

ASPHALT SHINGLES
12" FINISH O.H
R.T.M.C.
2x6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:

HUS26-2 -(XX)
LJS26DS -(V)
LUS24 -(O)

DENOTE:

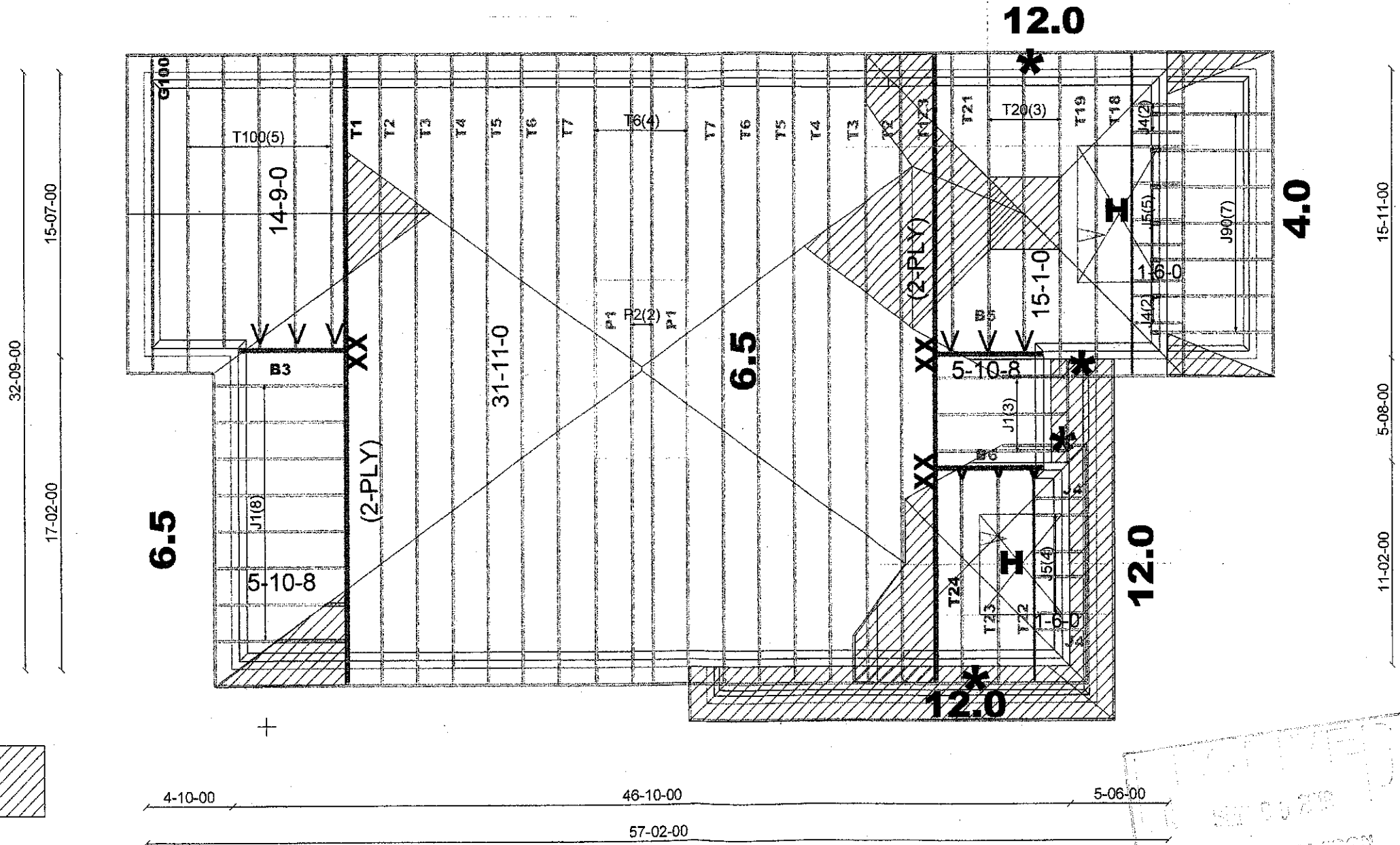
ALL B- 2-2X10 (FLUSH)

* -24" RAISED
FASCIA ONLY
H -24" RAISED CEILING

DENOTES:
CONVENTIONAL
FRAMING

9/12 PITCHES (TYP.)
UNLESS NOTED

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



ALL CONVENTIONAL ROOF FRAMING TO CONFORM TO PART9 OF THE OBC.LATEST EDITION
ROOF RAFTERS THAT MEET OR CROSS OVER TRUSSES ARE TO BE 2"x4"SPF@24"o.c.
WITH A 2"x4"SPF VERTICAL POST TO THE TRUSS UNDER AT EACH CROSS POINT.
POSTS LONGER THAN 6' TO BE Laterally BRACED SO THAT THE DISTANCE
BETWEEN END POINTS AND BETWEEN ROWS OF BRACING DOES NOT EXCEED 6'.

DESIGN LOADS:

GROUND SNOW LOAD $S_s = 1.8 \text{ kPa}$
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

M12257



Job Track: 50120
Plan Log: 200598
Layout ID: 401609

Builder / Location:
GREEN PARK HOMES / Caledon
Project: LAMBERTS LANE HOME CORP.
Date: 5/9/2019 Sales: Mario DiCano Designer: Sunny Jene

Model / Elevation:
KINMOUNT 6A/3-REAR UPGRADE Lot 74

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Mitek ver 8.2.3.229



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #: 74
 Elevation: 3-REAR UPGRADE

Job Track: 50120
 PlanLog: 200598
 Layout ID: 401609
 Ref #
 Page: 1 of 2
 Date: 05/09/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9 /12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	1 2-ply	T123 Hip Girder	9 /12	31-11-00	4-05-02	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	339.54 204.00		
	2	T2 Hip	9 /12	31-11-00	5-05-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	254.66 160.33		
	2	T3 Hip	9 /12	31-11-00	6-06-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	283.12 177.00		
	2	T4 Hip	9 /12	31-11-00	7-07-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	294.24 187.33		
	2	T5 Hip	9 /12	31-11-00	8-08-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	307.46 193.00		
	6	T6 Hip	9 /12	31-11-00	9-09-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	966.5 610.00		
	2	T7 Hip	9 /12	31-11-00	10-10-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	334.92 208.00		
	1	T18 Hip Girder	12 /12	15-01-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.69 61.50		
	1	T19 Hip	12 /12	15-01-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	94.02 60.83		
	3	T20 Hip	12 /12	15-01-00	9-04-08	2 x 4	1-03-08	3-10-08 3-10-08	247.52 159.00		
	1	T21 Hip	12 /12	15-01-00	7-04-08	2 x 4	1-03-08	3-10-08 3-10-08	85.26 54.83		
	1	T22 Hip Girder	12 /12	10-04-00	5-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	69.54 46.33		
	1	T23 Hip	12 /12	10-04-00	7-04-08	2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	81.01 53.67		



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERTS LANE HOME CORP.
 Location: Caledon
 Model: KINMOUNT 6A
 Lot #: 74
 Elevation: 3-REAR UPGRADE

Job Track: 50120
 PlanLog: 200598
 Layout ID: 401609
 Ref #
 Page: 2 of 2
 Date: 05/09/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T24 Hip	12 / 12	10-04-00	8-07-00	2 x 6 2 x 4	1-03-08 1-03-08	3-10-08 3-10-08	68.34 43.17		
	5	T100 Common	9 / 12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	330.72 215.83		
	1	G100 GABLE	9 / 12	14-09-00	7-00-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	69.08 45.17		
	2	P1 Piggyback	9 / 12	9-09-05	2-02-00	2 x 4			55.84 36.00		
	2	P2 Piggyback	9 / 12	9-09-05	3-03-00	2 x 4			54.88 36.67		
	11	J1 Jack-Open	6.5 / 12	5-10-08	4-04-14	2 x 4	1-03-08	1-02-11 4-04-14	187.42 117.33		
	6	J4 Jack-Open	12 / 12	1-06-00	5-04-08	2 x 4	1-03-08	3-10-08 5-04-08	81.06 56.00		
	9	J5 Jack-Open	12 / 12	1-06-00	3-04-08	2 x 4	1-03-08	1-10-08 3-04-08	86.6 63.00		
	7	J90 Jack-Open	4 / 12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	101.14 65.33		

TOTAL # TRUSS= 72

TOTAL BFT OF ALL TRUSSES= 3058.32

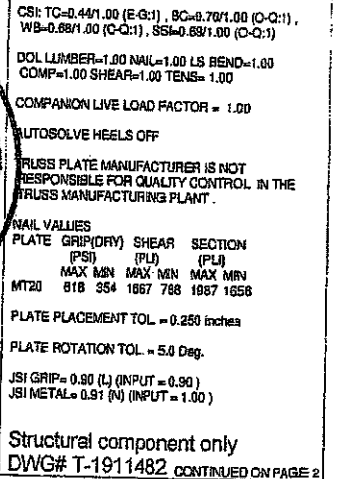
BFT.

TOTAL WEIGHT OF ALL TRSSES 4827.08 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS26-2	
9	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 12

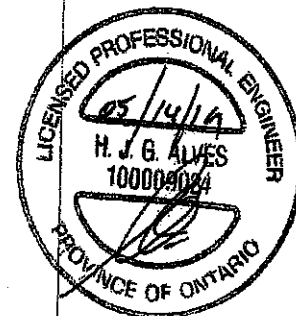
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:19 2019 Page 2
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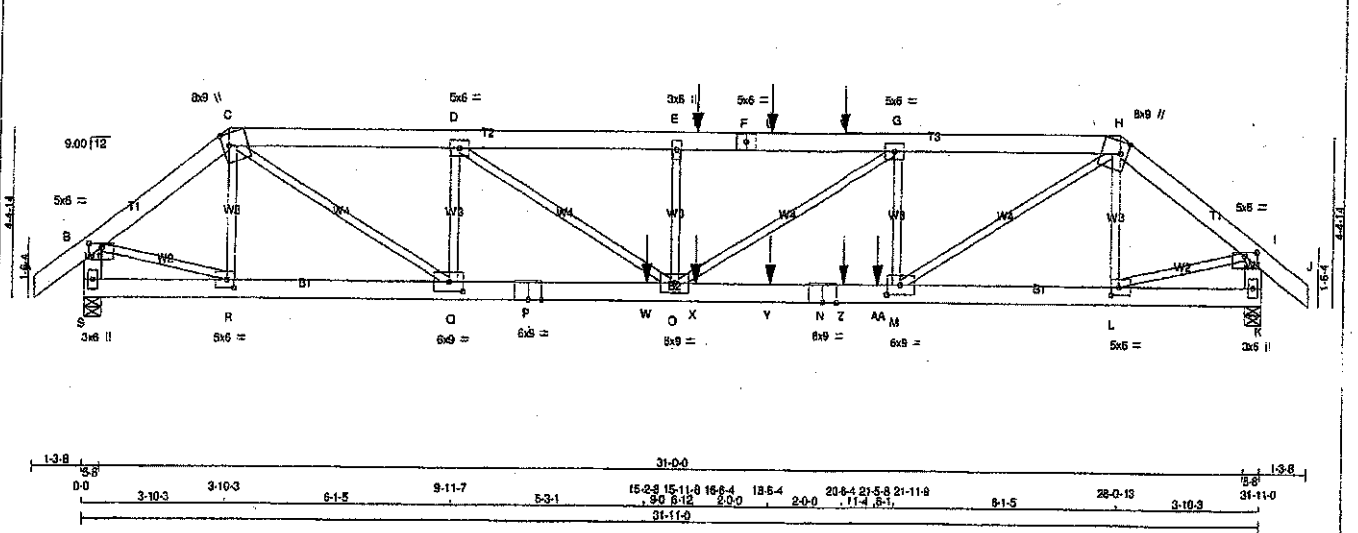
FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	20-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
W	22-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
X	24-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Y	26-9-12	-123	-123	---	FRONT	VERT	TOTAL	---	---
Z	14-10-8	-2332	-2332	---	FRONT	VERT	TOTAL	---	---
AA	16-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AB	18-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AC	20-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AD	22-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AE	24-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AF	26-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AG	28-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---
AH	30-9-12	-55	-70	---	FRONT	VERT	TOTAL	---	---



Structural component only
DWG# T-1911482

JOB NAME 403060	TRUSS NAME T1Z3	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.300 5 Apr 23 2018 Mitek Industries, Inc. Tue May 14 11:25:32 2019 Page 1			
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Scale = 1:30.3					



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
F - F	2x6	DRY	No.2
H - H	2x6	DRY	No.2
S - B	2x6	DRY	No.2
K - J	2x6	DRY	No.2
S - P	2x6	DRY	1850F 1.5E
P - N	2x6	DRY	1650F 1.5E
N - K	2x6	DRY	1650F 1.5E
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C	2	12
C-F	2	12
F-H	2	12
H-J	2	12
S-B	2	12
K-I	2	12
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
S-P	2	12
P-N	2	12
N-K	2	12
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	1	6

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	8.0	1.25	4.00
C TWWV-m	MT20	5.0	9.0	3.75	2.00
D TMVW-t	MT20	5.0	6.0		
E TMVW-w	MT20	3.0	6.0		
F TS-t	MT20	5.0	6.0		
G TMVW-l	MT20	5.0	6.0		
H TWWV-m	MT20	8.0	9.0	3.75	2.00
I TMVW-p	MT20	5.0	8.0	1.25	4.00
K BMV1-p	MT20	3.0	6.0		
L BMVW-t	MT20	5.0	6.0	2.50	2.25
M BMVW-t	MT20	6.0	9.0	3.00	4.25
N BS-t	MT20	6.0	9.0		
O BMVW-w	MT20	6.0	9.0		
P BS-t	MT20	6.0	9.0		
Q BMVW-t	MT20	5.0	8.0	3.00	4.25
R BMVW-t	MT20	5.0	8.0	2.50	2.25
S BMV1-p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
S	4185	0	4185	0	5-8
K	4678	0	4678	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
S	3103	1791/0	595/0
K	3485	2010/0	624/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRAIDED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MEMB.	FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED (LBS)	WEBS	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)
FR-TO	FROM					FR-TO	FROM		
A-B	0/44			-102.1	-102.1	R-C	-747/0		0.10 (1)
B-C	-4707/0			-102.1	-102.1	C-Q	0/5706		0.71 (1)
C-D	-8534/0			-102.1	-102.1	Q-D	-2659/0		0.35 (1)
D-E	-11632/0			-102.1	-102.1	D-O	0/3708		0.46 (1)
E-T	-11632/0			-102.1	-102.1	O-E	-922/0		0.12 (1)
T-F	-11632/0			-102.1	-102.1	F-G	0/2194		0.27 (1)
F-U	-11632/0			-102.1	-102.1	U-H	-2127/0		0.28 (1)
U-V	-11632/0			-102.1	-102.1	V-H	0/6522		0.82 (1)
V-G	-11632/0			-102.1	-102.1	H-L	-848/0		0.11 (1)
G-H	-9789/0			-102.1	-102.1	L-I	-7387/0		0.58 (1)
H-I	-5329/0			-102.1	-102.1	I-J	0/44		
I-J	0/44			-102.1	-102.1	J-K	0/15		10.00
S-B	-4139/0			0.0	0.0	K-I	-4641/0		0.0
K-I	-4641/0			0.0	0.0				
S-R	0/0			-38.5	-38.5	R-Q	0/3732		-38.5
R-Q	0/3732			-38.5	-38.5	Q-P	0/6534		-38.5
Q-P	0/6534			-38.5	-38.5	P-W	0/6534		-38.5
P-W	0/6534			-38.5	-38.5	W-O	0/6534		-38.5
W-O	0/6534			-38.5	-38.5	O-X	0/6799		-38.5
O-X	0/6799			-38.5	-38.5	X-Y	0/6799		-38.5
X-Y	0/6799			-38.5	-38.5	Y-N	0/6799		-38.5
Y-N	0/6799			-38.5	-38.5	N-Z	0/6799		-38.5
N-Z	0/6799			-38.5	-38.5	Z-AA	0/6799		-38.5
Z-AA	0/6799			-38.5	-38.5	AA-M	0/6799		-38.5
AA-M	0/6799			-38.5	-38.5	M-L	0/4228		-38.5
M-L	0/4228			-38.5	-38.5	L-K	0/0		-38.5
L-K	0/0			-38.5	-38.5				

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL	CONN.
T	18-6-4	-198	-198		BACK	VERT	TOTAL		
U	18-6-4	-198	-198		BACK	VERT	TOTAL		
V	20-6-4	-198	-198		BACK	VERT	TOTAL		
W	15-2-8	-1981	-1981		TOP	VERT	TOTAL		
X	18-6-4	-87	-87		BACK	VERT	TOTAL		
Y	18-6-4	-87	-87		BACK	VERT	TOTAL		
Z	20-6-4	-87	-87		BACK	VERT	TOTAL		
AA	21-5-8	-1328	-1328		TOP	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF CBC 2018, OBC 2012
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEFL. (LL) = L/999 (0.26")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEFL. (TL) = L/919 (0.42")

CSI: TC=0.50/1.00 (E-G:1), BC=0.55/1.00 (O-Q:1), WB=0.82/1.00 (H-M:1), SS=0.63/1.00 (O-Q:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1658

PLATE PLACEMENT TOL. = 0.250 inches

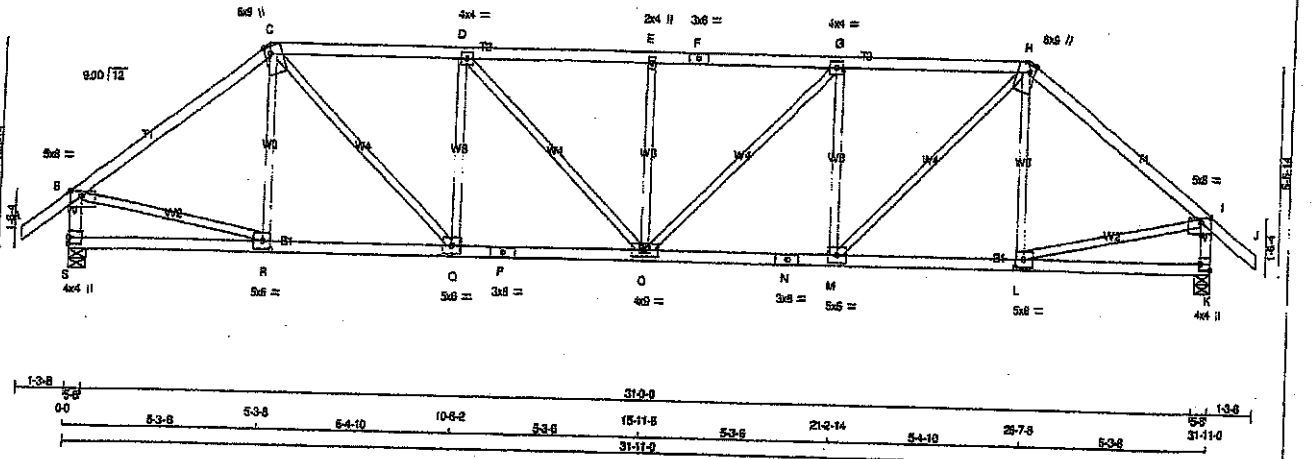
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.89 (G) (INPUT = 0.90)

JSI METAL = 0.85 (P) (INPUT = 1.00)

Structural component only
DWG# T-1911508 CONTINUED ON PAGE 2

JOB NAME 403045	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 10:41:21 2019 Page 1	
ID: D7uIPnW87DzGlo1HTFaFreyNCOCH-PmloJd7CUMW4UuHDPCLgigT1deT4MR7zGdD7				31-11-0 33-28	
1-3-8 0-0 5-3-8 5-3-8 5-4-10 10-8-2 5-3-8 15-11-8 5-3-8 21-2-14 5-4-10 20-7-8 5-3-8 31-11-0				Scale = 1/32" = 1'-0"	



LUMBER

N. L. & A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - H	2x4	DRY	No.2
H - J	2x4	DRY	No.2
J - K	2x4	DRY	No.2
K - L	2x4	DRY	No.2
L - M	2x4	DRY	No.2
M - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50 3.50
C	TTWW-m	MT20	8.0	9.0	Edge 2.00
D	TMVW-t	MT20	4.0	4.0	
E	TMVW-w	MT20	2.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-t	MT20	4.0	4.0	
H	TTWW-m	MT20	8.0	9.0	Edge 2.00
I	TMVW-p	MT20	5.0	8.0	1.50 3.50
K	BMVW-p	MT20	4.0	4.0	2.00 Edge
L	BMVW-t	MT20	5.0	8.0	2.50 2.75
M	BMVW-t	MT20	5.0	8.0	
N	BS-t	MT20	3.0	8.0	
O	BMVW-w	MT20	4.0	9.0	
P	BS-t	MT20	3.0	8.0	
Q	BMVW-t	MT20	5.0	8.0	
R	BMVW-t	MT20	5.0	8.0	2.50 2.75
S	BMVW-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
S	2385 0	2385 0	0/0	0/0
K	2385 0	2385 0	0/0	0/0

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED SNOW LIVE PERM.LIVE WIND DEAD SOIL
S	1778 1007/0 335/0 0/0 0/0 431/0 0/0
K	1778 1007/0 335/0 0/0 0/0 431/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.14 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0/42	-102.1 -102.1 0.14 (1)	10.00	R-C	-195/98	0.08 (1)	
B-C	-2428/0	-102.1 -102.1 0.73 (1)	3.67	C-Q	0/1897	0.38 (1)	
C-D	-3138/0	-102.1 -102.1 0.85 (1)	3.34	Q-D	-461/0	0.43 (1)	
D-E	-3494/0	-102.1 -102.1 0.89 (1)	3.14	D-O	0/500	0.11 (1)	
E-F	-3494/0	-102.1 -102.1 0.88 (1)	3.14	O-E	-497/0	0.22 (1)	
F-G	-3494/0	-102.1 -102.1 0.88 (1)	3.14	G-G	0/500	0.11 (1)	
G-H	-3138/0	-102.1 -102.1 0.85 (1)	3.34	M-G	-361/0	0.43 (1)	
H-I	-2428/0	-102.1 -102.1 0.73 (1)	3.67	M-H	0/1897	0.38 (1)	
I-J	0/42	-102.1 -102.1 0.14 (1)	10.00	L-H	-195/98	0.08 (1)	
S-B	-2301/0	0.0 0.0 0.24 (1)	5.92	S-R	0/1897	0.45 (1)	
K-I	-2301/0	0.0 0.0 0.24 (1)	5.92	L-I	0/1897	0.45 (1)	
S-R	0/0	-38.5 -38.5 0.21 (3)	10.00				
R-Q	0/1936	-38.5 -38.5 0.47 (2)	10.00				
Q-P	0/3138	-38.5 -38.5 0.80 (1)	10.00				
P-O	0/3138	-38.5 -38.5 0.80 (1)	10.00				
O-N	0/3138	-38.5 -38.5 0.80 (1)	10.00				
N-M	0/3138	-38.5 -38.5 0.80 (1)	10.00				
M-L	0/1936	-38.5 -38.5 0.47 (2)	10.00				
L-K	0/0	-38.5 -38.5 0.21 (3)	10.00				

TOTAL WEIGHT = 2 X 134 = 268 LB

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
-PART 9 OF NBCC 2010, CBC 2012
-CSA 086-09, CSA 086-14
-TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/800 (1.06")
CALCULATED VERT. DEFL. (LL) = L/989 (0.18")
ALLOWABLE DEFL. (TL) = L/800 (1.06")
CALCULATED VERT. DEFL. (TL) = L/989 (0.28")

CSK: TC=0.73/1.00 (H+1), EC=0.60/1.00 (C-D-1), WS=0.45/1.00 (L-1), SS=0.26/1.00 (C-D-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

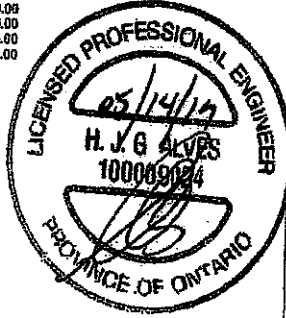
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRP(DRY) SHEAR SECTION (PSI) (PU) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 788 1837 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP = 0.88 (L) (INPUT = 0.90)
ISI METAL = 0.95 (P) (INPUT = 1.00)



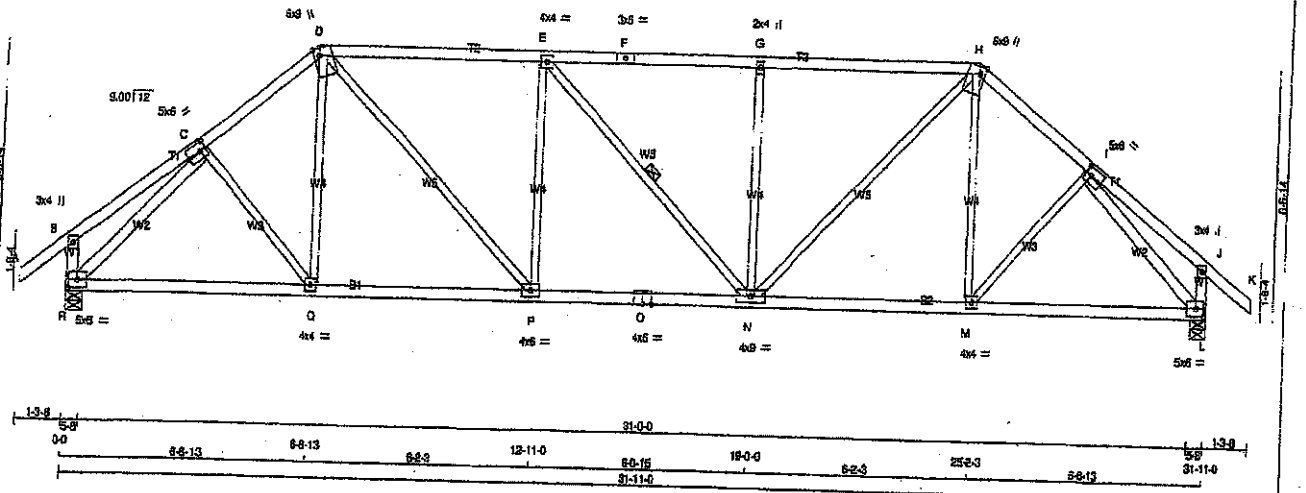
Structural component only
DWG# T-1911484

JOB NAME: 403045 TRUSS NAME: T3 QUANTITY: 2 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

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ID: D7uIPnW8?DzGkx1HTFaFreyNCCH-tys7fz2NlWqMKexu4nOJERYA?100PrHm7qv_RzGdOh

Scale = 1/32



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - D	2x4	DRY No.2
D - F	2x4	DRY No.2
F - H	2x4	DRY No.2
H - K	2x4	DRY No.2
R - B	2x4	DRY No.2
L - J	2x4	DRY No.2
O - L	2x4	DRY No.2
ALL WEBS	2x3	DRY No.2
EXCEPT		
R - C	2x4	DRY No.2
J - L	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (thickness in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+tp	MT20	3.0	4.0		
C TMVW+L	MT20	5.0	6.0	2.50	2.00
D TTWW+m	MT20	6.0	9.0	Edge	2.00
E TMVW+L	MT20	4.0	4.0		
F TS+L	MT20	3.0	6.0		
G TMVW+tp	MT20	2.0	4.0		
H TTWW+m	MT20	6.0	9.0	Edge	2.00
I TMVW+L	MT20	5.0	6.0	2.50	2.00
J TMV+tp	MT20	3.0	4.0		
L BMVW+L	MT20	5.0	6.0	2.50	2.75
M BMVW+L	MT20	4.0	4.0		
N BMVW+L	MT20	4.0	9.0		
O BS+L	MT20	4.0	6.0		
P BMVW+L	MT20	4.0	6.0		
Q BMVW+L	MT20	4.0	4.0		
R BMVW+L	MT20	5.0	6.0	2.50	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

BEARINGS

SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	5-8	5-8
R	HORIZ	HORIZ	5-8	5-8
L	HORIZ	UPLIFT	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX/MIN COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
R	1773 1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0
L	1773 1007 / 0 335 / 0 0 / 0 0 / 0 431 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.34 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 142	-102.1 -102.1 0.14 (1)	10.00	0 / 201 0.05 (2)
B-C	0 / 21	-102.1 -102.1 0.18 (1)	10.00	0 / 237 0.08 (2)
C-D	-2441 / 0	-102.1 -102.1 0.25 (1)	4.19	0 / 1265 0.28 (1)
D-E	-2822 / 0	-102.1 -102.1 0.79 (1)	3.34	P-E -877 / 0 0.48 (1)
E-F	-2822 / 0	-102.1 -102.1 0.78 (1)	3.34	E-N -2 / 0 0.00 (1)
F-G	-2822 / 0	-102.1 -102.1 0.78 (1)	3.34	N-G -578 / 0 0.48 (1)
G-H	-2822 / 0	-102.1 -102.1 0.78 (1)	3.34	N-H 0 / 1262 0.28 (1)
H-I	-2441 / 0	-102.1 -102.1 0.25 (1)	4.19	M-H 0 / 237 0.08 (2)
I-J	0 / 21	-102.1 -102.1 0.18 (1)	10.00	M-I 0 / 201 0.05 (2)
J-K	0 / 142	-102.1 -102.1 0.14 (1)	10.00	R-C -2888 / 0 0.77 (1)
R-B	-278 / 0	0.0 0.0 0.08 (1)	7.81	F-L -2888 / 0 0.77 (1)
L-J	-278 / 0	0.0 0.0 0.08 (1)	7.81	
R-Q	0 / 1809	-38.5 -38.5 0.55 (2)	10.00	
Q-P	0 / 1938	-38.5 -38.5 0.57 (2)	10.00	
P-Q	0 / 2822	-38.5 -38.5 0.59 (1)	10.00	
Q-N	0 / 2822	-38.5 -38.5 0.59 (1)	10.00	
N-M	0 / 1834	-38.5 -38.5 0.57 (2)	10.00	
M-L	0 / 1809	-38.5 -38.5 0.55 (2)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 088-08, CSA 089-14
- TPIC 2011, TPIC 2014

65% OF 27.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.06")
CALCULATED VERT. DEF. (LL) = L/899 (0.14")
ALLOWABLE DEFL. (TL) = L/360 (1.06")
CALCULATED VERT. DEF. (TL) = L/899 (0.24")

CS: TG=0.79/1.00 (D-E), SC=0.58/1.00 (N-P-1), WS=0.77/1.00 (L-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

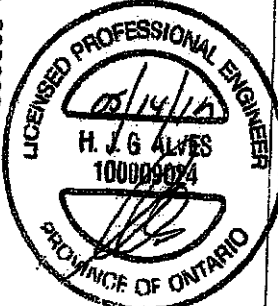
NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PS) (PL) (PL)
MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP = 0.89 (P) (INPUT = 0.90)

SI METAL = 0.73 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911485

Structural diagram of a truss bridge. The diagram shows a truss structure with various members labeled W1 through W15. The dimensions are given in feet and inches. The top chord consists of members W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11, W12, W13, W14, W15. The bottom chord consists of members W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11, W12, W13, W14, W15. The vertical height is 13'-0". The total length is 131'-0". The diagram includes various dimensions and labels for the truss members and their connections.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

Structural component only
DWG# T-1911486

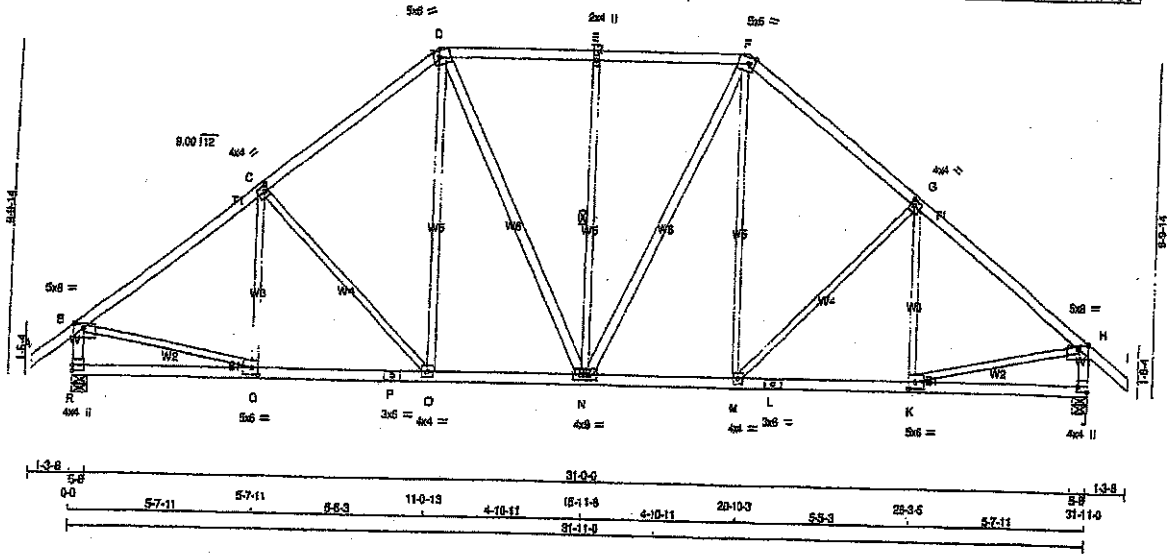
Structural drawing of a roof truss system. The drawing includes a plan view at the bottom showing horizontal dimensions and a side elevation view above it. The plan view shows a series of horizontal segments with dimensions: 1-3.8, 5.8, 4-11.0, 4-11.0, 4-9.8, 9-7.8, 5-4.0, 15-11.8, 5-4.0, 22-3.8, 4-8.8, 27-0.0, 4-11.0, 31-11.0, 5.8, and 1-3.8. The side elevation view shows a truss structure with various members labeled (e.g., W1, W2, W3, W4, W5, W6, W7, W8, W9, W10, W11, W12, W13, W14, W15, W16, W17, W18, W19, W20, W21, W22, W23, W24, W25, W26, W27, W28, W29, W30, W31, W32, W33, W34, W35, W36, W37, W38, W39, W40, W41, W42, W43, W44, W45, W46, W47, W48, W49, W50, W51, W52, W53, W54, W55, W56, W57, W58, W59, W60, W61, W62, W63, W64, W65, W66, W67, W68, W69, W70, W71, W72, W73, W74, W75, W76, W77, W78, W79, W80, W81, W82, W83, W84, W85, W86, W87, W88, W89, W90, W91, W92, W93, W94, W95, W96, W97, W98, W99, W100). The truss is supported by columns labeled A through J. The drawing is titled "Scale = 1/8\"/>

Structural component only
DWG# T-1911487

JOB NAME: 403045 TRUSS NAME: T6 QUANTITY: 6 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC: Tamarack Roof Truss, Burlington

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 ID: D7uPnW9?DzGkx1HTFaFreyNCCDH-XHYGeh0F2RCx85fTwyQr3a6y3XcFxCZ52ZamZGdDe

Scale = 1/8" = 1'-0"



LUMBER

N.L. & A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
R - P	2x4	DRY	No.2
P - L	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	8.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	8.0	Edge	2.00
G	TMVW-t	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	8.0	1.50	3.50
J	BMV1-p	MT20	4.0	4.0	2.00	Edge
K	BMVW-t	MT20	5.0	8.0	2.50	2.50
L	BS-t	MT20	3.0	8.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMVW-w	MT20	4.0	8.0		
O	BMVW-t	MT20	4.0	4.0		
P	BS-t	MT20	3.0	8.0		
Q	BMVW-t	MT20	5.0	8.0	2.50	2.50
R	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	DOWN	UP/LIFT	IN-SX
R 2385 0	2385 0	5-8	5-8
J 2385 0	2385 0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS
JT COMBINED	SNOW LIVE PERM. LIVE WIND DEAD SOIL
R 1773 1007 / 0	335 / 0 0 / 0 0 / 0 431 / 0 0 / 0
J 1773 1607 / 0	335 / 0 0 / 0 0 / 0 431 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.88 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO
FR-TO							
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-Q	-185 / 119	0.09 (1)
B-C	-2474 / 0	-102.1 -102.1	0.54 (1)	3.88	C-O	-422 / 0	0.52 (1)
C-D	-2181 / 0	-102.1 -102.1	0.49 (1)	4.13	D-N	0 / 498	0.11 (2)
D-E	-1916 / 0	-102.1 -102.1	0.35 (1)	4.50	D-N	0 / 440	0.07 (1)
E-F	-1916 / 0	-102.1 -102.1	0.35 (1)	4.50	N-F	-603 / 0	0.39 (1)
F-G	-2181 / 0	-102.1 -102.1	0.49 (1)	4.13	N-F	0 / 440	0.07 (1)
G-H	-2474 / 0	-102.1 -102.1	0.54 (1)	3.88	N-F	0 / 498	0.11 (2)
H-I	0 / 42	-102.1 -102.1	0.14 (1)	10.00	M-G	-422 / 0	0.52 (1)
I-J	-2285 / 0	0.0 0.0	0.24 (1)	5.83	K-G	-185 / 119	0.09 (1)
J-H	-2285 / 0	0.0 0.0	0.24 (1)	5.83	B-Q	0 / 2055	0.48 (1)
					K-H	0 / 2055	0.48 (1)
R-Q	0 / 0	-38.5 -38.5	0.22 (3)	10.00			
Q-P	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
P-O	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
O-N	0 / 1716	-38.5 -38.5	0.38 (1)	10.00			
N-M	0 / 1716	-38.5 -38.5	0.38 (1)	10.00			
M-L	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
L-K	0 / 2011	-38.5 -38.5	0.48 (2)	10.00			
K-J	0 / 0	-38.5 -38.5	0.22 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN/OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, NBC 2012
- CSA 086-08, CSA 086-14
- TFC 2011, TFC 2014

(55% OF 37.8 P.S.F. & 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.05)
 CALCULATED VERT. DEFL. (LL) = L/999 (0.09)
 ALLOWABLE DEFL. (TL) = L/360 (1.05)
 CALCULATED VERT. DEFL. (TL) = L/999 (0.15)

CSE TC=0.54/1.00 (B-C), BC=0.48/1.00 (D-C), WS=0.32/1.00 (C-O), SS=0.24/1.00 (D-E)

DO NOT LUMBER 1.00 NAIL 1.00 L.S. BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

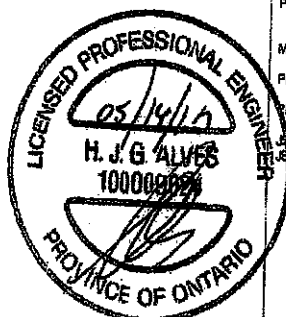
NAIL VALUES

PLATE	GRIP (OR) SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1657 788

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.85 (B) (INPUT = 0.90)
 METAL = 0.60 (P) (INPUT = 1.00)

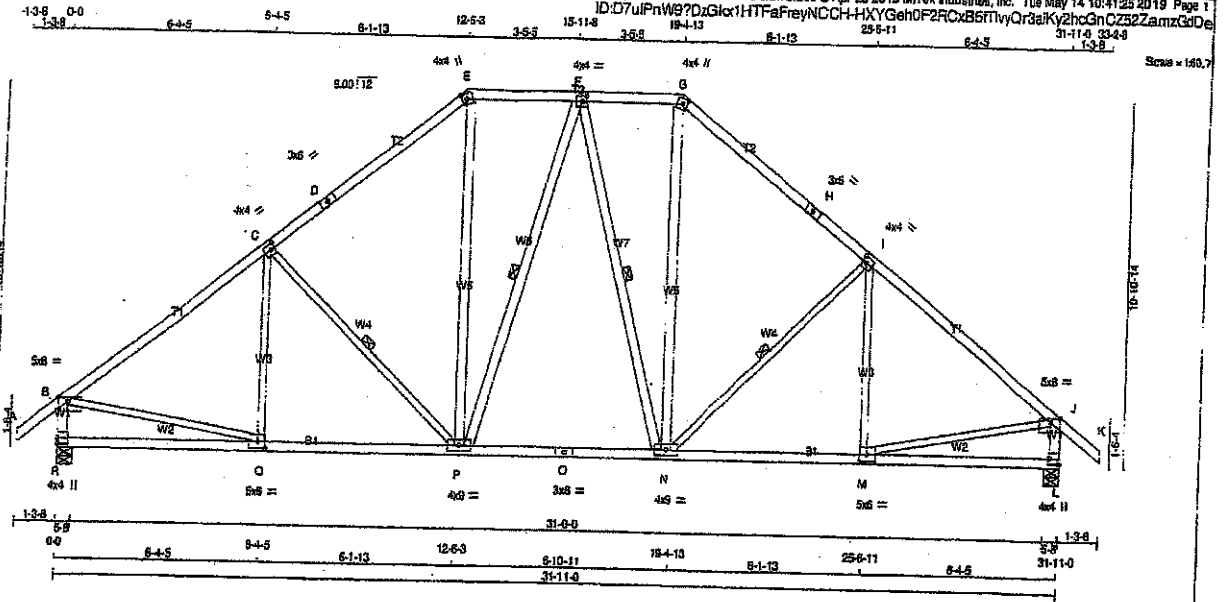


Structural component only
 DWG# T-1911488

JOB NAME 403045	TRUSS NAME T7	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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LUMBER	N. L. O. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - E	2x4	DRY	No.2	SPF		
E - G	2x4	DRY	No.2	SPF		
G - H	2x4	DRY	No.2	SPF		
H - K	2x4	DRY	No.2	SPF		
R - B	2x4	DRY	No.2	SPF		
L - J	2x4	DRY	No.2	SPF		
R - O	2x4	DRY	No.2	SPF		
O - L	2x4	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						
P - E	2x4	DRY	No.2	SPF		
P - F	2x4	DRY	No.2	SPF		
F - N	2x4	DRY	No.2	SPF		
N - G	2x4	DRY	No.2	SPF		

DRY: SEASONED LUMBER

PLATE TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50 3.50
C	TMVW-t	MT20	4.0	4.0	2.00 1.50
D	TS-t	MT20	3.0	6.0	
E	TTW+m	MT20	4.0	4.0	2.00 1.75
F	TMVW-t	MT20	4.0	4.0	2.00 1.75
G	TTW+m	MT20	4.0	4.0	2.00 1.75
H	TS-t	MT20	3.0	6.0	
I	TMVW-t	MT20	4.0	4.0	2.00 1.50
J	TMVW-p	MT20	5.0	8.0	1.50 3.50
L	EMV1-p	MT20	4.0	4.0	2.00 Edge
M	EMVW-t	MT20	5.0	5.0	2.50 2.75
N	EMVWV-t	MT20	4.0	9.0	
O	BS-t	MT20	3.0	6.0	
P	EMVW-t	MT20	4.0	4.0	2.00 Edge
Q	EMVW-t	MT20	5.0	5.0	2.50 2.75
R	EMV1-p	MT20	4.0	4.0	

Edges - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP/LIFT	IN-SX
R	2385	0	0	5-8
L	2385	0	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
R	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0
L	1773	1007 / 0	335 / 0	0 / 0	0 / 0	431 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.65 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-P, F-N, H-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. CS (LBS)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	CS (LBS)	FR-TO	
A-B	0 / 42	-102.1 -102.1	0.14 (1)	C-C	-111 / 174
B-C	-2481 / 0	-102.1 -102.1	0.72 (1)	C-P	-559 / 0
C-D	-2070 / 0	-102.1 -102.1	0.84 (1)	P-F	0 / 832
D-E	-2070 / 0	-102.1 -102.1	0.84 (1)	F-N	-212 / 0
E-F	-1627 / 0	-102.1 -102.1	0.78 (1)	N-G	-215 / 0
F-G	-1627 / 0	-102.1 -102.1	0.78 (1)	G-H	0 / 837
G-H	-2070 / 0	-102.1 -102.1	0.84 (1)	H-I	-559 / 0
H-I	-2070 / 0	-102.1 -102.1	0.84 (1)	I-J	-111 / 174
I-J	-2481 / 0	-102.1 -102.1	0.72 (1)	J-K	0 / 2057
J-K	0 / 42	-102.1 -102.1	0.14 (1)	K-L	0 / 2057
K-L	-2284 / 0	0.0 0.0	0.24 (1)	L-M	0.0 0.0
L-M	-2284 / 0	0.0 0.0	0.24 (1)	M-N	0.0 0.0
M-N	0 / 0	-38.5 -38.5	0.28 (3)	N-O	0.0 0.0
N-O	0 / 2022	-38.5 -38.5	0.54 (2)	O-P	0.0 0.0
O-P	0 / 1691	-38.5 -38.5	0.46 (2)	P-Q	0.0 0.0
P-Q	0 / 1691	-38.5 -38.5	0.46 (2)	Q-R	0.0 0.0
Q-R	0 / 2022	-38.5 -38.5	0.54 (2)	R-S	0.0 0.0
R-S	0 / 0	-38.5 -38.5	0.28 (3)	S-T	0.0 0.0

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:

- PART 9 OF NBC 2010, NBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(85 % OF 57.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.00")
CALCULATED VERT. DEFL. (LL) = L/999 (0.10")
ALLOWABLE DEFL. (TL) = L/360 (1.00")
CALCULATED VERT. DEFL. (TL) = L/999 (0.18")

CSL: TO=0.72/1.00 (B-C), BC=0.54/1.00 (P-Q), WB=0.48/1.00 (B-C), SB=0.25/1.00 (B-C)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

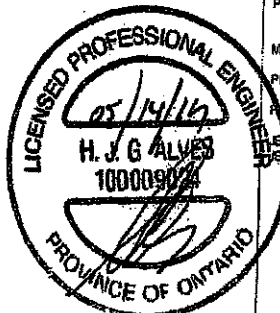
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 768 1987 1656

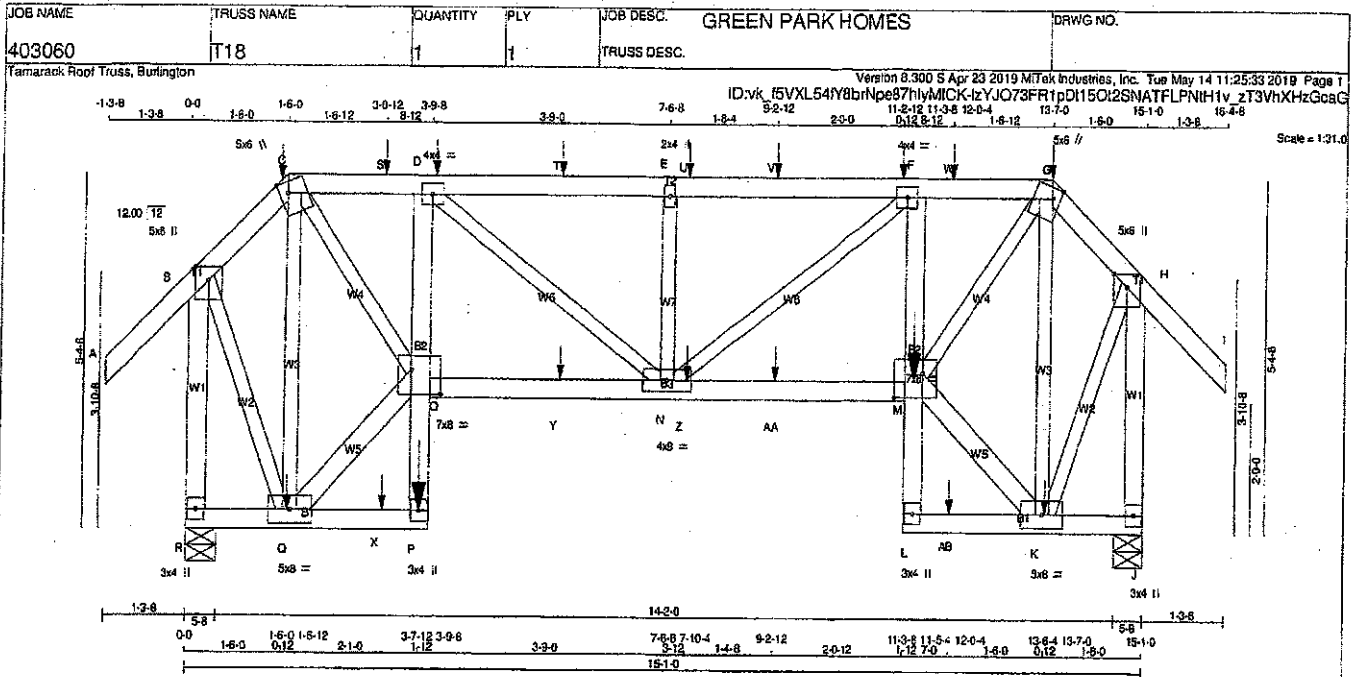
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 6.0 Deg.

GRIP = 0.88 (C) (INPUT = 0.90)
METAL = 0.89 (C) (INPUT = 1.00)



Structural component only
DWG# T-1911489



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - I	2x4	DRY	No.2
I - B	2x4	DRY	No.2
B - H	2x4	DRY	No.2
H - P	2x4	DRY	No.2
P - D	2x4	DRY	No.2
O - M	2x4	DRY	No.2
L - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
Q - O	2x4	DRY	No.2
M - K	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge	
C	TTWW+m	MT20	5.0	6.0	2.00	1.50
D	TMVW-t	MT20	4.0	4.0		
E	TMVW-w	MT20	2.0	4.0		
F	TMVW-t	MT20	4.0	4.0		
G	TTWW+m	MT20	5.0	6.0	2.00	1.50
H	TMVW+p	MT20	5.0	6.0	Edge	
J	BMV1+p	MT20	3.0	4.0		
K	BMVWW-t	MT20	5.0	8.0		
L	BMV+p	MT20	3.0	4.0		
M	BMVWW-t	MT20	7.0	8.0	4.50	5.50
N	BMVWW-t	MT20	4.0	9.0		
O	BMVWW-t	MT20	7.0	8.0	4.50	5.50
P	BMV+p	MT20	3.0	4.0		
Q	BMVWW-t	MT20	5.0	8.0		
R	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UPLIFT	IN-SX
R	1208	0	0	5-8
J	1208	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
R	895	519/0 181/0 0/0 0/0 215/0 0/0
J	895	519/0 181/0 0/0 0/0 215/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.

MAX. UNBRAIDED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS				WEBS			
MEMB.	MAX. FACTORED	FACTORED		MEMB.	MAX. FACTORED	FACTORED	
	FORCE	VERT. LOAD	CS1 (LO)		FORCE	MAX. CS1 (LO)	
FR-TO		FROM TO	LENGTH	FR-TO		FROM TO	LENGTH
A-B	0/50	-102.1 -102.1	0.16 (1)	10.00	Q-C	-800/0	0.36 (1)
B-C	-448/0	-102.1 -102.1	0.16 (1)	6.25	Q-D	0/369	0.07 (1)
C-S	-1018/0	-102.1 -102.1	0.14 (1)	6.02	C-O	0/1245	0.31 (1)
S-D	-1018/0	-102.1 -102.1	0.14 (1)	6.02	D-N	0/888	0.10 (1)
D-T	-1332/0	-102.1 -102.1	0.23 (1)	5.32	N-E	-422/0	0.09 (1)
T-E	-1332/0	-102.1 -102.1	0.23 (1)	5.32	N-F	0/388	0.10 (1)
E-U	-1332/0	-102.1 -102.1	0.23 (1)	5.32	M-K	0/369	0.07 (1)
U-V	-1332/0	-102.1 -102.1	0.23 (1)	5.32	M-G	0/1245	0.31 (1)
V-F	-1332/0	-102.1 -102.1	0.23 (1)	5.32	K-G	-800/0	0.36 (1)
F-W	-1018/0	-102.1 -102.1	0.14 (1)	6.02	B-O	0/677	0.17 (1)
W-G	-1018/0	-102.1 -102.1	0.14 (1)	6.02	K-H	0/677	0.17 (1)
G-H	-448/0	-102.1 -102.1	0.16 (1)	6.25			
H-I	0/50	-102.1 -102.1	0.16 (1)	10.00			
R-B	-1187/0	0.0	0.0	7.29			
J-H	-1187/0	0.0	0.0	7.29			
R-Q	0/0	-38.5	-38.5	0.03 (3)	10.00		
Q-X	0/15	-38.5	-38.5	0.03 (2)	10.00		
X-P	0/15	-38.5	-38.5	0.03 (2)	10.00		
P-O	0/47	0.0	0.0	0.06 (1)	10.00		
O-D	-563/0	0.0	0.0	0.06 (1)	7.81		
O-Y	0/1029	-38.5	-38.5	0.28 (2)	10.00		
Y-M	0/1029	-38.5	-38.5	0.28 (2)	10.00		
N-Z	0/1029	-38.5	-38.5	0.28 (2)	10.00		
Z-AA	0/1029	-38.5	-38.5	0.28 (2)	10.00		
AA-M	0/1029	-38.5	-38.5	0.28 (2)	10.00		
L-M	0/41	0.0	0.0	0.05 (1)	10.00		
M-F	-563/0	0.0	0.0	0.06 (1)	7.81		
L-AB	0/15	-38.5	-38.5	0.03 (2)	10.00		
AB-K	0/15	-38.5	-38.5	0.03 (2)	10.00		
K-J	0/0	-38.5	-38.5	0.03 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN
C	1-8-0	1	1	144	BACK	VERT	TOTAL		
D	3-10-4	1	1	144	BACK	VERT	TOTAL		
F	11-2-12	1	1	144	BACK	VERT	TOTAL		
G	13-7-0	1	1	144	BACK	VERT	TOTAL		
K	13-6-4	1	1		BACK	VERT	TOTAL		
M	11-5-4	-6	-7		BACK	VERT	TOTAL		
P	3-7-12	-6	-7		BACK	VERT	TOTAL		
Q	1-6-12	1	1		BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BDT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")

CALCULATED VERT. DEFL.(LL) = L/999 (0.04")

ALLOWABLE DEFL.(TL) = L/360 (0.50")

CALCULATED VERT. DEFL.(TL) = L/999 (0.07")

CSI: TC=0.31/1.00 (B-R-1), BC=0.28/1.00 (N-D-2), WB=0.38/1.00 (G-K-1), SS=0.18/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00

COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667

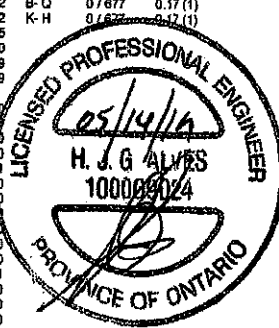
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.83 (G) (INPUT = 0.90)

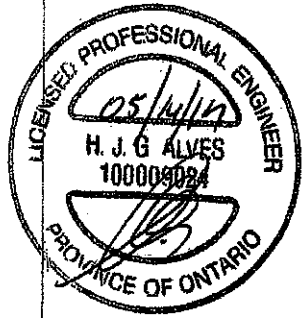
JSI METAL=0.32 (C) (INPUT = 1.00)

Structural component only
DWG# T-1911509 CONTINUED ON PAGE 2

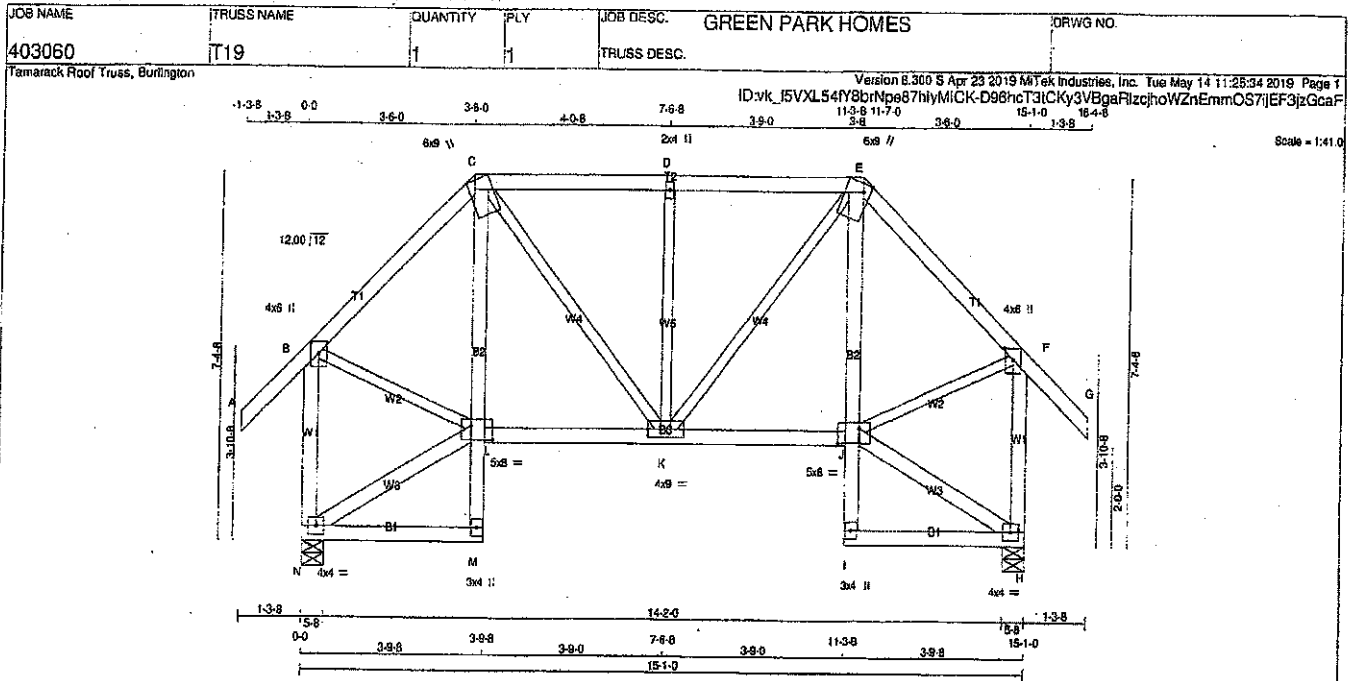


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
403060	T18	1	1	TRUSS DESC.		
Tamarack Roof Truss, Burlington						
Version 8.300 S Apr 23 2019 Mitek Industries, Inc. Tue May 14 11:25:33 2019 Page 2						
ID:vk f5VXL54fy8brNpe87hiyMiCK-jzYJO73FR1pDii15OI2SNATFLPNH1v zT3VhXH-zGcaG						

FACTORED CONCENTRATED LOADS (LBS)									
	LOC.	LC1	MAX	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	3-0-12	1	1	144	BACK	VERT	TOTAL	---	---
S	5-10-4	1	1	144	BACK	VERT	TOTAL	---	---
T	7-10-4	1	1	144	BACK	VERT	TOTAL	---	---
U	9-2-12	1	1	144	BACK	VERT	TOTAL	---	---
V	12-0-4	1	1	144	BACK	VERT	TOTAL	---	---
W	3-0-12	1	1	---	BACK	VERT	TOTAL	---	---
X	5-10-4	1	1	---	BACK	VERT	TOTAL	---	---
Y	7-10-4	1	1	---	BACK	VERT	TOTAL	---	---
Z	9-2-12	1	1	---	BACK	VERT	TOTAL	---	---
AA	12-0-4	1	1	---	BACK	VERT	TOTAL	---	---
AB									



Structural component only
DWG# T-1911509-2



LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
N - L	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	8.0	2.75	2.00
C TTVW+m	MT20	8.0	9.0	Edge	1.75
D TMVW+w	MT20	2.0	4.0		
E TTVW+m	MT20	8.0	9.0	Edge	1.75
F TMVW+p	MT20	4.0	8.0	2.75	2.00
H BMVW-1	MT20	4.0	4.0		
I BMVW+p	MT20	3.0	4.0		
J BMVW-W	MT20	5.0	8.0	3.50	5.50
K BMVW-W	MT20	4.0	9.0		
L BMVW-W	MT20	5.0	8.0	3.50	5.50
M BMVW+p	MT20	3.0	4.0		
N BMVW-1	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG IN-SX	REQD BRG IN-SX
JT VERT	1203	1203	5-8	5-8
H VERT	1203	1203	5-8	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT COMBINED	N	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
H COMBINED	N	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO				FR-TO			
A-B	0 / 50	-102.1	-102.1 0.14 (1)	N-L	-5 / 0	0.00 (1)	
B-C	-823 / 0	-102.1	-102.1 0.23 (1)	B-L	0 / 821	0.14 (1)	
C-D	-837 / 0	-102.1	-102.1 0.28 (1)	J-H	-5 / 0	0.00 (1)	
D-E	-837 / 0	-102.1	-102.1 0.28 (1)	J-F	0 / 821	0.14 (1)	
E-F	-823 / 0	-102.1	-102.1 0.23 (1)	K-D	-496 / 0	0.21 (1)	
F-G	0 / 50	-102.1	-102.1 0.14 (1)	C-K	0 / 414	0.09 (1)	
N-B	-1130 / 0	0.0	0.0 0.29 (1)	K-E	0 / 414	0.09 (1)	
H-F	-1130 / 0	0.0	0.0 0.29 (1)				
N-M	0 / 4	-38.5	-38.5 0.12 (3)				
M-L	0 / 89	0.0	0.0 0.02 (2)				
L-C	-82 / 67	0.0	0.0 0.02 (3)				
L-K	0 / 578	-38.5	-38.5 0.20 (2)				
K-J	0 / 578	-38.5	-38.5 0.20 (2)				
I-J	0 / 89	0.0	0.0 0.02 (2)				
J-E	-82 / 67	0.0	0.0 0.02 (3)				
I-H	0 / 4	-38.5	-38.5 0.12 (3)				

TOTAL WEIGHT = 94 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPO 2011, TPO 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.50")
CALCULATED VERT. DEFL.(LL) = L/989 (0.02")
ALLOWABLE DEFL.(TL) = L/860 (0.50")
CALCULATED VERT. DEFL.(TL) = L/989 (0.03")

CSI: TC=0.29/1.00 (B-N:1), BC=0.20/1.00 (J-K:2), WB=0.21/1.00 (D-K:1), SS=0.20/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.16 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

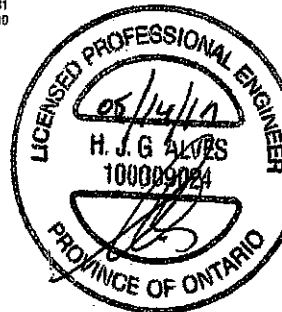
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (FSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

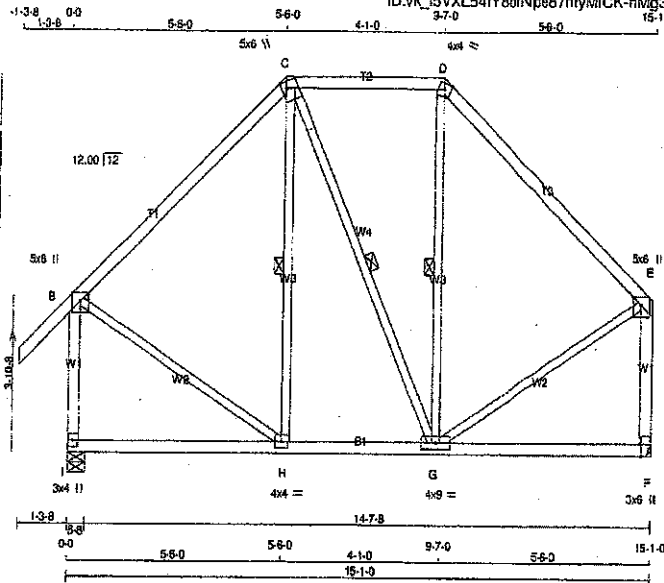
PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.79 (F) (INPUT = 0.90)
SI METAL= 0.39 (F) (INPUT = 1.00)



Structural component only
DWG# T-1911510

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403060	T20	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					



TOTAL WEIGHT = 3 X 83 = 249 lb

LUMBER

N. L. G. A. RULES				
CHORDS	SIZE			LUMBER
A - C	2x4	DRY		No.2
C - D	2x4	DRY		No.2
D - E	2x4	DRY		No.2
I - B	2x4	DRY		No.2
F - E	2x4	DRY		No.2
I - F	2x4	DRY		No.2

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TTW-m	MT20	4.0	4.0	Edge
E	TMVW+p	MT20	5.0	6.0	Edge
F	BMV1+H	MT20	3.0	6.0	Edge 0.50
H	BMWWV-1	MT20	4.0	6.0	
I	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESCR.	BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
SPF	FACTORED	GROSS REACTION	GROSS REACTION	BRG	BRG
SPF	VERT	HORIZ	DOWN	HORIZ	UPLIFT
SPF	I	1203	0	0	5-8
SPF	F	1060	0	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
I	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
F	792	438 / 0	158 / 0	0 / 0	0 / 0	198 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-H, C-G, D-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
PR-TO		FROM	TO	FROM	TO	FROM	TO
A-B	0 / 50	-102.1	-102.1	0.14 (1)	10.00	H-C	-95 / 125
B-C	-598 / 0	-102.1	-102.1	0.40 (1)	6.25	C-G	0 / 0
C-D	-421 / 0	-102.1	-102.1	0.22 (1)	6.25	G-D	-95 / 125
D-E	-598 / 0	-102.1	-102.1	0.40 (1)	6.25	B-H	0 / 498
I-B	-1112 / 0	0.0	0.0	0.28 (1)	7.51	G-E	0 / 498
F-E	-989 / 0	0.0	0.0	0.25 (1)	7.81		
I-H	0 / 0	-38.5	-38.5	0.20 (3)	10.00		
H-G	0 / 421	-38.5	-38.5	0.23 (2)	10.00		
G-F	0 / 0	-38.5	-38.5	0.20 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.50")
CALCULATED VERT. DEFL. (LL) = L/999 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.50")
CALCULATED VERT. DEFL. (TL) = L/999 (0.07")

CS: TC=0.40/1.00 (B-C:1), BC=0.23/1.00 (G-H:2), WB=0.11/1.00 (B-H:1), SSI=0.18/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

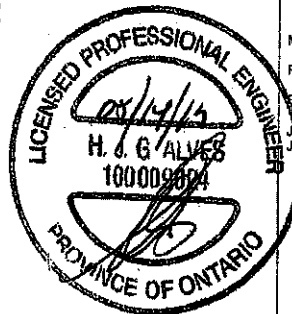
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX MIN	MAX MIN	MAX MIN	
MT20	618	354	1867

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

J1 GRIP=0.57 (B) (INPUT = 0.80)
J2 METAL=0.27 (B) (INPUT = 1.00)

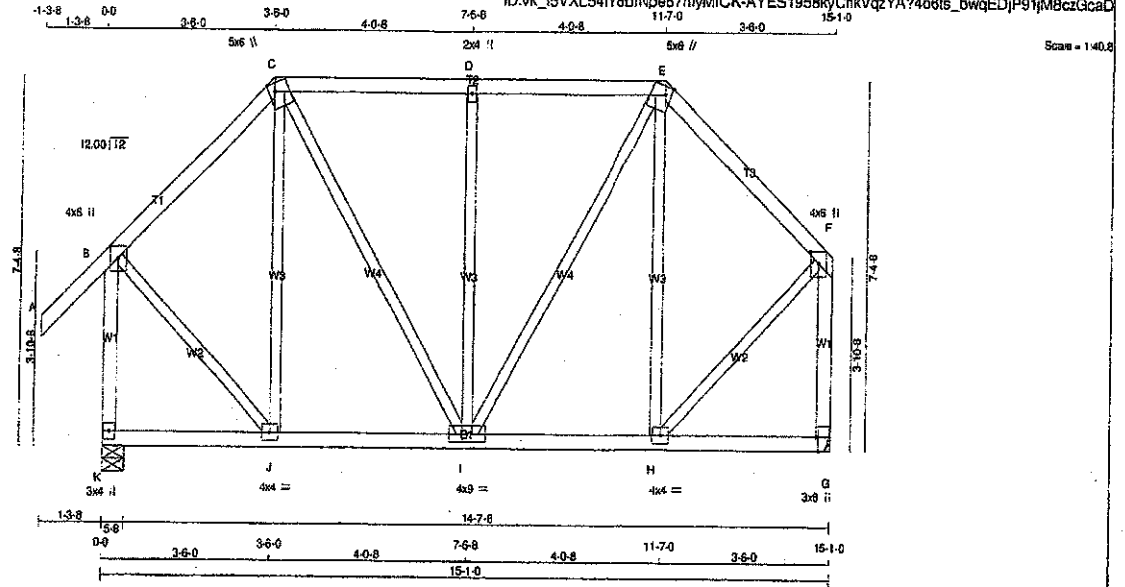


Structural component only
DWG# T-1911511

JOB NAME 403060	TRUSS NAME T21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:vk_15VXL54Y8brNpe87hlyMICK-AYES1958kyCnkVqzYA?4o8ts_bwqEDJP91jMBczGcaD



TOTAL WEIGHT = 85 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - F	2x4 DRY	No.2	SPF		
K - 8	2x4 DRY	No.2	SPF		
G - F	2x4 DRY	No.2	SPF		
K - G	2x4 DRY	No.2	SPF		
ALL WEBS	2x3 DRY	No.2	SPF		
EXCEPT					

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75 2.00
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMW+w	MT20	5.0	4.0	
E	TTWW+m	MT20	5.0	6.0	2.00 1.50
F	TMVW+p	MT20	4.0	6.0	2.75 2.00
G	BMV1+1	MT20	3.0	6.0	Edge 0.50
H	BMWW+1	MT20	4.0	4.0	
I	BMWW+1	MT20	4.0	9.0	
J	BMWW+1	MT20	4.0	4.0	
K	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
JT	GROSS REACTION	GROSS REACTION	SRG	BRG
K	VERT	HORZ	DOWN	HORZ
G	1203 0	1203 0	5-8 5-8	IN-SX
			MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
K	890	519 / 0	158 / 0	0 / 0	0 / 0	213 / 0	0 / 0
G	782	438 / 0	158 / 0	0 / 0	0 / 0	186 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED HORIZ. LOAD (LBS)
FR-TO					FR-TO		
A-B	0 / 50	-102.1	-102.1	0.14 (1)	J-C	-240 / 17	6.23 (1)
B-C	-583 / 0	-102.1	-102.1	0.22 (1)	C-D	0 / 368	0.08 (1)
C-D	-586 / 0	-102.1	-102.1	0.27 (1)	D-E	-508 / 6	0.48 (1)
D-E	-586 / 0	-102.1	-102.1	0.27 (1)	E-F	0 / 368	0.08 (1)
E-F	-583 / 0	-102.1	-102.1	0.22 (1)	F-G	-240 / 17	6.23 (1)
K-B	-1150 / 0	0.0	0.0	0.29 (1)	H-J	0 / 583	0.13 (1)
G-F	-1007 / 0	0.0	0.0	0.26 (1)	H-F	0 / 583	0.13 (1)
K-J	0 / 0	-38.5	-38.5	0.10 (3)			
J-I	0 / 414	-38.5	-38.5	0.16 (2)			
I-H	0 / 414	-38.5	-38.5	0.16 (2)			
H-G	0 / 0	-38.5	-38.5	0.10 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.50")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.02")
ALLOWABLE DEFL. (TL) = $L/360$ (0.50")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.03")

CSI: TC=0.29/1.00 (B-K:1), BC=0.16/1.00 (H-I:2), WB=0.48/1.00 (D-I:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 254	1667 788	1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.81 (P) (INPUT = 0.90)
JSI METAL = 0.39 (B) (INPUT = 1.00)

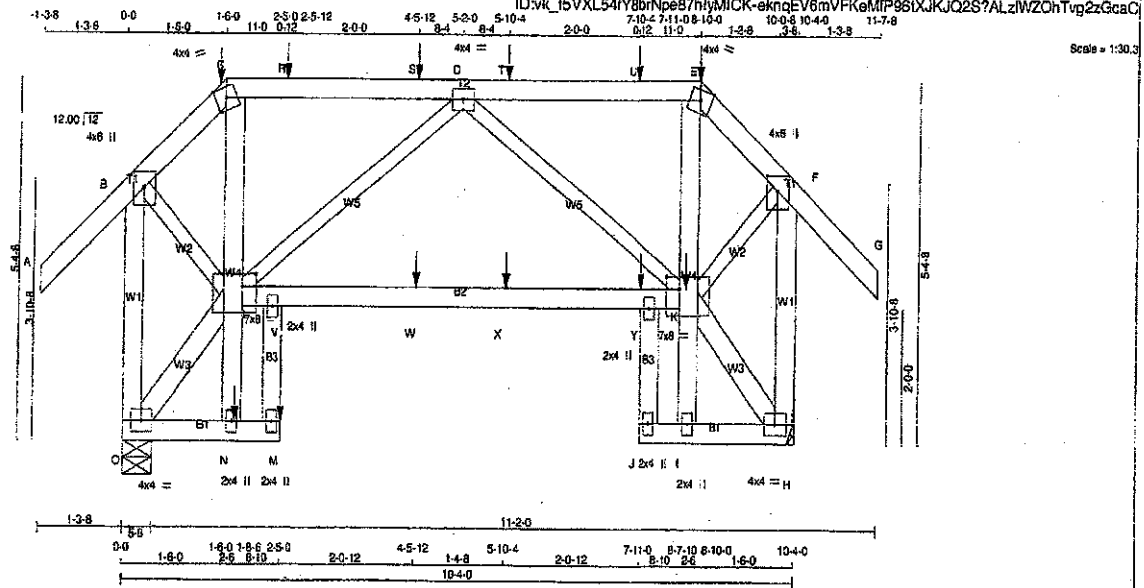


Structural component only
DWG# T-1911512

JOB NAME 403060	TRUSS NAME T22	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
TRUSS DESC.					

Tamworth Roof Truss, Burlington

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TOTAL WEIGHT = 70 LB

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2			SPF
C - E	2x4 DRY	No.2			SPF
E - G	2x4 DRY	No.2			SPF
O - B	2x4 DRY	No.2			SPF
H - F	2x4 DRY	No.2			SPF
O - M	2x4 DRY	No.2			SPF
L - K	2x4 DRY	No.2			SPF
J - H	2x4 DRY	No.2			SPF
ALL WEBS	2x4 DRY	No.2			SPF
EXCEPT					
B - L	2x3 DRY	No.2			SPF
K - F	2x3 DRY	No.2			SPF
L - D	2x3 DRY	No.2			SPF
D - K	2x3 DRY	No.2			SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TTW-m	MT20	4.0	4.0	Edge	
D	TMVW-i	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMVW-i	MT20	4.0	4.0		
I	BMVW-w	MT20	2.0	4.0		
J, M, P, Q						
J	NP+w	MT20	2.0	4.0		
K	BWMVWV-i	MT20	7.0	8.0	2.25	2.50
L	BWMVWV-w	MT20	7.0	8.0	2.25	2.50
N	BMVW-w	MT20	2.0	4.0		
O	BMVW-i	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
O	897	0	897	0	5-8
H	897	0	897	0	5-8

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-8.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
O	666	361 / 0	124 / 0
H	666	361 / 0	124 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

MAX. UNBRACED INTERIOR CHORD LENGTH = 10.00 FT

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (7)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	MAX. (LBS)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 50	-102.1	-102.1	0.18 (1)	10.00	N-L	0 / 78
B-C	-472 / 0	-102.1	-102.1	0.15 (1)	6.25	L-C	0 / 172
C-R	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	I-K	0 / 78
R-S	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	K-E	0 / 172
S-D	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	O-L	-12 / 0
D-T	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	B-L	0 / 400
T-U	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	K-H	-12 / 0
U-E	-311 / 0	-102.1	-102.1	0.25 (1)	6.25	K-F	0 / 400
E-F	-472 / 0	-102.1	-102.1	0.15 (1)	6.25	L-D	-344 / 0
F-G	0 / 50	-102.1	-102.1	0.18 (1)	10.00	D-K	-344 / 0
O-B	-856 / 0	0.0	0.0	0.23 (1)	7.81		
H-F	-856 / 0	0.0	0.0	0.23 (1)	7.81		
O-N	0 / 8	-38.5	-38.5	0.03 (3)	10.00		
N-M	0 / 0	-38.5	-38.5	0.03 (3)	10.00		
L-V	0 / 568	-38.5	-38.5	0.52 (2)	10.00		
V-W	0 / 568	-38.5	-38.5	0.52 (2)	10.00		
W-X	0 / 568	-38.5	-38.5	0.52 (2)	10.00		
X-Y	0 / 568	-38.5	-38.5	0.52 (2)	10.00		
Y-K	0 / 568	-38.5	-38.5	0.52 (2)	10.00		
J-I	0 / 0	-38.5	-38.5	0.02 (3)	10.00		
I-H	0 / 8	-38.5	-38.5	0.03 (3)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
E	1-8-0	1	1	144	FRONT	VERT	TOTAL	--	--
K	8-10-0	1	1	144	FRONT	VERT	TOTAL	--	--
M	2-5-0	1	1	---	FRONT	VERT	TOTAL	--	--
N	1-8-6	1	1	---	FRONT	VERT	TOTAL	--	--
R	2-5-12	1	1	143	FRONT	VERT	TOTAL	--	--
S	4-5-12	1	1	144	FRONT	VERT	TOTAL	--	--
T	5-10-4	1	1	144	FRONT	VERT	TOTAL	--	--
U	7-10-4	1	1	143	FRONT	VERT	TOTAL	--	--
W	4-5-12	1	1	---	FRONT	VERT	TOTAL	--	--
X	5-10-4	1	1	---	FRONT	VERT	TOTAL	--	--

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF	

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 088-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) =	L/360 (0.34")
CALCULATED VERT. DEFL.(LL) =	L/814 (0.15")
ALLOWABLE DEFL.(TL) =	L/360 (0.34")
CALCULATED VERT. DEFL.(TL) =	L/488 (0.25")

CSI: TC=0.25/1.00 (C-D:1), BC=0.52/1.00 (K-L:2), WB=0.13/1.00 (D-K:1), SS=0.20/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354 1857 788 1987 1856	

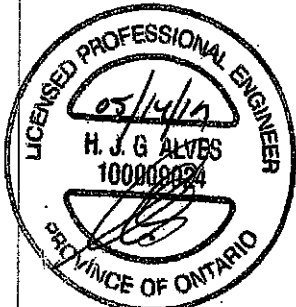
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (B) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)

Structural component only

DWG# T-1911513 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.																				
403060	T22	1	1	GREEN PARK HOMES																					
Tamarack Roof Truss, Burlington		Version 8.300 S Apr 23 2019 M/Tek Industries, Inc. Tue May 14 11:25:37 2019 Page 2																							
		ID:vk I5VXL54fy8brNpe87hlyMICK-ekngEV6mVFKaMIP96IXJKJQ2S7ALziWZOhTvg2zGcaC																							
FACTORED CONCENTRATED LOADS (LBS) <table border="1"> <thead> <tr> <th>JT</th> <th>LOC.</th> <th>LC1</th> <th>MAX-</th> <th>MAX+</th> <th>FACE</th> <th>DIR.</th> <th>TYPE</th> <th>HEEL</th> <th>CONN.</th> </tr> </thead> <tbody> <tr> <td>Y</td> <td>7-11-0</td> <td>1</td> <td>:</td> <td>---</td> <td>FRONT</td> <td>VERT</td> <td>TOTAL</td> <td>---</td> <td>---</td> </tr> </tbody> </table>						JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.	Y	7-11-0	1	:	---	FRONT	VERT	TOTAL	---	---
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.																
Y	7-11-0	1	:	---	FRONT	VERT	TOTAL	---	---																
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE																									
																									
Structural component only DWG# T-1911513 -2																									

JOB NAME: Tamarack Roof Truss, Burlington

JOB NO: 403060

TRUSS NAME: T23

QUANTITY: 1

PLY: 1

JOB DESC: GREEN PARK HOMES

DRWG NO: 1-1911514

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ID:vk_15VXL54Y8brNpe87hlyMlCK-ekngEV6mVFKmIP98XJKJQ22?H7zm2ZChTvg2zGcaC

Scale = 1:40.0

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - H	2x4	DRY	No.2
P - B	2x4	DRY	No.2
I - G	2x4	DRY	No.2
P - O	2x4	DRY	No.2
O - C	2x4	DRY	No.2
N - K	2x4	DRY	No.2
J - F	2x4	DRY	No.2
J - I	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT

P - N 2x4 DRY No.2

K - I 2x4 DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X	
B	TMVW+p	MT20	4.0	6.0	2.75	2.00
C	TMVW+p	MT20	5.0	6.0	Edge	2.25
D	TTWW+m	MT20	5.0	6.0	2.00	1.50
E	TTW-m	MT20	4.0	4.0	Edge	
F	TMVW+p	MT20	5.0	6.0	Edge	2.25
G	TMVW+p	MT20	4.0	6.0	2.75	2.00
H	BMVW-1	MT20	4.0	4.0		
I	BMVW+p	MT20	3.0	4.0		
J	K	BMVW-1	MT20	6.0	9.0	4.50 5.50
L	M	BMVW-1	MT20	5.0	6.0	
N	O	BMVW-1	MT20	4.0	6.0	
P	Q	BMVW-1	MT20	6.0	9.0	4.50 5.50
R	S	BMVW+p	MT20	3.0	4.0	
T	U	BMVW-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 23.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2014

(65% OF 37.5 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.34")

CALCULATED VERT. DEFL.(LL) = L/899 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.34")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TO=0.21/1.00 (G-I:1), BC=0.10/1.00 (K-L:2), WB=0.10/1.00 (G-K:1), SSI=0.13/1.00 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LBS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618	354
1667	768	1987
1656		

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

GRIP = 0.58 (G) (INPUT = 0.80)

GRIP METAL = 0.28 (G) (INPUT = 1.00)

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	FORCE (LBS)	MEMB.	FORCE (LBS)
A-B	0/50	C-M	-106/0
B-C	-509/0	M-D	0/153
C-D	-502/0	D-L	0/2
D-E	-338/0	L-E	0/153
E-F	-504/0	F-F	-103/0
F-G	-509/0	G-N	-5/0
G-H	0/80	H-N	0/425
P-B	-821/0	K-I	-5/0
I-G	-821/0	K-G	0/425
P-Q	0/4		
Q-N	0/58		
N-C	-102/0		
M-L	0/373		
L-M	0/328		
L-K	0/372		
J-K	0/56		
K-F	-105/0		
J-I	0/4		

BEARINGS

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	SNOW	DOWN	HORIZ
P	869	0	0
I	869	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX MIN	COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
P	841	381/0	109/0	0/0	0/0	151/0	0/0
I	841	381/0</					

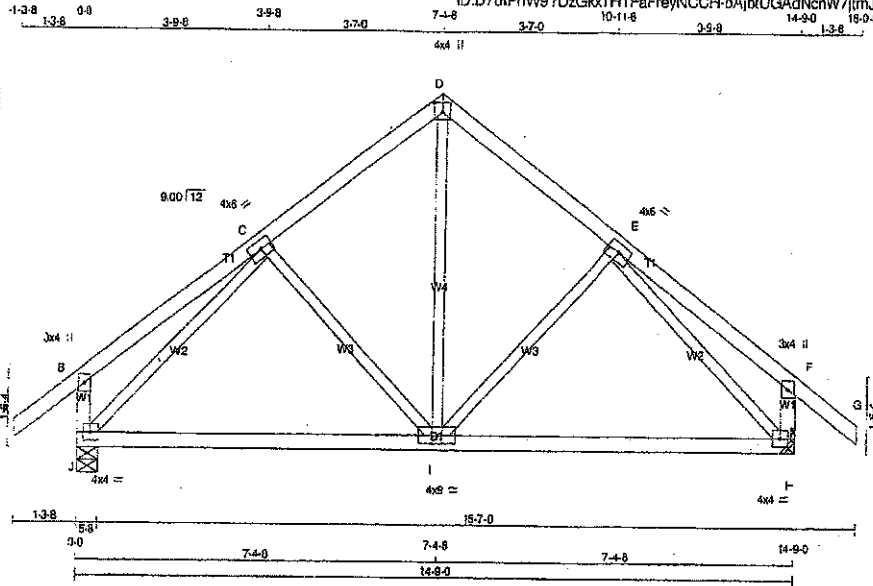
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401607	T100	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/40.4

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TTW+p	MT20	4.0	4.0	2.25 2.00
E	TMWW-t	MT20	4.0	6.0	
F	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	4.0	
I	BMWW1-t	MT20	4.0	9.0	
J	BMVW1-t	MT20	4.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
J	1178	0	1178	0	0
H	1178	0	1178	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERALIVE	WIND	DEAD	SOIL
J	872	508/0	155/0	0/0	0/0	208/0	0/0
H	872	508/0	155/0	0/0	0/0	208/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MEMB.	FORCE (LBS)	LC1 MAX (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/42	-102.1	-102.1 0.14 (1)	10.00	I-D	0/616	0.14 (1)
B-C	0/26	-102.1	-102.1 0.22 (1)	10.00	I-E	-181/24	0.09 (1)
C-D	-783/0	-102.1	-102.1 0.17 (1)	6.25	C-I	-181/24	0.09 (1)
D-E	-783/0	-102.1	-102.1 0.17 (1)	6.25	J-C	-1072/0	0.50 (1)
E-F	0/26	-102.1	-102.1 0.22 (1)	10.00	E-H	-1072/0	0.50 (1)
F-G	0/42	-102.1	-102.1 0.14 (1)	10.00			
J-B	-285/0	0.0	0.0 0.03 (1)	7.81			
H-F	-285/0	0.0	0.0 0.03 (1)	7.81			
J-I	0/730	-38.5	-38.5 0.54 (2)	10.00			
I-H	0/730	-38.5	-38.5 0.54 (2)	10.00			

TOTAL WEIGHT = 5 X 66 = 331 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, OBC 2012
- CSA 060-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.49")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.49")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.22/1.00 (B-C:1), BC=0.54/1.00 (I-J:2), WB=0.50/1.00 (C-J:1), SSI=0.19/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

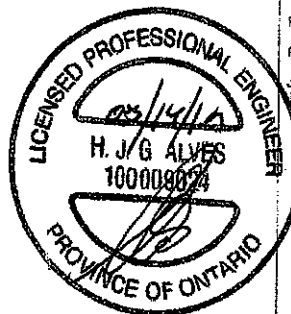
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.89 (D) (INPUT = 0.90)
JSI METAL=0.27 (J) (INPUT = 1.00)



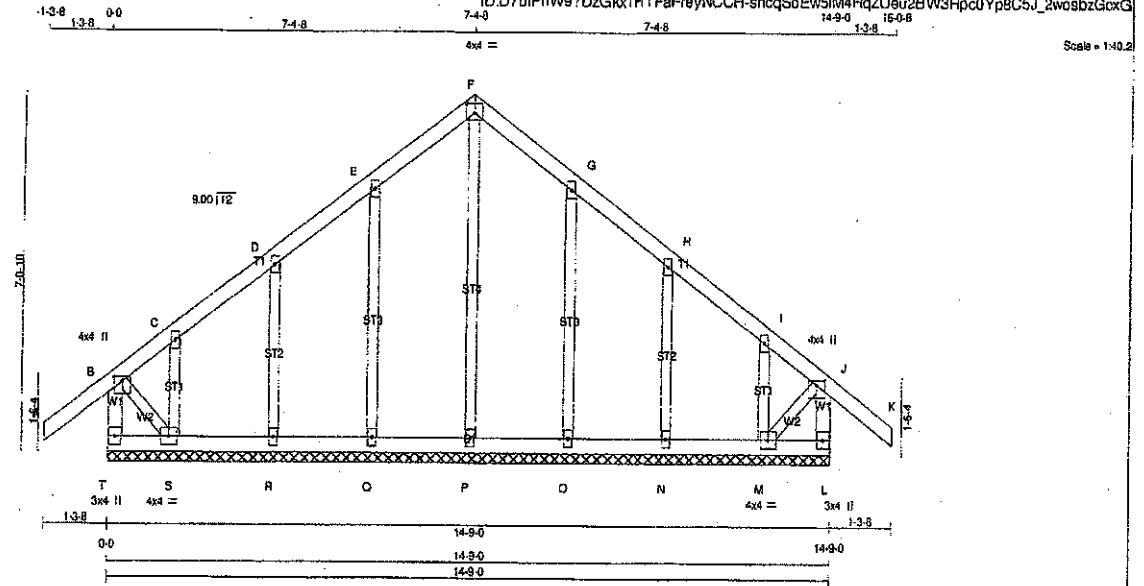
Structural component only
DWG# T-1911516

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401607	G100	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 69 lb

[M]

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
T - B	2x4	DRY	No.2	SPF	
A - F	2x4	DRY	No.2	SPF	
F - K	2x4	DRY	No.2	SPF	
L - J	2x4	DRY	No.2	SPF	
T - L	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
ALL GABLE WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER					
GABLE STUDS SPACED AT	2'-0" O.C.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (M-N:2), WB=0.13/1.00 (F-P:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1657 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)

JSI METAL= 0.12 (E) (INPUT = 1.00)

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, G, H, I					
C TMW+w	MT20	2.0	4.0		
F TTW+p	MT20	4.0	4.0	2.25	2.00
J TMVW+p	MT20	4.0	4.0	1.00	2.00
L BMV1+p	MT20	3.0	4.0		
M BMW1+w	MT20	4.0	4.0		
N, O, P, Q, R					
N BMW1+w	MT20	2.0	4.0		
S BMWV1+1	MT20	4.0	4.0		
T BMV1+p	MT20	3.0	4.0		

LOADING

TOTAL LOAD CASES: (4)

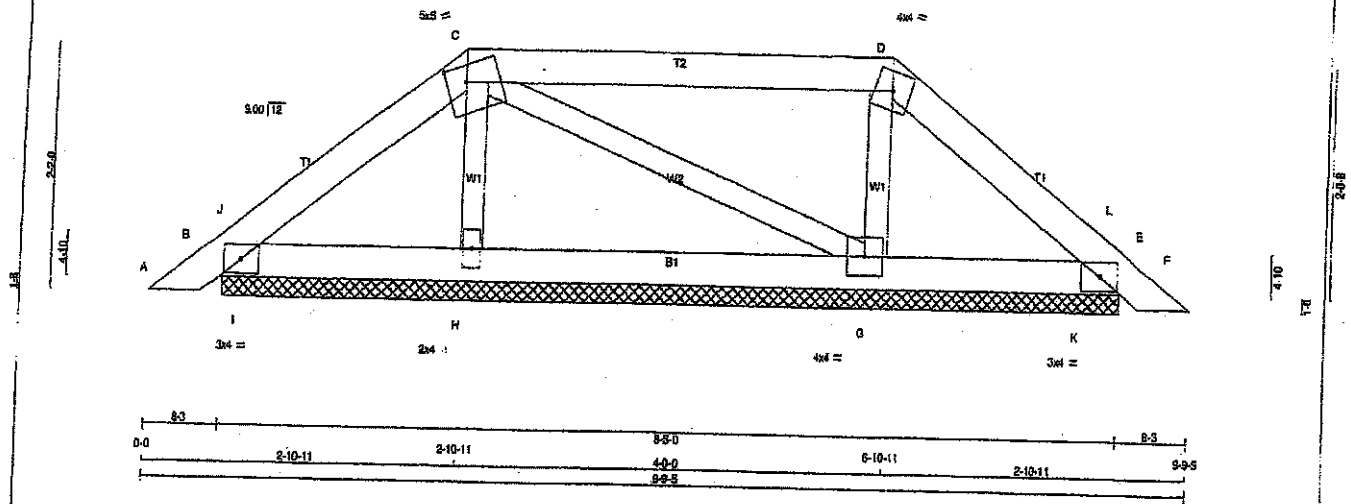
CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
T-B	-310 / 0	0.0	0.0	0.03 (1)	7.81	P-F	-157 / 0
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	Q-E	-229 / 0
B-C	-59 / 0	-102.1	-102.1	0.13 (1)	8.25	R-D	-210 / 0
C-D	-18 / 0	-102.1	-102.1	0.05 (1)	6.25	S-C	-88 / 0
D-E	-18 / 0	-102.1	-102.1	0.08 (1)	6.25	O-G	-229 / 0
E-F	-25 / 0	-102.1	-102.1	0.06 (1)	6.25	N-H	-210 / 0
F-G	-25 / 0	-102.1	-102.1	0.06 (1)	6.25	M-I	-88 / 0
G-H	-18 / 0	-102.1	-102.1	0.08 (1)	6.25	B-S	0 / 30
H-I	-18 / 0	-102.1	-102.1	0.05 (1)	6.25	M-J	0 / 30
I-J	-88 / 0	-102.1	-102.1	0.19 (1)	6.25		
J-K	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
L-J	-310 / 0	0.0	0.0	0.03 (1)	7.81		
T-S	0 / 0	-38.5	-38.5	0.02 (3)	10.00		
S-R	0 / 19	-38.5	-38.5	0.03 (2)	10.00		
R-D	0 / 14	-38.5	-38.5	0.02 (3)	10.00		
O-P	0 / 10	-38.5	-38.5	0.02 (3)	10.00		
P-Q	0 / 10	-38.5	-38.5	0.02 (3)	10.00		
Q-N	0 / 14	-38.5	-38.5	0.02 (3)	10.00		
N-M	0 / 19	-38.5	-38.5	0.03 (2)	10.00		
M-L	0 / 0	-38.5	-38.5	0.02 (3)	10.00		



Structural component only
DWG# T-1911497

JOB NAME 403045	TRUSS NAME P1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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Scale = 1:17.5



LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 28 = 56 LB				
N. L. G. A. RULES		SIZE	LUMBER	DESCR.	SPF	BEARINGS					DESIGN CRITERIA							
A - C	2x4	DRY	No.2	SPF	FACTORED					MAXIMUM FACTORED								
C - D	2x4	DRY	No.2	SPF	GROSS REACTION					GROSS REACTION								
D - F	2x4	DRY	No.2	SPF	JT	VERT	HORZ	DOWN	UPLIFT	IN-SX	IN-SX	SPECIFIED LOADS:						
B - E	2x4	DRY	No.2	SPF	B	257	0	257	0	0	8-5-0	TOP CH. LL	=	23.0	PSF			
					E	243	0	243	0	0	8-5-0	DL	=	8.0	PSF			
					H	388	0	388	0	0	8-5-0	BOY CH. LL	=	10.5	PSF			
					G	417	0	417	0	0	8-5-0	DL	=	7.0	PSF			
															TOTAL LOAD	=	52.5	PSF
ALL WEBS 2x3 DRY SEASONED LUMBER.					No.2	SPF												

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	14B14	MT20	3.0	4.0			
C	TTWW-m	MT20	5.0	6.0	1.75	2.25	
D	TTW-m	MT20	4.0	4.0			
E	14B14	MT20	3.0	4.0			
G	BMW14	MT20	4.0	4.0			
H	BMW14w	MT20	2.0	4.0			

UNFACTORED REACTIONS							
1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	183	130/0	16/0	0/0	0/0	37/0	0/0
E	173	122/0	16/0	0/0	0/0	35/0	0/0
H	295	144/0	73/0	0/0	0/0	78/0	0/0
G	316	161/0	73/0	0/0	0/0	82/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LC1 (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. LC1 (LBS)	MAX. UNBRACED LENGTH (LBS)	MEMB.
FR-TO					FR-TO				FR-TO
A-B	0/17	-102.1	-102.1	0.03 (1)	10.00	H-C	-232/0	0.03 (1)	10.00
B-J	-75/0	-102.1	-102.1	0.02 (1)	6.25	C-G	-21/0	0.04 (1)	6.25
J-C	-98/0	-102.1	-102.1	0.06 (1)	6.25	G-D	-255/0	0.04 (1)	6.25
C-D	-42/0	-102.1	-102.1	0.28 (1)	6.25	I-J	-148/0	0.00 (1)	6.25
D-L	-75/0	-102.1	-102.1	0.06 (1)	6.25	K-L	-148/0	0.00 (1)	6.25
L-E	-51/0	-102.1	-102.1	0.02 (1)	6.25				
E-F	0/17	-102.1	-102.1	0.03 (1)	10.00				
B-I	0/75	-38.5	-38.5	0.07 (1)	10.00				
I-H	0/75	-38.5	-38.5	0.09 (2)	10.00				
H-G	0/61	-38.5	-38.5	0.09 (2)	10.00				
G-K	0/37	-38.5	-38.5	0.09 (2)	10.00				
K-E	0/57	-38.5	-38.5	0.08 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 8.0/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2010, CBC 2012
- CSA C88-08, CSA C88-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS: TC=0.28/1.00 (C-D:1), BC=0.08/1.00 (H-C:1), WB=0.04/1.00 (D-G:1), SS=0.16/1.00 (C-D:1)

DOL LUMBER=1.00 NAL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

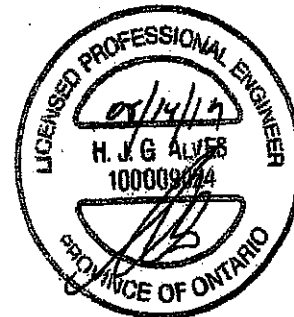
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1857 708 1857 1658

PLATE PLACEMENT TOL = 0.250 inches

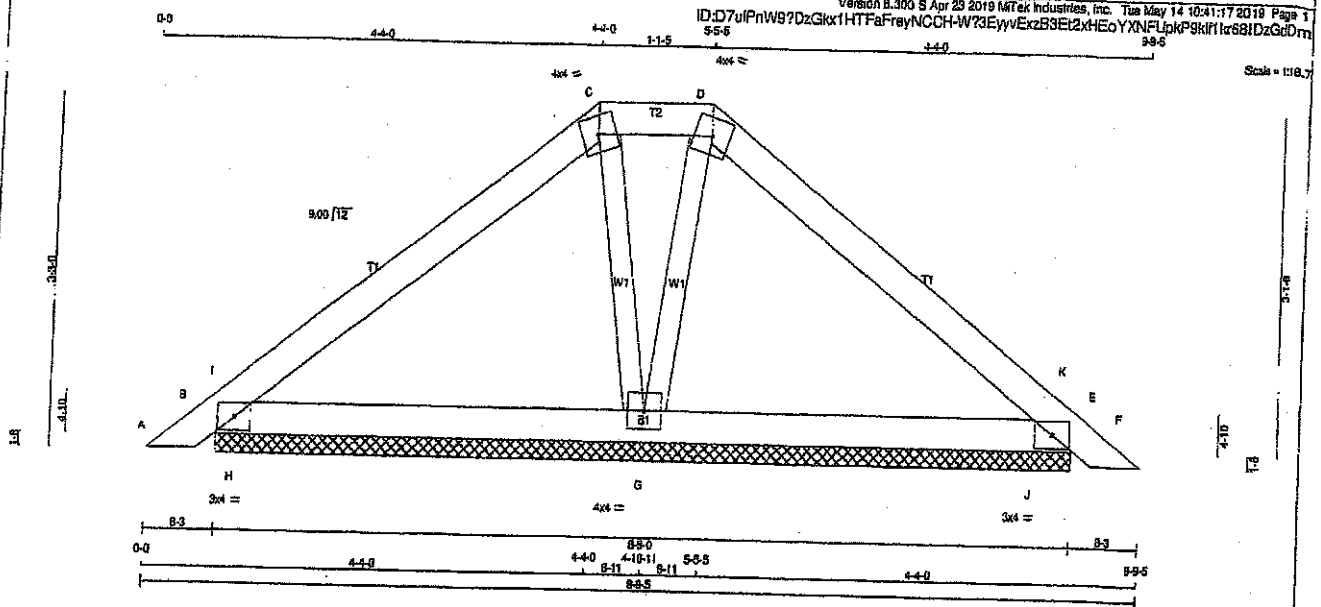
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.50)
JSI METAL = 0.05 (H) (INPUT = 1.00)



Structural component only
DWG# T-1911479

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
403045	P2	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.300 5 Apr 2019 Mitek Industries, Inc. Tue May 14 10:41:17 2019 Page 1					
ID: D7uIPnW8?DzGkx1HTFaFreyNCOH-W73EyyvExzBSEL2XHEoYXNfLpKp9kInr68IDzGdDm					
5-5-5					
8-9-6					



LUMBER			
N. L. G. A. RULES	SIZE	DRY	LUMBER
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table in inches)				
JT	TYPE	PLATES	W	LEN
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TTW-m	MT20	4.0	4.0
E	TMB1-I	MT20	3.0	4.0
G	BMMW1-I	MT20	4.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
DOWN	UP	DOWN	UP	IN-SX		IN-SX	
JT							
B	401	0	401	0	8-5-0	8-5-0	
E	390	0	390	0	8-5-0	8-5-0	
G	514	0	514	0	8-5-0	8-5-0	

UNFACTORED REACTIONS

1ST CASE	MAX/MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	283	182/0	43/0	0/0	0/0	67/0	0/0
E	285	179/0	42/0	0/0	0/0	65/0	0/0
G	390	197/0	52/0	0/0	0/0	102/0	0/0

BEARING MATERIAL TO BE 6PF NO.2 OR BETTER AT JOINT(S) B, E, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNSPACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX. FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (LBS)	MAX. (LBS)	CS1 (LBS)	UNBRAC. LENGTH (LBS)	MEMB.	FORCE (LBS)	MAX. (LBS)	CS1 (LBS)
FR-TO						FR-TO			
A-B	0/17	-102.1	-102.1	0.03 (1)	10.00	C-G	-127/0	0.02 (1)	
B-I	-117/0	-102.1	-102.1	0.05 (1)	8.25	G-D	-129/0	0.02 (1)	
I-C	-105/0	-102.1	-102.1	0.18 (1)	8.25	H-I	-304/76	0.00 (1)	
C-D	-98/0	-102.1	-102.1	0.02 (1)	6.25	J-K	-327/56	0.00 (1)	
D-K	-157/0	-102.1	-102.1	0.17 (1)	8.25				
K-E	-103/0	-102.1	-102.1	0.06 (1)	8.25				
E-F	0/17	-102.1	-102.1	0.03 (1)	10.00				
B-H	0/126	-38.5	-38.5	0.14 (1)	10.00				
H-G	0/126	-38.5	-38.5	0.20 (2)	10.00				
G-J	0/119	-38.5	-38.5	0.19 (2)	10.00				
J-E	0/119	-38.5	-38.5	0.15 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.03/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2015, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 3.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.18/1.00 (C-1), BC=0.20/1.00 (G-H-2), WS=0.02/1.00 (D-G-1), SS=0.28/1.00 (E-J-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

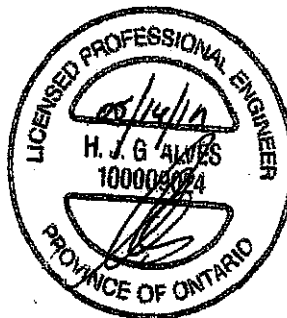
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1556

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

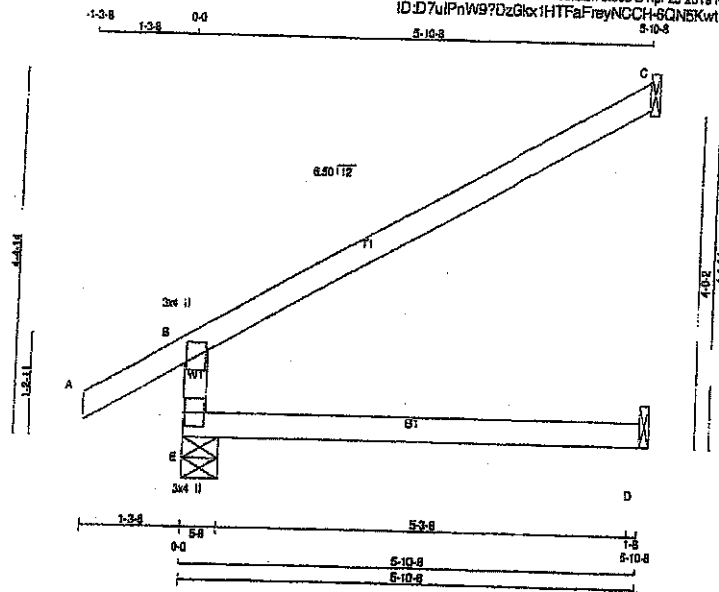
JSI GRIP = 0.30 (B) (INPUT = 0.90)
JSI METAL = 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911480

JOB NAME 403045	TRUSS NAME U1	QUANTITY 17	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington



Scale = 1/4" = 1'-0"

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	DRY	LUMBER
E - B	2x4	DRY	No.2		
A - C	2x4	DRY	No.2		
E - D	2x4	DRY	No.2		

DRY: SEASONED LUMBER.

PLATES (table in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	SMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
E	646	0	646	0	5-8	6-8		
C	225	0	225	0	1-8	1-8		
D	94	0	119	0	1-8	1-8		

SEE MITEK STANDARD DETAIL 897791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE		MAXIMUM COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	LIVE				
E	473	282/0	72/0	0/0	0/0	0/0	109/0	0/0
C	154	128/0	0/0	0/0	0/0	0/0	26/0	0/0
D	85	0/0	51/0	0/0	0/0	0/0	34/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	FACTORED VERT. LOAD (LC1)	MAX. CSH (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSH (LC)
FR-TO		FROM TO				FR-TO		
E-B	-514/0	0.0	8.0	0.21 (3)	7.81			
A-B	0/33	-102.1	-102.1	0.13 (1)	10.00			
B-C	-36/0	-102.1	-102.1	0.00 (1)	6.25			
E-D	0/0	-33.5	-33.5	0.21 (3)	10.00			

TOTAL WEIGHT = 17 X 17 = 289 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 32.5 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLES WITH:
- PART 9 OF NBC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TFC 2011, TFC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/800 (0.20")
CALCULATED VERT. DEFL. (LL) = L/899 (0.05")
ALLOWABLE DEFL. (LL) = L/800 (0.20")
CALCULATED VERT. DEFL. (LL) = L/930 (0.08")

CSH TO = 0.60/1.00 (B-C-1), BC = 0.21/1.00 (D-E-2), WB = 0.00/1.00 (n/c), SSI = 0.28/1.00 (B-C-1)

DOL LUMBER = 1.00 NAIL = 1.00 LS BEND = 1.10 COMP = 1.10 SHEAR = 1.10 TENS = 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

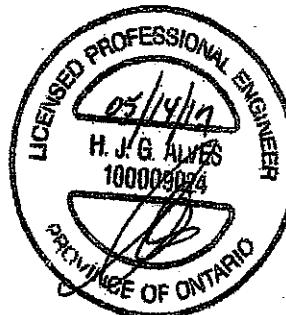
NAIL VALUES

PLATE GRP(DRY) SHEAR SECTION (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.22 (E) (INPUT = 0.99)
JSI METAL = 0.15 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911476

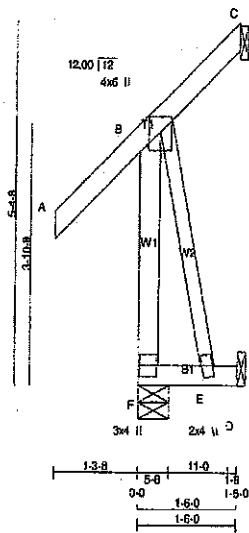
JOB NAME 403060	TRUSS NAME J4	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:vk_J5VXL54Y8bNpe87hlyMICK-LOsAm60N96Re0aMpCwgYrdspAvYqdoXn5G1wyzGcaJ

1-3-8 0-0 1-5-0 1-5-0

Scale = 1:30.3



LUMBER
N L G. A. RULES
CHORDS SIZE LUMBER
F - B 2x4 DRY No.2
A - C 2x4 DRY No.2
F - D 2x4 DRY No.2
ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMVW+p MT20 4.0 6.0 2.75 2.00
E BMW+p MT20 2.0 4.0
F BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	RECORD
JT	VERT	GROSS REACTION	GROSS REACTION	BRG	BRG
F	314	0	314	0	5-8
C	10	0	10	0	1-8
D	29	0	37	0	1-8

SEE MITEK STANDARD DETAIL B87791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS FACTORED UPLIFT

UNFACTORED REACTIONS		MAX./MIN. COMPONENT REACTIONS					
JT	1ST LCASE	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	222	162/0	16/0	0/0	0/0	44/0	0/0
C	7	6/36	0/0	0/0	0/0	1/0	0/0
D	26	0/0	16/0	0/0	0/0	11/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS		FACTORED		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED	MAX. FACTORED
F-B	-285/0	0.0	0.0	0.07 (1)	7.81	B-E	0/0
A-B	0/50	-102.1	-102.1	0.14 (1)	10.00		
B-C	-47/0	-102.1	-102.1	0.14 (1)	6.25		
F-E	0/0	-38.5	-38.5	0.02 (3)	10.00		
E-D	0/0	-38.5	-38.5	0.02 (3)	10.00		

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/998 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/998 (0.00")

CS: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (E-F:3), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

SOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	618	354	1867
	788	1987	1866

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.20 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911506

JOB NAME

TRUSS NAME

QUANTITY

PLY

JOB DESC.

GREEN PARK HOMES

DRWG NO.

403060

J5

9

1

TRUSS DESC.

Tamarack Roof Truss, Burlington

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ID: wk_15VXL54fY8brNpe87nlyMiCK-paQZzS1?wPZVekX0ldQv52A1YafnZ42g0l0bSQzGca

Scale = 1:20.0

LUMBER

N.L.G.A. RULES

CHORDS SIZE

F - B 2x4 DRY No.2 SPF

A - C 2x4 DRY No.2 SPF

F - D 2x4 DRY No.2 SPF

ALL WEBS 2x3 DRY No.2 SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

B TM/VW+p MT20 4.0 6.0 2.75 2.00

E BMV+p MT20 2.0 4.0

F BMV1+p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION

MAXIMUM FACTORED GROSS REACTION

INPUT BRG

RECORD BRG

JT VERT HORZ DOWN HORZ UPLIFT IN-SX IN-SX

F 314 0 314 0 0 5-8 5-8

C 10 0 10 0 -5-8 1-8 1-8

D 29 0 37 0 0 1-8 1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

PROVIDE ANCHORAGE AT BEARING JOINT C FOR 150 LBS. FACTORED UPLIFT

UNFACTORED REACTIONS

1ST LOASE MAX/MIN COMPONENT REACTIONS

JT COMBINED SNOW LIVE PERMLIVE WIND DEAD SOIL

F 222 182/0 16/0 0/0 0/0 44/0 0/0

C 7 6/38 0/0 0/0 0/0 1/0 0/0

D 28 0/0 16/0 0/0 0/0 11/0 0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (5)

CHORDS

MAX. FACTORED FORCE

FACTORED VERT. LOAD

LC1 MAX

MAX. MEMB. FORCE

UNBRAC LENGTH

FR-TO

F-B -285/0 0.0 0.0 0.03 (1) 7.81 B-E 0/0 0.00 (1)

A-B 0/50 -102.1 -102.1 0.14 (1) 10.00

B-C -47/0 -102.1 -102.1 0.14 (1) 6.25

F-E 0/0 -38.5 -38.5 0.02 (3) 10.00

E-D 0/0 -38.5 -38.5 0.02 (3) 10.00

WEBS

MAX. FACTORED FORCE

MAX. MEMB. FORCE

UNBRAC LENGTH

FR-TO

F-B -285/0 0.0 0.0 0.03 (1) 7.81 B-E 0/0 0.00 (1)

A-B 0/50 -102.1 -102.1 0.14 (1) 10.00

B-C -47/0 -102.1 -102.1 0.14 (1) 6.25

F-E 0/0 -38.5 -38.5 0.02 (3) 10.00

E-D 0/0 -38.5 -38.5 0.02 (3) 10.00

CANTILEVER ANALYSIS HAS BEEN CONSIDERED IN THIS DESIGN

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

PART 9 OF CBC 2015, CBC 2012

CSA 088-09, CSA 088-14

TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT-OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")

ALLOWABLE DEFL.(LL) = L/360 (0.19")

CALCULATED VERT. DEFL.(LL) = 1/999 (0.00")

CSI: TC=0.14/1.00 (A-B:1), BC=0.02/1.00 (E-F:2), WB=0.00/1.00 (B-E:1), SS=0.08/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1667 788 1997 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.20 (B) (INPUT = 0.90)

JSI METAL=0.07 (B) (INPUT = 1.00)

TOTAL WEIGHT = 9 X 10 = 87 lb

OK w/ 1/14/19

WHL

LICENSED PROFESSIONAL ENGINEER

05/14/19

H. J. G. ALVES

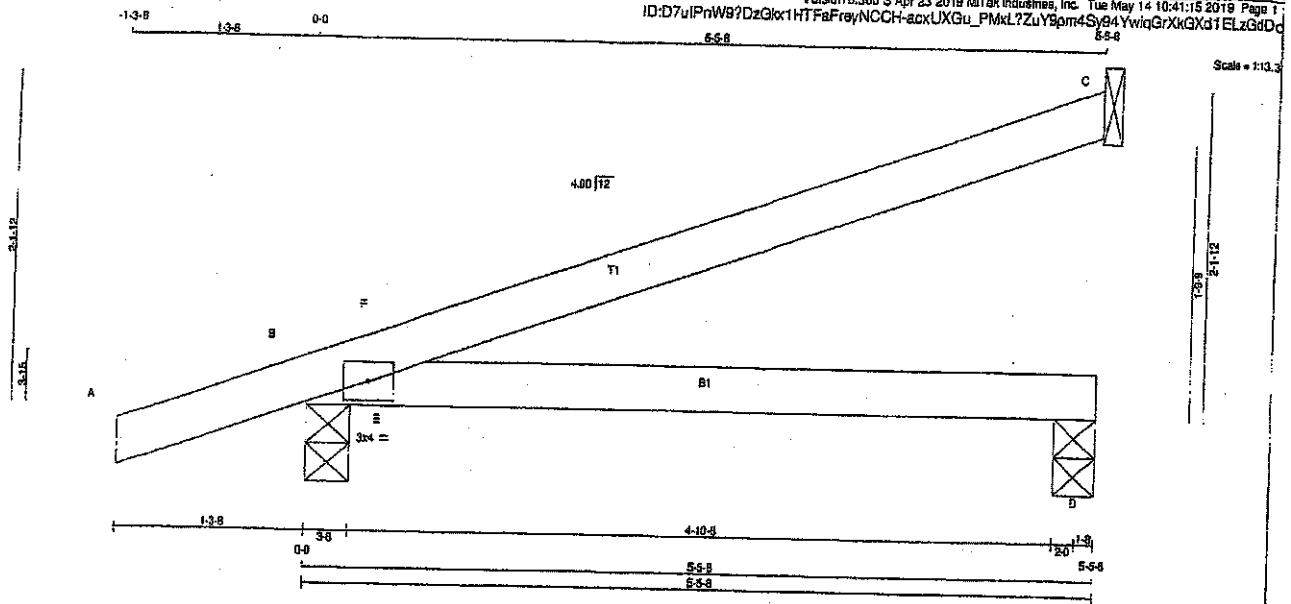
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PROVINCE OF ONTARIO

Structural component only

DWG# T-1911507

JOB NAME 403045	TRUSS NAME J90	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S Apr 23 2019 MITak Industries, Inc. Tue May 14 10:41:15 2019 Page 1		
ID: D7uIPnW9?DzGKx1HTFaFreyNCCt-axUXGu_PmXL?ZuY9pm4Sy94YwqGrXKXGd1ELZGdDc				Scale = 1:13.3		



LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
B - D 2x4 DRY
No.2
No.2
DRY, SEASONED LUMBER.

PLATES (table is in inches)
JT TYPE PLATES W LEN Y X
B TMB14 MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQ'D
GROSS REACTION	GROSS REACTION	DOWN	UP	DOWN
JT	VERT	HDRZ	DOWN	HDRZ
C	250	0	250	0
B	520	0	520	0
D	133	0	145	0

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	175	133 / 0	9 / 0	0 / 0	0 / 0	33 / 0	0 / 0
B	380	238 / 0	57 / 0	0 / 0	0 / 0	87 / 0	0 / 0
D	111	25 / 0	49 / 0	0 / 0	0 / 0	38 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF) FROM TO	MEMB.	FORCE (LBS) MAX
A-B	0 / 20	-102.1 -102.1 0.13 (1)	10.00	0.00 (1)
B-F	-51 / 0	-102.1 -102.1 0.13 (3)	8.25	0.00 (1)
F-C	0 / 5	-102.1 -102.1 0.42 (1)	10.00	0.00 (1)
B-E	0 / 0	-38.5 -38.5 0.35 (1)	10.00	0.00 (1)
E-D	0 / 0	-38.5 -38.5 0.31 (1)	10.00	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/G

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015

THIS DESIGN COMPLES WITH:
- PART 9 OF NBC2010, NBC2012
- CSA 086-06, CSA 089-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/942$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/489$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WS=0.00/1.00 (E-F:1), SS=0.28/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

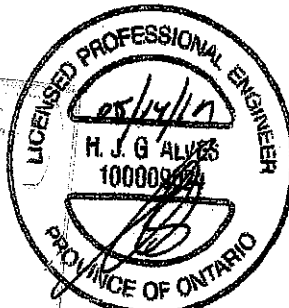
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1855

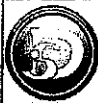
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.31 (B) (INPUT = 0.90)
JSI METAL = 0.09 (B) (INPUT = 1.00)



Structural component only
DWG# T-1911477



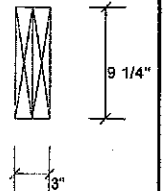
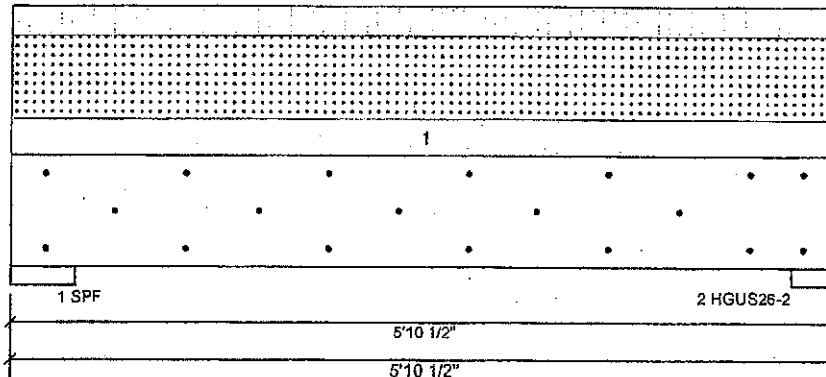
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 6 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	264	327	741	0
2	253	313	710	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	18% 408 / 1376	1784 L	1.25D+1.5S +L
2 - HGUS...	4.000"	23% 391 / 1318	1710 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2016 ft-lb	3'	6039 ft-lb	0.334 (33%)	1.25D+1.5S +L	L
Unbraced	2016 ft-lb	3'	5236 ft-lb	0.385 (39%)	1.25D+1.5S +L	L
Shear	1549 lb	1'2"	3984 lb	0.389 (39%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3535)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.024 (L/2573)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1911519
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-4-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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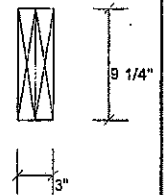
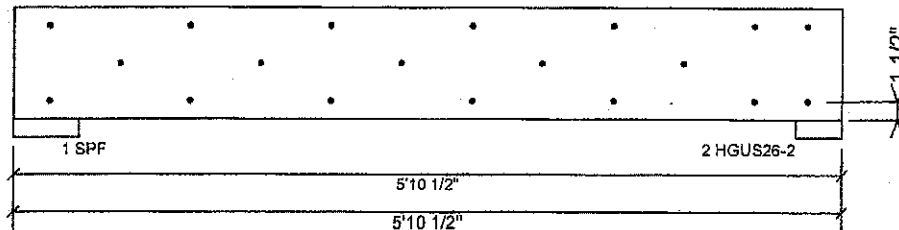
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 7 of 13

B3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



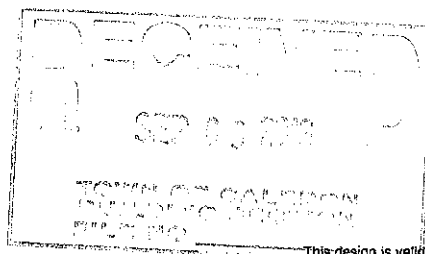
Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	87.4 %
Load	297.3 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 71911579
STRUCTURAL
COMPONENT ONLY 3/2



Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115





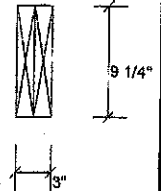
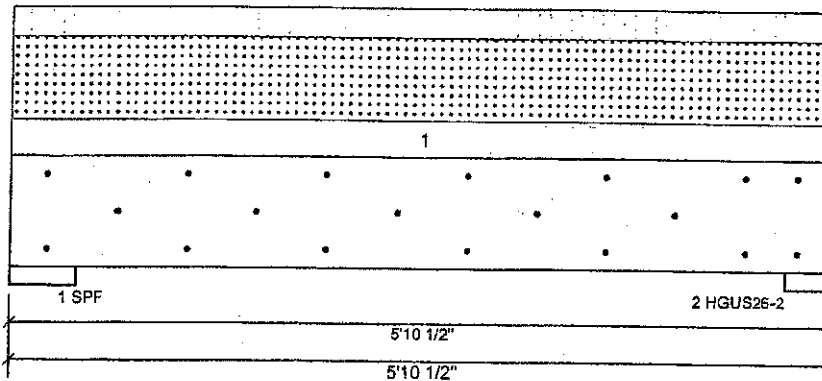
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 10 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Pties:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	269	333	756	0
2	258	319	724	0

Bearings and Factored Reactions

Bearing	Length	Cap. React	D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	18%	416 / 1403	1819 L	1.25D+1.5S +L
2 - HGUS...	4.000"	24%	399 / 1345	1744 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2058 ft-lb	3'	6039 ft-lb	0.341 (34%)	1.25D+1.5S +L	L
Unbraced	2056 ft-lb	3'	5236 ft-lb	0.393 (39%)	1.25D+1.5S +L	L
Shear	1579 lb	1'2"	3984 lb	0.396 (40%)	1.25D+1.5S +L	L
LL Defl inch	0.018 (L/3466)	3'	0.174 (L/360)	0.100 (10%)	S+0.5L	L
TL Defl inch	0.025 (L/2523)	3'	0.174 (L/360)	0.140 (14%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 194521
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-6-8	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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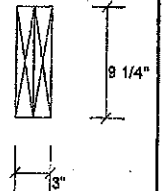
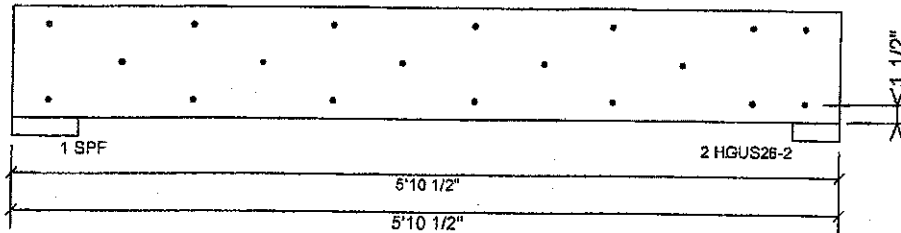
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 11 of 13

B5 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	89.2 %
Load	303.2 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 1791/521
STRUCTURAL
COMPONENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021



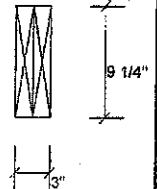
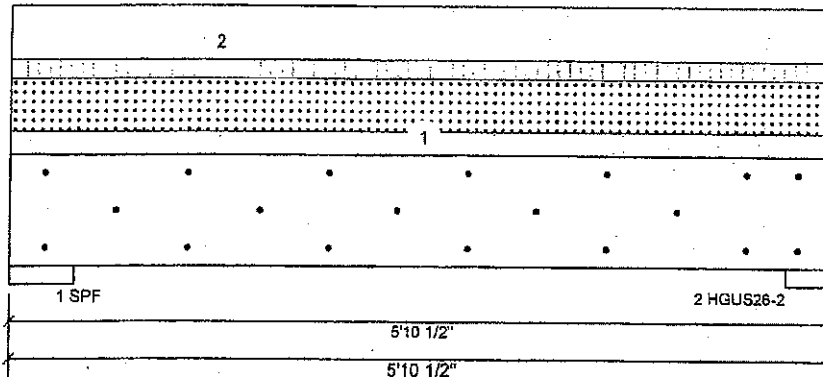
isDesign™

Client:
Project:
Address:Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 8 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	194	331	546	0
2	186	317	523	0

Bearings and Factored Reactions

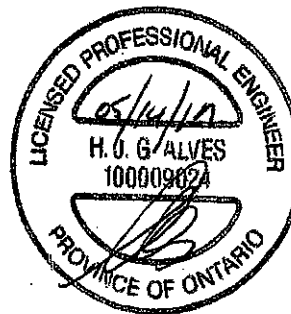
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	14%	413 / 1013	1426 L
2 - HGUS...	4.000"	19%	396 / 971	1367 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1612 ft-lb	3'	6039 ft-lb	0.267 (27%)	1.25D+1.5S	L
Unbraced	1612 ft-lb	3'	6236 ft-lb	0.308 (31%)	1.25D+1.5S	L
Shear	1209 lb	12"	3984 lb	0.303 (30%)	1.25D+1.5S	L
LL Defl inch	0.013 (L/4801)	3'	0.174 (L/360)	0.070 (7%)	S+0.5L	L
TL Defl inch	0.020 (L/3171)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top loads must be supported equally by all plies.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 17911520
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		8-2-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Uniform			Top	30 PLF	0 PLF	0 PLF	0 PLF	

RECEIVED

Manufacturer Info

Tamarack Roof Trusses
3268 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/31/2021





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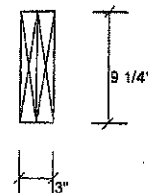
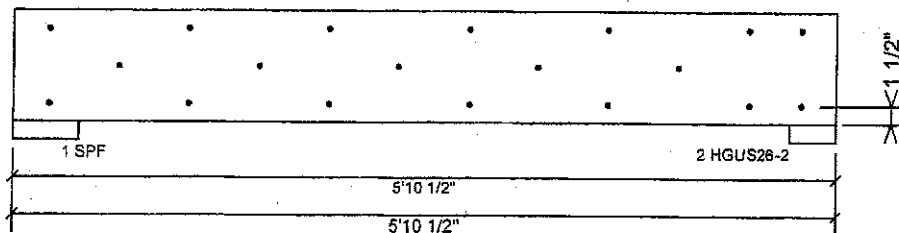
Client:
Project:
Address:

Date: 5/14/2019
Designer:
Job Name: 200598
Project #:

Page 9 of 13

B6 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	64.4 %
Load	218.9 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DWG NO. TAM 1711520
STRUCTURAL
COMPONENT ONLY 2/2

Manufacturer Info	Tamarack Roof Trusses 3268 North Service Rd., ON Canada L7N3G2 (905) 335-1115
	TAMARACK LUMBER INC ALPHA LUMBER GROUP

This design is valid until 12/11/2021

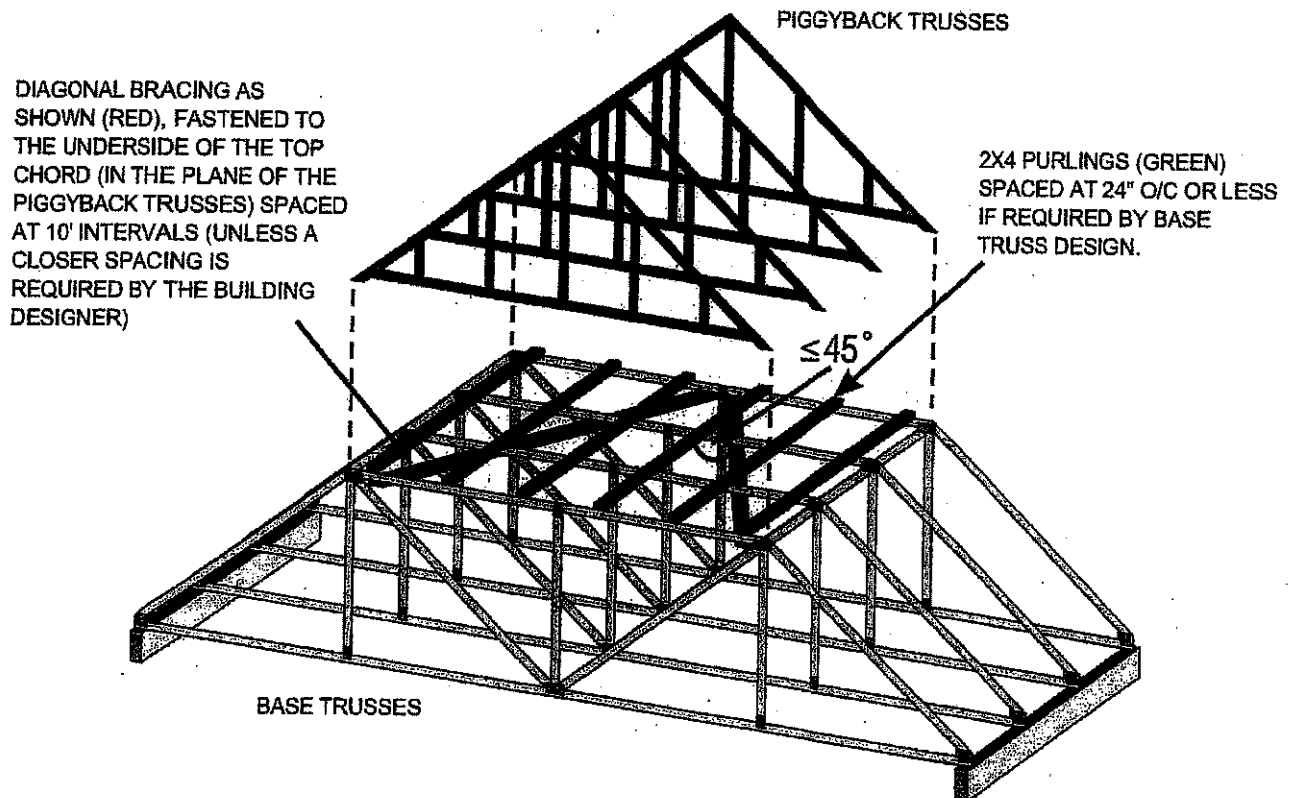


Overview:

Where piggybacks are connected overtop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

Finish: G90 galvanized

Design:

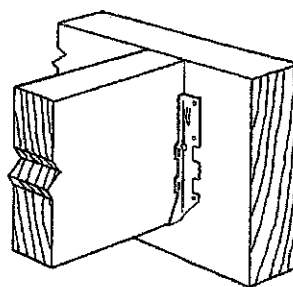
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

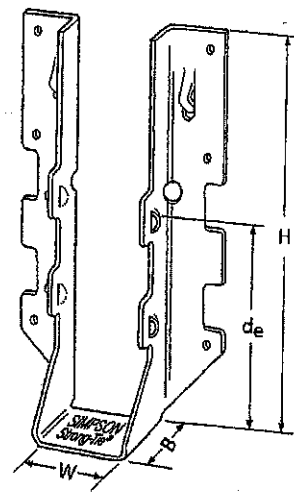
- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire,
10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.

Options:

- These hangers cannot be modified



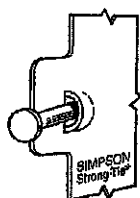
Typical LUS
Installation



LUS28

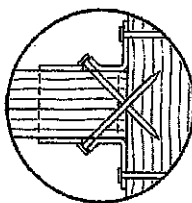
Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 15/16	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 15/16	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 3/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 7/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 3/8	4 7/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 3/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 3/8	6 1/4	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 13/16	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 3/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).

U.S. Patent
5,603,580



Double
Shear
Nailing
Top View.



UNITED
STATES
DESIGN

This technical bulletin is intended for use by the user of the product. It is not a contract. The user of the product is responsible for determining the applicability of this technical bulletin to their specific application. The user of the product is responsible for obtaining the necessary permits and approvals for the use of the product. The user of the product is responsible for the proper installation and use of the product. The user of the product is responsible for the safety of the installation and use of the product.

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15-00001-01-0000-0000-0000

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HUS/LJS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

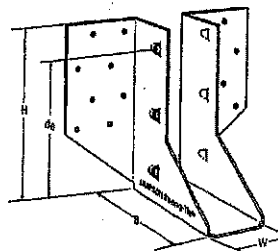
- Factored resistances are in accordance with CSA O86 -14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

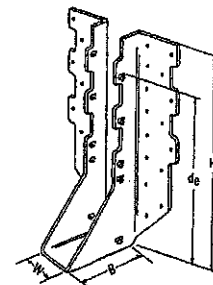
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

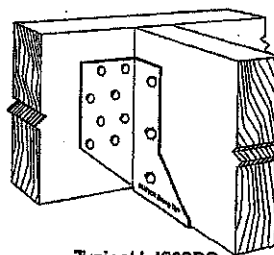
- See current catalogue for options



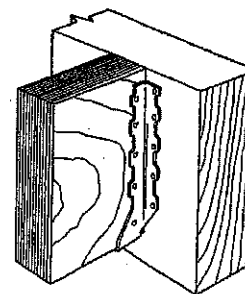
LJS26DS



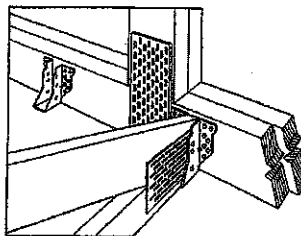
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



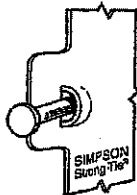
Typical HUS
Installation



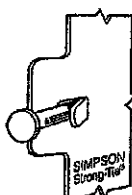
Typical HUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₀ ¹	Face	Joist		D.Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
LJS26DS	18	1 9/16	5	3 1/2	4 5/8	(16) 16d	(6) 16d		2055	4265	1460	4115
HUS26	16	1 5/8	5 5/8	3	3 15/16	(14) 16d	(6) 16d		2705	4940	2065	3875
HUS28	16	1 5/8	7 1/2	3	6 1/2	(22) 16d	(8) 16d		3605	5365	2675	4345
HUS210	16	1 5/8	9 3/4	3	7 3/4	(30) 16d	(10) 16d		4505	5795	4010	4740
HUS1.81/10	16	1 15/16	9	3	8	(30) 16d	(10) 16d		4505	6450	4010	5200

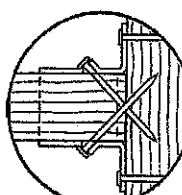
1. d₀ is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



DESIGN

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HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

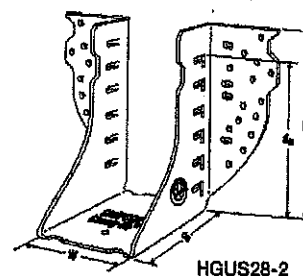
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

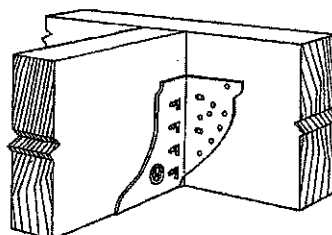
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3½" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

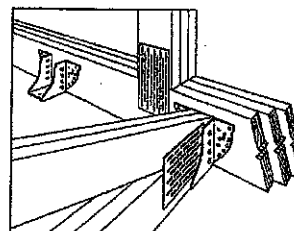
- See current catalogue for options



HGUS28-2



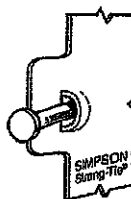
Typical HGUS
Installation



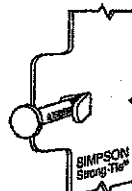
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d ₁	Face	Joist	D-Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS26	12	1½	5½	5	4½	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3¾	5¾	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4½	5½	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6¾	5¾	4	4½	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1½	7½	5	6½	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3¾	7¾	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4½	7¾	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6¾	7¾	4	6½	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3¾	9¾	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4½	9¾	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6¾	9¾	4	8½	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6¾	10¾	4	10½	(56) 16d	(20) 16d	7640	14995	5425	10845
HGUS214-4	12	6¾	12¾	4	11½	(66) 16d	(22) 16d	10130	16400	7195	11645

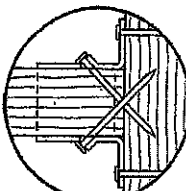
1. d₂ is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,680



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



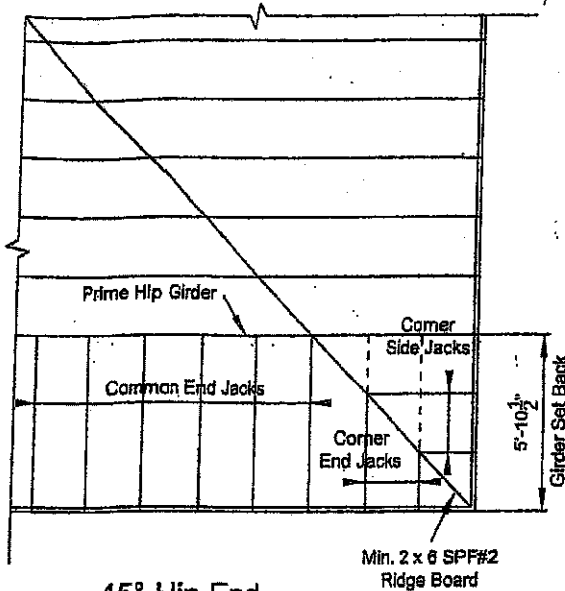
Double
Shear
Nailing
Top View.



DESIGN

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45° Hip End

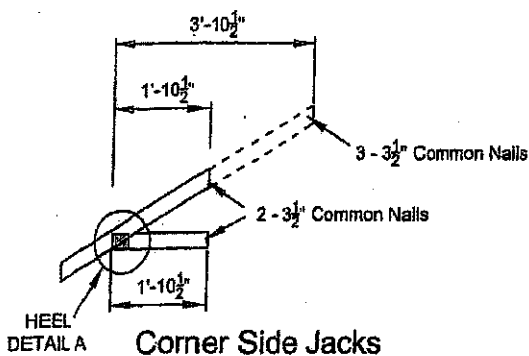
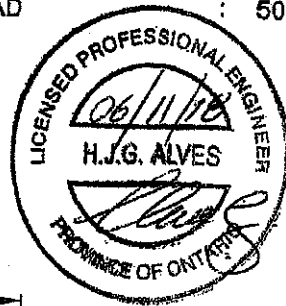
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

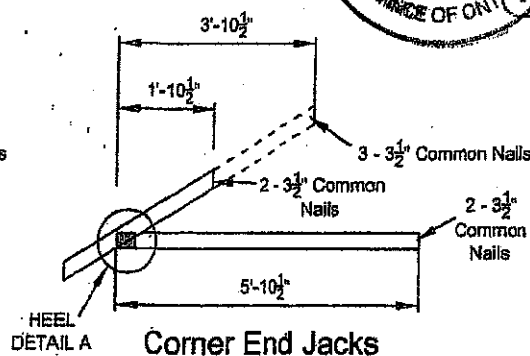
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

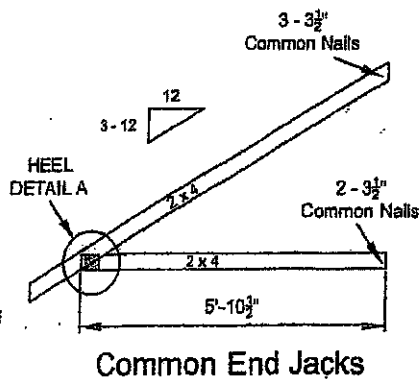
TOTAL LOAD : 50.5 P.S.F.



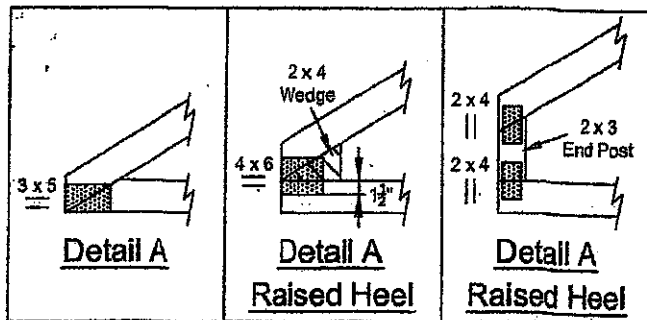
Corner Side Jacks



Corner End Jacks

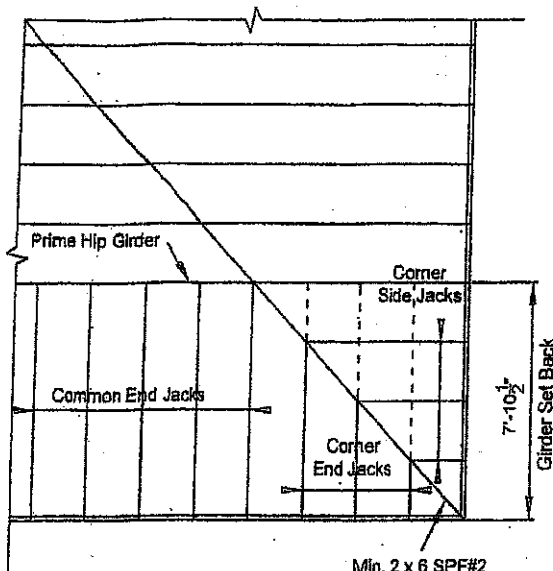


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

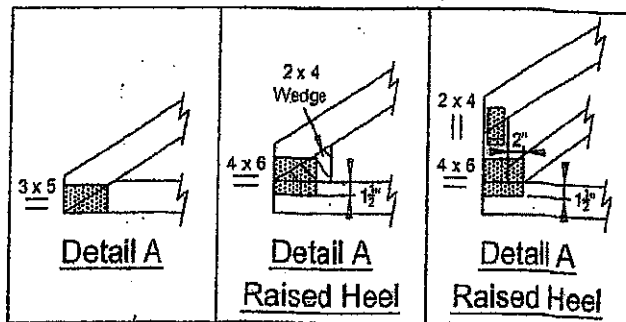
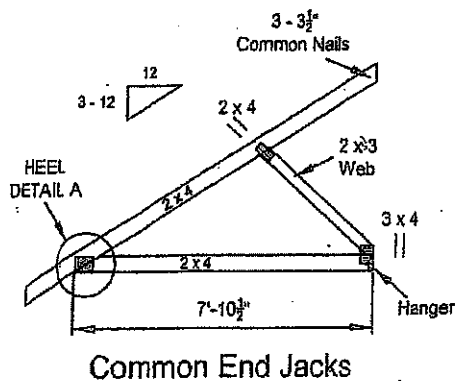
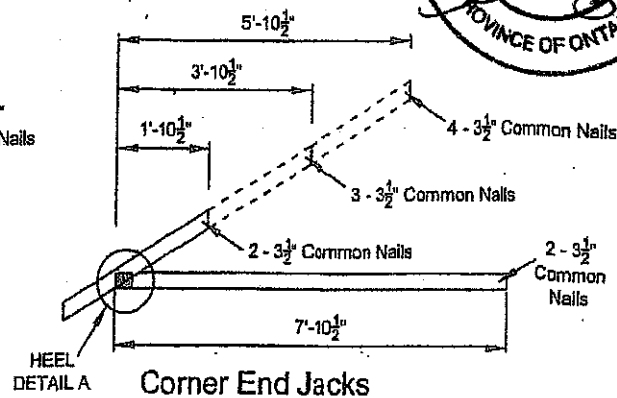
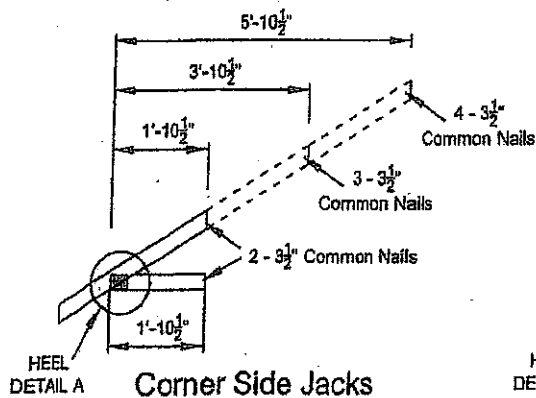
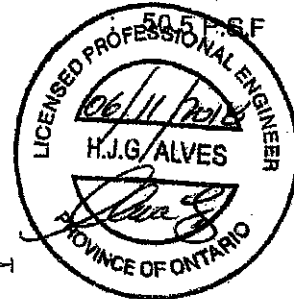
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD

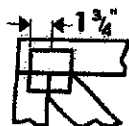


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

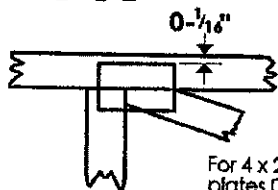
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4\" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

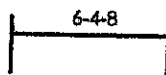


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

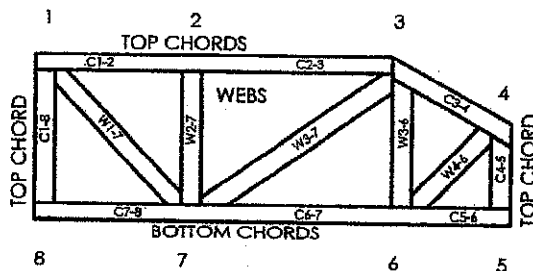
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

T-1300213

Feb 09, 2018