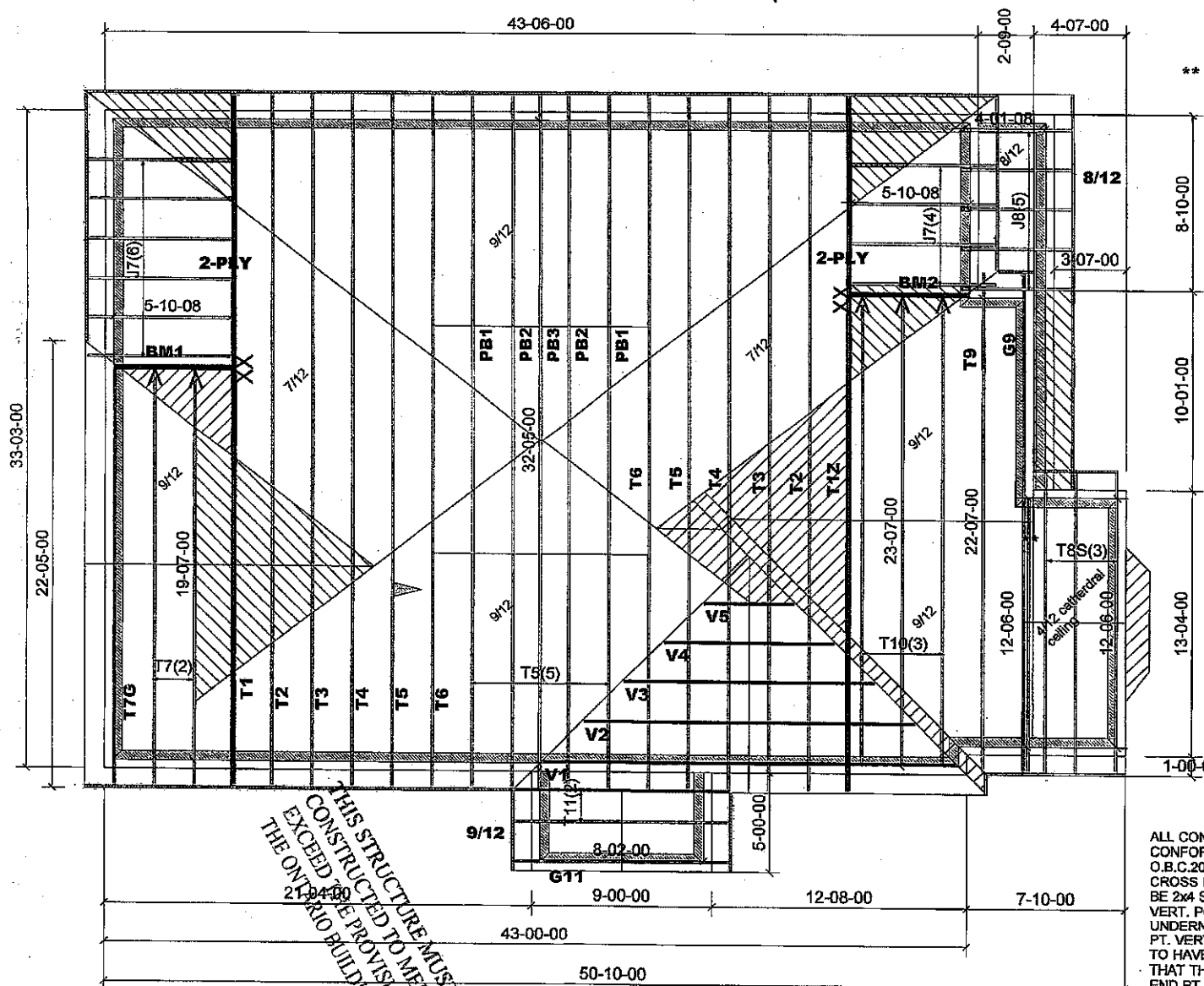




**** NAIL 2-2X10 BESIDE G9 FOR CONTINUOUS BEARING SUPPORT**

ASPHALT SHINGLES
12" FINISHED OH.
R.T.M.C.
2X6 EXTERIOR WALLS
2X6 FASCIA BOARD

HARDWARE:
LJS26DS - (V)
HGUS26-2 - (XX)

DENOTES
CONV.
FRAMING

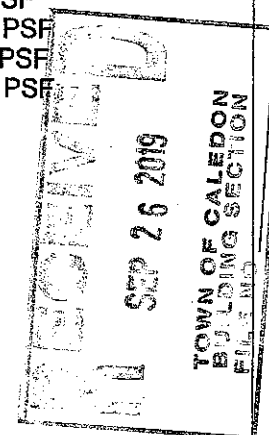
GSL 37.6 PSF

DESIGN CONFORMS
WITH O.B.C. 2018 PART 9
DESIGN LOADS:

TC LIVE 29 PSF
TC DEAD 6 PSF
BC LIVE 10.5 PSF
BC DEAD 7 PSF

BM1,2= 2- 2x10

ALL CONV. FRAMINGS TO
CONFORM WITH PART 9 OF
O.B.C.2012 ROOF RAFTERS THAT
CROSS MEET OVER TRUSSES TO
BE 2x4 SPF @ 24" O.C. WITH A 2x4
VERT. POST TO THE TRUSS
UNDERNEATH AT EACH CROSS
PT. VERT. POST LONGER THAN 6'
TO HAVE LATERAL BRACING SO
THAT THE DISTANCE BETWEEN
END PT. & BETWEEN ROWS OF
BRACING DOES NOT EXCEED 6'



Job Track: **50033**
Plan Log: **200666**
Layout ID: **401827**

Builder / Location:

GREEN PARK HOMES / CALEDON

Project: **LAMBERT LANE PH.2**

Date: 2019-03-21 Sales: Mario DiCano Designer: Chris JG

Model / Elevation:

KINMOUNT 11B / 1 Lot 45

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Milex ver 8.2.3.228



DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: KINMOUNT 11B
 Lot #: 45
 Elevation: 1

Job Track: 50120
 PlanLog: 200659
 Layout ID: 401811
 Ref #
 Page: 2 of 2
 Date: 03/13/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	G11 GABLE	9/12	8-02-00	4-07-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	38.15 26.00		
	1	V1 Valley	9/12	20-05-08	7-08-01	2 x 4			78.75 51.33		
	1	V2 Valley	9/12	16-05-08	6-02-01	2 x 4			57.19 35.67		
	1	V3 Valley	9/12	12-05-08	4-08-01	2 x 4			34 21.83		
	1	V4 Valley	9/12	8-05-08	3-02-01	2 x 4			22.51 14.17		
	1	V5 Valley	9/12	4-05-08	1-08-01	2 x 4			10.2 7.33		
	2	PB1 Piggyback	9/12	11-05-09	2-04-00	2 x 4			66.31 43.33		
	2	PB2 Piggyback	9/12	11-05-09	3-06-00	2 x 4			68.93 44.67		
	1	PB3 Piggyback	9/12	11-05-09	4-03-09	2 x 4			30.72 20.00		
	10	J7 Jack-Open	7/12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	186.14 118.67		
	5	J8 Jack-Open	8/12	4-01-08	3-11-09	2 x 4	1-03-08	4-07 3-01-07	64.24 43.33		

TOTAL # TRUSS= 58

TOTAL BFT OF ALL TRUSSES= 3020.49

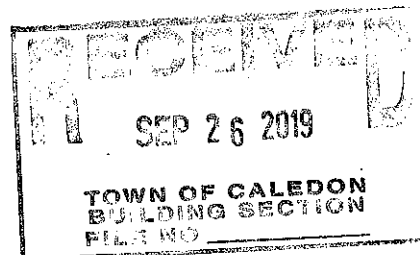
BFT.

TOTAL WEIGHT OF ALL TRSSES 4814.25 LBS

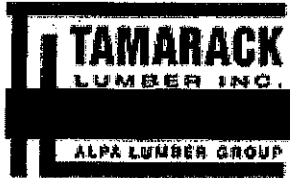
HARDWARE

QTY	TYPE	MODEL	LENGTH
2	Hardware	HGUS26-2	
5	Hardware	LJS26DS	

TOTAL NUMBER OF ITEMS= 7



DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: Lamberts Lane Home Corp.
 Location: Caledon
 Model: KINMOUNT 11B
 Lot #: 45
 Elevation: 1

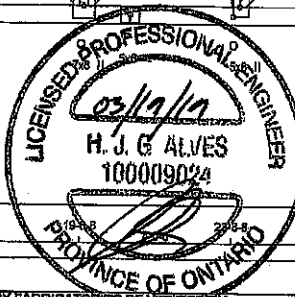
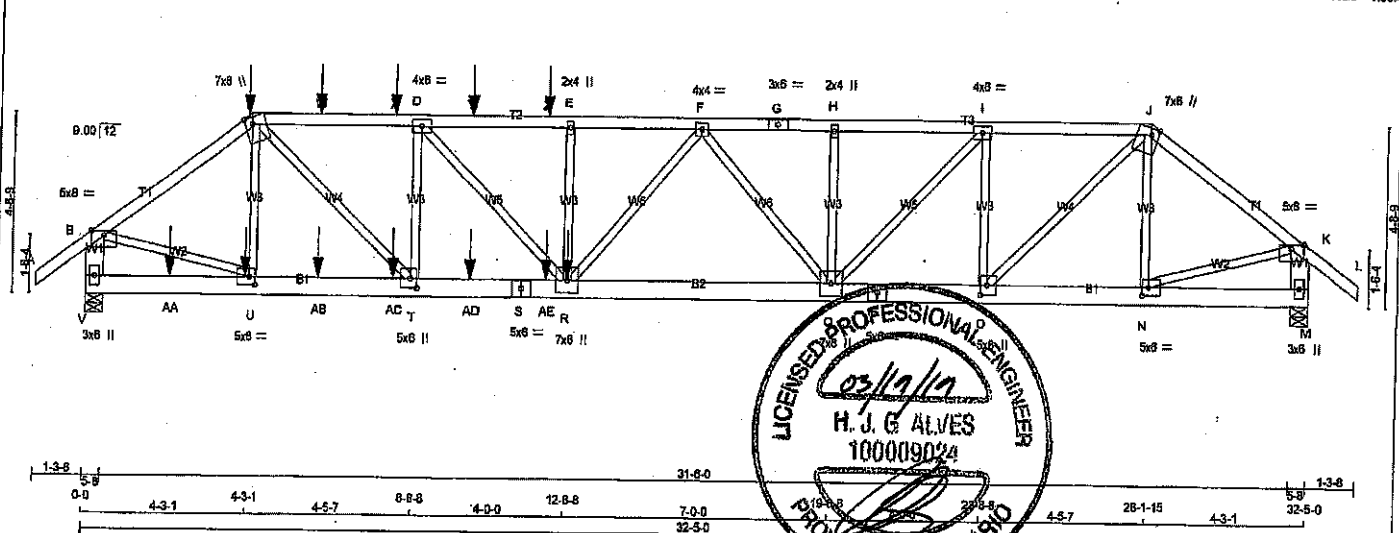
Job Track: 50120
 PlanLog: 200659
 Layout ID: 401811
 Ref #
 Page: 1 of 2
 Date: 03/13/2019
 Designer: Jane Gong
 Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T1 Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	1 2-ply	T12 Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	2	T2 Hip	9/12	32-05-00	5-10-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.98 186.00		
	2	T3 Hip	9/12	32-05-00	7-00-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	286.62 182.33		
	2	T4 Hip	9/12	32-05-00	8-02-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	303.91 191.33		
	7	T5 Hip	9/12	32-05-00	9-04-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	1116 698.83		
	2	T6 Hip	9/12	32-05-00	10-06-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	335.48 208.00		
	2	T7 Common	9/12	19-07-00	8-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	179.67 114.00		
	1	T7G GABLE	9/12	19-07-00	8-10-06	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	96.04 62.50		
	3	T8S Scissor	9/12 4/12	12-06-00	6-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	170.08 113.00		
	1	T9 Common	9/12	22-07-00	9-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	108.62 68.00		
	1	G9 GABLE	9/12	22-07-00	9-11-14	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	114.49 72.33		
	3	T10 Common	9/12	23-07-00	9-11-14	2 x 4		9-04 1-06-04	321.53 202.50		
	2	T11 Common	9/12	8-02-00	4-07-00	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	74.13 48.00		

JOB NAME 401811	TRUSS NAME T1	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 18 14:03:20 2018 Page 1
ID:mxsiFIQ02DDGzD9xpGq4yKxQY-VdTHIEQHofzIBIBndbU6XognhXjbnzOIJ2GxzZMcL
1-3-8 0-0 4-3-1 4-3-1 5-1-12 5-1-12 8-8-8 4-0-0 12-8-8 3-6-0 16-2-8 3-8-0 18-8-8 4-0-0 23-8-8 4-5-7 28-1-15 4-3-1 32-5-0 1-3-8
Scale = 1:59.4



LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - C	2x4	DRY	No.2
C - G	2x4	DRY	No.2
G - J	2x4	DRY	No.2
J - L	2x4	DRY	No.2
L - B	2x6	DRY	No.2
M - K	2x6	DRY	No.2
V - S	2x6	DRY	No.2
S - P	2x6	DRY	No.2
P - M	2x6	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2

DRY: SEASONED LUMBER.
DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS: (0.122"x3") SPIRAL NAILS		
A-C	12	SIDE (61.0)
C-G	12	SIDE (61.0)
G-J	12	TOP
J-L	12	TOP
L-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS: (0.122"x3") SPIRAL NAILS		
V-S	12	SIDE (183.1)
S-P	12	SIDE (183.1)
P-M	12	TOP
WEBS: (0.122"x3") SPIRAL NAILS		
U-C	6	SIDE (24.1)
E-R	6	SIDE (318.3)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.
GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.
TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWVW+m	MT20	7.0	8.0	Edge 2.50
D	TMWV-i	MT20	4.0	6.0	
E	TMWV-w	MT20	2.0	4.0	
F	TMWV-i	MT20	4.0	4.0	
G	TS-C	MT20	3.0	6.0	
H	TMWV-w	MT20	2.0	4.0	
I	TMWV-i	MT20	4.0	6.0	
J	TTWVW+m	MT20	7.0	8.0	Edge 2.50
K	TMWV-p	MT20	5.0	8.0	Edge
M	BMVW-i	MT20	3.0	6.0	
N	BMWV-i	MT20	5.0	6.0	2.50 2.00
O	BMWV-i	MT20	5.0	6.0	3.00 2.00
P	BS-4	MT20	5.0	6.0	
Q	BMWVW+i	MT20	7.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
V	4988	0	4988	0	5-8
M	3567	0	3567	0	5-8

UNFACTORED REACTIONS		1ST LCASE	MAX./MIN. COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE
V	3688	2132 / 0	672 / 0
M	2647	1522 / 0	486 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.66 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS		MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CS (LC) UNBRAC LENGTH	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
MEMB.	FR-TO	FROM	TO	FR-TO	MEMB.	FROM	TO
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	U-C	-780 / 0
B-C	-5831 / 0	-102.1	-102.1	0.34 (1)	3.85	C-T	0 / 4398
C-W	-7834 / 0	-102.1	-102.1	0.58 (1)	3.08	T-D	-2753 / 0
W-X	-7834 / 0	-102.1	-102.1	0.58 (1)	3.08	D-R	0 / 2426
X-D	-7834 / 0	-102.1	-102.1	0.58 (1)	3.08	R-E	-582 / 0
D-Y	-8279 / 0	-102.1	-102.1	0.68 (1)	2.88	E-F	0 / 1228
Y-Z	-8279 / 0	-102.1	-102.1	0.68 (1)	2.88	F-Q	-1695 / 0
Z-E	-8279 / 0	-102.1	-102.1	0.68 (1)	2.88	Q-H	-348 / 0
E-F	-8279 / 0	-102.1	-102.1	0.41 (1)	2.88	H-I	0 / 2587
F-G	-7444 / 0	-102.1	-102.1	0.29 (1)	3.43	I-J	-2385 / 0
G-H	-7444 / 0	-102.1	-102.1	0.29 (1)	3.43	J-K	0 / 3594
H-I	-7444 / 0	-102.1	-102.1	0.37 (1)	3.35	K-L	-578 / 0
I-J	-5683 / 0	-102.1	-102.1	0.29 (1)	3.88	L-M	0 / 4839
J-K	-3905 / 0	-102.1	-102.1	0.28 (1)	4.57	M-N	0 / 3217
K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00		
V-B	-4893 / 0	0.0	0.0	0.18 (1)	6.56		
M-K	-3503 / 0	0.0	0.0	0.13 (1)	7.48		

FACTORED CONCENTRATED LOADS (LBS)		LOC.	LOC	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
JT	C	4-3-1	-53	-59	-	FRONT	VERT	DEAD	-	-
C	C	4-3-1	-205	-205	-	FRONT	VERT	TOTAL	-	-
C	C	4-3-1	-303	-303	-	FRONT	VERT	SNOW	-	-
R	U	12-8-8	-1917	-1917	-	FRONT	VERT	TOTAL	-	-
U	U	4-1-12	-75	-95	-	FRONT	VERT	TOTAL	-	-

DWG NO. TAM 11905620
STRUCTURAL
COMPONENT ONLY

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT. NO FURTHER MODIFICATIONS WERE MADE.

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 240 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***
ADAPT USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018
- CSA 088-14
- TPC 2014

(65% OF 37.8 P.S.F., G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.22")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = L/999 (0.37")

CSI: TO=0.88/1.00 (D-E-I), BC=0.65/1.00 (Q-R-I), WB=0.57/1.00 (B-U-I), SS=0.24/1.00 (C-D-I)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1887 1873

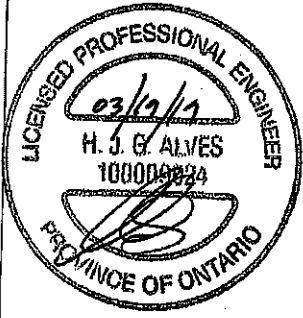
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.88 (I) (INPUT = 0.90)
JSI METAL= 0.72 (S) (INPUT = 1.00)

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T1	1	2	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:20 2019 Page 2	
ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-VdTIIErQHofizjBIBndbU6XqgnhXlbnzOI2GxzZMcL					

PLATES (table is in inches)					FACTORED CONCENTRATED LOADS (LBS)											
JT	TYPE	PLATES	W	LEN	Y	X	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
R	BMWWW+t	MT20	7.0	8.0			W	6-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
S	BS-t	MT20	5.0	8.0			X	8-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
T	BMWWH	MT20	5.0	8.0	3.00	2.00	Y	10-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
U	BMWWH	MT20	5.0	8.0	2.50	2.00	Z	12-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
V	BMV1+p	MT20	3.0	6.0			AA	2-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							AB	6-1-12	-75	-85	---	FRONT	VERT	TOTAL	---	---
							AC	8-1-12	-75	-85	---	FRONT	VERT	TOTAL	---	---
							AD	10-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---
							AE	12-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM 17905890
 STRUCTURAL
 COMPONENT ONLY 3/2

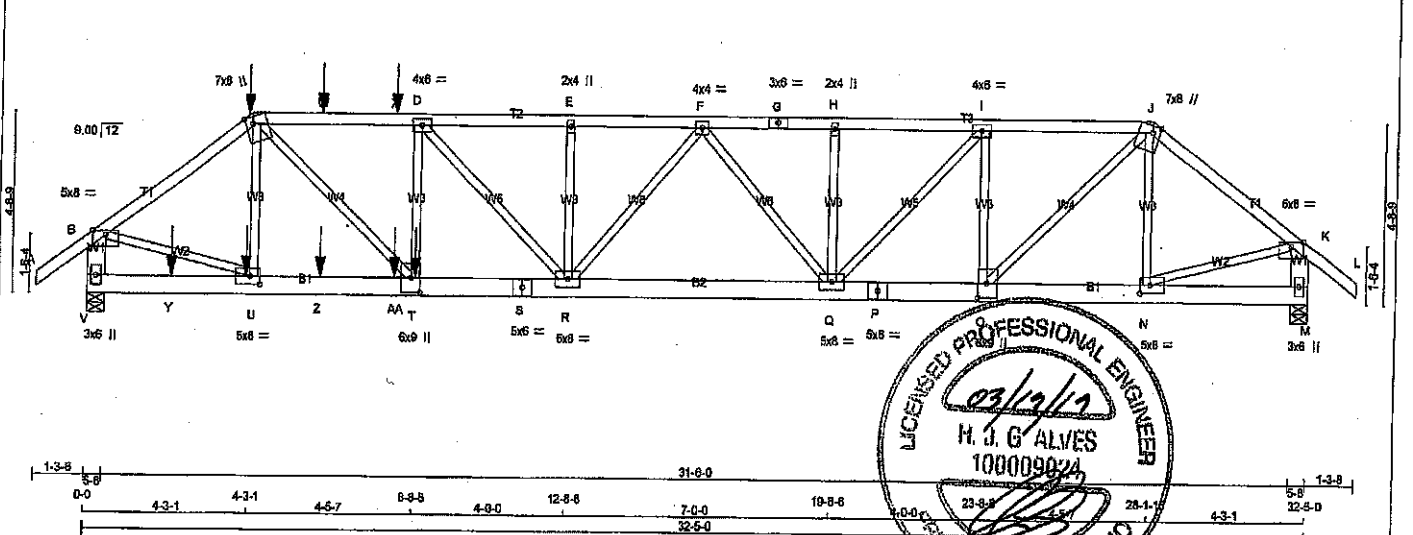
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	T12
401811	T12	1	2	TRUSS DESC.			

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2016 Mitek Industries, Inc. Tue Mar 19 14:03:22 2018 Page 1

ID:mxsiFIQQ02DDG5ZD9xpGq4yKxQY-R7bPAvggpPvQCCL5JCF3Zxc6bMZBTIFr3C8KqzZMcJ

Scale = 1:55.4



LUMBER	CHORDS	SIZE	LUMBER	DESCR
N. L. G. A. RULES				
A - C	2x4	DRY	No.2	SPF
C - G	2x4	DRY	No.2	SPF
G - J	2x4	DRY	No.2	SPF
J - L	2x4	DRY	No.2	SPF
V - B	2x6	DRY	No.2	SPF
M - K	2x6	DRY	No.2	SPF
V - S	2x6	DRY	No.2	SPF
S - P	2x6	DRY	No.2	SPF
P - M	2x6	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD(PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-C 1	12	SIDE(61.0)
C-G 1	12	SIDE(61.0)
G-J 1	12	TOP
J-L 1	12	TOP
V-B 2	12	TOP
M-K 2	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
V-S 2	12	SIDE(183.1)
S-P 2	12	TOP
P-M 2	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
2x3 1	6	SIDE(631.1)
D-T 1	3	
I-O 1	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
B TMWV-p	MT20	3.0	8.0	Edge		
C TTVWV+m	MT20	7.0	8.0	Edge 2.50		
D TMWV-t	MT20	4.0	8.0			
E TMWV-w	MT20	2.0	4.0			
F TMWV-t	MT20	4.0	4.0			
G TS-t	MT20	3.0	8.0			
H TMWV-w	MT20	2.0	4.0			
I TTVWV+m	MT20	4.0	8.0			
K TMWV-p	MT20	7.0	8.0	Edge 2.50		
L TMWV-t	MT20	5.0	8.0	Edge		
M BMV1+p	MT20	3.0	8.0			
N BMWV-t	MT20	5.0	8.0	2.50 3.25		
O BMWV-w	MT20	6.0	8.0	4.50 2.75		
P BS-t	MT20	5.0	8.0			
Q BMWV-t	MT20	6.0	8.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	0	0
V	5748	0	5748	5-8
M	3467	0	3467	5-8

UNFACTORED REACTIONS	1ST LOASE	MAX /MIN. COMPONENT REACTIONS	WIND	DEAD	SOIL
JT	COMBINED	SNOW	LIVE	PERM.LIVE	
V	4268	2442 / 0	780 / 0	0 / 0	1036 / 0
M	2576	1489 / 0	481 / 0	0 / 0	625 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.69 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1	MAX CS1 (LC)	UNBRAC LENGTH	FR-TO	WEBS	MAX. FACTORED FORCE (LBS)	MAX CS1 (LC)
FR-TO								
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	U-C	-872 / 0	0.14 (1)
B-C	-6591 / 0	-102.1	-102.1	0.40 (1)	3.53	C-T	0 / 5347	0.68 (1)
C-W	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69	T-D	-510 / 0	0.08 (1)
W-X	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69	D-R	-358 / 0	0.11 (1)
X-D	-9079 / 0	-102.1	-102.1	0.71 (1)	2.69	R-E	-301 / 0	0.05 (1)
D-E	-6837 / 0	-102.1	-102.1	0.55 (1)	2.89	E-F	0 / 1099	0.14 (1)
E-F	-6837 / 0	-102.1	-102.1	0.38 (1)	3.07	F-Q	-1600 / 0	0.42 (1)
F-G	-7142 / 0	-102.1	-102.1	0.28 (1)	3.50	Q-H	-343 / 0	0.05 (1)
G-H	-7142 / 0	-102.1	-102.1	0.28 (1)	3.50	H-I	0 / 2445	0.30 (1)
H-I	-7142 / 0	-102.1	-102.1	0.36 (1)	3.43	I-J	-2275 / 0	0.36 (1)
I-J	-5484 / 0	-102.1	-102.1	0.28 (1)	3.65	J-K	0 / 3465	0.43 (1)
J-K	-3779 / 0	-102.1	-102.1	0.25 (1)	4.63	K-L	-557 / 0	0.09 (1)
K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00	L-U	0 / 5430	0.67 (1)
V-B	-5668 / 0	0.0	0.0	0.21 (1)	6.18	U-N	0 / 3113	0.39 (1)
M-K	-3402 / 0	0.0	0.0	0.12 (1)	7.55			
V-Y	0 / 0	-38.5	-38.5	0.08 (2)	10.00			
Y-U	0 / 0	-38.5	-38.5	0.08 (2)	10.00			
U-Z	0 / 5241	-38.5	-38.5	0.44 (1)	10.00			
Z-AA	0 / 5241	-38.5	-38.5	0.44 (1)	10.00			
AA-T	0 / 5241	-38.5	-38.5	0.44 (1)	10.00			
T-S	0 / 9079	-38.5	-38.5	0.58 (1)	10.00			
S-R	0 / 9079	-38.5	-38.5	0.68 (1)	10.00			
R-Q	0 / 8147	-38.5	-38.5	0.81 (1)	10.00			
Q-P	0 / 5484	-38.5	-38.5	0.39 (1)	10.00			
P-O	0 / 5484	-38.5	-38.5	0.39 (1)	10.00			
O-N	0 / 3004	-38.5	-38.5	0.23 (1)	10.00			
N-M	0 / 0	-38.5	-38.5	0.04 (3)	10.00			

FACTORED CONCENTRATED LOADS (LBS)	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-3-1	-53	-59			FRONT	VERT	DEAD		
C	4-3-1	-205	-205			BACK	VERT	TOTAL		
C	4-3-1	-303	-303			FRONT	VERT	SNOW		
T	8-8-8	-3121	-3121			BACK	VERT	TOTAL		
U	4-1-12	-75	-95			BACK	VERT	TOTAL		
W	8-1-12	-198	-198			BACK	VERT	TOTAL		
X	8-1-12	-198	-198			BACK	VERT	TOTAL		
Y	2-1-12	-75	-95			BACK	VERT	TOTAL		
Z	6-1-12	-75	-95			BACK	VERT	TOTAL		

TOTAL WEIGHT = 2 X 158 = 316 lb

DESIGN CRITERIA
*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:	TOP CH.	LL	PSF
	DL	6.0	PSF
SPECIFIED LOADS:	BOT CH.	LL	PSF
	DL	7.0	PSF
	TOTAL LOAD	52.5	PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 0.001/2

*** NON STANDARD GIRDER ***
ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC2015
- CSA 086-14
- TFC2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = 1/999 (0.21")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = 1/999 (0.35")

CS1: TC=0.71/1.00 (C-D:1), BC=0.68/1.00 (R-T:1), WS=0.67/1.00 (B-U:1), SS=0.22/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)

JSI METAL = 0.81 (S) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	T12
401811	T1Z	1	2	TRUSS DESC.			

Tamarack Roof Truss, Burlington

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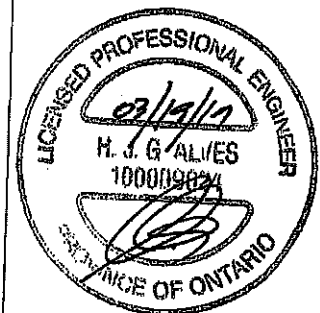
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWVW-t	MT20	5.0	8.0		
S	BS-t	MT20	5.0	8.0		
T	BMWWVW-t	MT20	6.0	9.0	4.50	2.75
U	BMWWVW-t	MT20	5.0	8.0	2.50	3.25
V	BMV1+p	MT20	3.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE
TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	8-1-12	-75	-95	---	BACK	VERT	TOTAL	---	---

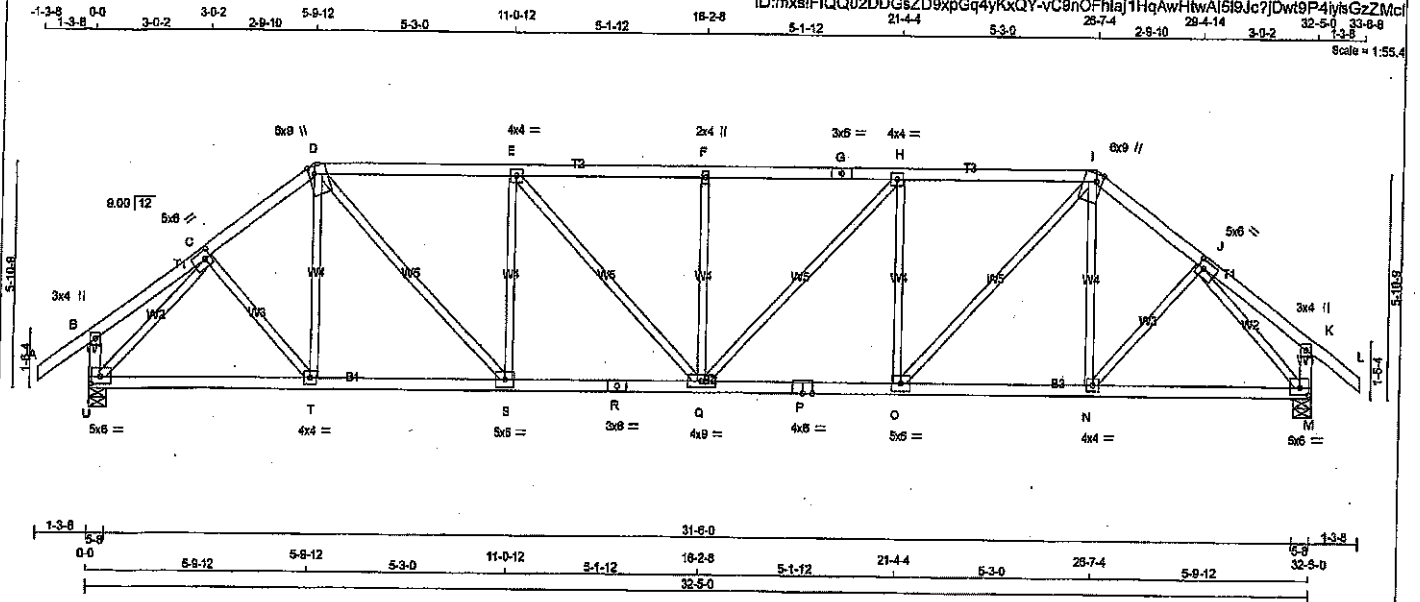


DWG NO. TAM 11905691
STRUCTURAL
COMPONENT ONLY 7/2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.	T2
401811	T2	2	1	TRUSS DESC.			

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:23 2019 Page 1
ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-vC9nOFHlaJ1HqAwHwAl5l9Jc7JDwt9P4lysGzZMcI



TOTAL WEIGHT = 2 X 142 = 285 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY No.2	SPF
D - G	2x4	DRY No.2	SPF
G - I	2x4	DRY No.2	SPF
I - L	2x4	DRY No.2	SPF
U - B	2x4	DRY No.2	SPF
M - K	2x4	DRY No.2	SPF
U - R	2x4	DRY No.2	SPF
R - P	2x4	DRY No.2	SPF
P - M	2x4	DRY No.2	SPF

ALL WEBS EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMWW-t	MT20	5.0	6.0	2.50	2.00
D	TTWW+m	MT20	6.0	9.0	Edge	2.00
E	TMWW-t	MT20	4.0	4.0		
F	TMWW-w	MT20	2.0	4.0		
G	TS-t	MT20	3.0	6.0		
H	TMWW-t	MT20	4.0	4.0		
I	TTWW+m	MT20	6.0	9.0	Edge	2.00
J	TMWW-t	MT20	5.0	6.0	2.50	2.00
K	TMV+p	MT20	3.0	4.0		
M	BMVW-t	MT20	5.0	6.0	2.25	2.75
N	BMWW-t	MT20	4.0	4.0		
O	BMWW-t	MT20	5.0	6.0		
P	BS-t	MT20	4.0	6.0		
Q	BMVWW-t	MT20	4.0	9.0		
R	BS-t	MT20	3.0	6.0		
S	BMWW-t	MT20	5.0	6.0		
T	BMWW-t	MT20	4.0	4.0		
U	BMVW-t	MT20	5.0	6.0	2.25	2.75

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
GROSS REACTION		GROSS REACTION		BRG		BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
U	2420	0	2420	0	0	5-8	5-8
M	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LCASE	SNOW	LIVE	PERMLIVE	WIND	DEAD	SOIL
U	1300	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0
M	1300	1021 / 0	340 / 0	0 / 0	0 / 0	438 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	MAX. UNBRACED LENGTH (LC)
FR-TO		FROM TO		FR-TO		FROM TO	
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	C-T	0 / 301	0.07 (1)
B-C	0 / 15	-102.1 -102.1	0.10 (1)	10.00	T-D	0 / 153	0.04 (3)
C-D	-2492 / 0	-102.1 -102.1	0.18 (1)	4.22	D-S	0 / 1542	0.35 (1)
D-E	-3033 / 0	-102.1 -102.1	0.80 (1)	3.44	S-E	-938 / 0	0.50 (1)
E-F	-3349 / 0	-102.1 -102.1	0.84 (1)	3.25	E-Q	0 / 468	0.10 (1)
F-G	-3349 / 0	-102.1 -102.1	0.84 (1)	3.25	Q-F	-484 / 0	0.26 (1)
G-H	-3349 / 0	-102.1 -102.1	0.84 (1)	3.25	Q-H	0 / 468	0.10 (1)
H-I	-3033 / 0	-102.1 -102.1	0.80 (1)	3.44	H-Q	-938 / 0	0.50 (1)
I-J	-2492 / 0	-102.1 -102.1	0.18 (1)	4.22	O-I	0 / 1542	0.35 (1)
J-K	0 / 15	-102.1 -102.1	0.10 (1)	10.00	N-I	0 / 153	0.04 (3)
K-L	0 / 42	-102.1 -102.1	0.14 (1)	10.00	N-J	0 / 301	0.07 (1)
U-B	-263 / 0	0.0 0.0	0.03 (1)	7.81	U-C	-2722 / 0	0.85 (1)
M-K	-263 / 0	0.0 0.0	0.03 (1)	7.81	J-M	-2722 / 0	0.85 (1)
U-T	0 / 1773	-38.5 -38.5	0.46 (2)	10.00			
T-S	0 / 1976	-38.5 -38.5	0.50 (2)	10.00			
S-R	0 / 3033	-38.5 -38.5	0.59 (1)	10.00			
R-Q	0 / 3033	-38.5 -38.5	0.59 (1)	10.00			
Q-P	0 / 3033	-38.5 -38.5	0.59 (1)	10.00			
P-O	0 / 3033	-38.5 -38.5	0.59 (1)	10.00			
O-N	0 / 1976	-38.5 -38.5	0.50 (2)	10.00			
N-M	0 / 1773	-38.5 -38.5	0.46 (2)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018
- CSA 086-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (1.08")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.17")
ALLOWABLE DEFL.(TL) = $L/360$ (1.08")
CALCULATED VERT. DEFL.(TL) = $L/999$ (0.26")

CSI: TO=0.64/1.00 (E-F:1), BC=0.59/1.00 (O-C:1), WB=0.85/1.00 (C-U:1), SS=0.25/1.00 (H-I:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

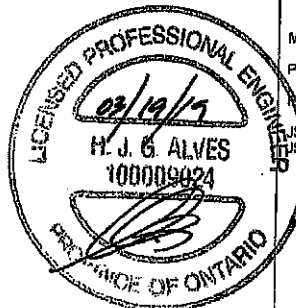
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1867 1873

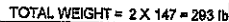
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

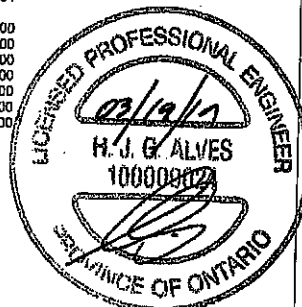
US GRIP= 0.90 (M) (INPUT = 0.90)
US METAL= 1.00 (R) (INPUT = 1.00)



DWG NO. TAM T190 5692
STRUCTURAL
COMPONENT ONLY

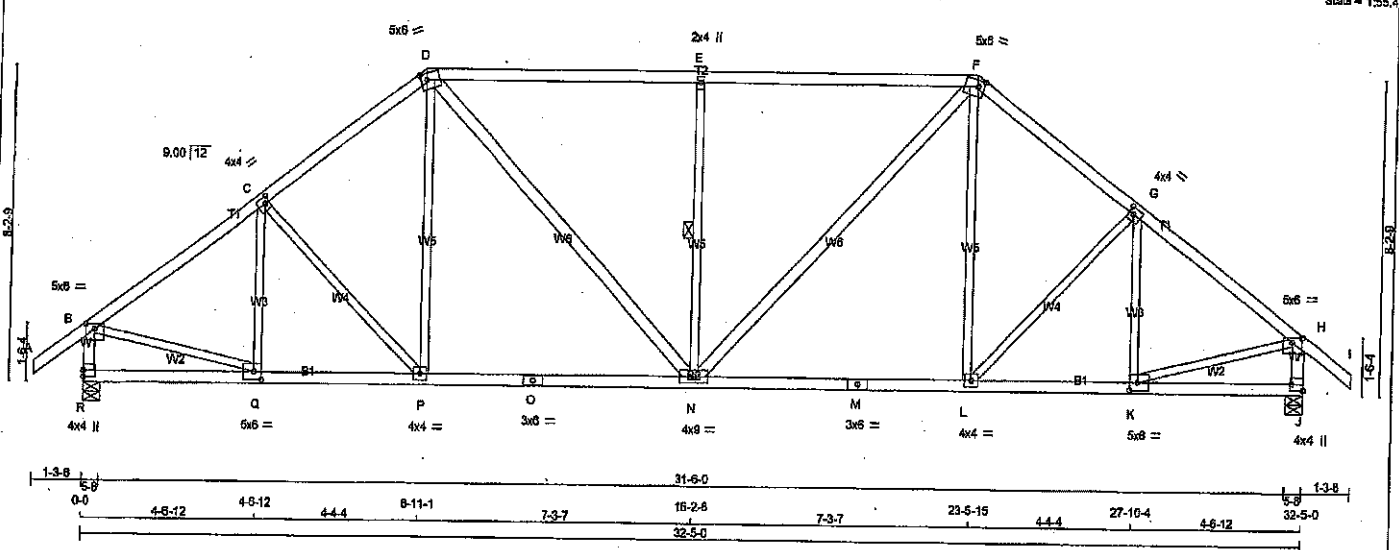
T3

SI GRIP= 0.87 (I) (INPUT = 0.90)
SI METAL= 0.62 (I) (INPUT = 1.00)



DWG NO. TAM 7905893
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington ID: mxsIFIQQ02DDGsZD9xpGq4yKxQY-saGYpxiY5KH_3U4g_KDmBAEbpqPnOikXDRpx8zZMcG Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:25 2018 Page 1 1-3-8 0-0 4-8-12 4-8-12 4-4-4 8-11-1 7-3-7 16-2-8 7-3-7 23-5-15 4-4-4 27-10-4 4-8-12 32-5-0 1-3-8 Scale = 1/55.4



TOTAL WEIGHT = 2 X 152 = 304 lb

LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - I	2x4	DRY	No.2
R - B	2x4	DRY	No.2
R - H	2x4	DRY	No.2
R - O	2x4	DRY	No.2
O - M	2x4	DRY	No.2
M - J	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - N	2x4	DRY	No.2
N - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TMWW-p	MT20	5.0	6.0	1.50 3.00
C	TMWW-i	MT20	4.0	4.0	2.00 1.50
D	TTWW-m	MT20	5.0	6.0	2.00 2.00
E	TMWW-w	MT20	2.0	4.0	
F	TTWW-m	MT20	5.0	6.0	2.00 2.00
G	TMWW-i	MT20	4.0	4.0	2.00 1.50
H	TMWW-p	MT20	5.0	6.0	1.50 3.00
J	BMV1-p	MT20	4.0	4.0	2.00 Edge
K	BMWW-i	MT20	5.0	6.0	2.50 2.50
L	BMWW-i	MT20	4.0	4.0	
M	BS-4	MT20	3.0	6.0	
N	BMWW-i	MT20	4.0	9.0	
O	BS-4	MT20	3.0	6.0	
P	BMWW-i	MT20	4.0	4.0	
Q	BMWW-i	MT20	5.0	6.0	2.50 2.50
R	BMV1-p	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	HORZ	DOWN	HORZ	UPLIFT
R	2420	0	2420	0	0
J	2420	0	2420	0	0

UNFACTORED REACTIONS		1ST LCASE	MAX/MIN	COMPONENT REACTIONS
JT	COMBINED	SNOW	LIVE	PERM.LIVE
R	1800	1021/0	340/0	0/0
J	1800	1021/0	340/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.28 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX LC1 (LC)
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/42	-102.1	-102.1	0.14 (1)	Q-C	-355/0	0.13 (1)
B-C	-2465/0	-102.1	-102.1	0.35 (1)	C-P	-173/0	0.13 (1)
C-D	-2383/0	-102.1	-102.1	0.33 (1)	P-D	0/392	0.09 (2)
D-E	-2437/0	-102.1	-102.1	0.85 (1)	D-N	0/821	0.13 (1)
E-F	-2437/0	-102.1	-102.1	0.85 (1)	N-E	-914/0	0.39 (1)
F-G	-2383/0	-102.1	-102.1	0.33 (1)	N-F	0/821	0.13 (1)
G-H	-2465/0	-102.1	-102.1	0.35 (1)	L-F	0/392	0.09 (2)
H-I	0/42	-102.1	-102.1	0.14 (1)	L-G	-173/0	0.13 (1)
R-B	-2342/0	0.0	0.0	0.24 (1)	K-G	-355/0	0.13 (1)
J-H	-2342/0	0.0	0.0	0.24 (1)	B-Q	0/2064	0.46 (1)
R-Q	0/0	-38.5	-38.5	0.13 (3)	K-H	0/2064	0.46 (1)
Q-P	0/1997	-38.5	-38.5	0.46 (2)			
P-O	0/1861	-38.5	-38.5	0.66 (2)			
O-N	0/1861	-38.5	-38.5	0.58 (2)			
N-M	0/1861	-38.5	-38.5	0.58 (2)			
M-L	0/1861	-38.5	-38.5	0.58 (2)			
L-K	0/1997	-38.5	-38.5	0.46 (2)			
K-J	0/0	-38.5	-38.5	0.13 (3)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (1.08")
CALCULATED VERT. DEFL. (LL) = L/999 (0.12")
ALLOWABLE DEFL. (TL) = L/360 (1.08")
CALCULATED VERT. DEFL. (TL) = L/999 (0.21")

CSI: TC=0.85/1.00 (E-F:1), BC=0.58/1.00 (L-N:2),
WB=0.46/1.00 (H-K:1), SS=0.36/1.00 (E-F:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

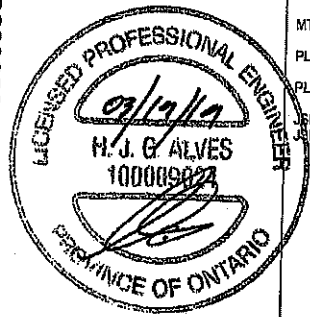
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1073

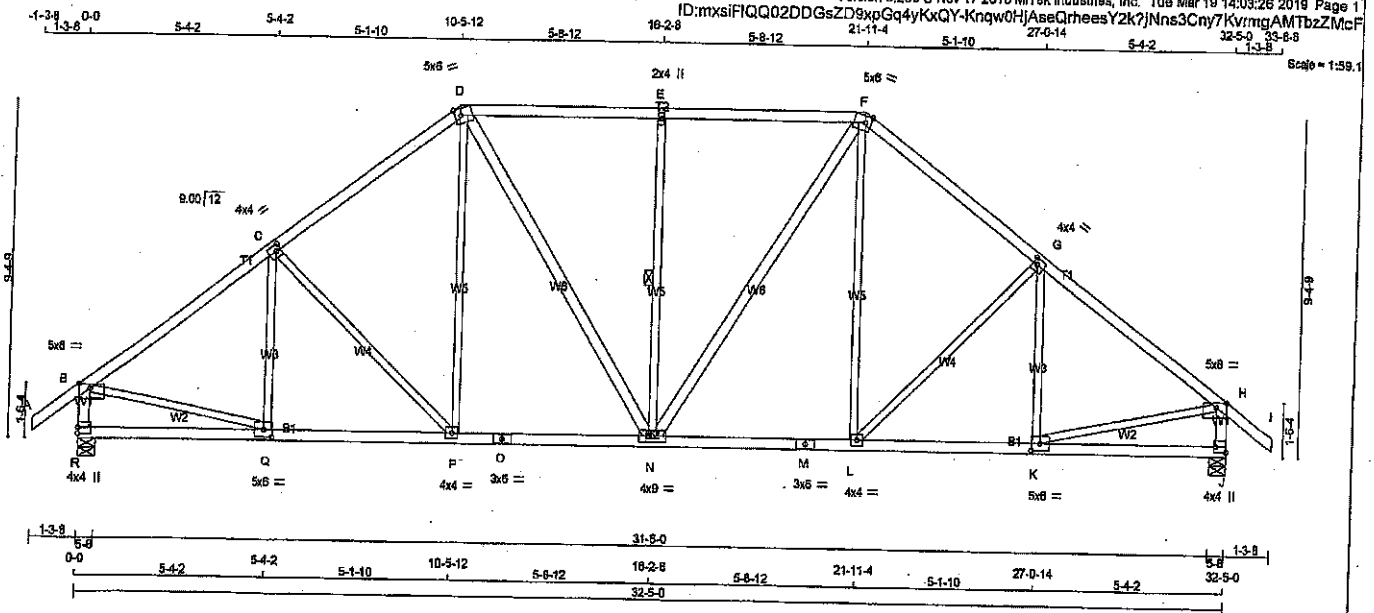
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

RI GRIP= 0.88 (H) (INPUT = 0.90)
METAL= 0.78 (O) (INPUT = 1.00)

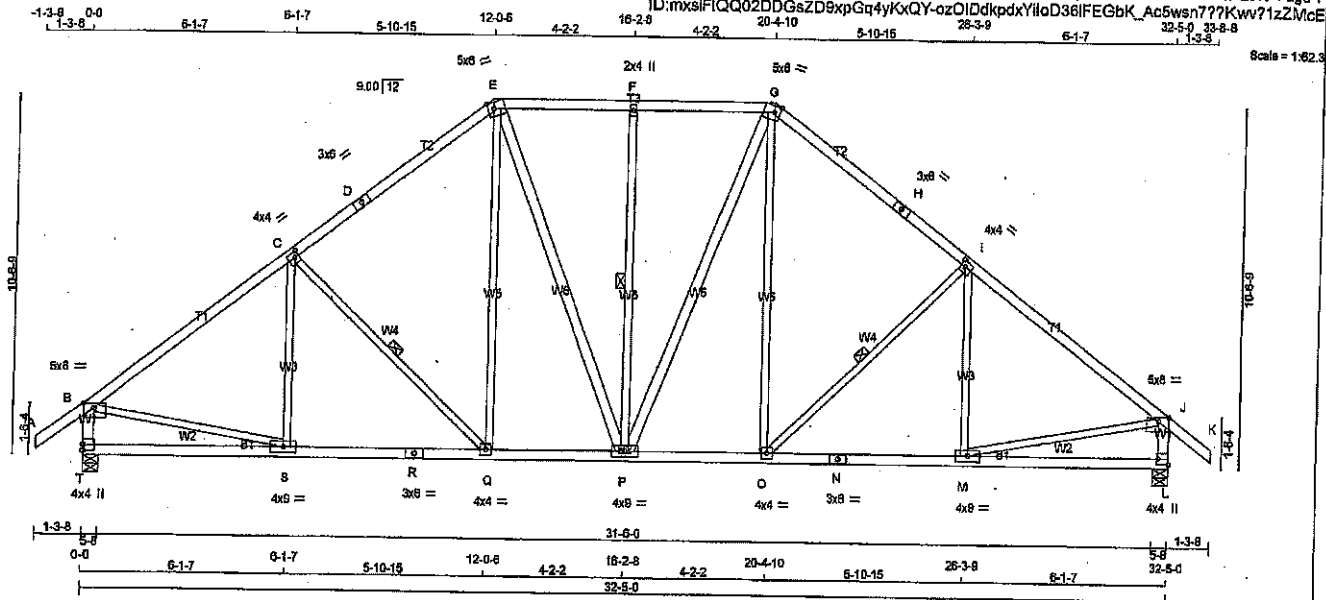


DRWG NO. TAM 7905894
STRUCTURAL
COMPONENT ONLY



Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 19 14:03:27 2019 Page 1
 ID:mxslFIQQ02DDG8ZD9xpGq4yKxQY-ozOIdkpdXyIeD36lFEGbK_Ac5wn777Kwv71zZMcE
 32-5-9 33-8-8 1.3-8 1.3-8



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - D	2x4	DRY	No.2
D - E	2x4	DRY	No.2
E - G	2x4	DRY	No.2
G - H	2x4	DRY	No.2
H - K	2x4	DRY	No.2
T - B	2x4	DRY	No.2
L - J	2x4	DRY	No.2
T - R	2x4	DRY	No.2
N - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
E - P	2x4	DRY	No.2
P - G	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	5.0	8.0	1.50	3.50
C	TMVW-1	MT20	4.0	4.0	2.00	1.50
D	TS-1	MT20	3.0	6.0		
E	TTMW-m	MT20	5.0	6.0	Edge	2.00
F	TTMW-m	MT20	2.0	4.0		
G	TTMW-m	MT20	5.0	6.0	Edge	2.00
H	TS-1	MT20	3.0	6.0		
I	TMVW-1	MT20	4.0	4.0	2.00	1.50
J	TMVW-p	MT20	5.0	8.0	1.50	3.50
L	BMV1-p	MT20	4.0	4.0	2.00	Edge
M	BMVW-1	MT20	4.0	9.0		
N	BS-1	MT20	3.0	6.0		
O	BMVW-1	MT20	4.0	4.0		
P	BMVW-1	MT20	4.0	9.0		
Q	BMVW-1	MT20	4.0	4.0		
R	BS-1	MT20	3.0	6.0		
S	BMVW-1	MT20	4.0	9.0		
T	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	DOWN	HORZ	UP/LIFT	IN-SX	IN-SX	IN-SX	IN-SX
T	2420	0	2420	0	0	5-8	5-8	5-8	5-8
L	2420	0	2420	0	0	5-8	5-8	5-8	5-8

UNFACTORED REACTIONS

1ST CASE		MAX/MIN. COMPONENT REACTIONS		PERM. LIVE WIND		DEAD		SOIL	
JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL	DEAD	SOIL
T	1800	1021/0	340/0	0/0	0/0	438/0	0/0	438/0	0/0
L	1800	1021/0	340/0	0/0	0/0	438/0	0/0	438/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) T, L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.70 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-Q, F-P, I-O.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		MAX.		WEBS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRACED LENGTH	MEMB.	FORCE (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)	MAX. FACTORED (LBS)
FR-TO					FR-TO				
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	S-C	-138/161	0.08 (1)	0.08 (1)
B-C	-2528/0	-102.1	-102.1	0.66 (1)	3.70	C-Q	-513/0	0.24 (1)	0.24 (1)
C-D	-2158/0	-102.1	-102.1	0.59 (1)	4.02	Q-E	0/550	0.12 (2)	0.12 (2)
D-E	-2158/0	-102.1	-102.1	0.59 (1)	4.02	E-P	0/349	0.08 (1)	0.08 (1)
E-F	-1825/0	-102.1	-102.1	0.26 (1)	4.72	P-F	-509/0	0.40 (1)	0.40 (1)
F-G	-1825/0	-102.1	-102.1	0.26 (1)	4.72	P-G	0/349	0.08 (1)	0.08 (1)
G-H	-2158/0	-102.1	-102.1	0.59 (1)	4.02	O-G	0/550	0.12 (2)	0.12 (2)
H-I	-2158/0	-102.1	-102.1	0.59 (1)	4.02	O-I	-513/0	0.24 (1)	0.24 (1)
I-J	-2528/0	-102.1	-102.1	0.66 (1)	3.70	M-I	-138/161	0.08 (1)	0.08 (1)
J-K	0/42	-102.1	-102.1	0.14 (1)	10.00	B-S	0/2096	0.47 (1)	0.47 (1)
T-B	-2324/0	0.0	0.0	0.24 (1)	5.60	M-J	0/2096	0.47 (1)	0.47 (1)
L-J	-2324/0	0.0	0.0	0.24 (1)	5.60				
T-S	0/0	-38.5	-38.5	0.27 (3)	10.00				
S-R	0/2058	-38.5	-38.5	0.54 (2)	10.00				
R-Q	0/2058	-38.5	-38.5	0.54 (2)	10.00				
Q-P	0/1893	-38.5	-38.5	0.38 (1)	10.00				
P-O	0/1893	-38.5	-38.5	0.38 (1)	10.00				
O-N	0/2058	-38.5	-38.5	0.54 (2)	10.00				
N-M	0/2058	-38.5	-38.5	0.54 (2)	10.00				
M-L	0/0	-38.5	-38.5	0.27 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0	PSF
DL = 6.0	PSF	
BOT CH.	LL = 10.5	PSF
DL = 7.0	PSF	
TOTAL LOAD = 52.5	PSF	

SPACING = 24.0 IN. C/G

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2016

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2016
- CSA 086-14
- TPIC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.08")
 ALLOWABLE DEFL.(TL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.16")

CSI: TC=0.65/1.00 (B-C-1), BC=0.54/1.00 (M-O-2)
 WE=0.47/1.00 (J-M-1), SS=0.24/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR= 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

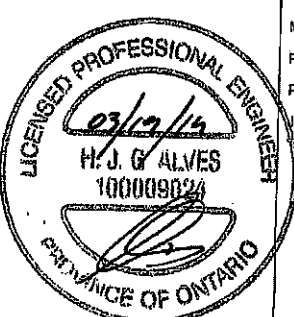
NAIL VALUES
 PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
 MAX MIN MAX MIN MAX MIN
 MT20 650 371 1747 768 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

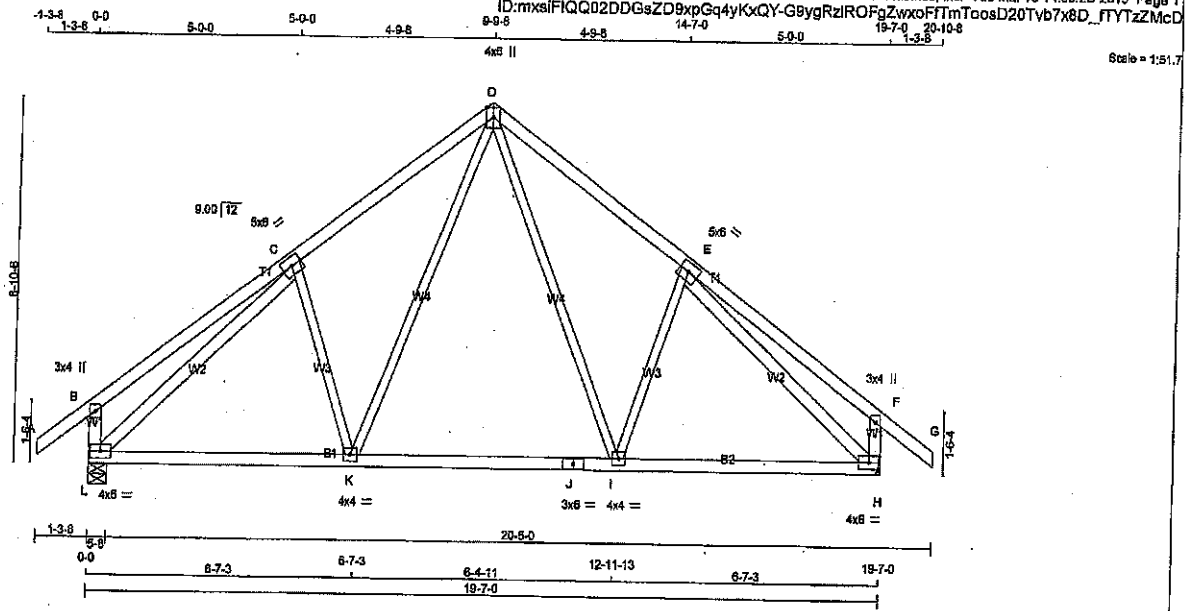
SI GRIP= 0.89 (S) (INPUT = 0.80)

SI METAL= 0.74 (R) (INPUT = 1.00)



DRWG NO. TAM 77905896
 STRUCTURAL
 COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC	DRWG NO.
401811	T7	2	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	



TOTAL WEIGHT = 2 X 94 = 188 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
L - B	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - J	2x4	DRY	No.2
J - H	2x4	DRY	No.2
ALL WEBS EXCEPT	2x3	DRY	No.2
L - C	2x4	DRY	No.2
E - H	2x4	DRY	No.2

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
L	1518 0	1518 0	5-8	5-8
H	1518 0	1518 0	MECHANICAL	MECHANICAL

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 1-10.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 086-14
- TPIC 2014

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+I	MT20	5.0	6.0		
D	TTWV+p	MT20	4.0	6.0	Edge	
E	TMVW+I	MT20	5.0	6.0		
F	TMV+p	MT20	3.0	4.0		
H	BMVW+I	MT20	4.0	6.0		
I	BMVW+I	MT20	4.0	6.0		
J	BS4	MT20	3.0	6.0		
K	BMVW+I	MT20	4.0	6.0		
L	BMVW+I	MT20	4.0	6.0		

UNFACTORED REACTIONS

JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS
L	COMBINED	1125 649 / 0
H	COMBINED	1125 649 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.32 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

LOADING

TOTAL LOAD CASES: (4)

(65 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.65")

CALCULATED VERT. DEFL. (LL) = $L/999$ (0.08")

ALLOWABLE DEFL. (TL) = $L/360$ (0.65")

CALCULATED VERT. DEFL. (TL) = $L/999$ (0.13")

CSI: TC=0.40/1.00 (B-C:1), BC=0.42/1.00 (H-I:2), WB=0.65/1.00 (C-L:1), SSI=0.10/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

CHORDS

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH (FT)
FR-TO				
A-B	0 / 42	-102.1	-102.1 0.14 (1)	10.00
B-C	0 / 36	-102.1	-102.1 0.40 (1)	10.00
C-D	-1277 / 0	-102.1	-102.1 0.33 (1)	5.32
D-E	-1277 / 0	-102.1	-102.1 0.33 (1)	5.32
E-F	0 / 36	-102.1	-102.1 0.40 (1)	10.00
F-G	0 / 42	-102.1	-102.1 0.14 (1)	10.00
L-B	-328 / 0	0.0	0.0 0.03 (1)	7.81
H-F	-328 / 0	0.0	0.0 0.03 (1)	7.81

WEBS

MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
D-I	0 / 589	0.13 (1)
I-E	-293 / 23	0.13 (1)
K-D	0 / 589	0.13 (1)
C-K	-293 / 23	0.13 (1)
L-C	-1540 / 0	0.95 (1)
E-H	-1540 / 0	0.95 (1)

NAIL VALUES

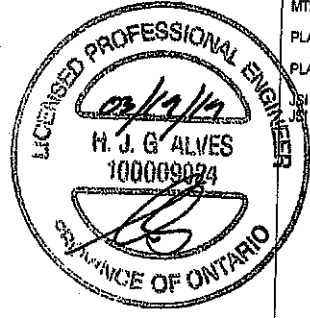
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JST GRIP= 0.88 (I) (INPUT = 0.90)

JST METAL= 0.37 (E) (INPUT = 1.00)

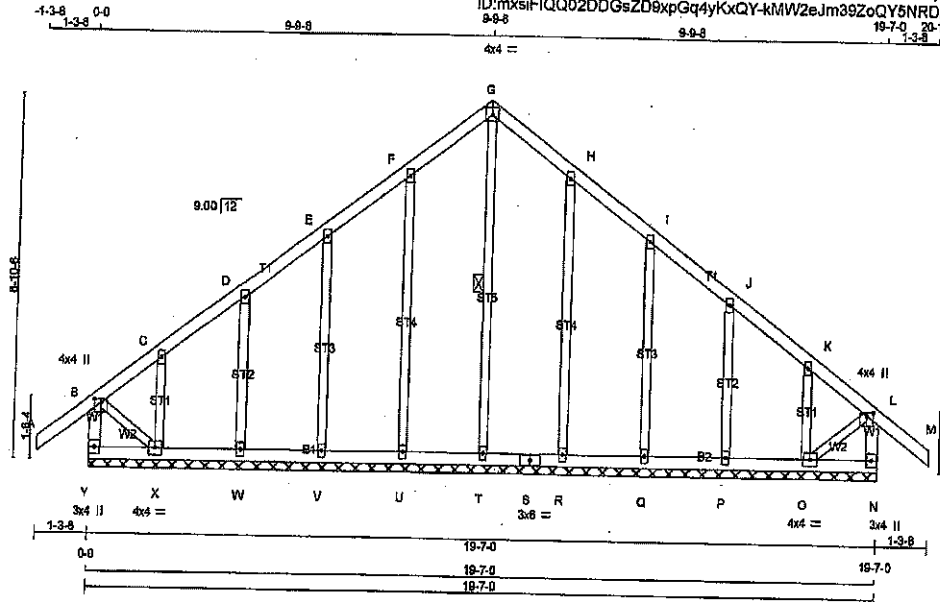


DRWG NO. TAM 17905897
STRUCTURAL
COMPONENT ONLY

Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-kMW2eJm392oQY5NRDAHL0PStPvLKITHSeP04wzZMcC
19-7-0 30-10-8 1-3-8

Scale = 1:51.7



TOTAL WEIGHT = 96 lb [M]

LUMBER			
N. L. G. A. RULES			
CHORDS	SIZE	LUMBER	DESCR.
Y - B	2x4	DRY No.2	SPF
A - G	2x4	DRY No.2	SPF
G - M	2x4	DRY No.2	SPF
N - L	2x4	DRY No.2	SPF
Y - S	2x4	DRY No.2	SPF
S - N	2x4	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
ALL GABLE WEBS	2x3	DRY No.2	SPF
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table is in inches)			
JT TYPE	PLATES	W	LEN Y X
B TMW+p	MT20	4.0	4.0 1.00 2.00
C, D, E, F, H, I, J, K			
C TMW+w	MT20	2.0	4.0
G TTW-p	MT20	4.0	4.0 2.25 2.00
L TMW+p	MT20	4.0	4.0 1.00 2.00
N BMW1+p	MT20	3.0	4.0
O BMW1+1	MT20	4.0	4.0
P, Q, R, T, U, V, W			
P BMW1+w	MT20	2.0	4.0
S BS-1	MT20	3.0	8.0
X BMW1+1	MT20	4.0	4.0
Y BMW1+p	MT20	3.0	4.0

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS1 (LC)
FR-TO			
Y-B	-320 / 0	0.0	0.03 (1)
A-B	0 / 42	-102.1	-102.1 0.14 (1)
B-C	-67 / 0	-102.1	-102.1 0.13 (1)
C-D	-25 / 0	-102.1	-102.1 0.05 (1)
D-E	-28 / 0	-102.1	-102.1 0.05 (1)
E-F	-19 / 0	-102.1	-102.1 0.06 (1)
F-G	-31 / 0	-102.1	-102.1 0.08 (1)
G-H	-31 / 0	-102.1	-102.1 0.08 (1)
H-I	-19 / 0	-102.1	-102.1 0.08 (1)
I-J	-28 / 0	-102.1	-102.1 0.05 (1)
J-K	-25 / 0	-102.1	-102.1 0.05 (1)
K-L	-67 / 0	-102.1	-102.1 0.13 (1)
L-M	0 / 42	-102.1	-102.1 0.14 (1)
N-L	-320 / 0	0.0	0.03 (1)
TO-TO			
Y-X	0 / 0	-38.5	-38.5 0.03 (3)
X-W	0 / 28	-38.5	-38.5 0.03 (2)
W-V	0 / 21	-38.5	-38.5 0.02 (2)
V-U	0 / 18	-38.5	-38.5 0.02 (2)
U-T	0 / 15	-38.5	-38.5 0.02 (3)
T-S	0 / 15	-38.5	-38.5 0.02 (3)
S-R	0 / 15	-38.5	-38.5 0.02 (3)
R-Q	0 / 18	-38.5	-38.5 0.02 (2)
Q-P	0 / 21	-38.5	-38.5 0.02 (2)
P-O	0 / 28	-38.5	-38.5 0.03 (2)
O-N	0 / 0	-38.5	-38.5 0.03 (3)

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	=	29.0	PSF
DL	=	6.0	PSF
BOT CH. LL	=	10.5	PSF
DL	=	7.0	PSF
TOTAL LOAD	=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(65 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.14/1.00 (L-M:1), BC=0.03/1.00 (W-X:2), WB=0.23/1.00 (F-U:1), SS1=0.09/1.00 (L-M:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

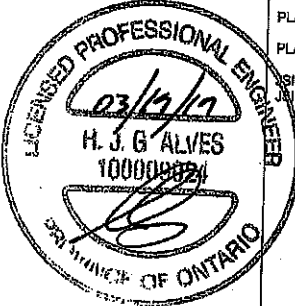
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

SI GRIP=0.60 (C) (INPUT = 0.90)
JAIL METAL=0.12 (F) (INPUT = 1.00)

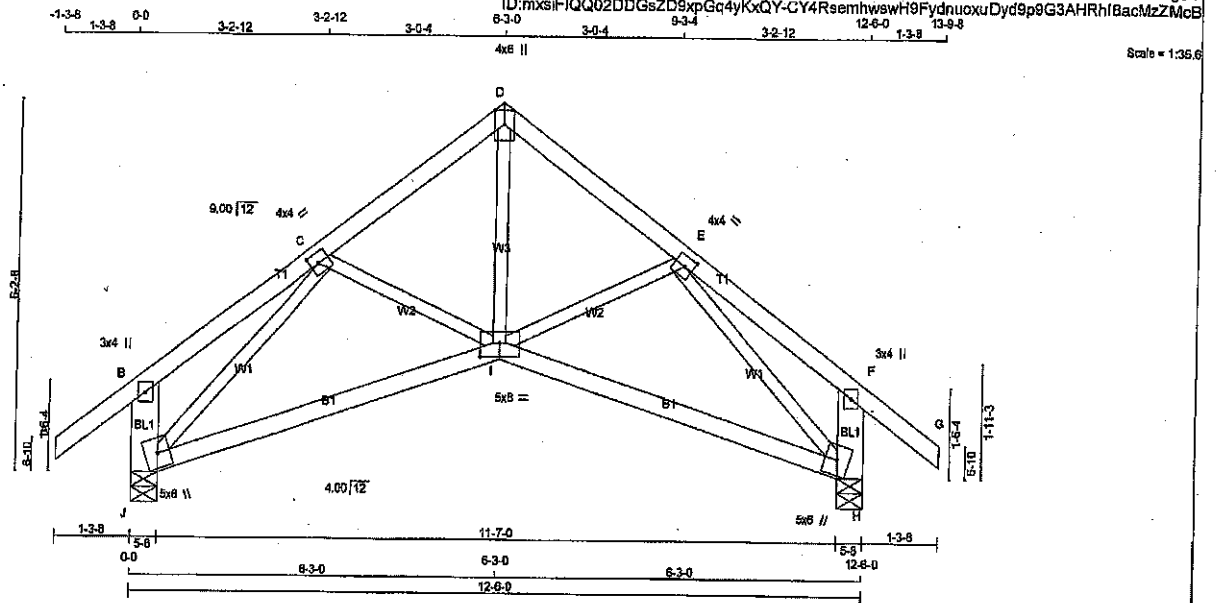


DRWG NO. TAM 7705698
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY.	JOB DESC.	DRWG NO.
401811	T8S	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:mxsIFIQ02DDGsZD9xpGc4yKxQY-CY4RsemhswH9FyduoxuDyd9p9G3AHRhI6acMzZMcB



TOTAL WEIGHT = 3 X 57 = 170 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
J - I	2x4	DRY	No.2	SPF
I - H	2x4	DRY	No.2	SPF
BEARING BLOCKS				
BL1	2-2x6	DRY	No.2	SPF
ALL WEBS				
	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMK+p	MT20	3.0	4.0		
C TMWV-I	MT20	4.0	4.0	2.00	1.75
D TTW+p	MT20	4.0	6.0	Edge	
E TMWV-I	MT20	4.0	4.0	2.00	1.75
F TMK+p	MT20	3.0	4.0		
H BWKM1+m	MT20	5.0	6.0		
I BBWVW-p	MT20	5.0	6.0	2.75	4.00
J BWKM1+m	MT20	5.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQRD
	GROSS REACTION	GROSS REACTION	BRG	BRG
	VERT	HORZ	DOWN	HORZ
JT	1020	0	1020	0
J	1020	0	1020	0
H	1020	0	1020	0

UNFACTORED REACTIONS

	1ST CASE	MAX / MIN	COMPONENT REACTIONS
	COMBINED	SNOW	LIVE
			PERM.LIVE
JT	753	443 / 0	131 / 0
J	753	443 / 0	131 / 0
H	753	443 / 0	131 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

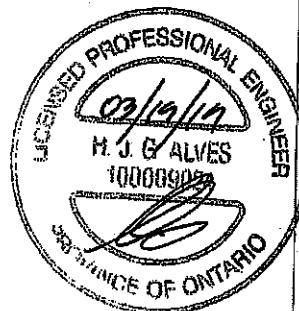
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO									
A-B	0 / 47	-102.1	-102.1	0.17 (1)	10.00				
B-C	-18 / 0	-102.1	-102.1	0.10 (1)	6.25				
C-D	-898 / 0	-102.1	-102.1	0.11 (1)	6.25				
D-E	-898 / 0	-102.1	-102.1	0.11 (1)	6.25				
E-F	-18 / 0	-102.1	-102.1	0.10 (1)	6.25				
F-G	0 / 47	-102.1	-102.1	0.17 (1)	10.00				
J-B	-289 / 0	0.0	0.0	0.02 (1)	7.81				
H-F	-289 / 0	0.0	0.0	0.02 (1)	7.81				
J-I	0 / 793	-38.5	-38.5	0.43 (2)	10.00				
I-H	0 / 793	-38.5	-38.5	0.43 (2)	10.00				



DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.0	PSF
	DL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	DL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.42")
CALCULATED VERT. DEFL. (LL) = L/899 (0.11")
ALLOWABLE DEFL. (TL) = L/360 (0.42")
CALCULATED VERT. DEFL. (TL) = L/799 (0.19")

CSI: TC=0.17/1.00 (A-B:1), BC=0.43/1.00 (I-J:2), WB=0.38/1.00 (C-J:1), SSI=0.12/1.00 (I-J:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX	MIN	MAX	MIN
MT20	650	371	1747
	788	1987	1873

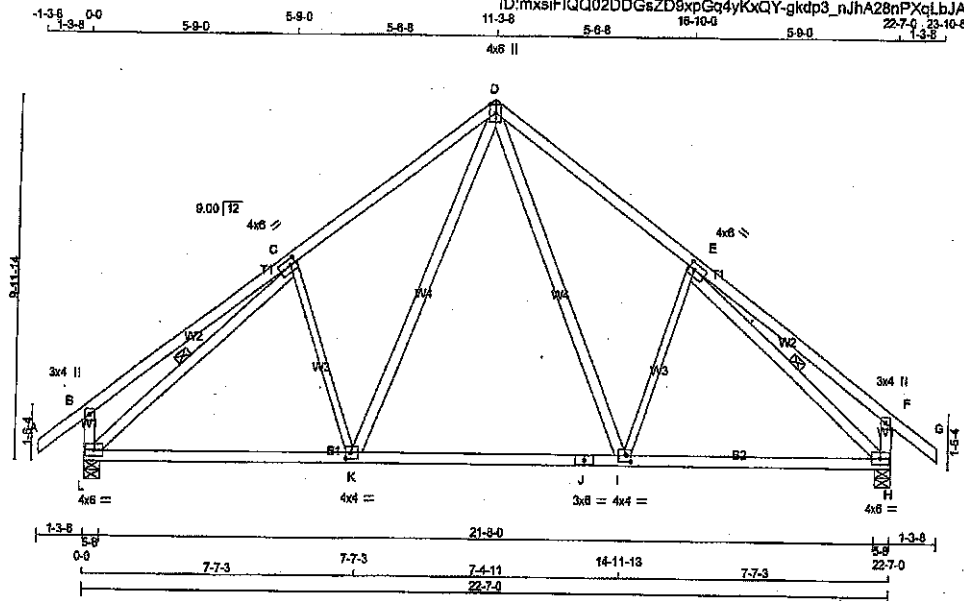
PLATE PLACEMENT TOL. = 0.250 Inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.88 (E) (INPUT = 0.80)
ISI METAL= 0.36 (C) (INPUT = 1.00)

DWG NO. TAM 7905699
STRUCTURAL
COMPONENT ONLY

JOB NAME 401811	TRUSS NAME T9	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T9
Tamerack Roof Truss, Burlington					



Scale = 1/8" = 1'-0"

TOTAL WEIGHT = 113 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF

ALL WEBS
EXCEPT
I - E
C - K

DRY, SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMV+p	MT20	3.0	4.0		
C	TMVW+1	MT20	4.0	6.0	2.00	2.00
D	TTWW+1	MT20	4.0	6.0	Edge	
E	TMVW+1	MT20	4.0	6.0	2.00	2.00
F	TMV+p	MT20	3.0	4.0		
H	BMVW+1	MT20	4.0	6.0		
I	BMVW+1	MT20	4.0	4.0	2.00	1.50
J	BS-1	MT20	3.0	6.0		
K	BMVW+1	MT20	4.0	4.0	2.00	1.50
L	BMVW+1	MT20	4.0	6.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	UPLIFT	IN-SX
L	1728	0	0	5-8
H	1728	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1283	738 / 0	237 / 0	0 / 0	0 / 0	310 / 0	0 / 0
H	1283	738 / 0	237 / 0	0 / 0	0 / 0	310 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.80 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	10.00	D-I	0 / 709	0.11 (1)
B-C	0 / 42	-102.1 -102.1	0.54 (1)	10.00	I-E	-368 / 14	0.21 (1)
C-D	-1514 / 0	-102.1 -102.1	0.47 (1)	4.80	K-D	0 / 709	0.11 (1)
D-E	-1514 / 0	-102.1 -102.1	0.47 (1)	4.80	C-K	-368 / 14	0.21 (1)
E-F	0 / 42	-102.1 -102.1	0.54 (1)	10.00	L-C	-1808 / 0	0.52 (1)
F-G	0 / 42	-102.1 -102.1	0.14 (1)	10.00	E-H	-1808 / 0	0.52 (1)
L-B	-358 / 0	0.0	0.0	0.04 (1)			
H-F	-358 / 0	0.0	0.0	0.04 (1)			
L-K	0 / 1303	-38.5	-38.5	0.55 (2)			
K-J	0 / 921	-38.5	-38.5	0.50 (2)			
J-I	0 / 921	-38.5	-38.5	0.50 (2)			
I-H	0 / 1303	-38.5	-38.5	0.55 (2)			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018
- CSA 086-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = L/360 (0.75")
CALCULATED VERT. DEFL. (LL) = L/999 (0.13")
ALLOWABLE DEFL. (TL) = L/360 (0.75")
CALCULATED VERT. DEFL. (TL) = L/889 (0.21")

CSI: TC=0.54/1.00 (E-F:1), BC=0.55/1.00 (H-I:2), WB=0.52/1.00 (E-H:1), SSI=0.22/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

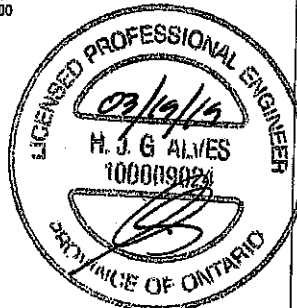
MAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN
MT20 650 371 1747 765 1887 1873

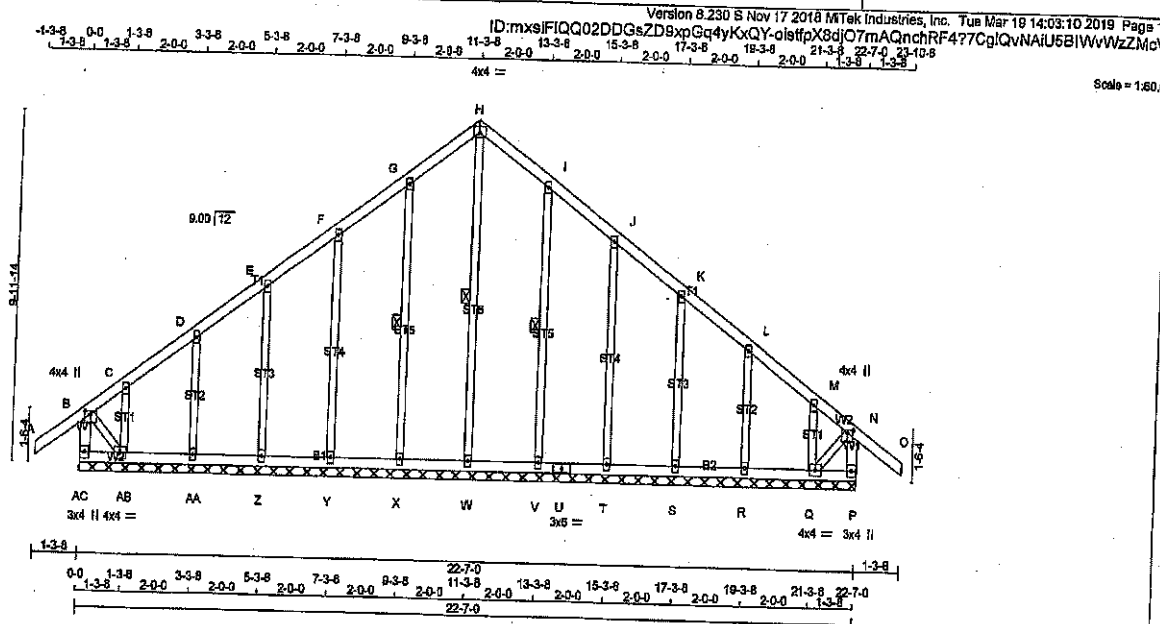
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.84 (E) (INPUT = 0.60)
JSI METAL = 0.50 (J) (INPUT = 1.00)



DRWG NO. TAM 17705700
STRUCTURAL
COMPONENT ONLY



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - H	2x4	DRY	No.2	SPF
H - O	2x4	DRY	No.2	SPF
AC - B	2x4	DRY	No.2	SPF
P - N	2x4	DRY	No.2	SPF
AC - U	2x4	DRY	No.2	SPF
U - P	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
ALL GABLE WEBS	2x3	DRY	No.2	SPF
DRY, SEASONED LUMBER.				

GABLE STUDS SPACED AT 2-0-0 OC.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, G, I, J, K, L, M					
C TMVW+w	MT20	2.0	4.0		
H TTW+p	MT20	4.0	4.0	2.25	2.00
N TMVW+p	MT20	4.0	4.0	1.00	2.00
P BMV1+p	MT20	3.0	4.0		
Q BMVW1+1	MT20	4.0	4.0		
R, S, T, V, W, X, Y, Z, AA					
R BMV1+w	MT20	2.0	4.0		
U BS4	MT20	3.0	6.0		
AB BMVW1+1	MT20	4.0	4.0		
AC BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)
BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF H-W, G-X, I-V.
END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING
TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (FT)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (FT)	MAX. UNBRACED LENGTH (FT)
FR-TO				FR-TO			
A-B	0/42	-102.1	-102.1 0.14 (1)	W-H	-152/0	0.10 (1)	
B-C	-81/0	-102.1	-102.1 0.13 (1)	X-G	-232/0	0.11 (1)	
C-D	-26/0	-102.1	-102.1 0.05 (1)	Y-F	-197/0	0.17 (1)	
D-E	-30/0	-102.1	-102.1 0.05 (1)	Z-E	-199/0	0.09 (1)	
E-F	-24/0	-102.1	-102.1 0.05 (1)	AA-D	-217/0	0.05 (1)	
F-G	-18/0	-102.1	-102.1 0.05 (1)	AB-C	-77/0	0.01 (1)	
G-H	-28/0	-102.1	-102.1 0.05 (1)	V-I	-232/0	0.11 (1)	
H-I	-29/0	-102.1	-102.1 0.05 (1)	T-J	-197/0	0.17 (1)	
I-J	-18/0	-102.1	-102.1 0.05 (1)	S-K	-199/0	0.09 (1)	
J-K	-24/0	-102.1	-102.1 0.05 (1)	R-L	-217/0	0.05 (1)	
K-L	-30/0	-102.1	-102.1 0.05 (1)	Q-M	-77/0	0.01 (1)	
L-M	-26/0	-102.1	-102.1 0.05 (1)	B-AB	0/42	0.01 (1)	
M-N	-81/0	-102.1	-102.1 0.13 (1)	Q-N	0/42	0.01 (1)	
N-O	0/42	-102.1	-102.1 0.14 (1)				
AC-B	-325/0	0.0	0.0 0.03 (1)				
P-N	-325/0	0.0	0.0 0.03 (1)				
AC-AB	0/0	-38.5	-38.5 0.02 (3)				
AB-AA	0/28	-38.5	-38.5 0.03 (2)				
AA-Z	0/22	-38.5	-38.5 0.03 (2)				
Z-Y	0/18	-38.5	-38.5 0.02 (2)				
Y-X	0/16	-38.5	-38.5 0.02 (2)				
X-W	0/14	-38.5	-38.5 0.02 (3)				
W-V	0/14	-38.5	-38.5 0.02 (3)				
V-U	0/16	-38.5	-38.5 0.02 (2)				
U-T	0/18	-38.5	-38.5 0.02 (2)				
T-S	0/19	-38.5	-38.5 0.02 (2)				
S-R	0/22	-38.5	-38.5 0.03 (2)				
R-Q	0/28	-38.5	-38.5 0.03 (2)				
Q-P	0/0	-38.5	-38.5 0.02 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015
- CSA 088-14
- TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (A-B:1), BC=0.03/1.00 (AA-AB:2), WB=0.17/1.00 (I-T:1), SS=0.09/1.00 (A-B:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

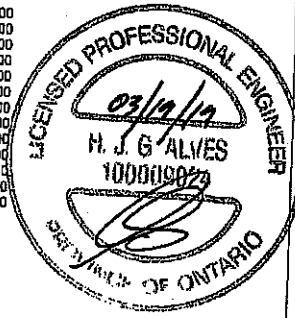
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

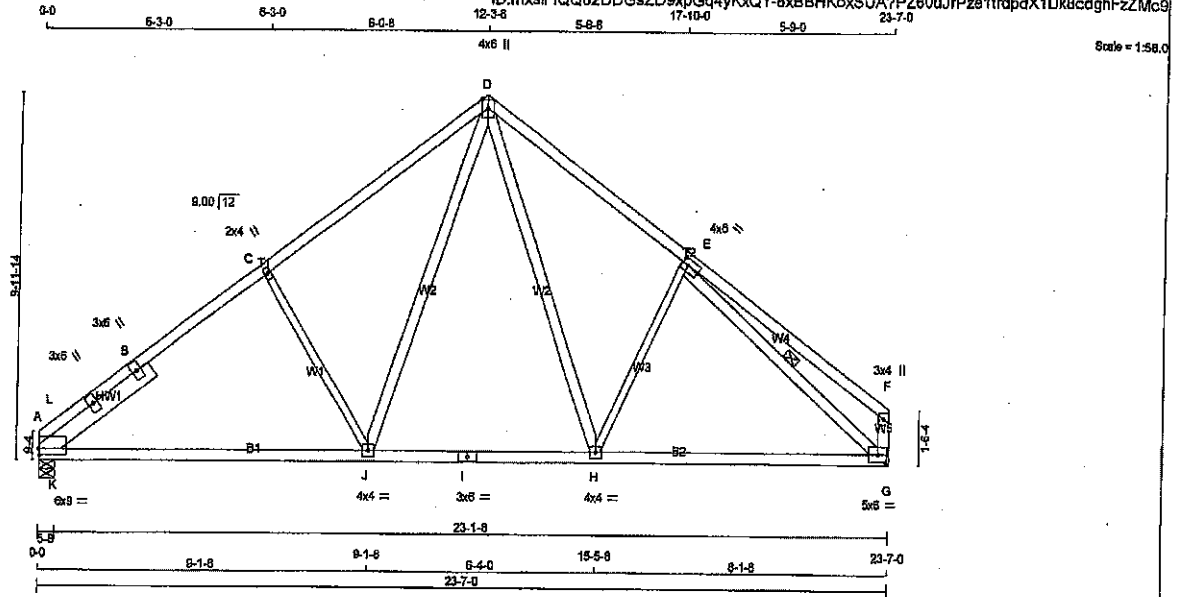
JSI GRIP= 0.47 (H) (INPUT = 0.90)
JSI METAL = 0.12 (I) (INPUT = 1.00)



JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401811	T10	3	1	TRUSS DESC.		

Tamrack Roof Truss, Burlington

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ID:mxsiFIQQ02DDG9pGq4yKxQY-8xBBHKoxSUA?PZ80uJrPze1trdpdX1Dk8cdghFzZMc9



TOTAL WEIGHT = 3 X 109 = 328 lb

LUMBER			
N. L. G. A. RULES	CHORDS	SIZE	LUMBER
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
G - F	2x4	DRY	No.2
A - I	2x4	DRY	No.2
I - G	2x4	DRY	No.2
REINFORCING MEMBERS			
HW1	2x6	DRY	No.2
ALL WEBS			
EXCEPT	2x4	DRY	No.2
C - J	2x3	DRY	No.2
H - E	2x3	DRY	No.2
DRY, SEASONED LUMBER.			

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
A	TMBR1-1	MT20	6.0	9.0	2.00 0.25
A	RT+1	MT20	3.0	6.0	
A	RT+1	MT20	3.0	6.0	
C	TMW+1	MT20	2.0	4.0	
D	TTWW+1	MT20	4.0	6.0	Edge
E	TMW+1	MT20	4.0	6.0	2.00 2.00
F	TMW+1	MT20	3.0	4.0	
G	BMVW1-1	MT20	5.0	6.0	
H	BMVW1-1	MT20	4.0	4.0	
I	BS1	MT20	3.0	6.0	
J	BMVW1-1	MT20	4.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	GROSS REACTION	HORZ	GROSS REACTION	HORZ	BRG	IN-SX	BRG	IN-SX
A	1658	0	1658	0	0	5-8	5-8		
G	1658	0	1658	0	0	MECHANICAL			

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT G. MINIMUM BEARING LENGTH AT JOINT G = 1-13.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN COMPONENT REACTIONS	DEAD	SOIL
A	1239	885 / 0	248 / 0	0 / 0
G	1239	885 / 0	248 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 4.38 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-G.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WEBS		FACTORED	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	CS1 (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM	TO		FR-TO		
A-L	-2387 / 0	-102.1	-102.1	0.10 (1)	4.38	C-J	-552 / 0
L-B	-1695 / 0	-102.1	-102.1	0.47 (1)	4.80	J-D	0 / 799
B-C	-1695 / 0	-102.1	-102.1	0.47 (1)	4.60	D-H	0 / 856
C-D	-1612 / 0	-102.1	-102.1	0.52 (1)	4.62	H-E	-347 / 16
D-E	-1560 / 0	-102.1	-102.1	0.47 (1)	4.74	E-G	-1891 / 0
E-F	0 / 42	-102.1	-102.1	0.54 (1)	10.00	K-L	0 / 929
G-F	-216 / 0	0.0	0.0	0.02 (1)	7.61	K-B	-438 / 173
A-K	0 / 1242	-38.5	-38.5	0.54 (2)	10.00		
K-J	0 / 1538	-38.5	-38.5	0.58 (2)	10.00		
J-I	0 / 1010	-38.5	-38.5	0.52 (2)	10.00		
I-H	0 / 1010	-38.5	-38.5	0.52 (2)	10.00		
H-G	0 / 1362	-38.5	-38.5	0.57 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL =	28.0	PSF
	DL =	6.0	PSF
BOT CH.	LL =	10.5	PSF
	DL =	7.0	PSF
TOTAL LOAD		32.5	PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA D89-09, CSA D86-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.79")
CALCULATED VERT. DEFL.(LL) = L/999 (0.18")
ALLOWABLE DEFL.(TL) = L/360 (0.79")
CALCULATED VERT. DEFL.(TL) = L/932 (0.30")

CSI: TC=0.54/1.00 (E-F:1), BC=0.57/1.00 (G-H:2), WB=0.54/1.00 (E-G:1), SSI=0.50/1.00 (A-K:3)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE LEFT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

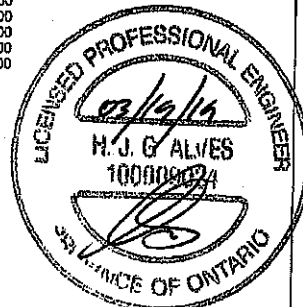
PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLI)	(PLI)
MT20	618	354	1687
	618	354	1687

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

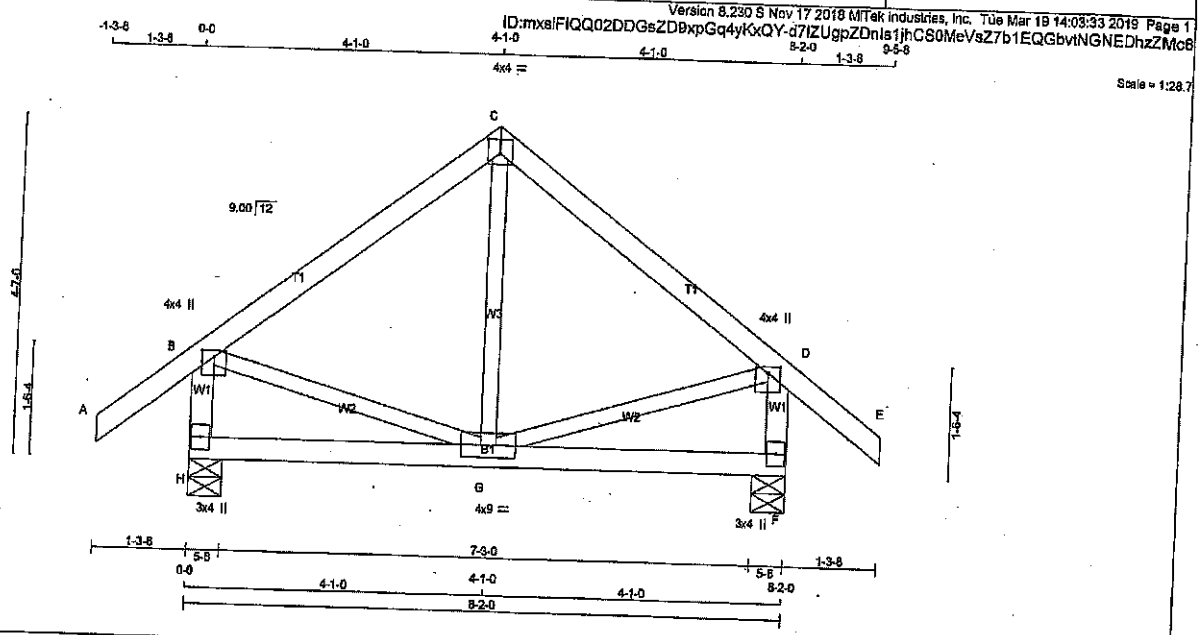
JSI GRIP= 0.90 (J) (INPUT = 0.90)

JSI METAL= 0.62 (A) (INPUT = 1.00)



DRWG NO. 17905702
STRUCTURAL
COMMENT ONLY

Tamarack Roof Truss, Burlington



Scale = 1/28.7

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - C	2x4	DRY No.2
C - E	2x4	DRY No.2
H - B	2x4	DRY No.2
F - D	2x4	DRY No.2
H - F	2x4	DRY No.2

ALL WEBS 2x3 DRY No.2 EXCEPT

DRY: SEASONED LUMBER.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C	TTW+p	MT20	4.0	4.0	2.25	2.00
D	TMVW+p	MT20	4.0	4.0	1.00	2.00
F	BMV+p	MT20	3.0	4.0		
G	BMVW+p	MT20	4.0	9.0		
H	BMV+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
H	715	0	715	0	5-8	5-8
F	715	0	715	0	5-8	5-8

UNFACTORED REACTIONS

JT	1ST LOASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
H	528	317 / 0	88 / 0	0 / 0	0 / 0	123 / 0	0 / 0
F	528	317 / 0	88 / 0	0 / 0	0 / 0	123 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	FACTORED FORCE (LBS)	FACTORED				MEMB.	MAX. FACTORED	
		VERT. LOAD	LC1	MAX (FLF) CSI (LC)	MAX UNBRAC LENGTH		FORCE (LBS)	MAX CSI (LC)
R-TO		FROM	TO			FR-TO		
B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	G-C	0 / 181	
C	-366 / 0	-102.1	-102.1	0.22 (1)	6.25	B-G	0 / 305	
D	-366 / 0	-102.1	-102.1	0.22 (1)	6.25	G-D	0 / 305	
E	0 / 42	-102.1	-102.1	0.14 (1)	10.00			
H	-654 / 0	0.0	0.0	0.07 (1)	7.81			
B	-654 / 0	0.0	0.0	0.07 (1)	7.81			
G	0 / 0	-38.5	-38.5	0.15 (3)	10.00			
F	0 / 0	-38.5	-38.5	0.15 (3)	10.00			

TOTAL WEIGHT = 2 X 37 = 74 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 085-14
- TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.27")
CALCULATED VERT. DEFL.(LL) = L/989 (0.01")
ALLOWABLE DEFL.(TL) = L/360 (0.27")
CALCULATED VERT. DEFL.(TL) = L/989 (0.02")

CSI: TC=0.22/1.00 (C-D:1), BC=0.15/1.00 (G-H:3), WS=0.07/1.00 (D-G:1), SSI=0.13/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

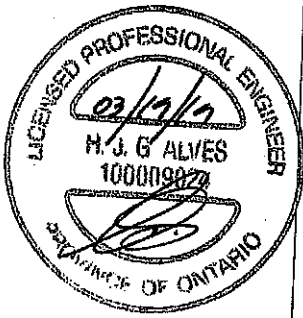
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 786 1687 1873

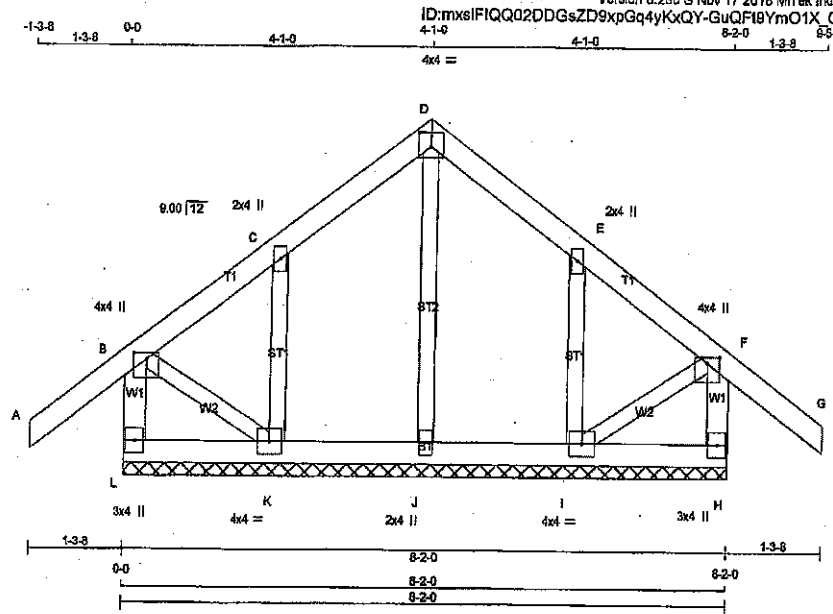
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (D) (INPUT = 0.80)
JSI METAL= 0.14 (D) (INPUT = 1.00)



DRWG NO. TAM 7905703
STRUCTURAL
GENERAL ONLY



TOTAL WEIGHT = 38 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
L - B	2x4	DRY	No.2
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
H - F	2x4	DRY	No.2
L - H	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY, SEASONED LUMBER.			
GABLE STUDS SPACED AT 2'-0" OC.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 28.0 PSF

CL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. G.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 088-14
- TRC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (F-G:1), BC=0.09/1.00 (I-J:3), WB=0.05/1.00 (E-I:1), SS=0.08/1.00 (F-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	650 371	1747 788	1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.18 (F) (INPUT = 0.80)

JSI METAL= 0.13 (E) (INPUT = 1.00)

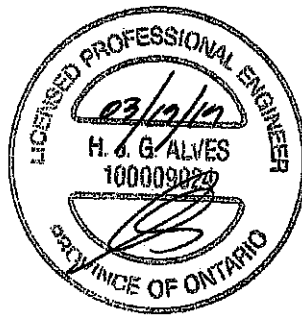
PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	4.0	4.0	1.00	2.00
C	TMW+w	MT20	2.0	4.0		
D	TTW-p	MT20	4.0	4.0	2.25	2.00
E	TMW+w	MT20	2.0	4.0		
F	TMW+p	MT20	4.0	4.0	1.00	2.00
H	BMW+p	MT20	3.0	4.0		
I	BMW+l	MT20	4.0	4.0		
J	BMW+w	MT20	2.0	4.0		
K	BMW+l	MT20	4.0	4.0		
L	BMW+p	MT20	3.0	4.0		

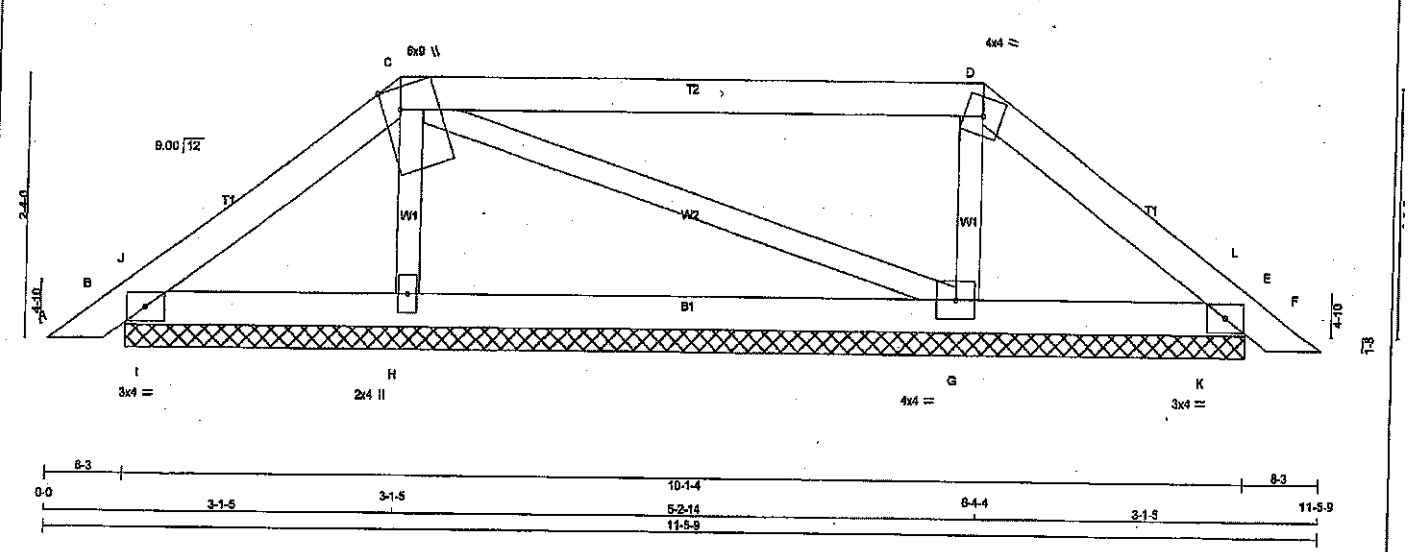
LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRAC LENGTH (LC)
FR-TO		FROM TO			FR-TO		
L-B	-245 / 0	0.0	0.0	0.03 (1)	7.81	J-D	-144 / 0
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	K-C	-253 / 0
B-C	-11 / 0	-102.1	-102.1	0.07 (1)	8.25	I-E	-253 / 0
C-D	-31 / 0	-102.1	-102.1	0.07 (1)	8.25	B-K	0 / 25
D-E	-31 / 0	-102.1	-102.1	0.07 (1)	8.25	I-F	0 / 25
E-F	-11 / 0	-102.1	-102.1	0.07 (1)	8.25		
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00		
H-F	-245 / 0	0.0	0.0	0.03 (1)	7.81		
L-K	0 / 0	-38.5	-38.5	0.03 (3)	10.00		
K-J	0 / 13	-38.5	-38.5	0.03 (3)	10.00		
J-I	0 / 13	-38.5	-38.5	0.03 (3)	10.00		
I-H	0 / 0	-38.5	-38.5	0.03 (3)	10.00		



DWG NO. TAM **77905704**
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 2 X 33 = 66 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR
A - G	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES Table 1a (in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-H	MT20	3.0	4.0	
C	TTWW+m	MT20	6.0	9.0	Edge 2.00
D	TTW-m	MT20	4.0	4.0	
E	TMB1-H	MT20	3.0	4.0	
G	BMWV1-H	MT20	4.0	4.0	
H	BMW1+W	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
B	270	0	270	0	10-1-4	10-1-4
E	268	0	256	0	10-1-4	10-1-4
H	494	0	494	0	10-1-4	10-1-4
G	521	0	521	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED	MAX./MIN. COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	190	142 / 0	11 / 0	0 / 0	0 / 0	37 / 0	0 / 0
E	181	134 / 0	11 / 0	0 / 0	0 / 0	35 / 0	0 / 0
H	378	182 / 0	95 / 0	0 / 0	0 / 0	101 / 0	0 / 0
G	398	188 / 0	95 / 0	0 / 0	0 / 0	104 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)		
FR-TO		FROM TO			FR-TO				
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	H-C	-288 / 0	0.05 (1)	
B-J	-63 / 0	-102.1	-102.1	0.03 (1)	8.25	C-G	-20 / 0	0.01 (1)	
J-C	-111 / 0	-102.1	-102.1	0.07 (1)	8.25	G-D	-319 / 0	0.05 (1)	
C-D	-49 / 0	-102.1	-102.1	0.49 (1)	8.25	I-J	-194 / 0	0.00 (1)	
D-L	-89 / 0	-102.1	-102.1	0.07 (1)	8.25	K-L	-196 / 0	0.00 (1)	
L-E	-38 / 1	-102.1	-102.1	0.03 (1)	8.25				
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00				
B-I	0 / 84	-38.5	-38.5	0.08 (1)	10.00				
I-H	0 / 84	-38.5	-38.5	0.13 (2)	10.00				
H-G	0 / 67	-38.5	-38.5	0.13 (2)	10.00				
G-K	0 / 66	-38.5	-38.5	0.13 (2)	10.00				
K-E	0 / 66	-38.5	-38.5	0.08 (1)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 9.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BOBC 2018
- CSA D86-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-D:1), BC=0.13/1.00 (H-I:2), WB=0.05/1.00 (D-G:1), SS=0.21/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

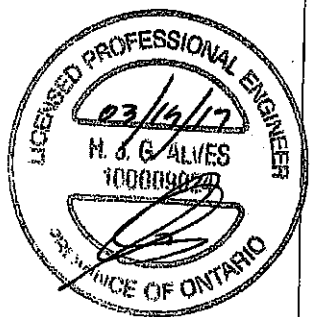
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

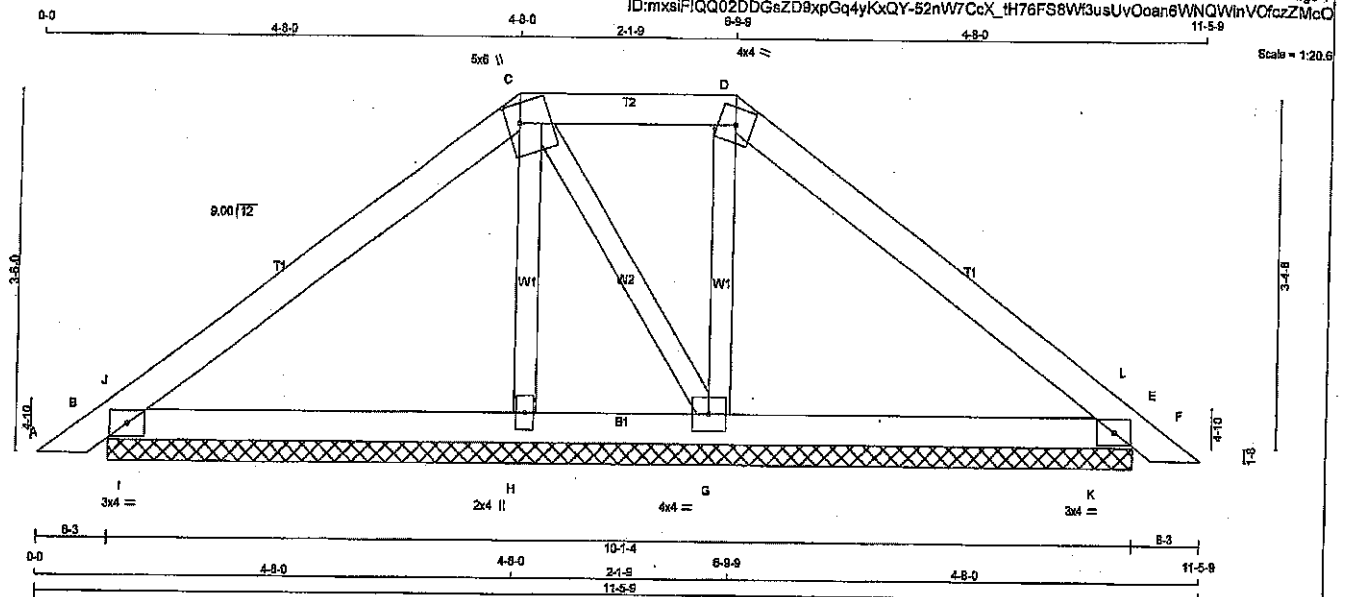
PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)



DRWG NO. TAM T1905705
STRUCTURAL
CONCRETE WORK ONLY



TOTAL WEIGHT = 2 X 34 = 68 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF	
C - D	2x4	DRY	No.2	SPF	
D - F	2x4	DRY	No.2	SPF	
B - E	2x4	DRY	No.2	SPF	
ALL WEBS	2x3	DRY	No.2	SPF	
DRY, SEASONED LUMBER.					

PLATES (table is in inches)	JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0			
C	TTWV+m	MT20	5.0	8.0	2.25	1.50	
D	TTW-m	MT20	4.0	4.0			
E	TMB1-I	MT20	3.0	4.0			
G	BMWV1-I	MT20	4.0	4.0			
H	BMW1+w	MT20	2.0	4.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQRD BRG
JT	VERT	HORZ	DOWN	HORZ
B	438	0	438	0
E	418	0	418	0
H	285	0	285	0
G	399	0	399	0

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN. COMPONENT REACTIONS	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
JT	COMBINED						
B	320	202 / 0	46 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	308	189 / 0	46 / 0	0 / 0	0 / 0	70 / 0	0 / 0
H	221	98 / 0	62 / 0	0 / 0	0 / 0	81 / 0	0 / 0
G	293	166 / 0	59 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CS (LC)	WEBS	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CS (LC)
MEMB.				MEMB.		
FR-TO		FROM TO		FR-TO		
A-B	0 / 17	-102.1 -102.1	0.03 (1)	10.00	H-C	-125 / 0
B-J	-122 / 0	-102.1 -102.1	0.07 (1)	6.25	C-G	-55 / 0
J-C	-197 / 0	-102.1 -102.1	0.21 (1)	6.25	G-D	-192 / 0
C-D	-116 / 0	-102.1 -102.1	0.08 (1)	6.25	I-J	-373 / 71
D-L	-162 / 0	-102.1 -102.1	0.21 (1)	6.25	K-L	-377 / 70
L-E	-118 / 0	-102.1 -102.1	0.07 (1)	6.25		
E-F	0 / 17	-102.1 -102.1	0.03 (1)	10.00		
B-I	0 / 151	-38.5 -38.5	0.17 (1)	10.00		
I-H	0 / 151	-38.5 -38.5	0.18 (1)	10.00		
H-G	0 / 147	-38.5 -38.5	0.14 (1)	10.00		
G-K	0 / 123	-38.5 -38.5	0.18 (1)	10.00		
K-E	0 / 123	-38.5 -38.5	0.17 (1)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018
- CSA 088-14
- TPC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J:1), BC=0.18/1.00 (H-I:1), WB=0.04/1.00 (D-G:1), SSI=0.32/1.00 (E-K:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

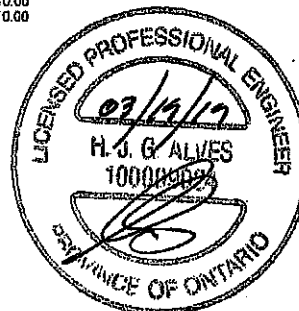
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLD) (PLI)
MT20 650 371 1747 788 1967 1873

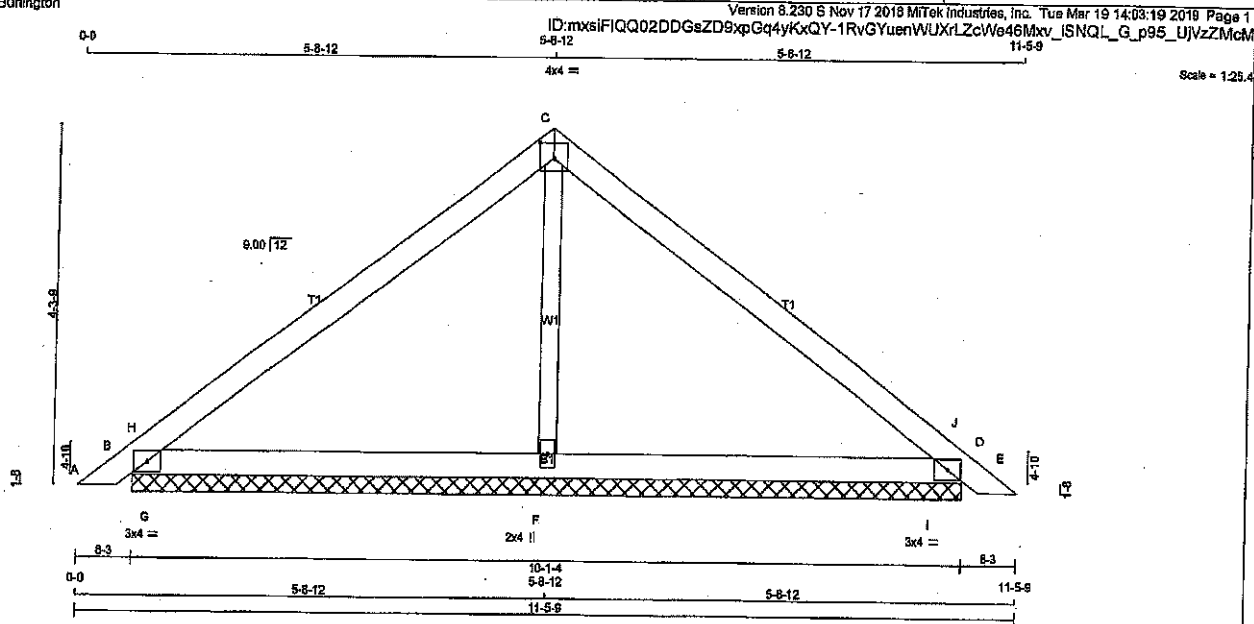
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.50)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NTA TAM T1905706
STRUCTURAL
COMPONENT ONLY



TOTAL WEIGHT = 31 lb

LUMBER			
N. L. G. A. RULES	SIZE	DRY	NO.2
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
B - D	2x4	DRY	No.2
ALL WEBS 2x3 DRY			
DRY, SEASONED LUMBER.			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER							
BEARINGS							
JT	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG			
	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
B	528	0	528	0	0	10-1-4	10-1-4
D	528	0	528	0	0	10-1-4	10-1-4
F	485	0	485	0	0	10-1-4	10-1-4

DESIGN CRITERIA	
SPECIFIED LOADS:	
TOP CH. LL =	29.0 PSF
DL =	8.0 PSF
BOT CH. LL =	10.5 PSF
DL =	7.0 PSF
TOTAL LOAD =	52.5 PSF

PLATES (table is in inches)					
JT	TYPE	PLATES	W	LEN	Y X
B	TM81-I	MT20	3.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TM81-I	MT20	3.0	4.0	
F	BMW1-w	MT20	2.0	4.0	

UNFACTORED REACTIONS							
JT	1ST LCASE	MAX. MIN. COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
B	364	246 / 0	53 / 0	0 / 0	0 / 0	88 / 0	0 / 0
D	364	246 / 0	53 / 0	0 / 0	0 / 0	88 / 0	0 / 0
F	377	164 / 0	107 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO					FR-TO				
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	F-C	-132 / 10	0.03 (1)	
B-H	-128 / 25	-102.1	-102.1	0.17 (1)	8.25	G-H	-883 / 45	0.00 (1)	
H-C	-270 / 0	-102.1	-102.1	0.39 (1)	8.25	I-J	-883 / 45	0.00 (1)	
C-J	-270 / 0	-102.1	-102.1	0.39 (1)	8.25				
J-D	-128 / 25	-102.1	-102.1	0.17 (1)	8.25				
D-E	0 / 17	-102.1	-102.1	0.03 (1)	10.00				
B-G	0 / 204	-38.5	-38.5	0.29 (1)	10.00				
G-F	0 / 204	-38.5	-38.5	0.33 (1)	10.00				
F-I	0 / 204	-38.5	-38.5	0.33 (1)	10.00				
I-D	0 / 204	-38.5	-38.5	0.29 (1)	10.00				

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 8, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF BCBC 2018
- CSA 088-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J:1), BC=0.33/1.00 (F-G:1), WB=0.03/1.00 (C-F:1), SSI=0.55/1.00 (D-I:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.50)
JSI METAL= 0.11 (B) (INPUT = 1.00)



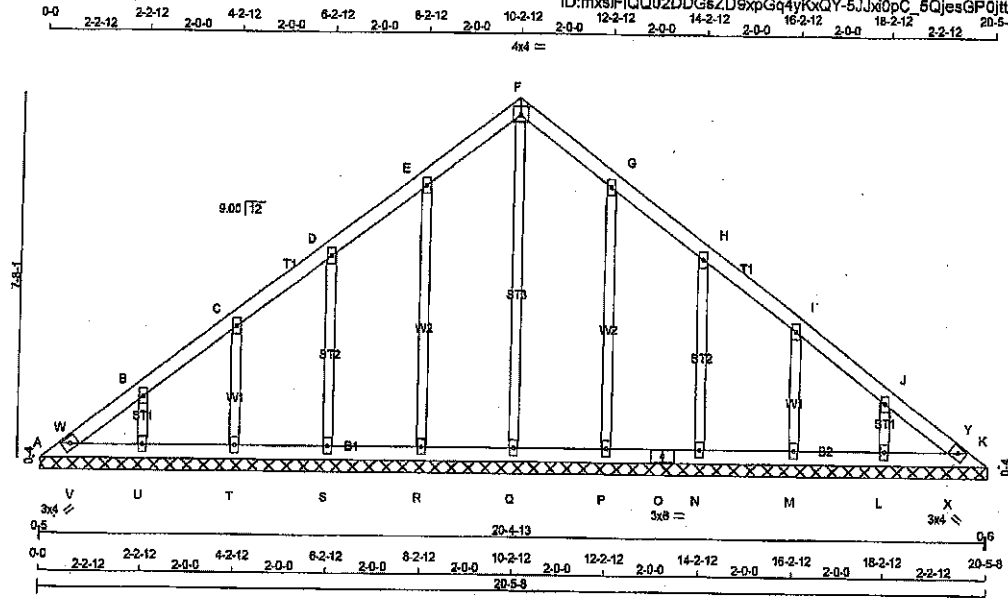
DWG NO. TAM **7795707**
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401811	V1	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsIFIQ02DDGsZD9xpGq4yKxQY-5JjM0pC_5QjesGPQIH236LrQc671a0cw6n7zZMc7



TOTAL WEIGHT = 79 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - F	2x4	DRY	No.2	SPF
F - K	2x4	DRY	No.2	SPF
A - O	2x4	DRY	No.2	SPF
O - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
		DRY, SEASONED LUMBER.		

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B, C, D, E, G, H, I, J						
B	TW1+w	MT20	2.0	4.0		
F	TTW-p	MT20	4.0	4.0	2.25	2.00
K	TBM1-h	MT20	3.0	4.0		
L, M, N, P, Q, R, S, T, U						
L	BW1+w	MT20	2.0	4.0		
O	BS-t	MT20	3.0	8.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	141	0	141	0	20-5-8 (6-11-22)5-8	
K	141	0	141	0	20-5-8 (6-11-22)5-8	
Q	238	0	238	0	20-5-8 (6-11-22)5-8	
S	276	0	276	0	20-5-8 (6-11-22)5-8	
U	329	0	329	0	20-5-8 (6-11-22)5-8	
N	276	0	276	0	20-5-8 (6-11-22)5-8	
L	329	0	329	0	20-5-8 (6-11-22)5-8	
R	308	0	308	0	20-5-8 (6-11-22)5-8	
T	281	0	281	0	20-5-8 (6-11-22)5-8	
P	308	0	308	0	20-5-8 (6-11-22)5-8	
M	281	0	281	0	20-5-8 (6-11-22)5-8	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	104	62/0	17/0	0/0	0/0	24/0	0/0
K	104	62/0	17/0	0/0	0/0	24/0	0/0
Q	182	98/0	48/0	0/0	0/0	48/0	0/0
S	207	113/0	42/0	0/0	0/0	52/0	0/0
U	246	135/0	50/0	0/0	0/0	81/0	0/0
N	207	113/0	42/0	0/0	0/0	52/0	0/0
L	246	135/0	50/0	0/0	0/0	81/0	0/0
R	229	132/0	42/0	0/0	0/0	55/0	0/0
T	198	107/0	40/0	0/0	0/0	49/0	0/0
P	229	132/0	42/0	0/0	0/0	55/0	0/0
M	198	107/0	40/0	0/0	0/0	49/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K, Q, S, U, N, L, R, T, P, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS			WEBS		
	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED CSI (LC)	MAX. UNBRACED LENGTH (FT)	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM TO				
A-W	-54/0	-102.1 -102.1	0.03 (1)	8.25	Q-F	-169/0 0.17 (1)
W-B	-17/8	-102.1 -102.1	0.08 (1)	8.25	S-D	-198/0 0.08 (1)
B-C	-29/0	-102.1 -102.1	0.08 (1)	8.25	U-B	-211/0 0.03 (1)
C-D	-19/4	-102.1 -102.1	0.04 (1)	8.25	N-H	-198/0 0.08 (1)
D-E	-10/5	-102.1 -102.1	0.08 (1)	8.25	L-J	-211/0 0.03 (1)
E-F	-22/3	-102.1 -102.1	0.08 (1)	8.25	R-E	-232/0 0.14 (1)
F-G	-22/3	-102.1 -102.1	0.08 (1)	8.25	T-C	-198/0 0.04 (1)
G-H	-10/6	-102.1 -102.1	0.08 (1)	8.25	P-G	-232/0 0.14 (1)
H-I	-19/4	-102.1 -102.1	0.04 (1)	8.25	M-I	-198/0 0.04 (1)
I-J	-29/0	-102.1 -102.1	0.08 (1)	8.25	V-W	-40/17 0.00 (1)
J-Y	-17/8	-102.1 -102.1	0.08 (1)	8.25	X-Y	-40/17 0.00 (1)
Y-K	-54/0	-102.1 -102.1	0.03 (1)	8.25		
A-V	-2/27	-38.5 -38.5	0.05 (1)	10.00		
V-U	0/35	-38.5 -38.5	0.05 (1)	10.00		
U-T	-2/21	-38.5 -38.5	0.04 (1)	10.00		
T-S	-3/15	-38.5 -38.5	0.02 (2)	10.00		
S-R	-3/11	-38.5 -38.5	0.02 (2)	10.00		
R-Q	-4/7	-38.5 -38.5	0.02 (3)	10.00		
Q-P	-4/7	-38.5 -38.5	0.02 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GIC

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.06/1.00 (E-F-1), BC=0.05/1.00 (L-X-1), WB=0.17/1.00 (F-Q-1), SSI=0.08/1.00 (F-Q-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1686

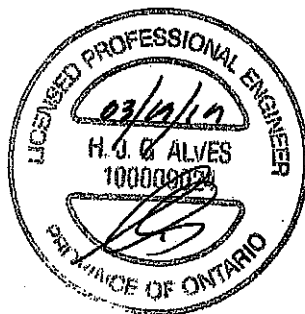
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.49 (F) (INPUT = 0.90)
JSI METAL= 0.12 (G) (INPUT = 1.00)

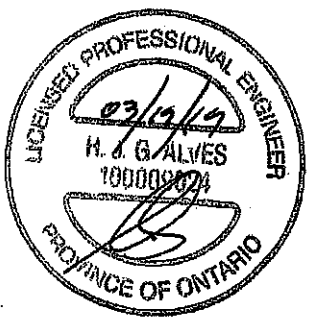
DRWG NO. 71905708
STRUCTURAL
COMMENT ONLY

CONTINUED ON PAGE 2



JOB NAME 401811	TRUSS NAME V1	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				Version 8.230 8 Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:34 2019 Page 2 ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-5JjX0pC 5QiesGP0H236LrQc671a0cw6nI7ZZMc7	

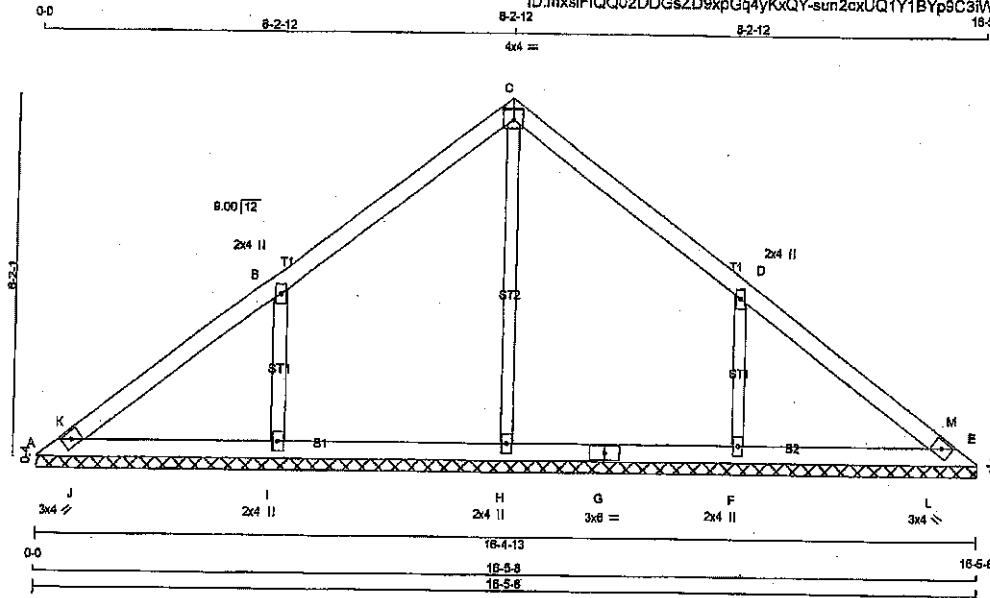
LOADING									
TOTAL LOAD CASES: (4)									
CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)	FR-TO	
P-O	-3/11	-38.5	-38.5	0.02 (2)	10.00				
O-N	-3/11	-38.5	-38.5	0.02 (2)	10.00				
N-M	-3/15	-38.5	-38.5	0.02 (2)	10.00				
M-L	-2/21	-38.5	-38.5	0.04 (1)	10.00				
L-X	0/35	-38.5	-38.5	0.05 (1)	10.00				
X-K	-2/27	-38.5	-38.5	0.05 (1)	10.00				



DWG NO. TAM 779057 08
 STRUCTURAL
 CONSULTANT ONLY 3/2

JOB NAME 401811	TRUSS NAME V2	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:mxsiFIQ02DDGsZD9xpGq4yKxQY-sun2cxUQ1Y1BYp9C3IWpW09RXJxfDZctE0f3NZzZLd
18-5-8



TOTAL WEIGHT = 50 lb [M]

LUMBER	SIZE	DRY	No.2	SPF
N. L. G. A. RULES				
CHORDS				
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - G	2x4	DRY	No.2	SPF
G - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)	W	LEN	Y	X
JT TYPE				
A TBM1-h	MT20	3.0	4.0	
B TMW+w	MT20	2.0	4.0	
C TTW-p	MT20	4.0	4.0	2.25
D TMW+w	MT20	2.0	4.0	
E TBM1-h	MT20	3.0	4.0	
F, H, I				
F BMW1+w	MT20	2.0	4.0	
G BS-t	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQD BRG	
JT	VERT	HORZ	DOWN	HORZ	UPLIFT	IN-SX	IN-SX
A	178	0	178	0	0	16-4-13 (6-5-10) 4-13	
E	178	0	178	0	0	16-4-13 (6-5-10) 4-13	
H	572	0	572	0	0	16-4-13 (6-5-10) 4-13	
I	889	0	889	0	0	16-4-13 (6-5-10) 4-13	
F	889	0	889	0	0	16-4-13 (6-5-10) 4-13	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	130	80/0	21/0	0/0	0/0	30/0	0/0
E	130	80/0	21/0	0/0	0/0	30/0	0/0
H	438	209/0	112/0	0/0	0/0	118/0	0/0
I	512	292/0	98/0	0/0	0/0	124/0	0/0
F	512	292/0	98/0	0/0	0/0	124/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-K	0/56	-102.1 -102.1	0.06 (2)	10.00	H-C	-444/0	0.28 (1)
K-B	0/124	-102.1 -102.1	0.28 (1)	10.00	I-B	-488/0	0.09 (1)
B-C	0/80	-102.1 -102.1	0.28 (1)	10.00	F-D	-488/0	0.09 (1)
C-D	0/80	-102.1 -102.1	0.28 (1)	10.00	J-K	-85/47	0.03 (1)
D-M	0/124	-102.1 -102.1	0.28 (1)	10.00	L-M	-85/47	0.03 (1)
M-E	0/56	-102.1 -102.1	0.06 (2)	10.00			
A-J	-39/0	-38.5 -38.5	0.10 (1)	6.25			
J-I	-70/0	-38.5 -38.5	0.13 (2)	6.25			
I-H	-82/0	-38.5 -38.5	0.13 (2)	6.25			
H-G	-82/0	-38.5 -38.5	0.13 (2)	6.25			
G-F	-82/0	-38.5 -38.5	0.13 (2)	6.25			
F-L	-70/0	-38.5 -38.5	0.13 (2)	6.25			
L-E	-39/0	-38.5 -38.5	0.10 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.28/1.00 (B-K:1), BC=0.13/1.00 (H-I:2), WB=0.28/1.00 (C-H:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

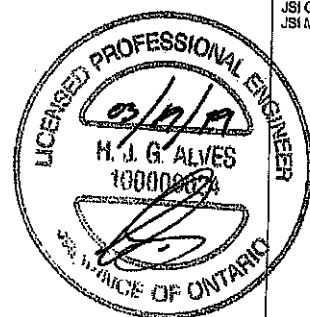
NAIL VALUES

PLATE	GRIP (GRY)	SHEAR (PSI)	SECTION (PLI)	MAX MIN	MAX MIN	MAX MIN
MT20	618	354	1687	783	1987	1688

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.40 (C) (INPUT = 0.60)
JSI METAL= 0.26 (D) (INPUT = 1.00)

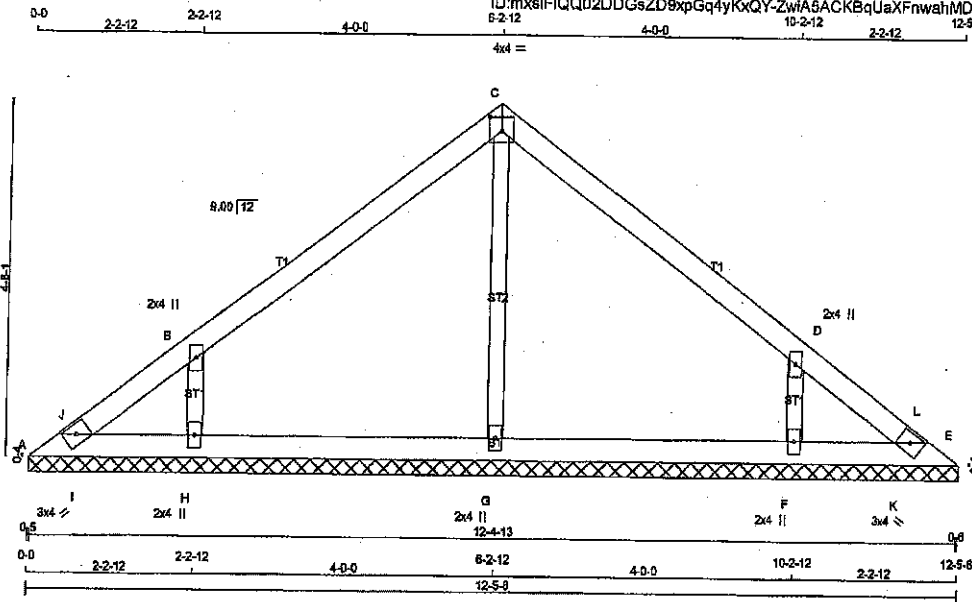


DRWG NO. TAM 17905709
STRUCTURAL
COMPONENT CWP V

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401811	V3	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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TOTAL WEIGHT = 36 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	DRY	LUMBER
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

DESCR.

SPF

SPF

SPF

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY

BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	110	0	110	0	12-5-8 (12-4-13)5-8	12-5-8
E	110	0	110	0	12-5-8 (12-4-13)5-8	12-5-8
G	409	0	409	0	12-5-8 (12-4-13)5-8	12-5-8
H	557	0	557	0	12-5-8 (12-4-13)5-8	12-5-8
F	557	0	557	0	12-5-8 (12-4-13)5-8	12-5-8

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	81	49/0	13/0	0/0	0/0	19/0	0/0
E	81	49/0	13/0	0/0	0/0	19/0	0/0
G	317	139/0	90/0	0/0	0/0	88/0	0/0
H	412	241/0	72/0	0/0	0/0	88/0	0/0
F	412	241/0	72/0	0/0	0/0	88/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. MAX. UNBRACED LENGTH (LC)	
FR-TO		FROM TO		FR-TO			
A-J	-120/0	-102.1 -102.1	0.03 (1)	6.25	G-C	-243/0	0.08 (1)
J-B	-32/0	-102.1 -102.1	0.23 (1)	6.25	H-B	-437/0	0.08 (1)
B-C	-102/0	-102.1 -102.1	0.23 (1)	6.25	F-D	-437/0	0.08 (1)
C-D	-102/0	-102.1 -102.1	0.23 (1)	6.25	I-J	0/45	0.00 (1)
D-L	-32/0	-102.1 -102.1	0.23 (1)	6.25	K-L	0/45	0.00 (1)
L-E	-120/0	-102.1 -102.1	0.03 (1)	6.25			
A-I	0/95	-38.5 -38.5	0.02 (1)	10.00			
I-H	0/92	-38.5 -38.5	0.08 (2)	10.00			
H-G	0/62	-38.5 -38.5	0.11 (2)	10.00			
G-F	0/62	-38.5 -38.5	0.11 (2)	10.00			
F-K	0/92	-38.5 -38.5	0.08 (2)	10.00			
K-E	0/85	-38.5 -38.5	0.02 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 29.0 PSF
	DL = 6.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN./C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBCC 2010, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.23/1.00 (B-C:1), BC=0.11/1.00 (C-H:2), WB=0.08/1.00 (C-G:1), SS=0.16/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

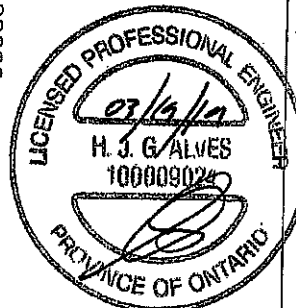
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667
			768
			1937
			1656

PLATE PLACEMENT TOL. = 0.250 inches

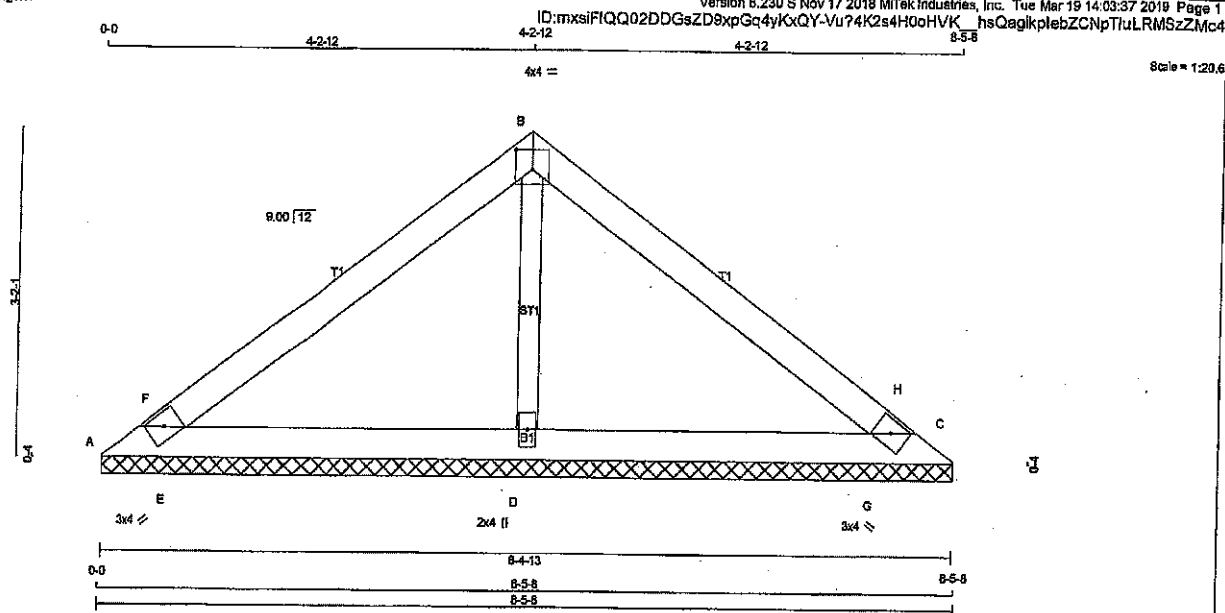
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.31 (D) (INPUT = 0.90)
JSI METAL= 0.23 (B) (INPUT = 1.00)



DWG NO. TAM 71905710
STRUCTURAL
COMPONENT ONLY

JOB NAME 401811	TRUSS NAME V4	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	



LUMBER

N.L.G.A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF
B - C	2x4	DRY	No.2	SPF
A - C	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TM1-h	MT20	3.0	4.0		
B	TTV+p	MT20	4.0	4.0	2.25	2.00
C	TM1-h	MT20	3.0	4.0		
D	BMV1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	70	0	70	0	8-4-13	8-4-13
C	70	0	70	0	8-4-13	8-4-13
D	1042	0	1042	0	8-4-13	8-4-13

UNFACTORED REACTIONS

JT	1ST LCASE	MAX/MIN COMPONENT REACTIONS					
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	52	29/0	10/0	0/0	0/0	13/0	0/0
C	52	29/0	10/0	0/0	0/0	13/0	0/0
D	779	430/0	156/0	0/0	0/0	193/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED CSI (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)	
FR-TO		FROM TO		FR-TO			
A-F	0/347	-102.1 -102.1	0.11 (1)	D-B	-755/0	0.14 (1)	
F-B	0/347	-102.1 -102.1	0.24 (1)	E-F	-222/19	0.00 (1)	
B-H	0/347	-102.1 -102.1	0.24 (1)	G-H	-222/19	0.00 (1)	
H-C	0/347	-102.1 -102.1	0.11 (1)				
A-E	-322/0	-38.5 -38.5	0.17 (1)				
E-D	-283/0	-38.5 -38.5	0.20 (1)				
D-G	-283/0	-38.5 -38.5	0.20 (1)				
G-C	-322/0	-38.5 -38.5	0.17 (1)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL = 28.0 PSF
	DL = 8.0 PSF
BOT CH.	LL = 10.5 PSF
	DL = 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 8 OF BCBC 2018, OBC 2012
- CSA 088-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/H.00 (B-F:1), BC=0.20/H.00 (D-E:1), WB=0.14/H.00 (B-D:1), SS=0.14/H.00 (A-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

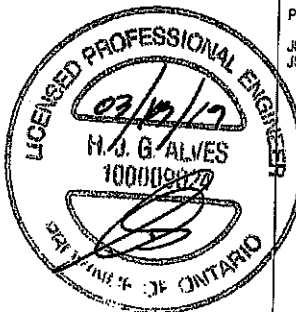
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1657

PLATE PLACEMENT TOL = 0.250 inches

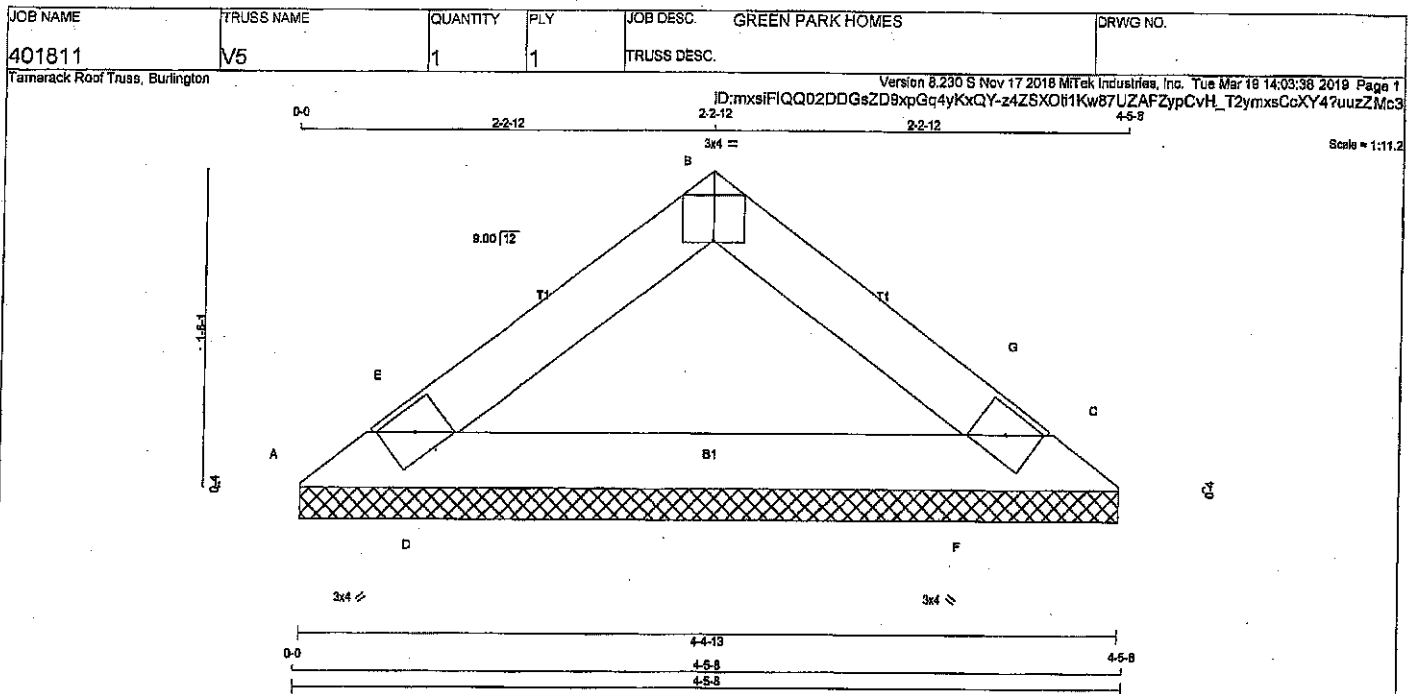
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.68 (B) (INPUT = 0.80)
JSI METAL= 0.21 (B) (INPUT = 1.00)



DRWG NO. TAM 71905711
STRUCTURAL
COMPONENT ONLY

TOTAL WEIGHT = 23 lb



TOTAL WEIGHT = 10 lb

LUMBER

N. L. G. A. RULES

CHORDS SIZE

A - B 2x4 DRY
B - C 2x4 DRY
A - C 2x4 DRY

LUMBER

No.2
No.2
No.2

DESCR.

SPF
SPF
SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TT-p	MT20	3.0	4.0	Edge	2.00
C	TBM1-h	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	309	0	309	0	4-4-13	4-4-13
C	309	0	309	0	4-4-13	4-4-13

UNFACTORED REACTIONS

JT	1ST LCASE COMBINED		MAX/MIN COMPONENT REACTIONS		PERM. LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
A	231	128 / 0	46 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0
C	231	128 / 0	46 / 0	0 / 0	0 / 0	0 / 0	57 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		WEBS		MEMB.	MAX. FACTORED	
	MAX. FACTORED	FACTORED	MAX. FACTORED	FACTORED		FORCE	MAX
	(LBS)	(PLF)	(LBS)	(PLF)		(LBS)	(LBS)
FR-TO		FROM TO	CS1 (LC)	UNBRAC LENGTH	FR-TO		
A-E	-291 / 0	-102.1 -102.1	0.15 (1)	8.25	D-E	0 / 104	0.00 (1)
E-B	-203 / 0	-102.1 -102.1	0.15 (1)	8.25	F-G	0 / 104	0.00 (1)
B-G	-203 / 0	-102.1 -102.1	0.15 (1)	8.25			
G-C	-291 / 0	-102.1 -102.1	0.15 (1)	8.25			
A-D	0 / 192	-38.5 -38.5	0.08 (1)	10.00			
D-F	0 / 197	-38.5 -38.5	0.13 (1)	10.00			
F-C	0 / 192	-38.5 -38.5	0.08 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CS1: TC=0.15/1.00 (C-G:1), BC=0.13/1.00 (D-F:1), WB=0.00/1.00 (D-E:1), SS=0.13/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PS)	(PL)	(PL)	(PL)
MT20	618	354	1887
			788
			1987
			1056

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.16 (C) (INPUT = 0.90)
JSI METAL= 0.09 (A) (INPUT = 1.00)

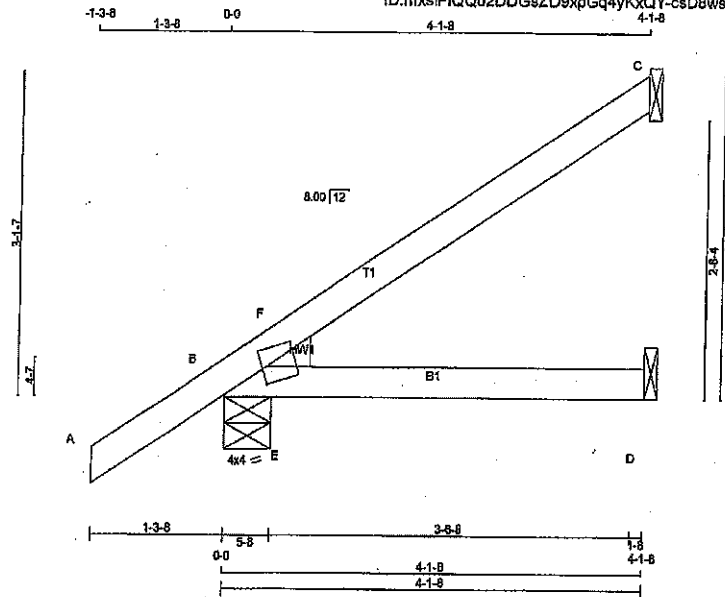


DWG NO. TAM 77905712
STRUCTURAL
CONSULTANT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	J8	5	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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ID:mxsIFIQQ02DDGsZD9xpGq4yKxQY-csD8wscvDZ9GU5bxyYfJGMDnAQenwmNT7iq6AzMcP

Scale = 1:20.5



LUMBER			
N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS	2x4	DRY	SPF
A - C	2x4	DRY	No.2
B - D	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)				
JT TYPE	PLATES	W	LEN	Y X
B	7MBH1-m	MT20	4.0	4.0 2.00 0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED		MAXIMUM FACTORED		INPUT		REQD		HEEL
	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	IN-SX	
C	184	0	184	0	0	0	1-8	1-8	WEDGE
B	430	0	430	0	0	0	5-8	5-8	2x4 L
D	106	0	113	0	0	0	1-8	1-8	

SEE MTEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
C	129	98/0	7/0	0/0	0/0	25/0	0/0
B	313	199/0	43/0	0/0	0/0	70/0	0/0
D	88	22/0	37/0	0/0	0/0	29/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		WEBS		MAX. FACTORED	
	MAX. FORCE	VERT. LOAD	LC1	MAX	MEMB.	MAX. FORCE	MAX	CS1 (LC)
FR-TO								
A-B	0/37	-102.1	-102.1	0.14 (1)	10.00	E-F	-171/62	0.00 (1)
B-F	-47/0	-102.1	-102.1	0.07 (3)	6.25			
F-C	0/0	-102.1	-102.1	0.23 (1)	10.00			
B-E	0/0	-38.5	-38.5	0.18 (1)	10.00			
E-D	0/0	-38.5	-38.5	0.20 (1)	10.00			

TOTAL WEIGHT = 5 X 13 = 64 lb

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 088-09, CSA 088-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/899$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/899$ (0.05")

CS1: TC=0.23/1.00 (C-F:1), BC=0.20/1.00 (D-E:1), WB=0.02/1.00 (E-F:1), SSI=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LG BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
(PSI)	(PLI)	(PLI)	
MAX	MIN	MAX	MIN
MT20	618	354	1567 788 1987 1555

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)
JSI METAL= 0.06 (B) (INPUT = 1.00)



DRWG NO. TAM 71905714
STRUCTURAL
COMPONENT ONLY



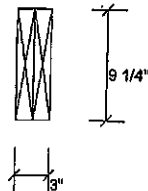
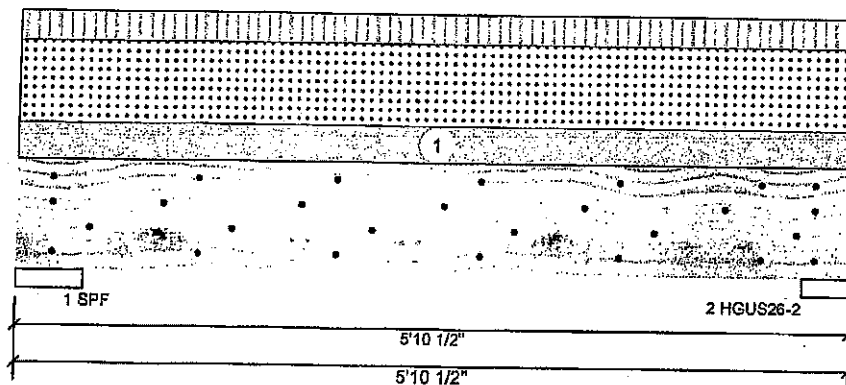
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 1 of 6

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	340	421	939	0
2	326	403	900	0

Bearings and Factored Reactions

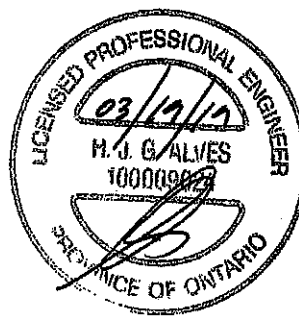
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.600"	22% 626 / 1748	2274 L	1.25D+1.5S +L
2 - HGUS...	4.000"	30% 504 / 1675	2180 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2571 ft-lb	3'	6039 ft-lb	0.426 (43%)	1.25D+1.5S +L	L
Unbraced	2571 ft-lb	3'	5236 ft-lb	0.491 (49%)	1.25D+1.5S +L	L
Shear	1974 lb	12"	3984 lb	0.496 (50%)	1.25D+1.5S +L	L
LL Defl inch	0.022 (L/2784)	3'	0.174 (L/360)	0.130 (13%)	S+0.5L	L
TL Defl inch	0.031 (L/2018)	3'	0.174 (L/360)	0.180 (18%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



BWS NO. TAM 7905728
STRUCTURAL
COMPONENT 10M V 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		10-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3209 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/11/2021





isDesign™

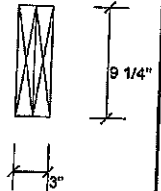
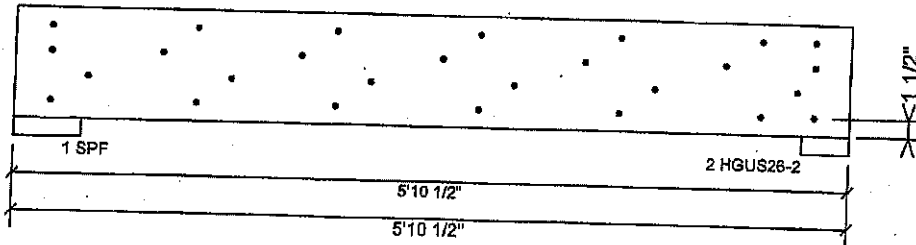
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 2 of 6

BM1 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

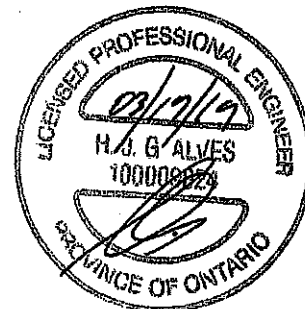
Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	83.6 %
Load	379.1 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DESIGN BY TAM 11705728
STRUCTURAL
CHECKMENT ONLY 3/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115





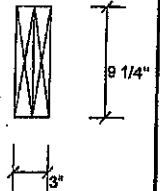
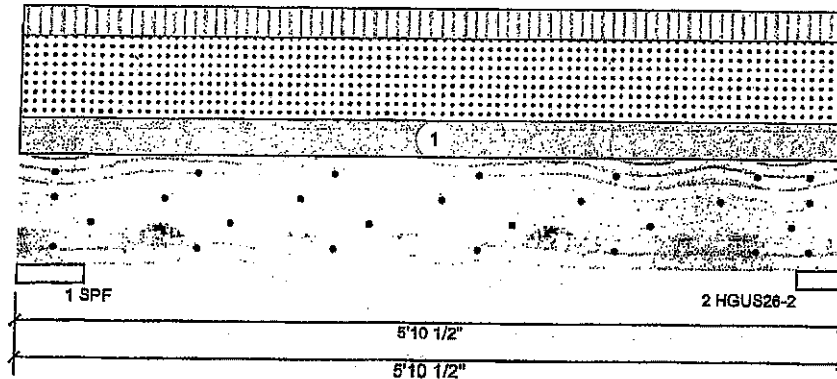
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 3 of 6

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Piles: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	403	499	1113	0
2	386	478	1067	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	27% 624 / 2072	2696 L	1.25D+1.5S +L
2 - HGUS...	4.000"	35% 598 / 1986	2584 L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3047 ft-lb	3'	6039 ft-lb	0.505 (50%)	1.25D+1.5S +L	L
Unbraced	3047 ft-lb	3'	5236 ft-lb	0.582 (58%)	1.25D+1.5S +L	L
Shear	2340 lb	4'10"	3984 lb	0.587 (59%)	1.25D+1.5S +L	L
LL Defl inch	0.027 (L/2348)	3'	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.037 (L/1702)	3'	0.174 (L/360)	0.210 (21%)	D+S+0.5L	L

Design Notes

- 1 Fasten all piles using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



ENG NO. TAM 71905724
STRUCTURAL
CONTRACT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		12-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(605) 235-1115



This design is valid until 12/11/2021





isDesign™

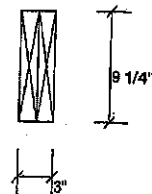
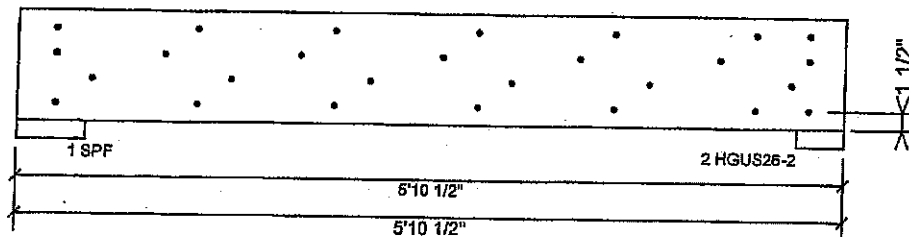
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 4 of 6

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

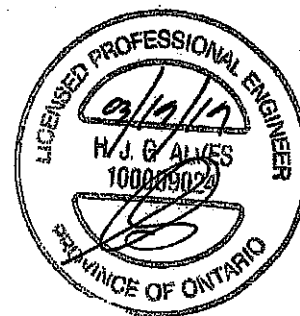
Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 5"

Capacity	99.1 %
Load	449.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



W. J. KO, TAM 77905724
STRUCTURAL
CONSULTANT ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3289 North Service Rd., ON Canada L7N3G2 (905) 335-1115
	TAMARACK LUMBER INC ALMA LUMBER GROUP

This design is valid until 12/11/2021



TECHNICAL BULLETIN

HUS/LJS - Double Shear Joist Hangers

SIMPSON
Strong-Tie

All hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: See table

FINISH: G90 galvanized

DESIGN:

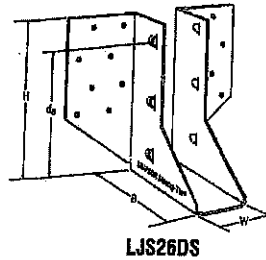
- Factored resistances are in accordance with CSA Q86-14
- Uplift resistances have been increased 15%. No further increase is permitted
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

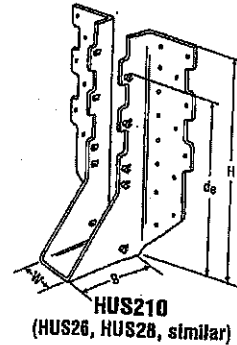
- Use all specified fasteners
- Nails: 16d = 0.162" dia. x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

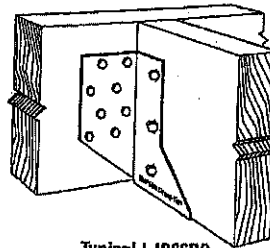
- See current catalogue for options



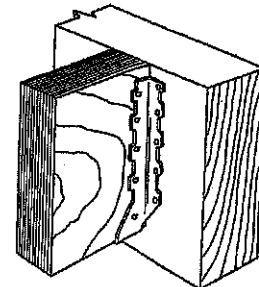
LJS26DS



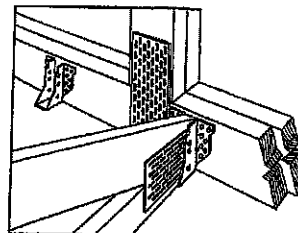
HUS210
(HUS26, HUS28, similar)



Typical LJS26DS
Installation



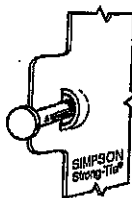
Typical HUS
Installation



Typical HUS Installation
(Truss Designer to provide fastener
quantity for connecting multiple
members together)

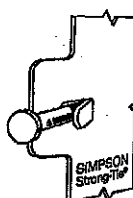
Model No.	Ga	Dimensions (in)			Fasteners			Factored Resistance (lbs)			
		W	H	B	dg1	Face	Joist	D.Fir-L		S-P-F	
								Uplift	Normal	Uplift	Normal
LJS26DS	18	1 1/16	5	3 1/2	4 1/2	16-16d	6-16d	2055	4265	1460	4115
HUS26	18	1 1/16	5 1/2	3	3 1/16	14-16d	6-16d	2705	4940	2065	3875
HUS28	18	1 1/16	7 1/2	3	6 1/2	22-16d	8-16d	3605	5385	2675	4345
HUS210	18	1 1/16	9 1/2	3	7 3/2	30-16d	10-16d	4505	5795	4010	4740
HUS1.81/10	15	1 1/16	9	3	8	30-16d	10-16d	4505	6450	4010	5200

1. dg1 is the distance from the seat of the hanger to the highest joist nail.

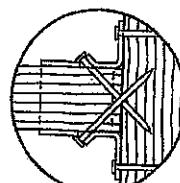


Double
Shear
Nailing
prevents tabs
breaking off
(available on
some models).

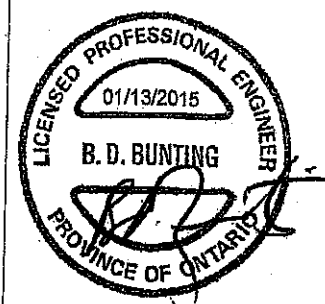
U.S. Patent
5,603,580



Double
Shear
Nailing
Side
View. Do
not bend
tab back.



Double
Shear
Nailing
Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

© 2015 Simpson Strong-Tie Company Inc.

1-SPEC-HUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com

TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

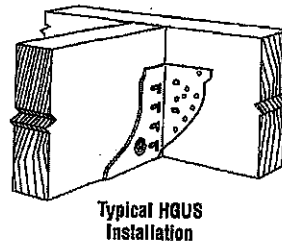
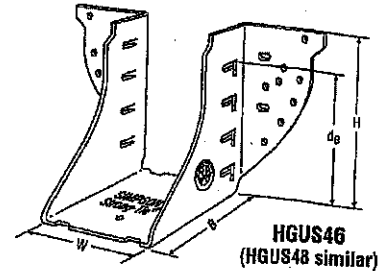
- Factored resistances are in accordance with CSA 086-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

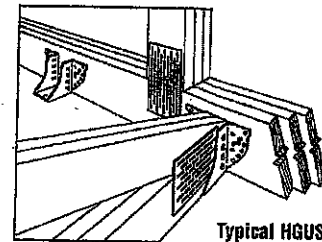
- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

OPTIONS:

- See current catalogue for options



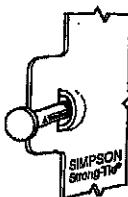
Typical HGUS Installation



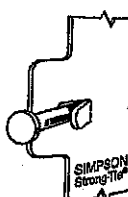
Typical HGUS Installation
(Truss Designer to provide fastener quantity for connecting multiple members together)

Model No.	Ga	Dimensions (In)					Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist		D-Fir-L		S-P-F	
									Uplift (K _u =1.15)	Normal (K _u =1.00)	Uplift (K _u =1.15)	Normal (K _u =1.00)
HGUS48	12	3 3/4	7 1/4	4	6 1/8	36-16d	12-16d		6070	12980	4310	9215
HGUS410	12	3 3/4	9	4	8 1/8	46-16d	16-16d		6840	14645	4855	10400
HGUS412	12	3 3/4	10 1/8	4	10 1/8	56-16d	20-16d		7840	14995	5425	10645
HGUS414	12	3 3/4	12 1/8	4	11 1/8	66-16d	22-16d		10130	16400	7195	11645
HGUS5.50/8	12	5 1/2	6 1/8	4	6 1/8	36-16d	12-16d		6070	12980	4310	9215
HGUS5.50/10	12	5 1/2	8 1/8	4	8 1/8	46-16d	16-16d		6840	14645	4855	10400
HGUS5.50/12	12	5 1/2	10 1/8	4	10 1/8	56-16d	20-16d		7840	14995	5425	10645
HGUS5.50/14	12	5 1/2	12 1/8	4	11 1/8	66-16d	22-16d		10130	16400	7195	11645
HGUS7.25/8	12	7 1/4	7 1/8	4	6 1/8	36-16d	12-16d		6070	12980	4310	9215
HGUS7.25/10	12	7 1/4	8 1/8	4	8 1/8	46-16d	16-16d		6840	15760	4855	11190
HGUS7.25/12	12	7 1/4	10 1/8	4	10 1/8	56-16d	20-16d		7640	16110	5425	11435
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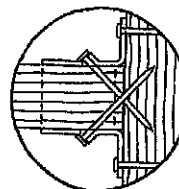
1. d_g is the distance from the seat of the hanger to the highest joist nail.



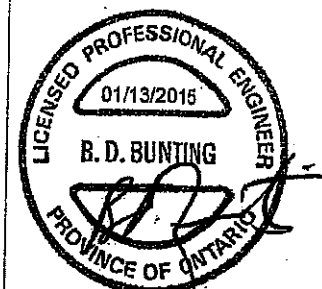
Double Shear Nailing prevents tabs breaking off (available on some models).
U.S. Patent 5,693,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



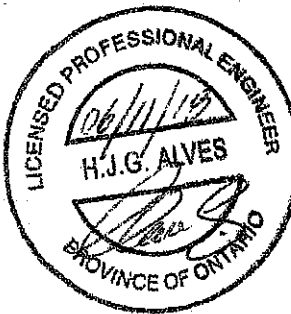
UNIT STATES DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016. Contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECDSHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

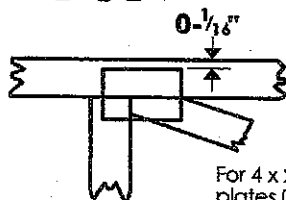
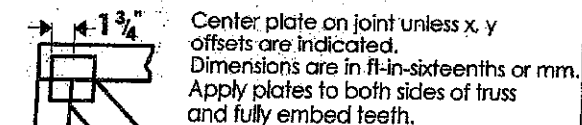
- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1800218

Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

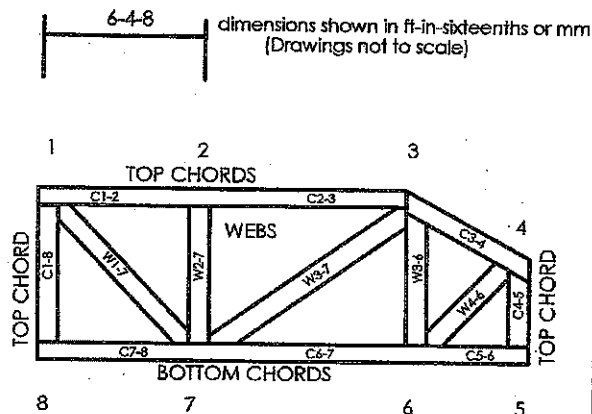


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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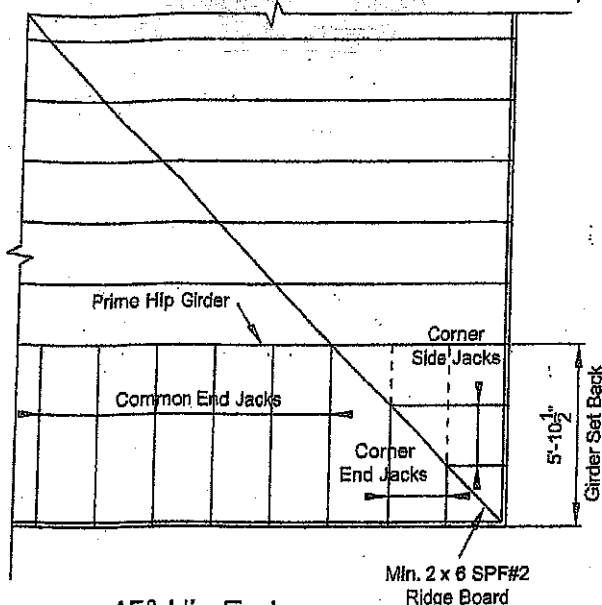
MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



45° Hip End

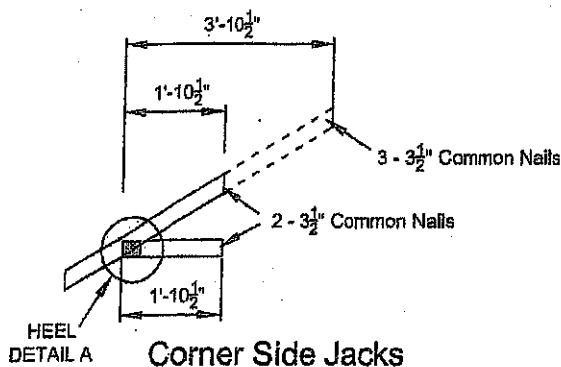
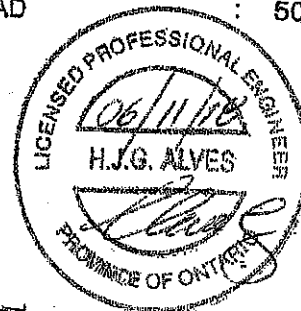
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

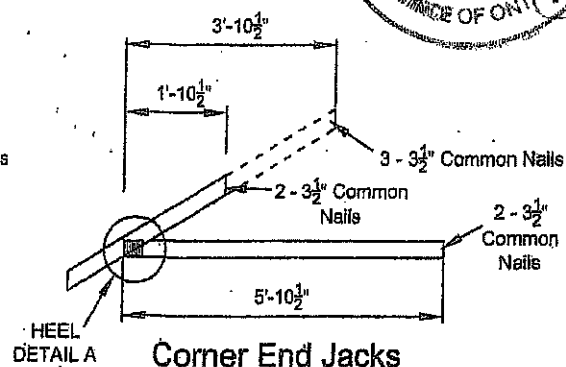
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

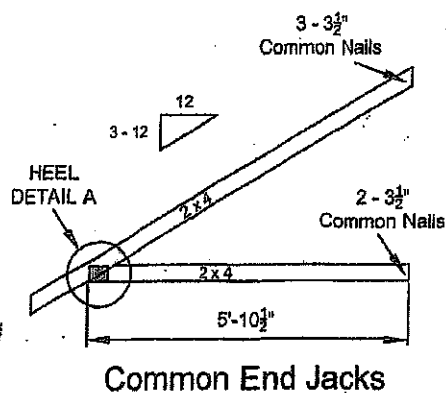
TOTAL LOAD : 50.5 P.S.F.



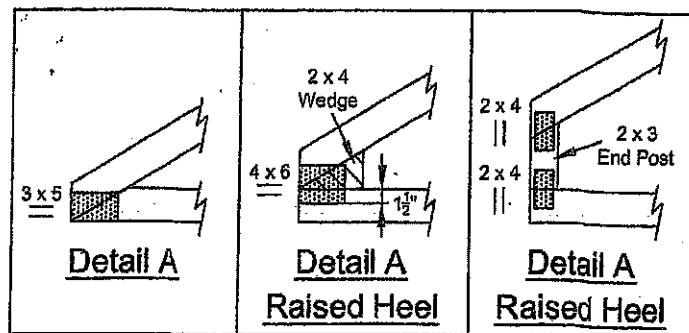
Corner Side Jacks



Corner End Jacks

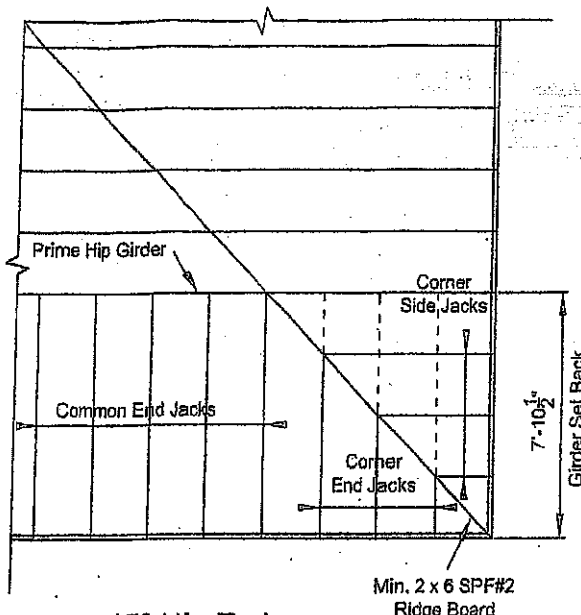


Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216



45° Hip End

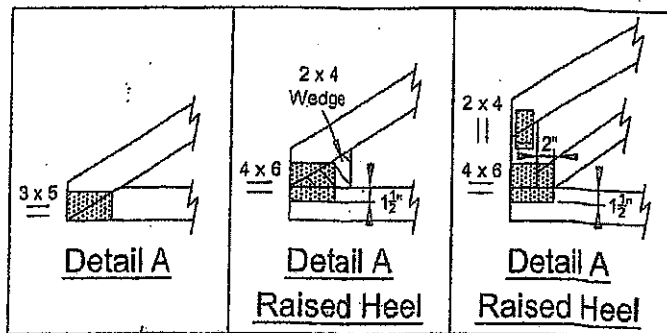
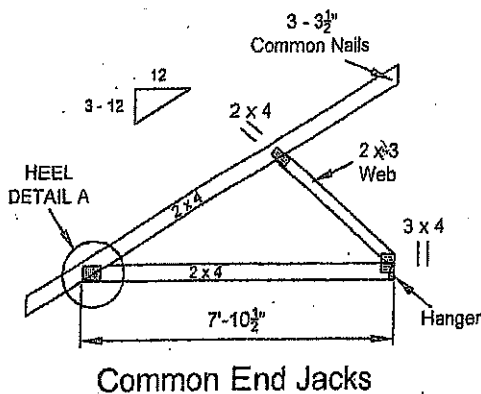
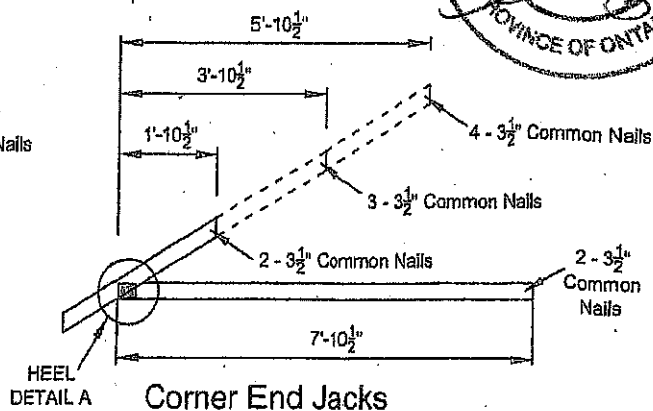
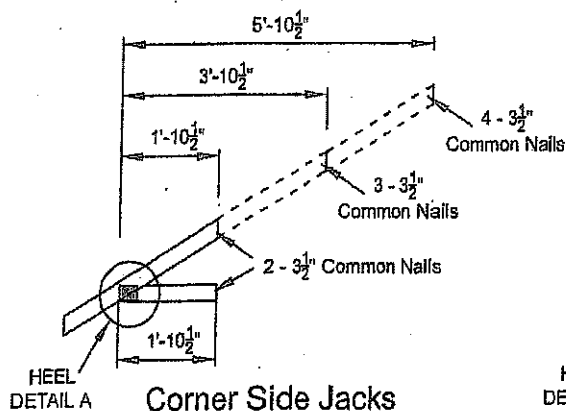
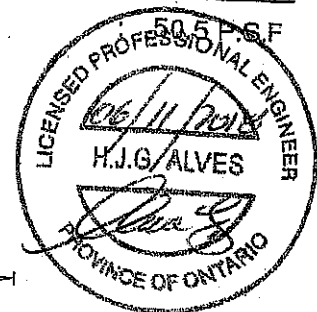
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

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