

	Job Track: 50033	Builder / Location:			Model / Elevation:		
	Plan Log: 200666	GREEN PARK HOMES / CALEDON			KINMOUNT 11 B -049 / 2 -LOT 49		
	Layout ID: 405804	Project: LAMBERT LANE PH.2		THESE DRAWINGS CONSTITUTE THE PROPERTY OF TAMARACK ROOF TRUSSES INC. SHALL NOT BE REPRODUCED, PUBLISHED, OR REDISTRIBUTED IN ANY MANNER OR UTILIZED FOR ANY PURPOSE OTHER THAN THE MANUFACTURE OF TRUSSES BY TAMARACK ROOF TRUSSES INC AND WILL BE RETRACTED BY TAMARACK ROOF TRUSSES INC IF UTILIZED FOR ANY OTHER PURPOSE.			
	Date: 9/25/2019	Sales: Mario DiCano	Designer: Chris/	Mitek ver 8.3.0.234			



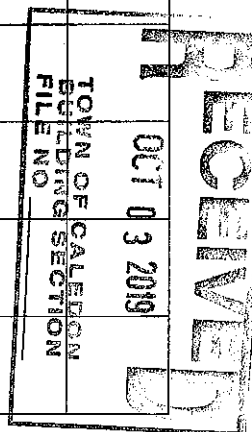
DELIVERY SHIPLIST

Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: KINMOUNT 11B
 Lot #:
 Elevation: 2 -LOT 49

Job Track: 50033
 Plan Log: 200666
 Layout ID: 405804
 Ref #
 Page: 1 of 3
 Date: 09/25/2019
 Designer:
 Sales Rep: Mario DiCano

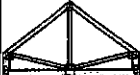
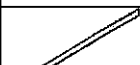
Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1 2-ply	T12 Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	1 2-ply	T12Z Hip Girder	9/12	32-05-00	4-08-13	2 x 6	1-03-08 1-03-08	1-06-04 1-06-04	382.78 224.67		
	2	T2 Hip	9/12	32-05-00	5-10-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	284.98 186.00		
	2	T3 Hip	9/12	32-05-00	7-00-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	286.62 182.33		
	2	T4 Hip	9/12	32-05-00	8-02-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	303.91 191.33		
	3	T5 Hip	9/12	32-05-00	9-04-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	478.29 299.50		
	4	T5A Hip	9/12	32-00-00	9-04-09	2 x 4	1-03-08	1-06-04 1-06-04	626.45 392.00		
	1	T6 Hip	9/12	32-05-00	10-06-09	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	167.74 104.00		
	1	T6A Hip	9/12	32-00-00	10-06-09	2 x 4	1-03-08	1-06-04 1-06-04	164.91 103.33		
	3	T8S Scissor	9/12 4/12	12-06-00	6-02-08	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	170.08 113.00		
	4	T10 Common	9/12	23-07-00	9-11-14	2 x 4	1-03-08	9-04 1-06-04	436.07 272.67		
	1	G10 GABLE	9/12	23-07-00	9-11-14	2 x 4	1-03-08	9-04 1-06-04	112.07 70.33		
	2	T14 Common	9/12	21-05-00	9-06-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	194.58 125.33		
	1	G14 GABLE	9/12	21-05-00	9-06-10	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	107.06 68.17		



 TAMARACK ROOF TRUSSES INC. ALPA LUMBER GROUP	DELIVERY SHIPLIST	
	Lumber Yard: TAMARACK LUMBER Builder: GREEN PARK HOMES Project: LAMBERT LANE PH.2 Location: CALEDON Model: KINMOUNT 11B Lot #: Elevation: 2 -LOT 49	Job Track: 50033 PlanLog: 200666 Layout ID: 405804 Ref # Page: 2 of 3 Date: 09/25/2019 Designer: Sales Rep: Mario DiCano

Roof Trusses

PROFILE	QTY PLY	MARK TYPE	PITCH	SPAN	HEIGHT	LUMBER	OVERHANG LEFT RIGHT	HEEL HEIGHT LEFT RIGHT	LBS. BFT.	BUNDLE # STACK #	LOAD BY REMARKS
	1	T17S Scissor	9 /12 6 /12	9-00-00	4-10-12	2 x 4	1-03-08 1-03-08	1-06-04 1-06-04	39.87 26.50		
	1 3-ply	T18 Common Girder	9 /12	9-00-00	4-10-12	2 x 4 2 x 6		1-06-04 1-06-04	129.41 88.50		
	1	V10 Valley	9 /12	23-03-09	8-08-13	2 x 4			93.83 60.50		
	1	V11 Valley	9 /12	19-03-09	7-02-13	2 x 4			68.17 42.83		
	1	V12 Valley	9 /12	15-03-09	5-08-13	2 x 4			52.77 35.00		
	1	V13 Valley	9 /12	11-03-09	4-02-13	2 x 4			30.66 19.33		
	1	V14 Valley	9 /12	7-03-09	2-08-13	2 x 4			19.18 13.50		
	2	PB1 Piggyback	9 /12	11-05-09	2-04-00	2 x 4			66.31 43.33		
	2	PB2 Piggyback	9 /12	11-05-09	3-06-00	2 x 4			68.93 44.67		
	1	PB3 Piggyback	9 /12	11-05-09	4-03-09	2 x 4			30.72 20.00		
	9	J7 Jack-Open	7 /12	5-10-08	4-08-09	2 x 4	1-03-08	1-03-07 4-08-09	167.53 105.00		
	5	J8 Jack-Open	8 /12	4-01-08	3-11-09	2 x 4	1-03-08	4-07 3-01-07	64.24 43.33		

• TOTAL # TRUSS= 58 TOTAL BFT OF ALL TRUSSES= 3099.82 BFT. TOTAL WEIGHT OF ALL TRSSES 4929.93 LBS

HARDWARE

QTY	TYPE	MODEL	LENGTH
10	Hardware	LJS26DS	
2	Hardware	HGUS26-2	

DELIVERY SHIPLIST



Lumber Yard: TAMARACK LUMBER
 Builder: GREEN PARK HOMES
 Project: LAMBERT LANE PH.2
 Location: CALEDON
 Model: KINMOUNT 11B
 Lot #:
 Elevation: 2 -LOT 49

Job Track: 50033
 PlanLog: 200666
 Layout ID: 405804
 Ref #
 Page: 3 of 3
 Date: 09/25/2019
 Designer:
 Sales Rep: Mario DiCano

HARDWARE

QTY	TYPE	MODEL	LENGTH
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TOTAL NUMBER OF
 ITFMS= 12

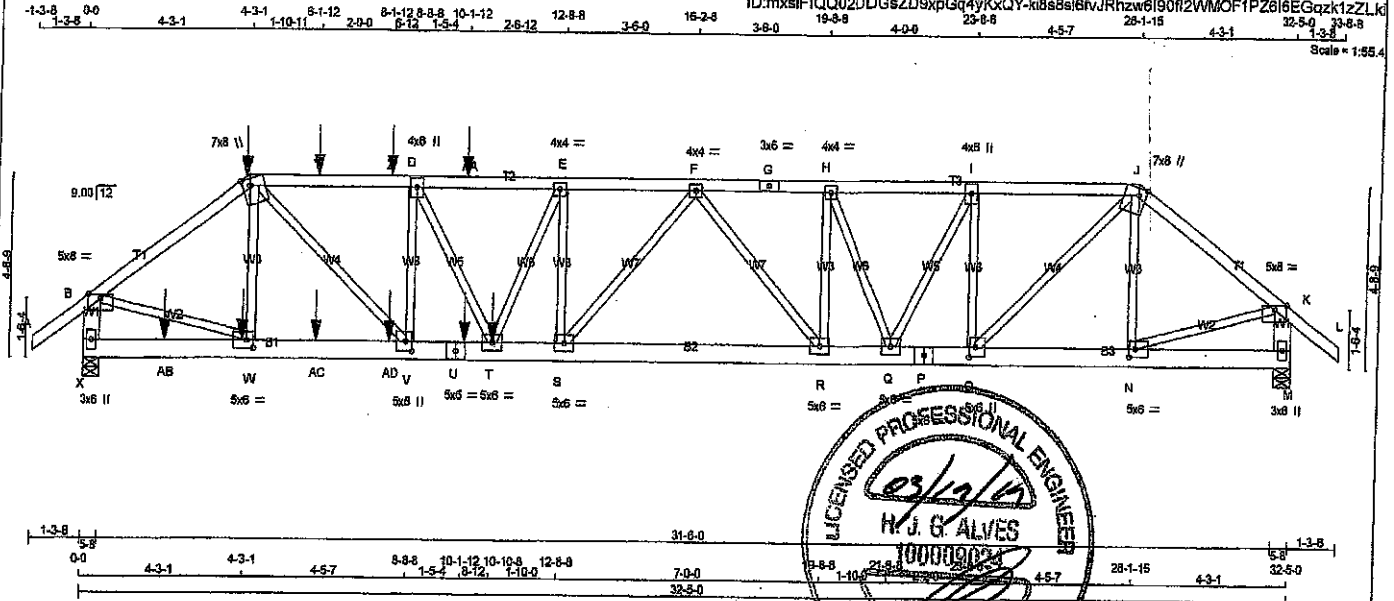
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T1Z2	1	2	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue Mar 19 15:02:41 2019 Page 1

ID:mxsIFIQ02DDGsZD9xpGq4yKxQY-kBs8sI6NjRhzw6I90R2WMOF1P28I6EGqzk1zZLk

Scale = 1:55.4



TOTAL WEIGHT = 2 X 163 = 327 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF		
C - G	2x4	DRY	No.2	SPF		
G - J	2x4	DRY	No.2	SPF		
J - L	2x4	DRY	No.2	SPF		
X - B	2x6	DRY	No.2	SPF		
M - K	2x6	DRY	No.2	SPF		
X - U	2x6	DRY	No.2	SPF		
U - P	2x6	DRY	No.2	SPF		
P - M	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A - C	12	SIDE (61.0)
C - G	12	SIDE (61.0)
G - J	12	TOP
J - L	12	TOP
X - B	12	TOP
M - K	12	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
X - U	12	SIDE (183.1)
U - P	12	SIDE (0.0)
P - M	12	TOP
WEBS : (0.122"x3") SPIRAL NAILS		
W - C	6	SIDE (24.1)
2x3	6	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERRING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMWV-p	MT20	5.0	8.0	Edge
C	TTWV-m	MT20	7.0	8.0	Edge 2.50
D	TMWV-h	MT20	4.0	6.0	
E, F, H					
G	TMWV-h	MT20	4.0	4.0	
H	TS-t	MT20	3.0	6.0	
I	TMWV-h	MT20	4.0	6.0	
J	TTWV-m	MT20	7.0	8.0	Edge 2.50
K	TMWV-p	MT20	5.0	8.0	Edge
M	BMV1-p	MT20	3.0	8.0	
N	BMWV-h	MT20	5.0	6.0	2.50 2.00
O	BMWV-h	MT20	5.0	6.0	3.00 2.00
P	BS-4	MT20	5.0	6.0	
Q, R, S, T					
Q	BMWV-h	MT20	5.0	6.0	
U	BS-1	MT20	5.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
X	5016	0	0	5-8
M	3401	0	0	5-8

UNFACTORED REACTIONS

1ST LCASE	MAX./MIN. COMPONENT REACTIONS						
JT	COMBINED	SNOW	LIVE	PERM. WIND	DEAD	SOIL	
X	3718	2147 / 0	674 / 0	0 / 0	888 / 0	0 / 0	
M	2524	1450 / 0	464 / 0	0 / 0	610 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) X, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MEMB.	MAX. FACTORED	FORCE (LBS)	VERT. LOAD LC1	MAX. FACTORED	CS1 (LC)	UNBRAC	MEMB.	MAX. FACTORED	FORCE (LBS)	MAX. FACTORED	CS1 (LC)
FR-TO								FR-TO				
A-B	0 / 42	-102.1	-102.1	0.08 (1)	10.00	W-C	-781 / 0	0.12 (1)				
B-C	-5663 / 0	-102.1	-102.1	0.34 (1)	3.83	C-V	0 / 4444	0.55 (1)				
C-Y	-7892 / 0	-102.1	-102.1	0.57 (1)	3.08	V-D	-2613 / 0	0.42 (1)				
Y-Z	-7892 / 0	-102.1	-102.1	0.57 (1)	3.08	D-T	0 / 1884	0.23 (1)				
Z-D	-7892 / 0	-102.1	-102.1	0.57 (1)	3.08	T-E	-90 / 0	0.02 (3)				
D-AA	-8534 / 0	-102.1	-102.1	0.58 (1)	2.89	S-F	-325 / 27	0.05 (1)				
AA-E	-8534 / 0	-102.1	-102.1	0.59 (1)	2.89	S-F	0 / 1052	0.13 (1)				
E-F	-8560 / 0	-102.1	-102.1	0.35 (1)	3.16	F-R	-1510 / 0	0.40 (1)				
F-G	-8951 / 0	-102.1	-102.1	0.28 (1)	3.57	R-H	0 / 1378	0.17 (1)				
G-H	-6951 / 0	-102.1	-102.1	0.28 (1)	3.57	H-Q	-1891 / 0	0.35 (1)				
H-I	-6215 / 0	-102.1	-102.1	0.30 (1)	3.71	Q-I	0 / 1918	0.24 (1)				
I-J	-5357 / 0	-102.1	-102.1	0.27 (1)	4.00	O-I	-2185 / 0	0.35 (1)				
J-K	-3668 / 0	-102.1	-102.1	0.25 (1)	4.68	O-J	0 / 3371	0.42 (1)				
K-L	0 / 42	-102.1	-102.1	0.08 (1)	10.00	N-J	-549 / 0	0.09 (1)				
X-B	-4919 / 0	0.0	0.0	0.18 (1)	6.55	B-W	0 / 4685	0.58 (1)				
M-K	-3335 / 0	0.0	0.0	0.12 (1)	7.81	N-K	0 / 3045	0.38 (1)				
X-AB	0 / 0	-38.5	-38.5	0.08 (3)	10.00							
AB-W	0 / 0	-38.5	-38.5	0.08 (3)	10.00							
W-AC	0 / 4502	-38.5	-38.5	0.33 (1)	10.00							
AC-AD	0 / 4502	-38.5	-38.5	0.33 (1)	10.00							
AD-V	0 / 4502	-38.5	-38.5	0.33 (1)	10.00							
V-U	0 / 7692	-38.5	-38.5	0.67 (1)	10.00							
U-T	0 / 7692	-38.5	-38.5	0.67 (1)	10.00							
T-S	0 / 8560	-38.5	-38.5	0.73 (1)	10.00							
S-R	0 / 7900	-38.5	-38.5	0.59 (1)	10.00							
R-Q	0 / 6851	-38.5	-38.5	0.49 (1)	10.00							
Q-P	0 / 5358	-38.5	-38.5	0.39 (1)	10.00							
P-O	0 / 5358	-38.5	-38.5	0.39 (1)	10.00							
O-N	0 / 2937	-38.5	-38.5	0.22 (1)	10.00							
N-M	0 / 0	-38.5	-38.5	0.04 (3)	10.00							

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX.	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
C	4-3-1	-53	-59		FRONT	VERT	DEAD		
C	4-3-1	-205	-205		FRONT	VERT	TOTAL		
C	4-3-1	-303	-303		FRONT	VERT	SNOW		
T	10-10-6	-2051	-2051		FRONT	VERT	TOTAL		
U	10-1-12	-75	-95		FRONT	VERT	TOTAL		
W	4-1-12	-75	-95		FRONT	VERT	TOTAL		

DESIGN CRITERIA

*** SPECIAL LOADS ANALYSIS ***
GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
LOADS WERE DERIVED FROM USER INPUT
NO FURTHER MODIFICATIONS WERE MADE

SPECIAL LOADS:

TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

*** NON STANDARD GIRDER ***

ADDTL. USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF CBC 2018
- CSA 088-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.I. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = L/999 (0.20")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = L/999 (0.34")

CSI: TC=0.59/1.00 (D-E:1), BC=0.73/1.00 (S-T:1),
WB=0.58/1.00 (B-W:1), SS=0.24/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

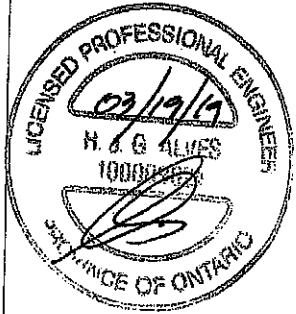
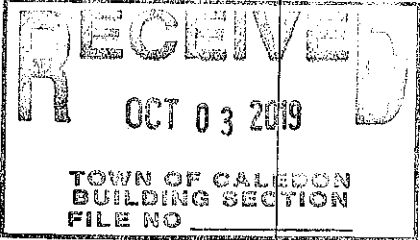
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.89 (V) (INPUT = 0.90)
JSI METAL = 0.79 (P) (INPUT = 1.00)

CONTINUED ON PAGE 2

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T1Z2	1	2	TRUSS DESC.		
Tamarack Roof Truss, Burlington				Version 8.290 8 Nov 17 2018 Mitak Industries, Inc. Tue Mar 19 15:02:41 2019 Page 2 ID:mxsIF1QQ020DGsZD9xpGq4yKxQY-ki8s8s19fvJRhzW6190f12VVMOF1PZ616EGqzk1zZ1k		

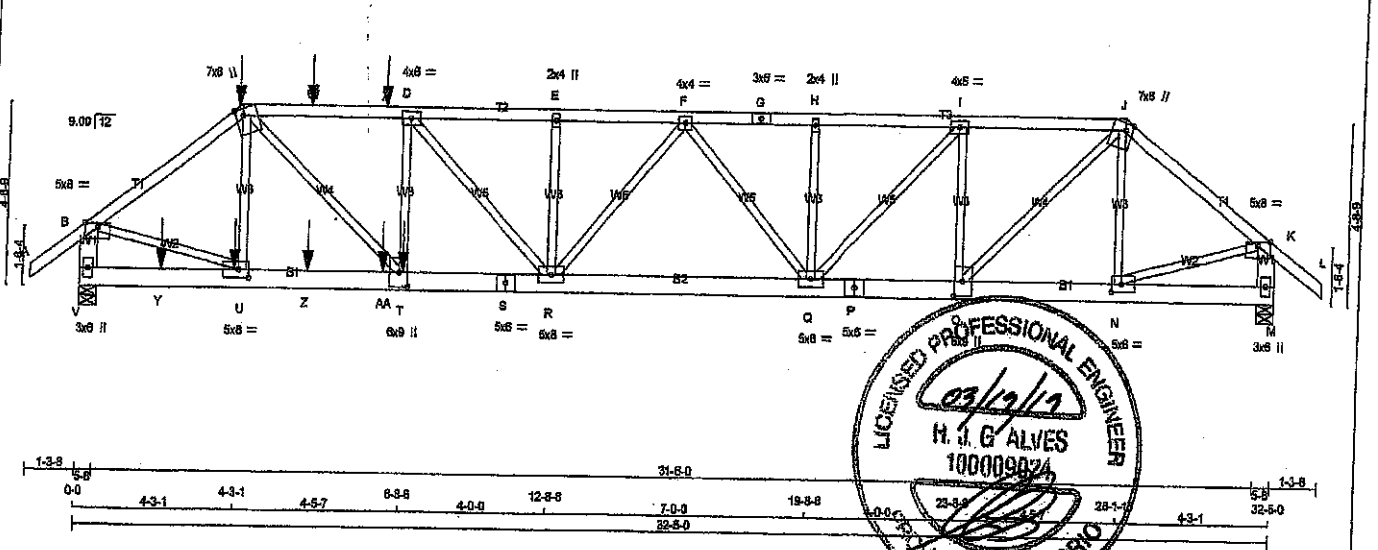
PLATES (table is in inches)							FACTORED CONCENTRATED LOADS (LBS)									
JT	TYPE	PLATES	W	LEN	Y	X	JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
V	BMVWH	MT20	5.0	6.0	3.00	2.00	Y	8-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
W	BMVWH	MT20	5.0	6.0	2.50	2.00	Z	8-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
X	BMV1+p	MT20	3.0	6.0			AA	10-1-12	-198	-198	---	FRONT	VERT	TOTAL	---	---
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.							AB	2-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---
							AC	8-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---
							AD	8-1-12	-75	-95	---	FRONT	VERT	TOTAL	---	---



DWG NO. TAM 11905717
STRUCTURAL
CADD - MEANT ONLY 2/2

JOB NAME 401811	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO. T1Z
Tamarack Roof Truss, Burlington				Version 8.230 6 Nov 17 2018 M&T Industries, Inc. Tue Mar 19 14:03:22 2019 Page 1	

ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-R7bPAvggpPvQC0L5JCf3ZYc8bMZBTfY3C8KqZMcJ
 1-3-8 0-0 4-3-1 4-3-1 1-10-11 8-1-12 8-1-12 8-8-8 4-0-0 12-8-8 3-8-0 15-2-8 19-8-8 4-0-0 23-8-8 4-5-7 28-1-15 4-3-1 32-5-0 1-3-8
 Scale = 1:55.4



LUMBER			
N.L.G.A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY No.2	SPF
C - G	2x4	DRY No.2	SPF
G - J	2x4	DRY No.2	SPF
V - L	2x4	DRY No.2	SPF
V - B	2x6	DRY No.2	SPF
M - K	2x6	DRY No.2	SPF
V - S	2x6	DRY No.2	SPF
S - P	2x6	DRY No.2	SPF
P - M	2x6	DRY No.2	SPF
ALL WEBS	2x3	DRY No.2	SPF
EXCEPT			

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 2 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122'X3') SPIRAL NAILS		
A-C	12	SIDE(61.0)
C-G	12	SIDE(61.0)
G-J	12	TOP
J-L	12	TOP
V-B	12	TOP
M-K	12	TOP
BOTTOM CHORDS : (0.122'X3') SPIRAL NAILS		
V-S	12	SIDE(183.1)
S-P	12	TOP
P-M	12	TOP
WEBS : (0.122'X3') SPIRAL NAILS		
2x3	8	SIDE(631.1)
D-T	3	
L-O	3	

NAILS TO BE DRIVEN FROM ONE SIDE ONLY.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLIES FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)	PLATES	W	LEN	Y	X
JT	TMVW-p	MT20	5.0	8.0	Edge
B	TTWW-m	MT20	7.0	8.0	Edge 2.50
C	TMVW-t	MT20	4.0	6.0	
D	TMVW-w	MT20	2.0	4.0	
E	TMVW-t	MT20	4.0	4.0	
F	TS-t	MT20	3.0	6.0	
G	TMVW-w	MT20	2.0	4.0	
H	TMVW-t	MT20	4.0	6.0	
I	TTWW-m	MT20	7.0	8.0	Edge 2.50
J	TMVW-p	MT20	5.0	8.0	Edge
K	BMVW-t	MT20	3.0	6.0	
M	BMVW-t	MT20	5.0	8.0	2.50 3.25
O	BMVW-t	MT20	6.0	9.0	4.50 2.75
P	BS-t	MT20	5.0	6.0	
Q	BMVW-t	MT20	5.0	8.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS			
FACTORED	GROSS REACTION	MAXIMUM FACTORED	INPUT
VERT	HORZ	DOWN	HORZ
JT	5748	0	5-8
V	5748	0	5-8
M	3487	0	5-8

UNFACTORED REACTIONS			
1ST CASE	MAX. MIN. COMPONENT REACTIONS		
COMBINED	SNOW	LIVE	PERM. LIVE
JT	4268	2442 / 0	790 / 0
V	4268	2442 / 0	790 / 0
M	2576	1459 / 0	481 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) V, M

BRACING
 TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 2.89 FT.
 MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING
 TOTAL LOAD CASES: (4)

CHORDS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO			
A-B	0 / 42	-102.1	-102.1
B-C	-6591 / 0	-102.1	-102.1
C-W	-8079 / 0	-102.1	-102.1
W-X	-8079 / 0	-102.1	-102.1
X-D	-8079 / 0	-102.1	-102.1
D-E	-8837 / 0	-102.1	-102.1
E-F	-8837 / 0	-102.1	-102.1
F-G	-7142 / 0	-102.1	-102.1
G-H	-7142 / 0	-102.1	-102.1
H-I	-7142 / 0	-102.1	-102.1
I-J	-5484 / 0	-102.1	-102.1
J-K	-3779 / 0	-102.1	-102.1
K-L	0 / 42	-102.1	-102.1
V-B	-5668 / 0	0.0	0.0
M-K	-3402 / 0	0.0	0.0
WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED VERT. LOAD (LBS)
FR-TO			
A-B	0 / 42	-102.1	-102.1
B-C	-6591 / 0	-102.1	-102.1
C-W	-8079 / 0	-102.1	-102.1
W-X	-8079 / 0	-102.1	-102.1
X-D	-8079 / 0	-102.1	-102.1
D-E	-8837 / 0	-102.1	-102.1
E-F	-8837 / 0	-102.1	-102.1
F-G	-7142 / 0	-102.1	-102.1
G-H	-7142 / 0	-102.1	-102.1
H-I	-7142 / 0	-102.1	-102.1
I-J	-5484 / 0	-102.1	-102.1
J-K	-3779 / 0	-102.1	-102.1
K-L	0 / 42	-102.1	-102.1
V-B	-5668 / 0	0.0	0.0
M-K	-3402 / 0	0.0	0.0

DWG NO. TAM 19056 91
 STRUCTURAL
 COMPONENTS FAMILY 1/2

FACTORED CONCENTRATED LOADS (LBS)			
LOC.	LC1	MAX.	FACE
JT	4-3-1	-53	FRONT
C	4-3-1	-205	BACK
C	4-3-1	-303	FRONT
T	8-8-8	-3121	BACK
U	4-1-12	-75	BACK
W	6-1-12	-198	BACK
X	8-1-12	-198	BACK
Y	2-1-12	-75	BACK
Z	6-1-12	-75	BACK

SPECIAL LOADS ANALYSIS
 GEOMETRY AND/OR BASIC LOADS CHANGED BY USER.
 LOADS WERE DERIVED FROM USER INPUT
 NO FURTHER MODIFICATIONS WERE MADE

SPECIFIED LOADS:
 TOP CH. LL = 28.0 PSF
 DL = 6.0 PSF
 BOT CH. LL = 10.5 PSF
 DL = 7.0 PSF
 TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

NON STANDARD GIRDER
 ADDTL USER-DEFINED LOADS APPLIED TO ALL LOAD CASES.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
 - PART 9 OF CBC 2015
 - CSA 086-14
 - TRC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(LL) = L/999 (0.21")
 ALLOWABLE DEFL.(TL) = L/360 (1.08")
 CALCULATED VERT. DEFL.(TL) = L/999 (0.35")

CSI: TC=0.71/1.00 (C-D:1), BC=0.68/1.00 (R-T:1), WB=0.67/1.00 (B-U:1), SSI=0.23/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.00
 COMP=1.00 SHEAR=1.00 TENS=1.00

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES						
PLATE	GRIP(DRY)		SHEAR		SECTION	
	(PSI)		(PL)		(PL)	
	MAX	MIN	MAX	MIN	MAX	MIN
MT20	650	371	1747	788	1987	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP = 0.90 (B) (INPUT = 0.90)
 JSI METAL = 0.81 (S) (INPUT = 1.00)

JOB NAME 401811	TRUSS NAME T1Z	QUANTITY 1	PLY 2	JOB DESC. GREEN PARK HOMES	DRWG NO. T1Z
Tamarack Roof Truss, Burlington				Version 8.230 5 Nov 17 2018 Mittek Industries, Inc. Tue Mar 19 14:03:22 2019 Page 2 ID:mxsiFIQQ02DDGsZD8xpGq4yKxQY-R7bPAVgggPvQC0L5JC3Zxc6bMZBTFr3C8KqzZMcJ	

PLATES (table is in inches)

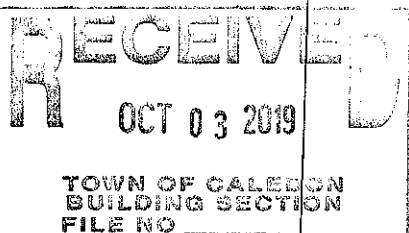
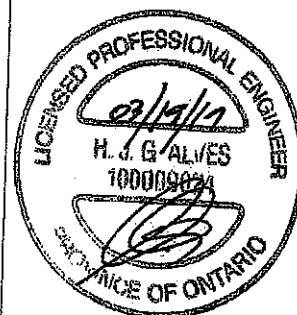
JT	TYPE	PLATES	W	LEN	Y	X
R	BMWWW-I	MT20	6.0	8.0		
S	BS-I	MT20	5.0	6.0		
T	BMWWW-I	MT20	6.0	9.0	4.50	2.75
U	BMWWW-I	MT20	5.0	8.0	2.50	3.25
V	BMV1-p	MT20	3.0	8.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

FACTORED CONCENTRATED LOADS (LB8)

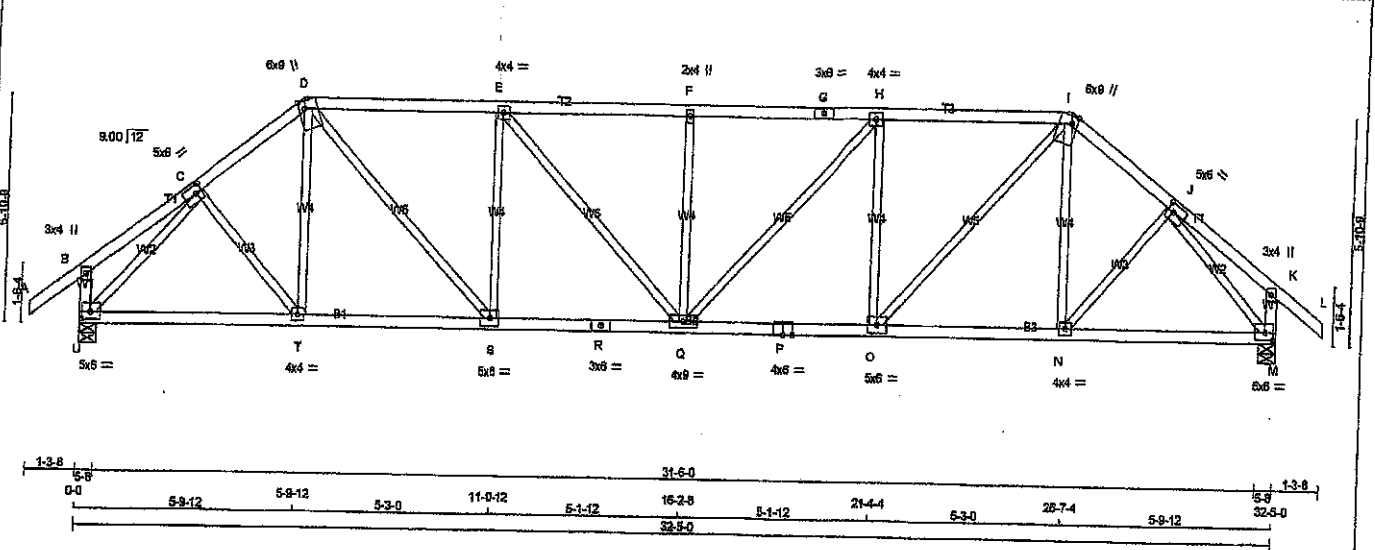
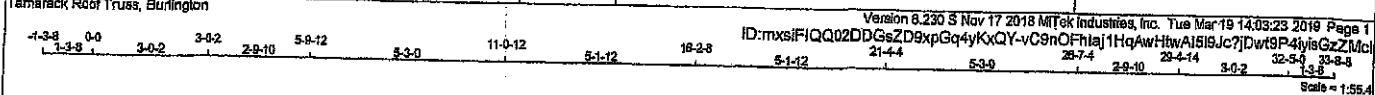
JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
AA	8-1-12	-75	-95	---	BACK	VERT	TOTAL	---	---

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



DWG NO. TAM 71905691
STRUCTURAL
COMPONENT ONLY 7/2

JOB NAME 401811	TRUSS NAME T2	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T2
Tamarack Roof Truss, Burlington				Version 8.230 S Nov 17 2018 MTek Industries, Inc. Tue Mar 19 14:03:23 2019 Page 1	



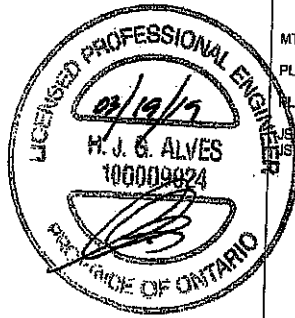
LUMBER N.L.G.A. RULES **CHORDS** SIZE **LUMBER** DESCR. **SPF** **BEARINGS** **FACTORED** **GROSS REACTION** **MAXIMUM FACTORED** **INPUT** **REQD** **BRG** **BRG** **IN-SX** **IN-SX** **U** **2420** **0** **2420** **0** **0** **5-8** **5-8** **M** **2420** **0** **2420** **0** **0** **5-8** **5-8** **UNFACTORED REACTIONS** **1ST LCASE** **MAX/MIN COMPONENT REACTIONS** **JT** **COMBINED** **SNOW** **LIVE** **PERM.LIVE** **WIND** **DEAD** **SOIL** **U** **1800** **1021 / 0** **340 / 0** **0 / 0** **0 / 0** **438 / 0** **0 / 0** **M** **1800** **1021 / 0** **340 / 0** **0 / 0** **0 / 0** **438 / 0** **0 / 0** **BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) U, M** **BRACING** **TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.25 FT.** **MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.** **ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.** **LOADING** **TOTAL LOAD CASES: (4)** **CHORDS** **MAX. FACTORED** **MEMB.** **FORCE** **VERT. LOAD** **LC1** **MAX** **MAX** **UNBRAC** **MEMB.** **MAX. FACTORED** **FORCE** **MAX** **CS1 (LC)** **CS1 (LC)** **FR-TO** **A-B** **0 / 42** **-102.1** **-102.1** **0.14 (1)** **10.00** **C-T** **0 / 301** **0.07 (1)** **B-C** **0 / 15** **-102.1** **-102.1** **0.10 (1)** **10.00** **D-D** **-2492 / 0** **-102.1** **-102.1** **0.18 (1)** **4.22** **D-S** **0 / 1542** **0.35 (1)** **D-E** **-3033 / 0** **-102.1** **-102.1** **0.60 (1)** **3.44** **S-E** **-938 / 0** **0.50 (1)** **E-F** **-3349 / 0** **-102.1** **-102.1** **0.84 (1)** **3.25** **E-Q** **0 / 486** **0.10 (1)** **F-G** **-3349 / 0** **-102.1** **-102.1** **0.84 (1)** **3.25** **Q-F** **-484 / 0** **0.28 (1)** **G-H** **-3349 / 0** **-102.1** **-102.1** **0.84 (1)** **3.25** **Q-H** **0 / 486** **0.10 (1)** **H-I** **-3033 / 0** **-102.1** **-102.1** **0.60 (1)** **3.44** **O-H** **-938 / 0** **0.50 (1)** **I-J** **-2492 / 0** **-102.1** **-102.1** **0.18 (1)** **4.22** **O-I** **0 / 1542** **0.35 (1)** **J-K** **0 / 15** **-102.1** **-102.1** **0.10 (1)** **10.00** **N-I** **0 / 153** **0.04 (3)** **K-L** **0 / 42** **-102.1** **-102.1** **0.14 (1)** **10.00** **N-J** **0 / 301** **0.07 (1)** **L-M** **-263 / 0** **0.0** **0.0** **0.03 (1)** **7.81** **U-C** **-2722 / 0** **0.85 (1)** **M-K** **-263 / 0** **0.0** **0.0** **0.03 (1)** **7.81** **J-M** **-2722 / 0** **0.85 (1)** **U-T** **0 / 1773** **-38.5** **-38.5** **0.46 (2)** **10.00** **T-S** **0 / 1976** **-38.5** **-38.5** **0.50 (2)** **10.00** **S-R** **0 / 3033** **-38.5** **-38.5** **0.59 (1)** **10.00** **R-Q** **0 / 3033** **-38.5** **-38.5** **0.59 (1)** **10.00** **Q-P** **0 / 3033** **-38.5** **-38.5** **0.59 (1)** **10.00** **P-O** **0 / 3033** **-38.5** **-38.5** **0.59 (1)** **10.00** **O-N** **0 / 1976** **-38.5** **-38.5** **0.50 (2)** **10.00** **N-M** **0 / 1773** **-38.5** **-38.5** **0.46 (2)** **10.00**

DESIGN CRITERIA **SPECIFIED LOADS:** **TOP CH.** **LL = 20.0 PSF** **DL = 6.0 PSF** **BOY CH.** **LL = 10.5 PSF** **DL = 7.0 PSF** **TOTAL LOAD = 52.5 PSF** **SPACING = 24.0 IN. C/C** **LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12** **THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015** **THIS DESIGN COMPLIES WITH:** **- PART 9 OF BCBC 2018** **- CSA 086-14** **- TPIC 2014** **(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD** **ALLOWABLE DEFL.(LL) = L/360 (1.08")** **CALCULATED VERT. DEFL.(LL) = 1/999 (0.17")** **ALLOWABLE DEFL.(TL) = L/360 (1.08")** **CALCULATED VERT. DEFL.(TL) = 1/999 (0.26")** **CS1: TC=0.64/1.00 (E-F:1), BC=0.59/1.00 (O-Q:1), WB=0.85/1.00 (G-L:1), SS=0.25/1.00 (H-I:1)** **DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10** **COMPANION LIVE LOAD FACTOR = 1.00** **TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.** **NAIL VALUES** **PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)** **MAX MIN MAX MIN MAX MIN** **MT20 650 371 1747 788 1987 1973** **PLATE PLACEMENT TOL = 0.250 inches** **PLATE ROTATION TOL = 5.0 Deg.** **GRIP= 0.80 (M) (INPUT = 0.90)** **METAL= 1.00 (R) (INPUT = 1.00)**

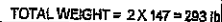
DRY: SEASONED LUMBER. **Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.**

JT	TYPE	PLATES	W	LEN	Y	X
B	TMW+p	MT20	3.0	4.0		
C	TMW+L	MT20	5.0	6.0	2.50	2.00
D	TTW+L	MT20	6.0	9.0	Edge	2.00
E	TMW+L	MT20	4.0	4.0		
F	TMW+L	MT20	2.0	4.0		
G	TS-L	MT20	3.0	6.0		
H	TMW+L	MT20	4.0	4.0		
I	TTW+L	MT20	6.0	9.0	Edge	2.00
J	TMW+L	MT20	5.0	6.0	2.50	2.00
K	TMW+p	MT20	3.0	4.0		
M	BMW+L	MT20	5.0	6.0	2.25	2.75
N	BMW+L	MT20	4.0	4.0		
O	BMW+L	MT20	5.0	6.0		
P	BS-L	MT20	4.0	6.0		
Q	BMW+L	MT20	4.0	6.0		
R	BS-L	MT20	3.0	6.0		
S	BMW+L	MT20	5.0	6.0		
T	BMW+L	MT20	4.0	4.0		
U	BMW+L	MT20	5.0	6.0	2.25	2.75

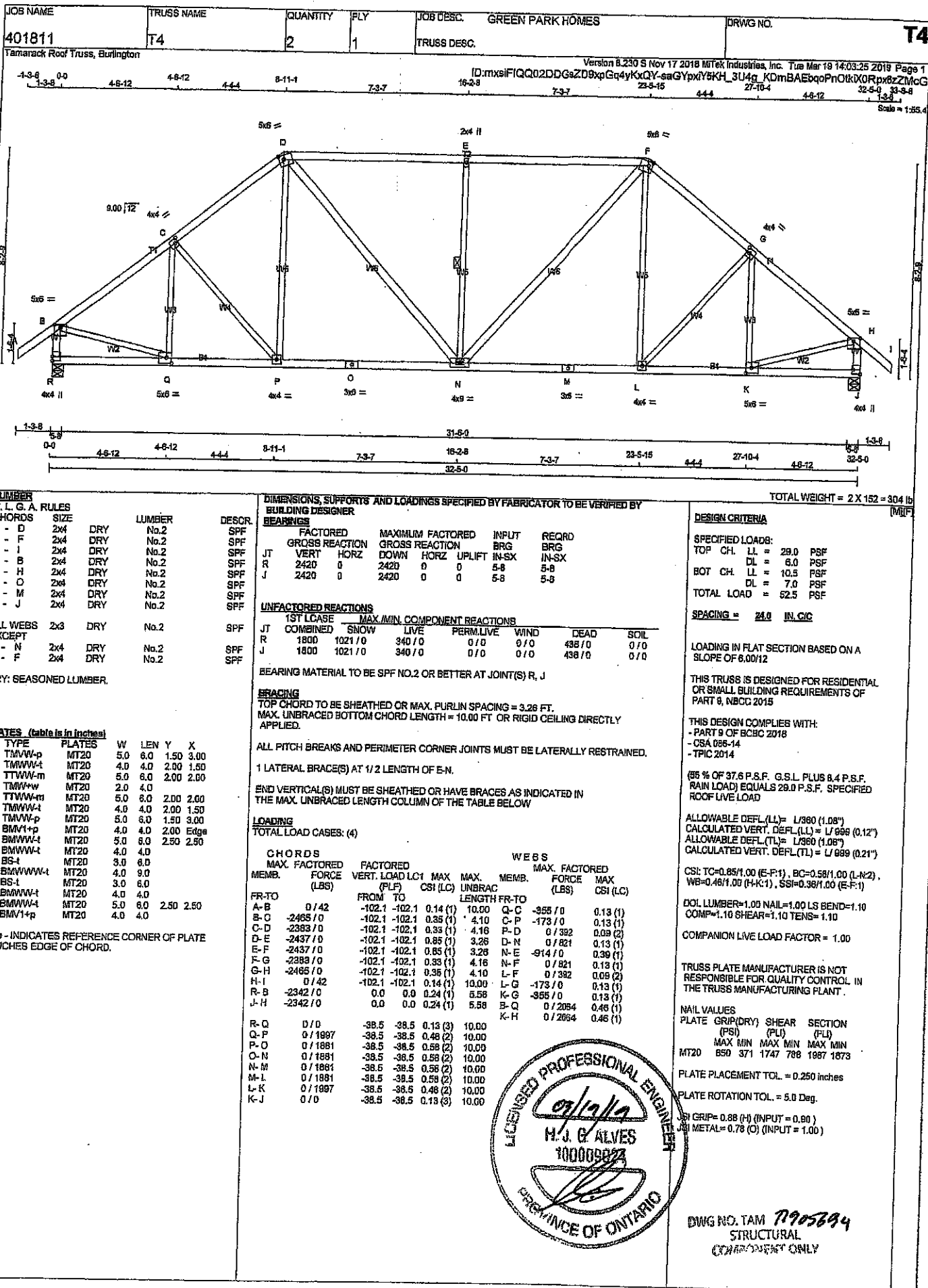
Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.



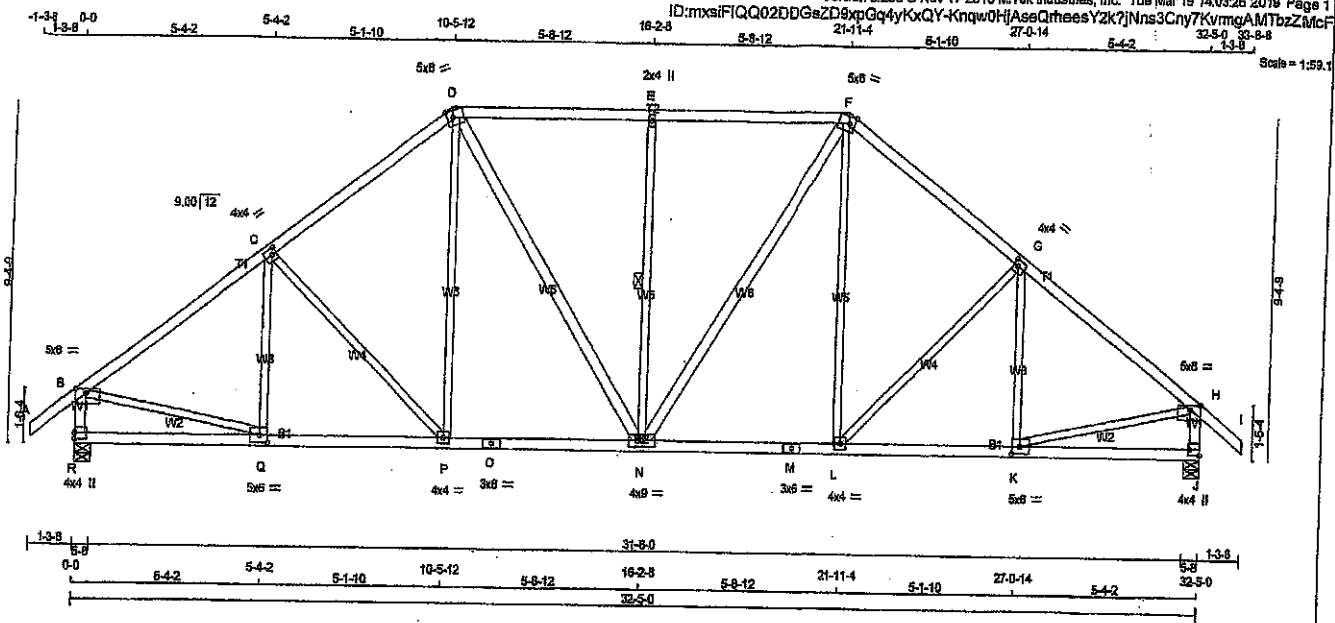
DRWG NO. TAM T190 5692
STRUCTURAL
COMPONENT ONLY

T3

DWG NO. TAM 11905693
STRUCTURAL
COMPONENT ONLY



JOB NAME 401811	TRUSS NAME T5	QUANTITY 7	FLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. T5
TRUSS DESC. Tamareck Roof Truss, Burlington			Version 8.230 S Nov 17 2018 MTEK Industries, Inc. Tue Mar 19 14:03:26 2018 Page 1		



LUMBER				
N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - F	2x4	DRY	No.2	SPF
F - I	2x4	DRY	No.2	SPF
R - B	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
R - O	2x4	DRY	No.2	SPF
O - M	2x4	DRY	No.2	SPF
M - J	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
D - N	2x4	DRY	No.2	SPF
N - F	2x4	DRY	No.2	SPF

PLATES (table is in inches)				
JT	TYPE	PLATES	W	LEN Y X
B	TMWw-p	MT20	5.0	8.0 1.50 3.50
C	TMWw-t	MT20	4.0	4.0 2.00 1.50
D	TTWw-m	MT20	5.0	8.0 Edge 2.00
E	TMWw-w	MT20	2.0	4.0
F	TTWw-m	MT20	5.0	8.0 Edge 2.00
G	TMWw-t	MT20	4.0	4.0 2.00 1.50
H	TMWw-p	MT20	5.0	8.0 1.50 3.50
J	BMV1-p	MT20	4.0	4.0 2.00 Edges
K	BMWw-t	MT20	5.0	6.0 2.50 2.75
L	BMWw-t	MT20	4.0	4.0
M	RS-t	MT20	3.0	6.0
N	BMWw-w	MT20	4.0	9.0
O	RS-t	MT20	3.0	6.0
P	BMWw-t	MT20	4.0	4.0
Q	BMWw-t	MT20	5.0	6.0 2.50 2.75
R	BMV1-p	MT20	4.0	4.0

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

FACTORED		MAXIMUM FACTORED		INPUT		REQD	
JT	VERT	HORZ	GROSS REACTION	DOWN	HORZ	BRG	IN-SX
R	2420	0	2420	0	0	5-8	5-8
J	2420	0	2420	0	0	5-8	5-8

UNFACTORED REACTIONS		1ST LCASE		MAX/MIN. COMPONENT REACTIONS	
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND
R	1830	1021/0	340/0	0/0	0/0
J	1800	1021/0	340/0	0/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) R, J

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.92 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

MAX. FACTORED				WEBS			
MEMB.	FORCE (LBS)	VERT. LOAD LC1 (PLF)	MAX CSI (LC)	MAX. UNBRACED LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)
FR-TO		FROM	TO		FR-TO		
A-B	0/42	-102.1	-102.1 0.14 (1)	10.00	Q-C	-233/82	0.11 (1)
B-C	-2509/0	-102.1	-102.1 0.48 (1)	3.92	C-P	-352/0	0.38 (1)
C-D	-2277/0	-102.1	-102.1 0.45 (1)	4.11	P-D	0/469	0.11 (2)
D-E	-2088/0	-102.1	-102.1 0.49 (1)	4.18	D-N	0/553	0.09 (1)
E-F	-2088/0	-102.1	-102.1 0.49 (1)	4.16	N-E	-713/0	0.41 (1)
F-G	-2277/0	-102.1	-102.1 0.45 (1)	4.11	N-F	0/553	0.09 (1)
G-H	-2509/0	-102.1	-102.1 0.48 (1)	3.92	L-F	0/469	0.11 (2)
H-I	0/42	-102.1	-102.1 0.14 (1)	10.00	L-G	-352/0	0.38 (1)
R-B	-2334/0	0.0	0.0 0.24 (1)	5.58	K-G	-233/82	0.11 (1)
J-H	-2334/0	0.0	0.0 0.24 (1)	5.58	B-Q	0/2087	0.47 (1)
R-Q	0/0	-38.5	-38.5 0.19 (3)	10.00	K-H	0/2087	0.47 (1)
Q-P	0/2037	-38.5	-38.5 0.46 (2)	10.00			
P-O	0/1782	-38.5	-38.5 0.43 (2)	10.00			
O-N	0/1782	-38.5	-38.5 0.43 (2)	10.00			
N-M	0/1782	-38.5	-38.5 0.43 (2)	10.00			
M-L	0/1782	-38.5	-38.5 0.43 (2)	10.00			
L-K	0/2037	-38.5	-38.5 0.46 (2)	10.00			
K-J	0/0	-38.5	-38.5 0.19 (3)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. OC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.08")
CALCULATED VERT. DEFL.(LL) = 1/988 (0.10")
ALLOWABLE DEFL.(TL) = L/360 (1.08")
CALCULATED VERT. DEFL.(TL) = 1/988 (0.16")

CS1: TC=0.48/1.00 (D-E-1), BC=0.48/1.00 (K-L-2), WB=0.47/1.00 (H-K-1), SS=0.28/1.00 (D-E-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

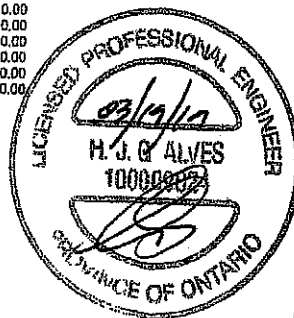
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(PRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 850 371 1747 788 1987 1873

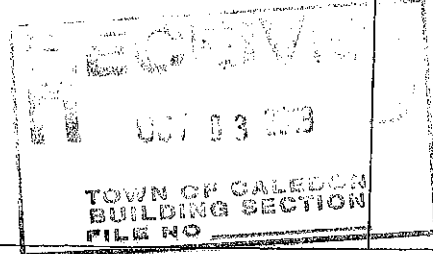
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP=0.88 (C) (INPUT = 0.90)
JSI METAL=0.55 (C) (INPUT = 1.00)



DRWG NO. TAM 17405695
STRUCTURAL
COMPONENT ONLY

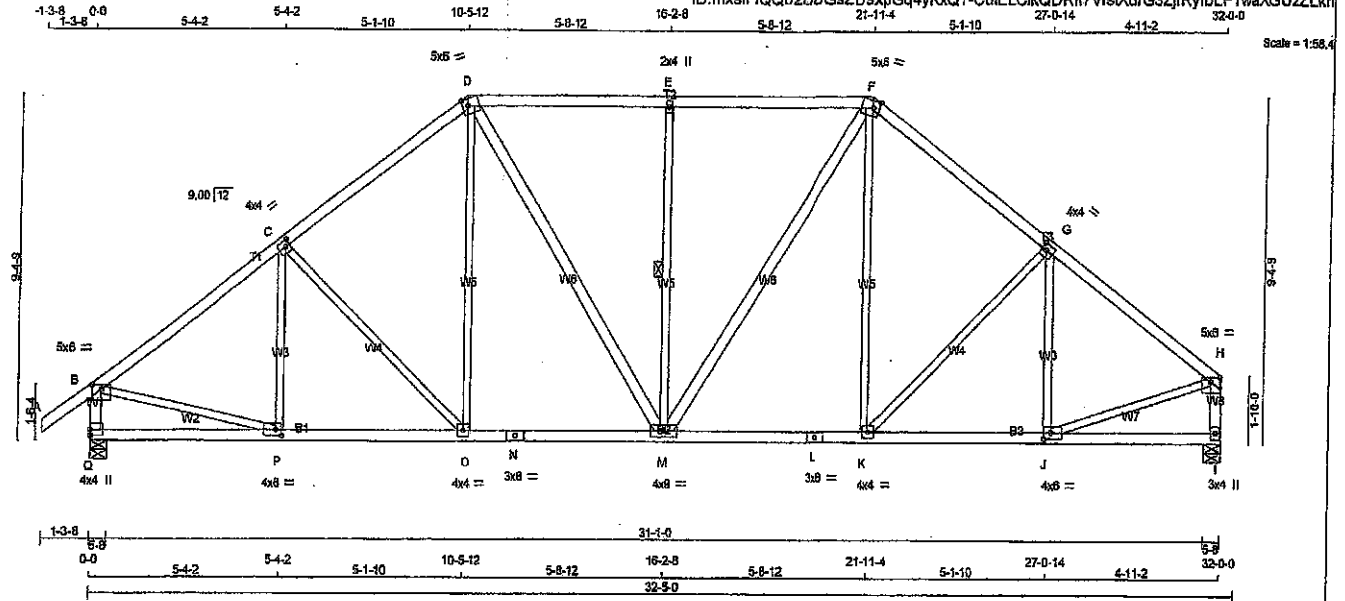


JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T5A	4	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsIFIQQ02DDGsZD9xpGq4yKxQY-CuiELCKQDRH7Vis0XurG3ZjRy1bLFTwaXGUzZLkh



TOTAL WEIGHT = 4 X 157 = 628 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
F - H	2x4	DRY	No.2
Q - B	2x4	DRY	No.2
I - H	2x4	DRY	No.2
Q - N	2x4	DRY	No.2
N - L	2x4	DRY	No.2
L - I	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
EXCEPT			
D - M	2x4	DRY	No.2
M - F	2x4	DRY	No.2

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-p	MT20	6.0	6.0	1.50	3.00
C	TMWW-I	MT20	4.0	4.0	2.00	1.50
D	TTWW-m	MT20	5.0	6.0	Edge	2.00
E	TMVW-w	MT20	2.0	4.0		
F	TTWW-m	MT20	5.0	6.0	Edge	2.00
G	TMWW-I	MT20	4.0	4.0	2.00	1.50
H	TMVW-p	MT20	5.0	6.0	1.50	3.00
I	BMV1-p	MT20	3.0	4.0		
J	BMWW-I	MT20	4.0	6.0	2.00	2.25
K	BS-I	MT20	4.0	4.0		
L	BS-I	MT20	3.0	6.0		
M	BMVWW-I	MT20	4.0	9.0		
N	BS-I	MT20	3.0	6.0		
O	BMWW-I	MT20	4.0	4.0		
P	BMWW-I	MT20	4.0	6.0	2.00	2.00
Q	BMV1-p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	RECORD BRG
JT	VERT	DOWN	UP	IN-SX
Q	2390	0	0	5-8
I	2250	0	0	5-8

UNFACTORED REACTIONS

	1ST LCASE	MAX./MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
Q	1778	1009 / 0	338 / 0	0 / 0	0 / 0	433 / 0	0 / 0
I	1681	929 / 0	338 / 0	0 / 0	0 / 0	416 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) Q, I

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.95 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF E-M.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. CS1 (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	
FR-TO		FROM TO		FR-TO			
A-B	0 / 42	-102.1 -102.1	0.14 (1)	P-C	-227 / 85	0.10 (1)	
B-C	-2471 / 0	-102.1 -102.1	0.48 (1)	C-O	-357 / 0	0.38 (1)	
C-D	-2234 / 0	-102.1 -102.1	0.45 (1)	O-D	0 / 473	0.11 (2)	
D-E	-2035 / 0	-102.1 -102.1	0.49 (1)	D-M	0 / 518	0.08 (1)	
E-F	-2035 / 0	-102.1 -102.1	0.49 (1)	M-E	-713 / 0	0.41 (1)	
F-G	-2187 / 0	-102.1 -102.1	0.42 (1)	M-F	0 / 337	0.09 (1)	
G-H	-2310 / 0	-102.1 -102.1	0.42 (1)	K-F	0 / 386	0.09 (2)	
Q-B	-2308 / 0	0.0 0.0	0.24 (1)	K-G	-225 / 0	0.24 (1)	
I-H	-2172 / 0	0.0 0.0	0.24 (1)	J-G	-363 / 14	0.17 (1)	
				B-P	0 / 2058	0.46 (1)	
				J-H	0 / 1963	0.44 (1)	
Q-P	0 / 0	-38.5 -38.5	0.19 (3)				
P-O	0 / 2007	-38.5 -38.5	0.45 (2)				
O-N	0 / 1759	-38.5 -38.5	0.43 (2)				
N-M	0 / 1759	-38.5 -38.5	0.43 (2)				
M-L	0 / 1722	-38.5 -38.5	0.42 (2)				
L-K	0 / 1722	-38.5 -38.5	0.42 (2)				
K-J	0 / 1878	-38.5 -38.5	0.42 (1)				
J-I	0 / 0	-38.5 -38.5	0.17 (3)				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. GC

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF BCBC 2018

- CSA 086-14

- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL)= L/360 (1.07")

CALCULATED VERT. DEFL.(LL)= L/999 (0.09")

ALLOWABLE DEFL.(TL)= L/360 (1.07")

CALCULATED VERT. DEFL.(TL)= L/999 (0.16")

CS1: TC=0.48/1.00 (D-E-1), BC=0.45/1.00 (O-P-2),

WB=0.48/1.00 (B-P-1), SS=0.28/1.00 (D-E-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10

COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION

(PSI) (PLI) (PLI)

MAX MIN MAX MIN MAX MIN

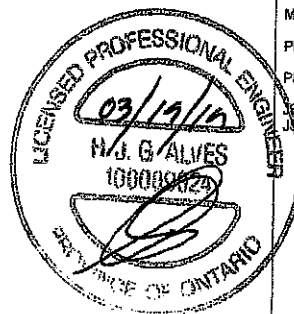
MT20 650 371 1747 786 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

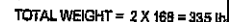
PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.89 (P) (INPUT = 0.90)

JSI METAL= 0.55 (N) (INPUT = 1.00)



DATE: MAR 19 2019
STRUCTURAL
COMPONENT ONLY

Scale = 1:62.5SI METAL= 0.74 (R) (INPUT = 1.00)

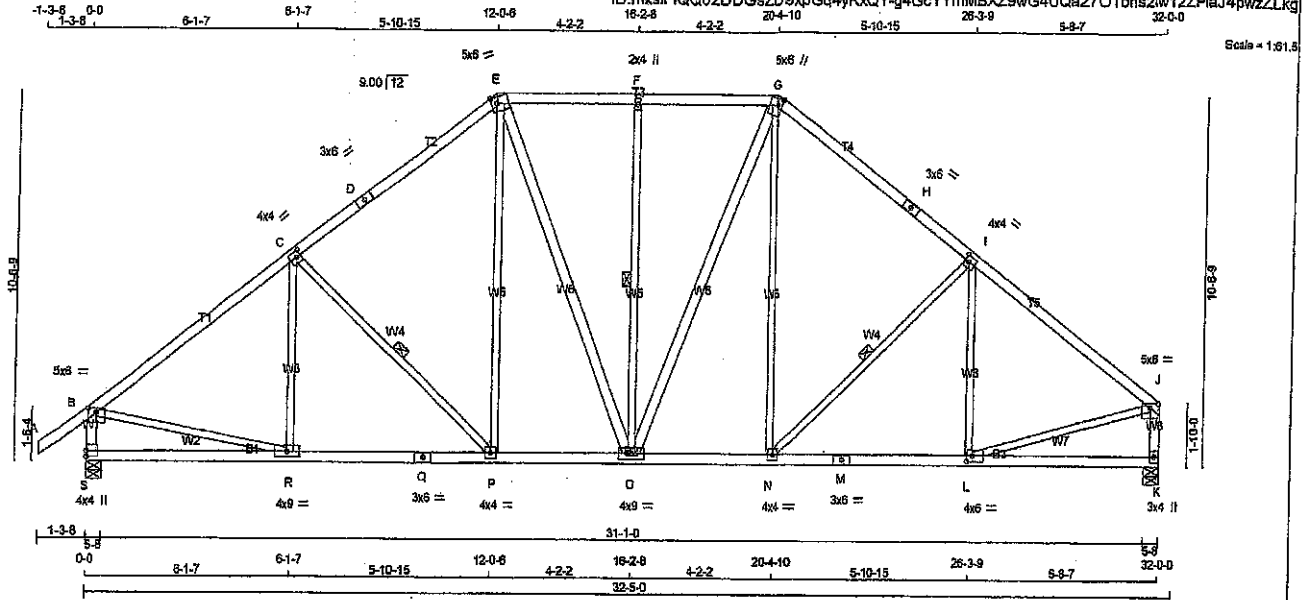
DWG NO. TAM 77905696
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T6A	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MTK Industries, Inc. Tue Mar 19 15:02:43 2019 Page 1

ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-g4GeYmMBXZ9wG4UQa27OTbhs2lw12ZPlaJ4pwzZLkg



TOTAL WEIGHT = 165 lb
[M/F]

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2	SPF
D - E	2x4	DRY	No.2	SPF
E - G	2x4	DRY	No.2	SPF
G - H	2x4	DRY	No.2	SPF
H - J	2x4	DRY	No.2	SPF
S - B	2x4	DRY	No.2	SPF
K - J	2x4	DRY	No.2	SPF
S - Q	2x4	DRY	No.2	SPF
Q - M	2x4	DRY	No.2	SPF
M - K	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
E - O	2x4	DRY	No.2	SPF
O - G	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW-p	MT20	5.0	6.0	1.50	3.00
C TMVW-t	MT20	4.0	4.0	2.00	1.50
D TS-t	MT20	3.0	5.0		
E TTVW-m	MT20	5.0	6.0	Edge	2.00
F TMVW-w	MT20	2.0	4.0		
G TTVW-t	MT20	5.0	6.0	2.25	1.50
H TS-t	MT20	3.0	6.0		
I TMVW-t	MT20	4.0	4.0	2.00	1.50
J TMVW-p	MT20	5.0	6.0	1.50	3.00
K BMV1+p	MT20	3.0	4.0		
L BMVW-t	MT20	4.0	6.0	2.00	2.00
M BS-t	MT20	3.0	6.0		
N BMVW-t	MT20	4.0	4.0		
O BMVW-w	MT20	4.0	9.0		
P BMVW-t	MT20	4.0	4.0		
Q BS-t	MT20	3.0	6.0		
R BMVW-t	MT20	4.0	9.0		
S BMV1+p	MT20	4.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
JT	VERT	HORZ	UP/LIFT	JT	VERT	HORZ	UP/LIFT	JT	VERT
S	2360	0	0	S	2360	0	0	S	2360
K	2250	0	0	K	2250	0	0	K	2250

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	1ST LCASE	MAX MIN. COMPONENT REACTIONS
S	COMBINED	SNOW	LIVE	PERM. LIVE
S	1778	1009 / 0	336 / 0	0 / 0
K	1681	928 / 0	336 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) S, K

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 3.72 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-P, F-Q, I-N.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. FACTORED VERT. LOAD (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED VERT. LOAD (LC)	MAX. FACTORED VERT. LOAD (LC)
FR-TO				FR-TO			
A-B	0 / 42	-102.1	-102.1	R-C	-129 / 164	0.08 (1)	0.25 (1)
B-C	-2489 / 0	-102.1	-102.1	C-P	-518 / 0	0.25 (1)	0.25 (1)
C-D	-2115 / 0	-102.1	-102.1	P-E	0 / 554	0.12 (2)	0.12 (2)
D-E	-2115 / 0	-102.1	-102.1	E-O	0 / 317	0.05 (1)	0.05 (1)
E-F	-1778 / 0	-102.1	-102.1	O-G	-509 / 0	0.40 (1)	0.40 (1)
F-G	-1778 / 0	-102.1	-102.1	G-H	0 / 380	0.06 (1)	0.06 (1)
G-H	-2083 / 0	-102.1	-102.1	H-I	0 / 488	0.11 (2)	0.11 (2)
H-I	-2083 / 0	-102.1	-102.1	I-N	-389 / 0	0.18 (1)	0.18 (1)
I-J	-2352 / 0	-102.1	-102.1	N-L	-251 / 99	0.15 (1)	0.15 (1)
J-K	-2285 / 0	0.0	0.0	L-J	0 / 2065	0.46 (1)	0.46 (1)
K-L	-2162 / 0	0.0	0.0	L-K	0 / 1983	0.45 (1)	0.45 (1)
S-R	0 / 0	-38.5	-38.5	R-Q	0 / 0	0.27 (3)	0.27 (3)
R-Q	0 / 2027	-38.5	-38.5	Q-P	0 / 2027	0.53 (2)	0.53 (2)
Q-P	0 / 1699	-38.5	-38.5	P-O	0 / 1699	0.37 (1)	0.37 (1)
P-O	0 / 1695	-38.5	-38.5	O-N	0 / 1695	0.38 (1)	0.38 (1)
O-N	0 / 1917	-38.5	-38.5	N-M	0 / 1917	0.50 (2)	0.50 (2)
N-M	0 / 1917	-38.5	-38.5	M-L	0 / 1917	0.50 (2)	0.50 (2)
M-L	0 / 0	-38.5	-38.5	L-K	0 / 0	0.25 (3)	0.25 (3)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2015
- CSA 086-14
- TFC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 28.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (1.07")
CALCULATED VERT. DEFL.(LL) = L/999 (0.09")
ALLOWABLE DEFL.(TL) = L/360 (1.07")
CALCULATED VERT. DEFL.(TL) = L/999 (0.15")

CSI: TO=0.88/1.00 (B-C:1), BC=0.53/1.00 (P-R:2),
WB=0.46/1.00 (B-R:1), SSI=0.24/1.00 (B-C:1)
DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

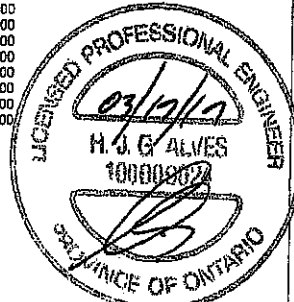
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.90 (G) (INPUT = 0.90)
ISI METAL= 0.73 (Q) (INPUT = 1.00)



DWG NO. TAM 11905719
STRUCTURAL
S.C. ROBERTSON INC.

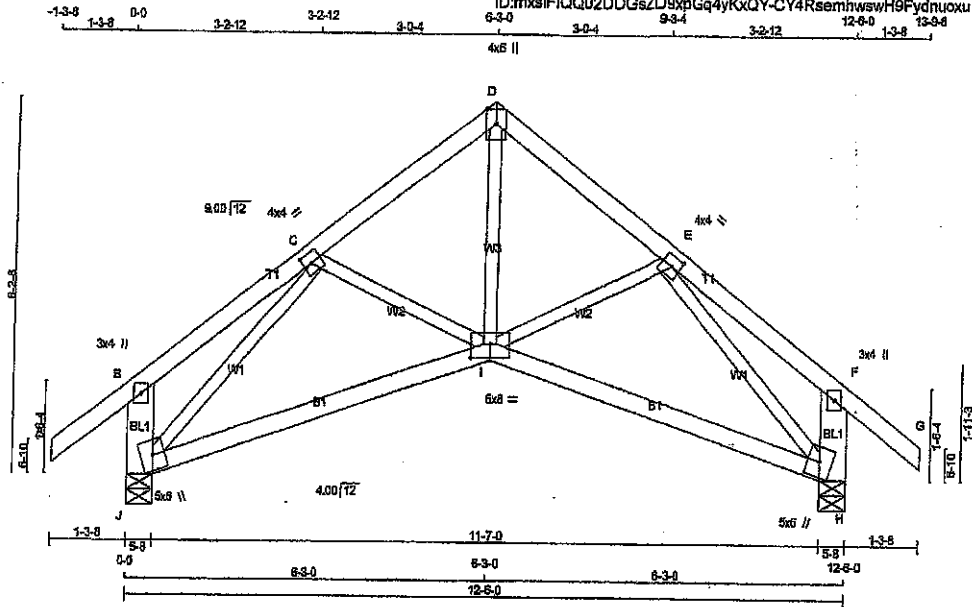
JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401811	T8S	3	1	GREEN PARK HOMES	

Tamarack Roof Truss, Burlington

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ID:mxfIQQ02DDGsZD9xpGq4yKxQY-CY4RsemhwsW9FydnuxuDyd9p9G3AHRH8acMzZMcB

Scale = 1:35.6



TOTAL WEIGHT = 3 X 57 = 170 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - D	2x4	DRY	No.2
D - G	2x4	DRY	No.2
J - I	2x4	DRY	No.2
I - H	2x4	DRY	No.2

BEARING BLOCKS

BL1	2-2x6	DRY	No.2	SPF
-----	-------	-----	------	-----

ALL WEBS

2x3	DRY	No.2	SPF
-----	-----	------	-----

DRY, SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED	MAXIMUM FACTORED	INPUT	REQD
	GROSS REACTION	GROSS REACTION	BRG	BRG
JT	VERT	DOWN	UP	IN-SX
J	1020	0	0	5-8
H	1020	0	0	5-8

UNFACTORED REACTIONS

	1ST CASE	MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
J	753	443 / 0	131 / 0	0 / 0	0 / 0	179 / 0	0 / 0
H	753	443 / 0	131 / 0	0 / 0	0 / 0	179 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) J, H

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS	MAX. FACTORED	FACTORED	VERT. LOAD	LC1	MAX	WEBS	MAX. FACTORED
MEMB.	FORCE (LBS)	VERT. (PLF)	CS1 (LC)	UNBRAC	LENGTH	MEMB.	FORCE (LBS)
FR-TO						FR-TO	
A-B	0 / 47	-102.1	-102.1	0.17 (1)	10.00	I-D	0 / 794
B-C	-18 / 0	-102.1	-102.1	0.10 (1)	8.25	I-E	-57 / 55
C-D	-886 / 0	-102.1	-102.1	0.11 (1)	6.25	C-I	-57 / 55
D-E	-886 / 0	-102.1	-102.1	0.11 (1)	6.25	J-C	-1118 / 0
E-F	-18 / 0	-102.1	-102.1	0.10 (1)	6.25	E-H	-1118 / 0
F-G	0 / 47	-102.1	-102.1	0.17 (1)	10.00		
J-B	-299 / 0	0.0	0.0	0.02 (1)	7.81		
H-F	-299 / 0	0.0	0.0	0.02 (1)	7.81		
J-I	0 / 793	-38.5	-38.5	0.43 (2)	10.00		
I-H	0 / 793	-38.5	-38.5	0.43 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	= 29.0	PSF
	DL	= 5.0	PSF
BOT CH.	LL	= 10.5	PSF
	DL	= 7.0	PSF
TOTAL LOAD		= 52.5	PSF

SPACING = 24.0 IN O/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 086-14
- TPI 2014

(55 % OF 37.5 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.42")
CALCULATED VERT. DEFL.(LL) = L/998 (0.11")
ALLOWABLE DEFL.(TL) = L/360 (0.42")
CALCULATED VERT. DEFL.(TL) = L/799 (0.19")

CSI: TC=0.17/1.00 (A-B-I), BC=0.43/1.00 (J-I), WB=0.38/1.00 (C-I), SI=0.12/1.00 (I-J)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

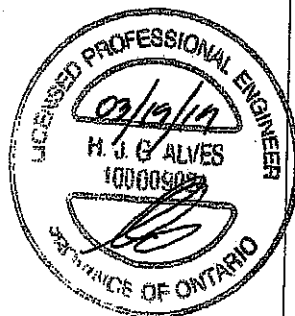
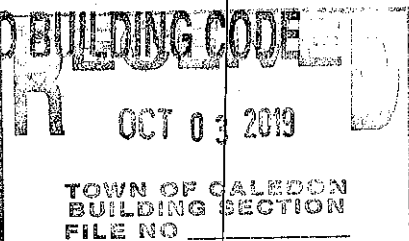
PLATE	GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MT20	650	371	1747
	788	1587	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

ISI GRIP= 0.86 (E) (INPUT = 0.80)
ISI METAL= 0.35 (C) (INPUT = 1.00)

THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE



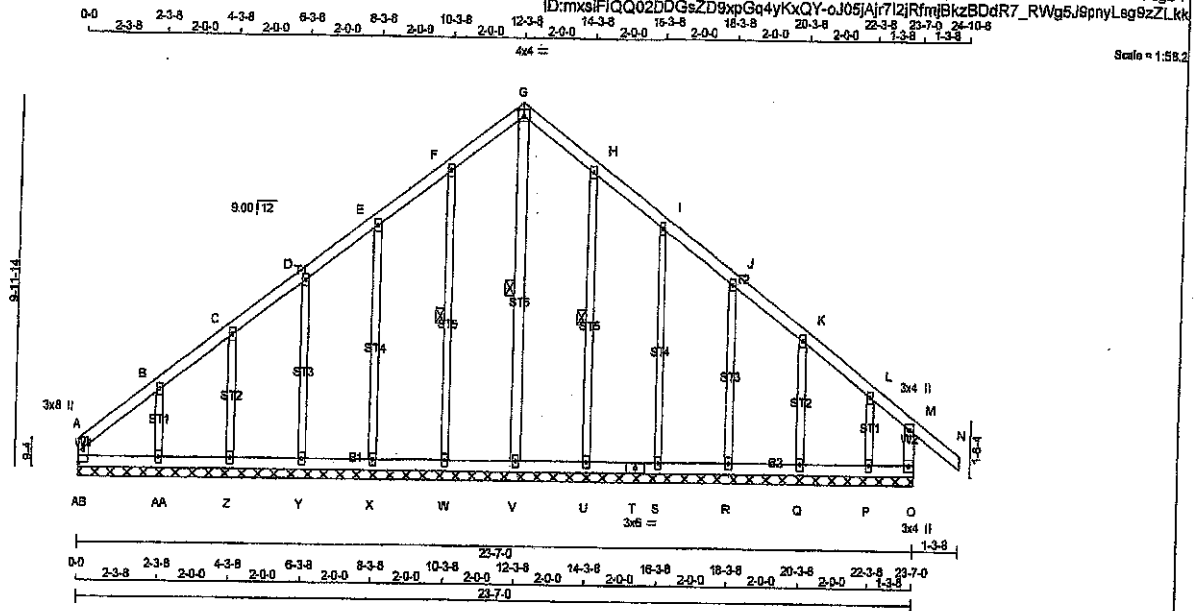
DWG NO. TAM 17405699
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	G10	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsIFiQQ02DDGsZD9xpGq4yKxQY-oJ05JAr7I2RfmBkzBDdR7_RWg5JspnyLeg9zZLkk



TOTAL WEIGHT = 115 lb [M]

LUMBER	CHORDS	SIZE	DRY	LUMBER	DESCR.
N. L. G. A. RULES	A - G	2x4	DRY	No.2	SPF
	G - N	2x4	DRY	No.2	SPF
	AB - A	2x4	DRY	No.2	SPF
	O - M	2x4	DRY	No.2	SPF
	AB - T	2x4	DRY	No.2	SPF
	T - O	2x4	DRY	No.2	SPF
	ALL WEBS EXCEPT V - G	2x4	DRY	No.2	SPF
	ALL GABLE WEBS EXCEPT ST2	2x4	DRY	No.2	SPF
	ST2	2x4	DRY	No.2	SPF
	DRY: SEASONED LUMBER.				
	GABLE STUDS SPACED AT 2-0-0 OC.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS
THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.
THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.
BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.
ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.
1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-V, F-W, H-U.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN./C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

DESIGN ASSUMPTIONS
- OVERHANG NOT TO BE ALTERED OR CUT OFF.

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (M-N:1), BC=0.04/1.00 (O-P:1), WB=0.17/1.00 (S:1), SS=0.09/1.00 (M-N:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 384 1857 788 1987 1856

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

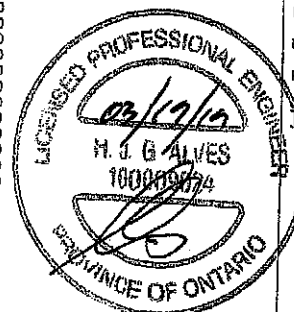
SL GRIP= 0.40 (F) (INPUT = 0.30)
SL METAL= 0.12 (B) (INPUT = 1.00)

DRWG NO. TAM 71905715
STRUCTURAL
02/24/19

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMBM1+p MT20	3.0	8.0	4.00	1.50
B, C, D, E, F, H, I, J, K, L	TTW+p MT20	2.0	4.0		
G	TTW+p MT20	4.0	4.0	2.25	2.00
M	TMV+p MT20	3.0	4.0		
O	BMV1+p MT20	3.0	4.0		
P, Q, R, S, U, V, W, X, Y, Z, AA	BMV1+p MT20	2.0	4.0		
T	BS-t MT20	3.0	8.0		
AB					

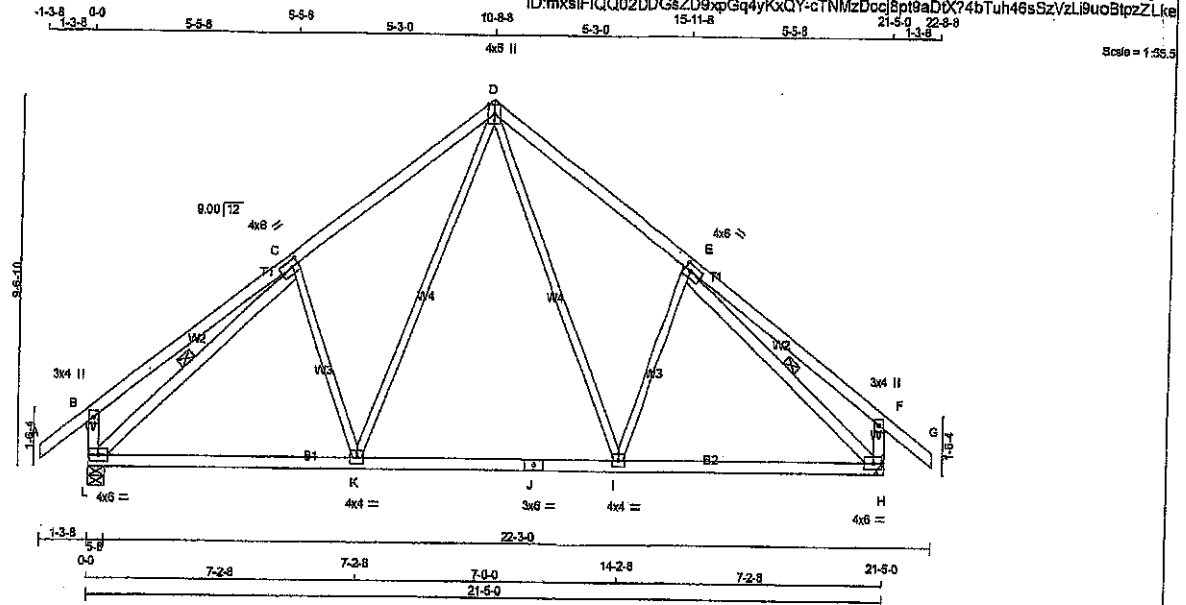
CHORDS		FACTORED		WEBS	
MEMB.	MAX. FORCE (LBS)	VERT. LOAD (PLF)	MAX. CS (LC)	MEMB.	MAX. FORCE (LBS)
FR-TO		FROM TO	LENGTH	FR-TO	
A-B	-14/0	-102.1 -102.1	0.05 (1)	8.25	V-G -228/0
B-C	-14/0	-102.1 -102.1	0.05 (1)	8.25	W-F -204/0
C-D	-3/0	-102.1 -102.1	0.05 (1)	10.00	X-E -202/0
D-E	-4/0	-102.1 -102.1	0.05 (1)	10.00	Y-D -203/0
E-F	-4/0	-102.1 -102.1	0.05 (1)	10.00	Z-C -193/0
F-G	-3/8	-102.1 -102.1	0.05 (1)	10.00	AA-B -227/0
G-H	-3/8	-102.1 -102.1	0.05 (1)	10.00	U-H -204/0
H-I	-4/4	-102.1 -102.1	0.05 (1)	10.00	S-I -203/0
I-J	-4/0	-102.1 -102.1	0.05 (1)	10.00	R-J -198/0
J-K	-6/0	-102.1 -102.1	0.05 (1)	10.00	Q-K -218/0
K-L	-5/0	-102.1 -102.1	0.05 (1)	10.00	P-L -90/0
L-M	-50/0	-102.1 -102.1	0.14 (1)	8.25	
M-N	0/42	-102.1 -102.1	0.14 (1)	10.00	
AB-A	-113/0	0.0	0.02 (1)	7.81	
O-M	-270/0	0.0	0.04 (1)	7.81	
AB-AA	0/17	-38.5 -38.5	0.04 (2)	10.00	
AA-Z	0/8	-38.5 -38.5	0.04 (2)	10.00	
Z-Y	0/4	-38.5 -38.5	0.02 (3)	10.00	
Y-X	0/3	-38.5 -38.5	0.02 (3)	10.00	
X-W	-3/3	-38.5 -38.5	0.02 (3)	10.00	
W-V	-5/3	-38.5 -38.5	0.02 (3)	10.00	
V-U	-5/3	-38.5 -38.5	0.02 (3)	10.00	
U-T	-3/3	-38.5 -38.5	0.02 (3)	10.00	
T-S	-3/3	-38.5 -38.5	0.02 (3)	10.00	
S-R	0/3	-38.5 -38.5	0.02 (3)	10.00	
R-Q	0/4	-38.5 -38.5	0.03 (3)	10.00	
Q-P	0/8	-38.5 -38.5	0.03 (2)	10.00	
P-O	0/12	-38.5 -38.5	0.04 (1)	10.00	



JOB NAME 401812	TRUSS NAME T14	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-cTNMzDccj8pt9aDXX74bTuh46sSzvzLi9uoBtpzZLke



Scale = 1/32

TOTAL WEIGHT = 2 X 102 = 204 lb

LUMBER	CHORDS	SIZE	LUMBER	DESCR.
N. L. G. A. RULES				
A - D	2x4	DRY	No.2	SPF
D - G	2x4	DRY	No.2	SPF
L - B	2x4	DRY	No.2	SPF
H - F	2x4	DRY	No.2	SPF
L - J	2x4	DRY	No.2	SPF
J - H	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
EXCEPT				
L - C	2x4	DRY	No.2	SPF
E - H	2x4	DRY	No.2	SPF

DRY: SEASONED LUMBER.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS		FACTORED		MAXIMUM FACTORED		INPUT		REQRD	
JT	GROSS REACTION	VERT	HORZ	GROSS REACTION	DOWN	HORZ	UPLIFT	BRG	BRG
L	1648	0	0	1648	0	0	5-8	5-8	5-8
H	1648	0	0	1648	0	0	MECHANICAL		

A SUITABLE HANGER/MECHANICAL CONNECTION IS REQUIRED AT JOINT H. MINIMUM BEARING LENGTH AT JOINT H = 3-8.

UNFACTORED REACTIONS

JT	1ST LCASE	MAX MIN. COMPONENT REACTIONS	COMBINED	SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
L	1222	702 / 0	225 / 0	0 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0
H	1222	702 / 0	225 / 0	0 / 0	0 / 0	0 / 0	0 / 0	285 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) L

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.01 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF C-L, E-H.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS		FACTORED		WESS		MAX. FACTORED	
MEMB.	FORCE (LBS)	VERT. LOAD	LC1 MAX (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	FORCE (LBS)	MAX. FACTORED (LC)
FR-TO					FR-TO		
A-B	0 / 42	-102.1	-102.1	0.14 (1)	10.00	D-I	0 / 667
B-C	0 / 40	-102.1	-102.1	0.48 (1)	10.00	E-E	-338 / 17
C-D	-1416 / 0	-102.1	-102.1	0.41 (1)	5.01	K-D	0 / 667
D-E	-1416 / 0	-102.1	-102.1	0.41 (1)	5.01	C-K	-336 / 17
E-F	0 / 40	-102.1	-102.1	0.48 (1)	10.00	L-C	-1702 / 0
F-G	0 / 42	-102.1	-102.1	0.14 (1)	10.00	E-H	-1702 / 0
L-B	-345 / 0	0.0	0.0	0.04 (1)	7.81		
H-F	-345 / 0	0.0	0.0	0.04 (1)	7.81		
L-K	0 / 1218	-38.5	-38.5	0.50 (2)	10.00		
K-J	0 / 668	-38.5	-38.5	0.45 (2)	10.00		
J-I	0 / 668	-38.5	-38.5	0.45 (2)	10.00		
I-H	0 / 1218	-38.5	-38.5	0.50 (2)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = 1/360 (0.71")
CALCULATED VERT. DEFL. (LL) = 1/899 (0.11")
ALLOWABLE DEFL. (TL) = 1/360 (0.71")
CALCULATED VERT. DEFL. (TL) = 1/999 (0.18")

CSI: TC=0.48/1.00 (E-F:1), SC=0.50/1.00 (K-L:2), WB=0.45/1.00 (E-H:1), SS=0.21/1.00 (D-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

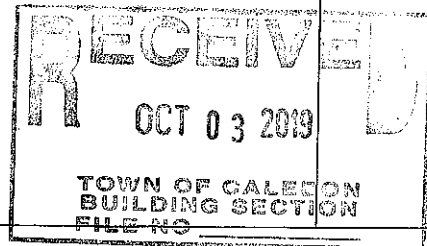
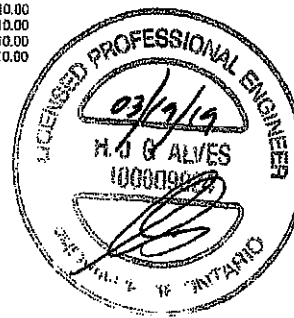
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 786 1887 1656

PLATE PLACEMENT TOL. = 0.250 inches

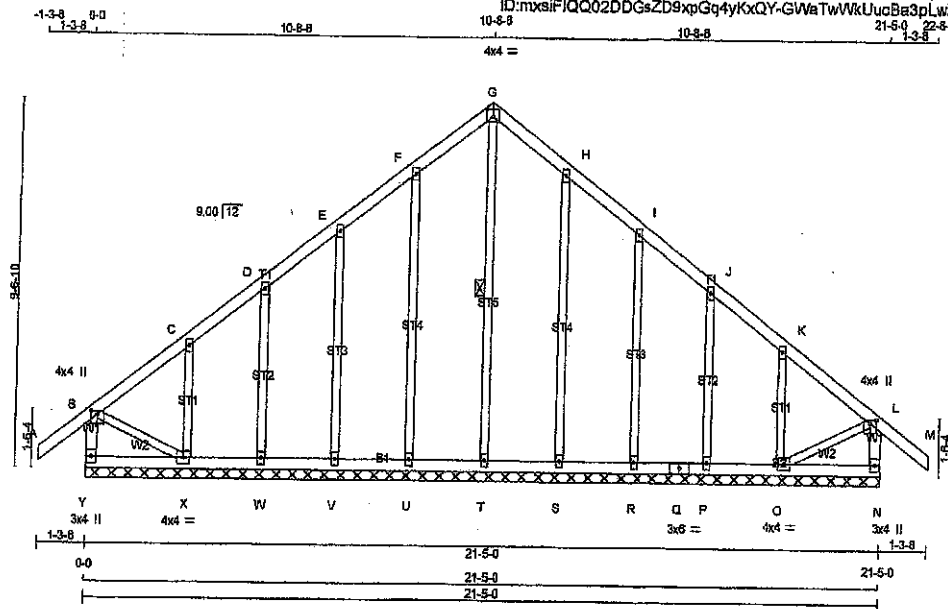
PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.87 (I) (INPUT= 0.90)
JSI METAL= 0.46 (C) (INPUT= 1.00)



DRWG NO. TAM 71905720
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	DRWG NO.
401812	G14	1	1	GREEN PARK HOMES	
Tamarack Roof Truss, Burlington					
Version 8.230 S Nov 17 2018 Mittek Industries, Inc. Tue Mar 19 15:02:40 2019 Page 1					
ID:mxsiflQQ02DDGsZD9xpGg4yKxQY-GWaTwWkUucBa3pLwISUQmzrljrmqkaz?c5QCbzZLk					
21-5-9 22-5-8 1-3-8					



Scale = 1:58.7

TOTAL WEIGHT = 107 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
Y - B	2x4	DRY	No.2
A - G	2x4	DRY	No.2
G - M	2x4	DRY	No.2
N - L	2x4	DRY	No.2
Y - Q	2x4	DRY	No.2
Q - N	2x4	DRY	No.2

ALL WEBS	2x3	DRY	No.2
ALL GABLE WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

GABLE STUDS SPACED AT 2'-0" OC.

PLATES (table in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW+p	MT20	4.0	4.0	1.00	2.00
C, D, E, F, H, I, J, K						
C	TMVW+p	MT20	2.0	4.0		
G	TTW+p	MT20	4.0	4.0	2.25	2.00
L	TMVW+p	MT20	4.0	4.0	1.00	2.00
N	BMV1+p	MT20	3.0	4.0		
O	BMVW1-t	MT20	4.0	4.0		
P, R, S, T, U, V, W						
P	BMV1+p	MT20	2.0	4.0		
Q	BS-4	MT20	3.0	4.0		
X	BMVW1-t	MT20	4.0	4.0		
Y	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

THIS TRUSS DESIGNED FOR CONTINUOUS BEARINGS.

THIS TRUSS REQUIRES RIGID SHEATHING ON EXPOSED FACE.

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S)

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF G-T.

END VERTICAL(S) MUST BE SHEATHED OR HAVE BRACES AS INDICATED IN THE MAX. UNBRACED LENGTH COLUMN OF THE TABLE BELOW

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (L.C.)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (L.C.)	
FR-TO		FROM TO		FR-TO			
Y-B	-294 / 0	0.0 0.0 0.03 (1)	7.81	T-G	-142 / 0	0.08 (1)	
A-B	0 / 42	-102.1 -102.1 0.14 (1)	10.00	U-F	-231 / 0	0.20 (1)	
B-C	-31 / 0	-102.1 -102.1 0.09 (1)	6.25	V-E	-201 / 0	0.14 (1)	
C-D	-48 / 0	-102.1 -102.1 0.09 (1)	6.25	W-D	-180 / 0	0.07 (1)	
D-E	-31 / 0	-102.1 -102.1 0.05 (1)	6.25	X-C	-273 / 0	0.06 (1)	
E-F	-28 / 0	-102.1 -102.1 0.06 (1)	6.25	S-H	-231 / 0	0.29 (1)	
F-G	-38 / 0	-102.1 -102.1 0.05 (1)	6.25	R-I	-201 / 0	0.14 (1)	
G-H	-38 / 0	-102.1 -102.1 0.03 (1)	6.25	P-J	-180 / 0	0.07 (1)	
H-I	-28 / 0	-102.1 -102.1 0.05 (1)	6.25	O-K	-273 / 0	0.06 (1)	
I-J	-31 / 0	-102.1 -102.1 0.05 (1)	6.25	B-X	0 / 40	0.01 (1)	
J-K	-48 / 0	-102.1 -102.1 0.09 (1)	6.25	O-L	0 / 40	0.01 (1)	
K-L	-31 / 0	-102.1 -102.1 0.09 (1)	6.25				
L-M	0 / 42	-102.1 -102.1 0.14 (1)	10.00				
N-L	-294 / 0	0.0 0.0 0.03 (1)	7.81				
Y-X	0 / 0	-38.5 -38.5 0.05 (3)	10.00				
X-W	0 / 29	-38.5 -38.5 0.05 (3)	10.00				
W-V	0 / 28	-38.5 -38.5 0.03 (2)	10.00				
V-U	0 / 23	-38.5 -38.5 0.03 (2)	10.00				
U-T	0 / 20	-38.5 -38.5 0.02 (2)	10.00				
T-S	0 / 20	-38.5 -38.5 0.03 (2)	10.00				
S-R	0 / 23	-38.5 -38.5 0.03 (2)	10.00				
R-Q	0 / 26	-38.5 -38.5 0.03 (2)	10.00				
Q-P	0 / 26	-38.5 -38.5 0.03 (2)	10.00				
P-O	0 / 29	-38.5 -38.5 0.05 (3)	10.00				
O-N	0 / 0	-38.5 -38.5 0.05 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH.	LL	=	28.0	PSF
	CL	=	6.0	PSF
BOT CH.	LL	=	10.5	PSF
	CL	=	7.0	PSF
TOTAL LOAD		=	52.5	PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.14/1.00 (L-M:1), SC=0.05/1.00 (O-P:3), WB=0.28/1.00 (F-U:1), SI=0.10/1.00 (K-L:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

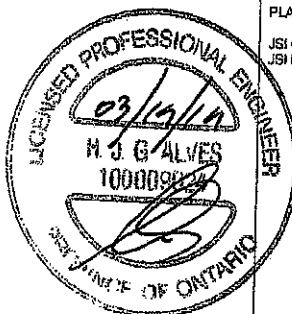
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(PSI)	(PLF)	(PLI)
MAX MIN	MAX MIN	MAX MIN	MAX MIN
MT20	618 354	1867 788	1987 1659

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.58 (O) (INPUT = 0.90)
JSI METAL= 0.14 (C) (INPUT = 1.00)



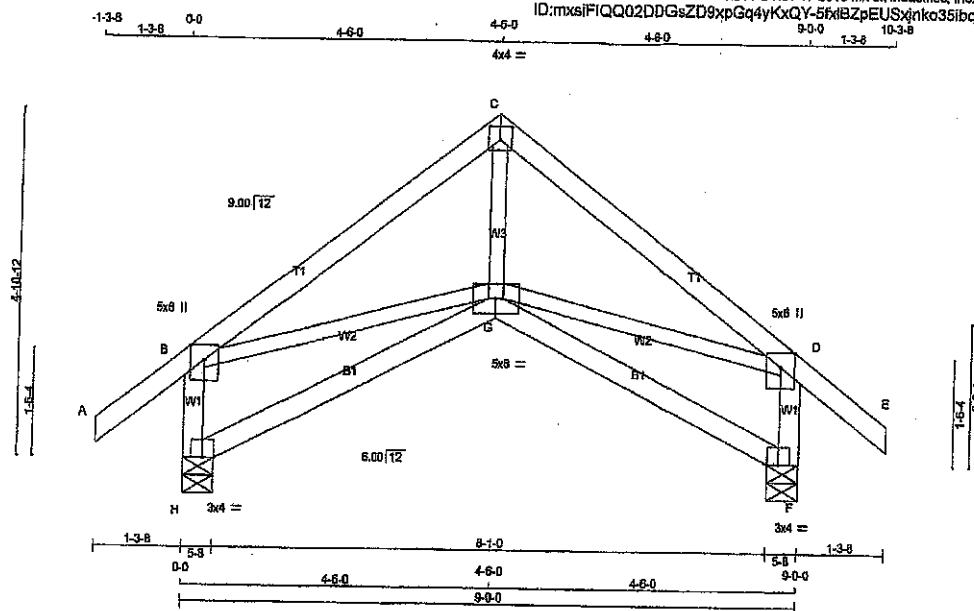
DWG NO. TAM T4905716
STRUCTURAL
ENGINEER

JOB NAME 401812	TRUSS NAME T17S	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	TRUSS DESC.	DRWG NO.
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 10 15:02:46 2019 Page 1
ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-5fMIBZpEUSXnko35ibq?6DH8G4EUyOYYkPFzZLkd

Scale = 1:30.4



TOTAL WEIGHT = 2 X 40 = 80 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
H - B	2x4	DRY	No.2	SPF
F - D	2x4	DRY	No.2	SPF
H - G	2x4	DRY	No.2	SPF
G - F	2x4	DRY	No.2	SPF

ALL WEBS 2x3 DRY No.2
EXCEPT

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B TMVW+p	MT20	5.0	8.0	Edge	
C TTW+p	MT20	4.0	4.0	2.25	2.00
D TMVW+p	MT20	5.0	6.0	Edge	
F BVM1+p	MT20	3.0	4.0		
G BBVWW+p	MT20	5.0	8.0		
H BVM1+p	MT20	3.0	4.0		

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
JT	VERT	DOWN	UPLIFT	IN-SX
H	774	0	0	5-8
F	774	0	0	5-8

UNFACTORED REACTIONS

JT	COMBINED	SNOW	LIVE	PERM LIVE	WIND	DEAD	SOIL
H	568	341/0	95/0	0/0	0/0	134/0	0/0
F	568	341/0	95/0	0/0	0/0	134/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (LBS)	MAX. FACTORED LC1 (LBS)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED LC1 (LBS)	
FR-TO		FROM	TO	FR-TO		FROM	TO
A-B	0/42	-102.1	-102.1	0.14 (1)	10.00	G-C	0/404
B-C	-703/0	-102.1	-102.1	0.27 (1)	8.25	B-G	0/573
C-D	-703/0	-102.1	-102.1	0.27 (1)	8.25	G-D	0/573
D-E	0/42	-102.1	-102.1	0.14 (1)	10.00		
H-B	-887/0	0.0	0.0	0.08 (1)	7.81		
F-D	-887/0	0.0	0.0	0.08 (1)	7.81		
H-G	0/0	-38.5	-38.5	0.18 (3)	10.00		
G-F	0/0	-38.5	-38.5	0.18 (3)	10.00		

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. CG

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL. (LL) = $L/360$ (0.30")
CALCULATED VERT. DEFL. (LL) = $L/899$ (0.04")
ALLOWABLE DEFL. (TL) = $L/360$ (0.30")
CALCULATED VERT. DEFL. (TL) = $L/999$ (0.08")

CSI: TC=0.27/1.00 (B-C:1), BC=0.18/1.00 (G-H:3), WB=0.19/1.00 (D-G:1), SS=0.14/1.00 (B-C:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

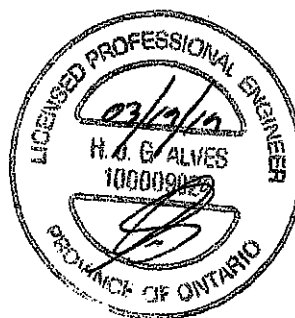
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP (DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 786 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.72 (C) (INPUT = 0.80)
JSI METAL= 0.40 (H) (INPUT = 1.00)



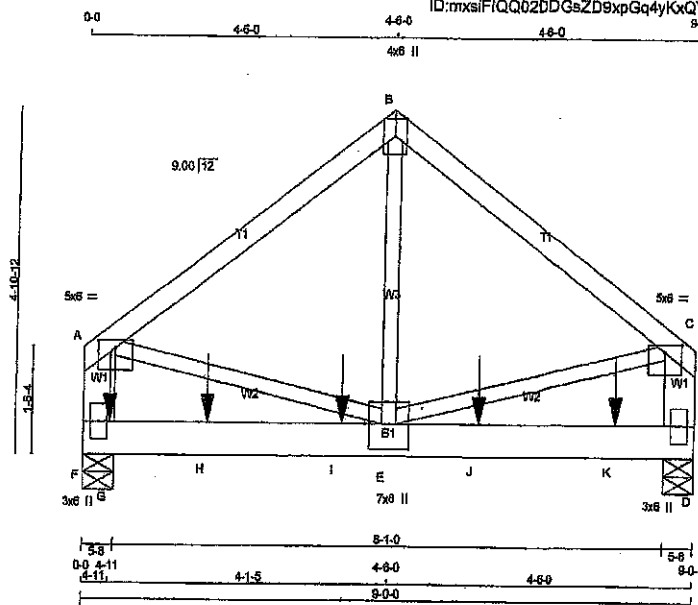
END OF DRAWING 71905721
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	T18	1	3	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.236 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 15:02:46 2019 Page 1
ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-5fzBZpEUSxjko35lq?6DJrGqERurOYYkPFzZLkd

Scale = 1/32" = 1'-0"



TOTAL WEIGHT = 3 X 43 = 129 lb

LUMBER	N. L. G. A. RULES	CHORDS	SIZE	DRY	LUMBER	DESCR.
A - B	2x4	DRY	No.2	SPF		
B - C	2x4	DRY	No.2	SPF		
F - A	2x6	DRY	No.2	SPF		
D - C	2x6	DRY	No.2	SPF		
F - D	2x6	DRY	No.2	SPF		
ALL WEBS	2x3	DRY	No.2	SPF		
EXCEPT						

DRY: SEASONED LUMBER.

DESIGN CONSISTS OF 3 TRUSSES BUILT SEPARATELY THEN FASTENED TOGETHER AS FOLLOWS:

CHORDS #ROWS	SURFACE SPACING (IN)	LOAD (PLF)
TOP CHORDS : (0.122"x3") SPIRAL NAILS		
A-B	12	TOP
B-C	12	TOP
F-A	4	SIDE(40.8)
D-C	4	TOP
BOTTOM CHORDS : (0.122"x3") SPIRAL NAILS		
F-D	5	SIDE(108.5)
WEBS : (0.122"x3") SPIRAL NAILS		
2x3	6	

STAGGER NAILS BY HALF THE SURFACE SPACING IN ADJACENT FLIES.

GIRDER NAILING ASSUMES NAILED HANGERS ARE FASTENED WITH MIN. 3-0 INCH NAILS.

TOP - COMPONENTS ARE LOADED FROM THE TOP AND MUST BE PLACED ON TOP EDGE OF ALL PLYS FOR THE LOAD TO BE TRANSFERRED TO EACH PLY.

SIDE - PLF SHOWN IS THE EQUIVALENT UDL APPLIED TO ONE SIDE THAT THE CORRESPONDING NAILING PATTERN SHALL BE CAPABLE OF TRANSFERING. REMAINING PLF MUST BE APPLIED ON THE OPPOSITE SIDE OR ON THE TOP.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TMVW-p	MT20	5.0	6.0	1.25 3.00
B	TTW-p	MT20	4.0	6.0	Edge
C	TMVW-p	MT20	5.0	6.0	1.25 3.00
D	BMV1-p	MT20	3.0	6.0	
E	BMVWVH	MT20	7.0	6.0	4.25 3.50
F	BMV1-p	MT20	3.0	6.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG		REQRD BRG	
	VERT	HORZ	DOWN	UPLIFT	IN-SX	IN-SX		
JT	6875	0	6875	0	5-8	5-8		
F	5458	0	5458	0	5-8	5-8		

UNFACTORED REACTIONS

1ST LCASE	MAX/MIN COMPONENT REACTIONS							
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL	
F	5122	2879 / 0	988 / 0	0 / 0	0 / 0	1254 / 0	0 / 0	
D	4065	2269 / 0	782 / 0	0 / 0	0 / 0	994 / 0	0 / 0	

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 5.26 FT. MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

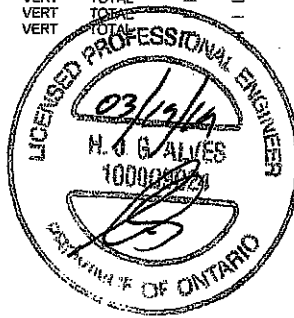
LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CS1	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)	WEBS	
									FR-TO	FROM TO
A-B	-4408 / 0	-102.1	-102.1	0.16 (1)	5.26	E-B	0 / 4832	0.38 (1)		
B-C	-4408 / 0	-102.1	-102.1	0.16 (1)	5.26	A-E	0 / 3833	0.27 (1)		
F-A	-3745 / 0	0.0	0.0	0.08 (1)	7.81	E-C	0 / 3833	0.27 (1)		
D-C	-3745 / 0	0.0	0.0	0.08 (1)	7.81					
F-G	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
G-H	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
H-I	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
I-E	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
E-J	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
J-K	0 / 0	-38.5	-38.5	0.85 (1)	10.00					
K-D	0 / 0	-38.5	-38.5	0.85 (1)	10.00					

FACTORED CONCENTRATED LOADS (LBS)

JT	LOC.	LC1	MAX-	MAX+	FACE	DIR.	TYPE	HEEL	CONN.
G	4-11	-2223	-2223		BACK	VERT	TOTAL		
H	1-9-12	-2211	-2211		BACK	VERT	TOTAL		
I	3-9-12	-2211	-2211		BACK	VERT	TOTAL		
J	5-9-12	-2211	-2211		TOP	VERT	TOTAL		
K	7-9-12	-2211	-2211		BACK	VERT	TOTAL		



DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.8 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = L/360 (0.30")
CALCULATED VERT. DEFL.(LL) = L/988 (0.04")
ALLOWABLE DEFL.(TL) = L/360 (0.30")
CALCULATED VERT. DEFL.(TL) = L/988 (0.07")

CSI: TC=0.16/1.00 (A-B-1), BC=0.65/1.00 (E-F-1), WB=0.36/1.00 (B-E-1), SS=0.74/1.00 (E-F-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE HEELS OFF

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

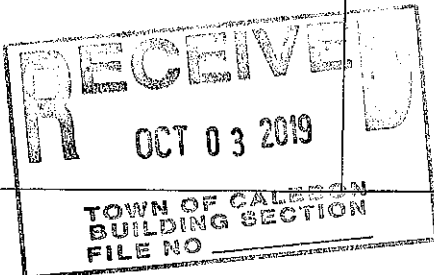
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	618	354	1667 788 1987 1656

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.84 (E) (INPUT = 0.90)
JSI METAL= 0.38 (B) (INPUT = 1.00)



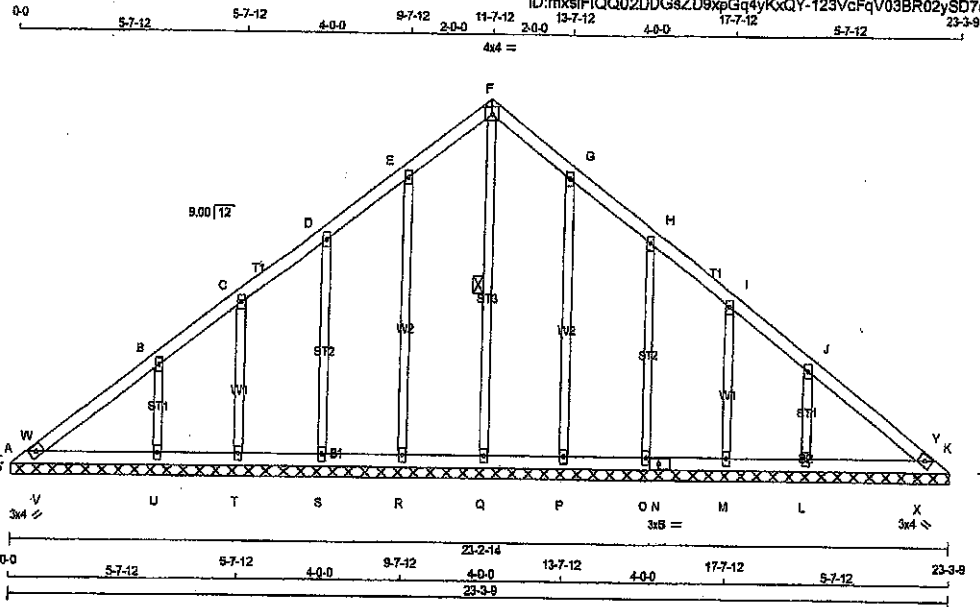
DRWG NO. TAM T1905722
STRUCTURAL
CONSTRUCTION ONLY

JOB NAME 401812	TRUSS NAME V10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
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Tamarack Roof Truss, Burlington

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ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-123VcFqV03BR02ySD7el5XJfm3ahNl8rs1rU7zZLkb



Scale = 1:51.0

TOTAL WEIGHT = 94 lb

LUMBER						
N. L. G. A. RULES						
CHORDS		SIZE		LUMBER	DESCR	
A - F	2x4	DRY	No.2			SPF
F - K	2x4	DRY	No.2			SPF
A - N	2x4	DRY	No.2			SPF
N - K	2x4	DRY	No.2			SPF
ALL WEBS		2x3	DRY	No.2		SPF
DRY: SEASONED LUMBER.						

PLATES (table is in inches)	JT TYPE	PLATES	W	LEN	Y	X
A TBM1-h	MT20	3.0	4.0			
B, C, D, E, G, H, I, J						
B TMW+w	MT20	2.0	4.0			
F TTW-p	MT20	4.0	4.0	2.25	2.00	
K TBM1-h	MT20	3.0	4.0			
L, M, O, P, Q, R, S, T, U						
L BMW1+w	MT20	2.0	4.0			
N BS-t	MT20	3.0	6.0			

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BUILDING DESIGNER									
BEARINGS									
	FACTORED			MAXIMUM FACTORED			INPUT	REQRD	
	GROSS REACTION			GROSS REACTION			BRG	BRG	
JT	VERT	HORZ		DOWN	HORZ	UPLIFT	IN-SX	IN-SX	
A	186	0		186	0	0	23-2-14	(7-3-23-2-14)	
K	186	0		186	0	0	23-2-14	(7-3-23-2-14)	
Q	325	0		325	0	0	23-2-14	(7-3-23-2-14)	
S	292	0		292	0	0	23-2-14	(7-3-23-2-14)	
U	494	0		494	0	0	23-2-14	(7-3-23-2-14)	
O	292	0		292	0	0	23-2-14	(7-3-23-2-14)	
L	494	0		494	0	0	23-2-14	(7-3-23-2-14)	
T	192	0		192	0	0	23-2-14	(7-3-23-2-14)	
R	306	0		306	0	0	23-2-14	(7-3-23-2-14)	
P	308	0		308	0	0	23-2-14	(7-3-23-2-14)	
M	192	0		192	0	0	23-2-14	(7-3-23-2-14)	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

1ST LCASE		MAX/MIN. COMPONENT REACTIONS					
JT	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
A	138	81/0	24/0	0/0	0/0	33/0	0/0
K	138	81/0	24/0	0/0	0/0	33/0	0/0
Q	247	124/0	58/0	0/0	0/0	65/0	0/0
S	219	118/0	45/0	0/0	0/0	55/0	0/0
U	370	202/0	75/0	0/0	0/0	92/0	0/0
O	219	118/0	45/0	0/0	0/0	55/0	0/0
L	370	202/0	75/0	0/0	0/0	92/0	0/0
T	143	79/0	29/0	0/0	0/0	35/0	0/0
R	227	131/0	42/0	0/0	0/0	55/0	0/0
P	227	131/0	42/0	0/0	0/0	55/0	0/0
M	143	79/0	29/0	0/0	0/0	35/0	0/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, K, Q, S, U, O, L, T, R, P, M

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

1 LATERAL BRACE(S) AT 1/2 LENGTH OF F-Q.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX CSI (LC)	
FR-TO		FROM TO			FR-TO			
A-W	0/17	-102.1	-102.1	0.04 (2)	10.00	Q-F	-247/0	0.12 (1)
W-B	0/81	-102.1	-102.1	0.13 (1)	10.00	S-D	-203/0	0.10 (1)
B-C	0/34	-102.1	-102.1	0.13 (1)	10.00	U-B	-313/0	0.05 (1)
C-D	0/60	-102.1	-102.1	0.08 (1)	10.00	O-H	-203/0	0.10 (1)
D-E	0/63	-102.1	-102.1	0.07 (1)	10.00	L-J	-313/0	0.05 (1)
E-F	0/51	-102.1	-102.1	0.07 (1)	10.00	T-C	-160/0	0.04 (1)
F-G	0/51	-102.1	-102.1	0.07 (1)	10.00	R-E	-233/0	0.21 (1)
G-H	0/63	-102.1	-102.1	0.07 (1)	10.00	P-G	-233/0	0.21 (1)
H-I	0/60	-102.1	-102.1	0.06 (1)	10.00	M-I	-160/0	0.04 (1)
I-J	0/34	-102.1	-102.1	0.13 (1)	10.00	V-W	-88/39	0.00 (1)
J-K	0/61	-102.1	-102.1	0.13 (1)	10.00	X-Y	-88/39	0.00 (1)
Y-K	0/17	-102.1	-102.1	0.04 (2)	10.00			
A-V	-52/0	-38.5	-38.5	0.11 (1)	6.25			
V-U	-32/0	-38.5	-38.5	0.11 (1)	6.25			
U-T	-41/0	-38.5	-38.5	0.09 (1)	6.25			
T-S	-45/0	-38.5	-38.5	0.03 (2)	6.25			
S-R	-48/0	-38.5	-38.5	0.03 (2)	6.25			
R-Q	-51/0	-38.5	-38.5	0.02 (3)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. G/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, OBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.13/1.00 (J-Y:1), BC=0.11/1.00 (J-X:1), WB=0.21/1.00 (G-P:1), SS=0.12/1.00 (J-Y:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 768 1987 1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP=0.40 (G) (INPUT = 0.90)
JSI METAL=0.17 (J) (INPUT = 1.00)

WWS NO. TAM 77905723
STRUCTURAL
C. LAMBERT ONLY 1/2

CONTINUED ON PAGE 2

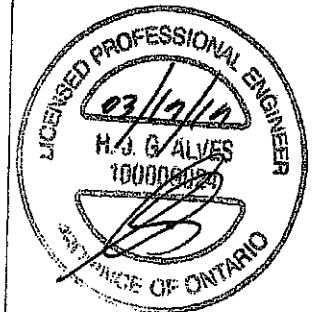


JOB NAME 401812	TRUSS NAME V10	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington				TRUSS DESC.	

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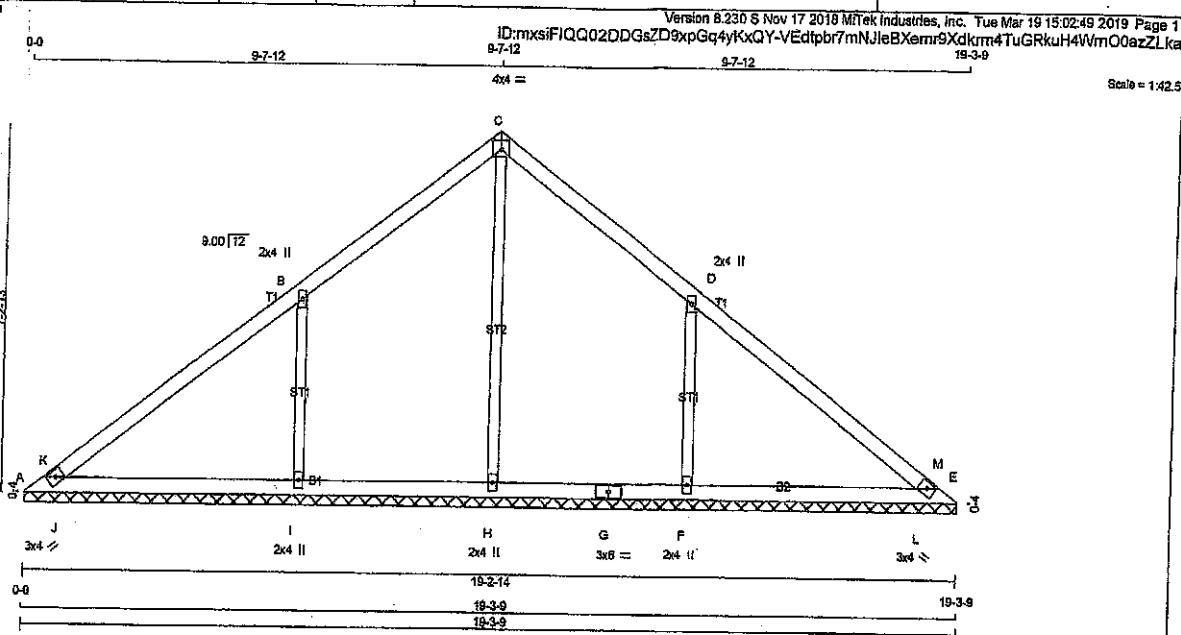
LOADING
TOTAL LOAD CASES: (4)

CHORDS					WEBS				
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX. (LC)	MAX. UNBRAC LENGTH
FR-TO		FROM	TO		FR-TO		FROM	TO	
Q-P	-51 / 0	-38.5	-38.5	0.02 (3)	6.25				
P-O	-48 / 0	-38.5	-38.5	0.03 (2)	6.25				
O-N	-45 / 0	-38.5	-38.5	0.03 (2)	6.25				
N-M	-45 / 0	-38.5	-38.5	0.03 (2)	6.25				
M-L	-41 / 0	-38.5	-38.5	0.09 (1)	6.25				
L-X	-32 / 0	-38.5	-38.5	0.11 (1)	6.25				
X-K	-52 / 0	-38.5	-38.5	0.11 (1)	6.25				



ENGINEER: H. J. G. ALVES 77905723
STRUCTURAL
CLIENTS ONLY 72

JOB NAME 401812	TRUSS NAME V11	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO.
Tamarack Roof Truss, Burlington		TRUSS DESC.		Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 15:02:49 2019 Page 1	



TOTAL WEIGHT = 60 lb

LUMBER
N. L. G. A. RULES
CHORDS SIZE
A - C 2x4 DRY
C - E 2x4 DRY
G - E 2x4 DRY

LUMBER
No.2
No.2
No.2

DESCR.
SPF
SPF
SPF

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0	
B	TBM1-w	MT20	2.0	4.0	
C	TTW-p	MT20	4.0	4.0	2.25 2.00
D	TBM1-w	MT20	2.0	4.0	
E	TBM1-h	MT20	3.0	4.0	
F, H, I					
F	BMW1+w	MT20	2.0	4.0	
G	BS-k	MT20	3.0	6.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	146	0	146	0	19-2-14 (7-3-2) 9-2-14	
E	146	0	146	0	19-2-14 (7-3-2) 9-2-14	
H	756	0	756	0	19-2-14 (7-3-2) 9-2-14	
I	829	0	829	0	19-2-14 (7-3-2) 9-2-14	
F	829	0	829	0	19-2-14 (7-3-2) 9-2-14	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST CASE	MAX/MIN COMPONENT REACTIONS					
		SNOW	LIVE	PERM. LIVE	WIND	DEAD	SOIL
A	108	88 / 0	16 / 0	0 / 0	0 / 0	24 / 0	0 / 0
E	108	88 / 0	16 / 0	0 / 0	0 / 0	24 / 0	0 / 0
H	574	290 / 0	134 / 0	0 / 0	0 / 0	150 / 0	0 / 0
I	617	347 / 0	119 / 0	0 / 0	0 / 0	151 / 0	0 / 0
F	617	347 / 0	119 / 0	0 / 0	0 / 0	151 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, H, I, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1 MAX CSI (LC)	MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX FACTORED CSI (LC)
FR-TO		FROM TO			FR-TO		
A-K	0 / 281	-102.1 -102.1	0.09 (3)	10.00	H-C	-674 / 0	0.60 (1)
K-B	0 / 340	-102.1 -102.1	0.42 (1)	10.00	I-B	-565 / 0	0.15 (1)
B-C	0 / 284	-102.1 -102.1	0.41 (1)	10.00	F-D	-565 / 0	0.15 (1)
C-D	0 / 284	-102.1 -102.1	0.41 (1)	10.00	J-K	-180 / 80	0.00 (1)
D-M	0 / 340	-102.1 -102.1	0.42 (1)	10.00	L-M	-180 / 80	0.00 (1)
M-E	0 / 281	-102.1 -102.1	0.08 (3)	10.00			
A-J	-287 / 0	-38.5 -38.5	0.18 (1)	6.25			
J-I	-246 / 0	-38.5 -38.5	0.22 (2)	6.25			
I-H	-259 / 0	-38.5 -38.5	0.21 (2)	6.25			
H-G	-259 / 0	-38.5 -38.5	0.21 (2)	6.25			
G-F	-258 / 0	-38.5 -38.5	0.21 (2)	6.25			
F-L	-246 / 0	-38.5 -38.5	0.22 (2)	6.25			
L-E	-287 / 0	-38.5 -38.5	0.18 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(85 % OF 37.8 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.42/1.00 (B-K-1), BC=0.22/1.00 (L-1/2), WB=0.60/1.00 (C-H-1), SSI=0.19/1.00 (B-K-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

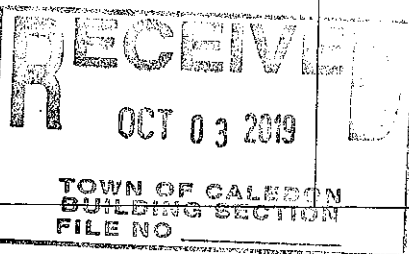
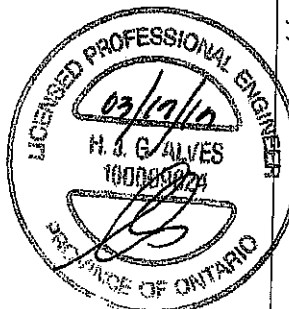
NAIL VALUES

PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1867 788 1987 1858

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.60 (C) (INPUT = 0.90)
JSI METAL= 0.30 (D) (INPUT = 1.00)



OWNER: TAM 11905724
STRUCTURAL
CONCRETE ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	V12	1	1	TRUSS DESC.		

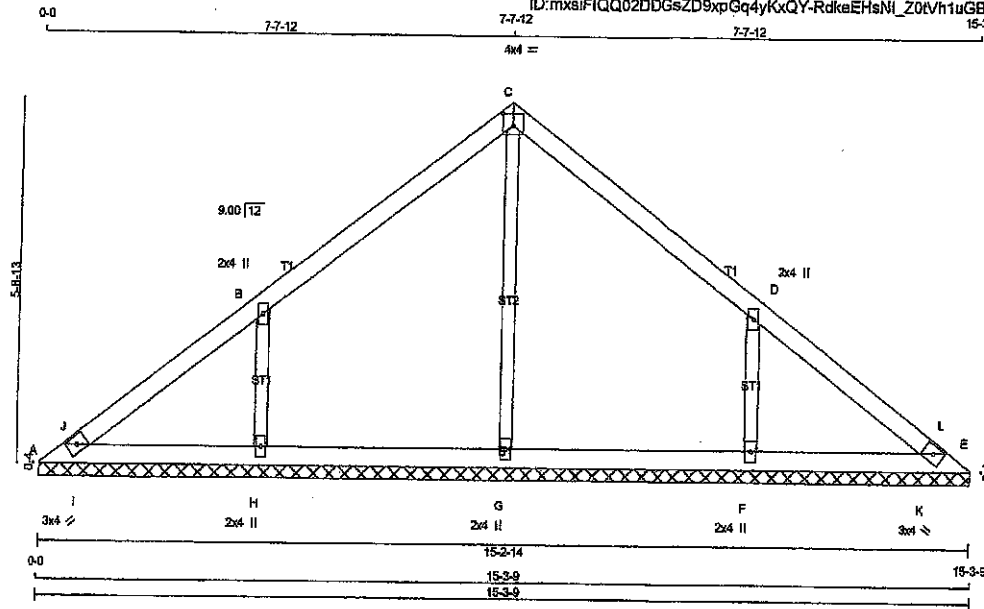
Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 MITek Industries, Inc. Tue Mar 19 15:02:51 2019 Page 1

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15-3-9

Scale = 1:33.0



TOTAL WEIGHT = 46 lb

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	DRY	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
A - E	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
DRY: SEASONED LUMBER.				

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
A	175	0	175	0	15-2-14	15-2-14
E	175	0	175	0	15-2-14	15-2-14
G	512	0	512	0	15-2-14	15-2-14
H	641	0	641	0	15-2-14	15-2-14
F	641	0	641	0	15-2-14	15-2-14

UNFACTORED REACTIONS

JT	1ST LOASE COMBINED	MAX./MIN. COMPONENT REACTIONS					SOIL
		SNOW	LIVE	PERM.LIVE	WIND	DEAD	
A	128	78 / 0	20 / 0	0 / 0	0 / 0	30 / 0	0 / 0
E	128	78 / 0	20 / 0	0 / 0	0 / 0	30 / 0	0 / 0
G	384	182 / 0	104 / 0	0 / 0	0 / 0	107 / 0	0 / 0
H	475	273 / 0	87 / 0	0 / 0	0 / 0	115 / 0	0 / 0
F	475	273 / 0	87 / 0	0 / 0	0 / 0	115 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1			MAX. UNBRAC LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
		FROM	TO	CSI (LC)				
FR-TO								
A-J	-14 / 17	-102.1	-102.1	0.05 (1)	6.25	G-C	-389 / 0	0.18 (1)
J-B	0 / 82	-102.1	-102.1	0.25 (1)	10.00	H-B	-481 / 0	0.08 (1)
B-C	0 / 36	-102.1	-102.1	0.25 (1)	10.00	F-D	-481 / 0	0.08 (1)
C-D	0 / 36	-102.1	-102.1	0.25 (1)	10.00	I-J	-48 / 36	0.00 (1)
D-L	0 / 62	-102.1	-102.1	0.25 (1)	10.00	K-L	-48 / 36	0.00 (1)
L-E	-14 / 17	-102.1	-102.1	0.05 (1)	6.25			
A-I	-34 / 0	-38.5	-38.5	0.07 (1)	6.25			
I-H	-30 / 0	-38.5	-38.5	0.10 (2)	8.25			
H-G	-33 / 0	-38.5	-38.5	0.10 (2)	8.25			
G-F	-33 / 0	-38.5	-38.5	0.10 (2)	8.25			
F-K	-30 / 0	-38.5	-38.5	0.10 (2)	8.25			
K-E	-34 / 0	-38.5	-38.5	0.07 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 8.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2018, CBC 2012
- CSA 086-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.25/1.00 (D-L:1), BC=0.10/1.00 (F-G:2), WB=0.18/1.00 (C-G:1), SSI=0.15/1.00 (C-D:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

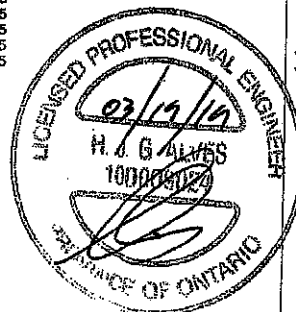
NAIL VALUES

PLATE GRIP(DRY)	SHEAR (PSI)	SECTION (PL)
MT20	618	354
	1557	788
	1987	1656

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP = 0.33 (B) (INPUT = 0.50)
JSI METAL = 0.24 (D) (INPUT = 1.00)



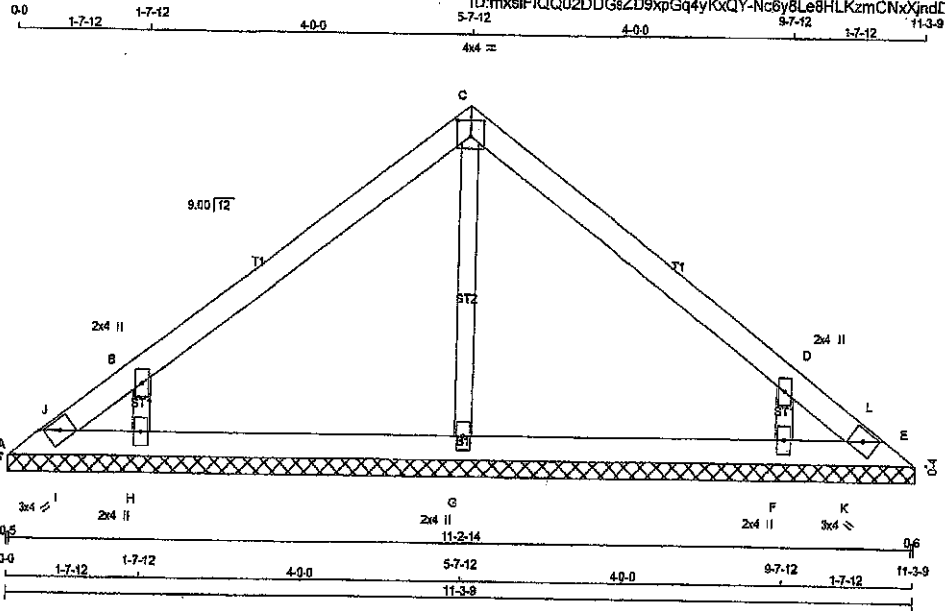
ENG. NO. TAM 71905725
STRUCTURAL
CL. 2 - 1985/1989

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	V13	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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Scale = 1/2\"

TOTAL WEIGHT = 32 lb

LUMBER

N. L. G. A. RULES	SIZE	LUMBER	DESCR.
CHORDS			
A - C	2x4	DRY	No.2
C - E	2x4	DRY	No.2
A - E	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
A	TBM1-h	MT20	3.0	4.0		
B	TWw-w	MT20	2.0	4.0		
C	TTW-p	MT20	4.0	4.0	2.25	2.00
D	TWw-w	MT20	2.0	4.0		
E	TBM1-h	MT20	3.0	4.0		
F, G, H						
F	BMW1-w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQRD BRG IN-SX
	VERT	HORZ	DOWN	HORZ		
A	43	0	43	0	11-3-9 (11-2-11)3-9	
E	43	0	43	0	11-3-9 (11-2-11)3-9	
G	394	0	394	0	11-3-9 (11-2-11)3-9	
H	550	0	550	0	11-3-9 (11-2-11)3-9	
F	550	0	550	0	11-3-9 (11-2-11)3-9	

VALUE IN PARENTHESIS INDICATES EFFECTIVE BEARING LENGTH

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS			
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD
A	32	19/0	5/0	0/0	0/0	7/0
E	32	19/0	5/0	0/0	0/0	7/0
G	305	134/0	88/0	0/0	0/0	85/0
H	406	240/0	70/0	0/0	0/0	95/0
F	406	240/0	70/0	0/0	0/0	95/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, E, G, H, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD LC1 (PLF)	MAX. CS1 (LC)	MAX. UNBRAC LENGTH FR-TO	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CS1 (LC)
FR-TO		FROM TO			FR-TO		
A-J	-135 / 0	-102.1 -102.1	0.01 (3)	6.25	G-C	-224 / 0	0.06 (1)
J-B	-26 / 0	-102.1 -102.1	0.23 (1)	8.25	H-B	-468 / 0	0.07 (1)
B-C	-116 / 0	-102.1 -102.1	0.24 (1)	8.25	F-D	-488 / 0	0.07 (1)
C-D	-116 / 0	-102.1 -102.1	0.24 (1)	8.25	I-J	-4 / 101	0.00 (1)
D-L	-26 / 0	-102.1 -102.1	0.23 (1)	8.25	K-L	-4 / 101	0.00 (1)
L-E	-135 / 0	-102.1 -102.1	0.01 (3)	8.25			
A-I	0 / 132	-38.5 -38.5	0.06 (1)	10.00			
J-H	0 / 121	-38.5 -38.5	0.07 (3)	10.00			
H-G	0 / 73	-38.5 -38.5	0.11 (2)	10.00			
G-F	0 / 73	-38.5 -38.5	0.11 (2)	10.00			
F-K	0 / 121	-38.5 -38.5	0.07 (3)	10.00			
K-E	0 / 132	-38.5 -38.5	0.06 (1)	10.00			

A circular seal with the word "PROFESSIONAL" curved along the bottom edge. The center of the seal is partially obscured by the table's content.

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, CBC 2012
- CSA 086-09, CSA 085-14
- TPIC 2011, TPIC 2014

(55% OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.24/1.00 (B-C:1), BC=0.11/1.00 (F-G:2),
WB=0.07/1.00 (B-H:1), SSI=0.16/1.00 (B-J:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

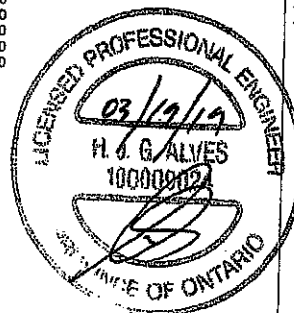
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1687 788 1987 1856

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.34 (D) (INPUT = 0.90)
JSI METAL= 0.25 (B) (INPUT = 1.00)



DESIGN TAM 71905726
STRUCTURAL
FOR PERMIT ONLY

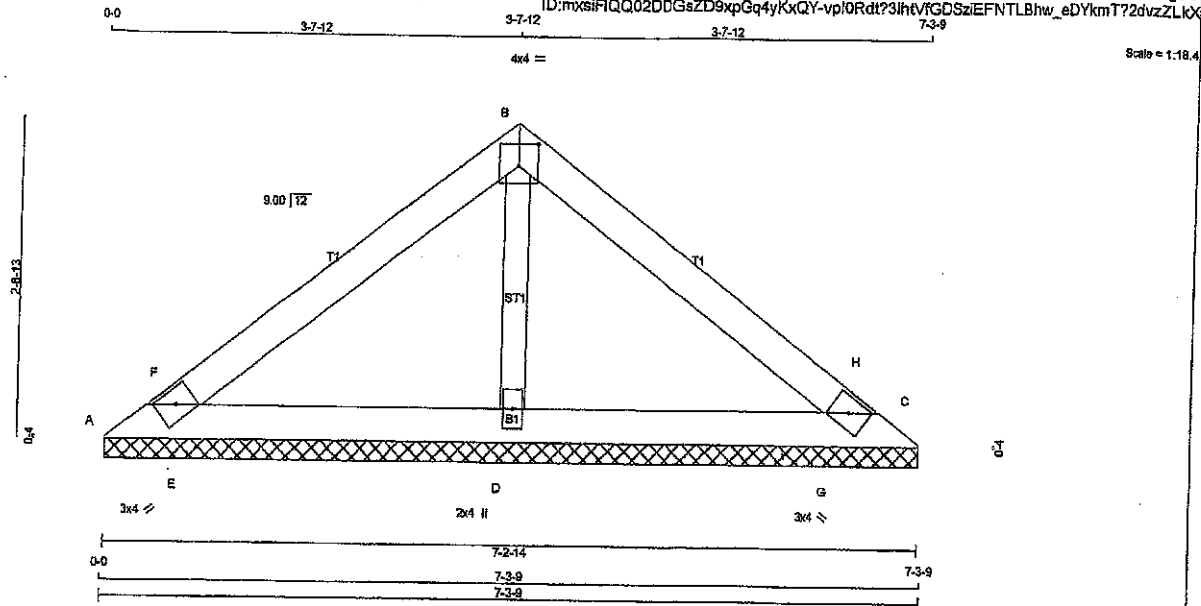
THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401812	V14	1	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-vp/0Rdt?3IhtVFGDSzEFNTLBhw_eDYkmT72dvzZLKX



Scale = 1:18.4

LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER
A - B	2x4	DRY
B - C	2x4	DRY
A - C	2x4	DRY

ALL WEBS 2x3 DRY
DRY: SEASONED LUMBER.

CHORDS	SIZE	LUMBER
A - B	2x4	DRY
B - C	2x4	DRY
A - C	2x4	DRY

CHORDS	SIZE	LUMBER
A - B	2x4	DRY
B - C	2x4	DRY
A - C	2x4	DRY

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
A	88	0	88	0	7-2-14	7-2-14
C	88	0	88	0	7-2-14	7-2-14
D	841	0	841	0	7-2-14	7-2-14

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
A	66	37.0	12.0	0.0	0.0	16.0	0.0
C	66	37.0	12.0	0.0	0.0	16.0	0.0
D	629	345.0	128.0	0.0	0.0	156.0	0.0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) A, C, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 6.25 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. UNBRACED LENGTH	MEMB.	WEBS	
	MAX. FACTORED FORCE (LBS)	VERT. LOAD LC1 (PLF)	TO	FROM			MAX. FACTORED FORCE (LBS)	MAX. (LC)
A-F	0 / 244	-102.1	-102.1	0.07 (1)	10.00	D-B	-587 / 0	0.10 (1)
F-B	0 / 251	-102.1	-102.1	0.17 (1)	10.00	E-F	-176 / 15	0.00 (1)
B-H	0 / 251	-102.1	-102.1	0.17 (1)	10.00	G-H	-176 / 15	0.00 (1)
H-C	0 / 244	-102.1	-102.1	0.07 (1)	10.00			
A-E	-234 / 0	-38.5	-38.5	0.13 (1)	6.25			
E-D	-204 / 0	-38.5	-38.5	0.15 (1)	6.25			
D-G	-204 / 0	-38.5	-38.5	0.15 (1)	6.25			
G-C	-234 / 0	-38.5	-38.5	0.13 (1)	6.25			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF CBC 2016, CBC 2012
- CSA 086-09, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.17/1.00 (B-F:1), BC=0.15/1.00 (D-E:1), WB=0.10/1.00 (E-D:1), SSI=0.11/1.00 (C-G:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10
COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

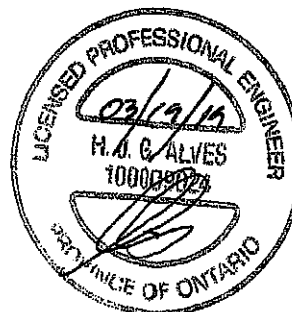
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION
(PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1657 788 1987 1658

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.53 (B) (INPUT = 0.90)
JSI METAL= 0.17 (B) (INPUT = 1.00)



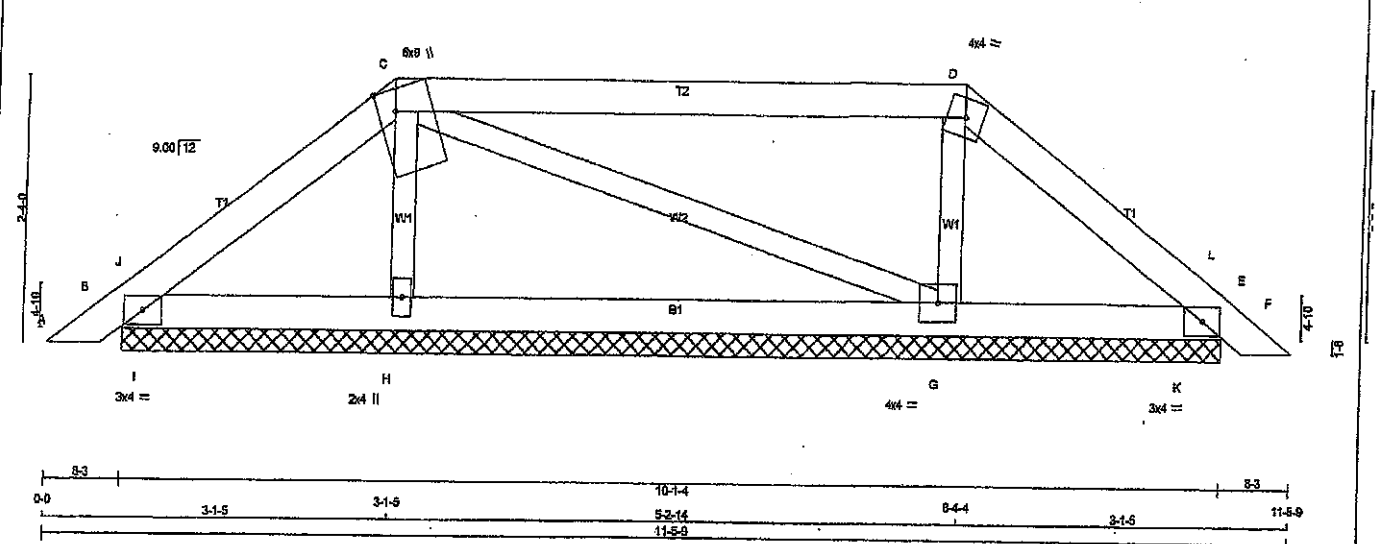
ENGINEER: 11905727
STRUCTURAL
CONSULTANT

JOB NAME 401811	TRUSS NAME PB1	QUANTITY 2	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. PB1
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Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:17 2018 Page 1
ID:mxsiFIQQ02DDGsZD9xpGq4yKxQY-52nW7CcX_th76FS8W3usUvKaansWNFWnVOtczZMcO

Scale = 1:18.8



LUMBER

N. L. G. A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	SPF
C - D	2x4	DRY	SPF
D - F	2x4	DRY	SPF
B - E	2x4	DRY	SPF
ALL WEBS	2x3	DRY	SPF
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0	
C	TTWW+m	MT20	6.0	9.0	Edge 2.00
D	TTWW-m	MT20	4.0	4.0	
E	TMB1-I	MT20	3.0	4.0	
G	BNWW1-t	MT20	4.0	4.0	
H	BNWW1-w	MT20	2.0	4.0	

Edge - INDICATES REFERENCE CORNER OF PLATE TOUCHES EDGE OF CHORD.

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

	FACTORED GROSS REACTION	MAXIMUM FACTORED GROSS REACTION	INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ
JT				
B	270	0	270	0
E	258	0	258	0
H	494	0	494	0
G	521	0	521	0

UNFACTORED REACTIONS

	1ST LOASE COMBINED	MAX/MIN. COMPONENT REACTIONS	PERM. LIVE	WIND	DEAD	SOIL
		SNOW	LIVE			
JT						
B	150	142/0	11/0	0/0	0/0	37/0
E	181	134/0	11/0	0/0	0/0	35/0
H	378	182/0	95/0	0/0	0/0	101/0
G	396	198/0	95/0	0/0	0/0	104/0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PLAIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	MAX. UNBRACED LENGTH (LC)	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. UNBRACED LENGTH (LC)	
FR-TO				FR-TO			
A-B	0/17	-102.1	-102.1 0.03 (1)	10.00	H-C	-298/0	0.05 (1)
B-J	-63/0	-102.1	-102.1 0.03 (1)	8.25	C-G	-20/0	0.01 (1)
J-C	-111/0	-102.1	-102.1 0.07 (1)	6.25	G-D	-319/0	0.05 (1)
C-D	-49/0	-102.1	-102.1 0.48 (1)	6.25	I-J	-194/0	0.00 (1)
D-L	-69/0	-102.1	-102.1 0.07 (1)	6.25	K-L	-198/0	0.00 (1)
L-E	-39/1	-102.1	-102.1 0.03 (1)	6.25			
E-F	0/17	-102.1	-102.1 0.03 (1)	10.00			
B-I	0/84	-38.5	-38.5 0.08 (1)	10.00			
I-H	0/84	-38.5	-38.5 0.13 (2)	10.00			
H-G	0/67	-38.5	-38.5 0.13 (2)	10.00			
G-K	0/68	-38.5	-38.5 0.13 (2)	10.00			
K-E	0/68	-38.5	-38.5 0.08 (1)	10.00			

TOTAL WEIGHT = 2 X 33 = 66 lb

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL	= 29.0 PSF
DL	= 6.0 PSF
BOT CH. LL	= 10.5 PSF
DL	= 7.0 PSF
TOTAL LOAD	= 52.5 PSF

SPACING = 24.0 IN/C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBCC 2015
- CSA 088-14
- TPC 2014

(55% OF 37.8 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.48/1.00 (C-D:1), BC=0.13/1.00 (H:2), WB=0.05/1.00 (D-G:1), SSI=0.21/1.00 (C-D:1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

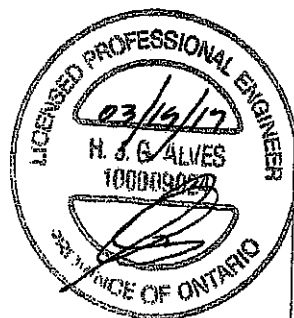
NAIL VALUES

PLATE GRIP (DRY)	SHEAR (PSI)	SECTION (PL)
MAX	MIN	MAX
MIN	MIN	MIN
MT20	650	371
	1747	788
	1987	1873

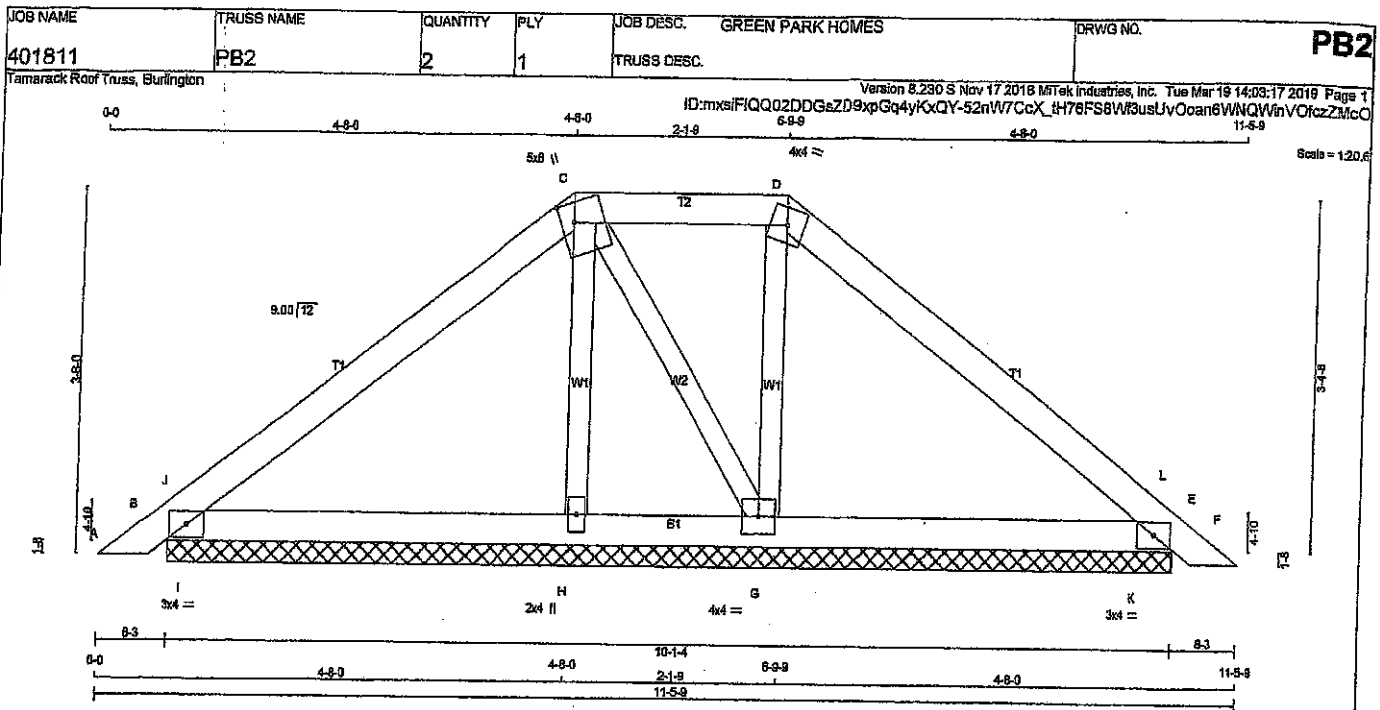
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 6.0 Deg.

JSI GRIP= 0.22 (D) (INPUT = 0.90)
JSI METAL= 0.06 (H) (INPUT = 1.00)



DRWG NO. TAM 17905705
STRUCTURAL
CHECKED: [Signature]



LUMBER
N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2
C - D	2x4	DRY	No.2
D - F	2x4	DRY	No.2
B - E	2x4	DRY	No.2

ALL WEBS 2x3 DRY No.2
DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TMB14	MT20	3.0	4.0	
C	TTWW+m	MT20	5.0	8.0	2.25 1.50
D	TTWW+m	MT20	4.0	4.0	
E	TMB14	MT20	3.0	4.0	
G	BMWW14	MT20	4.0	4.0	
H	BMWW14	MT20	2.0	4.0	

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	439	0	439	0	10-1-4	10-1-4
E	418	0	418	0	10-1-4	10-1-4
H	285	0	285	0	10-1-4	10-1-4
G	399	0	399	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN. COMPONENT REACTIONS		WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE	PERM. LIVE			
B	320	202 / 0	46 / 0	0 / 0	0 / 0	72 / 0	0 / 0
E	308	169 / 0	46 / 0	0 / 0	0 / 0	70 / 0	0 / 0
H	221	98 / 0	62 / 0	0 / 0	0 / 0	81 / 0	0 / 0
G	288	166 / 0	59 / 0	0 / 0	0 / 0	73 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, E, H, G

BRACING
TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 8.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING
TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	WEBS		MAX. FACTORED
	FORCE (LBS)	VERT. LOAD (LBS)	LC1 (PLF)	MAX. (LBS)		MEMB. FORCE (LBS)	MAX. (LBS)	
FR-TO						FR-TO		
A-B	0 / 17	-102.1	-102.1	0.03 (1)	10.00	H-C	-125 / 0	0.02 (1)
B-J	-122 / 0	-102.1	-102.1	0.07 (1)	8.25	C-G	-55 / 0	0.01 (1)
J-C	-197 / 0	-102.1	-102.1	0.21 (1)	6.25	G-D	-192 / 0	0.04 (1)
C-D	-116 / 0	-102.1	-102.1	0.08 (1)	8.25	I-J	-373 / 71	0.00 (1)
D-L	-162 / 0	-102.1	-102.1	0.21 (1)	6.25	K-L	-377 / 70	0.00 (1)
L-E	-118 / 0	-102.1	-102.1	0.07 (1)	8.25			
E-F	0 / 17	-102.1	-102.1	0.03 (1)	10.00			
B-I	0 / 151	-38.5	-38.5	0.17 (1)	10.00			
I-H	0 / 151	-38.5	-38.5	0.18 (1)	10.00			
H-G	0 / 147	-38.5	-38.5	0.14 (1)	10.00			
G-K	0 / 123	-38.5	-38.5	0.18 (1)	10.00			
K-E	0 / 123	-38.5	-38.5	0.17 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C

LOADING IN FLAT SECTION BASED ON A SLOPE OF 6.00/12

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(55% OF 37.5 P.S.F. G.S.L. PLUS 6.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.21/1.00 (C-J-1), BC=0.18/1.00 (H-I-1), WB=0.04/1.00 (D-G-1), SS=0.32/1.00 (E-K-1)

DOL LUMBER=1.00 NAIL=1.00 LB BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

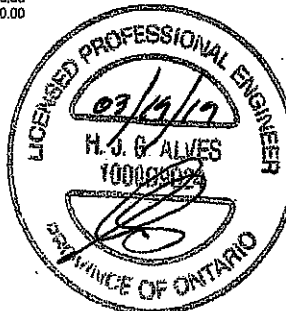
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PLI) (PLI)
MAX MIN MAX MIN MAX MIN
MT20 650 371 1747 788 1987 1873

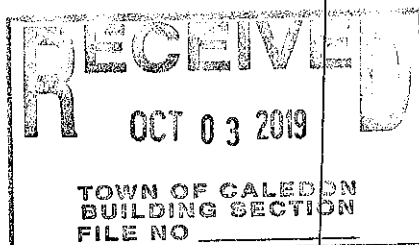
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.32 (B) (INPUT = 0.90)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DWG NO. 1AM T1905706
STRUCTURAL
COMPONENT ONLY



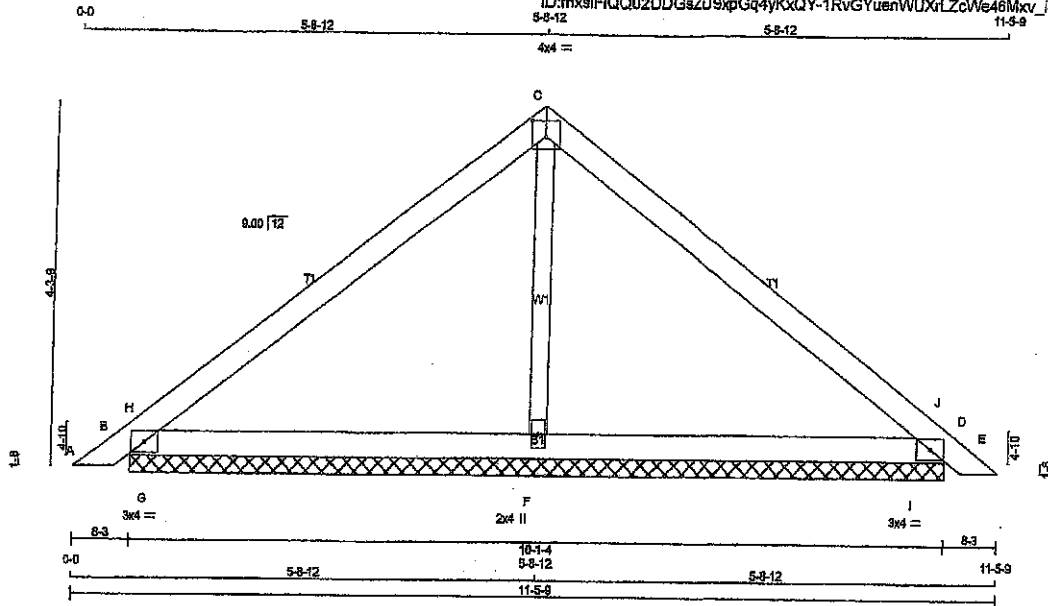
JOB NAME 401811	TRUSS NAME PB3	QUANTITY 1	PLY 1	JOB DESC. GREEN PARK HOMES	DRWG NO. PB3
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Tamarack Roof Truss, Burlington

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ID:mxslFIQQ02DDGsZD9xpGq4yKxQY-1RvGYuenWUXrLZcWe46Mxv_ISNQL_G_p95_UjVzZMc1d

Scale = 1:25.4



TOTAL WEIGHT = 31 lb

LUMBER

N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.
A - C	2x4	DRY	No.2	SPF
C - E	2x4	DRY	No.2	SPF
B - D	2x4	DRY	No.2	SPF
ALL WEBS	2x3	DRY	No.2	SPF
				SPF

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMB1-I	MT20	3.0	4.0		
C	YTW-p	MT20	4.0	4.0	2.25	2.00
D	TMB1-I	MT20	3.0	4.0		
F	BMW1+w	MT20	2.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REQD BRG
	VERT	HORZ	DOWN	HORZ		
B	528	0	528	0	10-1-4	10-1-4
D	528	0	528	0	10-1-4	10-1-4
F	485	0	485	0	10-1-4	10-1-4

UNFACTORED REACTIONS

JT	1ST LOASE		MAX./MIN. COMPONENT REACTIONS				
	COMBINED	SNOW	LIVE	PERM.LIVE	WIND	DEAD	SOIL
B	384	246 / 0	53 / 0	0 / 0	0 / 0	86 / 0	0 / 0
D	384	246 / 0	53 / 0	0 / 0	0 / 0	86 / 0	0 / 0
F	377	164 / 0	107 / 0	0 / 0	0 / 0	105 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D, F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT. OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS				WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	FACTORED LC1 (LC)	MAX. UNBRAC	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. FACTORED CSI (LC)
FR-TO		FROM	TO	LENGTH	FR-TO		
A-B	0 / 17	-102.1	-102.1	0.03 (1)	F-C	-132 / 10	0.03 (1)
B-H	-128 / 25	-102.1	-102.1	0.17 (1)	G-H	-883 / 45	0.00 (1)
H-C	-270 / 0	-102.1	-102.1	0.33 (1)	I-J	-883 / 45	0.00 (1)
C-J	-270 / 0	-102.1	-102.1	0.33 (1)			
J-D	-128 / 25	-102.1	-102.1	0.17 (1)			
D-E	0 / 17	-102.1	-102.1	0.03 (1)			
B-G	0 / 204	-38.5	-38.5	0.29 (1)			
G-F	0 / 204	-38.5	-38.5	0.33 (1)			
F-I	0 / 204	-38.5	-38.5	0.33 (1)			
I-D	0 / 204	-38.5	-38.5	0.29 (1)			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 29.0 PSF
DL = 6.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDINGS REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

CSI: TC=0.33/1.00 (C-J-1), BC=0.33/1.00 (F-G-1), WB=0.03/1.00 (G-F-1), SS=0.55/1.00 (D-I-1)

DCL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

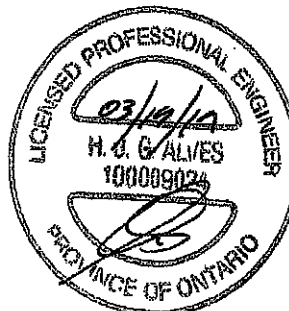
NAIL VALUES

PLATE	GRIP(DRY)	SHEAR	SECTION
	(FBI)	(PLI)	(PLI)
MT20	650	371	1747 788 1987 1873

PLATE PLACEMENT TOL = 0.250 inches

PLATE ROTATION TOL = 5.0 Deg.

JSI GRIP= 0.41 (D) (INPUT = 0.90)
JSI METAL= 0.11 (B) (INPUT = 1.00)



DRWG NO. TAM 71905707
STRUCTURAL
COMPONENT ONLY

JOB NAME
401811

TRUSS NAME
J7

QUANTITY
10

PLY
1

JOB DESC.
GREEN PARK HOMES

TRUSS DESC.

DRWG NO.
J7

Tamarack Roof Truss, Burlington

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ID:mxsiFIQQ02DDGsZD9xpGq4yFkxQY-gT9NVAAfhyVYFokZrXWBErHnSMjQJ0G47pGj2HzZMcR

Scale = 1/28.7

TOTAL WEIGHT = 10 X 19 = 186 lb

LUMBER

N.L.G.A. RULES

CHORDS	SIZE	LUMBER	DESCR.
F - B	2x4	DRY	No.2
A - C	2x4	DRY	No.2
F - D	2x4	DRY	No.2
ALL WEBS	2x3	DRY	No.2
DRY: SEASONED LUMBER.			

PLATES (table is in inches)

JT	TYPE	PLATES	W	LEN	Y	X
B	TMVW-H	MT20	4.0	4.0	2.00	1.25
E	BMVW-H	MT20	3.0	4.0		
F	BMV1+p	MT20	3.0	4.0		

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

BEARINGS

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG	REORD BRG
	VERT	HORZ	DOWN	HORZ		
F	552	0	552	0	5-8	5-8
C	300	0	300	0	1-8	1-8
D	113	0	144	0	1-8	1-8

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST LCASE		MAX/MIN COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
	COMBINED	SNOW	LIVE					
F	404	250 / 0	62 / 0	0 / 0	0 / 0	0 / 0	93 / 0	0 / 0
C	208	171 / 0	0 / 0	0 / 0	0 / 0	0 / 0	35 / 0	0 / 0
D	103	0 / 0	62 / 0	0 / 0	0 / 0	0 / 0	41 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) F

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 10.00 FT.

MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE Laterally RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

MEMB.	CHORDS		FACTORED		MAX. FACTORED	MEMB.	WEBS		MAX. FACTORED
	MAX. FORCE (LBS)	VERT. LOAD (LBS)	VERT. LOAD (LBS)	PLF (LBS)			MAX. FORCE (LBS)	MAX. FORCE (LBS)	
FR-TO						FR-TO			
F-B	-439 / 0	0.0	0.0	0.04 (1)	7.81	B-E	0 / 0	0.00 (1)	
A-B	0 / 35	-102.1	-102.1	0.14 (1)	10.00				
B-C	0 / 0	-102.1	-102.1	0.60 (1)	10.00				
F-E	0 / 0	-38.5	-38.5	0.24 (3)	10.00				
E-D	0 / 0	-38.5	-38.5	0.31 (3)	10.00				

DESIGN CRITERIA

SPECIFIED LOADS:

TOP CH. LL = 29.0 PSF

DL = 6.0 PSF

BOT CH. LL = 10.5 PSF

DL = 7.0 PSF

TOTAL LOAD = 52.5 PSF

SPACING = 24.0 IN. O.C.

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2015

THIS DESIGN COMPLIES WITH:

- PART 9 OF NBC 2015
- CSA 086-14
- TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = 1/360 (0.20")

CALCULATED VERT. DEFL.(LL) = 1/825 (0.08")

ALLOWABLE DEFL.(TL) = 1/360 (0.20")

CALCULATED VERT. DEFL.(TL) = 1/593 (0.13")

CS1: TC=0.60/1.00 (B-C-1), BC=0.31/1.00 (D-E-3), WB=0.00/1.00 (B-E-1), SS=0.20/1.00 (B-C-1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

AUTOSOLVE RIGHT HEEL ONLY

TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES

PLATE	GRIP(DRY)	SHEAR (PSI)	SECTION (PLI)
MT20	650	371	1747
	788	1957	1873

PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.90)

JSI METAL= 0.06 (B) (INPUT = 1.00)

RECEIVED

OCT 03 2019

TOWN OF CALEDON
BUILDING SECTION
FILE NO

DRWG NO. TAM 17905713

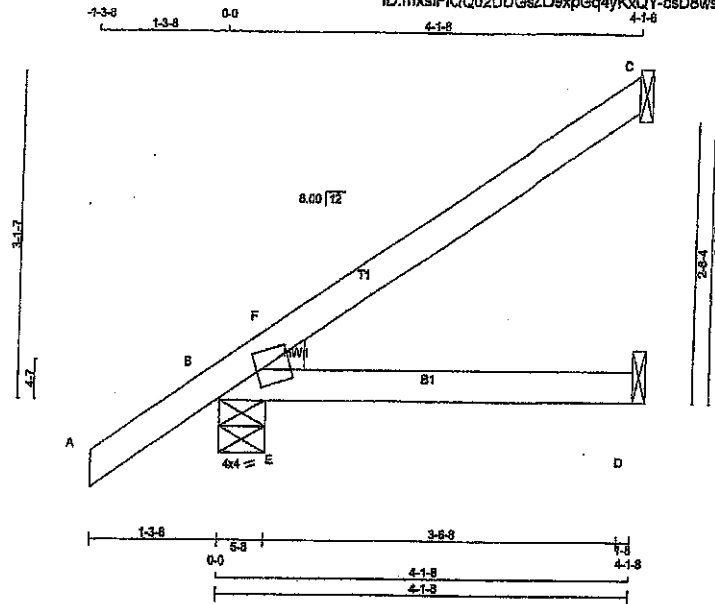
STRUCTURAL
COMPONENT ONLY

JOB NAME	TRUSS NAME	QUANTITY	PLY	JOB DESC.	GREEN PARK HOMES	DRWG NO.
401811	J8	5	1	TRUSS DESC.		

Tamarack Roof Truss, Burlington

Version 8.230 S Nov 17 2018 Mitek Industries, Inc. Tue Mar 19 14:03:16 2019 Page 1
ID: mxsiFIQ02DDGsdZDxpGq4yKxQY-osD8wscvDZ9GU5bxyVfJGMdNqenwmNT7lq6AZMcP

Scale = 1/20.0



LUMBER	N. L. G. A. RULES	CHORDS	SIZE	LUMBER	DESCR.	SPF
A - C	2x4	DRY	No.2			
B - D	2x4	DRY	No.2			

DRY: SEASONED LUMBER.

PLATES (table is in inches)

JT TYPE	PLATES	W	LEN	Y	X
B	TM8H1-m	MT20	4.0	4.0	2.00 0.50

DIMENSIONS, SUPPORTS AND LOADINGS SPECIFIED BY FABRICATOR TO BE VERIFIED BY BUILDING DESIGNER

JT	FACTORED GROSS REACTION		MAXIMUM FACTORED GROSS REACTION		INPUT BRG IN-SX	REQD BRG IN-SX	HEEL WEDGE
	VERT	HORZ	DOWN	HORZ			
C	184	0	184	0	1-8	1-8	
B	430	0	430	0	5-8	5-8	2x4 L
D	105	0	113	0	1-8	1-8	

SEE MITEK STANDARD DETAIL B37821H FOR CONNECTION TO JOINT(S) C, D

UNFACTORED REACTIONS

JT	1ST CASE COMBINED		MAX / MIN COMPONENT REACTIONS		PERM LIVE	WIND	DEAD	SOIL
	SNOW	LIVE	SNOW	LIVE				
C	129	98 / 0	7 / 0	0 / 0	0 / 0	0 / 0	25 / 0	0 / 0
B	313	189 / 0	43 / 0	0 / 0	0 / 0	0 / 0	70 / 0	0 / 0
D	88	22 / 0	37 / 0	0 / 0	0 / 0	0 / 0	29 / 0	0 / 0

BEARING MATERIAL TO BE SPF NO.2 OR BETTER AT JOINT(S) B, D

BRACING

TOP CHORD TO BE SHEATHED OR MAX. PURLIN SPACING = 6.25 FT.
MAX. UNBRACED BOTTOM CHORD LENGTH = 10.00 FT OR RIGID CEILING DIRECTLY APPLIED.

ALL PITCH BREAKS AND PERIMETER CORNER JOINTS MUST BE LATERALLY RESTRAINED.

LOADING

TOTAL LOAD CASES: (4)

CHORDS					WEBS			
MEMB.	MAX. FACTORED FORCE (LBS)	FACTORED VERT. LOAD (PLF)	LC1	MAX. CSI (LC)	MAX. UNBRAC. LENGTH	MEMB.	MAX. FACTORED FORCE (LBS)	MAX. CSI (LC)
FR-TO		FROM	TO		FR-TO			
A-B	0 / 37	-102.1	-102.1	0.14 (1)	10.00	E-F	-171 / 62	0.00 (1)
B-F	-47 / 0	-102.1	-102.1	0.07 (5)	8.25			
F-C	0 / 8	-102.1	-102.1	0.23 (1)	10.00			
B-E	0 / 0	-38.5	-38.5	0.18 (1)	10.00			
E-D	0 / 0	-38.5	-38.5	0.20 (1)	10.00			

DESIGN CRITERIA

SPECIFIED LOADS:
TOP CH. LL = 28.0 PSF
DL = 8.0 PSF
BOT CH. LL = 10.5 PSF
DL = 7.0 PSF
TOTAL LOAD = 62.5 PSF

SPACING = 24.0 IN. C/C

THIS TRUSS IS DESIGNED FOR RESIDENTIAL OR SMALL BUILDING REQUIREMENTS OF PART 9, NBC 2010, NBCC 2015

THIS DESIGN COMPLIES WITH:
- PART 9 OF BCBC 2018, OBC 2012
- CSA 088-08, CSA 086-14
- TPIC 2011, TPIC 2014

(55 % OF 37.6 P.S.F. G.S.L. PLUS 8.4 P.S.F. RAIN LOAD) EQUALS 29.0 P.S.F. SPECIFIED ROOF LIVE LOAD

ALLOWABLE DEFL.(LL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(LL) = $L/999$ (0.03")
ALLOWABLE DEFL.(TL) = $L/360$ (0.19")
CALCULATED VERT. DEFL.(TL) = $L/598$ (0.05")

CSI: TC=0.23/1.00 (C-F:1), BC=0.20/1.00 (D-E:1), WB=0.00/1.00 (E-F:1), SSI=0.18/1.00 (B-E:1)

DOL LUMBER=1.00 NAIL=1.00 LS BEND=1.10 COMP=1.10 SHEAR=1.10 TENS=1.10

COMPANION LIVE LOAD FACTOR = 1.00

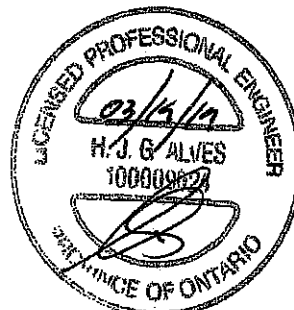
TRUSS PLATE MANUFACTURER IS NOT RESPONSIBLE FOR QUALITY CONTROL IN THE TRUSS MANUFACTURING PLANT.

NAIL VALUES
PLATE GRIP(DRY) SHEAR SECTION (PSI) (PL) (PL)
MAX MIN MAX MIN MAX MIN
MT20 618 354 1667 788 1867 1666

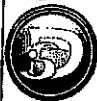
PLATE PLACEMENT TOL. = 0.250 inches

PLATE ROTATION TOL. = 5.0 Deg.

JSI GRIP= 0.26 (B) (INPUT = 0.80)
JSI METAL= 0.08 (B) (INPUT = 1.00)



DRWG NO. TAM 71905714
STRUCTURAL
COMPONENT ONLY



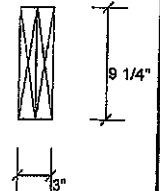
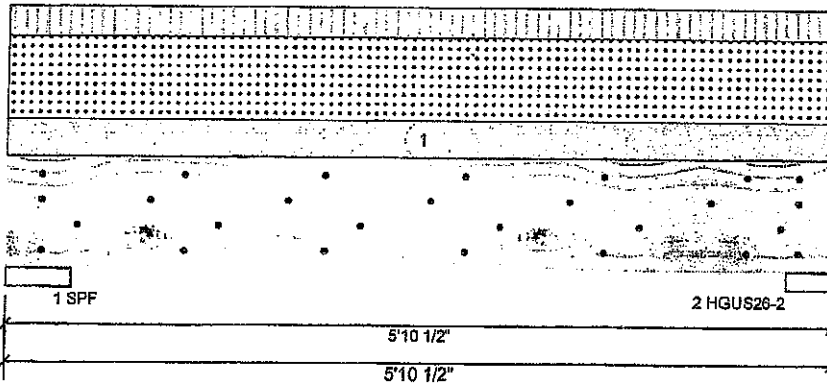
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 5 of 6

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 360
Deflection TL: 360
Importance: Normal

Application: Roof (Residential)
Slope: 0/12
Design Method: LSD
Building Code: NBCC 2015
Load Sharing: No
Deck: Not Checked
Vibration: Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	369	457	1019	0
2	353	438	976	0

Bearings and Factored Reactions

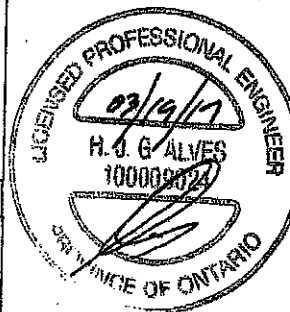
Bearing	Length	Cap. React D/L lb	Total Ld. Case	Ld. Comb.
1 - SPF	5.500"	24%	571 / 1897	2468 L
2 - HGUS...	4.000"	32%	547 / 1818	2385 L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2789 ft-lb	3'	6038 ft-lb	0.462 (46%)	1.25D+1.5S	L
Unbraced	2789 ft-lb	3'	5236 ft-lb	0.533 (53%)	1.25D+1.5S	L
Shear	2142 lb	1/2"	3984 lb	0.538 (54%)	1.25D+1.5S	L
LL Defl inch	0.024 (L/2566)	3'	0.174 (L/360)	0.140 (14%)	S+0.5L	L
TL Defl inch	0.034 (L/1860)	3'	0.174 (L/360)	0.190 (19%)	D+S+0.5L	L

Design Notes

- 1 Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



LONG RUN TAM 19405730
STRUCTURAL
COMPONENT ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		11-8-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	

Manufacturer Info

Tamareck Roof Trusses
3289 North Service Rd., ON
Canada
L7N8G2
(800) 335-1115



This design is valid until 12/11/2021





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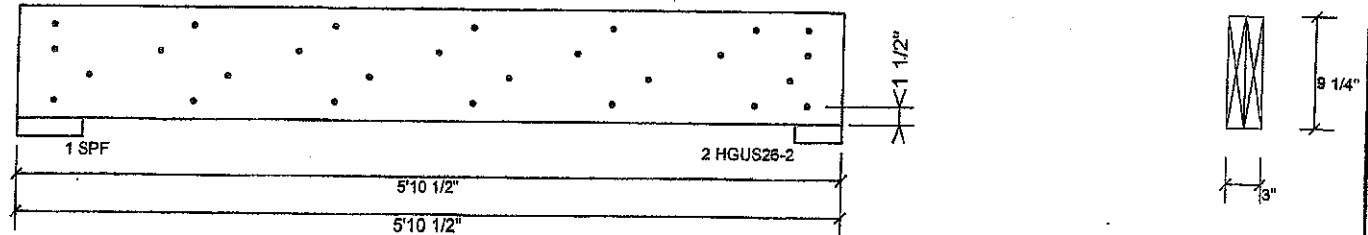
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project#:

Page 6 of 8

BM3 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

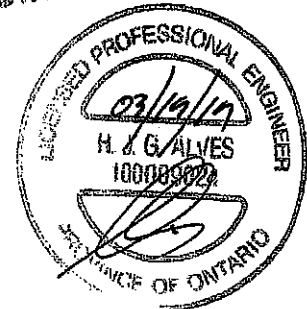


Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6"

Capacity	90.7 %
Load	411.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	9
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



DRUG ALA LAM 11905730
STRUCTURAL
CORPORATION ONLY 1/2

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115



This design is valid until 12/31/2021





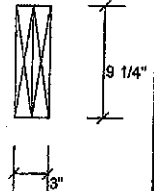
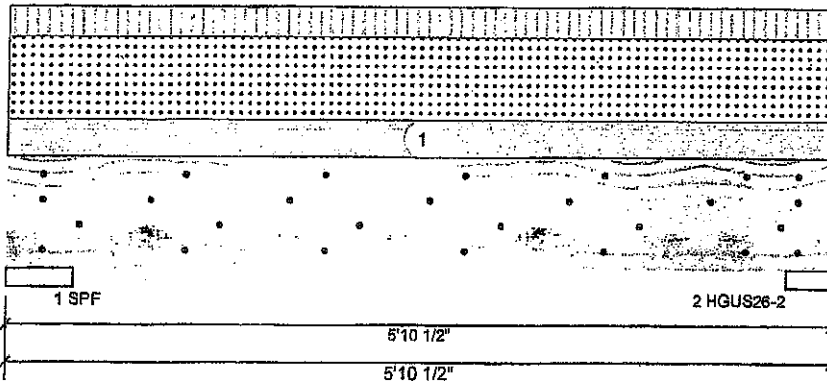
isDesign™

Client:
Project:
Address:Date: 3/19/2019
Designer:
Job Name: 200859
Project #:

Page 3 of 6

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level

**Member Information**

Type:	Girder	Application:	Roof (Residential)
Piles:	2	Slope:	0/12
Moisture Condition:	Dry	Design Method:	LSD
Deflection LL:	360	Building Code:	NBCC 2015
Deflection TL:	360	Load Sharing:	No
Importance:	Normal	Deck:	Not Checked
		Vibration:	Not Checked

Unfactored Reactions UNPATTERNED lb (Uplift)

Brg	Live	Dead	Snow	Wind
1	403	499	1113	0
2	386	478	1087	0

Bearings and Factored Reactions

Bearing	Length	Cap. React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	27% 624 / 2072	2696	L	1.25D+1.5S +L
2 - HGUS...	4.000"	35% 596 / 1986	2584	L	1.25D+1.5S +L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3047 ft-lb	3'	6039 ft-lb	0.505 (50%)	1.25D+1.5S +L	L
Unbraced	3047 ft-lb	3'	5236 ft-lb	0.582 (58%)	1.25D+1.5S +L	L
Shear	2340 lb	4'10"	3984 lb	0.587 (58%)	1.25D+1.5S +L	L
LL Defl inch	0.027 (L/2348)	3'	0.174 (L/360)	0.150 (15%)	S+0.5L	L
TL Defl inch	0.037 (L/1702)	3'	0.174 (L/360)	0.210 (21%)	D+S+0.5L	L

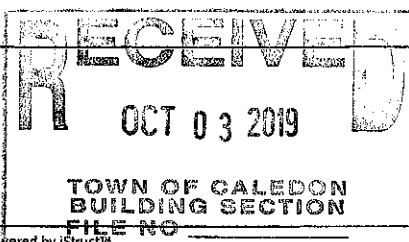
Design Notes

- 1 Fasten all piles using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c. Maximum end distance not to exceed 6".
- 2 Refer to last page of calculations for fasteners required for specified loads.
- 3 Girders are designed to be supported on the bottom edge only.
- 4 Top braced at bearings.
- 5 Bottom braced at bearings.
- 6 Lateral slenderness ratio based on single ply width.



LOUISIANA TAM 11905729
STRUCTURAL
DESIGN ONLY 1/2

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Comments
1	Uniform		12-9-8	Near Face	13 PSF	10.5 PSF	29 PSF	0 PSF	



This design is valid until 12/11/2021

Manufacturer Info

Tamarack Roof Trusses
3289 North Service Rd., ON
Canada
L7N3G2
(905) 335-1115





isDesign™

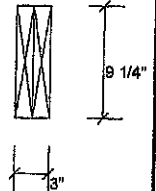
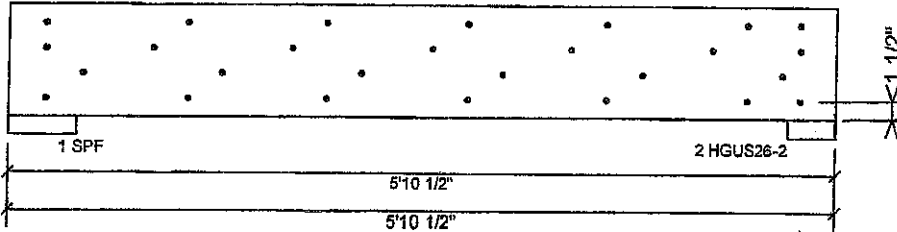
Client:
Project:
Address:

Date: 3/19/2019
Designer:
Job Name: 200659
Project #:

Page 4 of 8

BM2 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of Pneumatic Gun Nail (.120x3.25") at 12" o.c.. Maximum end distance not to exceed 6"

Capacity	99.1 %
Load	449.3 PLF
Yield Limit per Foot	453.4 PLF
Yield Limit per Fastener	113.3 lb.
Yield Mode	g
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	1.25D+1.5S+L
Duration Factor	1.00



DR. J. TAM 77907724
STRUCTURAL
ENGINEER ONLY 3/2

Manufacturer Info	Tamarack Roof Trusses 3269 North Service Rd., ON Canada L7N3G2 (805) 335-1115
	TAMARACK LUMBER INC ALUMINUM GROUP

This design is valid until 12/11/2021



LUL/LUS/LJS/HUS/HHUS/HGUS

HHUS/HGUS

See Hanger Options information on pp. 125–127.

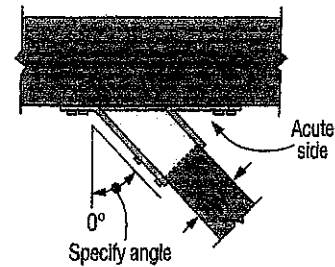
HHUS — Sloped and/or Skewed Seat

- HHUS hangers can be skewed to a maximum of 45° and/or sloped to a maximum of 45°
- For skew only, maximum factored down resistance is 0.85 of the table value
- For sloped only or sloped and skewed hangers, the maximum factored down resistance is 0.72 of the table value
- Uplift resistances for sloped/skewed conditions are 0.62 of the table value
- The joist must be bevel-cut to allow for double-shear nailing

HGUS — Skewed Seat

- HGUS hangers can be skewed only to a maximum of 45°. Factored resistances are:

HHUS Seat Width	Joist	Down Resistance	Uplift
$W < 2"$	Bevel or square cut	0.62 of table value	0.46 of table value
$2" < W < 6"$	Bevel cut	0.87 of table value	0.41 of table value
$2" < W < 6"$	Square cut	0.46 of table value	0.41 of table value
$W > 6"$	Bevel cut	0.75 of table value	0.41 of table value



**Top View HHUS Hanger
Skewed Right**
(joist must be bevel cut)
All joist nails installed on the
outside angle (non-acute side).

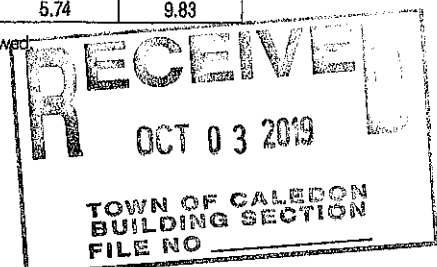
Standard and Double-Shear Joist Hangers (cont.)

These products are available with additional corrosion protection. For more information, see p. 24.

These products are approved for installation with the Strong-Drive® SD Connector screw. See pp. 32–34 for more information.

Model No.	Ga.	Dimensions (in.)				Fasteners		Factored Resistance			
		W	H	B	d _e ³	Header	Joist	D-Fit-L		S-P-F	
								Uplift (K _D = 1.15)	Normal (K _D = 1.00)	Uplift (K _D = 1.15)	Normal (K _D = 1.00)
								lb.	lb.	lb.	lb.
								kN	kN	kN	kN
Single 2x Sizes											
LUS24	18	1 1/8	3 1/2	1 1/4	2 1/4	(4) 10d	(2) 10d	710	1625	645	1155
LU24L	22	1 1/8	3	1 1/4	2 1/8	(4) 10d	(2) 10d x 1 1/2"	346	723	2.87	5.14
LU26L	22	1 1/8	5	1 1/4	4 1/4	(6) 10d	(4) 10d x 1 1/2"	360	1020	320	725
LUS26	18	1 1/8	4 1/4	1 1/4	3 1/4	(4) 10d	(4) 10d	160	454	1.42	3.22
HUS26	16	1 1/8	5 1/4	3	3 1/8	(14) 16d	(6) 16d	720	1605	645	1140
LJS26DS	18	1 1/8	5	3 1/2	4 1/4	(16) 16d	(6) 16d	320	714	2.87	5.07
HGUS26	12	1 1/8	5 1/4	5	4 1/4	(20) 16d	(8) 16d	1420	2170	1290	1630
LU28L	20	1 1/8	6 1/4	1 1/4	5 7/8	(8) 10d	(6) 10d x 1 1/2"	632	985	5.74	7.25
LUS28	18	1 1/8	6 1/4	1 1/4	3 3/4	(6) 10d	(4) 10d	2705	4940	2065	3875
HUS28	16	1 1/8	7 1/8	3	6 1/8	(22) 16d	(8) 16d	1130	2197	9.20	17.24
HGUS28	12	1 1/8	7 1/8	5	6 1/8	(36) 16d	(12) 16d	2055	4265	1460	4115
LU210L	20	1 1/8	8	1 1/4	7 1/4	(10) 10d	(6) 10d x 1 1/2"	914	1897	6.49	18.31
LUS210	18	1 1/8	7 1/8	1 1/4	3 3/4	(8) 10d	(4) 10d	2685	6625	2685	5700
								1196	2051	11.96	25.35
								1140	2185	1020	1550
								5.07	9.72	4.54	6.89
								1420	2520	1290	1790
								632	1121	5.74	7.96
								3605	5365	2675	4345
								1694	2386	11.90	19.33
								3310	7675	3310	6900
								1474	3419	14.74	30.73
								1140	2495	1020	1770
								5.07	11.10	4.54	7.87
								1420	2785	1290	2210
								632	1299	5.74	9.83

1. Factored uplift resistances have been increased 15% for wind or earthquake loading; no further increase is allowed.
2. Designer must ensure that hanger is compatible with truss when reduced heel height is used.
3. d_e is the distance from the bearing seat to the top joist nail.
4. Resistances shown require a minimum 2-ply girder truss. For fastening to single-ply truss request technical bulletin T-C-N10TRSSCN and/or see installation notes.
5. Nails: 16d = 0.162" dia. x 3 1/2" long. See pp. 27–28 for other nail sizes and information.



TECHNICAL BULLETIN

HGUS – Double Shear Joist Hangers

SIMPSON
Strong-Tie

All HGUS hangers have double shear nailing. This patented innovation distributes the load through two points on each joist nail for greater strength. It also allows the use of fewer nails, faster installation and the use of common nails for all connections. Do not bend or remove tabs.

MATERIAL: 12 gauge

FINISH: G90 galvanized

DESIGN:

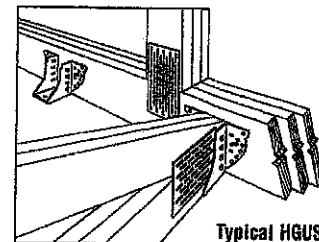
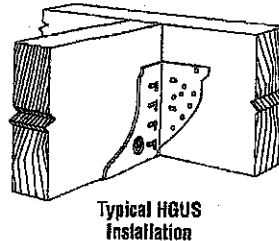
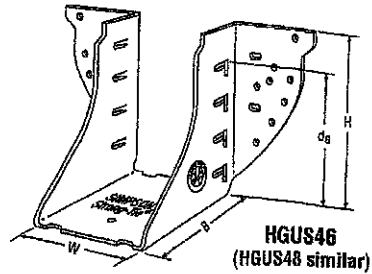
- Factored resistances are in accordance with CSA Q86-14
- Uplift resistances have been increased 15%. No further increase is permitted.
- Wood shear is not considered in the factored resistances given. The specifier must ensure that the joist and header capacities are capable of withstanding these loads.

INSTALLATION:

- Use all specified fasteners
- Nails: 16d = 0.162" dia x 3 1/2" long common wire
- Double shear nails must be driven at an angle through the joist or truss into the header to achieve the table loads
- Not designed for welded or nailer applications

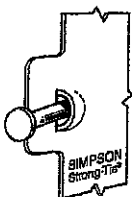
OPTIONS:

- See current catalogue for options



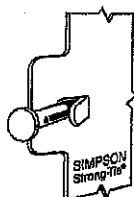
Model No.	Ga	Dimensions (in)				Fasteners		Factored Resistance (lbs)			
		W	H	B	d _g ¹	Face	Joist	D.Fir-L		S-P-F	
								Uplift (K _u =1.15)	Normal (K _n =1.00)	Uplift (K _u =1.15)	Normal (K _n =1.00)
HGUS48	12	3 3/4	7 1/4	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS410	12	3 3/4	9	4	8 1/8	46-16d	16-16d	8840	14645	4855	10400
HGUS412	12	3 3/4	10 1/4	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS414	12	3 3/4	12 1/4	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645
HGUS5.50/8	12	5 1/2	6 1/4	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS5.50/10	12	5 1/2	8 1/4	4	8 1/8	46-16d	16-16d	8840	14645	4855	10400
HGUS5.50/12	12	5 1/2	10 1/4	4	10 1/8	56-16d	20-16d	7640	14995	5425	10645
HGUS5.50/14	12	5 1/2	12 1/4	4	11 1/8	66-16d	22-16d	10130	16400	7195	11645
HGUS7.25/8	12	7 1/4	7 1/4	4	6 1/8	36-16d	12-16d	6070	12980	4310	9215
HGUS7.25/10	12	7 1/4	8 1/4	4	8 1/8	46-16d	16-16d	8840	15760	4855	11190
HGUS7.25/12	12	7 1/4	10 1/4	4	10 1/8	56-16d	20-16d	7640	16110	5425	11435
HGUS7.25/14	12	7 1/4	12 1/4	4	11 1/8	66-16d	22-16d	10130	18200	7195	12920

1. d_g is the distance from the seat of the hanger to the highest joist nail.

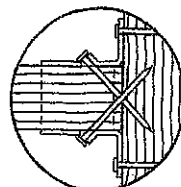


Dome Double Shear Nailing prevents tabs breaking off (available on some models).

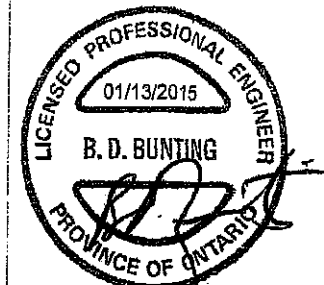
U.S. Patent
5,603,580



Double Shear Nailing Side View. Do not bend tab back.



Double Shear Nailing Top View.



LIMIT
STATES
DESIGN

This technical bulletin is effective until December 31, 2016, and reflects information available as of January 1, 2015. This information is updated periodically and should not be relied upon after December 31, 2016; contact Simpson Strong-Tie for current information and limited warranty or see www.strongtie.com.

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T-SPECDSHGUS15 1/15 exp. 12/16

800-999-5099
www.strongtie.com



Alves Engineering Services Inc.

5208 Easton road
Burlington, Ontario L7L 6N6
(289) 259 5455

RESPONSABILITIES

- 1-Alves Engineering Services Inc. is responsible for the design of trusses as individual components
- 2-It is the responsibility of others to ascertain that the design loads utilized on this drawing meet or exceed the actual dead load imposed by the structure and the live load imposed by the local building code or the authorities having jurisdictions.
- 3- All dimensions are to be verified by owner, contractor, architect or other authority before manufacture.
- 4- Alves Engineering Services Inc. bears no responsibility for the erection of the trusses. Persons erecting trusses are cautioned to seek professional advice regarding temporary and permanent bracing system. Bracing shown on Alves Engineering Services Inc. drawings is specified for the truss as a single component and forms an integral part of the truss design, but is not meant to represent the only required bracing for that truss when trusses are installed in a series of trusses forming a roof truss system.
- 5- It is the manufactures responsibility to ensure that the trusses are manufactured in conformance with Alves Engineering Services Inc. specifications outlined below.

SPECIFICATIONS

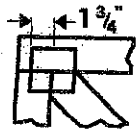
- 1-Truss components sealed by Alves Engineering Services Inc. conform to the relevant sections of the current Building Code of Ontario and Canada (part 4 or part 9) or the current Canadian code for Farm Buildings in accordance with the application specified on the sealed truss component drawing. All truss component design procedures must conform to the current design standard issued by the truss plate institute of Canada (TPIC). All lumber and nailing stresses to conform to the current CSA wood design standard identified on the current Building Code and TPIC.
- 2- Lumber is to be the sizes and grade specified on the truss drawing.
- 3- Moist content of lumber is not to exceed 19% in service unless otherwise specified.
- 4- Plates shall be applied to both faces of the each truss joint and shall be positioned as shown on the truss drawings
- 5- Lumber used on manufacture of trusses is not to be treated with chemicals unless otherwise specified on the truss drawings.
- 6- The top chord is assumed to be continuously laterally braced by the roof sheathing or purlins at intervals specified on the truss drawing but not exceeding 24" c/c for (part 9) and not exceeding 48" for (part 4 or farm design)
- 7- When rigid ceiling is not attached directly to the bottom chord, lateral bracing is required and it should not exceed more than 3m or 10' intervals.
- 8-Refer to Mitek sheet MII7473C REV.10-08 attached for information on symbols, numbering system and General Safety notes.

T-1300218

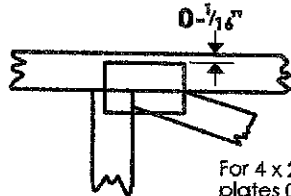
Feb 09, 2018

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths or mm. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- $\frac{3}{16}$ " from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING

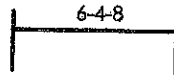


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

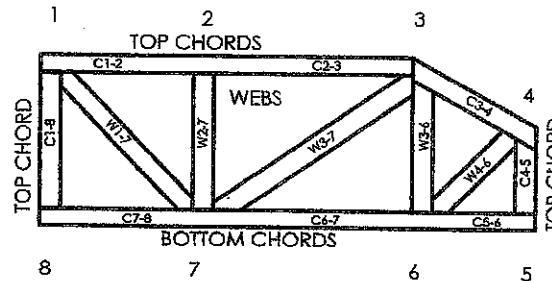
Industry Standards:

TPIC: Truss Design Procedures and Specifications for Light Metal Plate Connected Wood Trusses
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths or mm (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

CCMC Reports:

11996-L, 10319-L, 13270-L, 12691-R

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MiTek
POWER TO PERFORM.™

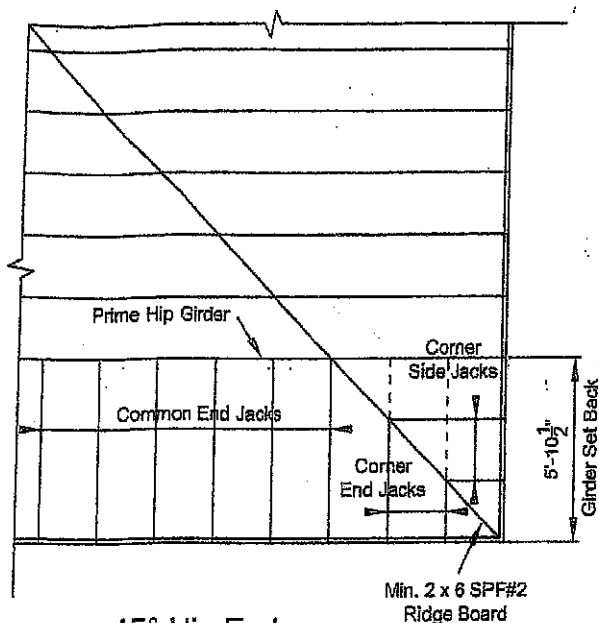
MiTek Engineering Reference Sheet: MII-7473C rev. 10-'08



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by TPIC.
7. Design assumes trusses will be suitably protected from the environment in accord with TPIC.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with TPIC Quality Criteria.



45° Hip End

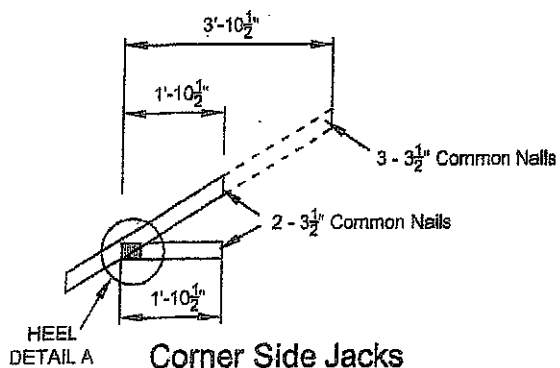
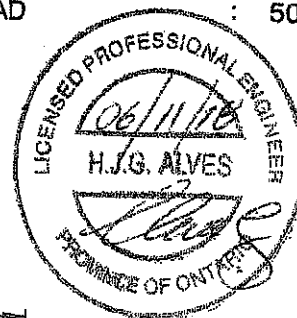
LUMBER SPECIFICATION

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

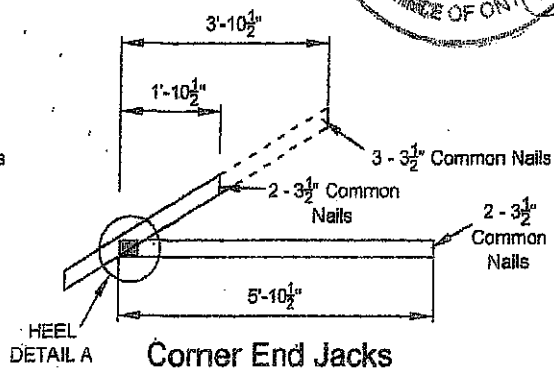
DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

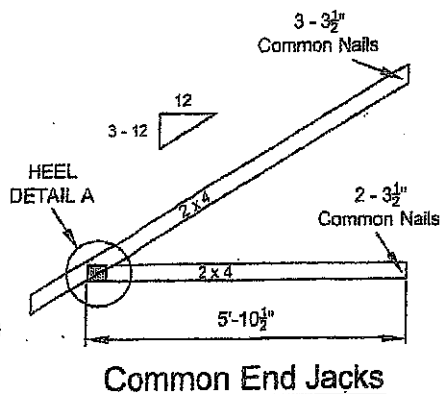
TOTAL LOAD : 50.5 P.S.F.



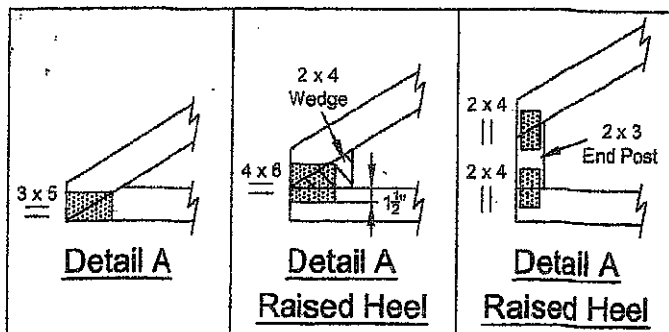
Corner Side Jacks



Corner End Jacks



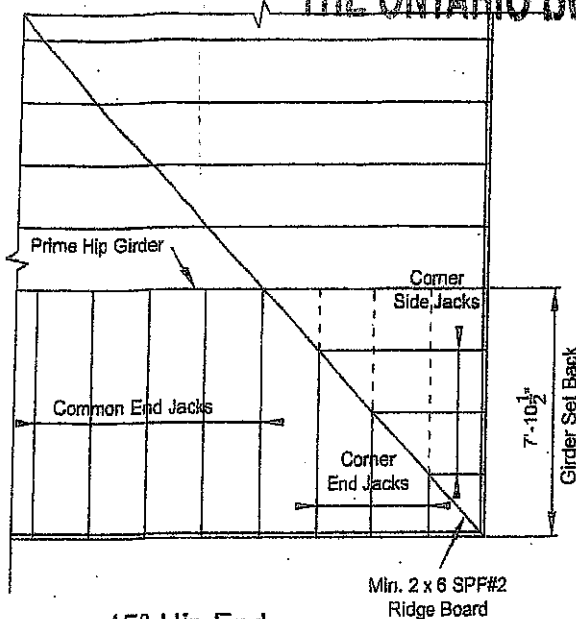
Common End Jacks



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800216

**THIS STRUCTURE MUST BE
CONSTRUCTED TO MEET OR
EXCEED THE PROVISIONS OF
THE ONTARIO BUILDING CODE**



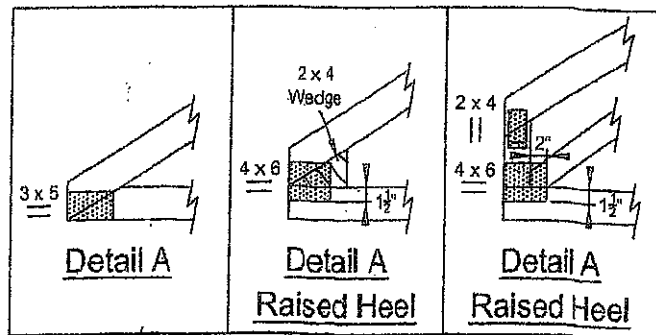
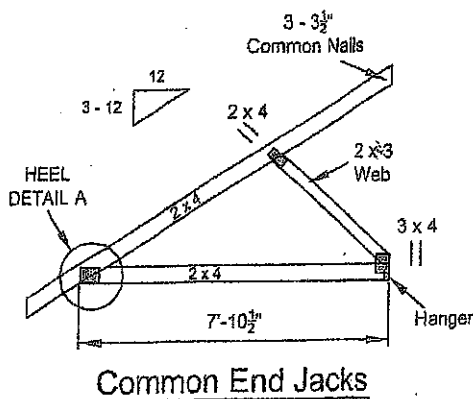
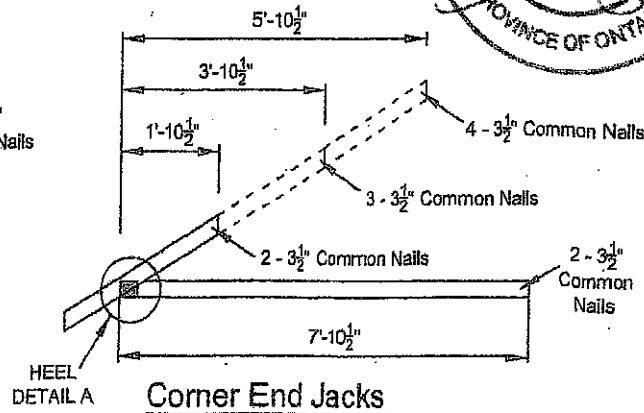
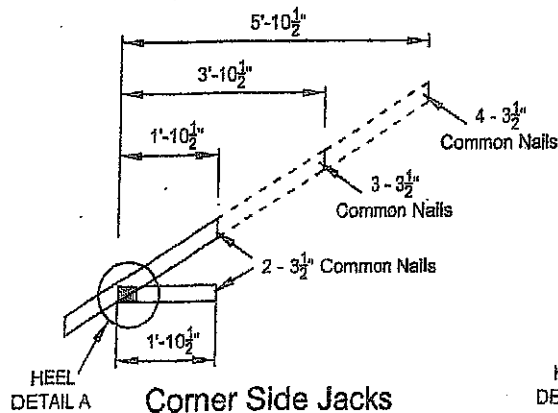
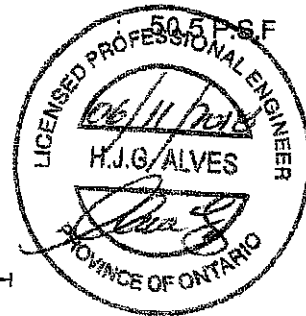
45° Hip End

TOP CHORD : 2 x 4 SPF#2
 BOTTOM CHORD : 2 x 4 SPF#2
 WEBS : 2 x 3 SPF#2
 UNLESS OTHERWISE SHOWN

DESIGN LOAD

TOP CHORD SNOW LOAD : 40.5 P.S.F.
 TOP CHORD DEAD LOAD : 3.0 P.S.F.
 BOTTOM CHORD LIVE LOAD : 0.0 P.S.F.
 BOTTOM CHORD DEAD LOAD : 7.0 P.S.F.

TOTAL LOAD



NOTE: DESIGN CONFORMS TO PART 9, O.B.C. 2012 (L.S.D. DESIGN)

T-1800217