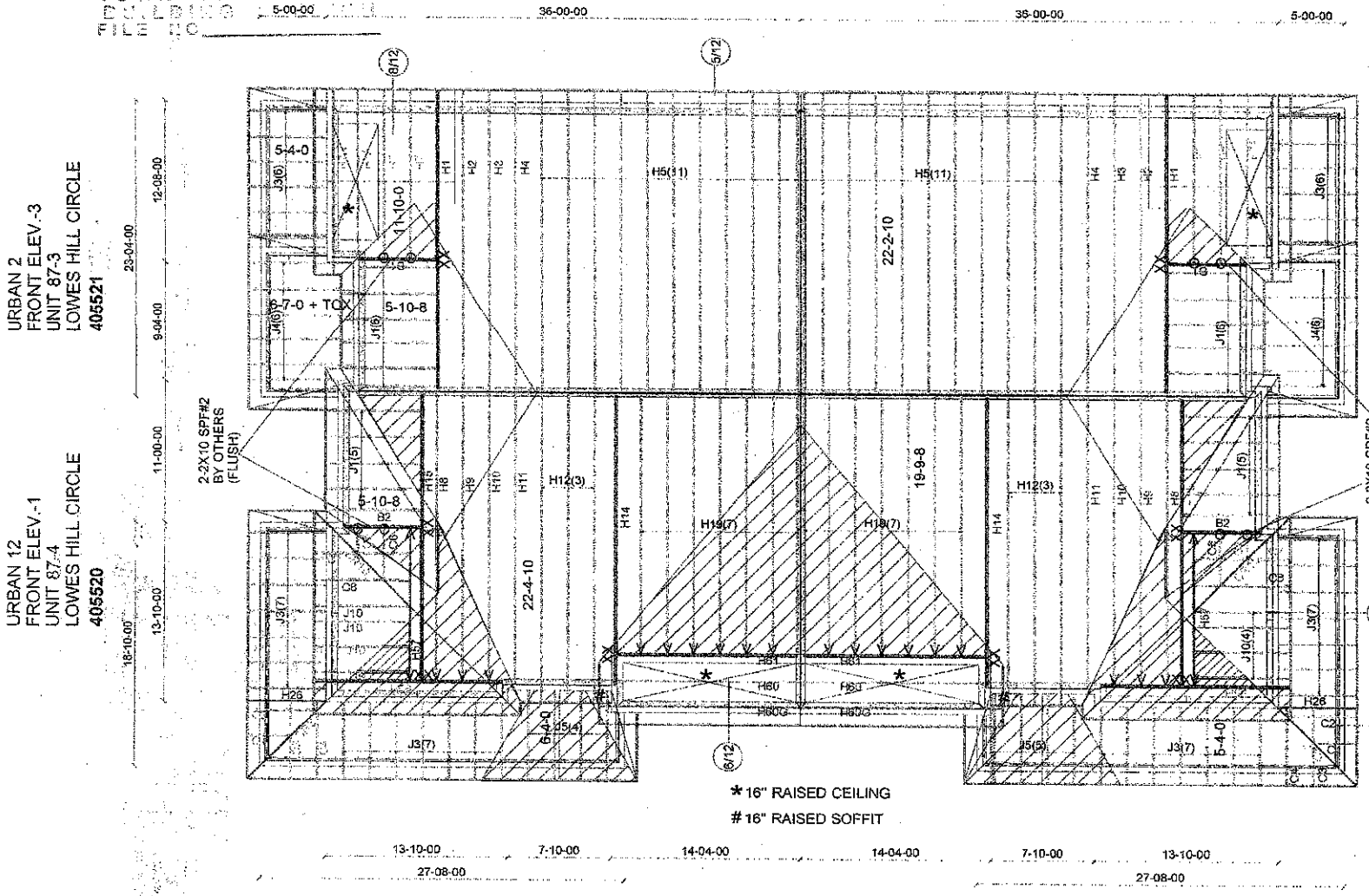
THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

BLA SEP 16 2019

TOWN OF CALEDON
PLANNING
FILE NO.



HARDWARE:
LUS24 - (O)
LJS26DS - (V)
HGUS26-2 - (XX)

URBAN 2
FRONT ELEV.-3
UNIT 87-3
LOWES HILL CIRCLE
405521

URBAN 12
FRONT ELEV.-1
UNIT 87-4
LOWES HILL CIRCLE
405520

ASPHALT SHINGLES
2X6 BRG./BRK.
12" PIN. OH.
R.T.M.C
W/ 2X6 FASCIA

PARTY WALL
2-2X4 STUD
W/ 1-2X4 STUD ABOVE

CALEDON
(29. 6. 0. 7.4)

BY JESSIE

URBAN 2
FRONT ELEV.-3
UNIT 87-2
DEER RIDGE TRAIL
405521

URBAN 12
FRONT ELEV.-1
UNIT 87-1
DEER RIDGE TRAIL
405520

M12631

TAMARACK

Job Track: **50033**
Plan Log: **201715**
Layout ID: **405515**

Builder / Location:
GREEN PARK HOMES / CALEDON

Model / Elevation:
BLOCK 87 / UNITS 87-1 - 87-4

Project: **LAMBERT LANE PH.2**
Date: **9/13/2019** Sales: Mario DiCano Designer: AC

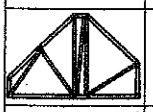
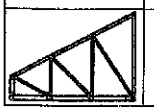
Mitek ver 8.3.0.234

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201715
Project:	LAMBERT LANE PH.2	Layout ID:	405520
Location:	CALEDON	Ref #	
Model:	BLOCK 87	Page:	1 of 2
Lot #:		Date:	09/12/2019
Elevation:	87-1, 87-4	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

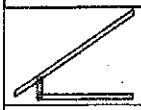
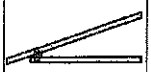
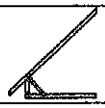
PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1	H8 Half Hip	5/12	22-00-02	5-03-13	2 x 4		1-02-07 5-03-13	87.8 55.33		
	1	H9 Half Hip	5/12	22-00-02	6-07-13	2 x 4		1-02-07 6-07-13	94.89 61.17		
	1	H10 Half Hip	5/12	22-00-02	7-11-13	2 x 4		1-02-07 7-11-13	96.28 60.83		
	1	H11 Half Hip	5/12	22-04-10	9-03-13	2 x 4		1-00-09 9-03-13	103.89 65.17		
	3	H12 Monopitch	5/12	22-04-10	10-04-08	2 x 4	1-03-08	1-00-09 10-04-08	286.88 178.50		
	1 2-ply	H14 Monopitch Girder	5/12	22-04-10	10-04-08	2 x 4 2 x 6		1-00-09 10-04-08	235.07 144.67		
	1 2-ply	H15 Hip Girder	5/12	22-00-02	5-03-13	2 x 4 2 x 6		1-02-07 1-03-10	201.49 127.33		
	7	H19 Common	5/12	19-09-08	10-04-08	2 x 4		2-01-09 10-04-08	620.06 385.00		
	1	H26 Monopitch Girder	4/12	4-07-00	2-05-10	2 x 4	1-03-08	7-15 2-02-04	20.23 14.00		
	1 2-ply	H56 Hip Girder	11/12	13-00-00	2-09-15	2 x 6	1-03-08 1-03-08	1-09-01 1-09-01	152.09 98.00		
	1	H57 Hip Girder	11/12	11-07-08	7-01-11	2 x 4		1-09-01 3-00-03	63.6 41.33		
	1	H60 Common	6/12	13-08-10	8-00-05	2 x 4		1-02-00 8-00-05	57.32 36.00		
	1	H60G GABLE	6/12	13-08-10	8-00-05	2 x 4	1-03-08	1-02-00 8-00-05	63.54 40.17		
	1 2-ply	H61 Jack-Closed Girder	6/12	13-08-10	9-04-05	2 x 4 2 x 6		2-06-00 9-04-05	160.85 97.33		

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201715
Project:	LAMBERT LANE PH.2	Layout ID:	405520
Location:	CALEDON	Ref #	
Model:	BLOCK 87	Page:	2 of 2
Lot #:		Date:	09/12/2019
Elevation:	87-1, 87-4	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	5	J1 Jack-Open	8 /12	**5-10-08**	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	89.15 53.33		
	14	J3 Jack-Open	4 /12	**5-04-00**	2-06-03	2 x 4	1-03-08	7-15 2-02-04	206.9 149.33		
	4	J5 Jack-Open	4 /12	**6-04-00**	2-10-03	2 x 4	1-03-08	7-15 2-06-04	68.48 48.00		
	4	J10 Jack-Open	11 /12	**5-10-08**	7-01-11	2 x 4	1-03-08	1-09-01 7-01-11	84.64 52.00		

TOTAL # TRUSS= **53** TOTAL BFT OF ALL TRUSSES= **1707.49** BFT. TOTAL WEIGHT OF ALL TRSSES **2695.17** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
11	Hardware	LJS26DS	
3	Hardware	HGUS26-2	

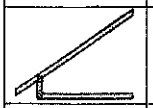
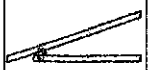
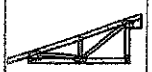
TOTAL NUMBER OF ITEMS= **16**

TAMARACK
ROOF TRUSSES INC.
ALPA LUMBER GROUP

DELIVERY SHIPLIST

Lumber Yard:	TAMARACK LUMBER	Job Track:	50033
Builder:	GREEN PARK HOMES	PlanLog:	201715
Project:	LAMBERT LANE PH.2	Layout ID:	405521
Location:	CALEDON	Ref #	
Model:	BLOCK 87	Page:	1 of 1
Lot #:		Date:	09/13/2019
Elevation:	87-2, 87-3	Designer:	Andrew Conway
		Sales Rep:	Mario DiCano

Roof Trusses

PROFILE	QTY / PLY	MARK / TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT / RIGHT	HEEL HEIGHT LEFT / RIGHT	LBS. / BFT.	BUNDLE # / STACK #	LOAD BY / REMARKS
	1 2-ply	H1 Half Hip Girder	5 /12	22-02-10	5-03-13	2 x 4 2 x 6	1-03-08	1-00-09 5-03-13	219.48 135.00		
	1	H2 Half Hip	5 /12	22-02-10	6-07-13	2 x 4	1-03-08	1-00-09 6-07-13	96.92 62.50		
	1	H3 Half Hip	5 /12	22-02-10	7-11-13	2 x 4	1-03-08	1-00-09 7-11-13	98.18 61.67		
	1	H4 Half Hip	5 /12	22-02-10	9-03-13	2 x 4	1-03-08	1-00-09 9-03-13	105.08 66.50		
	11	H5 Monopitch	5 /12	22-02-10	10-03-10	2 x 4	1-03-08	1-00-09 10-03-10	1051.67 654.50		
	2	H7 Common	8 /12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	104.75 63.67		
	2	H7A Roof Special Structural Gable	8 /12	11-10-00	5-04-02	2 x 4	1-03-08 1-03-08	1-04-13 1-04-13	103.8 69.33		
	6	J1 Jack-Open	8 /12	5-10-08	5-03-13	2 x 4	1-03-08	1-04-13 5-03-13	106.98 64.00		
	6	J3 Jack-Open	4 /12	5-04-00	2-06-03	2 x 4	1-03-08	7-15 2-02-04	88.67 64.00		
	6	J4 Jack-Closed	4 /12	6-07-00	3-02-03	2 x 4	1-03-08	7-15 2-10-04	174.1 123.00		

TOTAL # TRUSS= **38** TOTAL BFT OF ALL TRUSSES= **1364.17** BFT. TOTAL WEIGHT OF ALL TRSSES **2149.64** LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	LUS24	
1	Hardware	HGUS26-2	

TOTAL NUMBER OF ITEMS= **3**

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H1	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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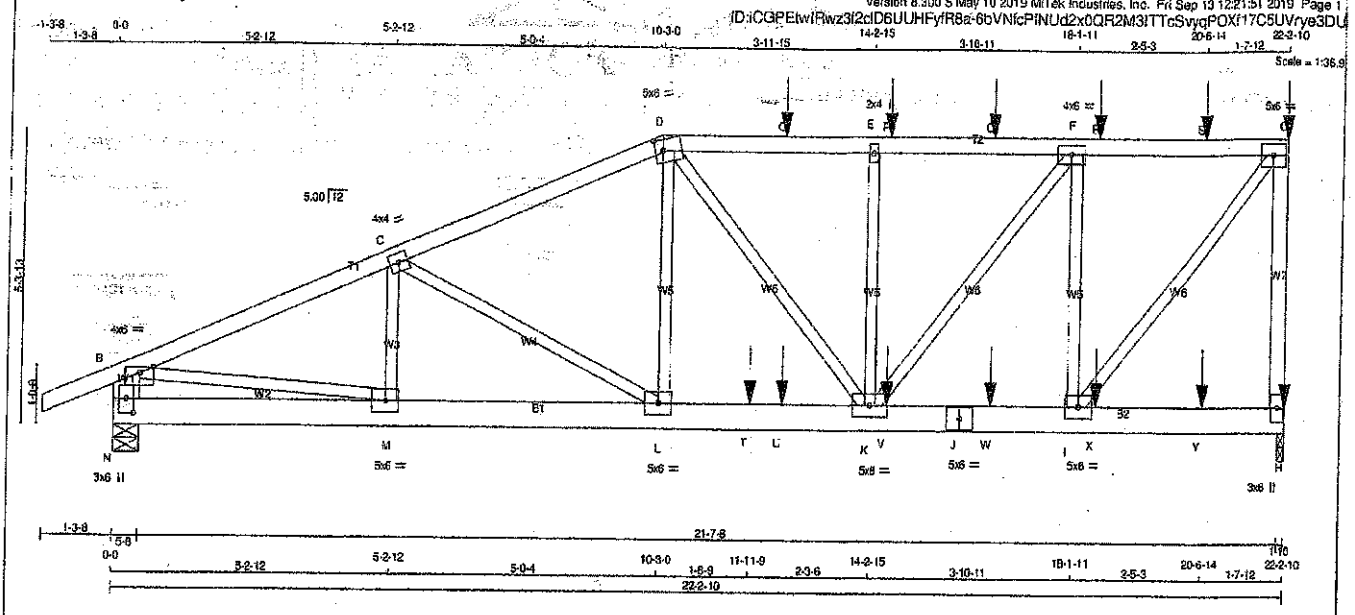

TOTAL WEIGHT = 4 X 110 = 439 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - B	2x6	DRY	No.2	SPF
N - J	2x6	DRY	No.2	SPF
J - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A- D	1	12	TOP
D- G	1	12	SIDE(61.0)
G- H	1	12	TOP
N- B	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N- J	2	12	SIDE(183.1)
J- H	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	4.0	6.0	Edge	
C	TMWW-t	MT20	4.0	4.0		
D	TTWW-m	MT20	5.0	6.0	2.50	1.75
E	TMW+w	MT20	2.0	4.0		
F	TMWW-t	MT20	4.0	6.0		
G	TMVW-t	MT20	5.0	6.0		
H	BMV1+p	MT20	3.0	6.0		
I, L, M						
I	BMWW-t	MT20	5.0	6.0		
J	BS-t	MT20	5.0	6.0		
K	BMWWW-t	MT20	5.0	6.0		
N	BMV1+p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	2745	0	2745	0	0	1-10	1-10
N	2223	0	2223	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1931	1327 / 0	0 / 0	0 / 0	0 / 0	604 / 0	0 / 0
N	1560	1094 / 0	0 / 0	0 / 0	0 / 0	466 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)
A- B	0 / 26	-102.0	-102.0	0.07 (1)	
B- C	-3551 / 0	-102.0	-102.0	0.30 (1)	
C- D	-3401 / 0	-102.0	-102.0	0.29 (1)	
D- O	-3024 / 0	-102.0	-102.0	0.20 (1)	
O- E	-3024 / 0	-102.0	-102.0	0.20 (1)	
E- P	-3024 / 0	-102.0	-102.0	0.23 (1)	
P- Q	-3024 / 0	-102.0	-102.0	0.23 (1)	
Q- F	-3024 / 0	-102.0	-102.0	0.23 (1)	
F- R	-1854 / 0	-102.0	-102.0	0.21 (1)	
R- S	-1854 / 0	-102.0	-102.0	0.21 (1)	
S- G	-1854 / 0	-102.0	-102.0	0.21 (1)	
H- G	-2643 / 0	0.0	0.0	0.61 (1)	
N- B	-2138 / 0	0.0	0.0	0.07 (1)	
N- M	0 / 0	-18.5	-18.5	0.07 (1)	
M- L	0 / 3298	-18.5	-18.5	0.35 (1)	
L- T	0 / 3139	-18.5	-18.5	0.52 (1)	
T- U	0 / 3139	-18.5	-18.5	0.52 (1)	
U- K	0 / 3139	-18.5	-18.5	0.52 (1)	
K- V	0 / 1854	-18.5	-18.5	0.21 (1)	
V- J	0 / 1854	-18.5	-18.5	0.21 (1)	
J- W	0 / 1854	-18.5	-18.5	0.21 (1)	
W- I	0 / 1854	-18.5	-18.5	0.21 (1)	
I- X	0 / 0	-18.5	-18.5	0.04 (4)	
X- Y	0 / 0	-18.5	-18.5	0.04 (4)	
Y- H	0 / 0	-18.5	-18.5	0.04 (4)	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	MAX. UNBRAC LENGTH
M- C	-466 / 0	0.05 (1)	10.00
C- L	-206 / 0	0.06 (1)	4.69
L- D	0 / 1040	0.13 (1)	4.78
I- G	0 / 2313	0.36 (1)	5.11
D- K	-184 / 0	0.06 (1)	5.11
I- F	-2208 / 0	0.45 (1)	5.07
K- E	-672 / 0	0.14 (1)	5.07
K- F	0 / 1690	0.23 (1)	5.07
B- M	0 / 3324	0.47 (1)	6.13

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HGHT	COMM
G	22-2-10	-164	-164	---	FRONT	VERT	TOTAL	---	C1
H	22-2-10	-35	-35	---	FRONT	VERT	TOTAL	---	C1
O	12-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
P	14-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
Q	16-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
R	18-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
S	20-6-14	-123	-123	---	FRONT	VERT	TOTAL	---	C1
T	11-11-9	-1206	-1206	---	FRONT	VERT	TOTAL	---	C1
U	12-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
V	14-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
W	16-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
X	18-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1
Y	20-6-14	-27	-27	---	FRONT	VERT	TOTAL	---	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	28.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.61/1.00 (G-H:1) , BC=0.52/1.00 (K-L:1) , WB=0.45/1.00 (F-I:1) , SSI=0.26/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI)	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1857	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (B) (INPUT = 0.90)
JSI METAL= 0.41 (M) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J/G ALVES
100009924
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923489 CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H2	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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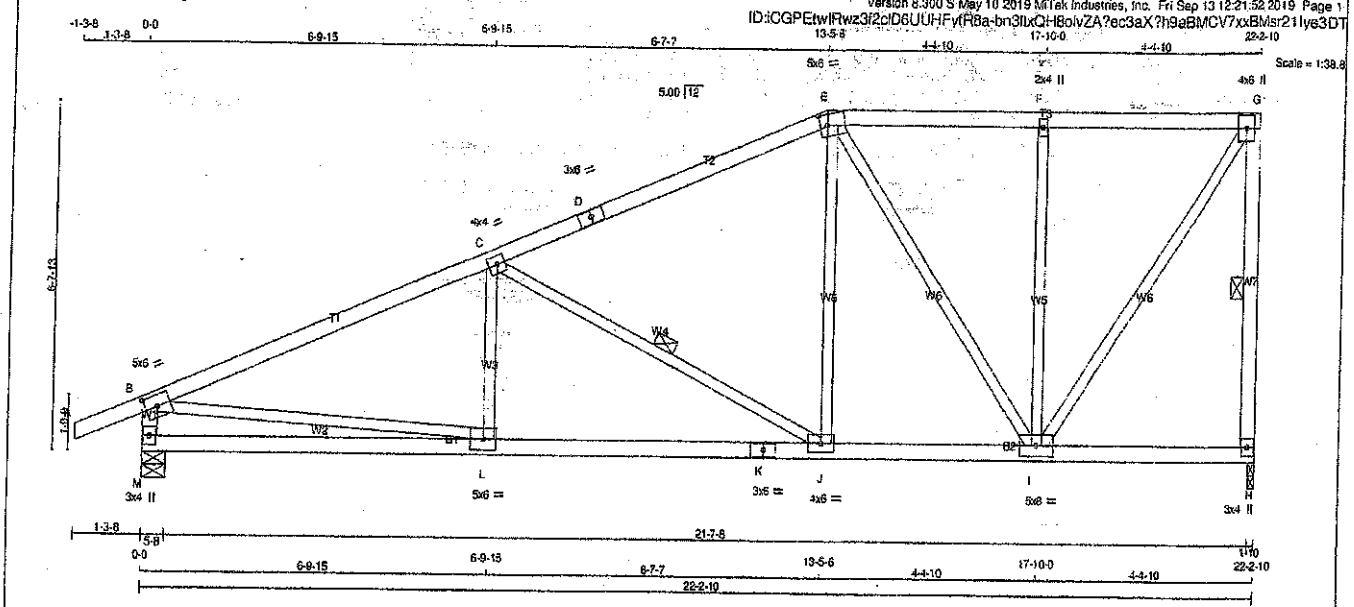

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	6.0		
E	TTWW-m	MT20	5.0	6.0	2.50	1.75
F	TMW-w	MT20	2.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	6.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, C-J.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	
B-C	-2068 / 0	-102.0	-102.0	0.70 (1)	3.85	
C-D	-1244 / 0	-102.0	-102.0	0.82 (1)	4.87	
D-E	-1244 / 0	-102.0	-102.0	0.62 (1)	4.87	
E-F	-786 / 0	-102.0	-102.0	0.25 (1)	6.25	
F-G	-786 / 0	-102.0	-102.0	0.25 (1)	6.25	
H-G	-1303 / 0	0.0	0.0	0.26 (1)	5.66	
M-B	-1423 / 0	0.0	0.0	0.14 (1)	6.84	
M-L	0 / 0	-18.5	-18.5	0.21 (4)	10.00	
L-K	0 / 1938	-18.5	-18.5	0.40 (1)	10.00	
K-J	0 / 1938	-18.5	-18.5	0.40 (1)	10.00	
J-I	0 / 1127	-18.5	-18.5	0.25 (1)	10.00	
I-H	0 / 0	-18.5	-18.5	0.07 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-72 / 86	0.03 (4)
C-J	-929 / 0	0.37 (1)
J-E	0 / 542	0.12 (1)
E-I	-504 / 0	0.70 (1)
I-F	-552 / 0	0.40 (1)
I-G	0 / 1376	0.31 (1)
B-L	0 / 1949	0.44 (1)

TOTAL WEIGHT = 2 X 97 = 194 lb [M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	5.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.70/1.00 (B-C:1) , BC=0.40/1.00 (J-L:1) , WB=0.70/1.00 (E-I:1) , SSI=0.31/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (I) (INPUT = 0.90)
JSI METAL= 0.58 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
04/13/19
H. J.G ALVES
100090024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923490

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H3	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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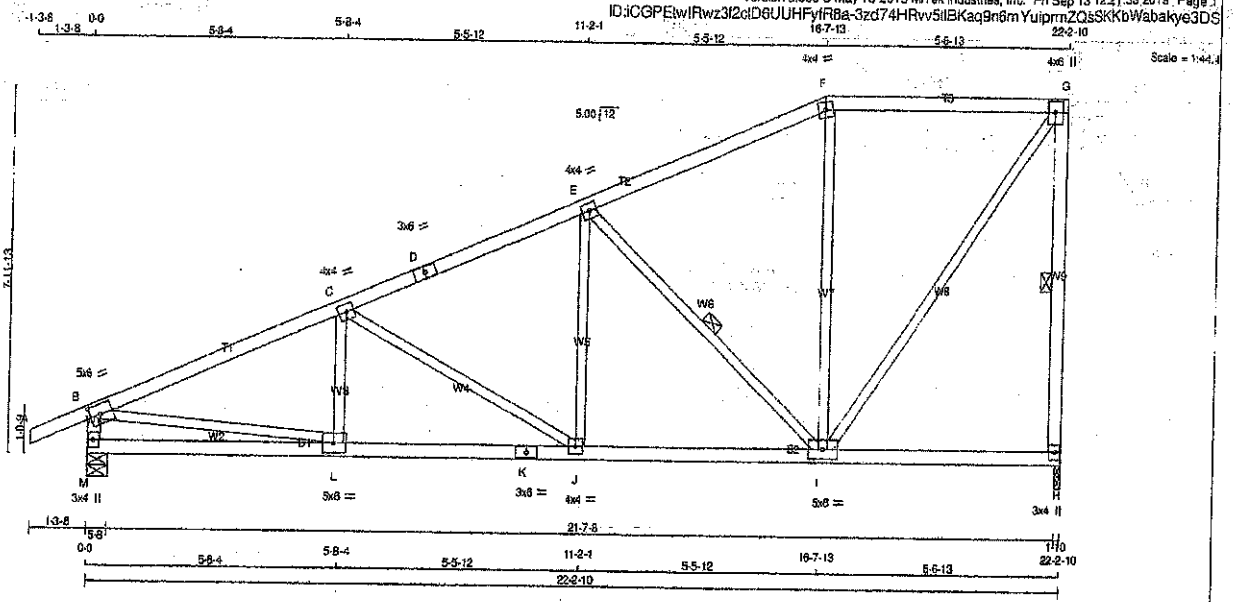

TOTAL WEIGHT = 2 X 98 = 196 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TB-t	MT20	3.0	6.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMVW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	728 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.26 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	8	0.13 (1)	10.00
B-C	-2087 / 0	-102.0	-102.0	1	0.45 (1)	4.26
C-D	-1561 / 0	-102.0	-102.0	1	0.36 (1)	4.90
D-E	-1561 / 0	-102.0	-102.0	1	0.36 (1)	4.90
E-F	-805 / 0	-102.0	-102.0	1	0.35 (1)	6.25
F-G	-721 / 0	-102.0	-102.0	1	0.41 (1)	6.25
H-G	-1298 / 0	0.0	0.0	1	0.37 (1)	5.66
M-B	-1430 / 0	0.0	0.0	1	0.14 (1)	6.82
M-L	0 / 0	-18.5	-18.5	1	0.13 (4)	10.00
L-K	0 / 1927	-18.5	-18.5	1	0.36 (1)	10.00
K-J	0 / 1927	-18.5	-18.5	1	0.36 (1)	10.00
J-I	0 / 1441	-18.5	-18.5	1	0.30 (1)	10.00
I-H	0 / 0	-18.5	-18.5	1	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-143 / 51	0.03 (1)
C-J	-559 / 0	0.40 (1)
J-E	0 / 376	0.08 (1)
E-I	997 / 0	0.40 (1)
I-F	-207 / 13	0.25 (1)
I-G	0 / 1245	0.26 (1)
B-L	0 / 1943	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. O/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.45/1.00 (B-C:1) , BC=0.36/1.00 (J-L:1) , WB=0.44/1.00 (B-L:1) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (L) (INPUT = 0.90)
JSI METAL= 0.57 (K) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923491

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	H4	2	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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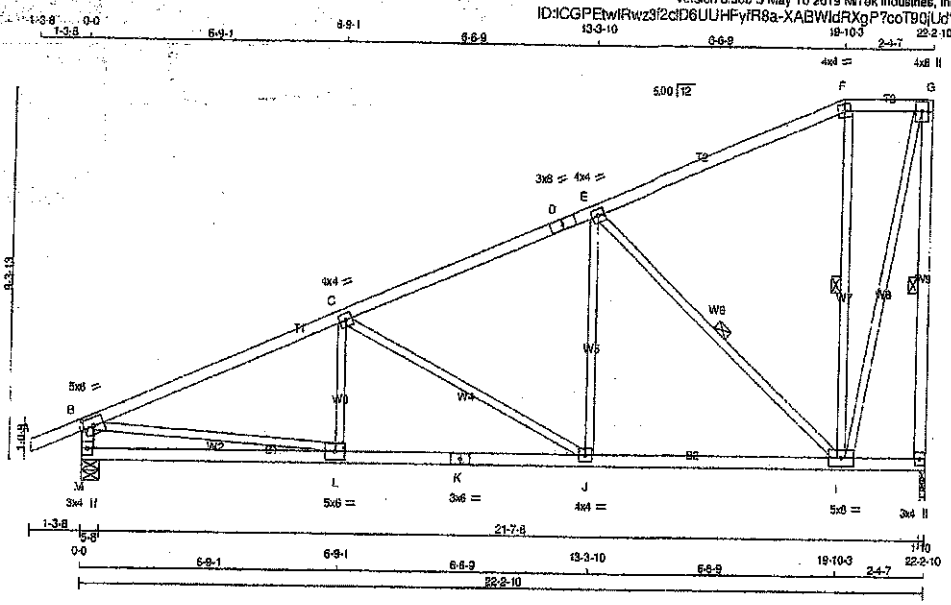

TOTAL WEIGHT = 2 X 105 = 210 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMWW-t	MT20	5.0	6.0	2.50	2.75
C	TMWW-t	MT20	4.0	4.0		
D	TS-t	MT20	3.0	8.0		
E	TMWW-t	MT20	4.0	4.0		
F	TTW-m	MT20	4.0	4.0		
G	TMWW+p	MT20	4.0	6.0		
H	BMV1+p	MT20	3.0	4.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	4.0	4.0		
K	BS-t	MT20	3.0	6.0		
L	BMWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
M	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
M	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.99 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, E-I, F-I.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00
B-C	-2042 / 0	-102.0	-102.0	0.65 (1)	3.99
C-D	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00
D-E	-1318 / 0	-102.0	-102.0	0.50 (1)	5.00
E-F	-358 / 0	-102.0	-102.0	0.51 (1)	6.25
F-G	-304 / 0	-102.0	-102.0	0.07 (1)	6.25
H-G	-1331 / 0	0.0	0.0	0.54 (1)	5.62
M-B	-1423 / 0	0.0	0.0	0.14 (1)	6.84
M-L	0 / 0	-18.5	-18.5	0.18 (4)	10.00
L-K	0 / 1908	-18.5	-18.5	0.40 (1)	10.00
K-J	0 / 1908	-18.5	-18.5	0.40 (1)	10.00
J-I	0 / 1217	-18.5	-18.5	0.29 (1)	10.00
I-H	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-79 / 88	0.03 (4)
C-J	-787 / 0	0.87 (1)
J-E	0 / 505	0.11 (1)
E-I	-1252 / 0	0.73 (1)
I-F	-261 / 0	0.15 (1)
I-G	0 / 1249	0.28 (1)
B-L	0 / 1920	0.43 (1)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65/1.00 (B-C:1) , BC=0.40/1.00 (J-L:1) , WB=0.87/1.00 (C-J:1) , SSI=0.29/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.86 (G) (INPUT = 0.90)
JSI METAL= 0.93 (K) (INPUT = 1.00)

Structural component only
DWG# T-1923492

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H5	22	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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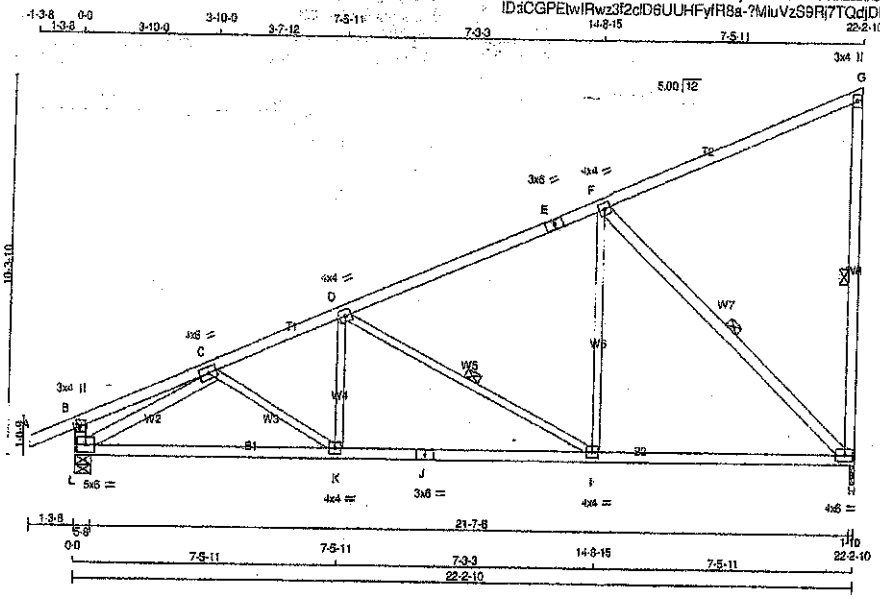


TOTAL WEIGHT = 22 X 100 = 2203 lb [M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TS-t	MT20	3.0	6.0		
F	TMWW-t	MT20	4.0	4.0		
G	TMV+p	MT20	3.0	4.0		
H	BMWW1-t	MT20	4.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	4.0	4.0		
L	BMVW1-t	MT20	5.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1339	0	1339	0	0	1-10	1-10
L	1476	0	1476	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	942	644 / 0	0 / 0	0 / 0	0 / 0	298 / 0	0 / 0
L	1037	723 / 0	0 / 0	0 / 0	0 / 0	314 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.50 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

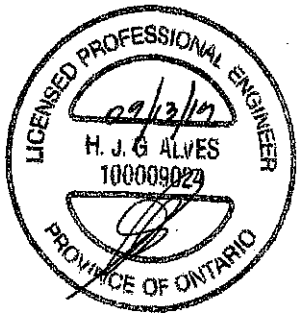
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	0.13 (1)	10.00	
B-C	0 / 8	-102.0	-102.0	0.20 (1)	10.00	
C-D	-1944 / 0	-102.0	-102.0	0.33 (1)	4.50	
D-E	-1140 / 0	-102.0	-102.0	0.69 (1)	4.86	
E-F	-1140 / 0	-102.0	-102.0	0.89 (1)	4.86	
F-G	-32 / 0	-102.0	-102.0	0.70 (1)	6.25	
H-G	-298 / 0	0.0	0.0	0.16 (1)	6.25	
L-B	-308 / 0	0.0	0.0	0.03 (1)	7.81	
L-K	0 / 1608	-18.5	-18.5	0.42 (1)	10.00	
K-J	0 / 1812	-18.5	-18.5	0.43 (1)	10.00	
J-I	0 / 1812	-18.5	-18.5	0.43 (1)	10.00	
I-H	0 / 1089	-18.5	-18.5	0.32 (4)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-D	0 / 142	0.05 (4)
D-I	-841 / 0	0.39 (1)
I-F	0 / 546	0.12 (1)
F-H	-1454 / 0	0.78 (1)
C-K	0 / 54	0.02 (4)
L-C	-2122 / 0	0.50 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.74")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.74")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.70/1.00 (F-G:1) , BC=0.43/1.00 (I-K:1) , WB=0.78/1.00 (F-H:1) , SSI=0.33/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (F) (INPUT = 0.90)
JSI METAL= 0.57 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923493

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H7	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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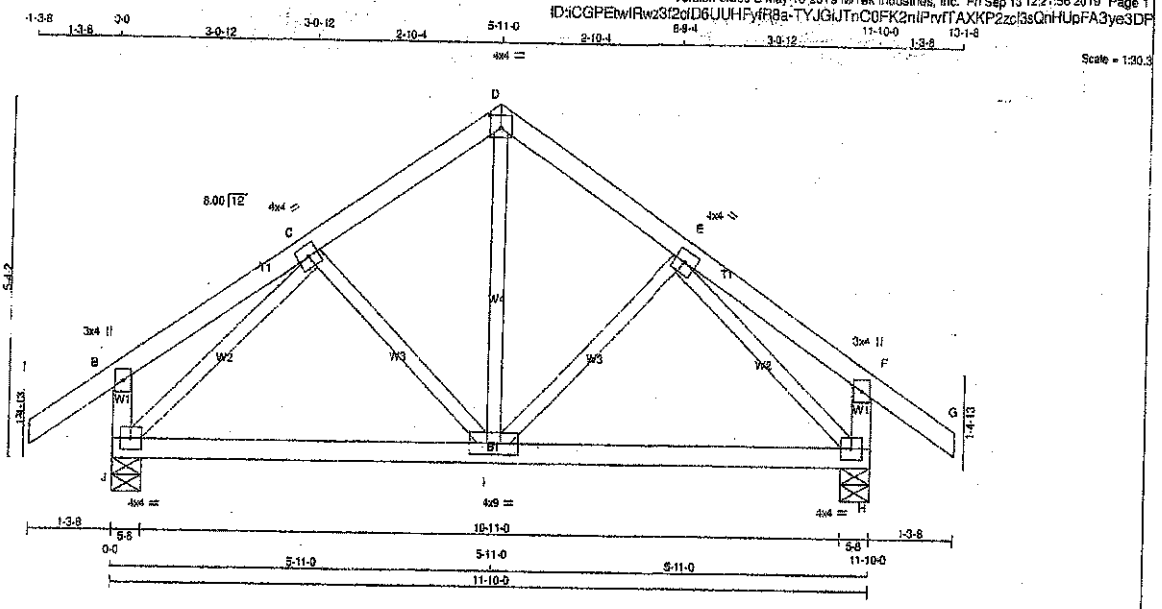

TOTAL WEIGHT = 6 X 52 = 312 lb
[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	4.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW1-t	MT20	4.0	4.0		
I	BMWWW-t	MT20	4.0	9.0		
J	BMVW1-t	MT20	4.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
J	853	0	853	0	0	5-8	5-8
H	853	0	853	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI	(LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
A-B	0 / 39	-102.0	-102.0	1	0.14	(1)	10.00	I-D	0 / 342	0.08	(1)
B-C	0 / 18	-102.0	-102.0	1	0.14	(1)	10.00	I-E	-145 / 0	0.04	(1)
C-D	-540 / 0	-102.0	-102.0	1	0.11	(1)	6.25	C-I	-145 / 0	0.04	(1)
D-E	-540 / 0	-102.0	-102.0	1	0.11	(1)	6.25	J-C	-770 / 0	0.22	(1)
E-F	0 / 18	-102.0	-102.0	1	0.14	(1)	10.00	E-H	-770 / 0	0.22	(1)
F-G	0 / 39	-102.0	-102.0	1	0.14	(1)	10.00				
J-B	-257 / 0	0.0	0.0	3	0.03	(1)	7.81				
H-F	-257 / 0	0.0	0.0	3	0.03	(1)	7.81				
J-I	0 / 538	-18.5	-18.5	4	0.22	(4)	10.00				
I-H	0 / 536	-18.5	-18.5	4	0.22	(4)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.04")

CSI: TC=0.14/1.00 (A-B:1) , BC=0.22/1.00 (I-J:4) , WB=0.22/1.00 (C-J:1) , SSI=0.12/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	818	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.70 (C) (INPUT = 0.90)
JSI METAL= 0.27 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923494

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H7A	4	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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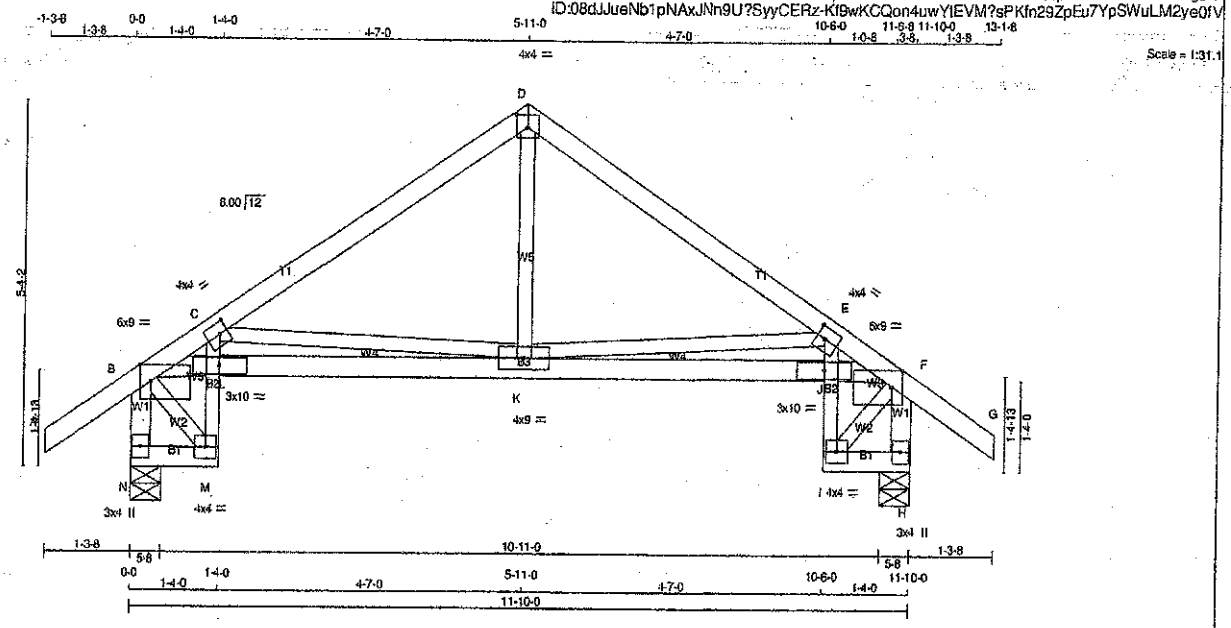


TOTAL WEIGHT = 4 X 52 = 210 lb

[M][F]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
N - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
N - M	2x4	DRY	No.2	SPF
M - C	2x3	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
I - E	2x3	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
B - L	2x4	DRY	No.2	SPF
J - F	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWW-p	MT20	6.0	9.0	Edge	2.00
C	TMVW-t	MT20	4.0	4.0	2.00	1.50
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMVW-t	MT20	4.0	4.0	2.00	1.50
F	TMVWW-p	MT20	6.0	9.0	Edge	2.00
H	BMV1+p	MT20	3.0	4.0		
I	BMVW-t	MT20	4.0	4.0		
J	BVMW-t	MT20	3.0	10.0		
K	BMWWW-t	MT20	4.0	9.0		
L	BVMW-t	MT20	3.0	10.0		
M	BMVW-t	MT20	4.0	4.0		
N	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
N	853	0	853	0	0	5-8	5-8
H	853	0	853	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
N	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0
H	598	423 / 0	0 / 0	0 / 0	0 / 0	175 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) N, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.04 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 7.81 FT OR RIGID CEILING DIRECTLY APPLIED.

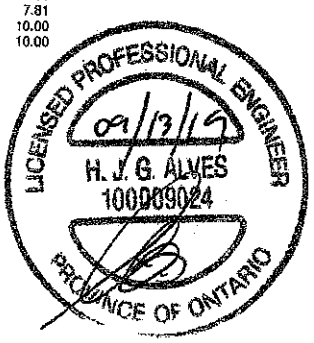
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 39	-102.0	-102.0	0.14 (1)	10.00
B- C	-1632 / 0	-102.0	-102.0	0.16 (1)	5.04
C- D	-687 / 0	-102.0	-102.0	0.30 (1)	6.25
D- E	-687 / 0	-102.0	-102.0	0.30 (1)	6.25
E- F	-1632 / 0	-102.0	-102.0	0.16 (1)	5.04
F- G	0 / 39	-102.0	-102.0	0.14 (1)	10.00
N- B	-842 / 0	0.0	0.0	0.09 (1)	7.81
H- F	-842 / 0	0.0	0.0	0.09 (1)	7.81
N- M	0 / 0	-18.5	-18.5	0.01 (4)	10.00
M- L	-15 / 5	0.0	0.0	0.08 (1)	7.81
L- C	0 / 294	0.0	0.0	0.17 (1)	10.00
L- K	0 / 1439	-18.5	-18.5	0.30 (1)	10.00
K- J	0 / 1439	-18.5	-18.5	0.30 (1)	10.00
I- J	-15 / 5	0.0	0.0	0.08 (1)	7.81
J- E	0 / 294	0.0	0.0	0.17 (1)	10.00
I- H	0 / 0	-18.5	-18.5	0.01 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- D	0 / 321	0.07 (1)
K- E	-886 / 0	0.34 (1)
C- K	-886 / 0	0.34 (1)
B- M	0 / 40	0.01 (1)
B- L	0 / 1368	0.22 (1)
I- F	0 / 39	0.01 (1)
J- F	0 / 1368	0.22 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.03")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.06")

CSI: TC=0.30/1.00 (C-D:1) , BC=0.30/1.00 (K-L:1) , WB=0.34/1.00 (C-K:1) , SSI=0.17/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.73 (E) (INPUT = 0.90)
JSI METAL= 0.41 (L) (INPUT = 1.00)

Structural component only
DWG# T-1923566

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H8	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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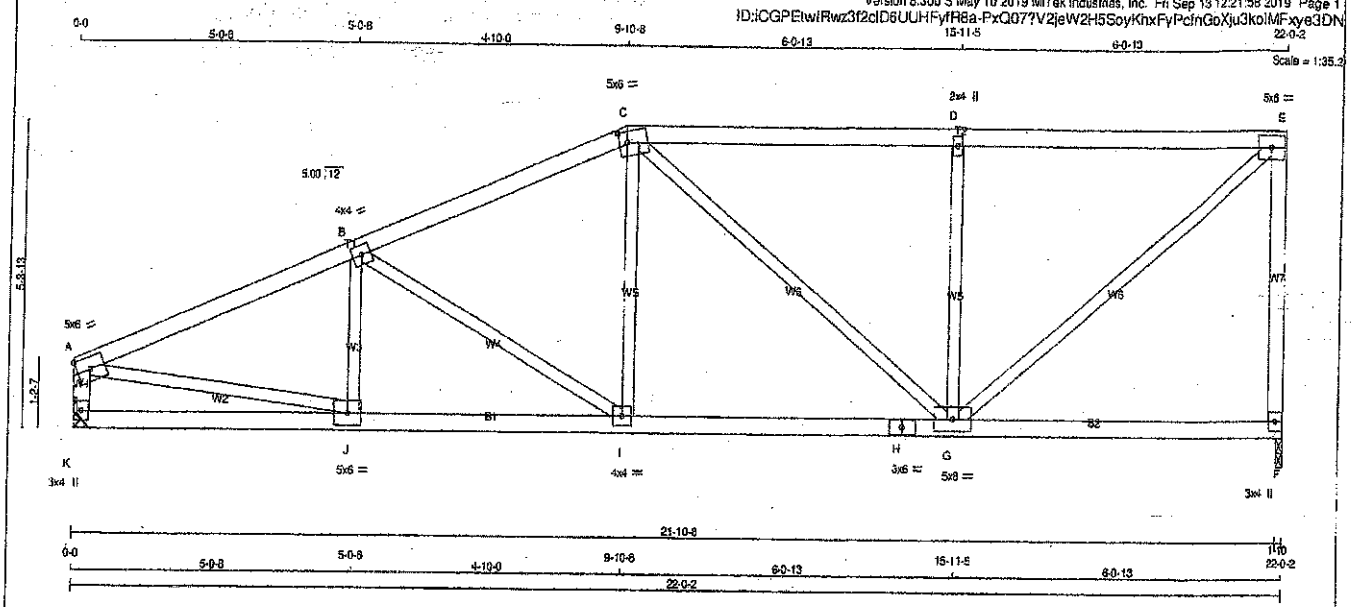

TOTAL WEIGHT = 2 X 88 = 176 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.25	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW-t	MT20	5.0	6.0		
F	SMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-r	MT20	4.0	4.0		
J	SMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1326	0	1326	0	0	1-10	1-10
K	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K. MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
K	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.27 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1950 / 0	-102.0	-102.0	0.49 (1)	4.27
B-C	-1584 / 0	-102.0	-102.0	0.46 (1)	4.68
C-D	-1267 / 0	-102.0	-102.0	0.70 (1)	4.60
D-E	-1267 / 0	-102.0	-102.0	0.71 (1)	4.60
F-E	-1280 / 0	0.0	0.0	0.63 (1)	7.13
K-A	-1284 / 0	0.0	0.0	0.13 (1)	7.11
K-J	0 / 0	-18.5	-18.5	0.09 (4)	10.00
J-I	0 / 1820	-18.5	-18.5	0.35 (1)	10.00
I-H	0 / 1448	-18.5	-18.5	0.31 (1)	10.00
H-G	0 / 1448	-18.5	-18.5	0.31 (1)	10.00
G-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-232 / 12	0.06 (1)
B-I	-444 / 0	0.25 (1)
I-C	0 / 332	0.07 (1)
C-G	-235 / 0	0.29 (1)
G-D	-767 / 0	0.32 (1)
G-E	0 / 1641	0.37 (1)
A-J	0 / 1849	0.42 (1)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009021
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.06")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.71/1.00 (D-E:1) , BC=0.35/1.00 (I-J:1) , WB=0.42/1.00 (A-J:1) , SSI=0.30/1.00 (D-E:1)

DOL 1.00BER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	789	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.87 (G) (INPUT = 0.90)
JSI METAL = 0.50 (A) (INPUT = 1.00)

Structural component only
DWG# T-1923496

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H9	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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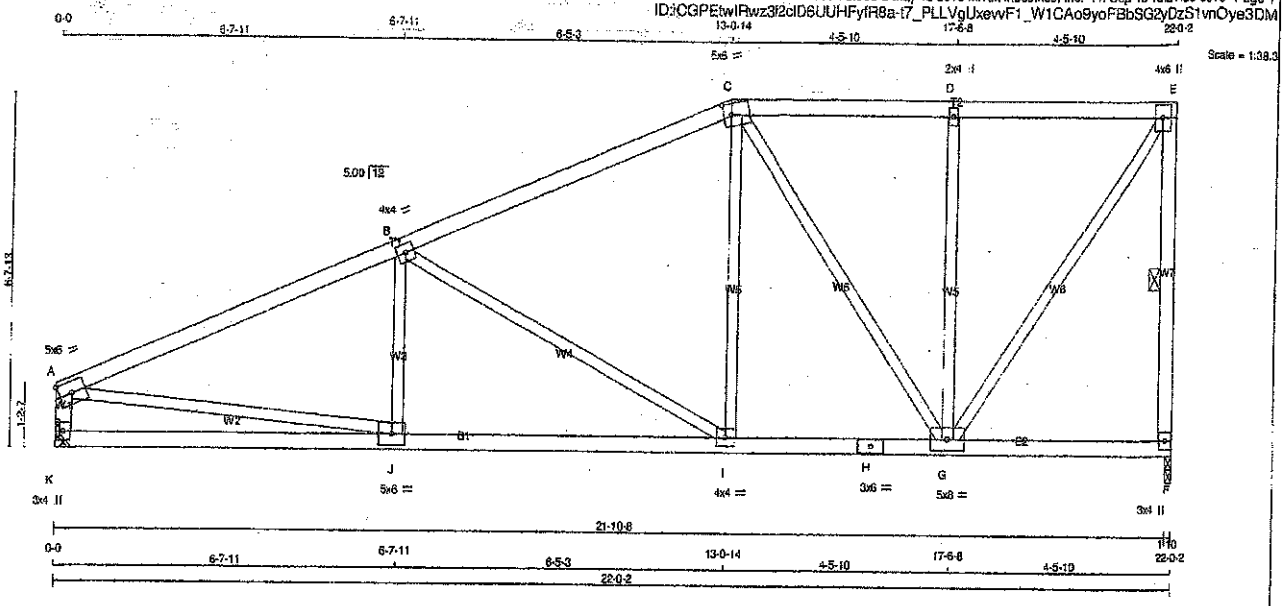

TOTAL WEIGHT = 2 X 95 = 190 lb

[M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
K - A	2x4	DRY	No.2	SPF
K - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TMW+w	MT20	2.0	4.0		
E	TMVW+p	MT20	4.0	8.0		
F	BMV1+p	MT20	3.0	4.0		
G	BMWWW-t	MT20	5.0	8.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	4.0	4.0		
J	BMWW-t	MT20	5.0	6.0		
K	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	1326	0	1326	0	0	1-10	1-10
K	1326	0	1326	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT K, MINIMUM BEARING LENGTH AT JOINT K = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
K	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.03 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LCI	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1961 / 0	-102.0	-102.0	0.65 (1)	4.03	
B-C	-1232 / 0	-102.0	-102.0	0.58 (1)	4.98	
C-D	-789 / 0	-102.0	-102.0	0.26 (1)	6.25	
D-E	-790 / 0	-102.0	-102.0	0.26 (1)	6.25	
F-E	-1290 / 0	0.0	0.0	0.26 (1)	5.66	
K-A	-1275 / 0	0.0	0.0	0.13 (1)	7.13	
K-J	0 / 0	-18.5	-18.5	0.19 (4)	10.00	
J-I	0 / 1838	-18.5	-18.5	0.38 (1)	10.00	
I-H	0 / 1116	-18.5	-18.5	0.24 (1)	10.00	
H-G	0 / 1116	-18.5	-18.5	0.24 (1)	10.00	
G-F	0 / 0	-18.5	-18.5	0.08 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
J-B	-114 / 80	0.03 (1)
B-I	-837 / 0	0.94 (1)
I-C	0 / 512	0.12 (1)
C-G	-571 / 0	0.65 (1)
G-D	-563 / 0	0.41 (1)
G-E	0 / 1385	0.31 (1)
A-J	0 / 1855	0.42 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.38/1.00 (I-J:1) , WB=0.94/1.00 (B-I:1) , SSI=0.30/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.83 (G) (INPUT = 0.90)
JSI METAL = 0.51 (A) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923497

RECEIVED
SEP 16 2019
TOWN OF GEORGETOWN
BUILDING SECTION
FILE NO.

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H10	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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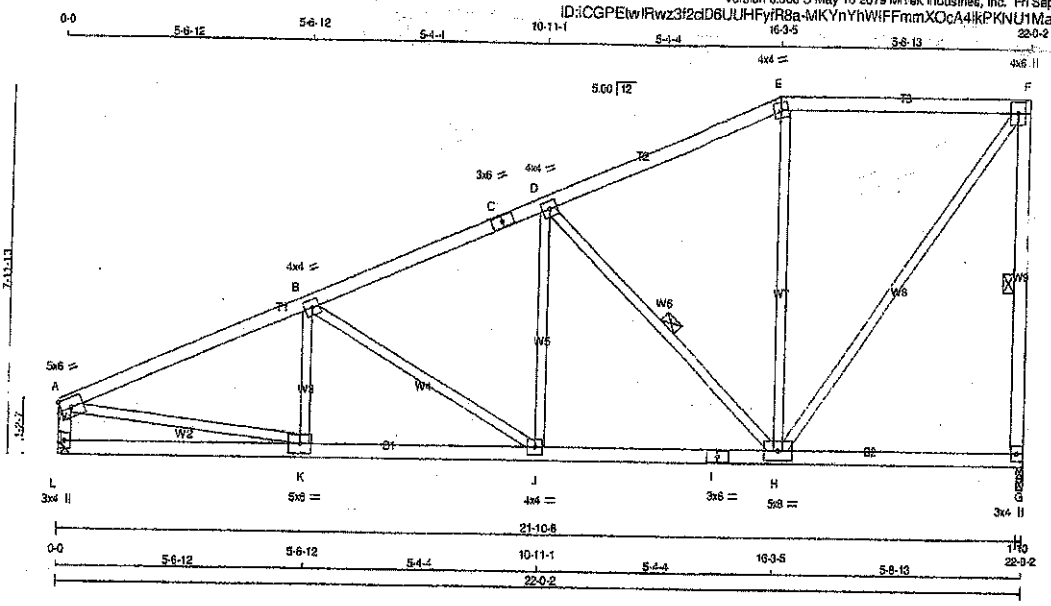

TOTAL WEIGHT = 2 X 98 = 193 lb

MiTek

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - I	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
A	TMVW-t	MT20	5.0	6.0	Edge
B	TMWW-t	MT20	4.0	4.0	
C	TS-t	MT20	3.0	8.0	
D	TMWW-t	MT20	4.0	4.0	
E	TTW-m	MT20	4.0	4.0	
F	TMVW+p	MT20	4.0	6.0	
G	BMV1+p	MT20	3.0	4.0	
H	BMWWW-t	MT20	5.0	8.0	
I	BS-t	MT20	3.0	6.0	
J	BMWW-t	MT20	4.0	4.0	
K	BMWW-t	MT20	5.0	6.0	
L	BMV1+p	MT20	3.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
G	1326	0	1326	0	0	1-10 IN-SX	1-10 IN-SX
L	1326	0	1326	0	0	1-10 MECHANICAL	1-10

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT L. MINIMUM BEARING LENGTH AT JOINT L = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0
L	933	638 / 0	0 / 0	0 / 0	0 / 0	295 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.40 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC	FACTORED (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-1945 / 0		-102.0	-102.0	0.42 (1)	4.40
B-C	-1515 / 0		-102.0	-102.0	0.34 (1)	4.97
C-D	-1515 / 0		-102.0	-102.0	0.34 (1)	4.97
D-E	-611 / 0		-102.0	-102.0	0.34 (1)	6.25
E-F	-726 / 0		-102.0	-102.0	0.44 (1)	6.25
G-F	-1294 / 0		0.0	0.0	0.37 (1)	5.69
L-A	-1281 / 0		0.0	0.0	0.13 (1)	7.13
L-K	0 / 0		-18.5	-18.5	0.12 (4)	10.00
K-J	0 / 1814		-18.5	-18.5	0.34 (1)	10.00
J-I	0 / 1398		-18.5	-18.5	0.30 (1)	10.00
I-H	0 / 1398		-18.5	-18.5	0.30 (1)	10.00
H-G	0 / 0		-18.5	-18.5	0.14 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K-B	-185 / 36	0.04 (1)
B-J	-485 / 0	0.34 (1)
J-D	0 / 345	0.08 (1)
D-H	-945 / 0	0.38 (1)
H-E	-208 / 13	0.25 (1)
H-F	0 / 1230	0.28 (1)
A-K	0 / 1837	0.41 (1)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.13")

CSI: TC=0.44/1.00 (E-F:1) , BC=0.34/1.00 (J-K:1) , WB=0.41/1.00 (A-K:1) , SSI=0.24/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.82 (K) (INPUT = 0.90)
JSI METAL = 0.50 (A) (INPUT = 1.00)

Structural component only
DWG# T-1923498

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H11	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss: Burlington

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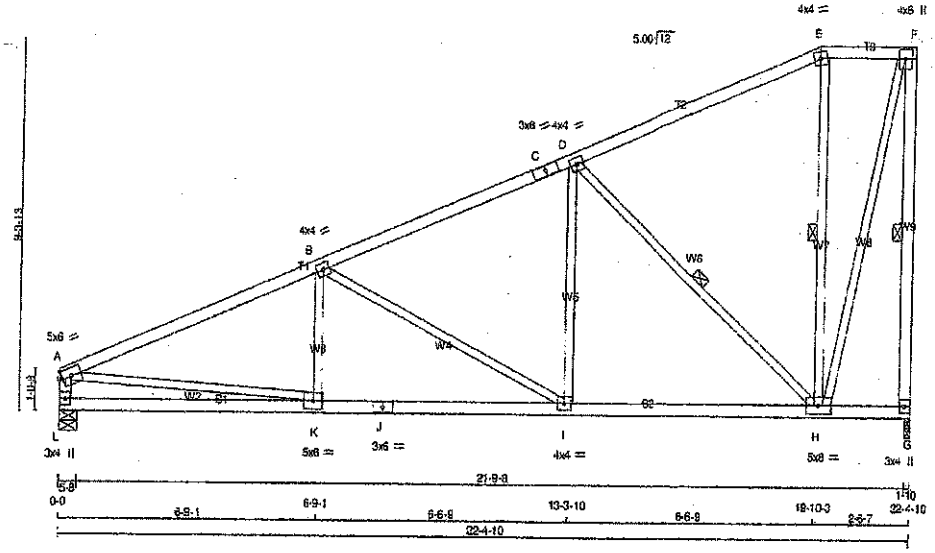

TOTAL WEIGHT = 2 X 104 = 208 lb [M][F]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
E - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
L - A	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - G	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-t	MT20	5.0	6.0		Edge
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	8.0		
D	TMWW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW+p	MT20	4.0	6.0		
G	BMV1+p	MT20	3.0	4.0		
H	BMWWW-t	MT20	5.0	8.0		
I	BMWW-t	MT20	4.0	4.0		
J	BS-t	MT20	3.0	6.0		
K	BMWW-t	MT20	5.0	6.0		
L	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION			INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
G	1349	0	1349	0	0	1-10	1-10
L	1349	0	1349	0	0	5-8	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
G	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
L	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.98 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF F-G, D-H, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-2083 / 0	-102.0	-102.0	0.65 (1)	3.98
B- C	-1341 / 0	-102.0	-102.0	0.50 (1)	4.97
C- D	-1341 / 0	-102.0	-102.0	0.50 (1)	4.97
D- E	-882 / 0	-102.0	-102.0	0.51 (1)	6.25
E- F	-326 / 0	-102.0	-102.0	0.08 (1)	6.25
G- F	-1340 / 0	0.0	0.0	0.55 (1)	5.60
L- A	-1295 / 0	0.0	0.0	0.13 (1)	7.10
L- K	0 / 0	-18.5	-18.5	0.18 (4)	10.00
K- J	0 / 1927	-18.5	-18.5	0.40 (1)	10.00
J- I	0 / 1927	-18.5	-18.5	0.40 (1)	10.00
I- H	0 / 1238	-18.5	-18.5	0.29 (1)	10.00
H- G	0 / 0	-18.5	-18.5	0.11 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
K- B	-81 / 87	0.03 (4)
B- I	-764 / 0	0.67 (1)
I- D	0 / 503	0.11 (1)
D- H	-1250 / 0	0.73 (1)
H- E	-261 / 0	0.15 (1)
H- F	0 / 1254	0.28 (1)
A- K	0 / 1939	0.44 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	29.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	0.0	PSF
	DL	=	7.4	PSF
TOTAL LOAD		=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.40/1.00 (I-K:1) , WB=0.87/1.00 (B-I:1) , SSI=0.29/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.56 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923499

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H12	6	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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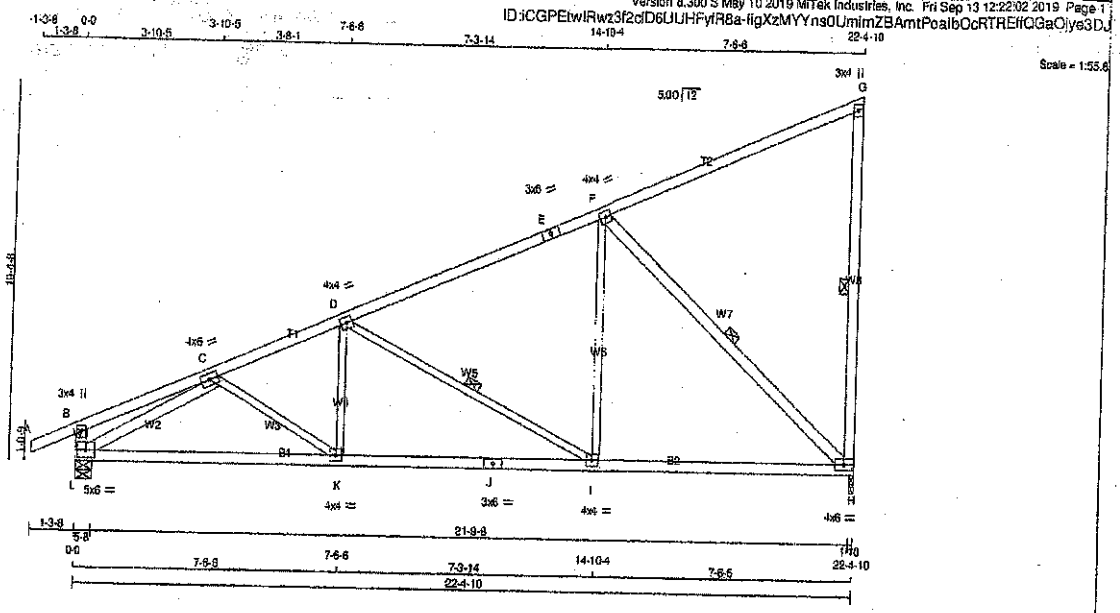

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
F - H	2x4	DRY	No.2	SPF
L - C	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN Y	X
B	TMV+p	MT20	3.0	4.0	
C	TMWW-t	MT20	4.0	6.0	
D	TMWW-t	MT20	4.0	4.0	
E	TS-t	MT20	3.0	6.0	
F	TMWW-t	MT20	4.0	4.0	
G	TMV+p	MT20	3.0	4.0	
H	BMVW1-t	MT20	4.0	6.0	
I	BMWW-t	MT20	4.0	4.0	
J	BS-t	MT20	3.0	6.0	
K	BMWW-t	MT20	4.0	4.0	
L	BMVW1-t	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1349	0	1349	0	0	1-10	1-10
L	1486	0	1486	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	949	649 / 0	0 / 0	0 / 0	0 / 0	300 / 0	0 / 0
L	1044	728 / 0	0 / 0	0 / 0	0 / 0	316 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, L

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.48 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H, D-I, F-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LCT FROM	TO	MAX (PLF) CSI	(LC)	MAX. UNBRAC LENGTH
A-B	0 / 26	-102.0	-102.0	0.13	(1)	10.00
B-C	0 / 8	-102.0	-102.0	0.20	(1)	10.00
C-D	-1982 / 0	-102.0	-102.0	0.34	(1)	4.46
D-E	-1149 / 0	-102.0	-102.0	0.71	(1)	4.81
E-F	-1149 / 0	-102.0	-102.0	0.71	(1)	4.81
F-G	-32 / 0	-102.0	-102.0	0.71	(1)	6.25
H-G	-309 / 0	0.0	0.0	0.18	(1)	6.25
L-B	-309 / 0	0.0	0.0	0.03	(1)	7.81
L-K	0 / 1826	-18.5	-18.5	0.42	(1)	10.00
K-J	0 / 1829	-18.5	-18.5	0.43	(1)	10.00
J-I	0 / 1829	-18.5	-18.5	0.43	(1)	10.00
I-H	0 / 1076	-18.5	-18.5	0.32	(4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI	(LC)
K-D	0 / 144	0.05	(4)
D-I	-650 / 0	0.40	(1)
I-F	0 / 551	0.12	(1)
F-H	-1465 / 0	0.78	(1)
C-K	0 / 54	0.02	(4)
L-C	-2141 / 0	0.51	(1)

TOTAL WEIGHT = 6 X 10 = 605 lb
[M][F]

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.15")

CSI: TC=0.71/1.00 (F-G:1) , BC=0.43/1.00 (I-K:1) , WB=0.78/1.00 (F-H:1) , SSI=0.34/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (F) (INPUT = 0.90)
JSI METAL= 0.59 (J) (INPUT = 1.00)

Structural component only
DWG# T-1923500

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H14	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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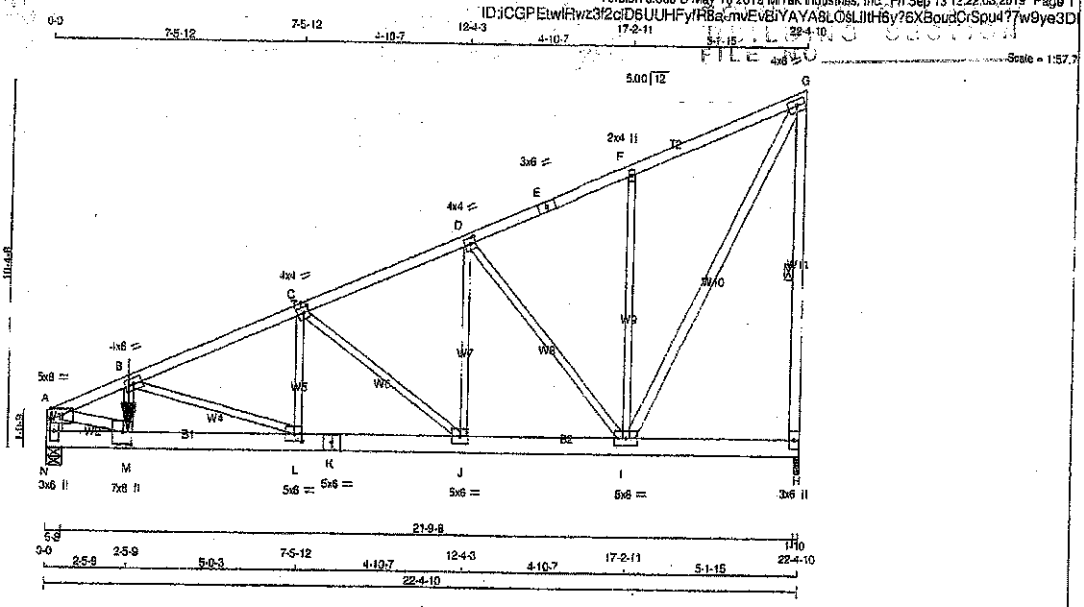

TOTAL WEIGHT = 4 X 123 = 494 lb [M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
N - A	2x6	DRY	No.2	SPF
N - K	2x6	DRY	No.2	SPF
K - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
M - B	2x4	DRY	No.2	SPF
I - G	2x4	DRY	No.2	SPF
A - M	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS		SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-E	1	12	TOP
E-G	1	12	TOP
G-H	1	12	TOP
N-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
N-K	2	4	SIDE(671.4)
K-H	2	12	TOP
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	
B-M	2	2	SIDE(466.0)
2x4	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	5.0	8.0	Edge	
B	TMWW-t	MT20	4.0	6.0		
C	TMWW-t	MT20	4.0	4.0	2.00	1.75
D	TMWW-t	MT20	4.0	4.0	2.00	1.75
E	TS-t	MT20	3.0	6.0		
F	TMW-sw	MT20	2.0	4.0		
G	TMWW-t	MT20	4.0	8.0		
H	BMV1-p	MT20	3.0	8.0		
I	BMWWW-t	MT20	5.0	8.0		
J	BMWW-t	MT20	5.0	8.0		
K	BS-t	MT20	5.0	6.0		
L	BMWW-t	MT20	5.0	6.0	2.50	2.75
M	BMWW-t	MT20	7.0	8.0	4.25	3.00
N	BMV1-p	MT20	3.0	6.0	3.25	1.50

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
H	1629	0	1629	0	0	1-10	1-10
N	5229	0	5229	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
H	1266	884 / 0	0 / 0	0 / 0	0 / 0	402 / 0	0 / 0
N	3674	2549 / 0	0 / 0	0 / 0	0 / 0	1125 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, N

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.39 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF G-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-7647 / 0	-102.0	-102.0	1	0.27 (1)	3.39
B-C	-4073 / 0	-102.0	-102.0	1	0.29 (1)	4.46
C-D	-2323 / 0	-102.0	-102.0	1	0.22 (1)	5.63
D-E	-1116 / 0	-102.0	-102.0	1	0.21 (1)	6.25
E-F	-1116 / 0	-102.0	-102.0	1	0.21 (1)	6.25
F-G	-1139 / 0	-102.0	-102.0	1	0.21 (1)	6.25
H-G	-1784 / 0	0.0	0.0	1	0.46 (1)	6.25
N-A	-4952 / 0	0.0	0.0	1	0.17 (1)	6.53
N-M	0 / 0	-18.5	-18.5	1	0.23 (1)	10.00
M-L	0 / 7062	-18.5	-18.5	1	0.69 (1)	10.00
L-K	0 / 3776	-18.5	-18.5	1	0.30 (1)	10.00
K-J	0 / 3776	-18.5	-18.5	1	0.30 (1)	10.00
J-I	0 / 2139	-18.5	-18.5	1	0.16 (1)	10.00
I-H	0 / 0	-18.5	-18.5	1	0.03 (4)	10.00

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
M-B	0 / 2131	0.19 (1)
B-L	-3466 / 0	0.83 (1)
L-C	0 / 1385	0.17 (1)
C-J	-2072 / 0	0.69 (1)
J-D	0 / 1314	0.16 (1)
D-I	-1713 / 0	0.97 (1)
I-F	-582 / 0	0.37 (1)
I-G	0 / 2251	0.20 (1)
A-M	0 / 7310	0.85 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE
M	2-5-9	-4360	-4360	---	BACK	VERT	TOTAL

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.75")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.75")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.14")

CSI: TC=0.46/1.00 (G-H:1) , BC=0.69/1.00 (L-M:1) , WB=0.97/1.00 (D-I:1) , SSI=0.13/1.00 (F-G:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (C) (INPUT = 0.90)
JSI METAL= 0.91 (M) (INPUT = 1.00)

LICENSEE PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923501

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H15	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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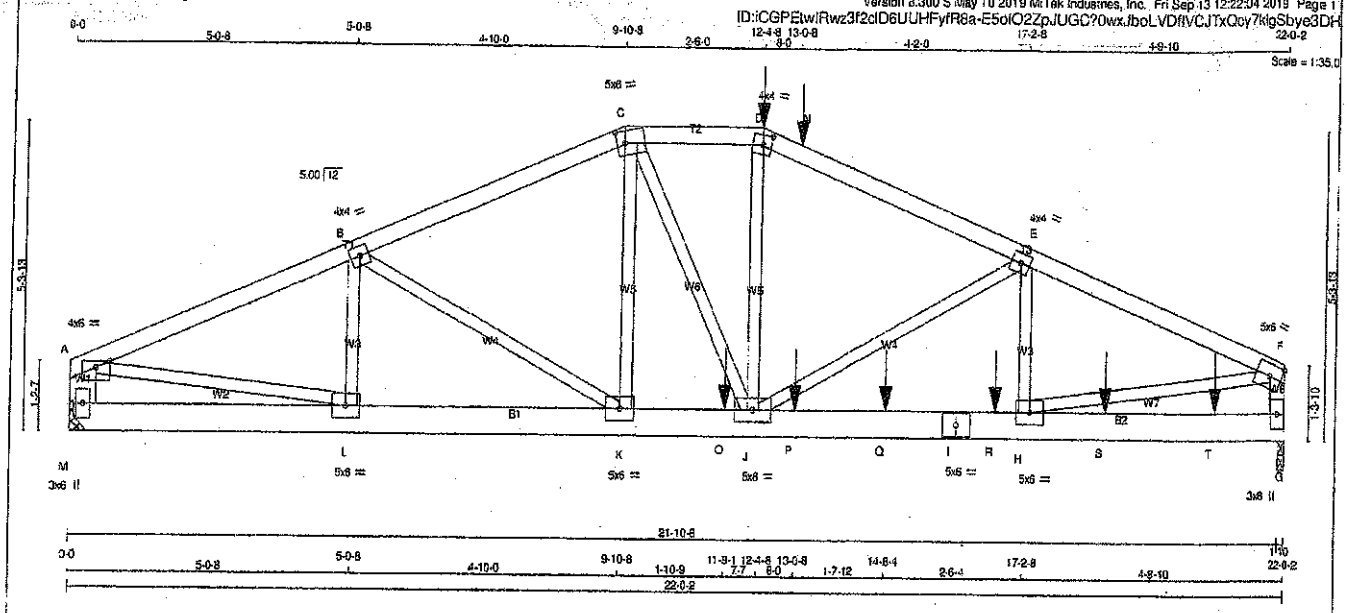

TOTAL WEIGHT = 4 X 100 = 401 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF
M - A	2x6	DRY	No.2	SPF
M - I	2x6	DRY	No.2	SPF
I - G	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	1	12	TOP
C-D	1	12	SIDE(61.0)
D-F	1	12	SIDE(61.0)
G-F	1	12	TOP
M-A	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-I	2	12	SIDE(0.0)
I-G	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
J-D	1	6	SIDE(66.5)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMWW-p	MT20	4.0	6.0	Edge	
B	TMWW-t	MT20	4.0	4.0		
C	TTWW-m	MT20	5.0	6.0	2.50	1.75
D	TTW-m	MT20	4.0	4.0	3.00	1.75
E	TMWW-t	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0		Edge
G	BMV1+p	MT20	3.0	6.0		
H, K, L						
H	BMWW-t	MT20	5.0	6.0		
I	BS-t	MT20	5.0	6.0		
J	BMWWW-t	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	2256	0	2256	0	0	MECHANICAL	
G	2527	0	2527	0	0	1-10	1-10

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT M MINIMUM BEARING LENGTH AT JOINT M = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	1582	1115 / 0	0 / 0	0 / 0	0 / 0	467 / 0	0 / 0
G	1777	1224 / 0	0 / 0	0 / 0	0 / 0	553 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-3680 / 0	-102.0	-102.0	0.29 (1)	4.64	
B-C	-3643 / 0	-102.0	-102.0	0.28 (1)	4.66	
C-D	-3684 / 0	-102.0	-102.0	0.10 (1)	4.83	
D-N	-3989 / 0	-102.0	-102.0	0.30 (1)	4.47	
N-E	-3989 / 0	-102.0	-102.0	0.30 (1)	4.47	
E-F	-3984 / 0	-102.0	-102.0	0.30 (1)	4.48	
G-F	-2433 / 0	0.0	0.0	0.13 (1)	7.23	
M-A	-2184 / 0	0.0	0.0	0.08 (1)	7.81	
M-L	0 / 0	-18.5	-18.5	0.05 (1)	10.00	
L-K	0 / 3423	-18.5	-18.5	0.27 (1)	10.00	
K-O	0 / 3349	-18.5	-18.5	0.33 (1)	10.00	
O-J	0 / 3349	-18.5	-18.5	0.33 (1)	10.00	
J-P	0 / 3698	-18.5	-18.5	0.29 (1)	10.00	
P-Q	0 / 3698	-18.5	-18.5	0.29 (1)	10.00	
Q-I	0 / 3698	-18.5	-18.5	0.29 (1)	10.00	
I-R	0 / 3698	-18.5	-18.5	0.29 (1)	10.00	
R-H	0 / 3698	-18.5	-18.5	0.29 (1)	10.00	
H-S	0 / 0	-18.5	-18.5	0.06 (1)	10.00	
S-T	0 / 0	-18.5	-18.5	0.06 (1)	10.00	
T-G	0 / 0	-18.5	-18.5	0.06 (1)	10.00	

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-B	-534 / 0	0.05 (1)
B-K	-96 / 0	0.03 (1)
K-C	0 / 389	0.05 (1)
C-J	0 / 787	0.09 (1)
J-D	0 / 262	0.04 (4)
J-E	-80 / 0	0.03 (4)
H-E	-610 / 0	0.06 (1)
H-F	0 / 3765	0.47 (1)
A-L	0 / 3468	0.43 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	CONN.
D	12-4-8	-125	-140	---	FRONT	VERT	DEAD	C1
D	12-4-8	-713	-713	---	FRONT	VERT	SNOW	C1
N	13-0-8	-140	-140	---	FRONT	VERT	TOTAL	C1
O	11-9-1	-1018	-1018	---	FRONT	VERT	TOTAL	C1
P	13-0-8	-27	-27	---	FRONT	VERT	TOTAL	C1
Q	14-8-4	-27	-27	---	FRONT	VERT	TOTAL	C1
R	16-8-4	-27	-27	---	FRONT	VERT	TOTAL	C1
S	18-8-4	-27	-27	---	FRONT	VERT	TOTAL	C1
T	20-8-4	-27	-27	---	FRONT	VERT	TOTAL	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDT'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.73")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.07")
ALLOWABLE DEFL.(TL)= L/360 (0.73")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.12")

CSI: TC=0.30/1.00 (D-E:1) , BC=0.33/1.00 (J-K:1) , WB=0.47/1.00 (F-H:1) , SSI=0.27/1.00 (J-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	768	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (A) (INPUT = 0.90)
JSI METAL= 0.50 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923502

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H19	12	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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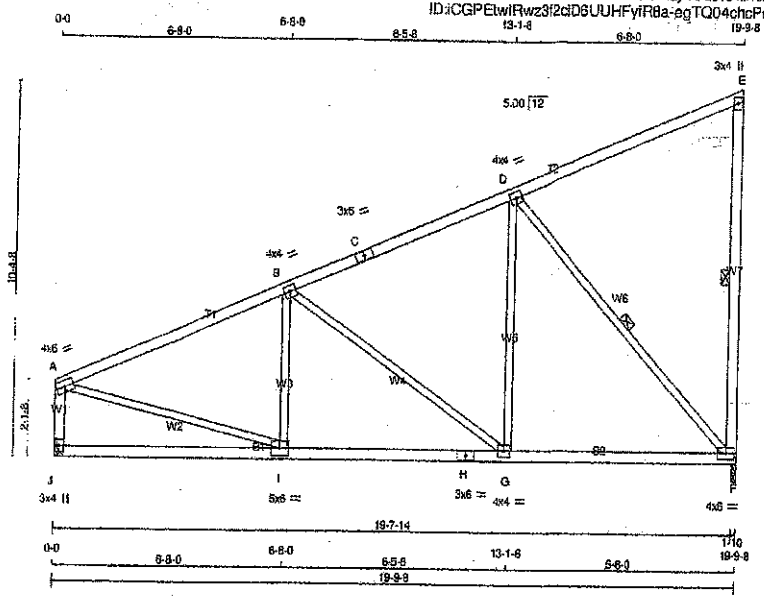

TOTAL WEIGHT = 12 X 89 = 1063 lb [M][P]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
J - A	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-t	MT20	4.0	6.0		
B	TMWW-t	MT20	4.0	4.0		
C	TS-t	MT20	3.0	6.0		
D	TMWW-t	MT20	4.0	4.0		
E	TMV+p	MT20	3.0	4.0		
F	BMWW1-t	MT20	4.0	6.0		
G	BMWW-t	MT20	4.0	4.0		
H	BS-t	MT20	3.0	6.0		
I	BMWW-t	MT20	5.0	6.0		
J	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
F	1193	0	1193	0	0	1-10	1-10
J	1193	0	1193	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT J. MINIMUM BEARING LENGTH AT JOINT J = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	SNOW	MAX./MIN. COMPONENT REACTIONS LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0
J	839	574 / 0	0 / 0	0 / 0	0 / 0	265 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.85 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF E-F, D-F.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	-1333 / 0	-102.0	-102.0	0.57 (1)	4.85	
B- C	-859 / 0	-102.0	-102.0	0.48 (1)	5.90	
C- D	-859 / 0	-102.0	-102.0	0.48 (1)	5.90	
D- E	-25 / 0	-102.0	-102.0	0.52 (1)	6.25	
F- E	-274 / 0	0.0	0.0	0.15 (1)	8.25	
J- A	-1141 / 0	0.0	0.0	0.13 (1)	7.44	
J- I	0 / 0	-18.5	-18.5	0.18 (4)	10.00	
I- H	0 / 1253	-18.5	-18.5	0.30 (1)	10.00	
H- G	0 / 1253	-18.5	-18.5	0.30 (1)	10.00	
G- F	0 / 794	-18.5	-18.5	0.25 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- B	-213 / 42	0.08 (1)
B- G	-564 / 0	0.74 (1)
G- D	0 / 461	0.10 (1)
D- F	-1176 / 0	0.80 (1)
A- I	0 / 1299	0.29 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.66")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.04")
ALLOWABLE DEFL.(TL)= L/360 (0.66")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.57/1.00 (A-B:1) , BC=0.30/1.00 (G-I:1) , WB=0.80/1.00 (D-F:1) , SSI=0.29/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1867	788	1987	1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.74 (D) (INPUT = 0.90)
JSI METAL= 0.39 (H) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923505

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	H26	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington.

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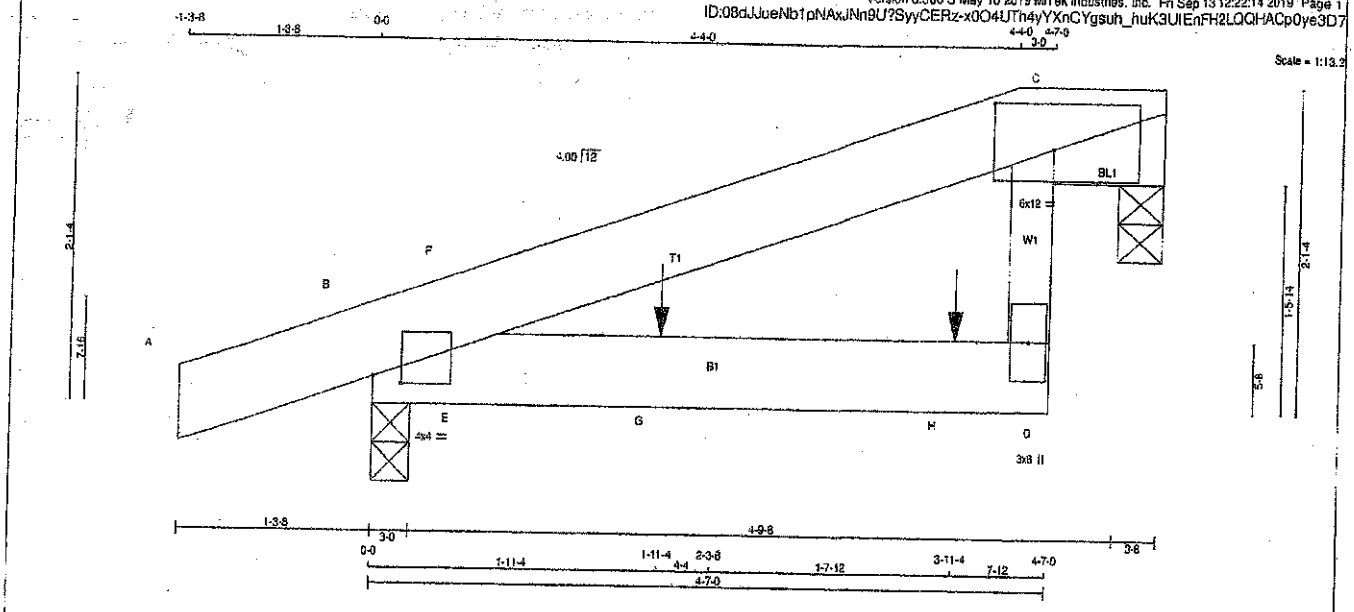


TOTAL WEIGHT = 2 X 24 = 47 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
B - D	2x6	DRY	No.2	SPF

BEARING BLOCKS

BL1	2x6	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	4.0	4.0	0.50	2.26
C	TMVK14-P	MT20	6.0	12.0	3.50	5.00
D	BMV+P	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
C	296	0	296	0	0	3-8	3-8
B	425	0	425	0	0	3-0	3-0

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	211	132 / 0	0 / 0	0 / 0	0 / 0	78 / 0	0 / 0
B	298	211 / 0	0 / 0	0 / 0	0 / 0	87 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) C, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
A-B	0 / 13	-102.0	-102.0	3	0.07 (1)	10.00	E-F	-214 / 29	0.00 (1)
B-F	-31 / 0	-102.0	-102.0	3	0.04 (4)	6.25			
F-C	0 / 3	-102.0	-102.0	3	0.15 (1)	10.00			
D-C	0 / 92	0.0	0.0	3	0.02 (4)	10.00			
B-E	0 / 0	-18.5	-18.5	3	0.11 (1)	10.00			
E-G	0 / 0	-18.5	-18.5	3	0.11 (1)	10.00			
G-H	0 / 0	-18.5	-18.5	3	0.11 (1)	10.00			
H-D	0 / 0	-18.5	-18.5	3	0.11 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	1-11-4	13	-14	---	FRONT	VERT	TOTAL	--	C1
H	3-11-4	17	-17	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.15/1.00 (C-F:1) , BC=0.11/1.00 (D-E:1) , WB=0.00/1.00 (E-F:1) , SSI=0.16/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

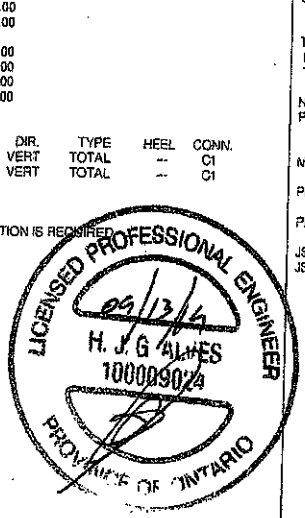
NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.19 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



Structural component only
DWG# T-1923558

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H56	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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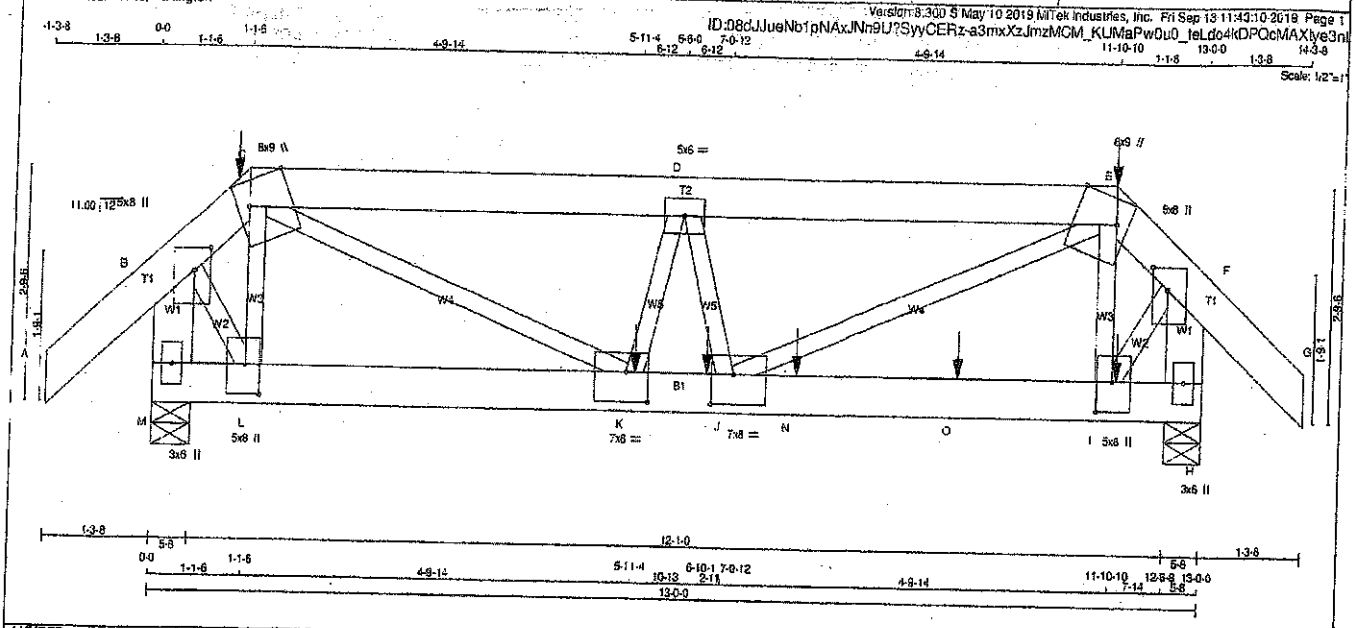

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x6	DRY	No.2	SPF
C - E	2x6	DRY	No.2	SPF
E - G	2x6	DRY	No.2	SPF
M - B	2x6	DRY	No.2	SPF
H - F	2x6	DRY	No.2	SPF
M - H	2x6	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
A-C	2	12	SIDE(122.0)
C-E	2	12	SIDE(80.7)
E-G	2	12	SIDE(122.0)
M-B	2	12	TOP
H-F	2	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
M-H	2	12	SIDE(182.9)
WEBS : (0.122"X3") SPIRAL NAILS			
2x9	1	3	
I-E	1	3	SIDE(285.8)
K-C	1	6	
J-E	1	6	
L-B	1	6	
I-F	1	6	
K-D	1	6	
D-J	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	8.0	3.25	2.25
C	TTWW+m	MT20	8.0	9.0	Edge	6.25
D	TMVW-t	MT20	5.0	6.0		
E	TTWW+m	MT20	8.0	9.0	Edge	6.25
F	TMVW+p	MT20	5.0	8.0	3.25	2.25
H	BMV1+p	MT20	3.0	6.0		
I	BMWW+t	MT20	5.0	8.0	4.25	2.25
J	BMWW-t	MT20	7.0	8.0	4.25	3.25
K	BMWW-t	MT20	7.0	8.0	4.25	3.25
L	BMWW+t	MT20	5.0	8.0	4.25	2.25
M	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
M	3795	0	3795	0	0	5-8	5-8
H	5939	0	5939	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
M	2683	1864 / 0	0 / 0	0 / 0	0 / 0	798 / 0	0 / 0
H	4169	2915 / 0	0 / 0	0 / 0	0 / 0	1253 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M, H

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.83 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 50	-102.0	-102.0	0.04 (1)	10.00
B-C	-2812 / 0	-102.0	-102.0	0.05 (1)	6.25
C-D	-7922 / 0	-103.1	-103.1	0.26 (1)	4.09
D-E	-9058 / 0	-103.1	-103.1	0.29 (1)	3.83
E-F	-4805 / 0	-102.0	-102.0	0.07 (1)	5.24
F-G	0 / 50	-102.0	-102.0	0.04 (1)	10.00
M-B	-4180 / 0	0.0	0.0	0.15 (1)	6.99
H-F	-6995 / 0	0.0	0.0	0.26 (1)	5.65
M-L	0 / 0	-18.7	-18.7	0.18 (1)	10.00
L-K	0 / 1928	-18.7	-18.7	0.33 (1)	10.00
K-J	0 / 5540	-18.7	-18.7	0.75 (1)	10.00
J-N	0 / 3441	-18.7	-18.7	0.70 (1)	10.00
N-O	0 / 3441	-18.7	-18.7	0.70 (1)	10.00
O-I	0 / 3441	-18.7	-18.7	0.70 (1)	10.00
I-H	0 / 0	-18.7	-18.7	0.48 (1)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
L-C	-1314 / 0	0.11 (1)
C-K	0 / 6644	0.82 (1)
J-E	0 / 6226	0.77 (1)
I-E	0 / 234	0.03 (1)
B-L	0 / 2809	0.35 (1)
I-F	0 / 4847	0.60 (1)
K-D	-2402 / 0	0.21 (1)
D-J	0 / 2010	0.25 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR	TYPE	HEIGHT
C	1-1-6	-19	-19	---	FRONT	VERT	TOTAL	
E	11-10-10	-19	-19	---	FRONT	VERT	TOTAL	
I	11-11-4	-1308	-1308	---	FRONT	VERT	TOTAL	
J	6-10-1	-2479	-2479	---	FRONT	VERT	TOTAL	
K	5-11-4	-1417	-1417	---	FRONT	VERT	TOTAL	
N	7-11-4	-1308	-1308	---	FRONT	VERT	TOTAL	
O	9-11-4	-1308	-1308	---	FRONT	VERT	TOTAL	

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. G. ALLOSS
100098024
PROVINCE OF ONTARIO

TOTAL WEIGHT = 4 X 76 = 304 lb (M)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

GIRDER TYPE: CPrimeHip
SIDE SETBACK = 1-1-6
END SETBACK = 2-6-0
END WALL WIDTH = 5-8
CORNER FRAMING TYPE: CONVENTIONAL
END JACK TYPE: CONVENTIONAL
APPLIED TO FRONT SIDE
- ADDT'L LOADS BASED ON 55 % OF GSL.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.43")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.08")
ALLOWABLE DEFL.(TL)= L/360 (0.43")
CALCULATED VERT. DEFL.(TL) = L/ 959 (0.16")

CSI: TC=0.29/1.00 (D-E:1) , BC=0.75/1.00 (J-K:1) , WB=0.82/1.00 (C-K:1) , SSI=0.47/1.00 (I-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1687	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.90 (C) (INPUT = 0.90)
JSI METAL= 0.78 (B) (INPUT = 1.00)

Structural component only
DWG# T-1923470

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H57	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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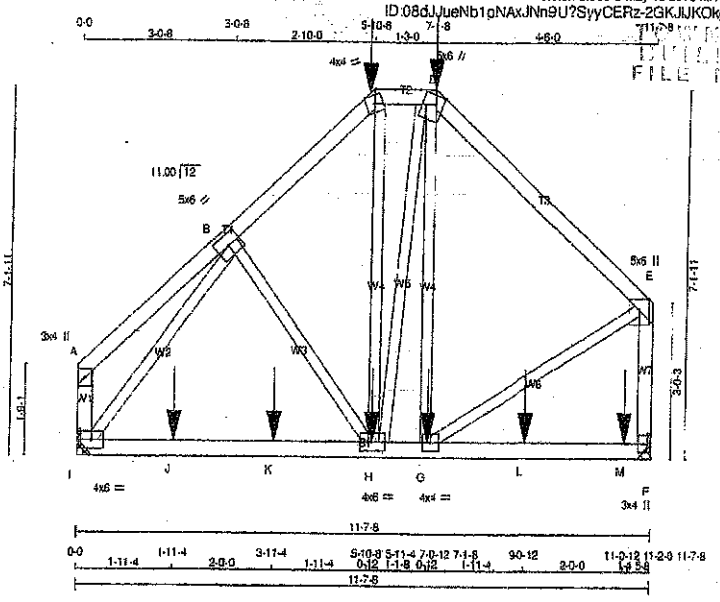

TOTAL WEIGHT = 2 X 64 = 127 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
I - A	2x4	DRY	No.2	SPF
F - E	2x4	DRY	No.2	SPF
I - F	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMV+p	MT20	3.0	4.0		
B	TMWW-t	MT20	5.0	6.0		
C	TTW-m	MT20	4.0	4.0		
D	TTWW+m	MT20	5.0	6.0	2.00	1.75
E	TMVW+p	MT20	5.0	6.0	Edge	
F	BMV1+p	MT20	3.0	4.0		
G	BMWW-t	MT20	4.0	4.0		
H	BMWWW-t	MT20	4.0	6.0	2.00	2.00
I	BMW1-t	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	RECMD BRG
F	1435	0	1435	0	0	IN-SX	IN-SX
I	1270	0	1270	0	0	MECHANICAL	

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F, I. MINIMUM BEARING LENGTH AT JOINT F = 3-8, JOINT I = 3-8,

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	1012	679 / 0	0 / 0	0 / 0	0 / 0	333 / 0	0 / 0
I	896	603 / 0	0 / 0	0 / 0	0 / 0	293 / 0	0 / 0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.57 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (1)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	0 / 22	-102.0	-102.0	0.14 (1)	10.00	
B-C	-1072 / 0	-102.0	-102.0	0.16 (1)	5.85	
C-D	-771 / 0	-102.0	-102.0	0.04 (1)	6.25	
D-E	-1021 / 0	-102.0	-102.0	0.44 (1)	5.57	
I-A	-115 / 0	0.0	0.0	0.01 (1)	7.81	
F-E	-1358 / 0	0.0	0.0	0.24 (1)	6.93	
I-J	0 / 769	-18.5	-18.5	0.37 (4)	10.00	
J-K	0 / 769	-18.5	-18.5	0.37 (4)	10.00	
K-H	0 / 769	-18.5	-18.5	0.37 (4)	10.00	
H-G	0 / 745	-18.5	-18.5	0.37 (4)	10.00	
G-L	0 / 0	-18.5	-18.5	0.17 (4)	10.00	
L-M	0 / 0	-18.5	-18.5	0.17 (4)	10.00	
M-F	0 / 0	-18.5	-18.5	0.17 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B-H	0 / 35	0.01 (4)
H-C	0 / 100	0.04 (4)
H-D	0 / 157	0.04 (4)
G-D	-335 / 0	0.30 (1)
I-B	-1326 / 0	0.55 (1)
G-E	0 / 888	0.21 (1)

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	5-10-8	-506	-506	---	FRONT	VERT	TOTAL	--	C1
D	7-1-8	-55	-82	---	FRONT	VERT	DEAD	--	C1
D	7-1-8	-205	-205	---	FRONT	VERT	TOTAL	--	C1
D	7-1-8	-320	-320	---	FRONT	VERT	SNOW	--	C1
G	7-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
H	5-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
J	1-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
K	3-11-4	-36	-36	---	FRONT	VERT	TOTAL	--	C1
L	9-0-12	-36	-36	---	FRONT	VERT	TOTAL	--	C1
M	11-0-12	-40	-40	---	FRONT	VERT	TOTAL	--	C1

CONNECTION REQUIREMENTS

1) C1: A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED.

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.39")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.39")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.08")

CSI: TC=0.44/1.00 (D-E;1) , BC=0.37/1.00 (G-H;4) , WB=0.55/1.00 (B-I;1) , SSI=0.16/1.00 (H-I;4)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	615	354	1867	786	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (G) (INPUT = 0.90)
JSI METAL= 0.37 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J G ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923471

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H60	2	1	GREEN PARK HOMES TRUSS DESC.	

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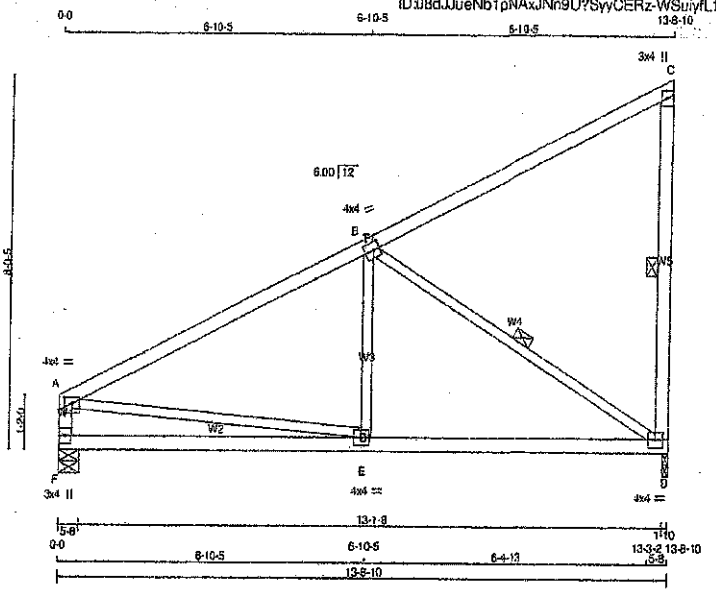


TOTAL WEIGHT = 2 X 57 = 115 lb

(M)(F)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
D - C	2x4	DRY	No.2	SPF
F - A	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	4.0	4.0	1.50	2.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.75
C	TMV+p	MT20	3.0	4.0		
D	BMWW1-t	MT20	4.0	4.0		
E	BMWW-t	MT20	4.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	827	0	827	0	0	1-10	1-10
F	827	0	827	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
D	582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0
F	582	398 / 0	0 / 0	0 / 0	0 / 0	184 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.46 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/ 2 LENGTH OF C-D, B-D.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

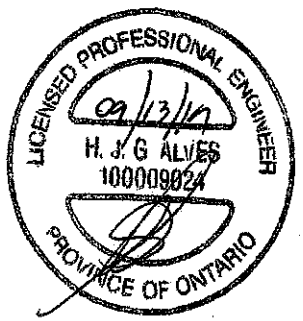
LOADING
TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	MAX CSI (LC)	MAX. UNBRAC LENGTH
A-B	-880 / 0	-102.0	-102.0	0.65 (1)	5.46
B-C	-36 / 0	-102.0	-102.0	0.63 (1)	6.25
D-C	-264 / 0	0.0	0.0	0.08 (1)	6.25
F-A	-777 / 0	0.0	0.0	0.08 (1)	7.81
F-E	0 / 0	-18.5	-18.5	0.25 (4)	10.00
E-D	0 / 821	-18.5	-18.5	0.31 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	0 / 144	0.05 (4)
B-D	-969 / 0	0.41 (1)
A-E	0 / 828	0.19 (1)



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.02")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.07")

CSI: TC=0.65/1.00 (A-B:1) , BC=0.31/1.00 (D-E:4) , WB=0.41/1.00 (B-D:1) , SSI=0.30/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1356

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.89 (D) (INPUT = 0.90)
JSI METAL= 0.31 (E) (INPUT = 1.00)

Structural component only
DWG# T-1923472

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H60G	2	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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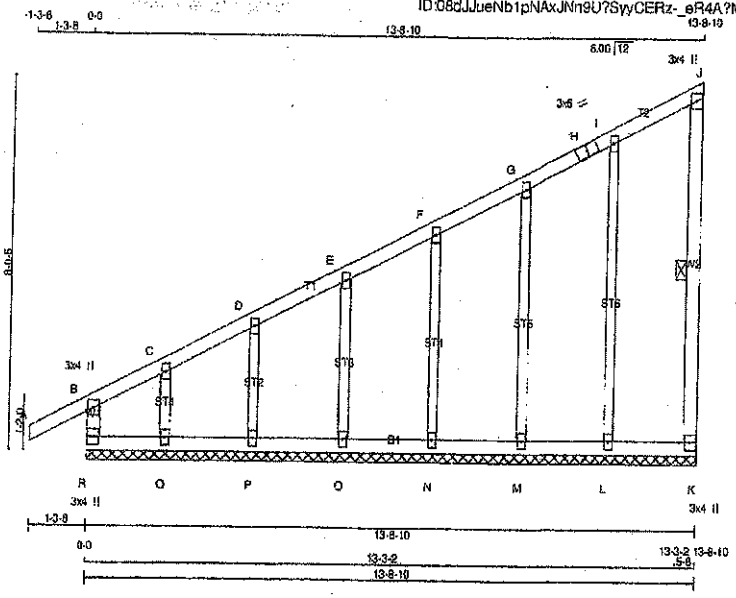

TOTAL WEIGHT = 2 X 64 = 127 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
R - B	2x4	DRY	No.2	SPF
A - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
R - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C, D, E, F, G, I						
C	TMW+w	MT20	2.0	4.0		
H	TS-t	MT20	3.0	6.0		
J	TMV+p	MT20	3.0	4.0		
K	BMV1+p	MT20	3.0	4.0		
L, M, N, O, P, Q						
L	BMW1+w	MT20	2.0	4.0		
R	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF J-K.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO							FR-TO		
R-B	-274 / 0	0.0	0.0	0.04 (1)		7.81	L-I	-221 / 0	0.19 (1)
A-B	0 / 31	-102.0	-102.0	0.13 (1)		10.00	M-G	-201 / 0	0.11 (1)
B-C	-44 / 0	-102.0	-102.0	0.11 (1)		6.25	N-F	-204 / 0	0.08 (1)
C-D	-15 / 0	-102.0	-102.0	0.05 (1)		6.25	O-E	-199 / 0	0.05 (1)
D-E	-15 / 0	-102.0	-102.0	0.05 (1)		6.25	P-D	-216 / 0	0.04 (1)
E-F	-10 / 0	-102.0	-102.0	0.05 (1)		6.25	Q-C	-132 / 0	0.02 (1)
F-G	-7 / 0	-102.0	-102.0	0.05 (1)		10.00			
G-H	-4 / 0	-102.0	-102.0	0.05 (1)		10.00			
H-I	-4 / 0	-102.0	-102.0	0.05 (1)		10.00			
I-J	-8 / 0	-102.0	-102.0	0.05 (1)		10.00			
K-J	-91 / 0	0.0	0.0	0.02 (1)		6.25			
R-Q	0 / 23	-18.5	-18.5	0.05 (1)		10.00			
Q-P	0 / 18	-18.5	-18.5	0.02 (4)		10.00			
P-O	0 / 13	-18.5	-18.5	0.02 (4)		10.00			
O-N	0 / 9	-18.5	-18.5	0.02 (4)		10.00			
N-M	0 / 6	-18.5	-18.5	0.02 (4)		10.00			
M-L	0 / 4	-18.5	-18.5	0.02 (4)		10.00			
L-K	0 / 2	-18.5	-18.5	0.02 (4)		10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 3012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1) , BC=0.05/1.00 (Q-R:1) , WB=0.19/1.00 (H-L:1) , SSI=0.10/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.13 (I) (INPUT = 0.90)
JSI METAL= 0.09 (I) (INPUT = 1.00)

THIS STRUCTURE MUST BE CONSTRUCTED TO MEET OR EXCEED THE PROVISIONS OF THE ONTARIO BUILDING CODE

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100090924
PROVINCE OF ONTARIO

SEP 16 2019

TOWN OF CALEDON
BUILDING DIVISION
FILE NO.

Structural component only
DWG# T-1923473

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	H61	2	2	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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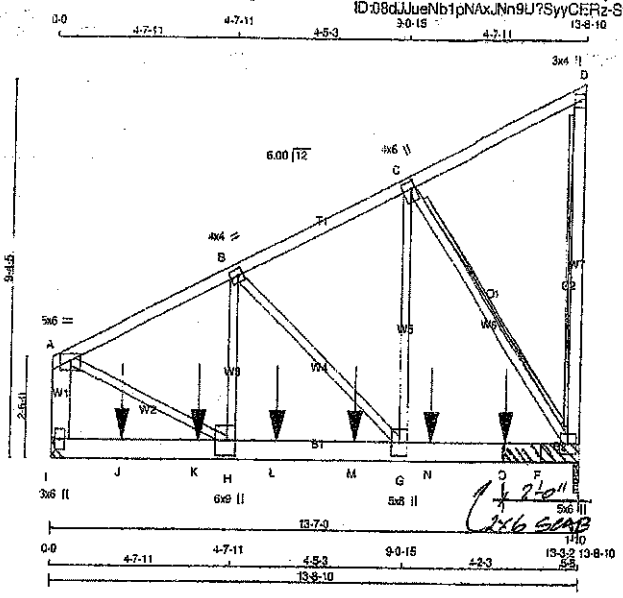


TOTAL WEIGHT = 4 X 80 = 322 lb (M)

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
I - A	2x6	DRY	No.2	SPF
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
I - E	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"X3") SPIRAL NAILS			
I- A	2	12	TOP
A- D	1	12	TOP
D- E	1	12	TOP
BOTTOM CHORDS : (0.122"X3") SPIRAL NAILS			
I-E	2	12	SIDE(0.0)
WEBS : (0.122"X3") SPIRAL NAILS			
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

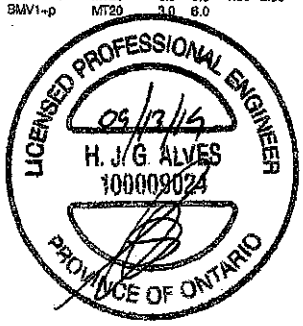
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	2.00	3.00
B	TMWW-t	MT20	4.0	4.0	2.00	1.50
C	TMWW+t	MT20	4.0	6.0	2.25	1.00
D	TMV+p	MT20	3.0	4.0		
E	BMVW1+p	MT20	5.0	6.0		
G	BMWW+t	MT20	5.0	8.0		
H	BMWW+t	MT20	6.0	9.0	4.50	2.50
I	BMV1+p	MT20	3.0	6.0		



DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG	REQRD BRG
I	4379	0	4379	0	0	IN-SX	IN-SX
E	4320	0	4320	0	0	MECHANICAL I-10	I-10 & BLOCK

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT I. MINIMUM BEARING LENGTH AT JOINT I = 4-0.

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
I	3076	2135 / 0	0 / 0	0 / 0	0 / 0	941 / 0	0 / 0
E	3055	2106 / 0	0 / 0	0 / 0	0 / 0	929 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

2x6 DRY SPF No.2 BEARING BLOCK 24" LONG AT JT. E ATTACHED TO FRONT SIDE WITH 3 ROWS OF (0.122"X3") SPIRAL NAILS SPACED 3" C.C. 21 NAILS TOTAL

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.53 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

2x4 DRY SPF No.2 T-BRACE AT D-E
2x6 DRY SPF No.2 T-BRACE AT C-E

FASTEN T AND I-BRACES TO NARROW EDGE OF WEB WITH ONE ROW PER PLY OF 3" COMMON WIRE NAILS @ 6" O.C. WITH 3" MINIMUM END DISTANCE. BRACE MUST COVER 90% OF WEB LENGTH.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
I- A	-3654 / 0	0.0	0.0	0.16 (1)	7.35	A- H	0 / 3972	0.49 (1)	
A- B	-4029 / 0	-102.0	-102.0	0.24 (1)	4.53	H- B	0 / 1318	0.16 (1)	
B- C	-2895 / 0	-102.0	-102.0	0.20 (1)	5.35	B- G	-1698 / 0	0.59 (1)	
C- D	-21 / 0	-102.0	-102.0	0.16 (1)	6.25	G- C	0 / 4143	0.51 (1)	
E- D	-191 / 0	0.0	0.0	0.07 (1)	7.81	C- E	-4218 / 0	0.85 (1)	
I- J	0 / 0	-18.5	-18.5	0.47 (1)	10.00				
J- K	0 / 0	-18.5	-18.5	0.47 (1)	10.00				
K- H	0 / 0	-18.5	-18.5	0.47 (1)	10.00				
H- L	0 / 3817	-18.5	-18.5	0.66 (1)	10.00				
L- M	0 / 3817	-18.5	-18.5	0.66 (1)	10.00				
M- G	0 / 3817	-18.5	-18.5	0.66 (1)	10.00				
G- N	0 / 2415	-18.5	-18.5	0.62 (1)	10.00				
N- O	0 / 2415	-18.5	-18.5	0.62 (1)	10.00				
O- F	0 / 2415	-18.5	-18.5	0.62 (1)	10.00				
F- E	0 / 2415	-18.5	-18.5	0.62 (1)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
J	1-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1
K	3-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1
L	5-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1
M	7-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1
N	9-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1
O	11-9-10	-1174	-1174	---	BACK	VERT	TOTAL	--	C1

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.46")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.05")
ALLOWABLE DEFL.(TL)= L/360 (0.46")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.09")

CSI: TC=0.24/1.00 (A-B:1) , BC=0.66/1.00 (G-H:1) , WB=0.85/1.00 (C-E:1) , SSI=0.58/1.00 (E-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS= 1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1997	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (C) (INPUT = 0.90)
JSI METAL= 0.55 (C) (INPUT = 1.00)

Structural component only
DWG# T-1923474

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J1	22	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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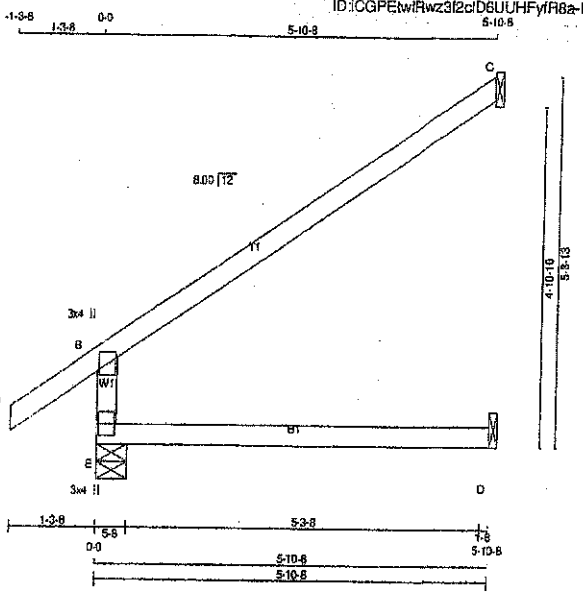

TOTAL WEIGHT = 22 X 18 = 392 lb [M]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (treble is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	578	0	578	0	0	5-8	5-8
C	225	0	225	0	0	1-8	1-8
D	46	0	51	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	404	283 / 0	0 / 0	0 / 0	0 / 0	111 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	38	0 / 0	0 / 0	0 / 0	0 / 0	36 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	WEBS MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E- B	-515 / 0	0.0	0.0	0	0.12 (4)	7.81			
A- B	0 / 39	-102.0	-102.0	0	0.14 (1)	10.00			
B- C	-42 / 0	-102.0	-102.0	0	0.60 (1)	6.25			
E- D	0 / 0	-18.5	-18.5	0	0.13 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.60/1.00 (B-C:1) , BC=0.13/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.24/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1967	788	1967	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.22 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)

Structural component only
DWG# T-1923559

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J3	42	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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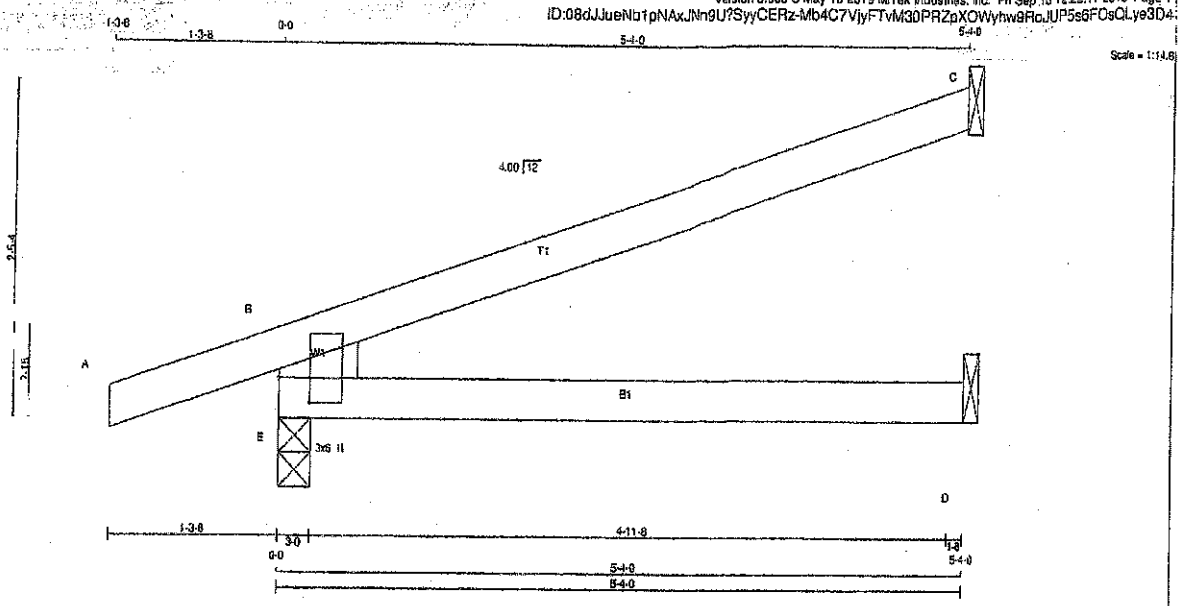

TOTAL WEIGHT = 42 X 15 = 621 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	6.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	538	0	538	0	0	3-0	3-0
C	204	0	204	0	0	1-8	1-8
D	37	0	42	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791N FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	376	271 / 0	0 / 0	0 / 0	0 / 0	105 / 0	0 / 0
C	140	118 / 0	0 / 0	0 / 0	0 / 0	24 / 0	0 / 0
D	30	0 / 0	0 / 0	0 / 0	0 / 0	30 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC	MAX CSI (LC)	MAX. UNBRAC LENGTH	WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-477 / 0	0.0	0.0	0	0.05 (4)	7.81			
A-B	0 / 22	-102.0	-102.0	0	0.13 (1)	10.00			
B-C	-22 / 0	-102.0	-102.0	0	0.49 (1)	8.25			
E-D	0 / 0	-18.5	-18.5	0	0.15 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(TL)= L/360 (0.19")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.02")

CSI: TC=0.49/1.00 (B-C:1) , BC=0.15/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.25/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.24 (E) (INPUT = 0.90)
JSI METAL= 0.09 (E) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G. ALVES
100090024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923560

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405481	J4	12	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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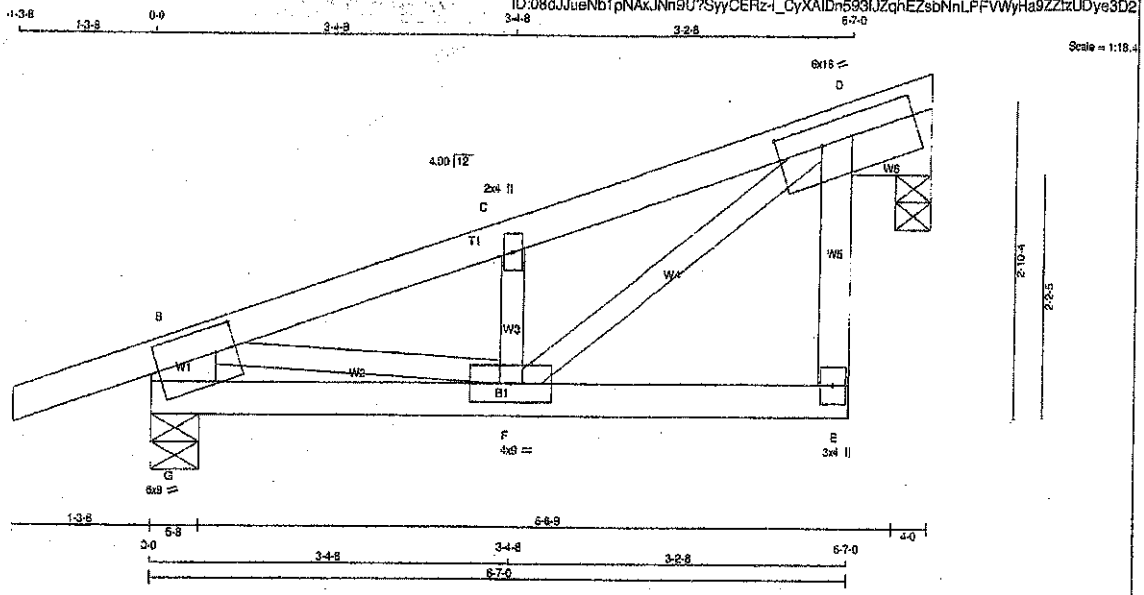

TOTAL WEIGHT = 12 X 29 = 348 lb

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESGR.
A - D	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS EXCEPT	2x3	DRY	No.2	SPF
G - B	2x8	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
C	TMW+w	MT20	2.0	4.0		
D	TMWWW1-t	MT20	6.0	16.0	2.25	5.00
E	BMV+p	MT20	3.0	4.0		
F	BMWWW-t	MT20	4.0	9.0		
G						
G	TMBMWW1-t	MT20	6.0	9.0	2.75	2.25

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
D	379	0	379	0	0	4-0 (3-15)	4-0
G	546	0	546	0	0	5-8	5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
D	268	182 / 0	0 / 0	0 / 0	0 / 0	84 / 0	0 / 0
G	381	277 / 0	0 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) D, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LCT	MAX CSI (LC)	MAX. UNBRAC LENGTH
A- B	0 / 26	-102.0	-102.0	0.19 (1)	10.00	
B- C	-494 / 0	-102.0	-102.0	0.15 (1)	6.25	
C- D	-479 / 0	-102.0	-102.0	0.15 (1)	6.25	
E- D	0 / 29	0.0	0.0	0.01 (4)	10.00	
G- F	0 / 0	-18.5	-18.5	0.04 (4)	10.00	
F- E	0 / 0	-18.5	-18.5	0.04 (4)	10.00	

WEBS MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
G- B	-517 / 0	0.02 (1)
B- F	0 / 484	0.11 (1)
F- C	-377 / 0	0.05 (1)
F- D	0 / 575	0.13 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015 .

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.01")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.01")

CSI: TC=0.19/1.00 (A-B:1) , BC=0.04/1.00 (E-F:4) , WB=0.13/1.00 (D-F:1) , SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.42 (G) (INPUT = 0.90)
JSI METAL= 0.13 (F) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/13/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

Structural component only
DWG# T-1923561

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC. GREEN PARK HOMES	DRWG NO.
405481	J5	10	1	TRUSS DESC.	

Tamarack Roof Truss, Burlington

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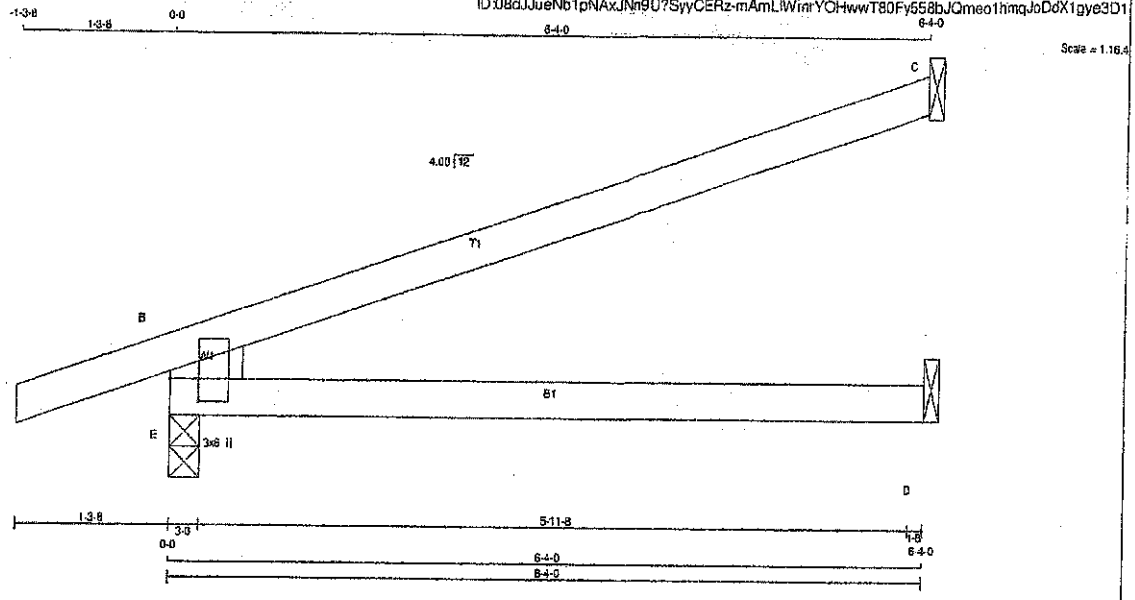

TOTAL WEIGHT = 10 X 17 = 171 lb

[MiTek]

LUMBER
N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
E - B	2x6	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B						
E						
E	TMBMV1+p	MT20	3.0	8.0	2.25	2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
E	613	0	613	0	0	3-0	3-0
C	242	0	242	0	0	1-8	1-8
D	44	0	50	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
E	429	307 / 0	0 / 0	0 / 0	0 / 0	122 / 0	0 / 0
C	166	138 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS MEMB.	MAX. FACTORED FORCE (LBS) FR-TO	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH FR-TO	WEBS MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
E-B	-540 / 0	0.0	0.0	0.05 (4)	7.81			
A-B	0 / 22	-102.0	-102.0	0.13 (1)	10.00			
B-C	-26 / 0	-102.0	-102.0	0.54 (1)	6.25			
E-D	0 / 0	-18.5	-18.5	0.21 (4)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	8.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
-OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.21")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.21")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.03")

CSI: TC=0.54/1.00 (B-C:1) , BC=0.21/1.00 (D-E:4) , WB=0.00/1.00 (n/a:0) , SSI=0.30/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1567	788	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.27 (E) (INPUT = 0.90)
JSI METAL= 0.11 (E) (INPUT = 1.00)

Structural component only
DWG# T-1923562

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
405515	J10	8	1	GREEN PARK HOMES TRUSS DESC.	

Tamarack Roof Truss, Burlington

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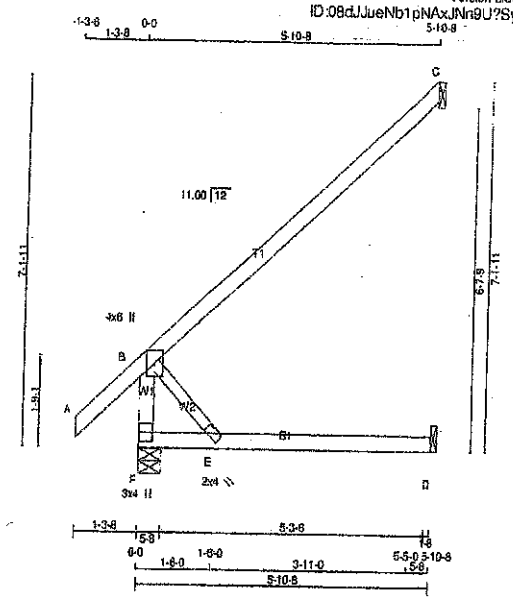


TOTAL WEIGHT = 8 X 21 = 169 lb

[M]

LUMBER

N. L. G. A. RULES

CHORDS	SIZE		LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	2100F 1.8E	SPF
F - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	6.0	Edge	
E	BMW+w	MT20	2.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION VERT	HORZ	MAXIMUM FACTORED GROSS REACTION DOWN	HORZ	UPLIFT	INPUT BRG IN-SX	REQRD BRG IN-SX
F	496	0	496	0	0	5-8	5-8
C	300	0	300	0	0	1-8	1-8
D	54	0	61	0	0	1-8	1-8

SEE MITEK STANDARD DETAIL B97791H FOR CONNECTION TO JOINT(S) C , D

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
F	346	251 / 0	0 / 0	0 / 0	0 / 0	95 / 0	0 / 0
C	206	170 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	43	0 / 0	0 / 0	0 / 0	0 / 0	43 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF) FROM	TO	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH
F- B	-442 / 0	0.0	0.0	0.05 (1)	7.81
A- B	0 / 46	-102.0	-102.0	0.09 (1)	10.00
B- C	0 / 0	-102.0	-102.0	0.40 (1)	10.00
F- E	0 / 0	-18.5	-18.5	0.14 (4)	10.00
E- D	0 / 0	-18.5	-18.5	0.19 (4)	10.00

WEBS

MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
B- E	0 / 0	0.00 (1)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	29.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	0.0	PSF
	DL =	7.4	PSF
TOTAL LOAD	=	42.4	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018 , OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 9.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/ 999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/ 999 (0.05")

CSI: TC=0.40/1.00 (B-C:1) , BC=0.19/1.00 (D-E:4) , WB=0.00/1.00 (B-E:1) , SSI=0.17/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS= 1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT .

NAIL VALUES

PLATE	GRIP(DRY) (PSI) MAX	MIN	SHEAR (PLI) MAX	MIN	SECTION (PLI) MAX	MIN
MT20	618	354	1667	798	1987	1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.12 (B) (INPUT = 1.00)

LICENSED PROFESSIONAL ENGINEER
09/12/19
H. J. G. ALVES
100009024
PROVINCE OF ONTARIO

THIS STRUCTURE MUST BE CONSTRUCTED IN ACCORDANCE WITH PART 9 OF THE ONTARIO BUILDING CODE

Structural component only
DWG# T-1923475

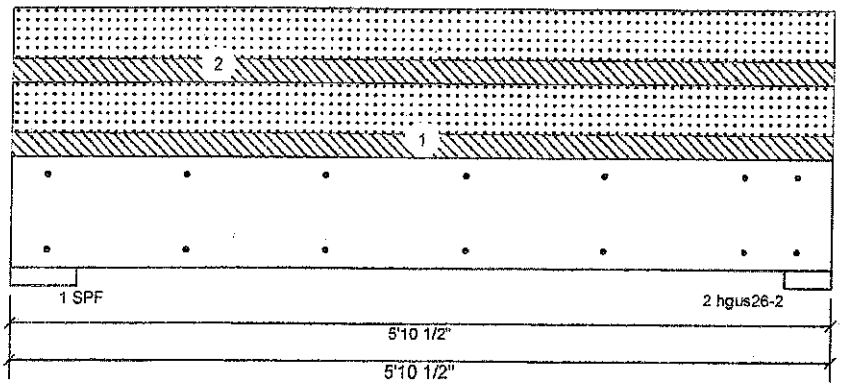
isDesign™

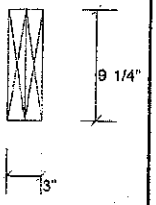
Client:
Project:
Address:

Date: 2/2019
Designer:
Job Name: 405515
Project #:

B1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level





Member Information

Type:	Girder	Application:	Roof (Residential)
Plies:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015 / OBC 2012
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	0	278	602	0
2	0	266	577	0

Bearings and Factored Reactions

Bearing	Length	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	12%	348 / 903	1250	L	1.25D+1.5S
2 - hgus2...	4.000"	16%	333 / 865	1198	L	1.25D+1.5S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1413 ft-lb	3'	6039 ft-lb	0.234 (23%)	1.25D+1.5S	L
Unbraced	1413 ft-lb	3'	5236 ft-lb	0.270 (27%)	1.25D+1.5S	L
Shear	1085 lb	1'2"	3984 lb	0.272 (27%)	1.25D+1.5S	L
LL Defl inch	0.012 (L/5129)	3'	0.174 (L/360)	0.070 (7%)	S	L
TL Defl inch	0.018 (L/3508)	3'	0.174 (L/360)	0.100 (10%)	D+S	L

LICENSED PROFESSIONAL ENGINEER
09/12/19
H.J. G ALVES
100009024
PROVINCE OF ONTARIO

DWG NO. TAM 1923476
STRUCTURAL COMPONENT ONLY 1/2

Design Notes

1 Fasten all plies using 2 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
2 Refer to last page of calculations for fasteners required for specified loads.
3 Girders are designed to be supported on the bottom edge only.
4 Top braced at bearings.
5 Bottom braced at bearings.
6 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		1-0-0	Far Face	13.4 PSF	0 PSF	29 PSF	0 PSF	
2	Uniform		5-11-0	Near Face	13.4 PSF	0 PSF	29 PSF	0 PSF	

This design is valid until 12/11/2021

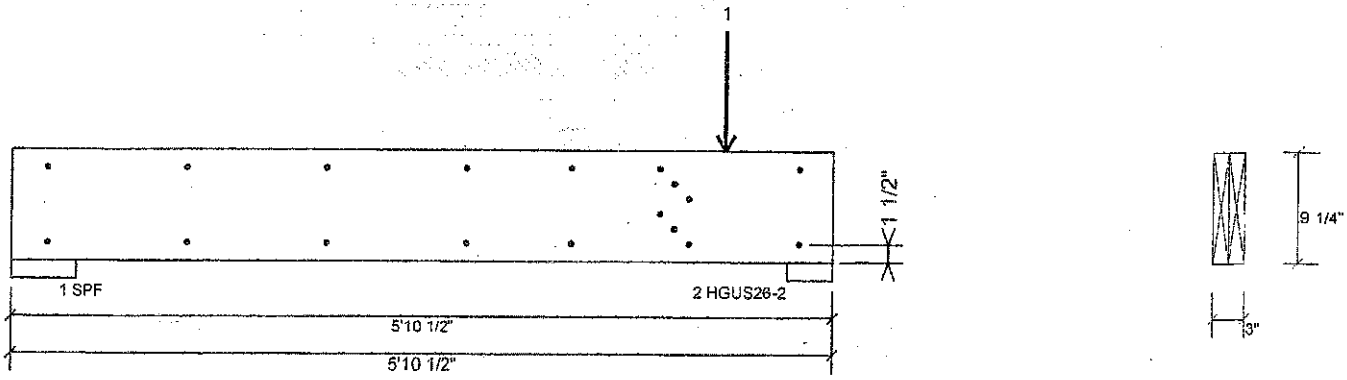
Manufacturer Info

Tamarack Roof Trusses
3255 North Service Road, ON
L7N 3G2
905-335-1115

TAMARACK LUMBER INC
ALPA LUMBER GROUP

B2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. except for regions covered by concentrated load fastening.
Maximum end distance not to exceed 6"

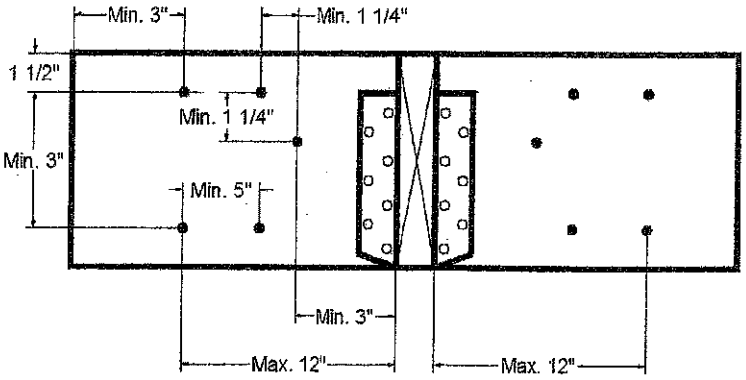
Capacity	34.6 %
Load	88.5 PLF
Yield Limit per Foot	255.6 PLF
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 5-1-4 with a minimum of (6) – 10d Box nails (.128x3") in the pattern shown.

Capacity	77.3 %
Load	592.8lb.
Total Yield Limit	766.9 lb.
Yield Limit per Fastener	127.8 lb.
Yield Mode	g
Load Combination	1.25D+1.5S
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DWG NO. TAM 1192377
STRUCTURAL COMPONENT ONLY

This design is valid until 12/11/2021

Manufacturer Info	Tamarack Roof Trusses 3255 North Service Road, ON L7N 3G2 905-335-1115

LUL/LUS/LJS/HUS/HHUS/HGUS

Standard and Double-Shear Joist Hangers

This product is preferable to similar connectors because of a) easier installation, b) higher capacities, c) lower installed cost, or a combination of these features.

Most hangers in this series have double-shear nailing — an innovation that distributes the load through two points on each joist nail for greater strength. This allows for fewer nails, faster installation, and the use of all common nails for the same connection. (Do not bend or remove tabs)

Double-shear hangers range from the light capacity LUS hangers to the highest capacity HGUS hangers. For medium load truss applications, the HUS offers a lower cost alternative and easier installation than the HGUS hangers, while providing greater load capacity and bearing than the LUS.

Material: See table on pp. 258–259.

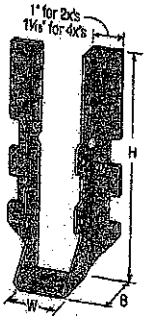
Finish: Galvanized. Some products available in stainless steel or ZMAX® coating; see Corrosion Information, pp. 20–24.

Installation:

- Use all specified fasteners; see General Notes.
- Nails must be driven at an angle through the joist or truss into the header to achieve the tabulated resistances (except LUL).
- Where 16d commons are specified, 10d commons may be used at 0.83 of the tabulated factored resistance.
- Not designed for welded or nailer applications.
- With single ply 2x carrying members, use 10d x 1½" nails into the header and 10d commons into the joist, and reduce the resistance to 0.64 of the table value where 16d nails are specified and 0.77 where 10d nails are specified.

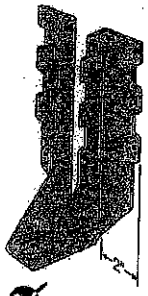
Options:

- LUS, LJS, LUL and HUS hangers cannot be modified.
- Other sizes available; consult your Simpson Strong-Tie representative.
- See Hanger Options information on p. 126.

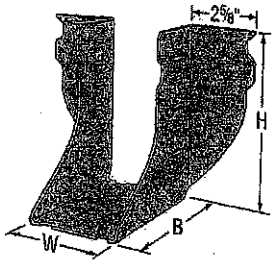


LUS28

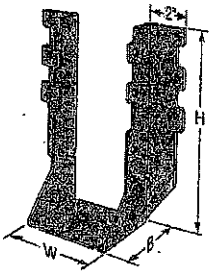
LU26L



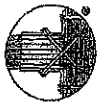
HUS210
(HUS26, HUS28, and HHUS similar)



HGUS28-2



HHUS210-2

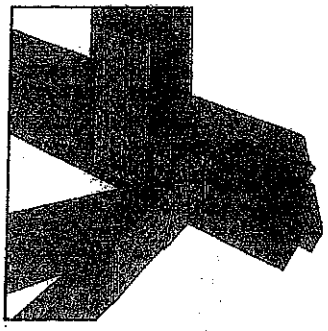


Double-Shear Nailing Top View

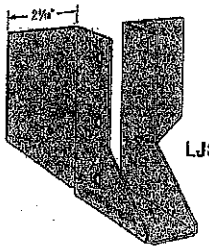
Double-Shear Nailing Side View; Do not bend tab

Dome Double-Shear Nailing Side View *(available on some models)*

U.S. Patent 5,603,580



Typical HUS26 Installation with Reduced Heel Height
(Truss Designer to provide fastener quantity for connecting multiple members together)



LJS26DS

LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

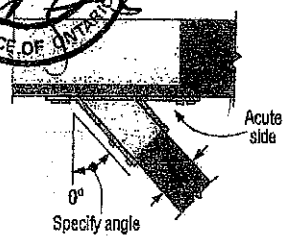
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HGUS Seat Width	Joist	Down Resistance	Uplift
W < 2"	Bevel or square cut	0.62 of table value	0.46 of table value
2" < W < 6"	Bevel cut	0.67 of table value	0.41 of table value
2" < W < 6"	Square cut	0.46 of table value	0.41 of table value
W > 6"	Bevel cut	0.75 of table value	0.41 of table value



Top View HHUS Hanger Skewed Right
(joist must be bevel cut)
All joist nails installed on the outside angle *(non-acute side)*.

Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance D.F./S.-P.-F. Uplift (K_D = 1.15) lb. / kN	Normal (K_D = 1.00) lb. / kN	S-P-F Uplift (K_D = 1.15) lb. / kN	S-P-F Normal (K_D = 1.00) lb. / kN
Single 2x Sizes											
LUS24	18	1⅜	3⅛	1¾	2⅛	(4) 10d	(2) 10d	710 / 3.16	1625 / 7.23	645 / 2.87	1155 / 5.14
LUL24	22	1⅜	3	1⅝	2⅛	(4) 10d	(2) 10d x 1½"	360 / 1.60	1020 / 4.54	320 / 1.42	725 / 3.22
LUL26	22	1⅜	5	1⅝	4⅝	(6) 10d	(4) 10d x 1½"	720 / 3.20	1605 / 7.14	645 / 2.87	1140 / 5.07
LUS26	18	1⅜	4¼	1¾	3⅜	(4) 10d	(4) 10d	1420 / 6.32	2170 / 9.65	1290 / 5.74	1630 / 7.25
HUS26	16	1⅜	5⅜	3	3⅝	(14) 16d	(6) 16d	2705 / 12.03	4940 / 21.97	2065 / 9.20	3875 / 17.24
LJS26DS	18	1⅜	5	3½	4⅝	(16) 16d	(6) 16d	2055 / 9.14	4265 / 18.97	1460 / 6.49	4115 / 18.31
HGUS26	12	1⅜	5⅜	5	4⅛	(20) 16d	(8) 16d	2685 / 11.94	6625 / 29.47	2685 / 11.96	5700 / 25.35
LUL28	20	1⅜	6¾	1⅝	5⅞	(8) 10d	(6) 10d x 1½"	1140 / 5.07	2185 / 9.72	1020 / 4.54	1550 / 6.89
LUS28	18	1⅜	6⅝	1¾	3¾	(6) 10d	(4) 10d	1420 / 6.32	2520 / 11.21	1290 / 5.74	1790 / 7.96
HUS28	16	1⅜	7⅛	3	6⅛	(22) 16d	(8) 16d	3605 / 16.04	5365 / 23.86	2675 / 11.90	4345 / 19.33
HGUS28	12	1⅜	7⅛	5	6⅛	(36) 16d	(12) 16d	3310 / 14.72	7675 / 34.14	3310 / 14.74	6900 / 30.73
LUL210	20	1⅜	8	1⅝	7⅝	(10) 10d	(6) 10d x 1½"	1140 / 5.07	2495 / 11.10	1020 / 4.54	1770 / 7.87
LUS210	18	1⅜	7¹³⁄₁₆	1¾	3¾	(8) 10d	(4) 10d	1420 / 6.32	2785 / 12.39	1290 / 5.74	2210 / 9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-NTOTRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3½" long. See pp. 27–28 for other nail sizes and information.

Face-Mount Hangers

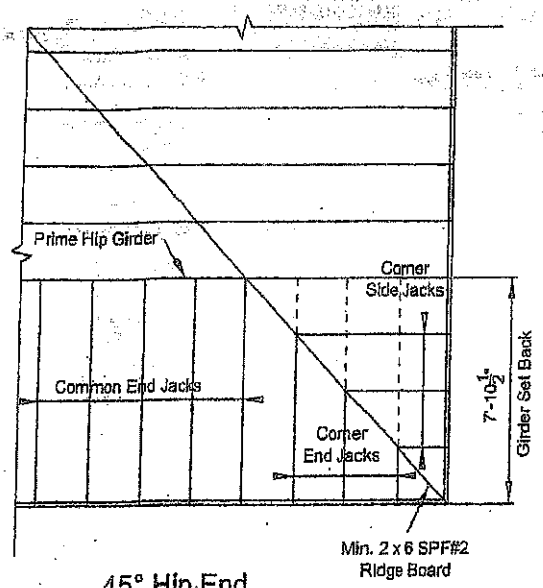
SIMPSON Strong-Tie®

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.) W	H	B	d_e[3]	Fasteners Header	Joist	Factored Resistance C-C-L Uplift (Kp = 1.15)	Normal (Kp = 1.00)	S-P-F Uplift (Kp = 1.15)	Normal (Kp = 1.60)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Double 2x Sizes											
LUS24-2	18	3⅛	3⅛	2	1½	(4) 16d	(2) 16d	835	2020	580	1435
								3.71	8.99	2.62	6.38
SS LUS26-2	18	3⅛	4⅞	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS26-2	14	3⅝	5⅝	3	3⅛	(14) 16d	(6) 16d	2850	7335	2065	5205
								12.68	32.63	9.20	23.15
HGUS26-2	12	3⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
SS LUS28-2	18	3⅛	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS28-2	14	3⅝	7⅝	3	6¼	(22) 16d	(8) 16d	3765	8940	2675	6345
								16.75	39.76	11.90	28.22
HGUS28-2	12	3⅝	7⅞	4	6¼	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
SS LUS210-2	18	3⅛	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HHUS210-2	14	3⅝	9⅛	3	8	(30) 16d	(10) 16d	4670	9660	4235	7000
								20.77	42.97	18.84	31.14
HGUS210-2	12	3⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
Triple 2x Sizes											
HGUS28-3	12	4⅝	5½	4	4⅛	(20) 16d	(0) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-3	12	4⅝	7¼	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-3	14	4⅝	9	3	7⅝	(30) 16d	(10) 16d	4670	9670	4235	6865
								20.77	43.02	18.84	30.54
HGUS210-3	12	4⅝	9¼	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.15	21.60	46.26
Quadruple 2x Sizes											
HGUS26-4	12	6⅝	5⅞	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
HGUS28-4	12	6⅝	7⅛	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
HHUS210-4	14	6⅛	8⅞	3	7⅝	(30) 16d	(10) 16d	4670	10155	4235	7210
								20.77	45.17	18.84	32.07
HGUS210-4	12	6⅝	9⅛	4	8⅛	(46) 16d	(16) 16d	6840	14645	4855	10400
								30.43	65.15	21.60	46.26
HGUS212-4	12	6⅝	10⅝	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS214-4	12	6⅝	12⅝	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.96	32.00	51.80
4x Sizes											
LUS48	18	3⅝	4¼	2	3⅞	(4) 16d	(4) 16d	1720	2595	1545	1920
								7.65	11.54	6.87	8.54
HHUS48	14	3⅝	5¼	3	3⅛	(14) 16d	(6) 16d	2540	7335	2065	5205
								11.30	32.63	9.20	23.15
HGUS48	12	3⅝	5¼	4	4⅛	(20) 16d	(8) 16d	4385	8950	3110	6355
								19.51	39.81	13.83	28.27
LUS48	18	3⅛	6¾	2	3⅞	(6) 16d	(4) 16d	1720	3325	1545	2575
								7.65	14.79	6.87	11.45
HHUS48	14	3⅝	7⅛	3	6⅛	(22) 16d	(8) 16d	3765	8840	2675	6345
								16.75	39.32	11.90	28.22
HGUS48	12	3⅝	7⅞	4	6⅛	(36) 16d	(12) 16d	6070	12980	4310	9215
								27.00	57.74	19.17	40.99
LUS410	18	3⅝	8¾	2	5⅜	(8) 16d	(6) 16d	2580	4500	2320	3195
								11.48	20.02	10.32	14.21
HGUS410	12	3⅝	9	4	8⅛	(46) 16d	(16) 16d	6840	14015	4855	10270
								30.43	62.34	21.60	45.69
HGUS412	12	3⅝	10⅞	4	10⅛	(56) 16d	(20) 16d	7640	14995	5425	10645
								33.98	66.70	24.13	47.35
HGUS414	12	3⅝	12⅞	4	11⅛	(66) 16d	(22) 16d	10130	16400	7195	11645
								45.06	72.96	32.00	51.80

See footnotes on p. 258.



45° Hip End

LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
BOTTOM CHORD : 2 x 4 SPF#2
WEBS : 2 x 3 SPF#2
UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD		50.5 P.S.F.

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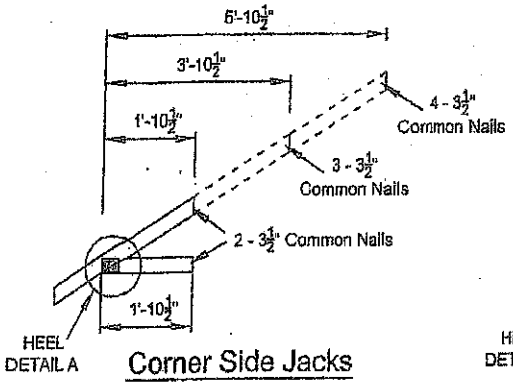
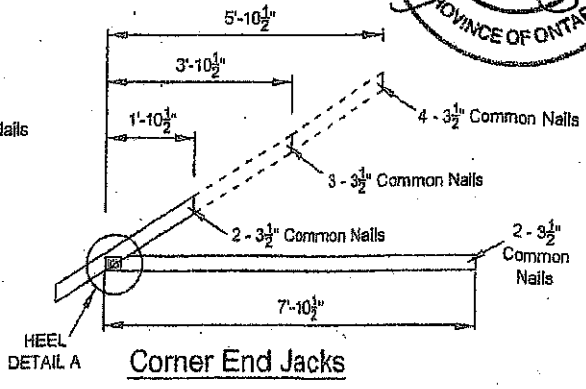
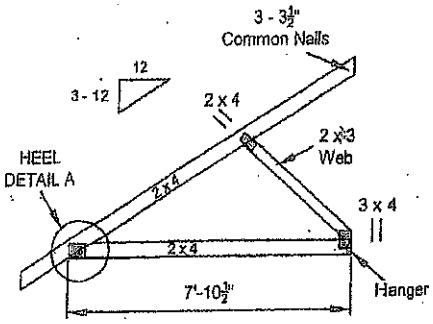
06/11/2018
H.J.G. ALVES
PROVINCE OF ONTARIO



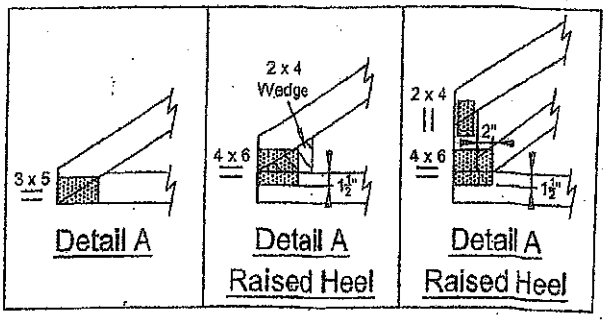
Corner Side Jacks



Corner End Jacks



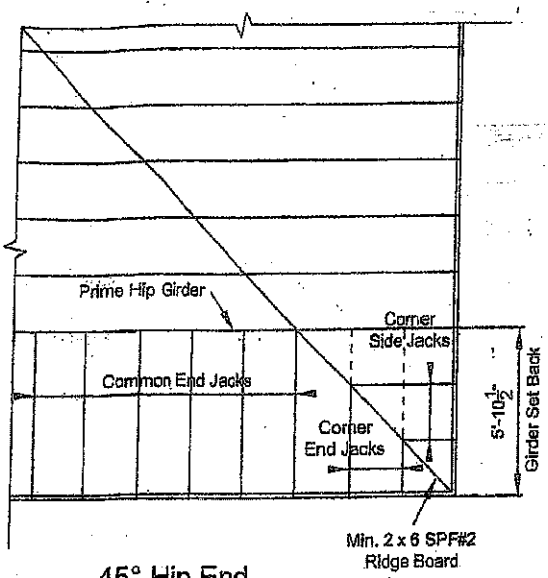
Common End Jacks



Detail A | Detail A Raised Heel | Detail A Raised Heel

NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217



45° Hip End

LUMBER SPECIFICATION

TOP CHORD	:	2 x 4 SPF#2
BOTTOM CHORD	:	2 x 4 SPF#2
WEBS	:	2 x 3 SPF#2

UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD	:	40.5 P.S.F.
TOP CHORD DEAD LOAD	:	3.0 P.S.F.
BOTTOM CHORD LIVE LOAD	:	0.0 P.S.F.
BOTTOM CHORD DEAD LOAD	:	7.0 P.S.F.
TOTAL LOAD	:	50.5 P.S.F

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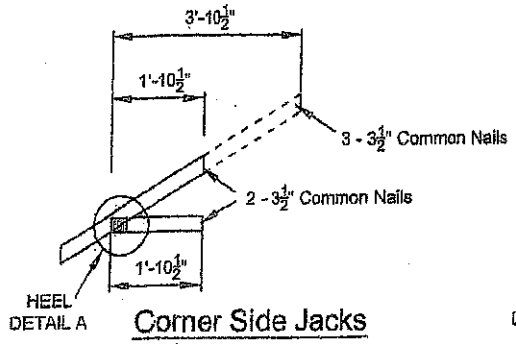
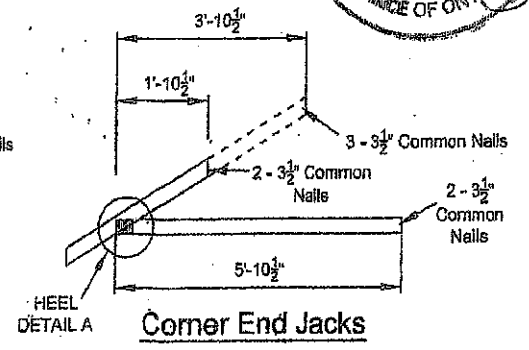
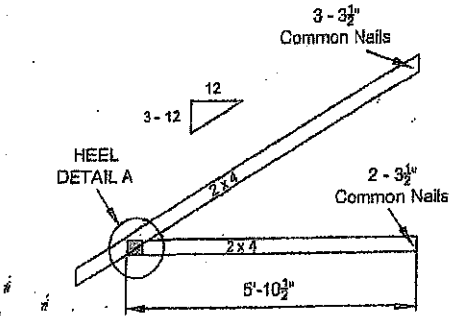
06/11/10
H.J.G. ALVES
PROVINCE OF ONTARIO



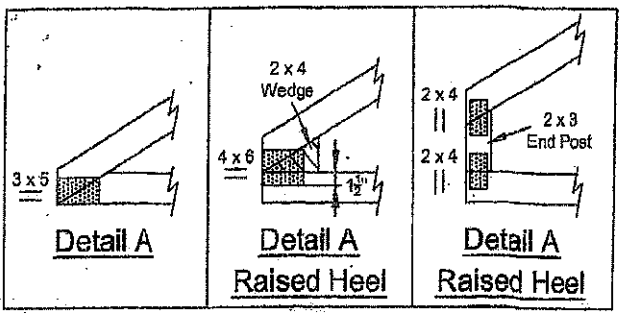
Corner Side Jacks



Corner End Jacks



Common End Jacks

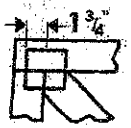


NOTE; DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

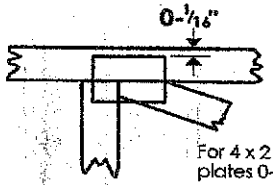
T-1800216

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

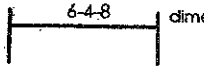
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

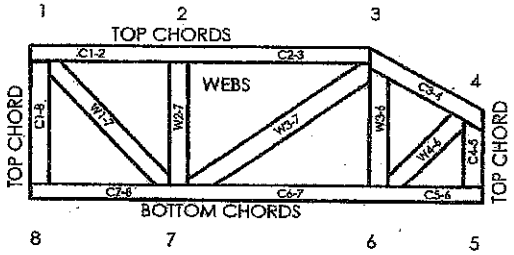
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

MiTek®

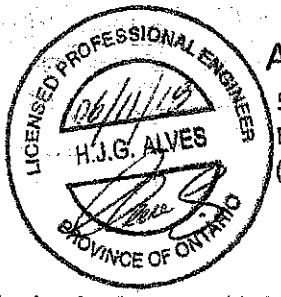
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

⚠ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.
5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components

2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.

3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.

4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.

5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.

2- Lumber is to be the sizes and grade specified on the truss drawing.

3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.

4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings

5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.

6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)

7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.

8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219 Feb 09, 2018