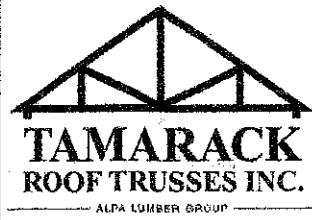




# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: GREEN PARK HOMES  
 Project: LAMBERT LANE PH.2  
 Location: CALEDON  
 Model: BLOCK 82, 85, 90  
 Lot #:   
 Elevation: 1 / UNITS 82-1, 82-3, 85-1, 85-3;

RECEIVED

Plan Log: 201705  
 Layout ID: 405492  
 Ref #:  
 Design of CALEDON BUILDING SECTION  
 DATE NO 09/09/2019

Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## Roof Trusses

PROFILE	QTY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG		HEEL HEIGHT		LBS. BFT.	BUNDLE #	LOAD BY
	PLY						LEFT RIGHT	LEFT RIGHT	STACK #	REMARKS			
	1 2-ply	H41 Half Hip Girder	8/12	22-02-10	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	210.24 132.00				
	1	H42 Half Hip	8/12	22-02-10	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	92.51 57.67				
	1	H43 Half Hip	8/12	22-02-10	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	100.46 64.00				
	1	H44 Half Hip	5/12	22-02-10	7-01-04	2 x 4	1-03-08	1-00-09 7-01-04	94.33 59.00				
	1	H45 Half Hip	5/12	22-02-10	8-01-04	2 x 4	1-03-08	1-00-09 8-01-04	98.76 62.17				
	1	H46 Half Hip	5/12	22-02-10	9-01-04	2 x 4	1-03-08	1-00-09 9-01-04	103.9 64.83				
	1	H47 Half Hip	5/12	22-02-10	10-01-04	2 x 4	1-03-08	1-00-09 10-01-04	101.28 62.83				
	8	H48 Monopitch	5/12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-11	764.85 476.00				
	3	H49 Common	8/12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	157.12 95.50				
	1	H49G GABLE	8/12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	51.66 32.83				
	6	J13 Jack-Open	4/12	6-00-00	2-08-14	2 x 4	1-03-08	7-15 2-04-15	98.04 64.00				
	6	J18 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00				

TOTAL # TRUSS= 32      TOTAL BFT OF ALL TRUSSES= 1234.83      BFT.      TOTAL WEIGHT OF ALL TRSSES 1973.92 LBS

## HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

# DELIVERY SHIPLIST

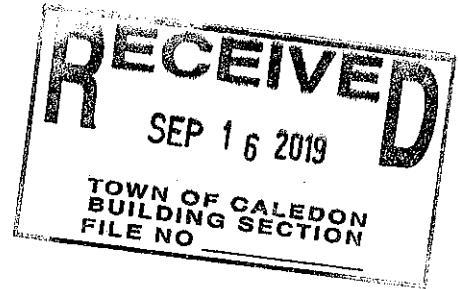


Lumber Yard: TAMARACK LUMBER  
 Builder: GREEN PARK HOMES  
 Project: LAMBERT LANE PH.2  
 Location: CALEDON  
 Model: BLOCK 82, 85, 90  
 Lot #:  
 Elevation: 1 / UNITS 82-1, 82-3, 85-1, 85-3,

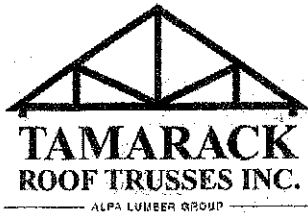
Job Track: 50033  
 PlanLog: 201705  
 Layout ID: 405492  
 Ref #  
 Page: 2 of 2  
 Date: 09/09/2019  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano

## HARDWARE

QTY	TYPE	MODEL	LENGTH
TOTAL NUMBER OF			3
ITEMS=			



# DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER  
 Builder: GREEN PARK HOMES  
 Project: LAMBERT LANE PH.2  
 Location: CALEDON  
 Model: BLOCK 82, 85, 90  
 Lot #:   
 Elevation: 1A / UNITS 82-2, 82-4, 85-2, 85-4,

Job Track: 5000  
 Plan No: 201105  
 Layout ID: 405491  
 Ref #: **RECEIVED**  
**SEP 16 2019**  
 Page: 1 of 2  
 Date: **TOWN OF CALEDON**  
**BUILDING SECTION**  
 Designer: Andrew Conway  
 Sales Rep: Mario DiCano


## Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	H41 Half Hip Girder	8/12	22-02-10	4-01-04	2 x 4 2 x 6	1-03-08	1-04-13 4-01-04	210.24 192.00		
	1	H42 Half Hip	8/12	22-02-10	5-01-04	2 x 4	1-03-08	1-04-13 5-01-04	92.51 57.67		
	1	H43 Half Hip	8/12	22-02-10	6-01-04	2 x 4	1-03-08	1-04-13 6-01-04	100.46 64.00		
	1	H44 Half Hip	5/12	22-02-10	7-01-04	2 x 4	1-03-08	1-00-09 7-01-04	94.33 59.00		
	1	H45 Half Hip	5/12	22-02-10	8-01-04	2 x 4	1-03-08	1-00-09 8-01-04	98.76 62.17		
	1	H46 Half Hip	5/12	22-02-10	9-01-04	2 x 4	1-03-08	1-00-09 9-01-04	103.9 64.83		
	1	H47 Half Hip	5/12	22-02-10	10-01-04	2 x 4	1-03-08	1-00-09 10-01-04	101.28 62.83		
	8	H48 Monopitch	5/12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-11	784.85 476.00		
	1	H49G GABLE	8/12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	51.66 32.83		
	3	H50 Common Structural Gable	8/12	11-10-00	8-04-13	2 x 4	1-03-08	1-04-13 7-06-02	188.01 119.50		
	6	J13 Jack-Open	4/12	6-00-00	2-08-14	2 x 4	1-03-08	7-15 2-04-15	98.04 64.00		
	6	J18 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	100.77 64.00		

TOTAL # TRUSS= 32      TOTAL BFT OF ALL TRUSSES= 1258.83      BFT.      TOTAL WEIGHT OF ALL TRSSES 2004.81      LBS

## HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

DELIVERY SHIPLIST		
 <p><b>TAMARACK</b> ROOF TRUSSES INC. <small>ALPHA LUMBER GROUP</small></p>	Lumber Yard: TAMARACK LUMBER	Job Track: 50033
	Builder: GREEN PARK HOMES	Plan Log: 201705
	Project: LAMBERT LANE PH.2	Layout ID: 405491
	Location: CALEDON	Ref #
	Model: BLOCK 82, 85, 90	Page: 2 of 2
	Lot #:	Date: 09/09/2019
	Elevation: 1A / UNITS 82-2, 82-4, 85-2, 85-4,	Designer: Andrew Conway

**HARDWARE**

QTY	TYPE	MODEL	LENGTH
TOTAL NUMBER OF ITEMS=			3

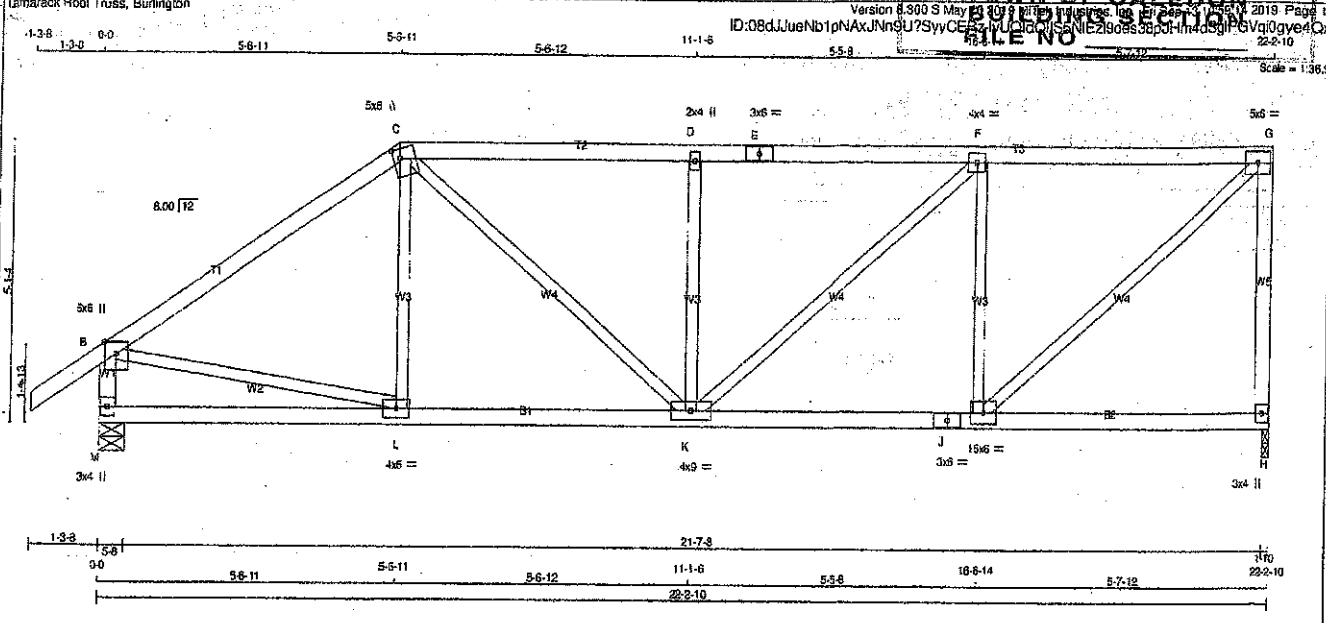
**RECEIVED**  
 SEP 16 2019  
 TOWN OF CALEDON  
 BUILDING SECTION  
 FILE NO \_\_\_\_\_



**RECEIVED**  
**SEP 16 2019**

JOB NAME: 405490 TRUSS NAME: H42 QUANTITY: 4 PLY: 1 JOB DESC: GREEN PARK HOMES TRUSS DESC:

Version 8.300.3 May 2019  
 ID:08dJjueNp1pNAXJNnsU79yCE5...  
**TOWN OF CALEDON**  
**BUILDING SECTION**  
 FILE NO: 22-2-10 Scale = 1/32



TOTAL WEIGHT = 4 X 93 = 372 lb

**LUMBER**

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DRY	No.2
A - C	2x4	DRY	No.2		
C - E	2x4	DRY	No.2		
E - G	2x4	DRY	No.2		
H - G	2x4	DRY	No.2		
M - B	2x4	DRY	No.2		
M - J	2x4	DRY	No.2		
J - H	2x4	DRY	No.2		

ALL WEBS 2x3 DRY No.2 EXCEPT  
 DRY-SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	5.0	6.0	Edge
C	TTWW+m	MT20	5.0	6.0	2.00 1.50
D	TMW+w	MT20	2.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMW-t	MT20	4.0	4.0	
G	TMW-t	MT20	5.0	6.0	
H	BMV1+p	MT20	3.0	4.0	
I	BMW-t	MT20	5.0	6.0	
J	BS-t	MT20	3.0	6.0	
K	BMW-t	MT20	4.0	6.0	
L	BMW-t	MT20	4.0	6.0	
M	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT VERT	DOWN	IN-SX	IN-SX
H	1339 0	1339 0	1-10 1-10
M	1479 0	1479 0	5-8 5-8

**UNFACTORED REACTIONS**

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT COMBINED	SNOW	LIVE PERM.LIVE WIND DEAD SOIL
H	942	844 / 0 0 / 0 0 / 0 0 / 0 280 / 0 0 / 0
M	1038	724 / 0 0 / 0 0 / 0 0 / 0 314 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.59 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. LCL (L.C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	PR-TO
A-B	0 / 89	-102.0	-102.0 0.14 (1)	10.00	L-C	-106 / 60	0.04 (1)
B-C	-1418 / 0	-102.0	-102.0 0.65 (1)	4.59	B-L	0 / 1199	0.27 (1)
C-D	-1812 / 0	-102.0	-102.0 0.48 (1)	4.82	I-G	0 / 1638	0.37 (1)
D-E	-1812 / 0	-102.0	-102.0 0.50 (1)	4.59	C-K	0 / 576	0.13 (1)
E-F	-1812 / 0	-102.0	-102.0 0.50 (1)	4.59	I-F	-948 / 0	0.36 (1)
F-G	-1247 / 0	-102.0	-102.0 0.46 (1)	5.10	K-D	-808 / 0	0.23 (1)
H-G	-1298 / 0	0.0	0.0 0.57 (1)	7.08	K-F	0 / 487	0.11 (1)
M-B	-1437 / 0	0.0	0.0 0.15 (1)	6.81			
M-L	0 / 0	-18.5	-18.5 0.14 (4)	10.00			
L-K	0 / 1178	-18.5	-18.5 0.27 (1)	10.00			
K-J	0 / 1247	-18.5	-18.5 0.27 (1)	10.00			
J-I	0 / 1247	-18.5	-18.5 0.27 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.14 (4)	10.00			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 29.0 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.4 PSF  
 TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN./C

LOADING IN PLAT SECTION BASED ON A SLOPE OF 5.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF NBC 2018, OSC 2012  
 - CSA 086-09, CSA 086-14  
 - TPIC 2011, TPIC 2014

(85% OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.06")  
 ALLOWABLE DEFL.(TL) = L/360 (0.74")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

OSI: TC=0.85/1.00 (B-C:1), SC=0.27/1.00 (I-K:1), WB=0.37/1.00 (G-I:1), SS=0.27/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LBS END=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

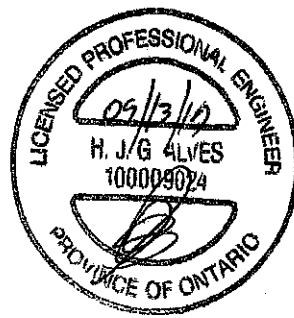
**MAIL VALUES**

PLATE GRIP (DRY) SHEAR (PSI)	SECTION (PLI)	MAX MIN MAX MIN MAX MIN
MT20	618 354 1667 788 1987 1656	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

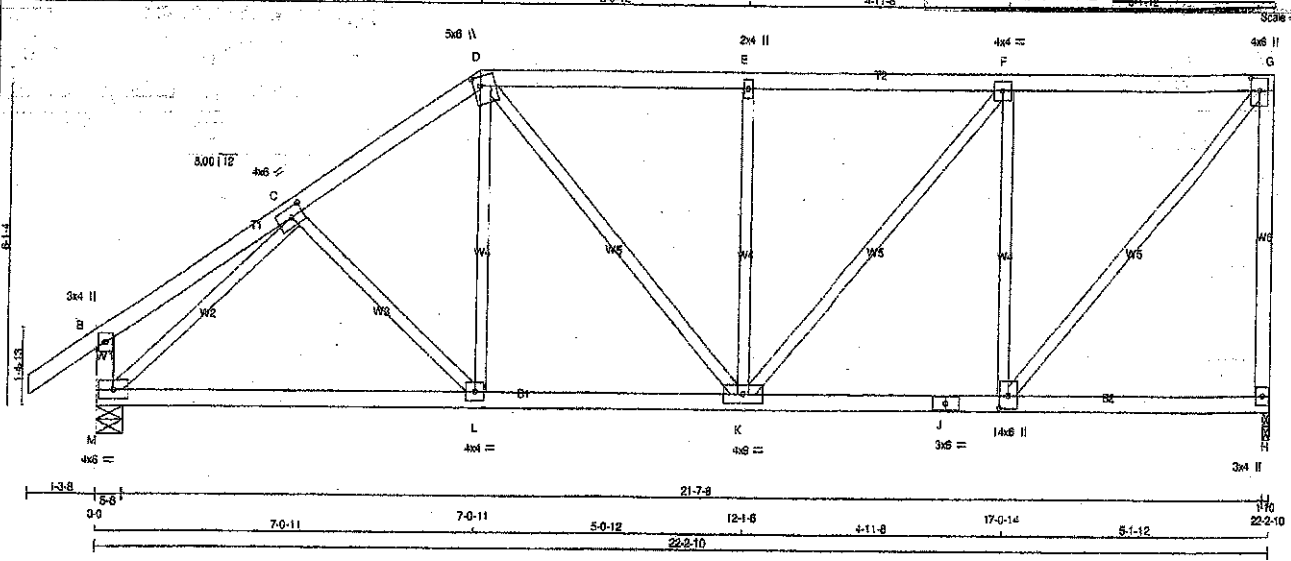
JSI GRIP = 0.89 (B) (INPUT = 0.80)  
 JSI METAL = 0.61 (B) (INPUT = 1.00)



Structural component only  
 DWG# T-1923441

**RECEIVED**  
SEP 16 2019

JOB NAME: 405490 TRUSS NAME: H43 QUANTITY: 4 PLY: 1 JOB DESC.: GREEN PARK HOMES TRUSS DESC.:  
 Tamarack Roof Truss, Burlington  
 Version 8.3005 May 10 2015 © 2015 Truss Industries, Inc. File: T-1923442.dwg Page 1 of 1  
 ID:08dJueNb1pNAxjNn9LU?SyyCERZD82oVzRWwIDENo2XW55MMVSP-REB31NOZV6ye4Qw  
 FILE NO: 22-2-10



**LUMBER**

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY, SEASONED LUMBER.

**PLATES (table in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+P	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	2.00 2.75
D	TTWW+m	MT20	5.0	6.0	2.00 1.50
E	TMWW-w	MT20	2.0	4.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMWW+P	MT20	4.0	6.0	2.75 2.00
H	BMV1+P	MT20	3.0	4.0	
I	BMWW+H	MT20	4.0	6.0	2.75 2.00
J	BS-t	MT20	3.0	6.0	
K	BMWW+H	MT20	4.0	6.0	
L	BMWW-t	MT20	4.0	4.0	
M	BMVV-t	MT20	4.0	6.0	

**BEARINGS**  
 UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1038	724 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M  
**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.13 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.  
 ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	UNBRACED LENGTH (FR-TO)
FR-TO		FROM	TO	FR-TO			
A-B	0 / 39	-102.0	-102.0	0.14 (1)	C-L	-70 / 28	0.03 (1)
B-C	0 / 22	-102.0	-102.0	0.19 (1)	L-D	0 / 185	0.35 (4)
C-D	-1375 / 0	-102.0	-102.0	0.22 (1)	M-G	-1638 / 0	0.63 (1)
D-E	-1313 / 0	-102.0	-102.0	0.38 (1)	F-G	0 / 1453	0.33 (1)
E-F	-1313 / 0	-102.0	-102.0	0.39 (1)	D-K	0 / 283	0.06 (1)
F-G	-983 / 0	-102.0	-102.0	0.38 (1)	L-F	-878 / 0	0.37 (1)
H-G	-1301 / 0	0.0	0.0	0.94 (1)	K-E	-553 / 0	0.32 (1)
M-B	-280 / 0	0.0	0.0	0.03 (1)	K-F	0 / 539	0.12 (1)
M-L	0 / 1175	-18.5	-18.5	0.30 (1)			
L-K	0 / 1127	-18.5	-18.5	0.29 (4)			
K-J	0 / 983	-18.5	-18.5	0.22 (1)			
J-I	0 / 963	-18.5	-18.5	0.22 (1)			
I-H	0 / 0	-18.5	-18.5	0.12 (4)			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 29.0 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.4 PSF  
 TOTAL LOAD = 42.4 PSF

**SPACING = 24.0 IN.C/C**

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC02010, NBC02015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF BC9C 2018, OBC 2012  
 - CSA 088-09, CSA 088-14  
 - TPC 2011, TPC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74)  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.05)  
 ALLOWABLE DEFL.(TL) = L/360 (0.74)  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.11)

CSI: TC=0.84/1.00 (G-H:1), BC=0.30/1.00 (L-M:1), WB=0.63/1.00 (C-M:1), SS=0.24/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1 10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

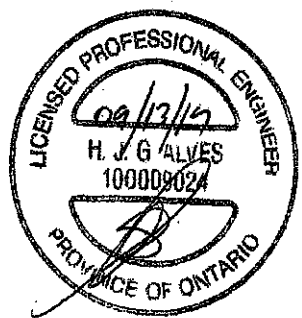
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT

**NAIL VALUES**

PLATE	GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	818	354	1687
	788	1987	1656

PLATE PLACEMENT TOL = 0.250 inches  
 PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.90 (M) (INPUT = 0.90)  
 JSI METAL = 0.43 (G) (INPUT = 1.00)



Structural component only  
 DWG# T-1923442



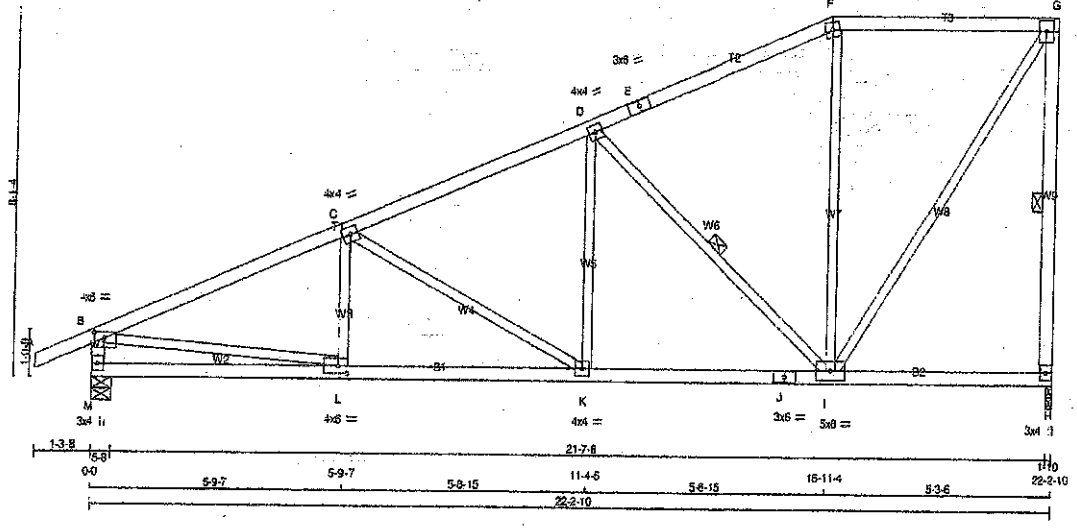


# RECEIVED

SEP 16 2019

JOB NAME <b>405490</b>	TRUSS NAME <b>H45</b>	QUANTITY <b>4</b>	PLY <b>1</b>	JOB DESC. <b>GREEN PARK HOMES</b>	TRUSS DESC.
---------------------------	--------------------------	----------------------	-----------------	--------------------------------------	-------------

Tamarack Roof Truss, Burlington  
Version 8.300 5 May 10 2010  
ID:08duJueNb1pNaxJNn9U?SyCERzAWA2r58411v0f0rZhu0v0r0ME0M0ye4Qu



TOTAL WEIGHT = 4 X 99 = 396 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF	
E - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER									
BEARINGS									
	FACTORED	GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT	RECORD		
		VERT	HORZ	DOWN	HORZ	BRG	BRG	IN-SX	IN-SX
JT		1339	0	1339	0	1-10	1-10		
M		1476	0	1476	0	5-8	5-8		

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 0.0 PSF
DL	= 7.4 PSF
TOTAL LOAD	= 42.4 PSF
SPACING = 24.0 IN. C/C	

ALL WEBS 2x3 DRY No.2  
EXCEPT  
DRY: SEASONED LUMBER.

UNFACTORED REACTIONS						
JT	1ST LOASE	MAX. MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD
H	942	844.0	0.0	0.0	0.0	298.0
M	1037	723.0	0.0	0.0	0.0	314.0

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMVW-p	MT20	4.0	6.0	1.50 2.75
C	TMVW-t	MT20	4.0	4.0	
D	TMVW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TTW-m	MT20	4.0	4.0	
G	TMVW-p	MT20	4.0	6.0	
H	BMV1-p	MT20	3.0	4.0	
I	BMV1-p	MT20	5.0	8.0	
J	BS-t	MT20	3.0	6.0	
K	BMVW-t	MT20	4.0	4.0	
L	BMVW-t	MT20	4.0	8.0	2.00 2.25
M	BMV1-p	MT20	3.0	4.0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.24 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-L.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF BCBC 2018, CBC 2012  
- CSA 086-09, CSA 086-14  
- TPC 2011, TPIC 2014

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**  
TOTAL LOAD CASES: (4)

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PL)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
A-B	0 / 28	-102.0	-102.0 0.13 (1)	10.00	L-C	-137 / 54	0.09 (1)
B-C	-2066 / 0	-102.0	-102.0 0.47 (1)	4.24	C-K	-580 / 0	0.43 (1)
C-D	-1540 / 0	-102.0	-102.0 0.37 (1)	4.80	K-D	0 / 388	0.08 (1)
D-E	-766 / 0	-102.0	-102.0 0.37 (1)	6.25	D-I	-1021 / 0	0.43 (1)
E-F	-766 / 0	-102.0	-102.0 0.37 (1)	6.25	I-F	-212 / 9	0.27 (1)
F-G	-684 / 0	-102.0	-102.0 0.37 (1)	6.25	I-G	0 / 1238	0.28 (1)
H-G	-1301 / 0	0.0	0.0 0.39 (1)	5.88	B-L	0 / 1942	0.44 (1)
M-B	-1429 / 0	0.0	0.0 0.14 (1)	6.83			
M-L	0 / 0	-18.5	-18.5 0.13 (4)	10.00			
L-K	0 / 1925	-18.5	-18.5 0.36 (1)	10.00			
K-J	0 / 1422	-18.5	-18.5 0.30 (1)	10.00			
J-I	0 / 1422	-18.5	-18.5 0.30 (1)	10.00			
I-H	0 / 0	-18.5	-18.5 0.13 (4)	10.00			

CSI: TC=0.47/1.00 (B-C:1), BC=0.36/1.00 (K-L:1), WB=0.44/1.00 (B-L:1), SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

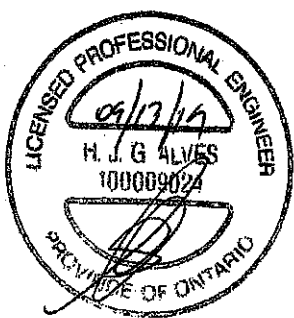
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)	SECTION (PL)
MAX	MIN	MAX	MIN	MAX
MT20	618	354	1687	788
		1987	1856	

PLATE PLACEMENT TOL. = 0.250 inches  
PLATE ROTATION TOL. = 5.0 Deg.

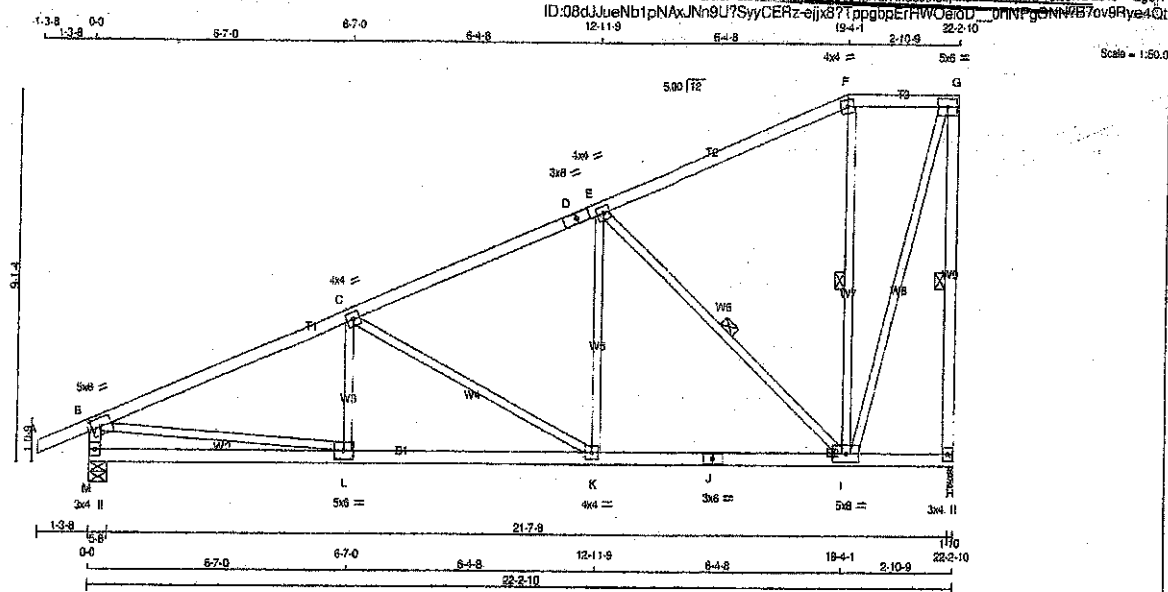
ISI GRIP = 0.89 (B) (INPUT = 0.90)  
ISI METAL = 0.48 (L) (INPUT = 1.00)



Structural component only  
DWG# T-1923444

JOB NAME 405490	TRUSS NAME H46	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES
Tamarack Roof Truss, Burlington				Version 6.000.S Max 10 2019 ID:08dJueNb1pNAxJn9U7SyyCERz-eijK371ppgpeRfW0e6D... 12-11-9 19-1 2-10-9 22-2-10

**TOWN OF CALEDON  
BUILDING SECTION  
FILE NO.**



**LUMBER**

N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
F - G	2x4	DRY	No.2	SPF	
H - G	2x4	DRY	No.2	SPF	
M - B	2x4	DRY	No.2	SPF	
M - J	2x4	DRY	No.2	SPF	
J - H	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT 1339	HORZ 0	0	0
H	VERT 1476	HORZ 0	0	0
M	VERT 1476	HORZ 0	0	0

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM	LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH. LL = 29.0 PSF  
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.0012

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBC2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMVW-1	MT20	5.0	6.0	2.50 2.75
C	TMVW-1	MT20	4.0	4.0	
D	TS-1	MT20	3.0	8.0	
E	TMVW-1	MT20	4.0	4.0	
F	TTW-m	MT20	4.0	4.9	
G	TMVW-1	MT20	5.0	8.0	
H	BMV1-p	MT20	3.0	4.0	
J	BMVW-1	MT20	5.0	8.0	
K	BS-1	MT20	3.0	6.0	
L	BMVW-1	MT20	4.0	4.0	
M	BMV1-p	MT20	5.0	8.0	
N	BMV1-p	MT20	3.0	4.0	

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.05 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	LC1	MAX	MEMB.	FORCE (LBS)	MAX	CSI (LC)
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	L-C	-89 / 82	0.03 (4)
B-C	-20 / 48 / 0	-102.0	-102.0	0.92 (1)	4.05	C-K	-751 / 0	0.78 (1)
C-D	-1359 / 0	-102.0	-102.0	0.47 (1)	4.99	K-E	0 / 485	0.11 (1)
D-E	-1359 / 0	-102.0	-102.0	0.47 (1)	4.99	E-I	-1212 / 0	0.67 (1)
E-F	-431 / 0	-102.0	-102.0	0.48 (1)	8.25	F	-252 / 0	0.14 (1)
F-G	-372 / 0	-102.0	-102.0	0.11 (1)	8.25	I-G	0 / 1293	0.28 (1)
H-G	-1325 / 0	0.0	0.0	0.51 (1)	5.62	B-L	0 / 1925	0.43 (1)
M-B	-1424 / 0	0.0	0.0	0.14 (1)	5.84			
M-L	0 / 0	-18.5	-18.5	0.17 (4)	10.00			
L-K	0 / 1913	-18.5	-18.5	0.39 (1)	10.00			
K-J	0 / 1254	-18.5	-18.5	0.29 (1)	10.00			
J-I	0 / 1254	-18.5	-18.5	0.29 (1)	10.00			
I-H	0 / 0	-18.5	-18.5	0.11 (4)	10.00			

ALLOWABLE DEFL. (LL) = L/360 (0.74")  
CALCULATED VERT. DEFL. (LL) = L/999 (0.08")  
ALLOWABLE DEFL. (TL) = L/380 (0.74")  
CALCULATED VERT. DEFL. (TL) = L/999 (0.15")

CSI: TC=0.62/1.00 (B-C-1), BC=0.39/1.00 (K-L-1),  
WB=0.78/1.00 (C-K-1), SSI=0.29/1.00 (G-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

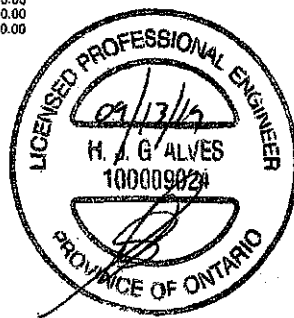
**NAIL VALUES**

PLATE	GRIP (DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	818	354	1667 768 1987 1659

PLATE PLACEMENT TOL. = 0.250 inches

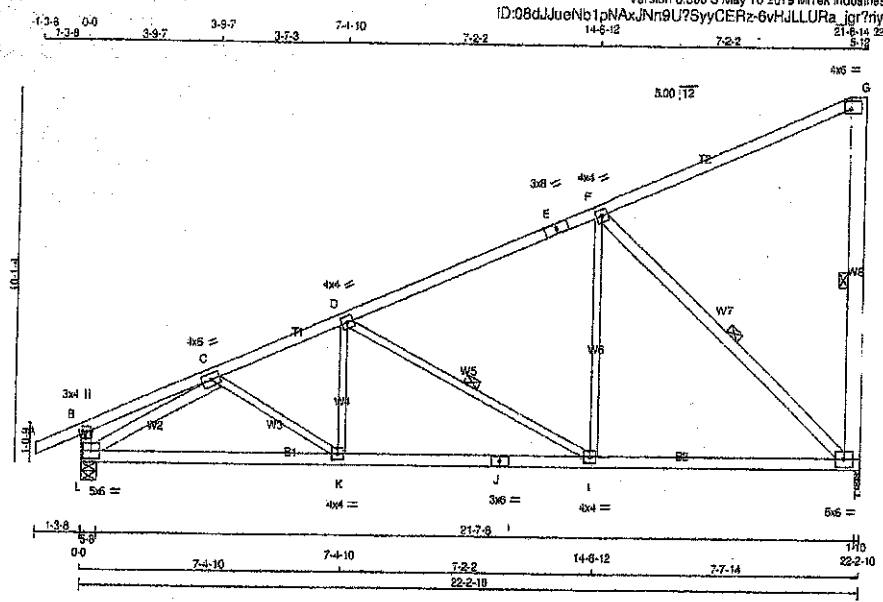
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (L) (INPUT = 6.80)  
JSI METAL = 0.55 (B) (INPUT = 1.00)



Structural component only  
DWG# T-1923445

JOB NAME 405490	TRUSS NAME H47	QUANTITY 4	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



Version 8.300 S May 10 2019 Mitek Industries, Inc. Fri Sep 13 10:59:19 2019 Page 1  
 ID:08dJueN61pNAxJNn9U7SyyCERz-6vHJLLURa\_igr?myLd1mCW9pnK7x8QnXThye4Cb  
 21-8-14 22-2-10  
 Scale = 1:50.0

TOTAL WEIGHT = 4 X 108 = 432 lb

**LUMBER**

N. L. & A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x6	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT F - H	2x3	DRY	No.2
L - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

SPF	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	IN-SX	IN-SX
H	1339	0	1-10	1-10
L	1476	0	5-8	5-8

**UNFACTORED REACTIONS**

1ST LCASE	MAX. MIN. COMBINED REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE		
H	942	844 / 0	0 / 0	298 / 0	0 / 0
L	1037	723 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

**DESIGN CRITERIA**

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF  
 DL = 6.0 PSF

BOT CH. LL = 0.0 PSF  
 DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF NBC 2018, OBC 2012  
 - CSA 088-08, CSA 086-14  
 - TRC 2011, TRC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")  
 CALCULATED VERT. DEFL.(LL) = L/989 (0.07")  
 ALLOWABLE DEFL.(TL) = L/360 (0.74")  
 CALCULATED VERT. DEFL.(TL) = L/989 (0.15")

CSI: TC=0.79/1.00 (F-G:1), BC=0.43/1.00 (I-K:1),  
 WB=0.76/1.00 (F-H:1), SS=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**PLATES (table is in inches)**

JT	TYPE	PLATES	W	LEN	Y	X
S	TMW-p	MT20	3.0	4.0		
C	TMW-W-1	MT20	4.0	6.0		
D	TMW-W-1	MT20	4.0	4.0		
E	TS-1	MT20	3.0	8.0		
F	TMW-W-1	MT20	4.0	4.0		
G	TMW-p	MT20	4.0	6.0	2.50	2.25
H	BMW-W-1	MT20	5.0	6.0		
I	BMW-W-1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMW-W-1	MT20	4.0	4.0		
L	BMW-W-1	MT20	5.0	6.0		

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.  
 MAX. UNSBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNSBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

**CHORDS**

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX.	MAX.	MEMB.	MAX. FACTORED FORCE (LBS)	MAX.
FR-TO		FROM TO				FR-TO		
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	K-D	0 / 137	0.05 (4)
B-C	0 / 8	-102.0	-102.0	0.19 (1)	10.00	D-I	-814 / 0	0.37 (1)
C-D	-1947 / 0	-102.0	-102.0	0.32 (1)	4.53	I-F	0 / 537	0.12 (1)
D-E	-1169 / 0	-102.0	-102.0	0.71 (1)	4.76	F-H	-1469 / 0	0.76 (1)
E-F	-1169 / 0	-102.0	-102.0	0.71 (1)	4.76	C-K	0 / 55	0.02 (4)
F-G	-32 / 0	-102.0	-102.0	0.73 (1)	5.25	L-C	-2121 / 0	0.48 (1)
H-G	-307 / 0	0.0	0.0	0.10 (1)	5.25			
L-B	-308 / 0	0.0	0.0	0.03 (1)	7.61			
L-K	0 / 1806	-18.5	-18.5	0.41 (1)	10.00			
K-J	0 / 1814	-18.5	-18.5	0.43 (1)	10.00			
J-I	0 / 1814	-18.5	-18.5	0.43 (1)	10.00			
I-H	0 / 1095	-18.5	-18.5	0.33 (4)	10.00			

MAIL VALUES

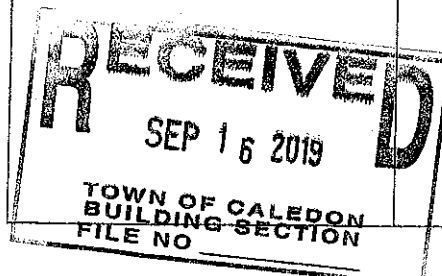
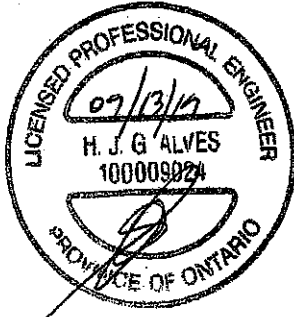
PLATE GRIP(DRY) SHEAR SECTION (PS) (PL) (PL)

MT20 619 354 1667 788 1987 1656

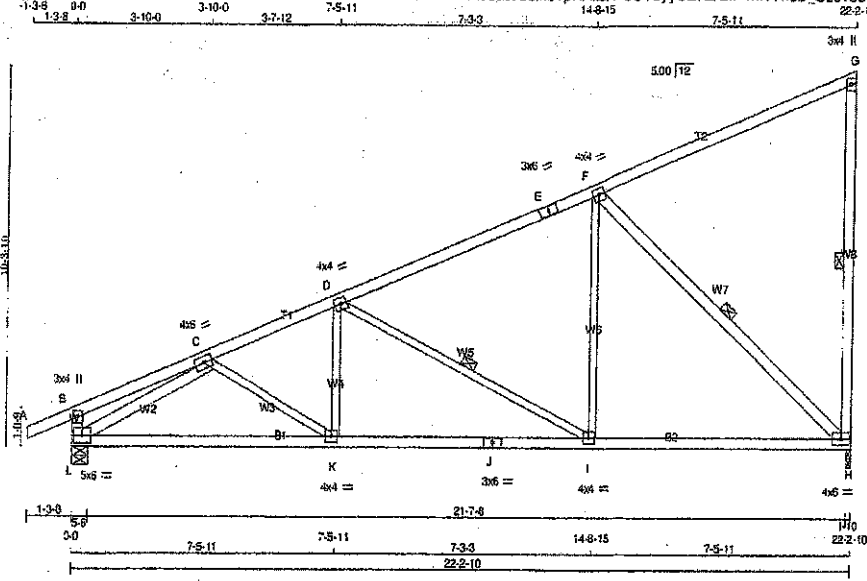
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.88 (F) (INPUT = 0.90)  
 JSI METAL = 0.56 (J) (INPUT = 1.00)



Structural component only  
 DWG# T-1923446



TOTAL WEIGHT = 32 X 100 = 3205 LB

**LUMBER**

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER
A - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
H - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT F - H	2x3	DRY	No.2
F - H	2x4	DRY	No.2
L - C	2x4	DRY	No.2

DRY: SEASONED LUMBER.

**PLATES (table in inches)**

JT	TYPE	PLATES	W	LEM	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW-1	MT20	4.0	6.0		
D	TMVW-1	MT20	4.0	4.0		
E	TS-1	MT20	3.0	8.0		
F	TMVW-1	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMVW-1	MT20	4.0	6.0		
I	BMVW-1	MT20	4.0	4.0		
J	BS-1	MT20	3.0	6.0		
K	BMVW-1	MT20	4.0	4.0		
L	BMVW-1	MT20	5.0	6.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQ'D BRG	
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
H	1339	0	1339	0	1-10	1-10		
L	1476	0	1476	0	5-8	5-8		

**UNFACTORED REACTIONS**

JT	1ST CASE		MAX./MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM.LIVE			
H	942	644/0	0/0	0/0	0/0	298/0	0/0
L	1037	723/0	0/0	0/0	0/0	314/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

**BRACING**  
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.  
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LCI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX LCI (LC)	
FR-TO		FROM	TO		FR-TO			
A-B	0/26	-102.0	-102.0	0.13 (1)	10.00	K-D	0/142	0.05 (4)
B-C	0/8	-102.0	-102.0	0.20 (1)	10.00	D-I	-841/0	0.38 (1)
C-D	-1944/0	-102.0	-102.0	0.33 (1)	4.50	F-F	0/546	0.12 (1)
D-E	-1140/0	-102.0	-102.0	0.59 (1)	4.88	F-H	-1454/0	0.76 (1)
E-F	-1140/0	-102.0	-102.0	0.59 (1)	4.88	C-K	0/54	0.02 (4)
F-G	-32/0	-102.0	-102.0	0.70 (1)	5.25	L-C	-2122/0	0.50 (1)
H-G	-298/0	0.0	0.0	0.16 (1)	6.25			
L-B	-308/0	0.0	0.0	0.03 (1)	7.61			
L-K	0/1808	-18.5	-18.5	0.42 (1)	10.00			
K-J	0/1812	-18.5	-18.5	0.43 (1)	10.00			
J-I	0/1812	-18.5	-18.5	0.43 (1)	10.00			
I-H	0/1069	-18.5	-18.5	0.32 (4)	10.00			

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
TOP CH. LL = 29.0 PSF  
DL = 8.0 PSF  
BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF  
TOTAL DL = 42.4 PSF

**SPACING = 24.0 IN. OC**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF CBC 2018, OBC 2012  
- CSA 086-08, CSA 086-14  
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.74")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.07")  
ALLOWABLE DEFL.(TL) = L/360 (0.74")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.70/1.00 (F-G:1), BC=0.43/1.00 (H-K:1), WB=0.70/1.00 (F-H:1), SSI=0.33/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY  
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

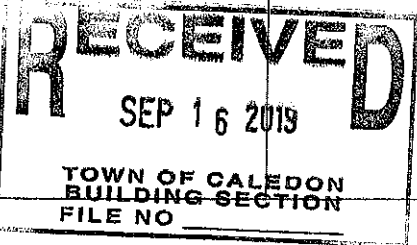
**NAIL VALUES**

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	818	354
	1667	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

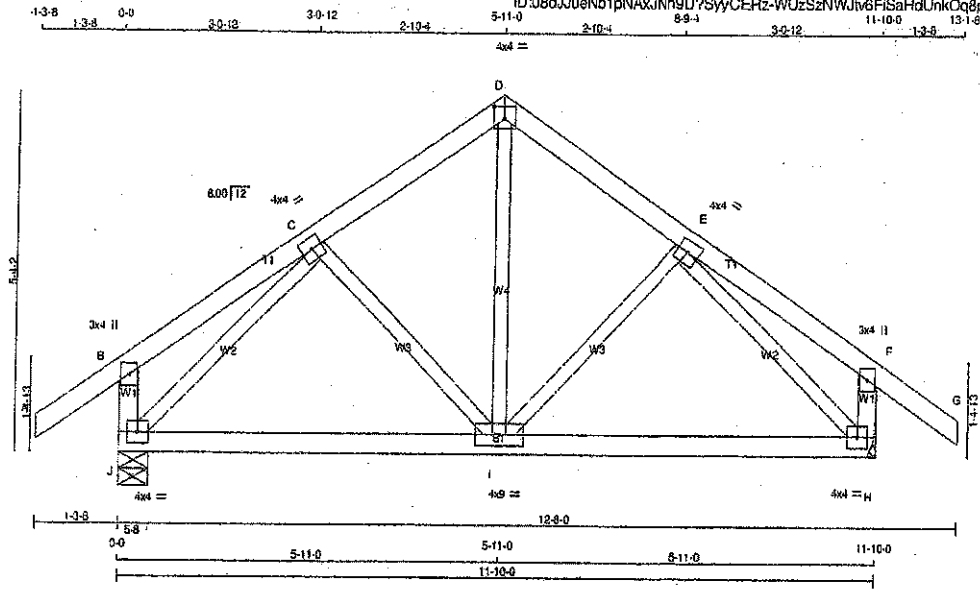
JSI GRIP = 0.88 (F) (INPUT = 0.90)  
JSI METAL = 0.57 (J) (INPUT = 1.00)



Structural component only  
DWG# T-1923447

JOB NAME 405490	TRUSS NAME H49-Cond1	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.300 S May 10 2019 M/Tek Industries, Inc. Fri Sep 13 10:59:22 2019 Page 1  
 ID:08dJlueNbtPNAxJInn9U7SyYcERz-WUzSzNWJt6FISaHdUnkOq8pG\_pHK0b6km7ICye4Qp  
 11-10-0 13-1-8 Scale = 1:30.3



TOTAL WEIGHT = 6 X 52 = 314 lb (M/F)

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF		
D - G	2x4	DRY	No.2	SPF		
J - B	2x4	DRY	No.2	SPF		
H - F	2x4	DRY	No.2	SPF		
J - H	2x4	DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF		

DRY: SEASONED LUMBER.

**BEARINGS**

JT	FACTORED		MAXIMUM FACTORED		INPUT BRG	RECORD BRG
	VERT	HORZ	GROSS	DOWN		
J	853	0	853	0	5-8	5-8
H	853	0	853	0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H, MINIMUM BEARING LENGTH AT JOINT H = 1-6.

**DESIGN CRITERIA**

SPECIFIED LOADS:

TOP CH. LL	= 29.0	PSF
DL	= 6.0	PSF
BOT CH. LL	= 0.0	PSF
DL	= 7.4	PSF
TOTAL LOAD	= 42.4	PSF

SPACING = 24.0 IN. C/C

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-1	MT20	4.0	4.0	
D	TTW-p	MT20	4.0	4.0	2.25 2.00
E	TMWW-1	MT20	4.0	4.0	
F	TMV+p	MT20	3.0	4.0	
H	BMWW-1	MT20	4.0	4.0	
I	BMWW-1	MT20	4.0	9.0	
J	BMWW-1	MT20	4.0	4.0	

**UNFACTORED REACTIONS**

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
J	598	423 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0
H	598	423 / 0	0 / 0	0 / 0	0 / 0	176 / 0	0 / 0

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2010, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.39")

CALCULATED VERT. DEFL. (LL) = L/999 (0.01")

ALLOWABLE DEFL. (TL) = L/360 (0.39")

CALCULATED VERT. DEFL. (TL) = L/999 (0.04")

**LOADING**

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MAX. FACTORED FORCE (LBS)
FR-TO				
A-B	0 / 39	-102.0	10.00	I-D 0 / 342
B-C	0 / 18	-102.0	10.00	I-E -145 / 0
C-D	-540 / 0	-102.0	6.25	C-I -145 / 0
D-E	-540 / 0	-102.0	6.25	J-C -770 / 0
E-F	0 / 18	-102.0	10.00	E-H -770 / 0
F-G	0 / 39	-102.0	10.00	
J-B	-257 / 0	0.0	7.81	
H-F	-257 / 0	0.0	7.81	
I-I	0 / 536	-18.5	10.00	
I-H	0 / 536	-18.5	10.00	

CSI: TC=0.147 (0.0) (F-G:1), BC=0.221 (0) (I-J:4), WB=0.221 (0) (C-J:1), SSI=0.121 (0) (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

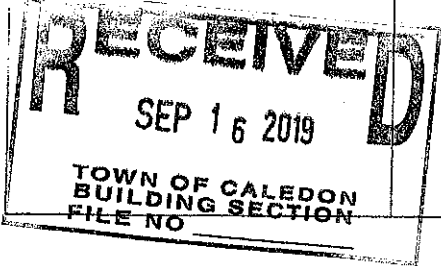
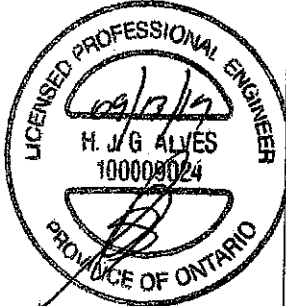
PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MAX MIN	MAX MIN	MAX MIN
MT20	616 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.70 (E) (INPUT = 0.90)

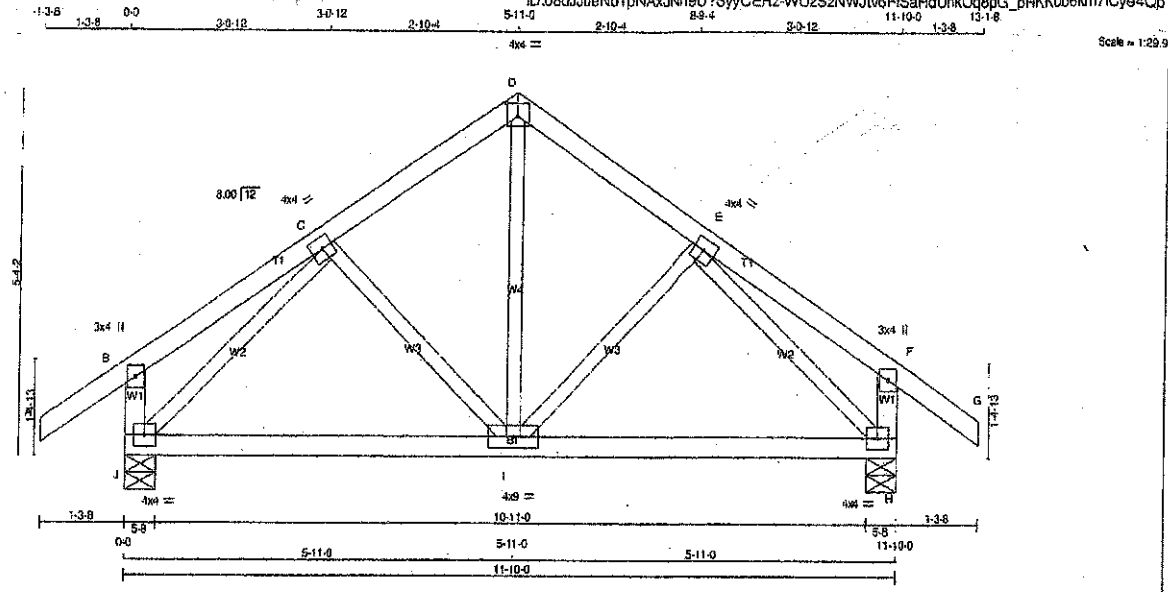
JSI METAL = 0.27 (E) (INPUT = 1.00)



Structural component only  
 DWG# T-1923448

JOB NAME 405490	TRUSS NAME H49-Cond2	QUANTITY 6	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.300 S May 10 2019 Milltek Industries, Inc. Fri Sep 13 10:59:22 2019 Page 1  
 ID:08dJjueNbtPNAxJNn9U?SyyCERz-WUzS2NWJw6RISaHdUnkCq9pG3\_pHKk0b6km71Cye4Qp



Scale = 1:29.9

TOTAL WEIGHT = 6 X 52 = 314 lb

**LUMBER**  
 N. L. G. A. RULES  
 CHORDS SIZE LUMBER DESCR.  
 A - D 2x4 DRY No.2 SPF  
 D - G 2x4 DRY No.2 SPF  
 J - B 2x4 DRY No.2 SPF  
 H - F 2x4 DRY No.2 SPF  
 J - H 2x4 DRY No.2 SPF  
 ALL WEBS 2x3 DRY No.2 SPF  
 EXCEPT  
 DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-t	MT20	4.0	4.0		
D TTW-p	MT20	4.0	4.0	2.25	2.00
E TMVW-t	MT20	4.0	4.0		
F TMV+p	MT20	3.0	4.0		
H BMVW-t	MT20	4.0	4.0		
I BMVW-t	MT20	4.0	9.0		
J BMVW-t	MT20	4.0	4.0		

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT VERT	853	853	5-8	5-8
JT HORZ	0	0		
JT DOWN	853	853		
JT UP	0	0		
H VERT	853	853	5-8	5-8
H HORZ	0	0		
H DOWN	853	853		
H UP	0	0		

**UNFACTORED REACTIONS**

JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

**BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H**

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNFRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS				WEBS			
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNI-RAC LENGTH (FT)	MAX. UNI-RAC CS (L/C)	MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. UNI-RAC LENGTH (FT)	MAX. UNI-RAC CS (L/C)
A-B	0 / 39	-102.0	10.00	0.14 (1)	I-D	0 / 342	10.00	0.08 (1)
B-C	0 / 18	-102.0	10.00	0.14 (1)	I-E	-145 / 0	10.00	0.04 (1)
C-D	-540 / 0	-102.0	8.25	0.11 (1)	C-I	-145 / 0	8.25	0.04 (1)
D-E	-540 / 0	-102.0	8.25	0.11 (1)	J-C	-770 / 0	8.25	0.22 (1)
E-F	0 / 18	-102.0	10.00	0.14 (1)	E-H	-770 / 0	10.00	0.22 (1)
F-G	0 / 39	-102.0	10.00	0.14 (1)				
J-B	-257 / 0	0.0	7.81	0.03 (1)				
H-F	-257 / 0	0.0	7.81	0.03 (1)				
J-I	0 / 538	-18.5	10.00	0.22 (4)				
I-H	0 / 538	-18.5	10.00	0.22 (4)				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 20.0 PSF  
 DL = 8.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.4 PSF  
 TOTAL LOAD = 42.4 PSF

**SPACING = 24.0 IN. C/C**

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC 2018, OBC 2012  
 - CSA 086-09, CSA 089-14  
 - TPIC 2011, TPIC 2014

(95 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")  
 CALCULATED VERT. DEFL.(LL) = L/999 (0.01")  
 ALLOWABLE DEFL.(TL) = L/360 (0.39")  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.04")

CSI: TC=0.14/1.00 (F-G:1), BC=0.22/1.00 (I-J:4), WB=0.23/1.00 (C-J:1), SI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

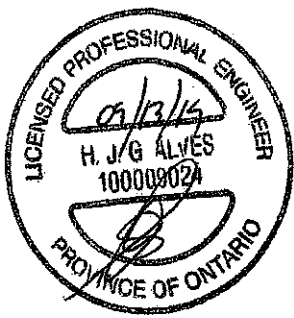
COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**  
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)  
 MAX MIN MAX MIN MAX MIN  
 MT20 618 354 1667 788 1987 1856

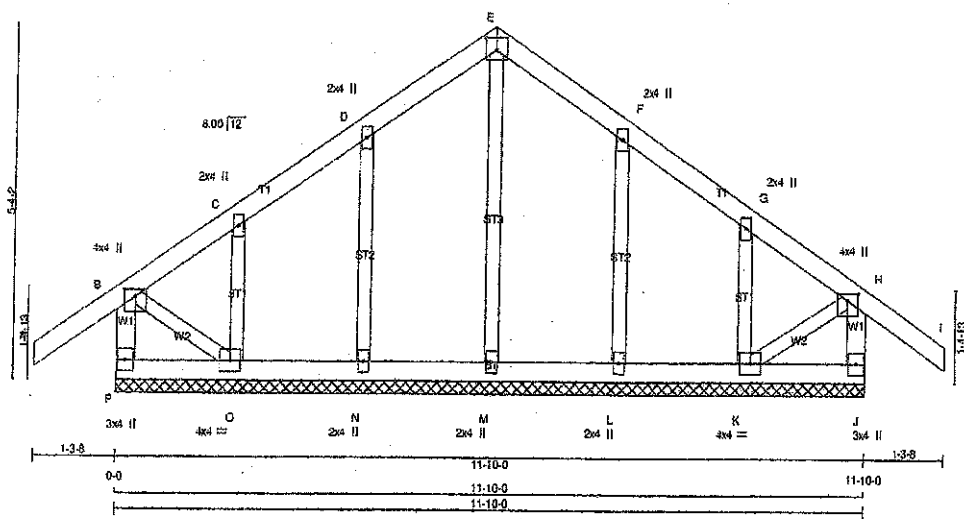
PLATE PLACEMENT TOL. = 0.250 inches  
 PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.70 (E) / INPUT = 0.90  
 JSI METAL = 0.27 (E) / INPUT = 1.00



**RECEIVED**  
 SEP 16 2019  
 TOWN OF CALEDON  
 BUILDING SECTION  
 FILE NO \_\_\_\_\_

Structural component only  
 DWG# T-1923449



**LUMBER**

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
P - B	2x4	DRY	No.2
A - E	2x4	DRY	No.2
E - I	2x4	DRY	No.2
J - H	2x4	DRY	No.2
P - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

**PLATES (table is in inches)**

PLATE TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.25 2.00
C, D, F, G					
C	TMW+w	MT20	2.0	4.0	
E	TTW+p	MT20	4.0	4.0	2.25 2.00
H	TMW+p	MT20	4.0	4.0	1.25 2.00
J	BMW1+p	MT20	3.0	4.0	
K	BMW1-l	MT20	4.0	4.0	
L, M, N					
L	BMW1+w	MT20	2.0	4.0	
O	BMW1-l	MT20	4.0	4.0	
P	BMW1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

**BEARINGS**

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

**LOADING**

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	CHORDS		WEBS	
		VERT. LOAD (PLF)	CS1 (LC)	LENGTH FR-TO	MAX. FACTORED FORCE (LBS)
FR-TO		FROM	TO		
P-B	-301 / 0	0.0	0.0	0.03 (1)	7.81
A-B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B-C	-47 / 0	-102.0	-102.0	0.13 (1)	6.25
C-D	-7 / 0	-102.0	-102.0	0.08 (1)	10.00
D-E	-23 / 0	-102.0	-102.0	0.06 (1)	6.25
E-F	-23 / 0	-102.0	-102.0	0.06 (1)	6.25
F-G	-7 / 0	-102.0	-102.0	0.08 (1)	10.00
G-H	-47 / 0	-102.0	-102.0	0.13 (1)	6.25
H-I	0 / 39	-102.0	-102.0	0.14 (1)	10.00
J-H	-301 / 0	0.0	0.0	0.03 (1)	7.81
P-O	0 / 0	-18.5	-18.5	0.02 (4)	10.00
O-N	0 / 14	-18.5	-18.5	0.02 (4)	10.00
N-M	0 / 8	-18.5	-18.5	0.02 (4)	10.00
M-L	3 / 8	-18.5	-18.5	0.02 (4)	10.00
L-K	0 / 14	-18.5	-18.5	0.02 (4)	10.00
K-J	0 / 0	-18.5	-18.5	0.02 (4)	10.00

TOTAL WEIGHT = 4 X 52 = 207 LB

**DESIGN CRITERIA**

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 8.0 PSF

BOT CH. LL = 0.0 PSF

DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2010, NBCG 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

**DESIGN ASSUMPTIONS**

OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55% OF 37.6 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.14/1.00 (H-1.1), BC=0.02/1.00 (N-C-4), WB=0.06/1.00 (E-M-1), SS1=0.09/1.00 (H-1.1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

**NAIL VALUES**

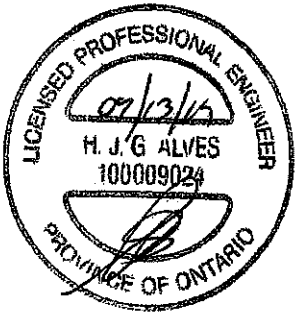
PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354 1667 788 1887 1858

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.21 (H) (INPUT = 0.80)

JSI METAL = 0.12 (F) (INPUT = 1.00)



**RECEIVED**

SEP 16 2019

TOWN OF CALEDON BUILDING SECTION

FILE NO \_\_\_\_\_

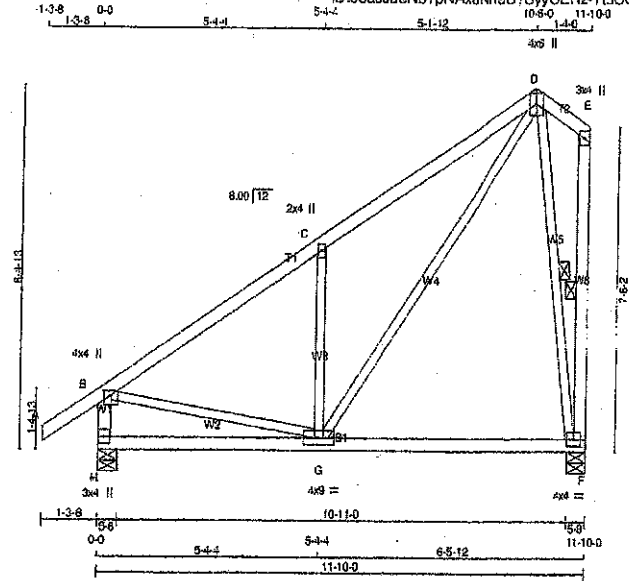
Structural component only

DWG# T-1923450





Tamarack Roof Truss, Burlington ID:08dJueNbpNAxJNnBU7SyyCERz-T15CO2YaPWWzYmKgiVpCTFE5FoWEoCkuZ2FEM4ye4Qn 10-9-0 11-10-0 Scale = 1:47.1



TOTAL WEIGHT = 6 X 63 = 378 lb

**LUMBER**

N.L. & A. RULES

CHORDS	SIZE	DRY	No.2	DESCR.	SPF
A - D	2x4	DRY	No.2	SPF	
D - E	2x4	DRY	No.2	SPF	
H - B	2x4	DRY	No.2	SPF	
F - E	2x4	DRY	No.2	SPF	
H - F	2x4	DRY	No.2	SPF	

ALL WEBS 2x3 DRY No.2 SPF

EXCEPT

DRY: SEASONED LUMBER.

**PLATES (table in inches)**

JT	TYPE	PLATES	W	LEN.	Y	X
B	TMVW+p	MT20	4.0	4.0	1.25	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTWV+p	MT20	4.0	6.0	Edge	
E	TMV+p	MT20	3.0	4.0		
F	BMVW1-t	MT20	4.0	4.0		
G	BMVWW1-t	MT20	4.0	4.0		
H	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
H	853	0	883	0	0	5-8	5-8
F	713	0	713	0	0	5-8	5-8

**UNFACTORED REACTIONS**

JT	1ST CASE	2ND CASE	3RD CASE	4TH CASE	5TH CASE	6TH CASE	7TH CASE	8TH CASE
H	598	423.0	0.0	0.0	0.0	175.0	0.0	0.0
F	502	343.0	0.0	0.0	0.0	159.0	0.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

**BRACING**

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

**LOADING**

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH	MEMB.
A-B	0.39	-102.0	10.00	G-C	-851.0	6.25	G-D
B-C	-817.0	-102.0	6.25	B-G	0.554	6.25	D-F
C-D	-964.0	-102.0	6.25	D-F	-604.0	6.25	G-D
D-E	0.0	-102.0	10.00	G-D	0.797	6.25	
H-B	-816.0	0.0	7.81				
F-E	-88.0	0.0	6.25				
H-G	0.0	-18.5	10.00				
G-F	0.98	-18.5	10.00				

**DESIGN CRITERIA**

**SPECIFIED LOADS:**

TOP CH. LL = 29.0 PSF  
DL = 6.0 PSF

BOT CH. LL = 0.0 PSF  
DL = 7.4 PSF

TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:  
- PART 9 OF SBC 2018, OBC 2012  
- CSA 088-09, CSA 088-14  
- TRC 2011, TRC 2014

(5% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.39")  
CALCULATED VERT. DEFL.(LL) = L/999 (0.02")  
ALLOWABLE DEFL.(TL) = L/360 (0.39")  
CALCULATED VERT. DEFL.(TL) = L/999 (0.06")

CSI: TC=0.38(1.00 (C-D)), BC=0.19(1.00 (F-G)), WB=0.27(1.00 (D-F)), SSI=0.22(1.00 (B-C:1))

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

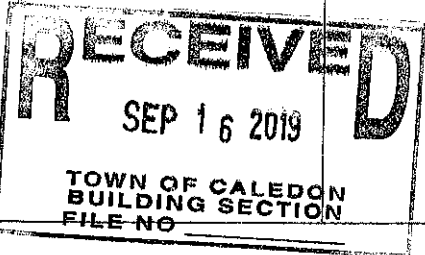
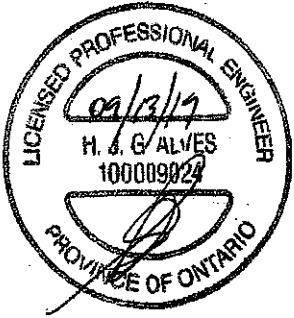
**NAIL VALUES**

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)	(PL)
MT20	618	354	1867 788 1957 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

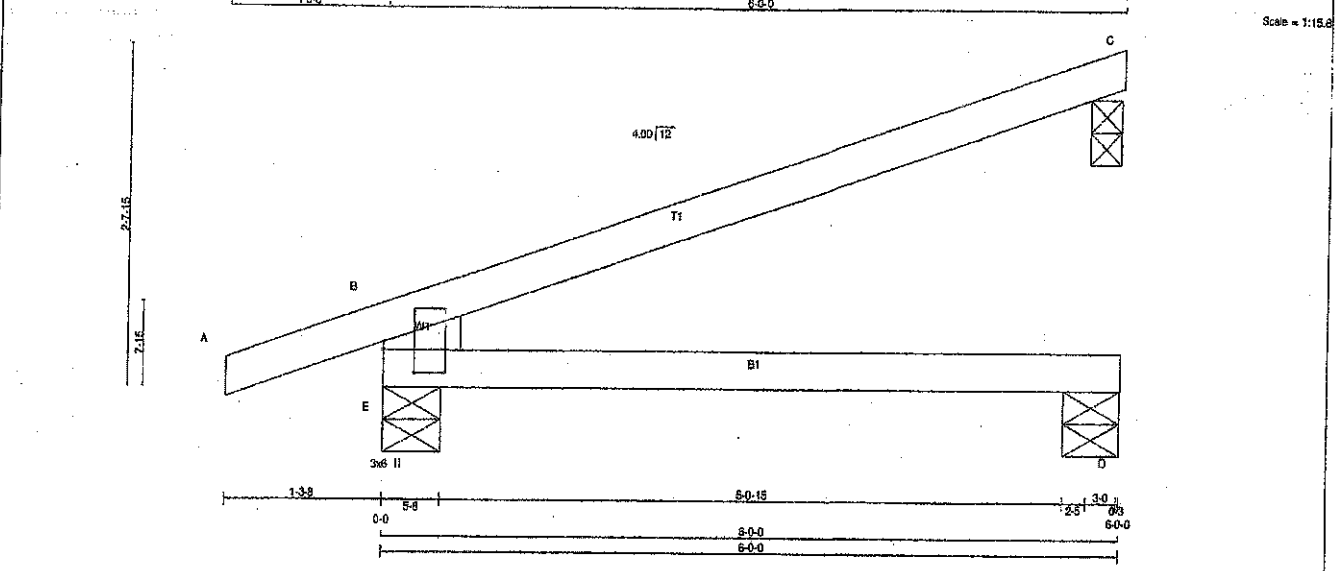
JSI GRIP = 0.78 (G) (INPUT = 0.90)  
JSI METAL = 0.33 (C) (INPUT = 1.00)



Structural component only  
DWG# T-1923452

JOB NAME 405490	TRUSS NAME J13	QUANTITY 24	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington Version 8.360 S May 10 2019 M/Tek Industries, Inc. Fri Sep 13 10:59:25 2019 Page 1  
 ID:08dJjueNb1pNAXJNn9U?SyyCERz-x3facOYC9qUqZwJslckR?TmCsBsSXID1oi\_nvXye4Cm



**LUMBER**

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x8	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

**PLATES (table is in inches)**

JT TYPE	PLATES	W	LEN	Y	X	
E	TMBM1+p	MT20	3.0	6.0	2.25	2.75

**DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER**

**BEARINGS**

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
E	588	588	5-8	5-8
C	230	230	3-0	3-0
D	42	47	5-8	5-8

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

**UNFACTORED REACTIONS**

JT	1ST LC CASE	MAX./MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
E	COMBINED	411	255 / 0	0 / 0	0 / 0	116 / 0
C		158	131 / 0	0 / 0	0 / 0	27 / 0
D		34	0 / 0	0 / 0	0 / 0	34 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C, D

**BRACING**  
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.  
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

**LOADING**  
 TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS	
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (L)	MAX. UNBRAC. LENGTH	MAX. FACTORED FORCE (LBS)
E-B	-519 / 0	0.0	0.0	0.06 (4)
A-B	0 / 22	-102.0	-102.0	0.13 (1)
B-C	-24 / 0	-102.0	-102.0	0.83 (1)
E-D	0 / 0	-18.5	-18.5	0.19 (4)

**DESIGN CRITERIA**

**SPECIFIED LOADS:**  
 TOP CH. LL = 29.0 PSF  
 DL = 6.0 PSF  
 BOT CH. LL = 0.0 PSF  
 DL = 7.4 PSF  
 TOTAL LOAD = 42.4 PSF

SPACING = 24.0 IN./C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:  
 - PART 9 OF CBC 2018, OBC 2012  
 - CSA 088-09, CSA 086-14  
 - TPIC 2011, TPIC 2014

**DESIGN ASSUMPTIONS**  
 - OVERHANG NOT TO BE ALTERED OR CUT OFF.

65% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL) = L/360 (0.20')  
 CALCULATED VERT. DEFL.(TL) = L/999 (0.02')

CSI: TC=0.63/1.00 (B-C:1), BC=0.19/1.00 (D-E:4),  
 WB=0.00/1.00 (n/a:0), SS=0.28/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10  
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

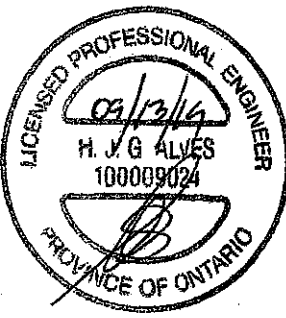
**NAIL VALUES**

PLATE GRIP (DRY) (PSI)	SHEAR (PLI)	SECTION (PLI)
MT20	818	354
	1857	788
	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.28 (E) (INPUT = 0.99)  
 JSI METAL = 0.10 (E) (INPUT = 1.00)



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 BUILDING SECTION  
 FILE NO

Structural component only  
 DWG# T-1923453





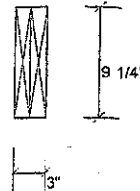
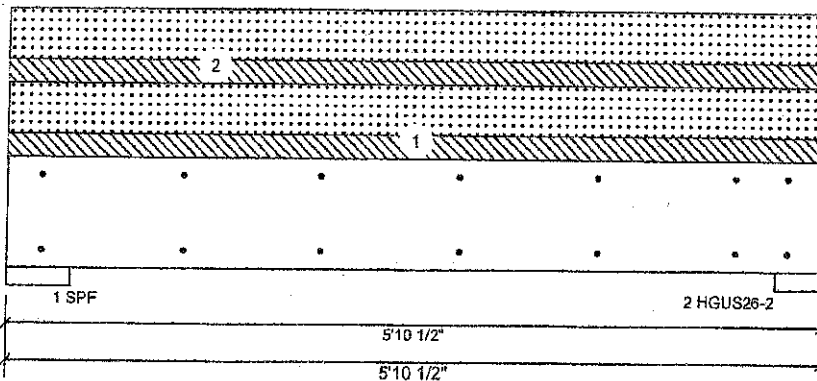
isDesign™

Client:  
Project:  
Address:

Date: 2/2019  
Designer:  
Job Name: 405481  
Project#:

### B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



#### Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

#### Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	278	602	0
2	0	266	577	0

#### Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	348 / 903	1250 L 1.25D+1.5S
2 - HGUS...	4.000"	16%	333 / 885	1198 L 1.25D+1.5S

#### Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1413 ft-lb	3'	6039 ft-lb	0.234 (23%)	1.25D+1.5S	L
Unbraced	1413 ft-lb	3'	5236 ft-lb	0.270 (27%)	1.25D+1.5S	L
Shear	1085 lb	1'2"	3984 lb	0.272 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5129)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3508)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

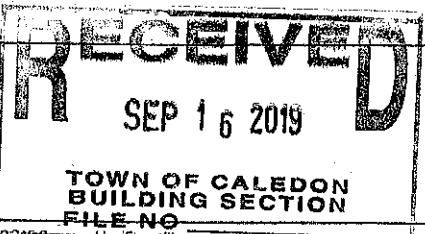
#### Design Notes

- 1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



DWG NO. TAM 19235 6A  
STRUCTURAL  
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	



Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115
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This design is valid until 12/1/2021



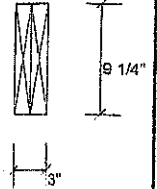
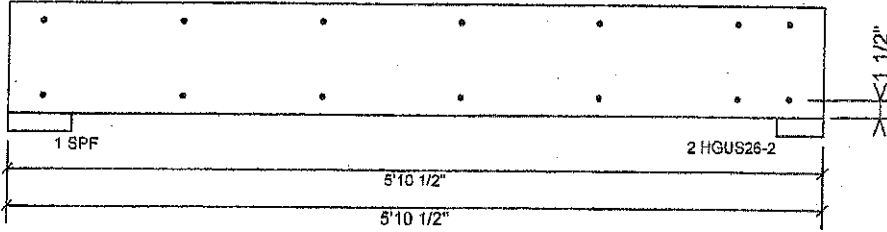
isDesign™

Client:  
Project:  
Address:

Date: 2/2019  
Designer:  
Job Name: 405481  
Project #:

**B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED**

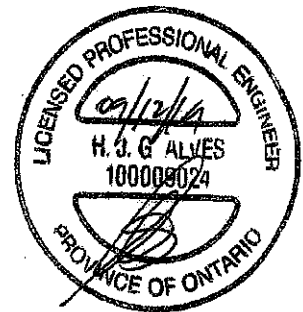
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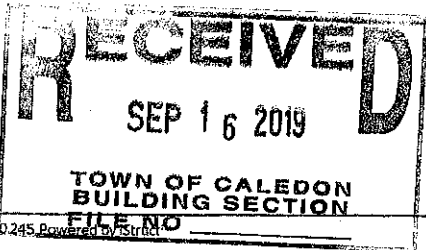
**Multi-Ply Analysis**

Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	78.6 %
Load	178.2 PLF
Yield Limit per Foot	226.7 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00



DWG NO. TAM R923564  
STRUCTURAL  
COMPONENT ONLY 3/2



This design is valid until 12/11/2021

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115
 ALTA LUMBER GROUP	



# LUL/LUS/LJS/HUS/HHUS/HGUS

## Standard and Double-Shear Joist Hangers



This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing – an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

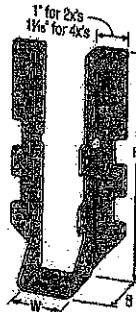
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

**Installation:**

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

**Options:**

- LJS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.



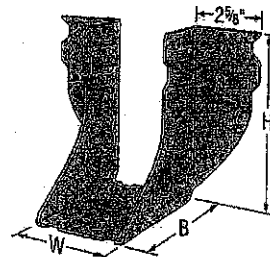
✓ LUS28



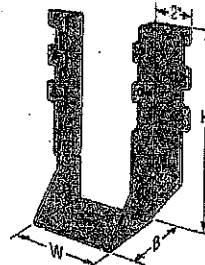
LU26L



✓ HUS210  
(HUS26, HUS28, and HHUS similar)



✓ HGUS28-2



✓ HHUS210-2



Double-Shear Nailing Top View

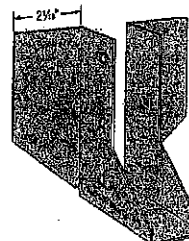
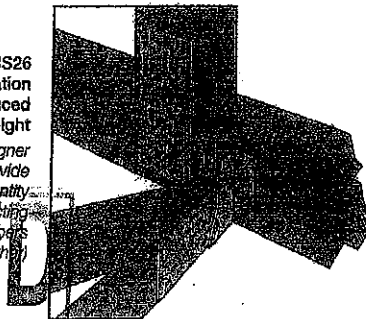


Double-Shear Nailing Side View; Do not bend tab



Dome Double-Shear Nailing Side View (available on some models) U.S. Patent 5,603,680

Typical HUS26 Installation with Reduced Heel Height (Truss Designer to provide fastener quantity for attaching multiple trusses together)



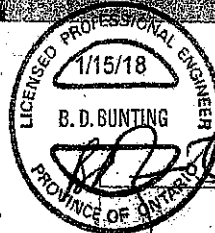
LJS26DS

Plated Truss Connectors

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# LUL/LUS/LJS/HUS/HHUS/HGUS

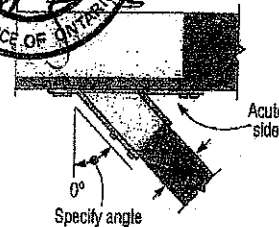


## HHUS/HGUS

See Hanger Options Information on pp. 125–127.

### HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.82 of the table value
- The joist must be bevel-cut to allow for double-shear nailing



Top View HHUS Hanger Skewed Right  
(joist must be bevel cut)  
All joist nails installed on the outside angle (non-acute side).

### HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value

## Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Plated Truss Connectors

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d <sub>e</sub> <sup>5</sup>	Header	Joist	S-P-F		Normal	
								Uplift (K <sub>6</sub> = 1.5)	Down (K <sub>6</sub> = 1.0)		Uplift (K <sub>6</sub> = 1.15)
		lb.		lb.		lb.		lb.			
		kN		kN		kN		kN			
Single 2x Sizes											
SS LUS24	18	1 1/8	3 1/4	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1165
								360	1020	320	725
SS LU24L	22	1 1/8	3	1 1/4	2 1/4	(4) 10d	(2) 10d x 1 1/2"	360	1020	320	725
								360	1020	320	725
SS LU28L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	720	1605	645	1140
								720	1605	645	1140
SS LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	1420	2170	1290	1630
								1420	2170	1290	1630
SS HUS26	16	1 1/8	5 1/4	3	3 1/4	(14) 16d	(6) 16d	2705	4940	2055	3875
								2705	4940	2055	3875
SS LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	2055	4285	1480	4115
								2055	4285	1480	4115
SS HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	2685	6625	2685	5700
								2685	6625	2685	5700
SS LU28L	20	1 1/8	6 1/4	1 1/4	5 1/4	(8) 10d	(6) 10d x 1 1/2"	1140	2185	1020	1550
								1140	2185	1020	1550
SS LUS28	18	1 1/8	6 1/4	1 1/4	3 1/4	(6) 10d	(4) 10d	1420	2520	1290	1790
								1420	2520	1290	1790
SS HUS28	16	1 1/8	7 1/4	3	6 1/4	(22) 16d	(8) 16d	3605	5365	2675	4945
								3605	5365	2675	4945
SS HGUS28	12	1 1/8	7 1/4	5	6 1/4	(36) 16d	(12) 16d	3310	7875	3310	6900
								3310	7875	3310	6900
SS LU240L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	1140	2495	1020	1770
								1140	2495	1020	1770
SS LUS210	18	1 1/8	7 1/4	1 1/4	3 1/4	(8) 10d	(4) 10d	1420	2785	1290	2210
								1420	2785	1290	2210

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d<sub>e</sub> is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin TR-100, "RESISTANCE AND/OR SEE INSTALLATION NOTES."
5. Nails: 16d #8, 2" x 3/4" long. See pp. 27–28 for other nail sizes and information.

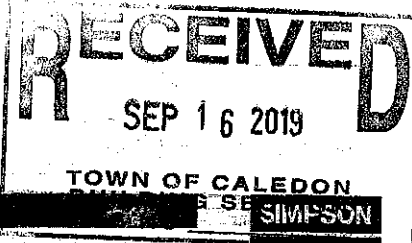
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Simpson Strong-Tie® Wood Construction Connectors - Canadian Limit States Design

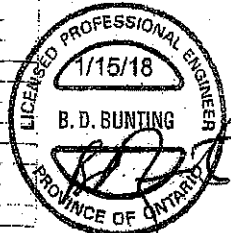
# Face-Mount Hangers

Strong-Tie

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32-34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance				
		W	H	B	d <sub>g</sub>	Header	Joist	U-P-L		S-P-F		
								Uplift (K <sub>u</sub> = 1.15)	Normal (K <sub>n</sub> = 1.00)	Uplift (K <sub>u</sub> = 1.15)	Normal (K <sub>n</sub> = 1.00)	
lb.	lb.	lb.	lb.	kN	kN	kN	kN					
<b>Double 2x Sizes</b>												
LUS24-2	18	3 1/8	3 1/8	2	1 1/2	(4) 16d	(2) 16d	835	2020	890	1435	
SS	LUS26-2	18	3 3/8	4 3/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
SS	HHUS26-2	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2850	7335	2065	5205
SS	HGUS26-2	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
SS	LUS28-2	18	3 3/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2675
SS	HHUS28-2	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3765	8940	2675	6345
SS	HGUS28-2	12	3 3/8	7 3/8	4	8 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
SS	LUS210-2	18	3 3/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
SS	HHUS210-2	14	3 3/8	9 3/8	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
SS	HGUS210-2	12	3 3/8	9 3/8	4	8 3/8	(48) 16d	(16) 16d	6840	14015	4855	10270
<b>Triple 2x Sizes</b>												
SS	HGUS28-3	12	4 1/8	5 1/2	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
SS	HGUS28-3	12	4 3/8	7 1/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
SS	HHUS210-3	14	4 1/8	9	3	7 1/4	(30) 16d	(10) 16d	4670	9670	4235	8865
SS	HGUS210-3	12	4 1/8	9 1/4	4	8 3/8	(48) 16d	(16) 16d	6840	14645	4855	10400
<b>Quadruple 2x Sizes</b>												
SS	HGUS28-4	12	6 1/8	8 3/8	4	4 1/4	(20) 16d	(8) 16d	4385	8950	3110	6355
SS	HGUS28-4	12	6 3/8	7 3/4	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
SS	HHUS210-4	14	6 1/8	8 3/8	3	7 3/8	(30) 16d	(10) 16d	4670	10155	4235	7210
SS	HGUS210-4	12	6 3/8	9 3/8	4	8 3/8	(48) 16d	(16) 16d	6840	14645	4855	10400
SS	HGUS212-4	12	8 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
SS	HGUS214-4	12	8 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645
<b>4x Sizes</b>												
SS	LUS46	18	3 3/8	4 3/8	2	3 3/8	(4) 16d	(4) 16d	1720	2595	1545	1920
SS	HHUS46	14	3 3/8	5 3/8	3	3 3/8	(14) 16d	(6) 16d	2540	7335	2065	5205
SS	HGUS46	12	3 3/8	5 3/8	4	4 3/8	(20) 16d	(8) 16d	4385	8950	3110	6355
SS	LUS48	18	3 3/8	6 3/8	2	3 3/8	(6) 16d	(4) 16d	1720	3325	1545	2675
SS	HHUS48	14	3 3/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3765	8940	2675	6345
SS	HGUS48	12	3 3/8	7 3/8	4	6 3/8	(36) 16d	(12) 16d	6070	12980	4310	9215
SS	LUS410	18	3 3/8	8 3/8	2	5 3/8	(8) 16d	(6) 16d	2580	4500	2320	3195
SS	HGUS410	12	3 3/8	9	4	8 3/8	(48) 16d	(16) 16d	6840	14015	4855	10270
SS	HGUS412	12	3 3/8	10 3/8	4	10 3/8	(56) 16d	(20) 16d	7640	14995	5425	10645
SS	HGUS414	12	3 3/8	12 3/8	4	11 3/8	(66) 16d	(22) 16d	10130	16400	7195	11645



Plated Truss Connectors

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See footnotes on p. 258.

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TOWN OF CALEDON  
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FILE NO.

**LUMBER SPECIFICATION**

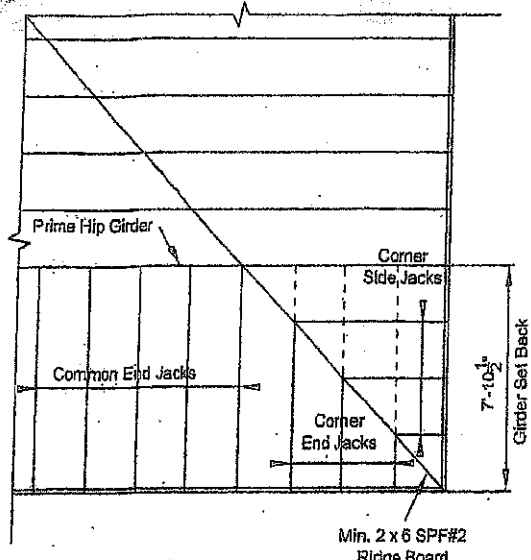
TOP CHORD : 2 x 4 SPF#2  
 BOTTOM CHORD : 2 x 4 SPF#2  
 WEBS : 2 x 3 SPF#2  
 UNLESS OTHERWISE SHOWN

**DESIGN LOAD**

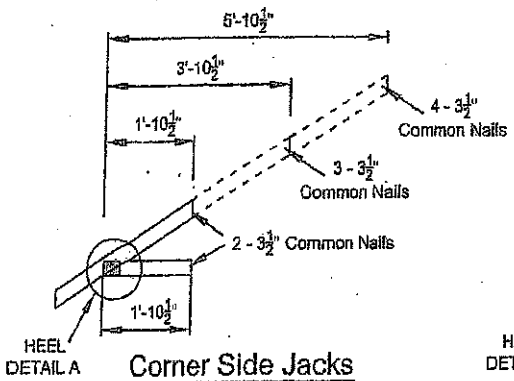
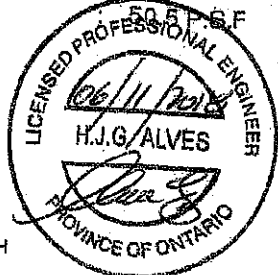
TOP CHORD SNOW LOAD : 40.5 P.S.F.  
 TOP CHORD DEAD LOAD : 3.0 P.S.F.  
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.  
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

**TOTAL LOAD**

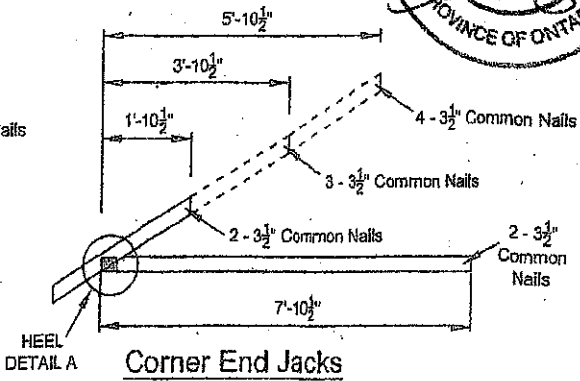
50.5 P.S.F.



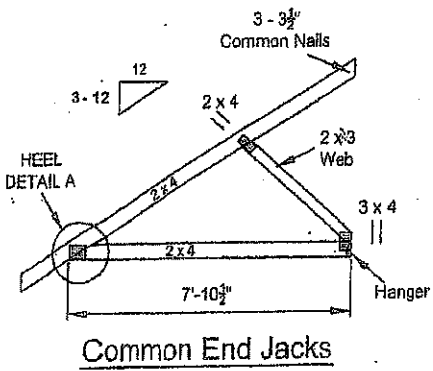
**45° Hip End**



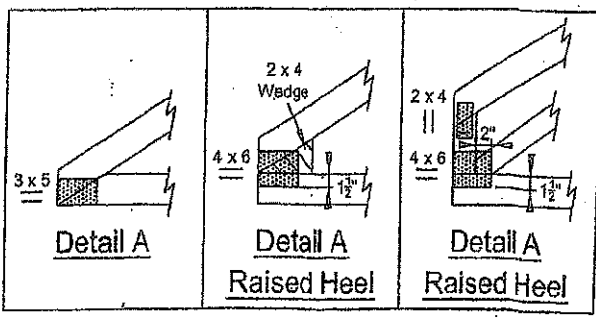
**Corner Side Jacks**



**Corner End Jacks**



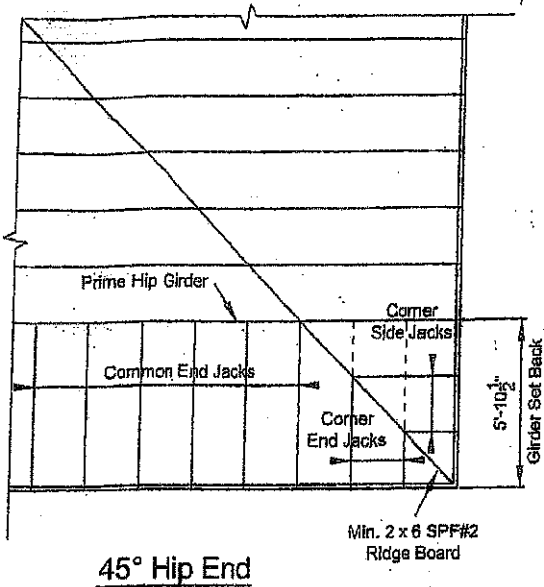
**Common End Jacks**



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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 BUILDING SECTION  
 FILE NO \_\_\_\_\_

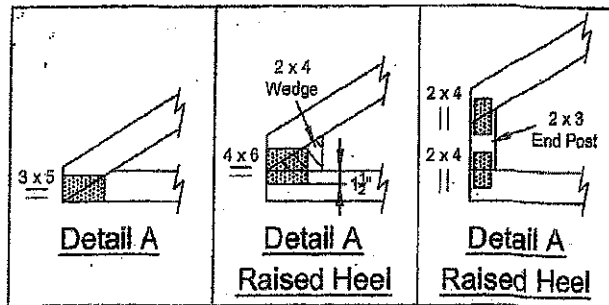
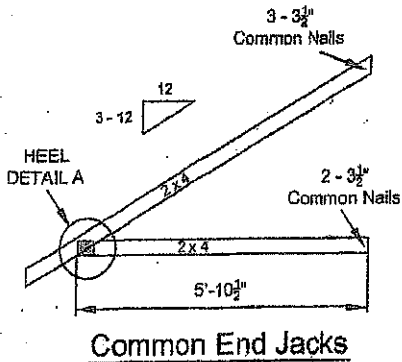
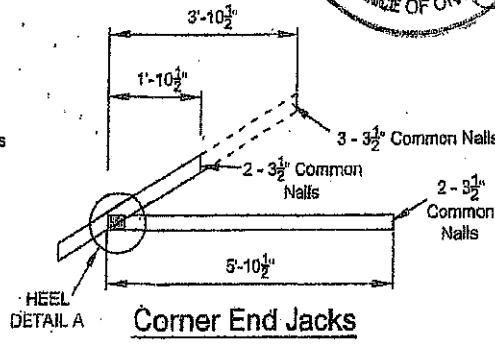
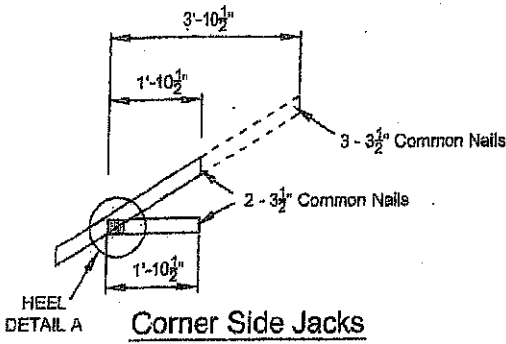
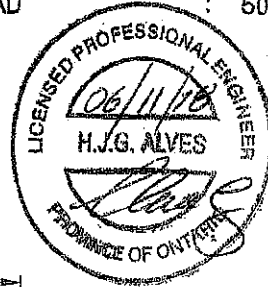


**LUMBER SPECIFICATION**

TOP CHORD : 2 x 4 SPF#2  
 BOTTOM CHORD : 2 x 4 SPF#2  
 WEBS : 2 x 3 SPF#2  
 UNLESS OTHERWISE SHOWN

**DESIGN LOAD**

TOP CHORD SNOW LOAD : 40.5 P.S.F.  
 TOP CHORD DEAD LOAD : 3.0 P.S.F.  
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.  
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.  
 TOTAL LOAD : 50.5 P.S.F



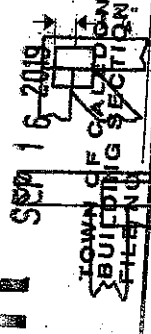
NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

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## Symbols

### PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.

For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

\*Plate location details available in MiTek software or upon request.

### PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

### LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

### BEARING



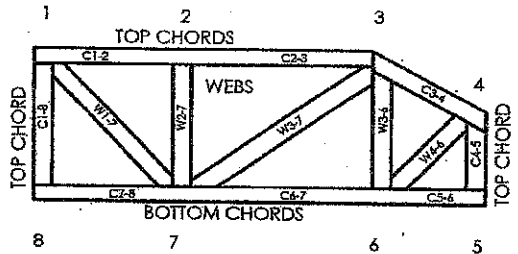
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

### Industry Standards:

- TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

## Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

### PRODUCT CODE APPROVALS

CCMC Reports:  
11996-L, 10319-L, 13270-L, 12691-R

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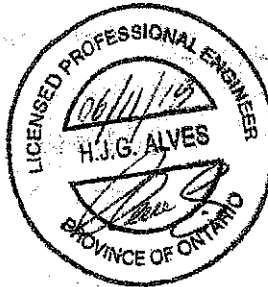


MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

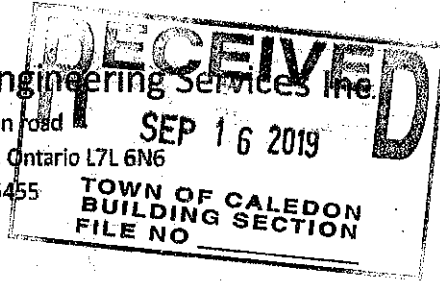
## General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.  
5208 Easton Road  
Burlington, Ontario L7L 6N6  
(289) 259 5455



### RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

### SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

F-1300218

Feb 09, 2018