

NOTICE: NO REVISIONS TO BE MADE TO THIS DRAWING

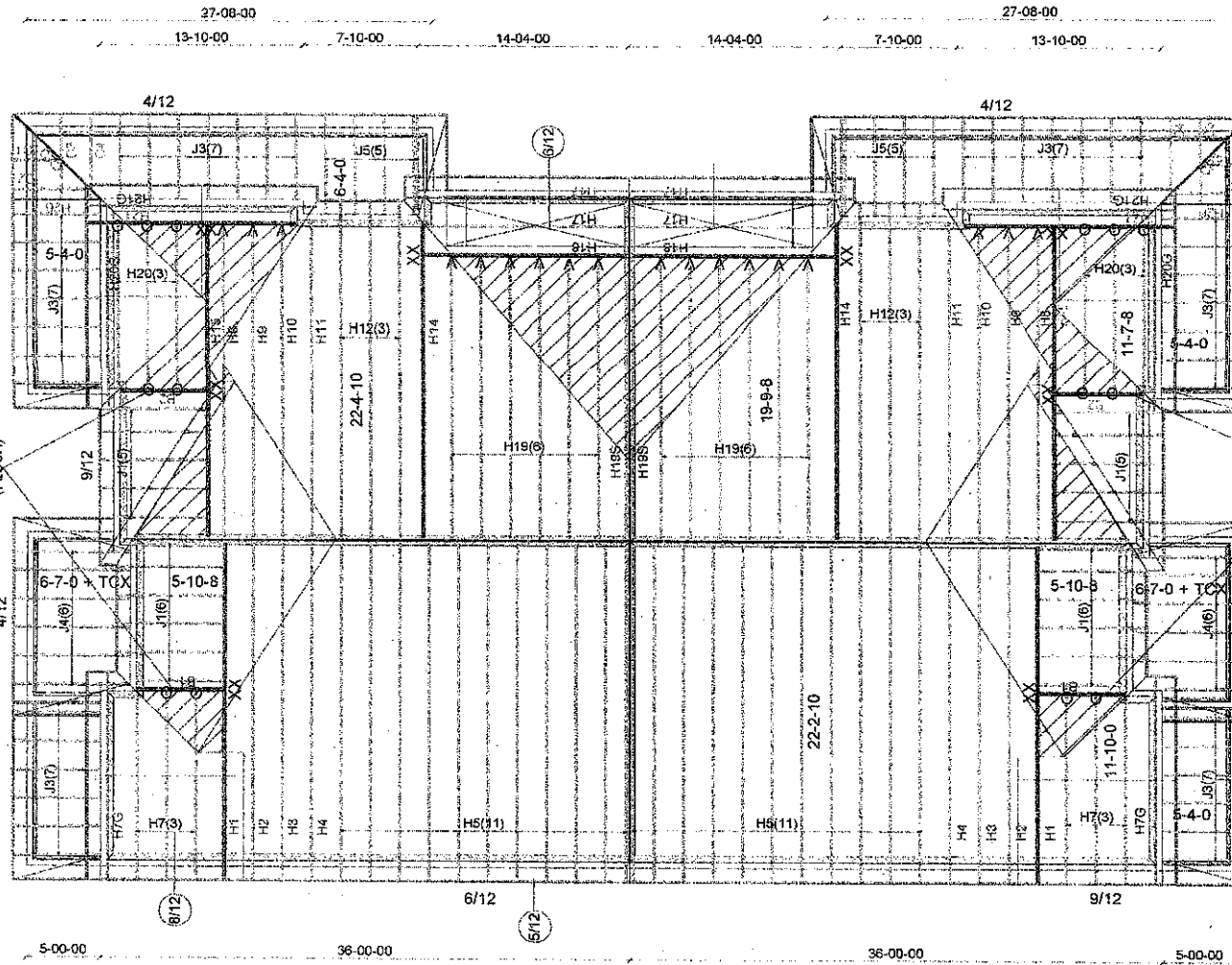
URBAN 12
FRONT ELEV.-2
UNIT 80-3
405483

URBAN 2
FRONT ELEV.-4
UNIT 80-4
405482

SEP 1 2019

12-08-00 23-04-00 9-04-00 11-00-00 13-10-00 18-10-00

2-2X10 SPF#2
BY OTHERS
(FLUSH)



HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

ASPHALT SHINGLES
2X6 BRG./BRK
12" FIN. OH
R.T.M.C.
W/ 2X6 FASCIA

PARTY WALL
2-2X4 STUD
W/ 1-2X4 STUD ABOVE

CALEDON
(29. 6. 0. 7. 4)

BY JESSIE

URBAN 12
FRONT ELEV.-2
UNIT 80-2
405483

URBAN 2
FRONT ELEV.-4
UNIT 80-1
405482

12-08-00 23-04-00 9-04-00 11-00-00 13-10-00 18-10-00

M12626



Job Track: 50033

Plan Log: 201703

Layout ID: 405481

Builder / Location:

GREENPARK HOMES / CALEDON

Project: LAMBERTS LANE PH.2

Date: 9/9/2019

Sales:

Mario DiCano

Designer: AC

Model / Elevation:

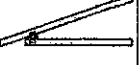
BLOCK 80 / UNITS 80-1 - 80-4

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Milek ver 8.3.0.23-4

 TAMARACK ROOF TRUSSES INC. ALPHA LUMBER GROUP	DELIVERY SHIPLIST				Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: BLOCK 80 Lot #: Elevation: UNITS 80-1, 80-4		Job Track: 50033 PlanLog: 201703 Layout ID: 405482 Ref # Page: 1 of 1 Date: 09/09/2019 Designer: Andrew Conway Sales Rep: Mario DiCano	
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Roof Trusses

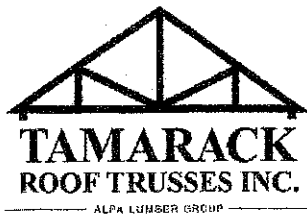
PROFLE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	H1 Half Hip Girder	5 /12	22-02-10	5-03-13	2 x 6	1-03-08	1-00-09 5-03-13	258.6 158.67		
	1	H2 Half Hip	5 /12	22-02-10	6-07-13	2 x 4	1-03-08	1-00-09 6-07-13	98.92 62.50		
	1	H3 Half Hip	5 /12	22-02-10	7-11-13	2 x 4	1-03-08	1-00-09 7-11-13	98.18 61.87		
	1	H4 Half Hip	5 /12	22-02-10	9-03-13	2 x 4	1-03-08	1-00-09 9-03-13	105.08 66.50		
	11	H5 Monopitch	5 /12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-10	1051.67 654.50		
	3	H7 Common	8 /12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	157.12 95.50		
	1	H7G GABLE	8 /12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	48.73 30.83		
	6	J1 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	106.98 64.00		
	7	J3 Jack-Open	4 /12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	103.45 74.67		
	6	J4 Jack-Open	4 /12	6-07-00	3-02-03	2 x 4	1-03-08	7-15 2-10-04	174.1 123.00		

TOTAL # TRUSS= 39 TOTAL BFT OF ALL TRUSSES= 1391.84 BFT. TOTAL WEIGHT OF ALL TRSSES 2200.84 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF
ITEMS= 3



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BLOCK 80
 Lot #:
 Elevation: UNITS 80-2, 80-3

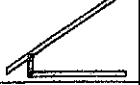
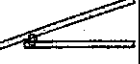
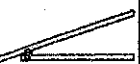
Job Track: 50033
 PlanLog: 201703
 Layout ID: 405483
 Ref #
 Page: 1 of 2
 Date: 09/09/2019
 Designer: Andrew Conway
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	H8 Half Hip	5/12	22-00-02	5-03-13	2 x 4		1-02-07 5-03-13	87.8 55.33		
	1	H9 Half Hip	5/12	22-00-02	6-07-13	2 x 4		1-02-07 6-07-13	94.89 81.17		
	1	H10 Half Hip	5/12	22-00-02	7-11-13	2 x 4		1-02-07 7-11-13	96.28 60.83		
	1	H11 Half Hip	5/12	22-04-10	9-03-13	2 x 4		1-00-09 9-03-13	103.89 65.17		
	3	H12 Monopitch	5/12	22-04-10	10-04-08	2 x 4	1-03-08	1-00-09 10-04-08	286.88 178.50		
	1 2-ply	H14 Monopitch Girder	5/12	22-04-10	10-04-08	2 x 6		1-00-09 10-04-08	278.01 173.00		
	1 2-ply	H15 Hip Girder	5/12	22-00-02	5-03-13	2 x 4 2 x 6		1-02-07 1-03-10	214.38 133.00		
	2	H17 Monopitch Structural Gable	6/12 12/12	13-08-10	8-00-05	2 x 4		1-02-00 6-08-05	115.24 77.33		
	1 2-ply	H18 Jack-Closed Girder	6/12	13-08-10	8-00-05	2 x 4 2 x 6		1-02-00 8-00-05	141.38 88.00		
	6	H19 Common	5/12	19-09-08	10-04-08	2 x 4		2-01-09 10-04-08	531.48 330.00		
	1	H19S GABLE	5/12	18-09-08	10-04-08	2 x 4		2-06-09 10-04-08	101.77 64.33		
	3	H20 Common	8/12	11-07-08	5-08-13	2 x 4		1-04-13 2-03-13	144.13 95.00		
	1	H20G GABLE	8/12	11-07-08	5-08-13	2 x 4	1-03-08	1-04-13 2-03-13	52.6 34.33		
	1 2-ply	H21 Common Girder	8/12	13-00-00	5-08-13	2 x 4 2 x 6	1-03-08 1-03-08	1-04-13 1-04-13	137.09 85.67		

 TAMARACK ROOF TRUSSES INC. ALFA LUMBER GROUP		DELIVERY SHIPLIST	
		Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: BLOCK 80 Lot #: Elevation: UNITS 80-2, 80-3	Job Track: 50033 PlanLog: 201703 Layout ID: 405483 Ref # Page: 2 of 2 Date: 09/09/2019 Designer: Andrew Conway Sales Rep: Mario DiCano

Roof Trusses

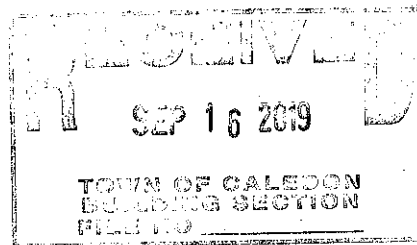
PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	H21G GABLE	8 / 12	13-00-00	5-08-13	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	57.01 36.33		
	1	H26 Monopitch Girder	4 / 12	4-07-00	2-03-03	2 x 4	1-03-08	7-15 2-02-04	17.85 12.67		
	5	J1 Jack-Open	8 / 12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	89.15 53.33		
	14	J3 Jack-Open	4 / 12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	206.9 149.33		
	5	J5 Jack-Open	4 / 12	6-04-00	2-10-03	2 x 4	1-03-08	7-15 2-06-04	85.6 60.00		

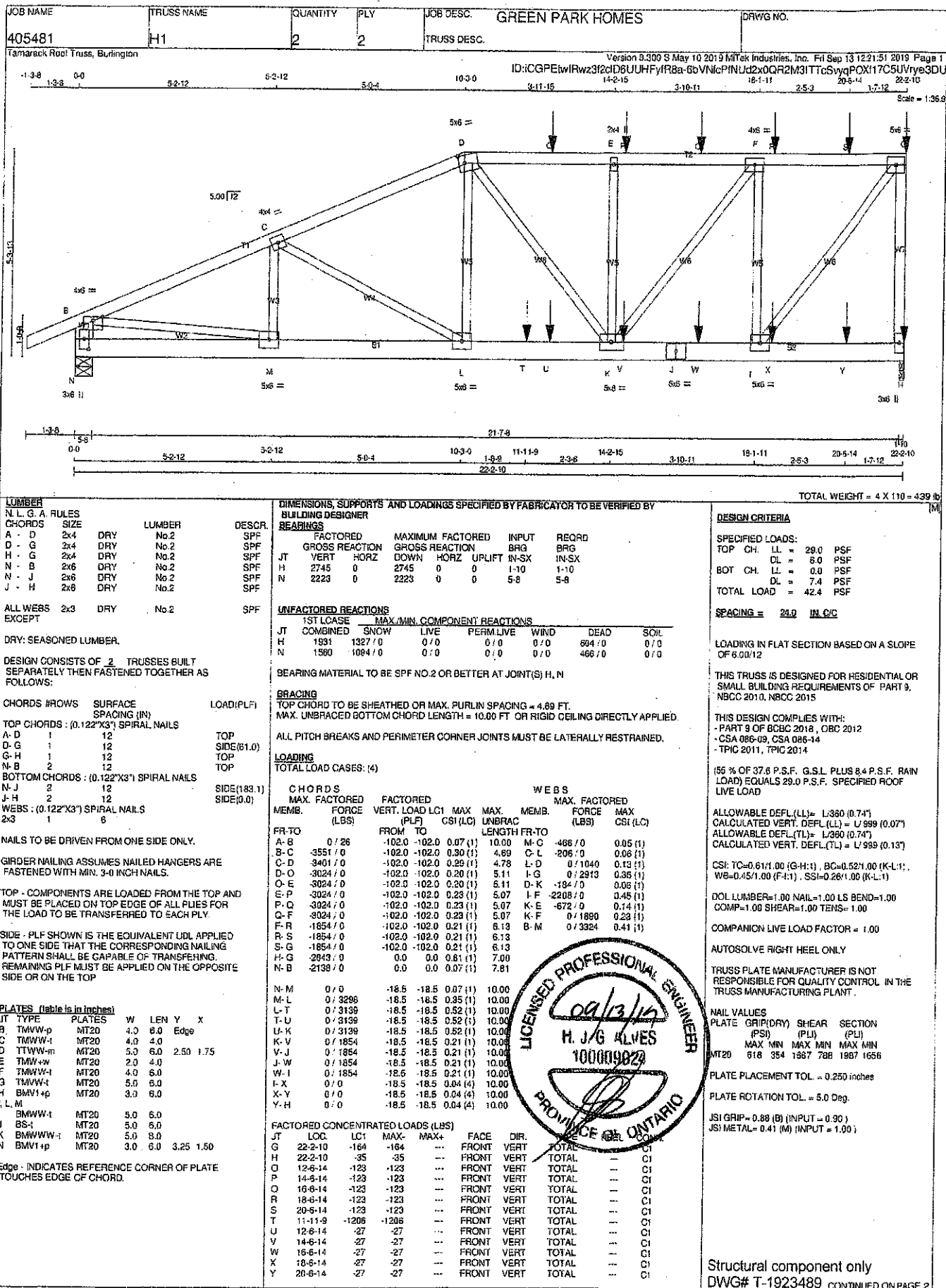
TOTAL # TRUSS= 54 TOTAL BFT OF ALL TRUSSES= 1813.32 BFT. TOTAL WEIGHT OF ALL TRSSES 2844.31 LBS

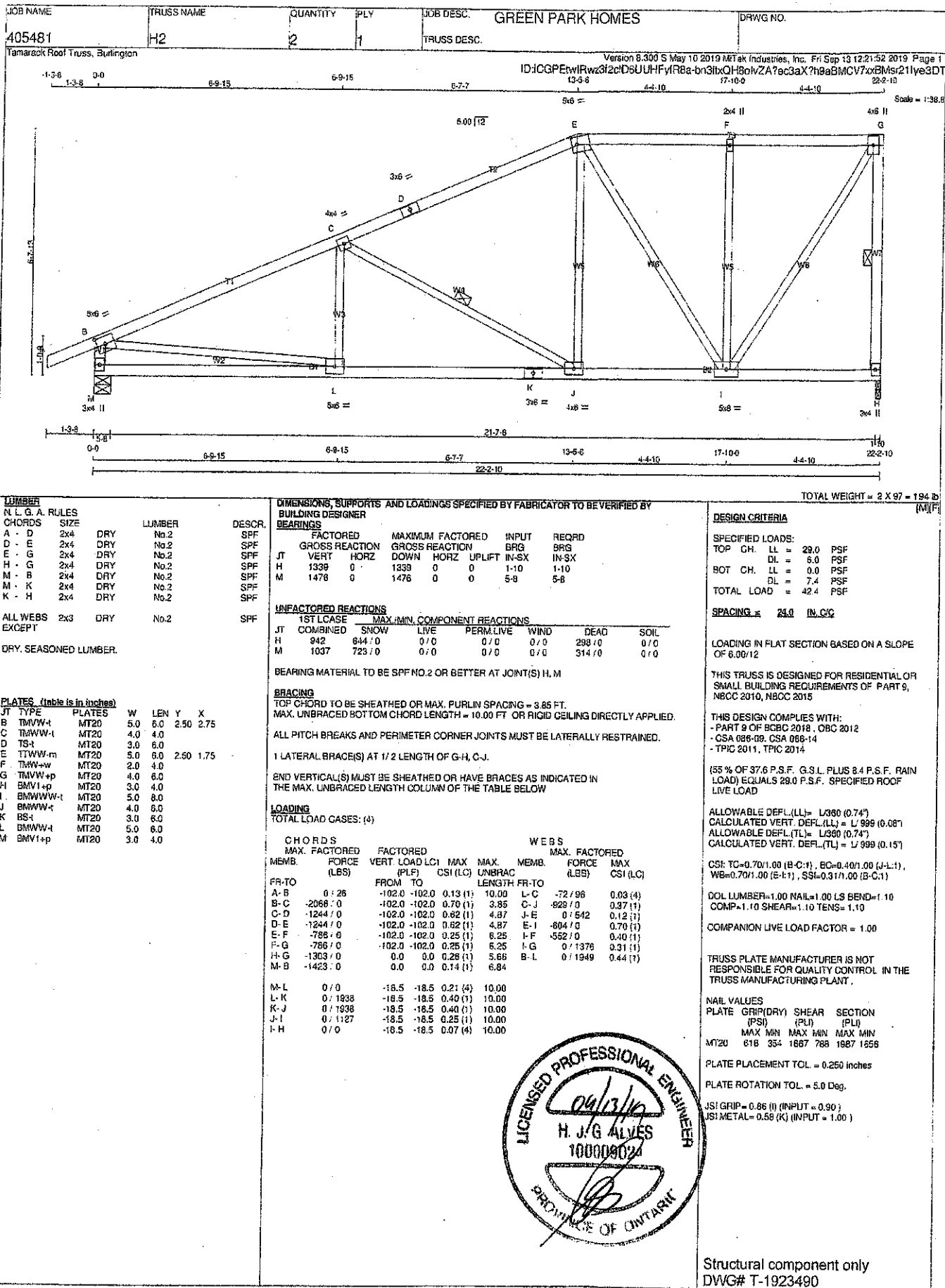
HARDWARE

QTY	TYPE	MODEL	LENGTH
5	Hardware	LUS24	
9	Hardware	LJS26DS	
3	Hardware	HGUS26-2	

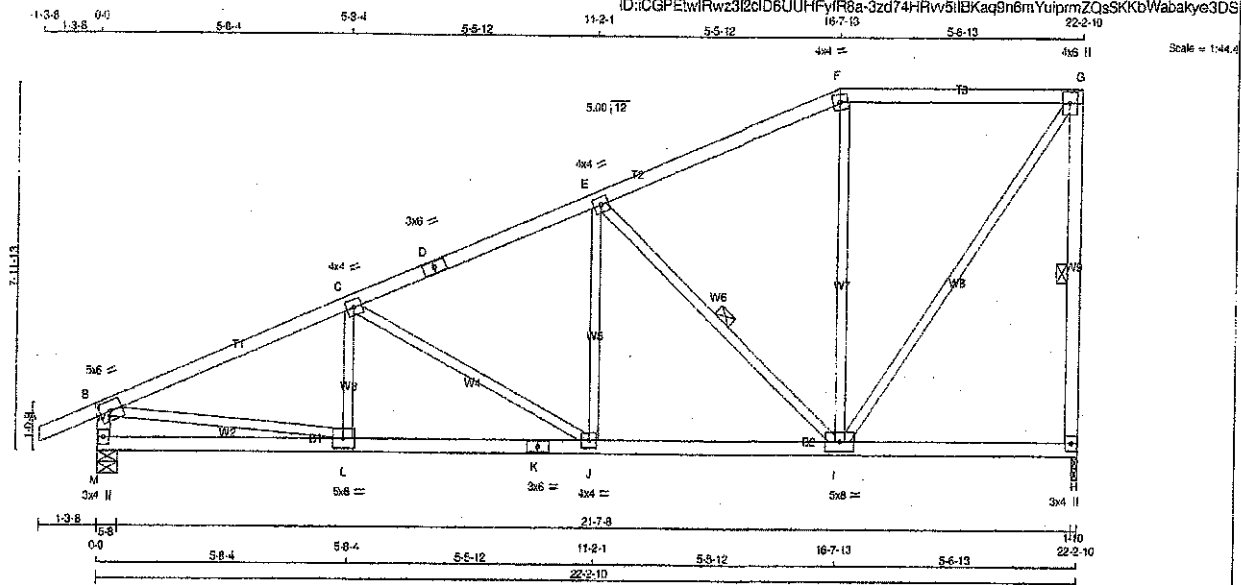
TOTAL NUMBER OF
ITEMS= 17







JOB NAME 405481	TRUSS NAME H3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 98 = 196 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2	
D - F	2x4	DRY	No.2	
F - G	2x4	DRY	No.2	
H - G	2x4	DRY	No.2	
M - B	2x4	DRY	No.2	
M - K	2x4	DRY	No.2	
K - H	2x4	DRY	No.2	

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY-SEASONED LUMBER.

DESCR.
SPF
SPF
SPF
SPF
SPF
SPF
SPF
SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ
JT	1359	0	1359	0
M	1476	0	1476	0

UNFACTORED REACTIONS

JT	1ST LC CASE	MAX. MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	288 / 0	0 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS1 (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0 / 26	-102.0	-102.0 0.13 (1)	10.00	L-C	-143 / 51	0.03 (1)
B-C	-2067 / 0	-102.0	-102.0 0.45 (1)	4.26	C-J	559 / 0	0.40 (1)
C-D	-1581 / 0	-102.0	-102.0 0.36 (1)	4.90	E-I	0 / 375	0.08 (1)
D-E	-1581 / 0	-102.0	-102.0 0.36 (1)	4.90	E-I	997 / 0	0.40 (1)
E-F	-805 / 0	-102.0	-102.0 0.35 (1)	6.25	I-F	-207 / 13	0.25 (1)
F-G	-721 / 0	-102.0	-102.0 0.41 (1)	6.25	I-G	0 / 1245	0.28 (1)
H-G	-1288 / 0	0.0	0.0 0.37 (1)	5.68	B-L	0 / 1943	0.44 (1)
M-B	-1420 / 0	0.0	0.0 0.14 (1)	6.82			
M-L	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
L-K	0 / 1927	-18.5	-18.5 0.36 (1)	10.00			
K-J	0 / 1927	-18.5	-18.5 0.36 (1)	10.00			
J-I	0 / 1441	-18.5	-18.5 0.30 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 20.0 PSF
DL	= 5.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.07")
ALLOWABLE DEFL. (TL) = $L/360$ (0.74")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.14")

CS1: TC=0.45/1.00 (B-C-1), BC=0.36/1.00 (J-L-1),
WB=0.44/1.00 (B-L-1), SSI=0.25/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

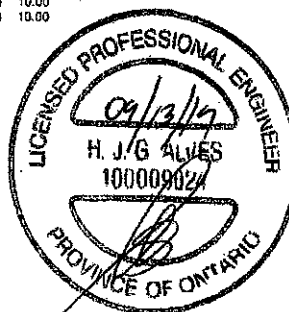
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788
		1957 1655

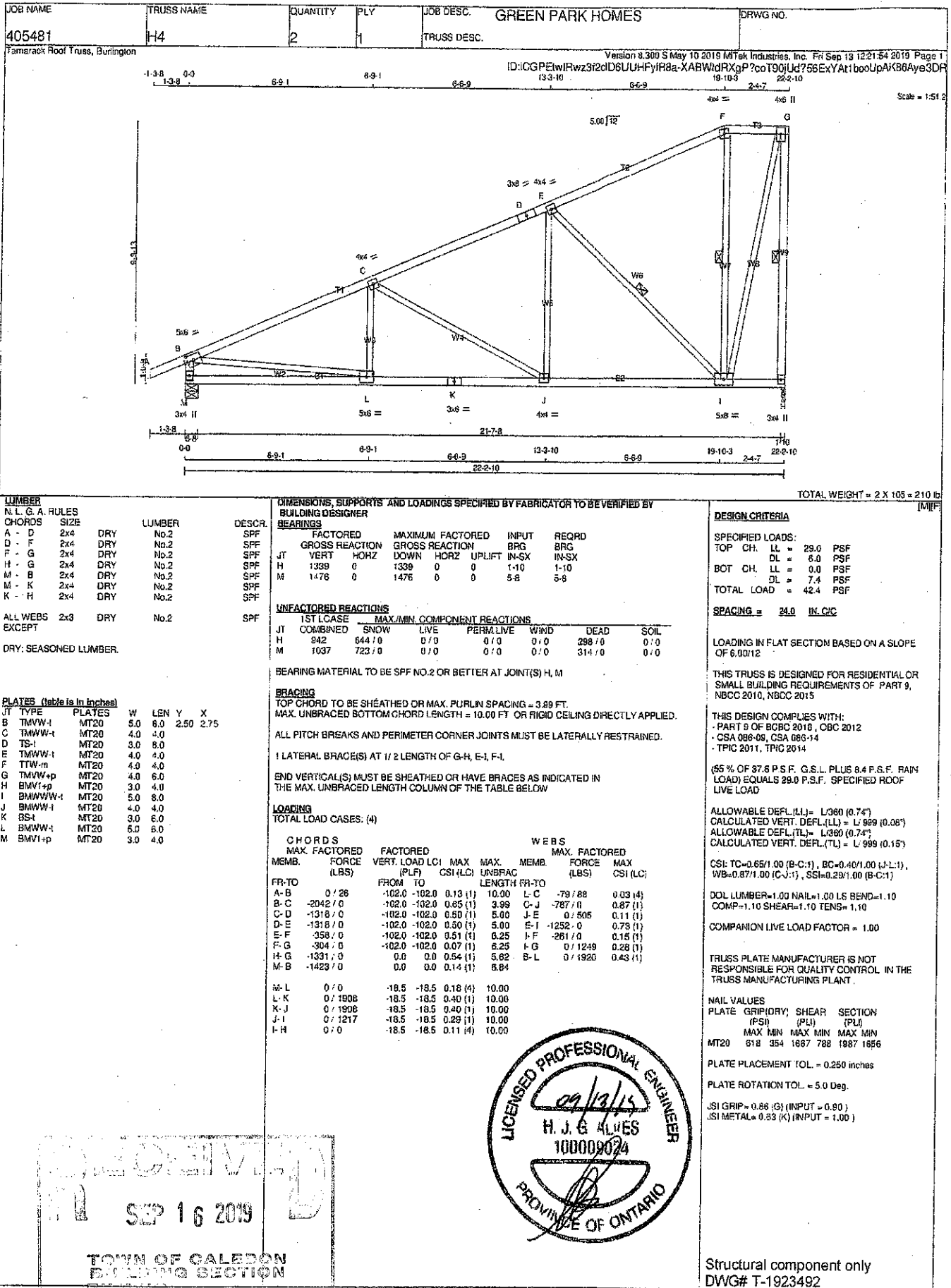
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.84 (L) (INPUT = 0.90)
JSI METAL = 0.57 (K) (INPUT = 1.00)



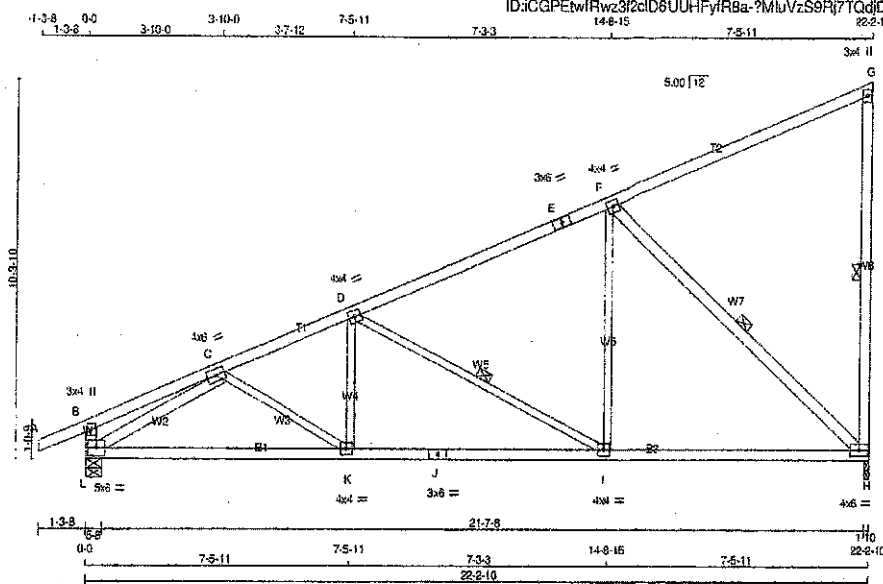
Structural component only
DWG# T-1923491



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H5	22	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.300 5 May 10 2019 MTEK Industries, Inc. Fri Sep 13 12:21:55 2019 Page 1



Scale = 1:55.4

TOTAL WEIGHT = 22 X 100 = 2203 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
H - G	2x4 DRY	No.2	SPF		
L - B	2x4 DRY	No.2	SPF		
L - J	2x4 DRY	No.2	SPF		
J - H	2x4 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		
F - H	2x4 DRY	No.2	SPF		
L - C	2x4 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REORD BRG
JT	VERT	DOWN	UP	IN-SX
H	1339	0	1339	0
L	1476	0	1476	0

UNFACTORED REACTIONS	1ST CASE	MAX./MIN. COMPONENT REACTIONS	PERM LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD
H	942	644 / 0	0 / 0	0 / 0	236 / 0	0 / 0
L	1037	723 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED MAX. LOAD (LC)	MAX. UNBRACED LENGTH	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH
FR-TO						FR-TO			
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	K-D	0 / 142	0.05 (4)	
B-C	0 / 8	-102.0	-102.0	0.20 (1)	10.00	D-I	-841 / 0	0.39 (1)	
C-D	-1944 / 0	-102.0	-102.0	0.33 (1)	4.50	I-F	0 / 546	0.12 (1)	
D-E	-1140 / 0	-102.0	-102.0	0.69 (1)	4.86	F-H	-1454 / 0	0.76 (1)	
E-F	-1140 / 0	-102.0	-102.0	0.69 (1)	4.86	C-K	0 / 54	0.02 (4)	
F-G	-92 / 0	-102.0	-102.0	0.70 (1)	6.25	L-C	-2122 / 0	0.50 (1)	
G-H	-288 / 0	0.0	0.0	0.15 (1)	6.25				
H-I	-308 / 0	0.0	0.0	0.03 (1)	7.81				
L-K	0 / 1808	-18.5	-18.5	0.42 (1)	10.00				
K-J	0 / 1812	-18.5	-18.5	0.43 (1)	10.00				
J-I	0 / 1812	-18.5	-18.5	0.43 (1)	10.00				
I-H	0 / 1089	-18.5	-18.5	0.32 (4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCRC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.70/1.00 (F-G-1), BC=0.48/1.00 (I-K-1), WB=0.78/1.00 (F-H-1), SSI=0.33/1.00 (F-G-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PS)	SECTION (PLI)

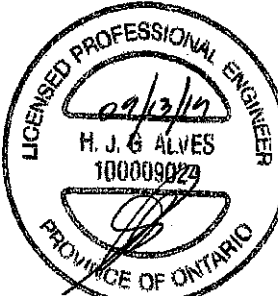
MT20 618 354 1867 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)

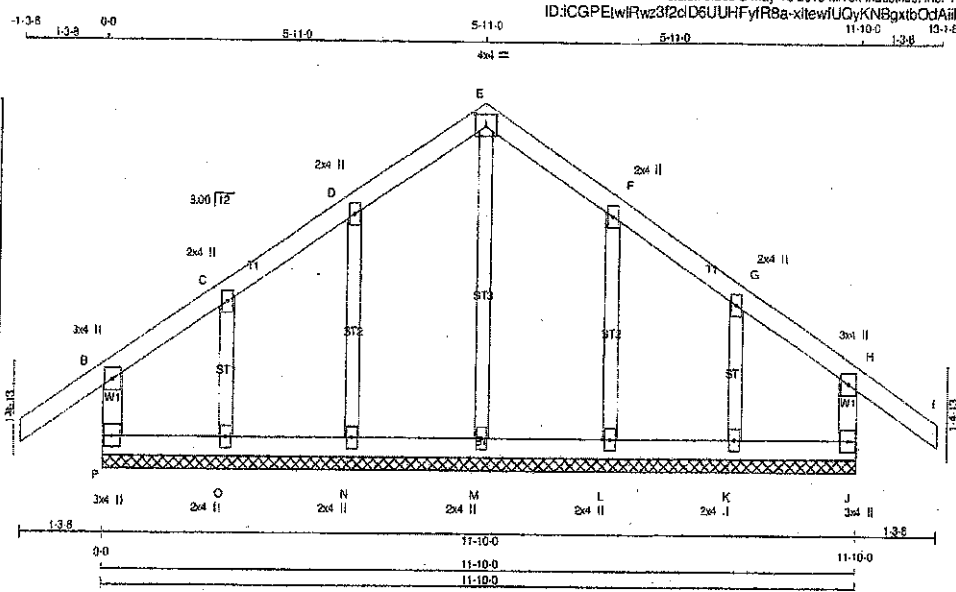
JSI METAL = 0.57 (J) (INPUT = 1.00)



Structural component only
DWG# T-1923493

[illegible]

JOB NAME 405481	TRUSS NAME H7G	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF

ALL GABLE WEBS 2x3 DRY No.2 SPF

DRY, SEASONED LUMBER.

GABLE STUDS SPACED AT 2'-0" OC

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV-p	MT20	3.0	4.0		
C D, F, G					
C TMV-w	MT20	2.0	4.0		
S TMV-p	MT20	4.0	4.0	2.25	2.00
H TMV-p	MT20	3.0	4.0		
J BMV1-p	MT20	3.0	4.0		
K, L, M, N, O					
K BMV1-w	MT20	2.0	4.0		
P BMV1-p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 8.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS			WEBS		
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 MAX (PLF)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
P-B	-241.0	0.0	0.0	0.04 (1)	7.81
A-B	0.39	-102.0	-102.0	0.14 (1)	10.00
B-C	0.15	-102.0	-102.0	0.09 (1)	10.00
C-D	0.39	-102.0	-102.0	0.06 (1)	10.00
D-E	0.38	-102.0	-102.0	0.06 (1)	10.00
E-F	0.38	-102.0	-102.0	0.06 (1)	10.00
F-G	0.39	-102.0	-102.0	0.06 (1)	10.00
G-H	0.15	-102.0	-102.0	0.09 (1)	10.00
H-I	0.39	-102.0	-102.0	0.14 (1)	10.00
J-H	-241.0	0.0	0.0	0.04 (1)	7.81
P-O	-23.0	-18.5	-18.5	0.02 (4)	6.25
O-N	-23.0	-18.5	-18.5	0.02 (4)	6.25
N-M	-33.0	-18.5	-18.5	0.01 (4)	6.25
M-L	-33.0	-18.5	-18.5	0.01 (4)	6.25
L-K	-23.0	-18.5	-18.5	0.02 (4)	6.25
K-J	-23.0	-18.5	-18.5	0.02 (4)	6.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCQ 2010, NBCQ 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(56 % OF 37.8 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSL TC=0.14/1.00 (A-B:1), SC=0.02/1.00 (K-L:4), WB=0.10/1.00 (E-M:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PS)

(PL)

MAX MIN MAX MIN MAX MIN

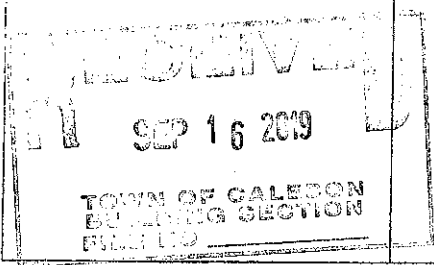
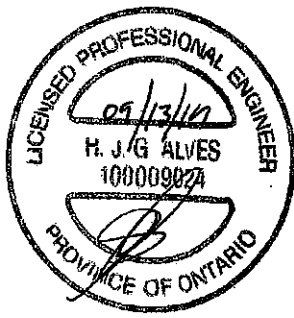
MT20 618 354 1637 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

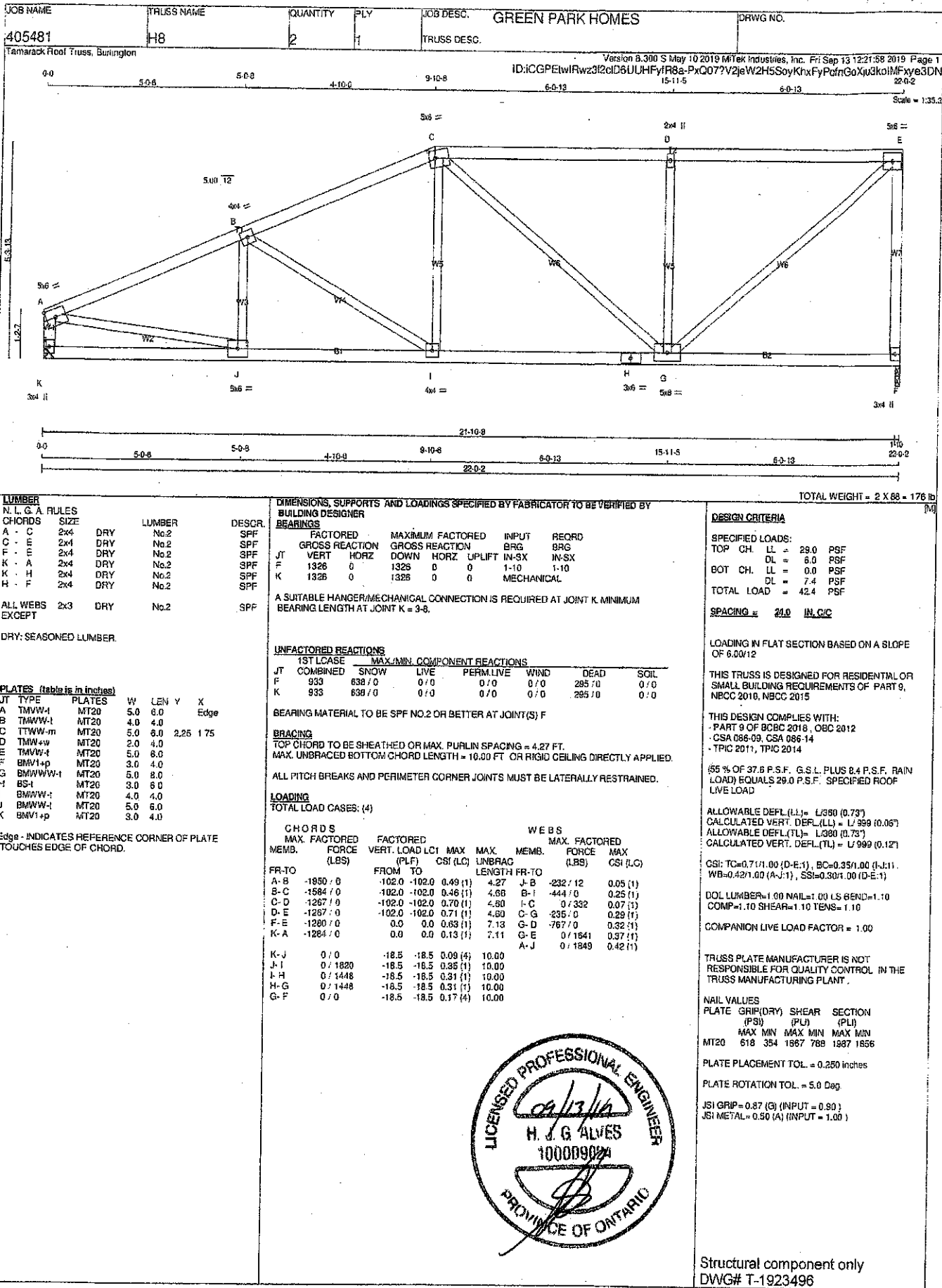
PLATE ROTATION TOL = 3.0 Deg.

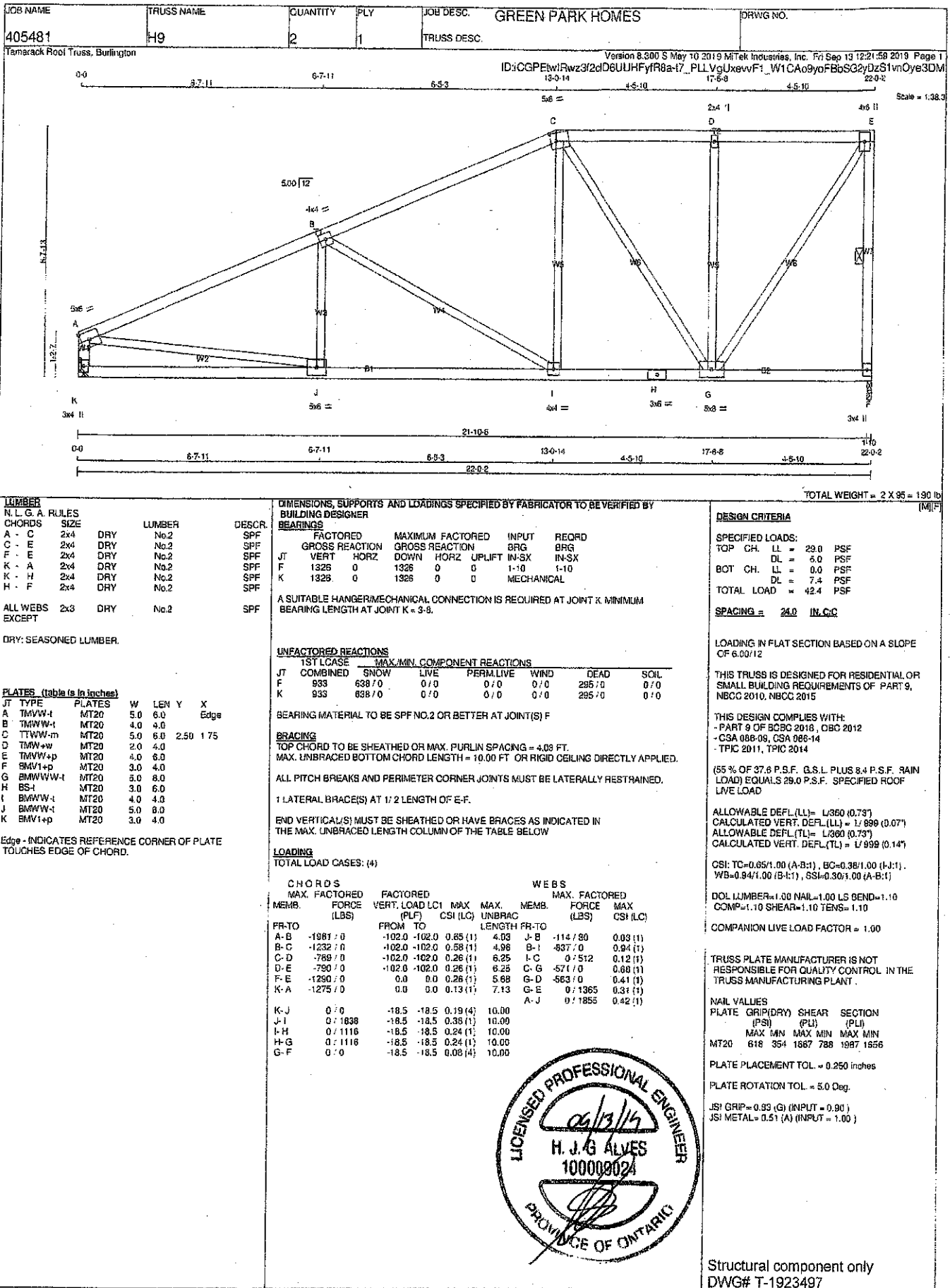
JSI GRIP = 0.21 (B) (INPUT = 0.90)

JSI METAL = 0.11 (D) (INPUT = 1.00)

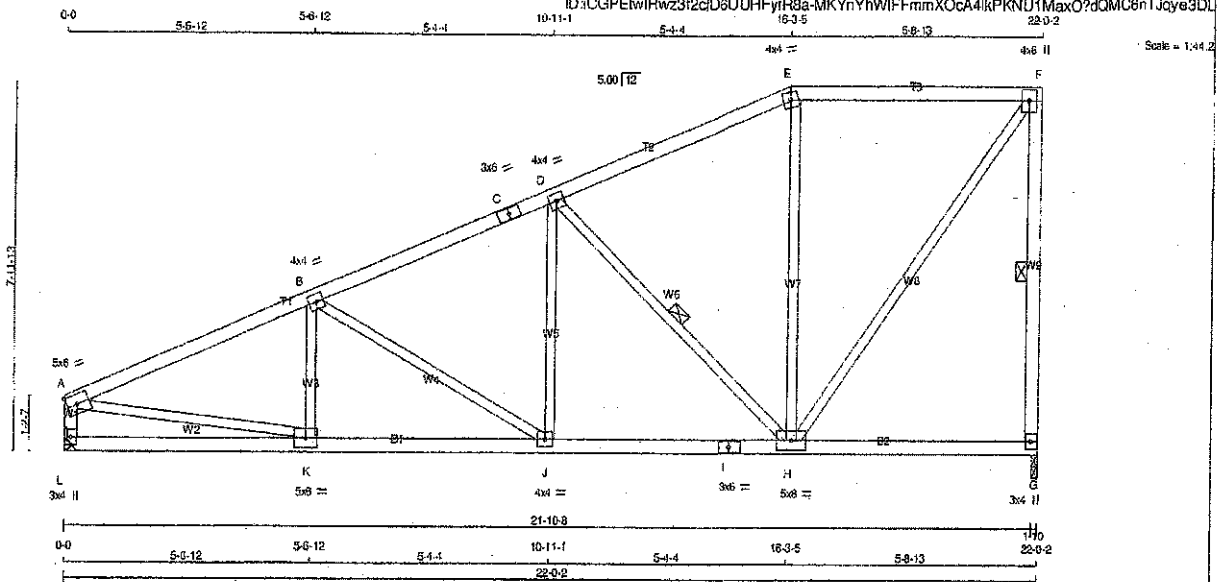


Structural component only
DWG# T-1923495





JOB NAME 405481	TRUSS NAME H10	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
L - A	2x4	DRY	No.2
L - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMWV-I	MT20	5.0	6.0		Edge
B - TMWW-I	MT20	4.0	4.0		
C - TS-I	MT20	3.0	6.0		
D - TMWV-I	MT20	4.0	4.0		
E - TWV-m	MT20	4.0	4.0		
F - TMWW-p	MT20	4.0	6.0		
G - BMV1-p	MT20	3.0	4.0		
H - BMWW-I	MT20	5.0	8.0		
I - SS-I	MT20	3.0	6.0		
J - BMWW-I	MT20	4.0	4.0		
K - BMWW-I	MT20	5.0	6.0		
L - BMV1-p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	VERT	GROSS	GROSS	BRG	BRG
G	1326	0	1326	0	0
L	1326	0	1326	0	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
G	933	838 / 0	0 / 0	0 / 0	0 / 0	295 / 0
L	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	-1945 / 0	A-B	-185 / 36
B-C	-1515 / 0	B-J	-485 / 0
C-D	-1515 / 0	J-D	0 / 345
D-E	-811 / 0	D-H	945 / 0
E-F	-726 / 0	H-E	-208 / 13
G-F	-1284 / 0	H-F	0 / 1230
L-A	-1281 / 0	A-K	0 / 1837
L-K	0 / 0		
K-J	0 / 1814		
J-I	0 / 1308		
I-H	0 / 1308		
H-G	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")
ALLOWABLE DEFL.(TL) = L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/999 (0.13")

CSI: TC=0.44/1.00 (E-F:1), BC=0.34/1.00 (J-K:1), WB=0.41/1.00 (A-K:1), SS=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

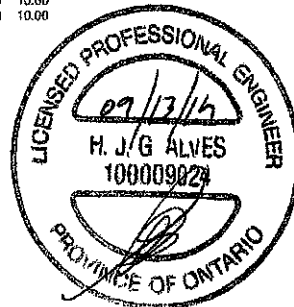
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 334 1667 788 1997 1656

PLATE PLACEMENT TOL = 0.250 inches

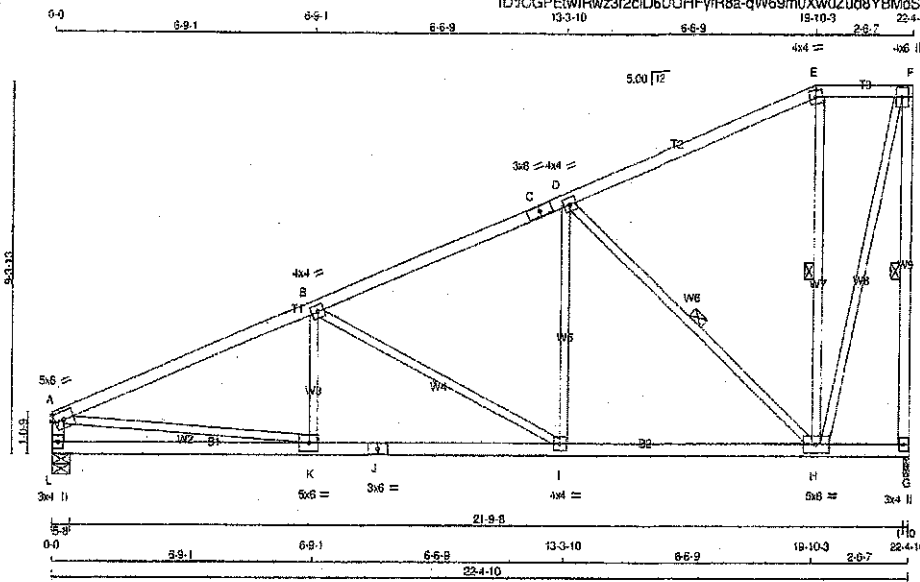
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.82 (KG) (INPUT = 0.90)
JSI METAL=0.50 (A) (INPUT = 1.00)



Structural component only
DWG# T-1923498

JOB NAME 405481	TRUSS NAME H11	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 5 May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:01 2019 Page 1	ID:CGPEWIRwz3f2cID6UHFYR8a-qW69m0Xw0Zud8YBMdSFeta18K_GekzaWQmW0sGye3DK



TOTAL WEIGHT = 2 X 104 = 208 lb
(M/F)

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A - C	TMW-t	MT20	5.0	8.0		Edge
B - TMWW-t	MT20	4.0	4.0			
C - TS-t	MT20	3.0	8.0			
D - TMWW-t	MT20	4.0	4.0			
E - TMW-t	MT20	4.0	4.0			
F - TMWW-p	MT20	4.0	8.0			
G - BMV1-p	MT20	3.0	4.0			
H - BMWWW-t	MT20	5.0	8.0			
I - BMWW-t	MT20	4.0	4.0			
J - BS-t	MT20	3.0	8.0			
K - BMWW-t	MT20	5.0	8.0			
L - BMV1-p	MT20	3.0	4.0			

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
G	1349	0	0	1-10
L	1349	0	0	5-8

UNFACTORED REACTIONS

1ST LOOSE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED					
G	949	649 / 0	0 / 0	0 / 0	300 / 0	0 / 0
L	949	649 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX.	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	LENGTH FR-TO		
A-B	-2083 / 0	-102.0 -102.0 0.65 (1)	3.98	K-B	-81 / 87
B-C	-1341 / 0	-102.0 -102.0 0.50 (1)	4.97	B-I	-784 / 0
C-D	-1341 / 0	-102.0 -102.0 0.50 (1)	4.97	I-D	0 / 503
D-E	-302 / 0	-102.0 -102.0 0.51 (1)	6.25	D-H	-1250 / 0
E-F	-326 / 0	-102.0 -102.0 0.08 (1)	6.25	H-E	-261 / 0
G-F	-1040 / 0	0.0 0.0 0.55 (1)	5.60	H-F	0 / 1254
L-A	-1255 / 0	0.0 0.0 0.19 (1)	7.10	A-K	0 / 1939
L-K	0 / 0	-18.5 -18.5 0.18 (4)	10.00		
K-J	0 / 1927	-18.5 -18.5 0.40 (1)	10.00		
J-I	0 / 1927	-18.5 -18.5 0.40 (1)	10.00		
I-H	0 / 1238	-18.5 -18.5 0.29 (1)	10.00		
H-G	0 / 0	-18.5 -18.5 0.11 (4)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 20.0 PSF
DL = 6.0 PSF
BOT CH. LL = 0.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, CBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

(55% OF 87.6 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CS1: TC=0.85/1.00 (A-B:1), BC=0.40/1.00 (I-K:1), WE=0.87/1.00 (B-E:1), SS=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

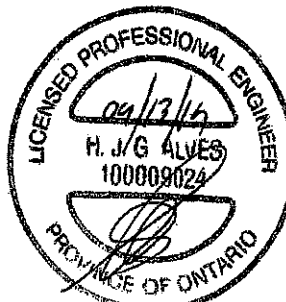
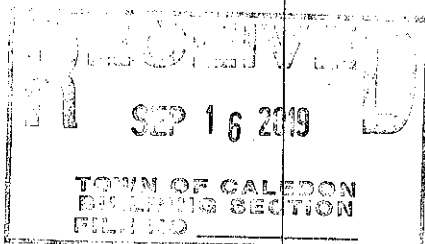
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 616 354 1667 786 1997 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

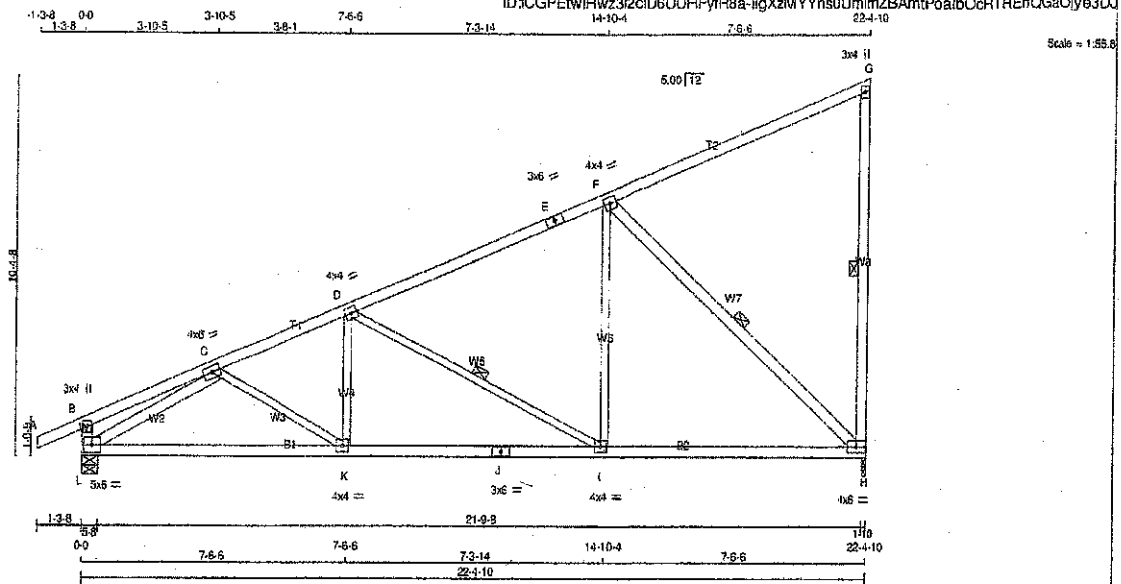
JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.58 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923499



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H12	6	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:02 2019 Page 1
ID:JGPEtWIRwz3ZcID6UJH-FyIR8a-IigXzMYns0UmimZBAmtPoaibOoRTREffQGaOye3DU



TOTAL WEIGHT = 6 X 101 = 606 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N.L.G.A. RULES				
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
F - H	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV-p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	TMVW-1	MT20	4.0	4.0		
E	TS-1	MT20	3.0	6.0		
F	TMVW-1	MT20	4.0	4.0		
G	TMV-p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMVW-1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMVW-1	MT20	4.0	4.0		
L	BMVW-1	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	1349	0	1-10	1-10
L	1486	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE	MAX. MIN. COMPONENT REACTIONS	DEAD	SOIL
JT	COMBINED	SNOW	LIVE
H	949	849 / 0	0 / 0
L	1044	728 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

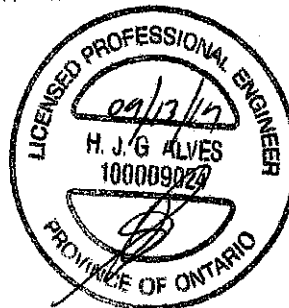
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD	LC1	MAX. FACTORED PERM. LIVE WIND UNBRACED LENGTH	FR-TO	WEBS	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
A-B	0 / 25	-102.0	-102.0	0.13 (1)	10.00	K-D	0 / 144	0.05 (4)		
B-C	0 / 8	-102.0	-102.0	0.20 (1)	10.00	D-I	-850 / 0	9.40 (1)		
C-D	-1982 / 0	-102.0	-102.0	0.34 (1)	4.48	I-F	0 / 551	0.12 (1)		
D-E	-1149 / 0	-102.0	-102.0	0.71 (1)	4.81	F-H	-1465 / 0	0.76 (1)		
E-F	-1149 / 0	-102.0	-102.0	0.71 (1)	4.81	C-K	0 / 54	0.02 (4)		
F-G	-32 / 0	-102.0	-102.0	0.71 (1)	6.25	L-C	-2141 / 0	0.51 (1)		
G-H	-309 / 0	0.0	0.0	0.16 (1)	6.25					
H-I	-309 / 0	0.0	0.0	0.03 (1)	7.81					
L-K	0 / 1826	-18.5	-18.5	0.42 (1)	10.00					
K-J	0 / 1829	-18.5	-18.5	0.43 (1)	10.00					
J-I	0 / 1829	-18.5	-18.5	0.43 (1)	10.00					
I-H	0 / 1078	-18.5	-18.5	0.32 (4)	10.00					



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF'L (LL) = L/960 (0.75")
CALCULATED VERT. DEF'L (LL) = L/999 (0.07")
ALLOWABLE DEF'L (TL) = L/360 (0.75")
CALCULATED VERT. DEF'L (TL) = L/999 (0.15")

CSI: TC=0.71/1.00 (F-G:1), BC=0.43/1.00 (H-K:1),
WB=0.78/1.00 (F-H:1), SS=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

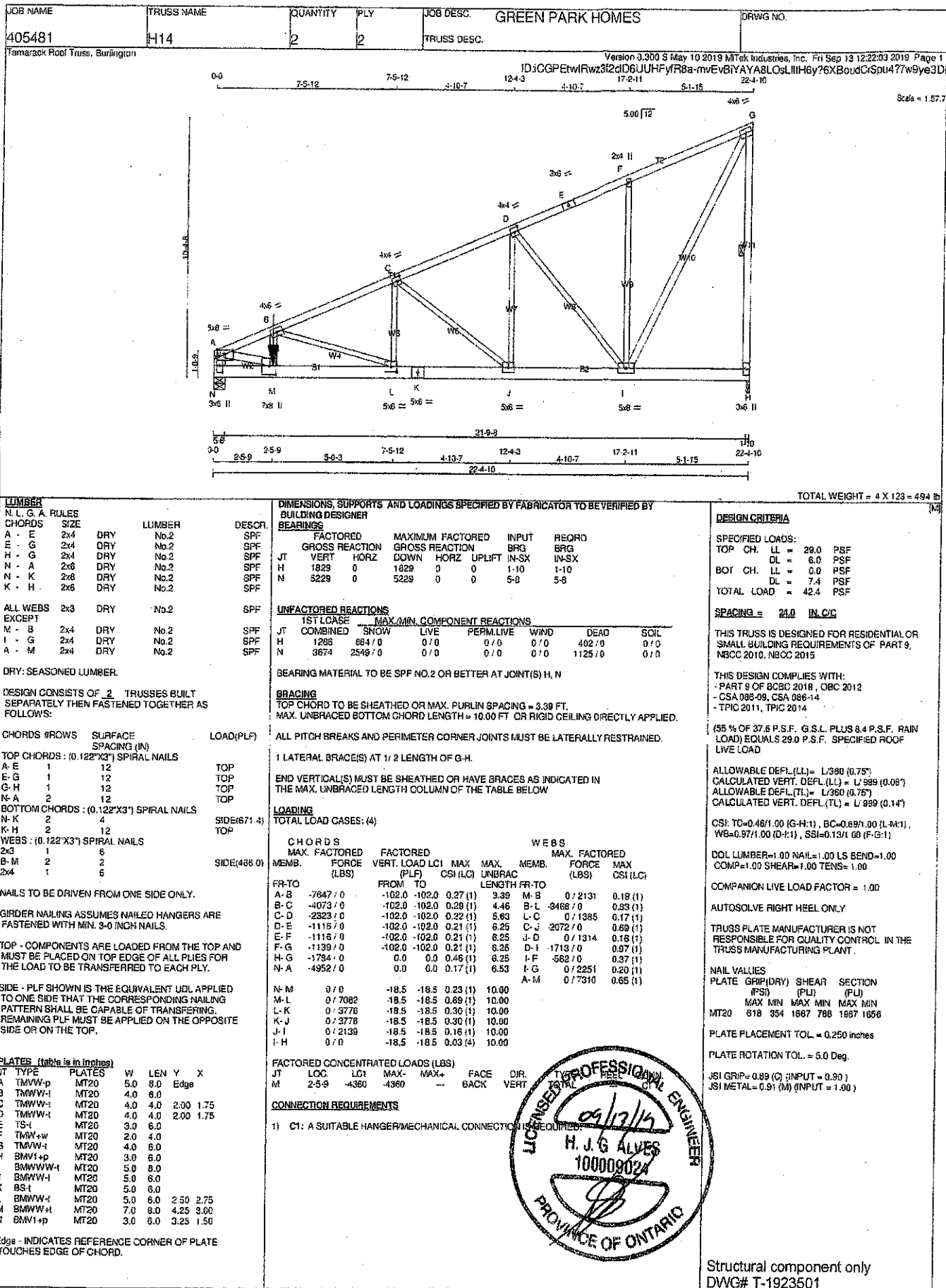
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg

JSI GRIP = 0.89 (F) (INPUT = 0.90)
JSI METAL = 0.88 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923500



JOB NAME: 405481

TRUSS NAME: H17

QUANTITY: 4

PLY: 1

JOB DESC: GREEN PARK HOMES

TRUSS DESC:

DRWG NO:

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ID:CGPEtWlRwz3f2dD8UJHFyIR8a-iHMgbOaR4nO3dAU8slJatQCvxbetGouSLOUE?Iye3DG

Tamarack Roof Truss, Burlington

Scale = 1/4" = 1'-0"

LUMBER

N.L. & A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.50 2.00
B	TMVW-i	MT20	4.0	6.0	
C	TMVW-i	MT20	4.0	6.0	
D	TMVW-i	MT20	4.0	4.0	2.00 1.75
E	TMV+p	MT20	3.0	4.0	
F	BMVW1-i	MT20	4.0	6.0	
G	BMVW-i	MT20	4.0	6.0	
H	BSWW-p	MT20	5.0	6.0	3.00 2.75
I	BSWW-m	MT20	5.0	6.0	1.75 2.00
J	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	HORZ	DOWN	HORZ
F	827	0	1-10
J	827	0	5-8

UNFACTORED REACTIONS

1ST CASE	MAX MIN. COMPONENT REACTIONS
JT COMBINED	SNOW
F	582
J	582

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.41 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO	-775 / 0	FR-TO	-975 / 0
A-B	-2318 / 0	B-H	0 / 1490
B-C	-849 / 0	H-C	0 / 488
C-D	-28 / 0	C-G	-1196 / 0
D-E	-220 / 0	G-D	0 / 334
E-F	-813 / 0	D-F	-1038 / 0
F-G	-18.5 / 0.01 (4)	A-I	0 / 784
G-H	-18.5 / 0.15 (1)		
H-I	-18.5 / 0.41 (1)		
I-J	-18.5 / 0.22 (4)		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(56 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.46")

CALCULATED VERT. DEFL. (LL) = L/999 (0.06")

ALLOWABLE DEFL. (TL) = L/380 (0.46")

CALCULATED VERT. DEFL. (TL) = L/999 (0.11")

CS: TC=0.381/0 (C-D:1), BC=0.41/1.00 (G-H:1), WB=0.78/1.00 (D-F:1), SSI=0.24/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	918	354
	1667	788
	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.85 (B) (INPUT = 0.90)

JSI METAL = 0.40 (B) (INPUT = 1.00)

09/13/19

H. J. G. ALVES

100009024

PROVINCE OF ONTARIO

Structural component only

DRWG# T-1923503

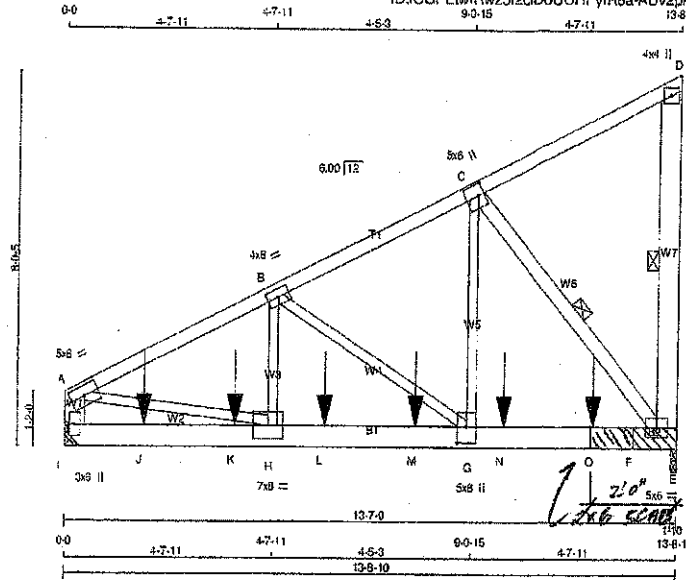
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
405481	H18	2	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:CGPEtWlRwz3f2dD6UHFyR8a-AUy2pkb3r5WvFJ3KQ0cpaek4V?uzPGMFa2EnXUye3DF

Scale = 1:43.7



TOTAL WEIGHT = 4 X 79 = 317 lb

LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
I - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x6	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
C - E	2x4	DRY	No.2	SPF

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
I - A 2	12	TOP
D - E 2	12	TOP
A - D 1	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
I - E 2	12	SIDE(0.0)
WEBS : (0.122'X3') SPIRAL NAILS		
2x3 1	6	
2x4 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A - TMWV-1	MT20	5.0	8.0		
B - TMWV-1	MT20	4.0	8.0		
C - TMWV-1	MT20	5.0	8.0	3.00	1.50
D - TMWV-1	MT20	4.0	4.0		
E - BMWV-1	MT20	5.0	8.0		
G - BMWV-1	MT20	5.0	8.0	4.25	2.50
H - BMWV-1	MT20	7.0	8.0		
I - BMWV-1	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	DOWN	HORIZ	IN-SX
I	4379	0	4379	0
E	4320	0	4320	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
I	3076	2135 / 0	0 / 0
E	3035	2106 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

2x6 DRY SPF NO.2 BEARING BLOCK 12" LONG AT JT. E ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122'X3') SPIRAL NAILS SPACED 3" C.C. 12 NAILS TOTAL.

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLINE SPACING = 3.83 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF D-E, C-E.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	MEMB.	FORCE (LBS)
FR-TO			FR-TO	
I-A	-3621 / 0	0.0	A-H	0 / 5187
A-B	-5708 / 0	-102.0	H-B	0 / 2024
B-C	-3385 / 0	-102.0	B-G	-2536 / 0
C-D	-22 / 0	-102.0	G-C	0 / 4480
D-E	-189 / 0	0.0	C-E	-4615 / 0
I-J	0 / 0	-18.5	J-K	-18.5
J-K	0 / 0	-18.5	K-L	-18.5
K-L	0 / 5116	-18.5	L-M	-18.5
L-M	0 / 5116	-18.5	M-N	-18.5
M-N	0 / 3037	-18.5	N-O	-18.5
N-O	0 / 3037	-18.5	O-F	-18.5
O-F	0 / 3037	-18.5	F-E	-18.5

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC1	MAX.	MAX+	FACE	BR.
J	1-9-10	-1174	-1174	---	FRONT	VERT
K	3-9-10	-1174	-1174	---	FRONT	VERT
L	5-9-10	-1174	-1174	---	FRONT	VERT
M	7-9-10	-1174	-1174	---	FRONT	VERT
N	9-9-10	-1174	-1174	---	FRONT	VERT
O	11-9-10	-1174	-1174	---	FRONT	VERT

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	6.0	PSF
	DL	0.0	PSF
	DL	7.4	PSF
TOTAL LOAD		42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 3, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.46)
CALCULATED VERT. DEFL.(LL) = L/959 (0.07)
ALLOWABLE DEFL.(TL) = L/360 (0.46)
CALCULATED VERT. DEFL.(TL) = L/999 (0.12)

CSI: TC=0.33/1.00 (A-B:1), BC=0.78/1.00 (G-H:1), WB=0.64/1.00 (A-H:1), SS=0.59/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

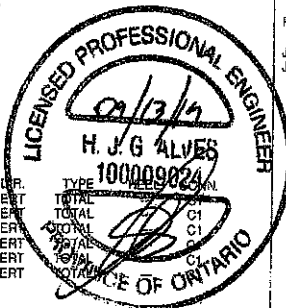
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(OR)Y SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

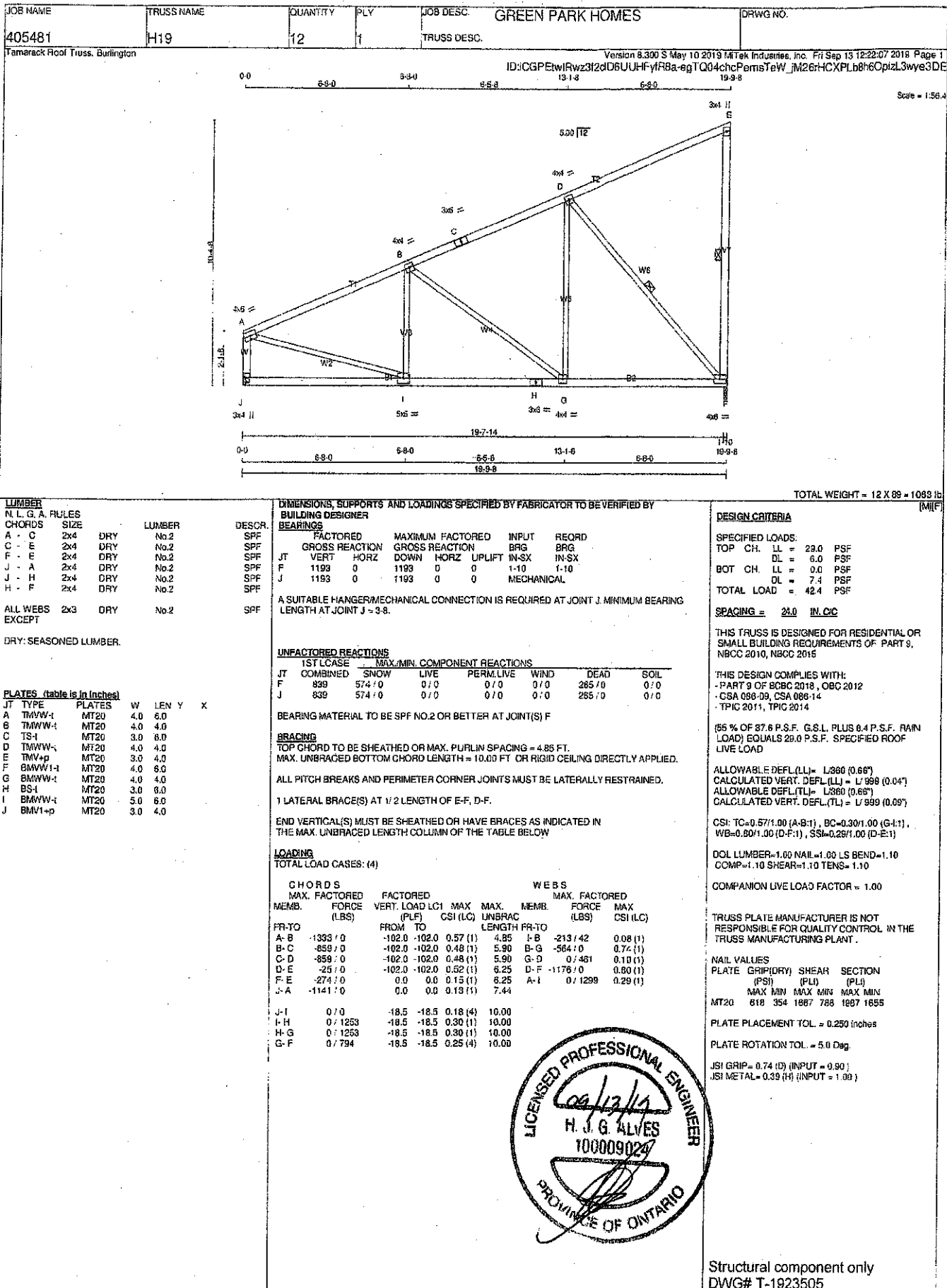
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (C) (INPUT = 0.90)
JSI METAL = 0.55 (C) (INPUT = 1.00)



Structural component only
DWG# T-1923504

TOWN OF CALEDON
BUILDING SECTION
FILE NO



JOB NAME

405481

TRUSS NAME

H19S

QUANTITY

2

PLY

1

JOB DESC.

GREEN PARK HOMES

DRWG NO.

TRUSS DESC.

Version 8.300.5 May 10 2019 MITek Industries, Inc. Fri Sep 13 12:22:08 2019 Page 1

ID:CGPEWIRwz3f2cID8UUHFyIR8a-7s1oEQcJNimUdDfYcHrI3qUMpIH8Y1MjbMMye3DD

Tamarack Roof Truss, Burlington

Scale = 1:50

LUMBER

N.L.G.A. RULES

CHORDS SIZE LUMBER

A - I 2x4 DRY No.2

I - L 2x4 DRY No.2

M - L 2x4 DRY No.2

X - A 2x4 DRY No.2

X - Q 2x4 DRY No.2

Q - M 2x4 DRY No.2

ALL WEBS 2x3 DRY No.2

ALL GABLE WEBS 2x3 DRY No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE PLATES W LEN Y X

A TMVW-1 MT20 4.0 4.0 2.00 1.50

B, C, D, E, F, G, H, J, K MT20 2.0 4.0

I TS-1 MT20 2.0 6.0

L TMV-p MT20 3.0 4.0

M BMV1-p MT20 3.0 4.0

N, O, P, R, S, T, U, V MT20 2.0 4.0

N BMW1-w MT20 3.0 6.0

Q BS-1 MT20 4.0 4.0

W BMWW1-t MT20 3.0 4.0

X BMV1-p MT20 3.0 4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF L-M, K-N, J-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. MAX. FACTORED FORCE (LBS) VERT. LOAD (PLF) LC1 MAX. UNBRACED LENGTH (FT) FROM TO

A-B -15 / 0 -102.0 -102.0 0.02 (1) 6.25 N-K -227 / 0 0.14 (1)

B-C -15 / 0 -102.0 -102.0 0.03 (1) 6.25 O-J -201 / 0 0.10 (1)

C-D -12 / 0 -102.0 -102.0 0.05 (1) 6.25 P-H -203 / 0 0.24 (1)

D-E -11 / 0 -102.0 -102.0 0.05 (1) 6.25 R-G -203 / 0 0.17 (1)

E-F -8 / 0 -102.0 -102.0 0.05 (1) 10.00 S-F -203 / 0 0.13 (1)

F-G -6 / 0 -102.0 -102.0 0.05 (1) 10.00 T-E -202 / 0 0.09 (1)

G-H -5 / 0 -102.0 -102.0 0.05 (1) 10.00 U-D -209 / 0 0.06 (1)

H-I -3 / 0 -102.0 -102.0 0.05 (1) 10.00 V-C -168 / 0 0.04 (1)

I-J -3 / 0 -102.0 -102.0 0.05 (1) 10.00 W-B -144 / 0 0.03 (1)

J-K 0 / 0 -102.0 -102.0 0.05 (1) 10.00 A-W 0 / 34 0.01 (1)

K-L -7 / 0 -102.0 -102.0 0.05 (1) 10.00

M-L -86 / 0 0.0 0.0 0.05 (1) 6.25

X-A -97 / 0 0.0 0.0 0.01 (1) 7.81

WEBS

MEMB. MAX. FACTORED FORCE (LBS) MAX. UNBRACED LENGTH (FT) FROM TO

X-W 0 / 0 -18.5 -18.5 0.01 (4) 10.00

W-V 0 / 16 -18.5 -18.5 0.01 (4) 10.00

V-U 0 / 13 -18.5 -18.5 0.02 (4) 10.00

U-T 0 / 10 -18.5 -18.5 0.02 (4) 10.00

T-S 0 / 8 -18.5 -18.5 0.01 (4) 10.00

S-R 0 / 6 -18.5 -18.5 0.01 (4) 10.00

R-Q 0 / 4 -18.5 -18.5 0.01 (4) 10.00

Q-P 0 / 4 -18.5 -18.5 0.01 (4) 10.00

P-O 0 / 3 -18.5 -18.5 0.01 (4) 10.00

O-N 0 / 1 -18.5 -18.5 0.02 (4) 10.00

N-M 0 / 0 -18.5 -18.5 0.02 (4) 10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012

- CSA 086-09, CSA 086-14

- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (K-L:1), BC=0.02/1.00 (N-O:1), WS=0.24/1.00 (H-P:1), SI=0.09/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

MT20 618 354 1857 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.53 (A) (INPUT = 0.90)

JSI METAL= 0.08 (K) (INPUT = 1.00)

RECEIVED

SEP 16 2019

TOWN OF CALEDON BUILDING SECTION FILE NO.

LICENSED PROFESSIONAL ENGINEER

09/13/19

H. J. G. ALVES

1000098024

PROVINCE OF ONTARIO

Structural component only

DWG# T-1923506

JOB NAME 405481	TRUSS NAME H20	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 MITAK Industries, Inc. Fri Sep 13 12:22:09 2019 Page 1	
Scale = 1/32"					

LUMBER N.L. G. A. RULES <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>CHORDS</th> <th>SIZE</th> <th>LUMBER</th> <th>DESCR</th> </tr> <tr> <td>A - C</td> <td>2x4 DRY</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td>C - D</td> <td>2x4 DRY</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td>G - A</td> <td>2x4 DRY</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td>E - D</td> <td>2x4 DRY</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td>G - E</td> <td>2x4 DRY</td> <td>No.2</td> <td>SPF</td> </tr> <tr> <td colspan="4">ALL WEBS 2x3 DRY No.2 SPF</td> </tr> <tr> <td colspan="4">EXCEPT</td> </tr> <tr> <td colspan="4">DRY, SEASONED LUMBER.</td> </tr> </table> PLATES (table is in inches) <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>JT</th> <th>TYPE</th> <th>PLATES</th> <th>W</th> <th>LEN</th> <th>Y</th> <th>X</th> </tr> <tr> <td>A</td> <td>TMV+p</td> <td>MT20</td> <td>3.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>B</td> <td>TMVW-1</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>C</td> <td>TTW-p</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td>2.25</td> <td>2.00</td> </tr> <tr> <td>D</td> <td>TMVW+p</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td>1.25</td> <td>2.00</td> </tr> <tr> <td>E</td> <td>BMV1+p</td> <td>MT20</td> <td>3.0</td> <td>4.0</td> <td></td> <td></td> </tr> <tr> <td>F</td> <td>BMVW-1</td> <td>MT20</td> <td>4.0</td> <td>9.0</td> <td></td> <td></td> </tr> <tr> <td>G</td> <td>BMVW-1</td> <td>MT20</td> <td>4.0</td> <td>4.0</td> <td></td> <td></td> </tr> </table>	CHORDS	SIZE	LUMBER	DESCR	A - C	2x4 DRY	No.2	SPF	C - D	2x4 DRY	No.2	SPF	G - A	2x4 DRY	No.2	SPF	E - D	2x4 DRY	No.2	SPF	G - E	2x4 DRY	No.2	SPF	ALL WEBS 2x3 DRY No.2 SPF				EXCEPT				DRY, SEASONED LUMBER.				JT	TYPE	PLATES	W	LEN	Y	X	A	TMV+p	MT20	3.0	4.0			B	TMVW-1	MT20	4.0	4.0			C	TTW-p	MT20	4.0	4.0	2.25	2.00	D	TMVW+p	MT20	4.0	4.0	1.25	2.00	E	BMV1+p	MT20	3.0	4.0			F	BMVW-1	MT20	4.0	9.0			G	BMVW-1	MT20	4.0	4.0			DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>JT</th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>RECORD BRG</th> </tr> <tr> <td>E</td> <td>701 0</td> <td>701 0</td> <td>0</td> <td>MECHANICAL</td> </tr> <tr> <td>G</td> <td>701 0</td> <td>701 0</td> <td>0</td> <td>MECHANICAL</td> </tr> </table> A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT E. G. MINIMUM BEARING LENGTH AT JOINT E = 1-8, JOINT G = 1-8. UNFACTORED REACTIONS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>JT</th> <th>1ST CASE COMBINED</th> <th>MAX. MIN. COMPONENT REACTIONS</th> <th>LIVE</th> <th>PERM. LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> <tr> <td>E</td> <td>493</td> <td>337 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>156 / 0</td> <td>0 / 0</td> </tr> <tr> <td>G</td> <td>493</td> <td>337 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>156 / 0</td> <td>0 / 0</td> </tr> </table> BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.	JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG	E	701 0	701 0	0	MECHANICAL	G	701 0	701 0	0	MECHANICAL	JT	1ST CASE COMBINED	MAX. MIN. COMPONENT REACTIONS	LIVE	PERM. LIVE	WIND	DEAD	SOIL	E	493	337 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0	G	493	337 / 0	0 / 0	0 / 0	0 / 0	156 / 0	0 / 0	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, CBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 (85 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.36") CALCULATED VERT. DEFL.(LL) = L/999 (0.01") ALLOWABLE DEFL.(TL) = L/360 (0.36") CALCULATED VERT. DEFL.(TL) = L/999 (0.05") CSI: TO=0.35/1.00 (C-D:1), BC=0.22/1.00 (F-G:4), WB=0.25/1.00 (B-G:1), SSI=0.17/1.00 (C-D:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR (PSI)</th> <th>SECTION (PLI)</th> </tr> <tr> <td>MT20</td> <td>618</td> <td>354</td> <td>1987</td> </tr> </table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP=0.64 (B) (INPUT = 0.90) JSI METAL=0.26 (B) (INPUT = 1.00)	PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)	MT20	618	354	1987
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LOADING TOTAL LOAD CASES: (4) <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th colspan="4">CHORDS</th> <th colspan="4">WEBS</th> </tr> <tr> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>FACTORED VERT. LOAD (PLF)</th> <th>MAX. CSI (LC)</th> <th>MEMB.</th> <th>MAX. FACTORED FORCE (LBS)</th> <th>MAX. CSI (LC)</th> <th></th> </tr> <tr> <td>FR-TO</td> <td></td> <td>FROM TO</td> <td></td> <td>FR-TO</td> <td></td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 21</td> <td>-102.0 -102.0</td> <td>0.17 (1)</td> <td>10.00</td> <td>B-F</td> <td>-197 / 0</td> <td>0.07 (1)</td> </tr> <tr> <td>B-C</td> <td>-483 / 0</td> <td>-102.0 -102.0</td> <td>0.13 (1)</td> <td>6.25</td> <td>F-C</td> <td>0 / 123</td> <td>0.04 (4)</td> </tr> <tr> <td>C-D</td> <td>-482 / 0</td> <td>-102.0 -102.0</td> <td>0.36 (1)</td> <td>6.25</td> <td>G-B</td> <td>-742 / 0</td> <td>0.25 (1)</td> </tr> <tr> <td>G-A</td> <td>-127 / 0</td> <td>0.0 0.0</td> <td>0.01 (1)</td> <td>7.81</td> <td>F-D</td> <td>0 / 413</td> <td>0.09 (1)</td> </tr> <tr> <td>E-D</td> <td>-668 / 0</td> <td>0.0 0.0</td> <td>0.06 (1)</td> <td>7.81</td> <td></td> <td></td> <td></td> </tr> <tr> <td>G-F</td> <td>0 / 525</td> <td>-18.5 -18.5</td> <td>0.22 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> <tr> <td>F-E</td> <td>0 / 0</td> <td>-18.5 -18.5</td> <td>0.19 (4)</td> <td>10.00</td> <td></td> <td></td> <td></td> </tr> </table>	CHORDS				WEBS				MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		FR-TO		FROM TO		FR-TO				A-B	0 / 21	-102.0 -102.0	0.17 (1)	10.00	B-F	-197 / 0	0.07 (1)	B-C	-483 / 0	-102.0 -102.0	0.13 (1)	6.25	F-C	0 / 123	0.04 (4)	C-D	-482 / 0	-102.0 -102.0	0.36 (1)	6.25	G-B	-742 / 0	0.25 (1)	G-A	-127 / 0	0.0 0.0	0.01 (1)	7.81	F-D	0 / 413	0.09 (1)	E-D	-668 / 0	0.0 0.0	0.06 (1)	7.81				G-F	0 / 525	-18.5 -18.5	0.22 (4)	10.00				F-E	0 / 0	-18.5 -18.5	0.19 (4)	10.00				Structural component only DWG# T-1923507
CHORDS				WEBS																																																																													
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JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H20G	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 9.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:11 2019 Page 1	
				ID:CGPEtWfRwz3l2dD6UHFyR8a-XRpsRfCld8CL5yHDZQ_HhSzG0nr4gQ_KkYChye3DA	
				Scale = 1:32.7	

LUMBER				DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER				DESIGN CRITERIA				
N.L.G.A. RULES				BEARINGS				SPECIFIED LOADS:				
CHORDS	SIZE	LUMBER	DESCR.	THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.				TOP CH. LL = 29.0 PSF				
O - B	2x4	DRY	No.2	THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.				DL = 6.0 PSF				
A - E	2x4	DRY	No.2	BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)				BOT CH. LL = 0.0 PSF				
E - H	2x4	DRY	No.2	BRACING				DL = 7.4 PSF				
I - H	2x4	DRY	No.2	TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.				TOTAL LOAD = 42.4 PSF				
O - I	2x4	DRY	No.2	MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.				SPACING = 24.0 IN. C/C				
ALL WEBS	2x3	DRY	No.2	ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.				THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015				
ALL GABLE WEBS	2x3	DRY	No.2	LOADING				THIS DESIGN COMPLIES WITH:				
DRY, SEASONED LUMBER.				TOTAL LOAD CASES: (4)				- PART 9 OF CBC 2010, OBC 2012				
GABLE STUDS SPACED AT 2-0-0 OC.								- CSA 086-09, CSA 086-14				
								- TPIC 2011, TPIC 2014				
PLATES (table is in inches)				CHORDS				(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD				
JT TYPE	PLATES	W	LEN	Y	X	MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. FACTORED UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED UNBRACED LENGTH (LC)
B TMVW+p	MT20	4.0	4.0	1.25	2.00	O-B	-267 / 0	0.0	0.0 (1)	L-E	-158 / 0	0.08 (1)
C, D, F, G						A-B	0 / 39	-102.0	-102.0 (1)	M-D	-216 / 0	0.06 (1)
C TMW+w	MT20	2.0	4.0			B-C	-16 / 0	-102.0	-102.0 (1)	N-C	-251 / 0	0.05 (1)
E TTW+p	MT20	4.0	4.0	2.25	2.00	C-D	-24 / 0	-102.0	-102.0 (1)	K-F	-238 / 0	0.07 (1)
H TMVW+p	MT20	4.0	4.0	1.25	2.00	D-E	-25 / 0	-102.0	-102.0 (1)	J-G	-163 / 0	0.03 (1)
I BMV1+p	MT20	3.0	4.0			E-F	-26 / 0	-102.0	-102.0 (1)	B-N	0 / 25	0.01 (1)
J BMVW1+1	MT20	4.0	4.0			F-G	-14 / 0	-102.0	-102.0 (1)	J-H	0 / 44	0.01 (1)
K, L, M						G-H	-14 / 0	-102.0	-102.0 (1)			
K BMW1+w	MT20	2.0	4.0			H-I	-84 / 0	0.0	0.0 (1)			
N BMVW1+1	MT20	4.0	4.0									
O BMV1+p	MT20	3.0	4.0			O-N	0 / 0	-18.5	-18.5 (4)			
						N-M	0 / 16	-18.5	-18.5 (4)			
						M-L	0 / 12	-18.5	-18.5 (4)			
						L-K	0 / 12	-18.5	-18.5 (4)			
						K-J	0 / 17	-18.5	-18.5 (4)			
						J-I	0 / 0	-18.5	-18.5 (4)			

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1987

PLATE PLACEMENT TOL = 0.250 inches

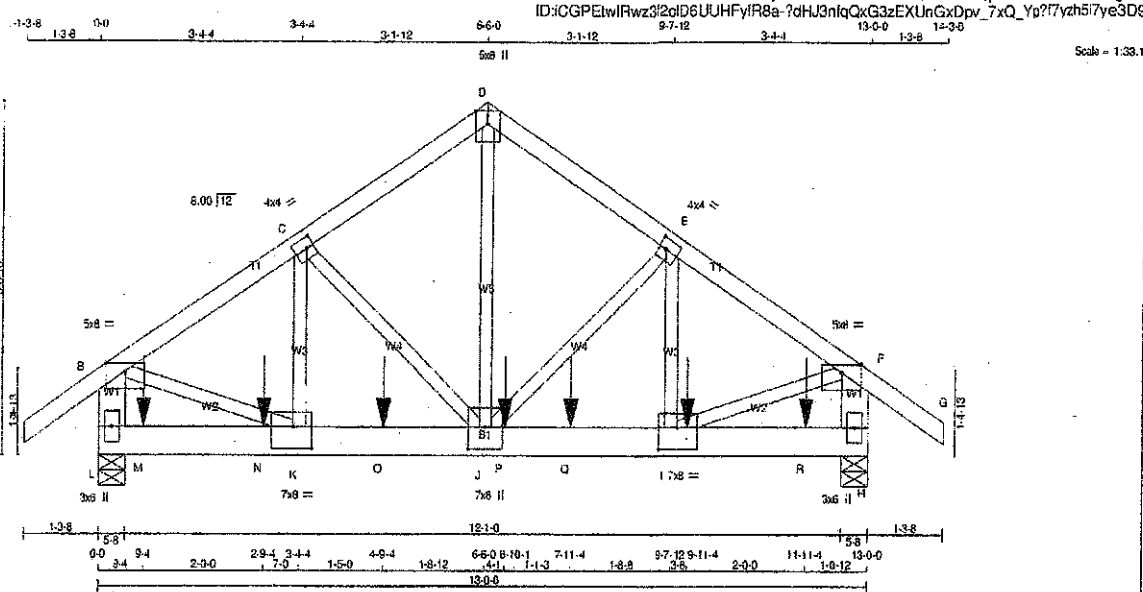
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.90)

JSI METAL = 0.13 (C) (INPUT = 1.00)

Structural component only
DWG# T-1923508

JOB NAME 405481	TRUSS NAME H21	QUANTITY 2	PLY 2	JOB DESC. GREEN PARK HOMES	ORWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	ORY	No.2
D - G	2x4	ORY	No.2
L - B	2x6	ORY	No.2
L - F	2x6	ORY	No.2
L - H	2x6	ORY	No.2
ALL WEBS	2x3	ORY	No.2

EXCEPT

DRY, SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-D 1	12	TOP
D-G 1	12	TOP
L-B 2	12	TOP
H-F 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
L-H 2	12	SIDE(183.1)
WEBS : (0.122"x3") SPIRAL NAILS		
E-I 1	6	SIDE(158.1)
J-D 1	6	SIDE(400.7)
2x3 1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMW-p	MT20	5.0	8.0	Edge	
C TMW-w	MT20	4.0	4.0	2.00	1.50
D TTW-p	MT20	5.0	6.0	Edge	
E TMW-w	MT20	4.0	4.0	2.00	1.50
F TMW-p	MT20	5.0	8.0	Edge	
H BMV-p	MT20	3.0	6.0		
I BMW-w	MT20	7.0	8.0	4.25	3.75
J BMW-w	MT20	7.0	8.0	4.25	3.50
K BMW-w	MT20	7.0	8.0	4.25	3.75
L BMV-p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	VERT	HORZ	GROSS REACTION	GROSS REACTION	BRG	IN-SX	BRG	IN-SX
L	4550	0	4550	0	0	5-8	5-8	5-8	5-8
H	5507	0	5507	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
L	3180	2247 / 0	0 / 0	0 / 0	0 / 0	943 / 0	0 / 0
H	3864	2709 / 0	0 / 0	0 / 0	0 / 0	1155 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB	FORCE (LBS)	FACTORED	MAX	MEMB	FORCE (LBS)	FACTORED	MAX
FR-TO				FR-TO			
A-B	0 / 39	-102.0	-102.0	0.08 (1)	10.00	J-D	0 / 4498
B-C	-4507 / 0	-102.0	-102.0	0.15 (1)	4.39	J-E	-1406 / 0
C-D	-4323 / 0	-102.0	-102.0	0.14 (1)	4.49	E-F	0 / 1300
D-E	-4320 / 0	-102.0	-102.0	0.15 (1)	4.49	C-J	-245 / 0
E-F	-5483 / 0	-102.0	-102.0	0.21 (1)	4.00	K-C	-81 / 27
F-G	0 / 39	-102.0	-102.0	0.08 (1)	10.00	B-K	0 / 3921
L-B	-3915 / 0	0.0	0.0	0.14 (1)	7.16	F-F	0 / 4768
H-F	-4700 / 0	0.0	0.0	0.17 (1)	6.67		
L-M	0 / 0	-18.5	-18.5	0.18 (1)	10.00		
M-N	0 / 0	-18.5	-18.5	0.18 (1)	10.00		
N-K	0 / 0	-18.5	-18.5	0.18 (1)	10.00		
K-O	0 / 3759	-18.5	-18.5	0.48 (1)	10.00		
O-J	0 / 3759	-18.5	-18.5	0.48 (1)	10.00		
J-P	0 / 4569	-18.5	-18.5	0.84 (1)	10.00		
P-Q	0 / 4569	-18.5	-18.5	0.84 (1)	10.00		
Q-R	0 / 4569	-18.5	-18.5	0.84 (1)	10.00		
R-H	0 / 0	-18.5	-18.5	0.31 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR
I	9-11-4	-1308	-1308	---	BACK	VERT
M	9-4	-684	-684	---	BACK	VERT
N	2-9-4	-682	-682	---	BACK	VERT
O	4-9-4	-682	-682	---	BACK	VERT
P	8-10-1	-2238	-2238	---	BACK	VERT
Q	7-11-4	-1308	-1308	---	BACK	VERT
R	11-11-4	-1308	-1308	---	BACK	VERT

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 6.0 PSF
DL = 7.4 PSF
TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/899 (0.05")
ALLOWABLE DEFL.(TL) = L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/899 (0.05")

CSI: TC=0.21/1.00 (E-F:1), BC=0.64/1.00 (I-J:1), WB=0.59/1.00 (F-I:1), SB=0.93/1.00 (I-J:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE FEELS OFF

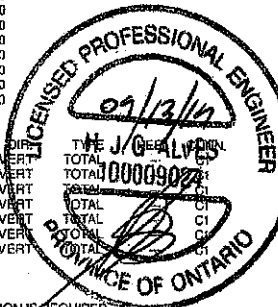
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1887 788 1887 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

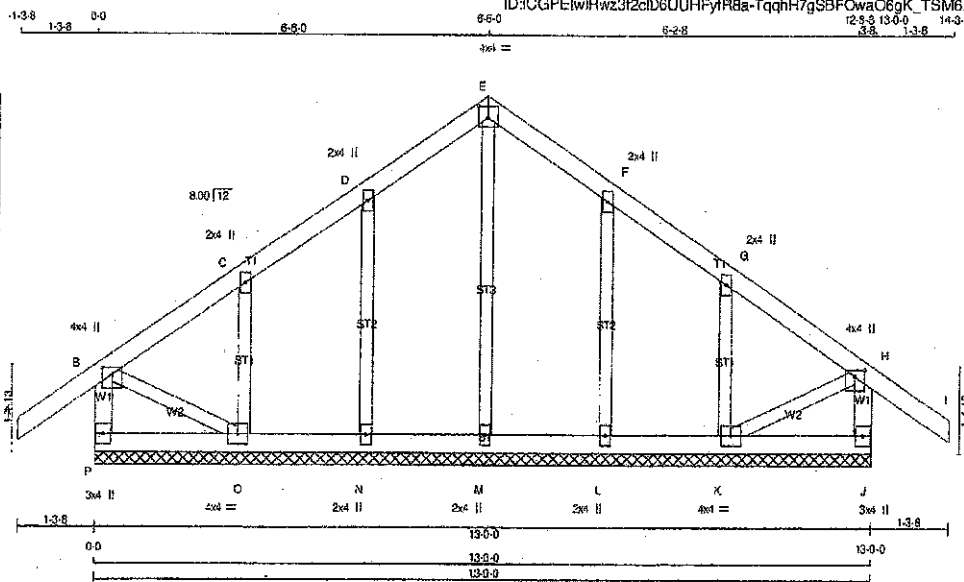
SI GRIP = 0.85 (D) (INPUT = 0.50)
SI METAL = 0.44 (F) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H21G	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:CGPEIwRwz3f2cd6UuHFyRba-TqnhH7gSBFOwaC6gK_TSM6XJmqSJYaxHBdQIHaye3D8



Scale = 1/32"

TOTAL WEIGHT = 2 X 57 = 114 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMW+p	MT20	4.0	4.0	1.25 2.00
C, D, F, G				
C TMW+u	MT20	2.0	4.0	
E TTW+p	MT20	4.0	4.0	2.25 2.00
H TMW+p	MT20	4.0	4.0	1.25 2.00
J BMV1+p	MT20	3.0	4.0	
K BMWV1+u	MT20	4.0	4.0	
L, M, N				
L BMW1+u	MT20	2.0	4.0	
O BMWV1+u	MT20	4.0	4.0	
P BMV1+p	MT20	3.0	4.0	

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L)
FR-TO			
P-B	-272 / 0	0.0	0.0 0.03 (1)
A-B	0 / 39	-102.0 -102.0	0.14 (1) 10.00
B-C	-21 / 0	-102.0 -102.0	0.07 (1) 6.25
C-D	-29 / 0	-102.0 -102.0	0.07 (1) 6.25
D-E	-30 / 0	-102.0 -102.0	0.05 (1) 6.25
E-F	-30 / 0	-102.0 -102.0	0.07 (1) 6.25
F-G	-29 / 0	-102.0 -102.0	0.07 (1) 6.25
G-H	-21 / 0	-102.0 -102.0	0.07 (1) 6.25
H-I	0 / 39	-102.0 -102.0	0.14 (1) 10.00
J-H	-272 / 0	0.0	0.0 0.03 (1) 7.81
P-O	0 / 0	-18.5 -18.5	0.03 (4) 10.00
O-N	0 / 21	-18.5 -18.5	0.03 (4) 10.00
N-M	0 / 16	-18.5 -18.5	0.02 (4) 10.00
M-L	0 / 16	-18.5 -18.5	0.02 (4) 10.00
L-K	0 / 21	-18.5 -18.5	0.03 (4) 10.00
K-J	0 / 0	-18.5 -18.5	0.03 (4) 10.00

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

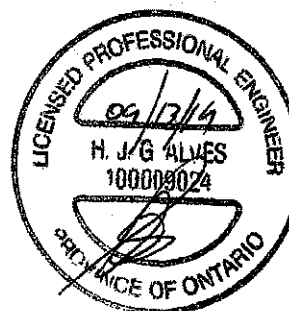
CS: TO=0.14/1.00 (A-B:1), BC=0.03/1.00 (N-O:4), WB=0.07/1.00 (E-M:1), SS=0.09/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

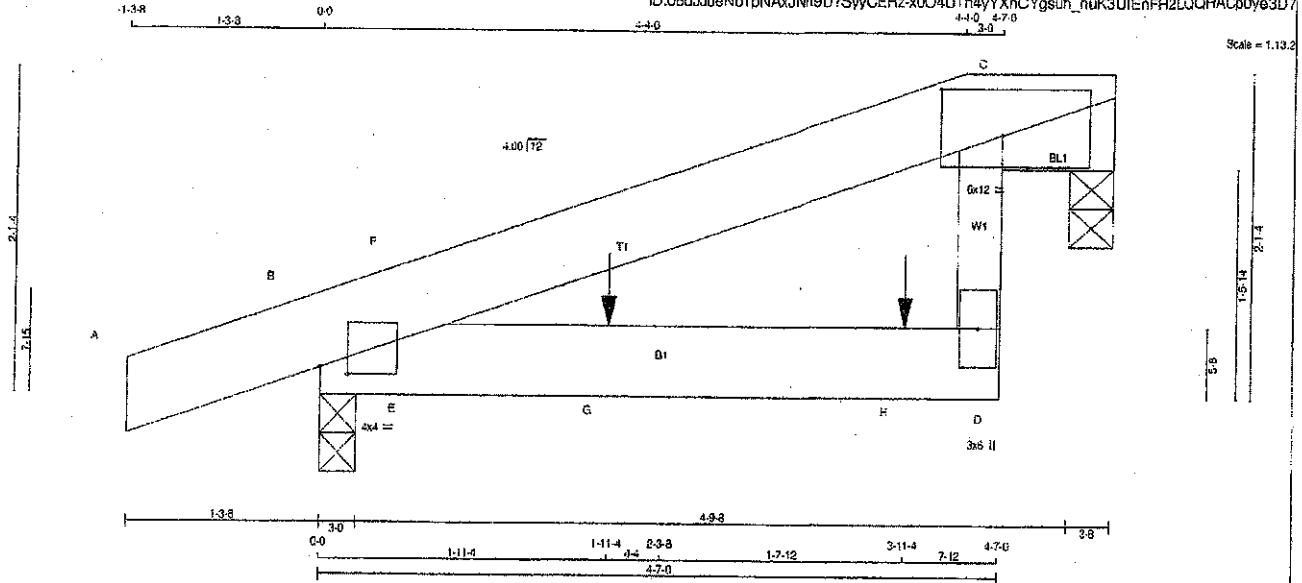
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES			
PLATE GRIP(DRY)	(PS)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	610 354	1667 768	1987 1856
PLATE PLACEMENT TOL. = 0.250 inches			
PLATE ROTATION TOL. = 5.0 Deg.			
JSI GRIP = 0.19 (H) (INPUT = 0.90)			
JSI METAL = 0.13 (C) (INPUT = 1.00)			



Structural component only
DWG# T-1923557

JOB NAME 405481	TRUSS NAME H26	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 MITek Industries, Inc. Fri Sep 13 12:22:14 2019 Page 1	
ID:0BdJlueNb1pNAXJN8U?SyyCERz-x0O4Uth4yYXnCYgshu_huK3UIEnFH2LQQHACp0ye3D7				4-10 4-7-0 3-0	



TOTAL WEIGHT = 2 X 24 = 47 lb

LUMBER			
N L G A. RULES	CHORDS	SIZE	LUMBER
A - C	2x6	DRY	No.2
D - C	2x4	DRY	No.2
B - D	2x6	DRY	No.2
BEARING BLOCKS			
BL1	2x6	DRY	No.2
SPF			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	4.0	4.0	0.50 2.25
C	TMVK1-p	MT20	6.0	12.0	3.50 5.00
D	BMV-p	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		RECORD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	BRG
C	296	0	296	0	0	3-8	3-8	3-8	3-8
B	425	0	425	0	0	3-0	3-0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

1ST CASE		MAX. MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
C	211	132.0	0.0	0.0	0.0	78.0	0.0
B	268	211.0	0.0	0.0	0.0	87.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LOC. MAX	MEMB.	FORCE (LBS)	MAX. FACTORED	LOC. MAX
FR-TO		FROM TO	PLF	CS1 (LC)	UNBRAC LENGTH	FR-TO	CS1 (LC)
A-B	0.13	-102.0	-102.0	0.07 (1)	10.00	E-F	-214.29
B-F	-31.0	-102.0	-102.0	0.04 (4)	6.25		0.00 (1)
F-C	0.3	-102.0	-102.0	0.15 (1)	10.00		
D-C	0.32	0.0	0.0	0.02 (4)	10.00		
B-E	0.0	-18.5	-18.5	0.11 (1)	10.00		
E-G	0.0	-18.5	-18.5	0.11 (1)	10.00		
G-H	0.0	-18.5	-18.5	0.11 (1)	10.00		
H-D	0.0	-18.5	-18.5	0.11 (1)	10.00		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LOC	MAX	MAX	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	-13	-14	---	FRONT	VERT	TOTAL	---	C1
H	3-11-4	-17	-17	---	FRONT	VERT	TOTAL	---	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
DL	=	8.0	PSF	
BOT CH.	LL	=	9.0	PSF
DL	=	7.4	PSF	
TOTAL LOAD	=	42.4	PSF	

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, NBCC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.19")
CALCULATED VERT. DEFL. (LL) = L/399 (0.01")
ALLOWABLE DEFL. (TL) = L/360 (0.19")
CALCULATED VERT. DEFL. (TL) = L/399 (0.02")

CS1: TC=0.15/1.00 (C-F:1), BC=0.11/1.00 (D-E:1),
WB=0.00/1.00 (E-F:1), SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

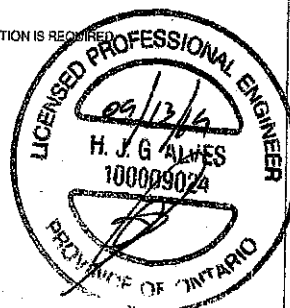
NAIL VALUES

PLATE GRIP (DRY) SHEAR	SECTION
(PSI)	(PLI)
MAX MIN	MAX MIN
MT20	618 354 1587 705 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.19 (B) (INPUT = 0.60)
JSI METAL = 0.06 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923558

JOB NAME 405481	TRUSS NAME J1	QUANTITY 22	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:15 2019 Page 1	
TOTAL WEIGHT = 22 X 18 = 392 lb					

LUMBER N. L. G. A. RULES CHORDS SIZE LUMBER DESCR. E - B 2x4 DRY No.2 SPF A - C 2x4 DRY No.2 SPF E - D 2x4 DRY No.2 SPF DRY, SEASONED LUMBER.	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">JT</th> <th colspan="2">FACTORED GROSS REACTION</th> <th colspan="2">MAXIMUM FACTORED GROSS REACTION</th> <th rowspan="2">INPUT BRG IN-SX</th> <th rowspan="2">REQD BRG IN-SX</th> </tr> <tr> <th>VERT</th> <th>HORZ</th> <th>DOWN</th> <th>HORZ</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>578</td> <td>0</td> <td>578</td> <td>0</td> <td>5-8</td> <td>5-8</td> </tr> <tr> <td>C</td> <td>225</td> <td>0</td> <td>225</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> <tr> <td>D</td> <td>46</td> <td>0</td> <td>51</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> </tbody> </table> SEE MITEK STANDARD DETAIL 697791H FOR CONNECTION TO JOINT(S) C, D UNFACTORED REACTIONS <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">JT</th> <th rowspan="2">COMBINED</th> <th colspan="6">MAX/MIN COMPONENT REACTIONS</th> </tr> <tr> <th>1ST CASE</th> <th>SNOW</th> <th>LIVE</th> <th>PERM LIVE</th> <th>WIND</th> <th>DEAD</th> <th>SOIL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>404</td> <td>293 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>111 / 0</td> <td>0 / 0</td> </tr> <tr> <td>C</td> <td>154</td> <td>128 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>26 / 0</td> <td>0 / 0</td> </tr> <tr> <td>D</td> <td>38</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>0 / 0</td> <td>36 / 0</td> <td>0 / 0</td> </tr> </tbody> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.	JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	VERT	HORZ	DOWN	HORZ	E	578	0	578	0	5-8	5-8	C	225	0	225	0	1-8	1-8	D	46	0	51	0	1-8	1-8	JT	COMBINED	MAX/MIN COMPONENT REACTIONS						1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL	E	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0	C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0	D	38	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX																																																																		
	VERT	HORZ	DOWN	HORZ																																																																				
E	578	0	578	0	5-8	5-8																																																																		
C	225	0	225	0	1-8	1-8																																																																		
D	46	0	51	0	1-8	1-8																																																																		
JT	COMBINED	MAX/MIN COMPONENT REACTIONS																																																																						
		1ST CASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL																																																																
E	404	293 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0																																																																	
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0																																																																	
D	38	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0																																																																	

LOADING TOTAL LOAD CASES: (4) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">MEMB.</th> <th rowspan="2">MAX. FACTORED FORCE (LBS)</th> <th colspan="2">FACTORED VERT. LOAD LC1</th> <th rowspan="2">MAX. CS1 (LC)</th> <th rowspan="2">MAX. UNBRAC LENGTH FR-TO</th> <th rowspan="2">MEMB. FORCE MAX (LBS)</th> <th rowspan="2">MAX. FACTORED CS1 (LC)</th> </tr> <tr> <th>FROM</th> <th>TO</th> </tr> </thead> <tbody> <tr> <td>E-B</td> <td>-515 / 0</td> <td>0.0</td> <td>0.0</td> <td>0.12 (4)</td> <td>7.81</td> <td></td> <td></td> </tr> <tr> <td>A-B</td> <td>0 / 39</td> <td>-102.0</td> <td>-102.0</td> <td>0.14 (1)</td> <td>10.00</td> <td></td> <td></td> </tr> <tr> <td>B-C</td> <td>-42 / 0</td> <td>-102.0</td> <td>-102.0</td> <td>0.80 (1)</td> <td>6.25</td> <td></td> <td></td> </tr> <tr> <td>E-D</td> <td>0 / 0</td> <td>-18.5</td> <td>-18.5</td> <td>0.13 (4)</td> <td>10.00</td> <td></td> <td></td> </tr> </tbody> </table>	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1		MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE MAX (LBS)	MAX. FACTORED CS1 (LC)	FROM	TO	E-B	-515 / 0	0.0	0.0	0.12 (4)	7.81			A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00			B-C	-42 / 0	-102.0	-102.0	0.80 (1)	6.25			E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00			DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 28.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCBC 2018, OBC 2012 - CSA 086-08, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.20") CALCULATED VERT. DEFL.(LL) = L/989 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.20") CALCULATED VERT. DEFL.(TL) = L/989 (0.03") CS1: TC=0.60/1.00 (B-C:1), BC=0.12/1.00 (D-E:4), WB=0.00/1.00 (A:0), SS=0.24/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1667 788 1987 1656 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.22 (B) (INPUT = 0.90) JSI METAL = 0.17 (B) (INPUT = 1.00)
MEMB.			MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1					MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB. FORCE MAX (LBS)	MAX. FACTORED CS1 (LC)																															
	FROM	TO																																									
E-B	-515 / 0	0.0	0.0	0.12 (4)	7.81																																						
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00																																						
B-C	-42 / 0	-102.0	-102.0	0.80 (1)	6.25																																						
E-D	0 / 0	-18.5	-18.5	0.13 (4)	10.00																																						

Structural component only
 DWG# T-1923559

JOB NAME 405481	TRUSS NAME J3	QUANTITY 42	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T-1923560
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 12:22:17 2019 Page 1 ID:08dJueNb1pNAXJnn9U?SyyCEHz-Mb4C7VjFTvM30PRZpXOWYhw9RqJUP5s6FOsQLye3D4	

Scale = 1:14.0

LUMBER N L G A RULES CHORDS SIZE LUMBER E - B 2x8 DRY No.2 A - C 2x4 DRY No.2 E - D 2x4 DRY No.2 DRY, SEASONED LUMBER.		DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">JT</th> <th colspan="2">FACTORED GROSS REACTION</th> <th colspan="2">MAXIMUM FACTORED GROSS REACTION</th> <th rowspan="2">INPUT BRG</th> <th rowspan="2">REQD BRG</th> </tr> <tr> <th>VERT</th> <th>HORZ</th> <th>DOWN</th> <th>HORZ</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>538</td> <td>0</td> <td>538</td> <td>0</td> <td>3-0</td> <td>3-0</td> </tr> <tr> <td>C</td> <td>204</td> <td>0</td> <td>204</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> <tr> <td>D</td> <td>37</td> <td>0</td> <td>42</td> <td>0</td> <td>1-8</td> <td>1-8</td> </tr> </tbody> </table>		JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG	VERT	HORZ	DOWN	HORZ	E	538	0	538	0	3-0	3-0	C	204	0	204	0	1-8	1-8	D	37	0	42	0	1-8	1-8	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 29.0 PSF OL = 6.0 PSF BOT CH. LL = 0.0 PSF OL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN. OC THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCG 2010, NBCG 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF BCG 2015, OBC 2012 - CSA 086-09, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS - OVERHANG NOT TO BE ALTERED OR CUT OFF. (55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(TL) = L/360 (0.19") CALCULATED VERT. DEFL.(TL) = U 989 (0.02") CSI: TC=0.49/1.00 (B-C:1), BC=0.15/1.00 (D-E:4), WB=0.00/1.00 (r/a:0), SSI=0.25/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI) MAX MIN MAX MIN MAX MIN MT20 618 354 1567 788 1987 1556 PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.24 (E) (INPUT = 0.90) JSI METAL = 0.09 (E) (INPUT = 1.00)	
JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG																															
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E	538	0	538	0	3-0	3-0																															
C	204	0	204	0	1-8	1-8																															
D	37	0	42	0	1-8	1-8																															

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
E	TMSMV1-p	MT20	3.0	6.0	2.25	2.75

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX-MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM LVE	WIND	DEAD	SOIL
E	376	271/0	0/0	0/0	0/0	105/0	0/0
C	140	118/0	0/0	0/0	0/0	24/0	0/0
D	30	0/0	0/0	0/0	0/0	30/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED LCI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LCI (LC)	
FR-TO		FROM	TO	FR-TO		FROM	TO
E-B	-477/0	0.0	0.0 (4)	7.81			
A-B	0/22	-102.0	-102.0 (1)	10.00			
B-C	-22/0	-102.0	-102.0 (1)	5.25			
E-D	0/0	-18.5	-18.5 (4)	10.00			

Structural component only
 DWG# T-1923560

JOB NAME 405481	TRUSS NAME J4	QUANTITY 12	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.300 S May 10 2019 M/Tak Industries, Inc. Fri Sep 13 12:22:19 2019 Page 1	
ID:08dJueNb1pNAxJnn9U?SyyCERz-1_CyXAIDn593UJZqEzsbNnLPFVWYHa9ZztUDye3D2				Scale = 1:18.4	

Dimensions, Supports and Loadings Specified by Fabricator to be Verified by Building Designer

BEARINGS

SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 379	DOWN 379	4-0 (3-15)	4-0
G	546	0	5-0	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
D	268	192 / 0	0 / 0	0 / 0	84 / 0	0 / 0
G	381	277 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	FR-TO
A-B	0 / 26	-102.0	10.00	G-B	-517 / 0	10.00	0.02 (1)
B-C	-494 / 0	-102.0	6.25	B-F	0 / 494	10.00	0.11 (1)
C-D	-479 / 0	-102.0	6.25	F-C	-377 / 0	10.00	0.05 (1)
E-D	0 / 29	0.0	10.00	F-D	0 / 575	10.00	0.13 (1)
G-F	0 / 0	-18.5	10.00				
F-E	0 / 0	-18.5	10.00				

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - D	2x4	DRY	No.2
E - D	2x4	DRY	No.2
G - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2

EXCEPT G - B 2x8 DRY No.2

DRY: SEASONED LUMBER.

PLATES (Table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMW-w	MT20	2.0	4.0		
D	TMVWW-1	MT20	6.0	18.0	2.25	5.00
E	BMV-p	MT20	3.0	4.0		
F	BMVWW-1	MT20	4.0	9.0		
G						
G	TMBMWW-1	MT20	6.0	9.0	2.75	2.25

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.21")

CALCULATED VERT. DEFL.(LL) = L/999 (0.01")

ALLOWABLE DEFL.(TL) = L/360 (0.21")

CALCULATED VERT. DEFL.(TL) = L/999 (0.01")

CSI: TC=0.19/1.00 (A-B:1), BC=0.04/1.00 (E-F:4), WB=0.13/1.00 (D-F:1), SS=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.42 (G) (INPUT = 0.90)

JSI METAL = 0.13 (F) (INPUT = 1.00)

Structural component only

DWG# T-1923561

JOB NAME 405481	TRUSS NAME J5	QUANTITY 10	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T-1923562
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Tamarack Roof Truss, Burlington
Version 8.300 S May 10 2019 Mitek Industries, Inc. Fr Sep 13 12:22:20 2019 Page 1

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Scale = 1:16.4

LUMBER N. L. G. A. RULES CHORDS SIZE E - B 2x6 DRY A - C 2x4 DRY E - D 2x4 DRY DRY, SEASONED LUMBER.		LUMBER No.2 No.2 No.2	DESCR. SPF SPF SPF	DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER BEARINGS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th></th> <th>FACTORED GROSS REACTION</th> <th>MAXIMUM FACTORED GROSS REACTION</th> <th>INPUT BRG</th> <th>REQD BRG</th> </tr> <tr> <td>JT</td> <td>VERT</td> <td>DOWN</td> <td>UPLIFT</td> <td>IN-SX</td> </tr> <tr> <td>E</td> <td>813</td> <td>0</td> <td>0</td> <td>3-0</td> </tr> <tr> <td>C</td> <td>242</td> <td>0</td> <td>0</td> <td>1-8</td> </tr> <tr> <td>D</td> <td>44</td> <td>0</td> <td>0</td> <td>1-8</td> </tr> </table> SEE MITEK STANDARD DETAIL R97791H FOR CONNECTION TO JOINT(S) E, C UNFACTORED REACTIONS <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>1ST LCASE</th> <th>MAX./MIN. COMPONENT REACTIONS</th> </tr> <tr> <td>JT</td> <td>COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOL</td> </tr> <tr> <td>E</td> <td>429 307 / 0 0 / 0 0 / 0 122 / 0 0 / 0</td> </tr> <tr> <td>C</td> <td>156 138 / 0 0 / 0 0 / 0 29 / 0 0 / 0</td> </tr> <tr> <td>D</td> <td>35 0 / 0 0 / 0 0 / 0 35 / 0 0 / 0</td> </tr> </table> BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C BRACING TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED. ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG	JT	VERT	DOWN	UPLIFT	IN-SX	E	813	0	0	3-0	C	242	0	0	1-8	D	44	0	0	1-8	1ST LCASE	MAX./MIN. COMPONENT REACTIONS	JT	COMBINED SNOW LIVE PERM. LIVE WIND DEAD SOL	E	429 307 / 0 0 / 0 0 / 0 122 / 0 0 / 0	C	156 138 / 0 0 / 0 0 / 0 29 / 0 0 / 0	D	35 0 / 0 0 / 0 0 / 0 35 / 0 0 / 0	DESIGN CRITERIA SPECIFIED LOADS: TOP CH. LL = 28.0 PSF DL = 6.0 PSF BOT CH. LL = 0.0 PSF DL = 7.4 PSF TOTAL LOAD = 42.4 PSF SPACING = 24.0 IN. C/C THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015 THIS DESIGN COMPLIES WITH: - PART 9 OF CBC 2018, OBC 2012 - CSA 086-08, CSA 086-14 - TPIC 2011, TPIC 2014 DESIGN ASSUMPTIONS OVERHANG NOT TO BE ALTERED OR CUT OFF. (55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD ALLOWABLE DEFL.(LL) = L/360 (0.21") CALCULATED VERT. DEFL.(LL) = L/999 (0.00") ALLOWABLE DEFL.(TL) = L/360 (0.21") CALCULATED VERT. DEFL.(TL) = L/999 (0.03") CSI: TC=0.54/1.00 (B-C:1), BC=0.21/1.00 (D-E:1), WB=0.00/1.00 (E:0), SSI=0.30/1.00 (B-C:1) DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10 COMPANION LIVE LOAD FACTOR = 1.00 AUTOSOLVE RIGHT HEEL ONLY TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT. NAIL VALUES <table border="1" style="width:100%; border-collapse: collapse;"> <tr> <th>PLATE</th> <th>GRIP(DRY)</th> <th>SHEAR</th> <th>SECTION</th> </tr> <tr> <th></th> <th>(PS)</th> <th>(PL)</th> <th>(PL)</th> </tr> <tr> <td>MT20</td> <td>618</td> <td>354</td> <td>1567</td> </tr> </table> PLATE PLACEMENT TOL. = 0.250 inches PLATE ROTATION TOL. = 5.0 Deg. JSI GRIP = 0.27 (E) (INPUT = 0.90) JSI METAL = 0.11 (E) (INPUT = 1.00)	PLATE	GRIP(DRY)	SHEAR	SECTION		(PS)	(PL)	(PL)	MT20	618	354	1567
	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG																																																
JT	VERT	DOWN	UPLIFT	IN-SX																																																
E	813	0	0	3-0																																																
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	(PS)	(PL)	(PL)																																																	
MT20	618	354	1567																																																	

Structural component only
 DWG# T-1923562



isDesign™

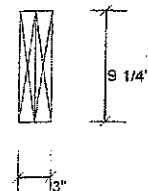
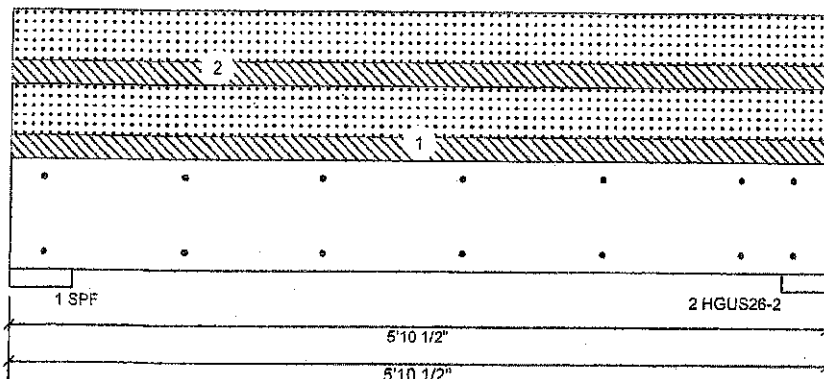
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405481
Project #:

Page 1 of 4

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	278	602	0
2	0	266	577	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	348 / 903	1250 L	1.25D+1.5S
2 - HGUS...	4.000"	16%	333 / 865	1198 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1413 ft-lb	3'	6039 ft-lb	0.234 (23%)	1.25D+1.5S	L
Unbraced	1413 ft-lb	3'	5236 ft-lb	0.270 (27%)	1.25D+1.5S	L
Shear	1085 lb	1'2"	3984 lb	0.272 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5129)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3508)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

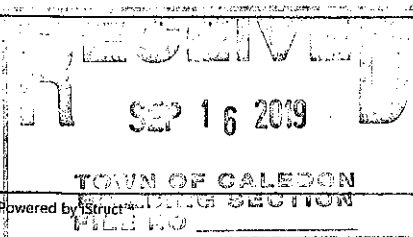
Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 11923564
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	



This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115





isDesign™

Client:
Project:
Address:

Date: 2/2019

Page 2 of 4

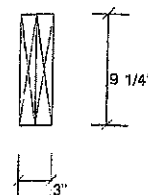
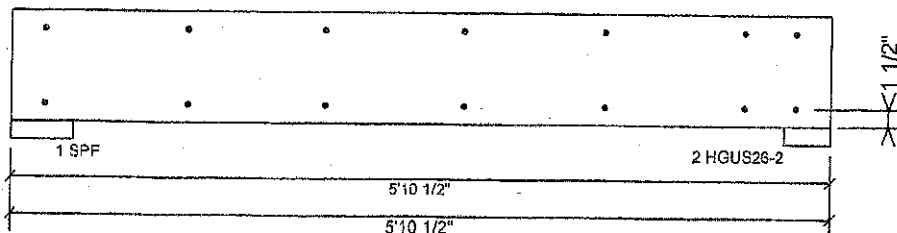
Designer:

Job Name: 405481

Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

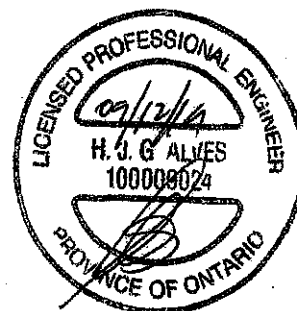
Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	78.6 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00



DWG NO. TAM **0923564**
STRUCTURAL
COMPONENT ONLY **3/2**

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021





isDesign™

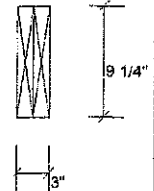
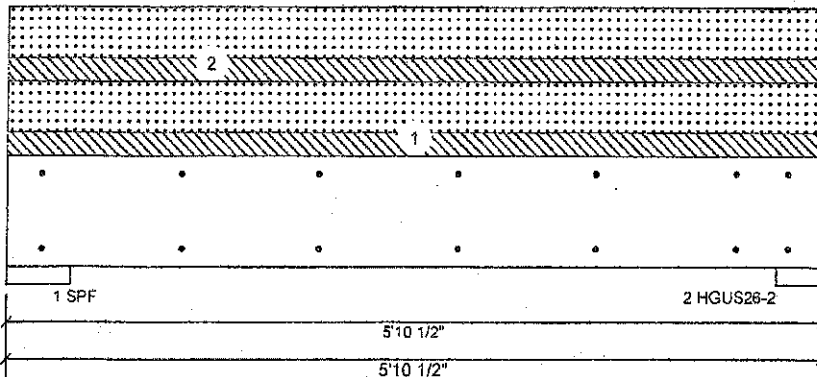
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405481
Project #:

Page 3 of 4

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	274	593	0
2	0	262	568	0

Bearings and Factored Reactions

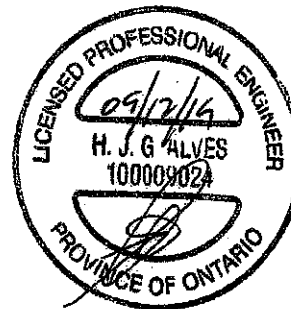
Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	342 / 889	1231 L	1.25D+1.5S
2 - HGUS...	4.000"	16%	328 / 852	1180 L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1392 ft-lb	3'	6039 ft-lb	0.230 (23%)	1.25D+1.5S	L
Unbraced	1392 ft-lb	3'	5236 ft-lb	0.266 (27%)	1.25D+1.5S	L
Shear	1069 lb	4'10"	3984 lb	0.268 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5208)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3562)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

Design Notes

- 1 Fasten all piles using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 1723563
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-9-12	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115



This design is valid until 12/11/2021





isDesign™

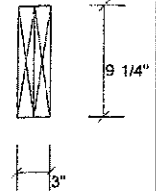
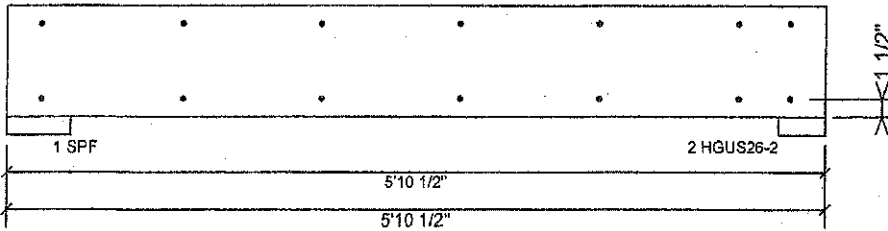
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405481
Project #:

Page 4 of 4

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

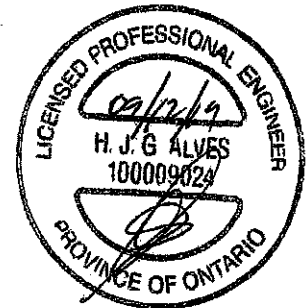
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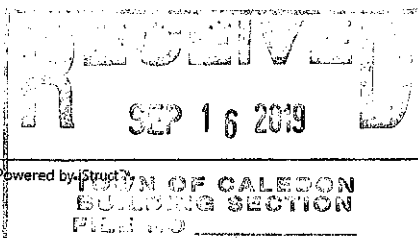
Multi-Ply Analysis

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	77.2 %
Load	175.1 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00



DWG NO. TAM T1923563
STRUCTURAL
COMPONENT ONLY 3/2



This design is valid until 12/11/2021

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115
	TAMARACK LUMBER INC ALMA LUMBER GROUP



LUL/LUS/LJS/HUS/HHUS/HGUS

SIMPSON
Strong-Tie

Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of
a) easier installation, b) higher capacities, c) lower installed
cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

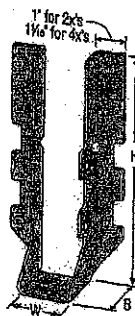
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



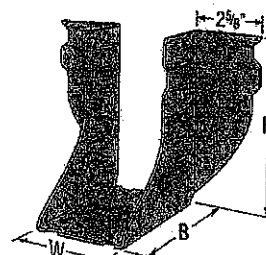
✓ LUS28



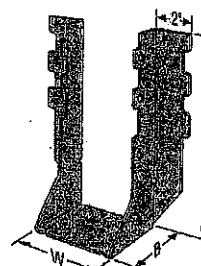
LU26L



✓ HUS210
(HUS26, HUS28,
and HHUS similar)



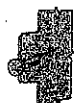
✓ HGUS28-2



✓ HHUS210-2



Double-Shear
Nailing
Top View

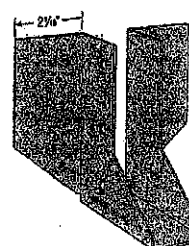
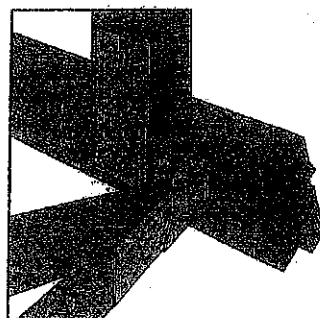


Double-Shear
Nailing
Side View;
Do not
bend tab



Dome Double-Shear
Nailing Side View
(available on
some models)
U.S. Patent 5,603,580

Typical HUS26
Installation
with Reduced
Heel Height
(Truss Designer
to provide
fastener quantity
for connecting
multiple members
together)



LJS26DS

Plated Truss Connectors

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SEP 16 2019

Simpson Strong-Tie® Wood Construction Connectors — Canadian Limit States Design

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

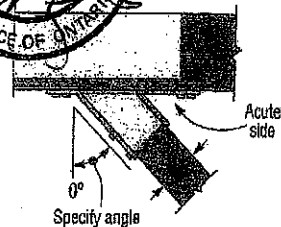
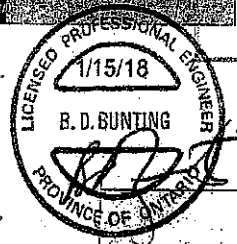
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger
Skewed Right
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	dg	Header	Joist		T & F		S-P-F	
									Uplift (K _u = 1.5)	Down (K _d = 1.75)	Uplift (K _u = 1.75)	Normal (K _d = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Single 2x Sizes												
SS	LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
	LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
SS	LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1805	645	1140
	LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
SS	HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2065	3875
	LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(8) 16d	2055	4265	1460	4115
SS	HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
	LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1560
SS	LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2520	1290	1790
	HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3605	5365	2875	4345
SS	HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7875	3310	6900
	LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
SS	LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
									8.32	32.39	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. dg is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/4" long. See pp. 27–28 for other nail sizes and information.

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Face-Mount Hangers

These products are available with additional corrosion protection. For more information, see p. 24.

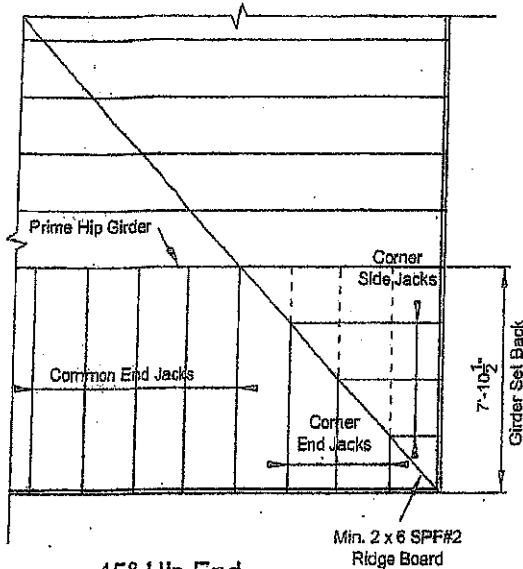
These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)					Fasteners		Factored Resistance			
		W	H	B	d _h ¹	Header	Joist		D-F-H		S-P-F	
									Uplift (K ₁ = 1.15)	Normal (K ₂ = 1.00)	Uplift (K ₁ = 1.15)	Normal (K ₂ = 1.00)
									lb.	lb.	lb.	lb.
									kN	kN	kN	kN
Double 2x Sizes												
LUS24-2	18	3 1/4	3 1/4	2	1 1/4	(4) 16d	(2) 16d		835	2020	590	1435
LUS26-2	18	3 1/4	4 1/4	2	4	(4) 16d	(4) 16d		1720	2595	1545	1920
HHUS26-2	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d		2850	7335	2085	5205
HGUS28-2	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
LUS28-2	18	3 1/4	7	2	4	(6) 16d	(4) 16d		1720	3325	1545	2575
HHUS28-2	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d		3765	8940	2675	6345
HGUS28-2	12	3 1/4	7 1/4	4	6 1/4	(26) 16d	(12) 16d		6070	12980	4310	9215
LUS210-2	18	3 1/4	9	2	6	(8) 16d	(5) 16d		2580	4500	2320	3195
HHUS210-2	14	3 1/4	9 1/4	3	8	(30) 16d	(10) 16d		4670	9660	4235	7000
HGUS210-2	12	3 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14015	4855	10270
Triple 2x Sizes												
HGUS26-3	12	4 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
HGUS28-3	12	4 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
HHUS210-3	14	4 1/4	9	3	7 1/4	(30) 16d	(10) 16d		4670	9670	4235	6865
HGUS210-3	12	4 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14845	4855	10400
Quadruple 2x Sizes												
HGUS26-4	12	6 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
HGUS28-4	12	6 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
HHUS210-4	14	6 1/4	9	3	7 1/4	(30) 16d	(10) 16d		4670	10155	4235	7210
HGUS210-4	12	6 1/4	9 1/4	4	8 1/4	(46) 16d	(16) 16d		6840	14845	4855	10400
HGUS212-4	12	6 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d		7640	14995	5425	10845
HGUS214-4	12	6 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645
4x Sizes												
LUS46	18	3 1/4	4 1/4	2	3 1/4	(4) 16d	(4) 16d		1720	2595	1545	1920
HHUS46	14	3 1/4	5 1/4	3	3 1/4	(14) 16d	(6) 16d		2850	7335	2085	5205
HGUS46	12	3 1/4	5 1/4	4	4 1/4	(20) 16d	(8) 16d		4385	8950	3110	6355
LUS48	18	3 1/4	6 1/4	2	3 1/4	(6) 16d	(4) 16d		1720	3325	1545	2575
HHUS48	14	3 1/4	7 1/4	3	6 1/4	(22) 16d	(8) 16d		3765	8940	2675	6345
HGUS48	12	3 1/4	7 1/4	4	6 1/4	(36) 16d	(12) 16d		6070	12980	4310	9215
LUS410	18	3 1/4	8 1/4	2	5 1/4	(8) 16d	(5) 16d		2580	4500	2320	3195
HGUS410	12	3 1/4	9	4	8 1/4	(46) 16d	(16) 16d		6840	14015	4855	10270
HGUS412	12	3 1/4	10 1/4	4	10 1/4	(56) 16d	(20) 16d		7640	14995	5425	10845
HGUS414	12	3 1/4	12 1/4	4	11 1/4	(66) 16d	(22) 16d		10130	16400	7195	11645



Plated Truss Connectors

See footnotes
on p. 258.



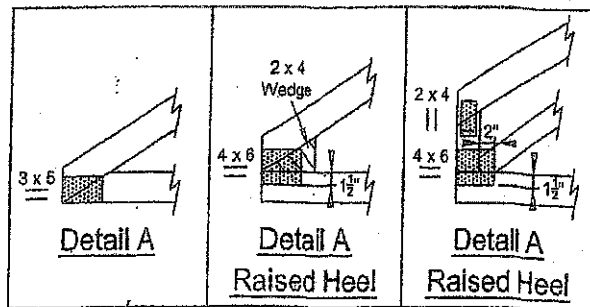
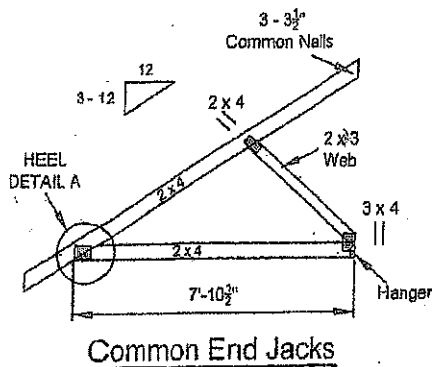
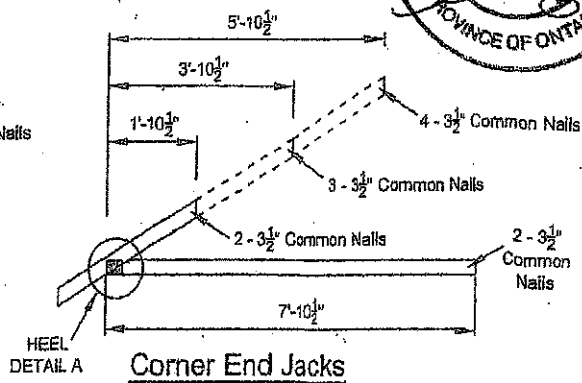
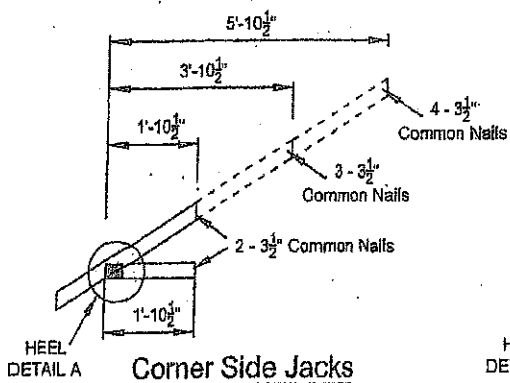
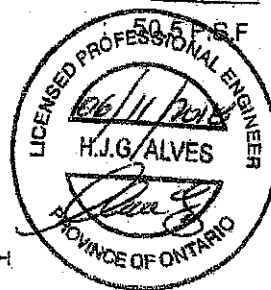
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

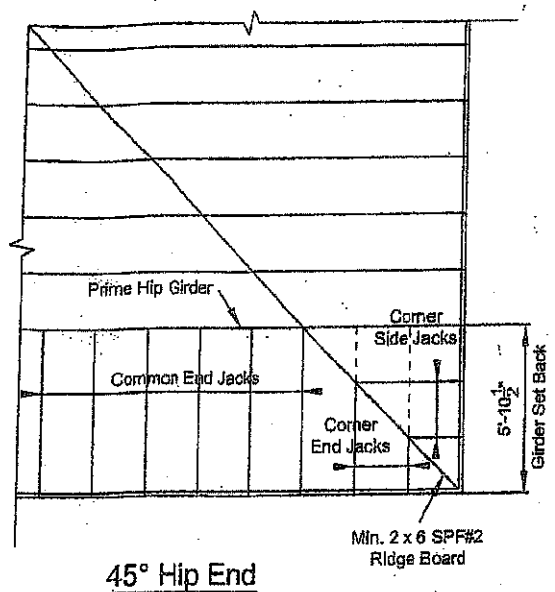
TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217

RECEIVED
SEP 16 2019
TOWN OF CALEDON
BUILDING SECTION
FILE NO.



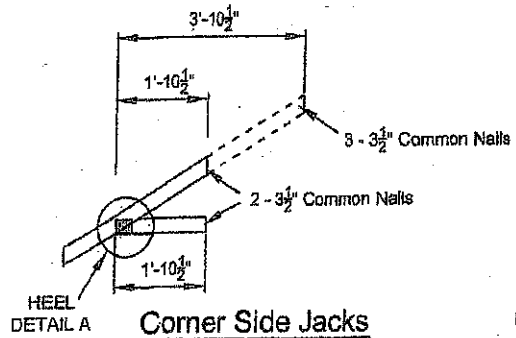
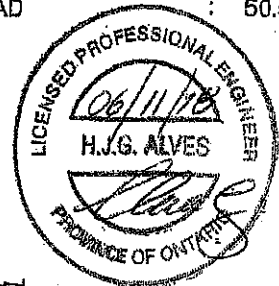
45° Hip End

LUMBER SPECIFICATION

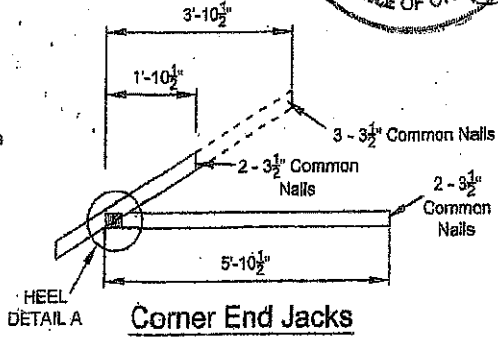
TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

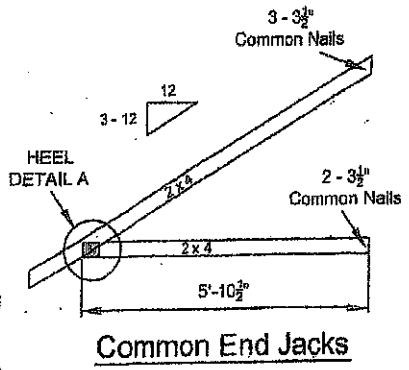
TOP CHORD SNOW LOAD : 40.5 P.S.F.
TOP CHORD DEAD LOAD : 3.0 P.S.F.
BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.
TOTAL LOAD : 50.5 P.S.F



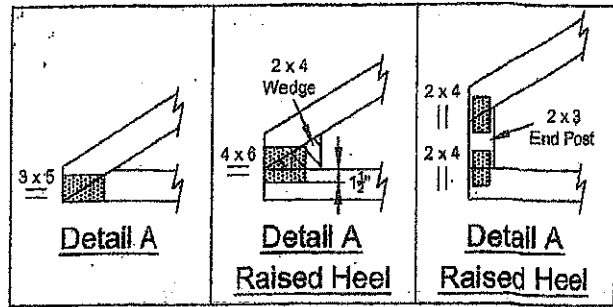
Corner Side Jacks



Corner End Jacks



Common End Jacks



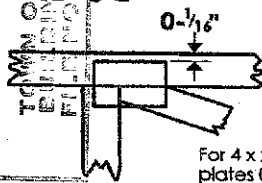
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



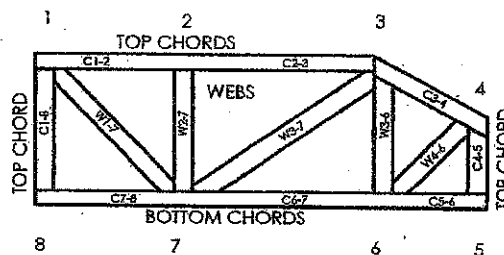
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek Engineering Reference Sheet: MII-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

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BUILDING SECTION

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900218

Feb 09, 2018