
Tamerack Roof Truss, Burlington					

Version 8.230 S Nov 17 2016 Mitek Industries, Inc. Tue May 7 17:41:42 2019 Page 1
ID:VdmZNIanRCr_yT1tp?OGrfzKFiu-nHNztSuBfX?YcJFadL3EK/CKXFHoyhffvY5jm2lqld



TOTAL WEIGHT = 2 X 130 = 259 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMVW+p	MT20	5.0	6.0	Edge 3.00
C TMWW-1	MT20	4.0	4.0	2.00 1.50
D TTWW+m	MT20	5.0	6.0	2.25 1.50
E TMVW+w	MT20	2.0	4.0	
F TTWW+m	MT20	5.0	6.0	2.25 1.50
G TMWW-1	MT20	4.0	4.0	2.00 1.50
H TMVW+p	MT20	5.0	6.0	Edge 3.00
J BMV1+p	MT20	3.0	4.0	
K BMWW-1	MT20	5.0	6.0	
L BMWW-1	MT20	4.0	4.0	
M BS-1	MT20	3.0	6.0	
N BMWW-1	MT20	4.0	4.0	
O BMWW-1	MT20	4.0	4.0	
P BMWW-1	MT20	5.0	6.0	
Q BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	UP	IN-SX
Q 2039 0	2039 0	0	5-8
J 2039 0	2039 0	0	5-8

UNFACTORED REACTIONS							
1ST LCASE	MAX. MIN.	COMPONENT REACTIONS					
JT COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
Q 1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	
J 1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	

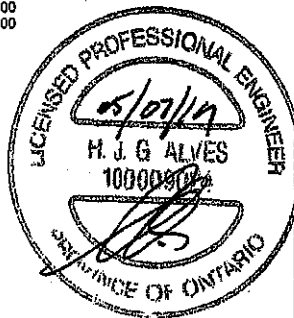
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. UNBRACED LENGTH (FT)
FR-TO			
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00
B-C	-1987 / 0	-102.1 -102.1 0.30 (1)	4.51
C-D	-1845 / 0	-102.1 -102.1 0.29 (1)	4.85
D-E	-1683 / 0	-102.1 -102.1 0.32 (1)	4.79
E-F	-1893 / 0	-102.1 -102.1 0.32 (1)	4.79
F-G	-1845 / 0	-102.1 -102.1 0.29 (1)	4.85
G-H	-1987 / 0	-102.1 -102.1 0.30 (1)	4.51
H-I	0 / 42	-102.1 -102.1 0.14 (1)	10.00
Q-B	-1987 / 0	0.0 0.0 0.21 (1)	6.01
J-H	-1987 / 0	0.0 0.0 0.21 (1)	6.01
Q-P	0 / 0	-38.5 -38.5 0.13 (3)	10.00
P-O	0 / 1622	-38.5 -38.5 0.34 (1)	10.00
O-N	0 / 1452	-38.5 -38.5 0.32 (1)	10.00
N-M	0 / 1452	-38.5 -38.5 0.32 (1)	10.00
M-L	0 / 1452	-38.5 -38.5 0.32 (1)	10.00
L-K	0 / 1622	-38.5 -38.5 0.34 (1)	10.00
K-J	0 / 0	-38.5 -38.5 0.13 (3)	10.00



DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 29.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = 1 / 898 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = 1 / 898 (0.11")

CSI: TC=0.32/1.00 (D-E-1), BC=0.34/1.00 (K-L-1), WB=0.74/1.00 (E-N-1), SSI=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP (DRY) SHEAR	SECTION (PSI)	(PL)	(PL)
MAX MIN MAX MIN MAX MIN			
MT20	618	354	1667 789 1697 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (H) (INPUT = 0.90)
JSI METAL = 0.61 (M) (INPUT = 1.00)

DRWG NO. TAM 1910764
STRUCTURAL
CONSULTANT ONLY

Tamarack Roof Truss, Burlington



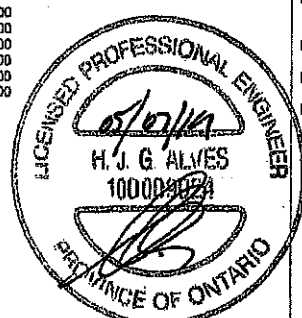
TOTAL WEIGHT = $6 \times 130 = 778$ lb

DRY: SEASONED LUMBER.

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

P-O	0/0	-38.5	-38.5	0.17 (3)	10.00
O-N	0/1842	-38.5	-38.5	0.43 (2)	10.00
N-M	0/1458	-38.5	-38.5	0.40 (2)	10.00
M-L	0/1458	-38.5	-38.5	0.40 (2)	10.00
L-K	0/1642	-38.5	-38.5	0.43 (2)	10.00
K-J	0/0	-38.5	-38.5	0.17 (3)	10.00

```
SI GRIP= 0.90 (B) (INPUT = 0.80 )
SI METAL= 0.48 (M) (INPUT = 1.00 )
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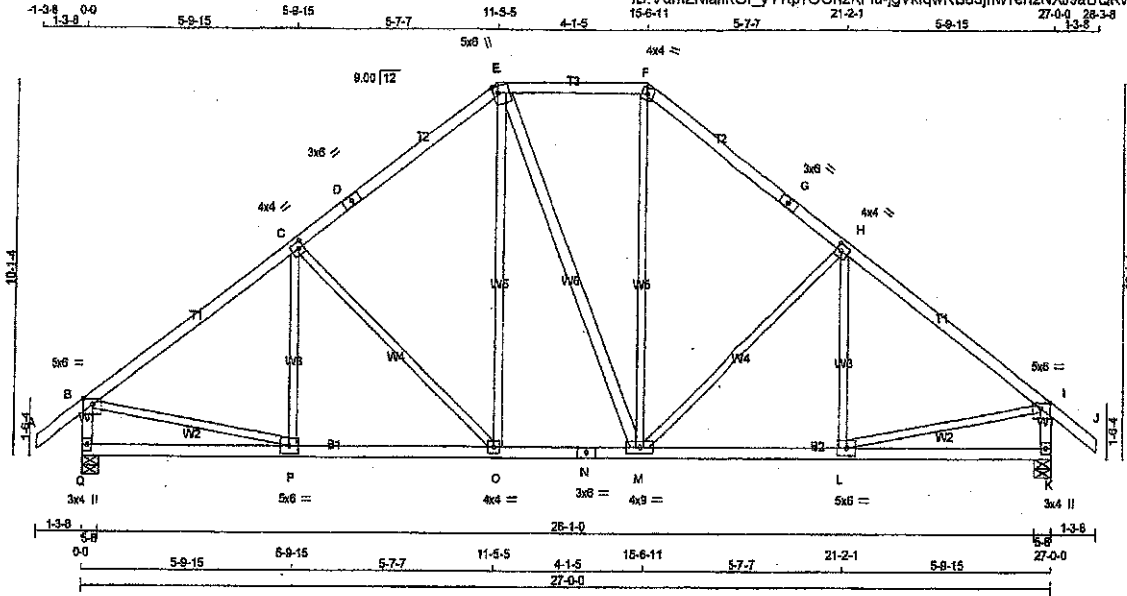


DMG NO. TAM 71910765
STRUCTURAL
CIVIL ENGINEER ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY.	JOB DESC.	DRWG NO.
402938	T7	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID: VdmZNlanRCr_yT1tp?OGrtzKFiu-jgVklqwRB85jnwTeh2NXJ9aUQKvltqy7D1Bofzljib



TOTAL WEIGHT = 2 X 132 = 264 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT	E - M	2x4	DRY
DRY: SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS					
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	
JT	VERT	DOWN	HORZ	UPLIFT	IN-SX
Q	2039	0	2039	0	5-8
K	2039	0	2039	0	5-8

UNFACTORED REACTIONS							
1ST LCASE	MAX	MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1515	864 / 0	264 / 0	0 / 0	0 / 0	368 / 0	0 / 0
K	1515	864 / 0	264 / 0	0 / 0	0 / 0	368 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	P-C	-83 / 176	0.04 (1)
B-C	-2028 / 0	-102.1 -102.1	0.54 (1)	4.20	C-O	-524 / 0	0.70 (1)
C-D	-1645 / 0	-102.1 -102.1	0.49 (1)	4.62	O-E	0 / 536	0.12 (1)
D-E	-1645 / 0	-102.1 -102.1	0.49 (1)	4.62	E-M	0 / 3	0.00 (1)
E-F	-1285 / 0	-102.1 -102.1	0.24 (1)	5.43	M-F	0 / 540	0.12 (1)
F-G	-1647 / 0	-102.1 -102.1	0.49 (1)	4.61	M-H	-522 / 0	0.69 (1)
G-H	-1647 / 0	-102.1 -102.1	0.49 (1)	4.61	L-H	-85 / 173	0.05 (1)
H-I	-2027 / 0	-102.1 -102.1	0.54 (1)	4.20	B-P	0 / 1690	0.38 (1)
I-J	0 / 42	-102.1 -102.1	0.14 (1)	10.00	L-I	0 / 1699	0.38 (1)
Q-B	-1949 / 0	0.0	0.0	6.03			
K-I	-1949 / 0	0.0	0.0	6.03			
Q-P	0 / 0	-38.5	-38.5	0.25 (3)	10.00		
P-O	0 / 1656	-38.5	-38.5	0.46 (2)	10.00		
O-N	0 / 1285	-38.5	-38.5	0.31 (1)	10.00		
N-M	0 / 1285	-38.5	-38.5	0.31 (1)	10.00		
M-L	0 / 1655	-38.5	-38.5	0.46 (2)	10.00		
L-K	0 / 0	-38.5	-38.5	0.25 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 26.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.12")

CS1: TO=0.54/1.00 (B-C-1), BC=0.46/1.00 (O-P-2),
WB=0.70/1.00 (C-O-1), SS=0.23/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

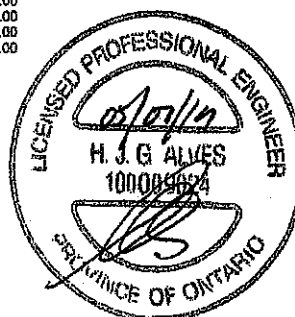
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1607 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

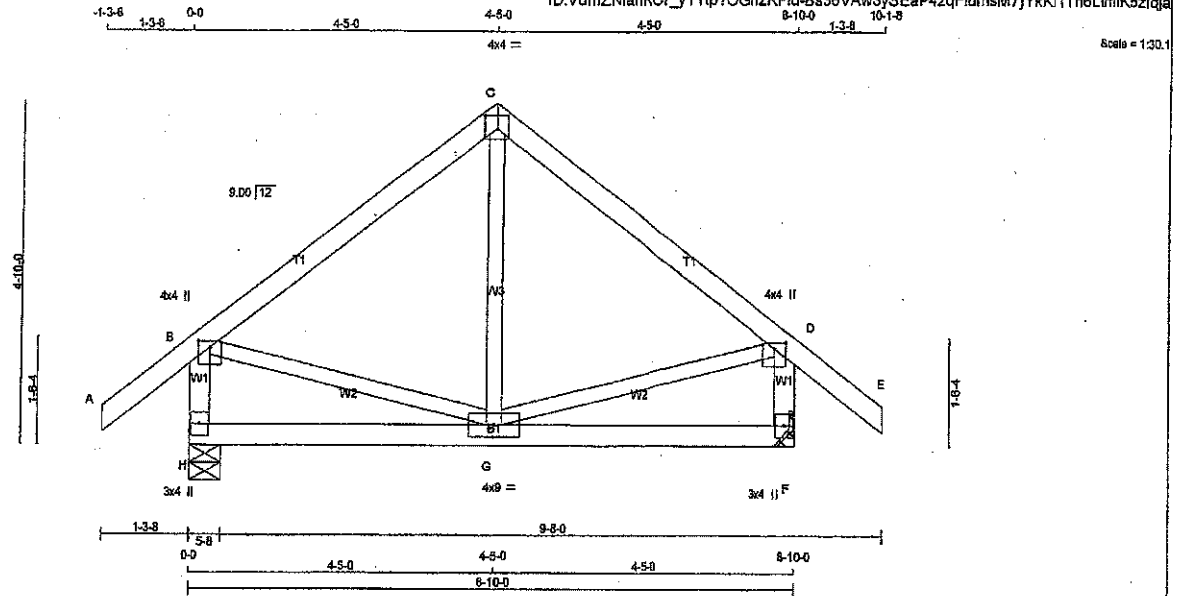
JSI GRIP= 0.89 (B) (INPUT= 0.93)
JSI METAL= 0.41 (P) (INPUT= 1.00)



DRWG NO. TAM T1910766
STRUCTURAL
CALCULATION ONLY

JOB NAME 402938	TRUSS NAME T9	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tramrack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Tue May 7 17:41:45 2019 Page 1
ID:VdmZNlanRCr_YT1tp?OGrfzKFiu-Bs36VAw3ySEaP42qFlumsM7jYkK1Tn6LtnIK5zloja



TOTAL WEIGHT = 2 X 39 = 79 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x6	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVV+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVV+p	MT20	4.0	4.0	1.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMVWW+T	MT20	4.0	9.0		
H	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UP	IN-SX
H	762	0	0	5-8
F	762	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0	0 / 0
F	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO			
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 180	0.04 (3)
B-C	-405 / 0	-102.1	-102.1	0.26 (1)	6.25	B-G	0 / 335	0.08 (1)
C-D	-405 / 0	-102.1	-102.1	0.26 (1)	6.25	G-D	0 / 335	0.08 (1)
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
H-B	-697 / 0	0.0	0.0	0.07 (1)	7.81			
F-D	-697 / 0	0.0	0.0	0.07 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	0.17 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-06, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/899 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/899 (0.02")

CSI: TC=0.28/1.00 (C-D:1) , BC=0.17/1.00 (F-G:3) ,
WB=0.08/1.00 (D-G:1) , SSI=0.14/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

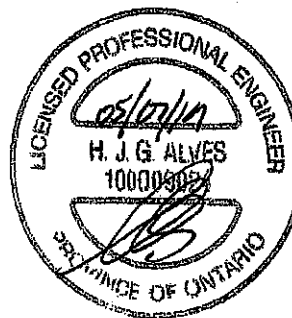
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(ORY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1687 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

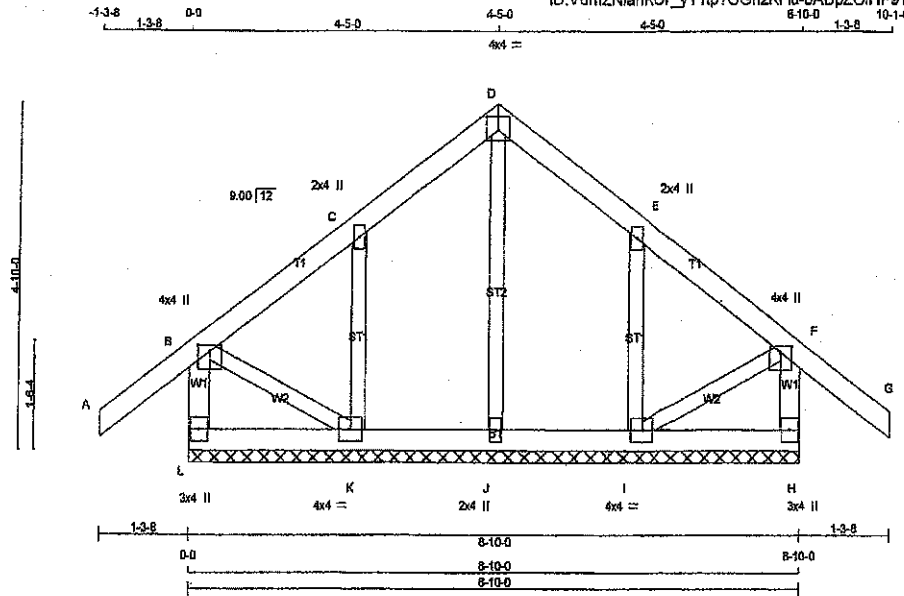
JSI GRIP= 0.64 (D) (INPUT = 0.90)
JSI METAL= 0.16 (D) (INPUT = 1.00)



DRWG NO. TAM 17910767
STRUCTURAL
CONSULTANT ONLY

JOB NAME 402938	TRUSS NAME G9	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 6.230 8 Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:31 2019 Page 1
ID:VdmZNIanRCr_yT1tp?OGrtzkFiu-bADpZOIHf9Tjwz8QpeTIPYmW6aQl8f27IN?qvzlgjo



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 41 lb

LUMBER			
N.L.G.A. RULES	CHORDS	SIZE	LUMBER
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2-0-0 OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWV+p	MT20	4.0	4.0 1.00 2.00
C	TMWV+w	MT20	2.0	4.0
D	TTW+p	MT20	4.0	4.0 2.25 2.00
E	TMWV+w	MT20	2.0	4.0
F	TMWV+p	MT20	4.0	4.0 1.00 2.00
H	BMWV+p	MT20	3.0	4.0
I	BMWV+l	MT20	4.0	4.0
J	BMWV+w	MT20	2.0	4.0
K	BMWV+l	MT20	4.0	4.0
L	BMV+p	MT20	3.0	4.0

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		FR-TO			
L-B	-259 / 0	0.0 0.0 0.03 (1)	7.81	J-D	-137 / 0	0.05 (1)	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	K-C	-275 / 0	0.05 (1)	
B-C	-10 / 0	-102.1 -102.1 0.08 (1)	10.00	I-E	-275 / 0	0.05 (1)	
C-D	-33 / 0	-102.1 -102.1 0.08 (1)	6.25	B-K	0 / 23	0.01 (1)	
D-E	-33 / 0	-102.1 -102.1 0.08 (1)	6.25	I-F	0 / 23	0.01 (1)	
E-F	-10 / 0	-102.1 -102.1 0.08 (1)	10.00				
F-G	0 / 42	-102.1 -102.1 0.14 (1)	10.00				
H-F	-259 / 0	0.0 0.0 0.03 (1)	7.81				
L-K	0 / 0	-38.5 -38.5 0.04 (3)	10.00				
K-J	0 / 12	-38.5 -38.5 0.04 (3)	10.00				
J-I	0 / 12	-38.5 -38.5 0.04 (3)	10.00				
I-H	0 / 0	-38.5 -38.5 0.04 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1), BC=0.04/1.00 (J-K:3), WB=0.05/1.00 (C-K:1), SSI=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

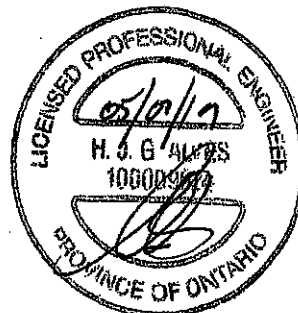
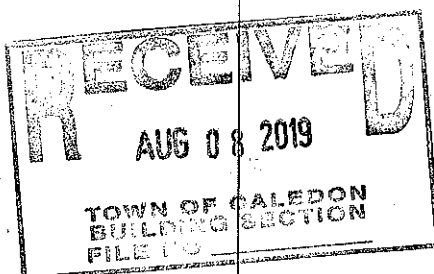
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1657 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

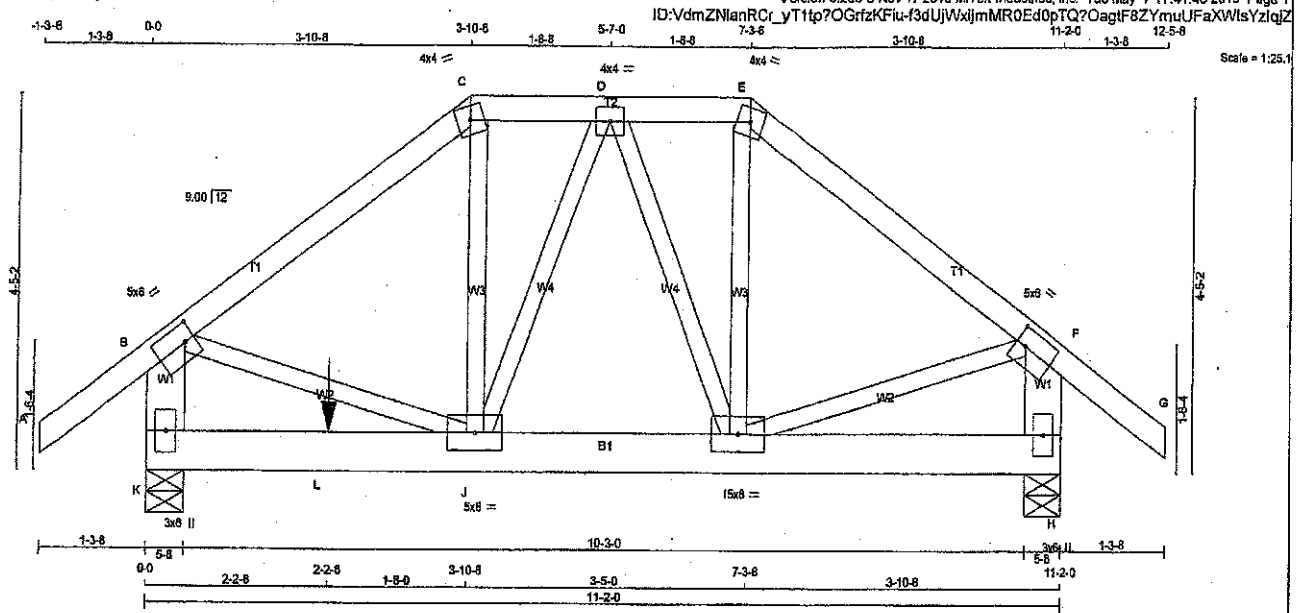
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.20 (F) (INPUT = 0.90)
JSI METAL= 0.15 (E) (INPUT = 1.00)

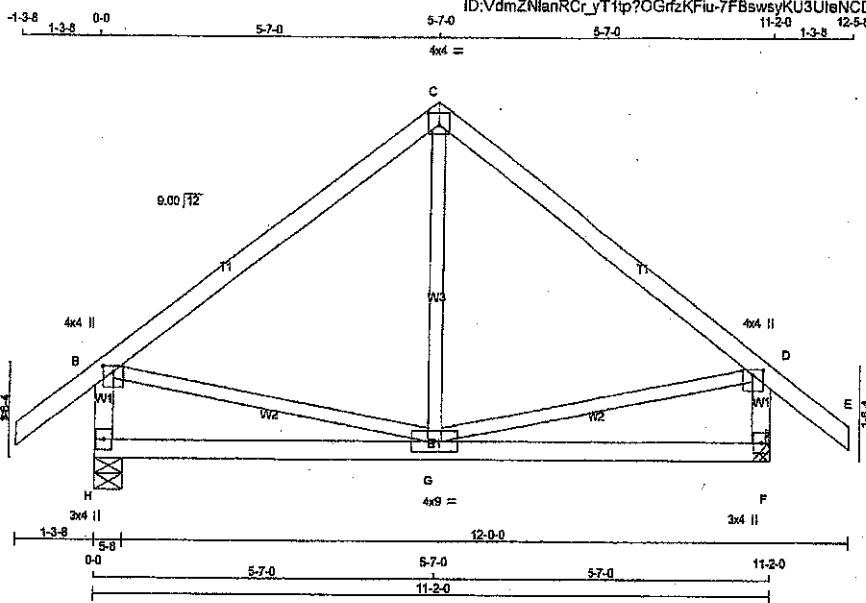


DWG NO. TAM 779/0752
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME T10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 9.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:41:46 2019 Page 1	



JOB NAME 402938	TRUSS NAME T11	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:47 2019 Page 1	



TOTAL WEIGHT = 3 X 48 = 144 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	DESCR.
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
H - B	2x4	DRY	No.2
F - D	2x4	DRY	No.2
H - F	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
C	TTW+p	MT20	4.0	4.0 2.25 2.00
D	TMVW+p	MT20	4.0	4.0 1.00 2.00
F	BMV1+p	MT20	3.0	4.0
G	BMVW+p	MT20	4.0	9.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
H	926	0	926	0	0
F	926	0	926	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
H	683	404 / 0	117 / 0	0 / 0	0 / 0	162 / 0
F	683	404 / 0	117 / 0	0 / 0	0 / 0	162 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD (PL)	CS (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED CS (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	G-C	0 / 246	0.06 (3)	
B-C	-543 / 0	-102.1 -102.1	0.41 (1)	B-G	0 / 444	0.10 (1)	
C-D	-543 / 0	-102.1 -102.1	0.41 (1)	G-D	0 / 444	0.10 (1)	
D-E	0 / 42	-102.1 -102.1	0.14 (1)				
H-B	-844 / 0	0.0 0.0	0.08 (1)				
F-D	-844 / 0	0.0 0.0	0.08 (1)				
H-G	0 / 0	-38.5 -38.5	0.27 (3)				
G-F	0 / 0	-38.5 -38.5	0.27 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.37")
CALCULATED VERT. DEFL.(LL) = L/999 (0.03")
ALLOWABLE DEFL.(TL) = L/360 (0.37")
CALCULATED VERT. DEFL.(TL) = L/999 (0.05")

CSI: TO=0.41/1.00 (B-C:1), BC=0.27/1.00 (F-G:3),
WB=0.10/1.00 (D-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

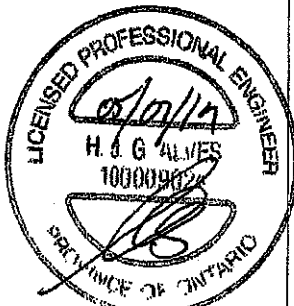
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 768 1687 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

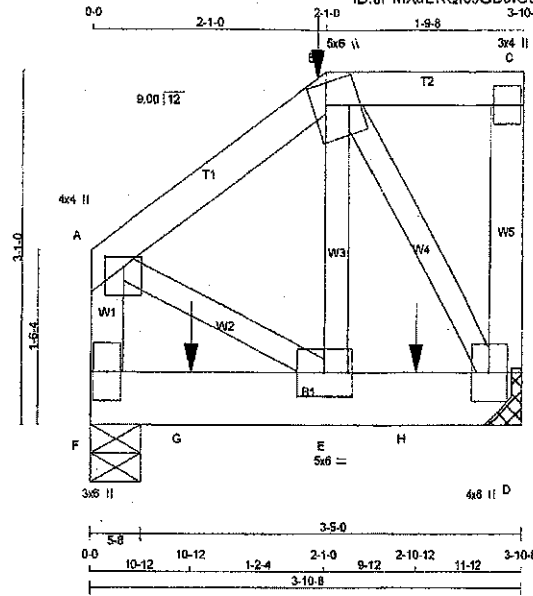


DRWS NO. TAM 77910769
STRUCTURAL
CALCULATIONS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402979	T12	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:JPMxderQr69GD8IO953aPwyJEIZ-13nRr_vE8TTLDPFIOZaC?_RIS32mJbJH_7XQZlXl



TOTAL WEIGHT = 2 X 22 = 43 lb

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY	No.2 SPF
A - B	2x4	DRY	No.2 SPF
B - C	2x4	DRY	No.2 SPF
D - C	2x4	DRY	No.2 SPF
F - D	2x6	DRY	No.2 SPF
ALL WEBS	2x3	DRY	No.2 SPF
DRY: SEASONED LUMBER.			

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS	(0.122"x3")	SPRIL NAILS	
F-A	1	12	TOP
A-B	1	12	SIDE(61.0)
B-C	1	12	SIDE(61.0)
C-D	1	12	TOP
BOTTOM CHORDS	(0.122"x3")	SPRIL NAILS	
F-D	2	12	SIDE(0.0)
WEBS	(0.122"x3")	SPRIL NAILS	
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTVW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW+m	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	803	0	903	0
D	803	0	883	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	667	394/0	115/0	0/0	0/0	158/0	0/0
D	651	386/0	111/0	0/0	0/0	154/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO		LENGTH	FR-TO			
F-A	-583/0	0.0	0.0	0.03 (1)	7.81	A-E	0/405	0.05 (1)
A-B	-483/0	-102.1	-102.1	0.04 (1)	6.25	E-B	0/579	0.07 (1)
B-C	0/0	-102.1	-102.1	0.03 (1)	10.00	B-D	-892/0	0.07 (1)
D-C	-91/0	0.0	0.0	0.01 (1)	7.81			
F-G	0/0	-38.5	-38.5	0.11 (1)	10.00			
G-E	0/0	-38.5	-38.5	0.11 (1)	10.00			
E-H	0/383	-38.5	-38.5	0.10 (1)	10.00			
H-D	0/383	-38.5	-38.5	0.10 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-1-0	-73	-73		BACK	VERT	TOTAL		
G	10-12	-585	-585		FRONT	VERT	TOTAL		
H	2-10-12	-583	-583		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(66% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/899 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.11/1.00 (E-F:1), WS=0.07/1.00 (B-E:1), SS=0.14/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

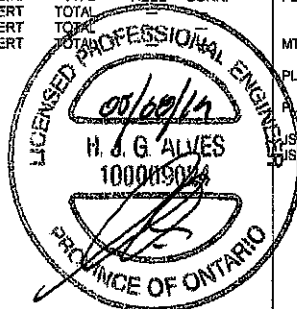
NAIL VALUES

PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PLI)		(PLI)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

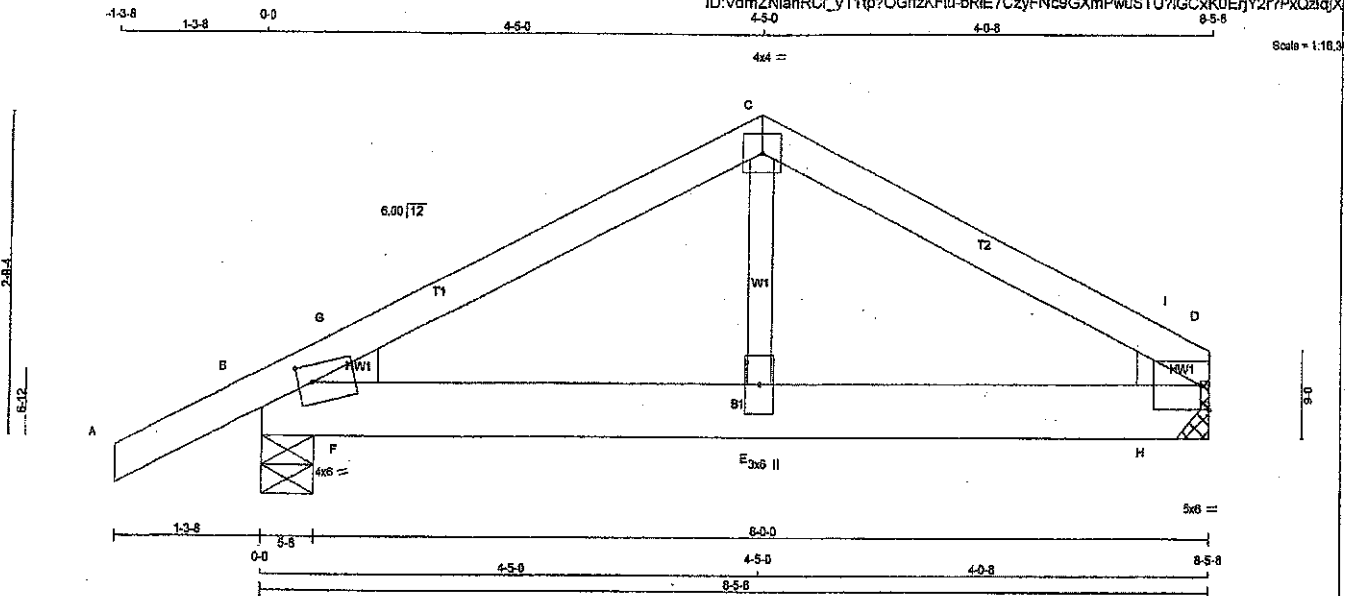
JS GRIP= 0.26 (A) (INPUT = 0.90)
JS METAL= 0.09 (D) (INPUT = 1.00)



DRWG NO. TAM 171104979
STRUCTURAL
EXAMINATION ONLY

JOB NAME 402938	TRUSS NAME T13	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:48 2018 Page 1
ID:VdmZNIanRC_yT1tp?OGrfzKFlu-bRIE7CzyFncSGXmPwuSTU7IGCxxK0EnY2r?PxQzlaqX



TOTAL WEIGHT = 31 lb

LUMBER	N. L. G. A. RULES	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY; SEASONED LUMBER.				

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMBH1-m	MT20	4.0	6.0	1.75
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-l	MT20	5.0	6.0	Edge
E	BMW-w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	HEEL
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
B	733	0	733	0	0
D	595	0	595	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3'-8".

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	540	324 / 0	89 / 0	0 / 0	0 / 0	126 / 0	0 / 0
D	444	246 / 0	89 / 0	0 / 0	0 / 0	110 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC2)	MAX. FACTORED VERT. LOAD (LC3)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO						FR-TO		
A-B	0 / 24	-102.1	-102.1	0.13 (1)	10.00	E-C	0 / 279	0.00 (2)
B-G	-835 / 0	-102.1	-102.1	0.10 (1)	6.25	F-G	-244 / 24	0.00 (1)
G-C	-650 / 0	-102.1	-102.1	0.17 (1)	6.25	H-I	-73 / 128	0.00 (1)
C-I	-655 / 0	-102.1	-102.1	0.14 (1)	6.25			
I-D	-713 / 0	-102.1	-102.1	0.13 (1)	6.25			
B-F	0 / 572	-38.5	-38.5	0.19 (1)	10.00			
F-E	0 / 572	-38.5	-38.5	0.19 (1)	10.00			
E-H	0 / 572	-38.5	-38.5	0.14 (1)	10.00			
H-D	0 / 572	-38.5	-38.5	0.10 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = L/999 (0.02")

CSI: TC=0.17/1.00 (C-G:1), BC=0.19/1.00 (E-F:1),
WB=0.06/1.00 (C-E:2), SG=0.16/1.00 (B-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

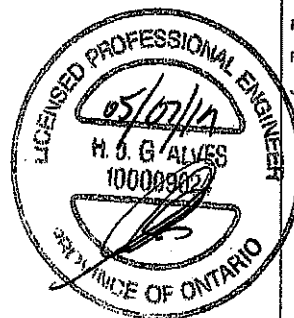
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

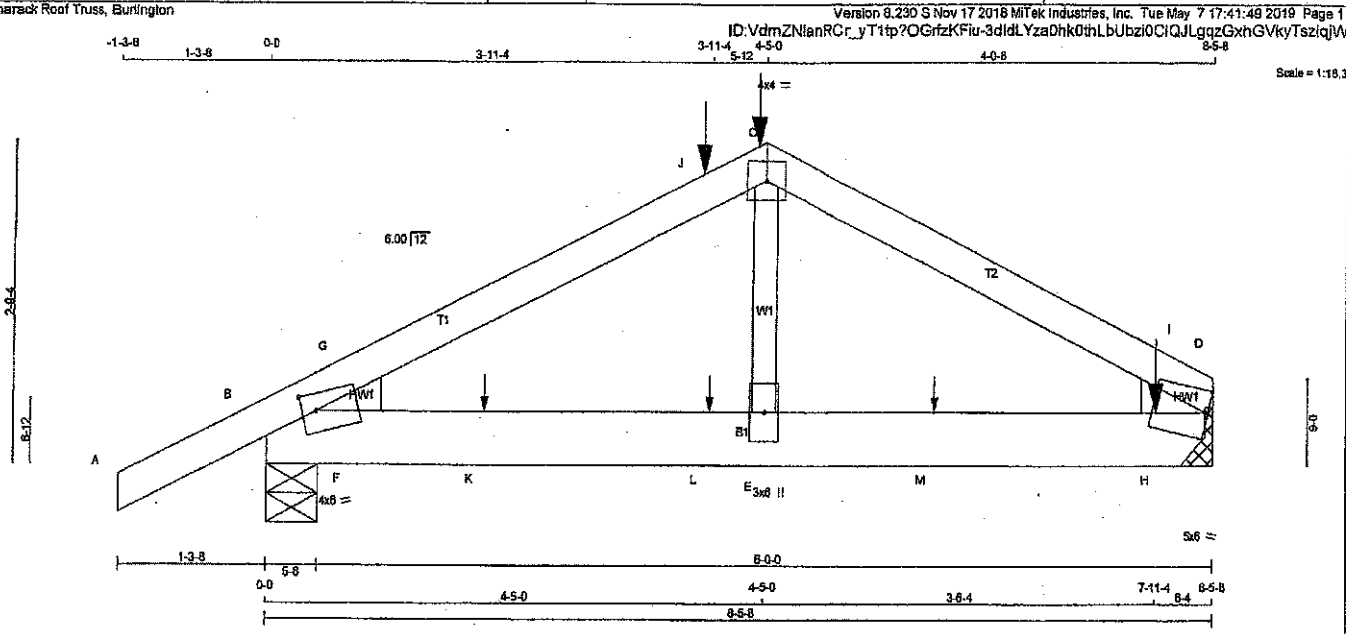
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (D) (INPUT = 0.80)
JSI METAL= 0.18 (B) (INPUT = 1.00)



DRWG NO. T13 19/07/14
STRUCTURAL
CLIENT ONLY

JOB NAME 402938	TRUSS NAME T13Z	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue May 7 17:41:48 2019 Page 1	



TOTAL WEIGHT = 31 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
B - D	2x6	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMBH1-m	MT20	4.0	6.0	1.75 1.50
C	TTW-p	MT20	4.0	4.0	
D	TMBH1-m	MT20	5.0	6.0	2.25 Edge
E	BMW-w	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS									
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG	HEEL WEDGE				
B	808	808	0	5-8	5-8	2x4 L			
D	680	680	0	MECHANICAL		2x4 R			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	591	357 / 0	89 / 0	0 / 0	0 / 0	135 / 0	0 / 0
D	504	291 / 0	91 / 0	0 / 0	0 / 0	121 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.16 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. MAX. (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. (LBS)	CS1 (LC)
A-B	0 / 24	-102.1 -102.1	0.15 (1)	10.00	E-C	0 / 261	0.06 (2)
B-G	-791 / 0	-102.1 -102.1	0.15 (1)	6.25	F-G	-218 / 28	0.00 (1)
G-J	-792 / 0	-102.1 -102.1	0.20 (1)	6.25	H-I	0 / 141	0.00 (1)
J-K	-792 / 0	-102.1 -102.1	0.20 (1)	6.25			
K-L	-797 / 0	-102.1 -102.1	0.21 (1)	6.25			
L-M	-806 / 0	-102.1 -102.1	0.20 (1)	6.18			
B-F	0 / 899	-38.5 -38.5	0.22 (1)	10.00			
F-K	0 / 899	-38.5 -38.5	0.22 (1)	10.00			
K-L	0 / 899	-38.5 -38.5	0.22 (1)	10.00			
L-E	0 / 899	-38.5 -38.5	0.22 (1)	10.00			
E-M	0 / 899	-38.5 -38.5	0.15 (1)	10.00			
M-H	0 / 899	-38.5 -38.5	0.15 (1)	10.00			
H-D	0 / 899	-38.5 -38.5	0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.	
C	4-5-0	-23	-25	FRONT	VERT	DEAD			
C	4-5-0	-129	-129	FRONT	VERT	SNOW			
H	7-11-4	-5	-7	BACK	VERT	TOTAL			
J	3-11-4	-3	-3	BACK	VERT	TOTAL			
K	1-11-4	1	1	BACK	VERT	TOTAL			
L	3-11-4	1	1	BACK	VERT	TOTAL			
M	5-11-4	1	1	BACK	VERT	TOTAL			

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

*** NON STANDARD GIRDER ***
ADDT. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2010, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.28")
CALCULATED VERT. DEFL.(LL) = 1/ 899 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.28")
CALCULATED VERT. DEFL.(TL) = 1/ 999 (0.02")

CS1: TC=0.21/1.00 (C-E-1), BC=0.22/1.00 (B-F-1),
WB=0.08/1.00 (C-E-2), SS=0.17/1.00 (C-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

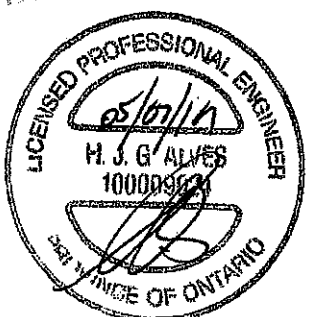
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 818 354 1887 788 1887 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.85 (D) (INPUT = 0.90)
JSI METAL= 0.21 (B) (INPUT = 1.00)

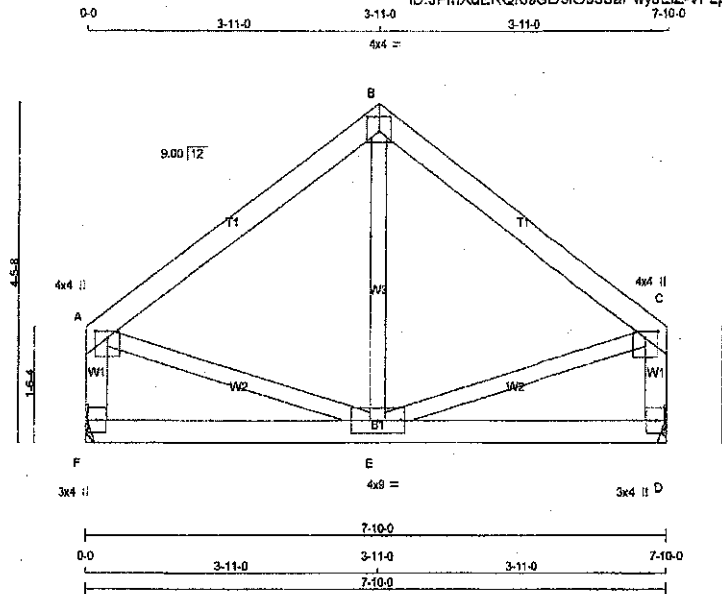
ONE NAIL TAM 7/19/07
STRUCTURAL
COMPUTATION ONLY



THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402979	T15	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 15:08:01 2019 Page 1
ID: JPMXdeRQr68GD6IO953aPwyJEIZ-VFLp2KvsbnbCHN_RJ5UpIDWZrNwVllkoxkZ4szlXt



Scale = 1/28.0

TOTAL WEIGHT = 32 lb (M)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4 DRY	No.2	SPF		
B - C	2x4 DRY	No.2	SPF		
F - A	2x4 DRY	No.2	SPF		
D - C	2x4 DRY	No.2	SPF		
F - D	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMV1+p	MT20	3.0	4.0		
E	BMVWW-t	MT20	4.0	9.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	HORIZ	UPLIFT
F	551	0	551	0
D	551	0	551	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
F	412	228 / 0	82 / 0	0 / 0	102 / 0	0 / 0
D	412	228 / 0	82 / 0	0 / 0	102 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO						FR-TO			
A-B	-347 / 0	-102.1	-102.1	0.20 (1)	6.25	E-B	0 / 152	0.04 (3)	
B-C	-347 / 0	-102.1	-102.1	0.20 (1)	6.25	A-E	0 / 290	0.07 (1)	
F-A	-493 / 0	0.0	0.0	0.05 (1)	7.81	E-C	0 / 290	0.07 (1)	
D-C	-493 / 0	0.0	0.0	0.05 (1)	7.81				
F-E	0 / 0	-38.5	-38.5	0.13 (3)	10.00				
E-D	0 / 0	-38.5	-38.5	0.13 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(58 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.25")
CALCULATED VERT. DEFL. (LL) = L/999 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.25")
CALCULATED VERT. DEFL. (TL) = L/999 (0.01")

CSI: TC=0.20/1.00 (A-B-1), BC=0.13/1.00 (E-F-3), WB=0.07/1.00 (A-E-1), SS=0.12/1.00 (A-B-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

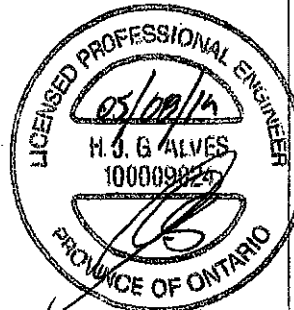
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 785 1987 1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP=0.47 (C) (INPUT = 0.90)
SI METAL=0.12 (C) (INPUT = 1.00)



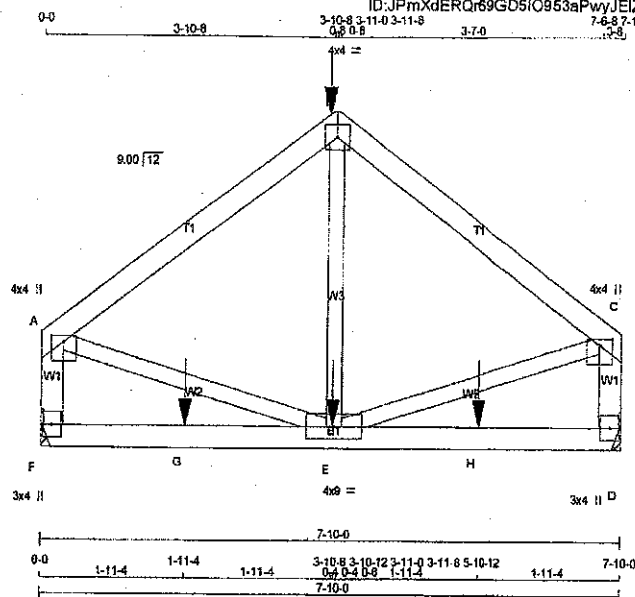
DRWG NO. TAM 07/10/980
STRUCTURAL
CALCULATIONS ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DRWG NO.
402979	T15CP	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: JPMxDERQr69GD5IC953aPwyJEIZ-zSuCGgwUf5j3vXZdp72IQ3j1FjGEBnuAbT6clzIXth



Scale = 1/28.0

LUMBER

N L G A RULES

CHORDS	SIZE	DRY	No.2
A - B	2x4	DRY	No.2
B - C	2x4	DRY	No.2
F - A	2x4	DRY	No.2
D - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2

ALL WEBS 2x3 DRY, SEASONED LUMBER.

LUMBER

DESCR.

SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
F	831	0	831	0
D	831	0	831	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. D. MINIMUM BEARING LENGTH AT JOINT F = 1-8, JOINT D = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX / MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE			
F	617	350 / 0	110 / 0	0 / 0	157 / 0	0 / 0
D	617	350 / 0	110 / 0	0 / 0	157 / 0	0 / 0

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
MEMB.					MEMB.		
FR-TO					FR-TO		
A-B	-668 / 0	-102.1	-102.1	0.31 (1)	E-B	-61 / 198	0.05 (3)
B-C	-666 / 0	-102.1	-102.1	0.31 (1)	A-E	0 / 557	0.14 (1)
F-A	-761 / 0	0.0	0.0	0.09 (1)	E-C	0 / 557	0.14 (1)
D-C	-761 / 0	0.0	0.0	0.09 (1)			
F-G	0 / 0	-38.5	-38.5	0.19 (3)			
G-E	0 / 0	-38.5	-38.5	0.19 (3)			
E-H	0 / 0	-38.5	-38.5	0.19 (3)			
H-D	0 / 0	-38.5	-38.5	0.19 (3)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	3-11-0	-459	-459	—	BACK	VERT	TOTAL	—	—
E	3-10-12	-36	-46	—	BACK	VERT	TOTAL	—	—
G	1-11-4	-33	-42	—	BACK	VERT	TOTAL	—	—
H	5-10-12	-33	-42	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 26.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.25")
CALCULATED VERT. DEFL. (LL) = $L/969$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.25")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CSI: TC=0.31/1.00 (A-B:1), BC=0.19/1.00 (E-F:3), WB=0.14/1.00 (C-E:1), SSI=0.14/1.00 (A-B:1)

DDL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

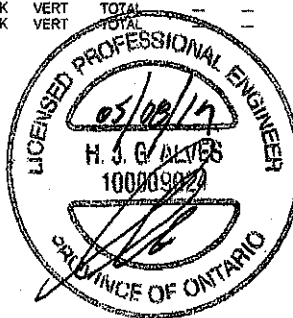
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1697 788

PLATE PLACEMENT TOL = 0.250 inches

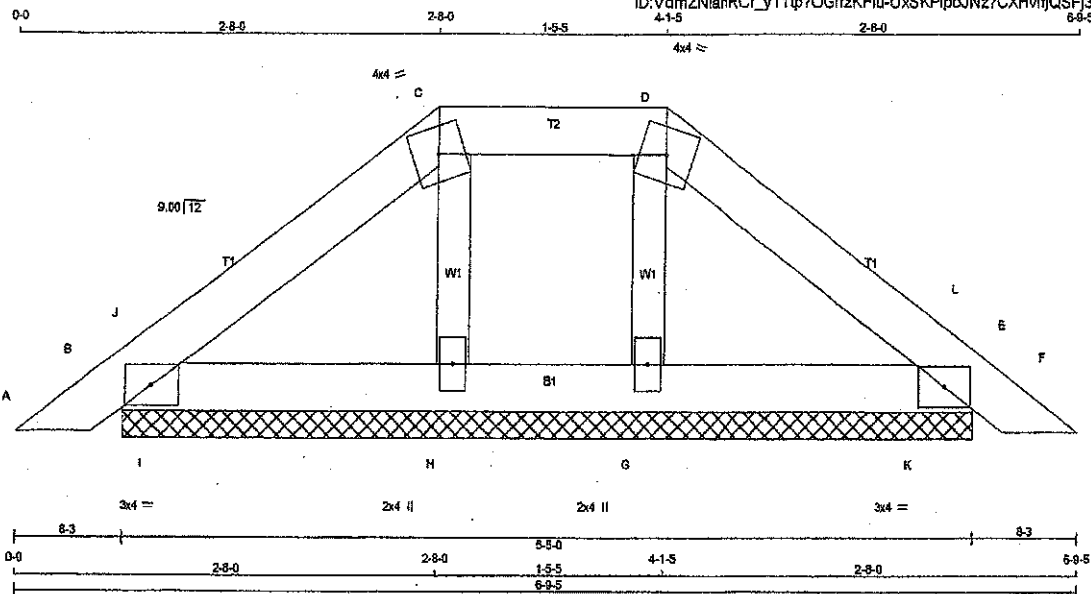
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.78 (A) (INPUT = 0.90)
JSI METAL = 0.21 (A) (INPUT = 1.00)



DRWG NO. TAM 17910981
STRUCTURAL
ALTERNATIVE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402938	PB1	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:35 2019 Page 1			
		ID: VdmZNlanRCr_yT1lp?OGfzKFIu-UxSKPpoJNz?CXHvtfjQSFj3fKxChyCd2JLD_gzljK			



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0		
C	TTW-m	MT20	4.0	4.0		
D	TTW-m	MT20	4.0	4.0		
E	TMB14	MT20	3.0	4.0		
G	BMW14w	MT20	2.0	4.0		
H	BMW14w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
B	233	0	233	0	5-5-0	5-5-0
E	233	0	233	0	5-5-0	5-5-0
H	208	0	208	0	5-5-0	5-5-0
G	208	0	208	0	5-5-0	5-5-0

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	169	111/0	21/0	0/0	0/0	37/0	0/0
E	169	111/0	21/0	0/0	0/0	37/0	0/0
H	158	81/0	38/0	0/0	0/0	41/0	0/0
G	158	81/0	38/0	0/0	0/0	41/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)	MAX. FACTORED UNBRACED LENGTH (FT)	MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORIZ. LOAD (LBS)	MAX. FACTORED MOMENT (LBS-FT)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO							FR-TO						
A-B	0/17						H-C	-121/0	0.02 (1)				
B-J	-84/0						G-D	-121/0	0.02 (1)				
J-C	-85/0						I-J	-103/22	0.00 (1)				
C-D	-43/0						K-L	-103/22	0.00 (1)				
D-L	-85/0												
L-E	-84/0												
E-F	0/17												
B-I	0/51												
I-H	0/51												
H-G	0/43												
G-K	0/51												
K-E	0/51												

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA D86-09, CSA D86-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-L-1), BC=0.05/1.00 (G-K-1), WS=0.02/1.00 (D-G-1), SSI=0.09/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

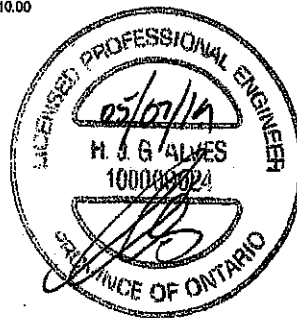
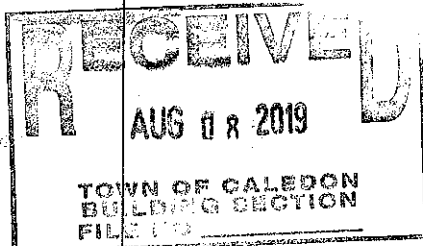
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.17 (B) (INPUT = 0.90)
JSI METAL = 0.04 (B) (INPUT = 1.00)

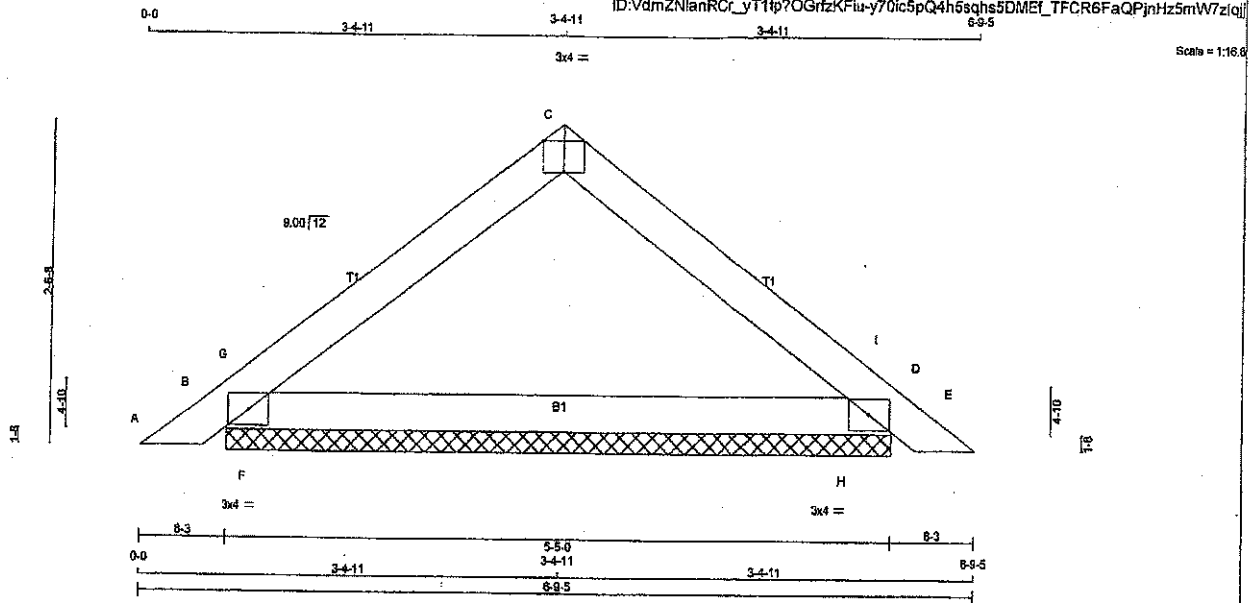


DRWG NO. TAM 174/0757
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402938	PB2	2	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:36 2019 Page 1
ID:VdmZNlanRCr_yT1tp?OGrtzKFlu-y70ic5pQ4h5sqhs5DMEI_TFCR6FaQPjnHz5mW7z/qj



LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - C 2x4 DRY No.2

C - E 2x4 DRY No.2

B - D 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMB1-I	MT20	3.0	4.0		
C TT-p	MT20	3.0	4.0	Edge	2.00
D TMB1-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
B	441	441	0	0
D	441	441	0	0

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
B	326	192 / 0	57 / 0	0 / 0	0 / 0	0 / 0
D	326	192 / 0	57 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD LC1	MAX	MAX.	MEMB.	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)
FR-TO							
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	F-G	0 / 330
B-G	-442 / 0	-102.1	-102.1	0.12 (3)	6.25	H-I	0 / 330
G-C	-235 / 0	-102.1	-102.1	0.15 (1)	6.25		
C-I	-235 / 0	-102.1	-102.1	0.15 (1)	6.25		
I-D	-442 / 0	-102.1	-102.1	0.12 (3)	6.25		
D-E	0 / 17	-102.1	-102.1	0.03 (1)	10.00		
B-F	0 / 201	-38.5	-38.5	0.09 (3)	10.00		
F-H	0 / 201	-38.5	-38.5	0.17 (2)	10.00		
H-D	0 / 201	-38.5	-38.5	0.09 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1 TC=0.15/1.00 (C-G:1), BC=0.17/1.00 (F-H:2), WB=0.00/1.00 (F-G:1), SS=0.18/1.00 (D-I:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618 354	1687 753	1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.80)

JSI METAL= 0.09 (B) (INPUT = 1.00)



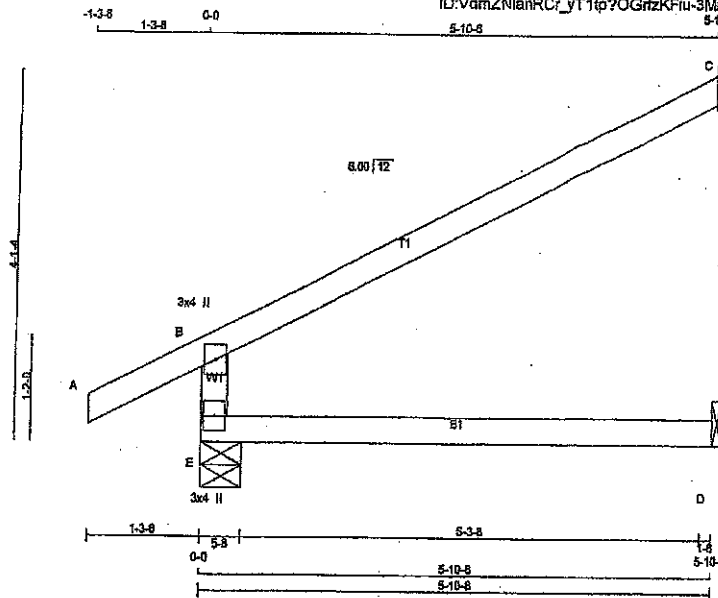
DWG NO. TAM 11910750
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J1	QUANTITY 19	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue May 7 17:41:32 2019 Page 1
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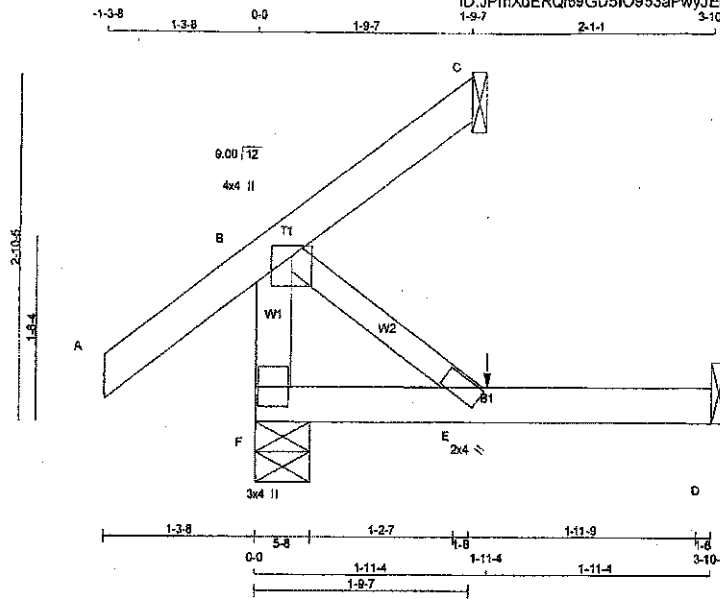
Scale = 1/2" = 1'-0"



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402979	J2	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
TRUSS DESC.					

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 15:11:04 2019 Page 1
ID:JPmXdERQ69GD5IO953aPwyJEIZ-eTu7Sv7foDeEkVTcX1GkgporZdrctXzg1vZk7lzlXqr

Scale = 1:17.7



TOTAL WEIGHT = 2 X 12 = 24 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORZ	DOWN	HORZ
F	361	0	361	0
C	37	0	37	0
D	75	0	95	0

SEE MI TEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	264	163/0	41/0	0/0	0/0	61/0	0/0
C	25	21/-31	0/0	0/0	0/0	4/0	0/0
D	68	0/0	41/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1	MAX. CS1 (LC)	UNBRACED LENGTH FR-TO
FR-TO		FROM TO		
F-B	-288/0	0.0	0.0	0.03 (1)
A-B	0/42	-102.1	-102.1	0.15 (1)
B-C	-33/0	-102.1	-102.1	0.15 (1)
F-E	0/0	-38.5	-38.5	0.14 (2)
E-D	0/0	-38.5	-38.5	0.14 (2)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-11-4	1	1		BACK	VERT	TOTAL		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/899 (0.02")

CS: TC=0.15/1.00 (A-B:1), BC=0.14/1.00 (D-E:2), WB=0.00/1.00 (B-E:1), SS=0.10/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

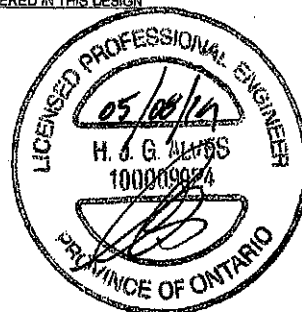
PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1667

PLATE PLACEMENT TOL = 0.250 inches

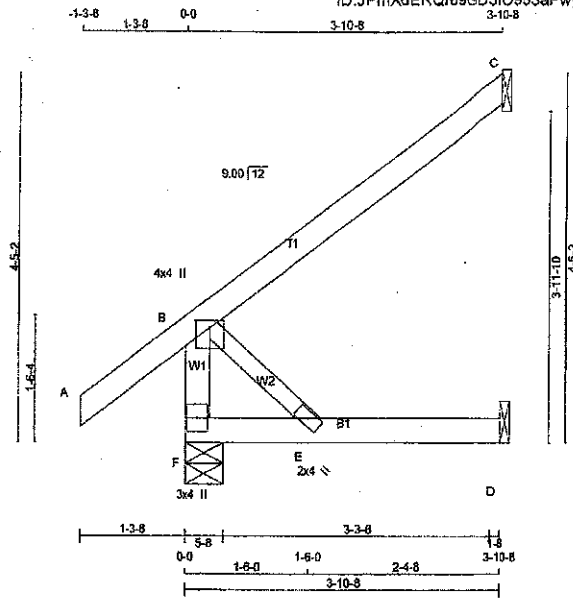
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)

DWG NO. TAM 11910972
STRUCTURAL
COMPONENT ONLY



JOB NAME 402979	TRUSS NAME J3	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 15:07:54 2019 Page 1 ID: JPMxdERQr69GD5IO953aPwyJEIZ-CvPAxqTYdiCxl5P7sAzkkLp1_AMcxikLXhKzmzIXtp			



Scale = 1:25.6

TOTAL WEIGHT = 15 lb [M]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
E	BMVW+p	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
F	413	0	413	0	5-8	5-8
C	198	0	198	0	1-8	1-8
D	75	0	95	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE		MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	300	193 / 0	41 / 0	0 / 0	0 / 0	67 / 0	0 / 0
C	136	113 / 0	0 / 0	0 / 0	0 / 0	23 / 0	0 / 0
D	68	0 / 0	41 / 0	0 / 0	0 / 0	27 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO	-339 / 0	0.0	0.0	0.0
A-B	0 / 42	-102.1	-102.1	0.14 (1)
B-C	0 / 0	-102.1	-102.1	0.26 (1)
F-E	0 / 0	-38.5	-38.5	0.13 (3)
E-D	0 / 0	-38.5	-38.5	0.14 (3)

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.01")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.02")

CSI: TC=0.26/1.00 (B-C:1), BC=0.14/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

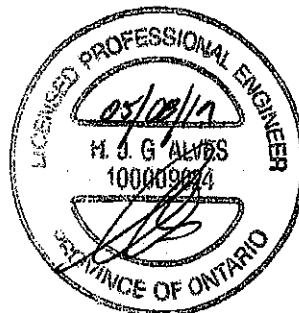
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PS)	SECTION (PL)
MT20	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

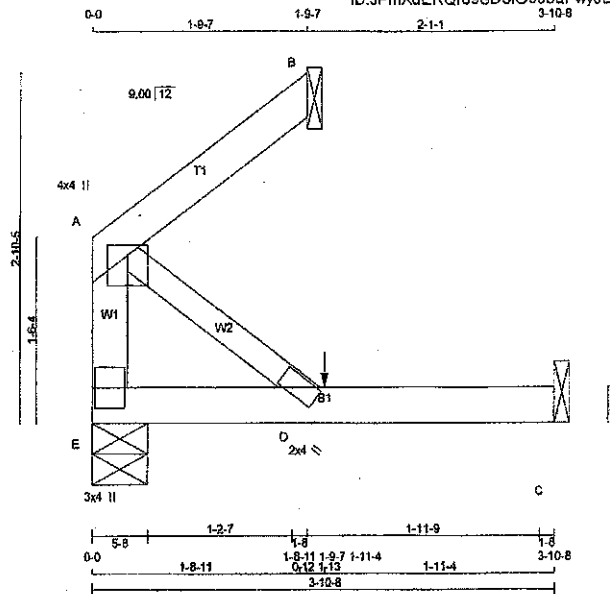
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)



DRWG NO. TAM 71910973
STRUCTURAL
COMPONENT ONLY

JOB NAME 402979	TRUSS NAME J5	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:07:56 2019 Page 1	ID:JPmXdERQr69GD5IO953aPwyJEIZ-8IXw7drk4FywBc5UWYue29plRgqdqVQ7of0oOfzIXln



Scale = 1:17.5

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - A	2x4	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
E - C	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
D	BMVW+p	MT20	2.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	HORZ	UPLIFT
E	166	0	0	5-8
B	91	0	0	1-8
C	75	0	0	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
E	130	52/0	41/0	0/0	38/0	0/0
B	63	52/0	0/0	0/0	11/0	0/0
C	68	0/0	41/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	FACTORED HORZ. LOAD (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (FT)
FR-TO					FR-TO		
E-A	0/0	0.0	0.0	0.01 (1)	A-D	0/0	0.00 (1)
A-B	0/0	-102.1	-102.1	0.08 (1)			
E-D	0/0	-38.5	-38.5	0.14 (2)			
D-C	0/0	-38.5	-38.5	0.14 (2)			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	1-11-4	1	1	1	BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/889$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/889$ (0.02")

CSI: TC=0.06/1.00 (A-B:1), BC=0.14/1.00 (C-D:2), WS=0.00/1.00 (A-D:1), SS=0.08/1.00 (D-E:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

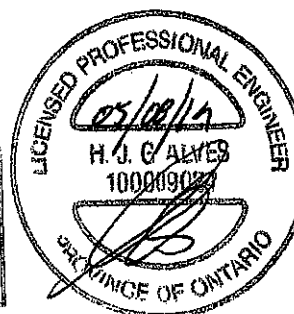
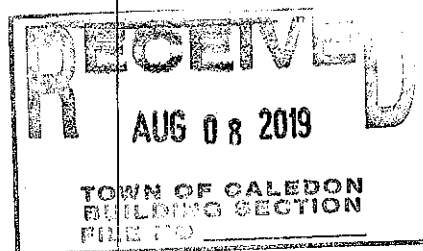
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1967 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

DWG NO. TAM 17910975
STRUCTURAL
COMPONENT ONLY



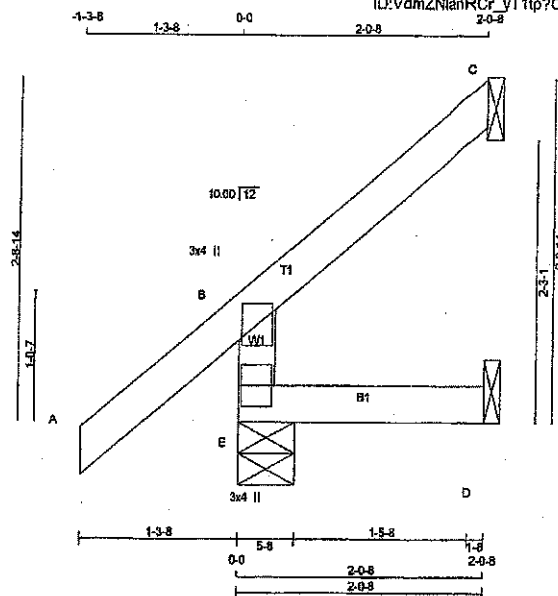
DWG NO. TAM 77910976
STRUCTURAL
EQUIPMENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402938	J7	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:35 2019 Page 1
ID:VdmZNlanRCr_yT1lp?OGfzKFiu-UxSKPipJnz?CXHvffjQSFj16xYhyTd2JLD_gzlaqk

Scale = 1:17.0



TOTAL WEIGHT = 4 X 8 = 33 lb

LUMBER	N L G A RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF		
A - C	2x4	DRY	No.2	SPF		
E - D	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0		
E	BMV+P	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	UPLIFT		
E	316	0	316	0	5-8	5-8
C	79	0	79	0	1-8	1-8
D	34	0	44	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	MAX / MIN	COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	226	194 / 0	24 / 0	0 / 0	0 / 0	48 / 0	0 / 0
C	54	45 / 0	0 / 0	0 / 0	0 / 0	9 / 0	0 / 0
D	31	0 / 0	19 / 0	0 / 0	0 / 0	13 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. UNBRACED LENGTH FR-TO	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)
		FROM	TO			
E-B	-271 / 0	0.0	0.0	0.02 (3)	7.81	
A-B	0 / 45	-102.1	-102.1	0.14 (1)	10.00	
B-C	-16 / 0	-102.1	-102.1	0.07 (1)	6.25	
E-D	0 / 0	-38.5	-38.5	0.03 (3)	10.00	

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.00")
ALLOWABLE DEFL. (TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SSI=0.08/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

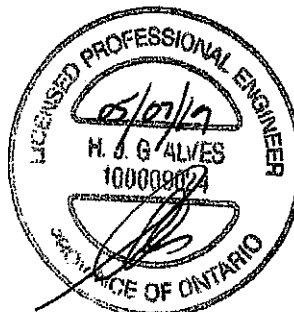
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (B) (INPUT = 0.90)
JSI METAL= 0.10 (B) (INPUT = 1.00)



DWG NO. TAM 171910762
STRUCTURAL
COMPONENT ONLY



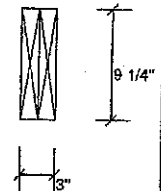
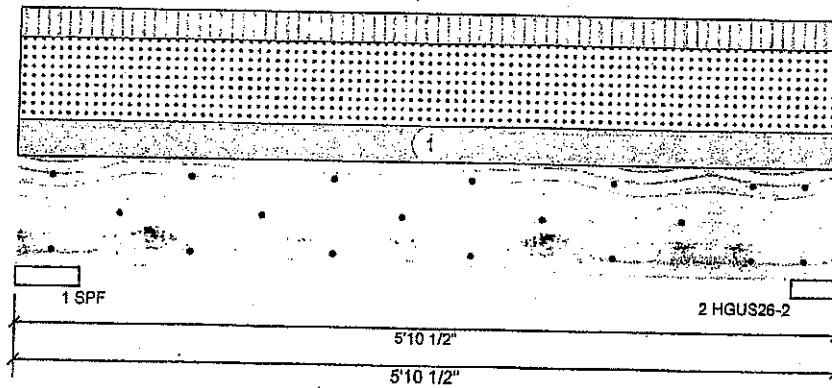
isDesign™

Client:
Project:
Address:Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 1 of 4

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Piles: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	207	257	583	0
2	199	246	558	0

Bearings and Factored Reactions

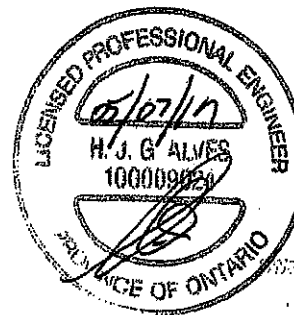
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	321 / 1081	1402 L	1.25D+1.5S +L
2 - HGUS...	4.000"	18%	308 / 1036	1344 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1585 ft-lb	3'	6039 ft-lb	0.262 (26%)	1.25D+1.5S +L	L
Unbraced	1585 ft-lb	3'	5236 ft-lb	0.303 (30%)	1.25D+1.5S +L	L
Shear	1217 lb	1'2"	3984 lb	0.306 (31%)	1.25D+1.5S +L	L
LL Defl inch	0.014 (L/4497)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.019 (L/3273)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



77910792

STRUCTURAL

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-7-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3259 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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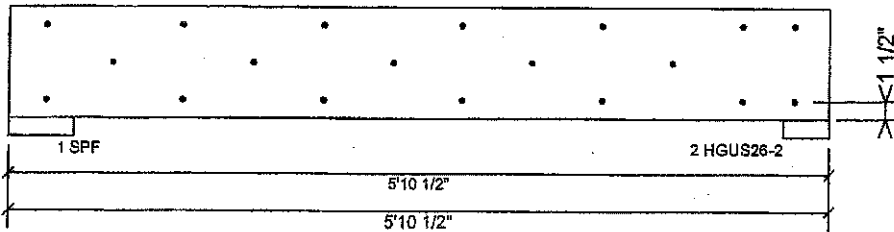
Client:
Project:
Address:

Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 2 of 4

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	88.7 %
Load	233.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



ENGINEER 05/07/19
STRUCTURAL
COMP. OF 201007
2/2

Manufacturer Info

Tamarack Roof Trusses
3288 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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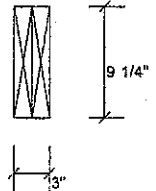
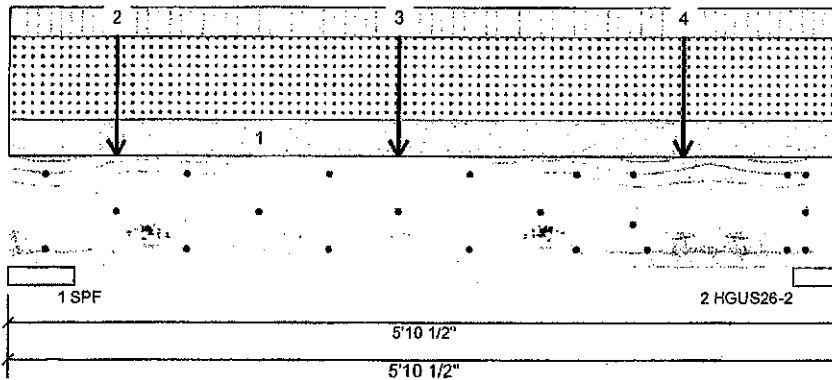
Client:
Project:
Address:

Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 1 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	155	203	456	0
2	180	233	523	0

Bearings and Factored Reactions

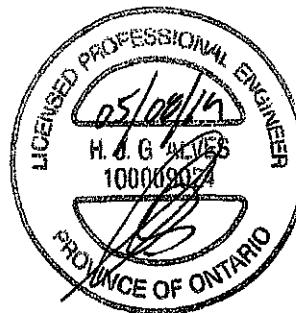
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	11%	254 / 839	1093 L
2 - HGUS...	4.000"	17%	291 / 964	1255 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1791 ft-lb	2'9"	6039 ft-lb	0.297 (30%)	1.25D+1.5S +L	L
Unbraced	1791 ft-lb	2'9"	5236 ft-lb	0.342 (34%)	1.25D+1.5S +L	L
Shear	1217 lb	4'10"	3984 lb	0.305 (31%)	1.25D+1.5S +L	L
LL Defl inch	0.014 (L/4361)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3157)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



ENGINEER TAM 7910994
STRUCTURAL
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	0-9-0		Far Face	24 lb	20 lb	52 lb	0 lb	j4
3	Point	2-9-0		Far Face	157 lb	110 lb	352 lb	0 lb	t15c
4	Point	4-9-0		Far Face	102 lb	82 lb	228 lb	0 lb	t15

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





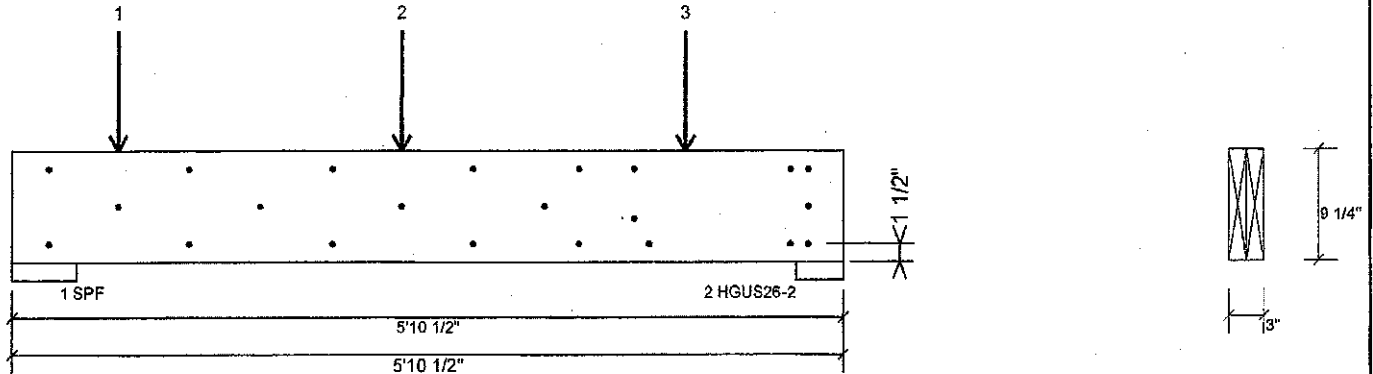
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 2 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

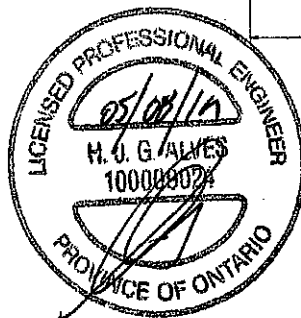
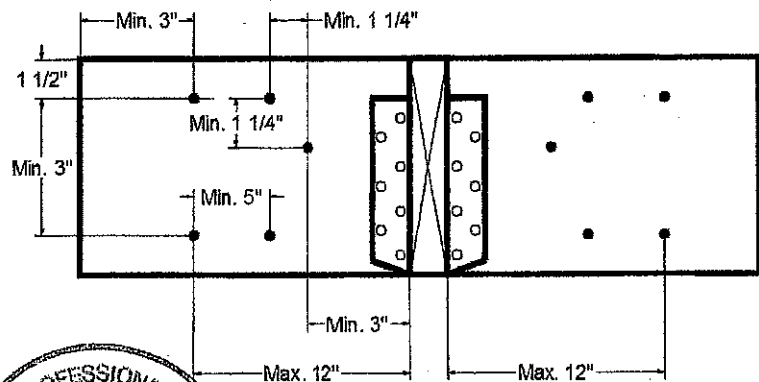
Fasten all plies using 3 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 4-9-0 with a minimum of (3) - Pneumatic Gun Nail (120x3.25") in the pattern shown.

Capacity	81.1 %
Load	275.8lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

DESIGN BY 7/9/0999
STRUCTURAL
CHECKED BY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(605) 335-1115



This design is valid until 12/11/2021





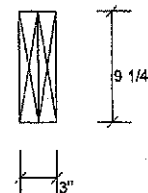
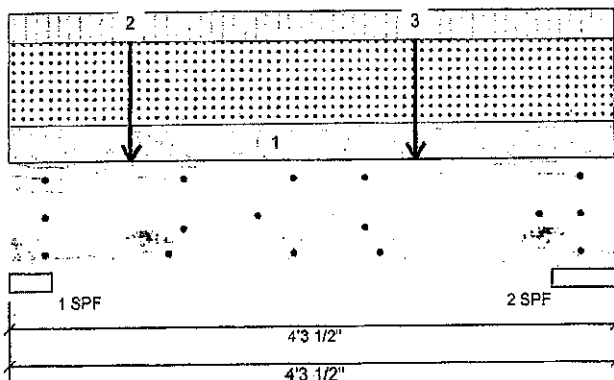
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 3 of 4

BM3 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	143	184	430	0
2	127	159	360	0

Bearings and Factored Reactions

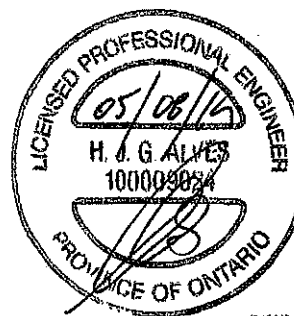
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	16%	230 / 788	1018	L	1.25D+1.5S +L
2 - SPF	5.500"	9%	198 / 887	866	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	769 ft-lb	2'4 5/8"	6039 ft-lb	0.127 (13%)	1.25D+1.5S +L	L
Unbraced	769 ft-lb	2'4 5/8"	5641 ft-lb	0.136 (14%)	1.25D+1.5S +L	L
Shear	985 lb	1'	3984 lb	0.247 (25%)	1.25D+1.5S +L	L
LL Defl inch	0.004 (L/12347)	2'7/8"	0.122 (L/360)	0.030 (3%)	S+0.5L	L
TL Defl inch	0.005 (L/9004)	2'7/8"	0.122 (L/360)	0.040 (4%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



STRUCTURAL

1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	0-10-8		Far Face	121 lb	91 lb	291 lb	0 lb	
3	Point	2-10-8		Far Face	110 lb	89 lb	246 lb	0 lb	

Manufacturer Info

Tamarack Roof Trusses

3259 North Service Rd., ON

Canada

L7N3G2

(805) 335-1115



This design is valid until 12/11/2021





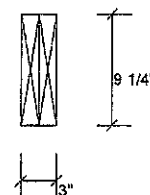
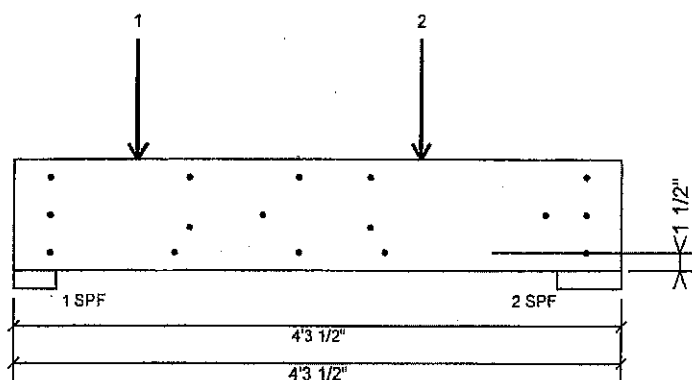
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 4 of 4

BM3 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Multi-Ply Analysis**

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c., except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

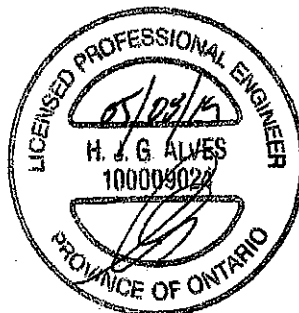
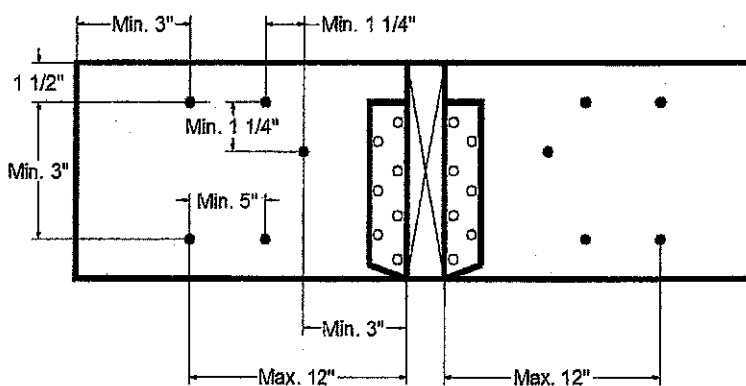
Fasten at concentrated side load at 0-10-8 with a minimum of (3) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	99.8 %
Load	339.4lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 2-10-8 with a minimum of (3) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	87.6 %
Load	297.8lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

DESIGNER: TAM 1910995
BY: J. G. ALVES
CHECKED: J. G. ALVES
DATE: 5/8/2019

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





ONTARIO WOOD TRUSS
FABRICATORS ASSOCIATION

TECH-NOTES

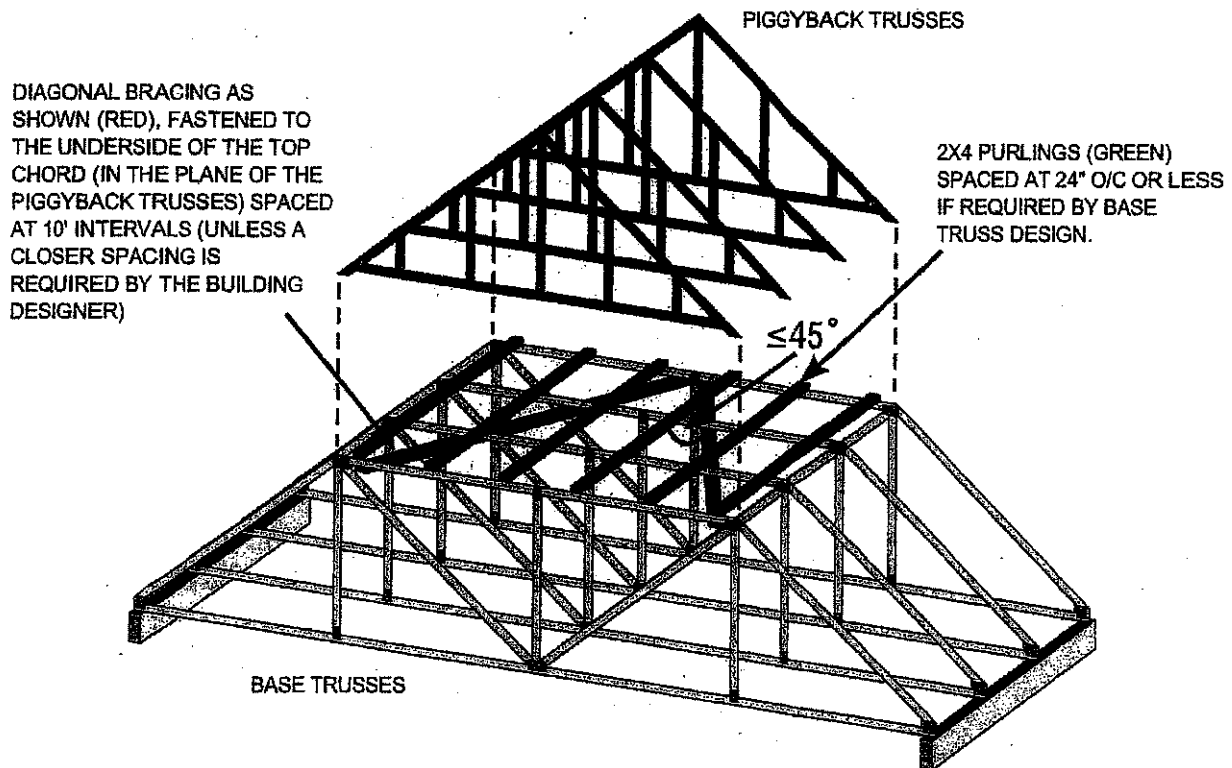
TN 15-001 Piggyback Bracing

Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

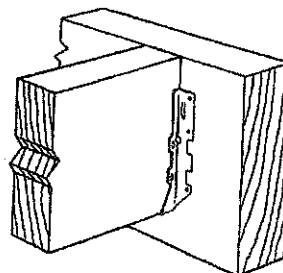
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

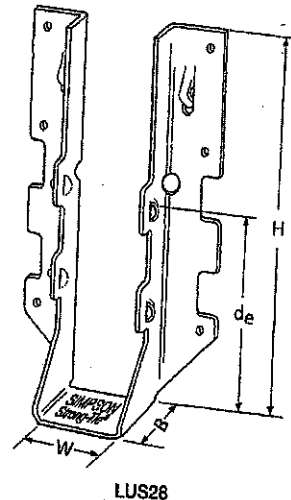
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified

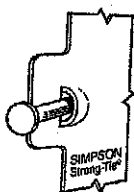


Typical LUS
Installation



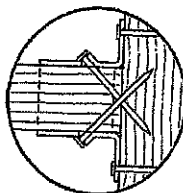
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LUS24	18	1 1/8	3 1/8	1 1/4	1 1/8	(4) 10d	(2) 10d	(K _o =1.15)	(K _o =1.00)	(K _o =1.15)	(K _o =1.00)
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	710	1630	645	1155
LUS26	18	1 1/8	4 3/8	1 1/4	3 3/8	(4) 10d	(4) 10d	835	2020	590	1435
LUS26-2	18	3 3/8	4 3/8	2	4	(4) 16d	(4) 16d	1420	2170	1290	1630
LUS26-3	18	4 3/8	4 3/8	2	3 1/4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS28	18	1 1/8	6 3/8	1 1/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 3/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 3/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_o is the distance from the seat of the hanger to the highest joist nail.

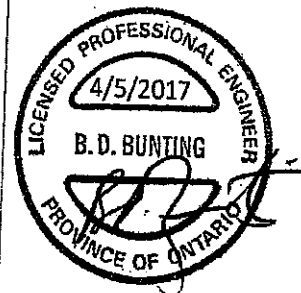


Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



DESIGN

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HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

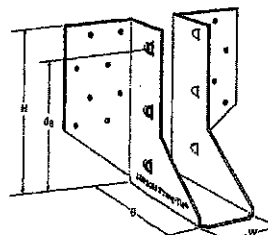
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

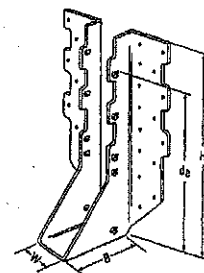
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

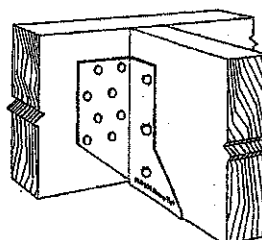
- See current catalogue for options



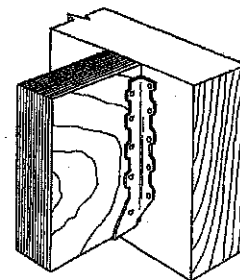
LJS26DS



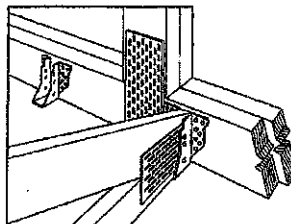
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



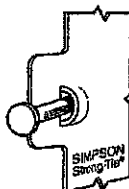
Typical HUS
Installation



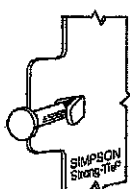
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4265	1480	4115
HUS26	16	1 1/8	5 1/4	3	3 1/16	(14) 16d	(8) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/4	3	6 1/2	(22) 16d	(8) 16d	3605	5365	2675	4345
HUS210	16	1 1/8	9 1/2	3	7 1/2	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.8/10	18	1 1/16	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

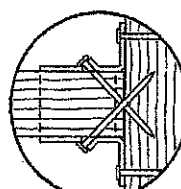
1. d_e is the distance from the seat of the hanger to the highest joist nail.



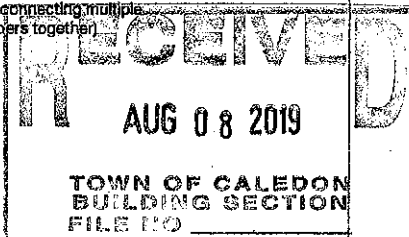
Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



DESIGN

Technical information is shown in this catalog for informational purposes only. It is not intended to be used as a substitute for the manufacturer's instructions. The manufacturer's instructions should be consulted for all details. © 2017 Simpson Strong-Tie Company. All rights reserved.

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HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

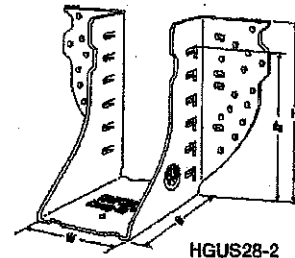
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

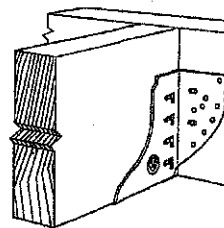
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

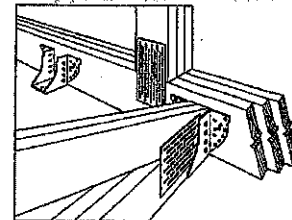
- See current catalogue for options



HGUS28-2



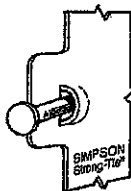
Typical HGUS Installation



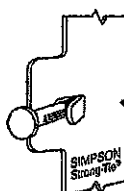
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d _o ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
HGUS26	12	1 1/8	5 1/8	5	4 1/2	(20) 16d	(8) 16d		2685	6625	2685	5700
HGUS26-2	12	3 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-3	12	4 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS26-4	12	6 1/8	5 1/8	4	4 1/8	(20) 16d	(8) 16d		4385	8950	3100	6355
HGUS28	12	1 1/8	7 1/8	5	8 1/8	(36) 16d	(12) 16d		3310	7675	3100	6900
HGUS28-2	12	3 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-3	12	4 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS28-4	12	6 1/8	7 1/8	4	6 1/8	(36) 16d	(12) 16d		6070	12980	4310	9215
HGUS210-2	12	3 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-3	12	4 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS210-4	12	6 1/8	9 1/8	4	8 1/8	(46) 16d	(16) 16d		6840	14645	4855	10400
HGUS212-4	12	6 1/8	10 1/8	4	10 1/8	(56) 16d	(20) 16d		7640	14995	5425	10645
HGUS214-4	12	6 1/8	12 1/8	4	11 1/8	(66) 16d	(22) 16d		10130	16400	7195	11645

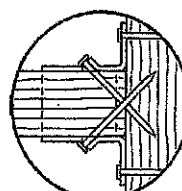
1. d_o is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



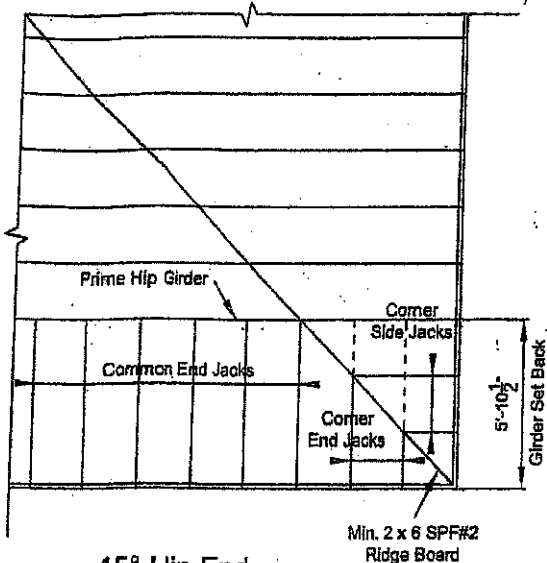
Double Shear Nailing Top View.



MADE IN CANADA

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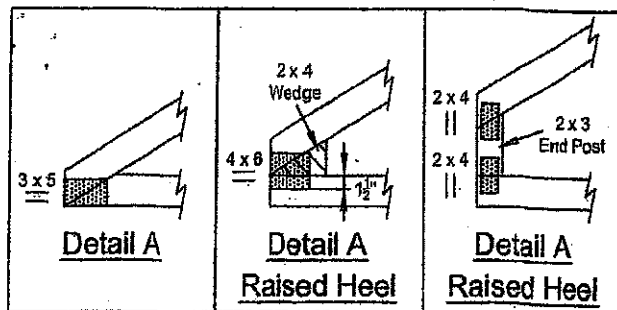
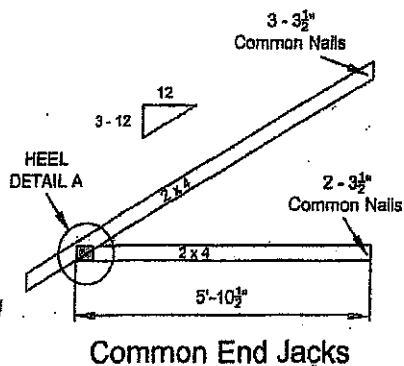
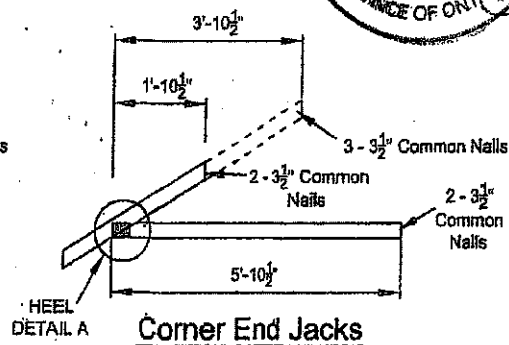
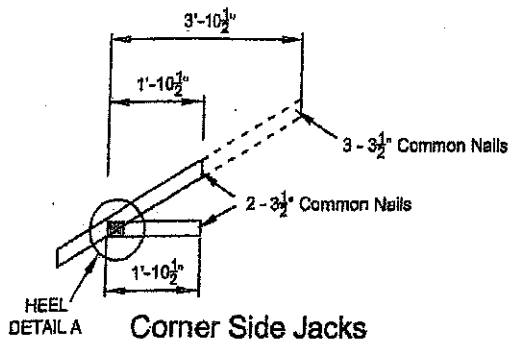
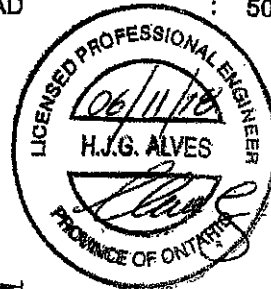
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

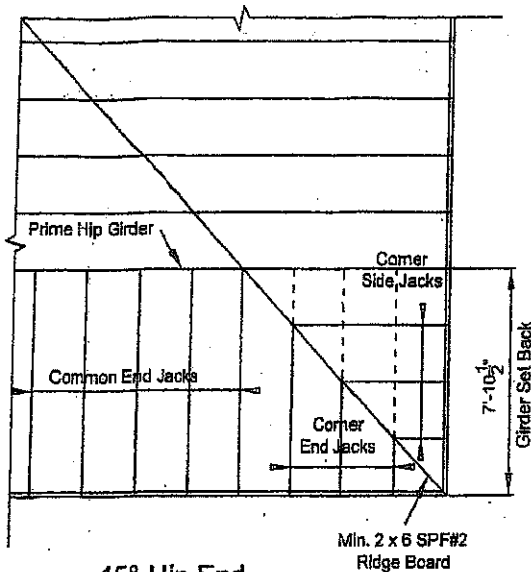
TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD : 50.5 P.S.F.



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

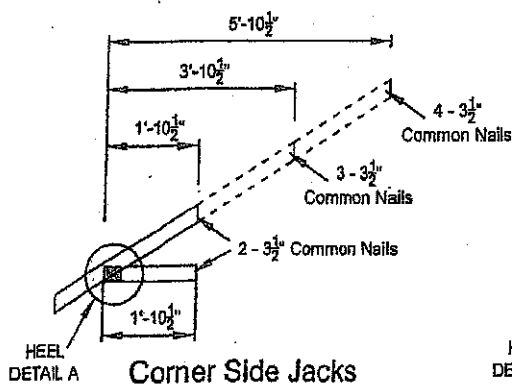
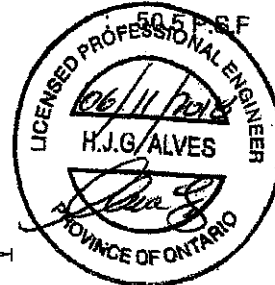
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

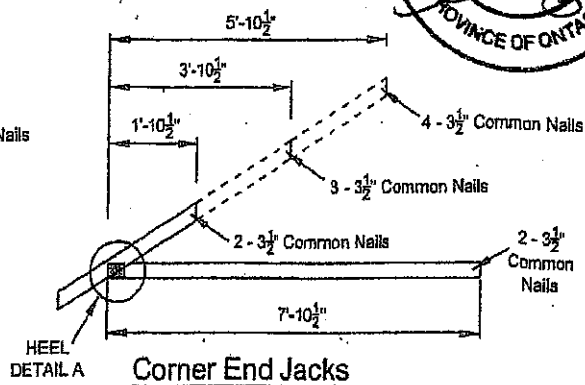
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

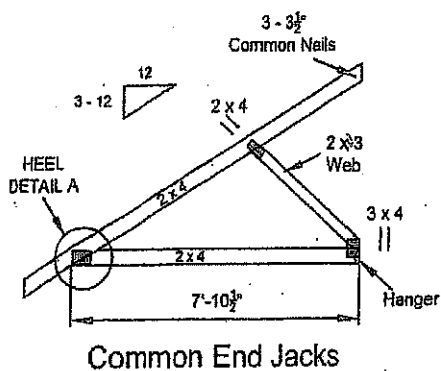
TOTAL LOAD



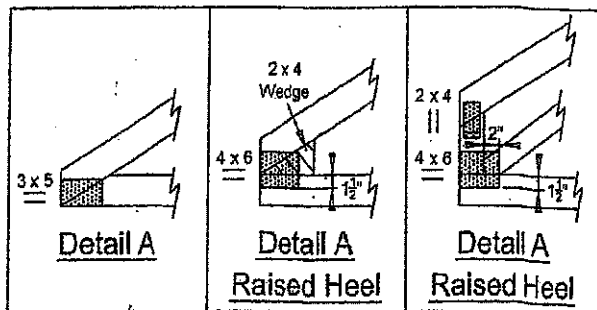
Corner Side Jacks



Corner End Jacks



Common End Jacks

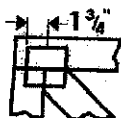


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

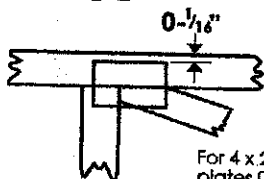
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

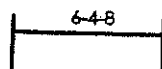


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

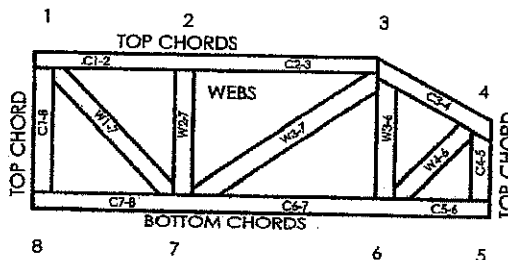
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal-Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

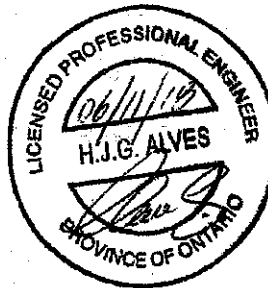
MiTek Engineering Reference Sheet: MII-7473C rev. 10-08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900213

Feb 09, 2018

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE