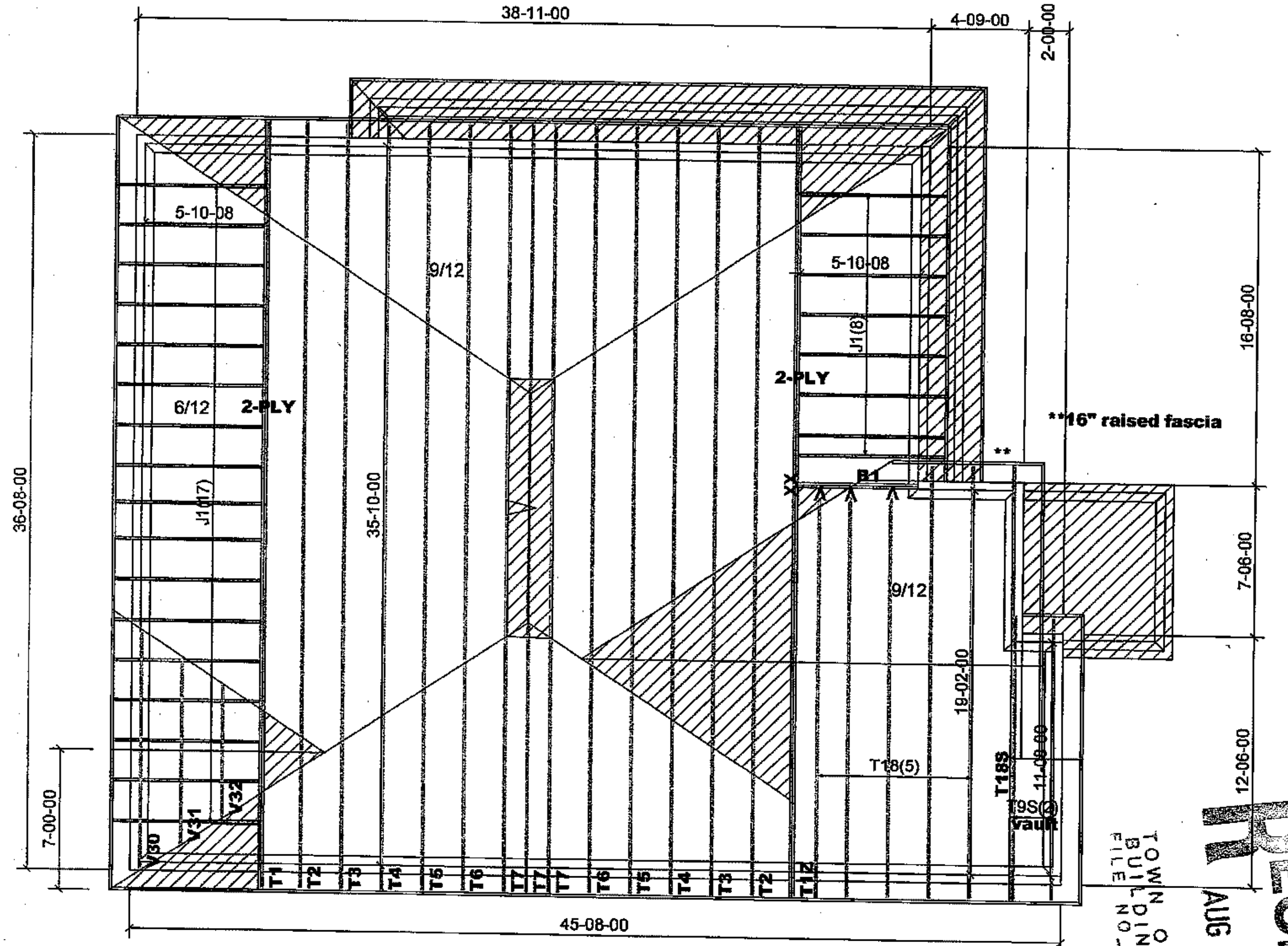


ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES CONV. FRAMING

GSL 37.6 PSF

DESIGN CONFORMS WITH O.B.C. 2012 PART 9
DESIGN LOADS:

TC LIVE	29	PSF
TC DEAD	6	PSF
BC LIVE	10.5	PSF
BC DEAD	7	PSF

T - 180737

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE ONTARIO BUILDING CODE

RECEIVED
AUG 28 2019
TOWN OF CALEDON BUILDING SECTION
FILE NO.

ALL CONV. FRAMINGS TO CONFORM WITH PART 9 OF O.B.C.2012
ROOF RAFTERS THAT CROSS MEET OVER TRUSSES TO BE 2x4 SPF @ 24" O.C. WITH A 2x4 VERT. POST TO THE TRUSS UNDERNEATH AT EACH CROSS PT. VERT. POST LONGER THAN 6' TO HAVE LATERAL BRACING SO THAT THE DISTANCE BETWEEN END PT. & BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'

TAMARACK LUMBER INC.

Job Track: **50033**	Builder / Location: **GREEN PARK HOMES / CALEDON**		Model / Elevation: **WOODCROFT 2 / 3- REAR UPGRADE Lot 51**
Plan Log: **200620**	Project: **LAMBERT LANE PH.2**		
Layout ID: **401690**	Date: 3/5/2019	Sales: Mario DiCano	Designer: Dan.s/JG

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC., SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.

Mitek ver 8.2.3.229

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200620
Project:	LAMBERT LANE PH.2	Layout ID:	401690
Location:	CALEDON	Ref #	
Model:	WOODCROFT 2	Page:	1 of 2
Lot #:	51	Date:	03/05/2019
Elevation:	3- REAR UPGRADE	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

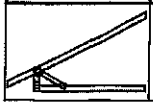
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9 /12	35-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	366.62 224.00		
	1 2-ply	T1Z Hip Girder	9 /12	35-10-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	366.62 224.00		
	2	T2 Hip	9 /12	35-10-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	299.62 188.33		
	2	T3 Hip	9 /12	35-10-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	309.43 194.67		
	2	T4 Hip	9 /12	35-10-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	326.47 209.00		
	2	T5 Hip	9 /12	35-10-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	322.29 202.00		
	2	T6 Hip	9 /12	35-10-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	345.79 219.33		
	3	T7 Hip	9 /12	35-10-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	505.25 319.00		
	2	T9S Scissor	9 /12 6 /12	11-08-00	5-10-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	106.86 73.33		
	5	T18 Common	9 /12	19-02-00	9-04-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	448.46 281.67		
	1	T18S Roof Special	9 /12 3 /12	19-02-00	9-04-08	2 x 4	1-03-08 1-03-08	1-06-04 2-10-04	92.4 57.83		
	1	V30 Valley	9 /12	11-10-00	4-05-04	2 x 4			36.06 22.33		
	1	V31 Valley	9 /12	9-02-00	3-05-04	2 x 4			24.56 16.17		
	1	V32 Valley	9 /12	6-06-00	2-05-04	2 x 4			16.9 11.00		

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	200620
Project:	LAMBERT LANE PH.2	Layout ID:	401690
Location:	CALEDON	Ref #	
Model:	WOODCROFT 2	Page:	2 of 2
Lot #:	51	Date:	03/05/2019
Elevation:	3- REAR UPGRADE	Designer:	Jane Gong
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	**25**	J1 Jack-Open	6 /12	**5-10-08**	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	451.94 291.67		

TOTAL # TRUSS= **53** TOTAL BFT OF ALL TRUSSES= **2534.33** BFT. TOTAL WEIGHT OF ALL TRSSES **4019.27** LBS

HARDWARE

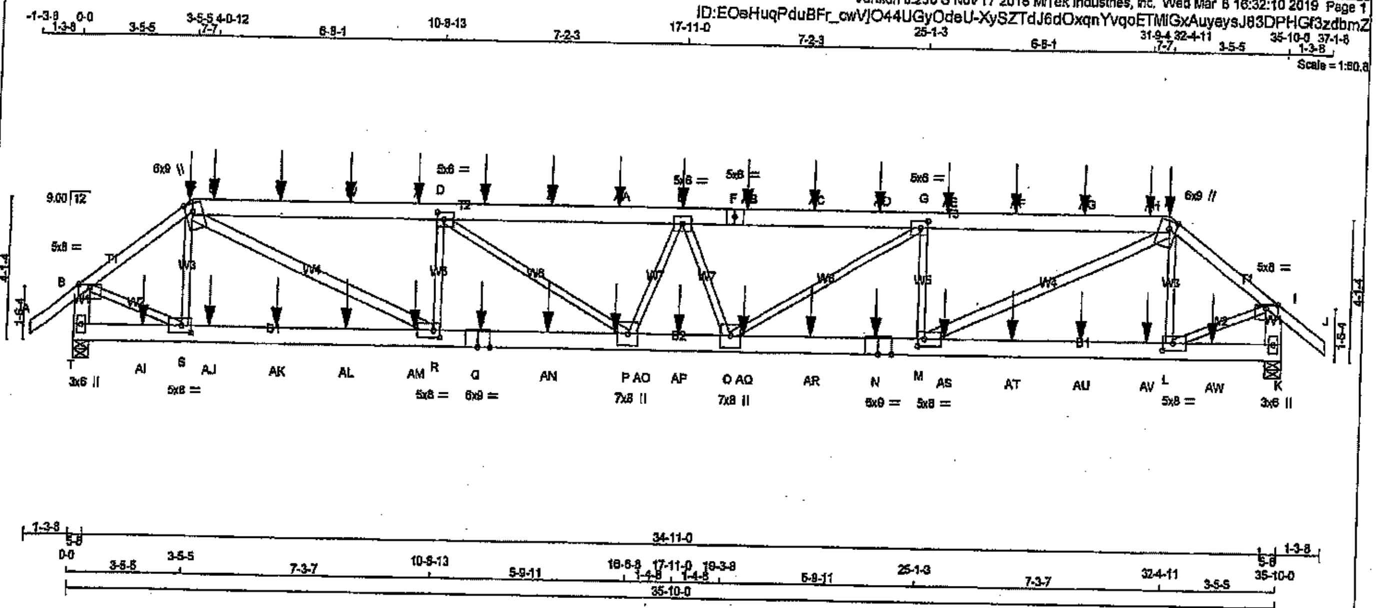
QTY	TYPE	MODEL	LENGTH
1	Hardware	HGUS26-2	
3	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= **4**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685'	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:10 2019 Page 1
ID:EOsHuqPduBFr_cwVJO44UGyOdeU-XySZTdJ6dOxqnYvqoETMlGxAuyeysJ83DPHGf3zdbmZ



TOTAL WEIGHT = 2 X 183 = 367 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x8	DRY	No.2	SPF
F - H	2x8	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x8	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x8	DRY	1650F 1.5E	SPF
Q - N	2x8	DRY	1650F 1.5E	SPF
N - K	2x6	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- C	1	12	SIDE(61.0)
H- J	1	12	SIDE(61.0)
C- F	2	12	SIDE(61.0)
F- H	2	12	SIDE(61.0)
T- B	2	12	TOP
K- I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T- Q	2	12	SIDE(0.0)
Q- N	2	12	SIDE(0.0)
N- K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TTWW+m	MT20	6.0	9.0	2.75	2.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	8.0		
G	TMWW-t	MT20	5.0	6.0	2.50	2.50
H	TTWW+m	MT20	6.0	9.0	2.75	2.50
I	TMVW-p	MT20	5.0	8.0	1.25	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.50
M	BMWW-t	MT20	5.0	8.0	2.50	2.25
N	BS-t	MT20	6.0	9.0		
O	BMWW+t	MT20	7.0	8.0		
P	BMWW+t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	5087	0	5087	0	0	5-8	5-8
K	5087	0	5087	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
T	3786	2140 / 0	722 / 0	0 / 0	0 / 0	924 / 0	0 / 0
K	3786	2140 / 0	722 / 0	0 / 0	0 / 0	924 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B- C	-5535 / 0	-102.1	-102.1	0.24 (1)	3.96
C- U	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
U- V	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
V- W	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
W- X	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
X- D	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
D- Y	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
Y- Z	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
Z-AA	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
AA- E	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
E- F	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
F-AB	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
AB-AC	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
AC-AD	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
AD- G	-12293 / 0	-102.1	-102.1	0.72 (1)	2.95
G-AE	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
AE-AF	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
AF-AG	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
AG-AH	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
AH- H	-10746 / 0	-102.1	-102.1	0.64 (1)	3.23
H- I	-5535 / 0	-102.1	-102.1	0.24 (1)	3.96
I- J	0 / 42	-102.1	-102.1	0.08 (1)	10.00
T- B	-5093 / 0	0.0	0.0	0.18 (1)	6.45
K- I	-5093 / 0	0.0	0.0	0.18 (1)	6.45
T-AI	0 / 0	-38.5	-38.5	0.10 (2)	10.00
AI- S	0 / 0	-38.5	-38.5	0.10 (2)	10.00
S-AJ	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AJ-AK	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AK-AL	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AL-AM	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AM- R	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
R- Q	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
Q-AN	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
AN-AO	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
AO- P	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
P-AP	0 / 12570	-38.5	-38.5	0.55 (1)	10.00
AP- O	0 / 12570	-38.5	-38.5	0.55 (1)	10.00
O-AQ	0 / 10745	-38.5	-38.5	0.51 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-948 / 0	0.12 (1)
C- R	0 / 7027	0.62 (1)
R- D	-2857 / 0	0.32 (1)
M- G	-2857 / 0	0.32 (1)
M- H	0 / 7027	0.62 (1)
L- H	-948 / 0	0.12 (1)
B- S	0 / 4681	0.58 (1)
L- I	0 / 4681	0.58 (1)
P- E	-741 / 0	0.10 (1)
E- O	-741 / 0	0.10 (1)
D- P	0 / 1836	0.23 (1)
O- G	0 / 1836	0.23 (1)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

DWG NO. TAM T1905098
STRUCTURAL
COMPONENT ONLY 1/2

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.33")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 789 (0.54")

CSI: TC=0.72/1.00 (E-G:1) , BC=0.55/1.00 (O-P:1) , WB=0.62/1.00 (C-R:1) , SSI=0.30/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1887	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.80 (Q) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T1	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Wed Mar 6 16:32:10 2019 Page 2

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-XySZTdJ6dOxgnYvgoETMIGxAuyevsJ83DPHGf3zdbmZ

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BS-t	MT20	6.0	9.0		
R	BMWW-t	MT20	5.0	8.0	2.50	2.25
S	BMWW-t	MT20	5.0	6.0	2.50	3.50
T	BMV1+p	MT20	3.0	6.0		

LOADING

TOTAL LOAD CASES: (4)

CHORDS

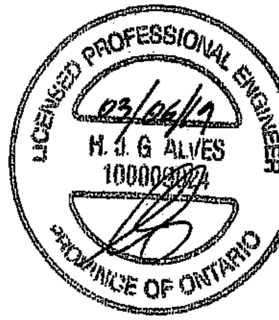
MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
AQ-AR	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
AR-N	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
N-M	0 / 10745	-38.5	-38.5	0.51 (1)	10.00
M-AS	0 / 4469	-38.6	-38.5	0.27 (1)	10.00
AS-AT	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AT-AU	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AU-AV	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
AV-L	0 / 4469	-38.5	-38.5	0.27 (1)	10.00
L-AW	0 / 0	-38.5	-38.5	0.10 (2)	10.00
AW-K	0 / 0	-38.5	-38.5	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-43	-48	—	FRONT	VERT	DEAD	—	—
C	3-5-5	-248	-248	—	FRONT	VERT	SNOW	—	—
E	17-11-0	-198	-198	—	BACK	VERT	TOTAL	—	—
H	32-4-11	-43	-48	—	FRONT	VERT	DEAD	—	—
H	32-4-11	-248	-248	—	FRONT	VERT	SNOW	—	—
N	23-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
Q	12-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
U	4-0-12	-217	-217	—	BACK	VERT	TOTAL	—	—
V	6-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
W	8-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
X	10-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
Y	12-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
Z	14-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
AA	16-0-12	-198	-198	—	BACK	VERT	TOTAL	—	—
AB	19-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AC	21-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AD	23-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AE	25-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AF	27-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AG	29-9-4	-198	-198	—	BACK	VERT	TOTAL	—	—
AH	31-9-4	-217	-217	—	BACK	VERT	TOTAL	—	—
AI	2-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AJ	4-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AK	6-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AL	8-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AM	10-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AN	14-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AO	16-0-12	-75	-95	—	BACK	VERT	TOTAL	—	—
AP	17-11-0	-75	-95	—	BACK	VERT	TOTAL	—	—
AQ	19-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AR	21-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AS	25-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AT	27-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AU	29-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AV	31-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—
AW	33-9-4	-75	-95	—	BACK	VERT	TOTAL	—	—



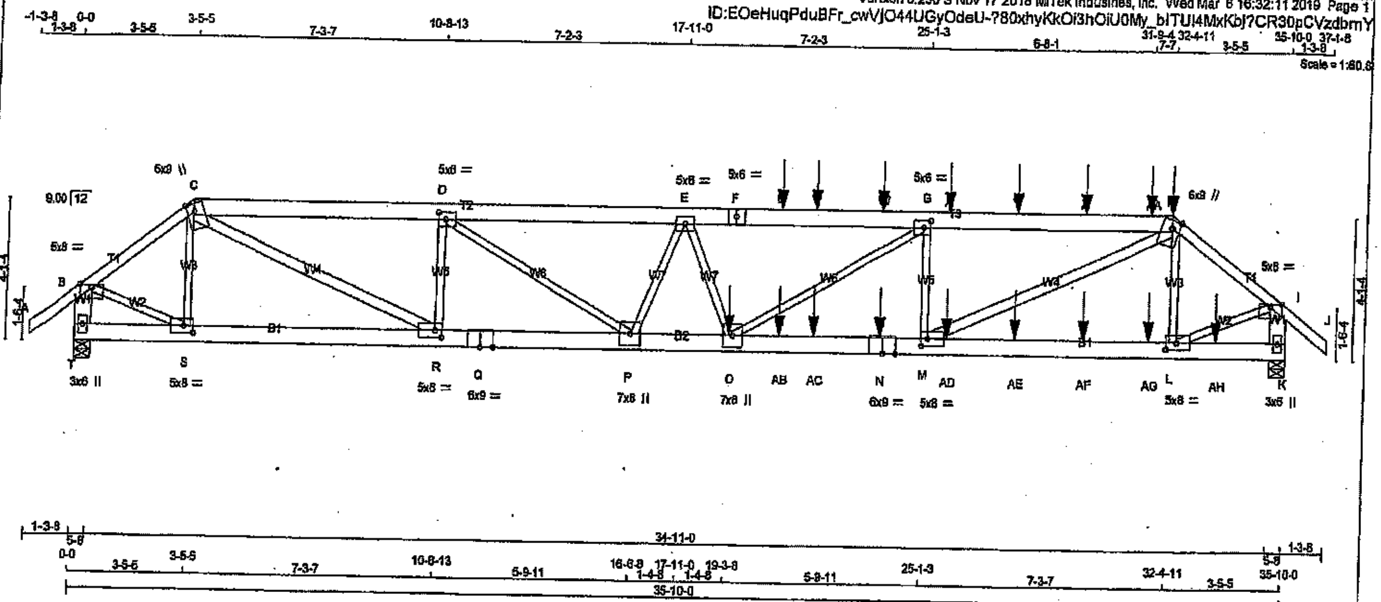
DWG NO. TAM T1905098
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:11 2019 Page 1

ID:EOeHuqPduBFr_cwVJO44UGyOdeU-?80xhyKkOi3hOiU0My_bITUI4MxKbJ?CR30pCVzdbmY



TOTAL WEIGHT = 2 X 183 = 367 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x6	DRY	No.2	SPF
F - H	2x6	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - Q	2x6	DRY	1650F 1.5E	SPF
Q - N	2x6	DRY	1650F 1.5E	SPF
N - K	2x8	DRY	1650F 1.5E	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
C - R	2x4	DRY	No.2	SPF
M - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
H-J	1	12	SIDE(61.0)
C-F	2	12	TOP
F-H	2	12	SIDE(61.0)
T-B	2	12	TOP
K-I	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
T-Q	2	12	TOP
Q-N	2	12	SIDE(0.0)
N-K	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.25	4.00
C	TTWW+m	MT20	6.0	9.0	2.75	2.50
D	TMWW-t	MT20	5.0	6.0	2.50	2.50
E	TMWW-t	MT20	5.0	6.0		
F	TS-t	MT20	5.0	6.0		
G	TMWW-t	MT20	5.0	8.0	2.50	2.50
H	TTWW+m	MT20	6.0	9.0	2.75	2.50
I	TMVW-p	MT20	5.0	8.0	1.25	4.00
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	8.0	2.50	3.50
M	BMWW-t	MT20	5.0	8.0	2.50	2.25
N	BS-t	MT20	6.0	9.0		
O	BMWW+t	MT20	7.0	8.0		
P	BMWW+t	MT20	7.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	4463	0	4463	0	0	5-8	5-8
K	5851	0	5851	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
T	3312	1903 / 0	609 / 0	0 / 0	0 / 0	799 / 0	0 / 0
K	4342	2494 / 0	799 / 0	0 / 0	0 / 0	1048 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.08 (1)	10.00
B- C	-4802 / 0	-102.1	-102.1	0.21 (1)	4.23
C- D	-10612 / 0	-102.1	-102.1	0.45 (1)	3.49
D- E	-14217 / 0	-102.1	-102.1	0.59 (1)	2.95
E- F	-15586 / 0	-102.1	-102.1	0.95 (1)	2.39
F- U	-15586 / 0	-102.1	-102.1	0.95 (1)	2.39
U- V	-15586 / 0	-102.1	-102.1	0.95 (1)	2.39
V- W	-15586 / 0	-102.1	-102.1	0.95 (1)	2.39
W- G	-15586 / 0	-102.1	-102.1	0.95 (1)	2.39
G- X	-13009 / 0	-102.1	-102.1	0.79 (1)	2.81
X- Y	-13009 / 0	-102.1	-102.1	0.79 (1)	2.81
Y- Z	-13009 / 0	-102.1	-102.1	0.79 (1)	2.81
Z-AA	-13009 / 0	-102.1	-102.1	0.79 (1)	2.81
AA- H	-13009 / 0	-102.1	-102.1	0.79 (1)	2.81
H- I	-8422 / 0	-102.1	-102.1	0.28 (1)	3.66
I- J	0 / 42	-102.1	-102.1	0.08 (1)	10.00
T- B	-4461 / 0	0.0	0.0	0.16 (1)	6.61
K- I	-5860 / 0	0.0	0.0	0.21 (1)	6.10
T- S	0 / 0	-38.5	-38.5	0.06 (2)	10.00
S- R	0 / 3862	-38.5	-38.5	0.21 (1)	10.00
R- Q	0 / 10612	-38.5	-38.5	0.49 (1)	10.00
Q- P	0 / 10612	-38.5	-38.5	0.49 (1)	10.00
P- O	0 / 15078	-38.5	-38.5	0.67 (1)	10.00
O-AB	0 / 13009	-38.5	-38.5	0.64 (1)	10.00
AB-AC	0 / 13009	-38.5	-38.5	0.64 (1)	10.00
AC- N	0 / 13009	-38.5	-38.5	0.64 (1)	10.00
N- M	0 / 13009	-38.5	-38.5	0.64 (1)	10.00
M-AD	0 / 5188	-38.5	-38.5	0.30 (1)	10.00
AD-AE	0 / 5188	-38.5	-38.5	0.30 (1)	10.00
AE-AF	0 / 5188	-38.5	-38.5	0.30 (1)	10.00
AF-AG	0 / 5188	-38.5	-38.5	0.30 (1)	10.00
AG- L	0 / 5188	-38.5	-38.5	0.30 (1)	10.00
L-AH	0 / 0	-38.5	-38.5	0.10 (2)	10.00
AH- K	0 / 0	-38.5	-38.5	0.10 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-985 / 0	0.12 (1)
C- R	0 / 7534	0.67 (1)
R- D	-3133 / 0	0.38 (1)
M- G	-3393 / 0	0.41 (1)
M- H	0 / 6756	0.77 (1)
L- H	-1183 / 0	0.15 (1)
B- S	0 / 4061	0.50 (1)
L- I	0 / 5432	0.67 (1)
P- E	-2309 / 0	0.31 (1)
E- O	0 / 1351	0.17 (1)
D- P	0 / 4487	[illegible]
O- G	0 / 3056	0.38 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
H	32-4-11	-43	-48	—	FRONT	VERT	DEAD	—	—
H	32-4-11	-248	-248	—	FRONT	VERT	SNOW	—	—

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER:
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.40")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 659 (0.65")

CSI: TC=0.95/1.00 (E-G:1) , BC=0.67/1.00 (O-P:1) , WB=0.77/1.00 (H-M:1) , SSI=0.30/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (B) (INPUT = 0.90)
JSI METAL= 0.98 (Q) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

DWG NO. TAM 7190509 5
STRUCTURAL COMPONENT ONLY 1/2

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T1Z	1	2	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:11 2019 Page 2

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-?80xhyKkOi3hOiU0My_bITUI4MxKbl?CR30pCVzdbmY

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
Q	BS-t	MT20	6.0	9.0		
R	BMWW-t	MT20	6.0	6.0	2.50	2.25
S	BMWW-t	MT20	5.0	6.0	2.50	3.50
T	BMV1+p	MT20	3.0	6.0		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
N	23-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
O	19-3-8	-2704	-2704	—	FRONT	VERT	TOTAL	—	—
U	20-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
V	21-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
W	23-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
X	25-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
Y	27-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
Z	29-9-4	-198	-198	—	FRONT	VERT	TOTAL	—	—
AA	31-9-4	-217	-217	—	FRONT	VERT	TOTAL	—	—
AB	20-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AC	21-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AD	25-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AE	27-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AF	29-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AG	31-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—
AH	33-9-4	-75	-95	—	FRONT	VERT	TOTAL	—	—

RECEIVED
AUG 28 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. D. GALVES
100009024
PROVINCE OF ONTARIO

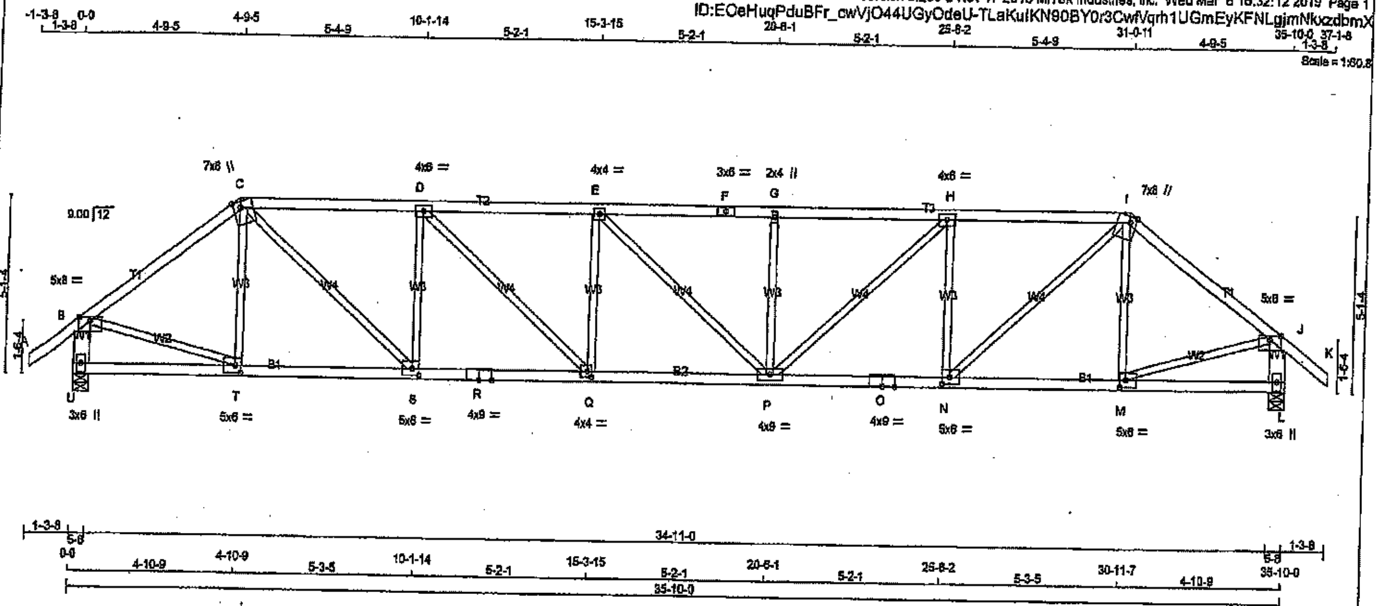
DWG NO. TAM 19005099
STRUCTURAL
COMPONENT ONLY 2/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T2	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:12 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-TLaKuIKN90BY0r3CwfVqrh1UGmEyKFNLgjmNkxzdbmX



TOTAL WEIGHT = 2 X 150 = 300 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
I - K	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
U - R	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	Edge	
C	TTWW+m	MT20	7.0	8.0	Edge	2.50
D	TMWW-t	MT20	4.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TMWW-t	MT20	4.0	6.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMVW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BMWW-t	MT20	5.0	6.0	2.25	2.50
O	BS-t	MT20	4.0	9.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0	2.00	1.50
R	BS-t	MT20	4.0	9.0		
S	BMWW-t	MT20	5.0	6.0	2.25	2.50
T	BMWW-t	MT20	5.0	6.0	2.50	2.00
U	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
U	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B-C	-2762 / 0	-102.1	-102.1	0.62 (1)	3.60
C-D	-3868 / 0	-102.1	-102.1	0.73 (1)	2.98
D-E	-4647 / 0	-102.1	-102.1	0.86 (1)	2.61
E-F	-4646 / 0	-102.1	-102.1	0.72 (1)	2.79
F-G	-4646 / 0	-102.1	-102.1	0.72 (1)	2.79
G-H	-4646 / 0	-102.1	-102.1	0.86 (1)	2.61
H-I	-3869 / 0	-102.1	-102.1	0.73 (1)	2.98
I-J	-2762 / 0	-102.1	-102.1	0.62 (1)	3.60
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00
U-B	-2586 / 0	0.0	0.0	0.18 (1)	6.45
L-J	-2586 / 0	0.0	0.0	0.18 (1)	6.45
U-T	0 / 0	-38.5	-38.5	0.18 (3)	10.00
T-S	0 / 2199	-38.5	-38.5	0.49 (1)	10.00
S-R	0 / 3868	-38.5	-38.5	0.72 (1)	10.00
R-Q	0 / 3868	-38.5	-38.5	0.72 (1)	10.00
Q-P	0 / 4647	-38.5	-38.5	0.84 (1)	10.00
P-O	0 / 3869	-38.5	-38.5	0.71 (1)	10.00
O-N	0 / 3869	-38.5	-38.5	0.71 (1)	10.00
N-M	0 / 2199	-38.5	-38.5	0.49 (1)	10.00
M-L	0 / 0	-38.5	-38.5	0.18 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-C	-319 / 29	0.12 (1)
M-I	-319 / 29	0.12 (1)
S-D	-1310 / 0	0.50 (1)
Q-E	-517 / 0	0.20 (1)
P-G	-516 / 0	0.20 (1)
N-H	-1310 / 0	0.50 (1)
C-S	0 / 2248	0.51 (1)
D-Q	0 / 1060	0.24 (1)
E-P	-2 / 0	0.00 (1)
P-H	0 / 1057	0.24 (1)
N-I	0 / 2248	0.51 (1)
B-T	0 / 2270	0.51 (1)
M-J	0 / 2270	0.51 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.30")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 872 (0.49")

CSI: TC=0.86/1.00 (G-H:1) , BC=0.84/1.00 (P-Q:1) , WB=0.51/1.00 (B-T:1) , SSI=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (N) (INPUT = 0.90)
JSI METAL= 0.80 (O) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

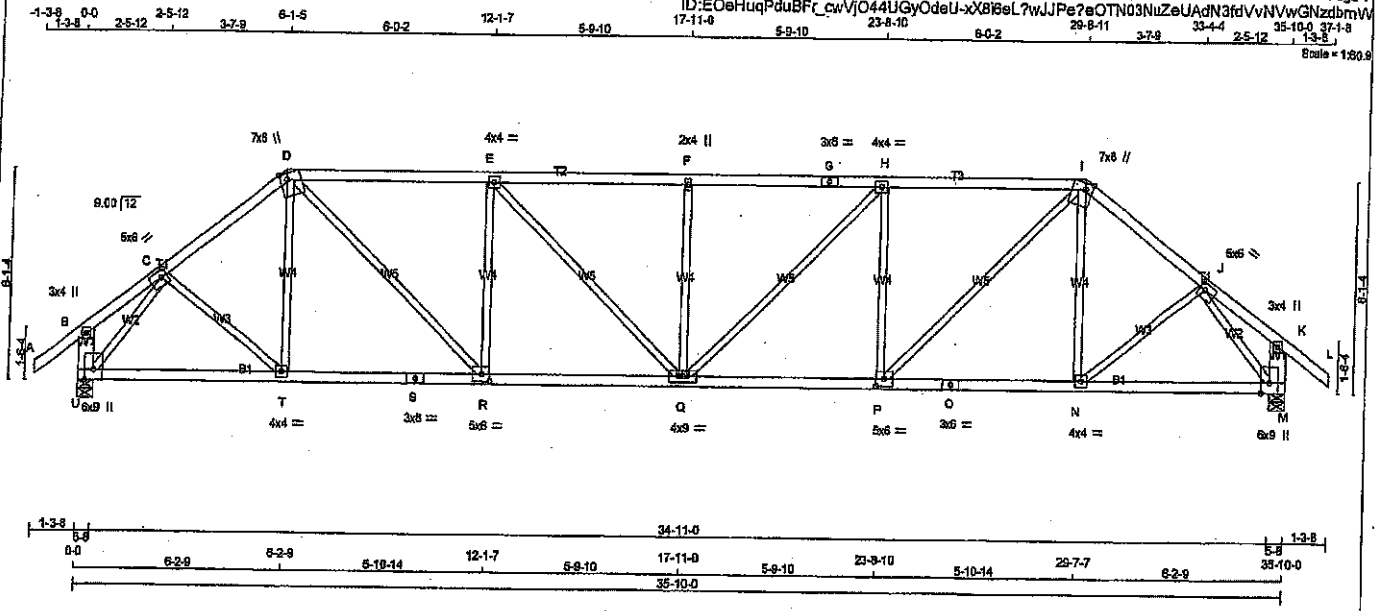
DWG NO. TAM 7190 5100
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T3	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:13 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-xX8i6eL?wJJPe?eOTN03NuZeUAdN3fdVvNVwGNzdbmW



TOTAL WEIGHT = 2 X 155 = 309 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.00
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	8.0	Edge	2.50
J	TMWW-t	MT20	5.0	6.0	2.25	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW1+p	MT20	6.0	9.0	Edge	
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW-t	MT20	5.0	6.0	2.50	2.75
Q	BMWWW-t	MT20	4.0	6.0		
R	BMWW-t	MT20	5.0	6.0	2.50	2.75
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1+p	MT20	6.0	9.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
M	2660	0	2660	0	0	5-8	5-8
U	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1979	1120 / 0	376 / 0	0 / 0	0 / 0	462 / 0	0 / 0
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.70 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 CSI (LC)	MAX UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 26	-102.1	-102.1	0.14 (1)	10.00
C- D	-2812 / 0	-102.1	-102.1	0.31 (1)	3.69
D- E	-3547 / 0	-102.1	-102.1	0.69 (1)	2.91
E- F	-3940 / 0	-102.1	-102.1	0.96 (1)	2.70
F- G	-3940 / 0	-102.1	-102.1	0.96 (1)	2.70
G- H	-3940 / 0	-102.1	-102.1	0.96 (1)	2.70
H- I	-3547 / 0	-102.1	-102.1	0.69 (1)	2.91
I- J	-2812 / 0	-102.1	-102.1	0.31 (1)	3.69
J- K	0 / 26	-102.1	-102.1	0.14 (1)	10.00
K- L	0 / 42	-102.1	-102.1	0.14 (1)	10.00
U- B	-219 / 0	0.0	0.0	0.01 (1)	7.81
M- K	-219 / 0	0.0	0.0	0.01 (1)	7.81
U- T	0 / 1868	-38.5	-38.5	0.51 (2)	10.00
T- S	0 / 2231	-38.5	-38.5	0.57 (2)	10.00
S- R	0 / 2231	-38.5	-38.5	0.57 (2)	10.00
R- Q	0 / 3547	-38.5	-38.5	0.70 (1)	10.00
Q- P	0 / 3547	-38.5	-38.5	0.70 (1)	10.00
P- O	0 / 2231	-38.5	-38.5	0.57 (2)	10.00
O- N	0 / 2231	-38.5	-38.5	0.57 (2)	10.00
N- M	0 / 1868	-38.5	-38.5	0.51 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T- D	-13 / 160	0.04 (3)
N- I	-13 / 160	0.04 (3)
C- T	0 / 458	0.10 (1)
D- R	0 / 1836	0.41 (1)
R- E	-1066 / 0	0.62 (1)
E- Q	0 / 553	0.12 (1)
Q- F	-551 / 0	0.32 (1)
Q- H	0 / 553	0.12 (1)
P- H	-1066 / 0	0.62 (1)
P- I	0 / 1836	0.41 (1)
N- J	0 / 458	0.10 (1)
U- C	-2999 / 0	0.70 (1)
J- M	-2999 / 0	0.70 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 989 (0.22")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 989 (0.37")

CSI: TC=0.96/1.00 (E-F:1) , BC=0.70/1.00 (Q-R:1) , WB=0.70/1.00 (J-M:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.76 (J) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009929
PROVINCE OF ONTARIO

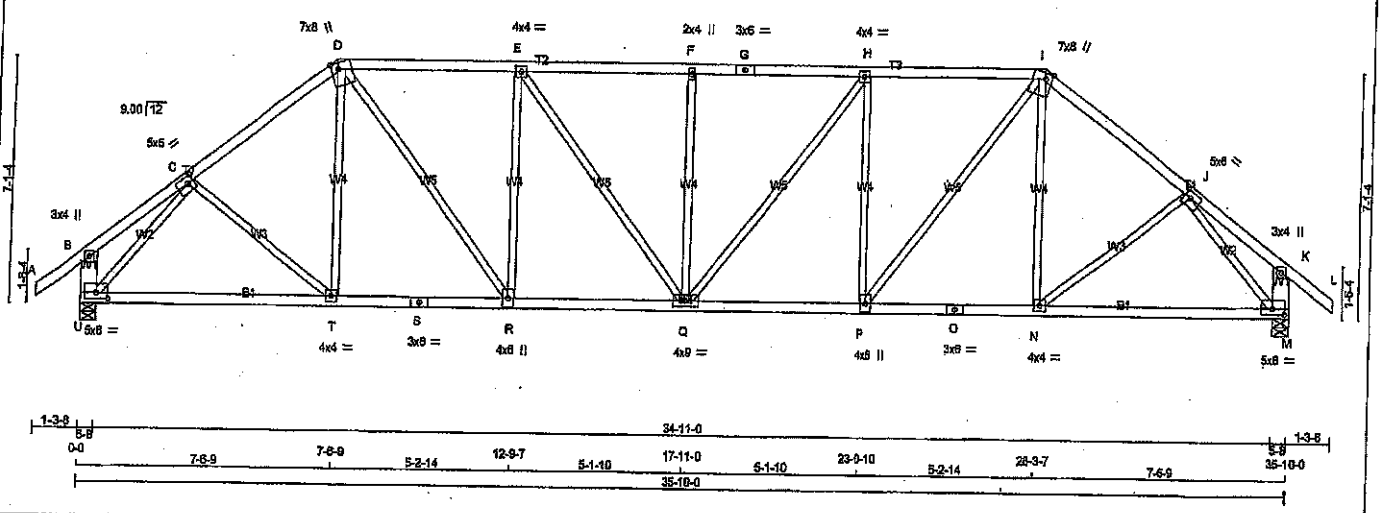
DWG NO. TAM F1905101
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401685	T4	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:14 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-Qji4J_MdhdSGF9Db14Xlw66t0azLo1Re71FToqzdbmV



TOTAL WEIGHT = 2 X 163 = 326 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - I	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF
U - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
U - S	2x4	DRY	No.2	SPF
S - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.25	2.25
D	TTWW+m	MT20	7.0	8.0	Edge	2.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	7.0	6.0	Edge	2.50
J	TMWW-t	MT20	5.0	6.0	2.25	2.25
K	TMV+p	MT20	3.0	4.0		
M	BMVW1-t	MT20	5.0	6.0	2.00	4.00
N	BMWW-t	MT20	4.0	4.0		
O	BS-t	MT20	3.0	6.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWWW-t	MT20	4.0	9.0		
R	BMWW+t	MT20	4.0	6.0		
S	BS-t	MT20	3.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW1-t	MT20	5.0	6.0	2.00	4.00

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER.

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2660	0	2660	0	0	5-8	5-8
U	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
U	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, U

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.23 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C	0 / 32	-102.1	-102.1	0.23 (1)	10.00	
C-D	-2797 / 0	-102.1	-102.1	0.40 (1)	3.83	
D-E	-3085 / 0	-102.1	-102.1	0.62 (1)	3.39	
E-F	-3352 / 0	-102.1	-102.1	0.65 (1)	3.23	
F-G	-3352 / 0	-102.1	-102.1	0.65 (1)	3.23	
G-H	-3352 / 0	-102.1	-102.1	0.65 (1)	3.23	
H-I	-3085 / 0	-102.1	-102.1	0.62 (1)	3.39	
I-J	-2797 / 0	-102.1	-102.1	0.40 (1)	3.83	
J-K	0 / 32	-102.1	-102.1	0.23 (1)	10.00	
K-L	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
U-B	-242 / 0	0.0	0.0	0.02 (1)	7.81	
M-K	-242 / 0	0.0	0.0	0.02 (1)	7.81	
U-T	0 / 2009	-38.5	-38.5	0.62 (2)	10.00	
T-S	0 / 2218	-38.5	-38.5	0.65 (2)	10.00	
S-R	0 / 2218	-38.5	-38.5	0.65 (2)	10.00	
R-Q	0 / 3085	-38.5	-38.5	0.61 (1)	10.00	
Q-P	0 / 3085	-38.5	-38.5	0.61 (1)	10.00	
P-O	0 / 2218	-38.5	-38.5	0.65 (2)	10.00	
O-N	0 / 2218	-38.5	-38.5	0.65 (2)	10.00	
N-M	0 / 2009	-38.5	-38.5	0.62 (2)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
T-D	0 / 246	0.06 (3)
N-I	0 / 246	0.06 (3)
C-T	0 / 277	0.05 (2)
D-R	0 / 1413	0.32 (1)
R-E	-950 / 0	0.83 (1)
E-Q	0 / 440	0.10 (1)
Q-F	-487 / 0	0.43 (1)
Q-H	0 / 440	0.10 (1)
P-H	-950 / 0	0.83 (1)
P-I	0 / 1413	0.32 (1)
N-J	0 / 277	0.06 (2)
U-C	-3053 / 0	0.99 (1)
J-M	-3053 / 0	0.99 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.18")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.30")

CSI: TC=0.65/1.00 (F-H:1) , BC=0.65/1.00 (R-T:2) , WB=0.99/1.00 (J-M:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) MAX	MIN	SHEAR (PSI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (M) (INPUT = 0.90)
JSI METAL= 0.76 (J) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009022
PROVINCE OF ONTARIO

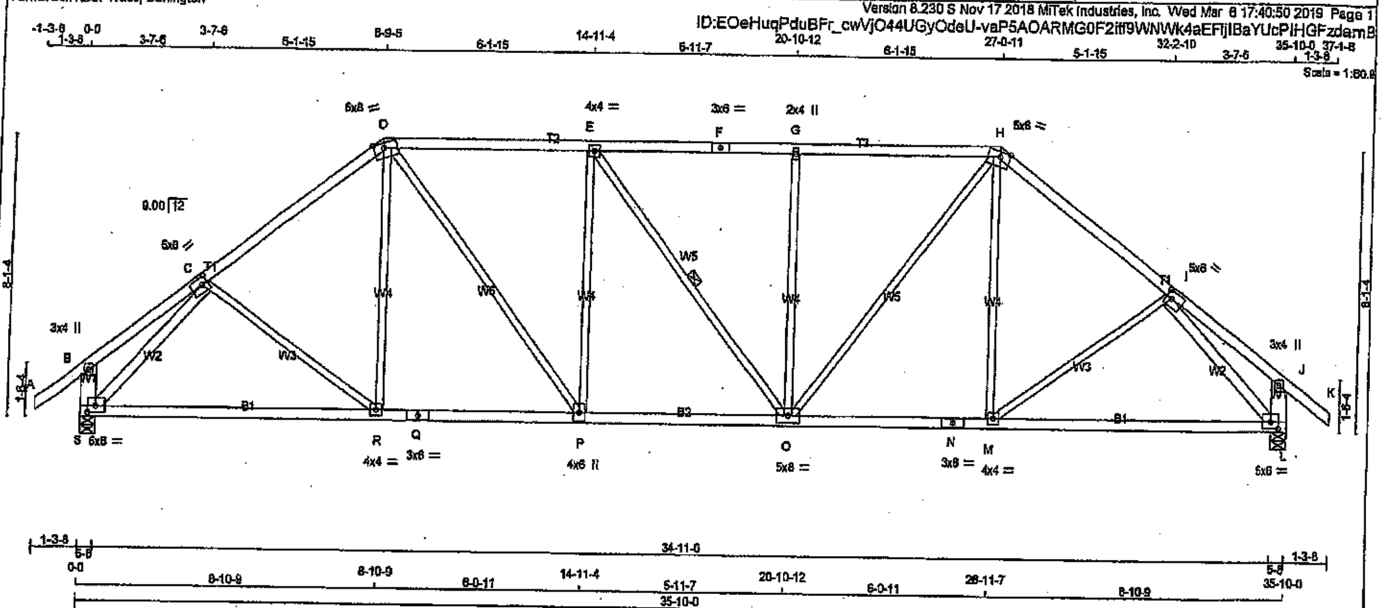
DWG NO. TAM T1905702
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
'401685'	T5	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:40:50 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-vaP5AOARMG0F2iff9WNWk4aEFljlBaYUcPIHGFzdamB



TOTAL WEIGHT = 2 X 164 = 328 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
S - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
S - C	2x4	DRY	No.2	SPF
I - L	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TS-t	MT20	3.0	6.0		
G	TMW+w	MT20	2.0	4.0		
H	TTWW-m	MT20	5.0	8.0	Edge	3.50
I	TMWW-t	MT20	5.0	6.0	2.50	2.00
J	TMV+p	MT20	3.0	4.0		
L	BMVW1-t	MT20	5.0	6.0	2.25	2.75
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	8.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BS-t	MT20	3.0	6.0		
R	BMWW-t	MT20	4.0	4.0		
S	BMVW1-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
S	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
S	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.33 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	R-D	0 / 352	0.08 (3)
B-C	0 / 43	-102.1	-102.1	0.35 (1)	10.00	M-H	0 / 353	0.08 (3)
C-D	-2752 / 0	-102.1	-102.1	0.59 (1)	3.70	P-E	-671 / 0	0.85 (1)
D-E	-2864 / 0	-102.1	-102.1	0.78 (1)	3.33	O-G	-670 / 0	0.85 (1)
E-F	-2863 / 0	-102.1	-102.1	0.77 (1)	3.33	C-R	0 / 179	0.04 (3)
F-G	-2863 / 0	-102.1	-102.1	0.77 (1)	3.33	D-P	0 / 1109	0.25 (1)
G-H	-2863 / 0	-102.1	-102.1	0.77 (1)	3.34	E-O	-2 / 0	0.00 (1)
H-I	-2752 / 0	-102.1	-102.1	0.59 (1)	3.70	O-H	0 / 1108	0.25 (1)
I-J	0 / 43	-102.1	-102.1	0.35 (1)	10.00	M-I	0 / 179	0.04 (3)
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	S-C	-3082 / 0	0.91 (1)
S-B	-249 / 0	0.0	0.0	0.02 (1)	7.81	I-L	-3082 / 0	0.91 (1)
L-J	-249 / 0	0.0	0.0	0.02 (1)	7.81			
S-R	0 / 2083	-38.5	-38.5	0.76 (2)	10.00			
R-Q	0 / 2179	-38.5	-38.5	0.78 (2)	10.00			
Q-P	0 / 2179	-38.5	-38.5	0.78 (2)	10.00			
P-O	0 / 2864	-38.5	-38.5	0.80 (1)	10.00			
O-N	0 / 2179	-38.5	-38.5	0.78 (2)	10.00			
N-M	0 / 2179	-38.5	-38.5	0.78 (2)	10.00			
M-L	0 / 2083	-38.5	-38.5	0.76 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14 -
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.26")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 981 (0.44")

CSI: TC=0.78/1.00 (D-E:1) , BC=0.78/1.00 (M-O:2) , WB=0.91/1.00 (I-L:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (I) (INPUT = 0.90)
JSI METAL= 0.99 (Q) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009824
PROVINCE OF ONTARIO

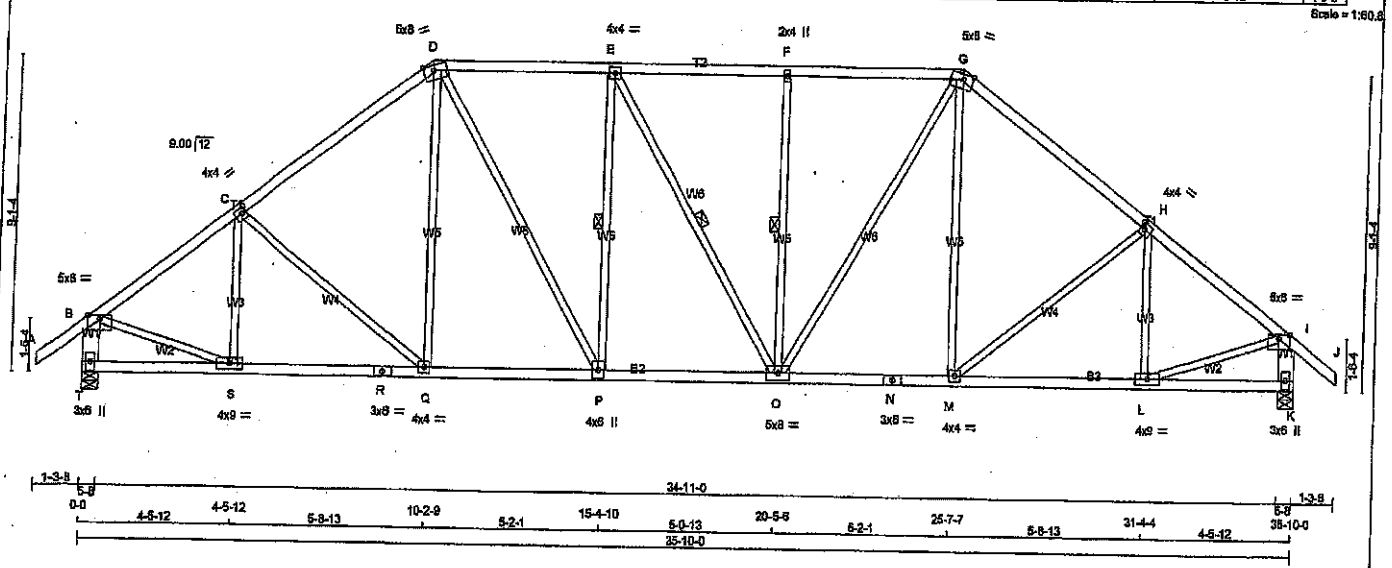
DWG NO. TAM T1905103
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401685	T6	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:16 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-M8pqkgOtDEl_VTMz9Vam?XBFINh_G27xbLkatizdbmT



TOTAL WEIGHT = 2 X 173 = 346 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
K - I	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - K	2x4	DRY	No.2	SPF
ALL WEBS, EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	3.50
E	TMWW-t	MT20	4.0	4.0		
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	8.0	Edge	3.50
H	TMWW-t	MT20	4.0	4.0	2.00	1.50
I	TMWW-p	MT20	5.0	8.0	Edge	
K	BMV1+p	MT20	3.0	6.0		
L	BMWW-t	MT20	4.0	9.0		
M	BMWW-t	MT20	4.0	4.0		
N	BS-t	MT20	3.0	6.0		
O	BMWWW-t	MT20	5.0	8.0		
P	BMWW+t	MT20	4.0	6.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	4.0	9.0		
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
T	2660	0	2660	0	0	5-8	5-8
K	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1979	1120 / 0	376 / 0	0 / 0	0 / 0	462 / 0	0 / 0
K	1979	1120 / 0	376 / 0	0 / 0	0 / 0	462 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.75 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-P, F-O, E-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B- C	-2776 / 0	-102.1	-102.1	0.46 (1)	3.75	
C- D	-2648 / 0	-102.1	-102.1	0.53 (1)	3.79	
D- E	-2547 / 0	-102.1	-102.1	0.40 (1)	3.96	
E- F	-2546 / 0	-102.1	-102.1	0.38 (1)	3.99	
F- G	-2546 / 0	-102.1	-102.1	0.40 (1)	3.96	
G- H	-2648 / 0	-102.1	-102.1	0.54 (1)	3.79	
H- I	-2776 / 0	-102.1	-102.1	0.46 (1)	3.75	
I- J	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
T- B	-2592 / 0	0.0	0.0	0.18 (1)	6.45	
K- I	-2592 / 0	0.0	0.0	0.18 (1)	6.45	
T- S	0 / 0	-38.5	-38.5	0.17 (3)	10.00	
S- R	0 / 2257	-38.5	-38.5	0.50 (1)	10.00	
R- Q	0 / 2257	-38.5	-38.5	0.50 (1)	10.00	
Q- P	0 / 2092	-38.5	-38.5	0.47 (1)	10.00	
P- O	0 / 2548	-38.5	-38.5	0.51 (1)	10.00	
O- N	0 / 2092	-38.5	-38.5	0.47 (1)	10.00	
N- M	0 / 2092	-38.5	-38.5	0.47 (1)	10.00	
M- L	0 / 2257	-38.5	-38.5	0.50 (1)	10.00	
L- K	0 / 0	-38.5	-38.5	0.17 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
Q- D	0 / 385	0.09 (2)
M- G	0 / 387	0.09 (2)
S- C	-390 / 5	0.14 (1)
P- E	-570 / 0	0.31 (1)
O- F	-569 / 0	0.31 (1)
L- H	-390 / 5	0.14 (1)
C- Q	-216 / 0	0.23 (1)
D- P	0 / 691	0.20 (1)
E- O	-3 / 0	0.00 (1)
G- G	0 / 867	0.20 (1)
M- H	-215 / 0	0.23 (1)
B- S	0 / 2335	0.53 (1)
L- I	0 / 2335	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.13")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.22")

CSI: TC=0.54/1.00 (G-H:1) , BC=0.51/1.00 (O-P:1) , WB=0.53/1.00 (B-S:1) , SSI=0.25/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

LS GRIP= 0.86 (B) (INPUT = 0.90)
SSI METAL= 0.88 (R) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009020
PROVINCE OF ONTARIO

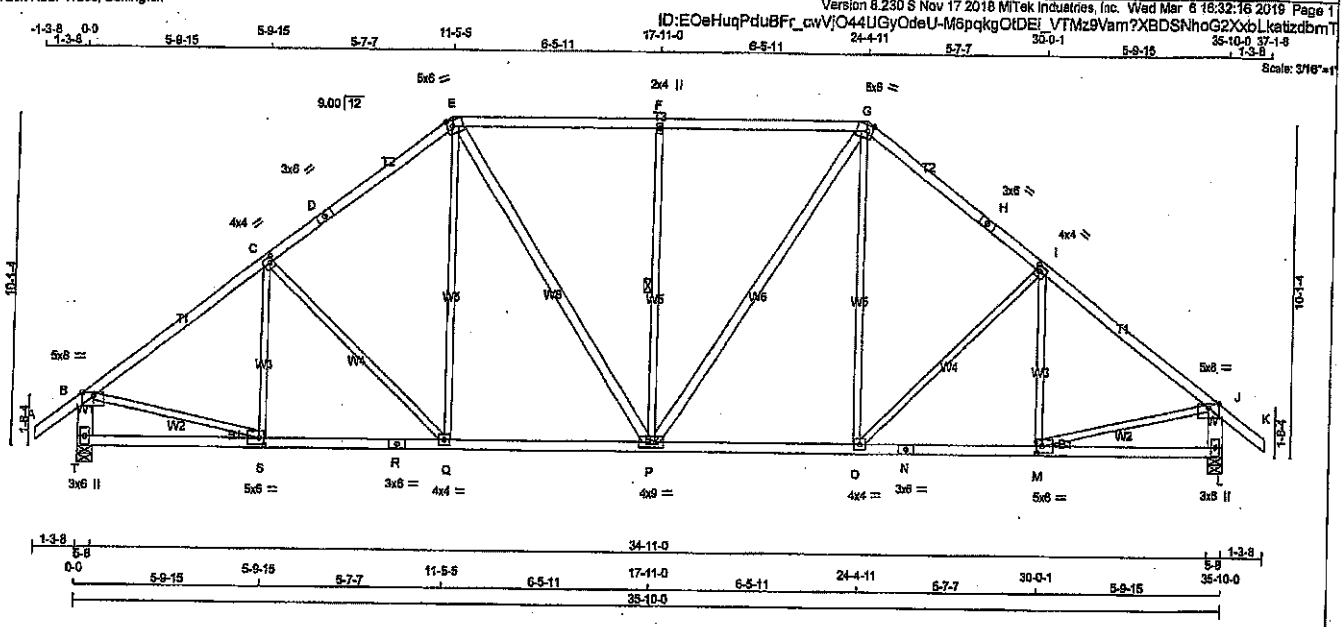
DWG NO. TAM 7190 5104
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685.	T7	3	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:16 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-M6pqkgOtDEi_VTMz9Vam?XBDSNhoG2XxbLkatizdbmT



TOTAL WEIGHT = 3 X 176 = 527 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - K	2x4	DRY	No.2	SPF
T - B	2x6	DRY	No.2	SPF
L - J	2x6	DRY	No.2	SPF
T - R	2x4	DRY	No.2	SPF
R - N	2x4	DRY	No.2	SPF
N - L	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
E - P	2x4	DRY	No.2	SPF
P - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-p	MT20	5.0	8.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	Edge	2.00
F	TMW+w	MT20	2.0	4.0		
G	TTWW-m	MT20	5.0	6.0	Edge	2.00
H	TS-t	MT20	3.0	6.0		
I	TMWW-t	MT20	4.0	4.0	2.00	1.50
J	TMWW-p	MT20	5.0	8.0	Edge	
L	BMV1+p	MT20	3.0	6.0		
M	BMWW-t	MT20	5.0	6.0	2.50	2.00
N	BS-t	MT20	3.0	6.0		
O	BMWW-t	MT20	4.0	4.0		
P	BMWWW-t	MT20	4.0	9.0		
Q	BMWW-t	MT20	4.0	4.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0	2.50	2.00
T	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
T	2660	0	2660	0	0	5-8	5-8
L	2660	0	2660	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
T	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0
L	1979	1120 / 0	376 / 0	0 / 0	0 / 0	482 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.55 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-P.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B- C	-2836 / 0	-102.1	-102.1	0.63 (1)	3.56	
C- D	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
D- E	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
E- F	-2356 / 0	-102.1	-102.1	0.65 (1)	3.70	
F- G	-2356 / 0	-102.1	-102.1	0.65 (1)	3.70	
G- H	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
H- I	-2560 / 0	-102.1	-102.1	0.57 (1)	3.77	
I- J	-2836 / 0	-102.1	-102.1	0.63 (1)	3.56	
J- K	0 / 42	-102.1	-102.1	0.14 (1)	10.00	
T- B	-2567 / 0	0.0	0.0	0.17 (1)	6.48	
L- J	-2567 / 0	0.0	0.0	0.17 (1)	6.48	
T- S	0 / 0	-38.5	-38.5	0.23 (3)	10.00	
S- R	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
R- Q	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
Q- P	0 / 2016	-38.5	-38.5	0.52 (2)	10.00	
P- O	0 / 2016	-38.5	-38.5	0.52 (2)	10.00	
O- N	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
N- M	0 / 2302	-38.5	-38.5	0.53 (2)	10.00	
M- L	0 / 0	-38.5	-38.5	0.23 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
S- C	-229 / 100	0.12 (1)
C- Q	-407 / 0	0.54 (1)
Q- E	0 / 529	0.12 (2)
E- P	0 / 636	0.10 (1)
P- F	-808 / 0	0.56 (1)
P- G	0 / 636	0.10 (1)
Q- G	0 / 529	0.12 (2)
Q- I	-407 / 0	0.54 (1)
M- I	-229 / 100	0.12 (1)
B- S	0 / 2350	0.53 (1)
M- J	0 / 2350	0.53 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.12")
ALLOWABLE DEFL.(TL)= L/360 (1.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.21")

CSI: TC=0.65/1.00 (F-G:1) , BC=0.53/1.00 (Q-S:2) , WB=0.56/1.00 (F-P:1) , SSI=0.32/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	766	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (G) (INPUT = 0.90)
JSI METAL= 0.69 (R) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009924
PROVINCE OF ONTARIO

RECEIVED
AUG 28 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

DWG NO. TAM 7905705
STRUCTURAL
COMPONENT ONLY

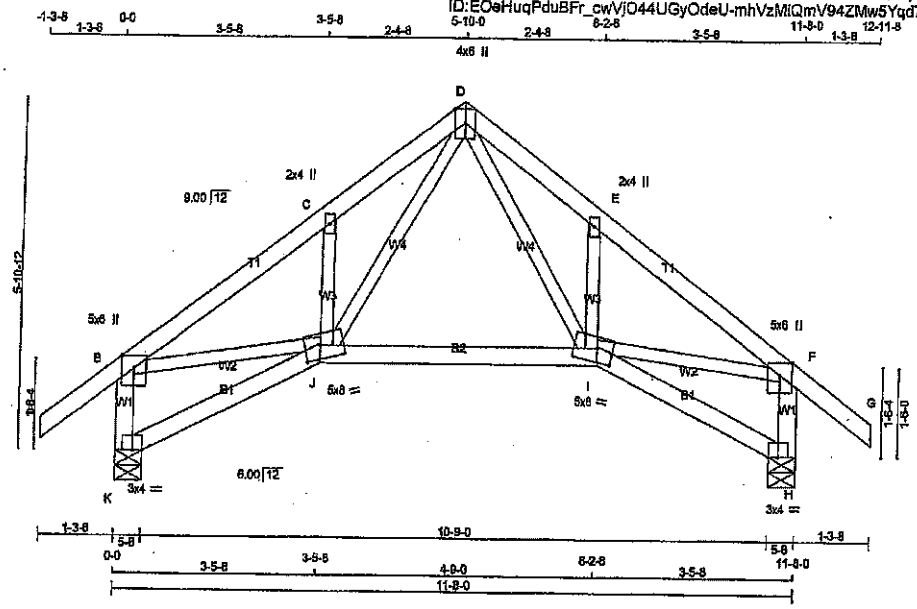
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401685	T9S	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:19 2019 Page 1

ID:EOsHuqPduBFr_cwVjO44UGyOdeU-mhVzMiQmV94ZMw5Yqd7Td9prObmbTTsNHJyEU1zdbmQ

Scale = 1:35.5



TOTAL WEIGHT = 2 X 53 = 107 lb

[MIF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW+p	MT20	5.0	6.0	Edge	
C	TMW+w	MT20	2.0	4.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMW+w	MT20	2.0	4.0		
F	TMWW+p	MT20	5.0	6.0	Edge	
H	BVM1-p	MT20	3.0	4.0		
I	BBWWW-m	MT20	5.0	8.0	2.75	3.25
J	BBWWW-m	MT20	5.0	8.0	2.75	3.25
K	BVM1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
K	961	0	961	0	0	5-8	5-8
H	961	0	961	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	710	419 / 0	123 / 0	0 / 0	0 / 0	168 / 0	0 / 0
H	710	419 / 0	123 / 0	0 / 0	0 / 0	168 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-1126 / 0	-102.1	-102.1	0.16 (1)	5.81
C- D	-1167 / 0	-102.1	-102.1	0.11 (1)	5.80
D- E	-1167 / 0	-102.1	-102.1	0.11 (1)	5.80
E- F	-1126 / 0	-102.1	-102.1	0.16 (1)	5.81
F- G	0 / 42	-102.1	-102.1	0.14 (1)	10.00
K- B	-894 / 0	0.0	0.0	0.10 (1)	7.81
H- F	-894 / 0	0.0	0.0	0.10 (1)	7.81
K- J	0 / 0	-38.5	-38.5	0.11 (3)	10.00
J- I	0 / 563	-38.5	-38.5	0.26 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.11 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- I	0 / 699	0.16 (1)
I- E	-362 / 0	0.06 (1)
J- D	0 / 699	0.16 (1)
J- C	-362 / 0	0.06 (1)
B- J	0 / 917	0.21 (1)
I- F	0 / 917	0.21 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.16/1.00 (B-C:1) , BC=0.26/1.00 (I-J:2) , WB=0.21/1.00 (B-J:1) , SSI=0.13/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

USI GRIP= 0.75 (H) (INPUT = 0.60)
USI METAL= 0.52 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. GALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 7905709
STRUCTURAL
COMPONENT ONLY

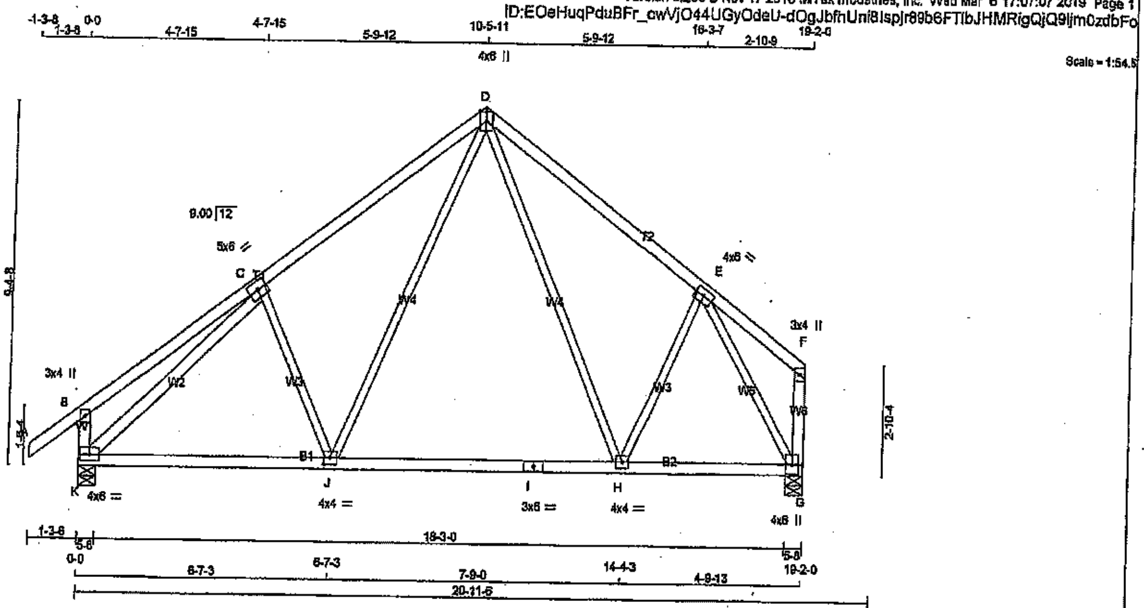
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401687'	T18	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:07:07 2019 Page 1

ID:EOeHuqPduBFr_owVjO44UGyOdeU-dOgJbfhUni8lsplr89b6FTlbJHMRigQjQ9ljm0zdbFo

Scale = 1:54.5



TOTAL WEIGHT = 2 X 90 = 180 lb

[MiIF]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
K - B	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
K - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
K - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0		
D	TTWW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	6.0		
F	TMV+p	MT20	3.0	4.0		
G	BMVW1+p	MT20	4.0	6.0		
H	BMWW-t	MT20	4.0	4.0		
I	BS-t	MT20	3.0	6.0		
J	BMWW-t	MT20	4.0	4.0	2.00	1.75
K	BMVW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
K	1488	0	1488	0	0	5-8	5-8
G	1347	0	1347	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0
G	1007	556 / 0	201 / 0	0 / 0	0 / 0	249 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.22 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	0 / 47	-102.1	-102.1	0.49 (1)	10.00
C- D	-1221 / 0	-102.1	-102.1	0.46 (1)	5.22
D- E	-1012 / 0	-102.1	-102.1	0.44 (1)	5.64
E- F	0 / 67	-102.1	-102.1	0.44 (1)	10.00
K- B	-292 / 0	0.0	0.0	0.03 (1)	7.81
G- F	-32 / 0	0.0	0.0	0.00 (1)	7.81
K- J	0 / 1082	-38.5	-38.5	0.50 (2)	10.00
J- I	0 / 695	-36.5	-38.5	0.45 (2)	10.00
I- H	0 / 695	-38.6	-38.5	0.45 (2)	10.00
H- G	0 / 760	-38.5	-38.5	0.36 (2)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
C- J	-316 / 20	0.14 (1)
J- D	0 / 649	0.15 (1)
D- H	0 / 247	0.08 (2)
H- E	0 / 164	0.04 (3)
K- C	-1542 / 0	0.80 (1)
E- G	-1465 / 0	0.68 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.50/1.00 (J-K:2) , WB=0.80/1.00 (C-K:1) , SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	786	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (E) (INPUT = 0.90)
JSI METAL= 0.42 (I) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

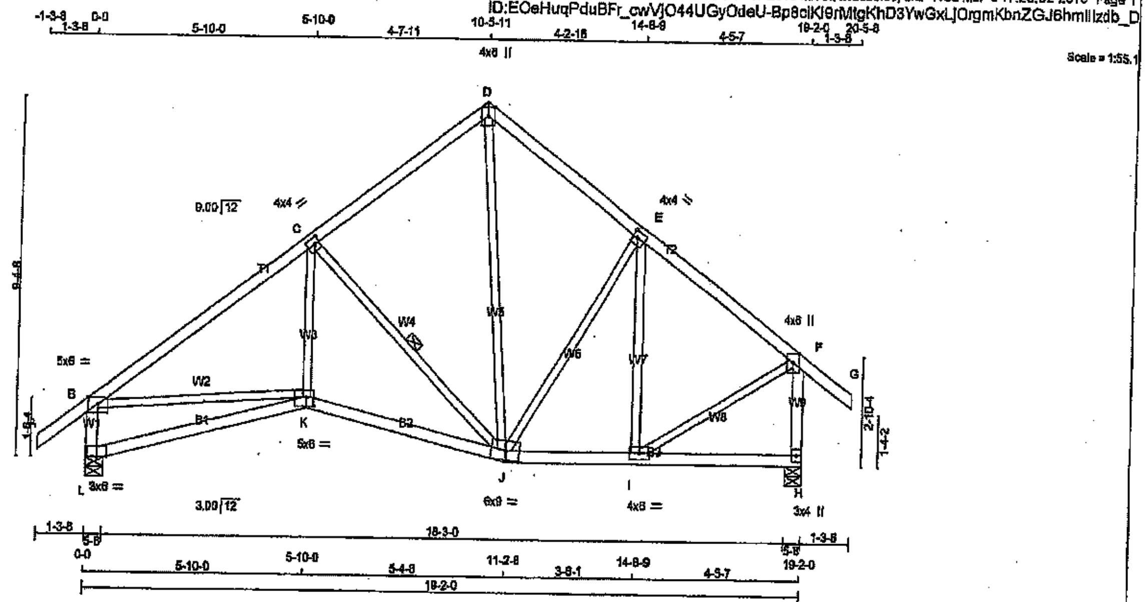
DWG NO. TAM 7905722
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401690	T18S	1	1	TRUSS DESC.	

Temarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:25:52 2019 Page 1
ID:EOeHuqPduBFr_cwVjO44UGyOdeU-Bp8clKl9rMtgKhD3YwGxLjOrgmKbnZGJ6hmIIzdb_D

Scale = 1:55.1



TOTAL WEIGHT = 92 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - K	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0	2.00	1.50
D	TTW+p	MT20	4.0	6.0	Edge	
E	TMWW-t	MT20	4.0	4.0	2.00	1.60
F	TMVW+p	MT20	4.0	6.0	Edge	
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	4.0	6.0		
J	BBWWW-m	MT20	6.0	9.0	2.75	4.25
K	BBWW-p	MT20	5.0	6.0	2.75	2.25
L	BVM1-p	MT20	3.0	6.0	Edge	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
L	1488	0	1488	0	0	5-8	5-8
H	1488	0	1488	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
L	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0
H	1104	637 / 0	201 / 0	0 / 0	0 / 0	266 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.61 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 42	-102.1	-102.1	0.14 (1)	10.00
B- C	-1686 / 0	-102.1	-102.1	0.47 (1)	4.61
C- D	-860 / 0	-102.1	-102.1	0.37 (1)	6.04
D- E	-842 / 0	-102.1	-102.1	0.25 (1)	6.08
E- F	-1014 / 0	-102.1	-102.1	0.26 (1)	5.91
F- G	0 / 42	-102.1	-102.1	0.14 (1)	10.00
L- B	-1376 / 0	0.0	0.0	0.15 (1)	6.93
H- F	-1418 / 0	0.0	0.0	0.22 (1)	6.65
L- K	0 / 0	-38.5	-38.5	0.30 (3)	10.00
K- J	0 / 1419	-38.5	-38.5	0.44 (2)	10.00
J- I	0 / 836	-38.5	-38.5	0.24 (2)	10.00
I- H	0 / 0	-38.5	-38.5	0.14 (3)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- C	0 / 602	0.14 (2)
C- J	-1019 / 0	0.40 (1)
D- J	0 / 724	0.16 (1)
J- E	-212 / 0	0.19 (1)
I- E	-286 / 29	0.17 (1)
B- K	0 / 1377	0.31 (1)
I- F	0 / 962	0.22 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.64")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.10")
ALLOWABLE DEFL.(TL)= L/360 (0.64")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.16")

CSI: TC=0.47/1.00 (B-C:1) , BC=0.44/1.00 (J-K:2) , WB=0.40/1.00 (C-J:1) , SSI=0.22/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.61 (L) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009624
PROVINCE OF ONTARIO

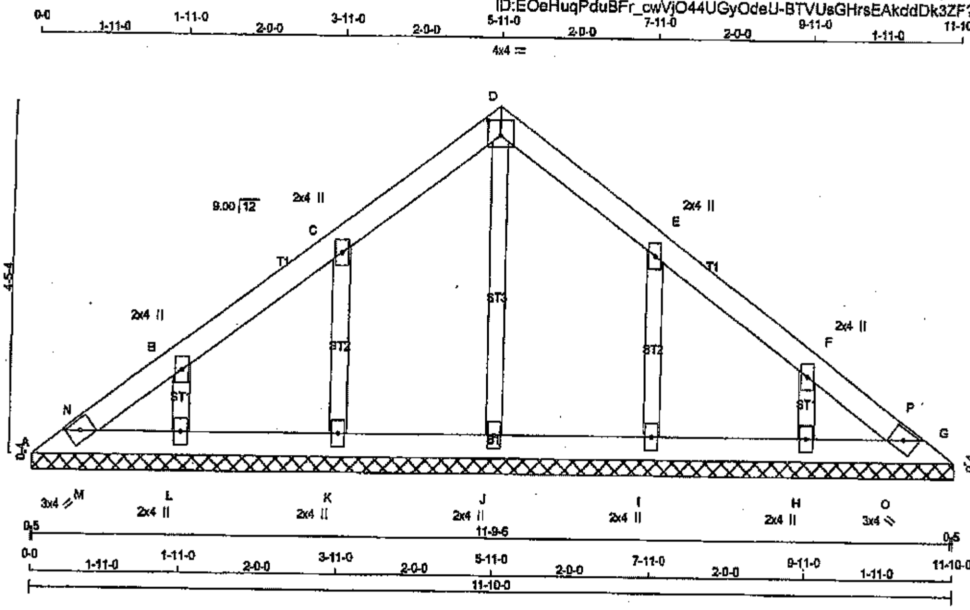
DWG No. 17905734
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401690	V30	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:24:27 2019 Page 1
ID:EOeHuqPduBFr_cwVjO44UGyOdeU-BTVUsGHrsEAkddDk3ZF?OoMRaVPxNDtoZzglGkzdb?Y

Scale = 1:26.7



TOTAL WEIGHT = 37 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, E, F						
B	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
G	TBM1-h	MT20	3.0	4.0		
H, I, J, K, L						
H	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	118	0	118	0	0	11-10-0 (11-9-8)	1-10-0
G	118	0	118	0	0	11-10-0 (11-9-8)	1-10-0
J	239	0	239	0	0	11-10-0 (11-9-8)	1-10-0
K	301	0	301	0	0	11-10-0 (11-9-8)	1-10-0
L	290	0	290	0	0	11-10-0 (11-9-8)	1-10-0
I	301	0	301	0	0	11-10-0 (11-9-8)	1-10-0
H	290	0	290	0	0	11-10-0 (11-9-8)	1-10-0

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	87	52 / 0	14 / 0	0 / 0	0 / 0	20 / 0	0 / 0
G	87	52 / 0	14 / 0	0 / 0	0 / 0	20 / 0	0 / 0
J	183	89 / 0	45 / 0	0 / 0	0 / 0	49 / 0	0 / 0
K	223	128 / 0	41 / 0	0 / 0	0 / 0	54 / 0	0 / 0
L	217	118 / 0	45 / 0	0 / 0	0 / 0	54 / 0	0 / 0
I	223	128 / 0	41 / 0	0 / 0	0 / 0	54 / 0	0 / 0
H	217	118 / 0	45 / 0	0 / 0	0 / 0	54 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, G, J, K, L, I, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- N	-46 / 0	-102.1	-102.1	0.02 (1)	6.25
N- B	-12 / 6	-102.1	-102.1	0.04 (1)	6.25
B- C	-16 / 2	-102.1	-102.1	0.06 (1)	6.25
C- D	-23 / 2	-102.1	-102.1	0.06 (1)	6.25
D- E	-23 / 2	-102.1	-102.1	0.06 (1)	6.25
E- F	-16 / 2	-102.1	-102.1	0.06 (1)	6.25
F- P	-12 / 8	-102.1	-102.1	0.04 (1)	6.25
P- G	-46 / 0	-102.1	-102.1	0.02 (1)	6.25
A- M	-1 / 25	-38.5	-38.5	0.04 (1)	10.00
M- L	0 / 31	-38.5	-38.5	0.04 (1)	10.00
L- K	-2 / 16	-38.5	-38.5	0.03 (2)	10.00
K- J	-3 / 8	-38.5	-38.5	0.02 (2)	10.00
J- I	-3 / 6	-38.5	-38.5	0.02 (2)	10.00
I- H	-2 / 16	-38.5	-38.5	0.03 (2)	10.00
H- O	0 / 31	-38.5	-38.5	0.04 (1)	10.00
O- G	-1 / 25	-38.5	-38.5	0.04 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J- D	-159 / 0	0.04 (1)
K- C	-231 / 0	0.04 (1)
L- B	-184 / 0	0.03 (1)
I- E	-231 / 0	0.04 (1)
H- F	-184 / 0	0.03 (1)
M- N	-31 / 12	0.00 (1)
O- P	-31 / 12	0.00 (1)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009029
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (C-D:1) , BC=0.04/1.00 (H-O:1) , WB=0.04/1.00 (D-J:1) , SSI=0.08/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.17 (C) (INPUT = 0.90)
JSI METAL= 0.12 (E) (INPUT = 1.00)

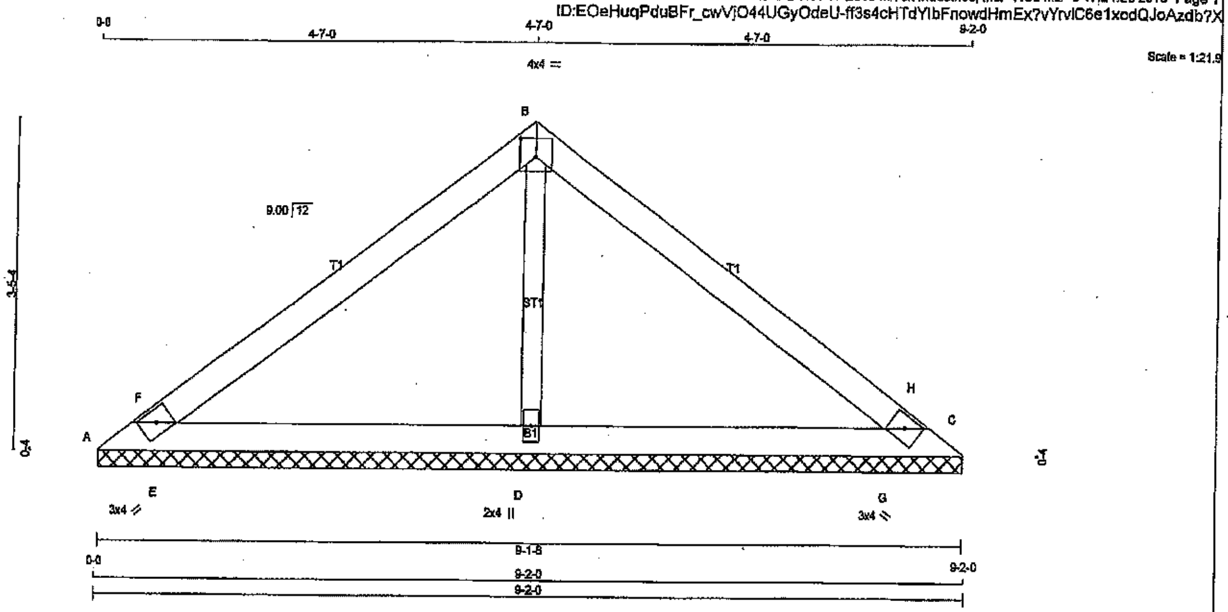
DWG NO. TAM 17905731
STRUCTURAL
[illegible]

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401690	V31	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:24:28 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-ff3s4cHTdYIbFnowdHmEx?vYrvlC6e1xodQJoAzdb?X



TOTAL WEIGHT = 25 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2 SPF
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	54	0	54	0	0	9-1-6	9-1-6
C	54	0	54	0	0	9-1-6	9-1-6
D	1174	0	1174	0	0	9-1-6	9-1-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	40	22 / 0	9 / 0	0 / 0	0 / 0	10 / 0	0 / 0
C	40	22 / 0	9 / 0	0 / 0	0 / 0	10 / 0	0 / 0
D	877	486 / 0	174 / 0	0 / 0	0 / 0	217 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- F	0 / 419	-102.1	-102.1	0.15 (1)	10.00
F- B	0 / 413	-102.1	-102.1	0.28 (1)	10.00
B- H	0 / 413	-102.1	-102.1	0.28 (1)	10.00
H- C	0 / 419	-102.1	-102.1	0.15 (1)	10.00
A- E	-382 / 0	-38.5	-38.5	0.19 (1)	6.25
E- D	-337 / 0	-38.5	-38.5	0.23 (1)	6.25
D- G	-337 / 0	-38.5	-38.5	0.23 (1)	6.25
G- C	-382 / 0	-38.5	-38.5	0.19 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- B	-867 / 0	0.17 (1)
E- F	-253 / 23	0.00 (1)
G- H	-253 / 23	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (B-H:1) , BC=0.23/1.00 (D-E:1) , WB=0.17/1.00 (B-D:1) , SSI=0.16/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (B) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. G. ALVES
100009894
PROVINCE OF ONTARIO

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

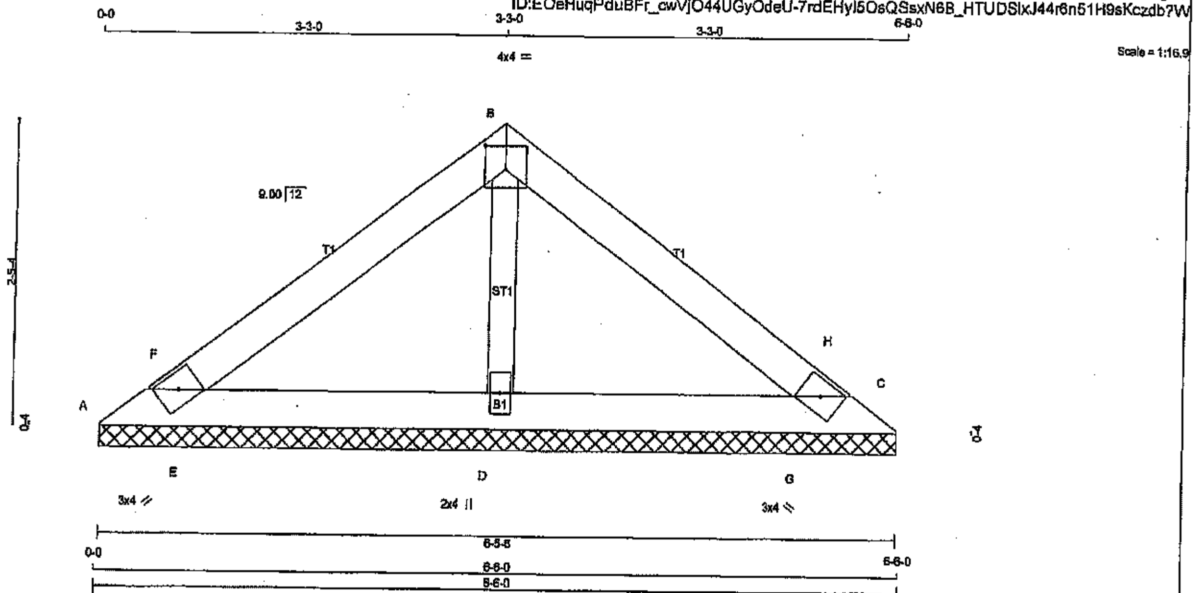
BUILDING DIVISION 17905732
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
401690	V32	1	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 17:24:29 2019 Page 1

ID:EOeHuqPduBFr_cwVjO44UGyOdeU-7rdEHyl5OsQSsxN6B_HTUDSIxJ44r6n51H9sKczdb?W



TOTAL WEIGHT = 17 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TTW-p	MT20	4.0	4.0	2.25	2.00
C	TBM1-h	MT20	3.0	4.0		
D	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
A	96	0	96	0	0	6-5-6	6-5-6
C	96	0	96	0	0	6-5-6	6-5-5
D	715	0	715	0	0	6-5-6	6-5-6

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
		MAX./MIN. COMPONENT REACTIONS					
A	71	41 / 0	13 / 0	0 / 0	0 / 0	17 / 0	0 / 0
C	71	41 / 0	13 / 0	0 / 0	0 / 0	17 / 0	0 / 0
D	535	292 / 0	109 / 0	0 / 0	0 / 0	133 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- F	0 / 184	-102.1	-102.1	0.04 (1)	10.00
F- B	0 / 193	-102.1	-102.1.	0.13 (1)	10.00
B- H	0 / 193	-102.1	-102.1	0.13 (1)	10.00
H- C	0 / 184	-102.1	-102.1	0.04 (1)	10.00
A- E	-162 / 0	-38.5	-38.5	0.11 (1)	6.25
E- D	-157 / 0	-38.5	-38.5	0.12 (1)	6.25
D- G	-157 / 0	-38.5	-38.5	0.12 (1)	6.25
G- C	-162 / 0	-38.5	-38.5	0.11 (1)	6.25

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
D- B	-482 / 0	0.07 (1)
E- F	-148 / 12	0.00 (1)
G- H	-148 / 12	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (B-H:1) , BC=0.12/1.00 (D-E:1) , WB=0.07/1.00 (B-D:1) , SSI=0.10/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	616	354	1667	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.43 (B) (INPUT = 0.90)
JSI METAL= 0.14 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009084
PROVINCE OF ONTARIO

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION

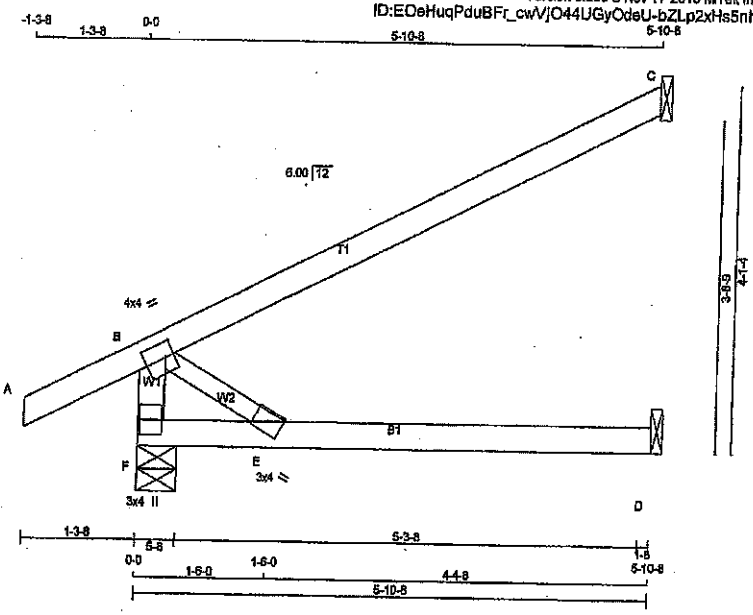
DWG NO. TAM 17905733
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401685	J1	25	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed Mar 6 16:32:08 2019 Page 1
ID:EOeHuqPduBFr_cwVjO44UGyOdeU-bZLp2xHs5nh7XElRhqRugrssK90JOYLmI5o9bAzdbmb

Scale = 1:23.6



TOTAL WEIGHT = 25 X 18 = 452 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	4.0	4.0	2.00	1.25
E	BMW+w	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	552	0	552	0	0	5-8	5-8
C	300	0	300	0	0	1-8	1-8
D	113	0	144	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	404	249 / 0	62 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	206	171 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-438 / 0	0.0	0.0	0.04 (1)	7.81	
A- B	0 / 31	-102.1	-102.1	0.13 (1)	10.00	
B- C	0 / 0	-102.1	-102.1	0.60 (1)	10.00	
F- E	0 / 0	-38.5	-38.5	0.24 (3)	10.00	
E- D	0 / 0	-38.5	-38.5	0.31 (3)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 925 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 555 (0.13")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.31/1.00 (D-E:3) , WB=0.00/1.00 (B-E:1) , SSI=0.21/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (B) (INPUT = 0.90)
JSI METAL= 0.07 (B) (INPUT = 1.00)

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO

LICENSED PROFESSIONAL ENGINEER
23/04/19
H. J. GALVES
100009029
PROVINCE OF ONTARIO

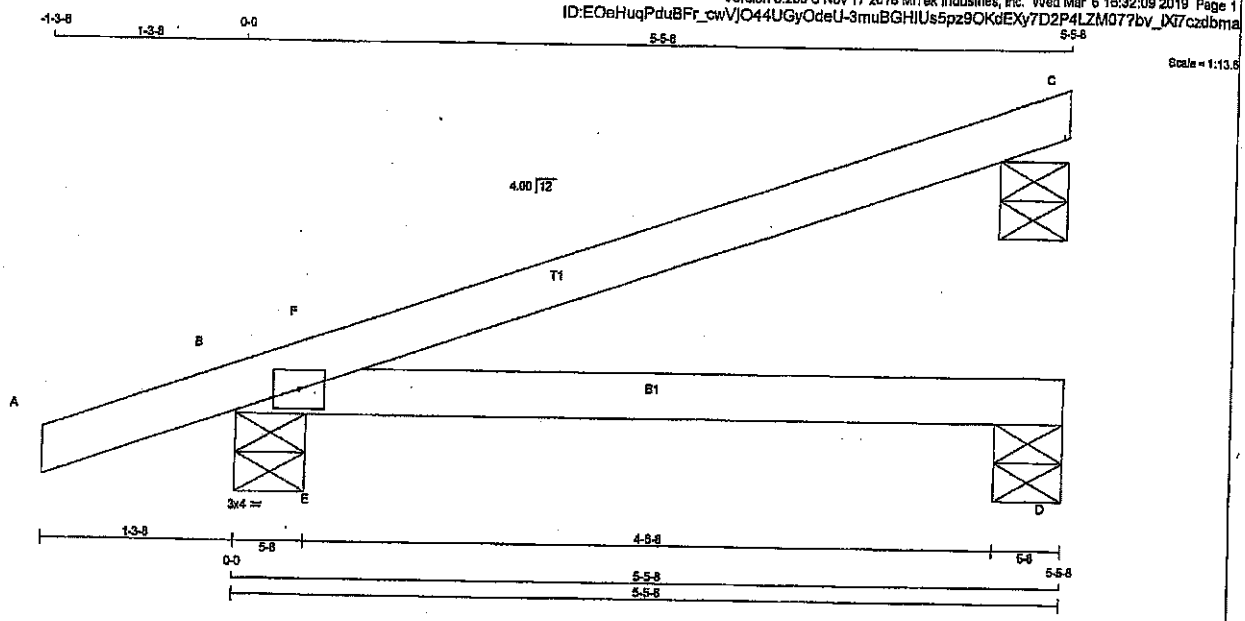
DWG NO. TAM 7905711
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401685	J3	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MiTek Industries, Inc. Wed Mar 6 16:32:09 2019 Page 1
ID:EOeHuqPduBFr_cwVJO44UGyOdeU-3muBGHIUs5pz9OKdEXy7D2P4LZM077bv_IX7czdbma

Scale = 1:13.6



TOTAL WEIGHT = 6 X 14 = 87 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
B	463	0	463	0	0	5-8	5-8
D	85	0	85	0	0	5-8	5-8
C	242	0	242	0	0	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	323	236 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	63	25 / 0	0 / 0	0 / 0	0 / 0	38 / 0	0 / 0
C	167	133 / 0	0 / 0	0 / 0	0 / 0	33 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 20	-102.1	-102.1	0.13 (1)	10.00
B- F	-17 / 17	-102.1	-102.1	0.06 (4)	6.25
F- C	-2 / 2	-102.1	-102.1	0.39 (1)	10.00
B- E	0 / 0	-17.5	-17.5	0.28 (1)	10.00
E- D	0 / 0	-17.5	-17.5	0.28 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- F	-331 / 7	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.0	PSF
TOTAL LOAD	=	42.0	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 564 (0.12")

CSI: TC=0.39/1.00 (C-F:1) , BC=0.28/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.27/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.90)
JSI METAL= 0.09 (B) (INPUT = 1.00)

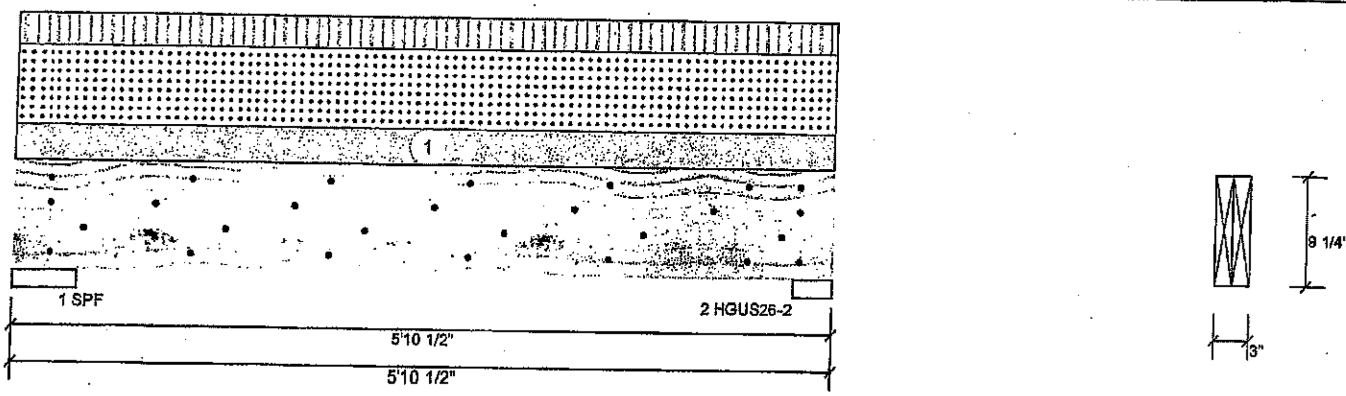
LICENSED PROFESSIONAL ENGINEER
03/06/19
M. J. G. ALVES
100100926
PROVINCE OF ONTARIO

DWG NO. TAM 77905112
STRUCTURAL
COMPONENT ONLY

isDesign™	Client: Project: Address:	Date: 3/6/2019 Designer: Job Name: 200620 Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	382	432	964	0
2	361	408	910	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	23%	540 / 1828	2368	L	1.25D+1.5S +L
2 - HGUS...	3.500"	35%	510 / 1727	2237	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2700 ft-lb	3' 1/4"	6039 ft-lb	0.447 (45%)	1.25D+1.5S +L	L
Unbraced	2700 ft-lb	3' 1/4"	5223 ft-lb	0.517 (52%)	1.25D+1.5S +L	L
Shear	2057 lb	1'2"	3984 lb	0.516 (52%)	1.25D+1.5S +L	L
LL Defl inch	0.024 (L/2828)	3' 1/4"	0.175 (L/360)	0.140 (14%)	S+0.5L	L
TL Defl inch	0.033 (L/1913)	3' 1/4"	0.175 (L/360)	0.190 (19%)	D+S+0.5L	L

Design Notes

1. Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2. Refer to last page of calculations for fasteners required for specified loads.
3. Girders are designed to be supported on the bottom edge only.
4. Top braced at bearings.
5. Bottom braced at bearings.
6. Lateral slenderness ratio based on single ply width.

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100000...
PROVINCE OF ONTARIO

DWG NO. TAM 19051/3
STRUCTURAL
COMPONENT ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		11-0-0	Near Face	13 PSF	11.5 PSF	29 PSF	0 PSF	

RECEIVED
AUG 2 8 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO.______

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

Version 18.80.245 Powered by iStruct™

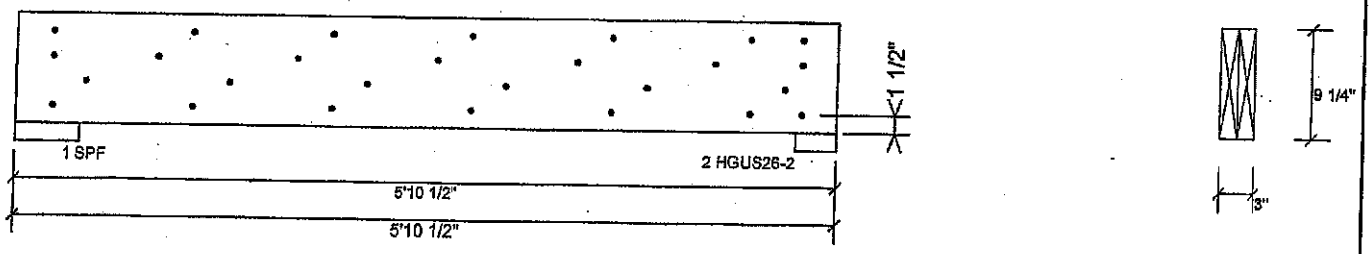
isDesign™

Client:
Project:
Address:

Date: 3/6/2019
Designer:
Job Name: 200620
Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	86.4 %
Load	391.9 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

LICENSED PROFESSIONAL ENGINEER
03/06/19
H. J. G. ALVES
100009
PROVINCE OF ONTARIO

DWG NO. TAM T1905113
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (905) 336-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

This design is valid until 12/11/2021

TECHNICAL BULLETIN

HUS/LJS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

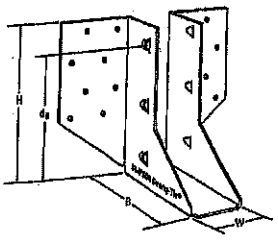
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15% No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

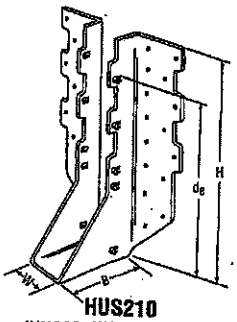
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

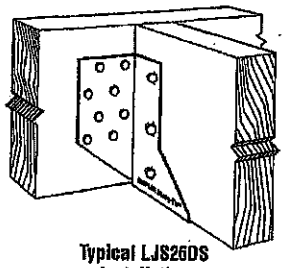
- See current catalogue for options



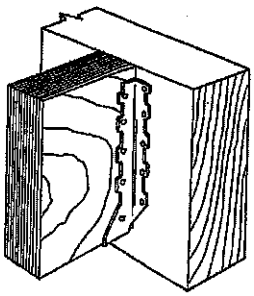
LJS26DS



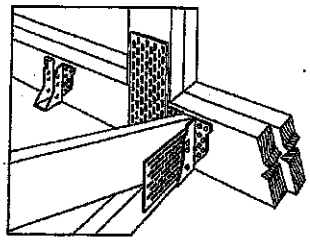
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS Installation



Typical HUS Installation

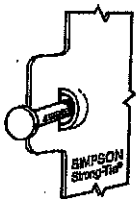


Typical HUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_e[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
LJS26DS	18	1 9/16	5	3½	4⅝	16-16d	6-16d	2055	4265	1460	4115
HUS26	16	1⅝	5⅜	3	3 15/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	16	1⅝	7 3/32	3	6 3/32	22-16d	8-16d	3605	5365	2675	4345
HUS210	16	1⅝	9 3/32	3	7 31/32	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	16	1 13/16	9	3	8	30-16d	10-16d	4505	6460	4010	5200

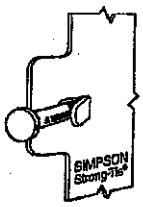
1. d_e is the distance from the seat of the hanger to the highest joist nail.

RECEIVED
AUG 28 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

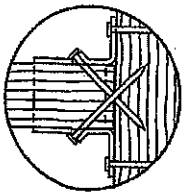


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
01/13/2015
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

©2015 Simpson Strong-Tie Company Inc.

T-SPECHUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON Strong-Tie®

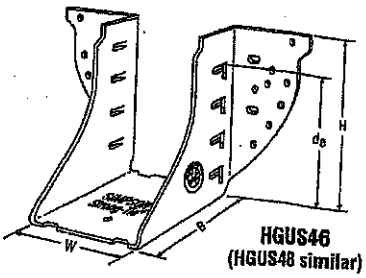
All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

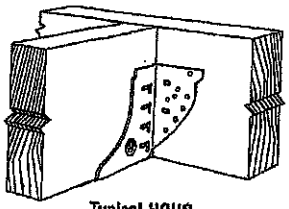
- Factored resistances are in accordance with CSA O86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.



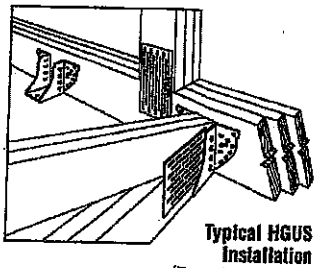
HGUS46
(HGUS48 similar)

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications



Typical HGUS Installation



Typical HGUS Installation (Truss Designer to provide fastener quantity for connecting multiple members together)

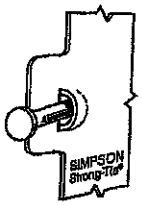
OPTIONS:

- See current catalogue for options

Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
								D.Fir-L		S-P-F	
		W	H	B	d_c[1]	Face	Joist	Uplift (K_D=1.15)	Normal (K_D=1.00)	Uplift (K_D=1.15)	Normal (K_D=1.00)
HGUS48	12	3⅝	7⅛	4	6 1/16	36-16d	12-16d	6070	12980	4310	9215
HGUS410	12	3⅝	9	4	8 1/16	46-16d	16-16d	6840	14645	4855	10400
HGUS412	12	3⅝	10 7/16	4	10 1/16	56-16d	20-16d	7640	14995	5425	10645
HGUS414	12	3⅝	12 7/16	4	11 1/16	66-16d	22-16d	10130	16400	7195	11645
HGUS5.50/8	12	5½	6 15/16	4	6⅛	36-16d	12-16d	6070	12980	4310	9215
HGUS5.50/10	12	5½	8 15/16	4	8⅛	46-16d	16-16d	6840	14645	4855	10400
HGUS5.50/12	12	5½	10½	4	10⅛	56-16d	20-16d	7640	14995	5425	10645
HGUS5.50/14	12	5½	12½	4	11⅛	66-16d	22-16d	10130	16400	7195	11645
HGUS7.25/8	12	7¼	7 7/32	4	6 15/32	36-16d	12-16d	6070	12980	4310	9215
HGUS7.25/10	12	7¼	8⅝	4	8¼	46-16d	16-16d	6840	15760	4855	11190
HGUS7.25/12	12	7¼	10⅝	4	10¼	56-16d	20-16d	7640	16110	5425	11435
HGUS7.25/14	12	7¼	12⅝	4	11¼	66-16d	22-16d	10130	18200	7195	12920

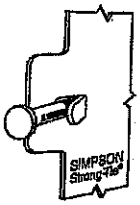
1. d_6 is the distance from the seat of the hanger to the highest joist nail.

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.

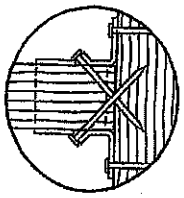


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.

LICENSED PROFESSIONAL ENGINEER
01/13/2015
B. D. BUNTING
PROVINCE OF ONTARIO

LIMIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.stronglie.com.

© 2015 Simpson Strong-Tie Company Inc.

800-999-5099
www.stronglie.com

Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

LICENSED PROFESSIONAL ENGINEER
06/11/19
H.J.G. ALVES
PROVINCE OF ONTARIO

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

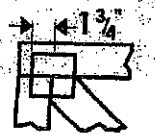
7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

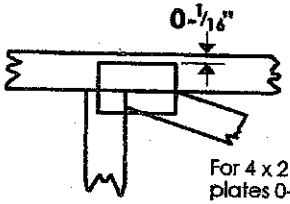
T-1900218 Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

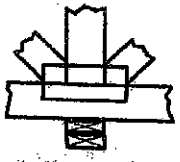
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

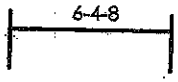
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses

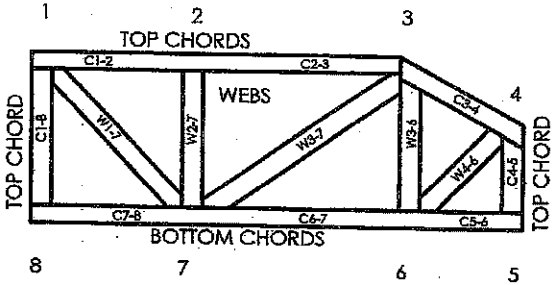
DSB-89: Design Standard for Bracing.

BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

© 2007 MiTek® All Rights Reserved

MiTek®

POWER TO PERFORM.™

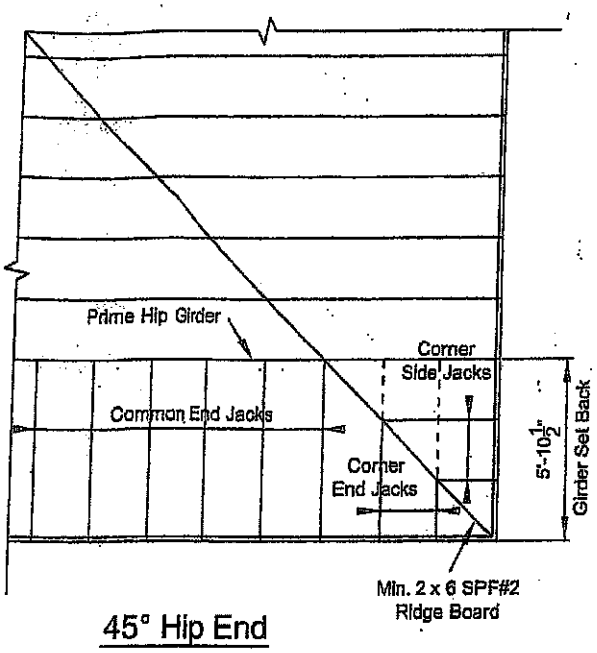
MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.

RECEIVED
AUG 2 8 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



45° Hip End

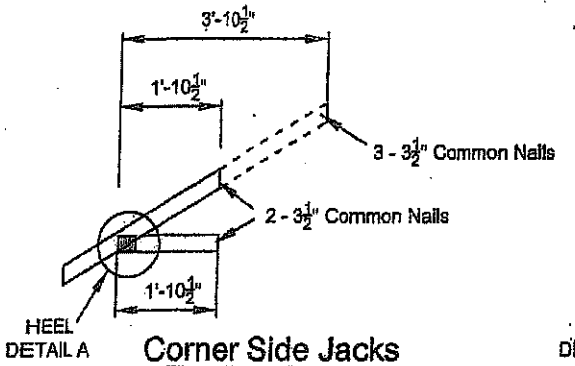
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

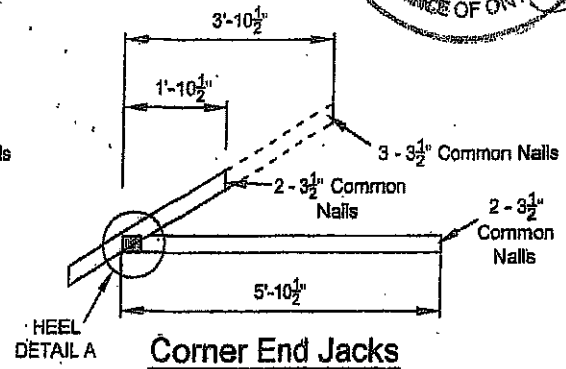
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

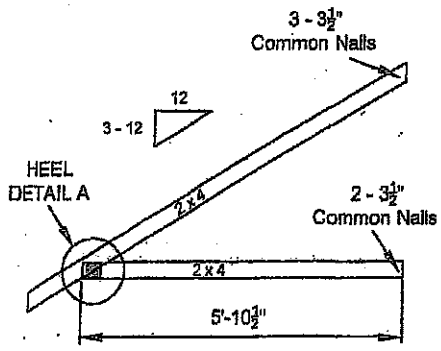
LICENSED PROFESSIONAL ENGINEER
06/11/10
H.J.G. ALVES
PROVINCE OF ONTARIO



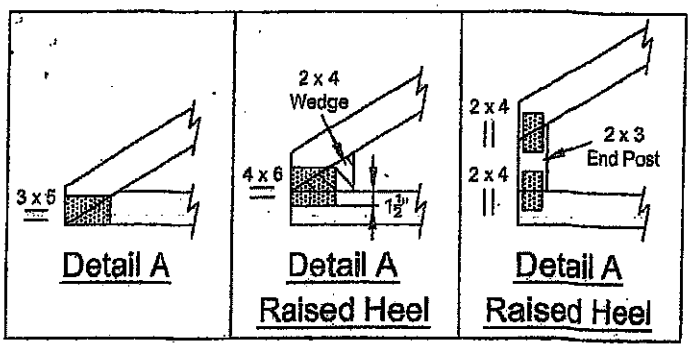
Corner Side Jacks



Corner End Jacks



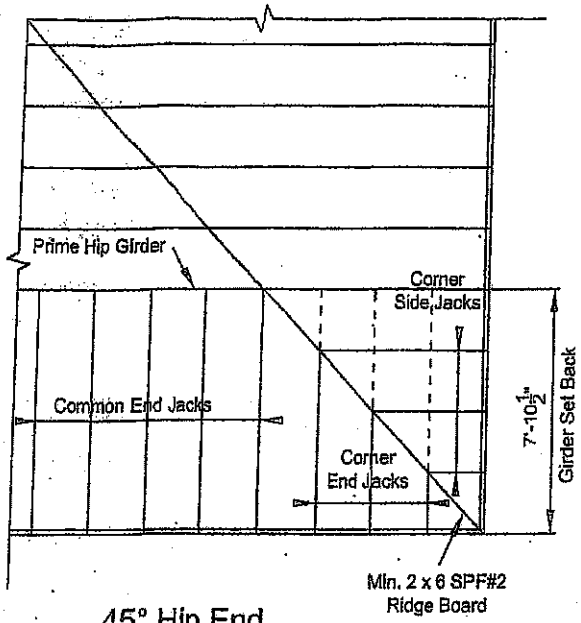
Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

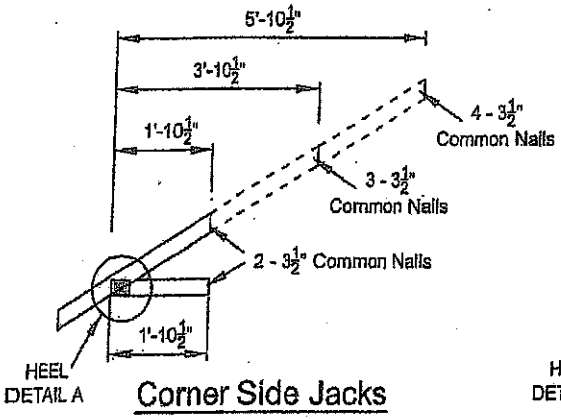
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

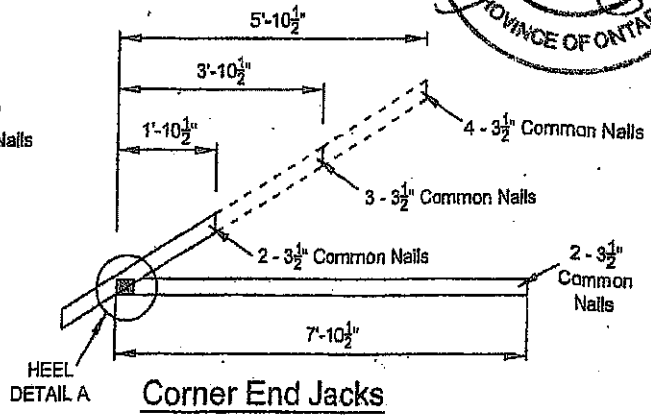
DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F

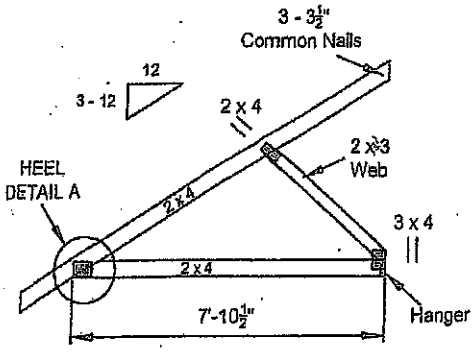
LICENSED PROFESSIONAL ENGINEER
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



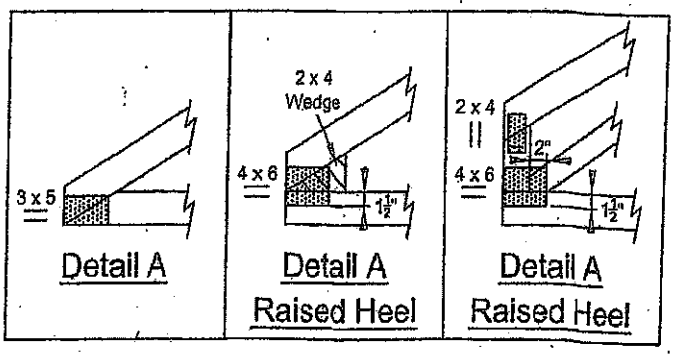
Corner Side Jacks



Corner End Jacks



Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217