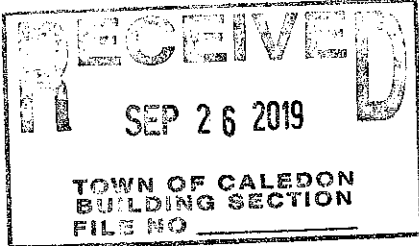


9/12 ROOF PITCH
UNLESS OTHERWISE
SPECIFIED



12 " FINISH O.H
R.T.M.C
2X6 EXTERIOR WALLS
ASPHALT SHINGLES
2X6 FASCIA BOARD

DESIGN LOADS:
SNOW LOAD 1.8 KPA
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

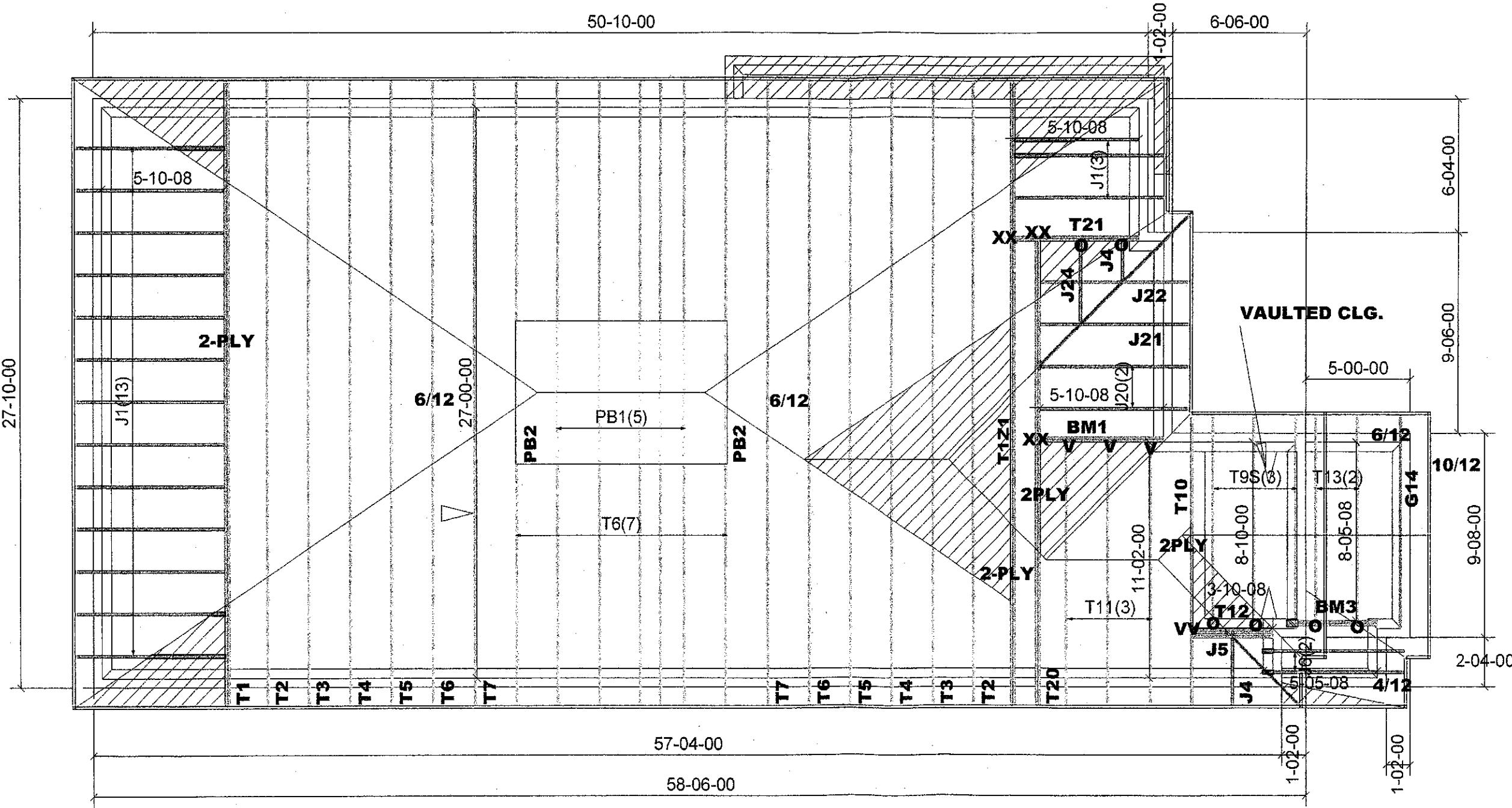
ALL CONVENTIONAL ROOF
FRAMING TO CONFORM TO
PART9 OF THE OBC.LATEST
EDITION ROOF RAFTERS
THAT MEET OR CROSS
OVER TRUSSES ARE TO BE
2"X4" SPF @24"o.c. WITH A
2"X4"SPF VERTICAL POST
TO THE TRUSS UNDER AT
EACH CROSS POINT. POSTS
LONGER THAN 6' TO BE
LATERALLY BRACED SO
THAT THE DISTANCE
BETWEEN END POINTS AND
BETWEEN ROWS OF
BRACING DOES NOT
EXCEED 6'.

TRUSSES DESIGNED
CONFORM TO THE
RELEVANT SECTION OF THE
LATEST EDITION OF THE
ONTARIO BUILDING CODE
(RESIDENTIAL PART 9)

HARDWARE
HGUS26-2(XX)
LUS26-2(VV)
LJS26DS(V) **BM2,4:2-2X10**
LUS24(O)

...
DENOTES
CONVENTIONAL
FRAMING

T- ...
**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



M1224-2



Job Track: **50033**
Plan Log: **201021**
Layout ID: **402980**

Builder / Location:
GREEN PARK HOMES / CALEDON
Project **LAMBERT LANE PH.2**
Date: 5/7/2019 Sales: Mario DiCano Designer: JG

Model / Elevation:
BARTON 4/2 Lot 32

THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC.. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 4
 Lot #: 32
 Elevation: 2

Job Track: 50033
 PlanLog: 201021
 Layout ID: 402980
 Ref #
 Page: 1 of 3
 Date: 05/07/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	279.07 176.67		
	1 2-ply	T1Z1 Hip Girder	9/12	27-00-00	4-01-04	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	254.79 164.67		
	2	T2 Hip	9/12	27-00-00	5-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	221.33 142.00		
	2	T3 Hip	9/12	27-00-00	6-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	242.71 157.33		
	2	T4 Hip	9/12	27-00-00	7-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	245.08 159.00		
	2	T5 Hip	9/12	27-00-00	8-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	259.05 166.67		
	9	T6 Hip	9/12	27-00-00	9-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1166.51 738.00		
	2	T7 Hip	9/12	27-00-00	10-01-04	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	264.11 169.33		
	3	T9S Scissor	9/12 6/12	8-10-00	4-10-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	117.75 79.50		
	1	T10 Hip Girder	9/12	11-02-00	4-05-02	2 x 4 2 x 6	1-05-00 1-03-08	1-06-04 1-06-04	59.88 37.33		
	3	T11 Common	9/12	11-02-00	5-08-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	144.14 95.00		
	1 2-ply	T12 Jack-Closed Girder	9/12	3-10-08	3-01-00	2 x 4 2 x 6		1-06-04 3-01-00	43.25 29.00		
	2	T13 Common	6/12	8-05-08	3-02-03	2 x 4 2 x 6	1-03-08	6-12 9-00	61.74 40.67		
	1	G14 GABLE	6/12	8-10-00	3-02-03	2 x 4	1-03-08 1-03-08	6-12 6-12	28.88 18.83		

SEP 26 2019

TOWN OF CALEDON
 BUILDING SECTION
 FILE NO.



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 4
 Lot #: 32
 Elevation: 2

Job Track: 50033
 PlanLog: 201021
 Layout ID: 402980
 Ref #
 Page: 2 of 3
 Date: 05/07/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T20 Hip Girder	9/12	20-08-00	5-11-02	2 x 4 2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	208.6 134.67		
	1 2-ply	T21 Jack-Closed Girder	6/12	5-10-08	4-01-04	2 x 4 2 x 6		1-02-00 4-01-04	58.52 37.67		
	5	PB1 Piggyback	9/12	6-09-05	2-06-08	2 x 4			79.19 53.33		
	2	PB2 Piggyback	9/12	6-09-05	2-00-00	2 x 4			35.06 25.33		
	16	J1 Jack-Open	6/12	5-10-08	4-01-04	2 x 4	1-03-08	1-02-00 4-01-04	268.71 170.67		
	2	J4 Jack-Open	9/12	1-09-07	2-10-05	2 x 4	1-03-08 1-01	1-06-04 2-10-05	18.63 14.00		
	1	J5 Jack-Open Girder	9/12	3-10-08	2-10-05	2 x 4		1-06-04 4-05-02	9.91 7.00		
	2	J6 GABLE	4/12	5-05-08	2-06-11	2 x 4	1-03-08	3-15 2-01-12	28.9 18.67		
	2	J20 Jack-Open	9/12	5-10-08	5-11-02	2 x 4	1-03-08	1-06-04 5-11-02	38.8 24.00		
	1	J21 Jack-Open	9/12	3-09-07	4-04-05	2 x 4	1-03-08 2-01-01	1-06-04 4-04-05	16.84 11.00		
	1	J22 Jack-Open Girder	9/12	1-09-07	2-10-05	2 x 4	1-03-08 4-01-01	1-06-04 2-10-05	14.15 9.67		
	1	J24 Jack-Open	9/12	1-10-08	4-04-05	2 x 4	1-10-15	1-06-04 2-11-02	10.38 7.00		

TOTAL # TRUSS= 72

TOTAL BFT OF ALL TRUSSES= 2687.01

BFT.

TOTAL WEIGHT OF ALL TRSSES 4173.77 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
3	Hardware	HGUS28-2	
3	Hardware	LJS26DS	

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: BARTON 4
 Lot #: 32
 Elevation: 2

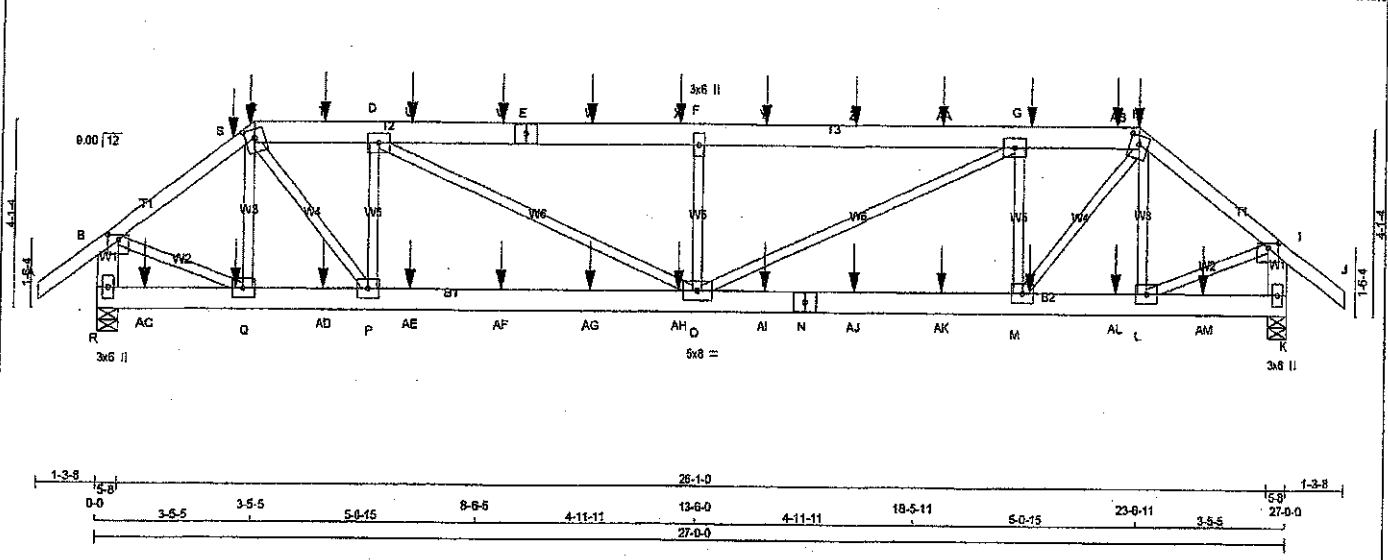
Job Track: 50033
 PlanLog: 201021
 Layout ID: 402980
 Ref #
 Page: 3 of 3
 Date: 05/07/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

HARDWARE

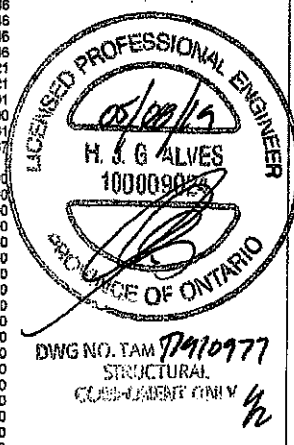
QTY	TYPE	MODEL	LENGTH
6	Hardware	LUS24	
1	Hardware	LUS26-2	

TOTAL NUMBER OF ITEMS= 13

JOB NAME 402979	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:07:58 2019 Page 1			
ID: JIMS6RLJVLhc2snmluv9GyJDZW-5hfhQit_csDdQvFszx87av0Yef6IKFIFzVVTXzIXd					



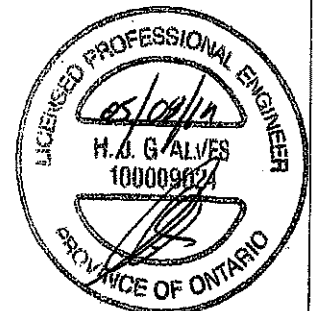
LUMBER										DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER										TOTAL WEIGHT = 2 X 140 = 279 lb									
N. L. G. A. RULES										BEARINGS										DESIGN CRITERIA									
CHORDS										UNFACTORED REACTIONS										*** SPECIAL LOADS ANALYSIS ***									
SIZE										1ST LCASE										GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.									
LUMBER										MAX/MIN. COMPONENT REACTIONS										LOADS WERE DERIVED FROM USER INPUT									
DESCR.										LIVE										NO FURTHER MODIFICATIONS WERE MADE									
SPF										PERM.LIVE										SPECIFIED LOADS:									
SPF										WIND										TOP CH. LL = 20.1 PSF									
SPF										DEAD										DL = 6.0 PSF									
SPF										SOIL										BOT CH. LL = 10.5 PSF									
SPF																				DL = 7.0 PSF									
SPF																				TOTAL LOAD = 52.5 PSF									
SPF																				SPACING = 24.0 IN. C/C									
ALL WEBS 2x3 DRY No.2 SPF										BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, K										LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM									
EXCEPT										BRACING										*** NON STANDARD GIRDER ***									
DRY: SEASONED LUMBER.										TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.46 FT.										ADD'L USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.									
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:										MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.										THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015									
CHORDS #ROWS SURFACE SPACING (IN) LOAD(PLF)										ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.										THIS DESIGN COMPLIES WITH:									
TOP CHORDS: (0.122"x3") SPIRAL NAILS										LOADING										- PART 9 OF CBC 2010, CBC 2012									
A-C 1 12 SIDE(61.0)										TOTAL LOAD CASES: (4)										- CSA 086-09, CSA 086-14									
H-J 1 12 SIDE(61.0)										CHORDS										- TPIC 2011, TPIC 2014									
C-E 2 12 SIDE(61.0)										MEMB. MAX. FACTORED FORCE (LBS)										THIS TRUSS COMPLIES WITH:									
E-H 2 12 SIDE(183.1)										FACTORED										- PART 9 OF CBC 2010, CBC 2012									
R-B 2 12 TOP										VERT. LOAD LC1										- CSA 086-09, CSA 086-14									
K-I 2 12 TOP										MAX. UNBRACED LENGTH										- TPIC 2011, TPIC 2014									
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS										FR-TO										ALLOWABLE DEFL.(LL) = L/360 (0.90")									
R-N 2 12 SIDE(183.1)										FROM TO										CALCULATED VERT. DEFL.(LL) = L/999 (0.11")									
N-K 2 12 SIDE(183.1)										CSI (LC)										ALLOWABLE DEFL.(TL) = L/360 (0.90")									
WEBS: (0.122"x3") SPIRAL NAILS										MEMB. MAX. FACTORED FORCE (LBS)										CALCULATED VERT. DEFL.(TL) = L/999 (0.19")									
2x3 1 6										CSI (LC)										CSI: TC=0.34/1.00 (F-G:1), BC=0.39/1.00 (M-O:1)									
NAILS TO BE DRIVEN FROM ONE SIDE ONLY.										FR-TO										, WB=0.36/1.00 (S-Q:1), SS=0.22/1.00 (D-F:1)									
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.										A-B 0 / 42										DOI LUMBER=1.00 NAIL=1.00 LS BEND=1.00									
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.										B-S -3441 / 0										COMP=1.00 SHEAR=1.00 TENS=1.00									
SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.										S-C -3441 / 0										COMPANION LIVE LOAD FACTOR = 1.00									
PLATES (table in inches)										C-T -4524 / 0										AUTOSOLVE HEELS OFF									
T TYPE PLATES W LEN Y X										T-D -4524 / 0										TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.									
										D-U -6227 / 0										NAIL VALUES									
										U-V -6227 / 0										PLATE GRIP(DRY) SHEAR SECTION									
										V-E -6227 / 0										(PSI) (PLI) (PLI)									
										E-W -6227 / 0										MAX MIN MAX MIN MAX MIN									
										W-X -6227 / 0										MT20 618 354 1987 788 1987 1856									
										X-F -6227 / 0										PLATE PLACEMENT TOL. = 0.250 inches									
										F-Y -6227 / 0										PLATE ROTATION TOL. = 5.0 Deg.									
										Y-Z -6227 / 0										JSI GRIP= 0.85 (L) (INPUT = 0.90)									
										Z-AA -6227 / 0										JSI METAL= 0.49 (N) (INPUT = 1.00)									
										AA-G -6227 / 0																			
										G-AB -4505 / 0																			
										AB-H -4505 / 0																			
										H-I -3429 / 0																			
										I-J 0 / 42																			
										R-B -3334 / 0																			
										K-I -3276 / 0																			



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402979	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:07:58 2019 Page 2
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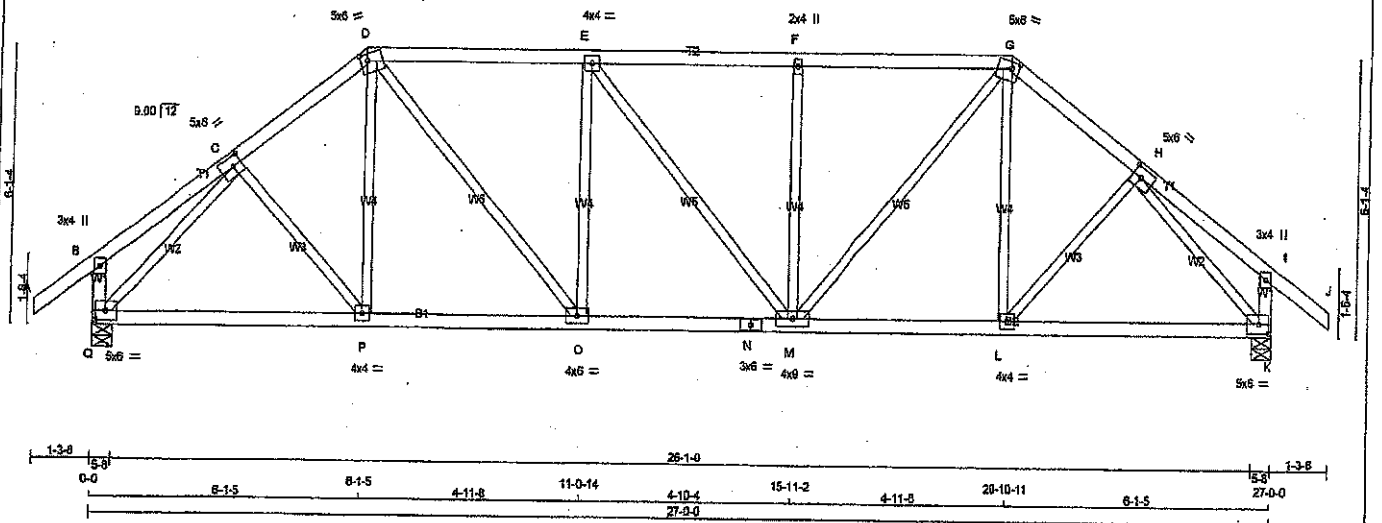
FACTORED CONCENTRATED LOADS (LBS)									
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	3-5-5	-44	-49	---	FRONT	VERT	DEAD	---	---
C	3-5-5	-251	-251	---	FRONT	VERT	SNOW	---	---
G	21-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
H	23-6-11	-44	-49	---	FRONT	VERT	DEAD	---	---
H	23-6-11	-251	-251	---	FRONT	VERT	SNOW	---	---
M	21-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
Q	3-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
S	3-0-12	-155	-155	---	BACK	VERT	TOTAL	---	---
T	5-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
U	7-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
V	9-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
W	11-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
X	13-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
Y	15-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
Z	17-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
AA	19-0-12	-123	-123	---	BACK	VERT	TOTAL	---	---
AB	23-0-12	-149	-149	---	BACK	VERT	TOTAL	---	---
AC	1-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AD	5-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AE	7-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AF	9-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AG	11-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AH	13-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AI	15-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AJ	17-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AK	19-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AL	23-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---
AM	25-0-12	-55	-70	---	BACK	VERT	TOTAL	---	---



DWG NO. TAM 77/10977
STRUCTURAL
CONSULT ONLY 7/2

JOB NAME 402938	TRUSS NAME T3	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:41 2018 Page 1
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 1-3-8 0-0 3-1-15 3-1-15 2-11-7 5-1-5 4-11-8 11-0-14 4-10-4 15-11-2 4-11-8 20-10-11 2-11-7 23-10-1 3-1-15 1-3-8
 Scale = 1/4" = 1'-0"



LUMBER			
N.L.G.A. RULES	SIZE	DRY	NO.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
DRY: SEASONED LUMBER.			

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B TMV+p	MT20	3.0	4.0	
C TMVW-1	MT20	5.0	6.0	2.50 2.50
D TMVW-m	MT20	5.0	6.0	1.75 2.00
E TMVW-t	MT20	4.0	4.0	
F TMVW-w	MT20	2.0	4.0	
G TMVW-m	MT20	5.0	6.0	1.75 2.00
H TMVW-t	MT20	5.0	6.0	2.50 2.50
I TMV+p	MT20	3.0	4.0	
K BMVW-1	MT20	5.0	6.0	2.50 2.75
L BMVW-t	MT20	4.0	4.0	
M BMVW-w	MT20	4.0	4.0	
N BS-1	MT20	3.0	6.0	
O BMVW-t	MT20	4.0	6.0	
P BMVW-t	MT20	4.0	4.0	
Q BMVW-1	MT20	5.0	6.0	2.50 2.75

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
Q	2039	0	2039	0	0
K	2039	0	2039	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Q	1515	864/0	284/0
K	1515	864/0	284/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.17 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	MAX	MEMB.	MAX. FACTORED	MAX
FR-TO	MEMB.	FORCE (LBS)	FROM	TO	(PLF)	CSI (LC)	UNBRAC	LENGTH	FR-TO	FORCE (LBS)
A-B	0/142	-102.1	-102.1	0.14 (1)	10.00	C-P	0/178	0.04 (2)		
B-C	0/118	-102.1	-102.1	0.13 (1)	10.00	P-D	0/205	0.05 (3)		
C-D	-1988/0	-102.1	-102.1	0.18 (1)	4.64	D-O	0/839	0.21 (1)		
D-E	-2184/0	-102.1	-102.1	0.42 (1)	4.17	O-E	-540/0	0.32 (1)		
E-F	-2182/0	-102.1	-102.1	0.42 (1)	4.17	E-M	-3/0	0.00 (1)		
F-G	-2182/0	-102.1	-102.1	0.42 (1)	4.18	M-F	-539/0	0.32 (1)		
G-H	-1988/0	-102.1	-102.1	0.18 (1)	4.63	M-G	0/835	0.21 (1)		
H-I	0/118	-102.1	-102.1	0.13 (1)	10.00	L-G	0/205	0.05 (3)		
I-J	0/142	-102.1	-102.1	0.14 (1)	10.00	L-H	0/178	0.04 (2)		
Q-B	-288/0	0.0	0.0	0.03 (1)	7.81	Q-C	-2228/0	0.75 (1)		
K-I	-288/0	0.0	0.0	0.03 (1)	7.81	H-K	-2228/0	0.75 (1)		
Q-P	0/1486	-38.5	-38.5	0.43 (2)	10.00					
P-O	0/1574	-38.5	-38.5	0.45 (2)	10.00					
O-N	0/2184	-38.5	-38.5	0.43 (1)	10.00					
N-M	0/2184	-38.5	-38.5	0.43 (1)	10.00					
M-L	0/1574	-38.5	-38.5	0.46 (2)	10.00					
L-K	0/1486	-38.5	-38.5	0.44 (2)	10.00					

TOTAL WEIGHT = 2 X 121 = 243 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, CBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/889 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/889 (0.15")

CSI: TC=0.42/1.00 (D-E-1), BC=0.46/1.00 (L-M-2),
WB=0.75/1.00 (H-K-1), SSI=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

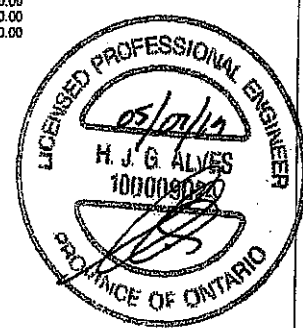
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 518 354 1687 768 1987 1686

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

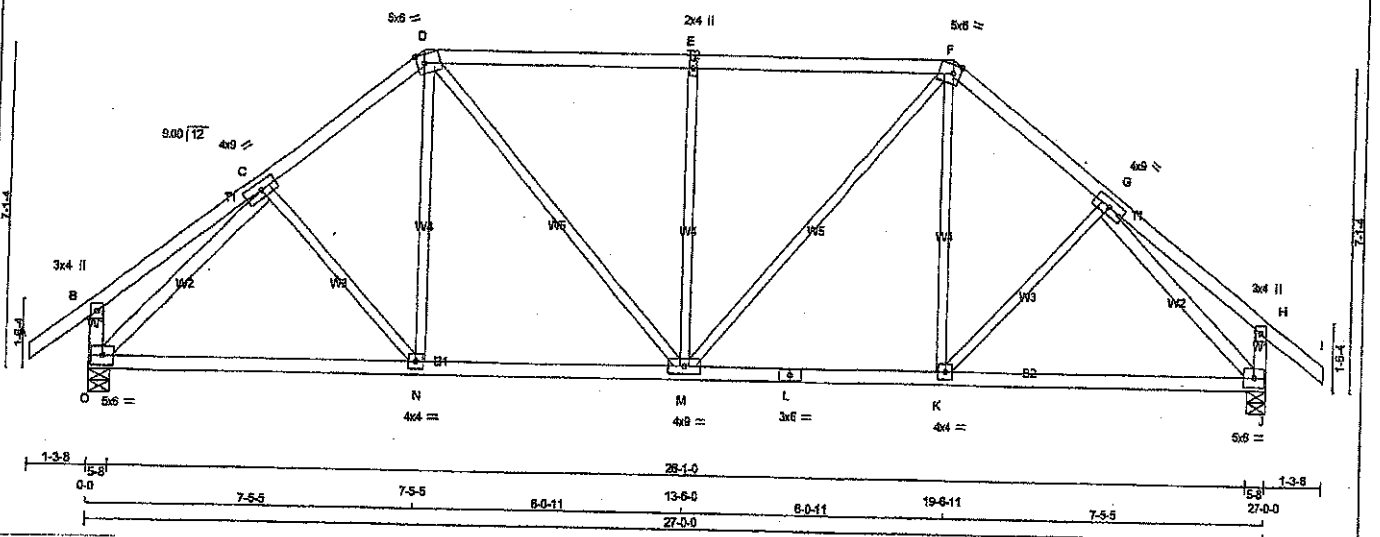
ISI GRIP= 0.87 (K) (INPUT = 0.80)
ISI METAL= 0.67 (N) (INPUT = 1.00)



DRWG NO. TAM T1910762
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME T4	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:42 2018 Page 1
 ID:VdmZNIanRCr_YT1tp?OGrtzKFu-nHNzt9uBFX?YcJFadL3EKV8sXBdyIfVY5jnzlgjd
 1-3-8 0-0 3-9-15 3-9-15 3-7-7 7-5-5 6-0-11 13-6-0 6-0-11 19-6-11 3-7-7 23-2-1 3-9-15 1-3-8 27-0-0 28-3-8
 Scale = 1/4" = 1'-0"



LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - F	2x4	DRY No.2	SPF
F - I	2x4	DRY No.2	SPF
O - B	2x4	DRY No.2	SPF
J - H	2x4	DRY No.2	SPF
O - L	2x4	DRY No.2	SPF
L - J	2x4	DRY No.2	SPF

ALL WEBS EXCEPT	2x3	DRY No.2	SPF
O - C	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
C TMVW-1	MT20	4.0	8.0		
D TMVW-m	MT20	5.0	8.0	Edge	2.00
E TMVW-w	MT20	2.0	4.0		
F TMVW-m	MT20	5.0	8.0	Edge	2.00
G TMVW-1	MT20	4.0	8.0		
H TMV+p	MT20	3.0	4.0		
J BMVW-1	MT20	5.0	6.0		
K BMVW-1	MT20	4.0	4.0		
L BS-1	MT20	4.0	4.0		
M BMVW-1	MT20	4.0	8.0		
N BMVW-1	MT20	4.0	4.0		
O BMVW-1	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
O	2039	0	2039	0	0	5-8	5-8
J	2039	0	2039	0	0	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX / MIN. COMPONENT REACTIONS		PERM. LIVE WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	COMBINED	SOIL
O	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	0 / 0	0 / 0
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	368 / 0	0 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) O, J

BRACING

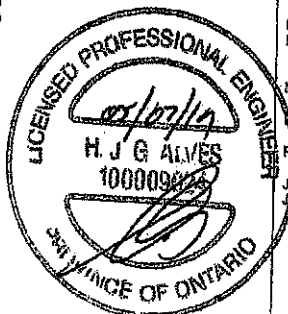
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	G-N	0 / 113
B-C	0 / 25	N-D	0 / 351
C-D	-1948 / 0	D-E	0 / 625
D-E	-1955 / 0	E-F	-758 / 0
E-F	-1955 / 0	F-G	0 / 625
F-G	-1948 / 0	G-H	0 / 351
G-H	0 / 25	H-I	0 / 113
H-I	0 / 42	I-J	-2234 / 0
I-J	-288 / 0	J-K	-2234 / 0
J-K	-288 / 0		
K-L	0 / 1524		
L-M	0 / 1539		
M-N	0 / 1539		
N-O	0 / 1524		



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.1 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2018, OBC 2012
 - CSA 086-09, CSA 088-14
 - TPC 2011, TPC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.13")
 ALLOWABLE DEFL.(TL) = L/360 (0.90")
 CALCULATED VERT. DEFL.(TL) = L/998 (0.22")

CS: TC=0.54/1.00 (D-E:1), BC=0.58/1.00 (K-M:2), WB=0.79/1.00 (G-J:1), SS=0.30/1.00 (E-F:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 618 354 1667 788 1987 1656

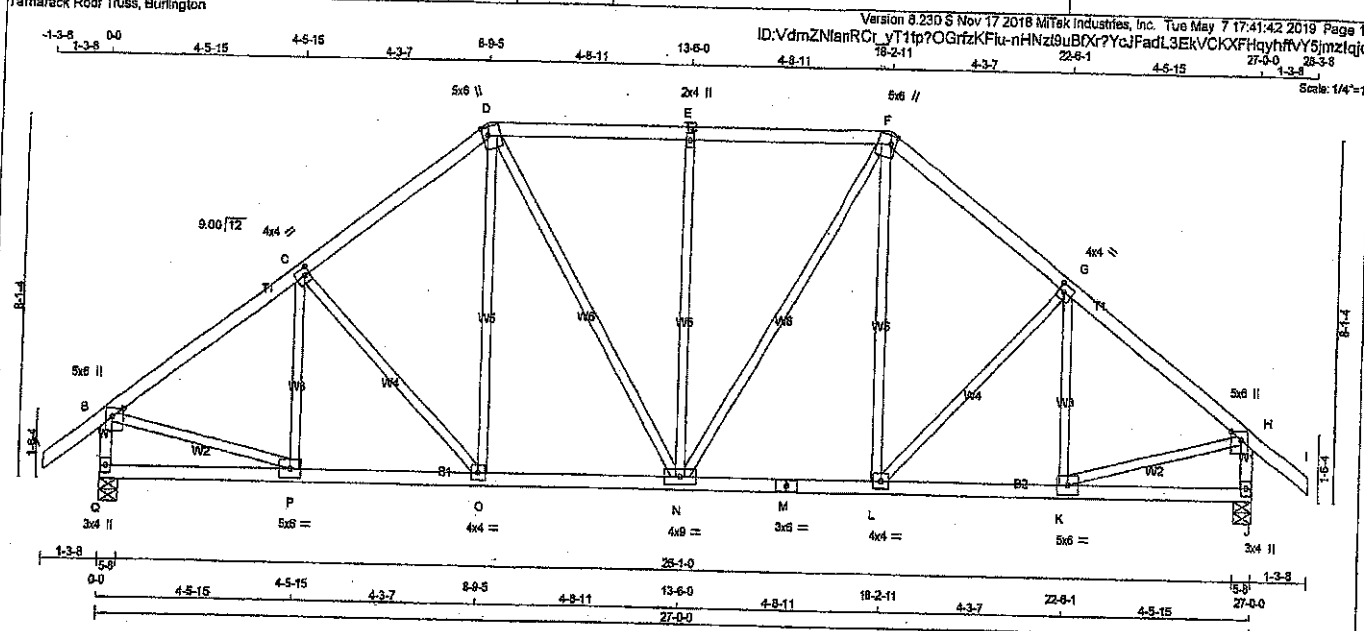
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JS: GRIP= 0.89 (C) (INPUT = 0.90)
 JS: METAL= 0.95 (L) (INPUT = 1.00)

DWG NO. TAM 171910763
 STRUCTURAL
 COMPONENT ONLY

JOB NAME 402938	TRUSS NAME T5	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington					



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
J - H	2x4	DRY	No.2
Q - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	8.0	Edge	3.00
C TMVW+1	MT20	4.0	4.0	2.00	1.50
D TMVW+m	MT20	5.0	8.0	2.25	1.50
E TMVW+w	MT20	2.0	4.0		
F TMVW+m	MT20	5.0	8.0	2.25	1.50
G TMVW+1	MT20	4.0	4.0	2.00	1.50
H TMVW+p	MT20	5.0	8.0	Edge	3.00
J BMV1+p	MT20	3.0	4.0		
K BMVW+1	MT20	5.0	8.0		
L BMVW+1	MT20	5.0	8.0		
M BS+1	MT20	4.0	4.0		
N BMVW+1	MT20	4.0	4.0		
O BMVW+1	MT20	4.0	4.0		
P BMVW+1	MT20	5.0	8.0		
Q BMV1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX	IN-SX	IN-SX
Q	2039	0	2039	0	0	5-8	5-8	5-8	5-8
J	2039	0	2039	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOOSE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
Q	1515	864 / 0	284 / 0	0 / 0	0 / 0	388 / 0	0 / 0	0 / 0
J	1515	864 / 0	284 / 0	0 / 0	0 / 0	388 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.51 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	P-C	-248 / 47
B-C	-1997 / 0	C-O	-252 / 0
C-D	-1845 / 0	O-D	0 / 370
D-E	-1683 / 0	D-N	0 / 446
E-F	-1683 / 0	N-E	-593 / 0
F-G	-1845 / 0	E-F	0 / 446
G-H	-1997 / 0	L-F	0 / 370
H-I	0 / 42	L-G	-252 / 0
Q-B	-1967 / 0	K-G	-248 / 47
J-H	-1967 / 0	B-P	0 / 1678
		K-H	0 / 1678
Q-P	0 / 0		
P-O	0 / 1622		
O-N	0 / 1452		
N-M	0 / 1452		
M-L	0 / 1452		
L-K	0 / 1622		
K-J	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 20.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN/C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 0.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.90")
CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
ALLOWABLE DEFL.(TL) = L/360 (0.90")
CALCULATED VERT. DEFL.(TL) = L/999 (0.11")

CSI: TC=0.32/1.00 (D-E-1), BC=0.34/1.00 (K-L-1),
WB=0.74/1.00 (E-N-1), SSI=0.23/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

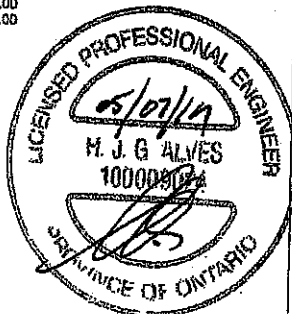
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

RAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 798 1987 1656

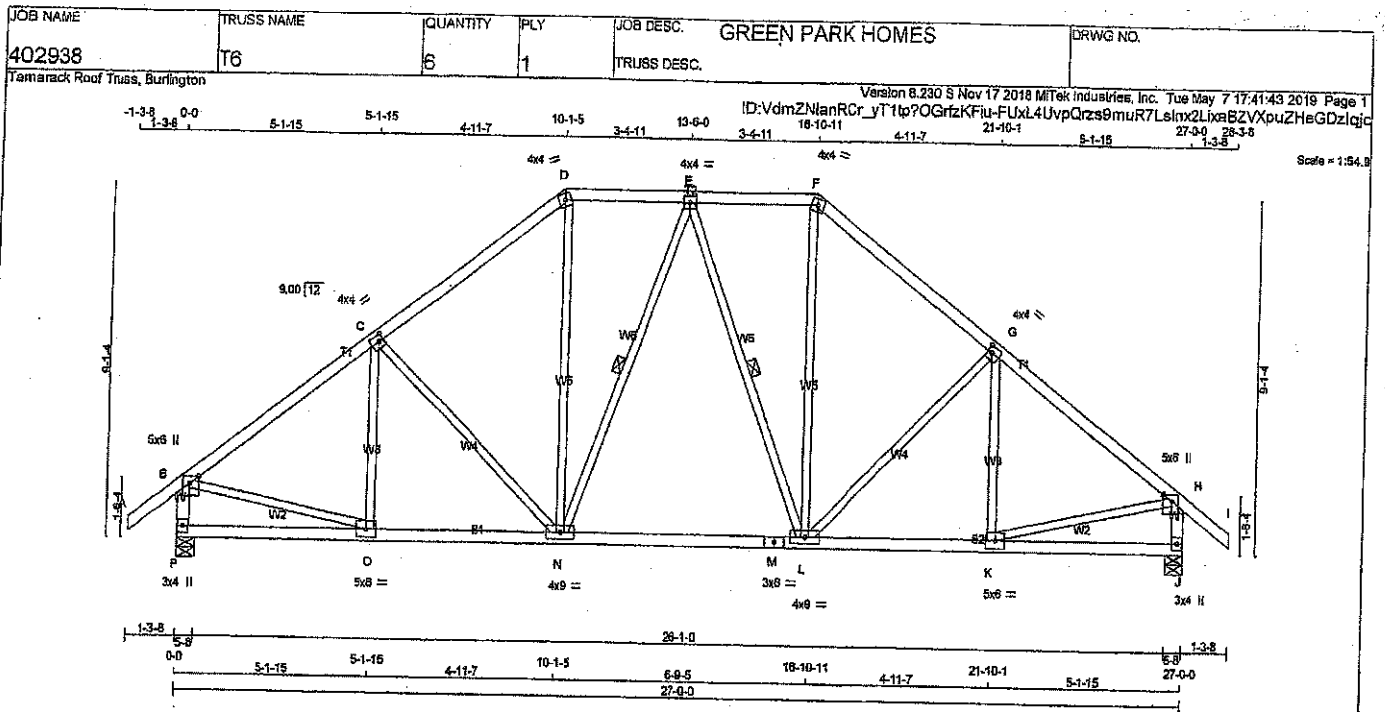
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (H) (INPUT = 0.90)
JSI METAL= 0.91 (M) (INPUT = 1.00)



DRWG NO. TAM 1910764
STRUCTURAL
CHECK SHEET ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
I - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2 SPF
EXCEPT
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVWHP	MT20	5.0	6.0	Edge	3.00
C	TMVWHP	MT20	4.0	4.0	2.00	1.50
D	TTWV-m	MT20	4.0	4.0		
E	TMVWHP	MT20	4.0	4.0		
F	TTWV-m	MT20	4.0	4.0		
G	TMVWHP	MT20	4.0	4.0	2.00	1.50
H	TMVWHP	MT20	5.0	6.0	Edge	3.00
J	BMV1+P	MT20	3.0	4.0		
K	BMVWHP	MT20	5.0	6.0		
L	BMVWHP	MT20	4.0	4.0		
M	BS-4	MT20	3.0	4.0		
N	BMVWHP	MT20	4.0	4.0		
O	BMVWHP	MT20	5.0	6.0		
P	BMV1+P	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT REQRD	
JT	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	BRG
P	2039	0	2039	0	0	5-8	5-8
J	2039	0	2039	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
P	1515	864/0	284/0	0/0	0/0	288/0	0/0
J	1515	864/0	284/0	0/0	0/0	368/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) P, J

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.37 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N, E-L

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	
FR-TO				FR-TO			
A-B	0/42	-102.1	-102.1 0.14 (1)	10.00	C-C	-175/94	0.08 (1)
B-C	-2017/0	-102.1	-102.1 0.41 (1)	4.37	C-N	-379/0	0.37 (1)
C-D	-1758/0	-102.1	-102.1 0.38 (1)	4.64	N-D	0/701	0.16 (1)
D-E	-1382/0	-102.1	-102.1 0.15 (1)	5.39	N-E	-215/0	0.13 (1)
E-F	-1382/0	-102.1	-102.1 0.15 (1)	5.39	E-L	-215/0	0.13 (1)
F-G	-1758/0	-102.1	-102.1 0.38 (1)	4.64	L-F	0/701	0.16 (1)
G-H	-2017/0	-102.1	-102.1 0.41 (1)	4.37	L-G	-379/0	0.37 (1)
H-I	0/42	-102.1	-102.1 0.14 (1)	10.00	K-G	-175/94	0.08 (1)
P-B	-1955/0	0.0	0.0 0.20 (1)	6.02	B-O	0/1685	0.38 (1)
J-H	-1955/0	0.0	0.0 0.20 (1)	6.02	K-H	0/1685	0.38 (1)
P-O	0/0	-38.5	-38.5 0.17 (3)	10.00			
O-N	0/1642	-38.5	-38.5 0.43 (2)	10.00			
N-M	0/1458	-38.5	-38.5 0.40 (2)	10.00			
M-L	0/1458	-38.5	-38.5 0.40 (2)	10.00			
L-K	0/1642	-38.5	-38.5 0.43 (2)	10.00			
K-J	0/0	-38.5	-38.5 0.17 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.80")
CALCULATED VERT. DEFL.(LL) = L/999 (0.06")
ALLOWABLE DEFL.(TL) = L/360 (0.80")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TC=0.41/1.00 (G-H:1), BC=0.43/1.00 (K-L:2), WB=0.38/1.00 (H-K:1), SS=0.20/1.00 (G-H:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

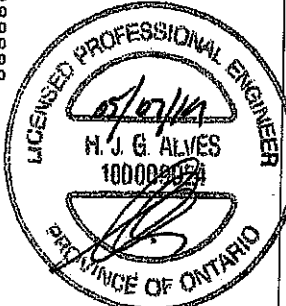
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

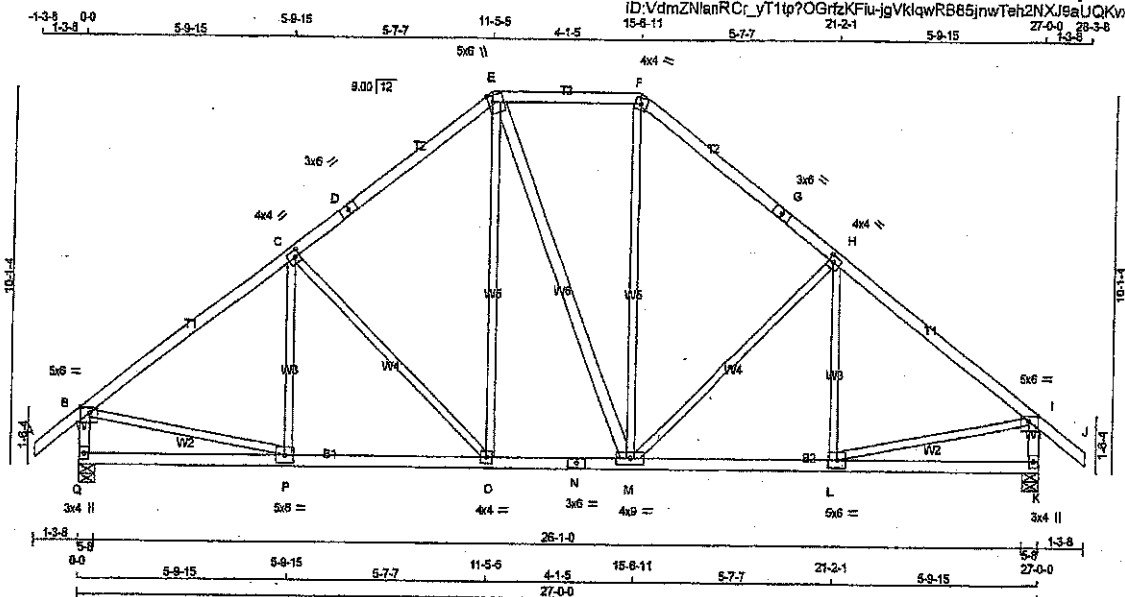
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

SI GRIP= 0.90 (S) (INPUT = 0.90)
SI METAL= 0.48 (M) (INPUT = 1.00)



DRWG NO. TAM 1910165
STRUCTURAL
CHECK ONLY



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - F	2x4	DRY	No.2
F - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
K - I	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - K	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
E - M	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.75	3.25
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	6.0		
E TTWW+m	MT20	5.0	6.0	2.25	1.50
F TTW-m	MT20	4.0	4.0		
G TS-t	MT20	3.0	6.0		
H TMVW-t	MT20	4.0	4.0	2.00	1.50
I TMVW-p	MT20	5.0	6.0	1.75	3.25
K BMV1+p	MT20	3.0	4.0		
L BMVW-t	MT20	5.0	6.0		
M BMVW-t	MT20	4.0	4.0		
N BS-t	MT20	3.0	6.0		
O BMVW-t	MT20	4.0	4.0		
P BMVW-t	MT20	5.0	6.0		
Q BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	BRG	BRG
Q	2039	0	0	5-8
K	2039	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
Q	1515	864 / 0	284 / 0
K	1515	864 / 0	284 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.20 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD (LBS)	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)
FR-TO					FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	P-C	-83 / 176
B-C	-2028 / 0	-102.1	-102.1	0.54 (1)	4.20	C-O	-524 / 0
C-D	-1645 / 0	-102.1	-102.1	0.49 (1)	4.62	O-E	0 / 596
D-E	-1645 / 0	-102.1	-102.1	0.49 (1)	4.62	E-M	0 / 3
E-F	-1285 / 0	-102.1	-102.1	0.24 (1)	5.43	M-F	0 / 540
F-G	-1647 / 0	-102.1	-102.1	0.49 (1)	4.61	M-H	-522 / 0
G-H	-1647 / 0	-102.1	-102.1	0.49 (1)	4.61	H-I	-88 / 173
H-I	-2027 / 0	-102.1	-102.1	0.54 (1)	4.20	B-P	0 / 1690
I-J	0 / 42	-102.1	-102.1	0.14 (1)	10.00	L-I	0 / 1699
Q-B	-1948 / 0	0.0	0.0	0.20 (1)	6.03		
K-I	-1948 / 0	0.0	0.0	0.20 (1)	6.03		
Q-P	0 / 0	-38.5	-38.5	0.25 (3)	10.00		
P-O	0 / 1656	-38.5	-38.5	0.46 (2)	10.00		
O-N	0 / 1285	-38.5	-38.5	0.31 (1)	10.00		
N-M	0 / 1285	-38.5	-38.5	0.31 (1)	10.00		
M-L	0 / 1655	-38.5	-38.5	0.46 (2)	10.00		
L-K	0 / 0	-38.5	-38.5	0.25 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 28.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPC 2011, TPC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.90")
CALCULATED VERT. DEFL. (LL) = L/999 (0.07")
ALLOWABLE DEFL. (TL) = L/360 (0.90")
CALCULATED VERT. DEFL. (TL) = L/999 (0.12")

CSI: TC=0.54/1.00 (B-C-T), BC=0.46/1.00 (O-P-T),
WB=0.70/1.00 (C-O-T), SS=0.23/1.00 (B-C-T)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

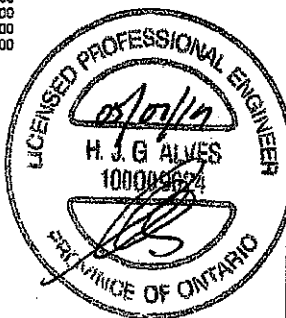
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1687	788
	1987	1655

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

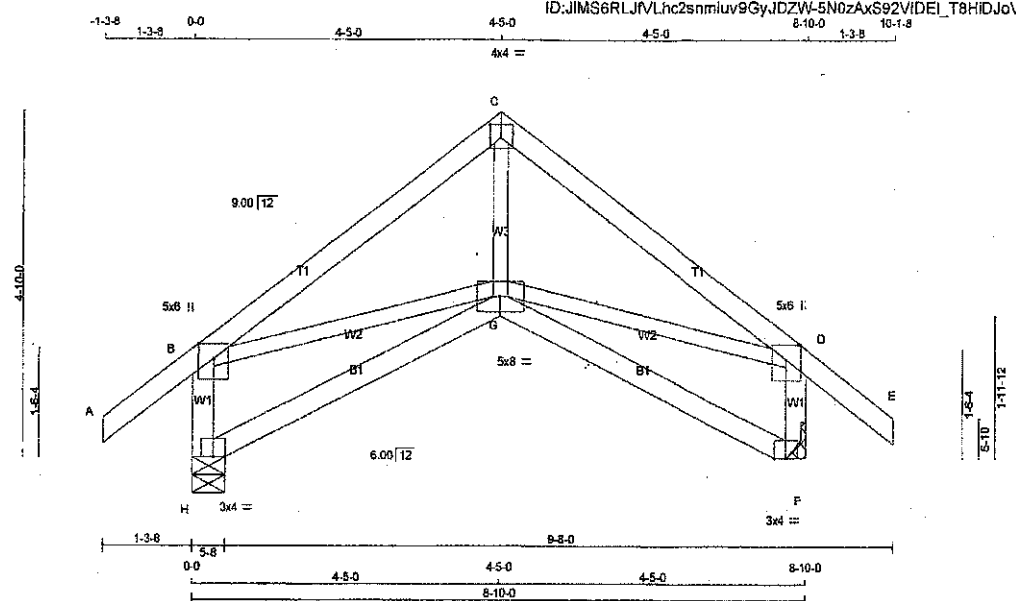
ISI GRIP= 0.89 (B) (INPUT = 0.90)
ISI METAL= 0.41 (P) (INPUT = 1.00)



DRWG NO. TAM T1910766
STRUCTURAL
CLARENCE ONLY

JOB NAME 402981	TRUSS NAME T9S	QUANTITY 3	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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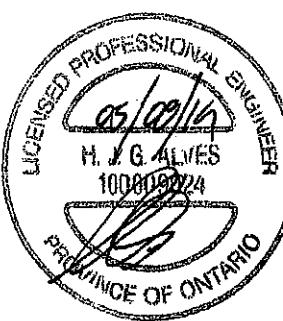
Version 8.230 S Nov 17 2018 MTek Industries, Inc. Wed May 8 15:15:39 2019 Page 1
ID:JIMS6RLJV/Lhc2snmliuv9GyJDZW-5N0zAxS92V/DEI_T8HIDJoVpStlpu59GBgo9JzlXmY



Scale = 1/30 ft
 TOTAL WEIGHT = 3 X 39 = 118 lb [M/F]

LUMBER							
N.L.G.A. RULES	SIZE	LUMBER	DESCR.				
A - C	2x4 DRY	No.2	SPF				
C - E	2x4 DRY	No.2	SPF				
H - B	2x4 DRY	No.2	SPF				
F - D	2x4 DRY	No.2	SPF				
H - G	2x4 DRY	No.2	SPF				
G - F	2x4 DRY	No.2	SPF				
ALL WEBS 2x3 DRY No.2 SPF							
EXCEPT							
DRY: SEASONED LUMBER.							

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER								
BEARINGS								
JT	FACTORED GROSS REACTION VERT	MAXIMUM FACTORED GROSS REACTION DOWN	INPUT BRG IN-SX	REQD BRG IN-SX				
H	762	0	0	5-8				
F	762	0	0	MECHANICAL				
A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3'-8".								
UNFACTORED REACTIONS								
JT	1ST CASE COMBINED	MAX/MIN COMPONENT REACTIONS SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
H	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0	0 / 0	
F	561	337 / 0	93 / 0	0 / 0	0 / 0	131 / 0	0 / 0	
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H								
BRACING								
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.								
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.								
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.								
LOADING								
TOTAL LOAD CASES: (4)								
CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CS1 (LC)	MAX. UNBRACED LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)	
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 389	0.09 (2)
B-C	-663 / 0	-102.1	-102.1	0.26 (1)	6.25	B-G	0 / 556	0.13 (1)
C-D	-663 / 0	-102.1	-102.1	0.26 (1)	6.25	G-D	0 / 556	0.13 (1)
D-E	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
H-B	-677 / 0	0.0	0.0	0.07 (1)	7.81			
F-D	-677 / 0	0.0	0.0	0.07 (1)	7.81			
H-G	0 / 0	-38.5	-38.5	0.18 (3)	10.00			
G-F	0 / 0	-38.5	-38.5	0.18 (3)	10.00			



DIVISION TAM 1910988

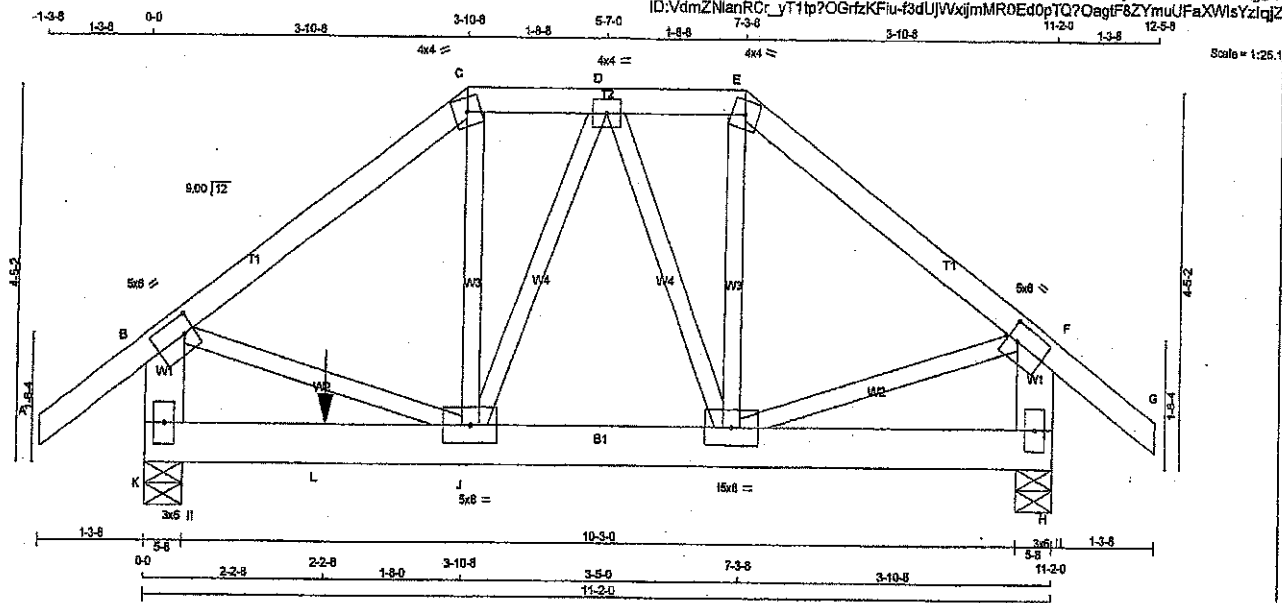
STRUCTURAL

CONSTRUCTION ONLY

JOB NAME 402938	TRUSS NAME T110	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 MTEK Industries, Inc. Tue May 7 17:41:46 2019 Page 1
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Scale = 1:25.1

TOTAL WEIGHT = 81 lb

LUMBER	N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4 DRY	No.2	SPF		
C - E	2x4 DRY	No.2	SPF		
E - G	2x4 DRY	No.2	SPF		
K - B	2x6 DRY	No.2	SPF		
H - F	2x6 DRY	No.2	SPF		
K - H	2x6 DRY	No.2	SPF		
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF		

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-t	MT20	5.0	6.0	2.50	1.50
C	TTW-m	MT20	4.0	4.0		
D	TMVW-t	MT20	4.0	4.0		
E	TTW-m	MT20	4.0	4.0		
F	TMVW-t	MT20	5.0	6.0	2.50	1.50
H	BMV1+p	MT20	3.0	6.0		
I	BMVWW-t	MT20	5.0	8.0		
J	BMVWW-t	MT20	5.0	8.0		
K	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORIZ	DOWN	HORIZ	UPLIFT
K	1684	0	1684	0	0
H	1115	0	1115	0	0

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS	PERM	LIVE	WIND	DEAD	SOIL
K	1240	788 / 0	168 / 0	0 / 0	0 / 0	288 / 0	0 / 0	0 / 0
H	821	494 / 0	135 / 0	0 / 0	0 / 0	192 / 0	0 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) K, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.47 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED HORZ. LOAD (LBS)
FR-TO			
A-B	0 / 42	-102.1	-102.1
B-C	-1160 / 0	-102.1	-102.1
C-D	-936 / 0	-102.1	-102.1
D-E	-672 / 0	-102.1	-102.1
E-F	-834 / 0	-102.1	-102.1
F-G	0 / 42	-102.1	-102.1
K-B	-1295 / 0	0.0	0.0
H-F	-1027 / 0	0.0	0.0
K-L	0 / 0	-38.5	-38.5
L-J	0 / 0	-38.5	-38.5
J-I	0 / 639	-38.5	-38.5
I-H	0 / 0	-38.5	-38.5

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
L	2-2-8	-958	-958		FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD	= 52.5	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2010, NBC 2012
- CSA 080-09, CSA 085-14
- TPIC 2011, TPIC 2014

(5% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEF. (LL) = 1/380 (0.37")
CALCULATED VERT. DEF. (LL) = 1/899 (0.04")
ALLOWABLE DEF. (TL) = 1/860 (0.37")
CALCULATED VERT. DEF. (TL) = 1/999 (0.08")

CSI: TC=0.32/1.00 (B-C-1), BC=0.58/1.00 (J-K-1), WB=0.24/1.00 (B-J-1), SS=0.47/1.00 (J-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

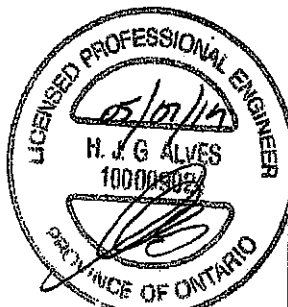
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1697 788 1697 1656

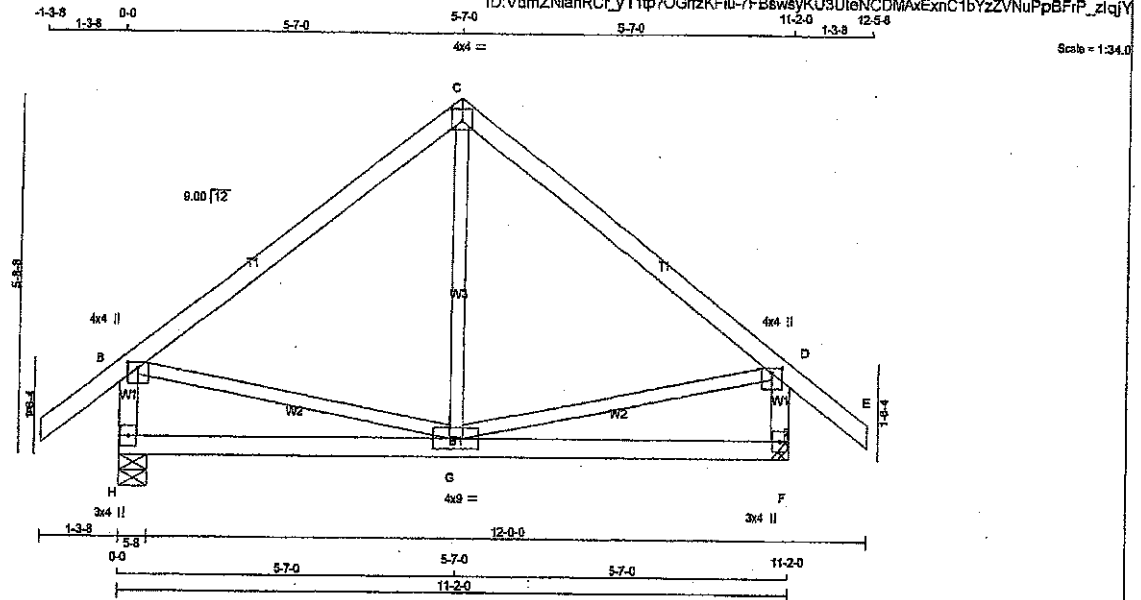
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.86 (F) (INPUT = 0.80)
JSI METAL = 0.34 (F) (INPUT = 1.00)



DRWG NO. TAM T110768
STRUCTURAL
C/C ONLY



TOTAL WEIGHT = 3 X 48 = 144 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4 DRY	No.2	SPF
C - E	2x4 DRY	No.2	SPF
H - B	2x4 DRY	No.2	SPF
F - D	2x4 DRY	No.2	SPF
H - F	2x4 DRY	No.2	SPF
ALL WEBS EXCEPT	2x3 DRY	No.2	SPF
DRY, SEASONED LUMBER.			

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMVW+p	MT20	4.0	4.0 1.00 2.00
C	TTW+p	MT20	4.0	4.0 2.25 2.00
D	TMVW+p	MT20	4.0	4.0 1.00 2.00
F	BMV1+p	MT20	3.0	4.0
G	BMVWW-t	MT20	4.0	9.0
H	BMV1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED	MAXIMUM FACTORED	INPUT	REQD
JT	GROSS REACTION	GROSS REACTION	DOWN	BRG	BRG
H	926 0	926 0	0 0	5-8	5-8
F	926 0	926 0	0 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT F. MINIMUM BEARING LENGTH AT JOINT F = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
H	883	404 / 0	117 / 0	0 / 0	162 / 0	0 / 0
F	883	404 / 0	117 / 0	0 / 0	162 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO		FR-TO	
A-B	0 / 42	G-C	0 / 248
B-C	-543 / 0	B-G	0 / 444
C-D	-543 / 0	G-D	0 / 444
D-E	0 / 42		
H-B	-844 / 0		
F-D	-844 / 0		
H-G	0 / 0		
G-F	0 / 0		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD = 52.5 PSF	

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (8.37")
CALCULATED VERT. DEFL.(LL) = 1/898 (0.03")
ALLOWABLE DEFL.(TL) = 1/880 (0.37")
CALCULATED VERT. DEFL.(TL) = 1/898 (0.05")

CSK: TC=0.41/1.00 (B-C:1), BC=0.27/1.00 (F-G:3), WB=0.10/1.00 (D-G:1), SS=0.18/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

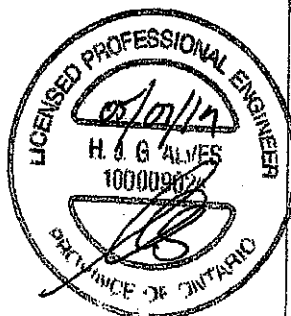
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618 354 1667 788 1987 1658	

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.81 (D) (INPUT = 0.80)
JSI METAL= 0.21 (B) (INPUT = 1.00)

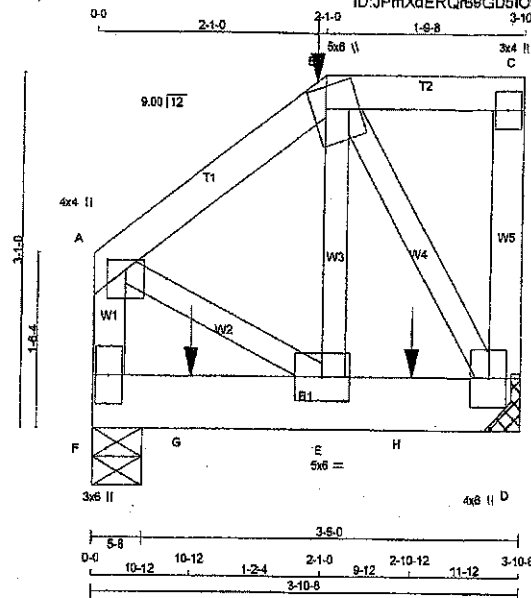


DRWG NO. TAM 779/0769
STRUCTURAL
CL. QUOTRY ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402979	T12	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 M/Tek Industries, Inc. Wed May 8 15:08:00 2019 Page 1
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Scale = 1:18.6

CHORDS	SIZE	LUMBER	DESCR.
F - A	2x4	DRY No.2	SPF
A - B	2x4	DRY No.2	SPF
B - C	2x4	DRY No.2	SPF
C - D	2x4	DRY No.2	SPF
F - D	2x6	DRY No.2	SPF

ALL WEBS 2x3 DRY SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
F-A	12	TOP
A-B	12	SIDE(61.0)
B-C	12	SIDE(61.0)
C-D	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	12	SIDE(0.0)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TMVW+p	MT20	4.0	4.0	1.00	2.00
B	TTWW+m	MT20	5.0	6.0	2.25	1.50
C	TMV+p	MT20	3.0	4.0		
D	BMVW1+p	MT20	4.0	6.0		
E	BMVW-l	MT20	5.0	6.0		
F	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
VERT	HORZ	UPLIFT	IN-SX
F	803	0	5-8
D	883	0	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT D. MINIMUM BEARING LENGTH AT JOINT D = 4-0.

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW LIVE PERM.LIVE WIND DEAD SOIL
F	667	394 / 0 115 / 0 0 / 0 0 / 0 158 / 0 0 / 0
D	851	386 / 0 111 / 0 0 / 0 0 / 0 154 / 0 0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MEMB.	FORCE (LBS)
FR-TO		FROM TO	FR-TO	
F-A	-583 / 0	0.0 0.0 0.03 (1)	A-E	0 / 405
A-B	-453 / 0	-102.1 -102.1 0.04 (1)	E-B	0 / 579
B-C	0 / 0	-102.1 -102.1 0.03 (1)	B-D	-692 / 0
D-C	-91 / 0	0.0 0.0 0.01 (1)		
F-G	0 / 0	-38.5 -38.5 0.11 (1)		
G-E	0 / 0	-38.5 -38.5 0.11 (1)		
E-H	0 / 383	-38.5 -38.5 0.10 (1)		
H-D	0 / 383	-38.5 -38.5 0.10 (1)		

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
B	2-1-0	-73	-73	-	BACK	VERT	TOTAL		
G	10-12	-585	-585	-	FRONT	VERT	TOTAL		
H	2-10-12	-583	-583	-	FRONT	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.1 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 096-09, CSA 096-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/380 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/380 (0.19")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.04/1.00 (A-B:1), BC=0.11/1.00 (E-F:1), WB=0.07/1.00 (B-E:1), SS=0.14/1.00 (D-E:1)

COL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

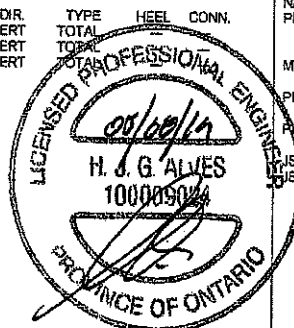
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PL)	SECTION (PL)
MT20	618	354
	1667	766
	1967	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

SLIP GRIP = 0.28 (A) (INPUT = 0.90)
SLIP METAL = 0.09 (D) (INPUT = 1.00)

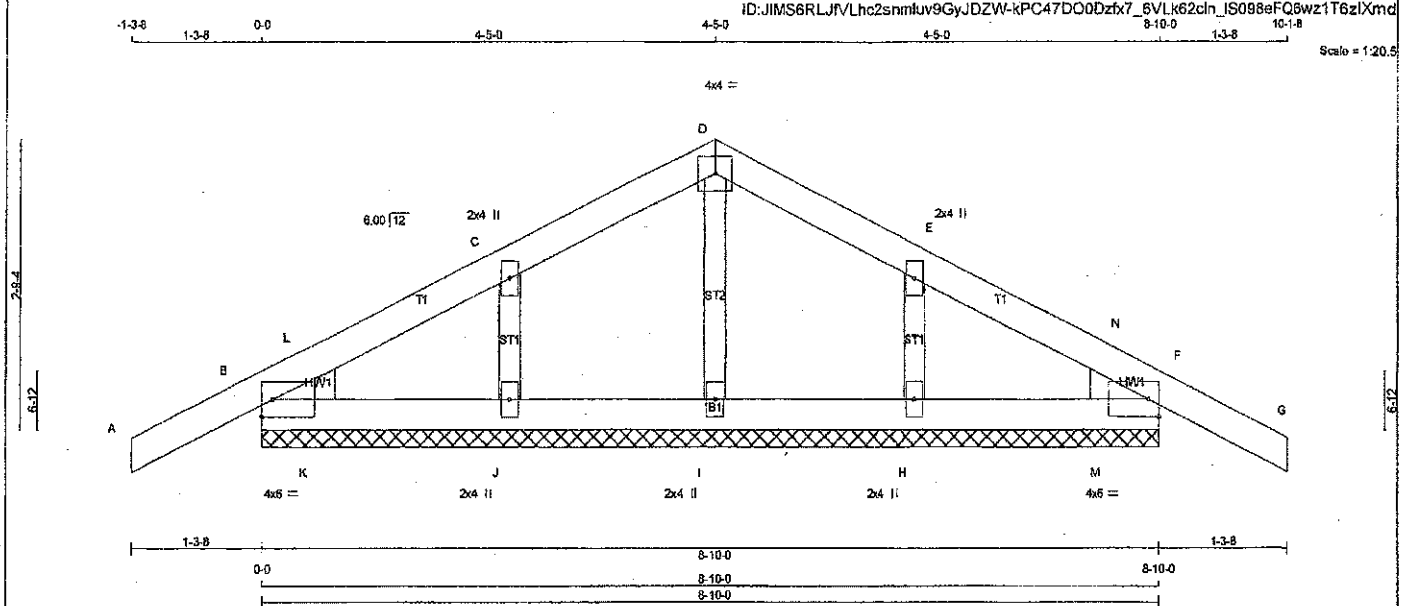


DRWG NO. TAM T12/10979
STRUCTURAL
CLIPPING ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
402981	G14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: JIMS6RLJfV/Lhc2snmluv9GyJDZW-kPC47D00Dzfx7_6VLk62cln_IS098eFQ6wz1T6zIXmd



TOTAL WEIGHT = 30 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
B - F	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			
GABLE STUDS SPACED AT	2'-0"-O.C.		

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMBH1-I	MT20	4.0	6.0	Edge
C	TMW+u	MT20	2.0	4.0	
D	TTW-p	MT20	4.0	4.0	
E	TMW+u	MT20	2.0	4.0	
F	TMBH1-I	MT20	4.0	6.0	Edge
H, I, J	BMW1+u	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
HEEL
WEDGE
2x4 L
2x4 R
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC1)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC1)	MAX. FACTORED VERT. LOAD (LC1)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/21	-102.1	-102.1	0.13 (1)	I-D	-94/6	0.02 (1)
B-L	-121/0	-102.1	-102.1	0.02 (3)	J-C	-254/0	0.04 (1)
L-C	-71/0	-102.1	-102.1	0.07 (1)	H-E	-254/0	0.04 (1)
C-D	-85/0	-102.1	-102.1	0.07 (1)	K-L	-29/31	0.00 (1)
D-E	-85/0	-102.1	-102.1	0.07 (1)	M-N	-29/31	0.00 (1)
E-N	-71/0	-102.1	-102.1	0.07 (1)			
N-F	-121/0	-102.1	-102.1	0.02 (3)			
F-G	0/21	-102.1	-102.1	0.13 (1)			
B-K	0/77	-38.5	-38.5	0.05 (1)			
K-J	0/77	-38.5	-38.5	0.05 (2)			
J-I	0/65	-38.5	-38.5	0.05 (2)			
I-H	0/65	-38.5	-38.5	0.05 (2)			
H-M	0/77	-38.5	-38.5	0.05 (2)			
M-F	0/77	-38.5	-38.5	0.05 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 5.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C
THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 20.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (A-B:1), BC=0.05/1.00 (J-K:2), WB=0.04/1.00 (E-H:1), SS=0.10/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

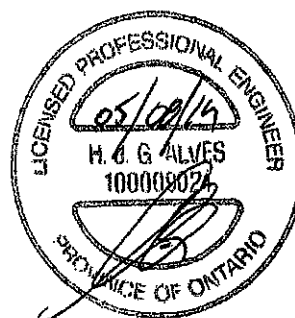
NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.31 (F) (INPUT = 0.80)

JSI METAL= 0.11 (C) (INPUT = 1.00)



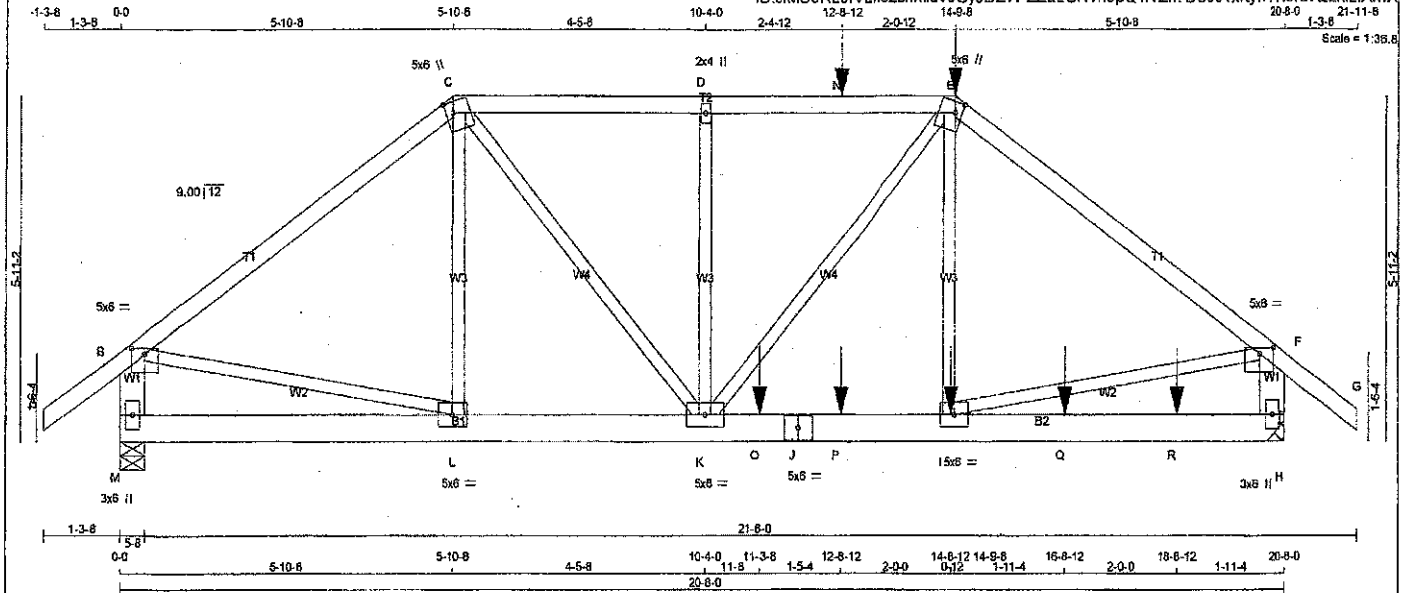
DRWG NO. TAM 779/0982
STRUCTURAL
SEE 1. DESIGN ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DRWG NO
402981	T20	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: JMS6RLJVLhc2snmuv9GyJDZW-ZZaLOHTnopQ4rvZfi?Ds01xrtYhYH3lUrQlhlzXmX



TOTAL WEIGHT = 2 X 104 = 208 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
E - G	2x4	DRY	No.2	SPF	
M - B	2x6	DRY	No.2	SPF	
H - F	2x6	DRY	No.2	SPF	
M - J	2x6	DRY	No.2	SPF	
J - H	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
EXCEPT					

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS			
A - C	1	12	TOP
C - E	1	12	SIDE(61.0)
E - G	1	12	SIDE(61.0)
M - B	2	12	TOP
H - F	2	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS			
M - J	2	12	SIDE(0.0)
J - H	2	12	SIDE(183.1)
WEBS: (0.122"x3") SPIRAL NAILS			
I - E	1	6	SIDE(14.7)
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	5.0	6.0	1.25	3.00
C	TTVW+m	MT20	5.0	6.0	2.25	1.50
D	TMVW+w	MT20	2.0	4.0		
E	TTVW+m	MT20	5.0	6.0	2.25	1.50
F	TMVW+p	MT20	5.0	6.0	1.25	3.00
H	BMV1+p	MT20	3.0	6.0		
I	BMVW-1	MT20	5.0	6.0		
J	BS-1	MT20	5.0	6.0		
K	BMVWV-1	MT20	5.0	6.0		
L	BMVW-1	MT20	5.0	6.0		
M	BMV1+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	HORIZ	UP/LIFT	IN-SX
M	2545	0	2545	0
H	3069	0	3069	0

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 4-0.

UNFACTORED REACTIONS

1ST CASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW
M	1874	1117 / 0
H	2266	1327 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.64 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED	FACTORED	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD	LC1	MEMB.	FORCE	MAX	CS(LC)
FR-TO		FROM TO	CS(LC)	FR-TO			
A - B	0 / 42	-102.1 -102.1	0.08 (1)	L - C	-275 / 98	0.07 (1)	
B - C	-2659 / 0	-102.1 -102.1	0.43 (1)	C - K	0 / 1790	0.22 (1)	
C - D	-3242 / 0	-102.1 -102.1	0.25 (1)	K - D	-608 / 0	0.18 (1)	
D - N	-3242 / 0	-102.1 -102.1	0.25 (1)	K - E	0 / 814	0.11 (1)	
N - E	-3242 / 0	-102.1 -102.1	0.25 (1)	L - E	0 / 407	0.06 (3)	
E - F	-3335 / 0	-102.1 -102.1	0.48 (1)	B - L	0 / 2480	0.27 (1)	
F - G	0 / 42	-102.1 -102.1	0.08 (1)	I - F	0 / 2480	0.27 (1)	
M - B	-2435 / 0	0.0	0.09 (1)				
H - F	-2841 / 0	0.0	0.11 (1)				
M - L	0 / 0	-38.5 -38.5	0.07 (2)				
L - K	0 / 2119	-38.5 -38.5	0.28 (1)				
K - O	0 / 2668	-38.5 -38.5	0.49 (1)				
O - J	0 / 2668	-38.5 -38.5	0.49 (1)				
J - P	0 / 2668	-38.5 -38.5	0.49 (1)				
P - I	0 / 2668	-38.5 -38.5	0.49 (1)				
I - Q	0 / 0	-38.5 -38.5	0.15 (2)				
Q - R	0 / 0	-38.5 -38.5	0.15 (2)				
R - H	0 / 0	-38.5 -38.5	0.15 (2)				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX	MAX	FACE	DIR
E	14-8-12	-524	-524		FRONT	VERT
N	12-8-12	-123	-123		FRONT	VERT
O	11-3-8	-1578	-1578		FRONT	VERT
P	12-8-12	-28	-28		FRONT	VERT
Q	16-8-12	-75	-75		FRONT	VERT
R	18-8-12	-75	-75		FRONT	VERT

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.1	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.6	PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.69")
CALCULATED VERT. DEFL. (LL) = L/998 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.69")
CALCULATED VERT. DEFL. (TL) = L/999 (0.09")

CSI: TC=0.48/1.00 (E-F-1), BC=0.48/1.00 (I-K-1), WB=0.34/1.00 (F-L-1), SS=0.44/1.00 (I-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
M20 618 354 1667 788 1667 1658

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.78 (E) (INPUT = 0.90)
JSI METAL= 0.54 (J) (INPUT = 1.00)

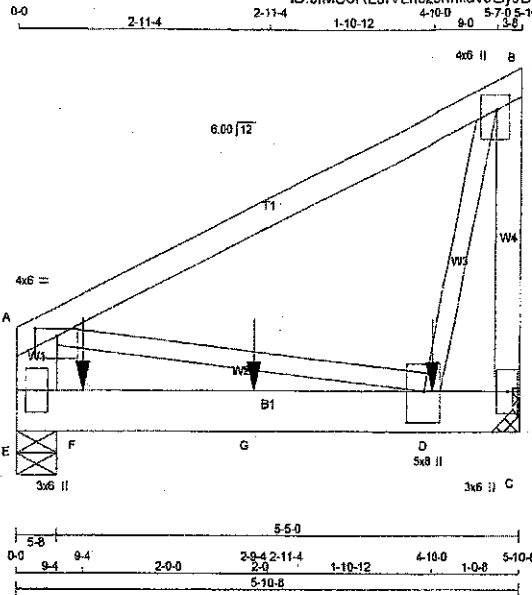
DATE NOTED 11/9/09

DESIGNER

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	GREEN PARK HOMES	DRWG NO.
402981	T21	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID: JIMS6RLJVLhc2anmluv9GyJDZW-117jbdUPZ7Yxt38rGikrODa8XHNbHjXRjv9vDCzIXmVW



Scale: 1/2"=1'

TOTAL WEIGHT = 2 X 29 = 58 lb

LUMBER	CHORDS	SIZE	SURFACE	LUMBER	DESCR.
N. L. G. A. RULES					
E - A	2x6	DRY	No.2	SPF	
A - B	2x4	DRY	No.2	SPF	
C - B	2x4	DRY	No.2	SPF	
E - C	2x6	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS	#ROWS	SURFACE	LOAD(PLF)
TOP CHORDS	2	(0.122"x3") SPIRAL NAILS	
E-A	2	12	TOP
A-B	1	12	TOP
B-C	1	12	TOP
BOTTOM CHORDS	2	(0.122"x3") SPIRAL NAILS	
E-C	2	12	SIDE(0.0)
WEBS	1	(0.122"x3") SPIRAL NAILS	
2x3	1	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TM/VV+p	MT20	4.0	6.0	1.00	3.00
B	TM/VV+p	MT20	4.0	6.0	2.00	1.75
C	BM/V+p	MT20	3.0	6.0		
D	BM/VV+t	MT20	5.0	8.0	4.25	2.50
E	BM/V+p	MT20	3.0	6.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED	MAXIMUM FACTORED	INPUT	REQD
GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORZ	DOWN	HORZ
E	1309	0	1309
C	3036	0	3036

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT C. MINIMUM BEARING LENGTH AT JOINT C = 4-0.

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN	COMPONENT REACTIONS
E	952	605 / 0	130 / 0
C	2233	1335 / 0	358 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	
FR-TO		FROM TO			FR-TO			
E-A	-858 / 0	0.0	0.0	0.03 (1)	7.81	A-D	0 / 852	0.11 (1)
A-B	-941 / 0	-102.1	-102.1	0.35 (1)	6.25	D-B	0 / 3115	0.39 (1)
C-B	-2878 / 0	0.0	0.0	0.35 (1)	6.77			
E-F	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
F-G	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
G-D	0 / 0	-38.5	-38.5	0.19 (1)	10.00			
D-C	0 / 0	-38.5	-38.5	0.05 (1)	10.00			

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	4-10-0	-3031	-3031		BACK	VERT	TOTAL		
F	8-4	-295	-295		BACK	VERT	TOTAL		
G	2-9-4	-192	-192		BACK	VERT	TOTAL		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 066-09, CSA 096-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.02")
ALLOWABLE DEFL.(TL) = 1/360 (0.20")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.04")

CSI: TC=0.35/1.00 (B-C:1), BC=0.19/1.00 (D-E:1), WB=0.39/1.00 (B-D:1), CSI=0.15/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MT20	618	354	1657

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

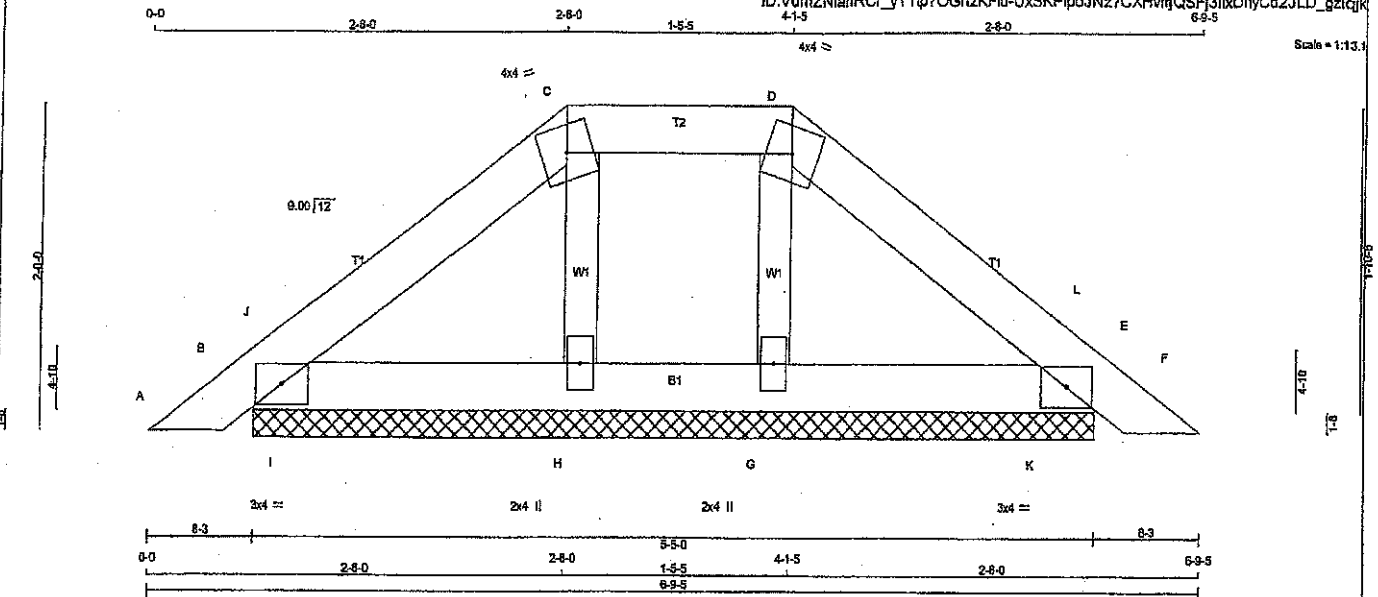
JSI GRIP= 0.84 (B) (INPUT = 0.90)
JSI METAL= 0.28 (D) (INPUT = 1.00)



DWG NO. TAM 1910990
STRUCTURAL
2019-05-08

JOB NAME 402938	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:35 2019 Page 1
ID:VdmZNlanRCr_yT1fp?OGfzKFlu-UxSKPipojNz?CXHvffQSF3ltxDhyCd2JLD_gzicjk



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		RECORD BRG	
JT	VERT	JT	HORZ	JT	IN-SX	JT	IN-SX
B	233	0	233	0	5-5-0	5-5-0	
E	233	0	233	0	5-5-0	5-5-0	
H	208	0	208	0	5-5-0	5-5-0	
G	208	0	208	0	5-5-0	5-5-0	

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 2.00/12 MINIMUM

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.05/1.00 (D-L:1), BC=0.05/1.00 (G-K:1), WB=0.02/1.00 (D-G:1), SS=0.09/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.17 (B) (INPUT = 0.90)
JSI METAL= 0.04 (B) (INPUT = 1.00)

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	TMB1-I	MT20	3.0	4.0
C	TTW-m	MT20	4.0	4.0
D	TTW-m	MT20	4.0	4.0
E	TMB1-J	MT20	3.0	4.0
G	BMW1-w	MT20	2.0	4.0
H	BMW1-w	MT20	2.0	4.0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS				
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
B	169	111/0	21/0	0/0	0/0
E	169	111/0	21/0	0/0	0/0
H	158	81/0	38/0	0/0	0/0
G	158	81/0	38/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

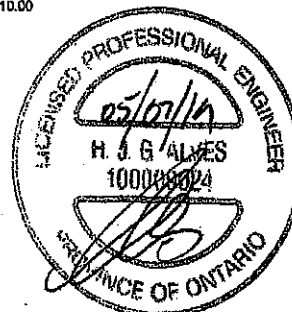
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 MAX. (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 MAX. (LC)	
FR-TO				FR-TO			
A-B	0/17	-102.1	-102.1 0.03 (1)	H-C	-121/0	0.02 (1)	
B-J	-64/0	-102.1	-102.1 0.01 (3)	G-D	-121/0	0.02 (1)	
J-C	-65/0	-102.1	-102.1 0.05 (1)	I-J	-103/22	0.00 (1)	
C-D	-43/0	-102.1	-102.1 0.04 (1)	K-L	-103/22	0.00 (1)	
D-L	-85/0	-102.1	-102.1 0.05 (1)				
L-E	-64/0	-102.1	-102.1 0.31 (3)				
E-F	0/17	-102.1	-102.1 0.03 (1)				
B-I	0/51	-38.5	-38.5 0.05 (1)				
I-H	0/51	-38.5	-38.5 0.05 (1)				
H-G	0/43	-38.5	-38.5 0.04 (1)				
G-K	0/51	-38.5	-38.5 0.05 (1)				
K-E	0/51	-38.5	-38.5 0.05 (1)				

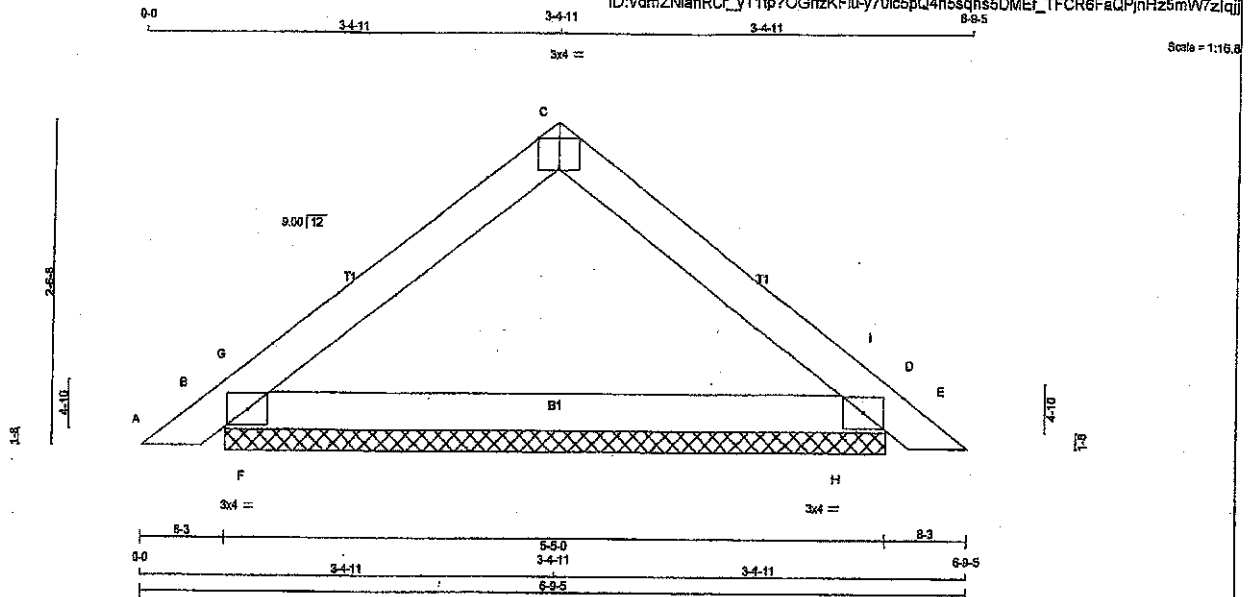


DRWG NO. TAM 71910757
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME PB2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:VdmZNIanRCr_yT1tp?OGrfzKFiu-y70lcSpQ4h5sqhs5DMEf_TFCR6FaQPjnHz5mV7zIqj



TOTAL WEIGHT = 2 X 16 = 32 lb

<u>LUMBER</u>					
N. L. G. A. RULES					
CHORDS		SIZE	LUMBER		DESCR.
A - C	2x4	DRY	No.2	SPF	
C - E	2x4	DRY	No.2	SPF	
B - D	2x4	DRY	No.2	SPF	
DRY: SEASONED LUMBER.					

DRY: SEASONED LUMBER.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B	TM81-I	MT20	3.0	4.0		
C	TT-p	MT20	3.0	4.0	Edge	2.00
D	TM81-I	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	HORZ	IN-SX	IN-SX	IN-SX	IN-SX
B	441	0	441	0	0	5-5-0	5-5-0	5-5-0
D	441	0	441	0	0	5-5-0	5-5-0	5-5-0

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	326	192/0	57/0	0/0	0/0	78/0	0/0
D	326	192/0	57/0	0/0	0/0	78/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LC)	MAX. FACTORED LC1 (LC)
FR-TO				FR-TO			
A-B	0/17	-102.1	-102.1 0.03 (1)	10.00	F-G	0/330	0.00 (1)
B-G	-442/0	-102.1	-102.1 0.12 (3)	6.25	H-I	0/330	0.00 (1)
G-C	-235/0	-102.1	-102.1 0.15 (1)	6.25			
C-I	-235/0	-102.1	-102.1 0.15 (1)	6.25			
I-D	-442/0	-102.1	-102.1 0.12 (3)	6.25			
D-E	0/17	-102.1	-102.1 0.03 (1)	10.00			
B-F	0/201	-38.5	-38.5 0.09 (3)	10.00			
F-H	0/201	-38.5	-38.5 0.17 (2)	10.00			
H-D	0/201	-38.5	-38.5 0.09 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 085-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.15/1.00 (C-G-1), 8C=0.17/1.00 (F-H-2),
WB=0.00/1.00 (F-G-1), 5SI=0.18/1.00 (D-I-3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

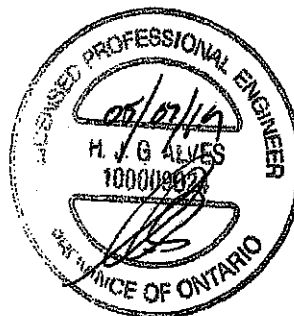
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1867
	788	1987	1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.28 (B) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)



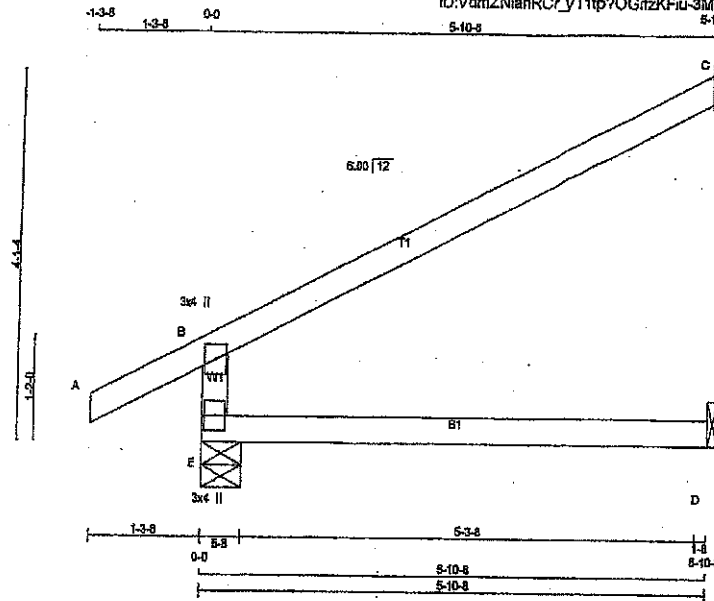
DRWG NO. TAM 1910758
STRUCTURAL
COMPONENT ONLY

JOB NAME 402938	TRUSS NAME J1	QUANTITY 19	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue May 7 17:41:32 2018 Page 1
ID:VdmZNianRCr_yT1tp?OG:fzKFiU-3MnBmkmw0SbQL4YK_XAlqd5QSVtyUbkBMM7ZNLzqjn

Scale = 1:25.0



LUMBER

N.L.G.A. RULES	SIZE	LUMBER
CHORDS	2x4	DRY No.2
E - B	2x4	DRY No.2
A - C	2x4	DRY No.2
E - D	2x4	DRY No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMV+p	MT20	3.0	4.0		
E BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
JT						
E	648	0	648	0	5-8	5-8
C	225	0	225	0	1-8	1-8
D	93	0	119	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
JT						
E	473	292/0	72/0	0/0	0/0	109/0
C	154	128/0	0/0	0/0	0/0	26/0
D	85	0/0	91/0	0/0	0/0	34/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LOAD LC1	MAX. CSI (LC)	MAX. UNBRAC LENGTH	MEMB. FR-TO	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM TO						
E-B	-514/0	0.0	0.0	0.22 (3)	7.81			
A-B	0/31	-102.1	-102.1	0.13 (1)	10.00			
B-C	-34/0	-102.1	-102.1	0.60 (1)	6.25			
E-D	0/0	-38.5	-38.5	0.22 (3)	10.00			

TOTAL WEIGHT = 19 X 17 = 319 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.1 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPC 2011, TPC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/898 (0.04")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/842 (0.07")

CSI: TC=0.60/1.00 (B-C:1), BC=0.22/1.00 (D-E:3), WB=0.00/1.00 (n/a:0), SS=0.26/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTO SOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

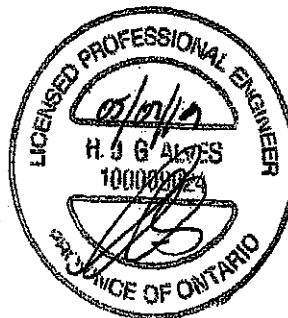
NAIL VALUES

PLATE GRIP (DRY)	SHEAR	SECTION
(PSI)	(PL)	(PL)
MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1667 788

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.21 (E) (INPUT = 0.90)
JSI METAL=0.14 (B) (INPUT = 1.00)



DWG NO. TAM 1910753
STRUCTURAL
COMPONENT ONLY

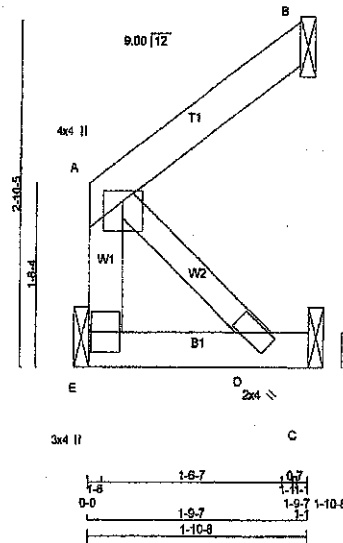
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
402979	J4	3	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					

Version 6.220 S Nov 17 2016 Mitek Industries, Inc. Wed May 8 15:07:55 2019 Page 1

ID:JPmXdERQr69GD5IO953aPwyJEIZ-g6zYoHr5Jxq3ZSVHqzNPVYHamRM353BsZ?HEsCzIXto

0-0 1-9.7 1-9.7 1-10.8

Scale = 1:17.7



TOTAL WEIGHT = 3 X 8 = 23 lb
(MIF)

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
E - A	2x4	DRY	No.2
A - B	2x4	DRY	No.2
E - C	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMVW+p	MT20	4.0	4.0	1.00 2.00
D	BMVW+p	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	DOWN	HORZ	BRG	IN-SX	BRG	IN-SX
E	127	0	127	0	0	1-8	1-8	1-8	1-8
B	91	0	91	0	0	1-8	1-8	1-8	1-8
C	38	0	46	0	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) E, B, C

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND
E	85	52/0	20/0	0/0	0/0
B	63	52/0	0/0	0/0	0/0
C	33	0/0	20/0	0/0	0/0

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS	
MEMB.	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX. CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)
FR-TO				FR-TO	
E-A	-91/0	0.0	0.0 (1)	A-D	0/0
A-B	0/0	-102.1	-102.1 (1)		0.00 (1)
E-D	0/0	-38.5	-38.5 (3)		10.00
D-C	0/0	-38.5	-38.5 (3)		10.00

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.19')
CALCULATED VERT. DEFL.(LL) = L/989 (0.00')
ALLOWABLE DEFL.(TL) = L/360 (0.19')
CALCULATED VERT. DEFL.(TL) = L/989 (0.00')

CSI: TC=0.05/1.00 (A-B:1), BC=0.03/1.00 (D-E:3),
WB=0.00/1.00 (A-D:1), SS=0.06/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

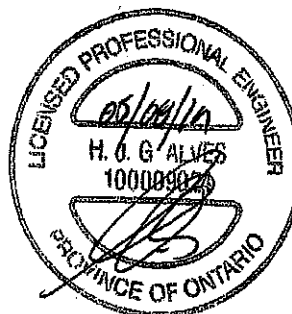
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1655

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.80)
JSI METAL= 0.02 (A) (INPUT = 1.00)

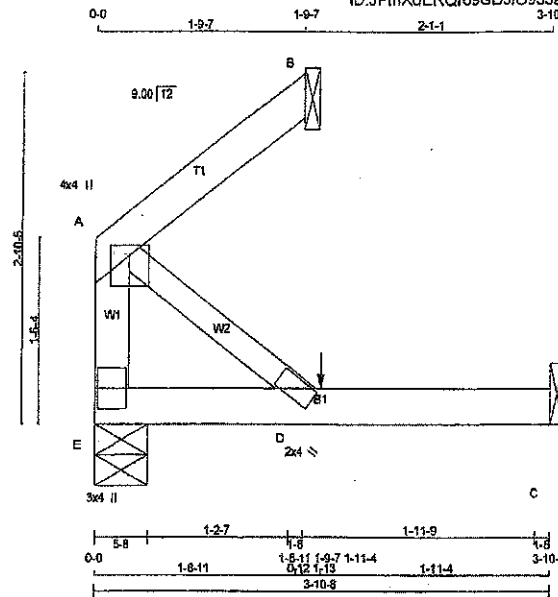


DWG NO. TAM 199/0974
STRUCTURAL
COMPONENT ONLY

JOB NAME 402979	TRUSS NAME J5	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Wed May 8 15:07:56 2018 Page 1
ID: JPMXdeRQr69GD5IO953aPwyJEIZ-8IXw7drk4FywBc5UWYue29piRqgdqVQ7of0oOfzIXin



Scale = 1:17.5

TOTAL WEIGHT = 10 lb [M]

LUMBER				DESCR.
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	
E - A	2x4	DRY	No.2	SPF
A - B	2x4	DRY	No.2	SPF
E - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

PLATES (table in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TW/VW+p	MT20	4.0	4.0	1.00 2.00
D	BMW+w	MT20	2.0	4.0	
E	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	BRG
E	166	0	188	0	0	5-8	5-8		
B	91	0	91	0	0	1-8	1-8		
C	75	0	95	0	0	1-8	1-8		

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) B, C

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
E	130	52/0	41/0	0/0	0/0	38/0	0/0
B	63	52/0	0/0	0/0	0/0	11/0	0/0
C	68	0/0	41/0	0/0	0/0	27/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E, B

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MEMB.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT.	LOAD (PLF)	CS1 (LC)	MAX.	MEMB.	FORCE (LBS)	MAX.	CS1 (LC)
FR-TO						FR-TO			
E-A	-91/0	0.0	0.0	0.01 (1)	7.81	A-D	0/0	0.00 (1)	
A-B	0/0	-102.1	-102.1	0.06 (1)	10.00				
E-D	0/0	-38.5	-38.5	0.14 (2)	10.00				
D-C	0/0	-38.5	-38.5	0.14 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
D	1-11-4	1	1	—	BACK	VERT	TOTAL	—	—

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.1 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.18")
CALCULATED VERT. DEFL. (LL) = $L/999$ (0.01")
ALLOWABLE DEFL. (TL) = $L/360$ (0.18")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.02")

CS1: TC=0.08/1.00 (A-B-1), BC=0.14/1.00 (C-D-2), WB=0.00/1.00 (A-D-1), SS=0.08/1.00 (D-E-2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

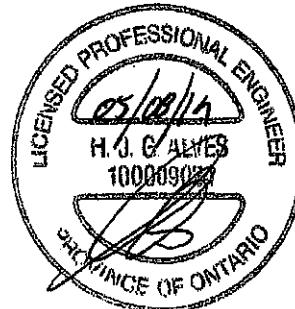
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(F5I) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1887 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.07 (A) (INPUT = 0.90)
JSI METAL= 0.02 (A) (INPUT = 1.00)

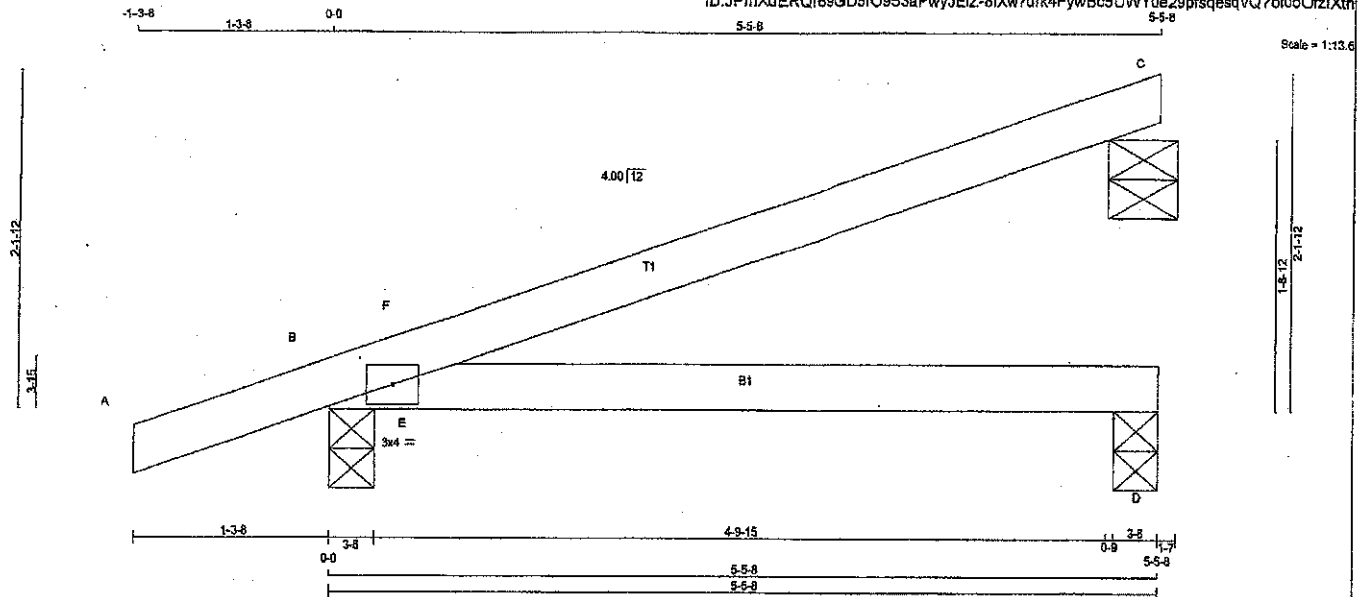


DWG NO. TAM 71910975
STRUCTURAL
COMPONENT ONLY

JOB NAME 402979	TRUSS NAME J6	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 6.230 S Nov 17 2016 M-Tek Industries, Inc. Wed May 8 15:07:58 2019 Page 1
ID:JPmXdERQr69GDSiO953aPwyJEIZ-8lXw?drk4FywBc5UWYue29pfsqesqVQ?of0cOfziXln



TOTAL WEIGHT = 2 X 14 = 28 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES	A - C	2x4	DRY	No.2
	B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEVELED PLATE OR SHIM REQUIRED TO PROVIDE FULL BEARING SURFACE WITH TRUSS CHORD AT JT(S): C.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LBS)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. (LBS)
FR-TO		FROM TO			FR-TO		
A-B	0/20	-102.1 -102.1	0.13 (1)	10.00	E-F	-254 / 122	0.00 (1)
B-F	-51 / 0	-102.1 -102.1	0.13 (3)	6.25			
F-C	0/6	-102.1 -102.1	0.42 (1)	10.00			
B-E	0/0	-38.5 -38.5	0.26 (1)	10.00			
E-D	0/0	-38.5 -38.5	0.31 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL =	8.0 PSF
BOT CH.	LL = 10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	52.6 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/842$ (0.08")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/459$ (0.13")

CSI: TC=0.42/1.00 (C-F:1), BC=0.31/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SS=0.25/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

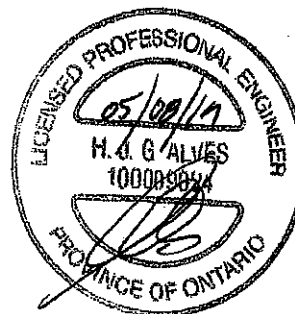
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1867 788 1987 1656

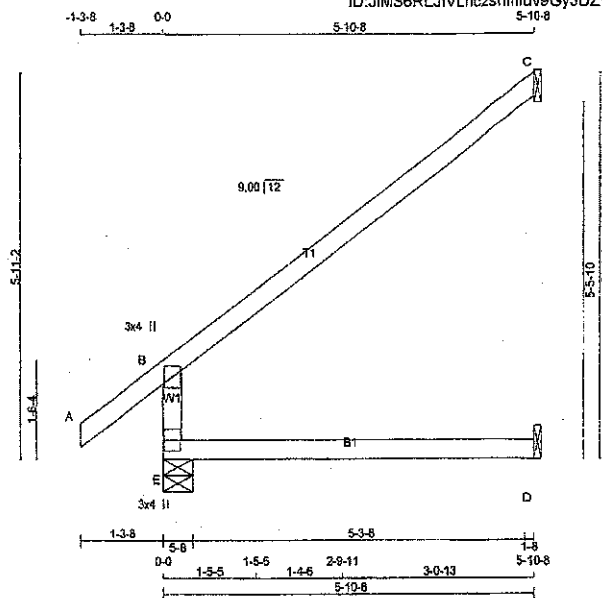
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (B) (INPUT = 0.80)
JSI METAL= 0.09 (B) (INPUT = 1.00)



DWG NO. TAM 71910976
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 18 = 37 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
E - B	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
E - D	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
E	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT	REQD
	VERT	HORZ	DOWN	HORZ
JT	575	0	575	0
E	225	0	225	0
C	43	0	43	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
E	401	293 / 0	0 / 0	0 / 0	0 / 0	108 / 0	0 / 0
C	154	128 / 0	0 / 0	0 / 0	0 / 0	26 / 0	0 / 0
D	35	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) E

BRACING

TOP CHORD TO BE SHEATHED OR MAX. FURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED HORIZ. LOAD (LC1)	MAX. MEMB. FORCE (LBS)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED FORCE (LBS)
FR-TO							
E-B		-516 / 0	0.0	0.0	0.11 (4)	7.81	
A-B		0 / 42	-102.1	-102.1	0.14 (1)	10.00	
B-C		-45 / 0	-102.1	-102.1	0.80 (1)	6.25	
E-D		0 / 0	-17.5	-17.5	0.13 (4)	10.00	

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 6.0 PSF	
BOT CH.	LL = 0.0 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 42.0 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS

- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.20")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL) = L/360 (0.20")
CALCULATED VERT. DEFL.(TL) = L/999 (0.03")

CSI: TC=0.60/1.00 (B-C:1), BC=0.13/1.00 (D-E:4), WB=0.00/1.00 (A:0), SS=0.23/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1687 788	1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.24 (B) (INPUT = 0.90)
JSI METAL = 0.18 (B) (INPUT = 1.00)

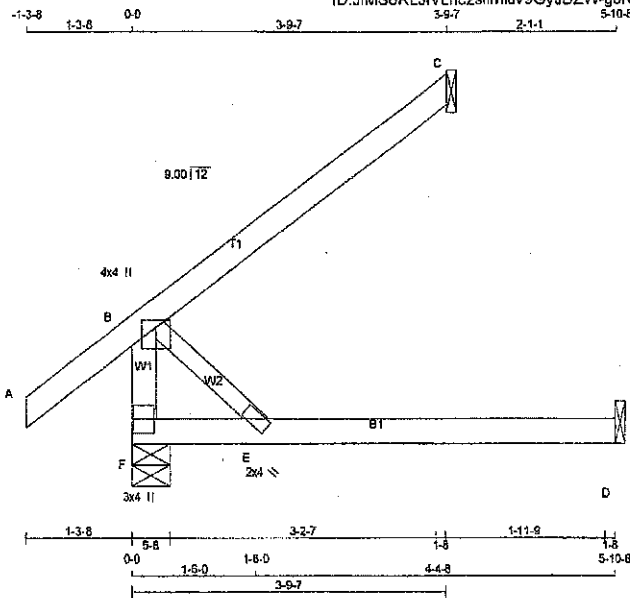


DRWG. NO. TAM 77910983
STRUCTURAL
CHECKED ONLY

JOB NAME 402981	TRUSS NAME J21	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 \$ Nov 17 2018 Mitek Industries, Inc. Wed May 8 15:15:36 2019 Page 1
ID: JIMS6RLJVLhc2snmluv9GyJDZW-gokqYvQGlaweNiGU9WhAJPGemcYJJaESBY_zlXmb



Scale = 1/25.3

TOTAL WEIGHT = 17 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2	SPF	
A - C	2x4	DRY	No.2	SPF	
F - D	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table in inches)	W	LEN	Y	X
JT TYPE PLATES	4.0	4.0	1.00	2.00
B T&Ww+p	MT20	2.0	4.0	
E BMVw+p	MT20	3.0	4.0	
F BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	RECORD
GROSS REACTION	GROSS REACTION	GROSS REACTION	BRG	BRG
JT VERT	HORIZ	DOWN	HORIZ	UPLIFT
IN-SX	IN-SX	IN-SX	IN-SX	IN-SX
F	447	0	447	0
C	193	0	193	0
D	113	0	113	0

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
F	332	190/0	62/0	0/0	0/0	80/0	0/0
C	133	110/0	0/0	0/0	0/0	23/0	0/0
D	103	0/0	62/0	0/0	0/0	41/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	MAX. FACTORED	WEBS	MAX. FACTORED
MEMB.	FORCE	VERT. LOAD LC1	MAX. CSI (LC)	MEMB.	FORCE
FR-TO	(LBS)	FROM TO	LENGTH FR-TO	FR-TO	(LBS)
F-B	-334/0	0.0	0.03 (1)	7.81	0/0
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00
B-C	0/0	-102.1	-102.1	0.25 (1)	10.00
F-E	0/0	-38.5	-38.5	0.23 (3)	10.00
E-D	0/0	-38.5	-38.5	0.30 (3)	10.00

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.1 PSF
DL = 8.0 PSF	
BOT CH.	LL = 10.5 PSF
DL = 7.0 PSF	
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NSCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2016, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = 1/925 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = 1/555 (0.13")

CSI: TC=0.25/1.00 (B-C:1), BC=0.30/1.00 (D-E:3), WB=0.00/1.00 (B-E:1), SS=0.12/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

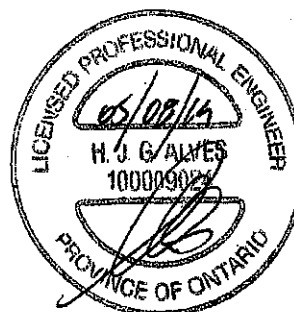
NAIL VALUES

PLATE	GRIP (DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1687
	788	1987	1655

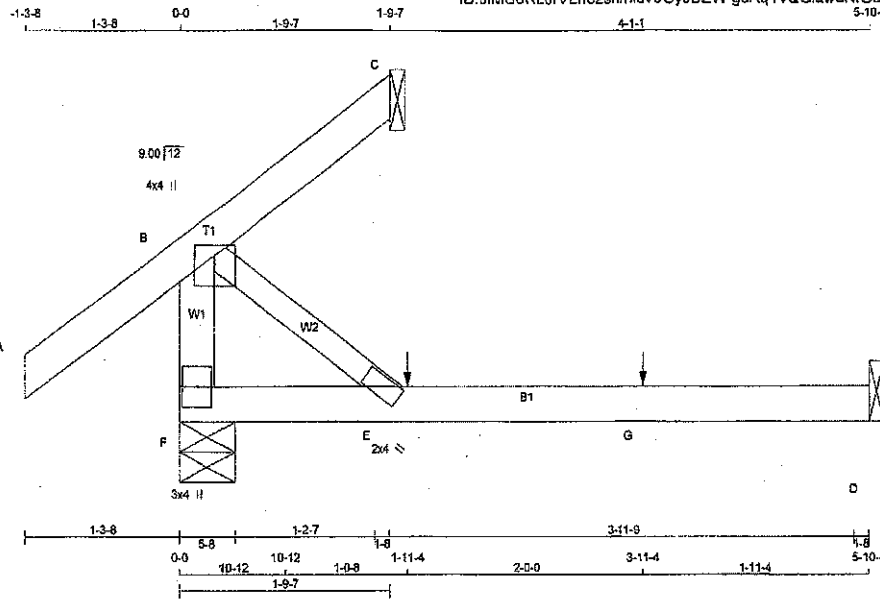
PLATE PLACEMENT TOL = 0.250 Inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.27 (B) (INPUT = 0.90)
JSI METAL = 0.07 (B) (INPUT = 1.00)



DATE: 05/08/19
TAM 7910984
STRUCTURAL
CONSULTANT ONLY



TOTAL WEIGHT = 14 lb

LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00 2.00
E	BMW+w	MT20	2.0	4.0	
F	BMV1+p	MT20	3.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	VERT	GROSS REACTION	HORIZ	GROSS REACTION	HORIZ	BRG	IN-SX	BRG	IN-SX
F	400	0	0	400	0	5-8	5-8	5-8	5-8
C	37	0	0	37	0	1-8	1-8	1-8	1-8
D	113	0	0	144	0	1-8	1-8	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS							
JT	1ST LCASE	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
F	289	163/0	62/0	0/0	0/0	75/0	0/0
C	25	21/0	0/0	0/0	0/0	4/0	0/0
D	103	0/0	62/0	0/0	0/0	41/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, C

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS (L.C)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS (L.C)	
FR-TO		FROM TO		FR-TO			
F-B	-286/0	0.0 0.0 0.03 (1)	7.81	B-E	0/0	0.00 (1)	
A-B	0/42	-102.1 -102.1 0.15 (1)	10.00				
B-C	-33/0	-102.1 -102.1 0.15 (1)	6.25				
F-E	0/0	-38.5 -38.5 0.25 (2)	10.00				
E-G	0/0	-38.5 -38.5 0.32 (2)	10.00				
G-D	0/0	-38.5 -38.5 0.32 (2)	10.00				

FACTORED CONCENTRATED LOADS (LBS)				FACE DIR.			
JT	LOC.	LC1	MAX.	FACE	DIR.	TYPE	HEEL
E	1-11-4	1	1	FRONT	VERT	TOTAL	CONN.
G	3-11-4	1	1	FRONT	VERT	TOTAL	CONN.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.1 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.20")
CALCULATED VERT. DEFL. (LL) = L/925 (0.08")
ALLOWABLE DEFL. (TL) = L/360 (0.20")
CALCULATED VERT. DEFL. (TL) = L/555 (0.13")

CS: TC=0.15/1.00 (A-B:1), BC=0.32/1.00 (D-E:2), WB=0.00/1.00 (B-E:1), SSI=0.12/1.00 (E-F:2)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

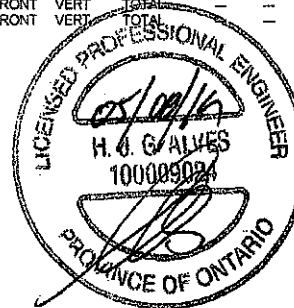
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1987 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.23 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DWG NO. TAM 1910985
STRUCTURAL
CHECK: NOT ONLY

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MiTek Industries, Inc. Wed May 8 15:15:37 2019 Page 1



TOTAL WEIGHT = 10 lb

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

DESIGN CRITERIA

SPECIFIED LOADS:

SPECIFIED LOADS:			
TOP CH.	LL =	29.1	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		=	52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL
OR SMALL BUILDING REQUIREMENTS OF
PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.1 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (0.19")
CALCULATED VERT. DEFL.(LL) = L/999 (0.00")
ALLOWABLE DEFL.(TL)= L/360 (0.18")
CALCULATED VERT. DEFL.(TL) = L/999 (0.00")

CSI: TC=0.25/1.00 (A-B:1), BC=0.03/1.00 (D-E:3),
WB=0.00/1.00 (A-D:1), SS=0.12/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES					
PLATE	GRIP (DRY) (PSI)		SHEAR (PLI)		SECTION (FLI)
	MAX	MIN	MAX	MIN	MAX MIN
MT20	618	354	1667	788	1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.15 (A) (INPUT = 0.90)
JSI METAL= 0.04 (A) (INPUT = 1.00)



BMG COL TAM 77910986
STRUCTURAL
CLASSIFICATION ONLY



isDesign™

Client:
Project:
Address:

Date: 5/7/2019

Page 1 of 4

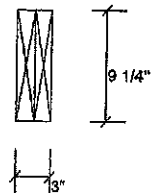
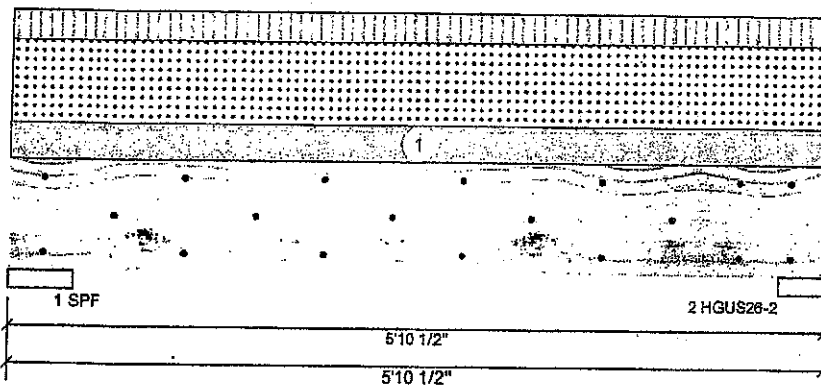
Designer:

Job Name: 201007

Project #:

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	207	257	583	0
2	199	246	558	0

Bearings and Factored Reactions

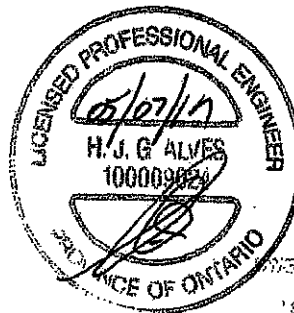
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	6.500"	14%	321 / 1081	1402	L	1.25D+1.5S +L
2 - HGUS...	4.000"	18%	308 / 1036	1344	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1585 ft-lb	3'	6039 ft-lb	0.262 (26%)	1.25D+1.5S +L	L
Unbraced	1585 ft-lb	3'	5236 ft-lb	0.303 (30%)	1.25D+1.5S +L	L
Shear	1217 lb	1'2"	3984 lb	0.306 (31%)	1.25D+1.5S +L	L
LL Defl inch	0.014 (L/4497)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.019 (L/3273)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



17910792
STRUCTURAL
ENGINEER ONLY
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		6-7-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3269 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021





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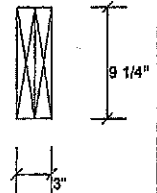
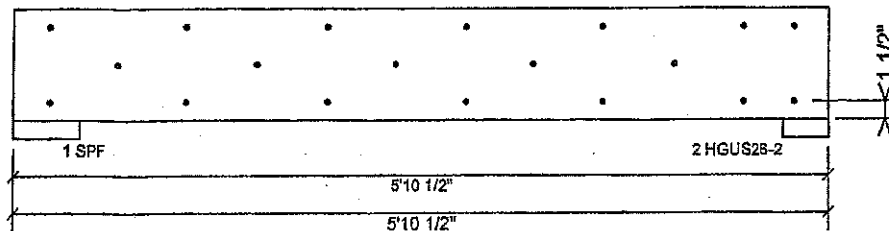
Client:
Project:
Address:

Date: 5/7/2019
Designer:
Job Name: 201007
Project #:

Page 2 of 4

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

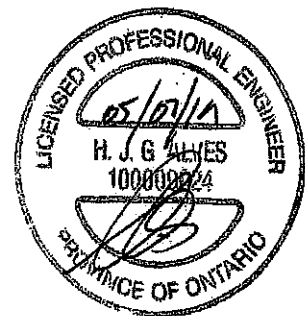
Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (120x3.25") at 12" o.c., Maximum end distance not to exceed 6"

Capacity	88.7 %
Load	233.7 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



FILE NO. 1910792
STRUCTURAL
CITY OF TORONTO
7/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 336-1115



This design is valid until 12/11/2021





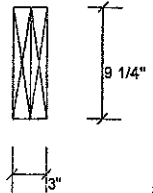
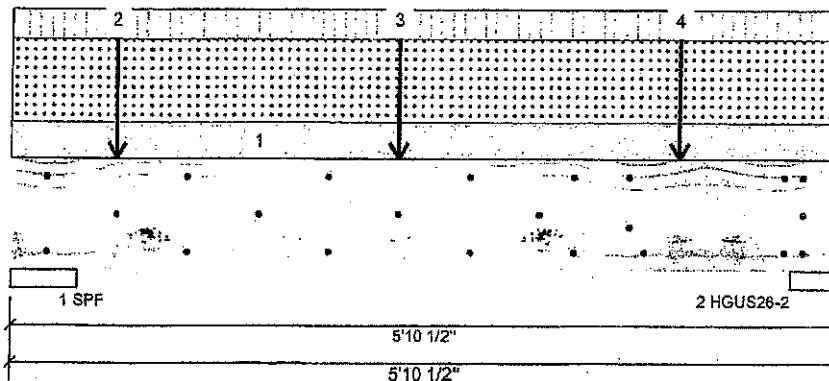
isDesign™

Client:
Project:
Address:Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 1 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	155	203	456	0
2	180	233	523	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	11%	254 / 839	1093 L	1,25D+1.5S +L
2 - HGUS...	4.000"	17%	291 / 984	1255 L	1,25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1791 ft-lb	2'9"	6039 ft-lb	0.297 (30%)	1.25D+1.5S +L	L
Unbraced	1791 ft-lb	2'9"	5236 ft-lb	0.342 (34%)	1.25D+1.5S +L	L
Shear	1217 lb	4'10"	3984 lb	0.305 (31%)	1.25D+1.5S +L	L
LL Defl inch	0.014 (L/4361)	3'	0.174 (L/360)	0.080 (8%)	S+0.5L	L
TL Defl inch	0.020 (L/3157)	3'	0.174 (L/360)	0.110 (11%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



ENGINEER TAM 71910994
STRUCTURAL
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	0-9-0		Far Face	24 lb	20 lb	52 lb	0 lb	j4
3	Point	2-9-0		Far Face	157 lb	110 lb	352 lb	0 lb	t15c
4	Point	4-9-0		Far Face	102 lb	82 lb	228 lb	0 lb	t15

Manufacturer Info

Tamarack Roof Trusses
3259 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021





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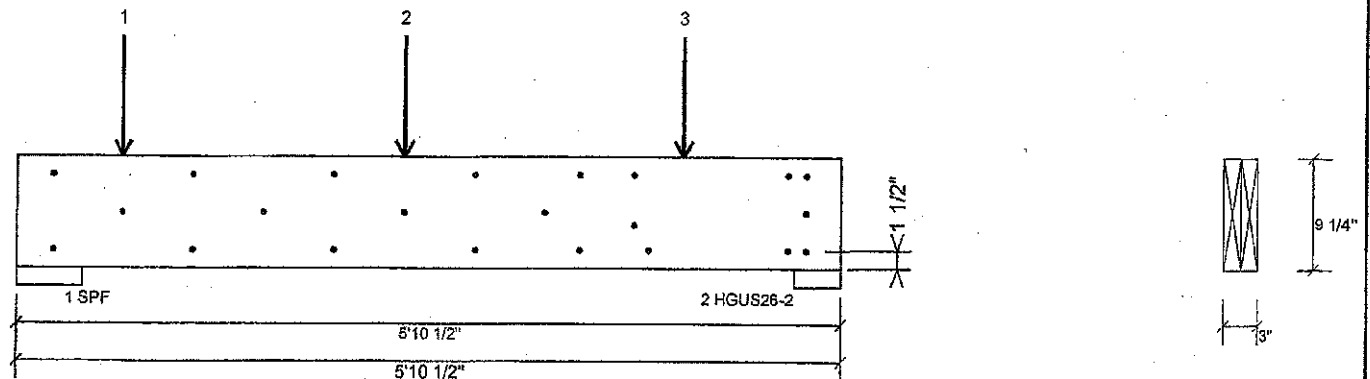
Client:
Project:
Address:

Date: 5/8/2019
Designer:
Job Name: 402979
Project #:

Page 2 of 4

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

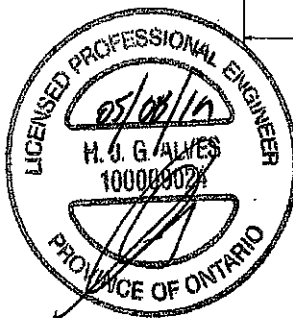
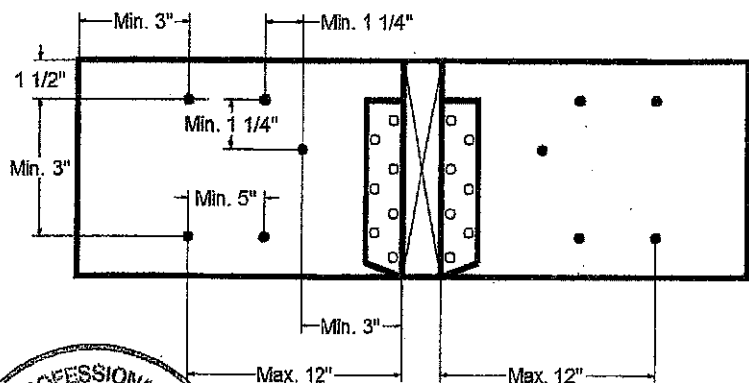
Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 4-9-0 with a minimum of (3) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	81.1 %
Load	275.8 lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads



DESIGN BY 19/0999
STRUCTURAL
CHECKED ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021



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Client:
Project:
Address:

Date: 5/8/2019

Page 3 of 4

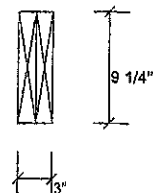
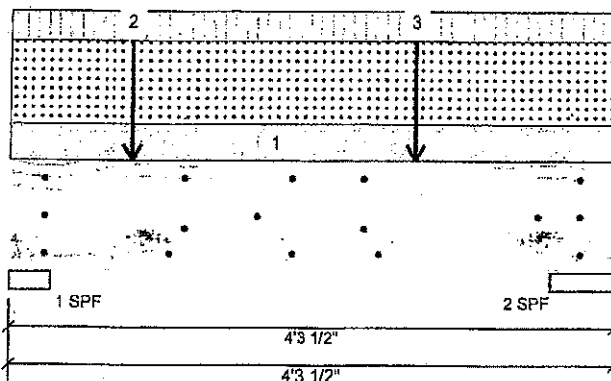
Designer:

Job Name: 402679

Project #:

BM3 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2016
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	143	184	430	0
2	127	159	360	0

Bearings and Factored Reactions

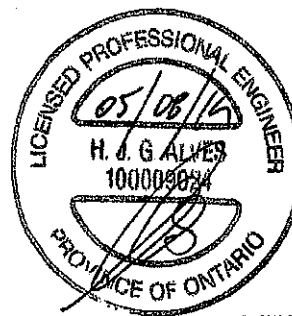
Bearing	Length	Cap. React	D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	16%	230 / 788	1018	L	1.25D+1.5S +L
2 - SPF	5.500"	9%	198 / 667	866	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	769 ft-lb	2'4 5/8"	8039 ft-lb	0.127 (13%)	1.25D+1.5S +L	L
Unbraced	769 ft-lb	2'4 5/8"	5641 ft-lb	0.136 (14%)	1.25D+1.5S +L	L
Shear	985 lb	1'	3984 lb	0.247 (25%)	1.25D+1.5S +L	L
LL Defl inch	0.004 (L12347)	2' 7/8"	0.122 (L/360)	0.030 (3%)	S+0.5L	L
TL Defl inch	0.005 (L8004)	2' 7/8"	0.122 (L/360)	0.040 (4%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top braced at bearings.
- 6 Bottom braced at bearings.
- 7 Lateral slenderness ratio based on single ply width.



DESIGNER: TAM 17910995
STRUCTURAL
1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		2-0-0	Near Face	13 PSF	10.5 PSF	29.5 PSF	0 PSF	
2	Point	0-10-8		Far Face	121 lb	91 lb	291 lb	0 lb	
3	Point	2-10-8		Far Face	110 lb	89 lb	246 lb	0 lb	

Manufacturer Info

Tamarack Roof Trusses
3268 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





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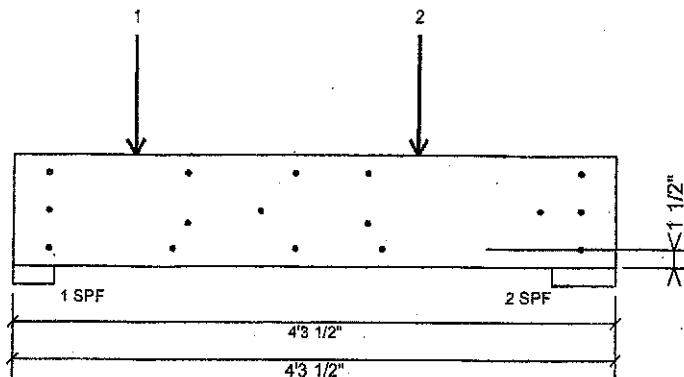
Client:
Project:
Address:

Date: 5/8/2019

Page 4 of 4

Designer:
Job Name: 402979
Project #:**BM3 S-P-F #1 2.000" X 10.000" 2-Ply - PASSED**

Level: Level

**Multi-Ply Analysis**

Fasten all plies using 3 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 6"

Capacity	20.9 %
Load	71.0 PLF
Yield Limit per Foot	340.0 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

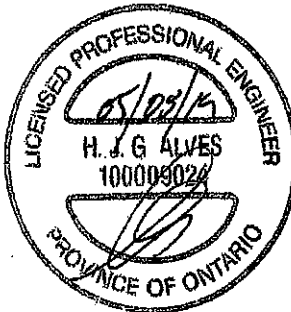
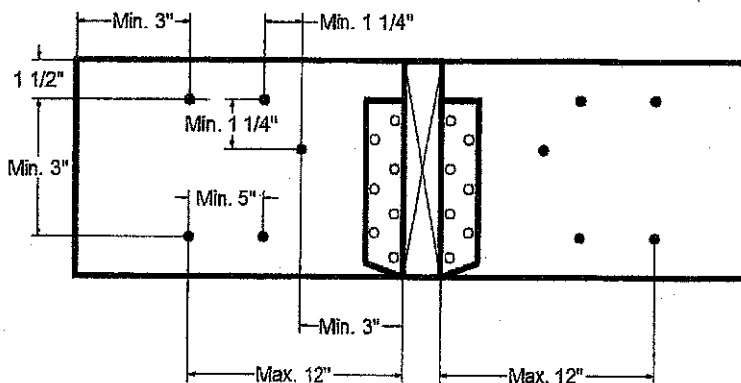
Fasten at concentrated side load at 0-10-8 with a minimum of (3) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	99.8 %
Load	339.4lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Concentrated Load

Fasten at concentrated side load at 2-10-8 with a minimum of (3) - Pneumatic Gun Nail (.120x3.25") in the pattern shown.

Capacity	87.6 %
Load	297.8lb.
Total Yield Limit	340.0 lb.
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

Min/Max fastener distances for Concentrated Side Loads

PROFESSIONAL ENGINEER
1610995
STRUCTURAL
DESIGN ONLY 7/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(805) 335-1115



This design is valid until 12/11/2021

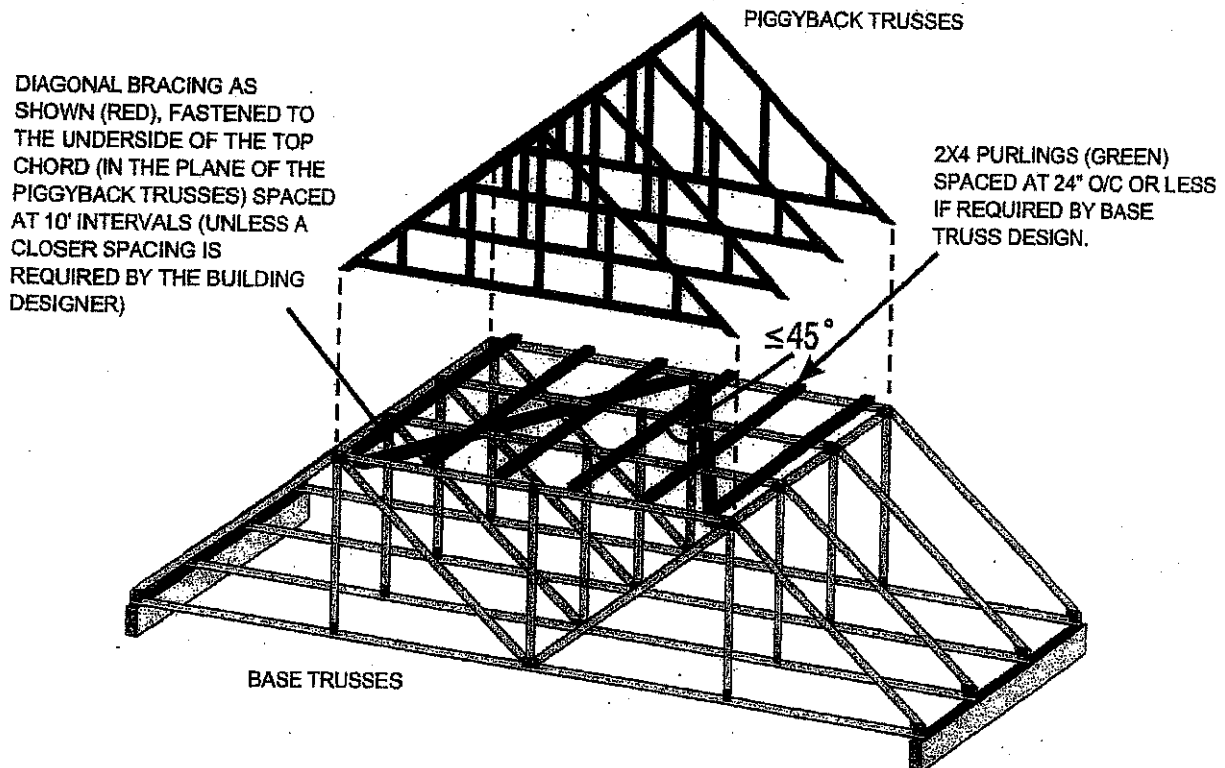


Overview:

Where piggybacks are connected ovetop of base trusses, 2x4 purlins must be first added to the flat portion of the base truss at a spacing no more than 24" o/c. These purlins not only provide support for the piggyback trusses above, but are required to laterally support the top chord of the base truss which will not have the sheathing directly connected to the flat portion of the base truss. This ensures the top chord, most often in compression, will not buckle laterally.

Further, the purlins in the plane of the flat portion require diagonal bracing to prevent lateral displacement of the purlins themselves where under certain conditions, the trusses may in fact all buckle in the same direction if this additional bracing is not added in the plane of the purlins.

Detail:



NOTE: THE SLOPED PORTION OF THE TOP CHORD OF THE BASE TRUSS AND PIGGYBACK TRUSS IN THIS SKETCH IS ASSUMED TO BE SHEATHED IN ACCORDANCE WITH THE OBC.

SKETCH FROM BCSI-CANADA 2013

Disclaimer:

OWTFA Tech Notes are intended to provide guidance to the design community both within the membership as well as to third party designers who might benefit from the information. The details have been developed by the OWTFA technical committee and although there may be professional engineers involved in development, the information contained in the tech-note are not intended to be used without having a professional engineer review the information for a specific application. The OWTFA takes no responsibility with respect to the information provided but has developed this tech-note to offer guidance where it is not currently readily available.

LUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All LUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections.

Material: 18 gauge

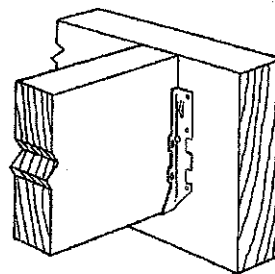
Finish: G90 galvanized

Design:

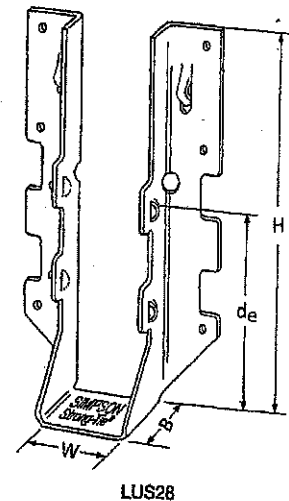
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

- Use all specified fasteners.
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire, 10d = 0.148" x 3" long common wire.
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads.
- Not designed for welded or nailer applications.



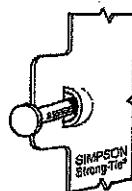
Typical LUS Installation



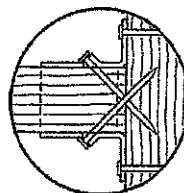
LUS28

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _e ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _o =1.15)	Normal (K _o =1.00)	Uplift (K _o =1.15)	Normal (K _o =1.00)
LUS24	18	1 1/8	3 1/8	1 3/4	1 1/8	(4) 10d	(2) 10d	710	1630	645	1155
LUS24-2	18	3 1/8	3 1/8	2	1 1/8	(4) 16d	(2) 16d	835	2020	590	1435
LUS26	18	1 1/8	4 1/8	1 3/4	3 3/8	(4) 10d	(4) 10d	1420	2170	1290	1630
LUS26-2	18	3 1/8	4 1/8	2	4	(4) 16d	(4) 16d	1720	2595	1545	1920
LUS26-3	18	4 1/8	4 1/8	2	3 3/4	(4) 16d	(4) 16d	1720	2595	1545	2340
LUS28	18	1 1/8	6 1/8	1 3/4	3 3/8	(6) 10d	(6) 10d	1420	2520	1290	1790
LUS28-2	18	3 1/8	7	2	4	(6) 16d	(4) 16d	1720	3325	1545	2575
LUS28-3	18	4 1/8	6 1/8	2	3 3/4	(6) 16d	(4) 16d	1720	3325	1545	2375
LUS210	18	1 1/8	7 1/8	1 3/4	3 3/8	(8) 10d	(4) 10d	1420	2785	1290	2210
LUS210-2	18	3 1/8	9	2	6	(8) 16d	(6) 16d	2580	4500	2320	3195
LUS210-3	18	4 1/8	8 1/8	2	5 1/4	(8) 16d	(6) 16d	2580	3345	2320	2375

1. d_e is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,680



Double Shear Nailing Top View.



For more information, visit www.simpsonstrongtie.com
Simpson Strong-Tie Company, Inc.
1000 Simpson Strong-Tie Company, Inc.
P.O. Box 1000, Simpson Strong-Tie Company, Inc.
Simpson Strong-Tie Company, Inc.

(800) 999-5099
strongtie.com

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: See table

Finish: G90 galvanized

Design:

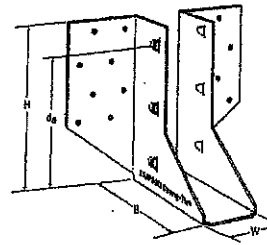
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

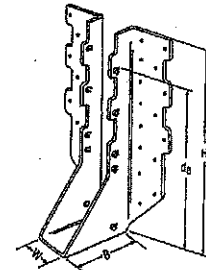
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

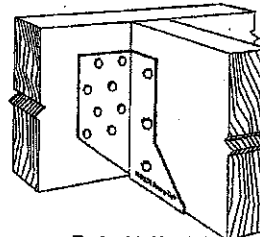
- See current catalogue for options



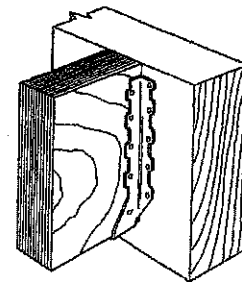
LJS26DS



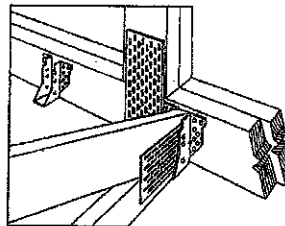
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



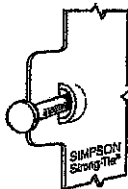
Typical HUS
Installation



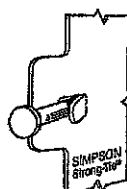
Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _s ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _o =1.00)	Uplift (K _u =1.15)	Normal (K _o =1.00)
LJS26DS	18	1 1/16	5	3 1/2	4 1/8	(16) 16d	(6) 16d	2055	4265	1460	4115
HUS26	16	1 1/8	5 1/8	3	3 1/8	(14) 16d	(6) 16d	2705	4940	2065	3875
HUS28	16	1 1/8	7 3/8	3	6 3/8	(22) 16d	(8) 16d	3605	5385	2875	4345
HUS210	16	1 1/8	9 3/8	3	7 3/8	(30) 16d	(10) 16d	4505	5795	4010	4740
HUS1.81/10	16	1 1/8	9	3	8	(30) 16d	(10) 16d	4505	6450	4010	5200

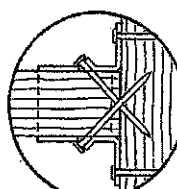
1. d_s is the distance from the seat of the hanger to the highest joist nail.



Dome Double
Shear Nailing
prevents tabs
breaking off
(available on
some models).
U.S. Patent
5,603,580



Double
Shear
Nailing
Side View.
Do not
bend tab
back.



Double
Shear
Nailing
Top View.



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HGUS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

Material: 12 gauge

Finish: G90 galvanized

Design:

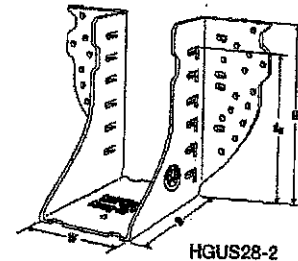
- Factored resistances are in accordance with CSA O86-14.
- Uplift resistances have been increased 15%.
No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

Installation:

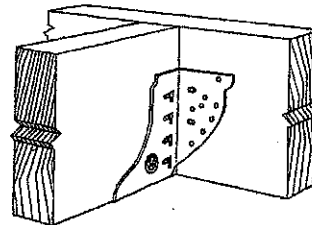
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

Options:

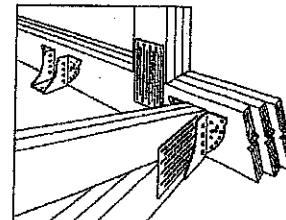
- See current catalogue for options



HGUS28-2



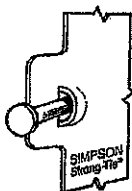
Typical HGUS
Installation



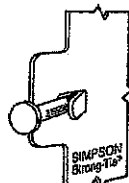
Typical HGUS
Installation
(Truss Designer to
provide fastener
quantity for
connecting multiple
members together)

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance (lb.)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
HGUS26	12	1 1/8"	5 1/8"	5	4 1/2"	(20) 16d	(8) 16d	2685	6625	2685	5700
HGUS26-2	12	3 1/8"	5 1/8"	4	4 1/8"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-3	12	4 1/8"	5 1/8"	4	4 1/8"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS26-4	12	6 1/8"	5 1/8"	4	4 1/8"	(20) 16d	(8) 16d	4385	8950	3100	6355
HGUS28	12	1 1/8"	7 1/8"	5	6 1/8"	(36) 16d	(12) 16d	3310	7675	3100	6900
HGUS28-2	12	3 1/8"	7 1/8"	4	6 1/8"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-3	12	4 1/8"	7 1/8"	4	6 1/8"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS28-4	12	6 1/8"	7 1/8"	4	6 1/8"	(36) 16d	(12) 16d	6070	12980	4310	9215
HGUS210-2	12	3 1/8"	9 1/8"	4	8 1/8"	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-3	12	4 1/8"	9 1/8"	4	8 1/8"	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS210-4	12	6 1/8"	9 1/8"	4	8 1/8"	(46) 16d	(16) 16d	6840	14645	4855	10400
HGUS212-4	12	6 1/8"	10 1/8"	4	10 1/8"	(56) 16d	(20) 16d	7640	14995	5425	10645
HGUS214-4	12	6 1/8"	12 1/8"	4	11 1/8"	(86) 16d	(22) 16d	10130	16400	7185	11645

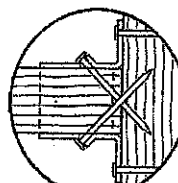
1. d_g is the distance from the seat of the hanger to the highest joist nail.



Dome Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,603,580



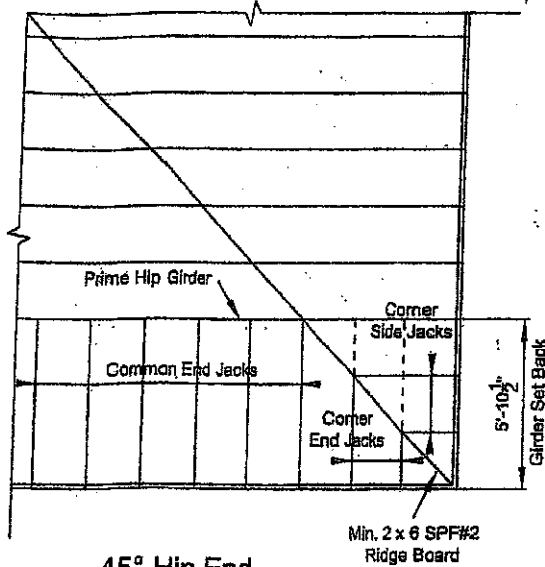
Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



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45° Hip End

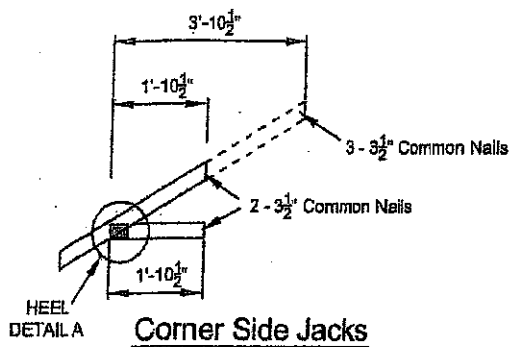
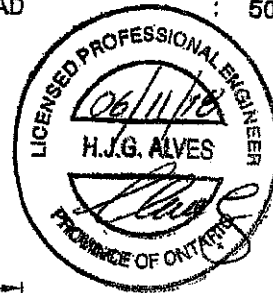
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

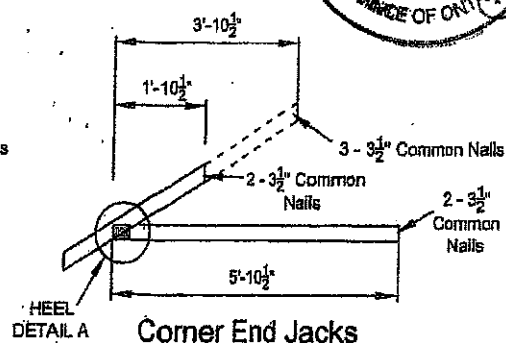
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

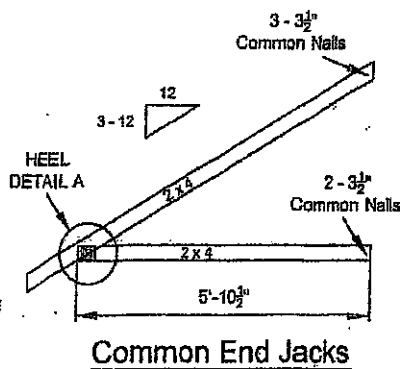
TOTAL LOAD : 50.5 P.S.F



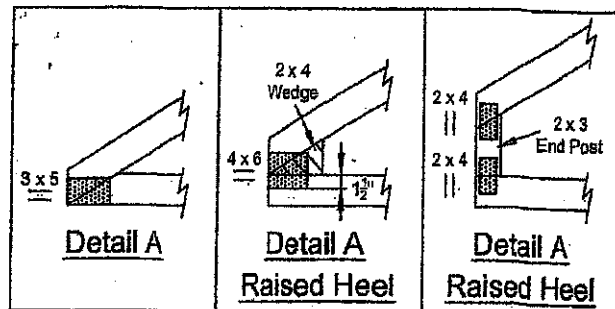
Corner Side Jacks



Corner End Jacks

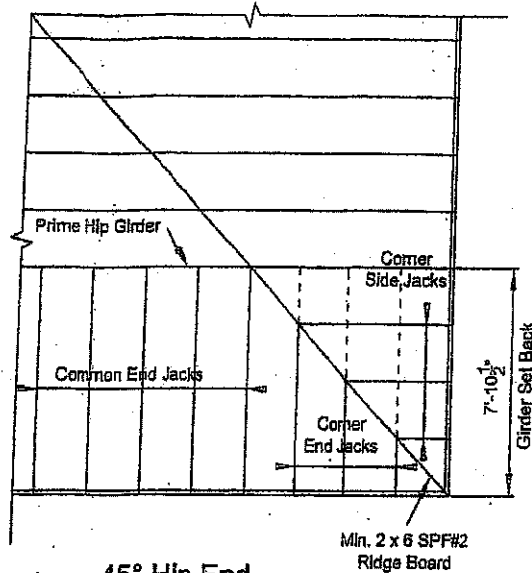


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

Min. 2 x 6 SPF#2
Ridge Board

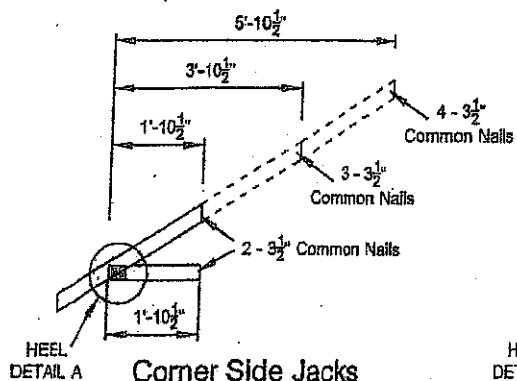
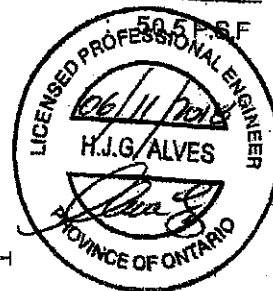
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

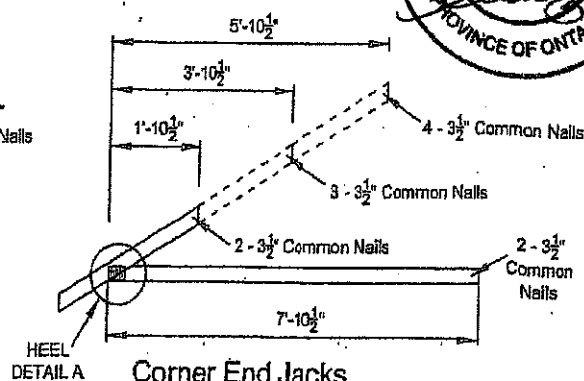
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

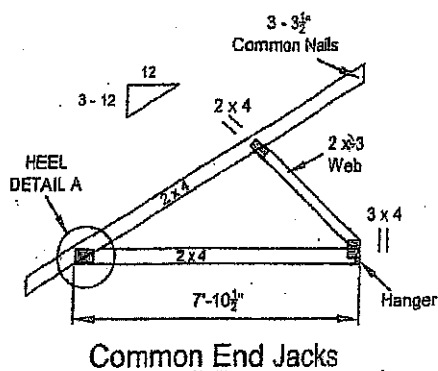
TOTAL LOAD



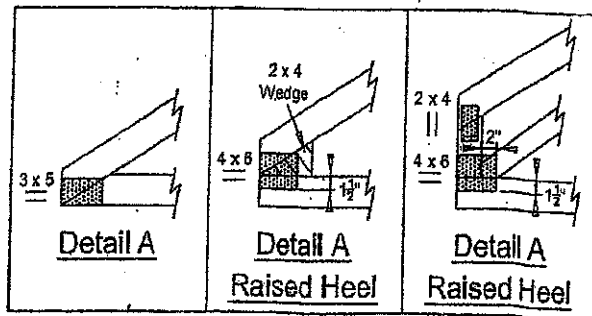
Corner Side Jacks



Corner End Jacks



Common End Jacks

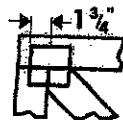


NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

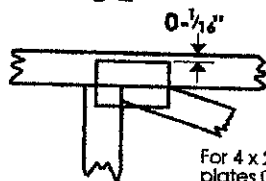
T-1800217

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/8" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



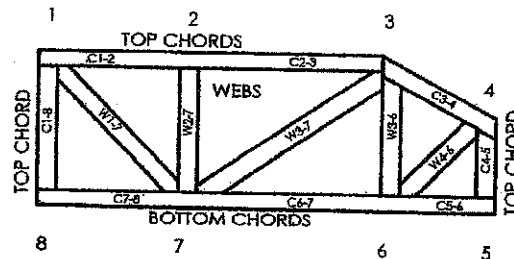
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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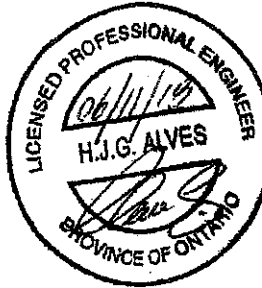
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MHI-7473C rev. 10-08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1900219

Feb 09, 2018